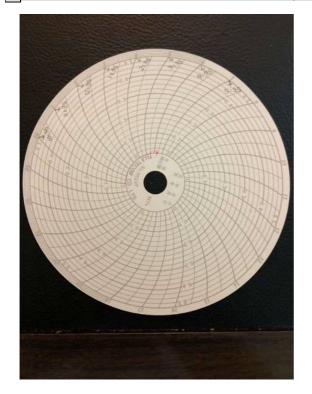
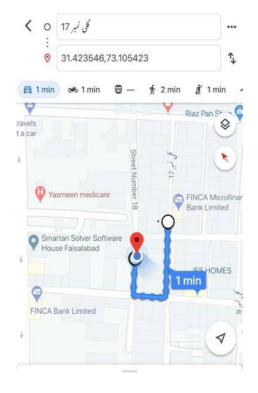
29 26-2-2020 (Consumer Comments: low water pressure, use water by pumping)







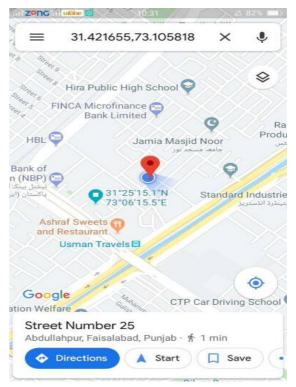






30 24-10-2019(Consumer Comments: Pressure is very low and water quality problem)







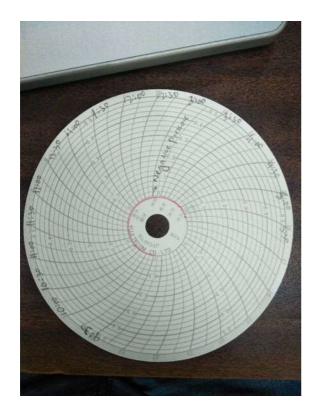


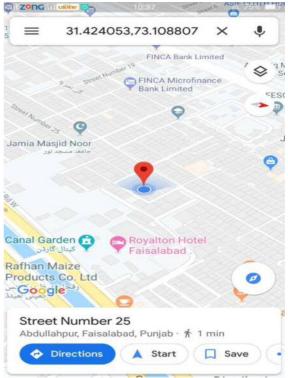






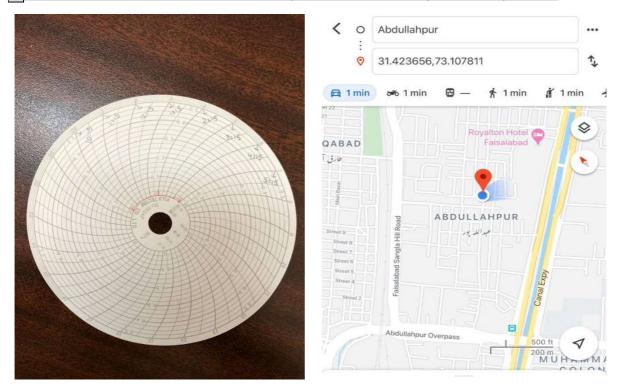
31 29-10-2019(Consumer Comments: water not come whole day)







32 9-3-2020 (Consumer Comments: Low water pressure, not used daily due to low pressure)



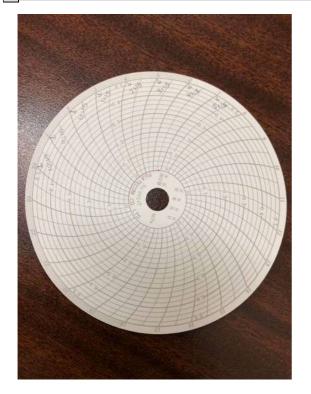


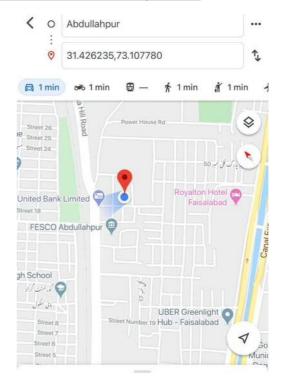






33 5-3-2020 (Consumer Comments: disconnect water connection due to low pressure)



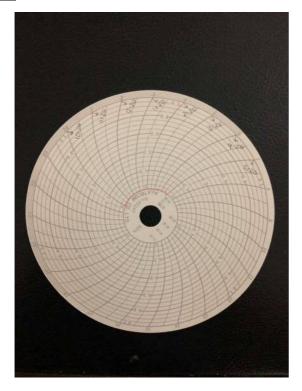


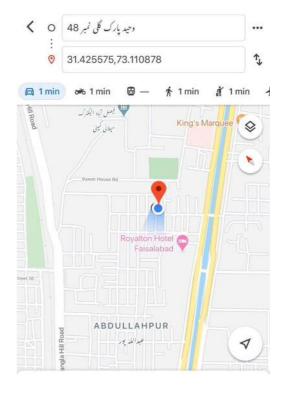






34 4-3-2020 (Consumer Comments: not using water due to low pressure and mixing with sewerage)







