

THE DETAILED DESIGN STUDY  
ON  
THE OUTER CIRCULAR HIGHWAY  
TO  
THE CITY OF COLOMBO

FINAL REPORT  
(FOR NORTHERN SECTION 1)  
VOLUME VI – 2/3:  
DRAWINGS - BRIDGES  
9 of 10

February 2008

JAPAN INTERNATIONAL COOPERATION AGENCY

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Oriental Consultants Company Limited  
Pacific Consultants International

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K08 - 09	DIMENSION DETAILS OF PIER P1, P2, P3, P4, P5, P6, P7, & P8
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K09-00-C	TYP. CROSS SECTION OF ABUTMENTS AND PIERS
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K09 - 02	LAYOUT PLAN (TYPICAL TO PIER P1 TO PIER P8)
K09 - 03	LAYOUT PLAN (PIER P8 TO ABUTMENT A2)
K09 - 04	TYPICAL PCI GIRDER LAYOUT DIMENSION DETAIL
K09 - 05	GIRDER LAYOUT DIMENSION DETAIL (TYPICAL)
K09 - 06	TYPICAL ARRANGEMENT OF PC TENDON (1/2)
K09 - 07	TYPICAL GIRDER CABLE ARRANGEMENT (2/2)
K09 - 08	LAYOUT OF PC TENDON
K09 - 09	DIMENSION DETAILS OF ABUTMENT A1
K09 - 10	DIMENSION DETAILS OF PIER P1, P2, P3, P4, P5, P6, P7, P8, P9, P10 & P11
K09 - 11	DIMENSION DETAILS OF ABUTMENT A2



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K10 - 03	LAYOUT PLAN OF PIER P7 TO PIER P0
K10 - 04	TYPICAL PCI GIRDER LAYOUT DIMENSION DETAIL
K10 - 05	PCI GIRDER LAYOUT DIMENSION DETAIL (ABUTMENT A1 TO PIER P1)
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K13 - 04	STEEL GIRDER LAYOUT & DIMENSION DETAIL FOR PIER P2 TO V1-P2
K13 - 05	DIMENSION DETAIL FOR ABUTMENT A1
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K14 - 03	STEEL GIRDER LAYOUT & DIMENSION DETAIL FOR PIERS P1 TO P2
K14 - 04	STEEL GIRDER LAYOUT & DIMENSION DETAIL FOR PIER P2 TO V1-P4
K14 - 05	DIMENSION DETAIL FOR ABUTMENT A1
K14 - 06	DIMENSION DETAIL FOR PIERS P1 & P2
K14 - 07	DIMENSION DETAIL FOR VIADUCT V1 - PIER P5

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K15-00-B	TYPICAL CROSS SECTION OF ABUTMENT & PIER
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K15 - 06	TYPICAL PCI GIRDER LAYOUT DIMENSION DETAIL (ABUT A1 TO P2)
K15 - 07	TYPICAL PCI GIRDER LAYOUT DIMENSION DETAIL (PIER P2 TO V4-A1)
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K15 - 09	PCI GIRDER LAYOUT DIMENSION DETAIL (PIER P1 TO P2)
K15 - 10	PCI GIRDER LAYOUT DIMENSION DETAIL (PIER P2 TO P3)
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K15 - 13	PC CABLE ARRANGEMENT (1/5)
K15 - 14	PC CABLE ARRANGEMENT (2/5)
K15 - 15	PC CABLE ARRANGEMENT (3/5)
K15 - 16	PC CABLE ARRANGEMENT (4/5)
K15 - 17	PC CABLE ARRANGEMENT (5/5)
K15 - 18	DIMENSION DETAILS OF ABUTMENT A1
K15 - 19	DIMENSION DETAILS OF PIER P1 & P2
K15 - 20	DIMENSION DETAILS OF PIER P3 & P4
VIADUCT NO.10 (V10) – B214 IC RAMP–2 BRIDGE	
K16-00-A	GENERAL VIEW
K16-00-B	TYPICAL CROSS SECTION OF ABUTMENT & PIER
K16 - 01	LAYOUT PLAN (ABUTMENT A1 TO PIER P1)
K16 - 02	LAYOUT PLAN (PIER P1 TO P2)
K16 - 03	LAYOUT PLAN (PIER P2 TO P3)
K16 - 04	LAYOUT PLAN (PIER P3 TO P4)
K16 - 05	LAYOUT PLAN (PIER P4 TO P5)
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K16 - 08	TYPICAL PCI GIRDER LAYOUT DIMENSION DETAIL (1/2)
K16 - 09	TYPICAL PCI GIRDER LAYOUT DIMENSION DETAIL (2/2)
K16 - 10	PCI GIRDER LAYOUT DIMENSION DETAIL (ABUTMENT A1 TO PIER P1)
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K16 - 13	PCI GIRDER LAYOUT DIMENSION DETAIL (PIER P3 TO P4)
K16 - 14	PCI GIRDER LAYOUT DIMENSION DETAIL (PIER P4 TO P5)
K16 - 15	PCI GIRDER LAYOUT DIMENSION DETAIL (PIER P5 TO P6)
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K16 - 17	PC CABLE ARRANGEMENT (1/5)
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K16 - 19	PC CABLE ARRANGEMENT (3/5)
K16 - 20	PC CABLE ARRANGEMENT (4/5)
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K16 - 24	DIMENSION DETAILS OF PIER P3 & P4
K16 - 25	DIMENSION DETAILS OF PIER P5 & P6

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GENERAL NOTES

1. DESIGN CRITERIA

- Geometric Design Standards of Roads (1998), RDA
- Bridge Design Manual (1997), RDA
- Standard Specifications for Construction and Bridges (1998), RDA
- Bridge Construction Manual (1997), RDA
- British Standard BS 5400, BS 5911, BS 8002, BS 8004, BS 8007, BS EN 10025
- Specification of Highway Bridges (Japan Road Association,1996)
- Design Manuals for Roads and Bridges (British Highway Agency)
- A Design Manual for Small Bridges (Transport and Road Research Laboratory, UK)
- A Guide to Design Loadings for Buried Rigid Pipes (Transport and Road Research Laboratory, UK)

2. LOADS

- Permanent Loads
  - Reinforced/Prestressed Concrete : 25.0 kN/m3
  - Plain Concrete : 23.5 kN/m3
  - Steel and Cast Steel : 78.5 kN/m3
  - Bituminous Wearing Surfaces : 23.0 kN/m3
  - Bridge Parapet : 7.60 kN/m
  - Handrail for one side : 0.50 kN/m
  - Compacted Soil : 19.0 kN/m3
  - Loose Soil : 16.0 kN/m3
- Live Load : HA Loading, 30 Units HB Loading, HA Single Wheel Loading
- Pedestrian Load : 3.0 kN/m2
- Basic Wind Load : 39.4 m/s
- Temperature Range : 25°C – 31°C
- Seismic Effect : No Consideration
- Vessel Collision Force : River Class III

3. MATERIALS

3-1 CONCRETE

- 1) Unless indicated otherwise, the characteristic cube strength of concrete (fcu) shall be as follows:

Concrete Class	fcu (MPa)	Typical Use
A	50	Cast-in-situ prestressed concrete box girder Precast prestressed concrete girder (PC I-Girder)
B	35	Crossbeam of PC I-Girder. Cast-in-place concrete deck slab, Precast reinforced concrete panel
C	30	Reinforced concrete pier (cantilever pier head, pier column and footing included) Reinforced concrete abutment (wing wall included) Reinforced concrete retaining wall, Box culvert
A'	40	For Kelani River Bridge, Viaduct 4, Ramps 1 & 2 (very severe condition). Crossbeam of PC I-Girder. Cast-in-place concrete deck slab, Precast reinforced concrete panel Reinforced concrete pier (cantilever pier head, pier column and footing included) Reinforced concrete abutment (wing wall included) Reinforced concrete retaining wall, Box culvert Cast-in-situ concrete pile Precast reinforced concrete pile Bridge parapet, Street lighting pole foundation
D-1	30	Cast-in-situ concrete pile
D-2	30	Precast reinforced concrete pile
D-3	30	Bridge parapet, Street lighting pole foundation
E	20	Approach Slab Pipe culvert bedding (Class A) Precast concrete curbs
F	15	Leveling concrete, Lean concrete

- 2) All exposed edges of concrete shall be chamfered 25\*25 mm unless otherwise noted.
- 3) All construction joints are to be located as shown on the drawings or as approved by the Engineer.
- 4) Unless otherwise noted, concrete surface finish shall be as specified in the General Specifications.

3-2 REINFORCING BAR

- 1) Steel reinforcement shall be of Grade 460 (yield strength of 460 MPa), Type-2 deformed bar in accordance with Bs 4449:1997. Type deformed bar shall have the required ultimate anchorage bond stress in Table 15 of BS 5400-4:1990.
- 2) Scheduling,dimensioning,bending an cutting of steel reinforcement shall be in accordance with BS 4466:1989.
- 3) Minimum splice length shall be in accordance with BS 5400-4:1990.
- 4) Splices in adjacent bars shall be staggered except where noted on the Drawings. Splices other than those shown on the drawings may only be made with the approval of the Engineer.
- 5) Unless otherwise indicated on the drawings, the minimum cover to any reinforcement shall be as follows:

Bored Pile	: 75 mm
Pile Cap, Footing	: 50 mm
Pile Cap, Footing (bottom)	: 75 mm
Precast Pile, Abutment, Pier, Approach Slab	: 50 mm
Precast PC Girder	: 35 mm
Cast-in-situ Girder	: 40 mm
Retaining Wall, Box Culvert	: 50 mm

3-3. PRESTRESSING CABLE

- 1) Prestressing tendons to be used for the Project are specified as follows:

Utilization	Designation	Equivalent
Longitudinal Strand for PC I-Girder	12S 12.7	D12.7mm
	9S 12.7	D12.7mm
Strand for Cross Beam	1S 21.8	D21.8mm

- 2) Prestressing tendons 12.7mm shall be formed using 7-wire and 21.8mm using 19-wire low relaxation strands conforming to BS 4486 & 5896.
- 3) Properties of prestressing tendons are:  
Characteristic Strength : fpu=1,860 MPa  
Modulus of Elasticity : E = 200,000 MPa
- 4) Jacking force used is 70% of the characteristic strength (fpu).
- 5) Ducts for interval tendons shall be semi-grid galvanized sheating, unless otherwise noted, and shall be rigidly supported at not greater than 500 mm intervals.
- 6) Tendon profiles are specified to the center of sheating. Tendons are to be placed to smooth profiles passing through the specified points.
- 7) Anchorages shall be set at right angles to the tendon profiles. Each tendon shall be kept straight for a minimum length of 1,000 mm from anchorage face.
- 8) Grout points shall be provided at all crown points, sag points and anchorages.

3-4. STRUCTURAL STEEL

- All structural steel shall conform to the requirements of BS or JRS as follows:
- a. Structural steel --- BS EN 10025, JIS 3106, JIS 3114
- b. High Strength Friction Grip Bolts --- BS 4395, BS 4604, JIS B 1186, JSS II 09-1996

4. WATERPROOFING

- 4-1 All reinforced concrete surfaces in contact with backfill shall be coated with coats of bituminous membrane.
- 4-2 The bridge deck shall be waterproof with parafor solo ponts or similar approved proprietary waterproofing system applied in accordance with manufacture's recommendation.

5. OTHERS

- 5-1 Unless otherwise noted on the Drawings, these notes are applicable to all structure drawings.
- 5-2 All coordinates and levels are given in meters. All dimension are given in millimeters unless otherwise noted on the drawings.
- 5-3 Levels are measured by National Level 1972 system (Sea Level datum at Hon Dau-Do Son).
- 5-4 Coordinates are measured by National Grid HN-72 with meridian of 106° 45'.
- 5-5 Where reference is made to proprietary component names, the Contractor may propose alternatives provided that they are equivalent and satisfy the requirements of the Specifications and Design Criteria.

6. CONSTRUCTION

6-1 BORED PILING WORKS

- 1) The dimension and minimum length of bored piles shall be as shown on the plans. Bottom of bored piles shall be embedded at least two (2) pile diameter into the bearing stratum and shall be approved by the engineer.
- 2) Suitable pile boring equipment shall be used to properly drill into the bearing stratum.
- 3) Bentonite slurry or other form of slurry to be used shall be able to properly hold in place the vertical soil surface in contact with the cast-in-place reinforced concrete shaft.
- 4) Shaft concreting shall be carried out using tremie pipe that will reach the bottom of the shaft. The tremie pipe shall always be embedded into the fresh concrete at least 2.0m as it goes up displacing the slurry.
- 5) For the pile terminating depth, bearing stratum shall be confirmed and approved by the engineer before the piling work.

- 6-2 Spread footings shall be as shown on the plans embedded into the bearing stratum with a minimum allowable bearing capacity (serviceability limit state) of 450kPa. Footing bottom shall be at least 1.00m below the bearing stratum. If actual footing levels change the height of the substructure, piers/abutments shall be redesigned.

7. FUTURE EXPANSION

- For future expansion the inner portion of the slab (500mm wide) shall be chipped off to expose slab reinforcement for the required lap splice with the new rebars. All existing slab rebars shall be carefully straightened-up without damaging the rebars.
- Prestressing steel wires for the diaphragms shall be connected by mechanical devices(coupler). Provisions of these couplers shall be anticipated at the sides of the exterior girders (inner portion only). These connection points by couplers shall be protected against corrosion as per requirement of prestressing supplier.

THE DEMOCRATIC SOCIALIST REPUBLIC OF SRI LANKA  
MINISTRY OF HIGHWAYS & ROAD DEVELOPMENT



Road Development Authority



JAPAN INTERNATIONAL COOPERATION AGENCY



ORIENTAL CONSULTANTS COMPANY LIMITED

in association with

PACIFIC CONSULTANTS INTERNATIONAL



No

REVISION

DATE

COLOMBO OUTER CIRCULAR HIGHWAY PROJECT  
(NORTHERN SECTION 1)

BRIDGE STRUCTURE  
GENERAL NOTES FOR LONG SPAN BRIDGES

DESIGNED BY:

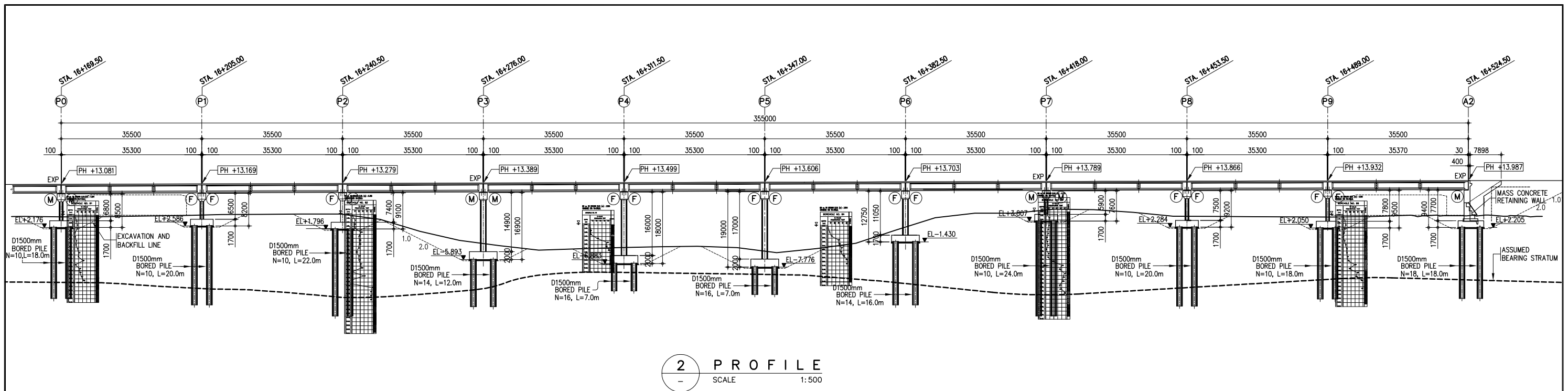
CHECKED BY:

APPROVED BY:

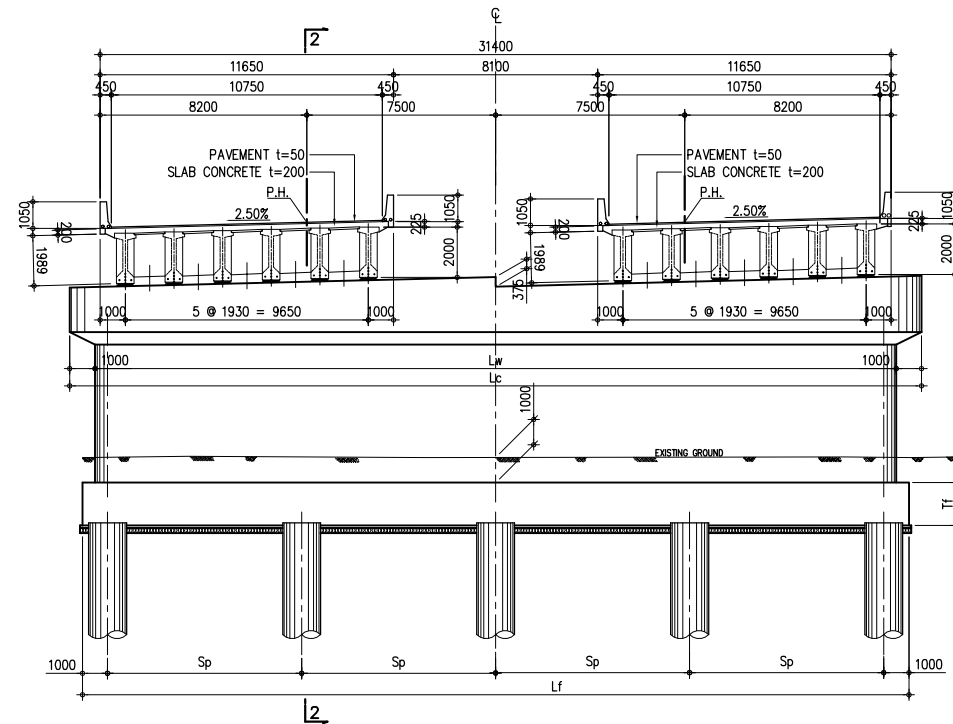
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K00

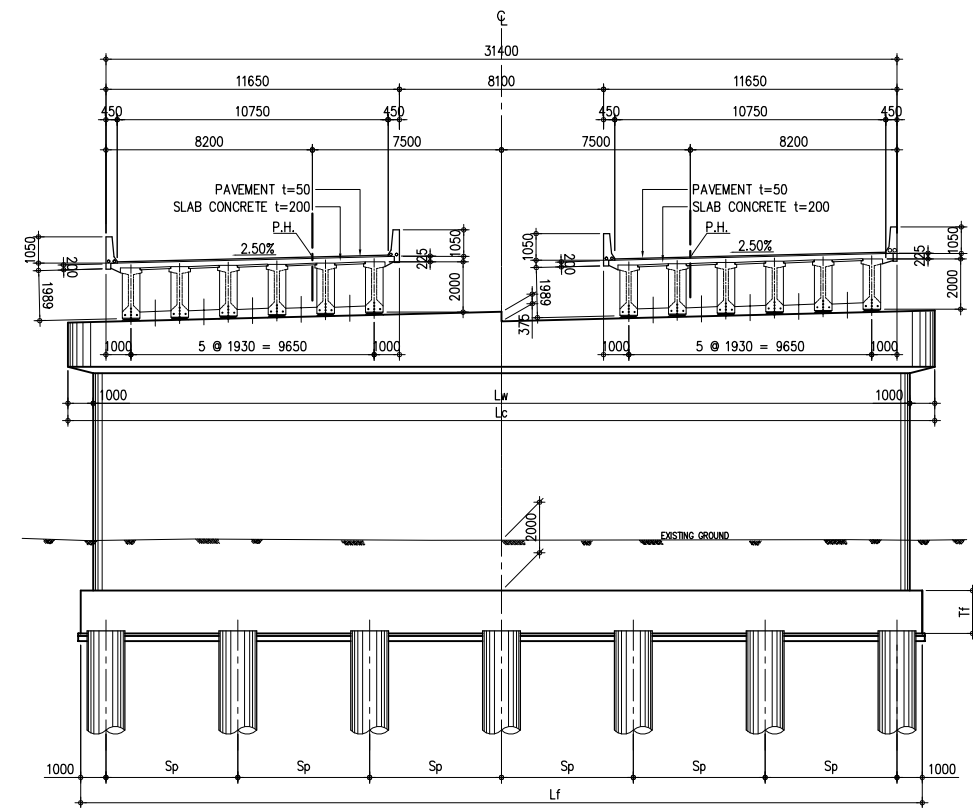
**K01** HIGHWAY BRIDGE No.9(H9)-KELANI RIVER  
CROSSING BRIDGE



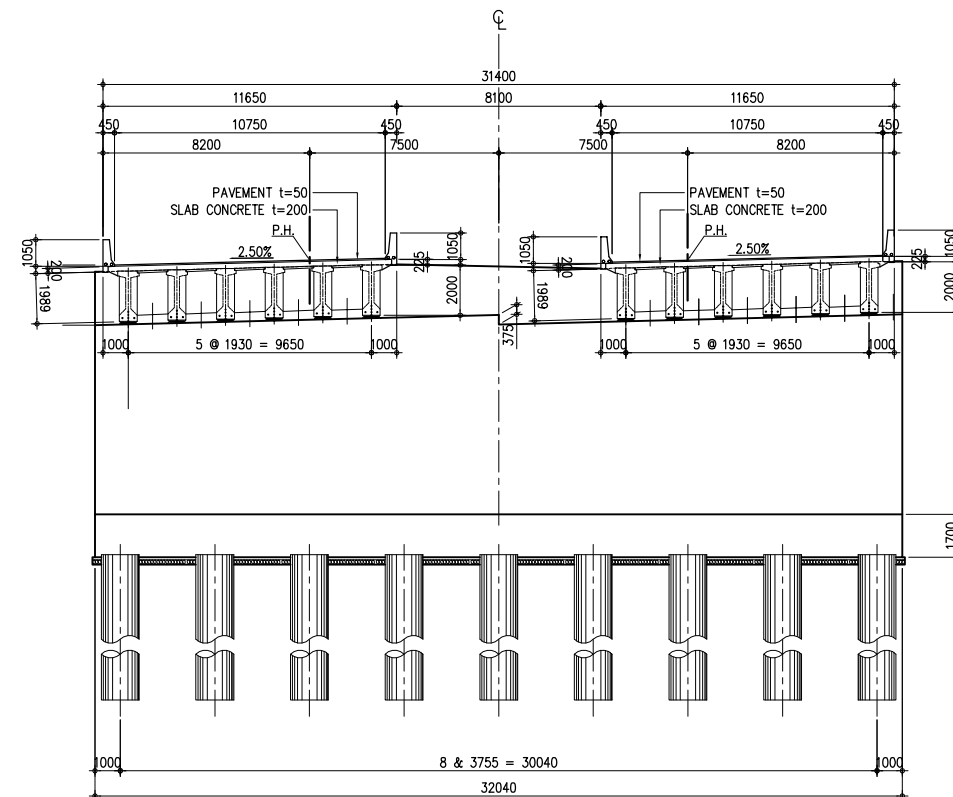
GRADE	
FINISHED ROAD LEVELS	13.091, 13.153, 13.215, 13.277, 13.339, 13.401, 13.463, 13.525, 13.587, 13.642, 13.696, 13.747, 13.794, 13.838, 13.879, 13.916, 13.950, 13.981, 13.983
EXISTING GROUND LEVELS	4.911, 5.221, 5.511, 5.801, 6.091, 6.381, 6.671, 6.961, 7.251, 7.541, 7.831, 8.121, 8.411, 8.701, 8.991, 9.281, 9.571, 9.861, 10.151, 10.441, 10.731, 11.021, 11.311, 11.601, 11.891, 12.181, 12.471, 12.761, 13.051, 13.341, 13.631, 13.921, 14.211, 14.501, 14.791, 15.081, 15.371, 15.661, 15.951, 16.241, 16.531, 16.821, 17.111, 17.401, 17.691, 17.981, 18.271, 18.561, 18.851, 19.141, 19.431, 19.721, 20.011, 20.301, 20.591, 20.881, 21.171, 21.461, 21.751, 22.041, 22.331, 22.621, 22.911, 23.201, 23.491, 23.781, 24.071, 24.361, 24.651, 24.941, 25.231, 25.521, 25.811, 26.101, 26.391, 26.681, 26.971, 27.261, 27.551, 27.841, 28.131, 28.421, 28.711, 29.001, 29.291, 29.581, 29.871, 30.161, 30.451, 30.741, 31.031, 31.321, 31.611, 31.901, 32.191, 32.481, 32.771, 33.061, 33.351, 33.641, 33.931, 34.221, 34.511, 34.801, 35.091, 35.381, 35.671, 35.961, 36.251, 36.541, 36.831, 37.121, 37.411, 37.701, 37.991, 38.281, 38.571, 38.861, 39.151, 39.441, 39.731, 40.021, 40.311, 40.601, 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401.651,



2 TYPICAL CROSS SECTION OF PIER P0, P1, P2, P7, P8, & P9  
SCALE 1:100

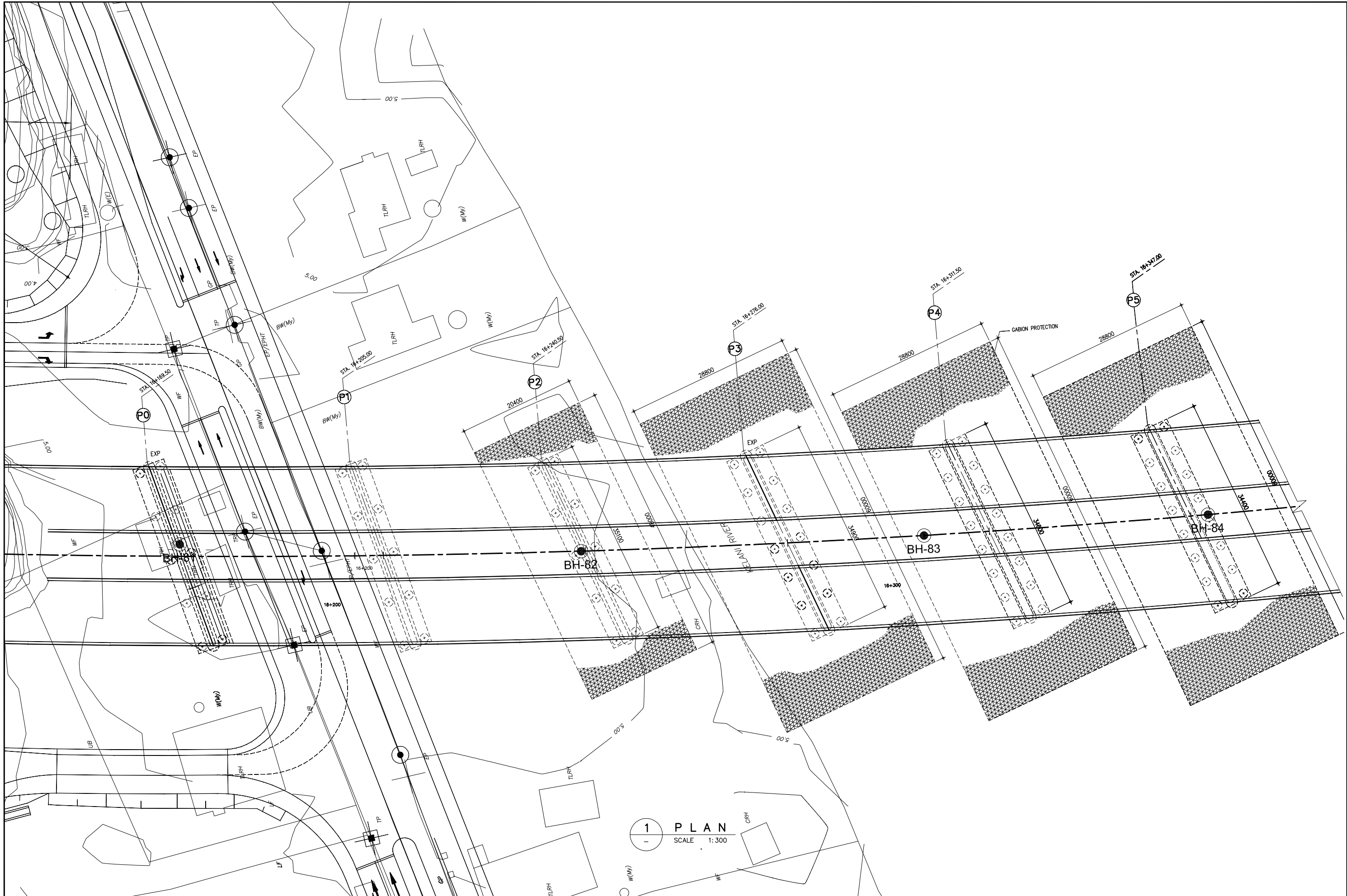


3 TYPICAL CROSS SECTION OF PIER P3, P4, P5 & P6  
SCALE 1:100



1 CROSS SECTION OF ABUTMENT A2  
SCALE 1:100

TABLE FOR P.H	
PIER NO.	P.H
P0	13.081
P1	13.169
P2	13.279
P3	13.389
P4	13.499
P5	13.606
P6	13.703
P7	13.789
P8	13.866
P9	13.932
ABUT-A2	13.987



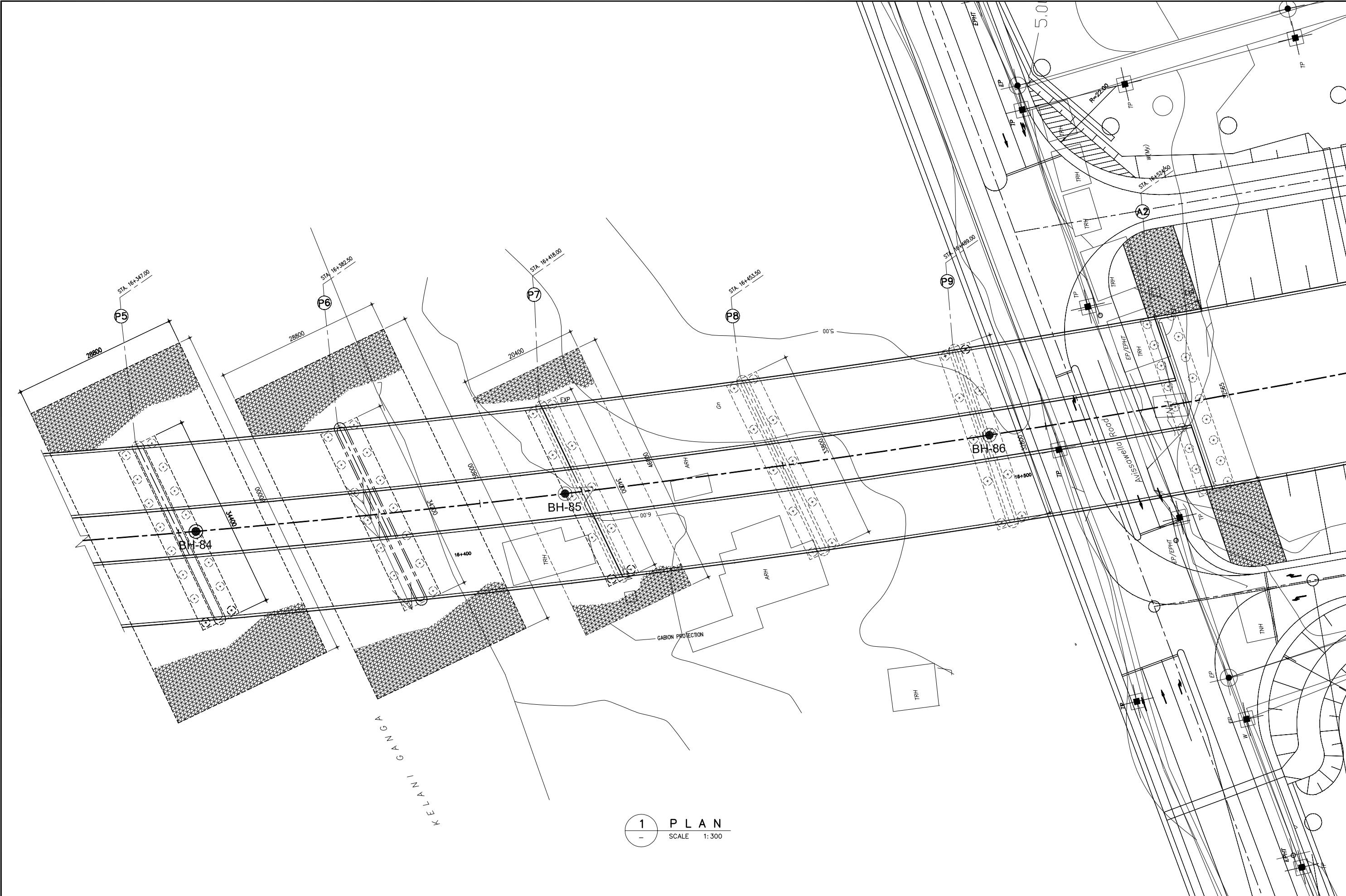
1 PLAN  
SCALE 1:300

THE DEMOCRATIC SOCIALIST REPUBLIC OF SRI LANKA  
MINISTRY OF HIGHWAYS & ROAD DEVELOPMENT  
**Road Development Authority**

**JICA** JAPAN INTERNATIONAL COOPERATION AGENCY  
**ORIENTAL CONSULTANTS COMPANY LIMITED**  
in association with  
**PACIFIC CONSULTANTS INTERNATIONAL**

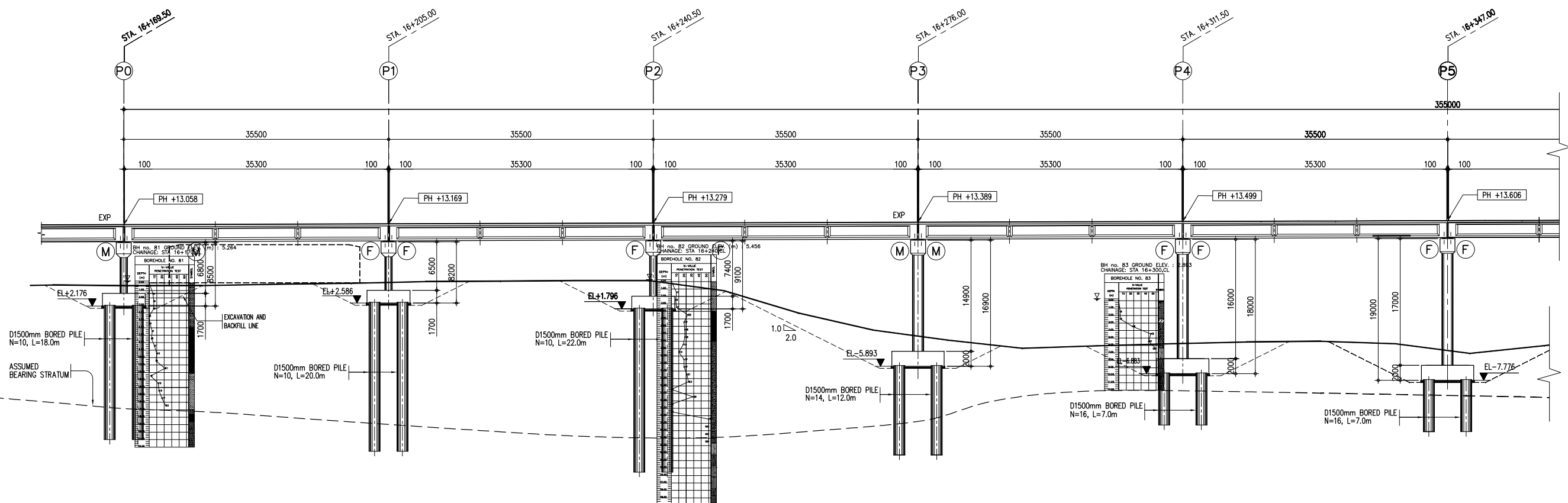
No	REVISION	DATE

<b>COLOMBO OUTER CIRCULAR HIGHWAY PROJECT</b> (NORTHERN SECTION 1)		DESIGNED BY:	
HIGHWAY BRIDGE NO.9 (H9) - KELANI RIVER CROSSING BRIDGE		CHECKED BY:	
GENERAL VIEW - STA. 16+169.50 - STA. 16+524.50		APPROVED BY:	
DWG. NO.	K01-00-C		



1 PLAN  
SCALE 1:300

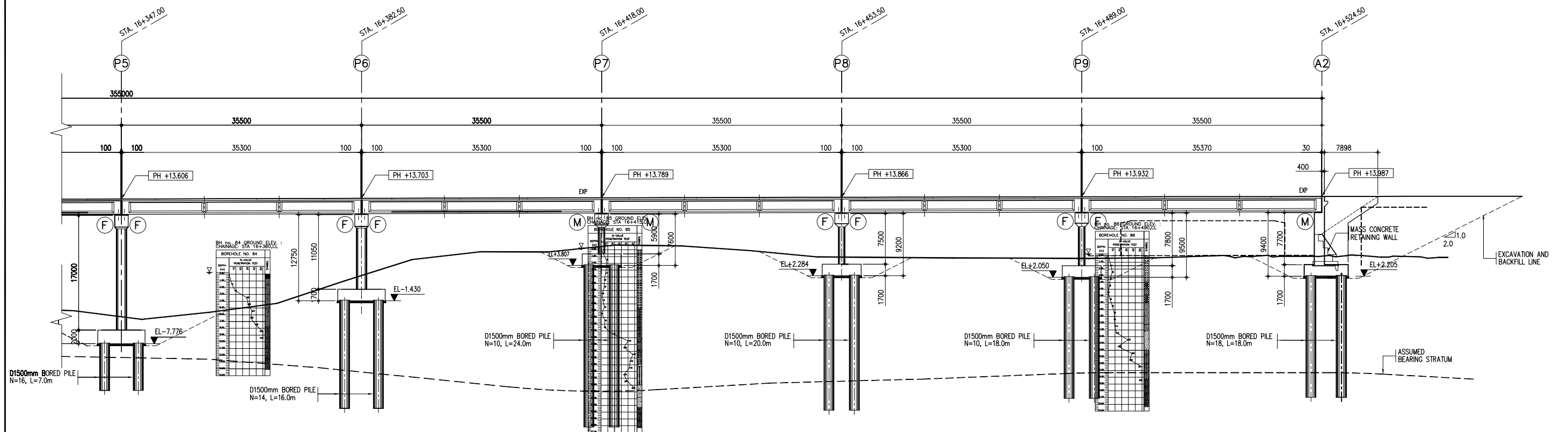
No	REVISION	DATE



GRADE		0.310% L=1,200m											
FINISHED ROAD LEVELS		13.091	13.153	13.215	13.277	13.339	13.401	13.463	13.525	13.585	13.642		
EXISTING GROUND LEVELS		4.911	5.221	5.511	5.581	1.241	-2.389	-3.159	-2.659	-2.889	-3.139		
STATION (km)		16180	16200	16220	16240	16260	16280	16300	16320	16340	16360		
CURVE BAND		R=2000 L=991.683 m.											
SUPERELEVATION		+2.5 % -2.5 %											

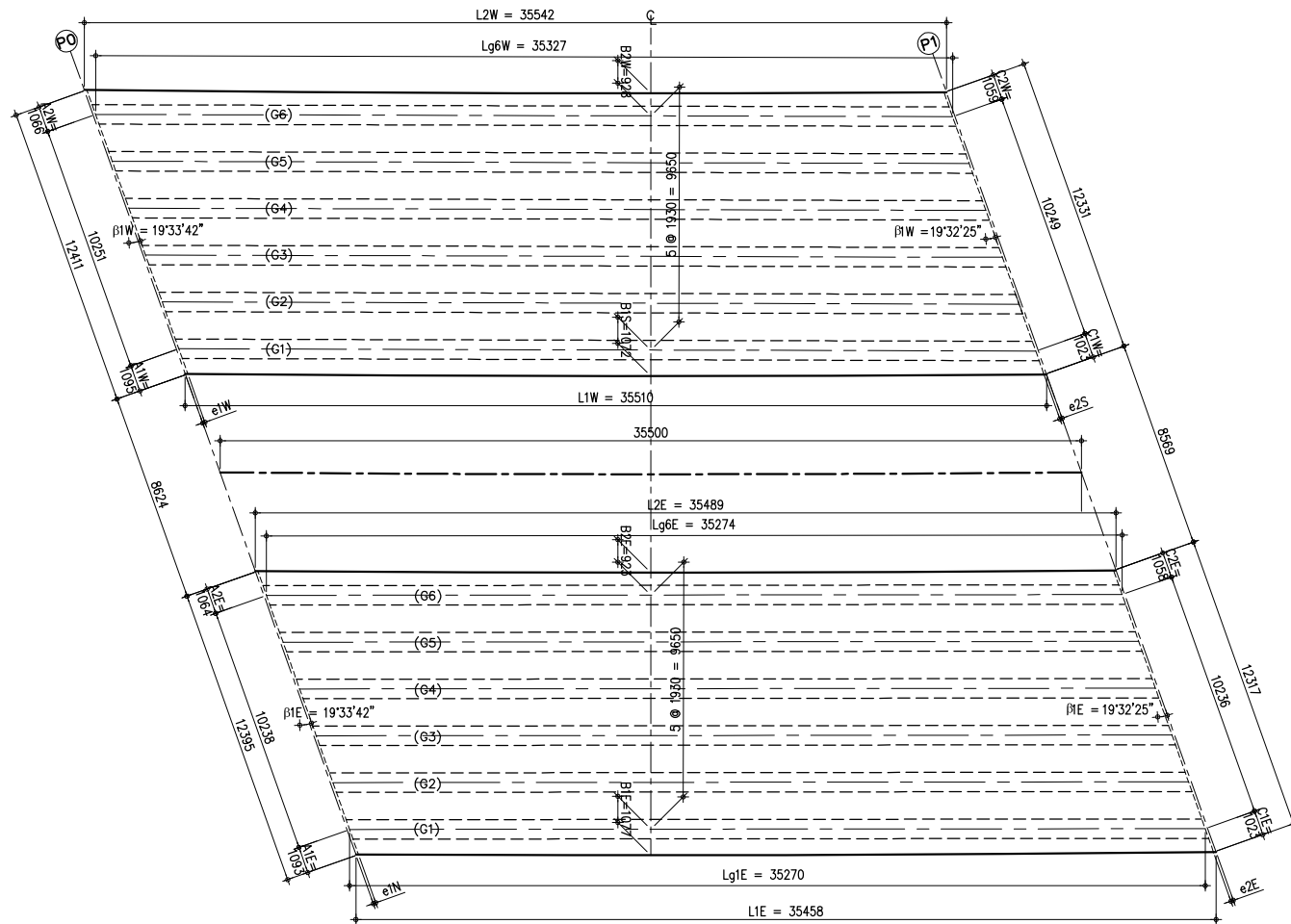
1 PROFILE  
SCALE 1:300





16340	16380	16380	16400	16420	16440	16460	16480	16500	16520	16524.5
-2.889	-3.139	1.531	5.811	6.791	6.000	4.971	4.861	4.931	5.149	13.783
13.585	13.642	13.696	13.747	13.794	13.838	13.879	13.916	13.950	13.981	13.783

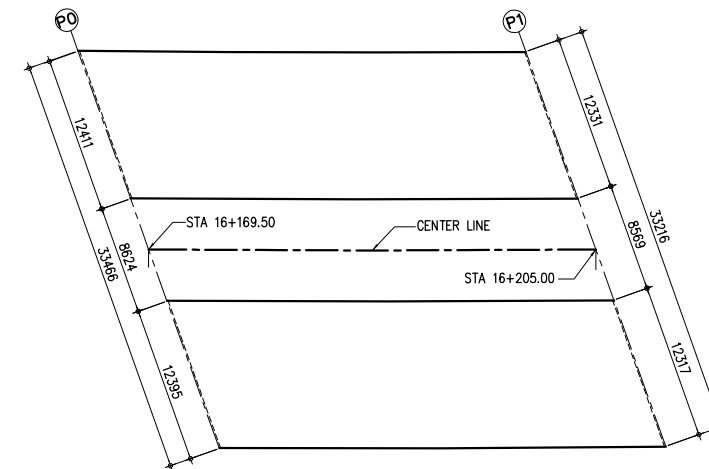
1 PROFILE  
SCALE 1:300

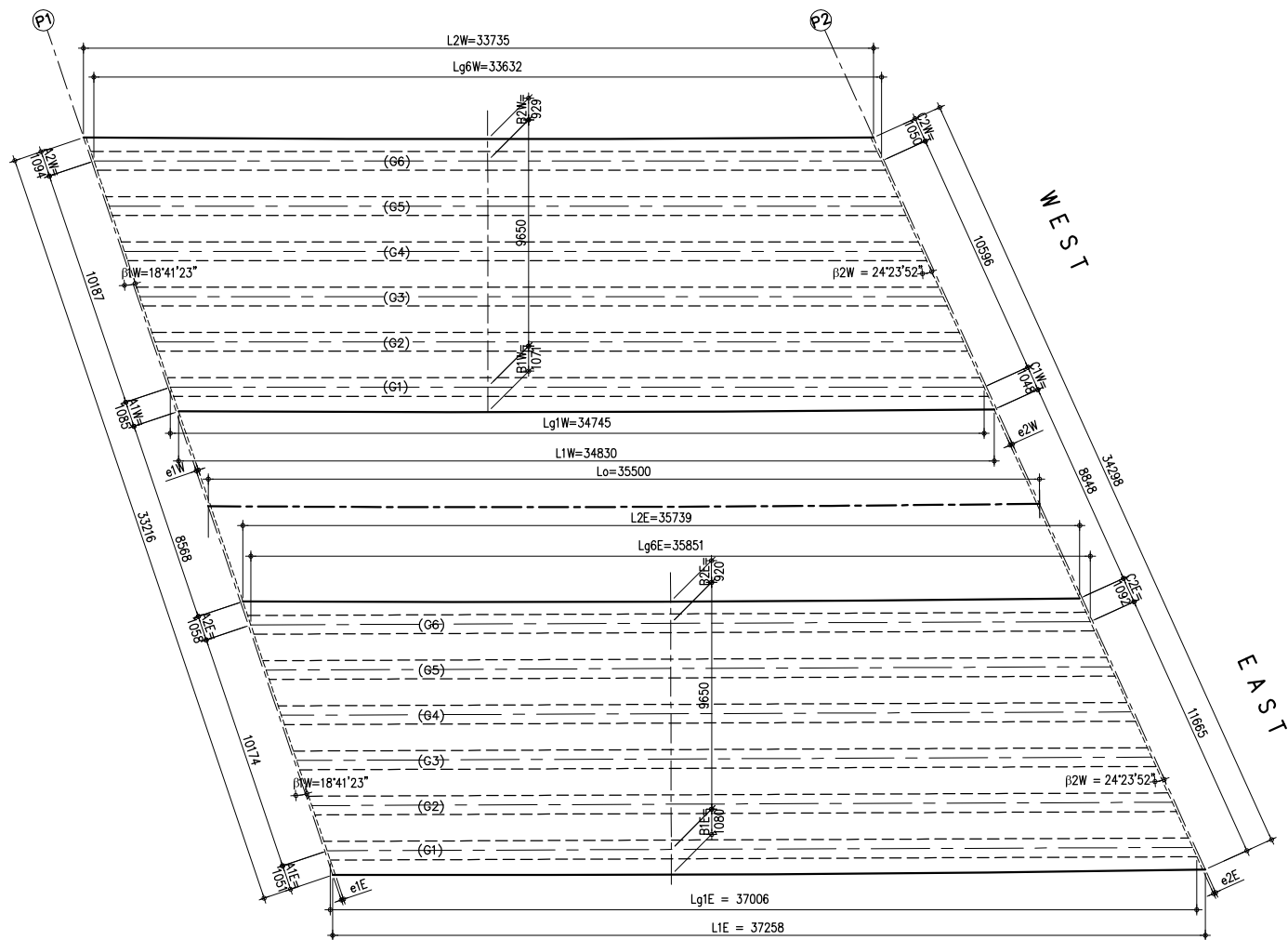


1 LAYOUT PLAN  
SCALE 1:150

	SOUTH BOUND
	P0~P1
Lo (m)	35.500
L1W (m)	35.510
L2W (m)	35.542
$\beta 1W$ (°)	19°33'42"
$\beta 2W$ (°)	19°32'25"
e1W/e2W (mm)	100/100
i1W (%)	2.5
i2W (%)	2.5
A1W/A2W (mm)	1095/1066
B1W/B2W (mm)	1072/928
C1W/C2W (mm)	1023/1059
Lg1W (m)	35.323
Lg2W (m)	35.323
Lg3W (m)	35.324
Lg4W (m)	34.077
Lg5W (m)	33.854
Lg6W (m)	33.632
SPAN LENGTH (m)	L=35.500
REMARKS	

	NORTH BOUND
	P0~P1
Lo (m)	35.500
L1E (m)	35.458
L2E (m)	35.489
$\beta 1E$ (°)	19°33'42"
$\beta 2E$ (°)	19°32'25"
e1E/e2E (mm)	100/100
i1E (%)	2.5
i2E (%)	2.5
A1E/A2E (mm)	1093/1064
B1E/B2E (mm)	1077/923
C1E/C2E (mm)	1023/1058
Lg1E (m)	35.270
Lg2E (m)	35.271
Lg3E (m)	35.272
Lg4E (m)	35.272
Lg5E (m)	35.273
Lg6E (m)	35.274
SPAN LENGTH (m)	L=35.500
REMARKS	

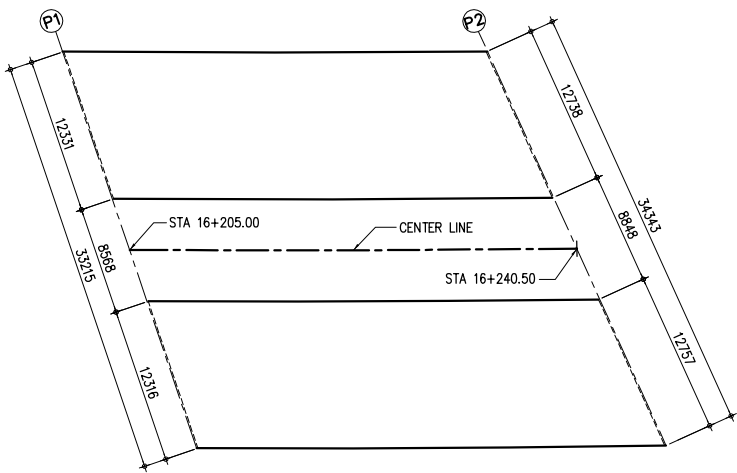


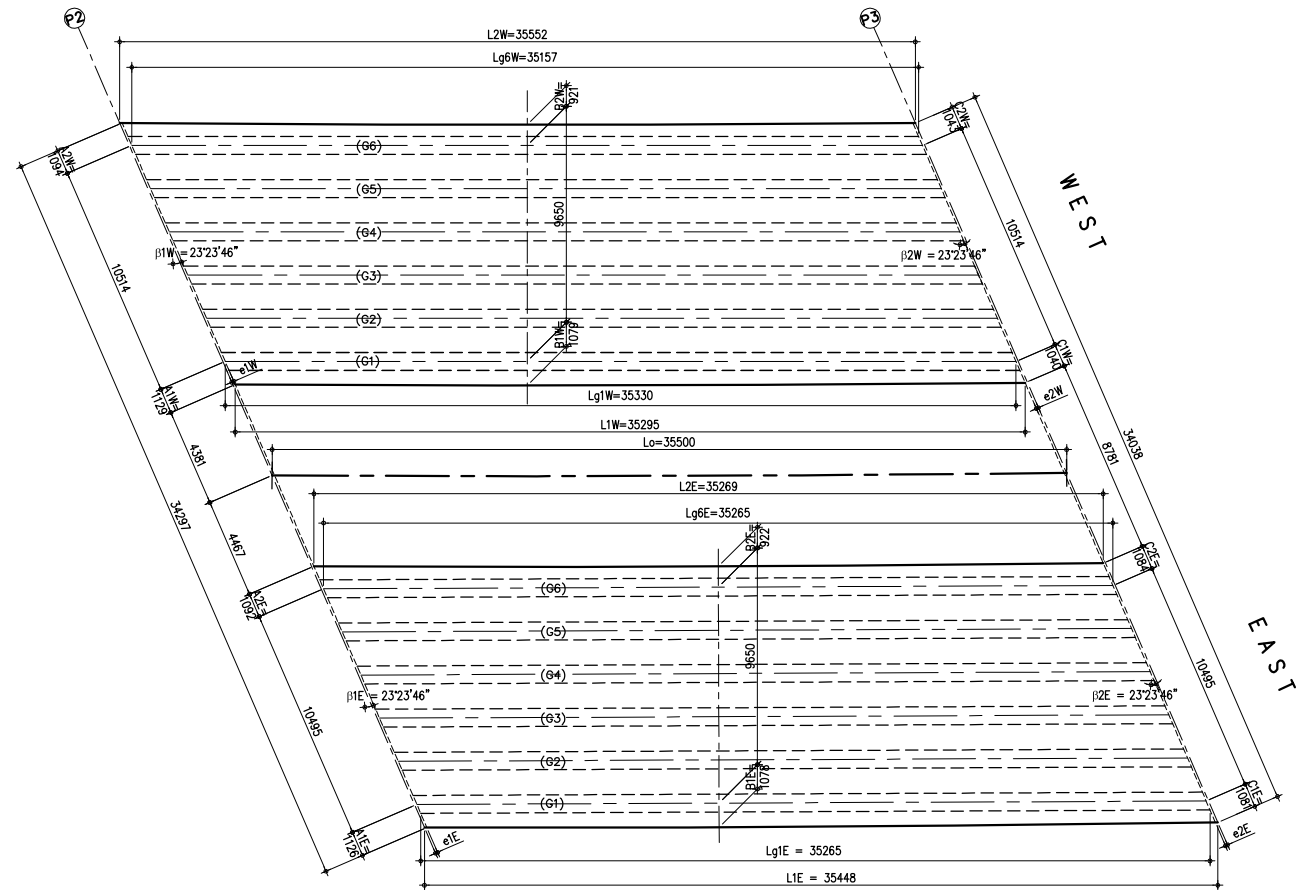


1 LAYOUT PLAN  
SCALE 1:150

	WEST BOUND
	P0~P1
Lo (m)	35.500
L1W (m)	34.830
L2W (m)	33.735
β1W (°)	18°41'23"
β2W (°)	23°43'21"
e1W/e2W (mm)	100/100
i1W (%)	2.5
i2W (%)	2.5
A1W/A2W (mm)	1085/1094
B1W/B2W (mm)	929/1071
C1W/C2W (mm)	1048/1050
Lg1W (m)	34.745
Lg2W (m)	34.522
Lg3W (m)	34.299
Lg4W (m)	34.077
Lg5W (m)	33.854
Lg6W (m)	33.632
SPAN LENGTH (m)	L=35.500
REMARKS	

	EAST BOUND
	P0~P1
Lo (m)	35.500
L1E (m)	37.258
L2E (m)	35.739
β1E (°)	18°41'23"
β2E (°)	23°43'21"
e1E/e2E (mm)	100/100
i1E (%)	2.5
i2E (%)	2.5
A1E/A2E (mm)	1085/1058
B1E/B2E (mm)	1080/920
C1E/C2E (mm)	1088/1092
Lg1E (m)	36.960
Lg2E (m)	36.738
Lg3E (m)	36.517
Lg4E (m)	36.295
Lg5E (m)	36.073
Lg6E (m)	35.851
SPAN LENGTH (m)	L=35.500
REMARKS	

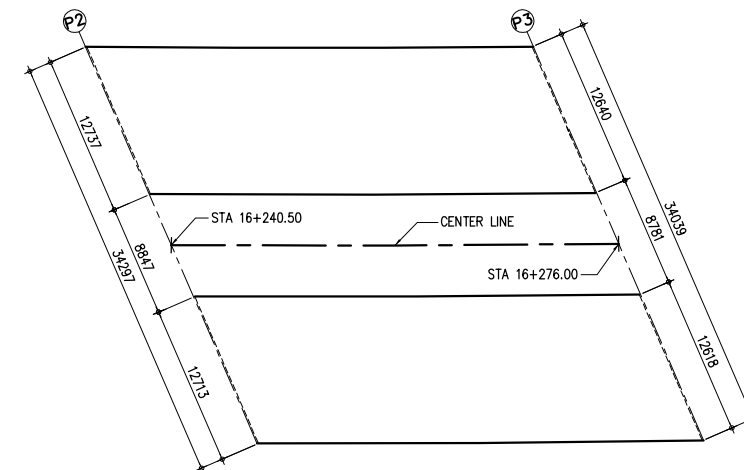


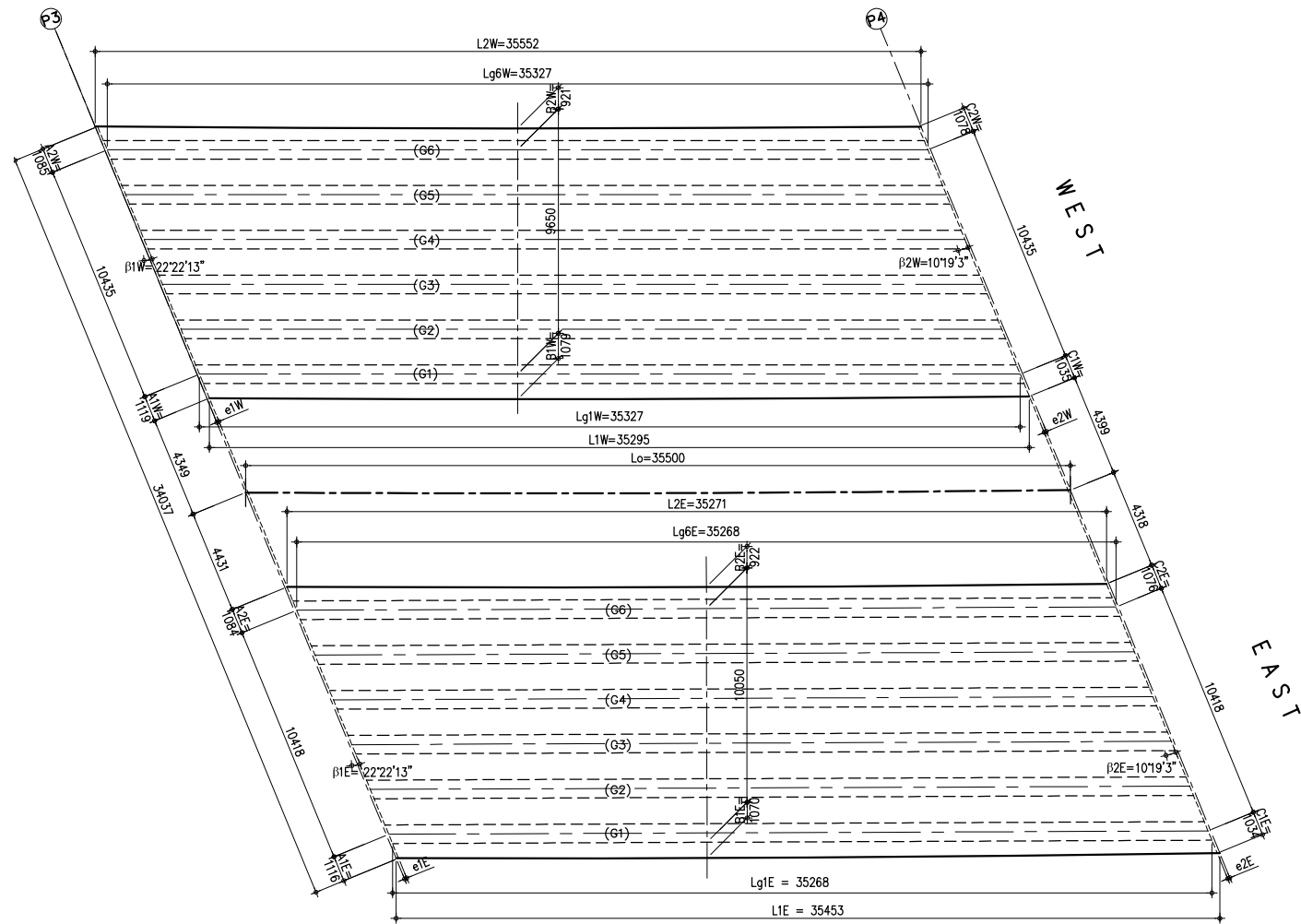


1 LAYOUT PLAN  
SCALE 1:150

	WEST BOUND
P2~P3	
Lo (m)	35.500
L1W (m)	35.295
L2W (m)	35.552
$\beta 1W$ (°)	23°43'21"
$\beta 2W$ (°)	22°42'20"
e1W/e2W (mm)	100/100
i1W (%)	2.5
i2W (%)	2.5
A1W/A2W (mm)	1129/1094
B1W/B2W (mm)	1079/921
C1W/C2W (mm)	1040/1043
Lg1W (m)	35.330
Lg2W (m)	35.330
Lg3W (m)	35.330
Lg4W (m)	35.330
Lg5W (m)	35.330
Lg6W (m)	35.330
SPAN LENGTH (m)	L=35.500
REMARKS	

	EAST BOUND
P2~P3	
Lo (m)	35.500
L1E (m)	35.448
L2E (m)	35.269
$\beta 1E$ (°)	23°43'21"
$\beta 2E$ (°)	22°42'20"
e1E/e2E (mm)	100/100
i1E (%)	2.5
i2E (%)	2.5
A1E/A2E (mm)	1126/1092
B1E/B2E (mm)	1078/922
C1E/C2E (mm)	1081/1084
Lg1E (m)	35.265
Lg2E (m)	35.266
Lg3E (m)	35.266
Lg4E (m)	35.266
Lg5E (m)	35.266
Lg6E (m)	35.266
SPAN LENGTH (m)	L=35.500
REMARKS	

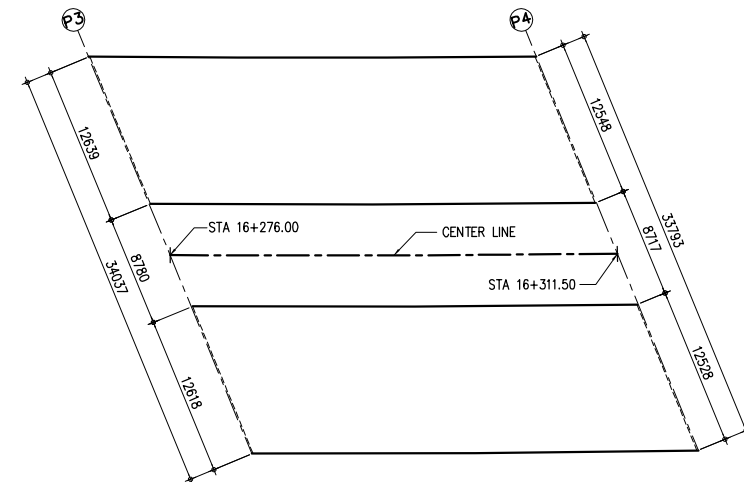


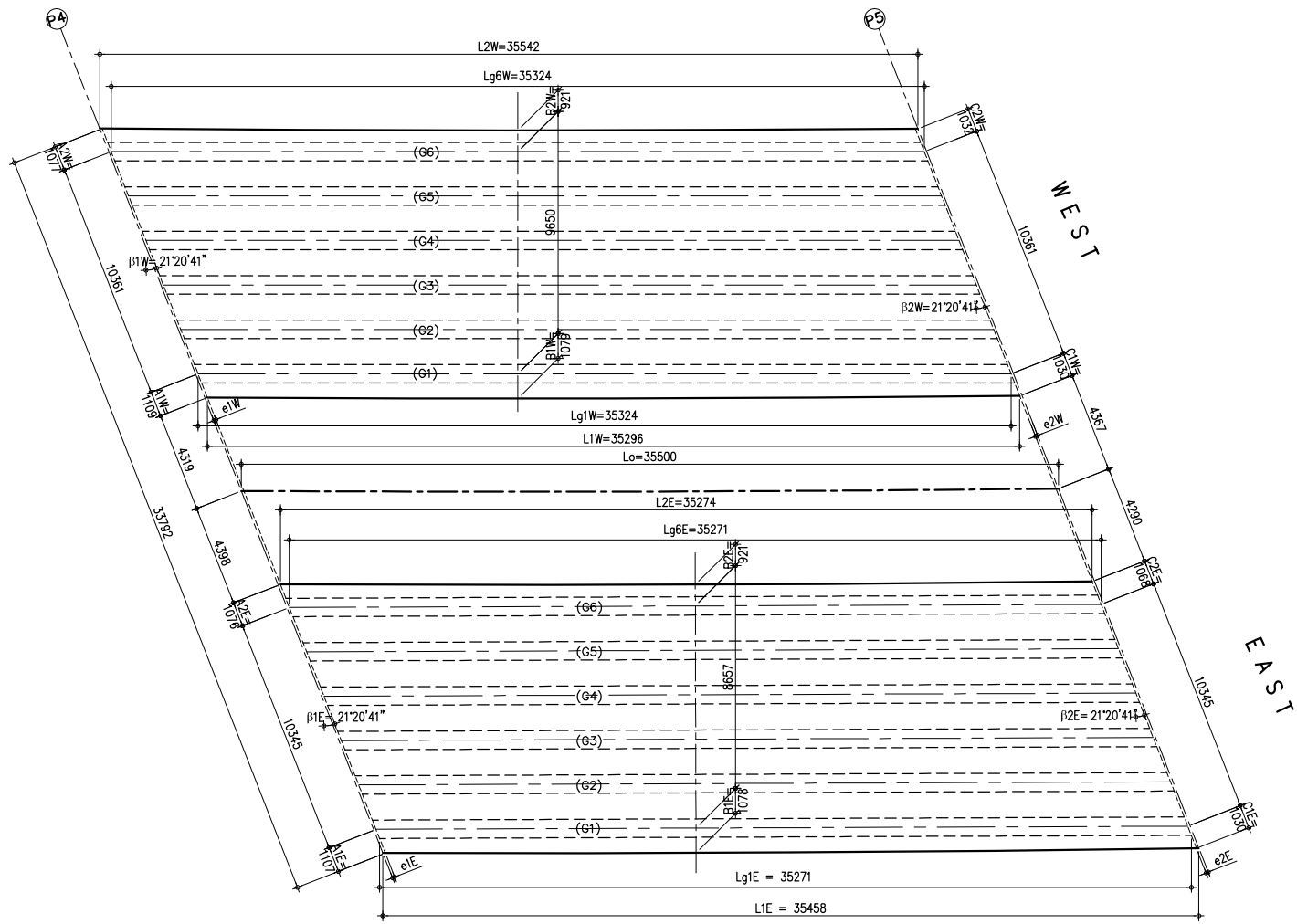


1 LAYOUT PLAN  
SCALE 1:150

	WEST BOUND
	P3~P4
Lo (m)	35.500
L1W (m)	35.295
L2W (m)	35.552
β1W (°)	22°42'20"
β2W (°)	21°41'19"
e1W/e2W (mm)	100/100
i1W (%)	2.5
i2W (%)	2.5
A1W/A2W (mm)	1119/1085
B1W/B2W (mm)	1079/921
C1W/C2W (mm)	1035/1078
Lg1W (m)	35.327
Lg2W (m)	35.327
Lg3W (m)	35.327
Lg4W (m)	35.327
Lg5W (m)	35.327
Lg6W (m)	35.327
SPAN LENGTH (m)	L=35.500
REMARKS	

	EAST BOUND
	P3~P4
Lo (m)	35.500
L1E (m)	35.453
L2E (m)	35.271
β1E (°)	22°42'20"
β2E (°)	21°41'19"
e1E/e2E (mm)	100/100
i1E (%)	2.5
i2E (%)	2.5
A1E/A2E (mm)	1116/1084
B1E/B2E (mm)	1070/922
C1E/C2E (mm)	1034/1076
Lg1E (m)	35.268
Lg2E (m)	35.269
Lg3E (m)	35.269
Lg4E (m)	35.269
Lg5E (m)	35.269
Lg6E (m)	35.269
SPAN LENGTH (m)	L=35.500
REMARKS	

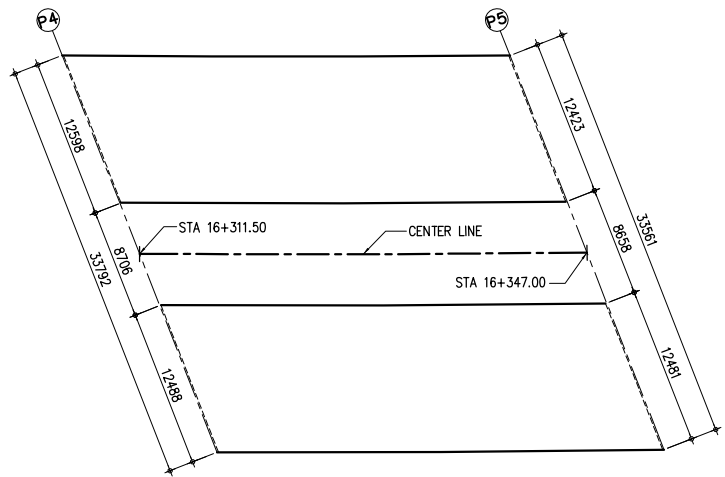


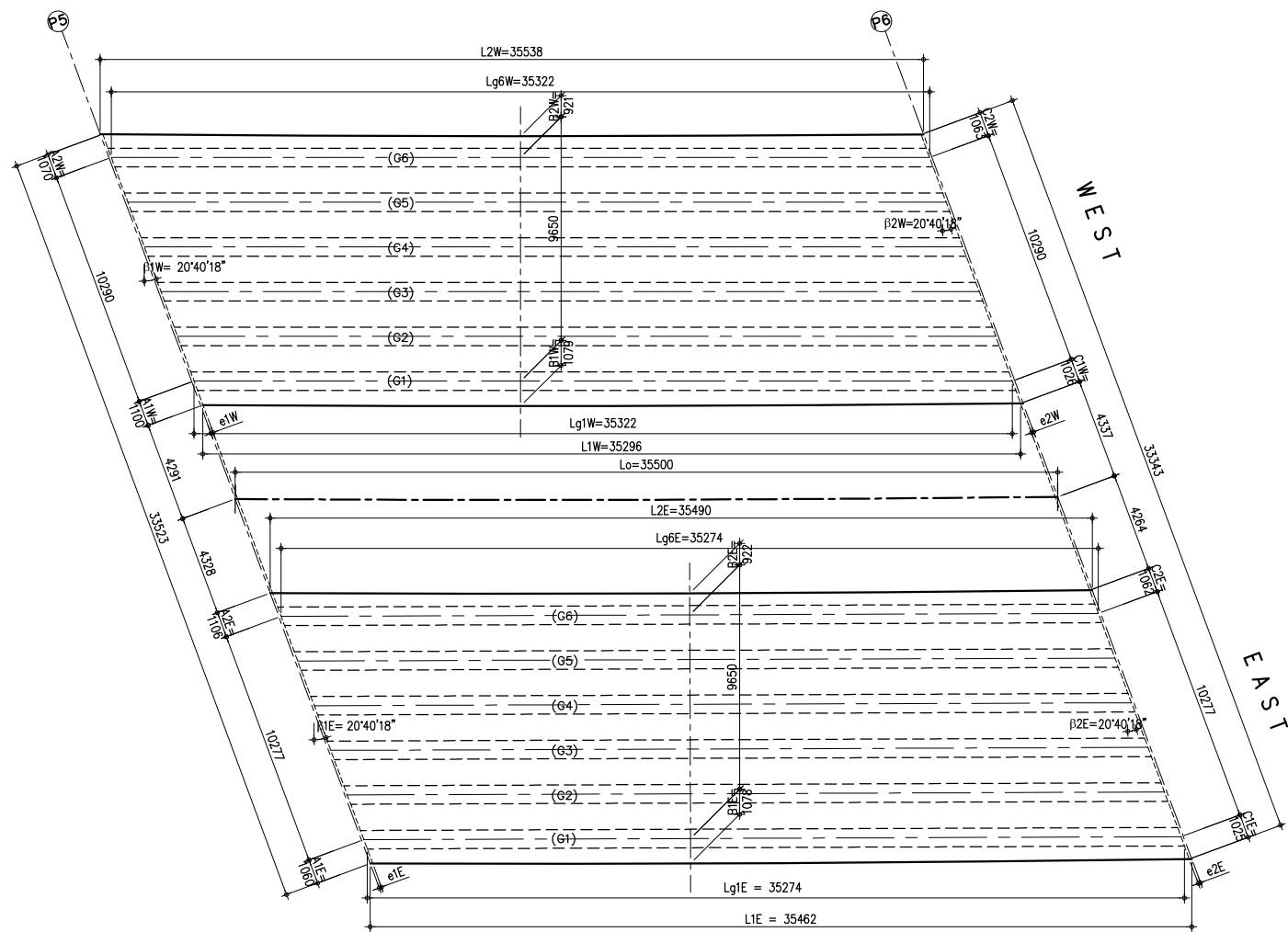


1 LAYOUT PLAN  
SCALE 1:150

	WEST BOUND	
	P4~P5	
Lo (m)	35.500	
L1W (m)	35.296	
L2W (m)	35.542	
$\beta 1W (^{\circ})$	21'41'19"	
$\beta 2W (^{\circ})$	20'40'18"	
e1W/e2W (mm)	100/100	
i1W (%)	2.5	
i2W (%)	2.5	
A1W/A2W (mm)	1109/1077	
B1W/B2W(mm)	1079/921	
C1W/C2W (mm)	1030/1032	
Lg1W (m)	35.324	
Lg2W (m)	35.324	
Lg3W (m)	35.324	
Lg4W (m)	35.324	
Lg5W (m)	35.324	
Lg6W (m)	35.324	
SPAN LENGTH (m)	L=35.500	
REMARKS		

	EAST BOUND	
	P4~P5	
Lo (m)	35.500	
L1E (m)	35.458	
L2E (m)	35.274	
$\beta 1E (^{\circ})$	21'41'19"	
$\beta 2E (^{\circ})$	20'40'18"	
e1E/e2E (mm)	100/100	
i1E (%)	2.5	
i2E (%)	2.5	
A1E/A2E (mm)	1107/1076	
B1E/B2E (mm)	1078/921	
C1E/C2E (mm)	1030/1068	
Lg1E (m)	35.271	
Lg2E (m)	35.272	
Lg3E (m)	35.272	
Lg4E (m)	35.272	
Lg5E (m)	35.272	
Lg6E (m)	35.272	
SPAN LENGTH (m)	L=35.500	
REMARKS		

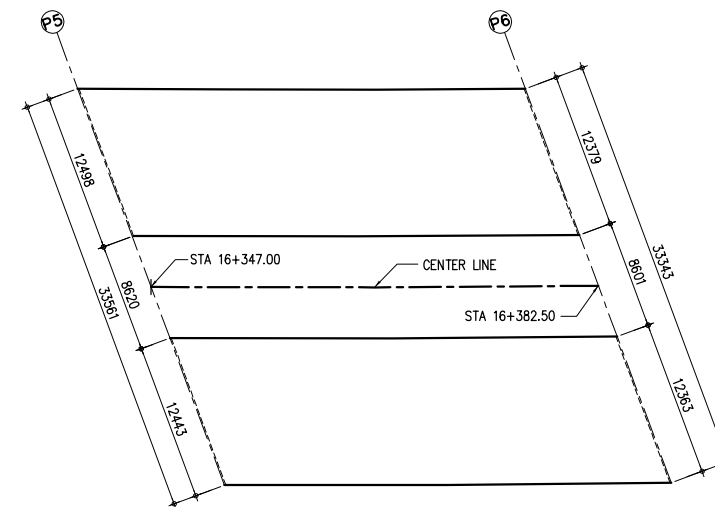


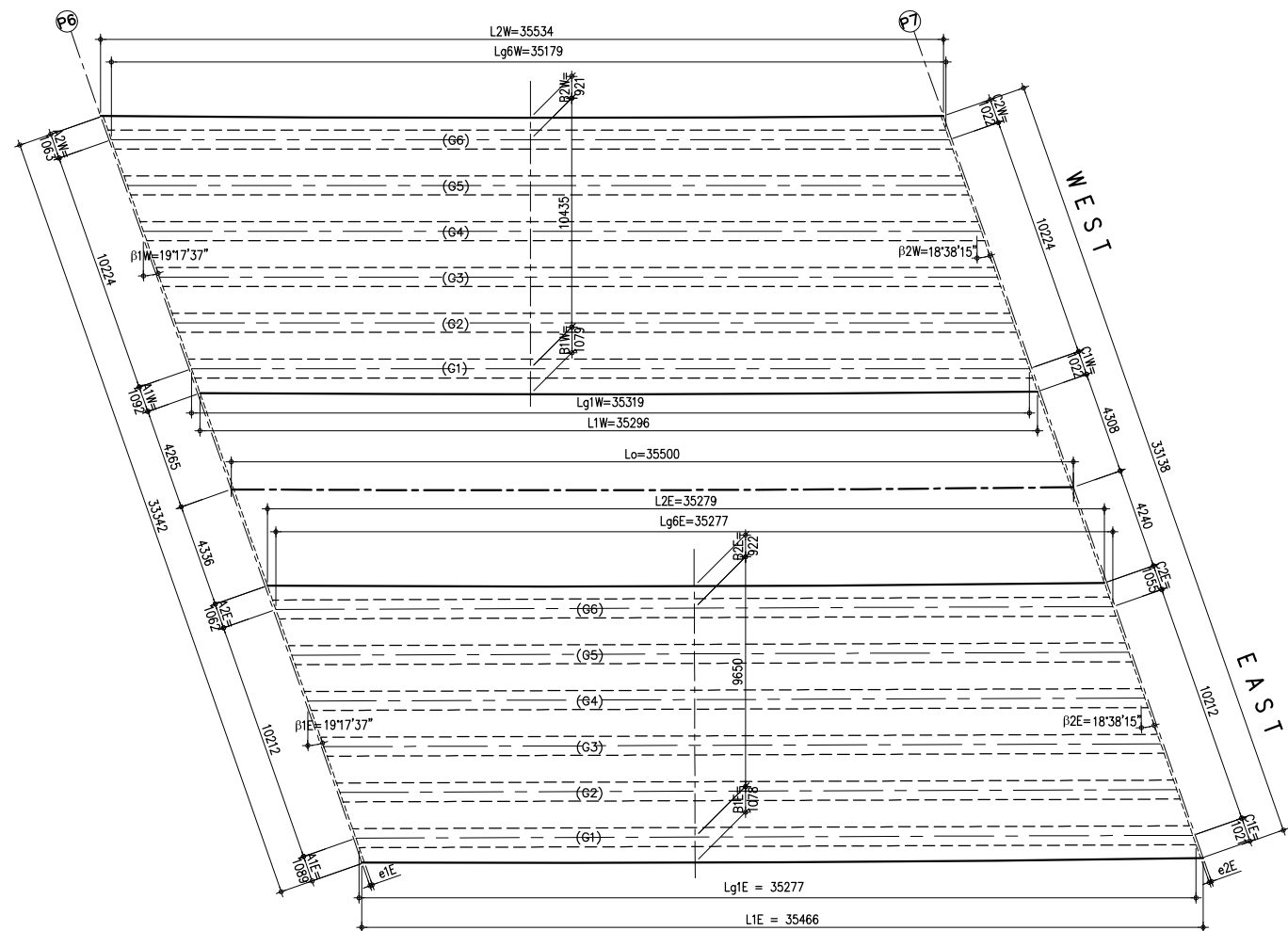


1 LAYOUT PLAN  
SCALE 1:150

	WEST BOUND
	P5~P6
Lo (m)	35.500
L1W (m)	35.402
L2W (m)	35.538
β1W (°)	20°40'18"
β2W (°)	20°40'18"
e1W/e2W (mm)	100/100
i1W (%)	2.5
i2W (%)	2.5
A1W/A2W (mm)	1100/1070
B1W/B2W (mm)	1079/921
C1W/C2W (mm)	1026/1063
Lg1W (m)	35.322
Lg2W (m)	35.322
Lg3W (m)	35.322
Lg4W (m)	35.322
Lg5W (m)	35.322
Lg6W (m)	35.322
SPAN LENGTH (m)	L=35.500
REMARKS	

	EAST BOUND
	P5~P6
Lo (m)	35.500
L1E (m)	35.462
L2E (m)	35.490
β1E (°)	20°40'18"
β2E (°)	20°40'18"
e1E/e2E (mm)	100/100
i1E (%)	2.5
i2E (%)	2.5
A1E/A2E (mm)	1060/1106
B1E/B2E (mm)	1078/922
C1E/C2E (mm)	1025/1062
Lg1E (m)	35.274
Lg2E (m)	35.275
Lg3E (m)	35.275
Lg4E (m)	35.275
Lg5E (m)	35.275
Lg6E (m)	35.275
SPAN LENGTH (m)	L=35.500
REMARKS	

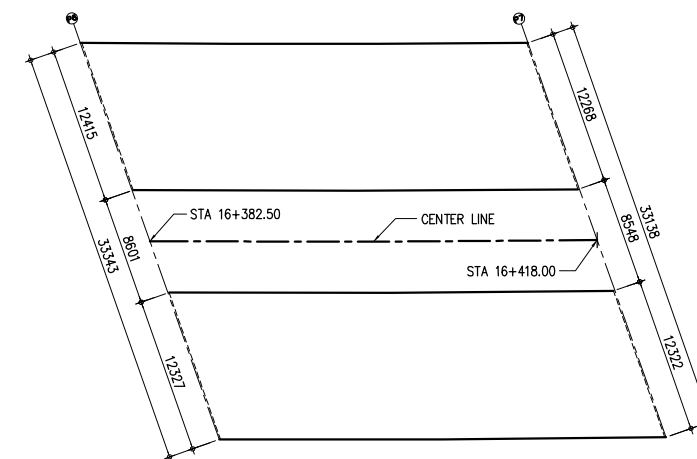




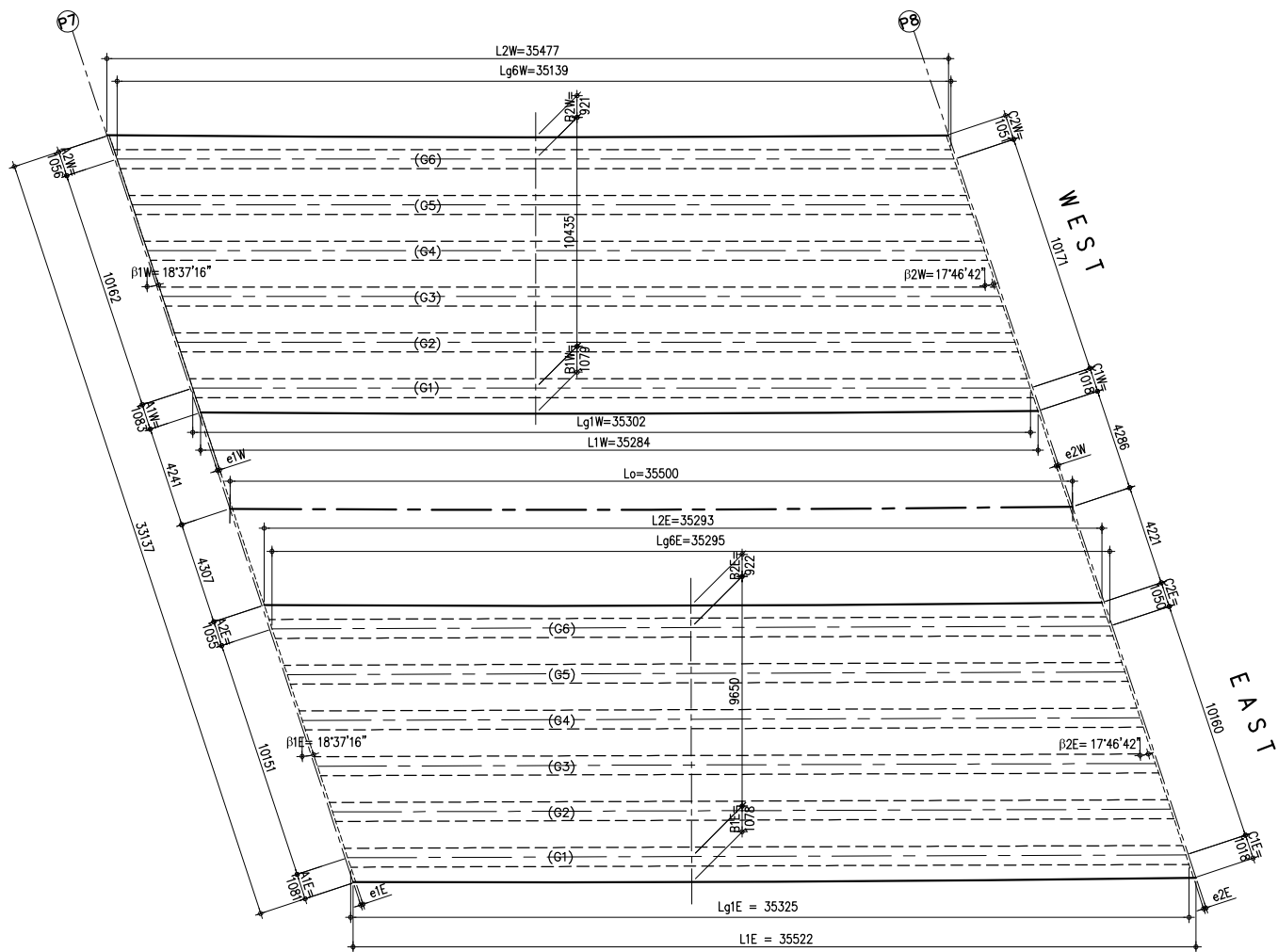
1 LAYOUT PLAN  
SCALE 1:150

	EAST BOUND
	P6~P7
Lo (m)	35.500
L1W (m)	35.296
L2W (m)	35.534
$\beta 1W$ (°)	19°17'37"
$\beta 2W$ (°)	18°38'15"
e1W/e2W (mm)	100/100
i1W (%)	2.5
i2W (%)	2.5
A1W/A2W (mm)	1092/1063
B1W/B2W (mm)	1079/921
C1W/C2W (mm)	1022/1022
Lg1W (m)	35.319
Lg2W (m)	35.319
Lg3W (m)	35.319
Lg4W (m)	35.319
Lg5W (m)	35.319
Lg6W (m)	35.319
SPAN LENGTH (m)	L=35.500
REMARKS	

	WEST BOUND
	P6~P7
Lo (m)	35.500
L1E (m)	35.466
L2E (m)	35.279
$\beta 1E$ (°)	19°17'37"
$\beta 2E$ (°)	18°38'15"
e1E/e2E (mm)	100/100
i1E (%)	2.5
i2E (%)	2.5
A1E/A2E (mm)	1089/1062
B1E/B2E (mm)	1078/922
C1E/C2E (mm)	1021/1055
Lg1E (m)	35.277
Lg2E (m)	35.277
Lg3E (m)	35.277
Lg4E (m)	35.277
Lg5E (m)	35.277
Lg6E (m)	35.277
SPAN LENGTH (m)	L=35.500
REMARKS	



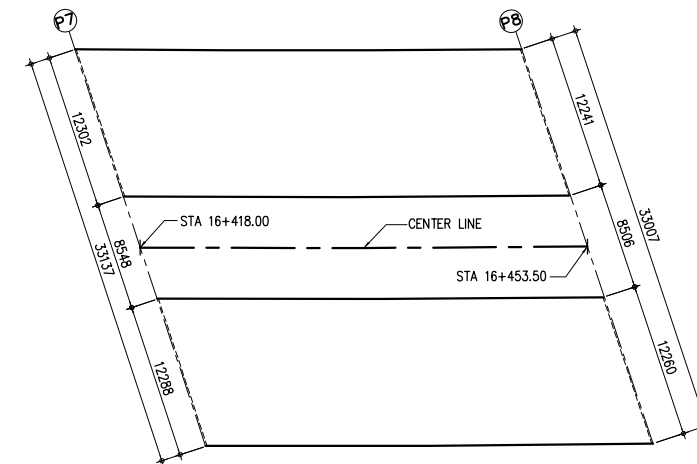


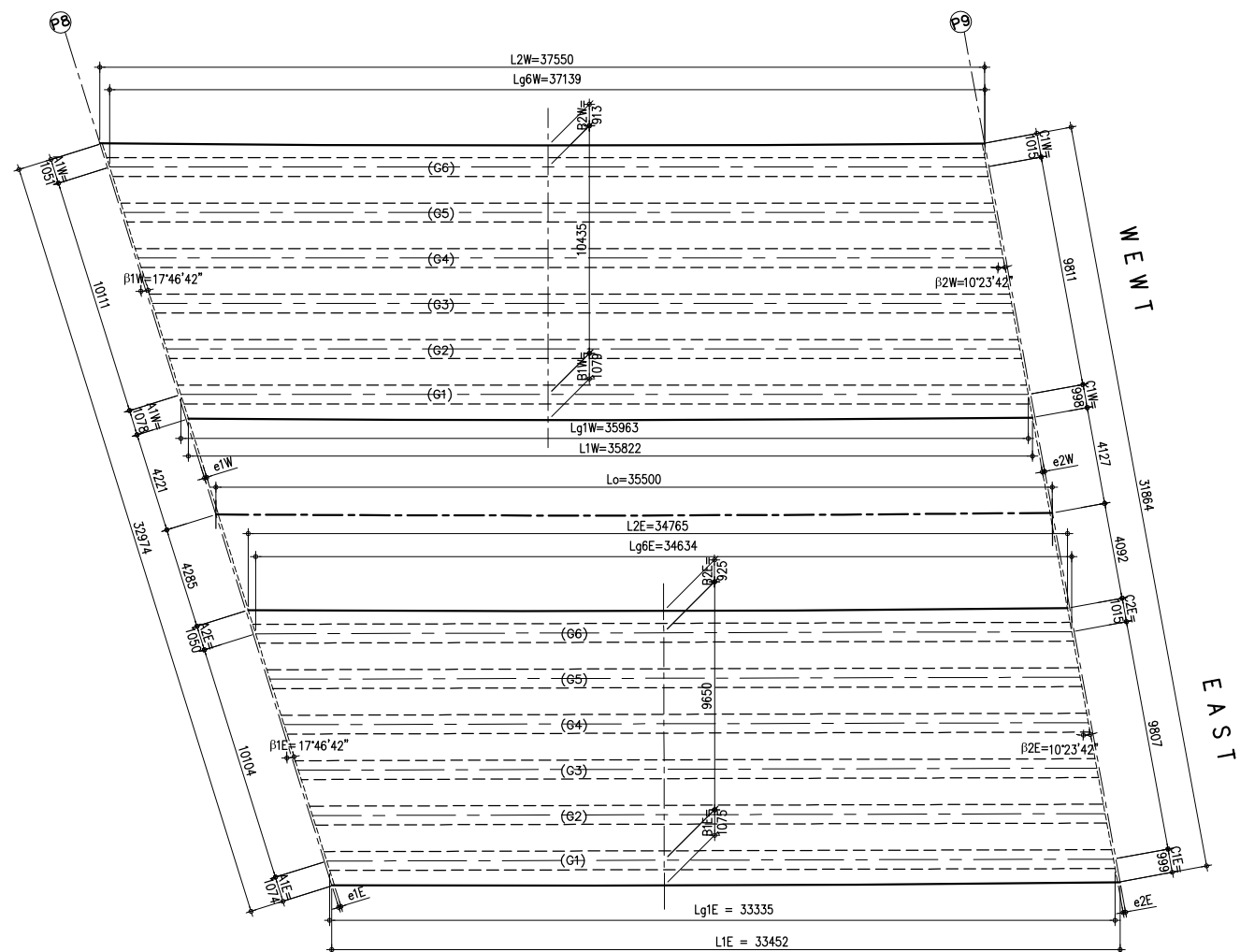


**1 LAYOUT PLAN**  
SCALE 1:150

	WEST BOUND
	P7~P8
Lo (m)	35.500
L1W (m)	35.302
L2W (m)	35.477
$\beta 1W$ (°)	17°46'42"
$\beta 2W$ (°)	18°38'15"
e1W/e2W (mm)	100/100
i1W (%)	2.5
i2W (%)	2.5
A1W/A2W (mm)	1018/1051
B1W/B2W (mm)	1079/921
C1W/C2W (mm)	1018/1019
Lg1W (m)	35.284
Lg2W (m)	35.296
Lg3W (m)	35.290
Lg4W (m)	35.284
Lg5W (m)	35.278
Lg6W (m)	35.319
SPAN LENGTH (m)	L=35.500
REMARKS	

	EAST BOUND
	P7~P8
Lo (m)	35.500
L1E (m)	35.522
L2E (m)	35.293
$\beta 1E$ (°)	17°46'42"
$\beta 2E$ (°)	18°38'15"
e1E/e2E (mm)	100/100
i1E (%)	2.5
i2E (%)	2.5
A1E/A2E (mm)	1081/1055
B1E/B2E (mm)	1078/922
C1E/C2E (mm)	1018/1050
Lg1E (m)	35.325
Lg2E (m)	35.318
Lg3E (m)	35.312
Lg4E (m)	35.307
Lg5E (m)	35.301
Lg6E (m)	35.295
SPAN LENGTH (m)	L=35.500
REMARKS	

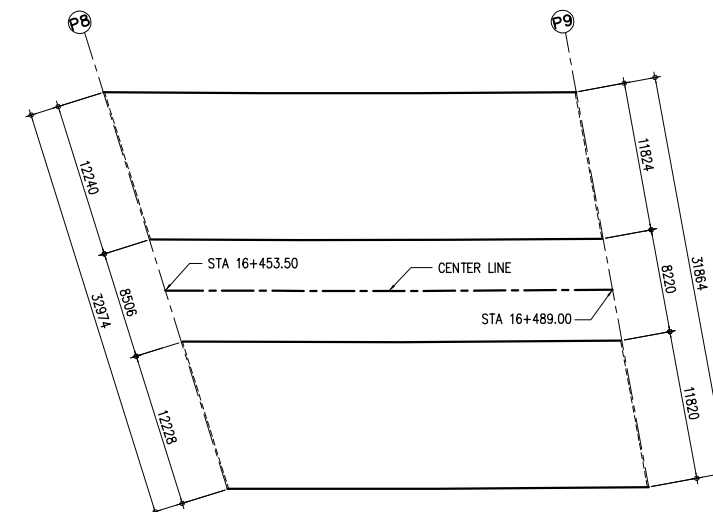


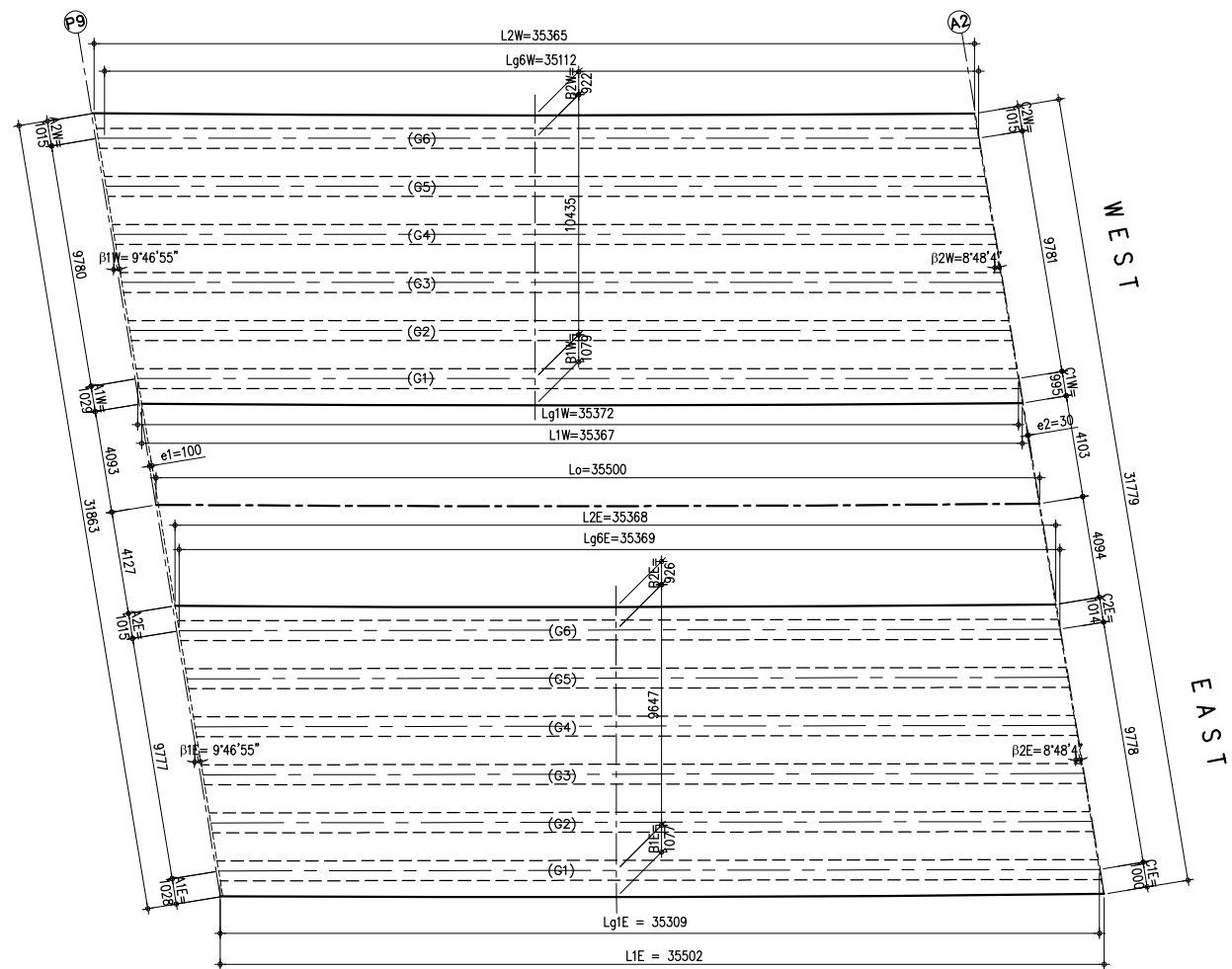


1 LAYOUT PLAN  
SCALE 1:150

	WEST BOUND
	P8~P9
Lo (m)	35.500
L1W(m)	35.822
L2W(m)	37.550
β1W(°)	17°46'42"
β2W(°)	10°23'42"
e1S/e2W(mm)	100/100
i1W(%)	2.5
i2W(%)	2.5
A1S/A2W(mm)	1078/1051
B1S/B2W(mm)	1079/921
C1S/C2W(mm)	998/1015
Lg1W(m)	35.963
Lg2W(m)	36.213
Lg3W(m)	36.463
Lg4W(m)	36.713
Lg5W(m)	36.963
Lg6W(m)	37.213
SPAN LENGTH (m)	L=35.500
REMARKS	

	EAST BOUND
	P8~P9
Lo (m)	35.500
L1E (m)	33.452
L2E (m)	34.765
β1E (°)	17°46'42"
β2E (°)	10°23'42"
e1E/e2E (mm)	100/100
i1E (%)	2.5
i2E (%)	2.5
A1E/A2E (mm)	1074/1050
B1E/B2E (mm)	1075/925
C1E/C2E (mm)	999/1015
Lg1E (m)	33.335
Lg2E (m)	33.636
Lg3E (m)	33.886
Lg4E (m)	34.135
Lg5E (m)	34.385
Lg6E (m)	34.634
SPAN LENGTH (m)	L=35.500
REMARKS	

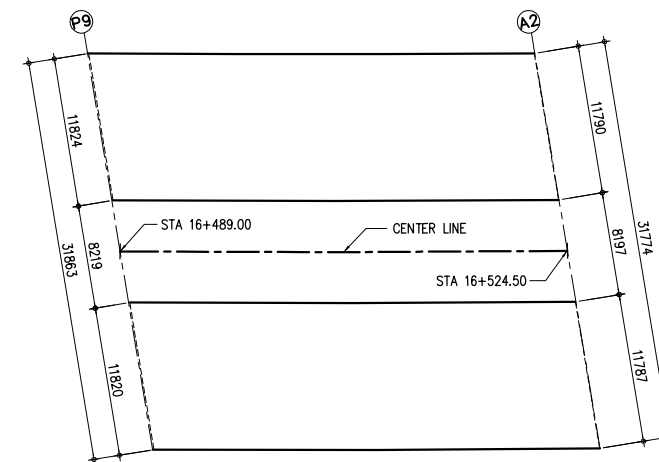


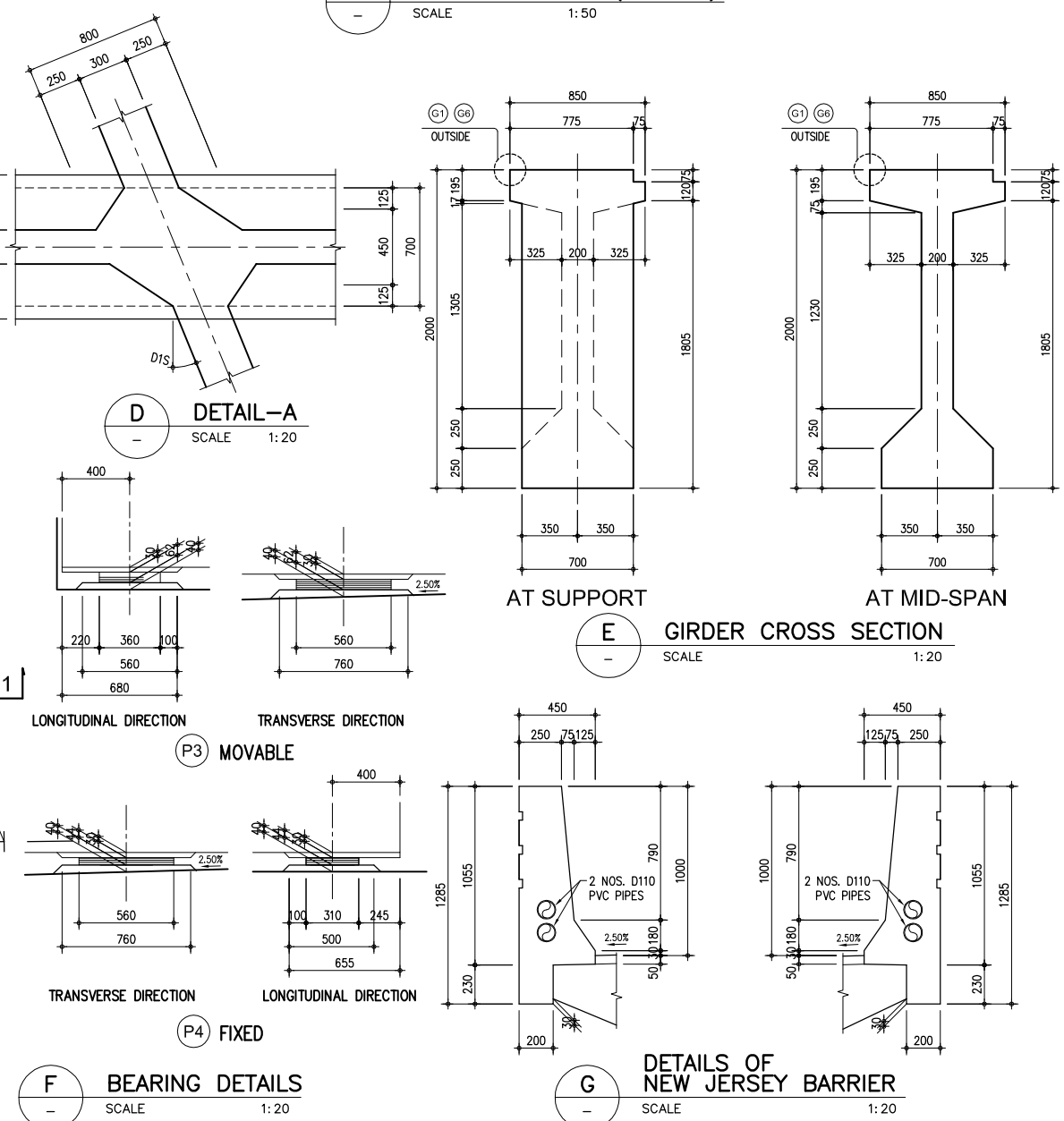
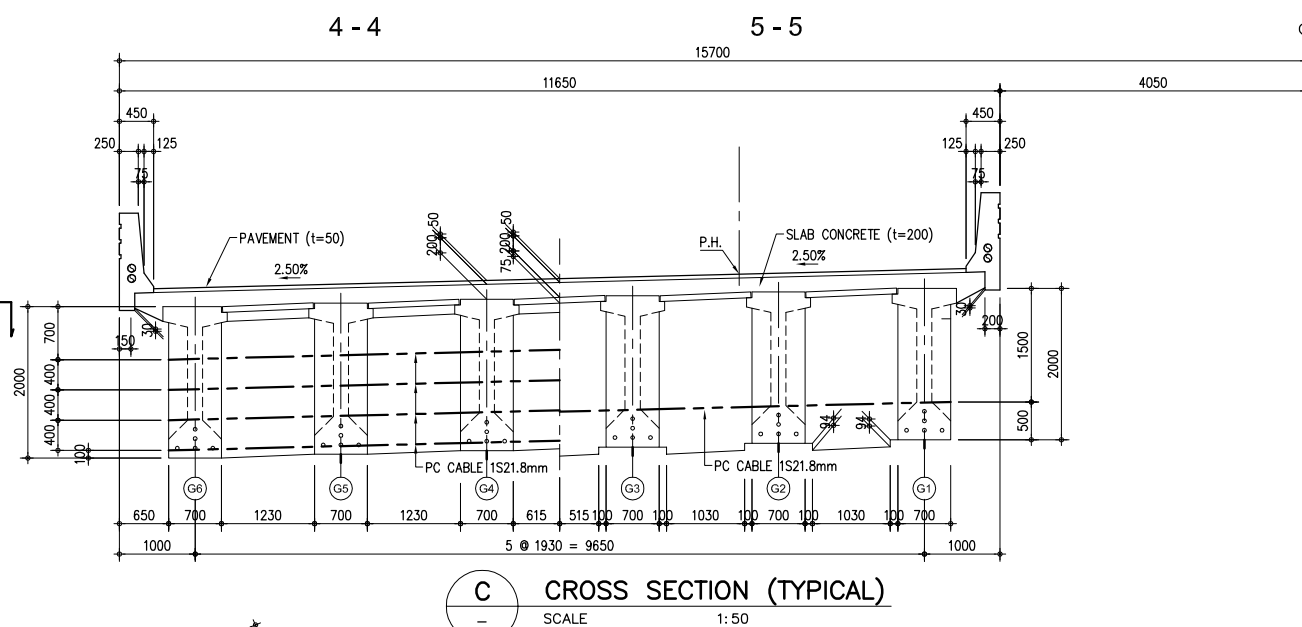


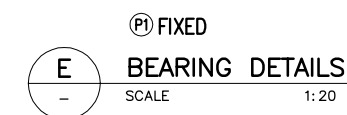
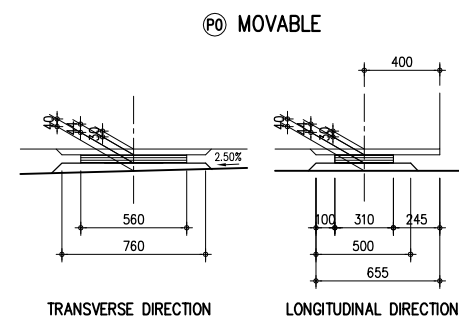
1 LAYOUT PLAN  
- WCALE 1:150

	WEWT BOUED
	P9~A2
Lo (m)	35.500
L1W (m)	35.367
L2W (m)	37.365
β1W (°)	9°46'55"
β2W (°)	8°48'4"
e1W/e2W (mm)	100/30
i1W (%)	2.5
i2W (%)	2.5
A1W/A2W (mm)	1029/1015
B1W/B2W(mm)	1079/922
C1W/C2W (mm)	995/1015
Lg1W (m)	35.372
Lg2W (m)	35.371
Lg3W (m)	35.369
Lg4W (m)	35.368
Lg5W (m)	35.367
Lg6W (m)	35.366
SPAN LENGTH (m)	L=35.500
REMARKS	

	EAWT BOUED
	P9~A2
Lo (m)	35.500
L1E (m)	35.502
L2E (m)	35.368
β1E (°)	9°46'55"
β2E (°)	8°48'4"
e1E/e2E (mm)	100/30
i1E (%)	2.5
i2E (%)	2.5
A1E/A2E (mm)	1028/1015
B1E/B2E (mm)	1077/926
C1E/C2E (mm)	1000/1014
Lg1E (m)	35.309
Lg2E (m)	35.374
Lg3E (m)	35.373
Lg4E (m)	35.371
Lg5E (m)	35.370
Lg6E (m)	35.369
SPAN LENGTH (m)	L=35.500
REMARKS	

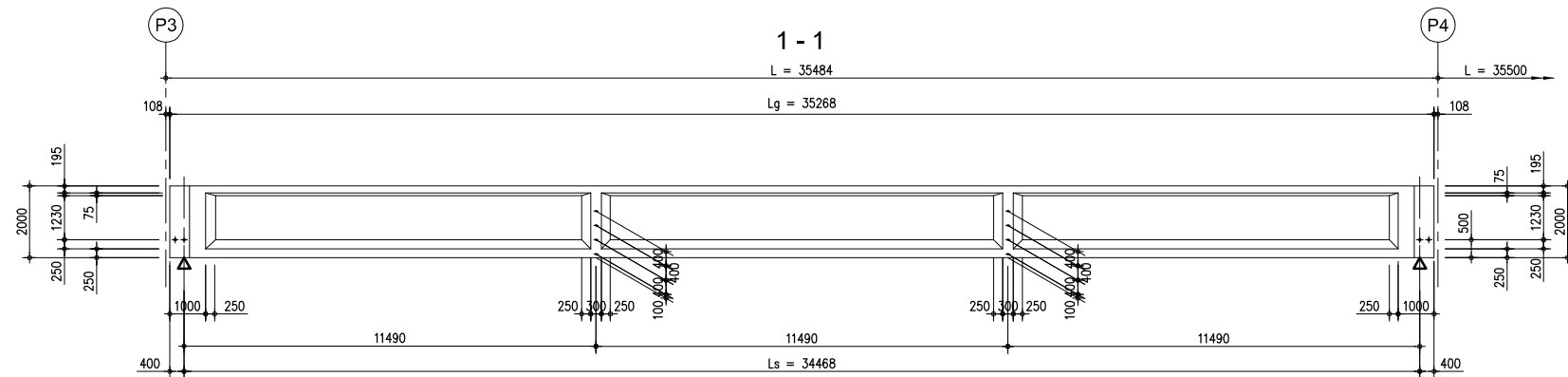




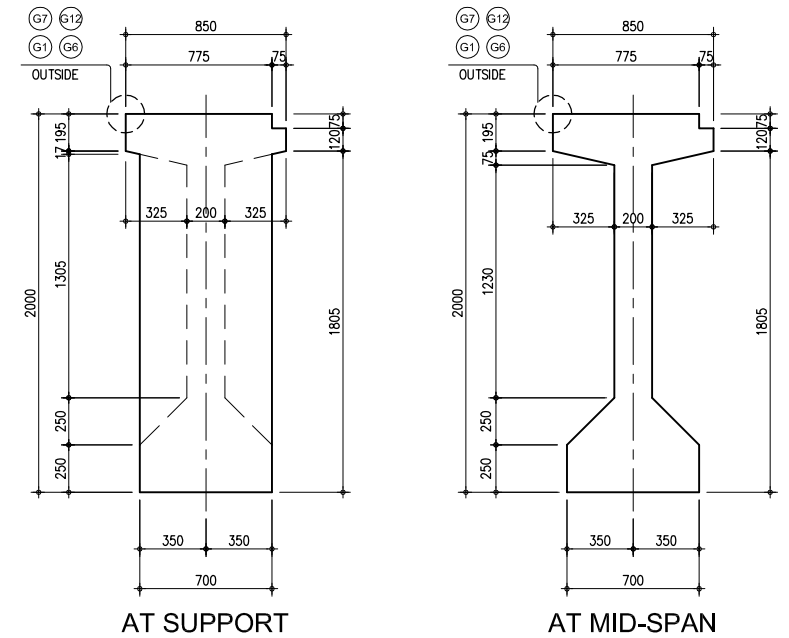




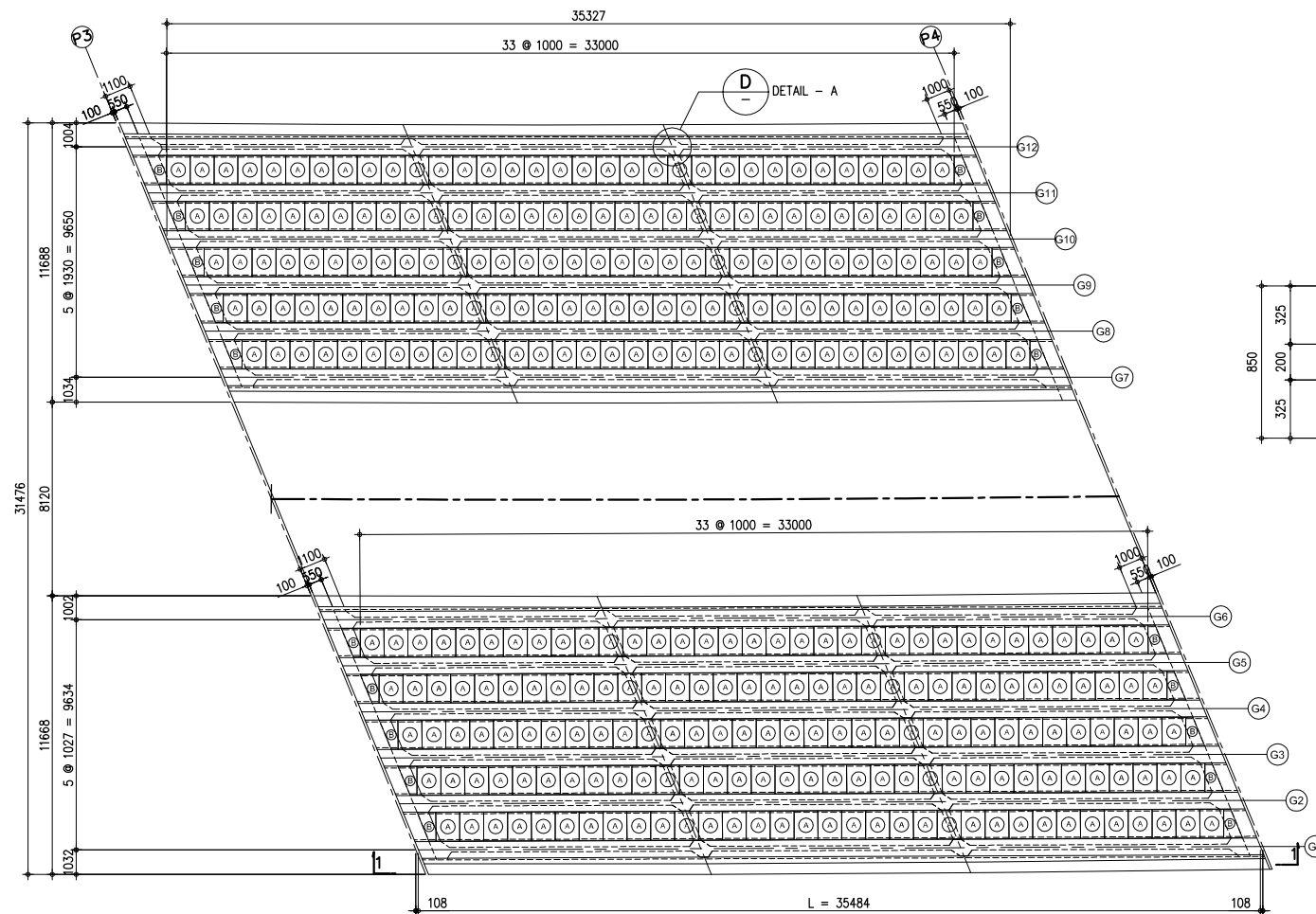




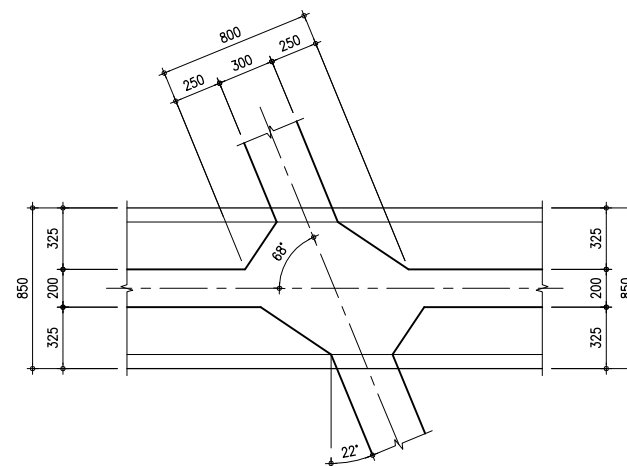
**B** SIDE VIEW  
SCALE 1:100



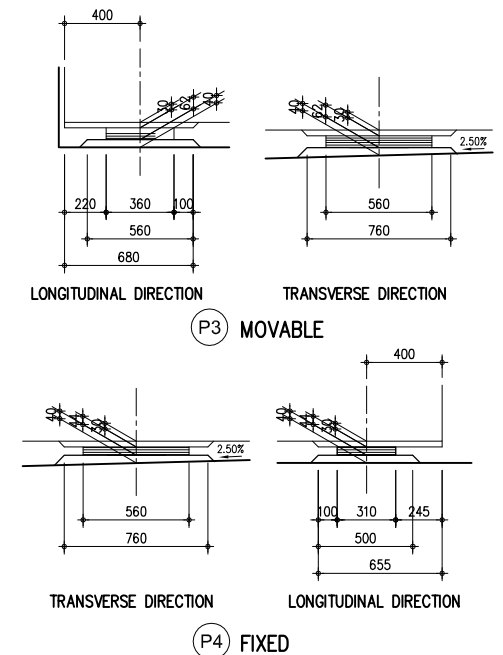
**C** GIRDER CROSS SECTION  
SCALE 1:20



**A** PRECAST PLATES LAYOUT  
SCALE 1:150

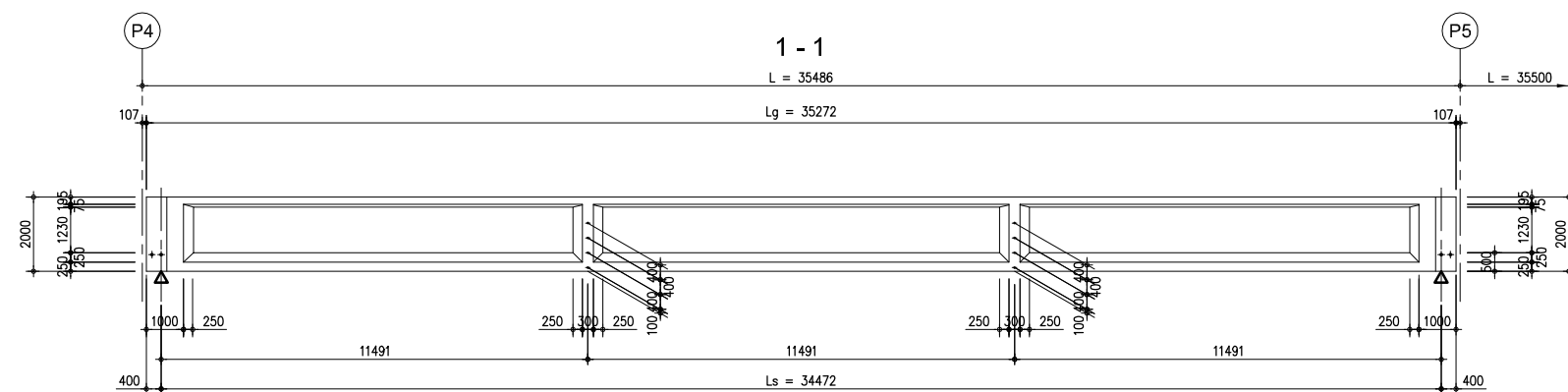


**D** DETAIL-A  
SCALE 1:20

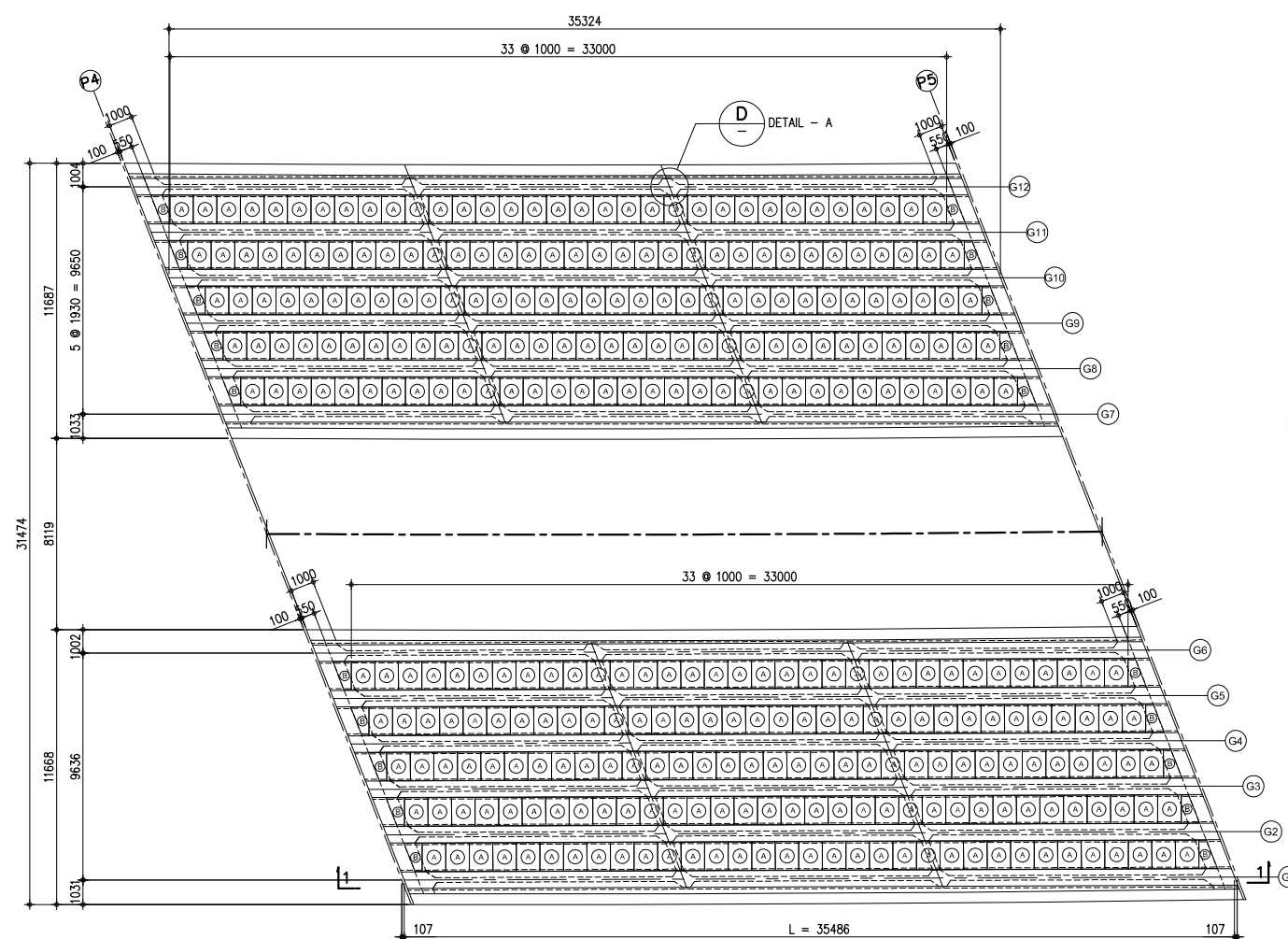


**E** BEARING DETAILS  
SCALE 1:20

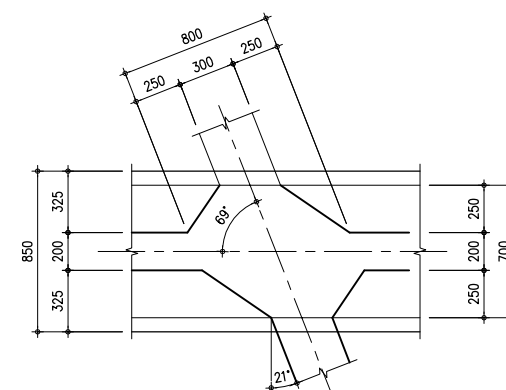




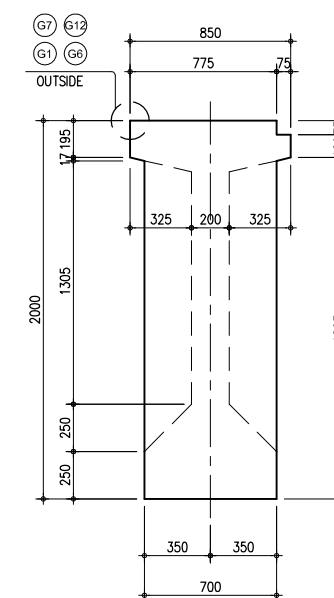
**B** SIDE VIEW  
SCALE 1:100



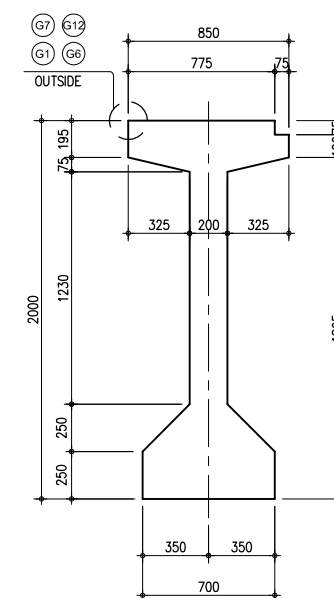
**A** PRECAST PLATES LAYOUT  
SCALE 1:150



**D** DETAIL-A  
SCALE 1:20

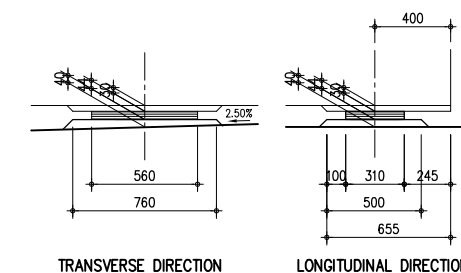


AT SUPPORT

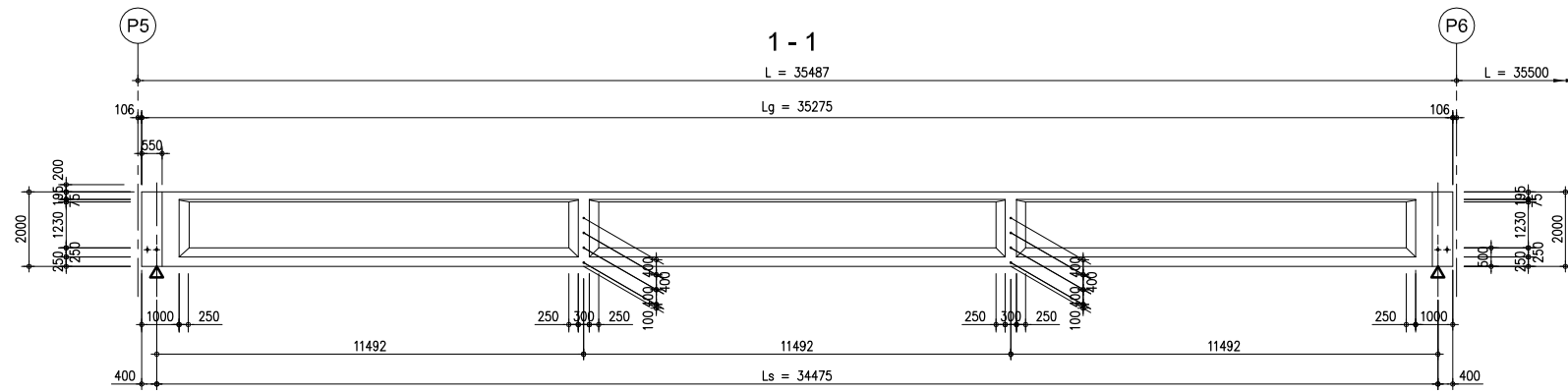


AT MID-SPAN

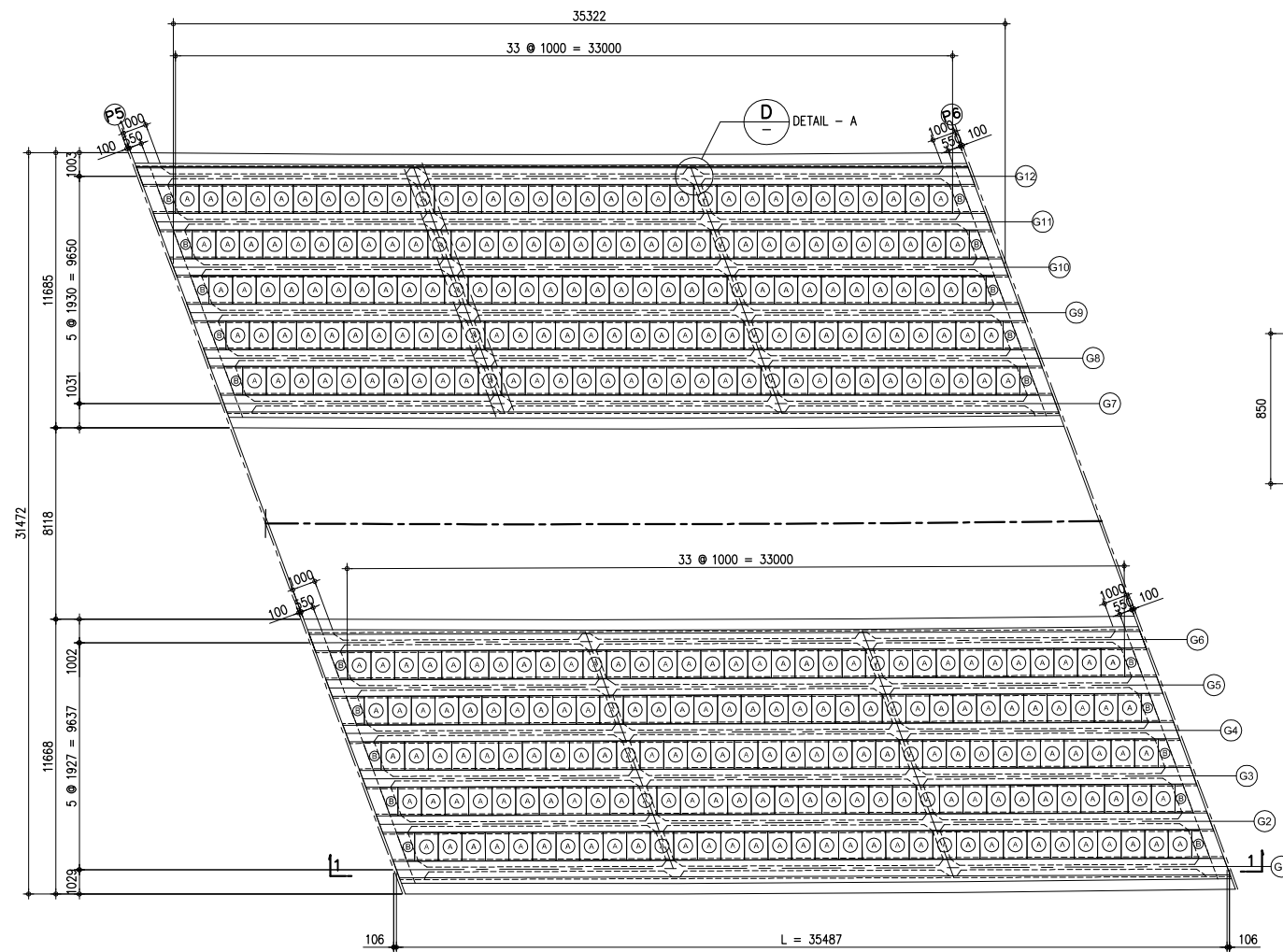
**C** GIRDER CROSS SECTION  
SCALE 1:20



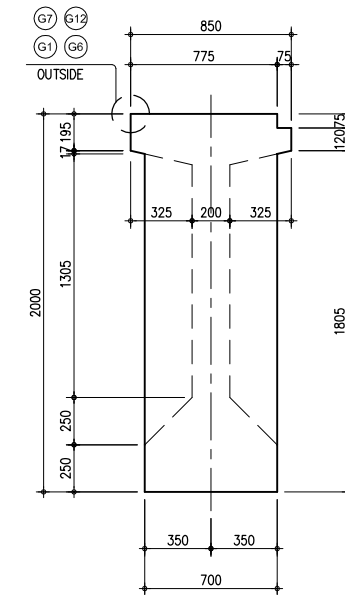
**E** BEARING DETAILS  
SCALE 1:20



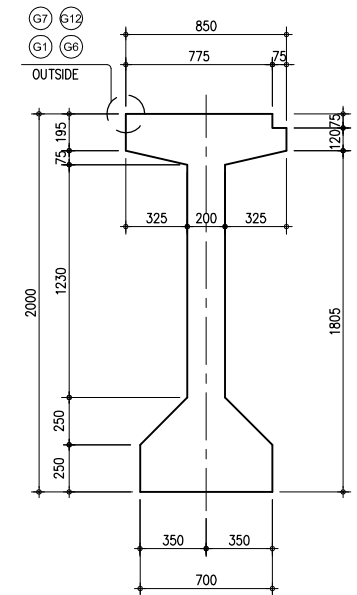
**B** SIDE VIEW  
SCALE 1:100



**A** PRECAST PLATES LAYOUT  
SCALE 1:150

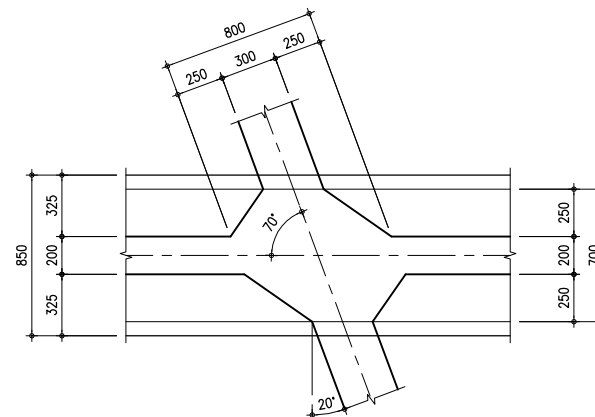


AT SUPPORT

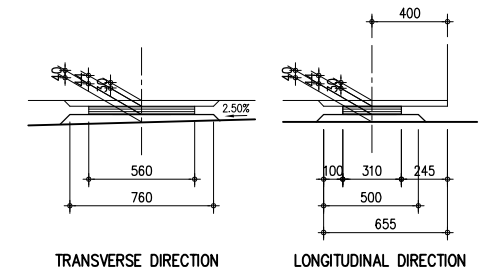


AT MID-SPAN

**C** GIRDER CROSS SECTION  
SCALE 1:20

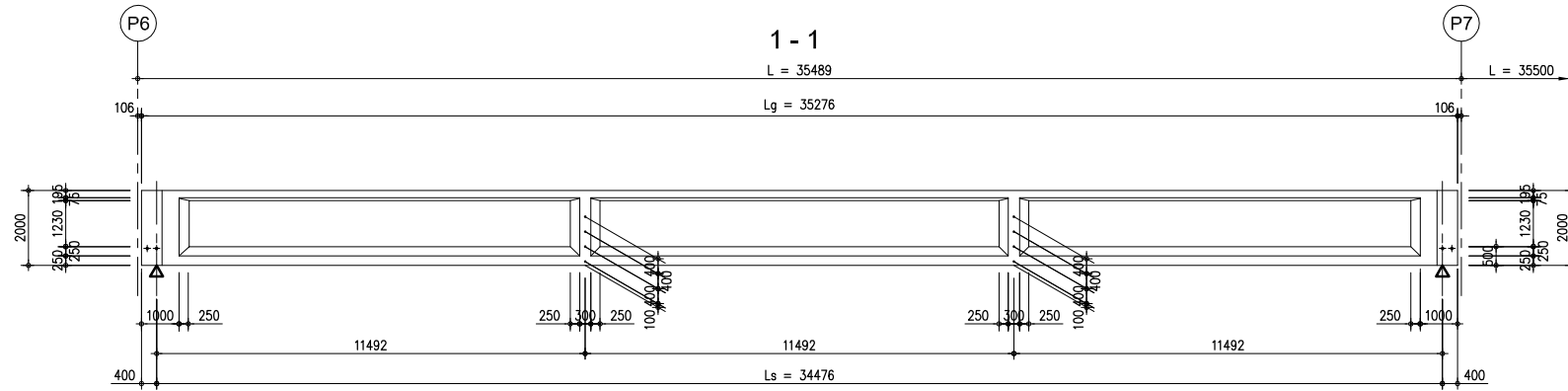


**D** DETAIL-A  
SCALE 1:20

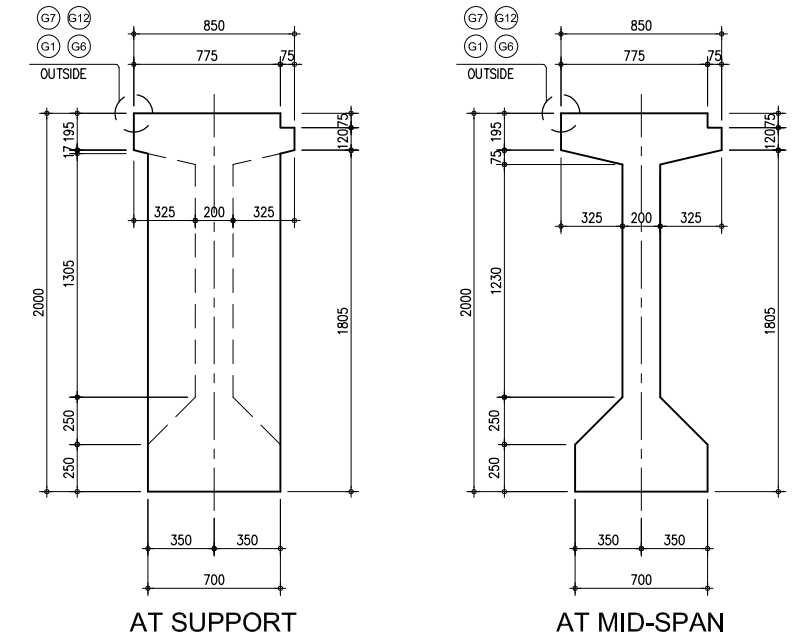


P5 P6 FIXED

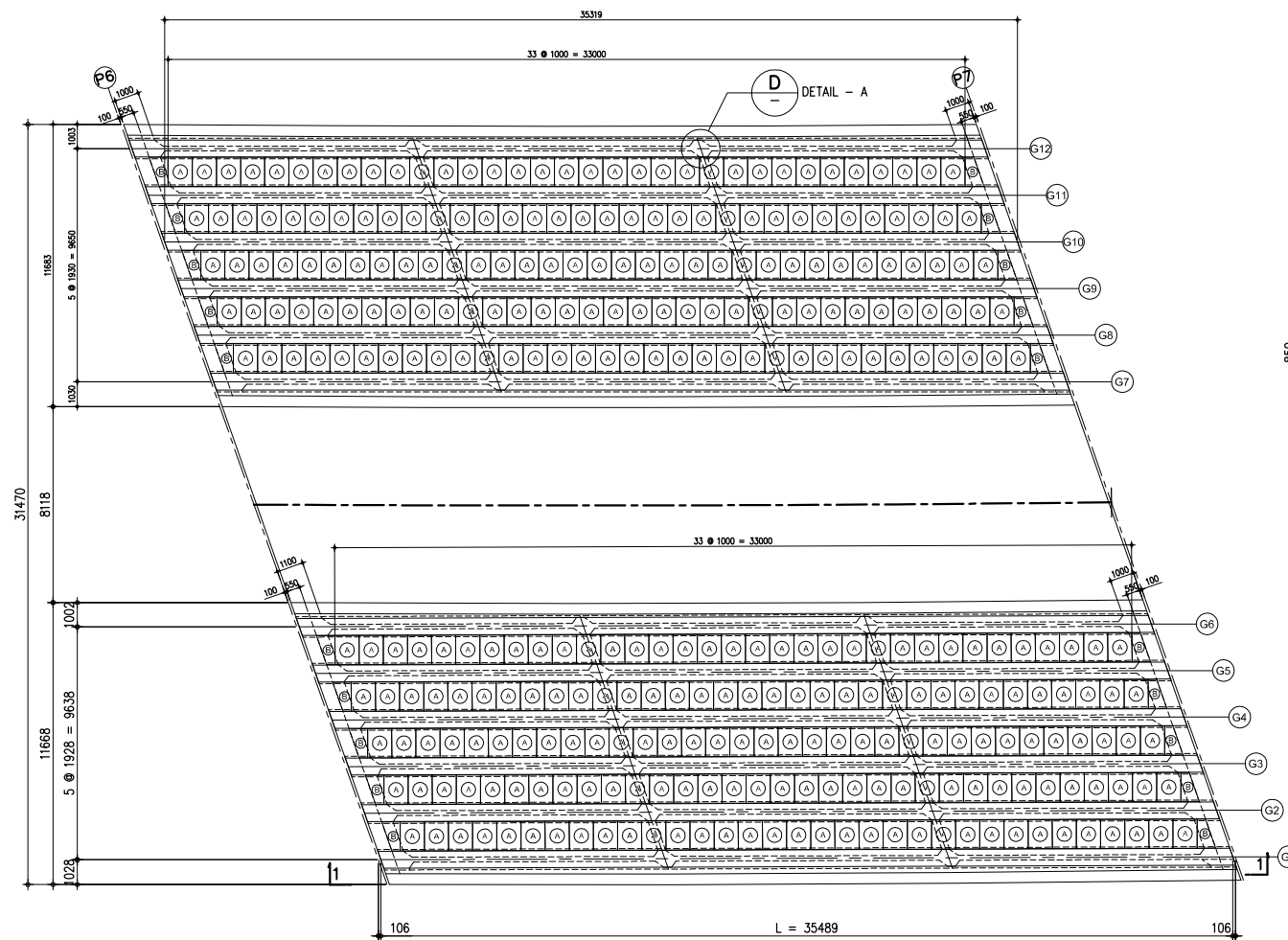
**E** BEARING DETAILS  
SCALE 1:20



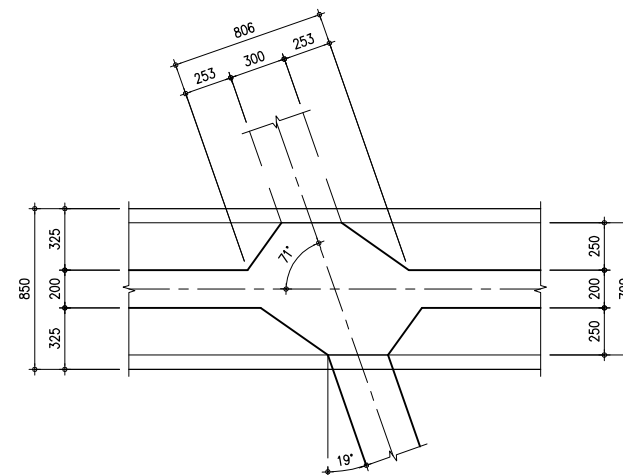
**B** SIDE VIEW  
SCALE 1:100



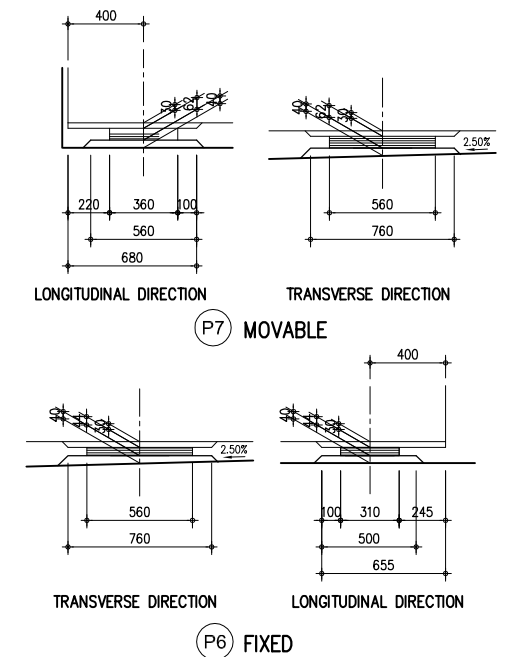
**C** GIRDER CROSS SECTION  
SCALE 1:20



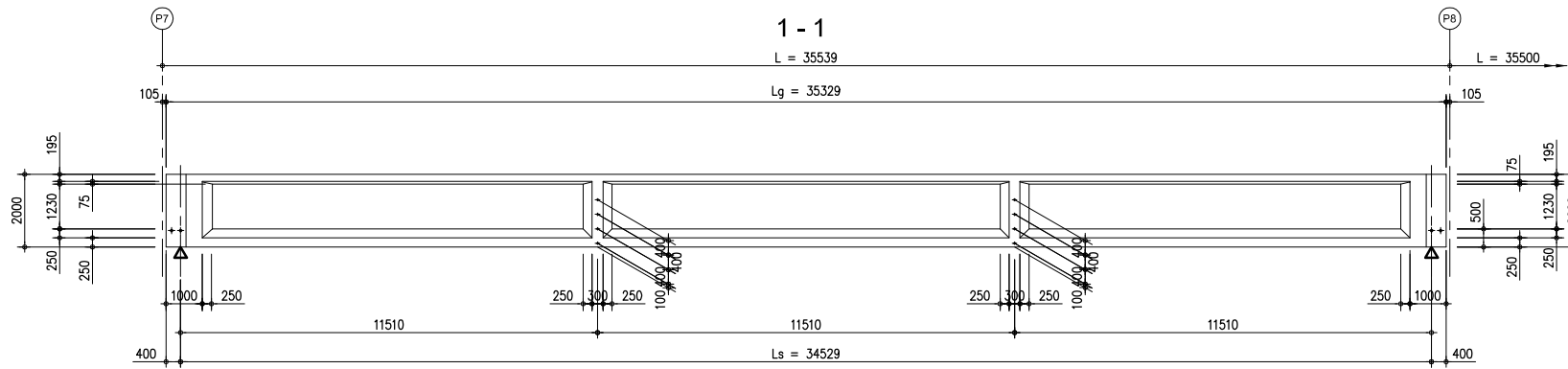
**A** PRECAST PLATES LAYOUT  
SCALE 1:150



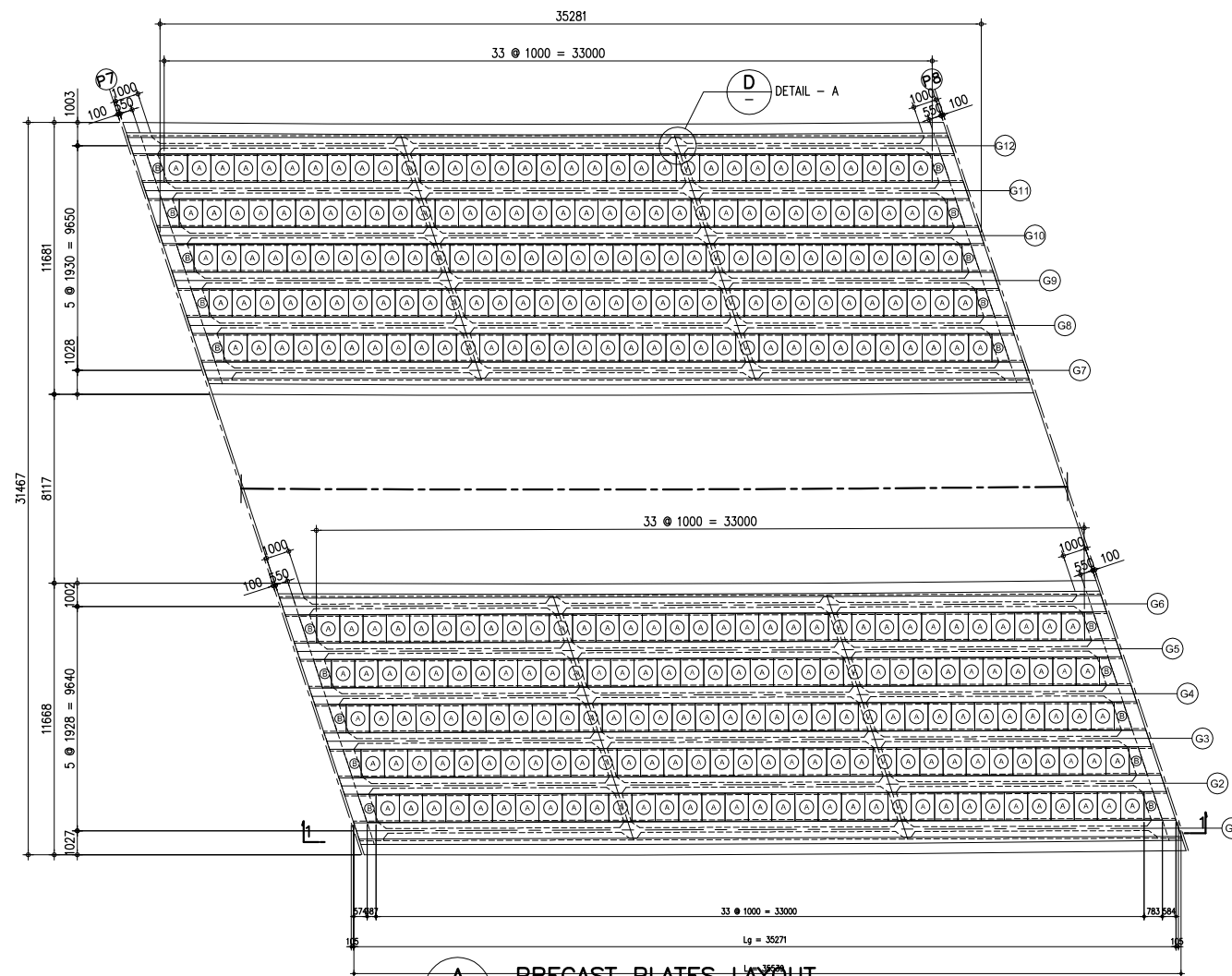
**D** DETAIL-A  
SCALE 1:20



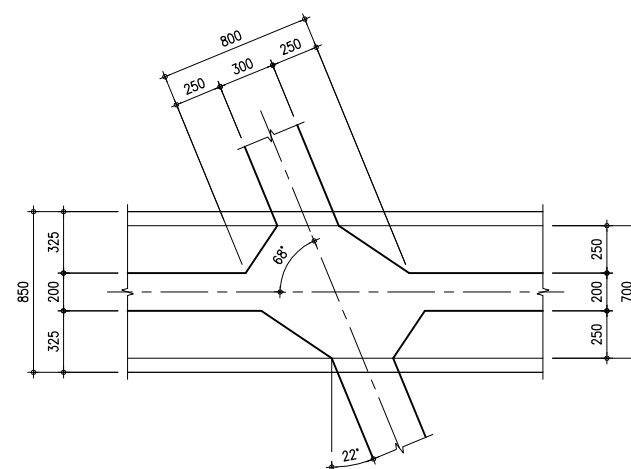
**E** BEARING DETAILS  
SCALE 1:20



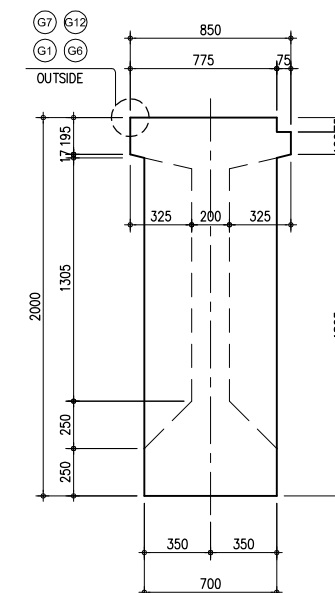
**B** SIDE VIEW  
SCALE 1:100



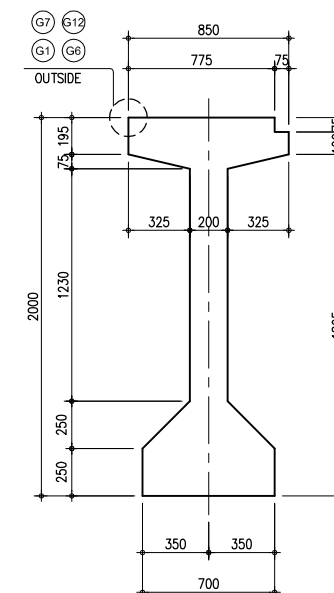
**A** PRECAST PLATES LAYOUT  
SCALE 1:150