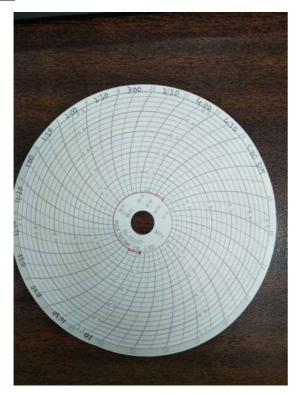


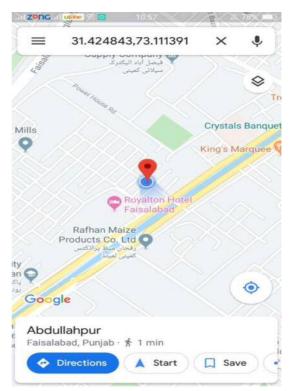






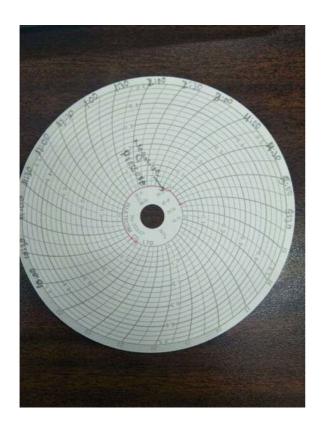
35 31-10-2019(Consumer Comments: Water pressure and water quality problem)

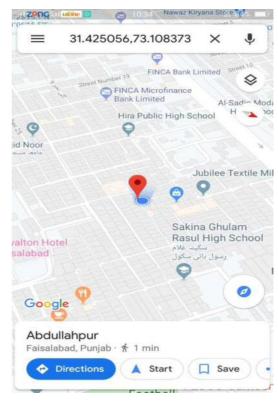






36 5-11-2019(Consumer Comments: Water has zero pressure)





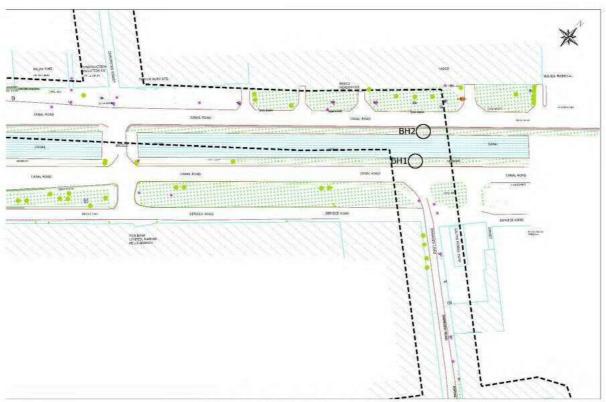




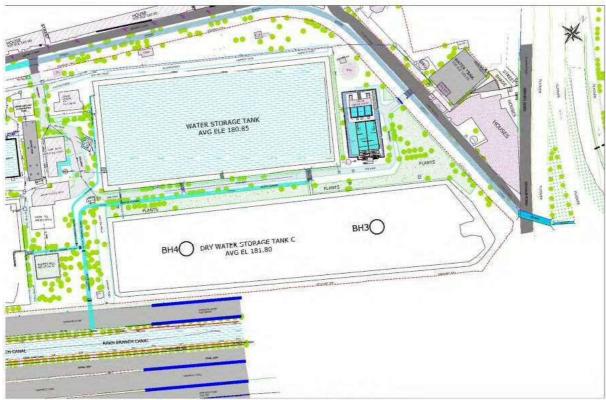




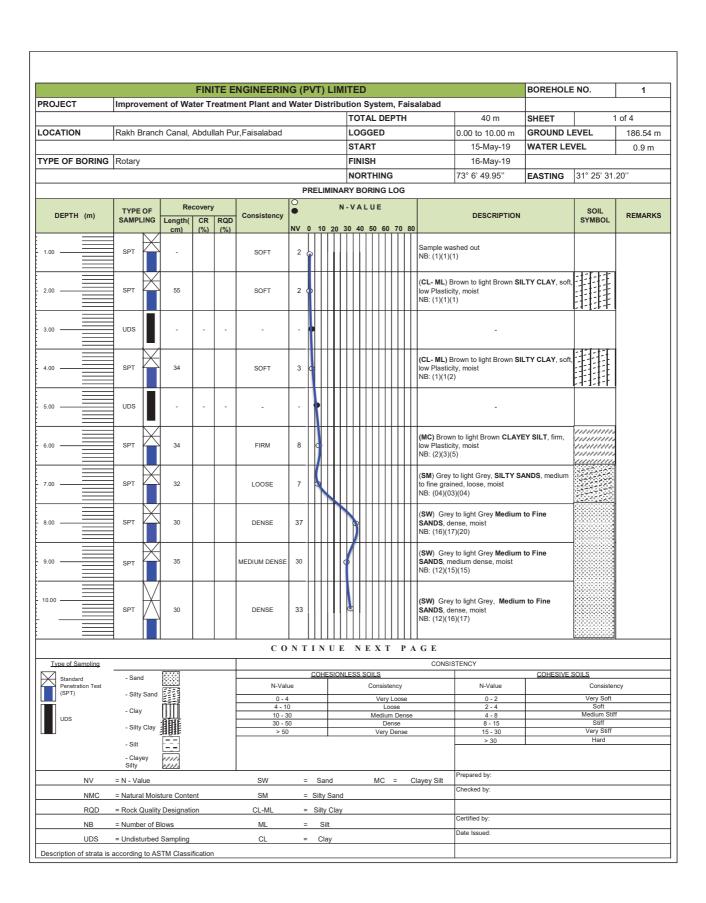
Appendix7 References (2) Results on Geotechnical Survey