**D** DETAIL-A  
SCALE 1:20

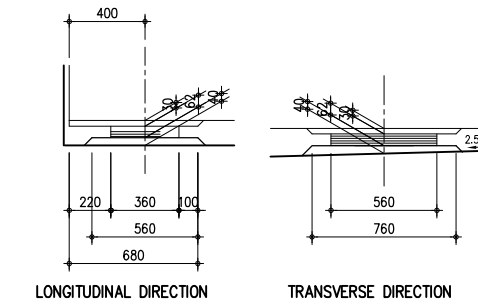


AT SUPPORT

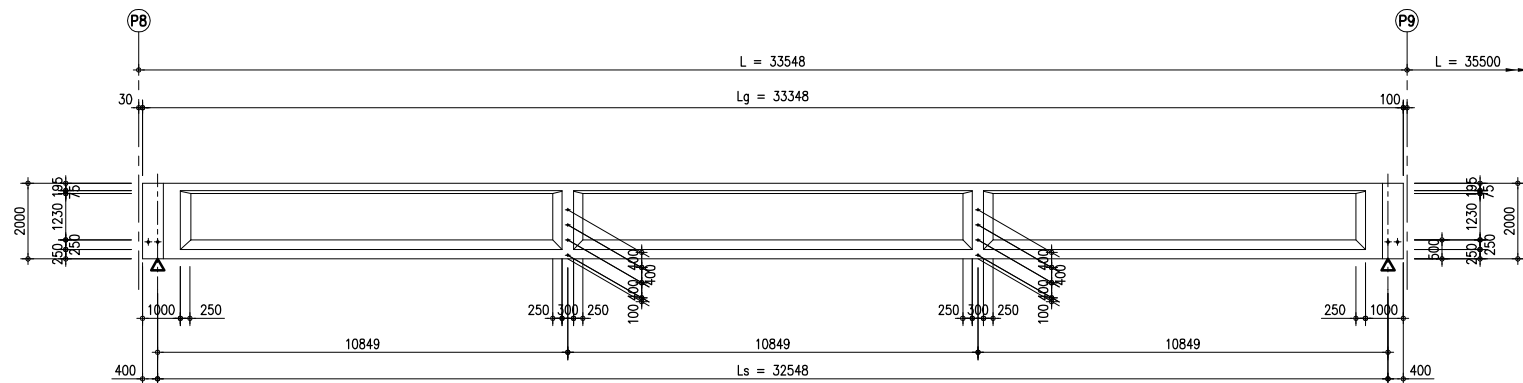


AT MID-SPAN

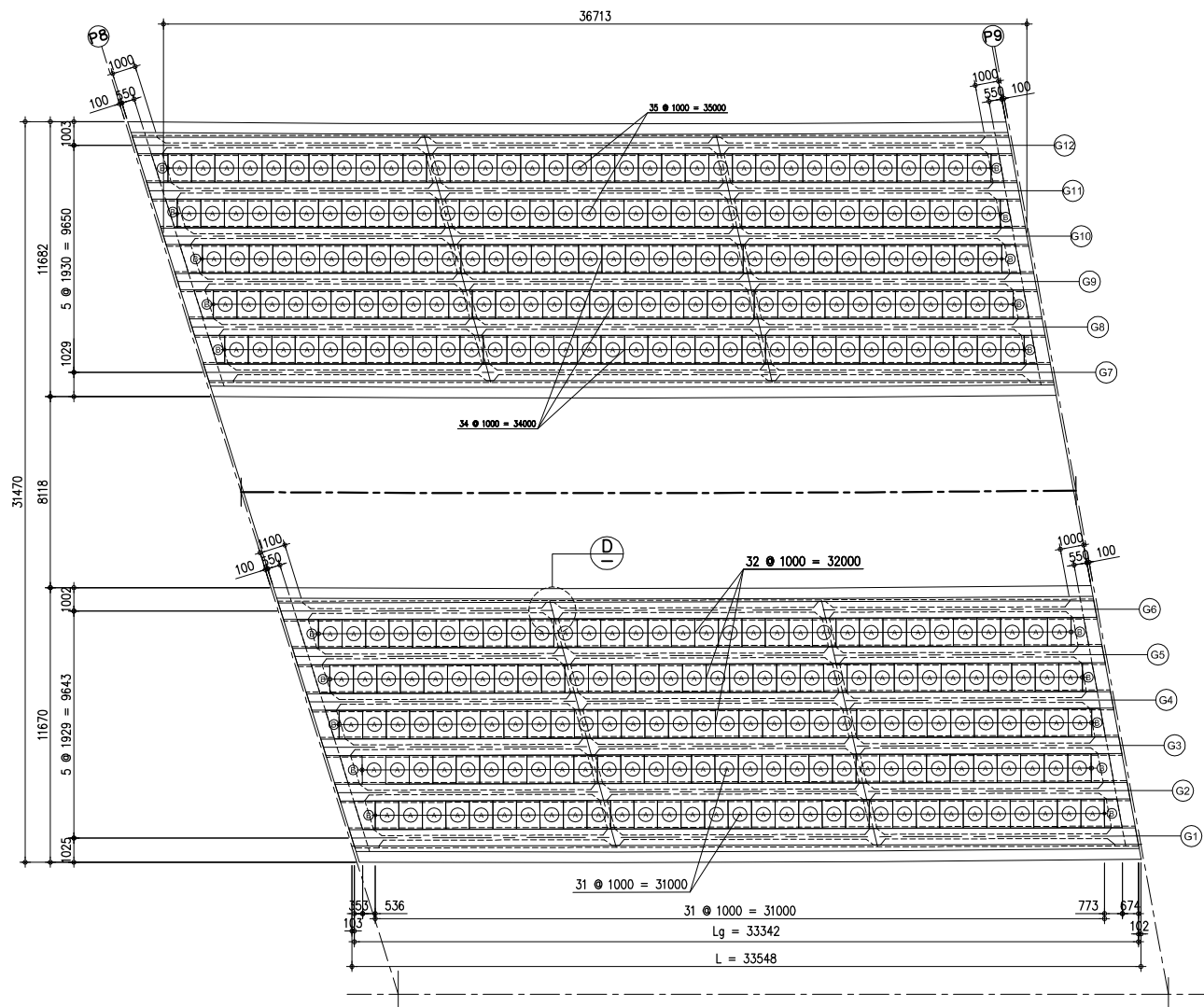
**C** GIRDER CROSS SECTION  
SCALE 1:20



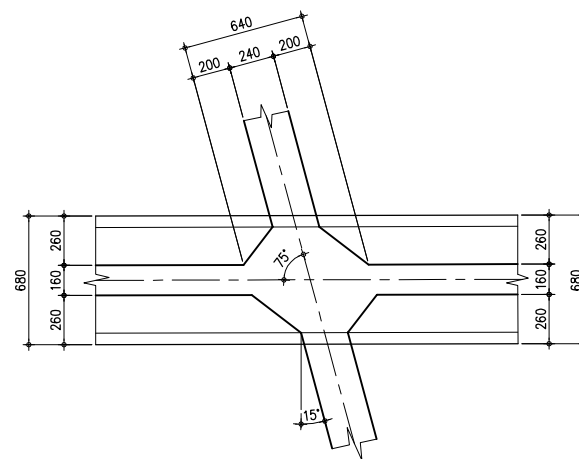
**E** BEARING DETAILS  
SCALE 1:20



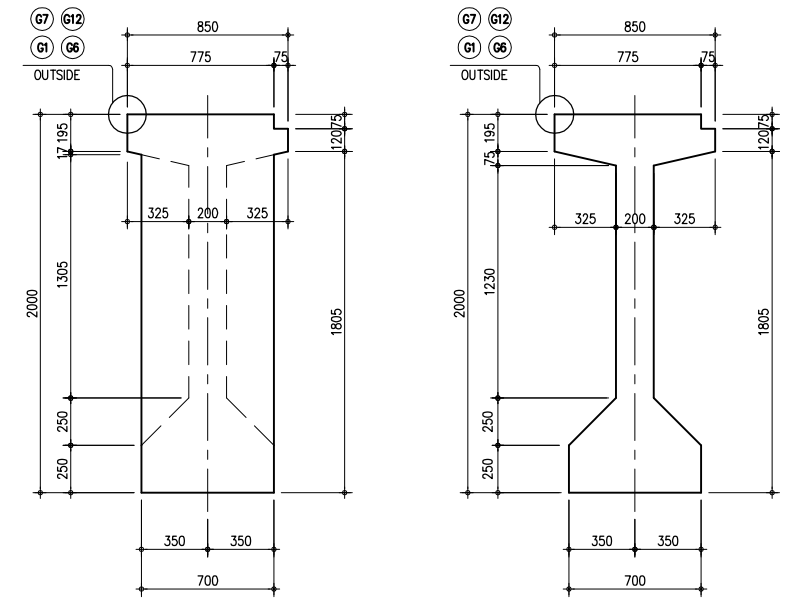
**B** SIDE VIEW  
SCALE 1:100



**A** PRECAST PLATES LAYOUT  
SCALE 1:150



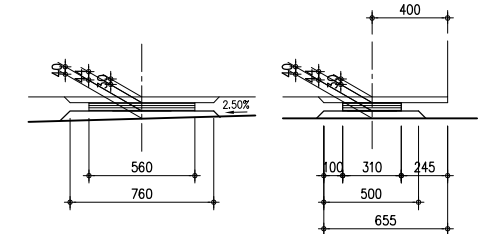
**D** DETAIL - A  
SCALE 1:20



AT SUPPORT

AT MID-SPAN

**E** GIRDER CROSS SECTION  
SCALE 1:20

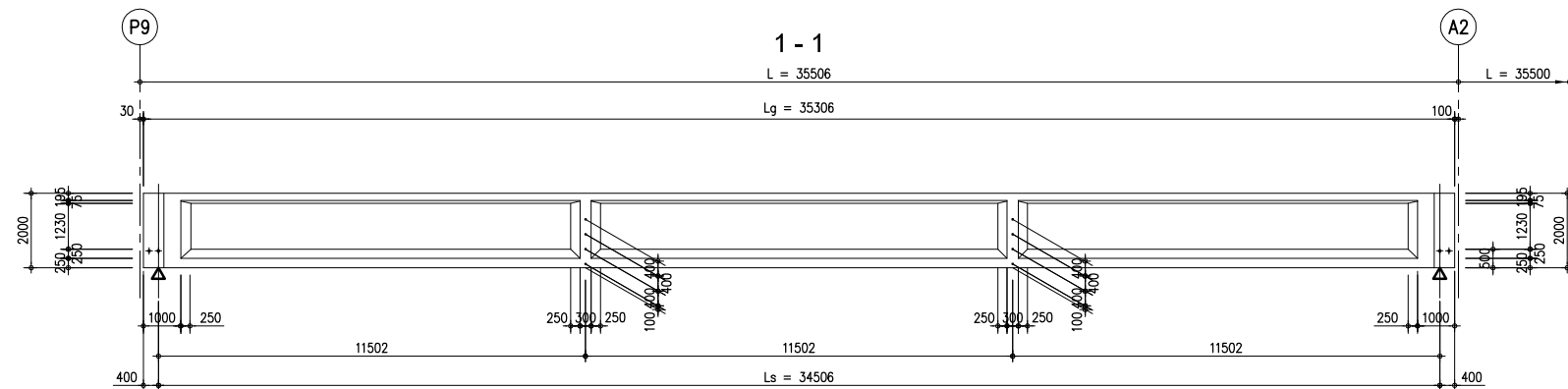


TRANSVERSE DIRECTION

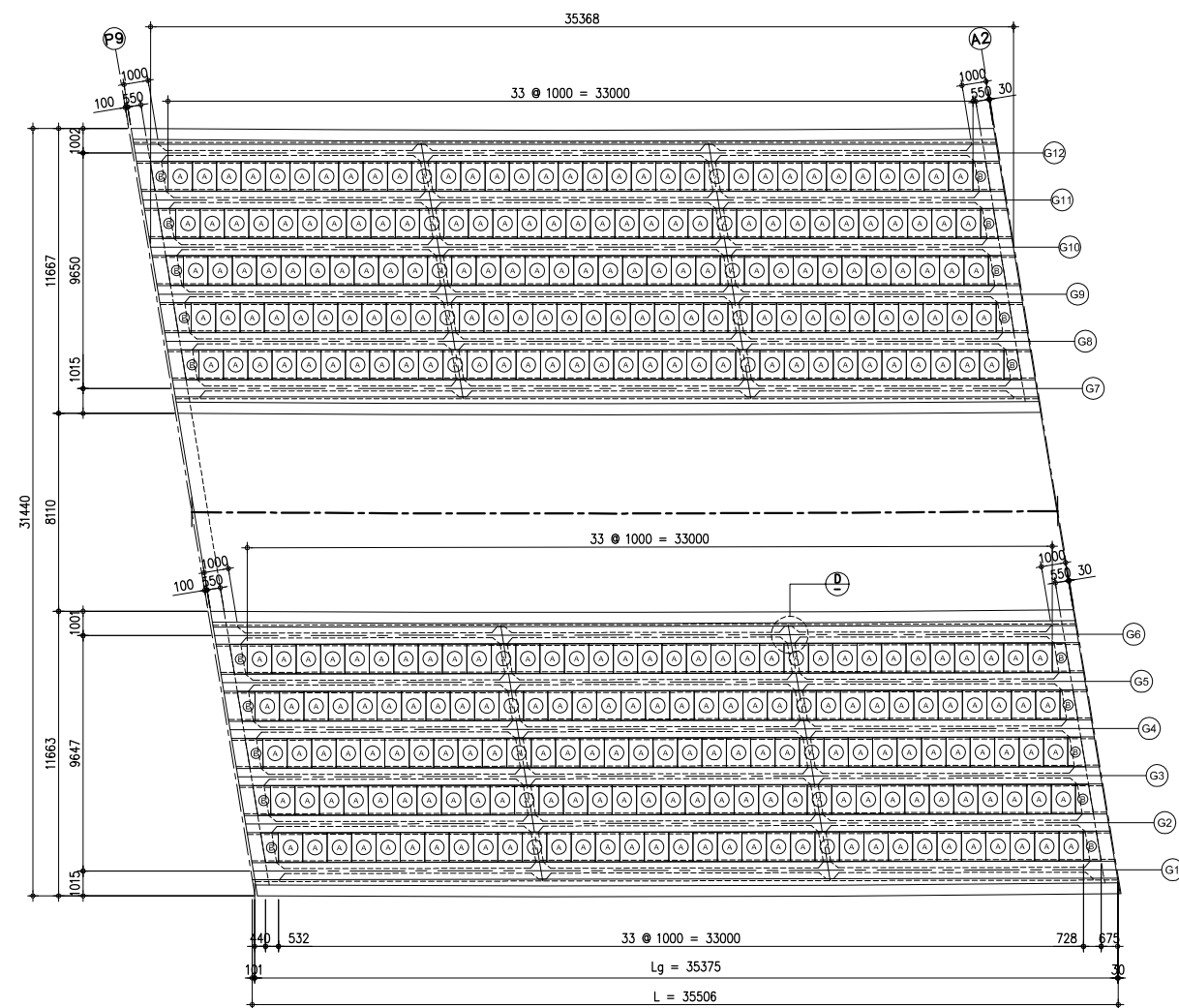
LONGITUDINAL DIRECTION

P8/P9 FIXED

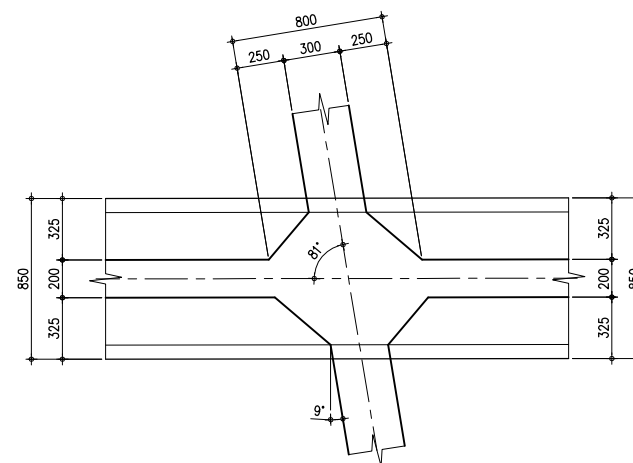
**F** BEARING DETAILS  
SCALE 1:20



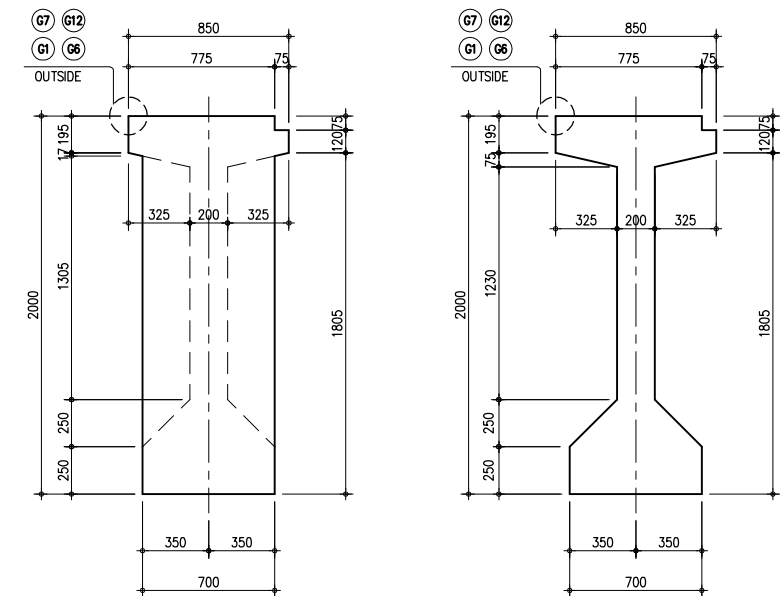
**B** SIDE VIEW  
SCALE 1:100



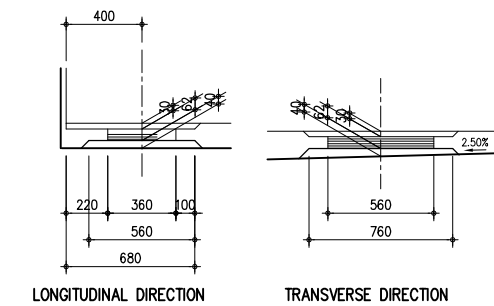
**A** PRECAST PLATES LAYOUT  
SCALE 1:150



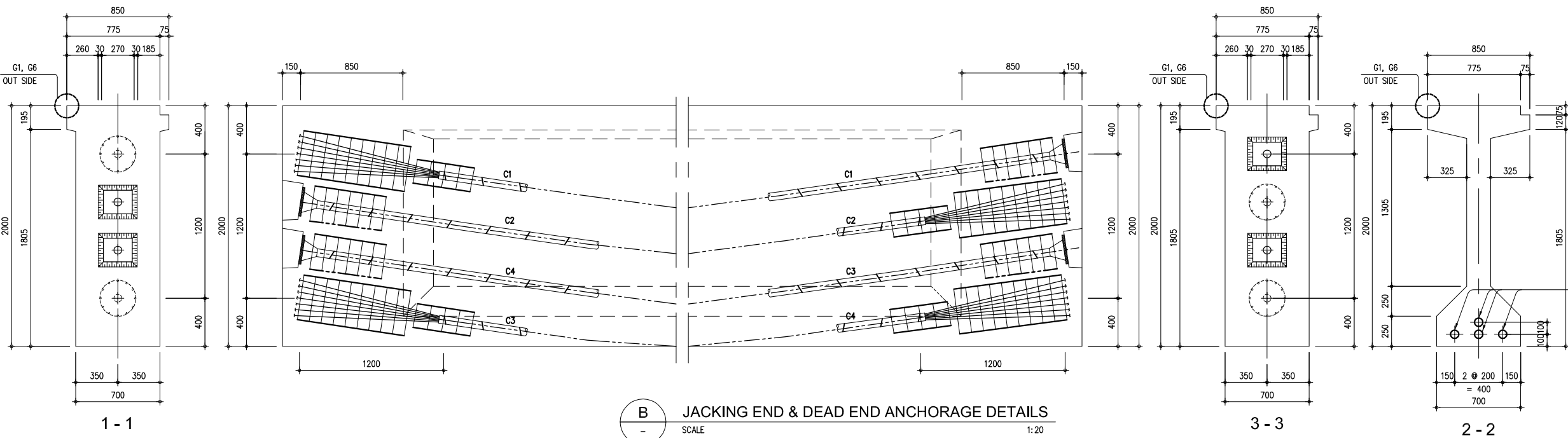
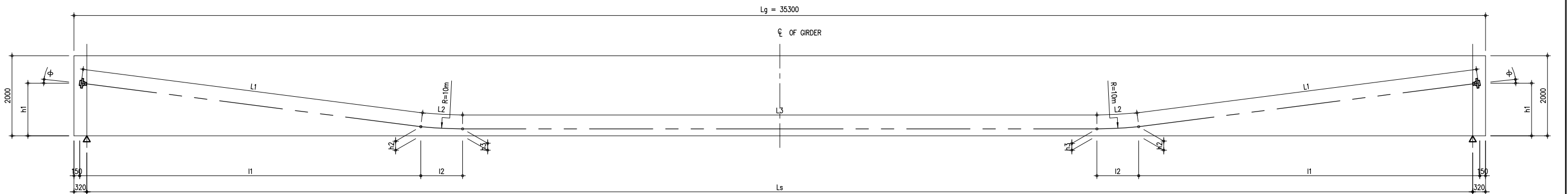
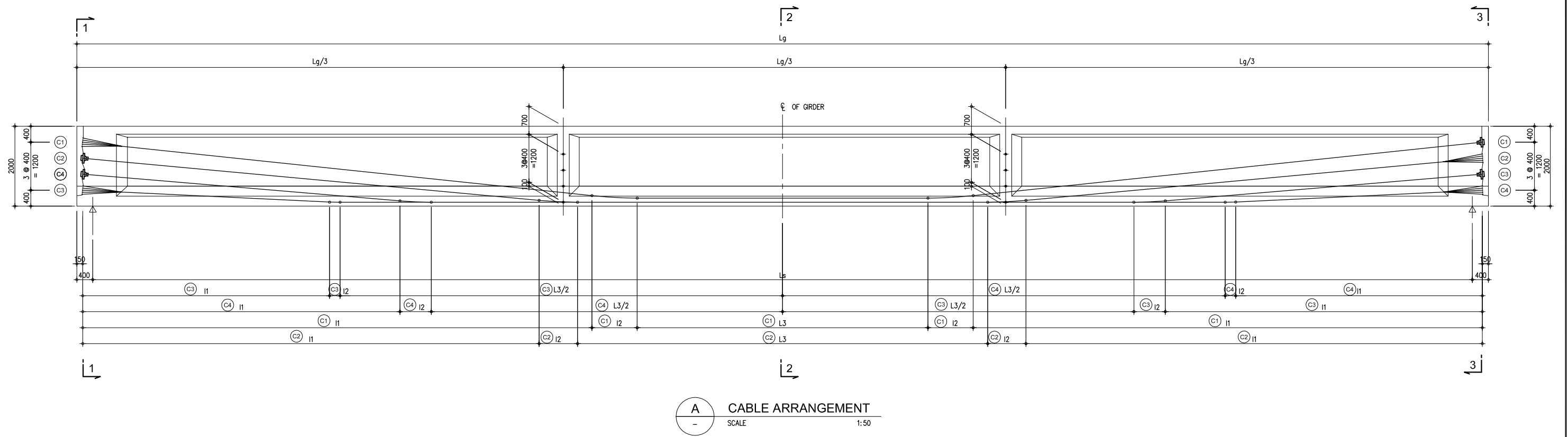
**D** DETAIL - A  
SCALE 1:20



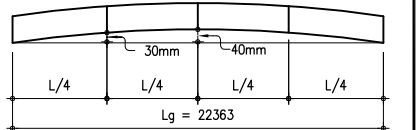
**E** GIRDER CROSS SECTION  
SCALE 1:20

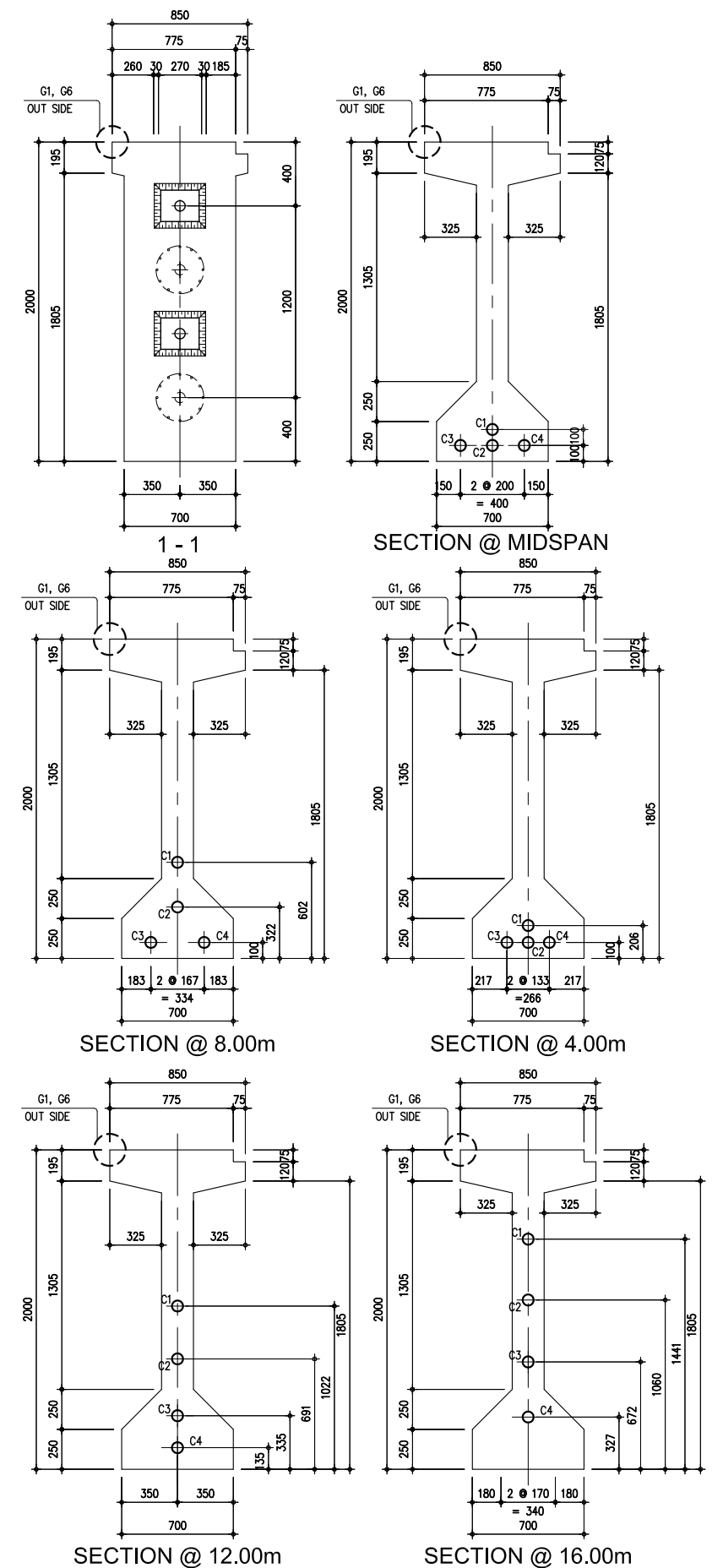


**F** BEARING DETAILS  
SCALE 1:20



- NOTES:**
- PRESTRESSED CONCRETE:**
    - \* CONCRETE CUBE STRENGTH,  $f_{cu} = 50 \text{ MPa}$
    - \* AT TRANSFER OF PRESTRESS,  $f_{ci} = 36 \text{ MPa}$
  - PRESTRESSING STRANDS:**
    - \* 1 -  $\phi 12.7$  IS A 7 - WIRE STRAND WITH NOMINAL TENSILE STRENGTH,  $f_{pu} = 1860 \text{ MPa}$
    - \* TOTAL NO. OF STRANDS =  $4 \times 12012.7 = 48012.7$
    - \* JACKING STRESS,  $f_{po} = 1302 \text{ MPa}$
    - \* TOTAL JACKING FORCE,  $P_o = 4536 \text{ KN}$
    - \* CABLE DUCT INSIDE DIAMETER ( $12 - \phi 12.7$ ) = 65mm.
  - REINFORCING STEEL :**
    - \* ALL REINFORCING STEEL SHALL BE GRADE 460 WITH MINIMUM CHARACTERISTIC STRENGTH,  $f_y = 460 \text{ MPa}$
  - GIRDER PRE-CAMBER PRIOR TO STRESSING SHALL BE AS SHOWN BELOW

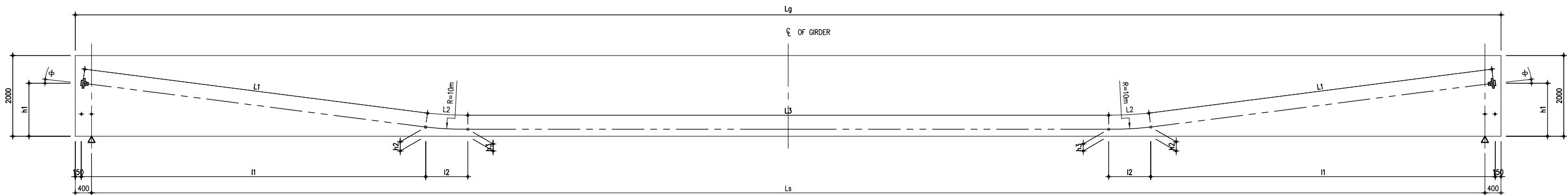




**B** GIRDER CROSS SECTION  
- SCALE 1:20

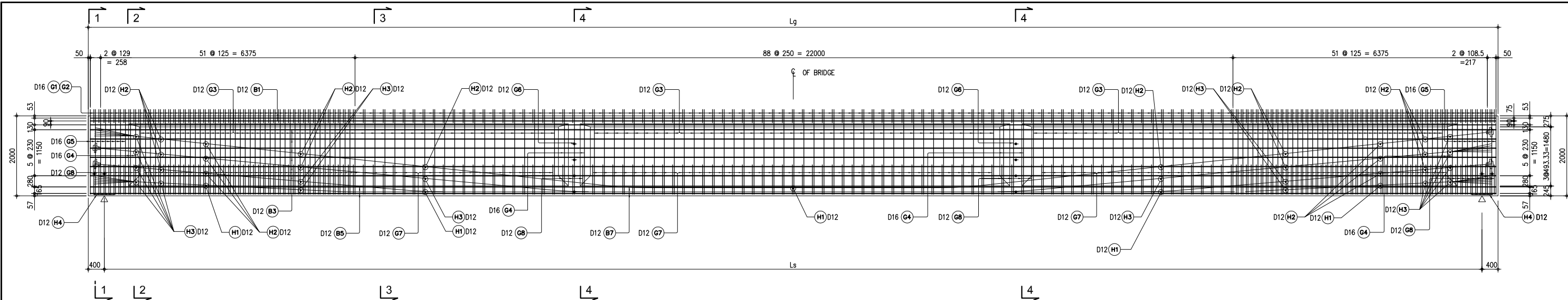


SPAN	CABLE NO.	I1	I2	L1	L2	L3	$2\frac{L^2}{L+3}$	h1	h2	h3	$\phi = (\text{degree})$	TOTAL LENGTH	TOTAL WEIGHT
P0 - P1	C1	12733	1132	12803	1135	7295	35171	1600	244	180	6°5'	140.42 m	1,304.22 kg./1girder
	C2	11413	958	11462	960	10281	35125	1200	136	90	5°20'		
	C3	7931	785	7959	786	17594	35084	800	121	90	4°54'		
	C4	6170	262	6177	262	22161	35039	400	93	90	2°51'		
P1 - P2	C1	12733	1132	12803	1134	6047	33921	1600	244	180	6°5'	135.42 m	1,257.78 kg./1girder
	C2	11413	958	11462	960	9034	33878	1200	136	90	5°20'		
	C3	7931	785	7959	785	16346	33834	800	121	90	4°54'		
	C4	6170	262	6177	262	20913	33791	400	93	90	2°51'		
P2 - P3	C1	12733	1132	12803	1134	7300	35174	1600	244	180	6°5'	140.44 m	1,304.41 kg./1girder
	C2	11413	958	11462	960	10286	35130	1200	136	90	5°20'		
	C3	7931	785	7959	785	17599	35087	800	121	90	4°54'		
	C4	6170	262	6177	262	22166	35044	400	93	90	2°51'		
P3 - P4	C1	12733	1132	12803	1134	7297	36171	1600	244	180	6°5'	141.42 m	1,313.51 kg./1girder
	C2	11413	958	11462	960	10284	35128	1200	136	90	5°20'		
	C3	7931	785	7959	785	17596	35084	800	121	90	4°54'		
	C4	6170	262	6177	262	22163	35041	400	93	90	2°51'		
P4 - P5	C1	12733	1132	12803	1134	7294	35168	1600	244	180	6°5'	140.41 m	1,304.13 kg./1girder
	C2	11413	958	11462	960	10281	35125	1200	136	90	5°20'		
	C3	7931	785	7959	785	17593	35081	800	121	90	4°54'		
	C4	6170	262	6177	262	22160	35038	400	93	90	2°51'		
P5 - P6	C1	12733	1132	12803	1134	7294	35168	1600	244	180	6°5'	140.41 m	1,304.13 kg./1girder
	C2	11413	958	11462	960	10279	35123	1200	136	90	5°20'		
	C3	7931	785	7959	785	17591	35079	800	121	90	4°54'		
	C4	6170	262	6177	262	22158	35036	400	93	90	2°51'		
P6 - P7	C1	12733	1134	12803	1134	7289	35163	1600	244	180	6°5'	140.39 m	1,303.94 kg./1girder
	C2	11413	958	11462	960	10276	35120	1200	136	90	5°20'		
	C3	7931	785	7959	785	17588	35076	800	121	90	4°54'		
	C4	6170	262	6177	262	22155	35033	400	93	90	2°51'		
P7 - P8	C1	12733	1132	12802	1134	7254	35126	1600	244	180	6°5'	140.25 m	1,302.64 kg./1girder
	C2	11413	958	11462	960	10241	35085	1200	136	90	5°20'		
	C3	7931	785	7959	785	17553	35041	800	121	90	4°54'		
	C4	6170	262	6177	262	22120	34998	400	93	90	2°51'		
P8 - P9	C1	12733	1132	12803	1134	8683	36557	1600	244	180	6°5'	145.97 m	1,355.77 kg./1girder
	C2	11413	958	11462	960	11670	36514	1200	136	90	5°20'		
	C3	7931	785	7959	785	18982	36470	800	121	90	4°54'		
	C4	6170	262	6177	262	23549	36427	400	93	90	2°51'		
P9 - A2	C1	12733	1132	12803	1134	7338	35212	1600	244	180	6°5'	140.59 m	1,305.8 kg./1girder
	C2	11413	960	11462	960	10325	35169	1200	136	90	5°20'		
	C3	7931	785	7959	785	17637	35125	800	121	90	4°54'		
	C4	6170	262	6177	262	22204	35082	400	93	90	2°51'		

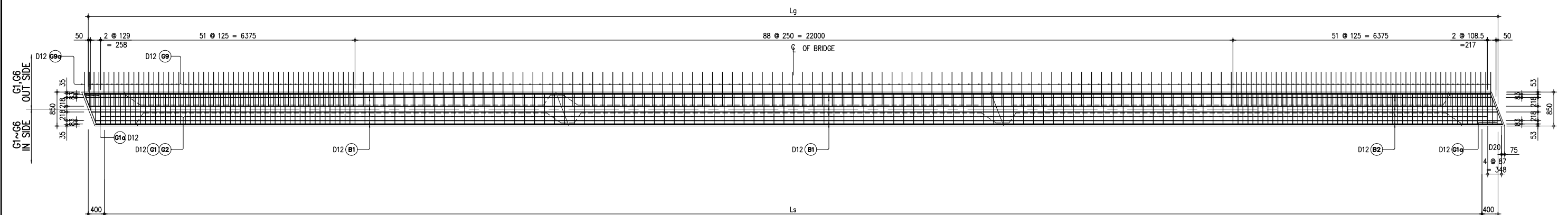


C CABLE LAYOUT  
SCALE 1:50

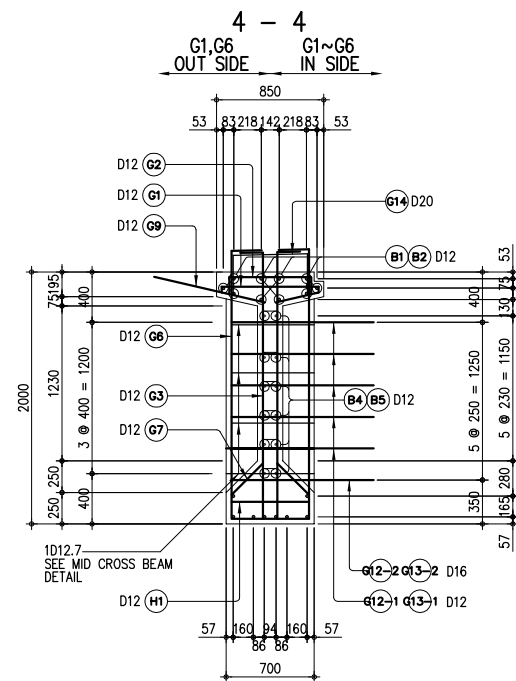
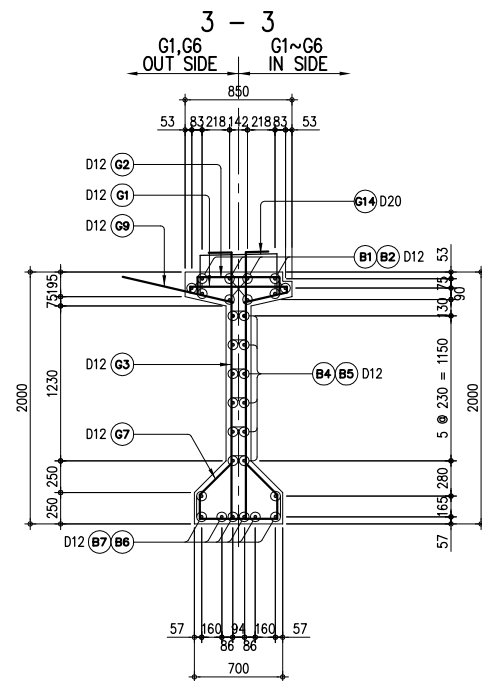
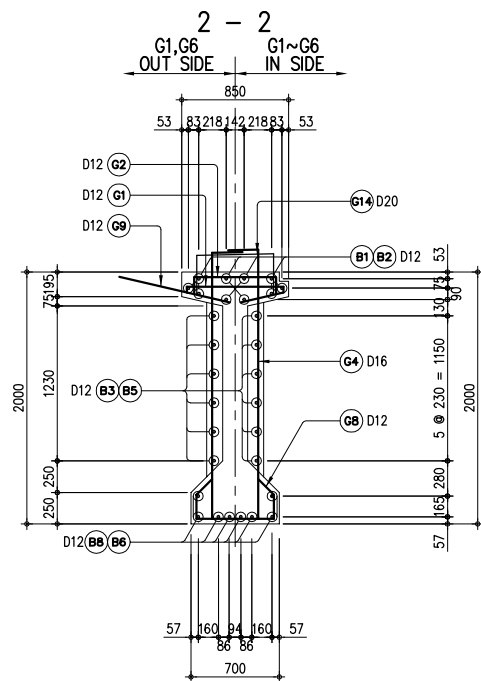
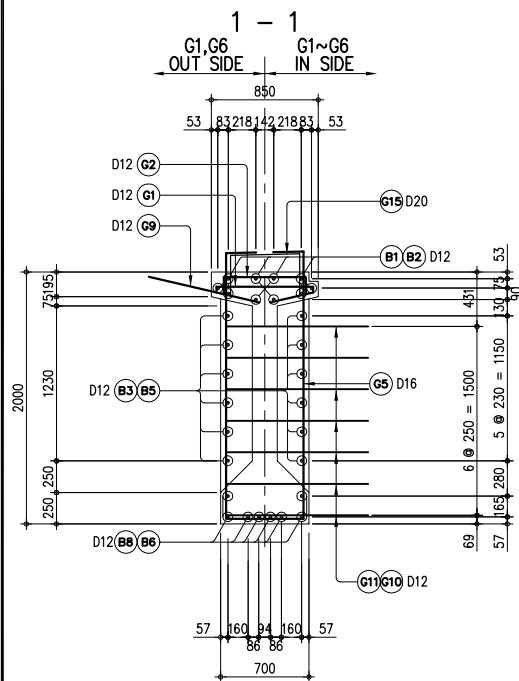




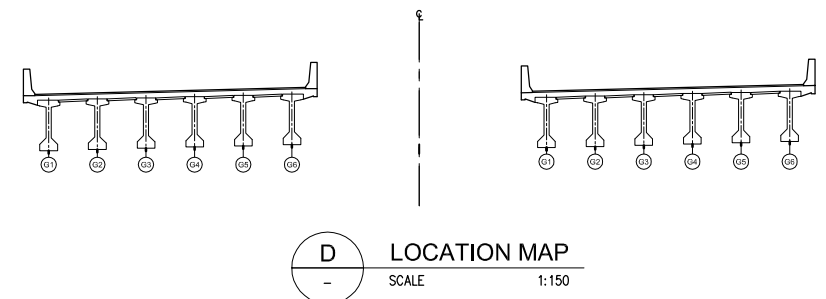
**B SIDE VIEW**  
SCALE 1:50



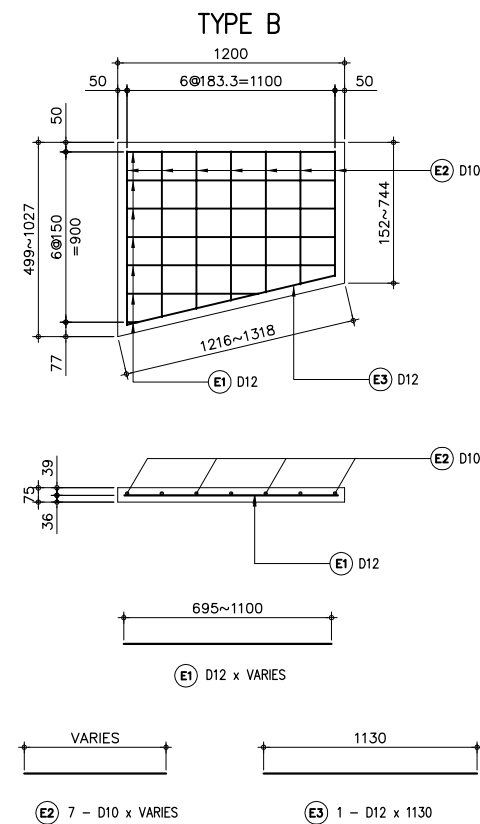
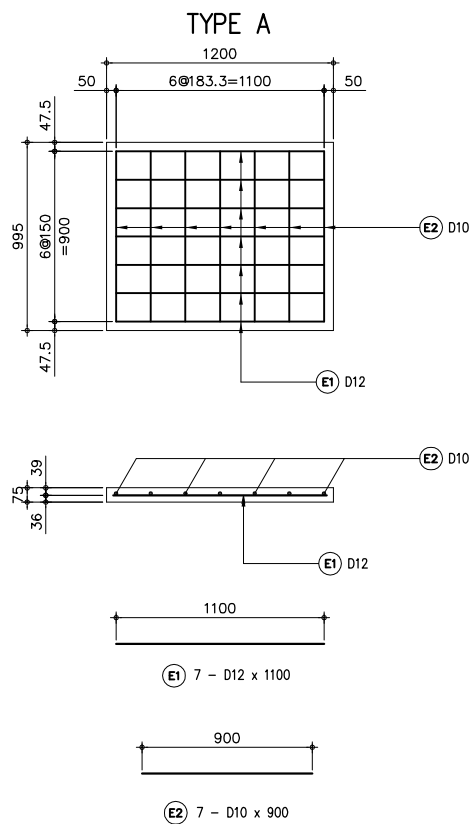
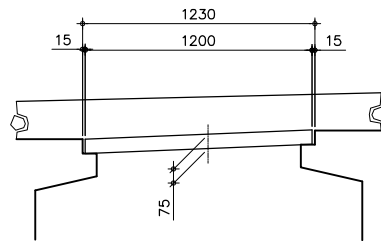
**A PLAN**  
SCALE 1:50



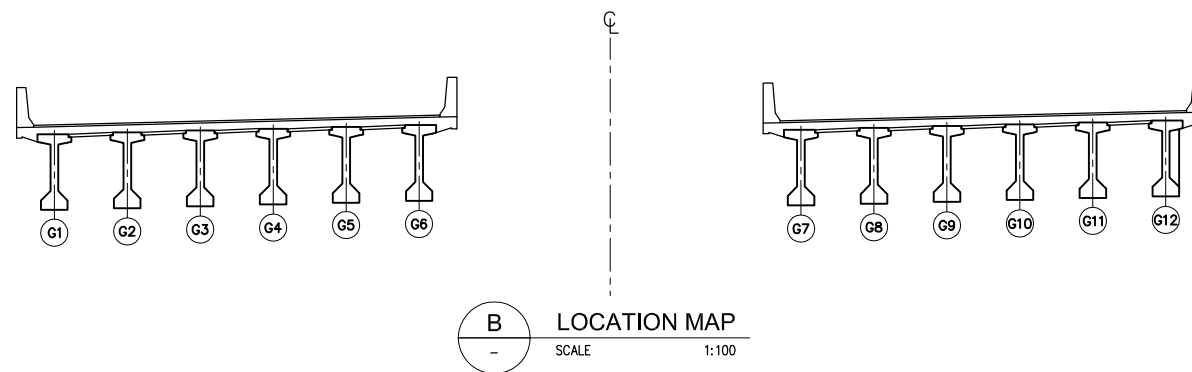
**C GIRDER CROSS SECTION**  
SCALE 1:30

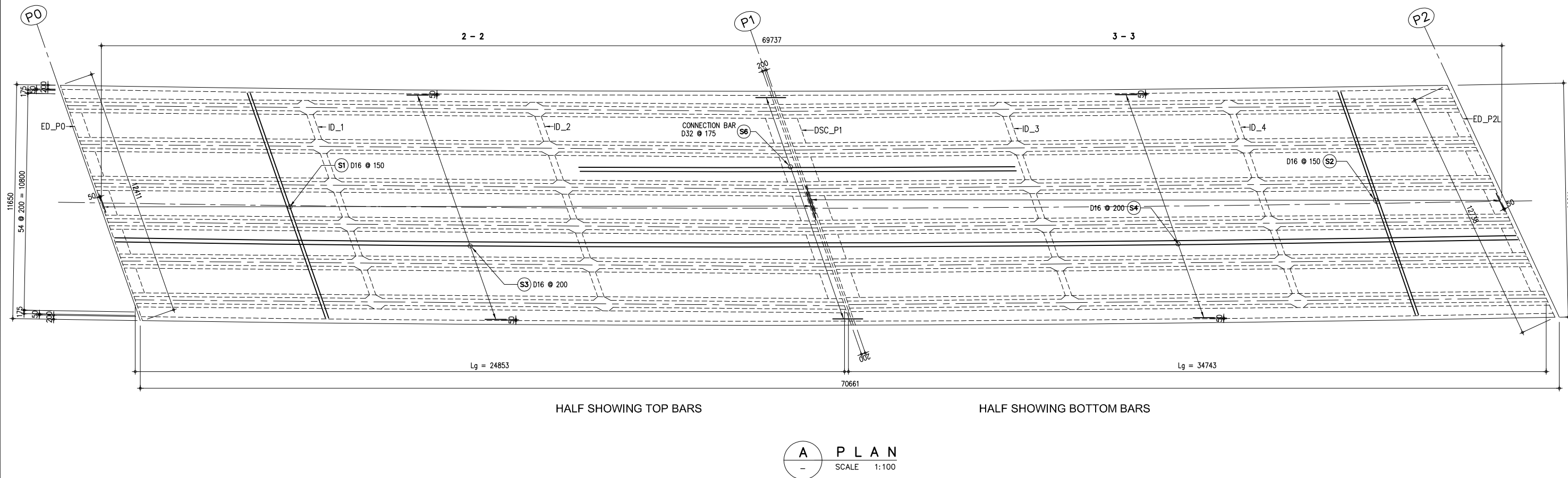
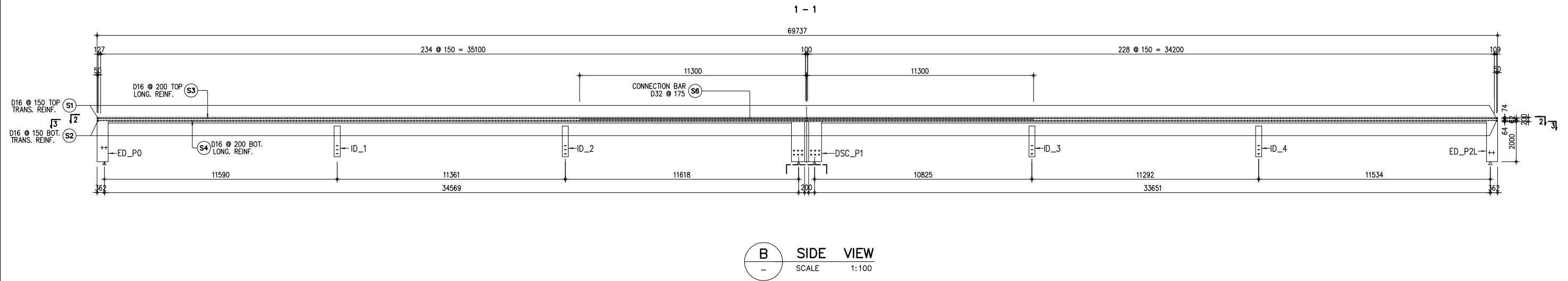


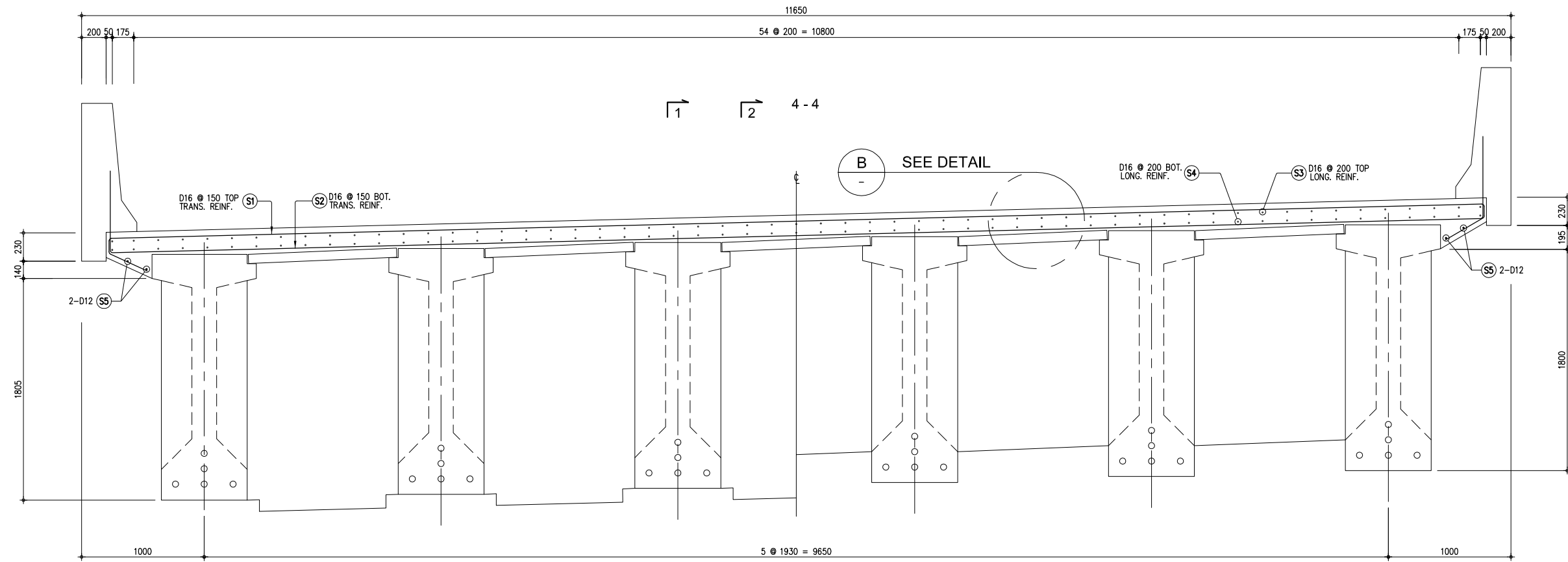
**D LOCATION MAP**  
SCALE 1:150



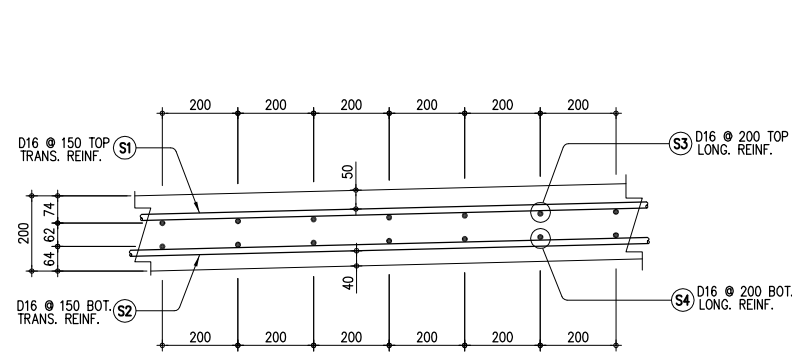
**A** DETAIL OF PRECAST PLATE  
SCALE 1:20



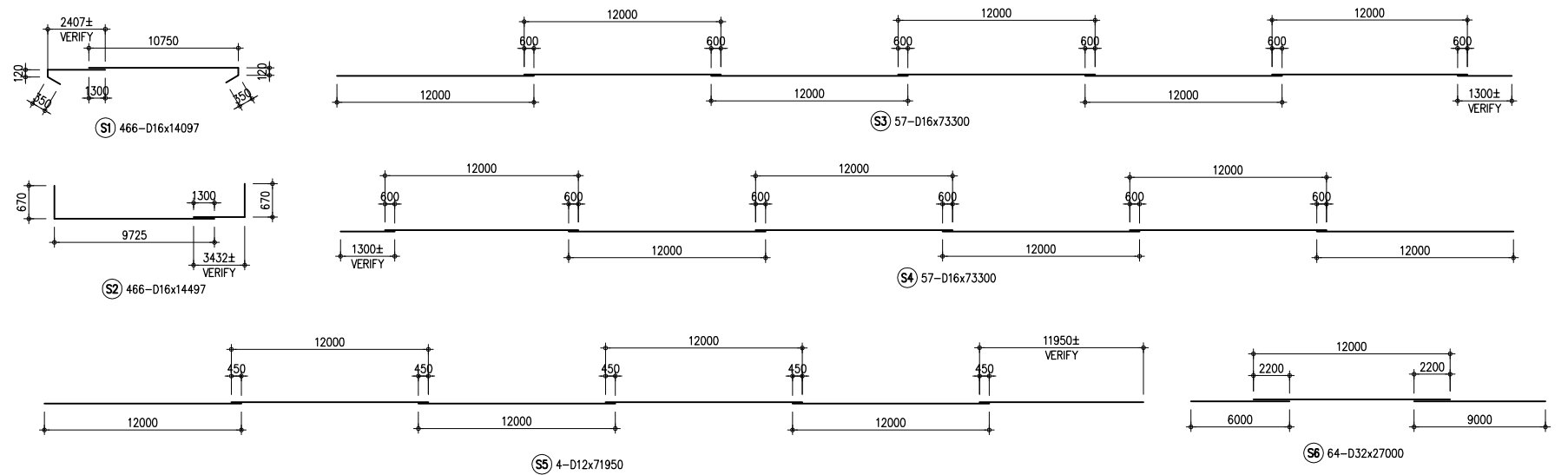




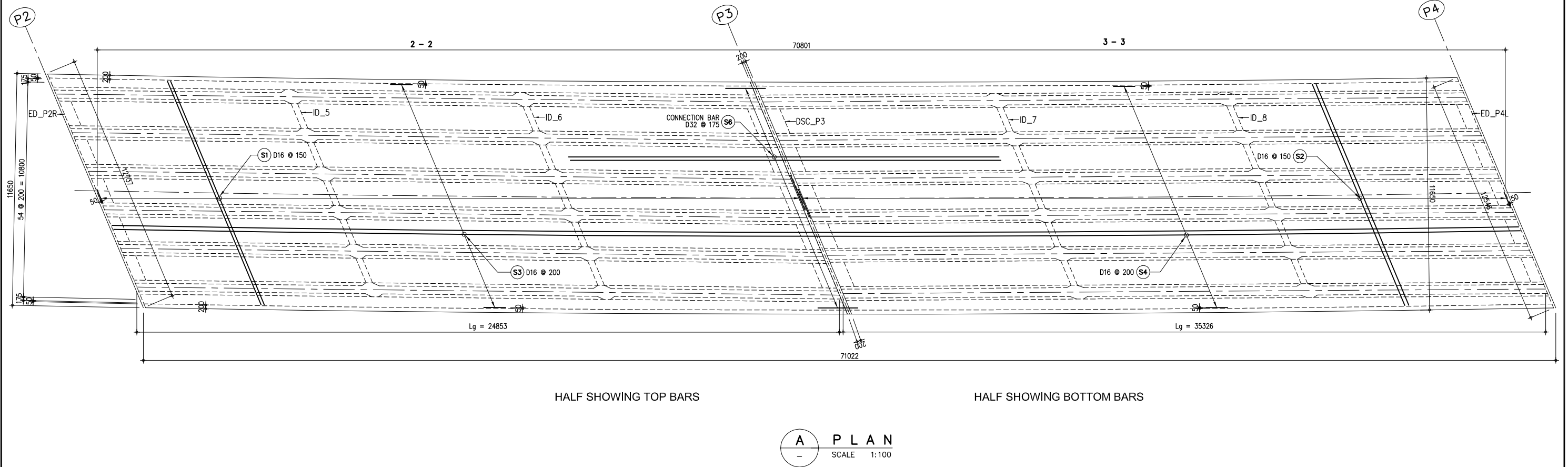
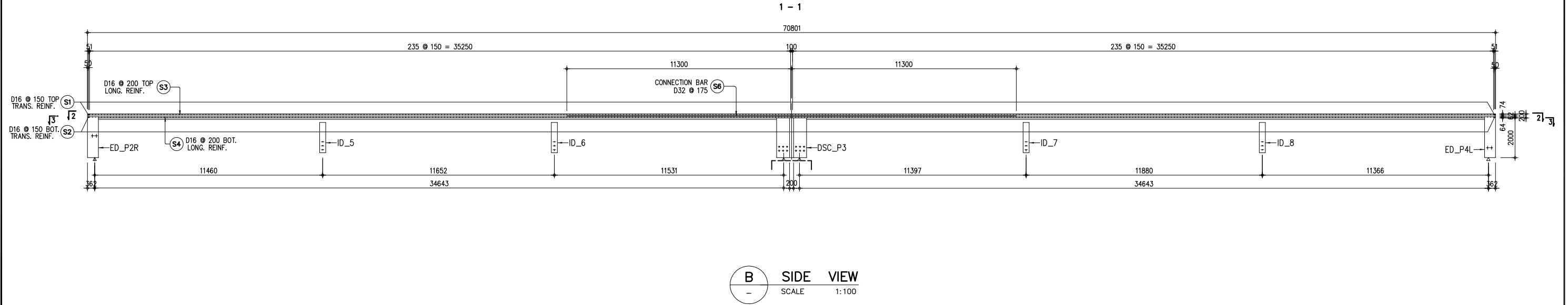
**A CROSS SECTION**  
SCALE 1:20

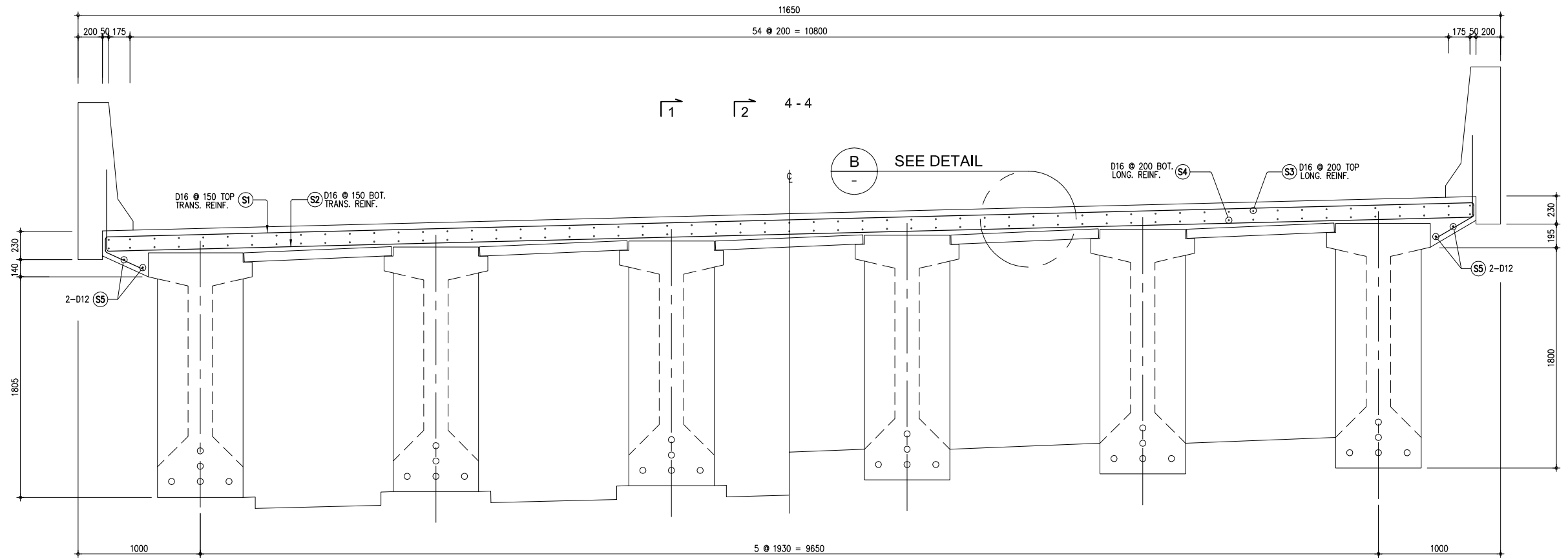


**B DETAIL**  
SCALE 1:10

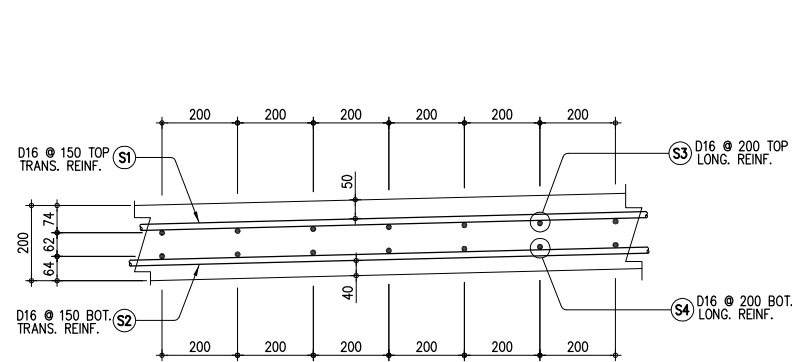


**C BAR BENDING LAYOUT**  
SCALE NTS

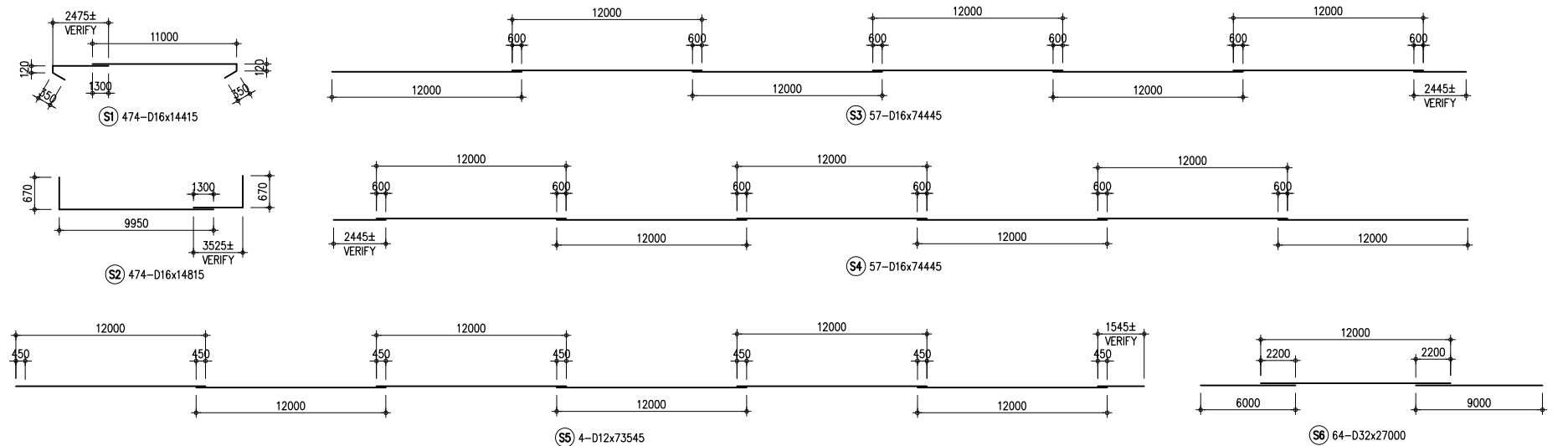




**A** CROSS SECTION  
SCALE 1:20

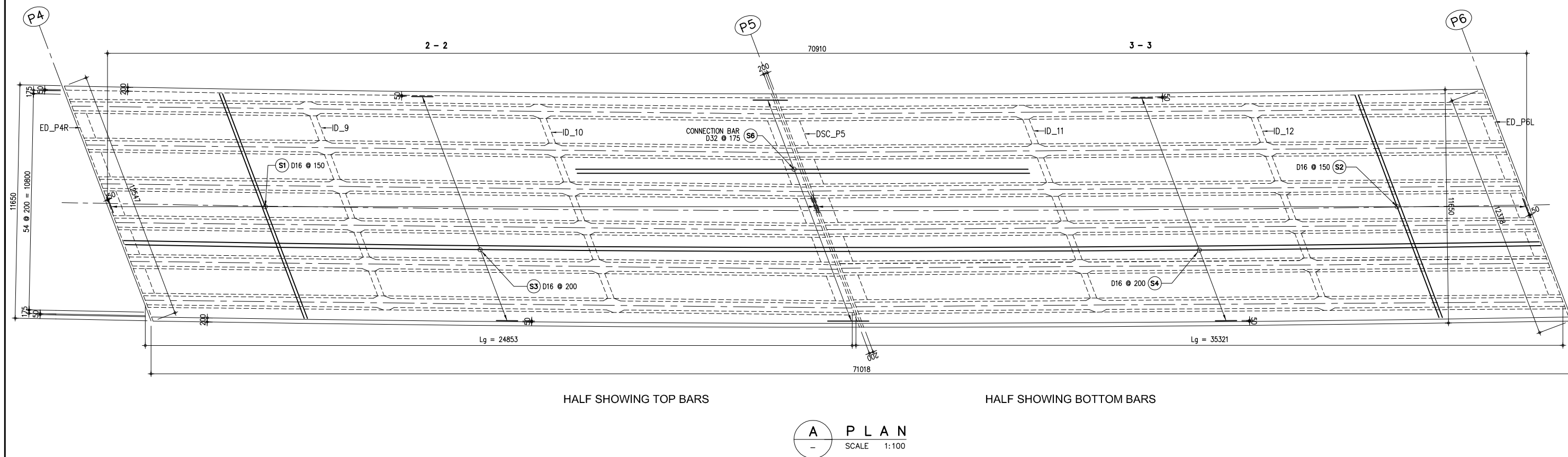
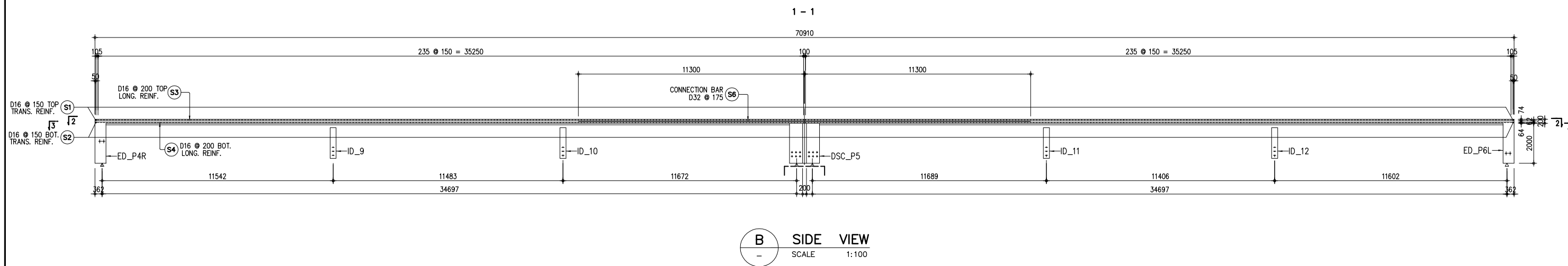


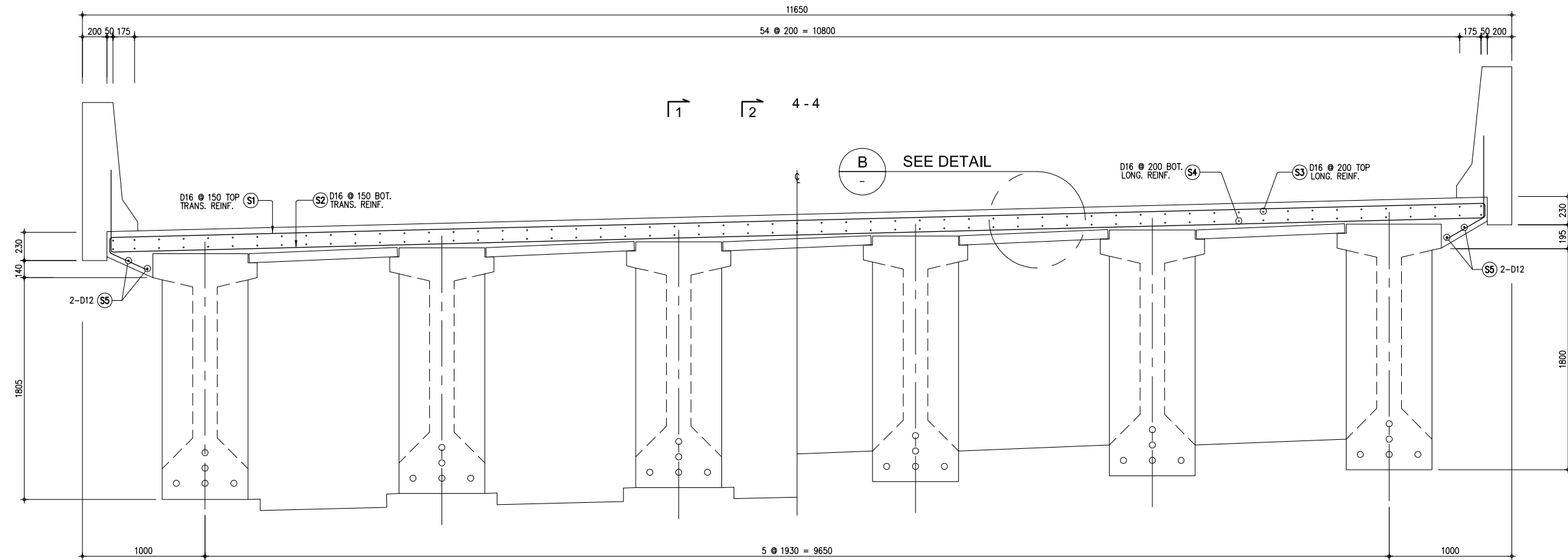
**B** DETAIL  
SCALE 1:10



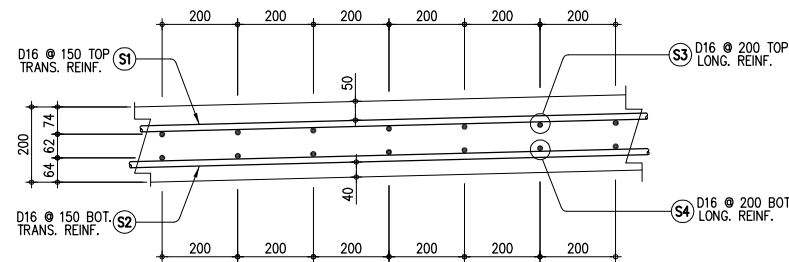
**C** BAR BENDING LAYOUT  
SCALE NTS



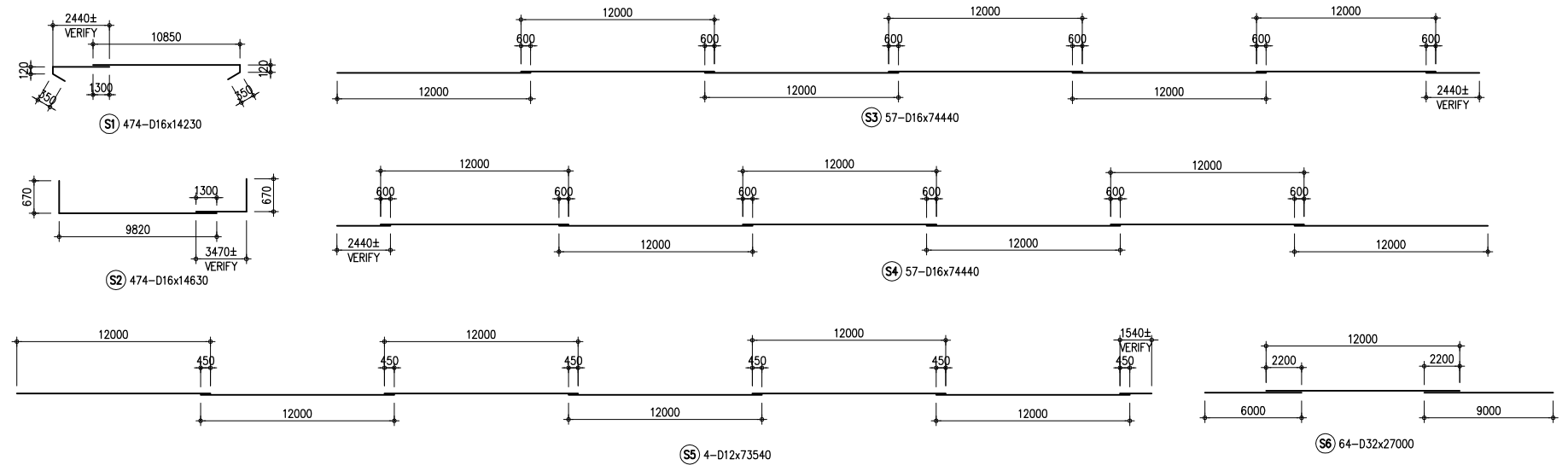




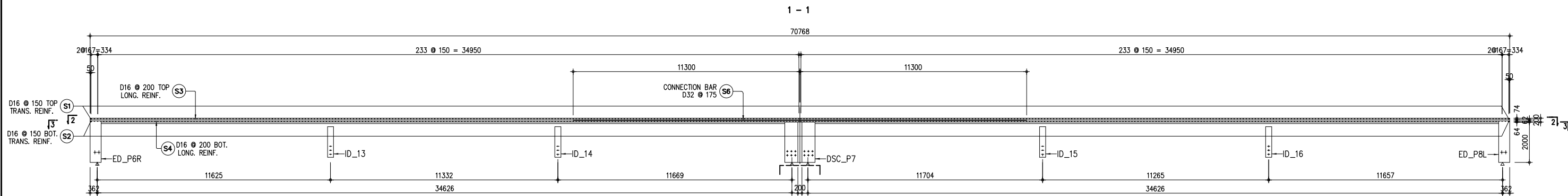
**A** CROSS SECTION  
SCALE 1:20



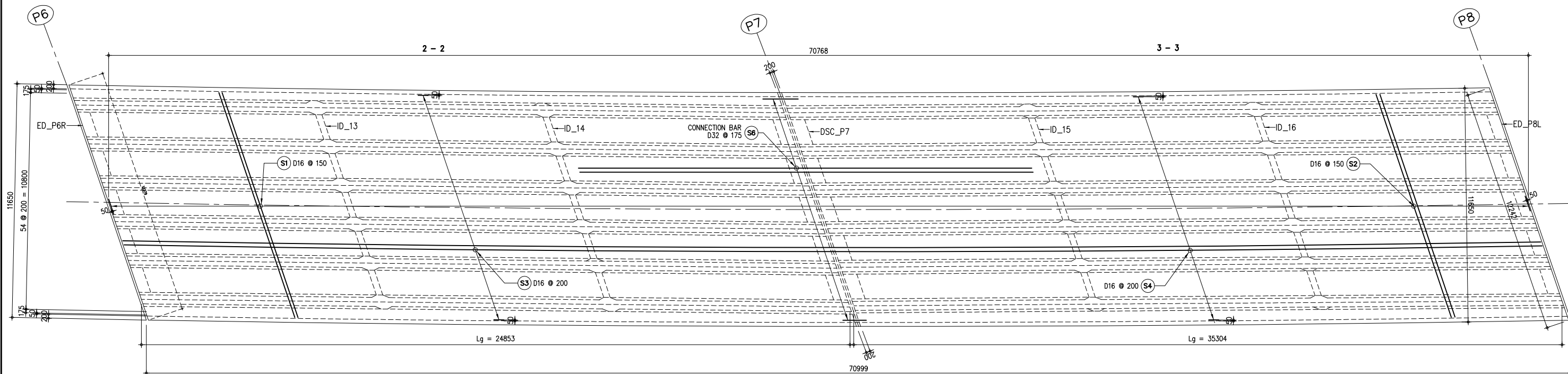
**B** DETAIL  
SCALE 1:10



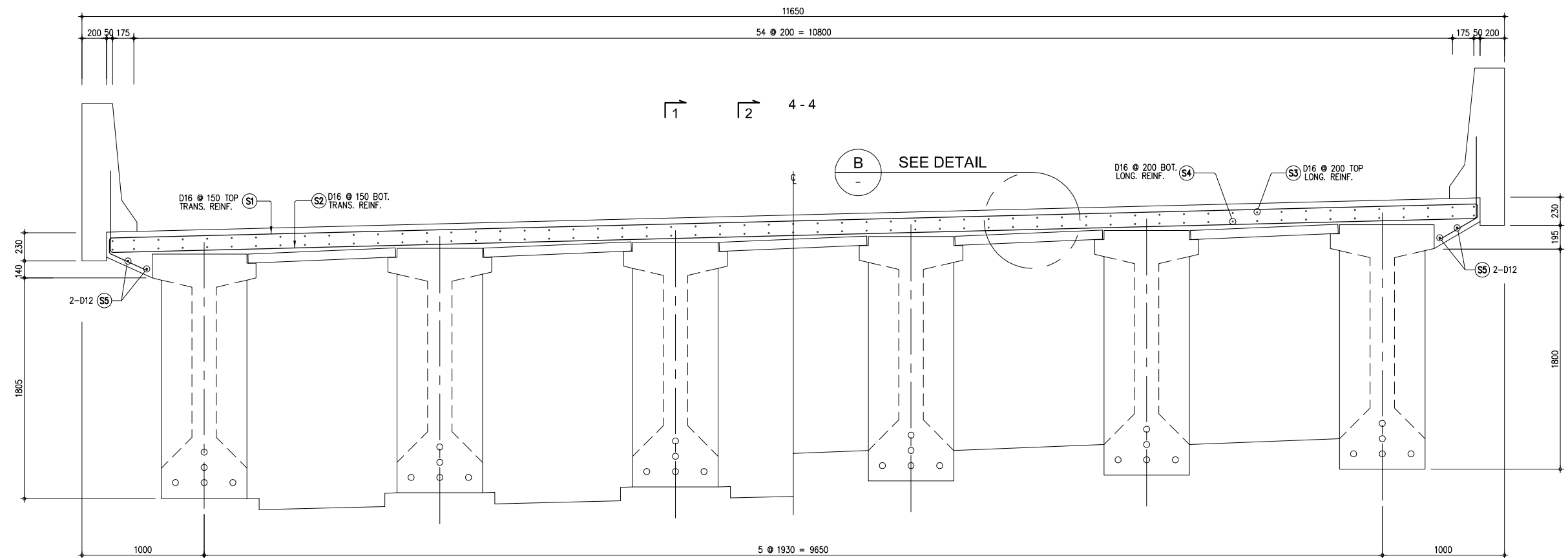
**C** BAR BENDING LAYOUT  
SCALE NTS



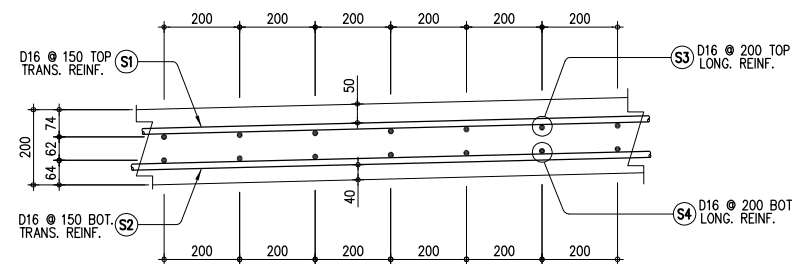
**B** SIDE VIEW  
SCALE 1:100



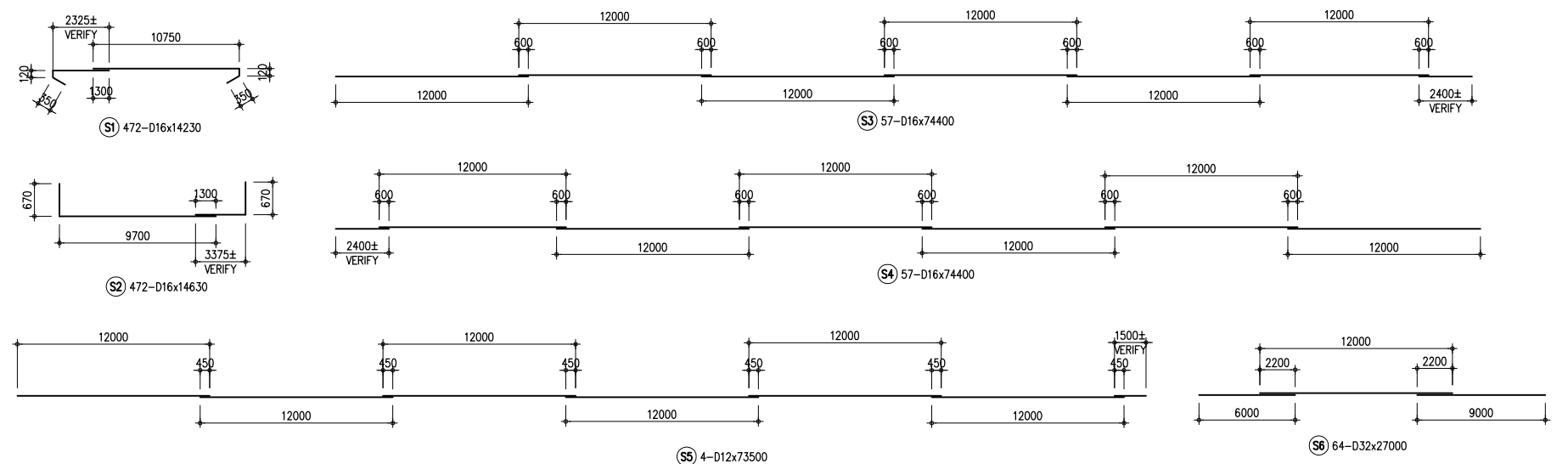
**A** PLAN  
SCALE 1:100



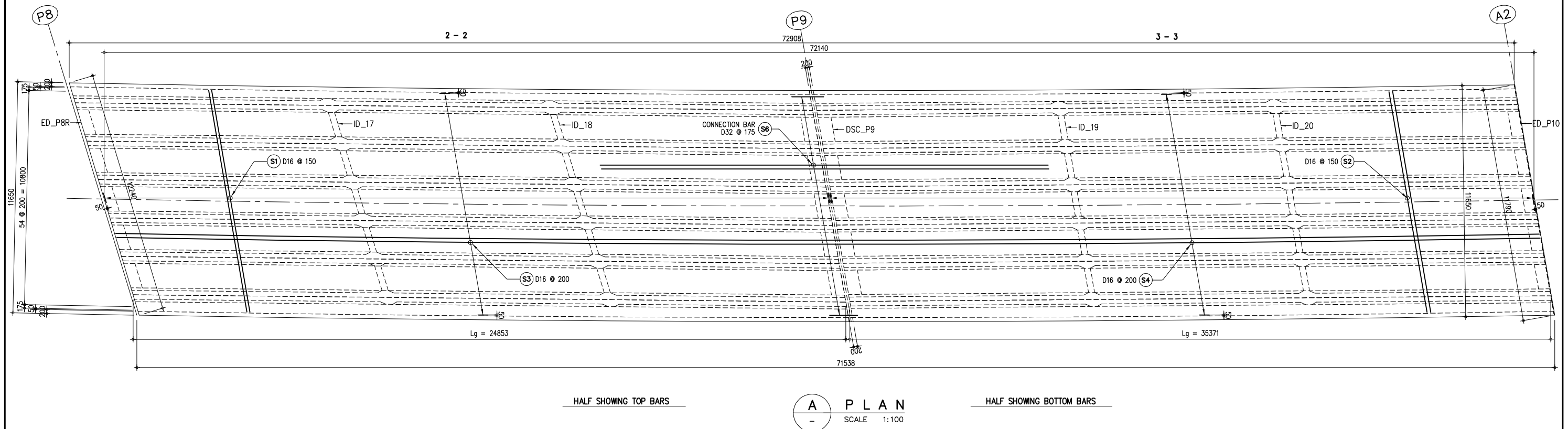
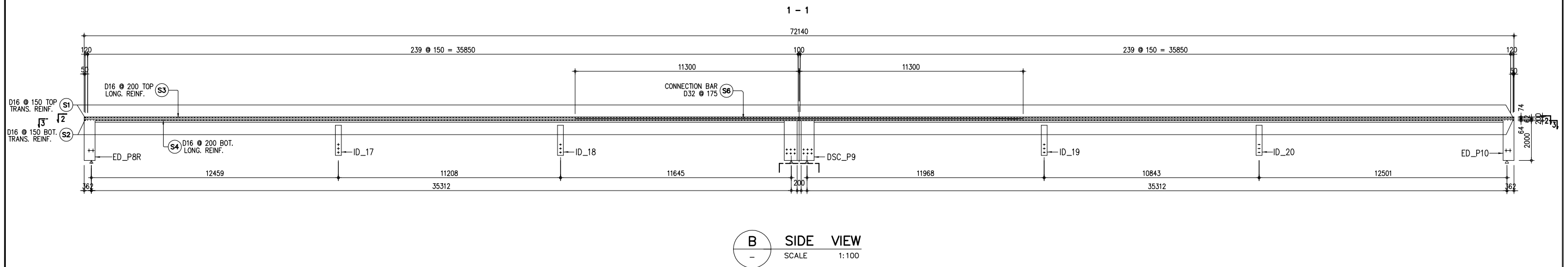
**A CROSS SECTION**  
SCALE 1:20

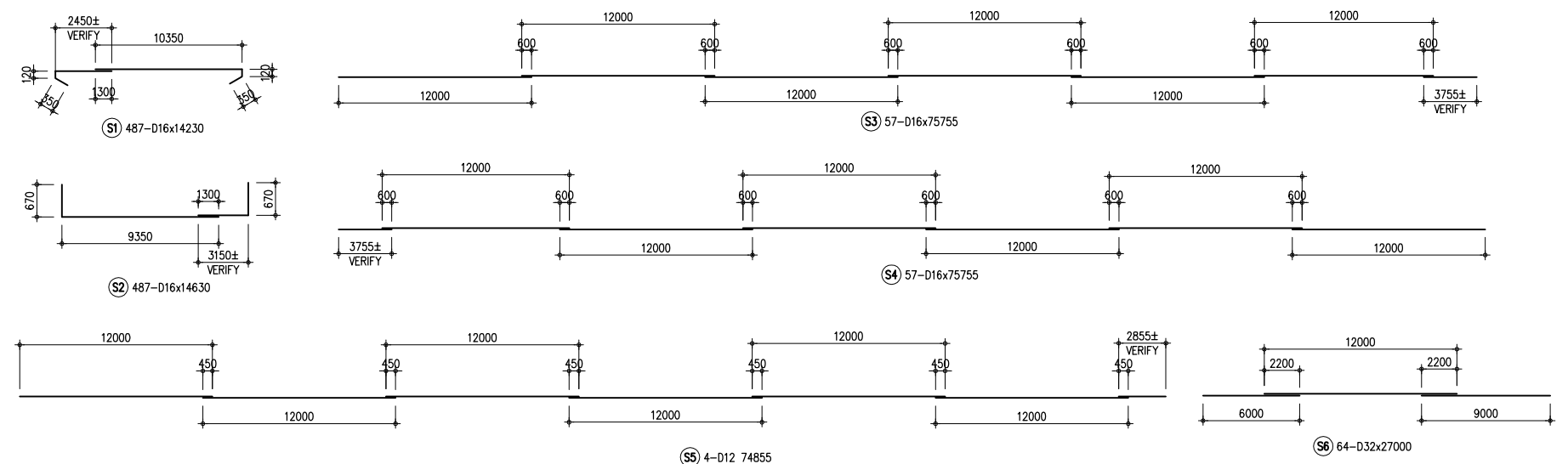


**B DETAIL**  
SCALE 1:10



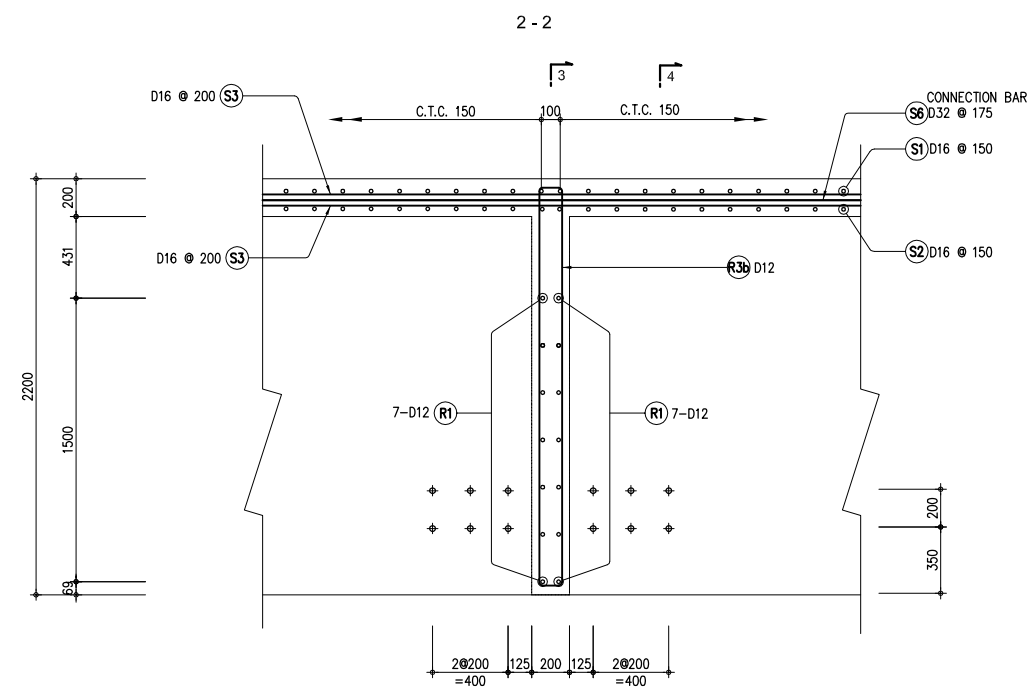
**C BAR BENDING LAYOUT**  
SCALE NTS



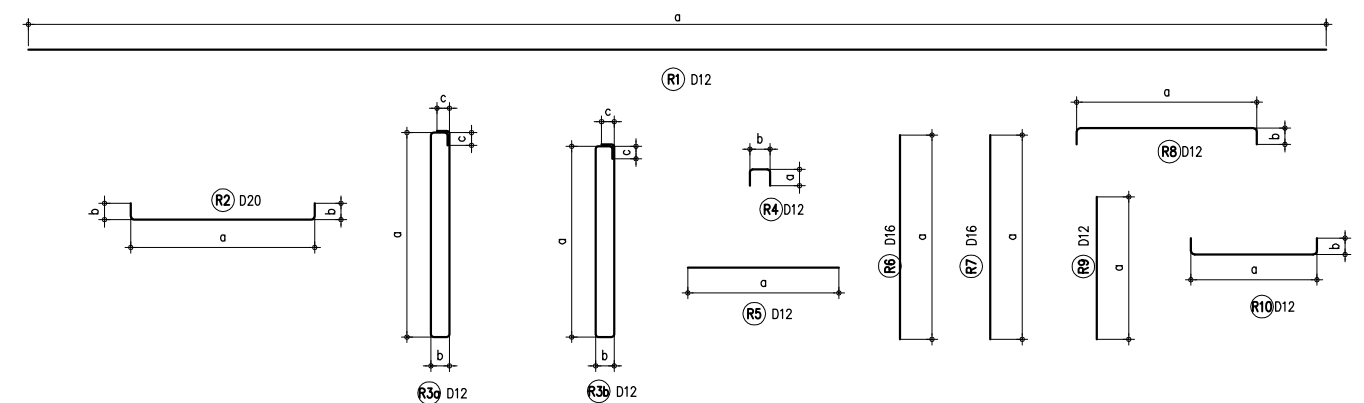


**C BAR BENDING LAYOUT**  
SCALE NTS

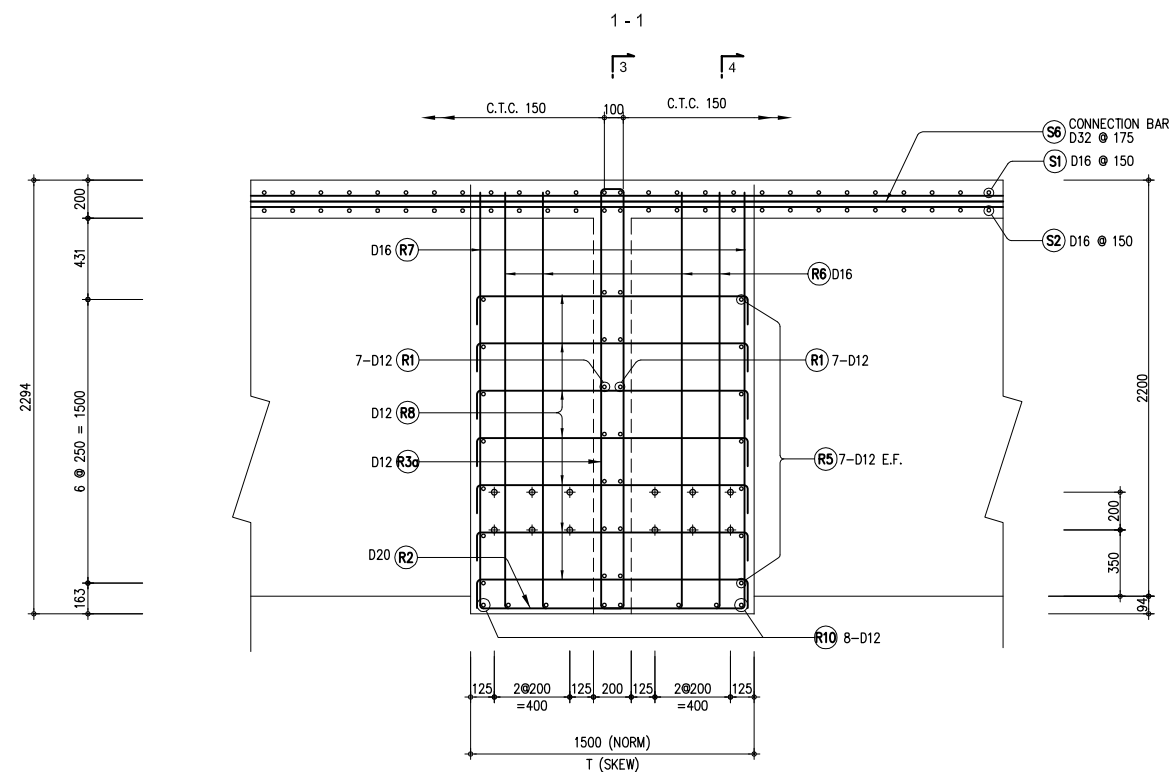




**B** SIDE VIEW  
SCALE 1:20

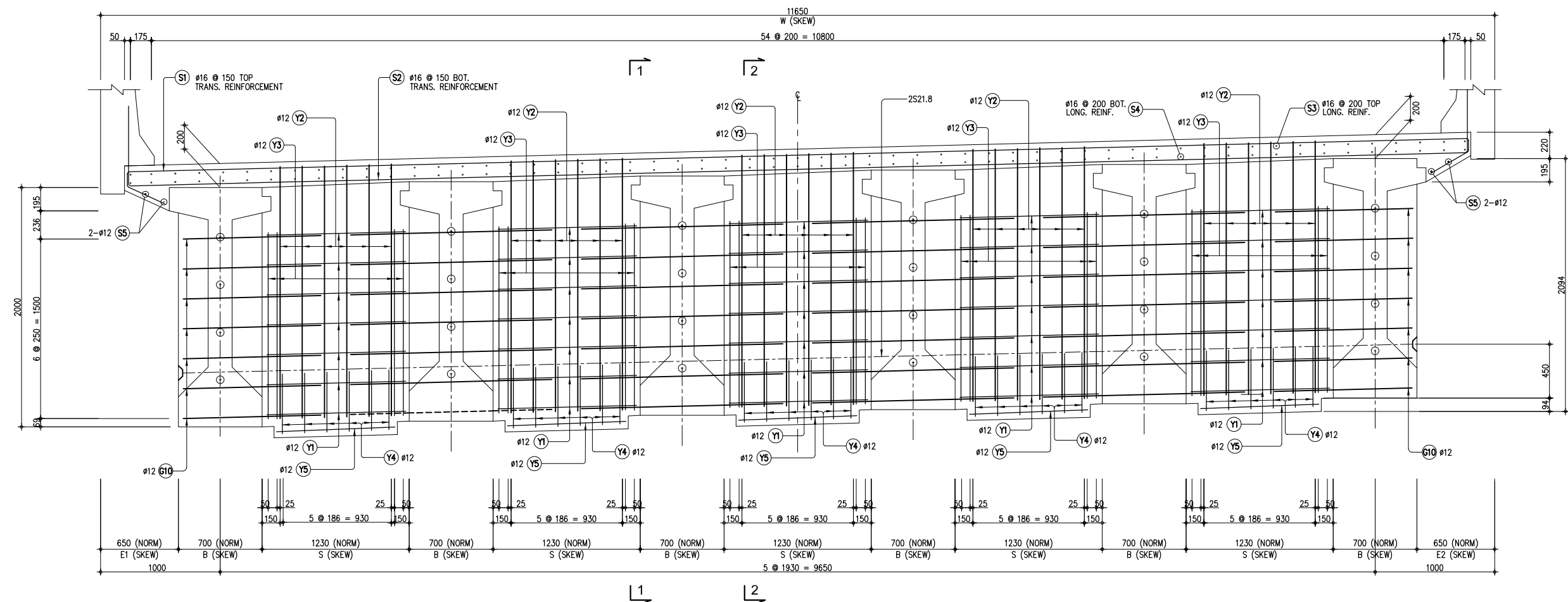


**D** BAR BENDING LAYOUT  
SCALE NTS

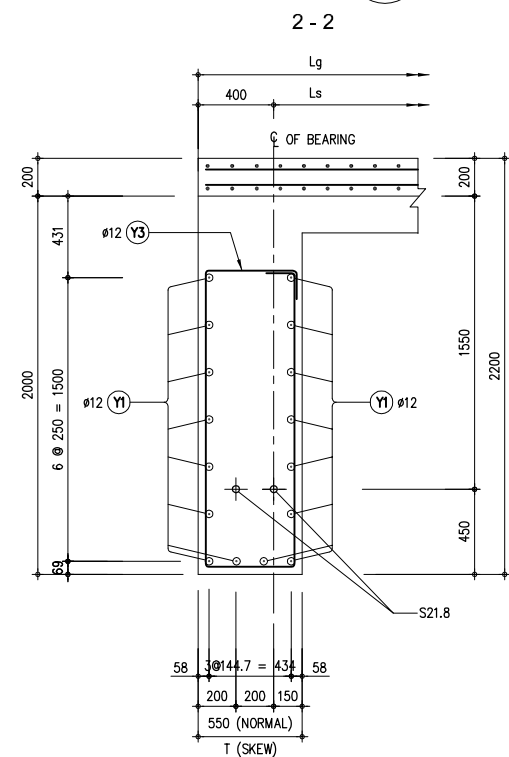
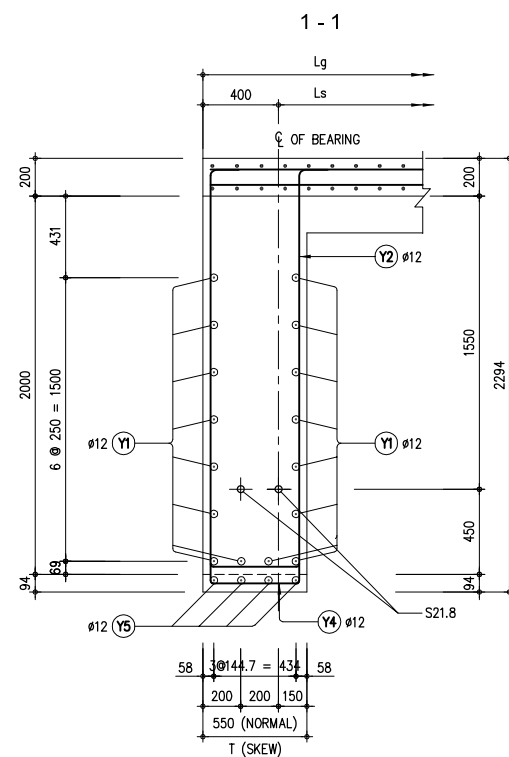


**C** SIDE VIEW  
SCALE 1:20

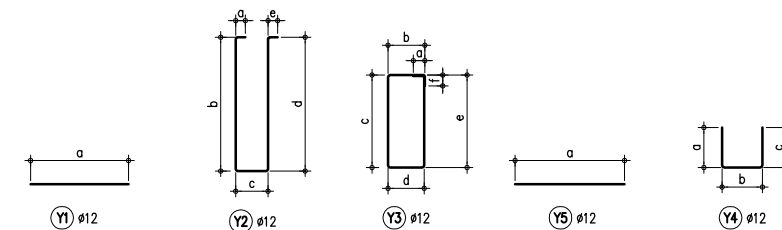




**A2** CROSS SECTION  
SCALE 1:20



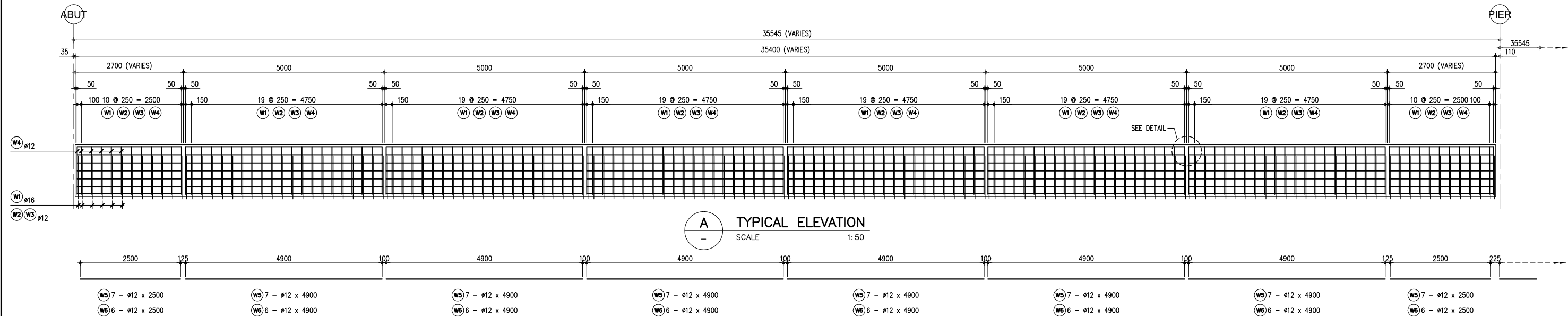
**B** SIDE VIEW  
SCALE 1:20



**C** BAR BENDING LAYOUT  
SCALE NTS

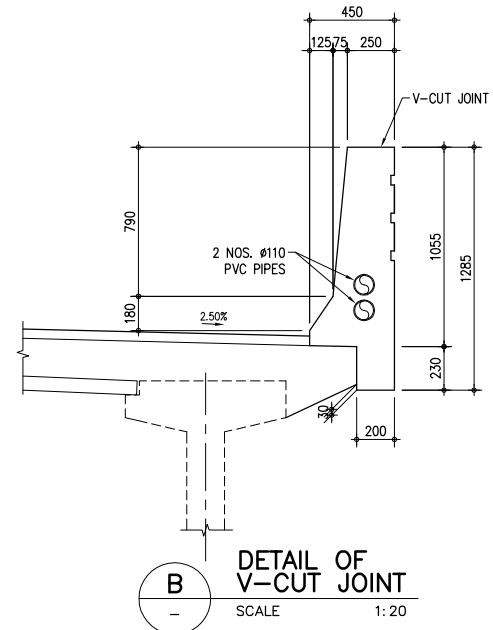
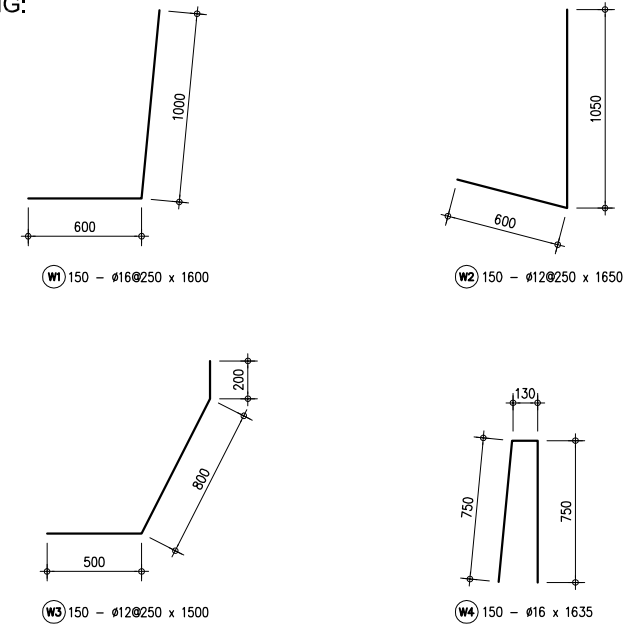


PARAPET REINFORCEMENT AT P0 TO A2

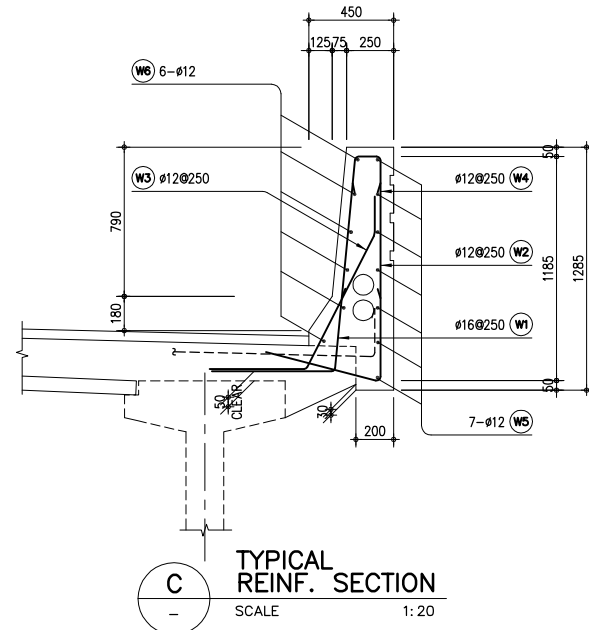


A TYPICAL ELEVATION  
SCALE 1:50

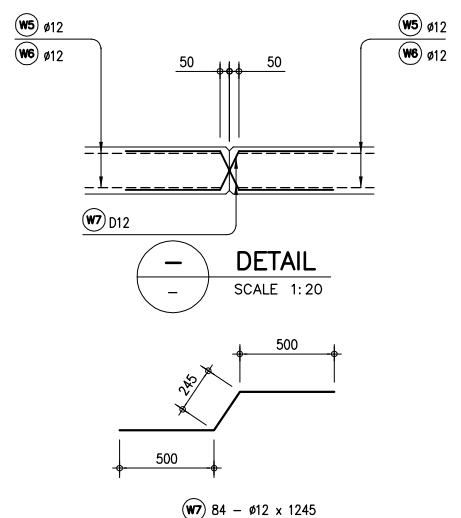
BAR BENDING:



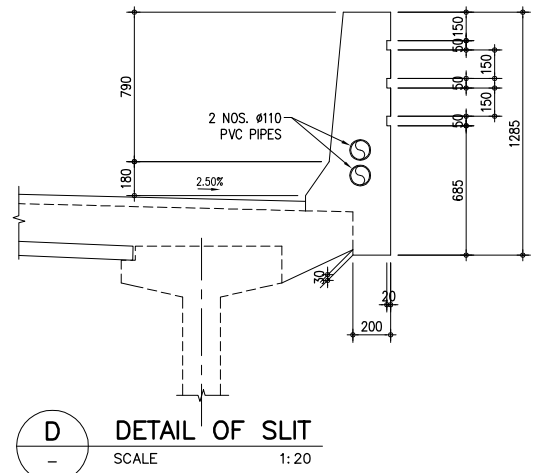
B DETAIL OF V-CUT JOINT  
SCALE 1:20



C TYPICAL REINF. SECTION  
SCALE 1:20



DETAIL  
SCALE 1:20



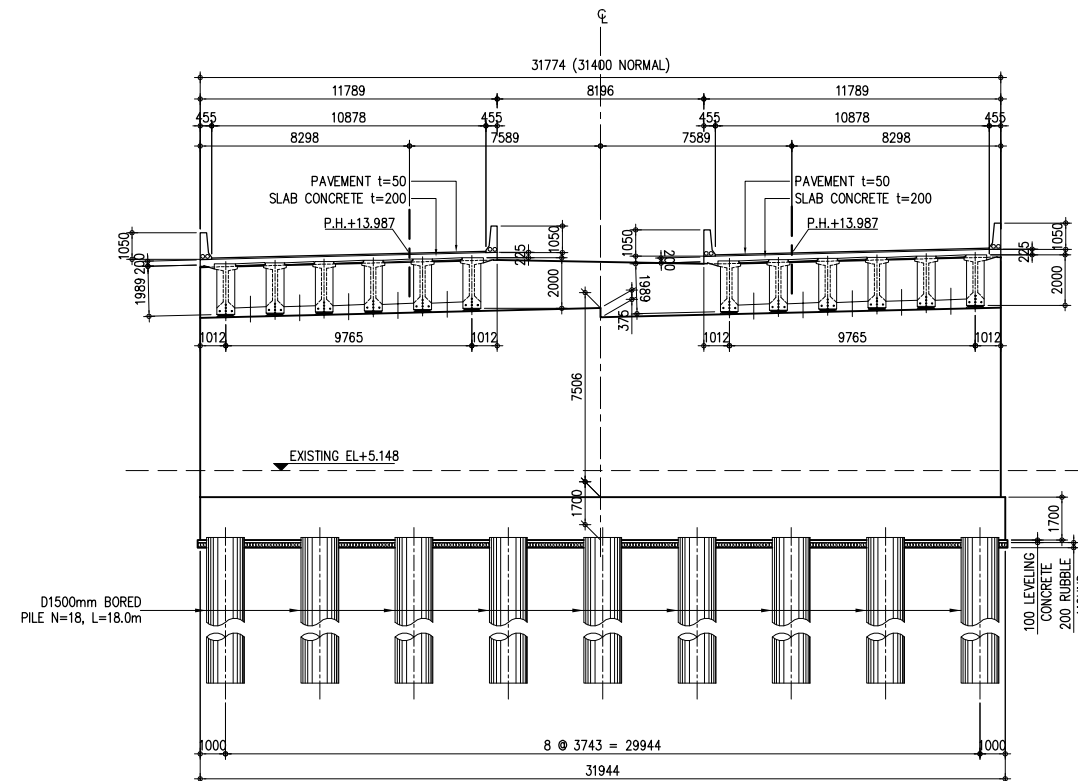
D DETAIL OF SLIT  
SCALE 1:20



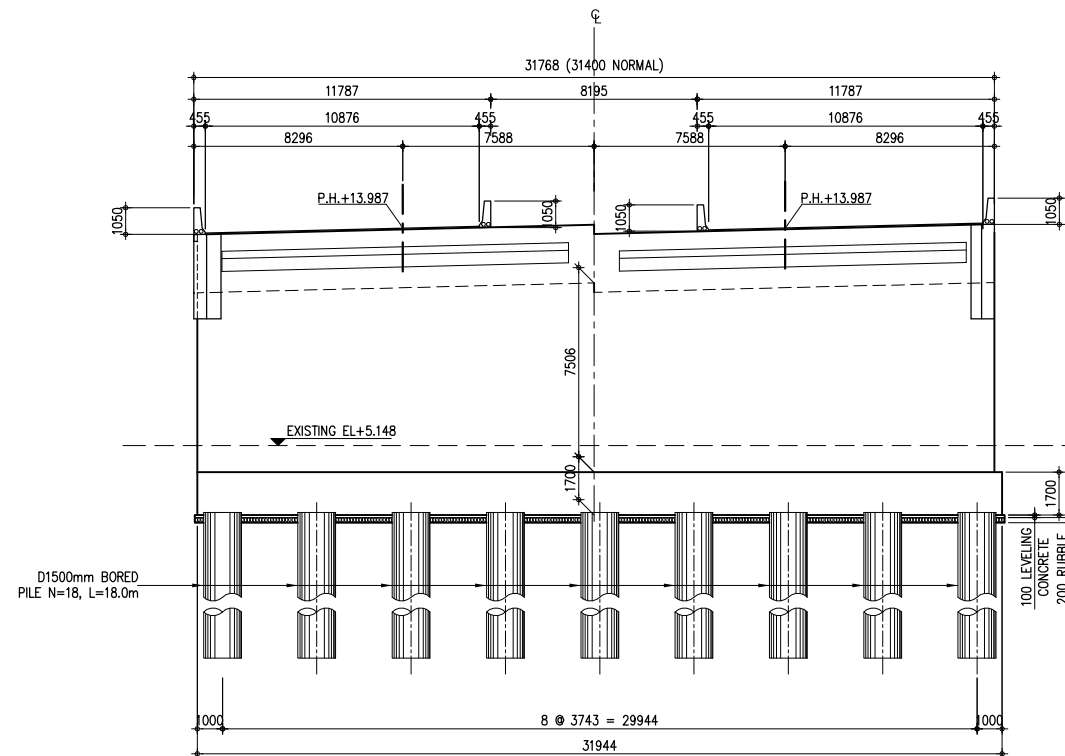
ESTIMATED QUANTITIES FOR SLAB															
REINFORCING BARS															
LOCATION	BAR MARK	SPACING	QTY. (pcs)	SIZE (mm)	DIMENSION mm ( OUT TO OUT )						LENGTH PER BAR (mm)	TOTAL LENGTH (m)	UNIT WEIGHT (kg/m)	TOTAL WEIGHT (kg)	CONCRETE VOLUME (m³)
SLAB - P0 to P1 to P2	S1	150	466	16	350	120	2407	10750	120	350		14097	6569.20	1.579	10,372.77
	S2	150	466	16	670	9725	3432	670				14497	6755.60	1.579	10,667.10
	S3	200	57	16	12000	12000	12000	12000	12000	1300		73300	4178.10	1.579	6,597.22
	S4	200	57	16	1300	12000	12000	12000	12000	12000		73300	4178.10	1.579	6,597.22
	S5	as shown	4	12	12000	12000	12000	12000	12000	11950		71950	287.80	0.888	255.57
	S6	175	64	32	6000	12000	9000					27000	1728.00	6.313	10,908.86
	A. TOTAL QUANTITY														45,398.74
SLAB - P2 to P3 to P4	S1	150	474	16	350	120	2475	11000	120	350		14415	6832.71	1.579	10,788.85
	S2	150	474	16	670	9950	3525	670				14815	7022.31	1.579	11,088.23
	S3	200	57	16	12000	12000	12000	12000	12000	2445		74445	4243.37	1.579	6,700.27
	S4	200	57	16	2445	12000	12000	12000	12000	12000		74445	4243.37	1.579	6,700.27
	S5	as shown	4	12	12000	12000	12000	12000	12000	1545		73545	294.18	0.888	261.23
	S6	175	64	32	6000	12000	9000					27000	1728.00	6.313	10,908.86
	B. TOTAL QUANTITY														46,447.72
SLAB - P4 to P5 to P6	S1	150	474	16	350	120	2440	10850	120	350		14230	6745.02	1.579	10,650.39
	S2	150	474	16	670	9820	3470	670				14630	6934.62	1.579	10,949.76
	S3	200	57	16	12000	12000	12000	12000	12000	12000	2440	74440	4243.08	1.579	6,699.82
	S4	200	57	16	2440	12000	12000	12000	12000	12000		74440	4243.08	1.579	6,699.82
	S5	as shown	4	12	12000	12000	12000	12000	12000	1540		73540	294.16	0.888	261.21
	S6	175	64	32	6000	12000	9000					27000	1728.00	6.313	10,908.86
	C. TOTAL QUANTITY														46,169.88
SLAB - P6 to P7 to P8	S1	150	472	16	350	120	2325	10750	120	350		14015	6615.08	1.579	10,445.21
	S2	150	472	16	670	9700	3375	670				14415	6803.88	1.579	10,743.33
	S3	200	57	16	12000	12000	12000	12000	12000	12000	2400	74400	4240.80	1.579	6,696.22
	S4	200	57	16	2400	12000	12000	12000	12000	12000		74400	4240.80	1.579	6,696.22
	S5	as shown	4	12	12000	12000	12000	12000	12000	1500		73500	294.00	0.888	261.07
	S6	175	64	32	6000	12000	9000					27000	1728.00	6.313	10,908.86
	D. TOTAL QUANTITY														45,750.92
SLAB - P8 to P9 to A2	S1	150	487	16	350	120	2450	10350	120	350		13740	6691.38	1.579	10,565.69
	S2	150	487	16	670	9350	3150	670				13840	6740.08	1.579	10,642.59
	S3	200	57	16	12000	12000	12000	12000	12000	12000	3755	75755	4318.04	1.579	6,818.18
	S4	200	57	16	3755	12000	12000	12000	12000	12000	12000	75755	4318.04	1.579	6,818.18
	S5	as shown	4	12	12000	12000	12000	12000	12000	12000	2855	74855	299.42	0.888	265.88
	S6	175	64	32	6000	12000	9000					27000	1728.00	6.313	10,908.86
	E. TOTAL QUANTITY														46,019.38
PARAPET	E. TOTAL QUANTITY														229,786.63
	W1	250	150	16	600	1000						1600	240.000	1.579	378.96
	W2	250	150	12	600	1050						1650	247.500	0.888	219.78
	W3	250	150	12	500	800	200					1500	225.000	0.888	199.80
	W4	250	150	16	750	130	750					1630	244.500	1.579	386.07
	W5	as shown	14	12	2500							2500	35.000	0.888	31.08
	W5a	as shown	42	12	4900							4900	205.800	0.888	182.75
	W6	as shown	12	12	2500							2500	30.000	0.888	26.64
	W6a	as shown	36	12	4900							4900	176.400	0.888	156.64
	W7	as shown	84	12	500	245	500					1245	104.580	0.888	92.87
	A. TOTAL QUANTITY PER PARAPET														1,674.59
	B. NUMBER OF SPANS x 2														20
	GRAND TOTAL QUANTITY FOR PARAPET														33,491.72

ESTIMATED QUANTITIES FOR THE DIAPHRAGMS SLAB CONNECTION														
REINFORCING BARS														
LOCATION	BAR MARK	QTY. (pcs)	SIZE (mm)	DIMENSION mm ( OUT TO OUT )						LENGTH PER BAR (mm)	TOTAL LENGTH (m)	UNIT WEIGHT (kg/m)	TOTAL WEIGHT (kg)	
				a	b	c	d	e	f					g
DSC_P1	R1	14	12	10908							10908	152.712	0.888	135.61
	R2	30	20	130	1429	130					1689	50.670	2.466	124.95
	R3a	30	12	100	148	2204	148	2204	100		4904	147.120	0.888	130.64
	R3b	30	12	100	148	2110	148	2110	100		4716	141.480	0.888	125.63
	R4	14	12	130	160	130					420	5.880	0.888	5.22
	R5	70	12	1226							1226	85.820	0.888	76.21
	R6	120	16	2204							2204	264.480	1.579	417.61
	R7	60	16	2204							2204	132.240	1.579	208.81
	R8	210	12	130	1509	130					1769	371.490	0.888	329.88
	R9	60	12	1536							1536	92.160	0.888	81.84
	R10	40	12	130	1026	130					1286	51.440	0.888	45.68
	A.TOTAL QUANTITY FOR PIER												1,682.08	
DSC_P3	R1	14	12	11198							11198	156.772	0.888	139.21
	R2	30	20	130	1467	130					1727	51.810	2.466	127.76
	R3a	30	12	100	148	2204	148	2204	100		4904	147.120	0.888	130.64
	R3b	30	12	100	148	2110	148	2110	100		4716	141.480	0.888	125.63
	R4	14	12	130	160	130					420	5.880	0.888	5.22
	R5	70	12	1260							1260	88.200	0.888	78.32
	R6	120	16	2204							2204	264.480	1.579	417.61
	R7	60	16	2204							2204	132.240	1.579	208.81
	R8	210	12	130	1547	130					1807	379.470	0.888	336.97
	R9	60	12	1536							1536	92.160	0.888	81.84
	R10	40	12	130	1060	130					1320	52.800	0.888	46.89
	B.TOTAL QUANTITY FOR PIER												1,698.90	
DSC_P5	R1	14	12	11037							11037	154.518	0.888	137.21
	R2	30	20	130	1444	130					1704	51.120	2.466	126.06
	R3a	30	12	100	148	2204	148	2204	100		4904	147.120	0.888	130.64
	R3b	30	12	100	148	2110	148	2110	100		4716	141.480	0.888	125.63
	R4	14	12	130	160	130					420	5.880	0.888	5.22
	R5	70	12	1241							1241	86.870	0.888	77.14
	R6	120	16	2204							2204	264.480	1.579	417.61
	R7	60	16	2204							2204	132.240	1.579	208.81
	R8	210	12	130	1524	130					1784	374.640	0.888	332.68
	R9	60	12	1536							1536	92.160	0.888	81.84
	R10	40	12	130	1041	130					1301	52.040	0.888	46.21
	C.TOTAL QUANTITY FOR PIER												1,689.05	
DSC_P7	R1	14	12	10887							10887	152.418	0.888	135.35
	R2	30	20	130	1424	130					1684	50.520	2.466	124.58
	R3a	30	12	100	148	2204	148	2204	100		4904	147.120	0.888	130.64
	R3b	30	12	100	148	2110	148	2110	100		4716	141.480	0.888	125.63
	R4	14	12	130	160	130					420	5.880	0.888	5.22
	R5	70	12	1223							1223	85.610	0.888	76.02
	R6	120	16	2204							2204	264.480	1.579	417.61
	R7	60	16	2204							2204	132.240	1.579	208.81
	R8	210	12	130	1504	130					1764	370.440	0.888	328.95
	R9	60	12	1536							1536	92.160	0.888	81.84
	R10	40	12	130	1023	130					1283	51.320	0.888	45.57
	D.TOTAL QUANTITY FOR PIER												1,680.22	
DSC_P9	R1	14	12	10447							10447	146.258	0.888	129.88
	R2	30	20	130	1362	130					1622	48.660	2.466	120.00
	R3a	30	12	100	148	2204	148	2204	100		4904	147.120	0.888	130.64
	R3b	30	12	100	148	2110	148	2110	100		4716	141.480	0.888	125.63
	R4	14	12	130	160	130					420	5.880	0.888	5.22
	R5	70	12	1171							1171	81.970	0.888	72.79
	R6	120	16	2204							2204	264.480	1.579	417.61
	R7	60	16	2204							2204	132.240	1.579	208.81
	R8	210	12	130	1442	130					1702	357.420	0.888	317.39
	R9	60	12	1536							1536	92.160	0.888	81.84
	R10	40	12	130	971	130					1231	49.240	0.888	43.73
	E.TOTAL QUANTITY FOR PIER												1,653.54	
	GRAND TOTAL												8,403.79	

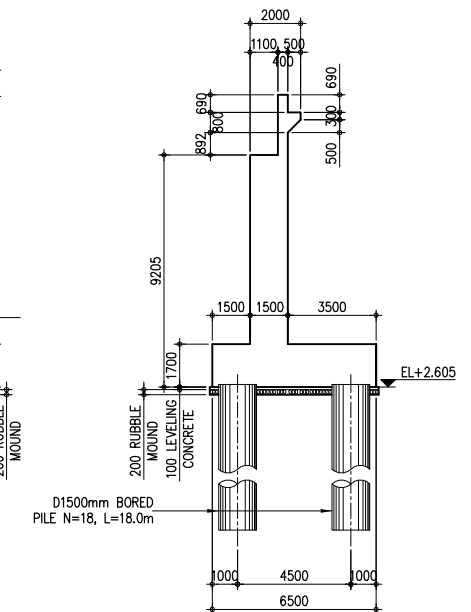
### HIGHWAY BRIDGE NO.9 (H9) - KELANI RIVER CROSSING BRIDGE (SCALE 1:150)



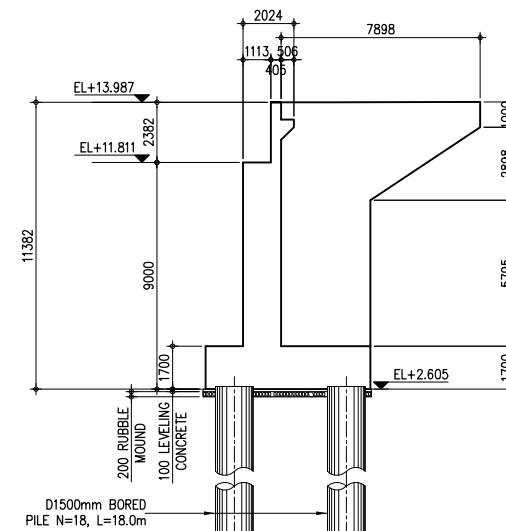
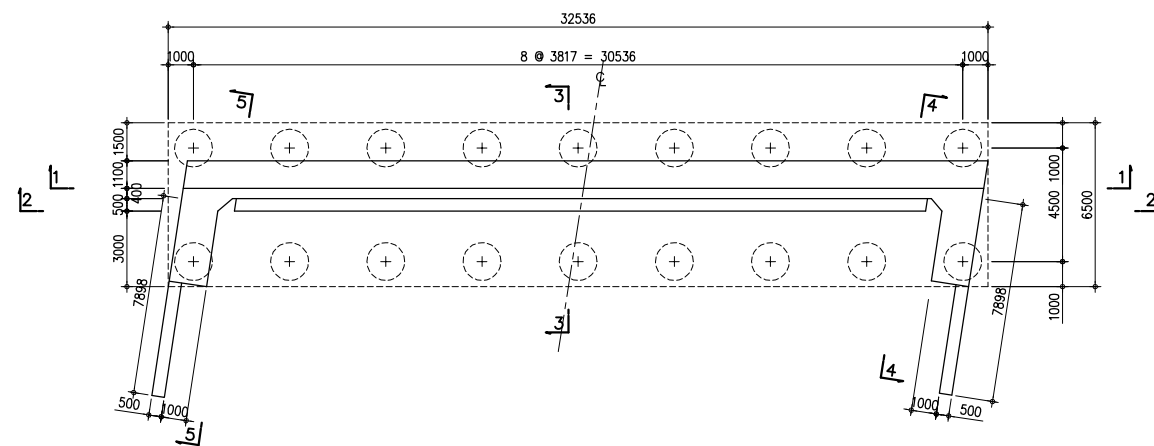
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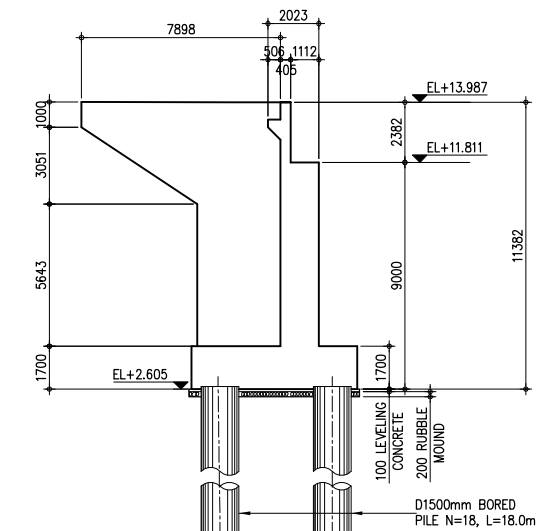
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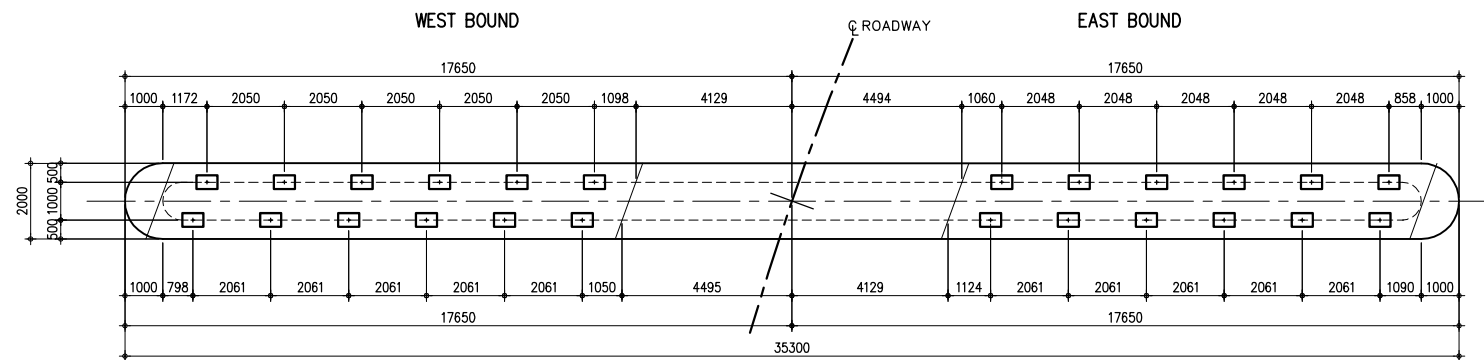
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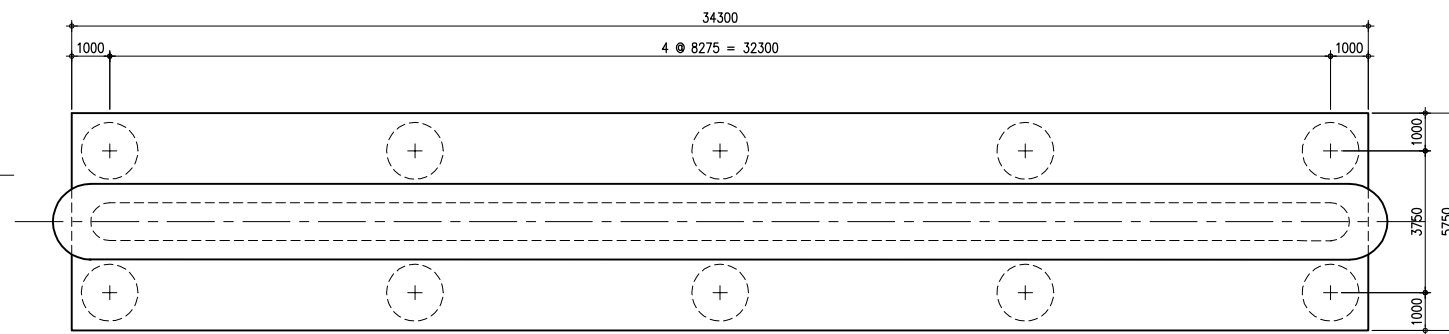
4 - 4



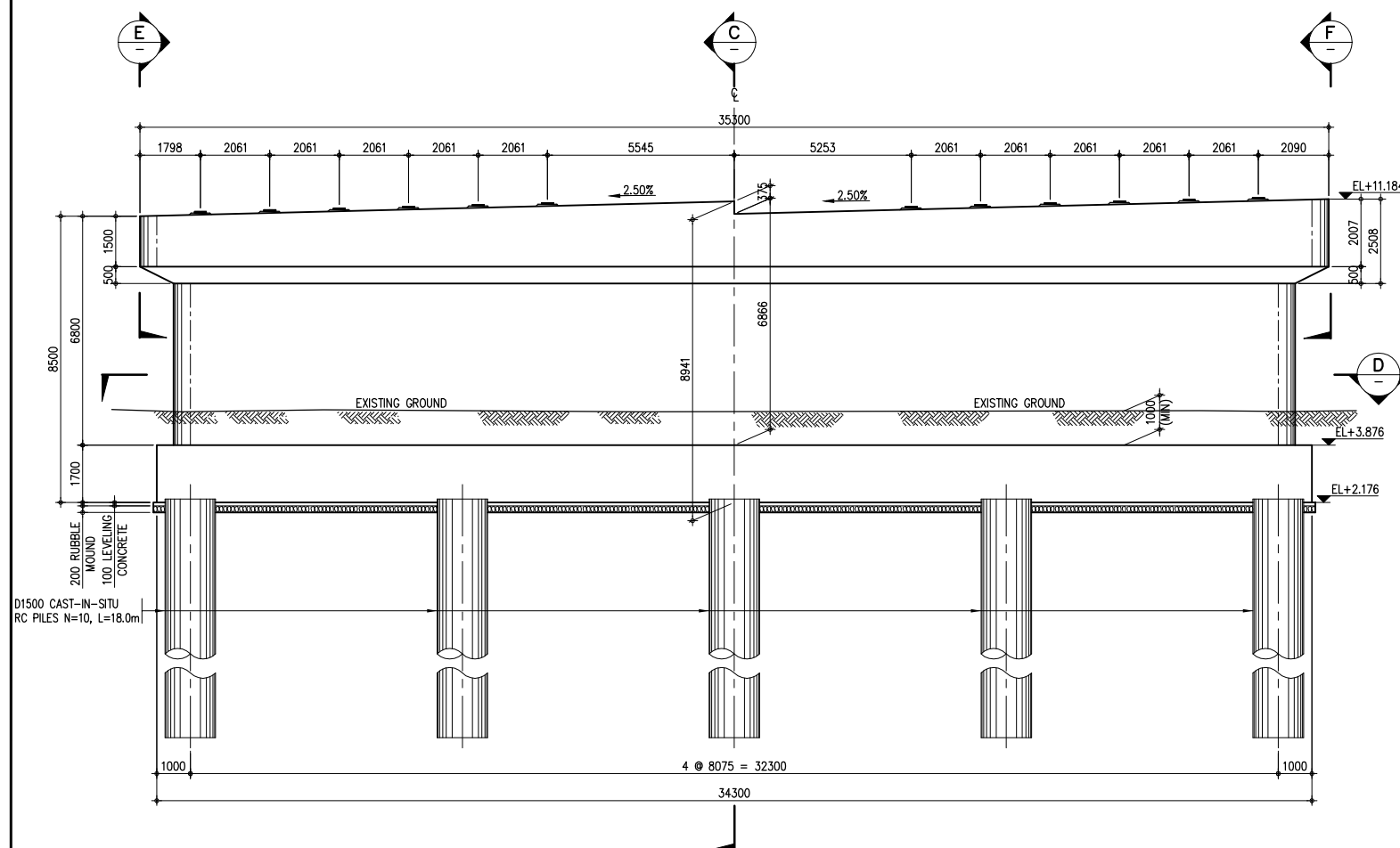
5 - 5



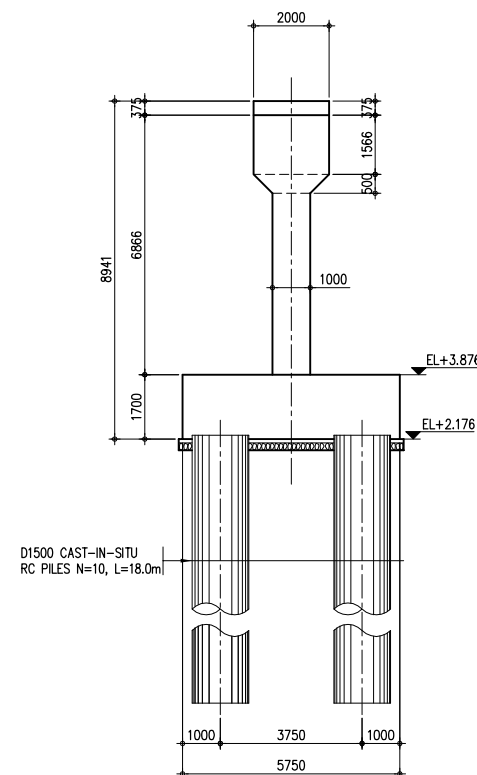
**A** COPING PLAN OF PIER P0  
SCALE 1:100



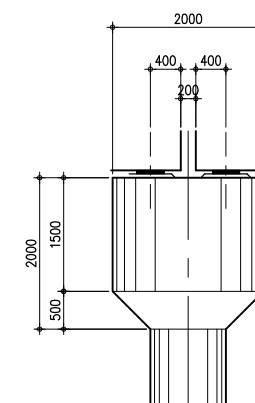
**D** FOUNDATION PLAN  
SCALE 1:100



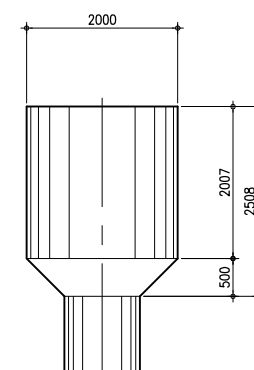
**B** FRONT ELEVATION  
SCALE 1:100



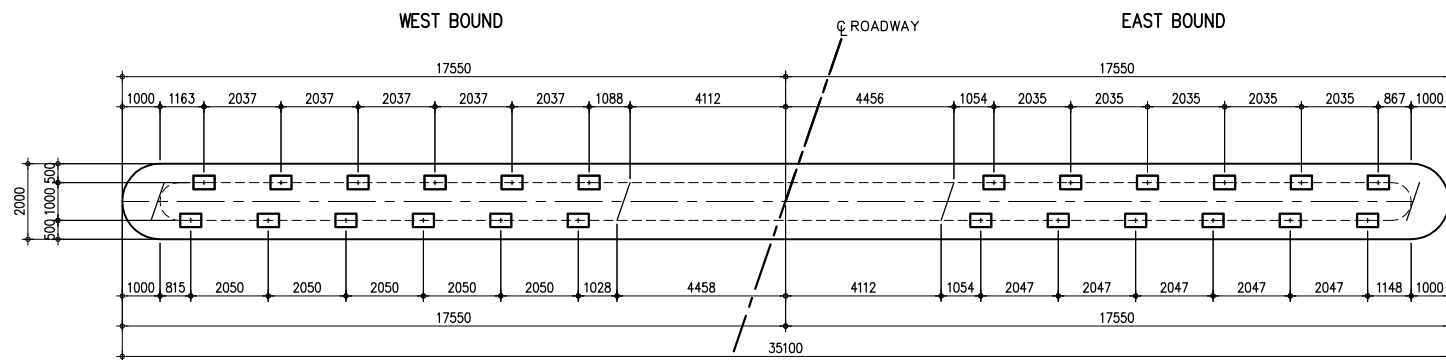
**C** SECTION @ ROADWAY  
SCALE 1:100



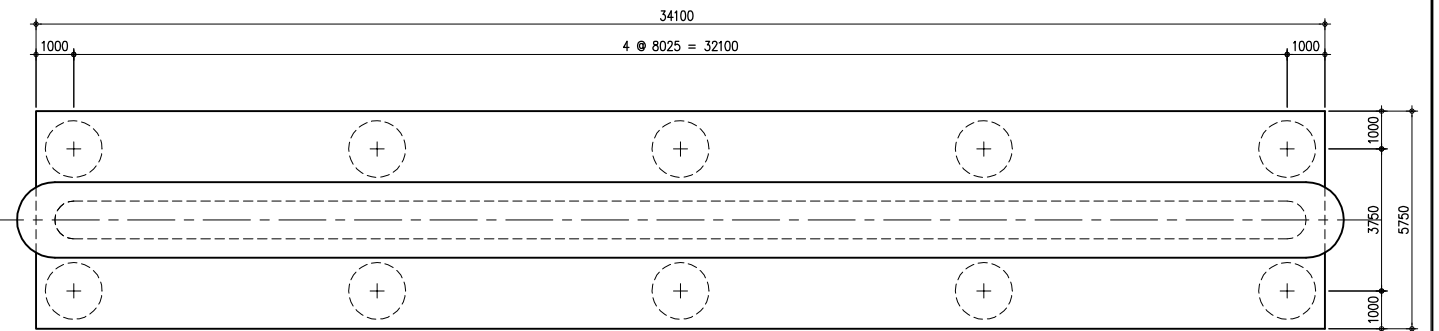
**E** SECTION  
SCALE 1:50



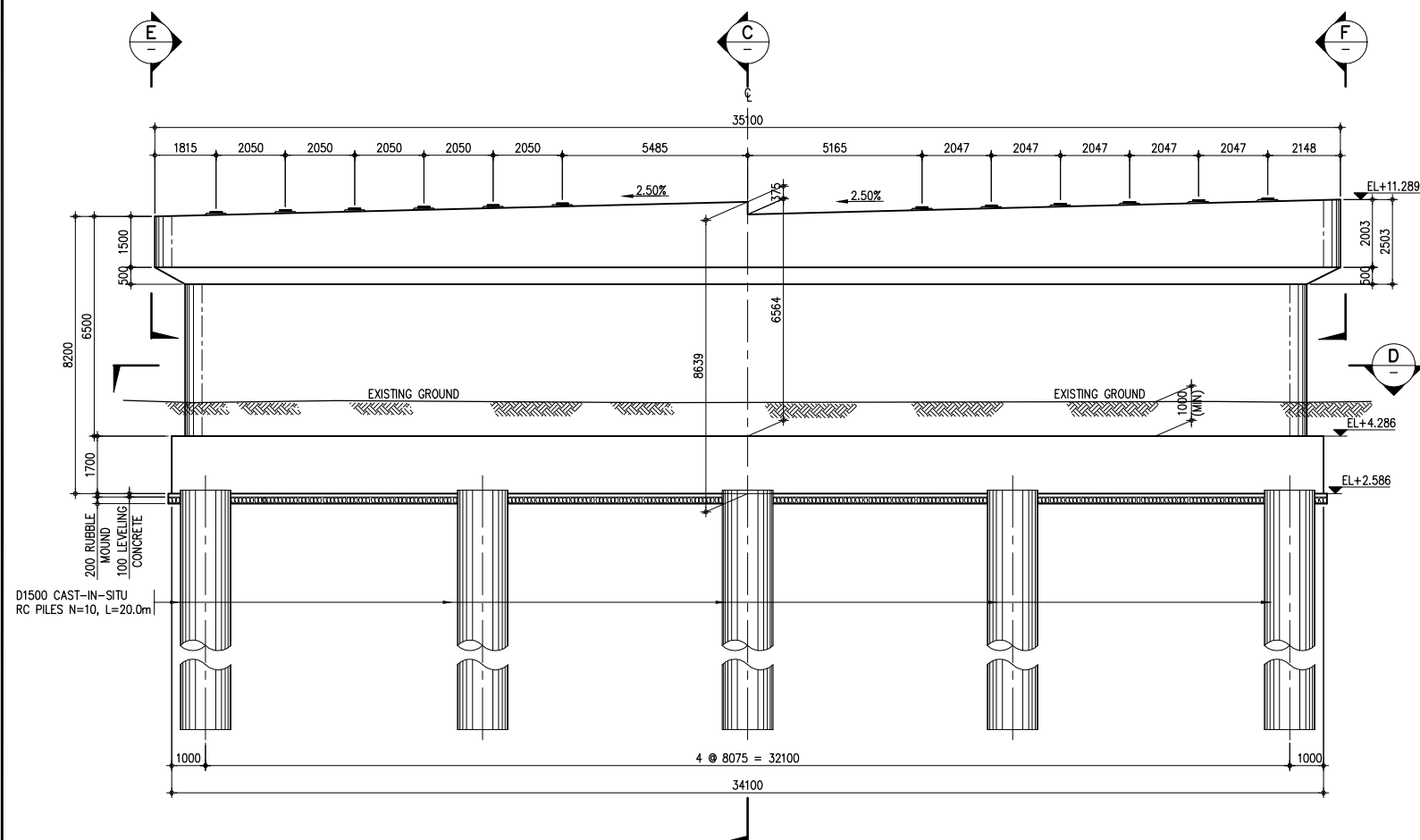
**F** SECTION  
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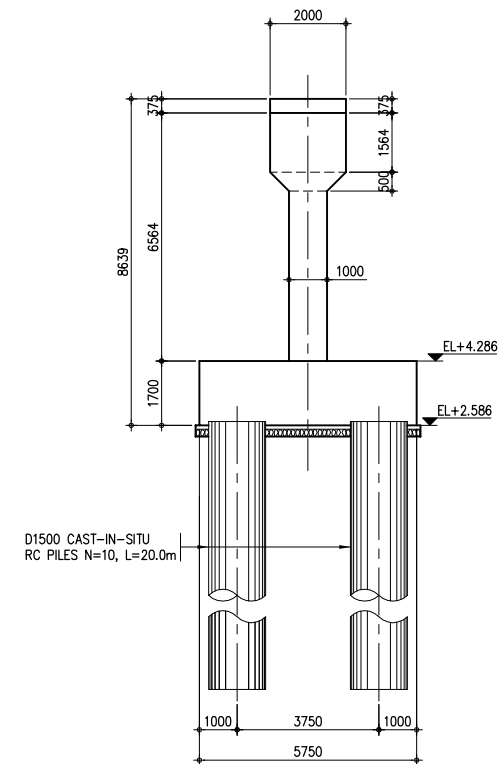
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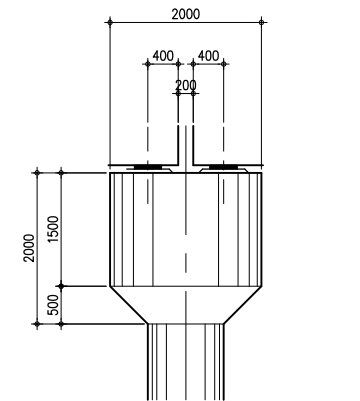
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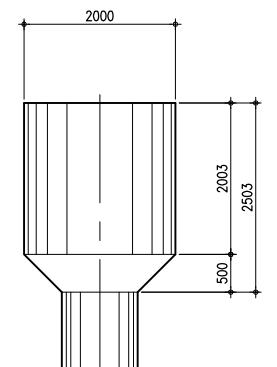
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SCALE 1:100



**C** SECTION @ ROADWAY  
SCALE 1:100

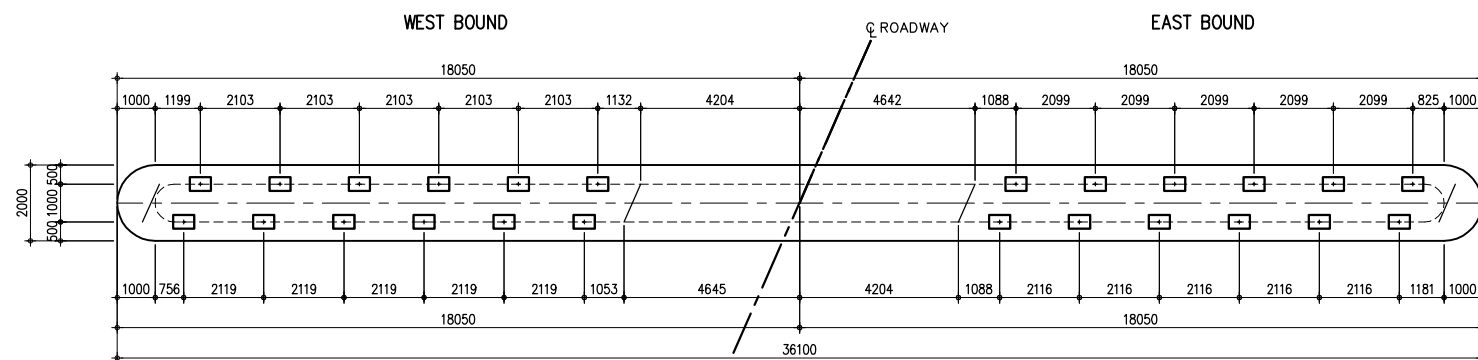


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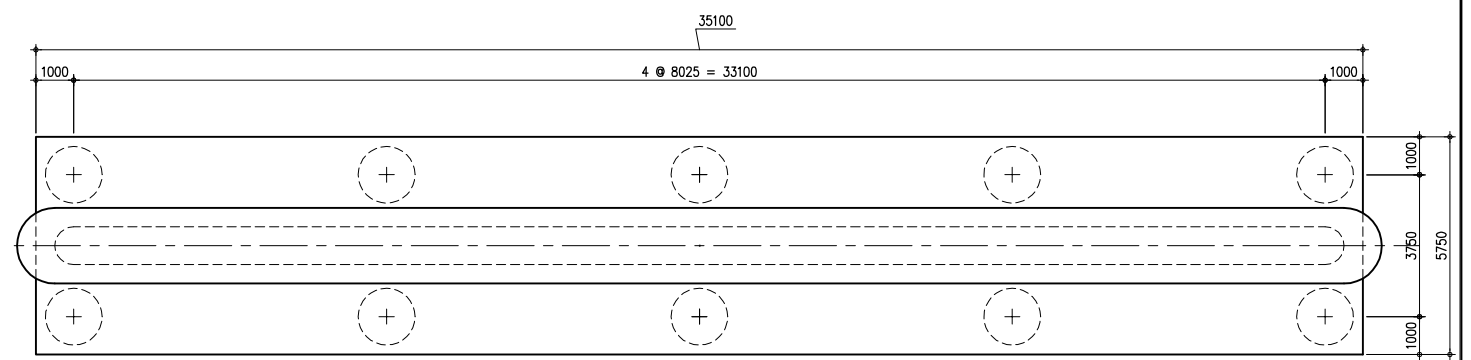


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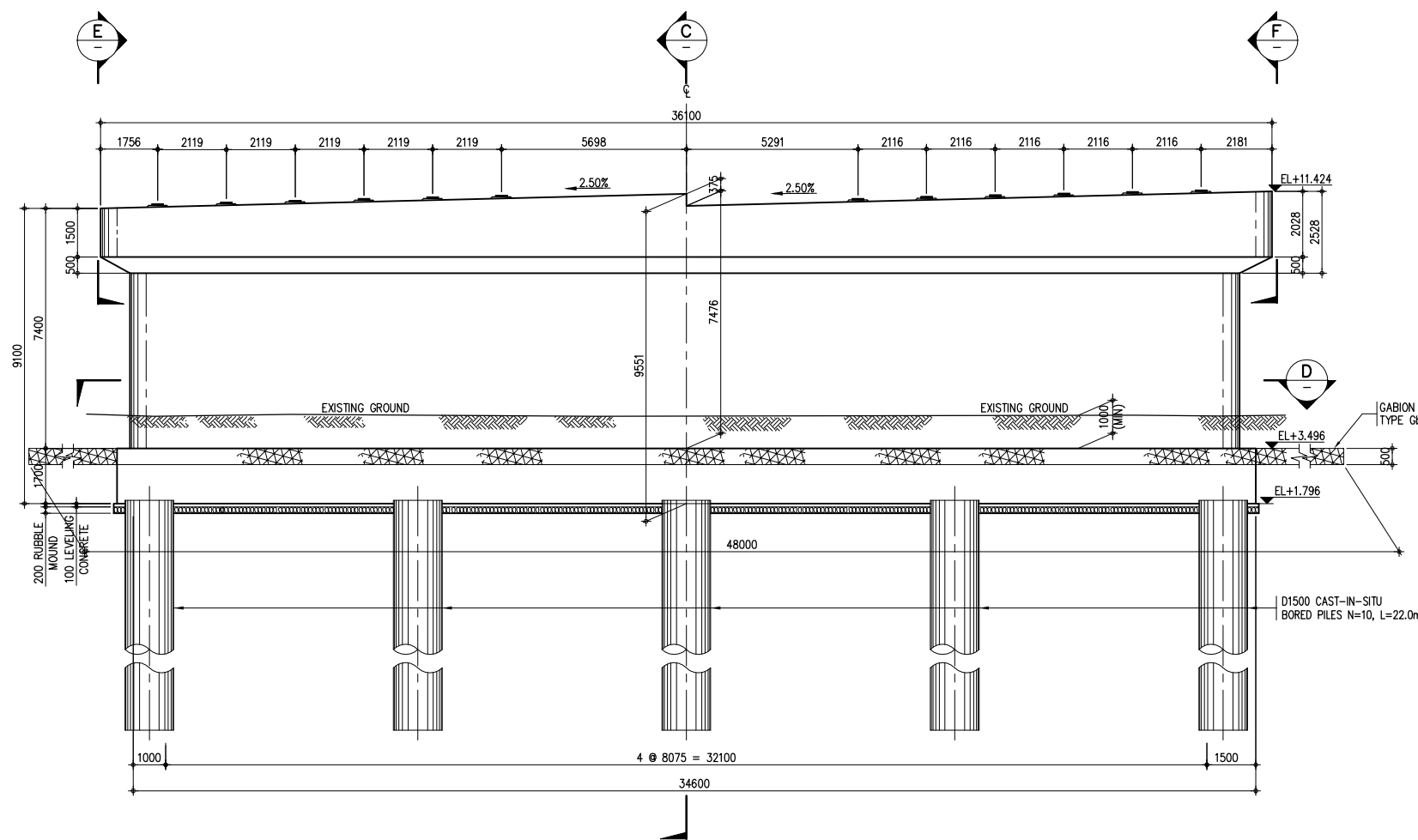




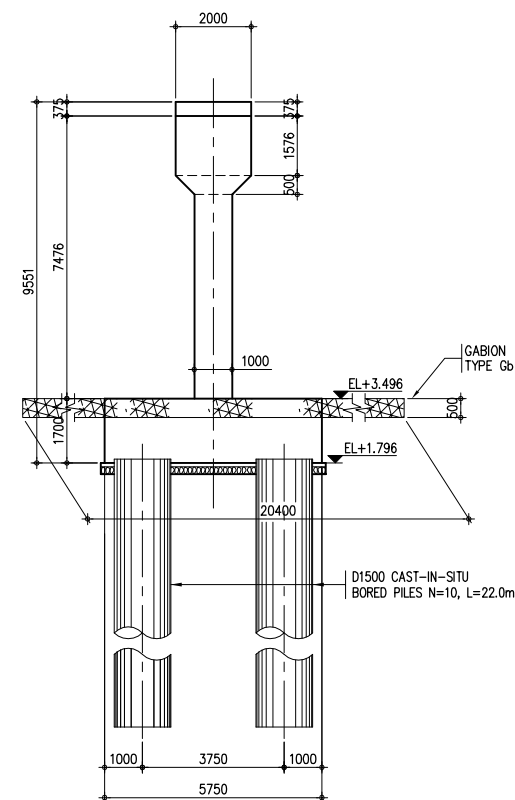
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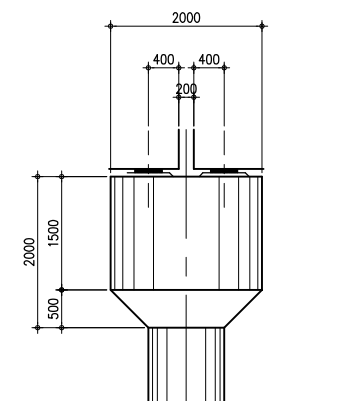
**D** FOUNDATION PLAN  
SCALE 1:150



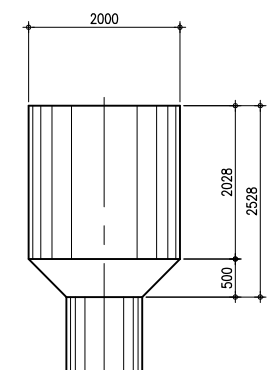
**B** FRONT ELEVATION  
SCALE 1:100



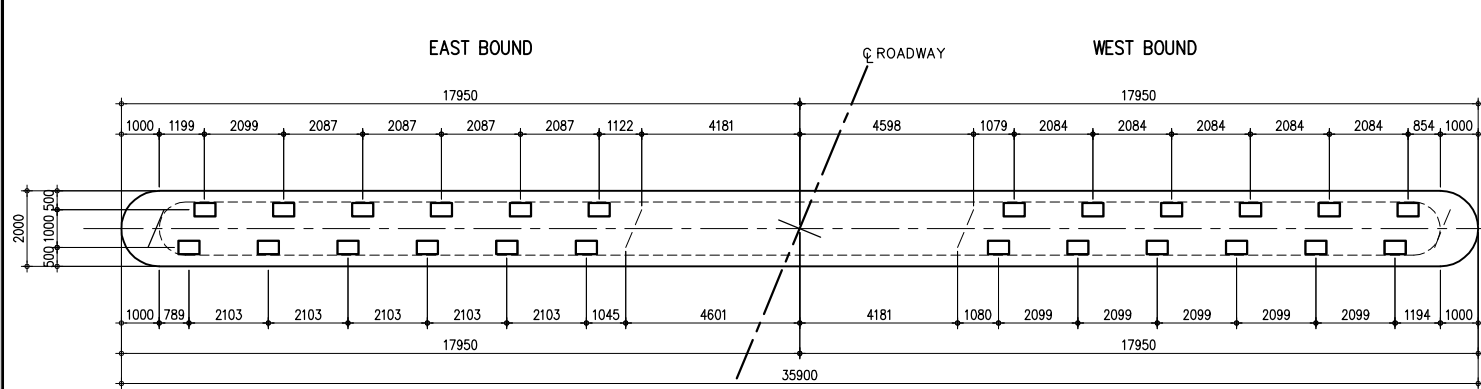
**C** SECTION @ ROADWAY C  
SCALE 1:100



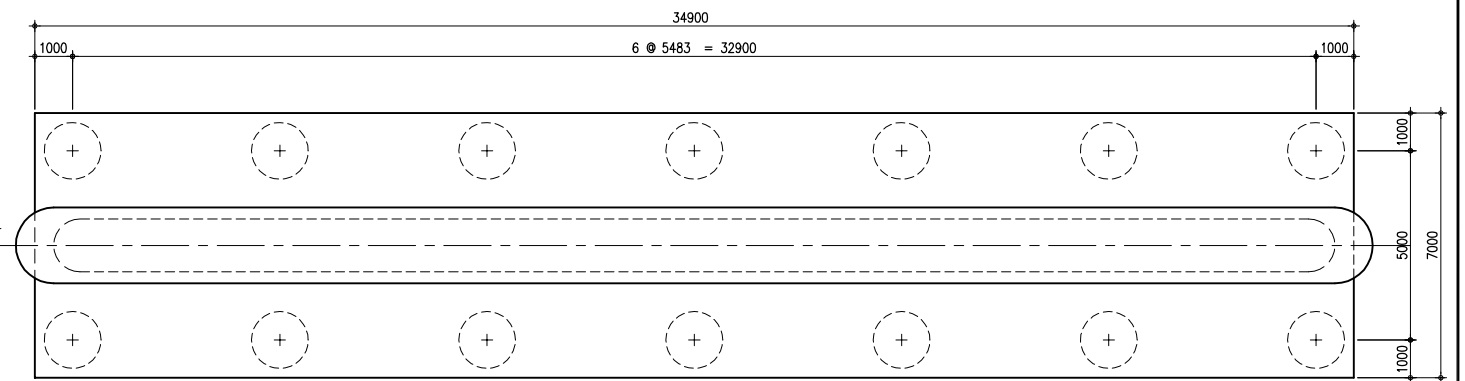
**E** SECTION  
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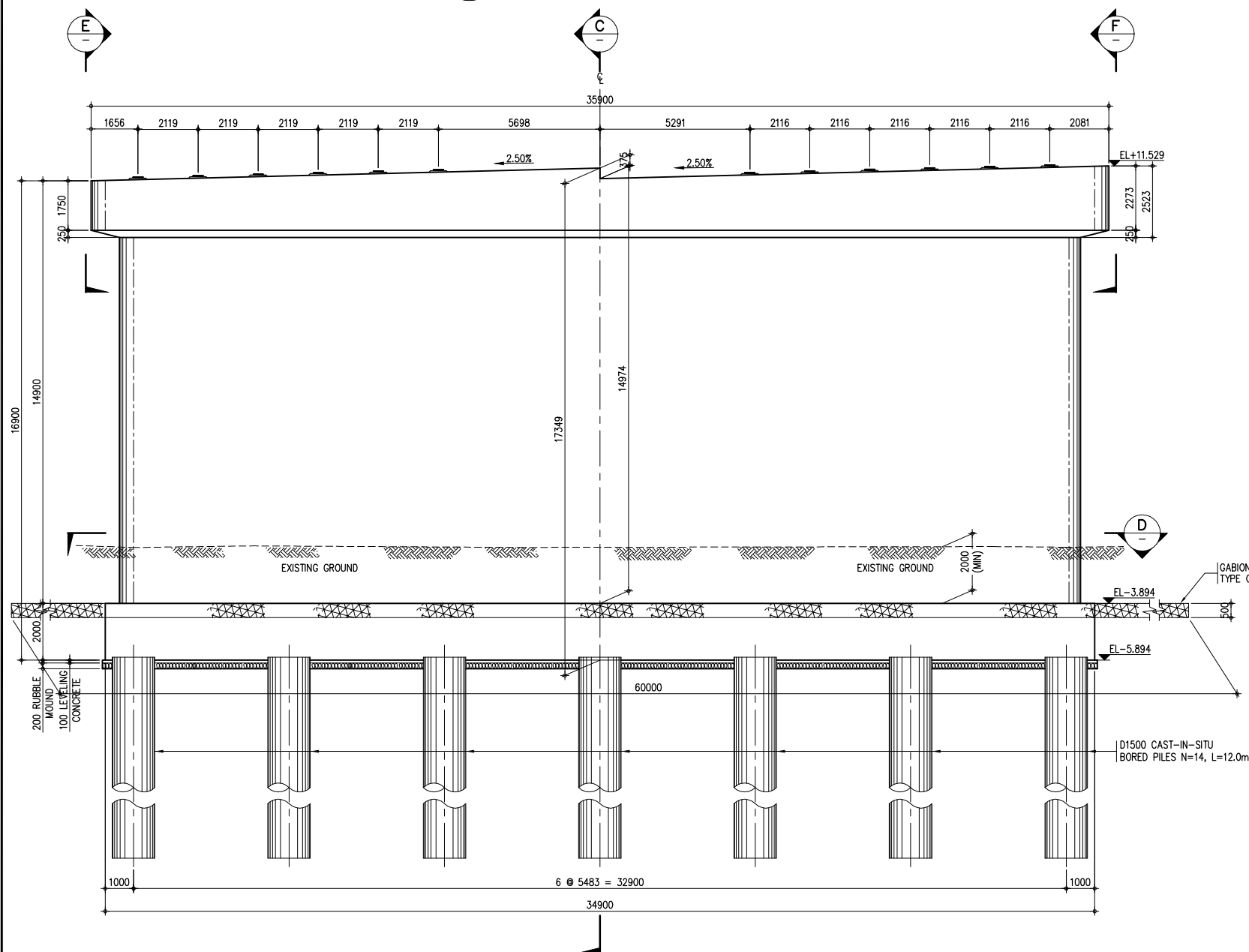
**F** SECTION  
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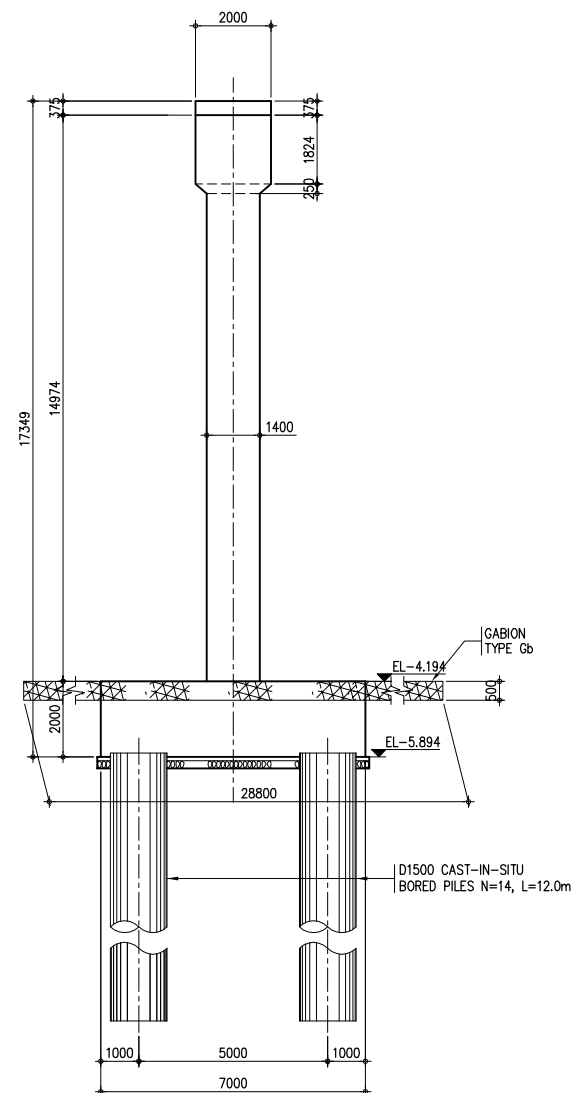
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SCALE 1:100



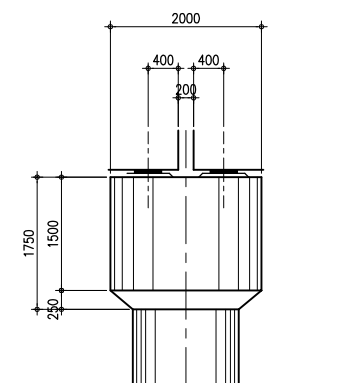
**D** FOUNDATION PLAN  
SCALE 1:100



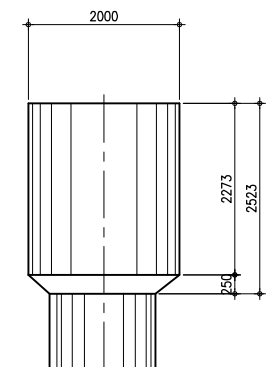
**B** FRONT ELEVATION  
SCALE 1:100



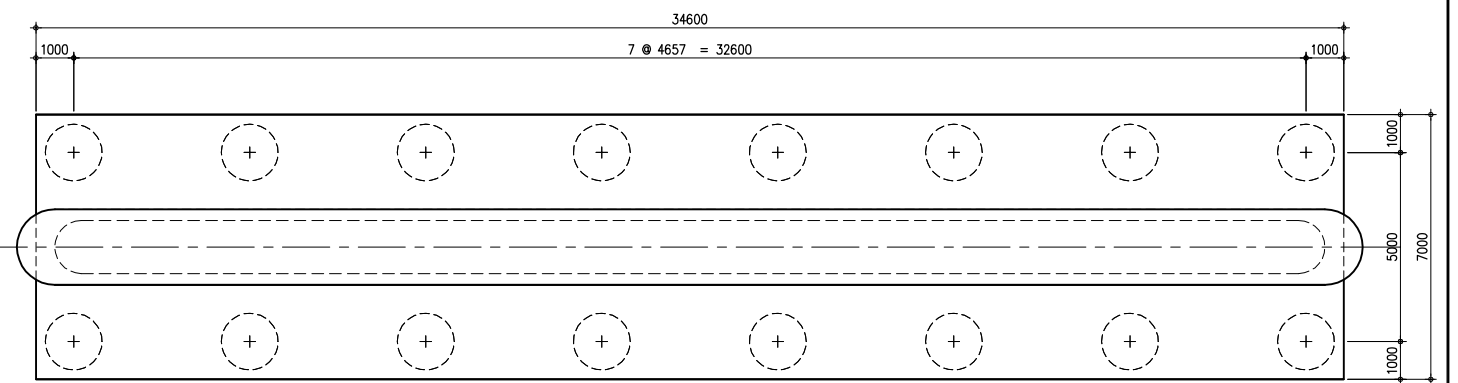
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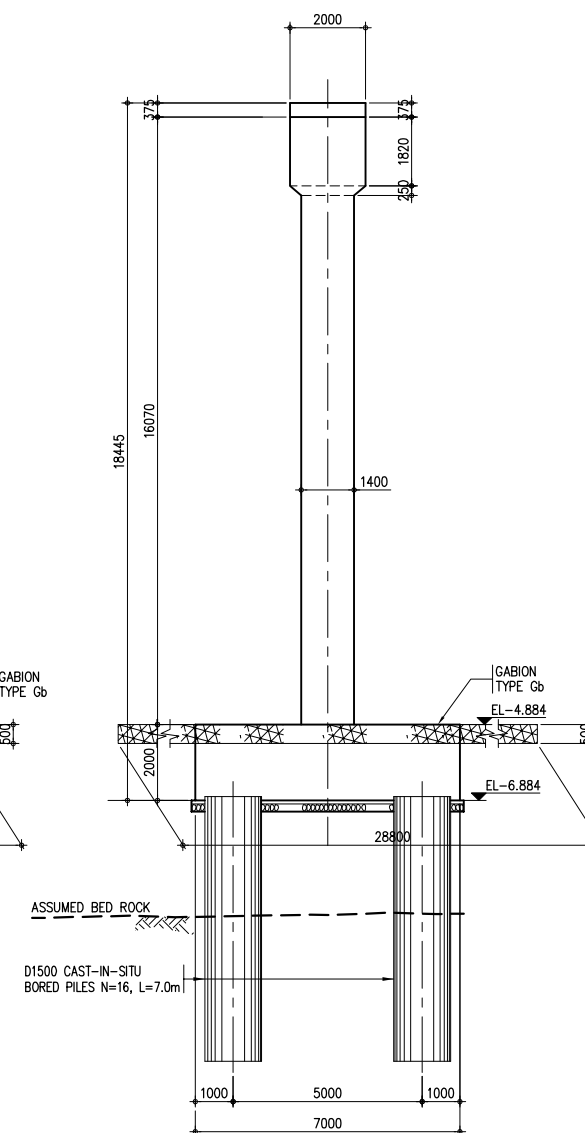
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**F** SECTION  
SCALE 1:50

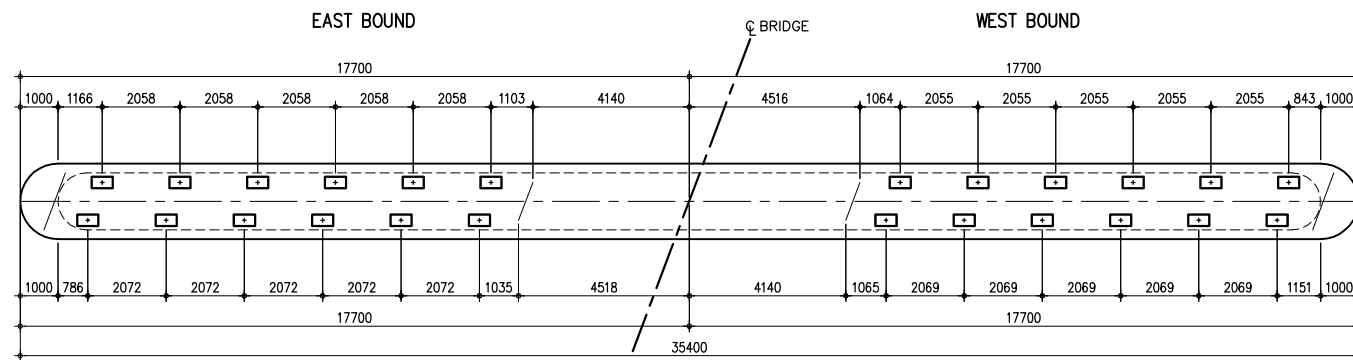


**D FOUNDATION PLAN**  
SCALE 1:100

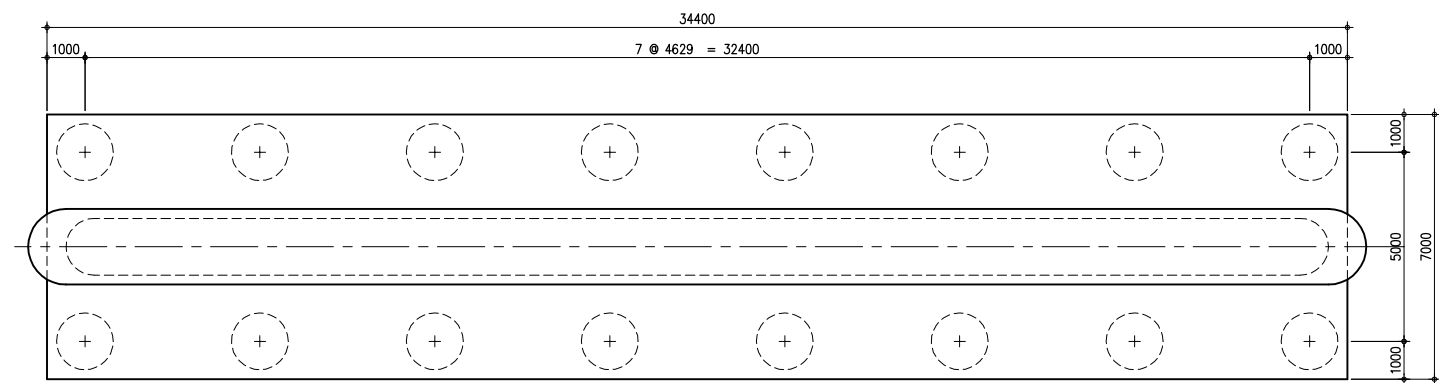


C SECTION @ ROADWAY C  
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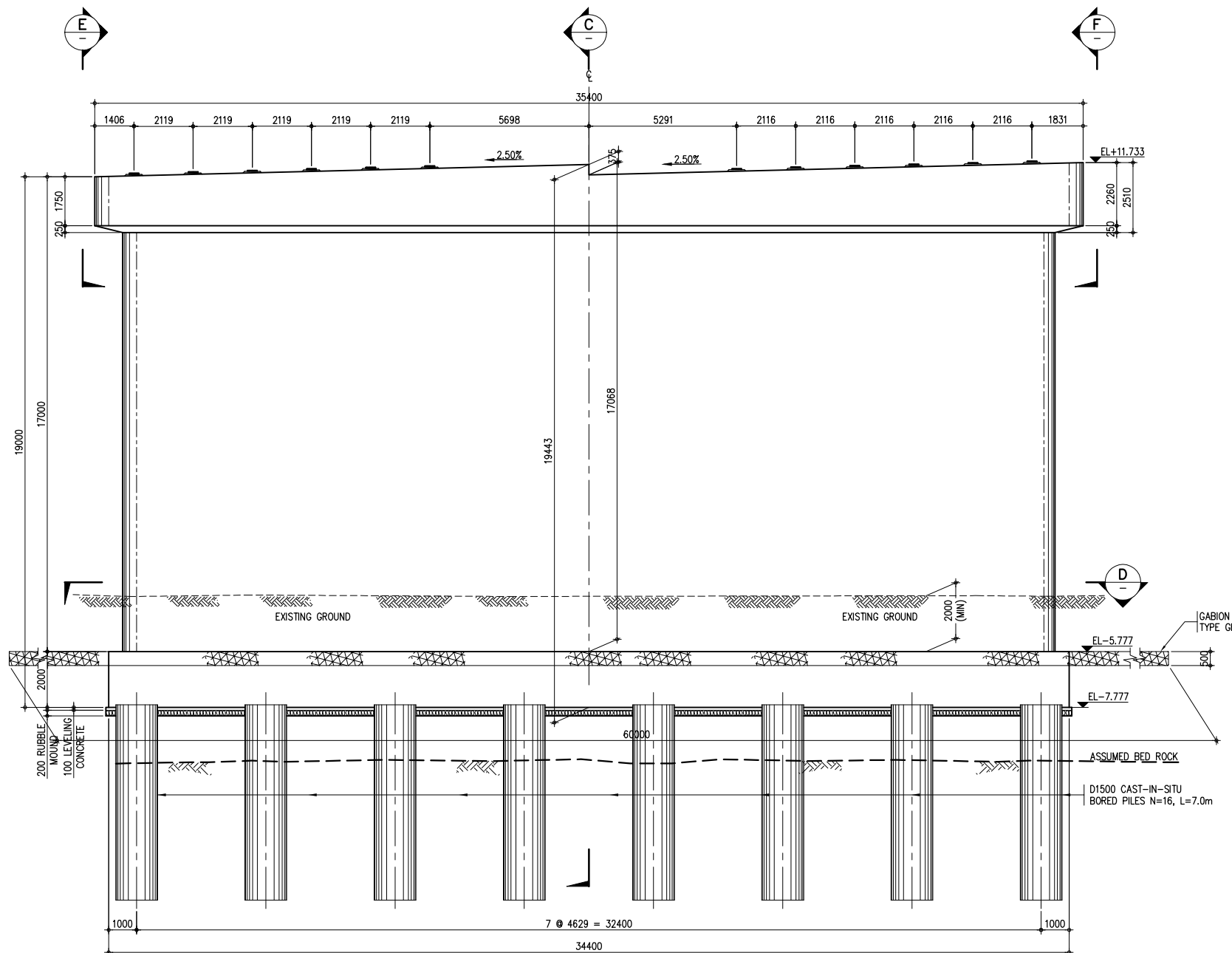




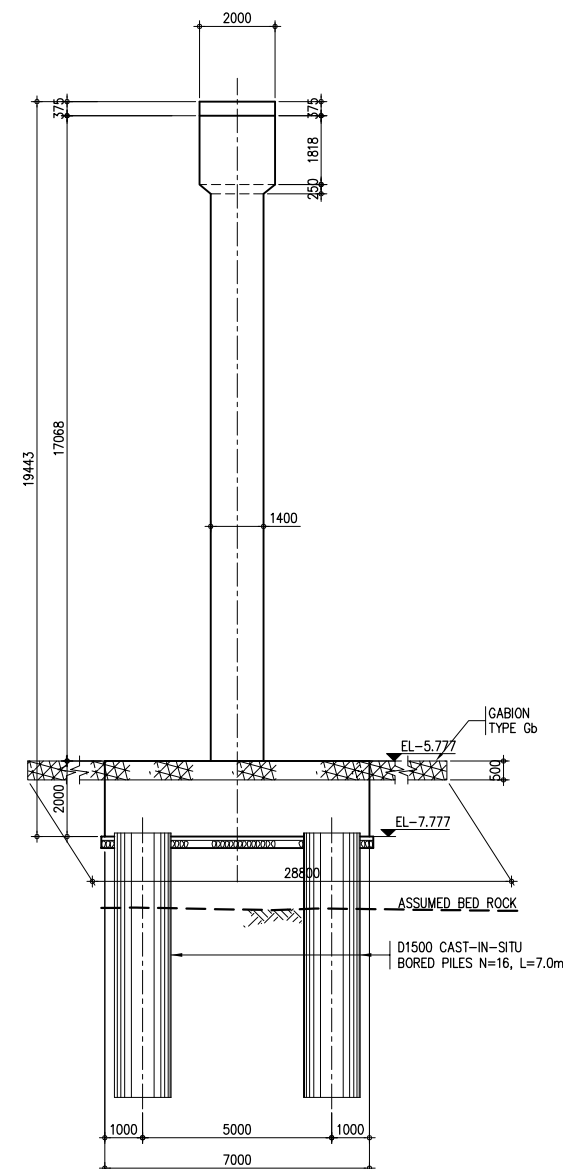
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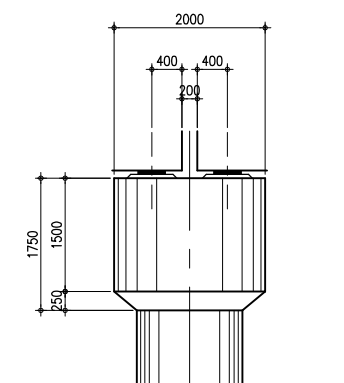
**D** FOUNDATION PLAN  
SCALE 1:100



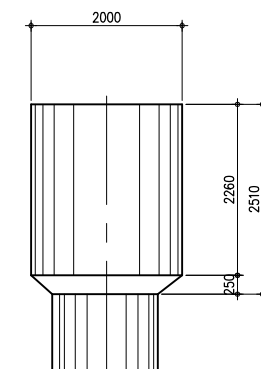
**B** FRONT ELEVATION  
SCALE 1:100



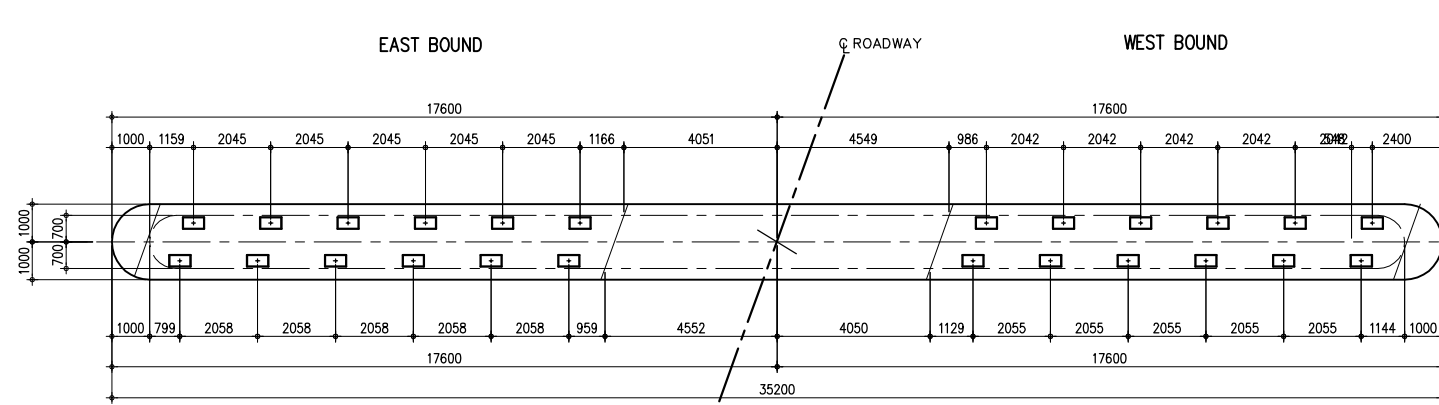
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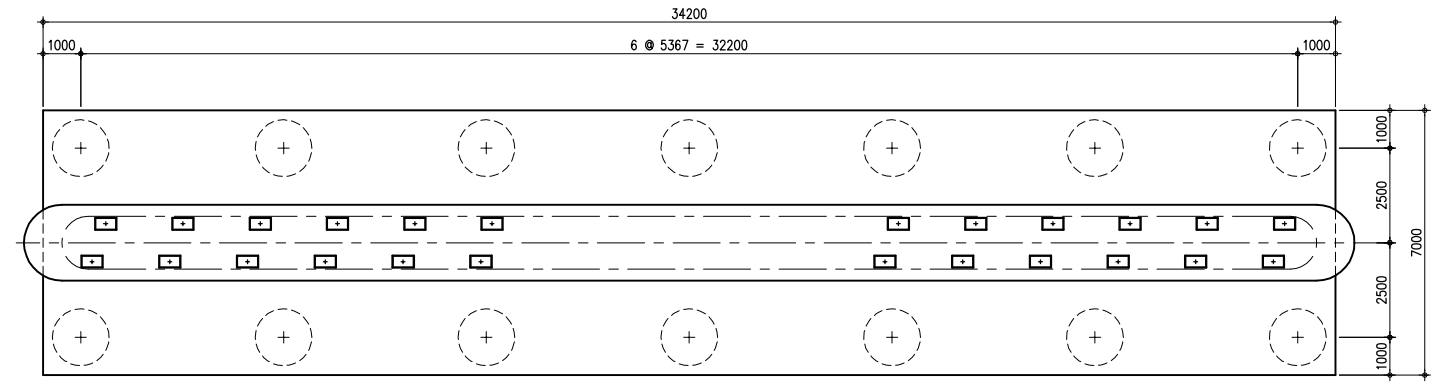
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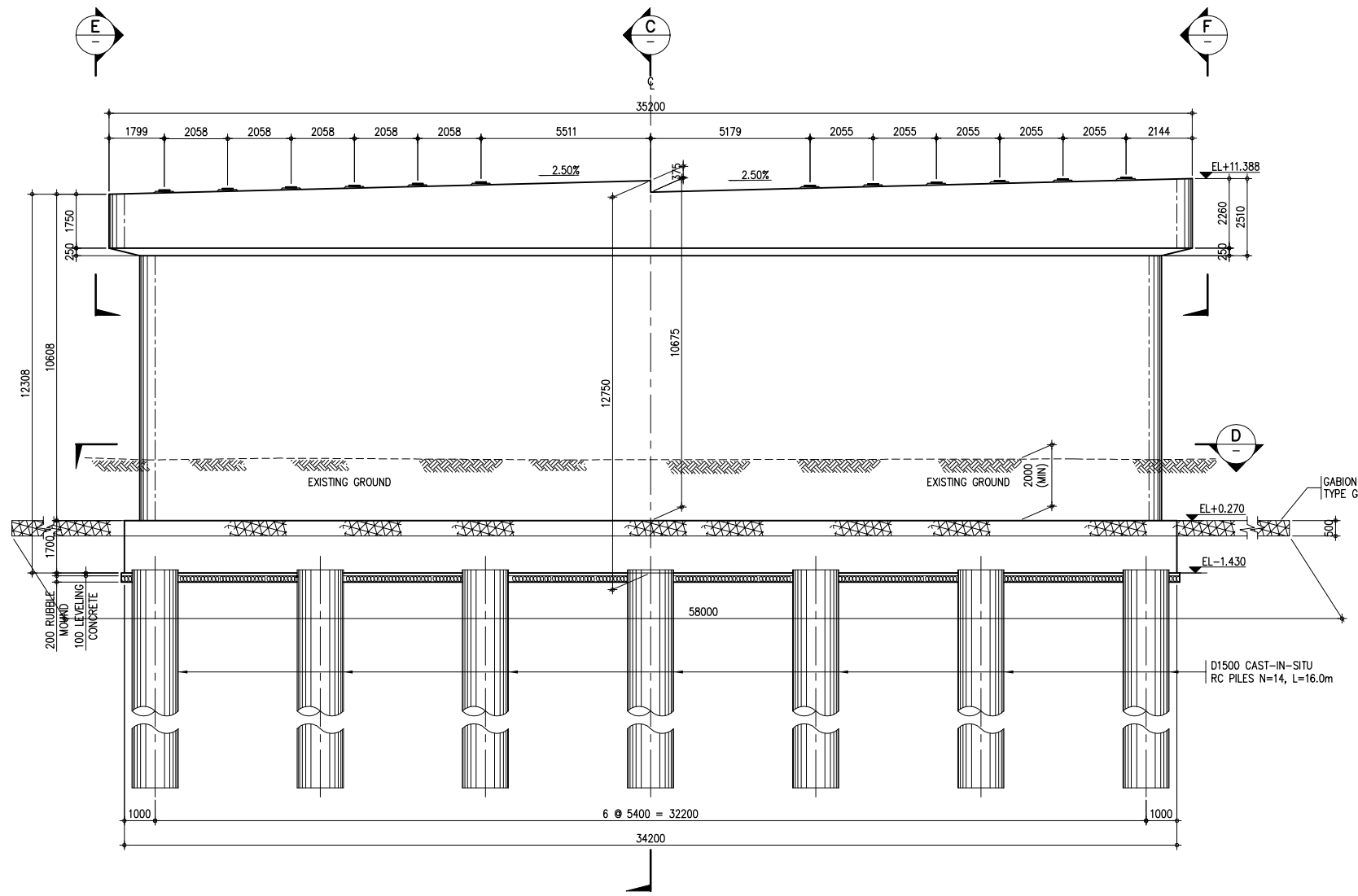
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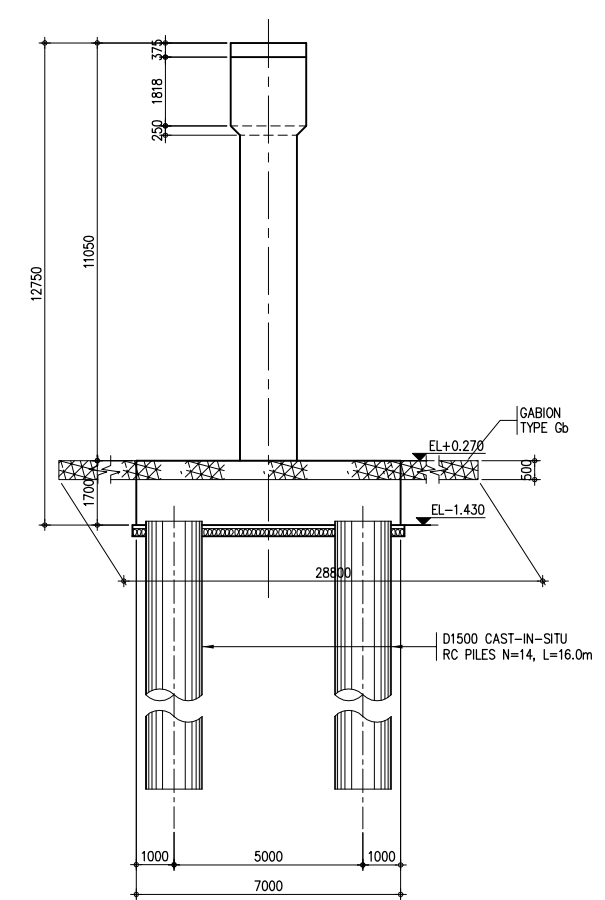
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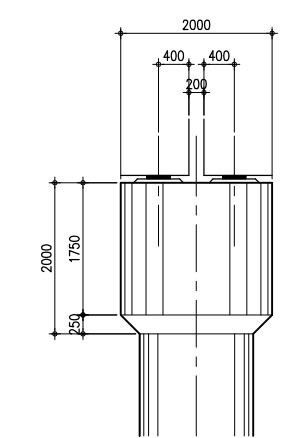
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SCALE 1:100



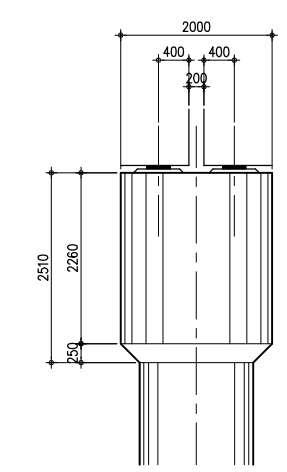
**B** FRONT ELEVATION  
SCALE 1:100



**C** SECTION @ ROADWAY C  
SCALE 1:100

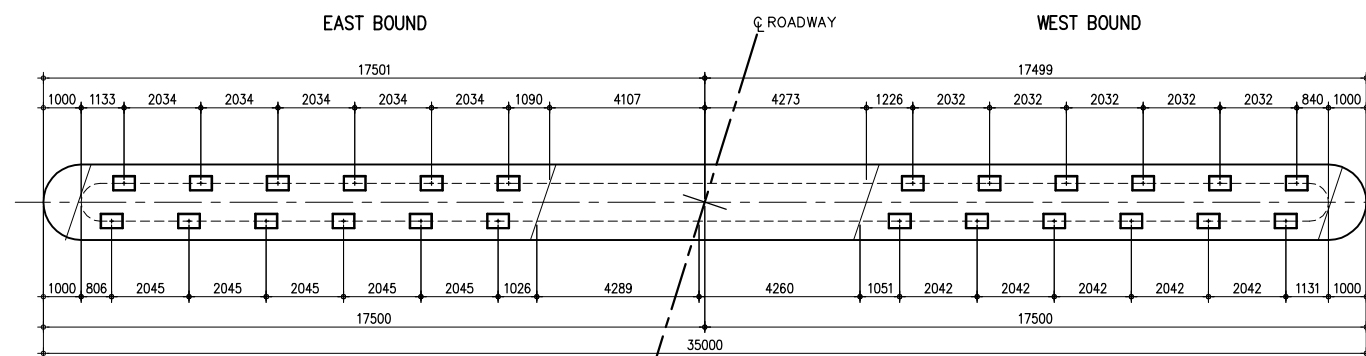


**E** SECTION  
SCALE 1:50

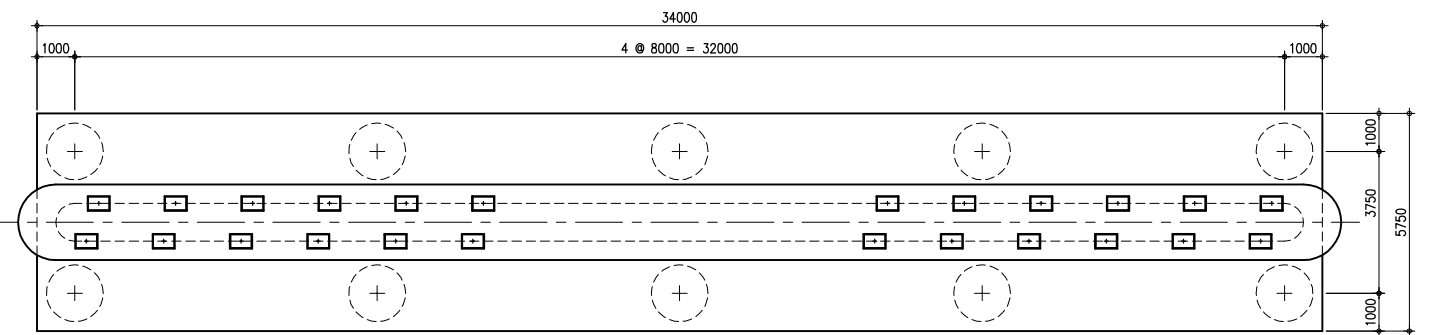


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SCALE 1:50

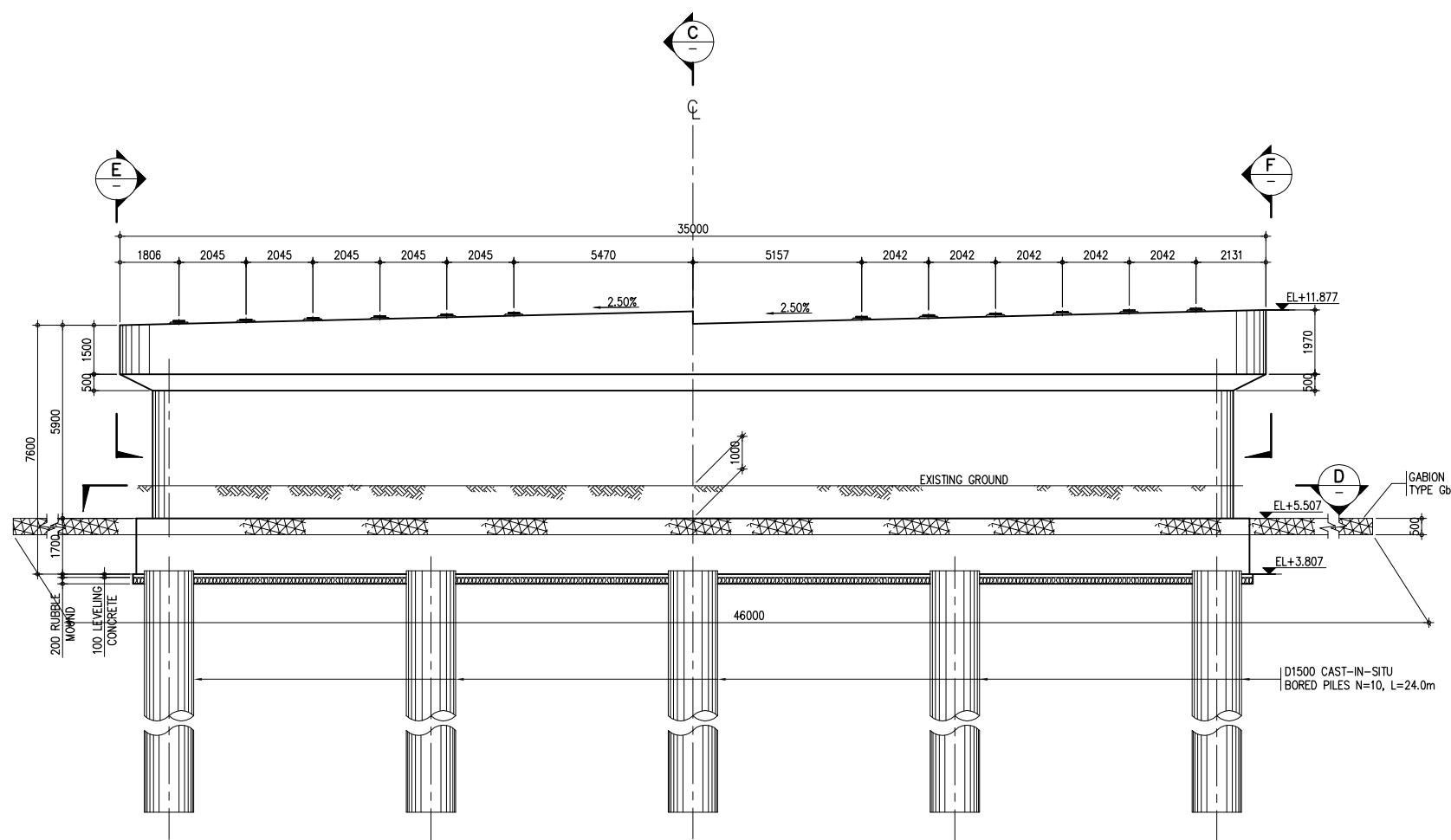
No	REVISION	DATE



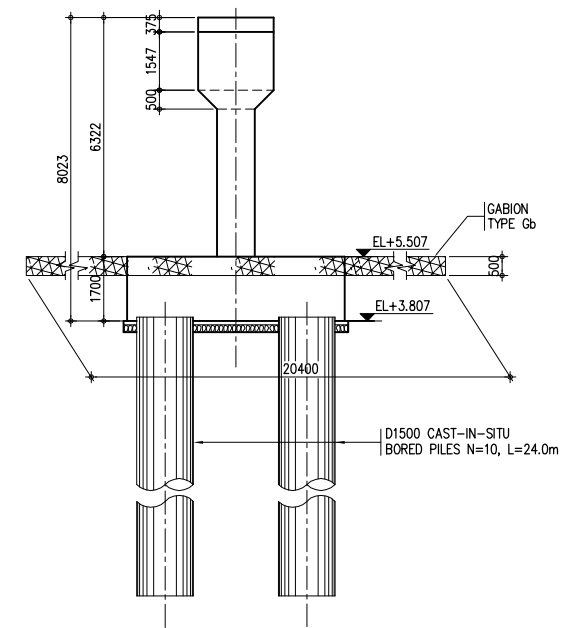
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SCALE 1:100



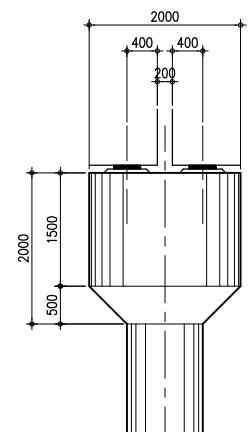
**D** FOUNDATION PLAN  
SCALE 1:100



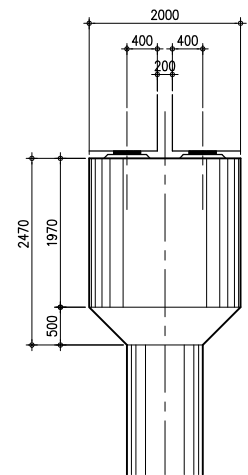
**B** FRONT ELEVATION  
SCALE 1:100



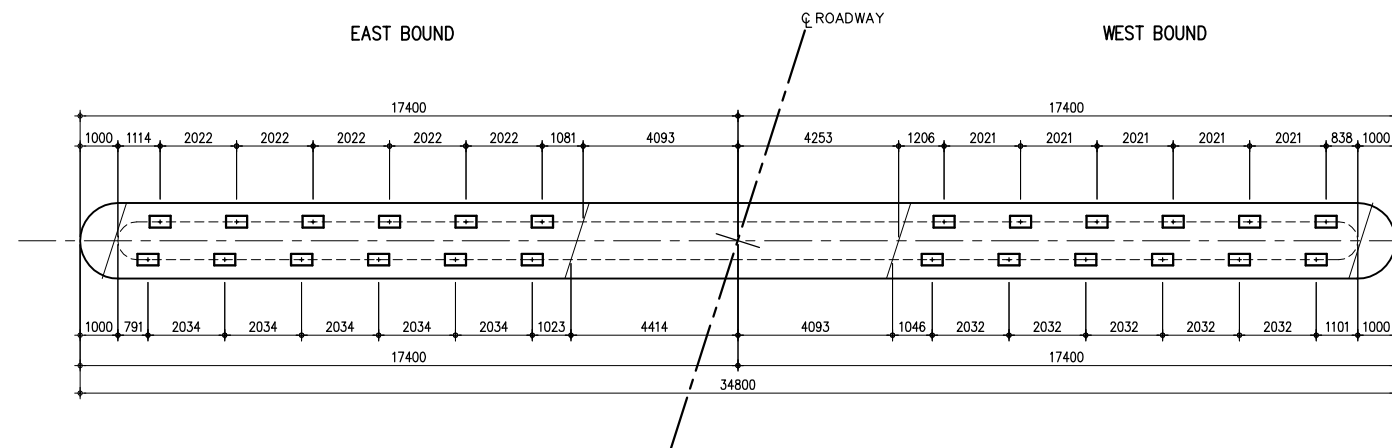
**C** SECTION @ ROADWAY  
SCALE 1:100



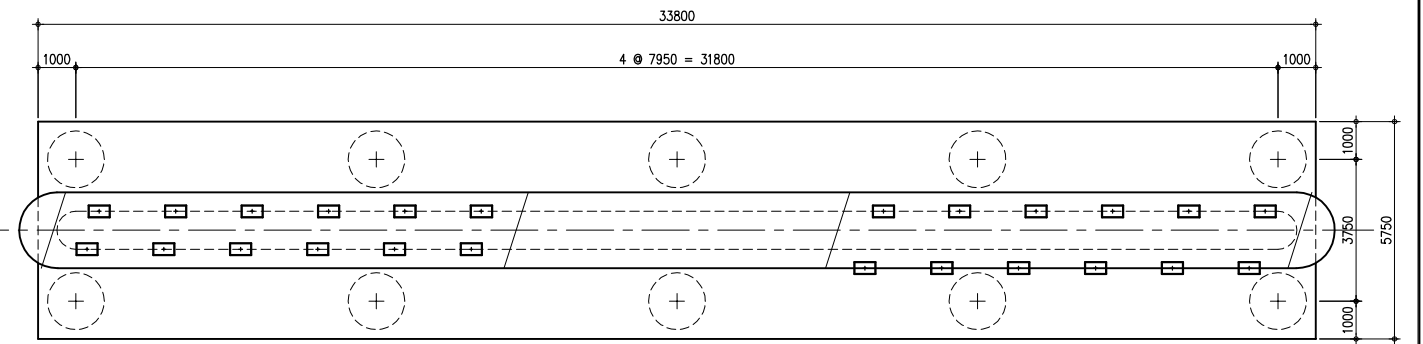
**E** SECTION  
SCALE 1:50



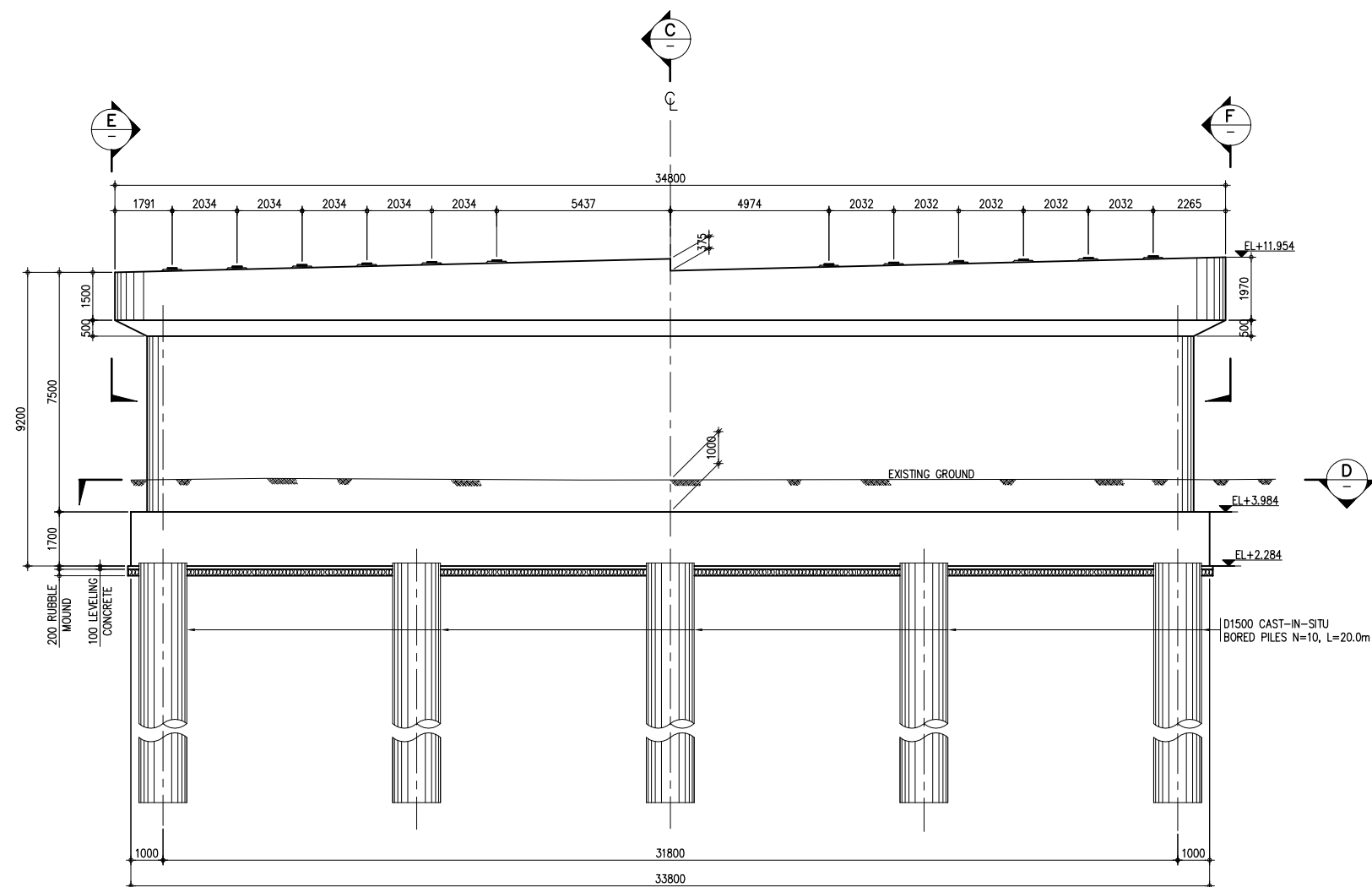
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SCALE 1:50



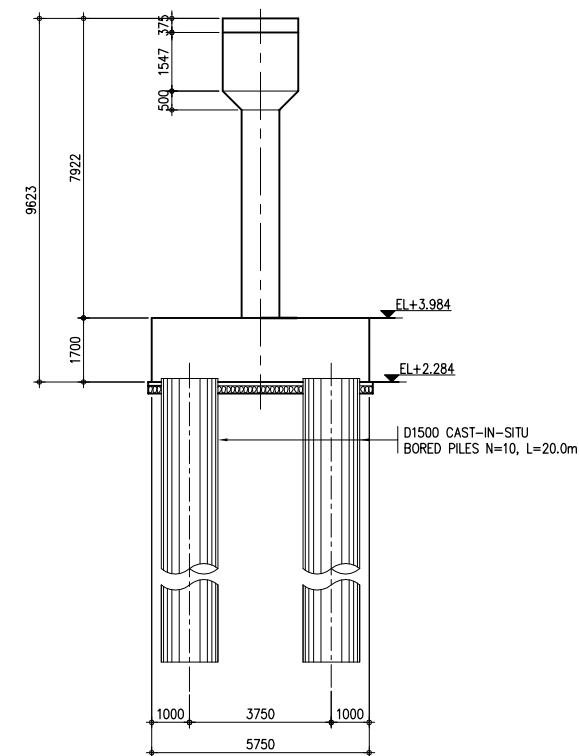
**A** COPING PLAN OF PIER P8  
SCALE 1:100



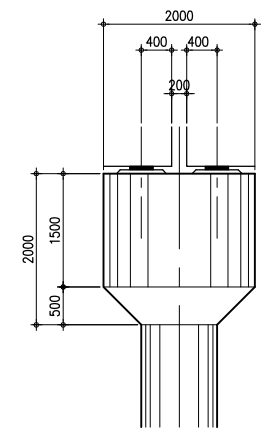
**D** FOUNDATION PLAN  
SCALE 1:100



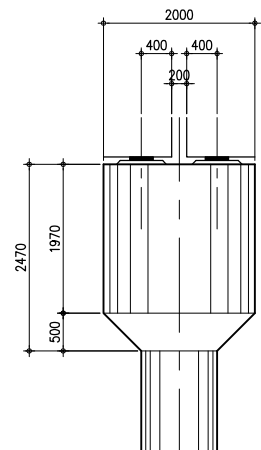
**B** FRONT ELEVATION  
SCALE 1:100



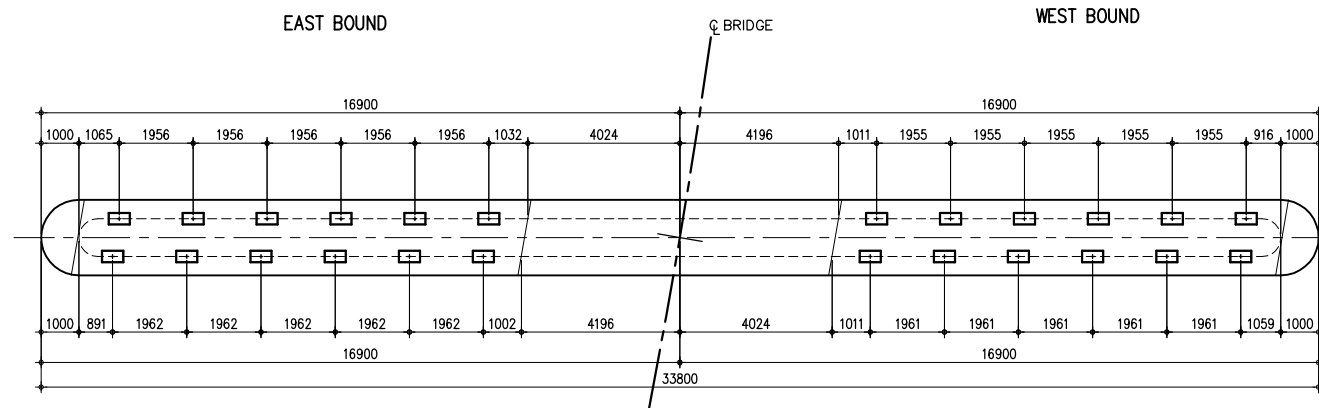
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SCALE 1:100



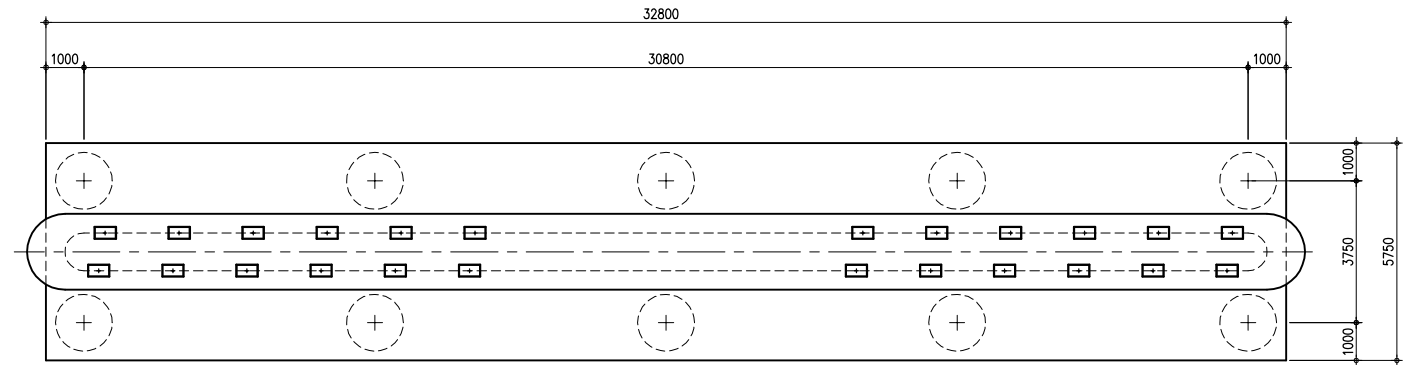
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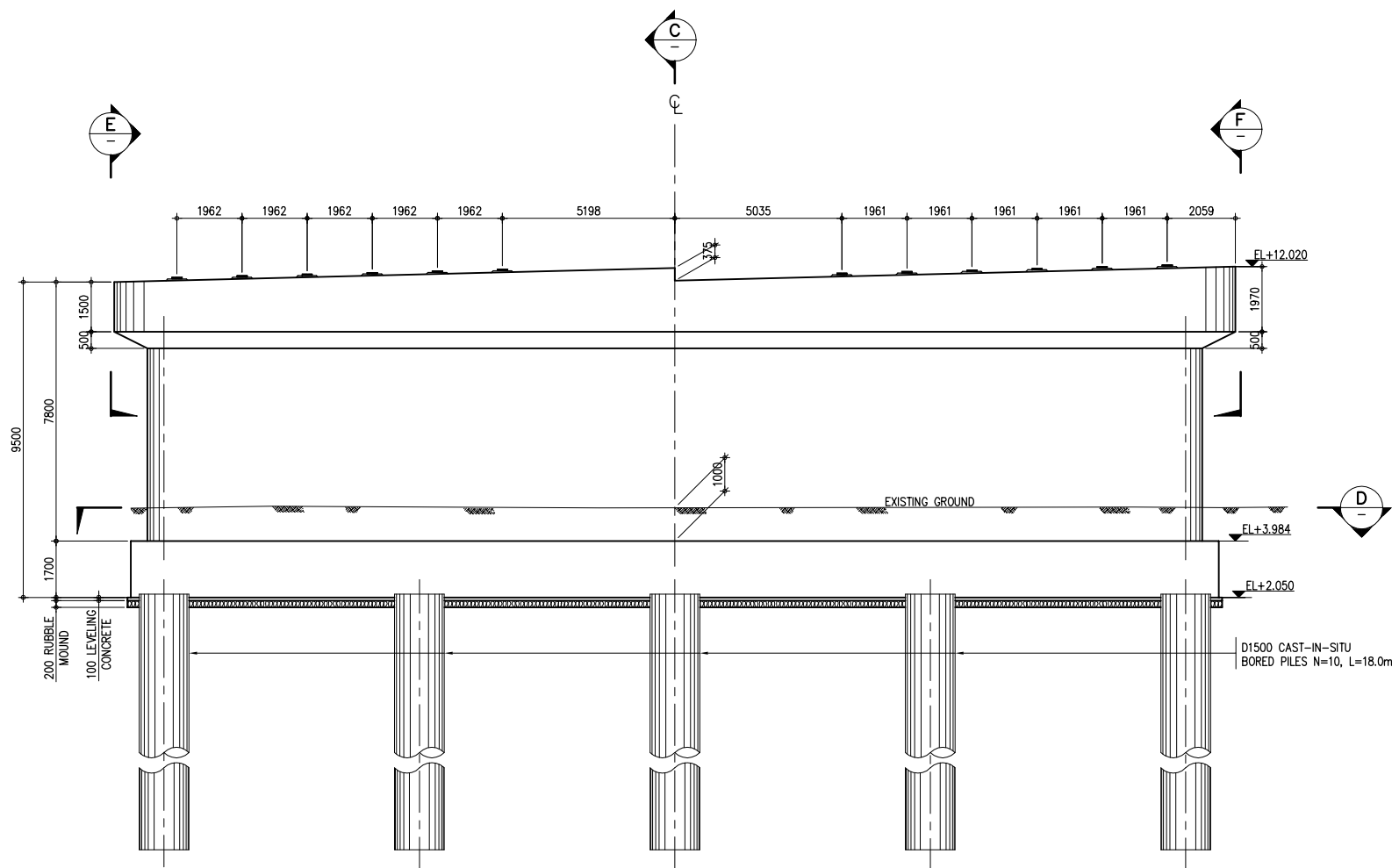
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SCALE 1:50



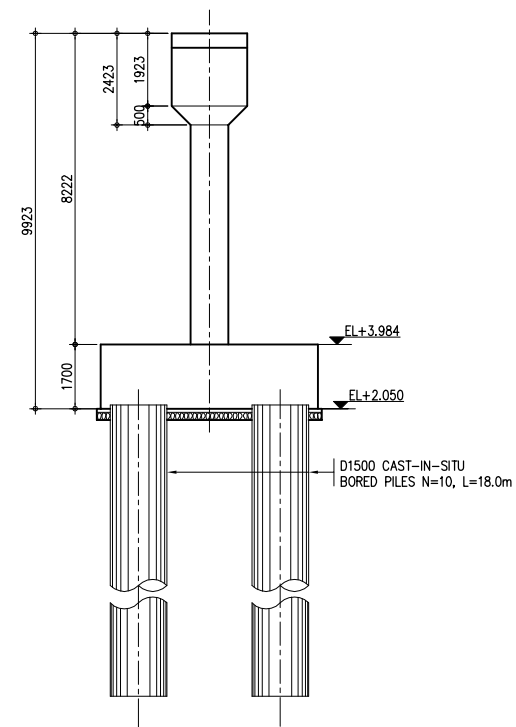
**A** COPING PLAN OF PIER P9  
SCALE 1:100



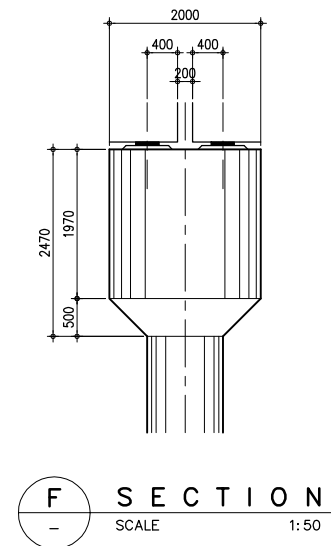
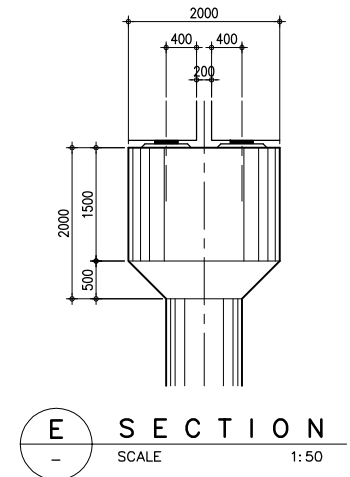
**D** FOUNDATION PLAN  
SCALE 1:100



**B** FRONT ELEVATION  
SCALE 1:100

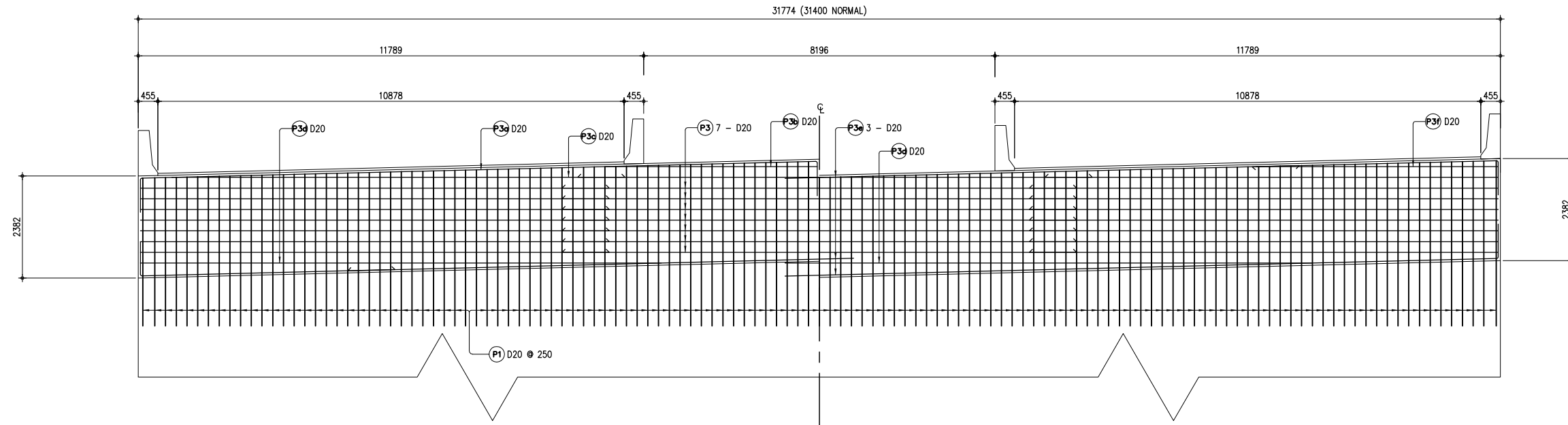


**C** SECTION @ ROADWAY CL  
SCALE 1:100

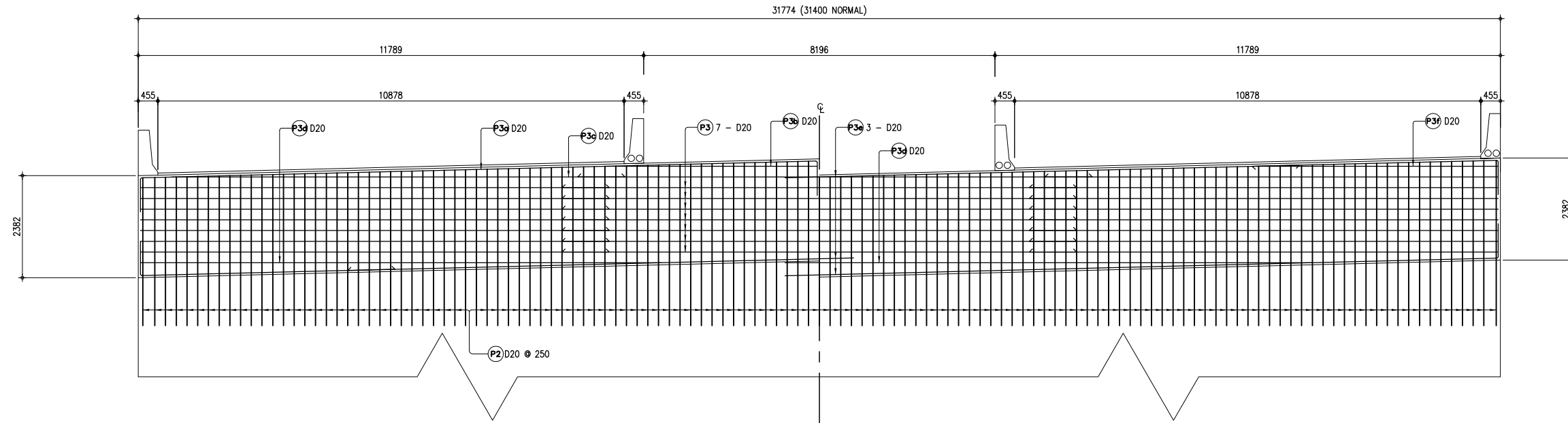




1 - 1



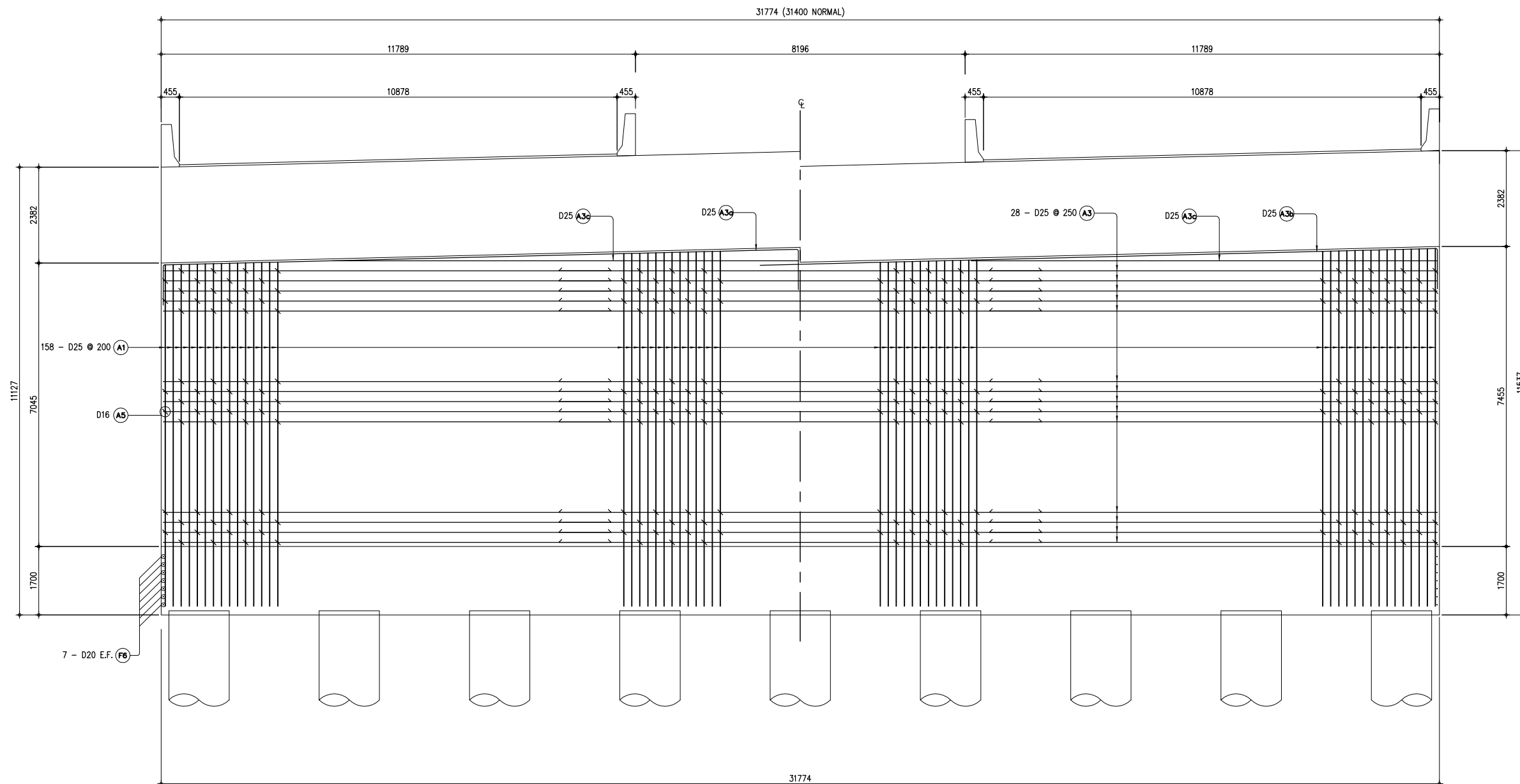
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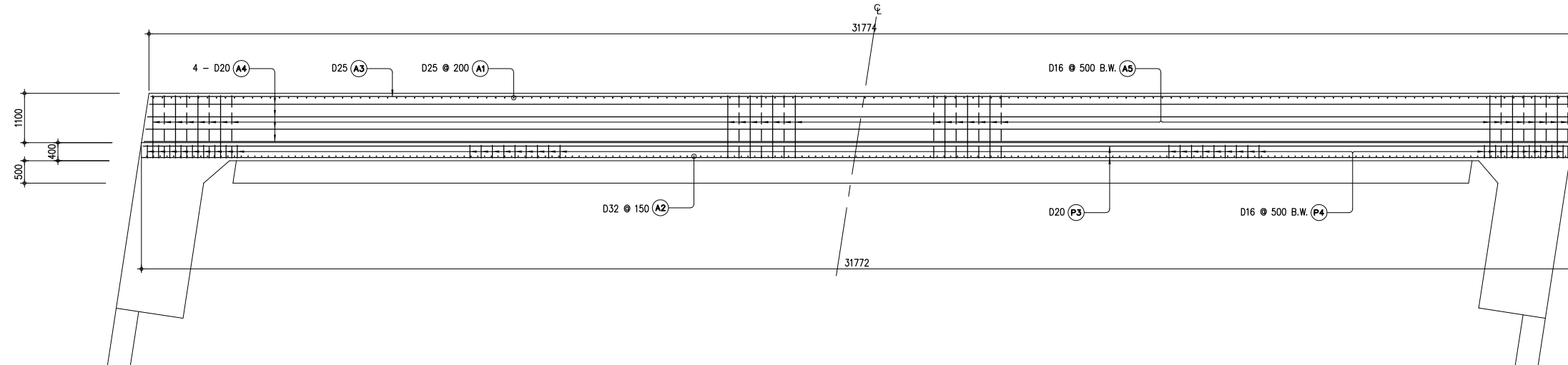
# HIGHWAY BRIDGE NO. 9 (H9) - KELANI RIVER CROSSING BRIDGE (SCALE 1:60)

## REINFORCEMENT OF A2 ABUTMENT (2)

3 - 3



6 - 6



THE DEMOCRATIC SOCIALIST REPUBLIC OF SRI LANKA  
MINISTRY OF HIGHWAYS & ROAD DEVELOPMENT



JAPAN INTERNATIONAL COOPERATION AGENCY



ORIENTAL CONSULTANTS COMPANY LIMITED  
in association with  
PACIFIC CONSULTANTS INTERNATIONAL



No

REVISION

DATE

COLOMBO OUTER CIRCULAR HIGHWAY PROJECT  
(NORTHERN SECTION 1)

HIGHWAY BRIDGE NO. 9 (H9) - KELANI RIVER CROSSING BRIDGE  
REINFORCEMENT BAR ARRANGEMENT OF A2 ABUTMENT (2/7)

DESIGNED BY:

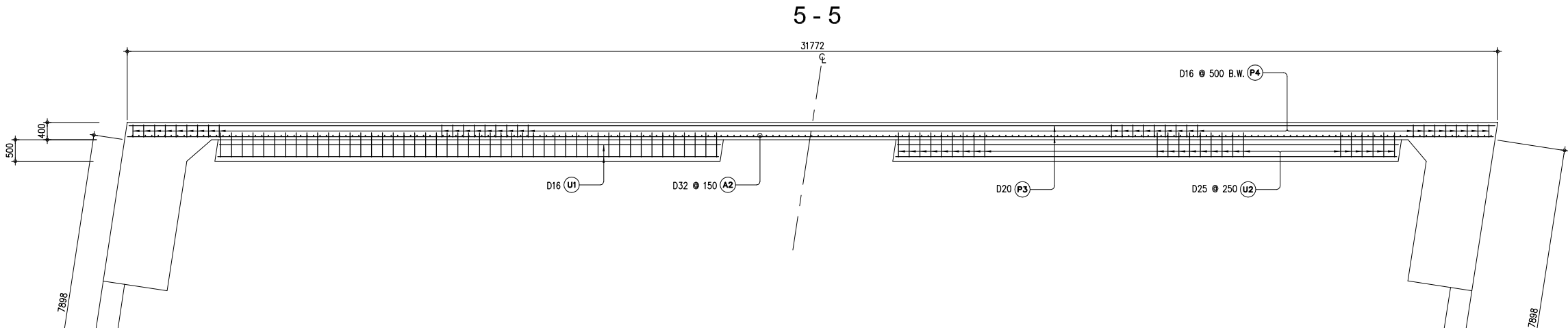
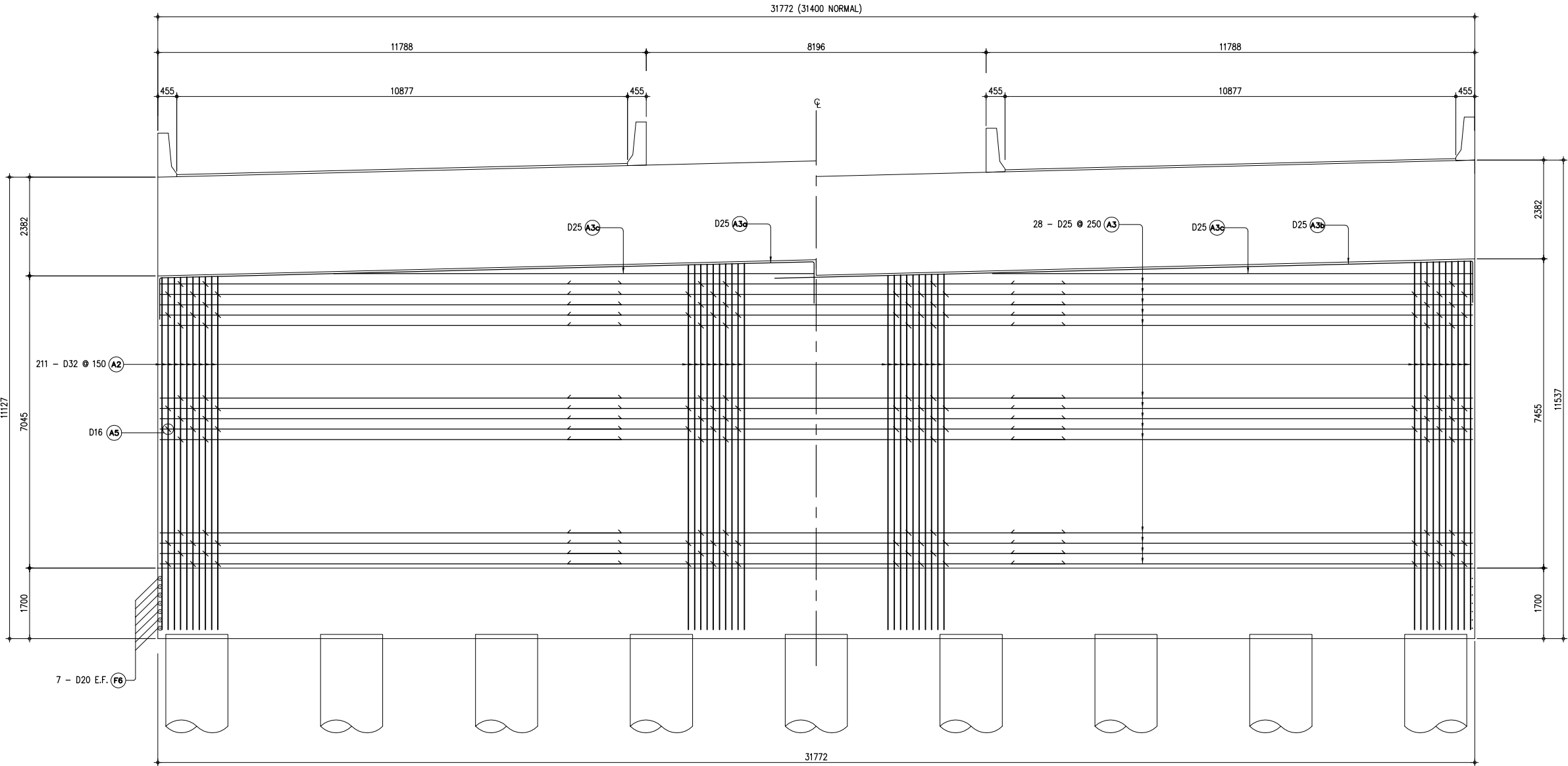
CHECKED BY:

APPROVED BY:

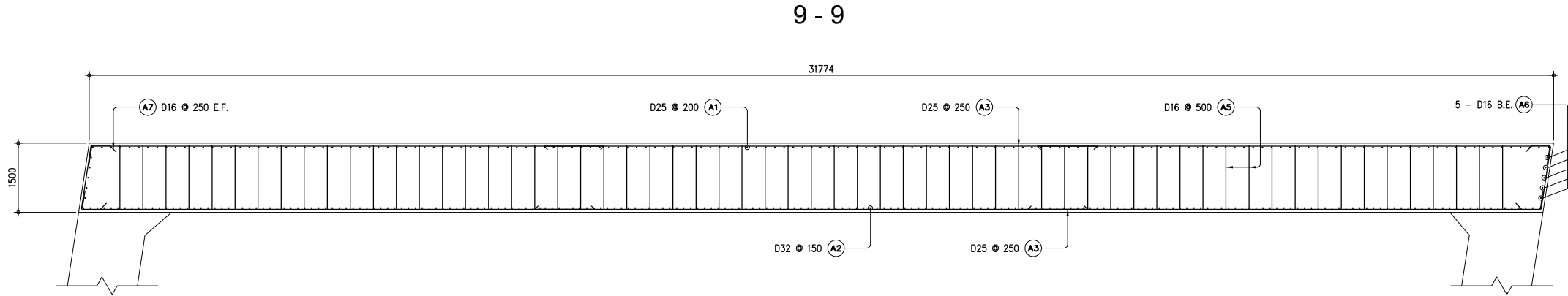
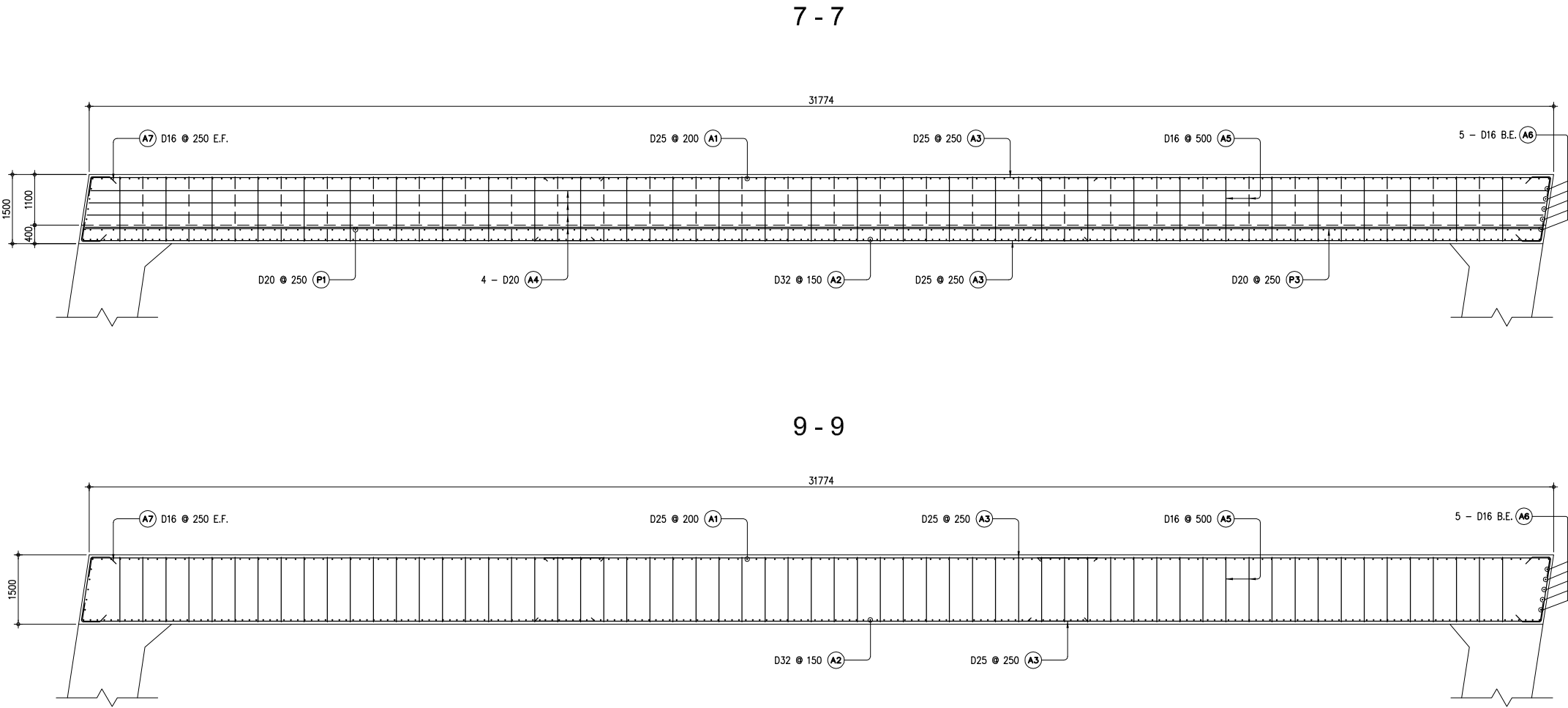
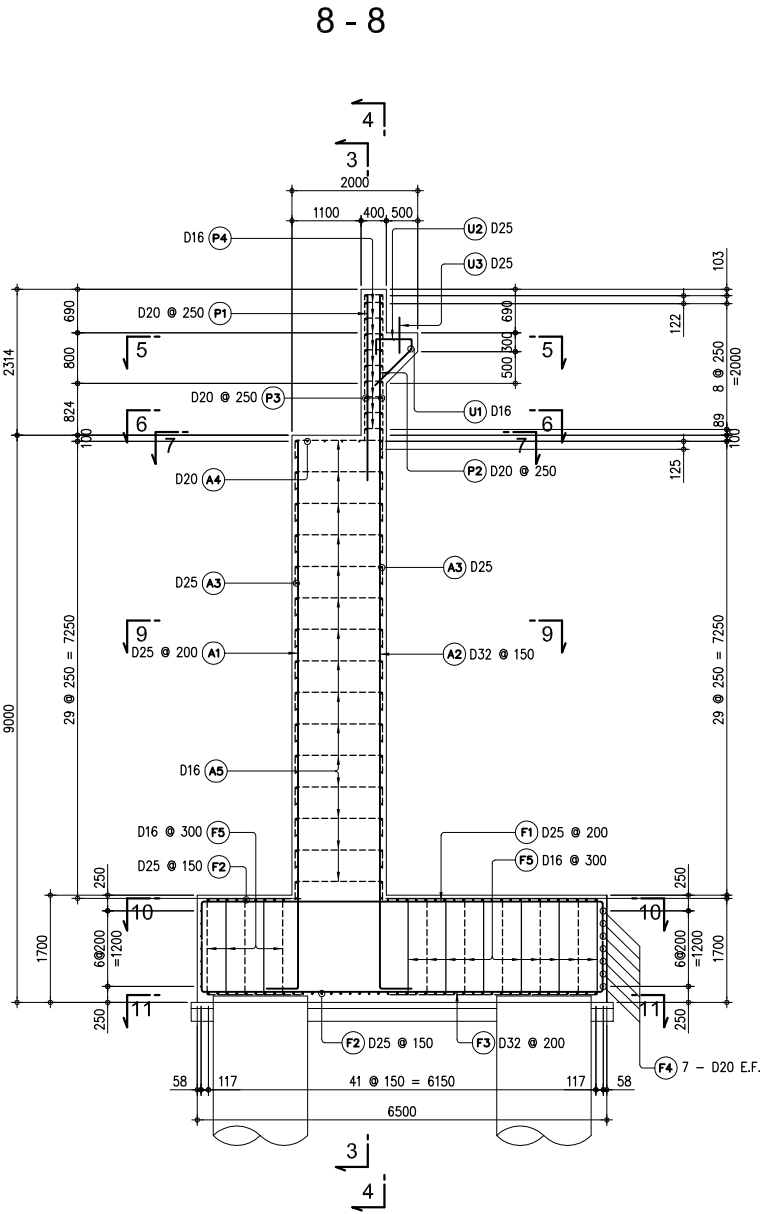
DWG. NO.

K01-57

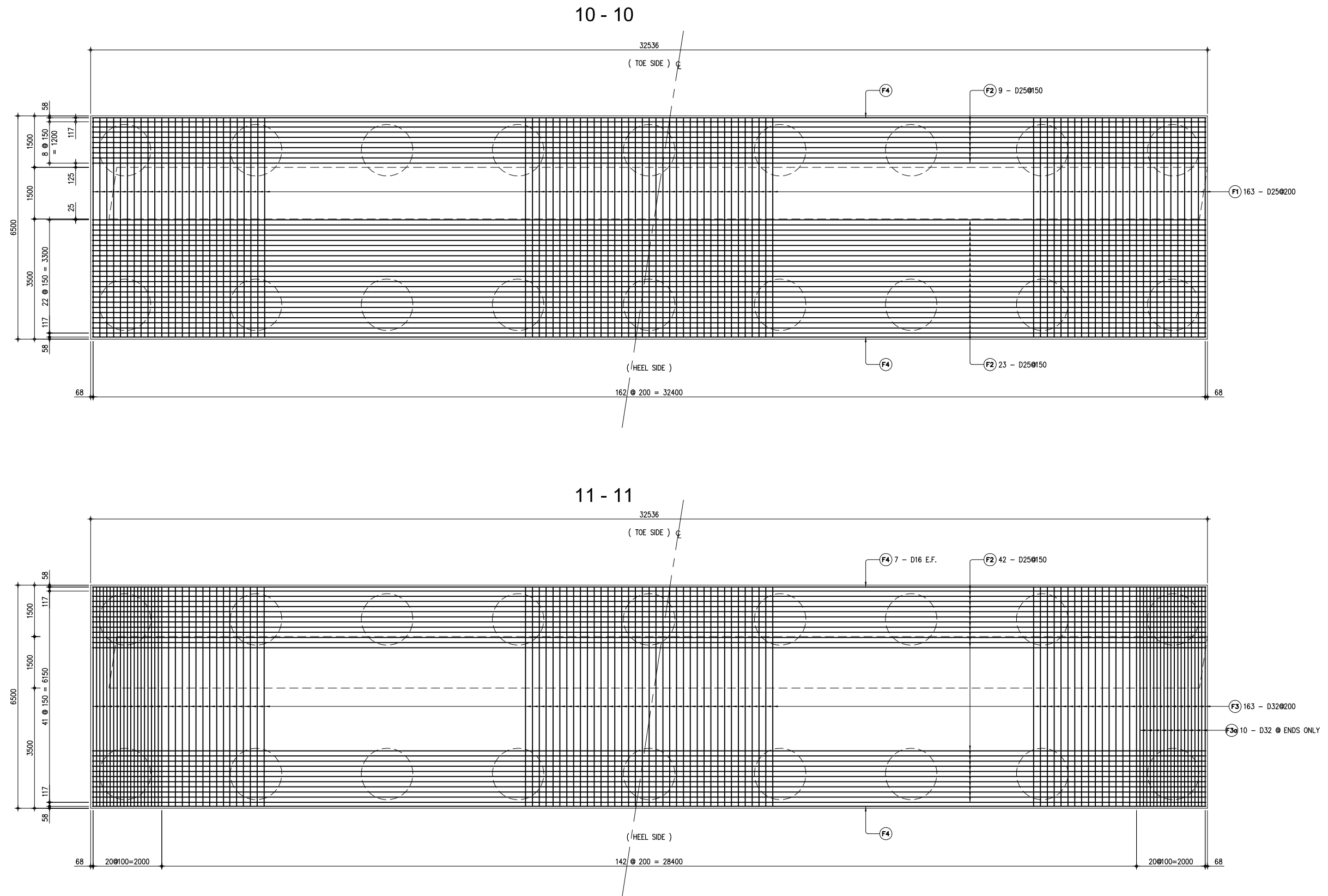
HIGHWAY BRIDGE NO. 9 (H9) - KELANI RIVER CROSSING BRIDGE (SCALE 1:60)
REINFORCEMENT OF A2 ABUTMENT (3)

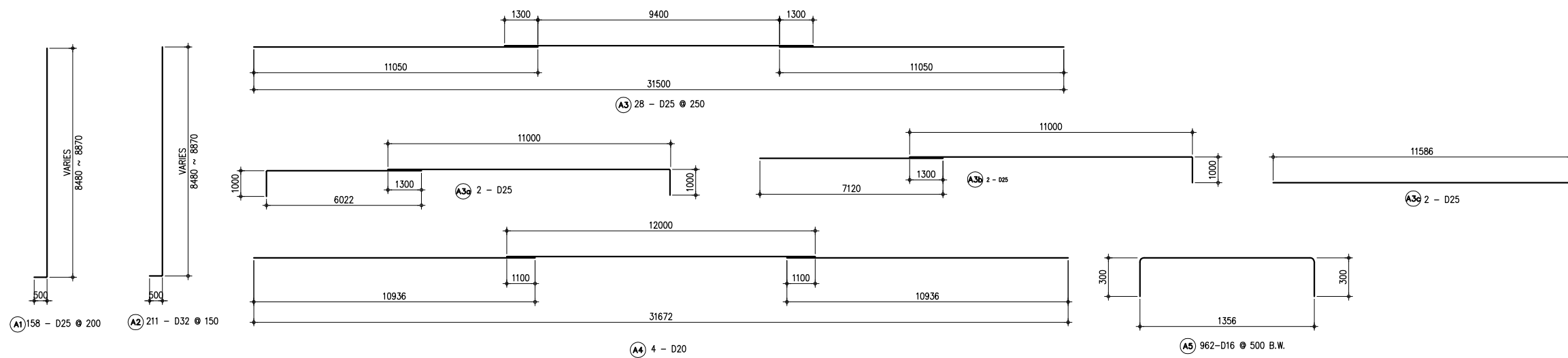
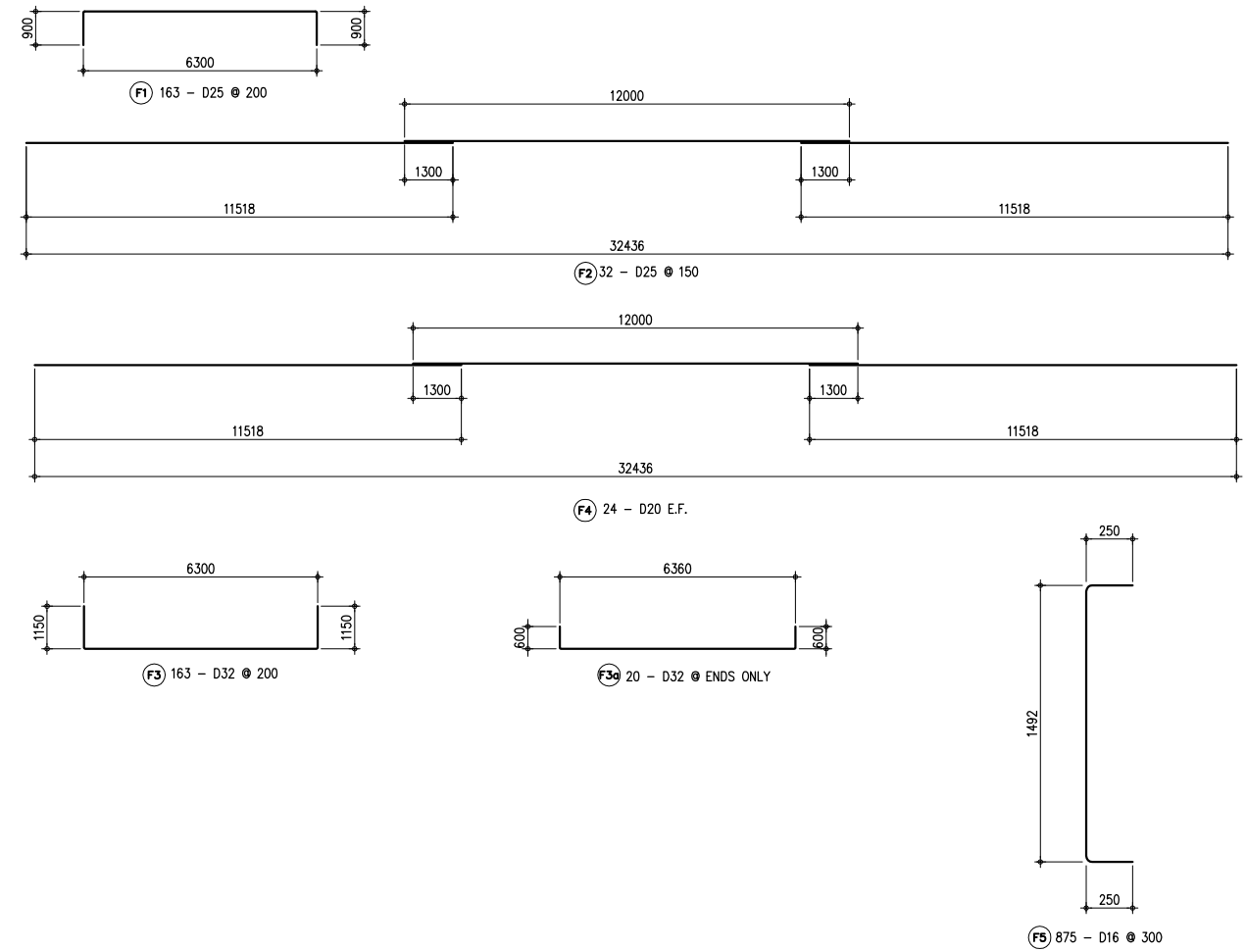
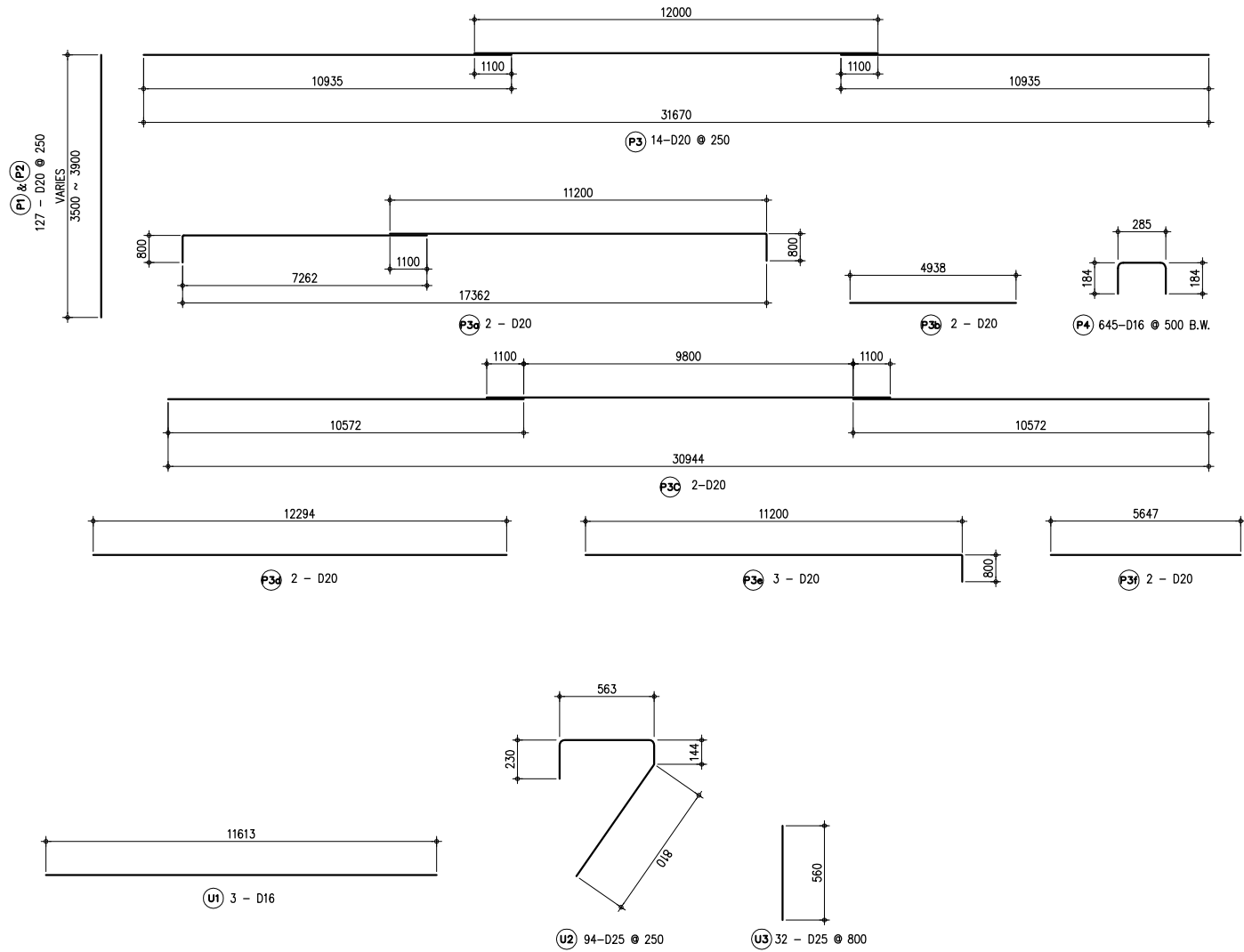


HIGHWAY BRIDGE NO. 9 (H9) - KELANI RIVER CROSSING BRIDGE (SCALE 1:60)  
REINFORCEMENT OF A2 ABUTMENT (4)



# **HIGHWAY BRIDGE NO. 9 (H9) - KELANI RIVER CROSSING BRIDGE (SCALE 1:60)** **REINFORCEMENT OF A2 ABUTMENT (5)**

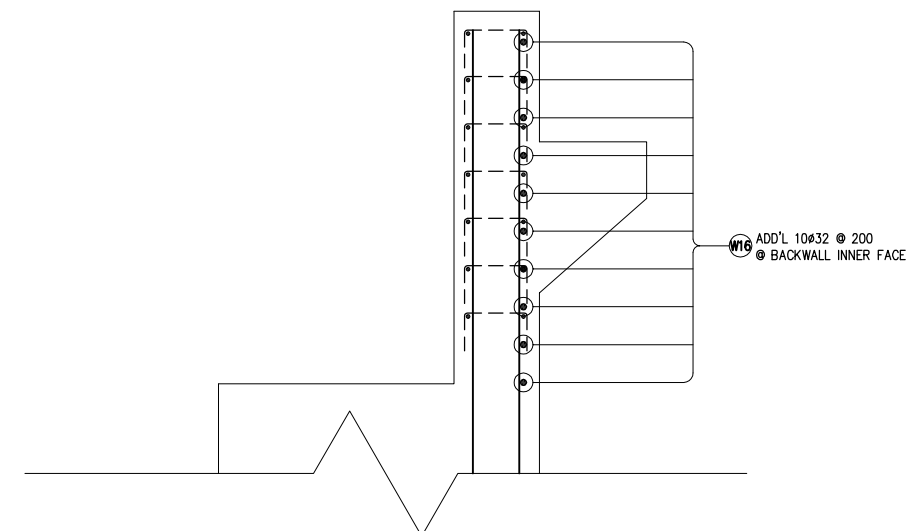
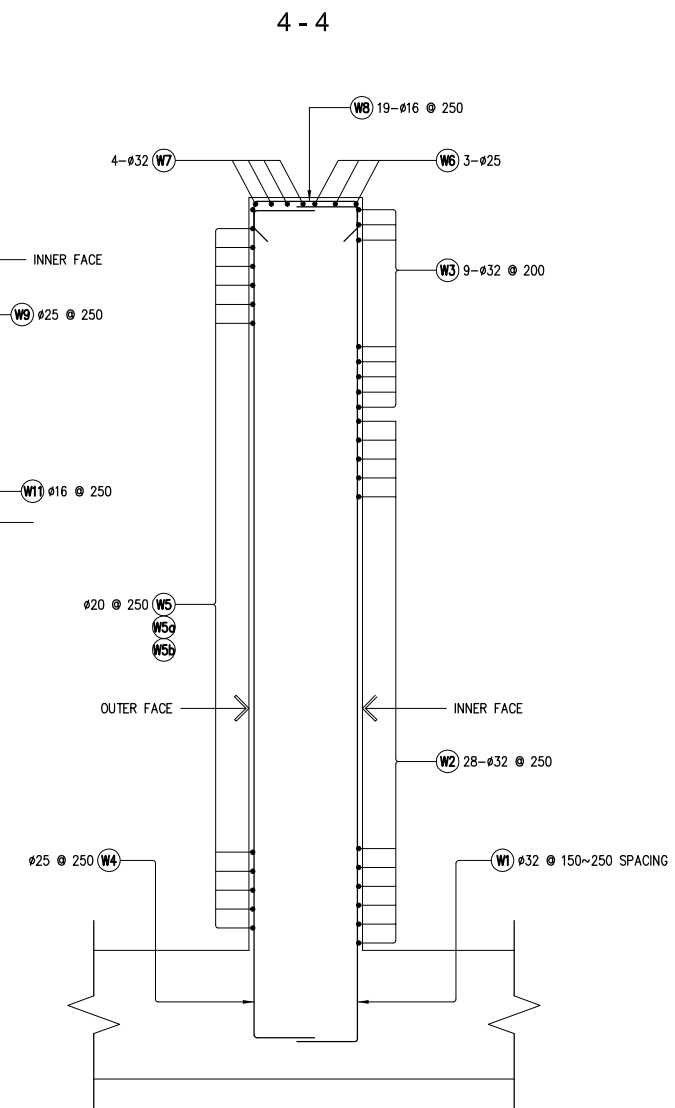
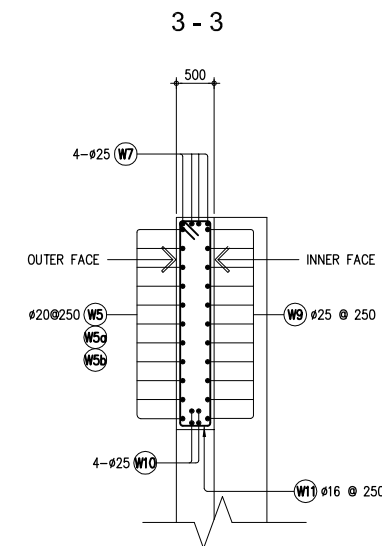
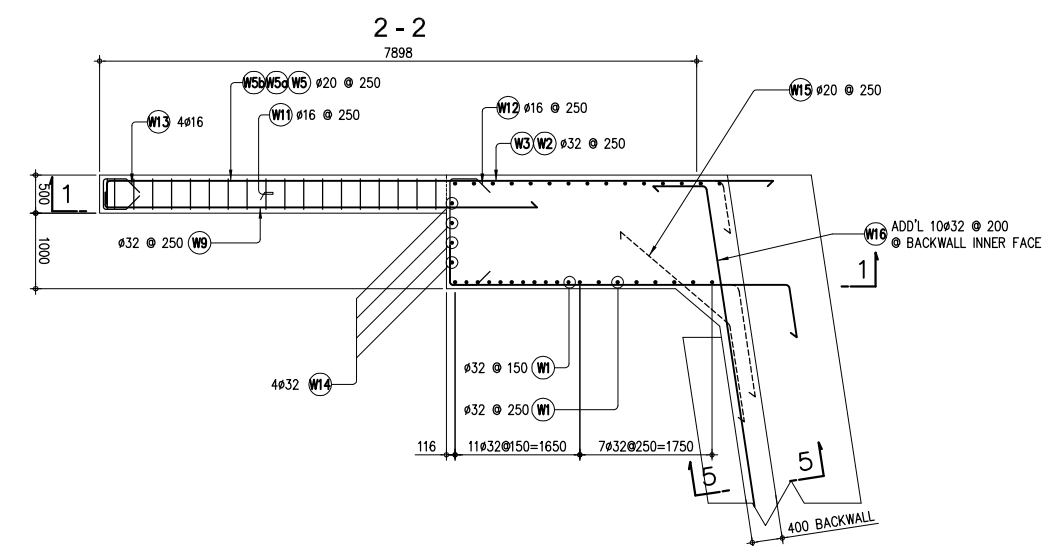
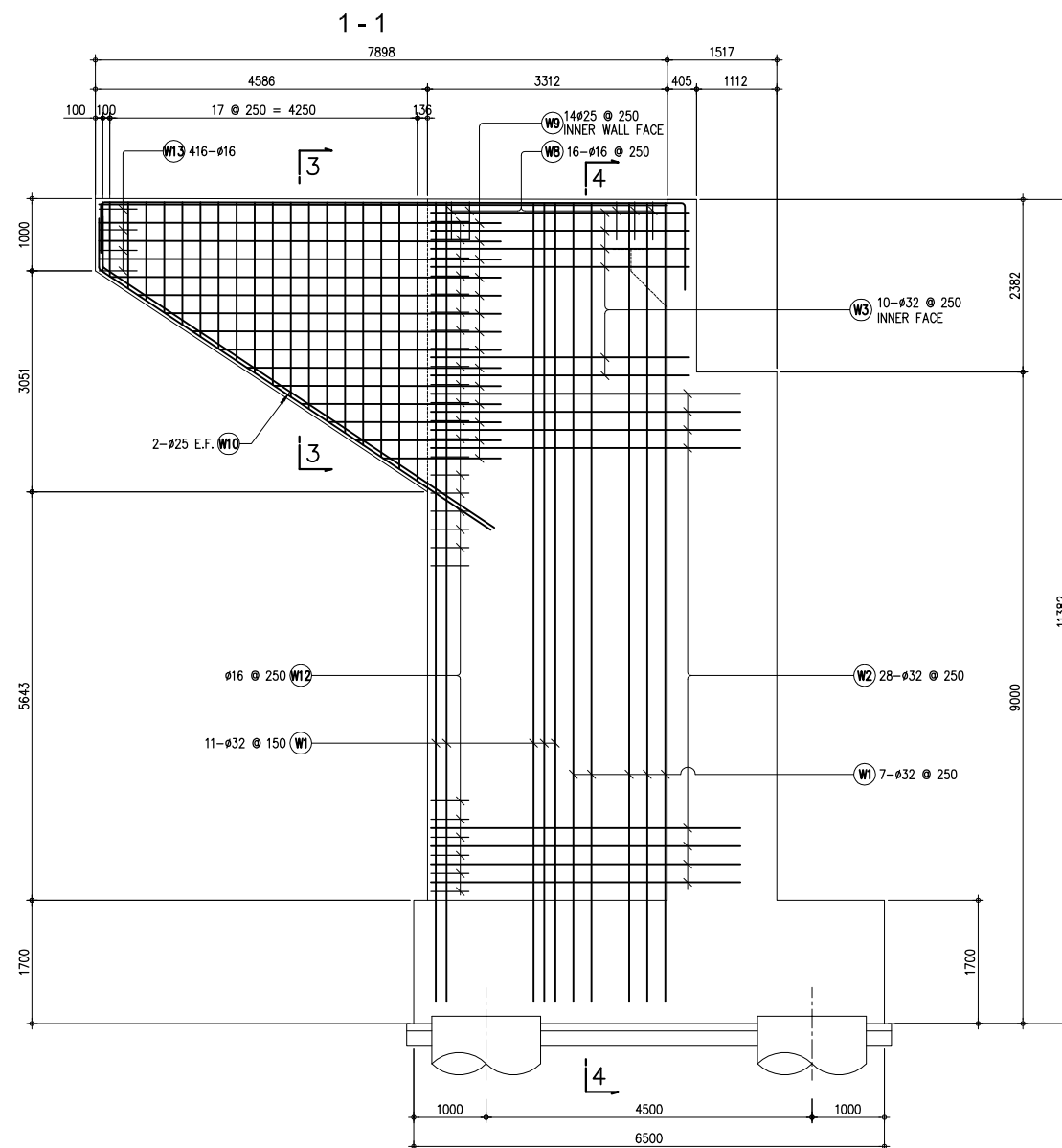




ESTIMATED QUANTITIES FOR ABUTMENT A2 (7)															
REINFORCING BARS															
LOCATION	BAR MARK	SPACING	QTY. (pcs)	SIZE (mm)	DIMENSION mm ( OUT TO OUT )						LENGTH PER BAR (mm)	TOTAL LENGTH (m)	UNIT WEIGHT (kg/m)	TOTAL WEIGHT (kg)	CONCRETE VOLUME (m³)
					a	b	c	d	e	f	g				
BACKWALL	P1		127	20	3700							3700	469.90	2.466	1158.77
	P2		127	20	3700							3700	469.90	2.466	1158.77
	P3		14	20	10935	12000	10935					33870	474.18	2.466	1169.33
	P3a		2	20	800	7262	11200	800				20062	40.12	2.466	98.95
	P3b		2	20	4938							4938	9.88	2.466	24.35
	P3c		2	20	10572	12000	10572					33144	66.29	2.466	163.47
	P3d		2	20	12294							12294	24.59	2.466	60.63
	P3e		3	20	11200	800						12000	36.00	2.466	88.78
	P3f		2	20	5647							5647	11.29	2.466	27.85
	P4		645	16	184	285	184					653	421.19	1.579	665.05
	U1		3	16	11613							11613	34.84	1.579	55.01
	U2		94	25	230	563	144	810				1747	164.22	3.854	632.90
	U3		32	25	560							560	17.92	3.854	69.06
	SUB-TOTAL QUANTITY FOR PARAPET														5,372.92
STEM	A1		158	25	500	8675						9175	1449.65	3.854	5586.95
	A2		211	32	500	8675						9175	1935.93	6.313	12221.49
	A3		56	25	11050	12000	11050					34100	1909.60	3.854	7359.60
	A3a		2	25	1000	6022	11000	1000				19022	38.04	3.854	146.62
	A3b		2	25	7120	11000	1000					19120	38.24	3.854	147.38
	A3c		2	25	11586							11586	23.17	3.854	89.30
	A4		4	20	10936	12000	10936					33872	135.49	2.466	334.11
	A5		962	16	274	1356	274					1904	1831.65	1.579	2892.17
	SUB-TOTAL QUANTITY FOR PARAPET														28,777.63
FOOTING	F1		163	25	600	6300	600					7500	1222.50	3.854	4711.52
	F2		32	25	11518	12000	11518					35036	1121.15	3.854	4320.92
	F3		163	32	600	6300	600					7500	1222.50	6.313	7717.64
	F3a		20	32	600	6360	600					7560	151.20	6.313	954.53
	F4		24	20	11518	12000	11518					35036	840.86	2.466	2073.57
	F5		875	16	250	1492	250					1992	1743.00	1.579	2752.20
	SUB-TOTAL QUANTITY FOR FOOTING														22,530.37
WINGWALL (X2)	W1		7	32	500	11500	3790	500				16290	114.03	6.313	719.87
	W1a		11	32	500	9500	5790	500				16290	179.19	6.313	1131.23
	W2		28	32	500	5200	500					6200	173.60	6.313	1095.94
	W3		10	32	500	4050	1500					6050	60.50	6.313	381.94
	W4		15	25	400	11600	2890	400				15290	229.35	3.854	883.91
	W4a		15	25	400	9600	4890	400				15290	229.35	3.854	883.91
	W5		4	20	300	8103	700					9103	36.41	2.466	89.79
	W5a		11	20	300	6560	1000					7860	86.46	2.466	213.21
	W5b		23	20	300	4400	1000					5700	131.10	2.466	323.29
	W6		3	25	400	8103	700					9203	27.61	3.854	106.41
	W7		4	32	500	8103	1200					9803	39.21	6.313	247.55
	W8		19	16	250	350	1400	350	250			2600	49.40	1.579	78.00
	W9		14	32	500	9500	4300	2000				16300	228.20	6.313	1440.63
	W10		4	25	6000	400						6400	25.60	3.854	98.66
	W11		17	16	700	2266	700	2266	100	100		6132	104.24	1.579	164.60
	W12		38	16	250	350	1400	350	250			2600	98.80	1.579	156.01
	W13		4	16	250	350	700	350	250			1900	7.60	1.579	12.00
	W14		4	32	500	11500	3790	500				16290	65.16	6.313	411.36
	W15		78	20	1300	1700						3000	234.00	2.466	577.04
			10	32	500	5300						5800	58.00	6.313	366.15
	SUB-TOTAL QUANTITY WINGWALL														9,381.50
	NUMBER OF WINGWALL														2.00
	TOTAL QUANTITY OF WINGWALLS														18,763.00
	GRAND TOTAL FOR SUBSTRUCTURE														75,443.92

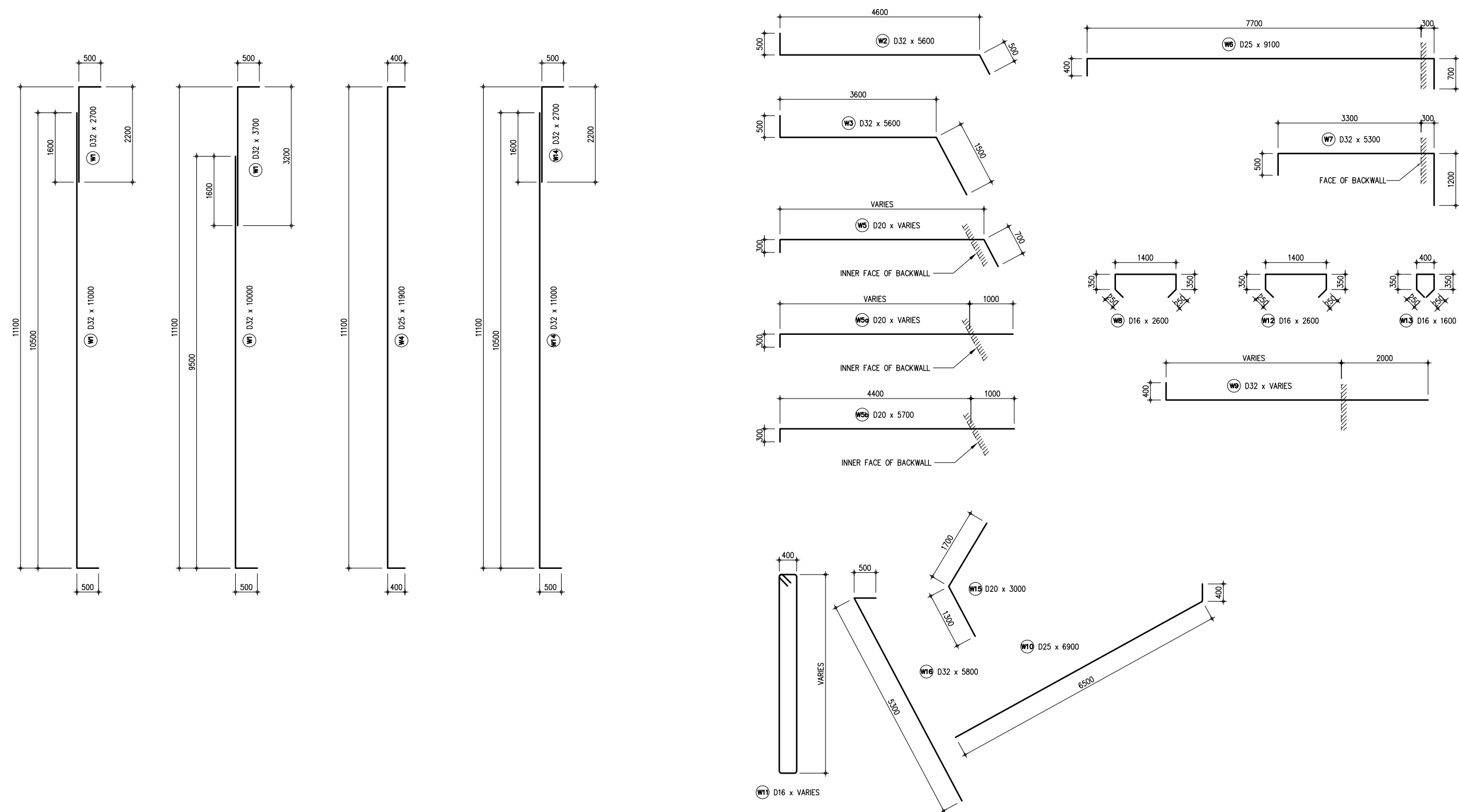
# HIGHWAY BRIDGE NO. 9 (H9) - KELANI RIVER CROSSING BRIGE (SCALE 1:150)

## REINFORCEMENT OF WINGWALL ABUTMENT A2 (1)





HIGHWAY BRIDGE NO.9 (H9) - KELANI RIVER CROSSING BRIGE (SCALE 1:150)
REINFORCEMENT OF WINGWALL ABUTMENT A2 (2)

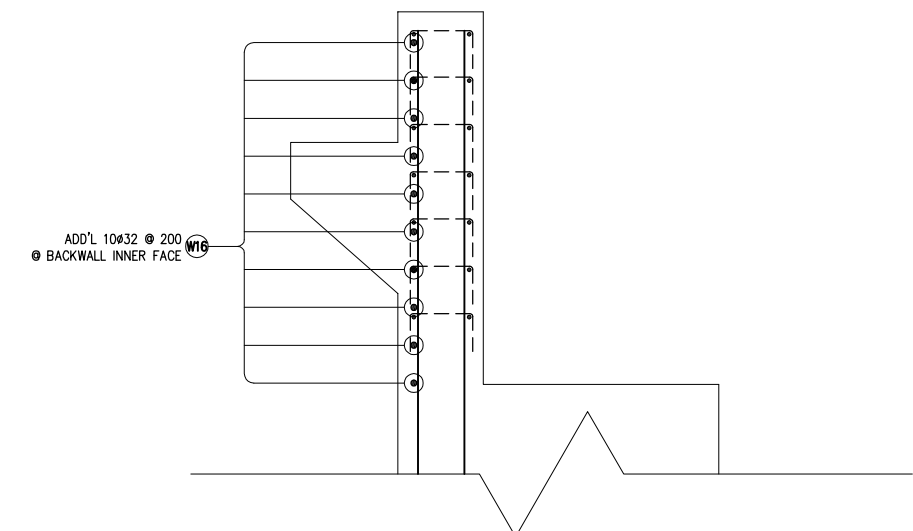
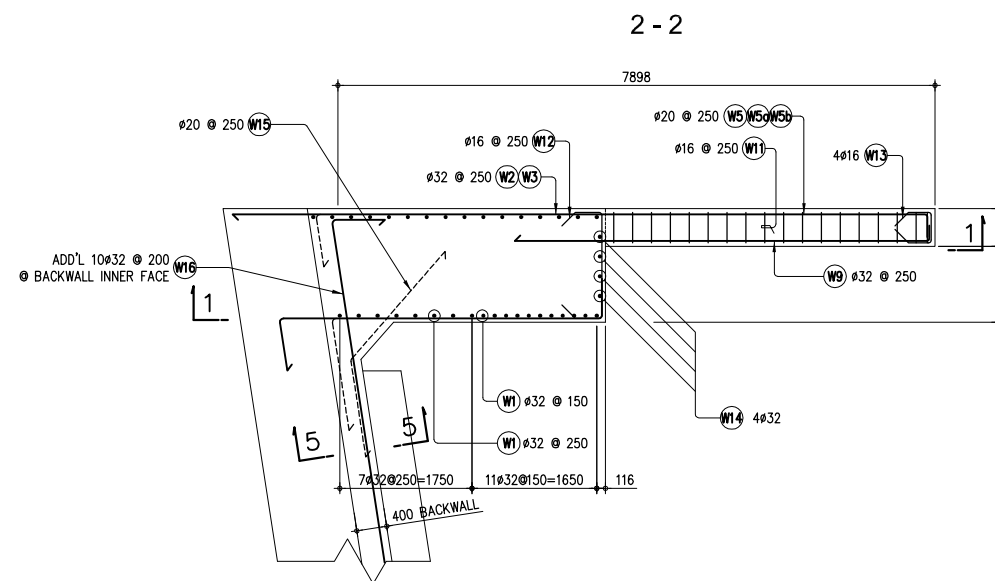
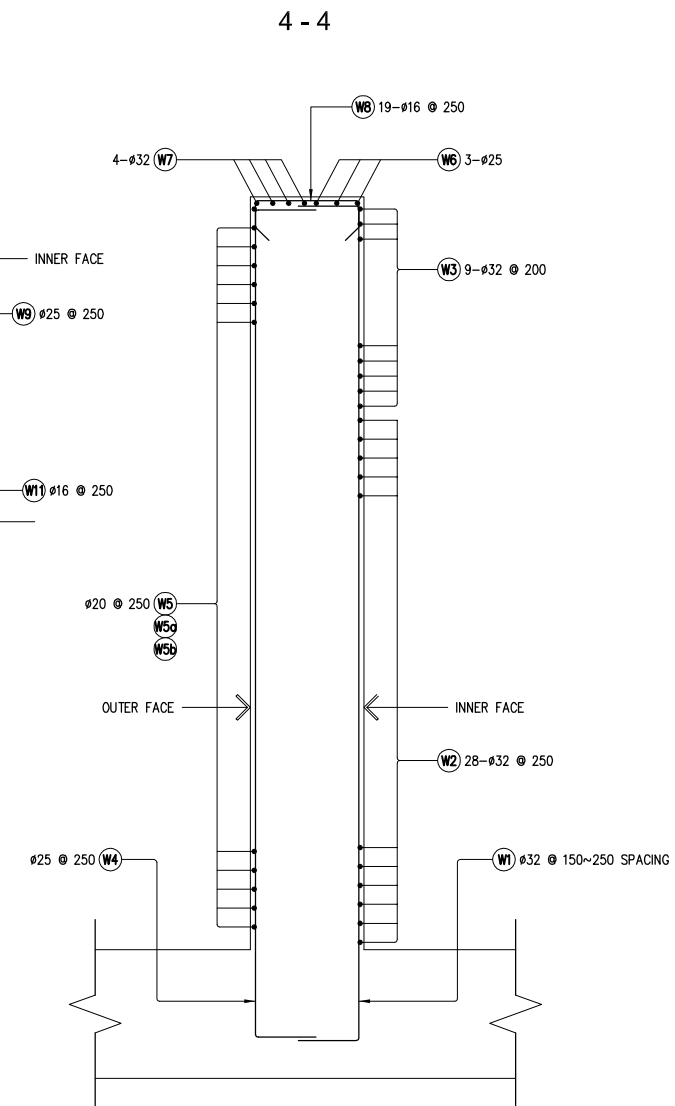
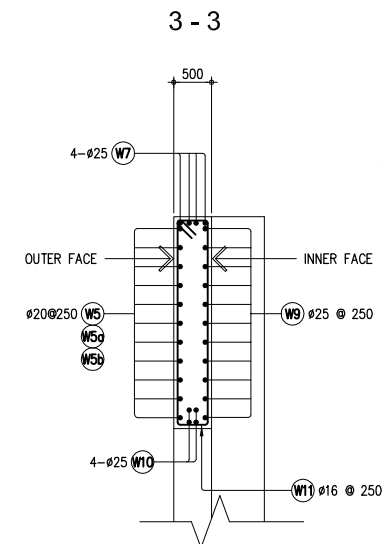
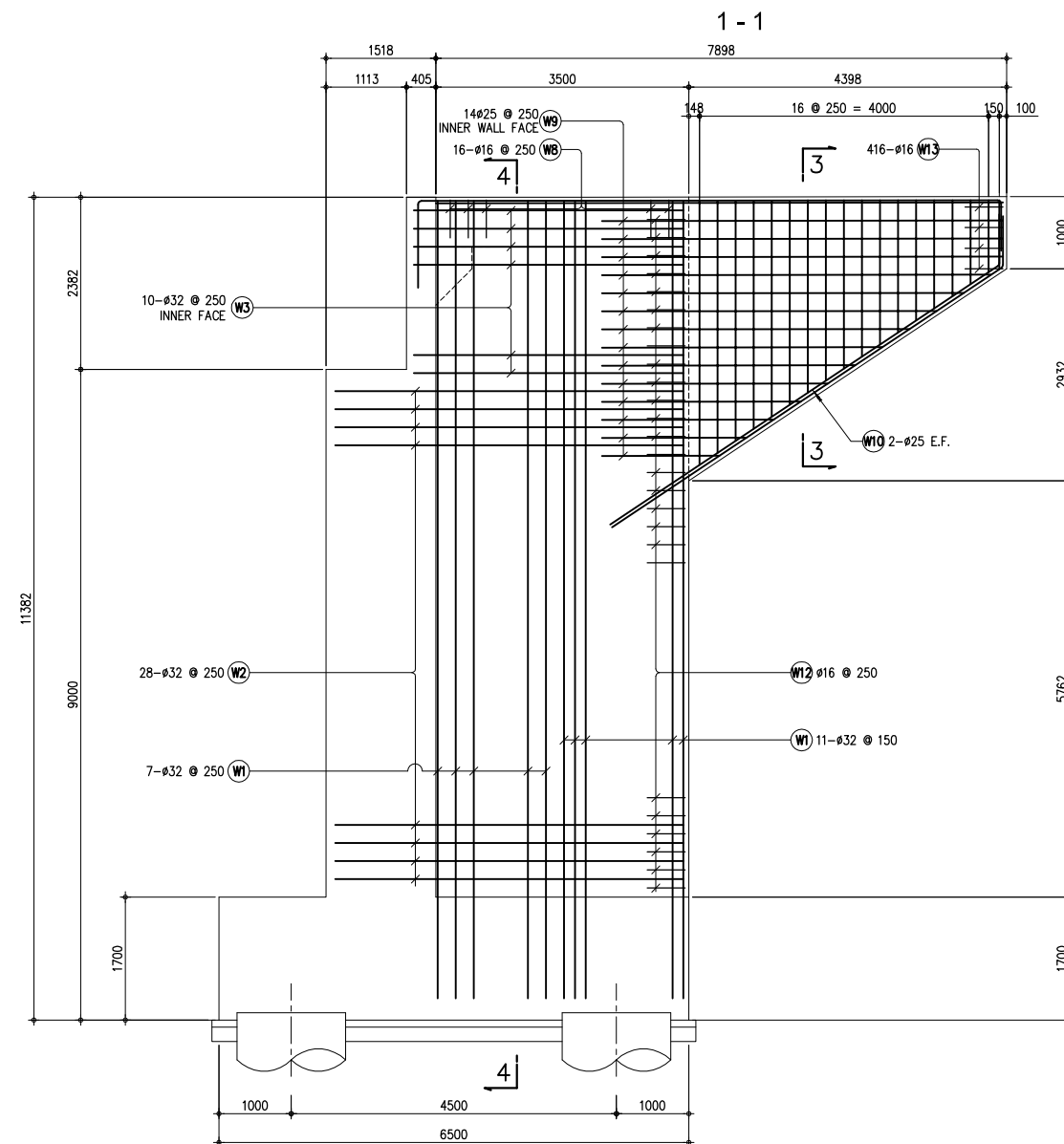


No		
	REVISION	DATE

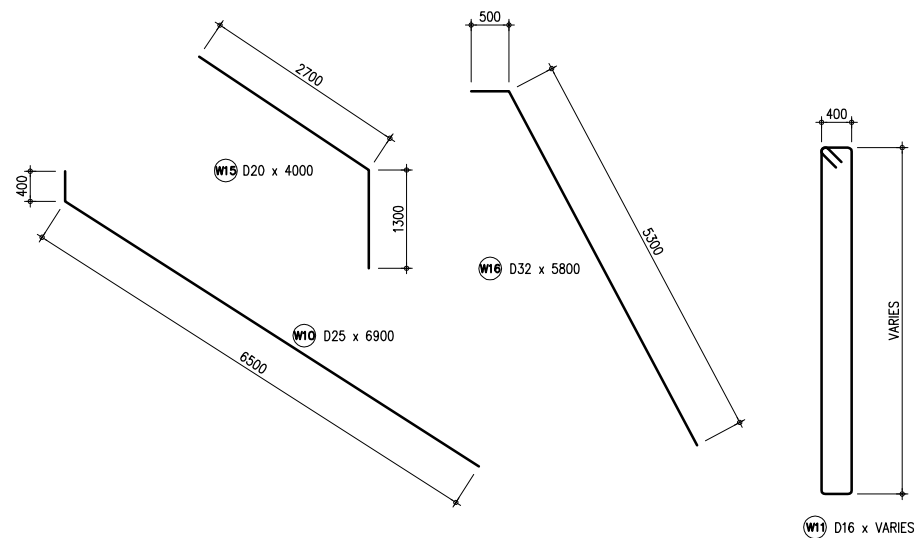
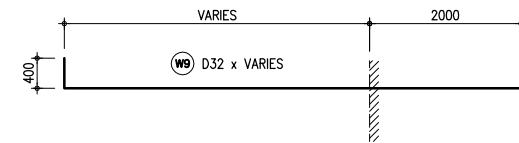
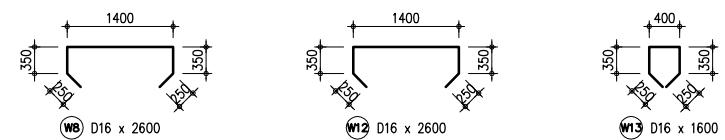
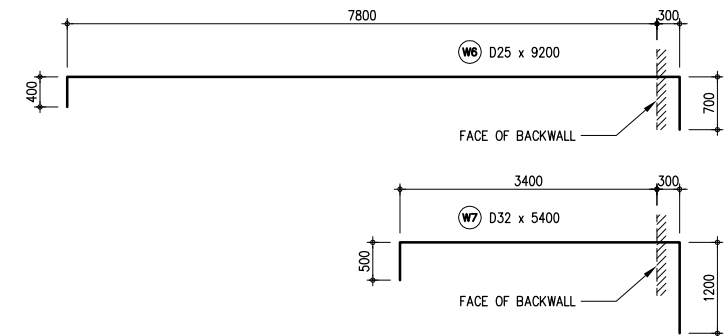
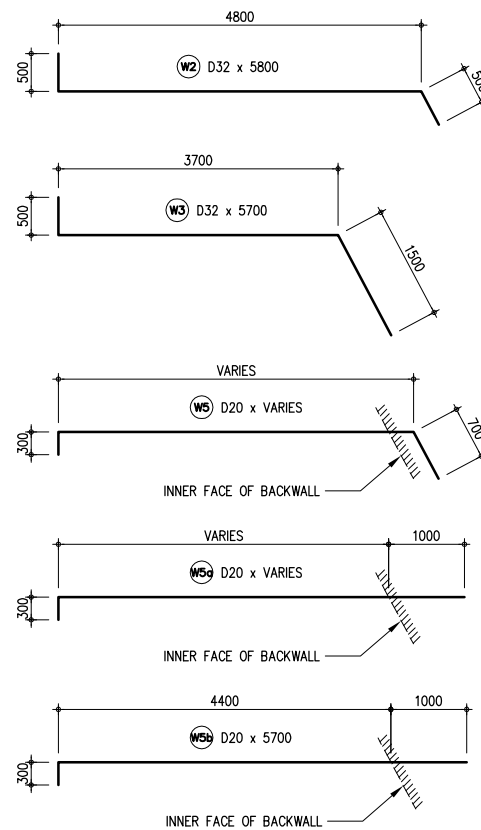
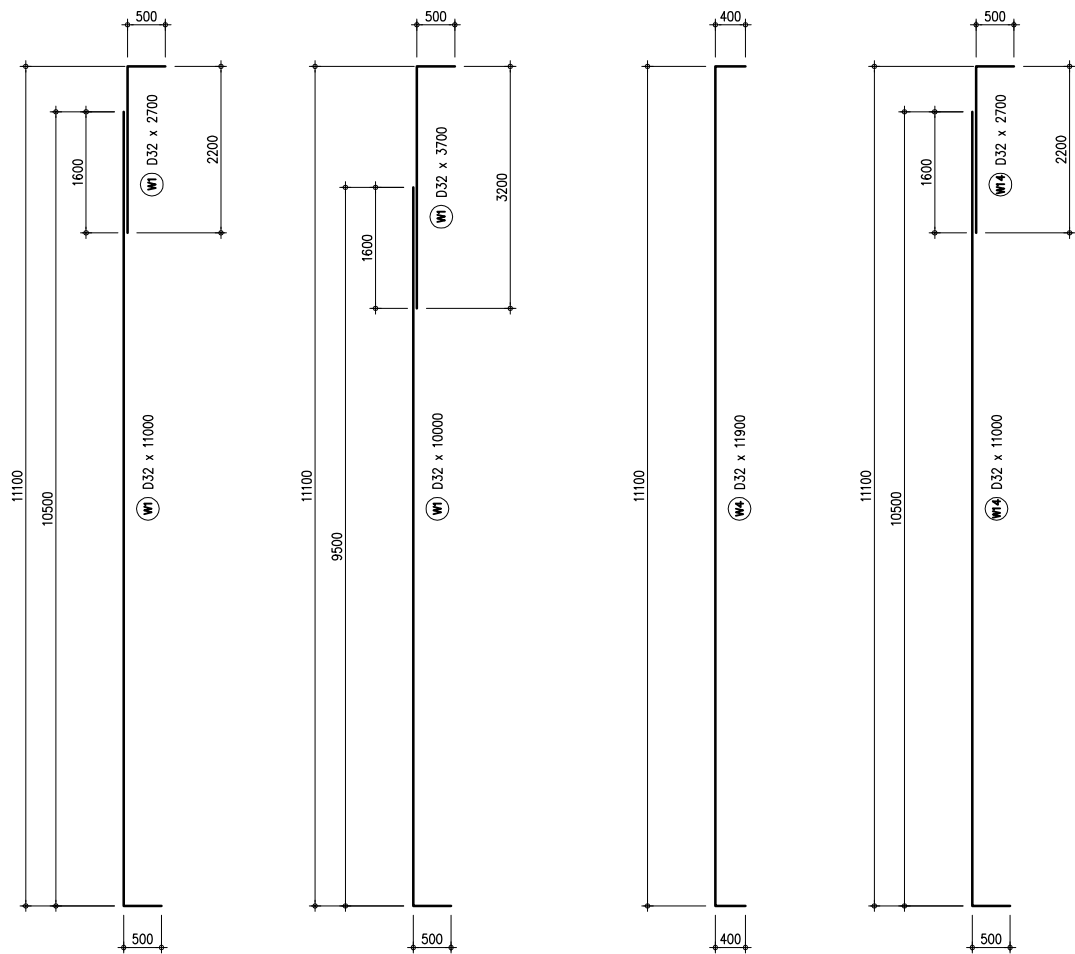
COLOMBO OUTER CIRCULAR HIGHWAY PROJECT (NORTHERN SECTION 1)		DESIGNED BY:	
HIGHWAY BRIDGE NO. 9 (H9) - KELANI RIVER CROSSING BRIGE REINFORCEMENT OF WINGWALL ABUTMENT A2 (2/4)		CHECKED BY:	
		APPROVED BY:	
		DWG. NO.	K01-64

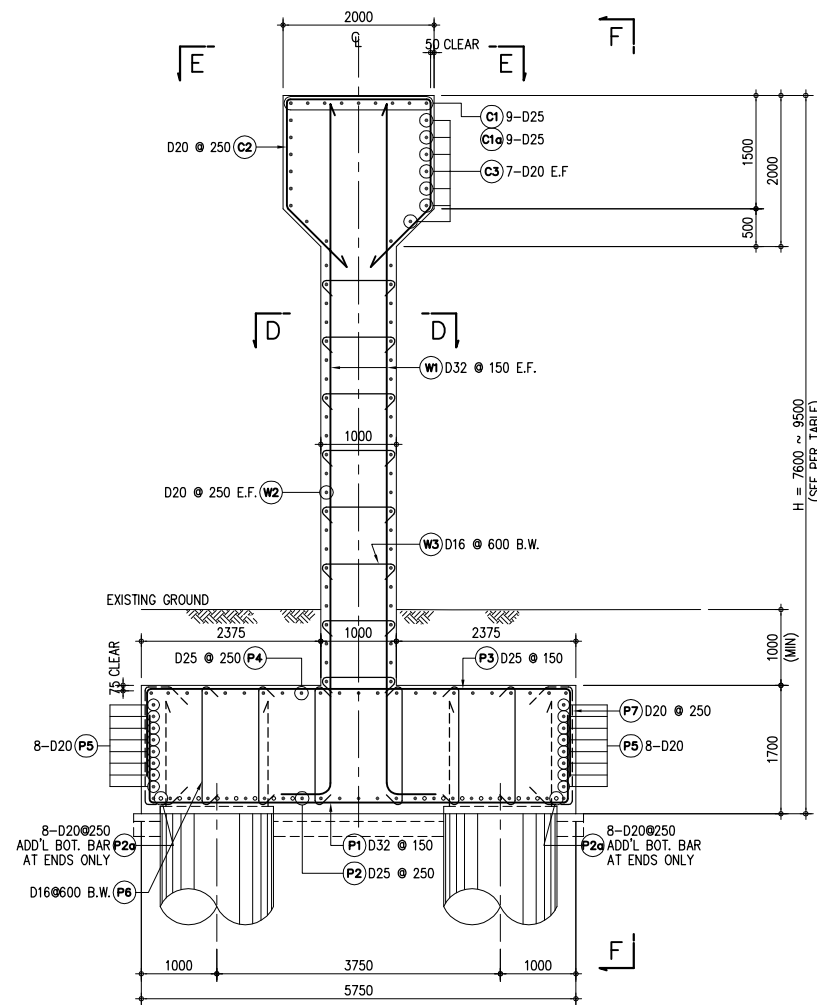
# HIGHWAY BRIDGE NO. 9 (H9) - KELANI RIVER CROSSING BRIGE (SCALE 1:150)

## REINFORCEMENT OF WINGWALL ABUTMENT A2 (3)



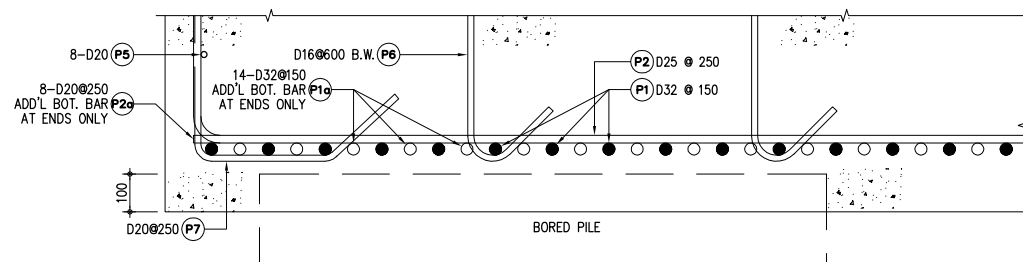
HIGHWAY BRIDGE NO. 9 (H9) - KELANI RIVER CROSSING BRIGE (SCALE 1:150)
REINFORCEMENT OF WINGWALL ABUTMENT A2 (4)



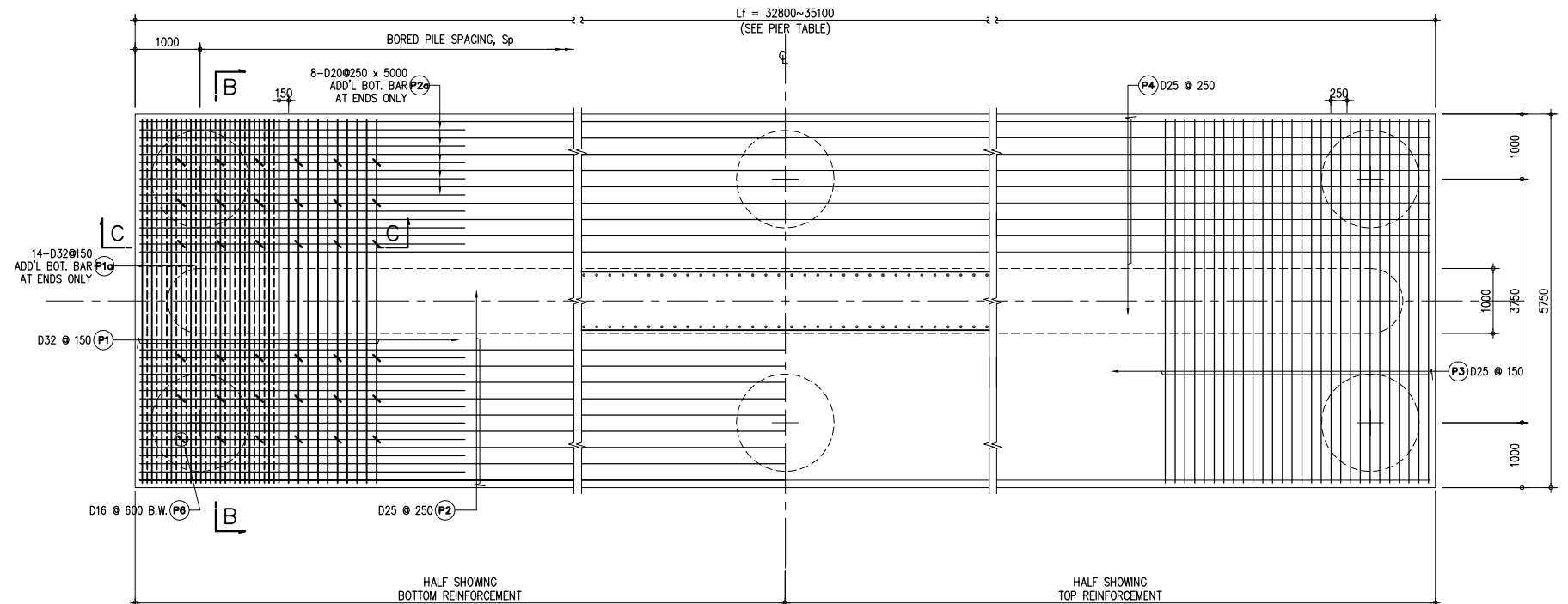


**B-B TRANSVERSE SECTION**  
SCALE 1:50

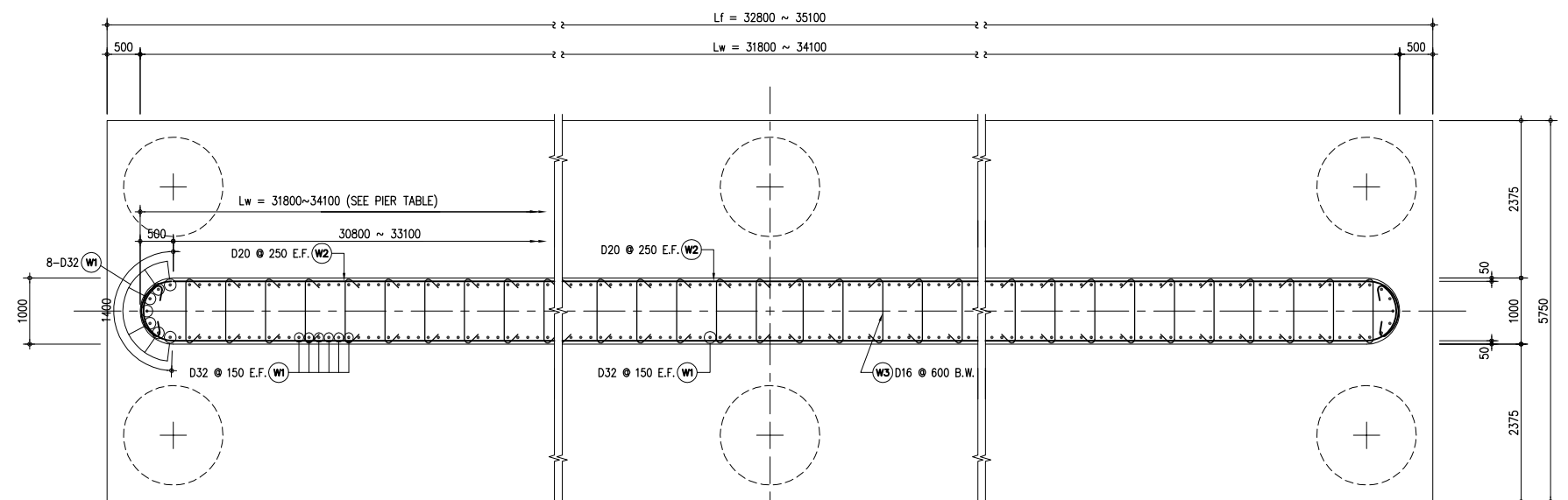
	H (mm)	Lw (mm)	Lc (mm)	Tw (mm)	Lf (mm)	Tf (mm)	Sp (mm)
P0	8,500	33,300	35,300	1,000	34,300	1,700	8,075
P1	8,200	33,100	35,100	1,000	34,100	1,700	8,025
P2	9,100	34,100	36,100	1,000	35,100	1,700	8,275
P7	7,600	33,000	35,000	1,000	34,000	1,700	8,000
P8	9,200	32,800	34,800	1,000	33,800	1,700	7,950
P9	9,500	31,800	33,800	1,000	32,800	1,700	7,700



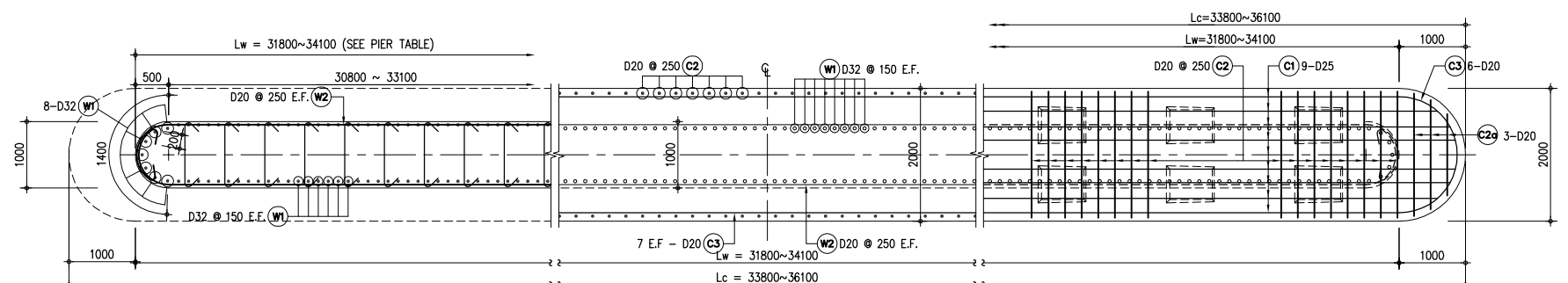
**C-C DETAILED SECTION**  
SCALE 1:10



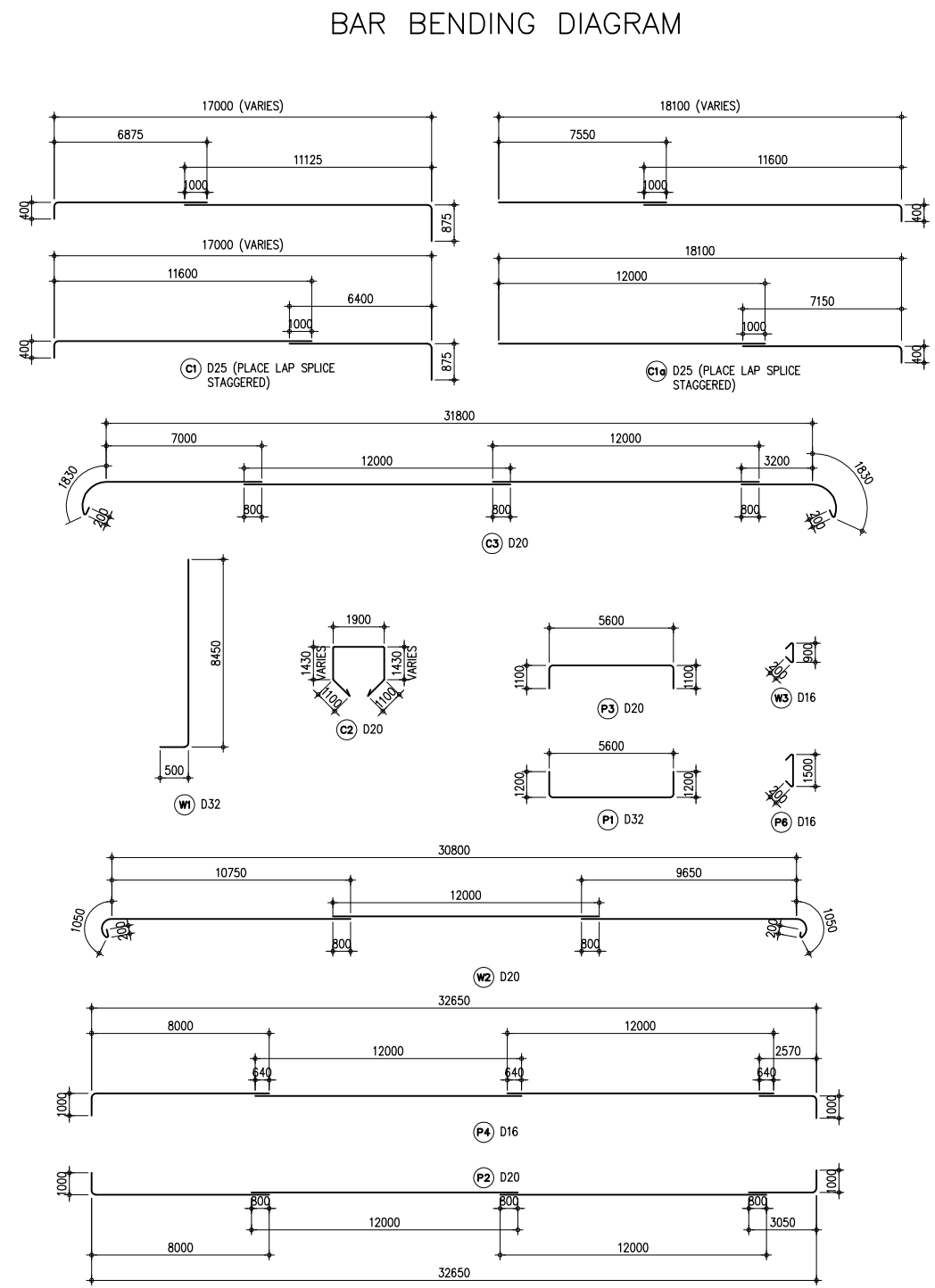
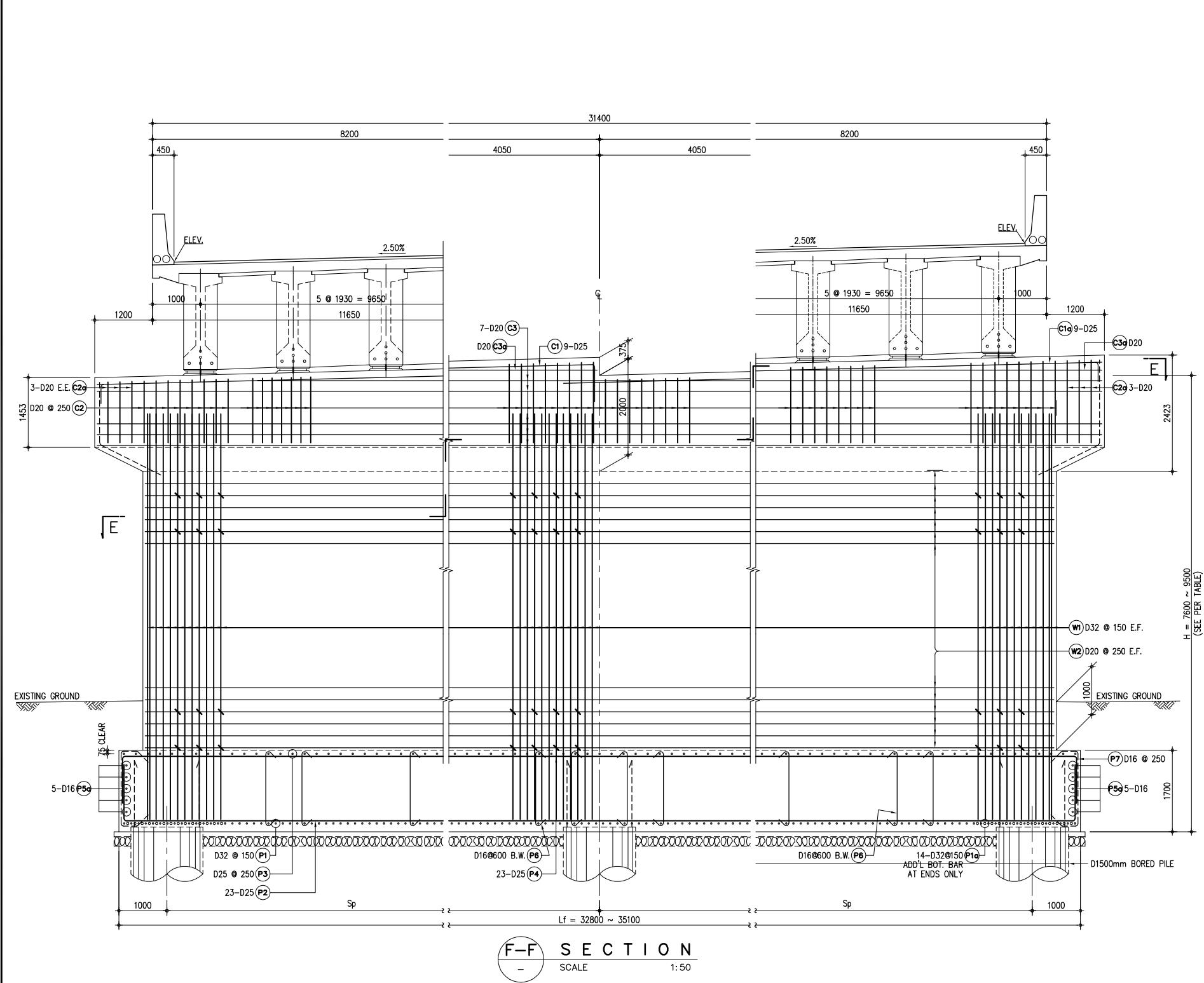
**A REINFORCEMENT PLAN**  
SCALE 1:50

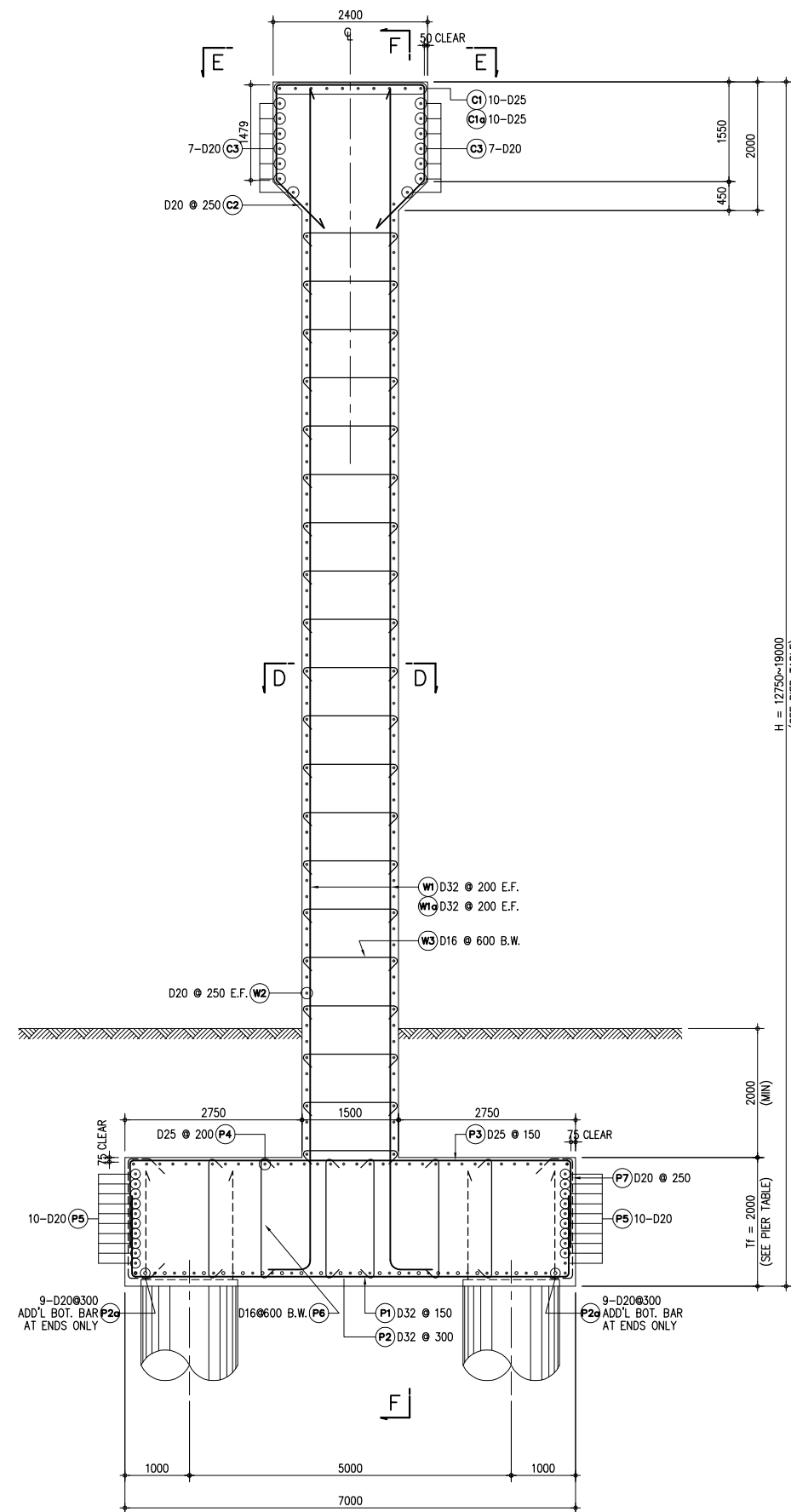


**D-D SECTION**  
SCALE 1:50

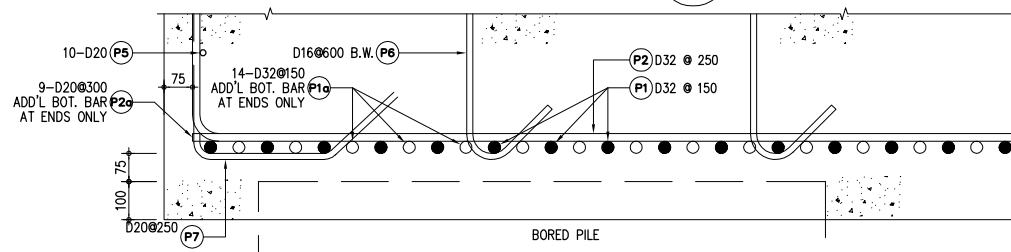
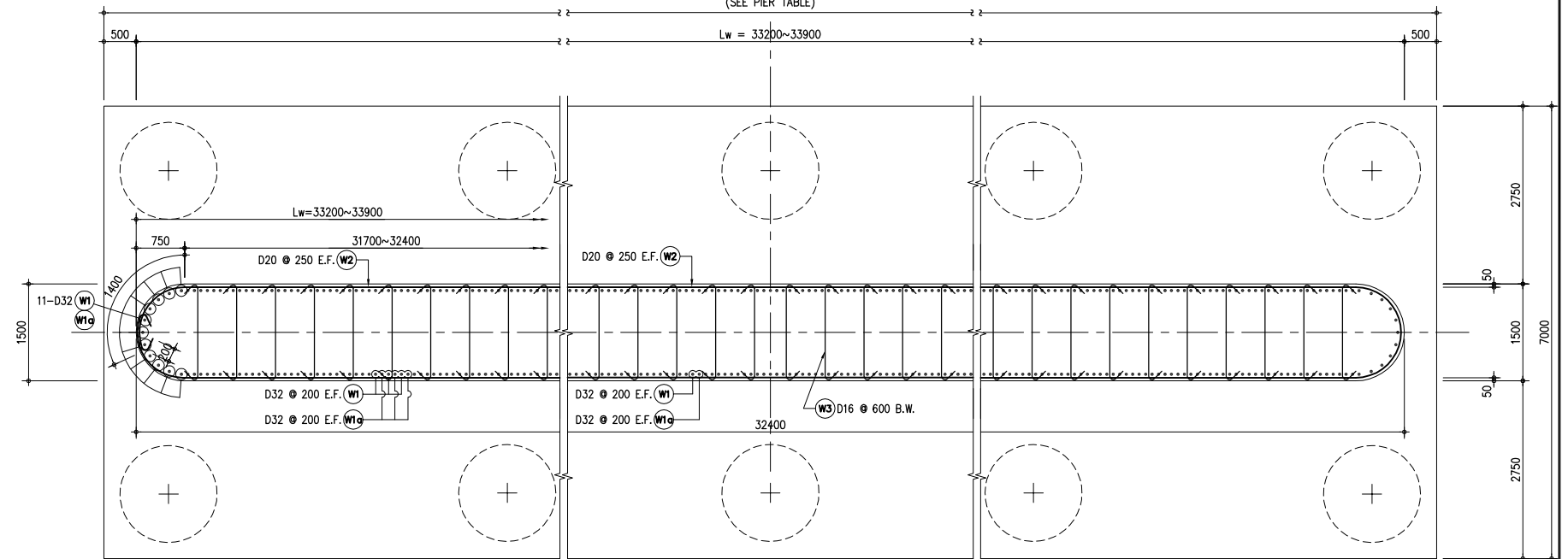
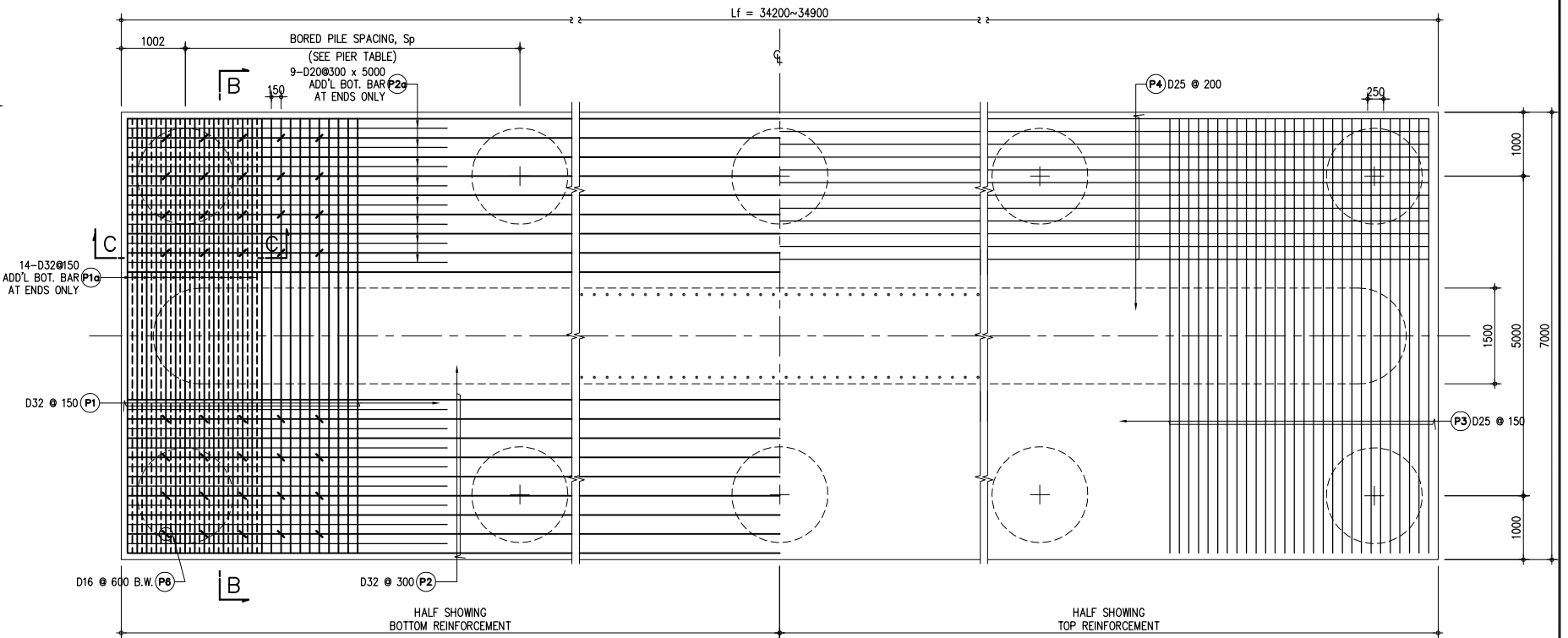


**E-E SECTION**  
SCALE 1:50



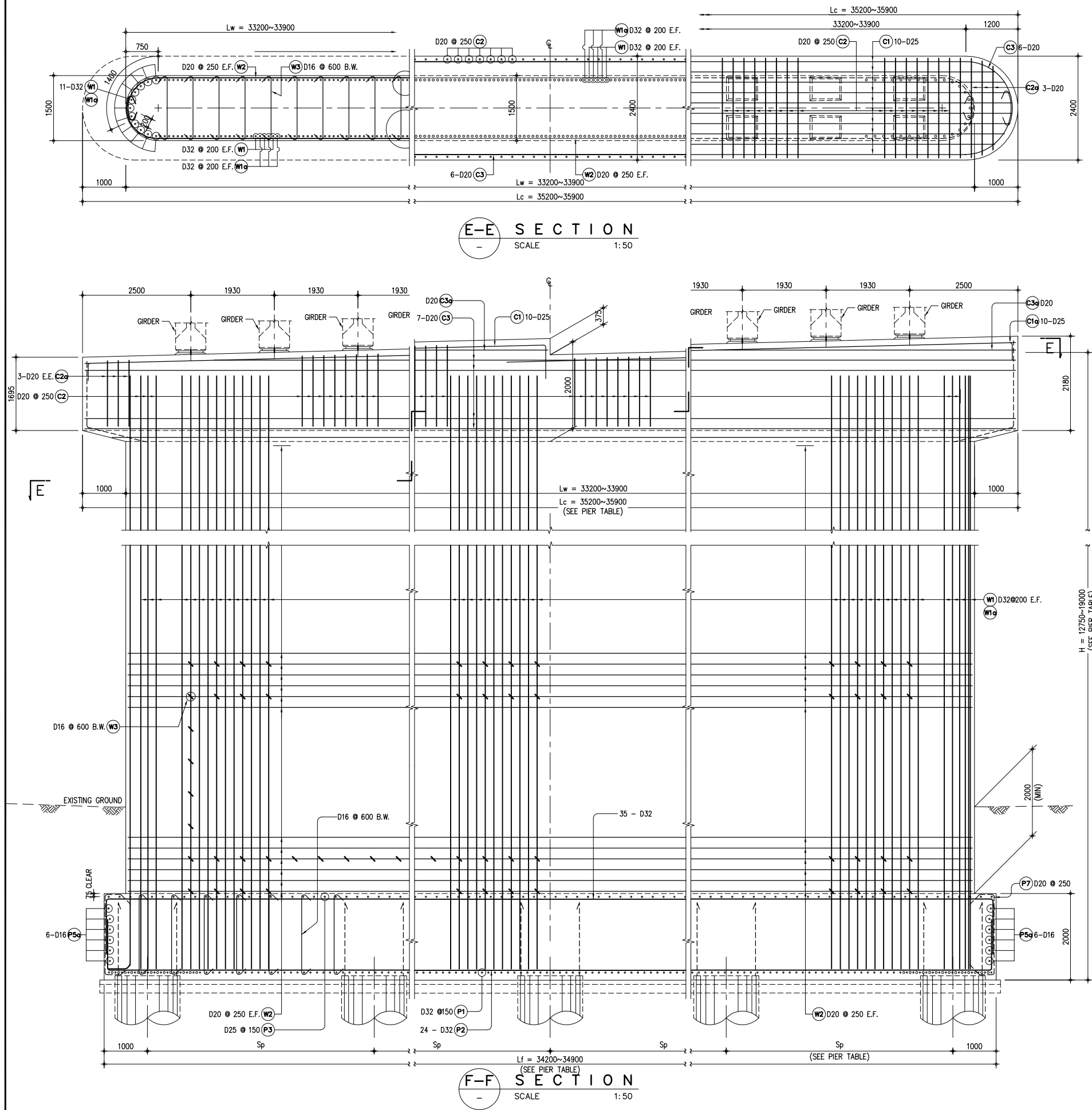


H = 12750~19000  
(SEE PIER TABLE)

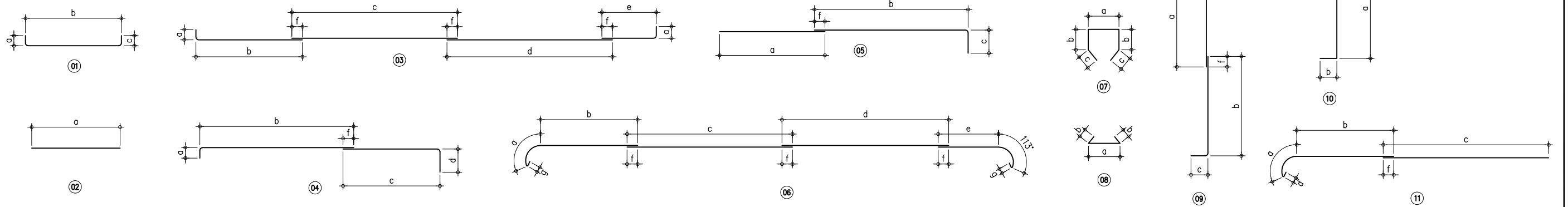


PIER TABLE

	H (mm)	Lw (mm)	Lc (mm)	Tw (mm)	Lf (mm)	Tf (mm)	Sp (mm)
P3	16900	33900	35900	1500	34900	2000	5483
P4	18000	33600	35600	1500	34600	2000	5433
P5	19000	33400	35400	1500	34400	2000	5400
P6	12750	33200	35200	1500	34200	1700	5367



### BAR BENDING DIAGRAM



ESTIMATED QUANTITIES FOR PIER 0																			
REINFORCING BARS																			
LOCATION	BAR MARK	BAR SHAPE	SPACING	QTY. (pcs)	SIZE (mm)	DIMENSION mm ( OUT TO OUT )								LENGTH PER BAR (mm)	TOTAL LENGTH (m)	UNIT WEIGHT (kg/m)	TOTAL WEIGHT (kg)	CONCRETE VOLUME (m³)	
PILE CAP	P1	01		228	32	1200	5600	1200						8000	1824.00	6.313	11,514.91		
	P1a	02		28	32	5600								5600	156.80	6.313	989.88		
	P2	03		23	25	1000	8000	12000	12000	5150	1000			39150	900.45	3.854	3,470.33		
	P2a	02		32	20	5000								5000	160.00	2.466	394.56		
	P3	01		228	25	1100	5600	1100						7800	1778.40	3.854	6,853.95		
	P4	03		23	25	1040	7960	12000	12000	5310	1040			39350	905.05	3.854	3,488.06		
	P5	03		16	20	1040	7960	12000	12000	5310	1040			39350	629.60	2.466	1,552.59		
	P5a	01		10	16	1040	5600	1040						7680	76.80	1.579	121.27		
	P6	08		440	16	1450	200							1850	814.00	1.579	1,285.31		
P7	07		318	20	1450	600	250						3150	1001.70	2.466	2,470.19			
SUB-TOTAL QUANTITY FOR PILE CAP																	32,141.06		335.28
WALL	W1	10		444	32	8200	500							8700	3862.80	6.313	24,385.86		
	W2	06		20	20	1080	7920	12000	12000	3380	1000	200		37860	757.20	2.466	1,867.26		
	W3	08		432	16	900	200							1300	561.60	1.579	886.77		
	SUB-TOTAL QUANTITY FOR STEM																	27,139.88	
COPING	C1	04		9	25	11600	7305	400	875			1250		20180	181.62	3.854	699.96		
	C1a	05		9	25	12000	7605	400				1250		20005	180.05	3.854	693.89		
	C2	07		134	20	1900	1590	1100						7280	975.52	2.466	2,405.63		
	C2a	07		6	20	1750	1590	1100						7130	42.78	2.466	105.50		
	C3	06		14	20	1830	7170	12000	12000	5130	1000	200		40360	565.04	2.466	1,393.39		
	C3a	11		6	20	1830	5650	12000	200		1000			19680	78.72	2.466	194.12		
	SUB-TOTAL QUANTITY FOR FOOTING																	5,492.50	
GRAND TOTAL FOR SUBSTRUCTURE PIER – 0																	64,773.43		639.72

ESTIMATED QUANTITIES FOR PIER 1																	
REINFORCING BARS																	
LOCATION	BAR MARK	BAR SHAPE	SPACING	QTY. (pcs)	SIZE (mm)	DIMENSION mm ( OUT TO OUT )							LENGTH PER BAR (mm)	TOTAL LENGTH (m)	UNIT WEIGHT (kg/m)	TOTAL WEIGHT (kg)	CONCRETE VOLUME (m³)
						a	b	c	d	e	f	g					
PILE CAP	P1	01		226	32	1200	5600	1200					8000	1808.00	6.313	11,413.90	
	P1a	02		28	32	5600							5600	156.80	6.313	989.88	
	P2	03		23	25	1000	7450	12000	12000	5500	1000		38950	895.85	3.854	3,452.61	
	P2a	02		32	20	5000							5000	160.00	2.466	394.56	
	P3	01		226	25	1100	5600	1100					7800	1762.80	3.854	6,793.83	
	P4	03		23	25	1040	7760	12000	12000	5310	1040		39150	900.45	3.854	3,470.33	
	P5	03		16	20	1040	7760	12000	12000	5310	1040		39150	626.40	2.466	1,544.70	
	P5a	01		10	16	1040	5600	1040					7680	76.80	1.579	121.27	
	P6	08		440	16	1450	200						1850	814.00	1.579	1,285.31	
P7	07		316	20	1450	600	250					3150	995.40	2.466	2,454.66		
SUB-TOTAL QUANTITY FOR PILE CAP																31,921.05	144.81
WALL	W1	10		442	32	7900	500						8400	3712.80	6.313	23,438.91	
	W2	06		19	20	1080	7600	12000	12000	3500	1000	200	37660	715.54	2.466	1,764.52	
	W3	08		432	16	900	200						1300	561.60	1.579	886.77	
SUB-TOTAL QUANTITY FOR STEM																26,090.19	147.98
COPING	C1	04		09	25	11600	7205	400	875		1250		20080	180.72	3.854	696.49	
	C1a	05		09	25	12000	7505	400			1250		19905	179.15	3.854	690.42	
	C2	07		133	20	1900	1590	1100					7280	968.24	2.466	2,387.68	
	C2a	07		06	20	1750	1590	1100					7130	42.78	2.466	105.50	
	C3	06		14	20	1830	7170	12000	12000	4930	1000	200	40160	562.24	2.466	1,386.48	
	C3a	11		04	20	1830	5550	12000	200		1000		19580	78.32	2.466	193.14	
	SUB-TOTAL QUANTITY FOR FOOTING																
GRAND TOTAL FOR SUBSTRUCTURE PIER - 1																63,470.96	626.12

ESTIMATED QUANTITIES FOR PIER 2																	
REINFORCING BARS																	
LOCATION	BAR MARK	BAR SHAPE	SPACING	QTY. (pcs)	SIZE (mm)	DIMENSION mm ( OUT TO OUT )							LENGTH PER BAR (mm)	TOTAL LENGTH (m)	UNIT WEIGHT (kg/m)	TOTAL WEIGHT (kg)	CONCRETE VOLUME (m³)
PILE CAP	P1	01		233	32	1200	5600	1200					8000	1864.00	6.313	11,767.43	
	P1a	02		28	32	5600						5600	156.80	6.313	989.88		
	P2	03		23	25	1000	8000	12000	12000	5950	1000	39950	918.85	3.854	3,541.25		
	P2a	02		32	20	5000						5000	160.00	2.466	394.56		
	P3	01		233	25	1100	5600	1100				7800	1817.40	3.854	7004.26		
	P4	03		23	25	1040	7960	12000	12000	6110	1040	40150	923.45	3.854	3,558.98		
	P5	03		16	20	1040	7960	12000	12000	6110	1040	40150	642.40	2.466	1,584.16		
	P5a	01		10	16	1040	5600	1040				7680	76.80	1.579	121.27		
	P6	08		448	16	1450	200					1850	828.80	1.579	1,308.68		
	P7	07		324	20	1450	600	250				3150	1020.60	2.466	2,516.80		
SUB-TOTAL QUANTITY FOR PILE CAP															32,787.25	343.10	
WALL	W1	10		456	32	8200	500					9300	4240.80	6.313	26,772.17		
	W2	06		23	20	1080	7920	12000	12000	4180	1000	200	38660	889.18	2.466		2,192.72
	W3	08		560	16	900	200					1300	728.00	1.579	1,149.51		
	SUB-TOTAL QUANTITY FOR STEM																30,114.40
COPING	C1	04		09	25	11600	7705	400	875		1250		20580	185.22	3.854	713.84	
	C1a	05		09	25	12000	8005	400			1250		20405	183.65	3.854	707.77	
	C2	07		137	20	1900	1590	1100					7280	997.36	2.466	2,459.49	
	C2a	07		06	20	1750	1590	1100					7130	42.78	2.466	105.50	
	C3	06		14	20	1830	7170	12000	12000	5930	1000	200	41160	576.24	2.466	1,421.01	
	C3a	11		04	20	1830	6050	12000	200		1000		20080	80.32	2.466	198.07	
	SUB-TOTAL QUANTITY FOR FOOTING															5,407.60	
GRAND TOTAL FOR SUBSTRUCTURE PIER – 2															68,309.25	675.01	

ESTIMATED QUANTITIES FOR PIER 3																		
REINFORCING BARS																		
LOCATION	BAR MARK	BAR SHAPE	SPACING	QTY. (pcs)	SIZE (mm)	DIMENSION mm ( OUT TO OUT )							LENGTH PER BAR (mm)	TOTAL LENGTH (m)	UNIT WEIGHT (kg/m)	TOTAL WEIGHT (kg)	CONCRETE VOLUME (m³)	
						a	b	c	d	e	f	g						
PILE CAP	P1	01		232	32	1350	6850	1350					9550	2215.60	6.313	13,987.08		
	P1a	02		28	32	6850							6850	191.80	6.313	1,210.83		
	P2	03		24	32	1250	6500	12000	12000	8000	1250		41000	984.00	6.313	6,211.99		
	P2a	02		36	20	5000							5000	180.00	2.466	443.88		
	P3	01		232	25	1300	6850	1300					9450	2129.40	3.854	8,449.51		
	P4	03		35	25	1200	7800	12000	12000	6550	1200		40750	1426.25	3.854	5,496.77		
	P5	03		20	20	1200	7800	12000	12000	6550	1200		40750	815.00	2.466	2,009.79		
	P5a	01		12	16	1200	6850	1200					9250	111.00	1.579	175.27		
	P6	08		560	16	1750	200						2150	1204.00	1.579	1,901.12		
	P7	07		332	20	1750	600	250					3450	1145.40	2.466	2,824.56		
SUB-TOTAL QUANTITY FOR PILE CAP															42,710.80	488.60		
WALL	W1	09		338	32	10000	8310	500			1600		18810	6357.78	6.313	40,136.67		
	W1a	09		334	32	6810	11500	500			1600		18810	6282.54	6.313	39,661.68		
	W2	06		53	20	1450	6000	12000		12000	5400	1000	200	38700	2051.10	2.466		5,058.01
	W3	08		1188	16	1400	200						1800	2138.40	1.579	3376.53		
	SUB-TOTAL QUANTITY FOR STEM															88,232.89		649.77
COPING	C1	04		10	25	11600	7605	400		875		1250		20480	204.80	3.854	789.30	
	C1a	05		10	25	1200	7905	400				1250		20350	203.05	3.854	782.55	
	C2	07		134	20	2300	1765	1050						7930	1062.62	2.466	2,620.42	
	C2a	07		06	20	2150	1765	1050						7780	46.68	2.466	115.11	
	C3	06		14	20	2110	7000	12000	12000	5500	1000	200	41120	575.68	2.466	1419.63		
	C3a	11		04	20	2110	5750	12000	200		1000			20060	80.24	2.466	197.87	
	SUB-TOTAL QUANTITY FOR FOOTING															5,924.89	180.22	
GRAND TOTAL FOR SUBSTRUCTURE PIER – 3															136,868.57	1,318.59		



ESTIMATED QUANTITIES FOR PIER 4																		
REINFORCING BARS																		
LOCATION	BAR MARK	BAR SHAPE	SPACING	QTY. (pcs)	SIZE (mm)	DIMENSION mm ( OUT TO OUT )							LENGTH PER BAR (mm)	TOTAL LENGTH (m)	UNIT WEIGHT (kg/m)	TOTAL WEIGHT (kg)	CONCRETE VOLUME (m³)	
						a	b	c	d	e	f	g						
PILE CAP	P1	01		230	32	1350	6850	1350					9550	2196.50	6.313	13,866.50		
	P1a	02		28	32	6850							6850	191.80	6.313	1,210.83		
	P2	03		24	32	1250	6200	12000	12000	8000	1250		40700	976.80	6.313	6,166.54		
	P2a	02		36	20	5000							5000	180.00	2.466	443.88		
	P3	01		230	25	1300	6850	1300					9450	2173.50	3.854	8,376.67		
	P4	03		35	25	1200	7800	12000	12000	6650	1200		40450	1415.75	3.854	5,456.30		
	P5	03		20	20	1200	7800	12000	12000	6650	1200		40450	809.00	2.466	1,994.99		
	P5a	01		12	16	1200	6850	1200					9250	111.00	1.579	175.27		
	P6	08		550	16	1750	200						2150	1182.50	1.579	1,867.17		
	P7	07		330	20	1750	600	250					3450	1138.50	2.466	2,807.54		
SUB-TOTAL QUANTITY FOR PILE CAP															42,365.70	484.40		
WALL	W1	09		336	32	10000	9410	500			1600		19910	6689.76	6.313	42,232.45		
	W1a	09		330	32	7910	11500	500			1600		19910	6570.30	6.313	41,478.30		
	W2	06		57	20	1450	6000	12000		12000	5100	1000	200	38400	2188.80	2.466		5,397.58
	W3	08		1296	16	1400	200						1800	2332.80	1.579	3,683.49		
	SUB-TOTAL QUANTITY FOR STEM															92,791.83		711.27
COPING	C1	04		10	25	11600	7455	400	875		1250		20330	203.30	3.854	783.52		
	C1a	05		10	25	12000	7755	400			1250		20155	201.55	3.854	776.77		
	C2	07		133	20	2300	1765	1050					7930	1054.69	2.466	2600.87		
	C2a	07		06	20	2150	1765	1050					7780	46.68	2.466	115.11		
	C3	06		14	20	2110	7000	12000	12000	5200	1000	200	40820	571.48	2.466	1409.27		
	C3a	11		04	20	2110	5600	12000	200		1000		19910	79.64	2.466	196.39		
	SUB-TOTAL QUANTITY FOR FOOTING															5,881.93		178.71
GRAND TOTAL FOR SUBSTRUCTURE PIER – 4															141,039.46	1,374.38		

ESTIMATED QUANTITIES FOR PIER 6																		
REINFORCING BARS																		
LOCATION	BAR MARK	BAR SHAPE	SPACING	QTY. (pcs)	SIZE (mm)	DIMENSION mm ( OUT TO OUT )							LENGTH PER BAR (mm)	TOTAL LENGTH (m)	UNIT WEIGHT (kg/m)	TOTAL WEIGHT (kg)	CONCRETE VOLUME (m³)	
						a	b	c	d	e	f	g						
PILE CAP	P1	01		227	32	1350	6850	1350					9550	2167.85	6.313	13,685.64		
	P1a	02		28	32	6850							6850	191.80	6.313	1,210.83		
	P2	03		24	32	1250	5800	12000	12000	8000	1250		40300	967.20	6.313	6,105.93		
	P2a	02		36	20	5000							5000	180.00	2.466	443.88		
	P3	01		227	25	1300	6850	1300					9450	2145.15	3.854	8,267.41		
	P4	03		35	25	1200	7800	12000	12000	5850	1200		40050	1401.75	3.854	5,402.34		
	P5	03		20	20	1200	7800	12000	12000	5850	1200		40050	801.00	2.466	1,975.27		
	P5a	01		12	16	1200	6850	1200					9250	111.00	1.579	175.27		
	P6	08		550	16	1750	200						2150	1182.50	1.579	1,867.17		
	P7	07		328	20	1750	600	250					3450	1131.60	2.466	2,790.53		
SUB-TOTAL QUANTITY FOR PILECAP															41,924.26	406.98		
WALL	W1	09		332	32	8000	6160	500			1600		14660	4867.12	6.313	30,726.13		
	W1a	09		326	32	4660	9500	500			1600		14660	4779.16	6.313	30,170.84		
	W2	06		37	20	1450	6000	12000	12000	4700	1000	200	38000	1406.00	2.466	3,467.20		
	W3	08		848	16	1400	200						1800	1526.40	1.579	2,410.19		
	SUB-TOTAL QUANTITY FOR PILECAP															66,774.35		446.35
COPING	C1	04		10	25	11600	7255	400	875		1250		20130	201.30	3.854	775.81		
	C1a	05		10	25	12000	7555	400			1250		19955	199.55	3.854	769.07		
	C2	07		132	20	2300	1765	1050					7930	1046.76	2.466	2,581.31		
	C2a	07		6	20	2150	1765	1050					7780	46.68	2.466	115.11		
	C3	06		14	20	2110	7000	12000	12000	4800	1000	200	40420	565.88	2.466	1,395.46		
	C3a	11		4	20	2110	5400	12000	200		1000		19710	78.84	2.466	194.42		
	SUB-TOTAL QUANTITY FOR PILECAP															5,831.18		176.70
	GRAND TOTAL FOR SUBSTRUCTURE PIER – 6															114,529.79		1030.03

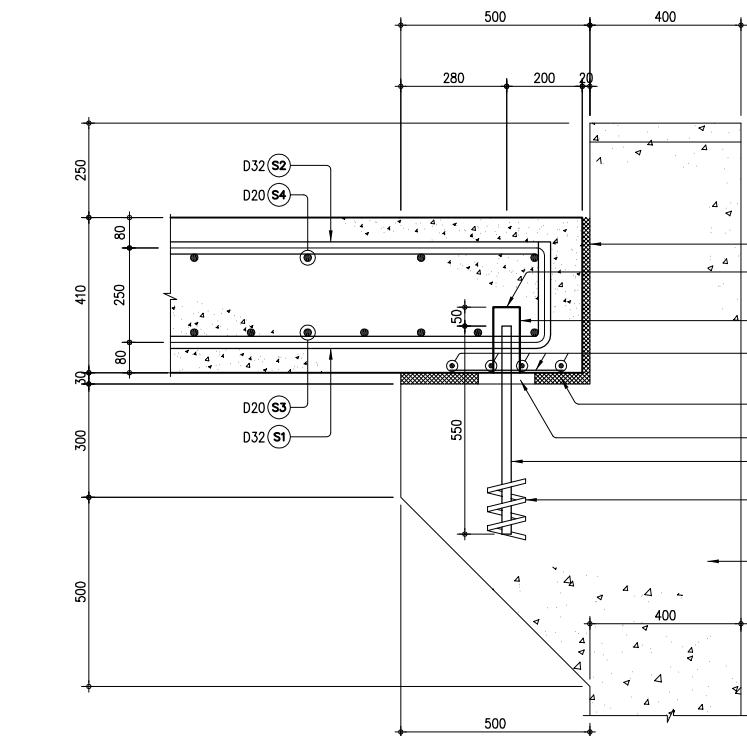
ESTIMATED QUANTITIES FOR PIER 5																	
REINFORCING BARS																	
LOCATION	BAR MARK	BAR SHAPE	SPACING	QTY. (pcs)	SIZE (mm)	DIMENSION mm ( OUT TO OUT )							LENGTH PER BAR (mm)	TOTAL LENGTH (m)	UNIT WEIGHT (kg/m)	TOTAL WEIGHT (kg)	CONCRETE VOLUME (m³)
						a	b	c	d	e	f	g					
PILE CAP	P1	01		228	32	1350	6850	1350					9550	2177.40	6.313	13745.93	481.60
	P1a	02		28	32	6850							6850	191.80	6.313	1,210.83	
	P2	03		24	32	1250	6000	12000	12000	8000	1250		40500	972.00	6.313	6316.24	
	P2a	02		36	20	5000							5000	180.00	2.466	443.88	
	P3	01		228	25	1300	6850	1300					9450	2154.60	3.854	8303.83	
	P4	03		35	25	1200	7800	12000	12000	6050	1200		40250	1408.75	3.854	5429.32	
	P5	03		20	20	1200	7800	12000	12000	6050	1200		40250	805.00	2.466	1985.13	
	P5a	01		12	16	1200	6850	1200					9250	111.00	1.579	175.27	
	P6	08		550	16	1750	200						2150	1182.50	1.579	1867.17	
	P7	06		328	20	1750	600	250					3450	1131.60	2.466	2790.53	
SUB-TOTAL QUANTITY FOR PILE CAP															42088.12	481.60	
WALL	W1	09		334	32	10000	10410	500			1600		20910	6983.94	6.313	44089.61	744.30
	W1a	09		328	32	8910	11500	500			1600		20910	6858.48	6.313	43297.58	
	W2	06		61	20	1450	6000	12000	12000	4900	1000	200	38200	2330.20	2.466	5746.27	
	W3	01		1350	16	1450	200						1800	2430.00	1.579	3836.97	
SUB-TOTAL QUANTITY FOR STEM															96970.44	744.30	
COPING	C1	04		10	25	11600	7355	400	875		1250		20230	202.30	3.854	779.66	177.71
	C1a	05		10	25	12000	7655	400			1250		20055	200.55	3.854	772.92	
	C2	07		132	20	2300	1765	1050					7930	1046.76	2.466	2581.31	
	C2a	07		6	20	2150	1765	1050					7780	46.68	2.466	115.11	
	C3	06		14	20	2100	7000	12000	12000	5000	1000	200	40620	568.68	2.466	1402.36	
	C3a	11		4	20	2100	5500	122000	200		1000		19810	79.24	2.466	195.41	
SUB-TOTAL QUANTITY FOR FOOTING															5846.78	177.71	
GRAND TOTAL FOR SUBSTRUCTURE PIER – 6															144905.34	1403.61	

ESTIMATED QUANTITIES FOR PIER 8																	
REINFORCING BARS																	
LOCATION	BAR MARK	BAR SHAPE	SPACING	QTY. (pcs)	SIZE (mm)	DIMENSION mm ( OUT TO OUT )							LENGTH PER BAR (mm)	TOTAL LENGTH (m)	UNIT WEIGHT (kg/m)	TOTAL WEIGHT (kg)	CONCRETE VOLUME (m³)
						a	b	c	d	e	f	g					
PILE CAP	P1	01		224	32	1200	5600	1200					8000	1792.00	6.313	11,312.90	
	P1a	02		28	32	5600							5600	156.80	6.313	989.88	
	P2	03		23	25	1000	7150	12000	12000	5500	1000		38650	888.95	3.854	3,426.01	
	P2a	02		32	20	5000							5000	160.00	2.466	394.56	
	P3	01		224	25	1100	5600	1100					7800	1747.20	3.854	6,733.71	
	P4	03		23	25	1040	7460	12000	12000	5310	1040		38850	893.55	3.854	3,443.74	
	P5	03		16	20	1040	7460	12000	12000	5310	1040		38850	621.60	2.466	1,532.87	
	P5a	01		10	16	1040	5600	1040					7680	76.80	1.579	121.27	
	P6	08		432	16	1450	200						1850	799.20	1.579	1,261.94	
	P7	07		314	20	1450	600	250					3150	989.10	2.466	2,439.12	
SUB-TOTAL QUANTITY FOR PILE CAP															31,655.99	330.40	
WALL	W1	10		438	32	8900	500						9400	4117.20	6.313	25,991.88	
	W2	06		23	20	1080	7300	12000	12000	3500	1000	200	37360	859.28	2.466	2,118.98	
	W3	08		530	16	900	200						1300	689.00	1.579	1087.93	
SUB-TOTAL QUANTITY FOR PILECAP															29,198.80	179.22	
COPING	C1	04		9	25	11600	7055	400	875		1250		19930	179.37	3.854	691.29	
	C1a	05		9	25	12000	7355	400			1250		19755	177.80	3.854	685.22	
	C2	07		132	20	1900	1590	1100					7280	960.96	2.466	2,369.73	
	C2a	07		6	20	1750	1590	1100					7130	42.78	2.466	105.50	
	C3	06		14	20	1830	7170	12000	12000	4630	1000	200	39860	558.04	2.466	1,376.13	
	C3a	11		4	20	1830	5400	12000	200		1000		19430	77.72	2.466	191.66	
	SUB-TOTAL QUANTITY FOR PILECAP															5,419.52	
GRAND TOTAL FOR SUBSTRUCTURE PIER – 8															66,274.31	653.19	

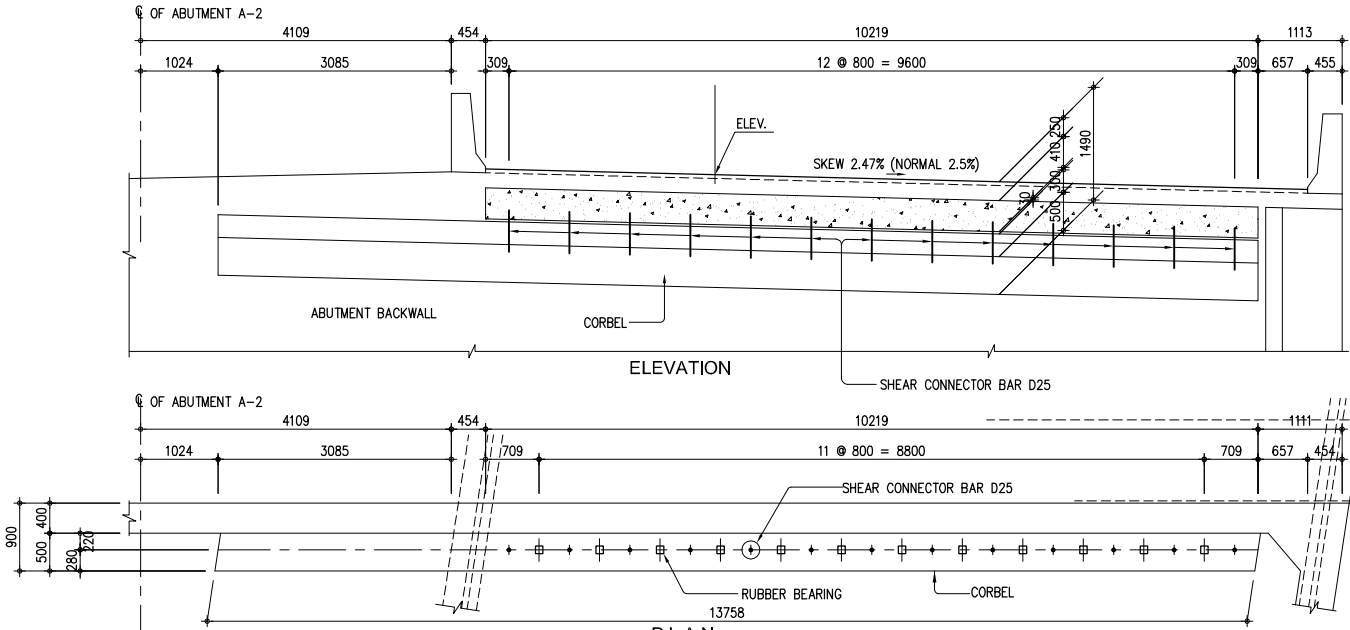
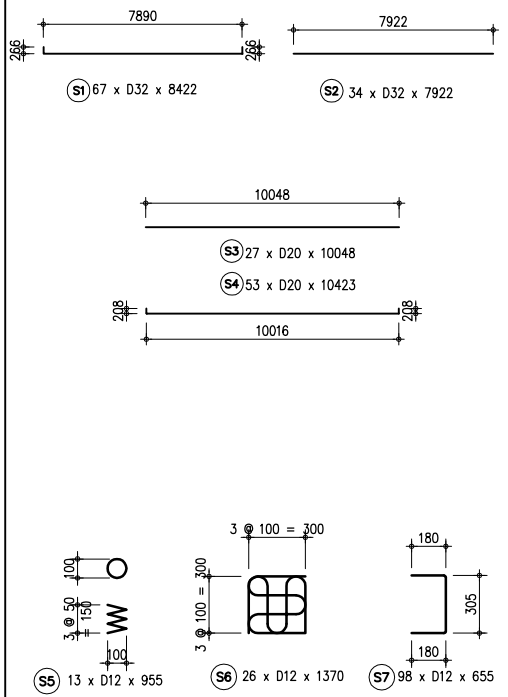
ESTIMATED QUANTITIES FOR PIER 9																	
REINFORCING BARS																	
LOCATION	BAR MARK	BAR SHAPE	SPACING	QTY. (pcs)	SIZE (mm)	DIMENSION mm ( OUT TO OUT )							LENGTH PER BAR (mm)	TOTAL LENGTH (m)	UNIT WEIGHT (kg/m)	TOTAL WEIGHT (kg)	CONCRETE VOLUME (m³)
						a	b	c	d	e	f	g					
PILE CAP	P1	01		218	32	1200	5600	1200					8000	1744.00	6.313	11,009.87	
	P1a	02		28	32	5600							5600	156.80	6.313	989.88	
	P2	03		23	25	1000	6500	12000	12000	5150	1000		37650	865.95	3.854	3,337.37	
	P2a	02		32	20	5000							5000	160.00	2.466	394.56	
	P3	01		218	25	1100	5600	1100					7800	1700.40	3.854	6,553.34	
	P4	03		23	25	1040	6460	12000	12000	5310	1040		37850	870.55	3.854	3,355.10	
	P5	03		16	20	1040	6460	12000	12000	5310	1040		37850	605.60	2.466	1,493.41	
	P5a	01		10	16	1040	5600	1040					7680	76.80	1.579	121.27	
	P6	08		416	16	1450	200						1850	769.60	1.579	1,215.20	
	P7	07		306	20	1450	600	250					3150	963.90	2.466	2,376.98	
SUB-TOTAL QUANTITY FOR PILE CAP															30,846.98	320.62	
WALL	W1	10		424	32	9200	500						9700	4112.80	6.313	25,964.11	
	W2	06		24	20	1080	6300	12000	12000	3550	1000	200	36360	872.64	2.466	2,151.93	
	W3	08		520	16	900	200						1300	676.00	1.579	1,067.40	
SUB-TOTAL QUANTITY FOR STEM															29,183.44	183.19	
COPING	C1	04		9	25	11600	6555	400	875		1250		19430	174.87	3.854	673.95	
	C1a	05		9	25	12000	6855	400			1250		19255	173.30	3.854	667.88	
	C2	07		128	20	1900	1590	1100					7280	913.84	2.466	2,297.92	
	C2a	07		6	20	1750	1590	1100					7130	42.78	2.466	105.50	
	C3	06		14	20	1830	7170	12000	12000	3630	1000	200	38860	544.04	2.466	1,341.60	
	C3a	11		4	20	1830	4900	12000	200		1000		18930	75.72	2.466	186.73	
	SUB-TOTAL QUANTITY FOR FOOTING															5,273.57	139.44
GRAND TOTAL FOR SUBSTRUCTURE PIER – 0															65,303.99	643.25	

# REINFORCEMENT OF APPROACH SLAB AT ABUTMENT A-2

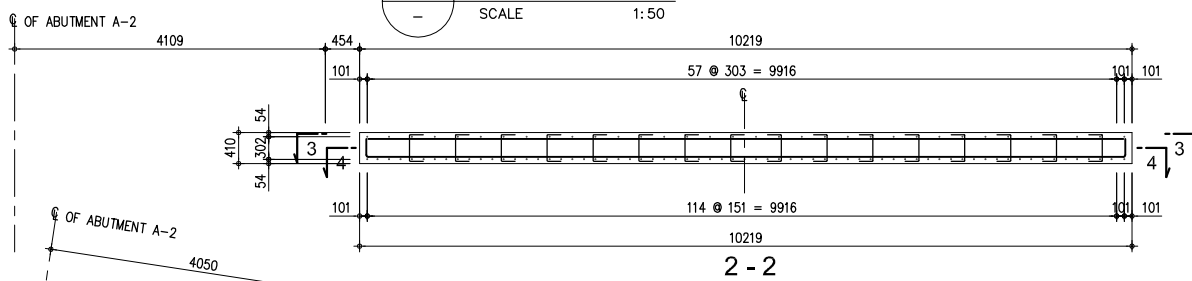
# BAR BENDING DIAGRAM



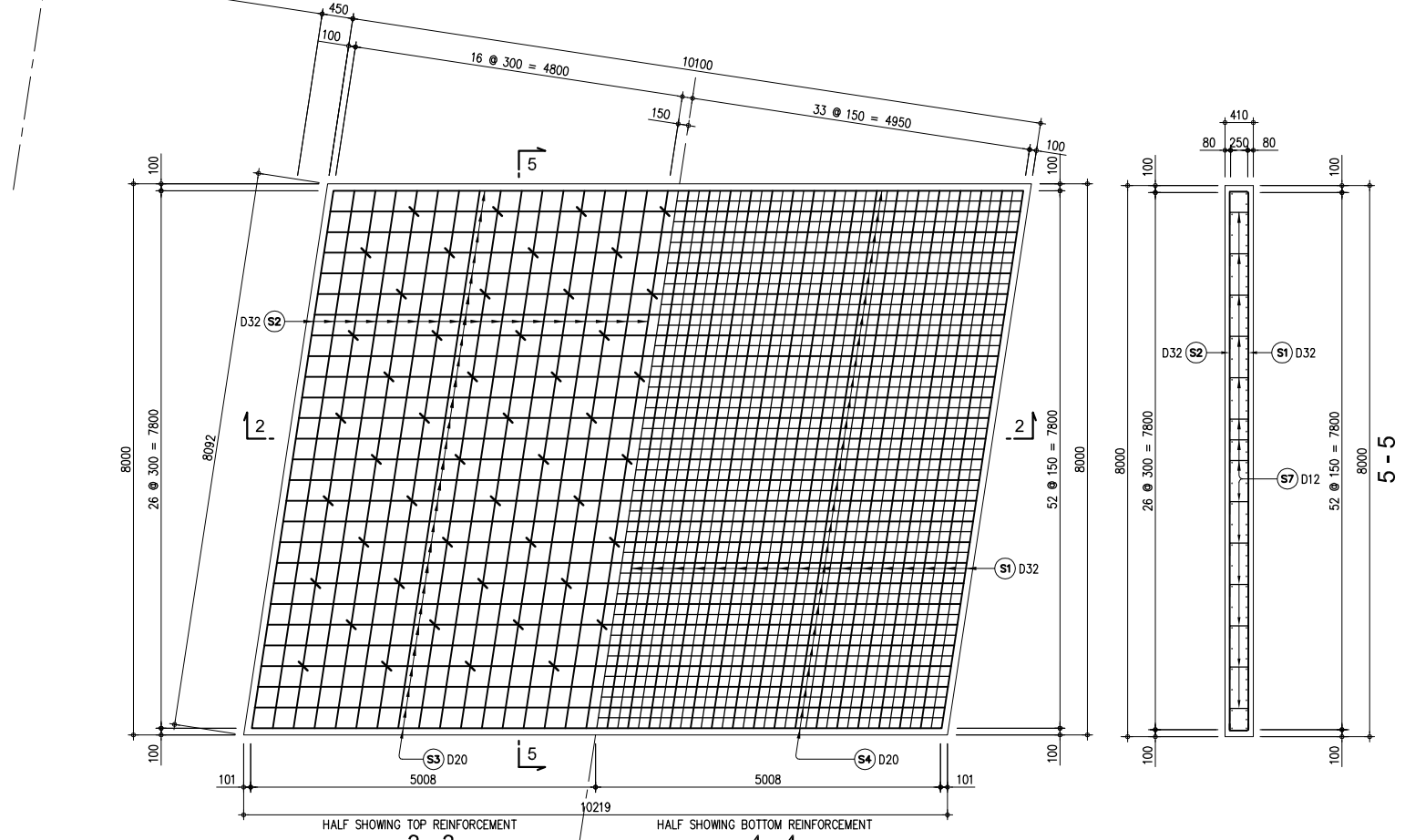
2  
TYPICAL SECTION  
SCALE 1:10



3  
CORBEL  
SCALE 1:50



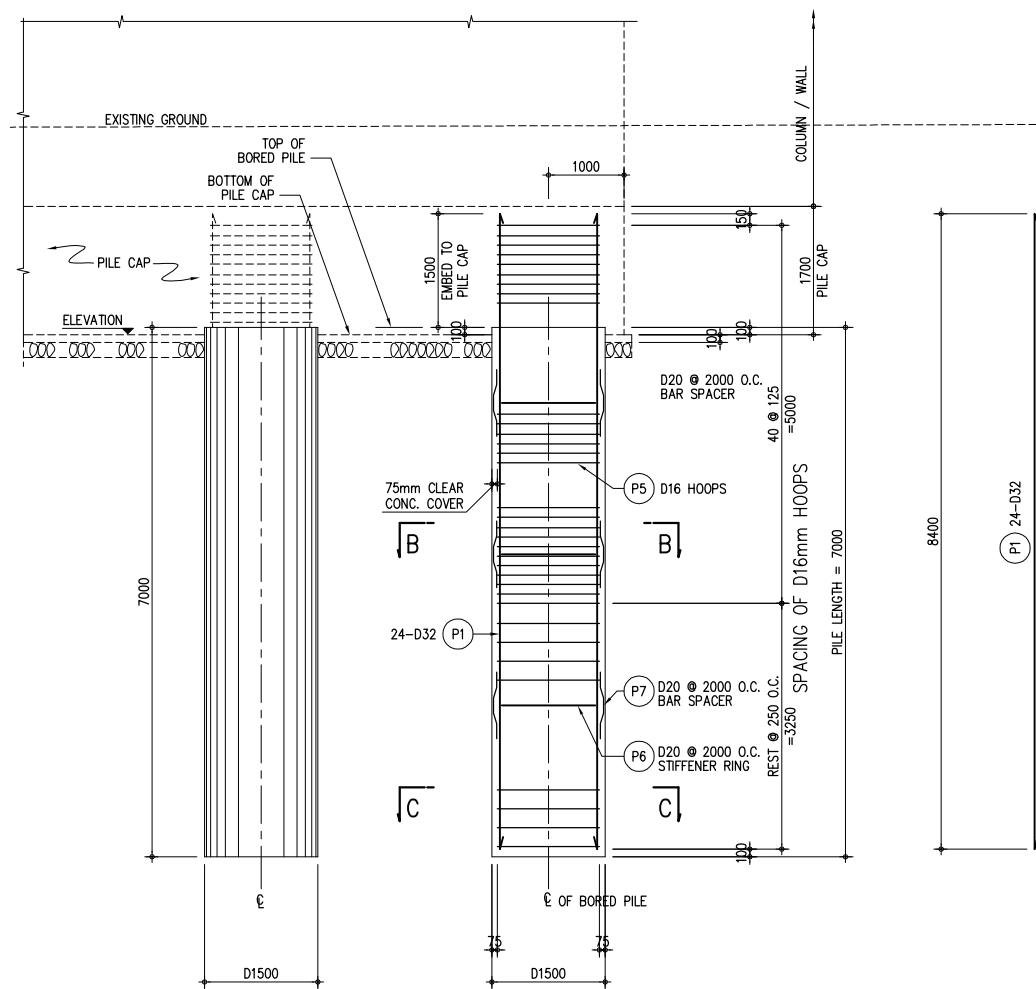
2 - 2



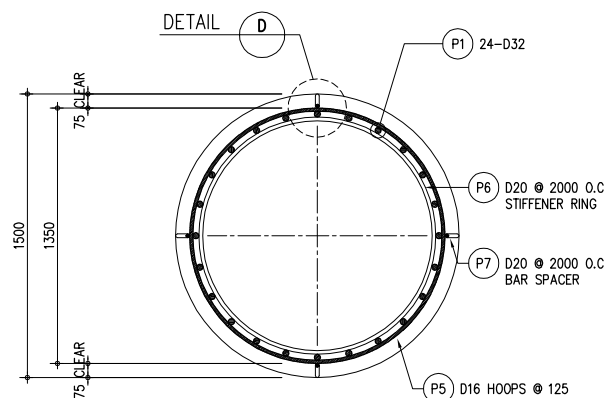
1  
REINFORCEMENT OF APPROACH SLAB  
SCALE 1:50

REINFORCING BARS												
BAR BENDING	BAR MARK	BAR SHAPE	QTY.	SIZE	DIMENSIONS			LENGTH PER BAR (mm)	TOTAL LENGTH (m)	UNIT WEIGHT (kg/m)	TOTAL WEIGHT (kg)	
					a	b	c					
<div><div><div><div><div></div><div>a</div><div>b</div></div></div><div><div><div>A</div></div></div><div><div><div>a</div></div></div><div><div><div>B</div></div></div><div><div><div>W</div></div></div><div><div><div>C</div></div></div><div><div><div>S</div></div></div><div><div><div>D</div></div></div></div></div>	S1	A	67	D32	7890	266	–	8422	564.27	6.314	3,562.83	
	S2	B	34	D32	7922	–	–	7922	269.35	6.314	1,700.66	
	S3	B	27	D20	10048	–	–	10048	271.30	2.466	669.02	
	S4	A	53	D20	10016	208	–	10432	552.90	2.466	1,363.44	
	S5	C	13	D12	955	–	–	955	12.42	0.888	11.02	
	S6	D	26	D12	1370	–	–	1370	35.62	0.888	31.63	
	S7	A	98	D12	305	180	180	665	65.17	0.888	57.87	
											D32	5,263.49
											D20	2,032.46
											D12	100.52
											TOTAL	7,396.47
	SGP	50A	–	–	–	–	–	–	–	–	–	GAS PIPE
	PL	G	–	–	–	–	–	–	–	–	–	CAP
	CONCRETE											
	FORM											
	BEARING (T=30mm)											
	JOINT FILLER (T=20mm)											
JOINT FILLER (T=30mm)												

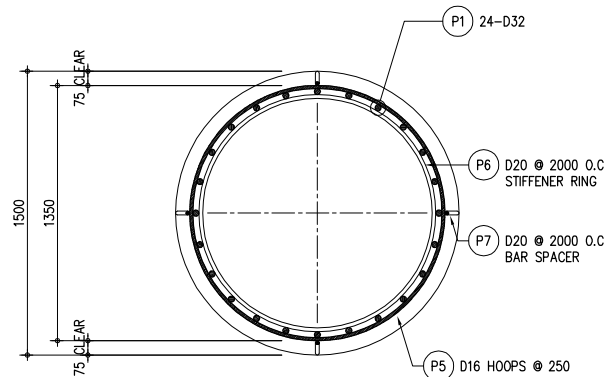
NOTE: QUANTITIES ARE FOR (1) ONE APPROACH ONLY.



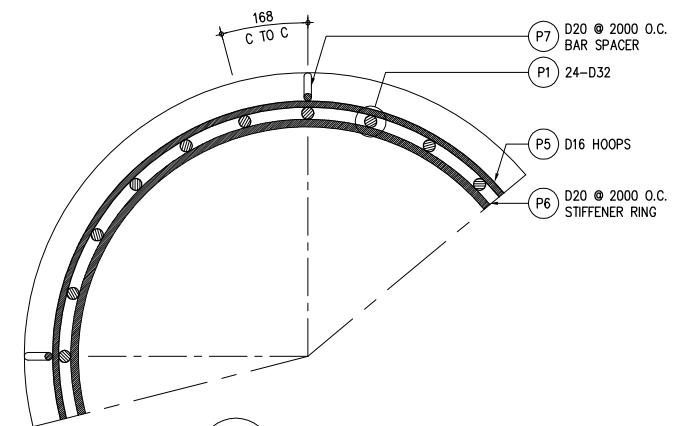
**A** TRANSVERSE ELEVATION  
SCALE 1:50



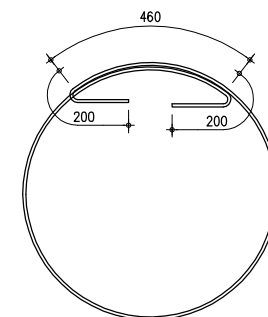
**B** SECTION  
SCALE 1:20







**C** SECTION  
SCALE 1:20

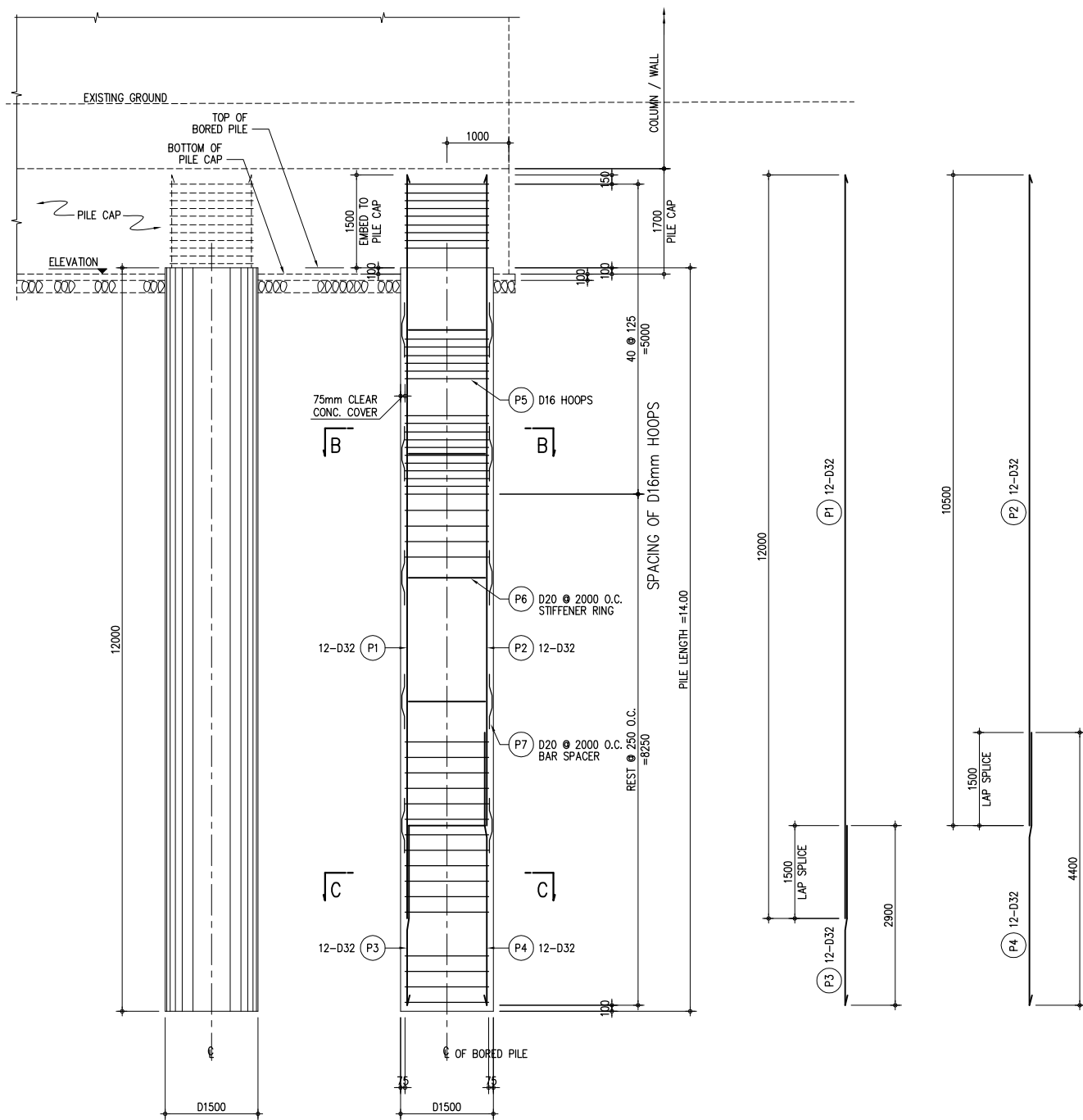


**D** DETAIL  
SCALE 1:10

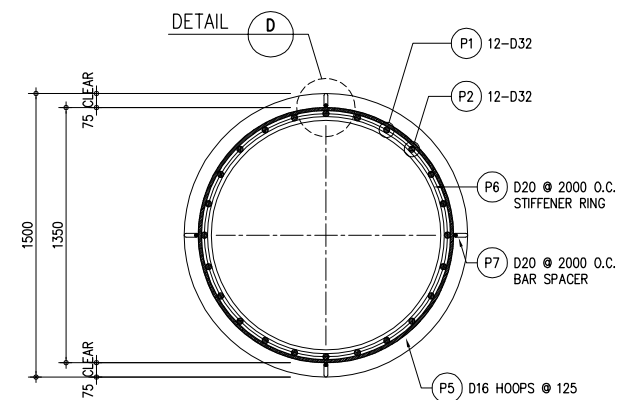


**E** HOOPS SPLICE DETAIL  
SCALE NONE

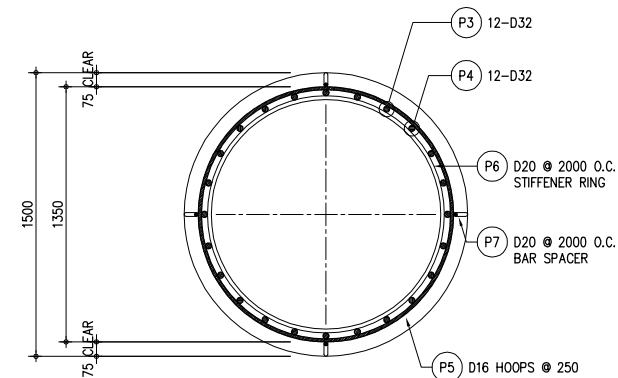
REINFORCING BARS															CONCRETE	
BAR BENDING	LOCATION	BAR MARK	BAR SHAPE	SPACING (mm)	QTY	SIZE	DIMENSION (mm)					LENGTH PER BAR (m)	TOTAL LENGTH (m)	UNIT WEIGHT (kg/m)	TOTAL WEIGHT (kg)	VOLUME (m³)
							a	b	c	d	e					
<div>   </div>	BORED PILE	P1	1	168	24	D32	8400	—	—	—	—	8.4	201.6	6.313	1272.70	12.37
		P2	1		0	D32	—	—	—	—	—	0	0	6.313	0.0	
		P3	1		0	D32	—	—	—	—	—	0	0	6.313	0.0	
		P4	1		0	D32	—	—	—	—	—	0	0	6.313	0.0	
		P5	2	125/250	49	D16	1350	460	200	—	—	5.10	249.9	1.579	394.6	
		P6	3	2000	3	D20	1254	800	—	—	—	4.74	14.22	2.466	35.07	
		P7	4	2000	28	D20	200	140	200	140	200	0.88	10.56	2.466	26.04	
															1728.41	



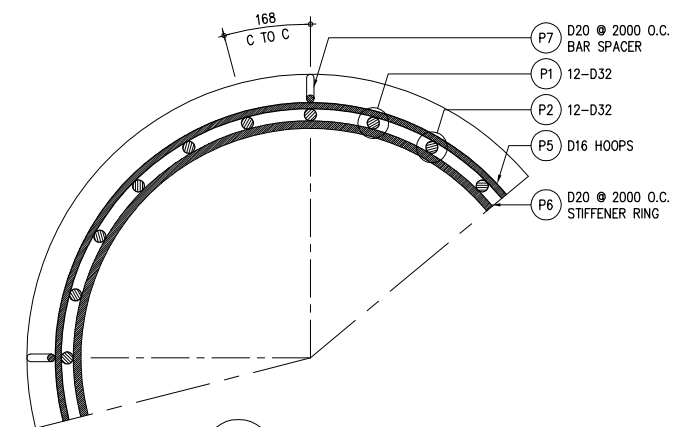
**A** TRANSVERSE ELEVATION  
SCALE 1: 50



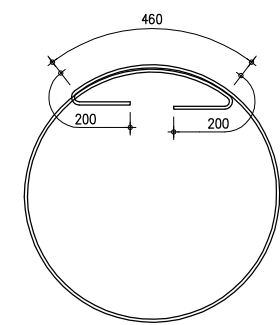
**B** SECTION  
SCALE 1: 20



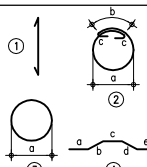
**C** SECTION  
SCALE 1: 20

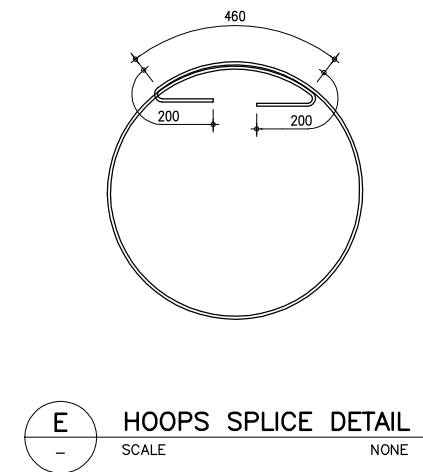
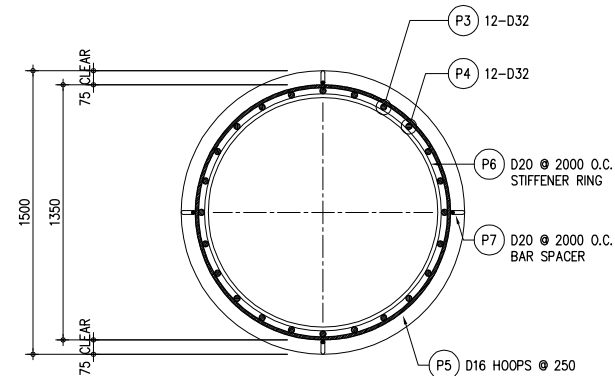
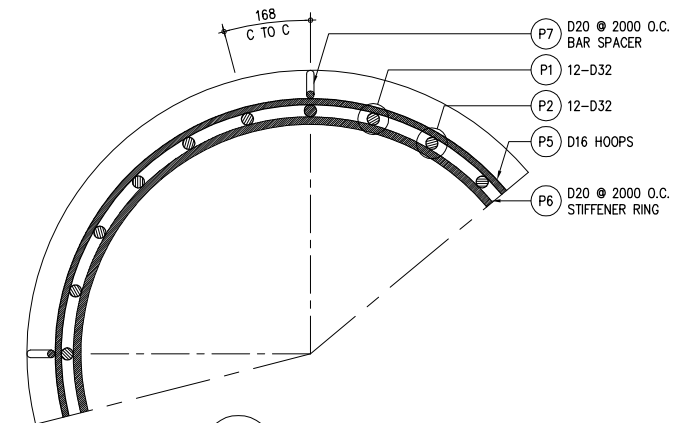
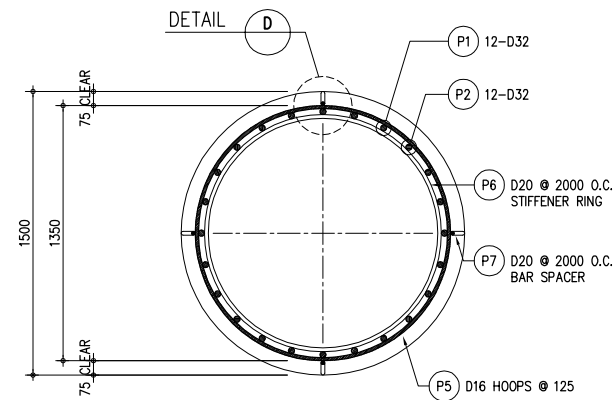
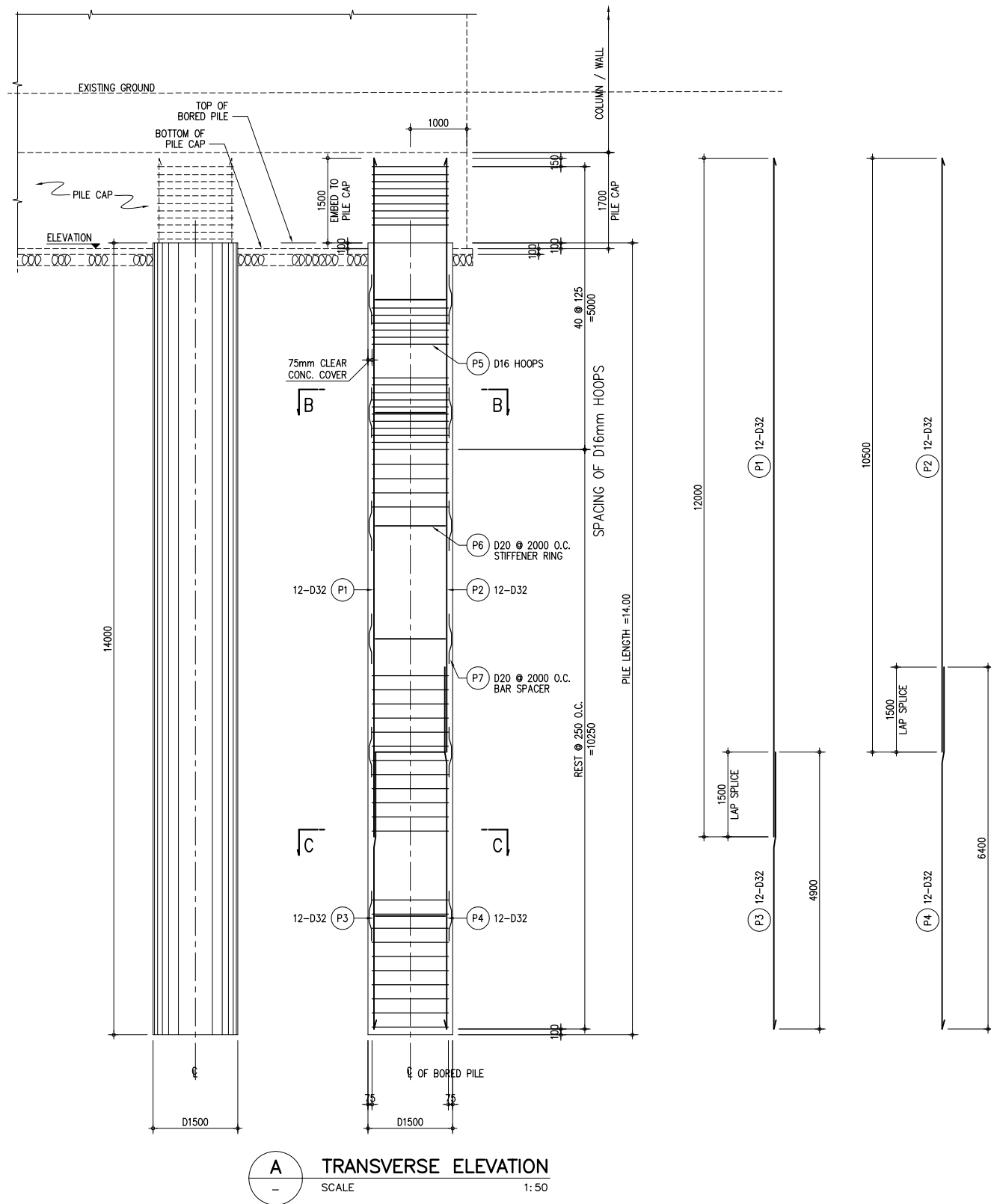


**D** DETAIL  
SCALE 1: 10

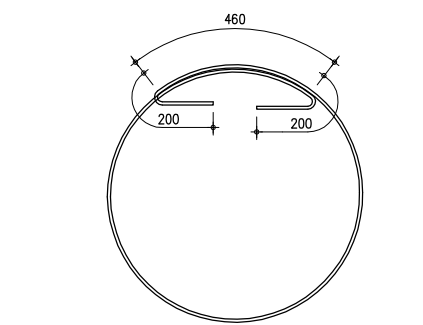
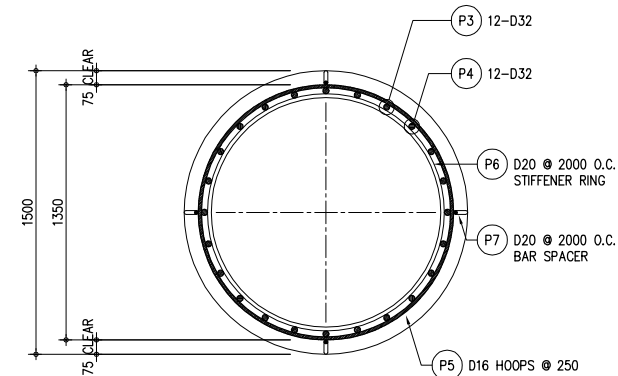
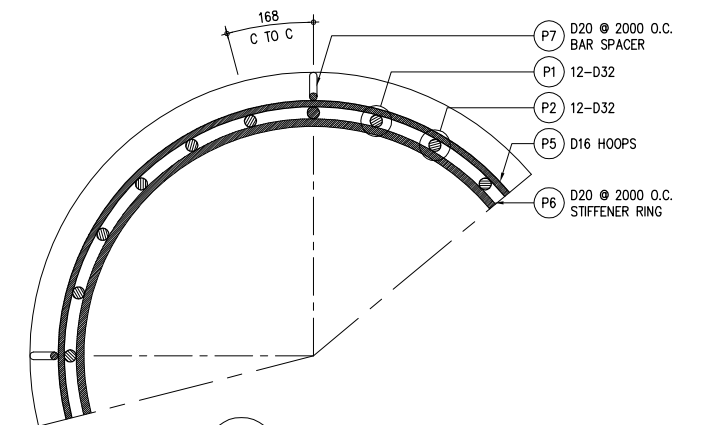
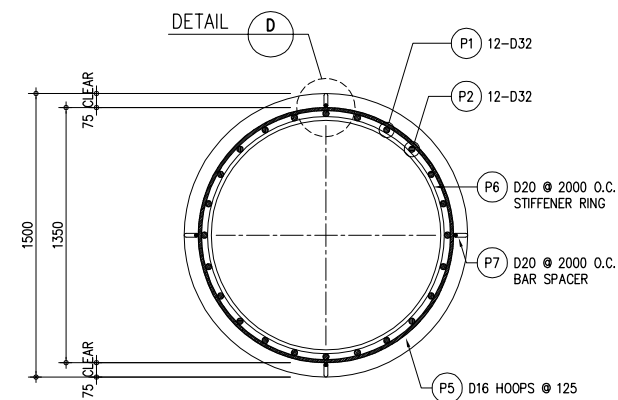
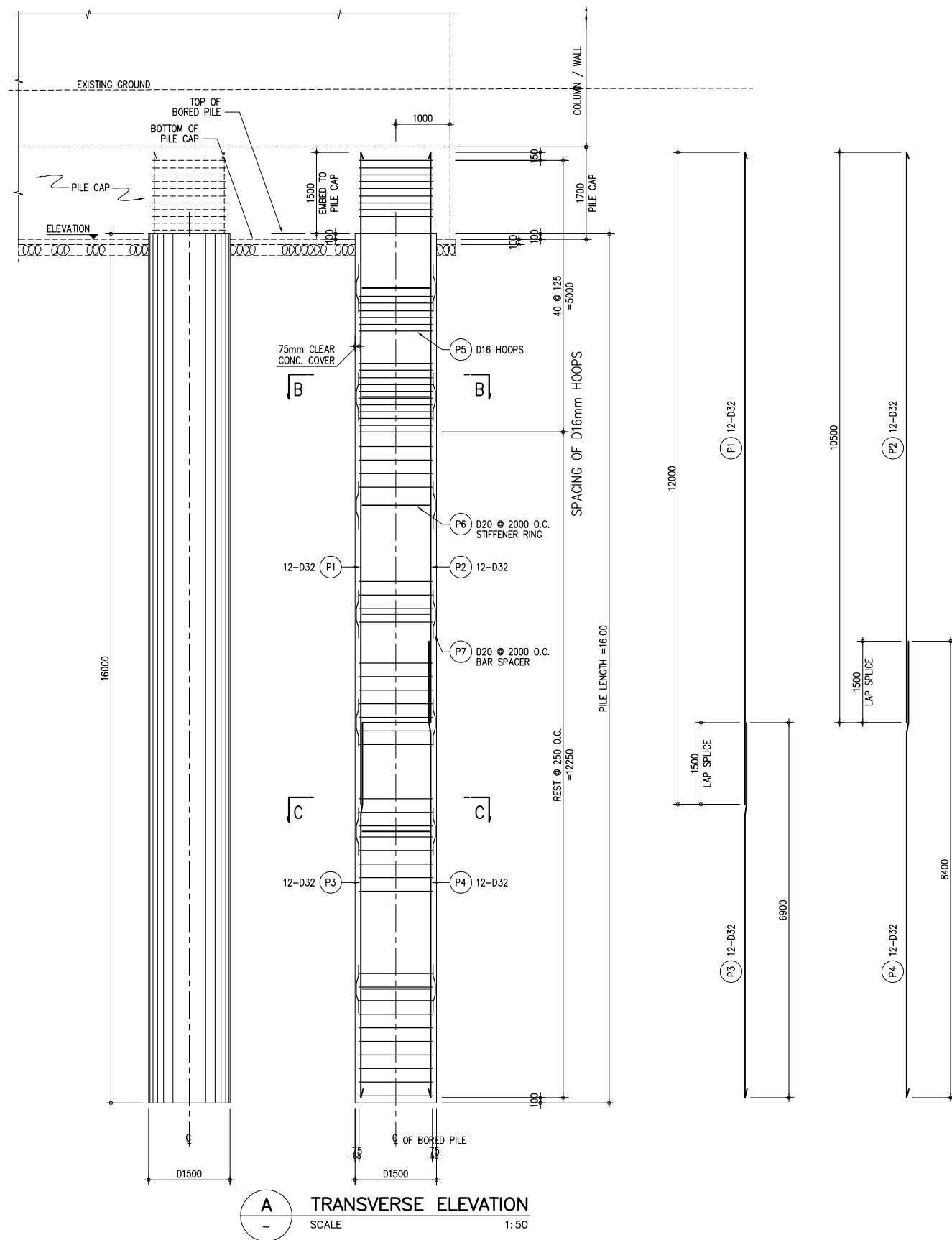





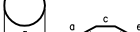
**E** HOOPS SPLICE DETAIL  
SCALE NONE

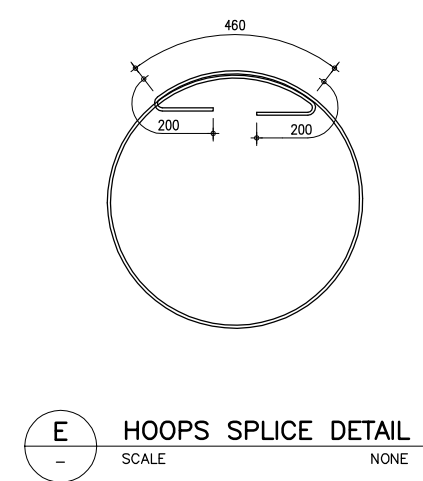
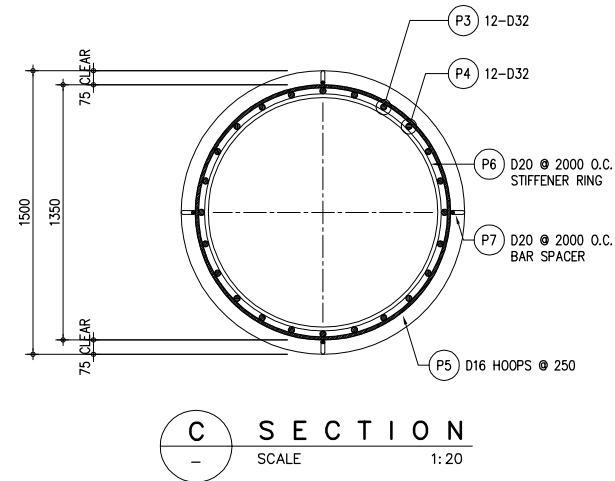
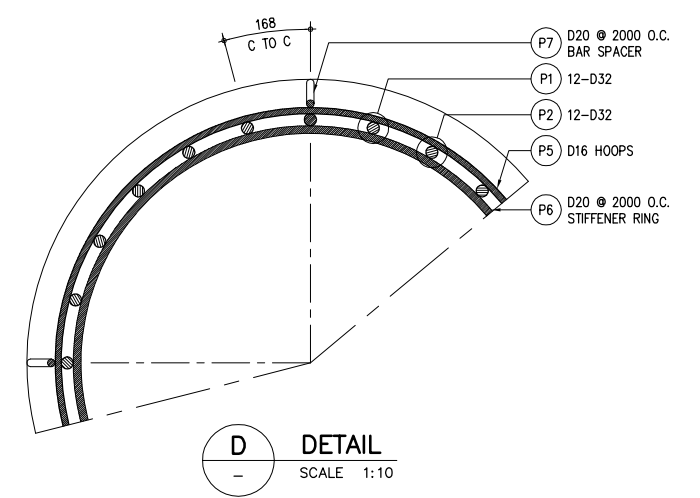
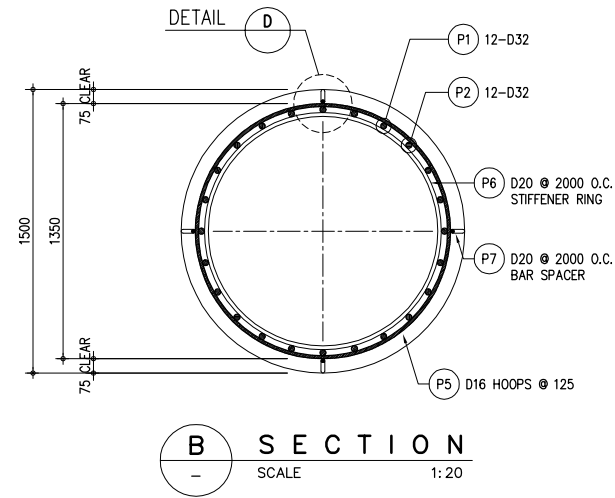
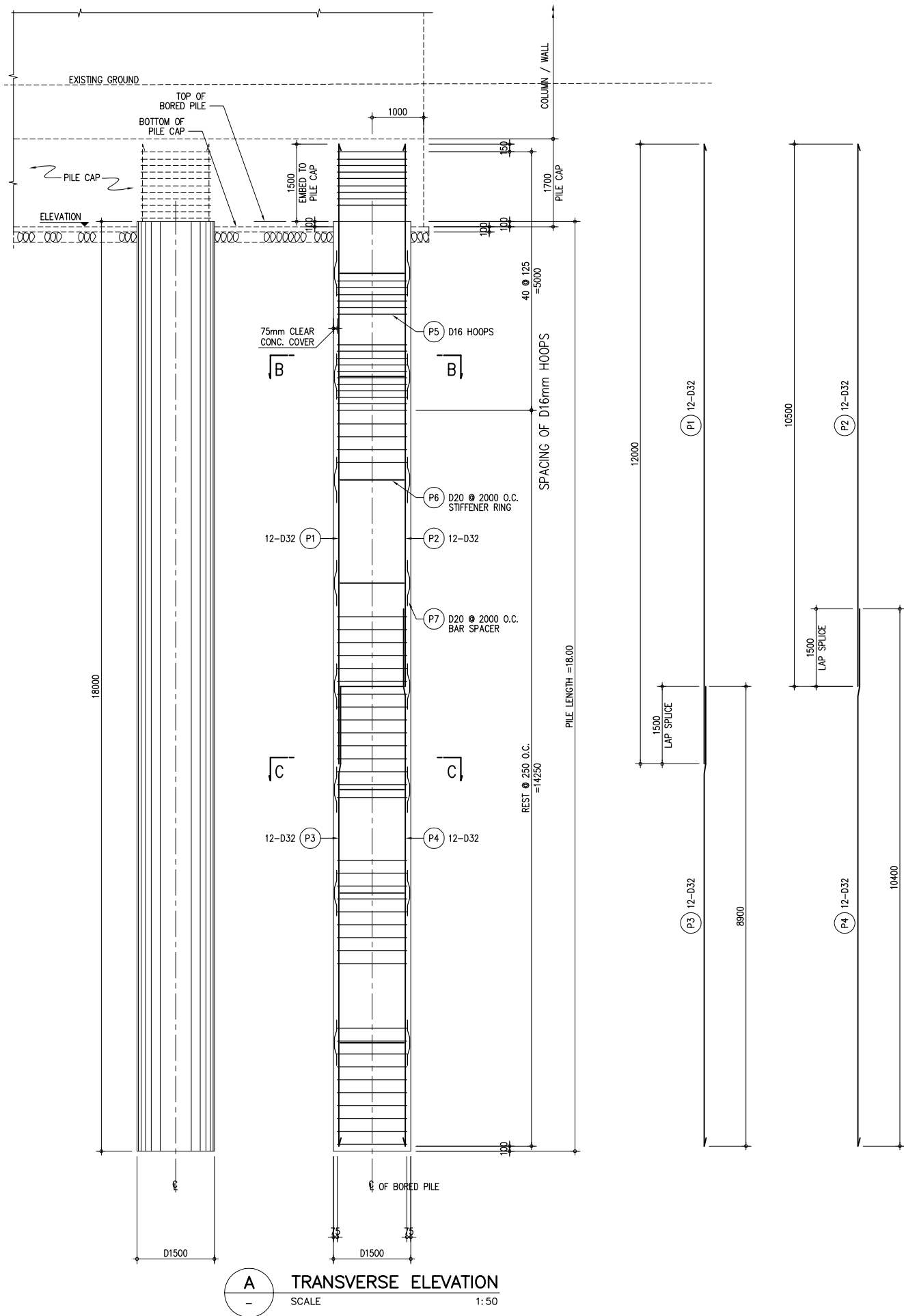
REINFORCING BARS															CONCRETE	
BAR BENDING	LOCATION	BAR MARK	BAR SHAPE	SPACING (mm)	QTY	SIZE	DIMENSION (mm)					LENGTH PER BAR (m)	TOTAL LENGTH (m)	UNIT WEIGHT (kg/m)	TOTAL WEIGHT (kg)	VOLUME (m³)
							a	b	c	d	e					
	BORED PILE	P1	1	168	12	D32	12000	–	–	–	–	12.00	144.00	6.313	909.07	21.21
		P2	1	168	12	D32	10500	–	–	–	–	10.50	126.00	6.313	795.44	
		P3	1	168	12	D32	2900	–	–	–	–	2.90	34.80	6.313	219.69	
		P4	1	168	12	D32	4400	–	–	–	–	4.40	52.80	6.313	333.33	
		P5	2	125/250	82	D16	1350	460	200	–	–	5.10	418.20	1.579	660.34	
		P6	3	2000	6	D20	1254	800	–	–	–	4.74	28.49	2.466	70.13	
		P7	4	2000	24	D20	200	140	200	140	200	0.88	21.12	2.466	52.08	
														3,040.08		

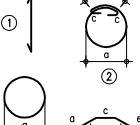


REINFORCING BARS															CONCRETE	
BAR BENDING	LOCATION	BAR MARK	BAR SHAPE	SPACING (mm)	QTY	SIZE	DIMENSION (mm)					LENGTH PER BAR (m)	TOTAL LENGTH (m)	UNIT WEIGHT (kg/m)	TOTAL WEIGHT (kg)	VOLUME (m³)
							a	b	c	d	e					
	BORED PILE	P1	1	168	12	D32	12000	—	—	—	—	12.00	144.00	6.313	909.07	24.74
		P2	1	168	12	D32	10500	—	—	—	—	10.50	126.00	6.313	795.44	
		P3	1	168	12	D32	4900	—	—	—	—	4.90	58.80	6.313	371.20	
		P4	1	168	12	D32	6400	—	—	—	—	6.40	76.80	6.313	484.84	
		P5	2	125/250	82	D16	1350	460	200	—	—	5.10	418.20	1.579	660.34	
		P6	3	2000	7	D20	1254	800	—	—	—	4.74	33.18	2.466	81.82	
		P7	4	2000	28	D20	200	140	200	140	200	0.88	24.64	2.466	60.76	
																3,363.47

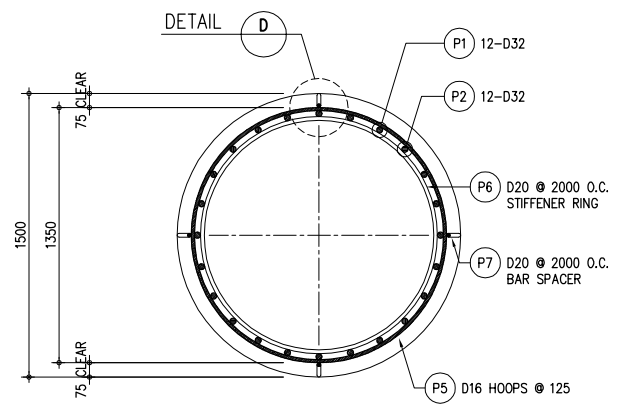
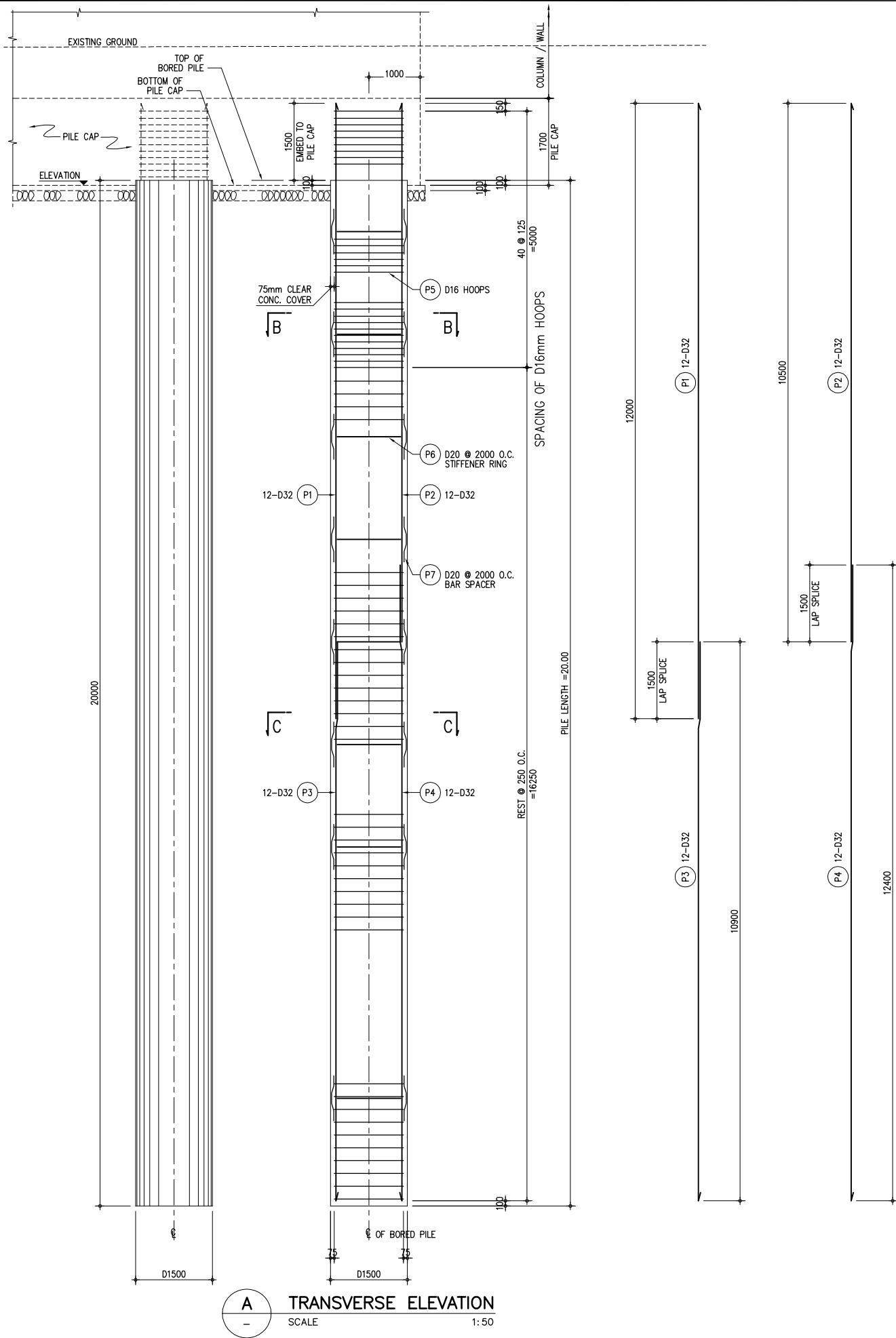


REINFORCING BARS															CONCRETE	
BAR BENDING	LOCATION	BAR MARK	BAR SHAPE	SPACING (mm)	QTY	SIZE	DIMENSION (mm)					LENGTH PER BAR (m)	TOTAL LENGTH (m)	UNIT WEIGHT (kg/m)	TOTAL WEIGHT (kg)	VOLUME (m³)
							a	b	c	d	e					
	BORED PILE	P1	1	168	12	D32	12000	—	—	—	—	12.00	144.00	6.313	909.07	28.27
		P2	1	168	12	D32	10500	—	—	—	—	10.50	126.00	6.313	795.44	
		P3	1	168	12	D32	8000	—	—	—	—	6.90	82.80	6.313	522.72	
		P4	1	168	12	D32	9500	—	—	—	—	8.40	100.80	6.313	636.35	
		P5	2	125/250	90	D16	1350	460	200	—	—	5.10	459.00	1.579	724.76	
		P6	3	2000	8	D20	1254	800	—	—	—	4.74	37.92	2.466	93.51	
		P7	4	2000	32	D20	200	140	200	140	200	0.88	28.16	2.466	69.44	
															3,751.29	

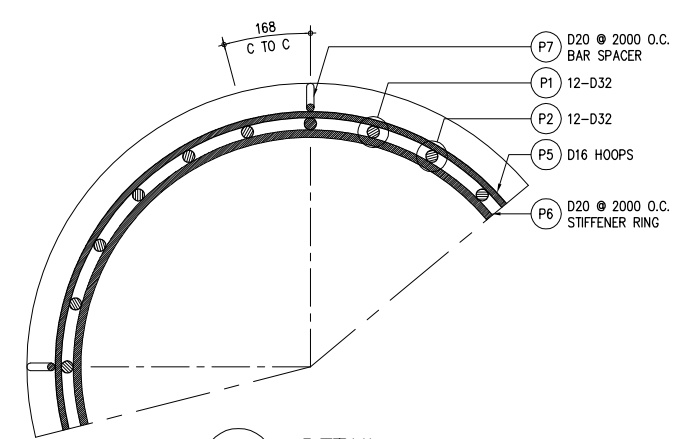


REINFORCING BARS															CONCRETE	
BAR BENDING	LOCATION	BAR MARK	BAR SHAPE	SPACING (mm)	QTY	SIZE	DIMENSION (mm)					LENGTH PER BAR (m)	TOTAL LENGTH (m)	UNIT WEIGHT (kg/m)	TOTAL WEIGHT (kg)	VOLUME (m³)
							a	b	c	d	e					
	BORED PILE	P1	1	168	12	D32	12000	—	—	—	—	12.00	144.00	6.313	909.07	31.81
		P2	1	168	12	D32	10500	—	—	—	—	10.50	126.00	6.313	795.44	
		P3	1	168	12	D32	8900	—	—	—	—	8.90	106.80	6.313	674.23	
		P4	1	168	12	D32	10400	—	—	—	—	10.40	124.80	6.313	787.86	
		P5	2	125/250	98	D16	1350	460	200	—	—	5.10	499.80	1.579	789.18	
		P6	3	2000	9	D20	1254	800	—	—	—	4.40	42.66	2.466	105.20	
		P7	4	2000	36	D20	200	140	200	140	200	0.88	31.68	2.466	78.12	
														4,139.10		

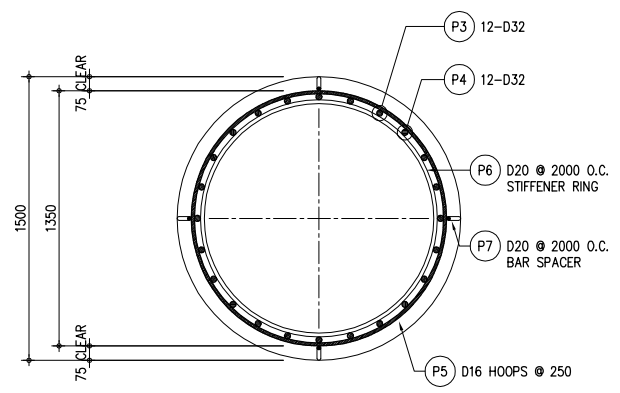




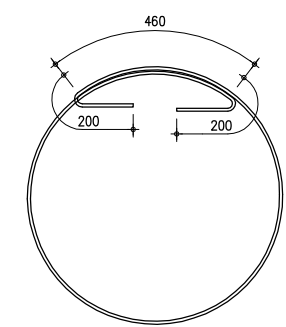
**B SECTION**  
SCALE 1: 20




**D DETAIL**  
SCALE 1: 10

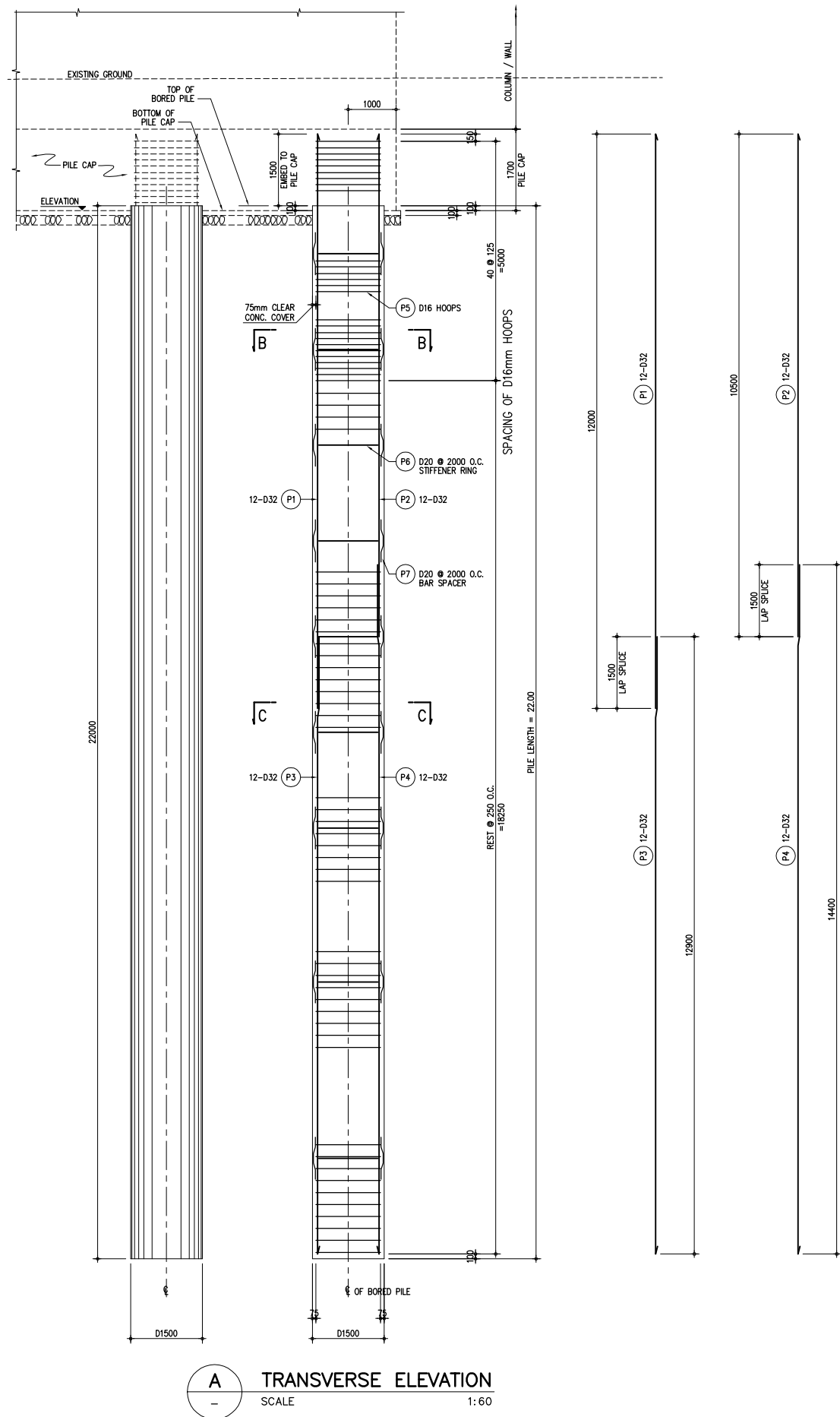


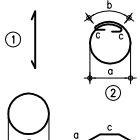
**C SECTION**  
SCALE 1: 20

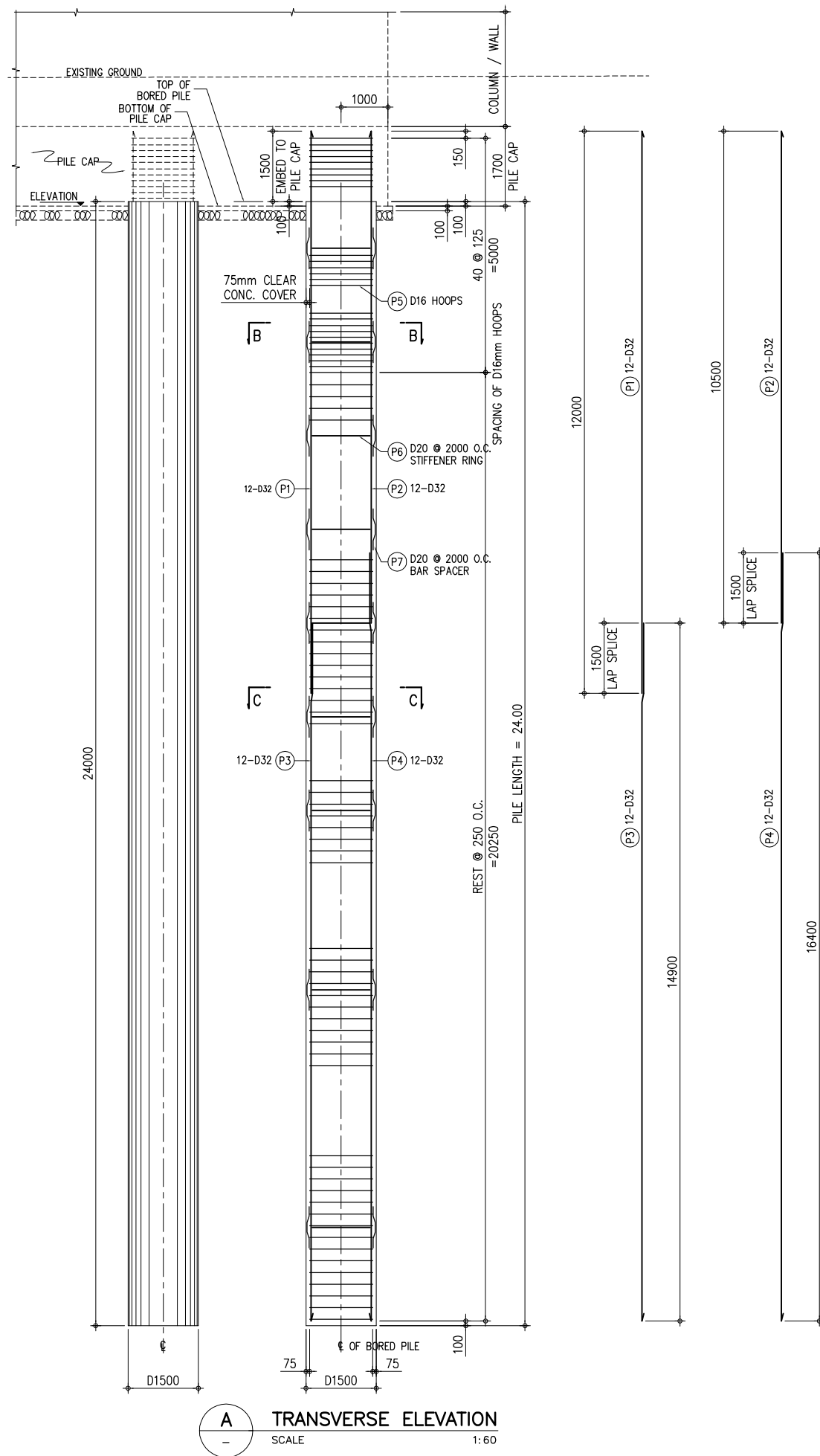


**E HOOPS SPLICE DETAIL**  
SCALE NONE

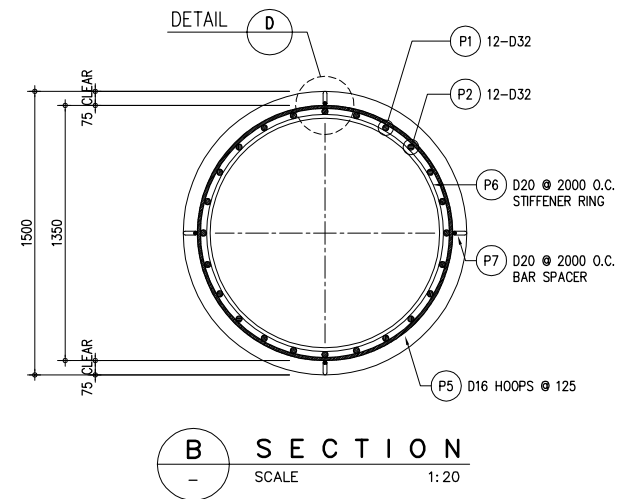
REINFORCING BARS															CONCRETE	
BAR BENDING	LOCATION	BAR MARK	BAR SHAPE	SPACING (mm)	QTY	SIZE	DIMENSION (mm)					LENGTH PER BAR (m)	TOTAL LENGTH (m)	UNIT WEIGHT (kg/m)	TOTAL WEIGHT (kg)	VOLUME (m³)
							a	b	c	d	e					
	BORED PILE	P1	1	168	12	D32	12000	–	–	–	–	12.00	144.00	6.313	909.07	35.34
		P2	1	168	12	D32	10900	–	–	–	–	10.90	130.80	6.313	825.74	
		P3	1	168	12	D32	10900	–	–	–	–	10.90	130.80	6.313	825.74	
		P4	1	168	12	D32	12000	–	–	–	–	12.00	144.00	6.313	909.07	
		P5	2	125/250	106	D16	1350	460	200	–	–	5.10	540.60	1.579	853.61	
		P6	3	2000	10	D20	1254	800	–	–	–	4.74	47.40	2.466	116.89	
		P7	4	2000	40	D20	200	140	200	140	200	0.88	35.20	2.466	86.80	
															4,526.92	



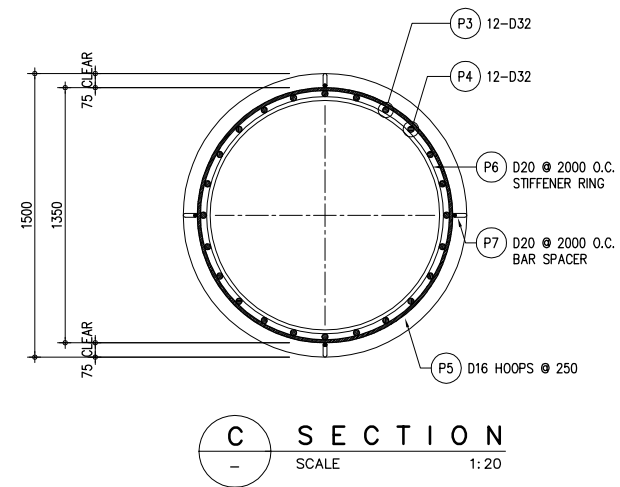
REINFORCING BARS															CONCRETE	
BAR BENDING	LOCATION	BAR MARK	BAR SHAPE	SPACING (mm)	QTY	SIZE	DIMENSION (mm)					LENGTH PER BAR (m)	TOTAL LENGTH (m)	UNIT WEIGHT (kg/m)	TOTAL WEIGHT (kg)	VOLUME (m <sup>3</sup> )
							a	b	c	d	e					
	BORED PILE	P1	1	168	12	D32	7200	—	—	—	—	7.20	86.40	6.313	545.44	38.87
		P2	1	168	12	D32	7950	—	—	—	—	7.95	95.40	6.313	602.26	
		P3	1	168	12	D32	12000	—	—	—	—	12.00	144.00	6.313	909.07	
		P4	1	168	12	D32	10500	—	—	—	—	10.50	126.00	6.313	795.44	
		P5	2	125/250	114	D16	1350	460	200	—	—	5.10	581.40	1.579	918.03	
		P6	3	2000	11	D20	1254	800	—	—	—	4.40	52.14	2.466	128.58	
		P7	4	2000	44	D20	200	140	200	140	200	0.88	38.72	2.466	95.48	
														3,994.30		



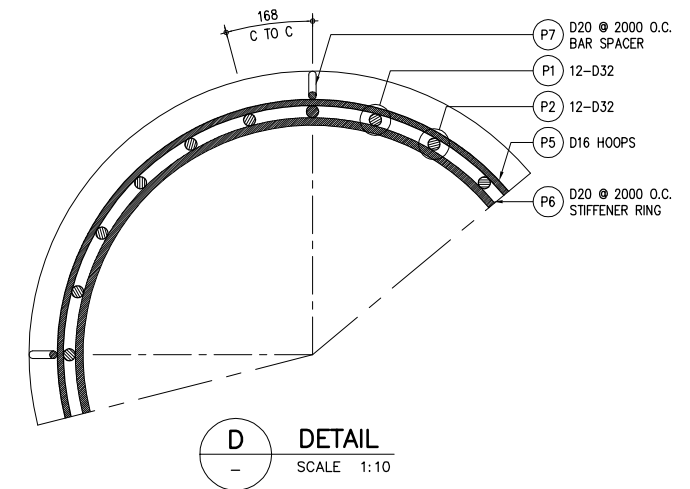
**A** TRANSVERSE ELEVATION  
SCALE 1:60



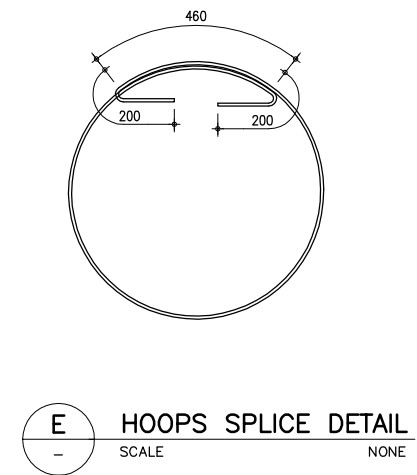
**B** SECTION  
SCALE 1:20



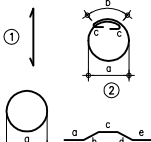
**C** SECTION  
SCALE 1:20

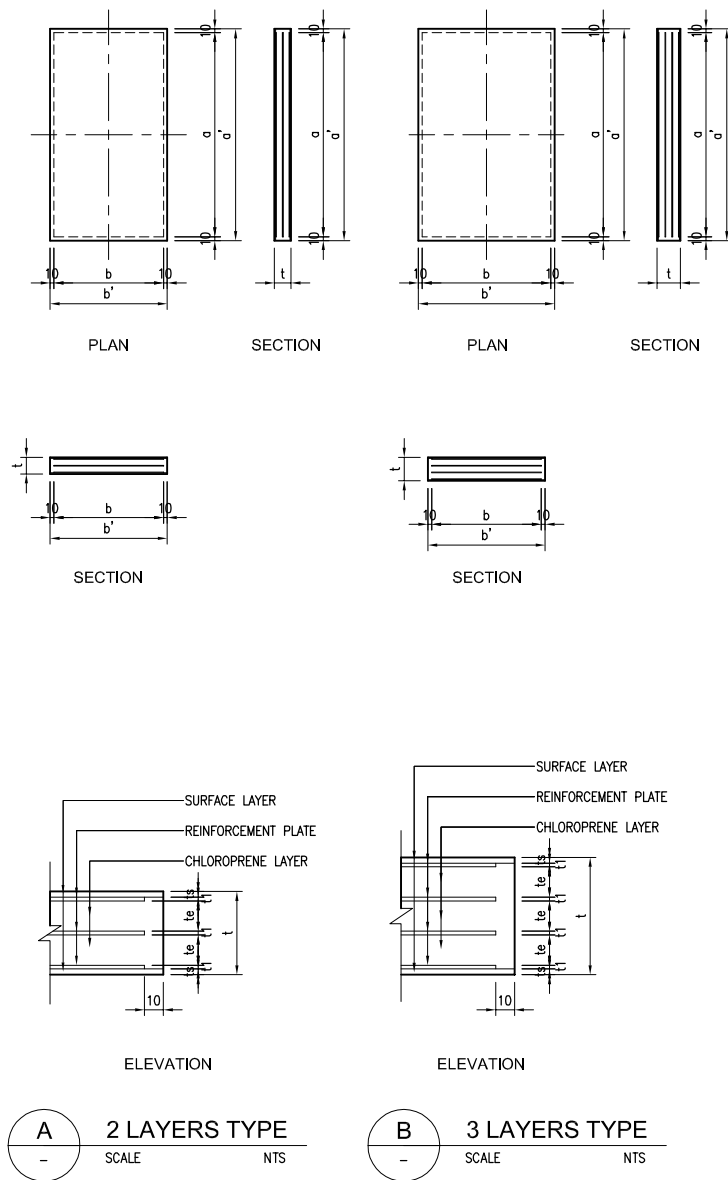
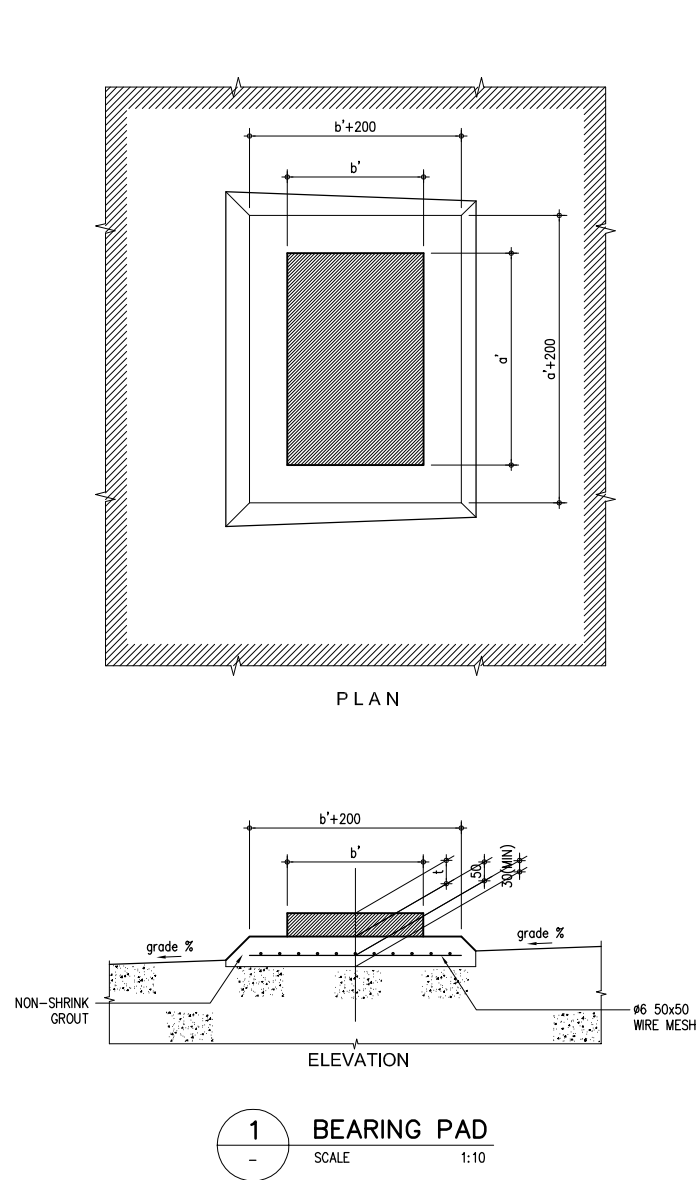


**D** DETAIL  
SCALE 1:10



**E** HOOPS SPLICE DETAIL  
SCALE NONE

REINFORCING BARS															CONCRETE	
BAR BENDING	LOCATION	BAR MARK	BAR SHAPE	SPACING (mm)	QTY	SIZE	DIMENSION (mm)					LENGTH PER BAR (m)	TOTAL LENGTH (m)	UNIT WEIGHT (kg/m)	TOTAL WEIGHT (kg)	VOLUME (m³)
							a	b	c	d	e					
	BORED PILE	P1	1	168	12	D32	8200	—	—	—	—	8.20	98.40	6.313	621.20	42.41
		P2	1	168	12	D32	8950	—	—	—	—	8.95	107.40	6.313	678.02	
		P3	1	168	12	D32	12000	—	—	—	—	12.00	144.00	6.313	909.07	
		P4	1	168	12	D32	10500	—	—	—	—	10.50	126.00	6.313	795.44	
		P5	2	125/250	122	D16	1350	460	200	—	—	5.10	622.20	1.579	982.45	
		P6	3	2000	12	D20	1254	800	—	—	—	4.40	56.88	2.466	140.27	
		P7	4	2000	48	D20	200	140	200	140	200	0.88	42.24	2.466	104.16	
															4,230.61	



#### DIMENSIONS OF BEARING PAD

TYPE	Dimensions a' x b' x t (mm)	Chloroprene Layers		Reinforcement Plate				Surface Layer
		Thickness of Layer te (mm)	Number of Layers ne	Transversal Width a (mm)	Longitudinal Width b (mm)	Thickness & Number		Thickness & Number
						t1 x n (mm)	t2 x n (mm)	ts x n (mm)
Fix	560 x 310 x 44	16	2	540	290	2 x 3	-	3 x 2
Move	560 x 360 x 68	16	3	540	340	2 x 4	-	3 x 2

#### ELASTOMERIC MATERIAL (POLY-CHLOROPRENE)

ITEM	UNIT	REQUIRED VALUE	TEST METHOD
Static Shearing Elasticity Modulus	kgf/cm <sup>2</sup>	10 ± 1	JIS K6254
Hardness	-	A60 ± 5	JIS K6253
Elongation	%	440 or more	JIS K6251
Tensile Strength	Kg/cm <sup>2</sup>	150 or more	JIS K6251
Fatigue Test	Strength Changing Ratio for 25 % Elongation	%	between -10 and +100
	Elongation Ratio	%	-50 or more
	Ratio of Compressive Permanent Strain	%	35 or less
Ozone Deterioration	-	No crack to be observed by naked eye	JIS K6259
Moisture Absorption (Mass Change Ratio Due to Water)	%	10 or less	JIS K6258
Low Temperature Resistance	Degree	-30°C or less	JIS K6260
Resistance to Stripping	kgf/cm	7 or more	JIS K6256

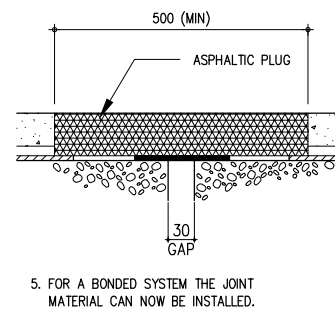
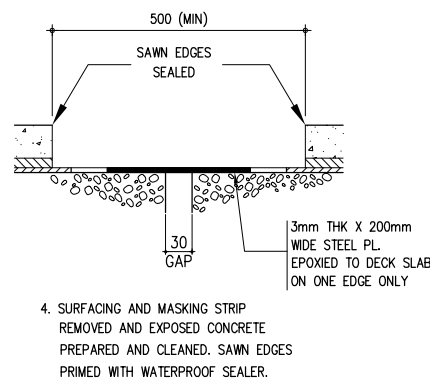
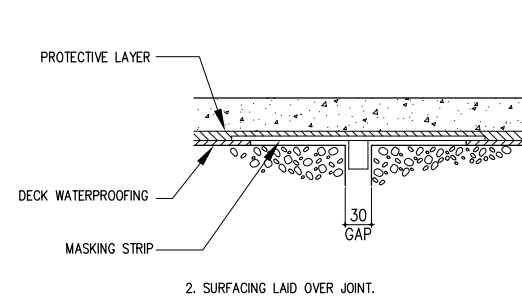
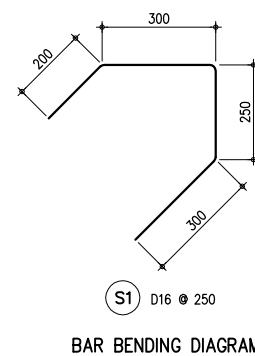
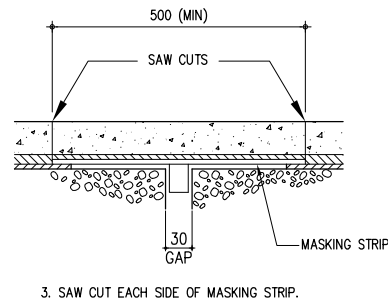
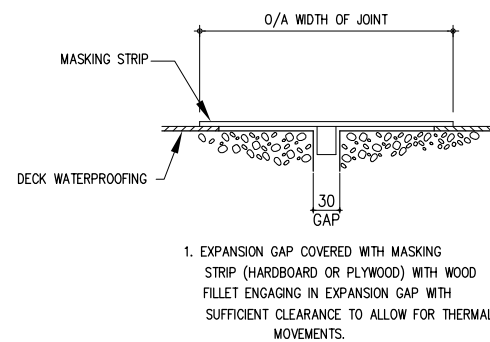
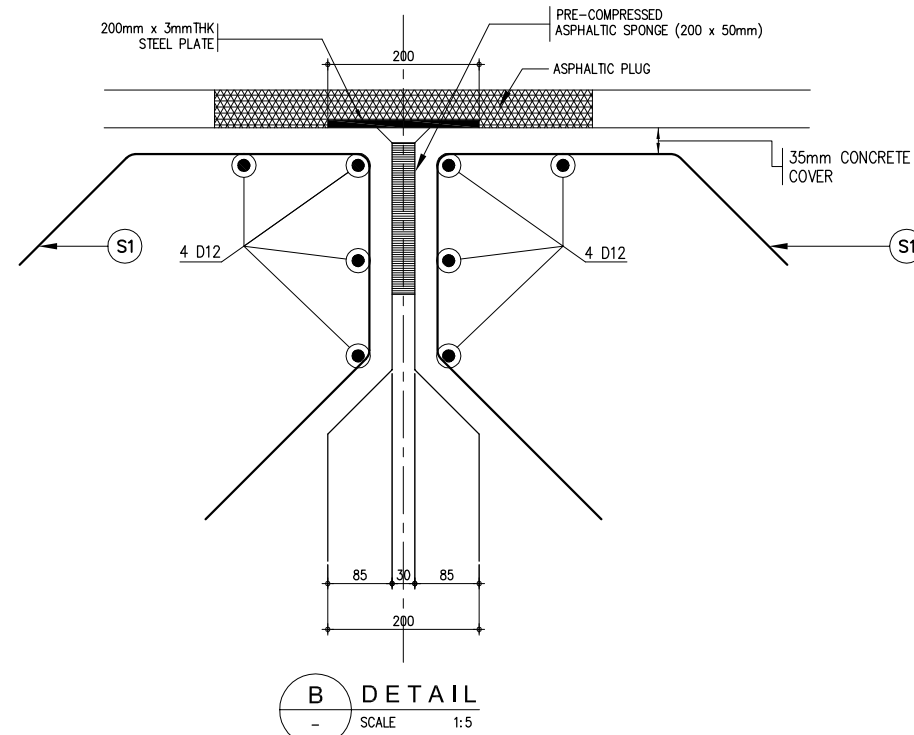
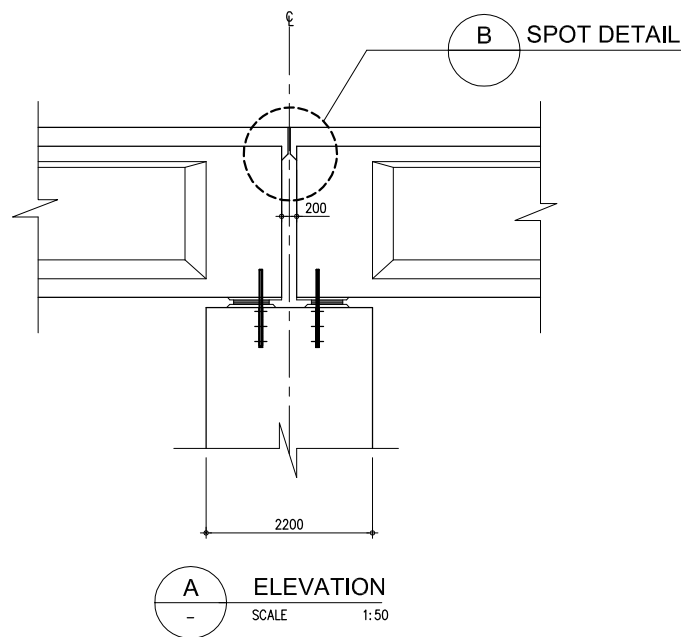
#### REINFORCEMENT PLATE (STEEL PLATE)

ITEM	UNIT	REQUIRED VALUE	TEST METHOD
Ultimate Strength	N/mm <sup>2</sup>	400 or more	JIS G3101
Elongation	%	21 or more	
Yield Strength	N/mm <sup>2</sup>	245 or more	

Note: All Elastomeric Bearing Pads shall be designed in accordance with Japan Road Association Standard or other equivalent Standard

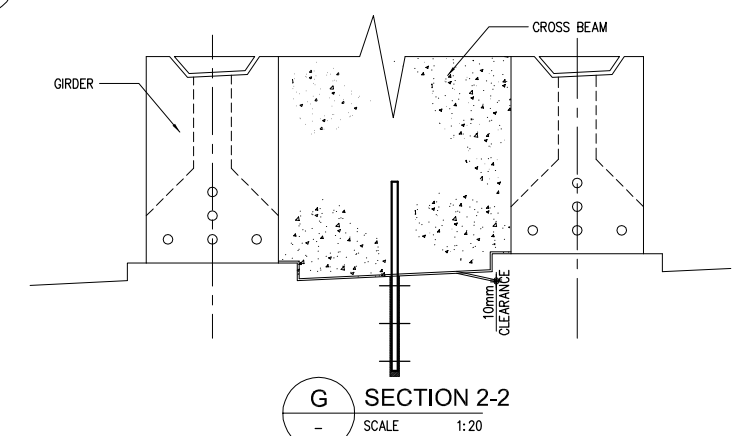
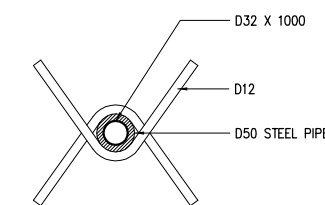
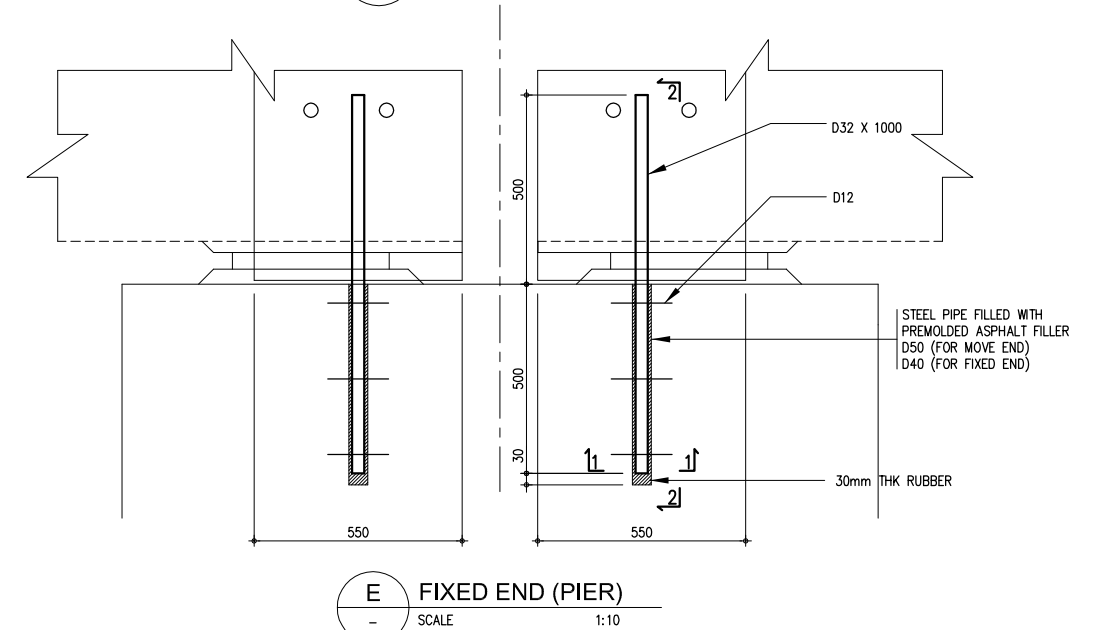
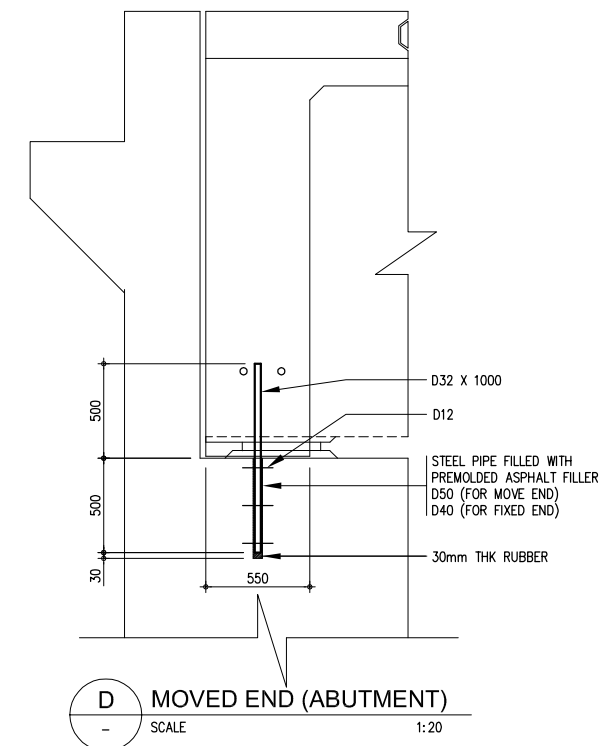
#### LOCATIONS OF ELASTOMERIC BEARING PADS

BRIDGE NO.			HB 09 (KELANI)											Total Number
BRIDGE TYPE			PC-I											
LOCATION			P0	P1	P2	P3	P4	P5	P6	P7	P8	P9	A2	
SUPPORT CONDITION			M	F	F	M	F	F	F	M	F	F	M	
ELASTOMERIC BEARING PAD	FIX	560 x 310 x 44		24	24		24	24	24		24	24		168
	MOVE	560 x 360 x 68	12			12				12			12	48

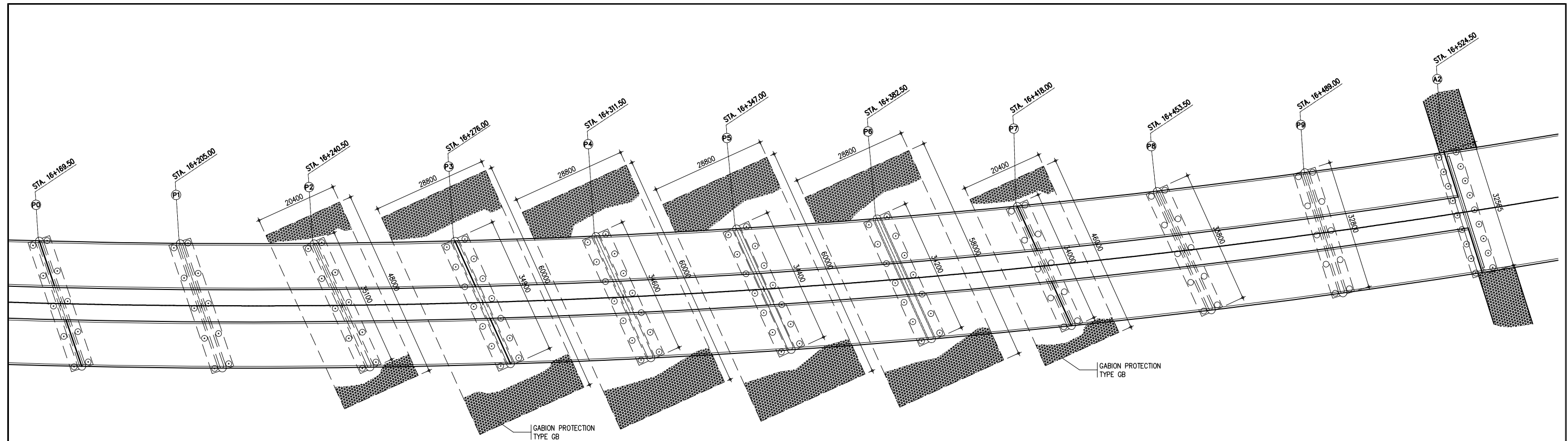


**C TYPICAL INSTALLATION PROCEDURE**  
SCALE N T S

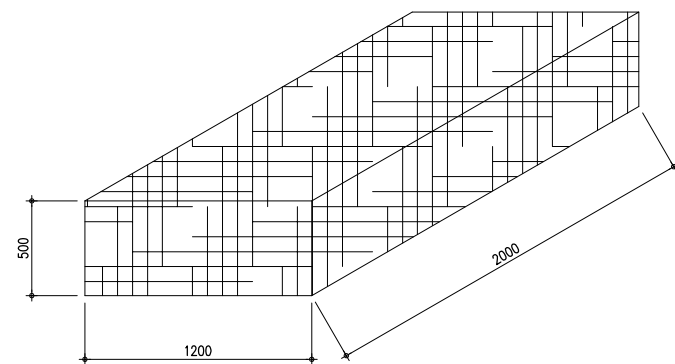
**1 DETAIL OF EXPANSION JOINT**  
SCALE AS SHOWN



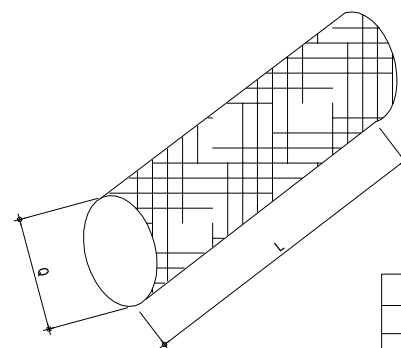
**2 ANCHORAGE DETAIL TYPICAL (MOVED AND FIXED)**  
SCALE AS SHOWN



1 PLAN (KELANI RIVER CROSSING BRIDGE)  
SCALE 1:500

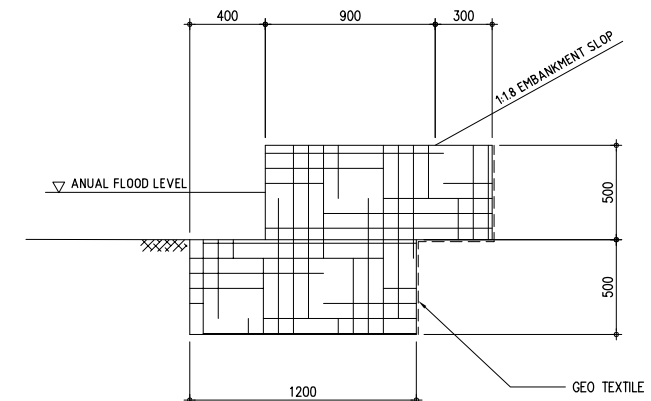


A GABIONS BOX (TYPE "GB")  
SCALE N T S

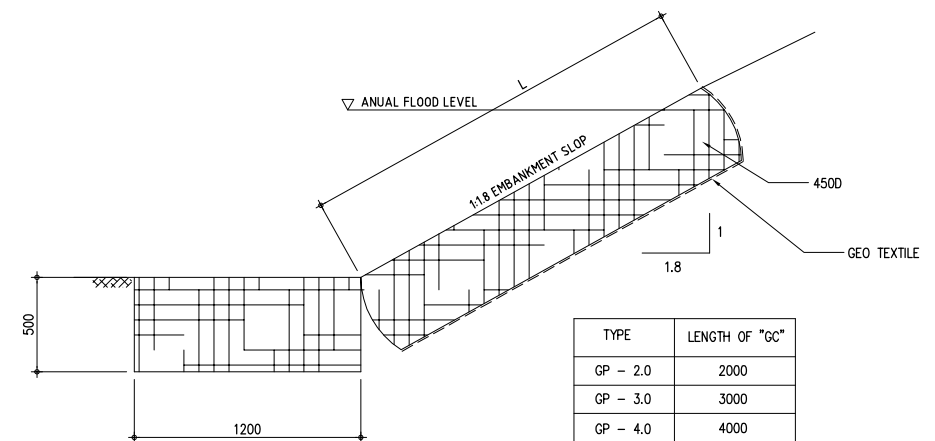


B GABIONS CYLINDER (TYPE "GC-L")  
SCALE N T S

TYPE	Q
GCa	450
Gcb	600



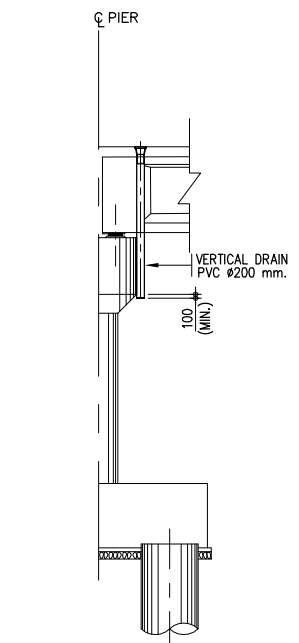
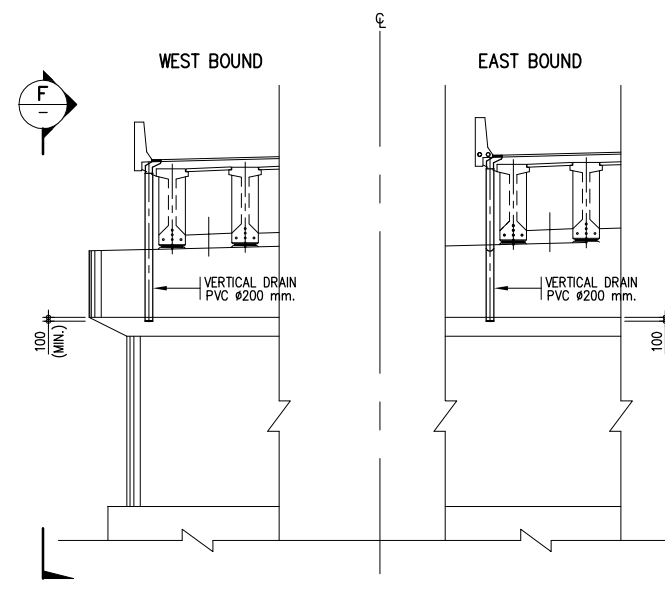
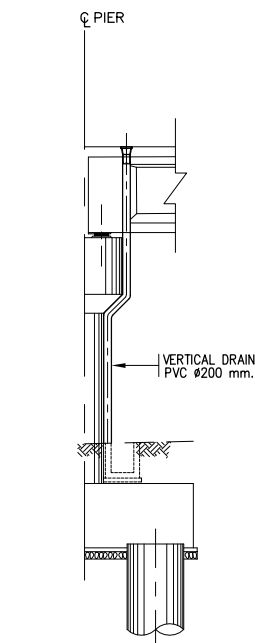
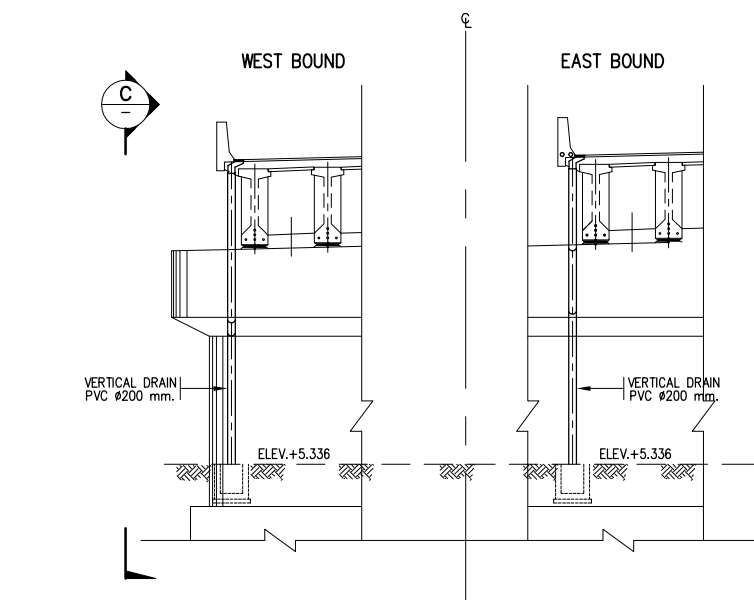
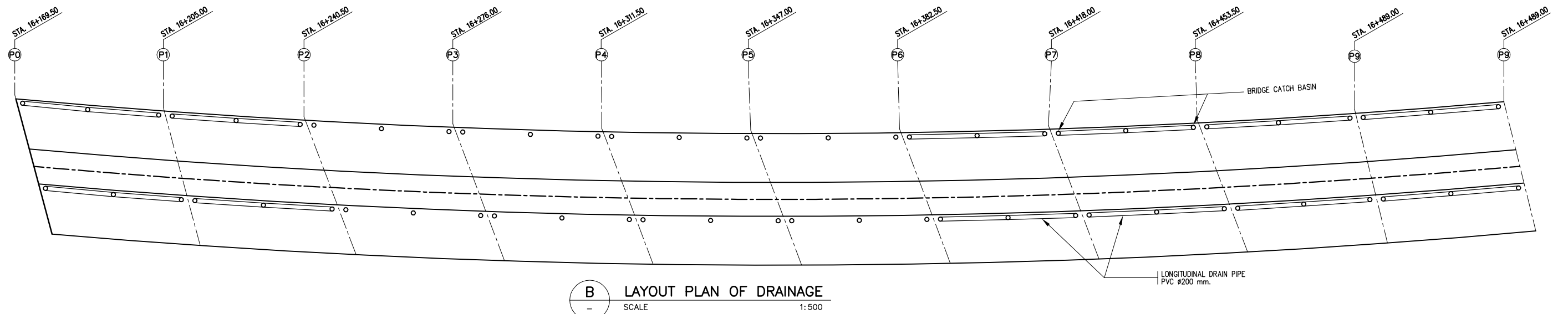
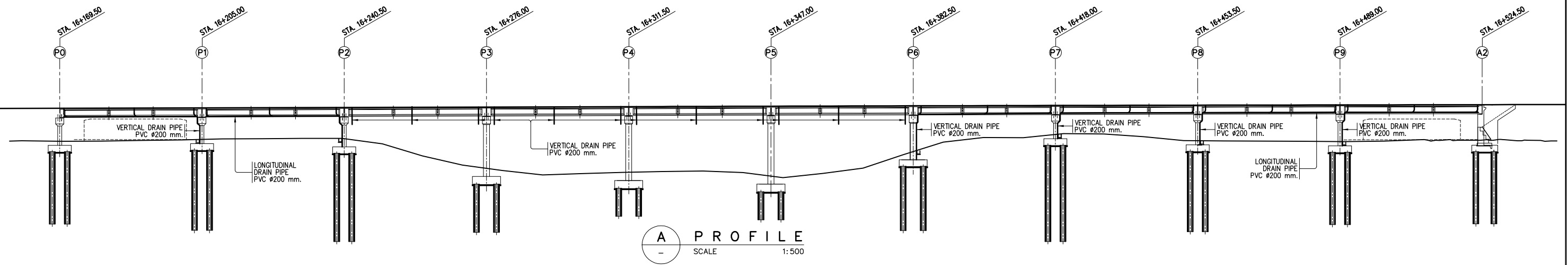
TYPE "GPa"



TYPE "GPb-L"

TYPE	LENGTH OF "GC"
GP - 2.0	2000
GP - 3.0	3000
GP - 4.0	4000
GP - 5.0	5000
GP - 6.0	6000
GP - 7.0	7000

C GABIONS PROTECTION  
SCALE N T S



BRIDGE MESH FENCE  
TYPE-2