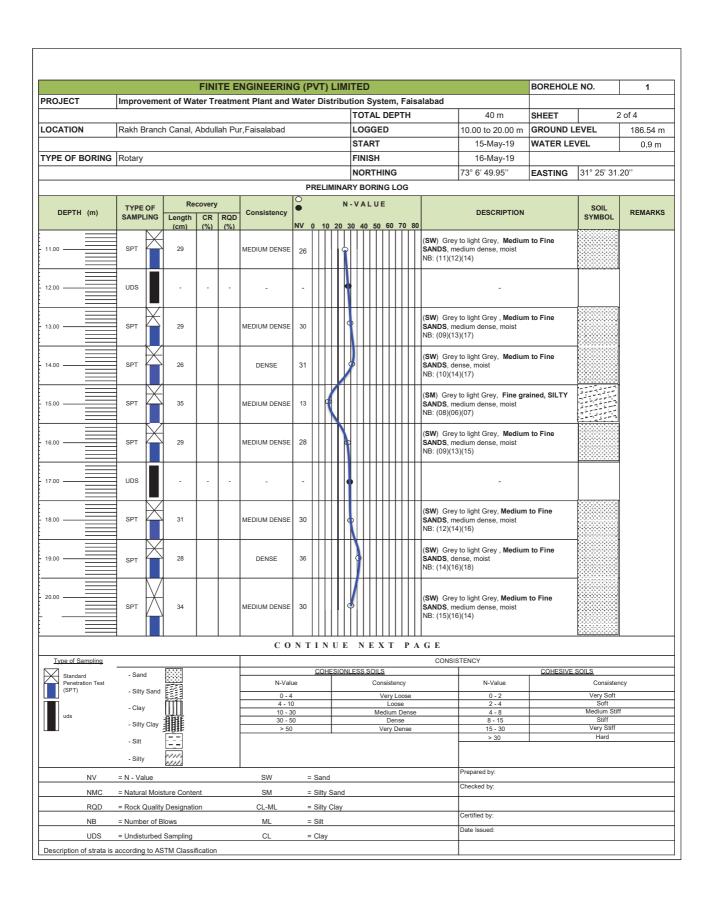


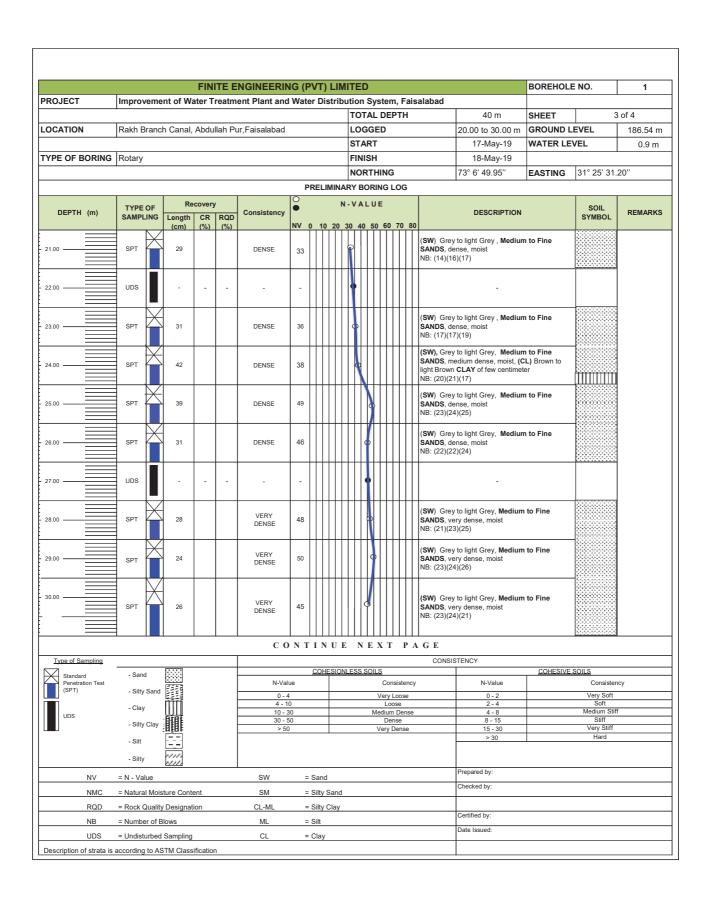
LOCATION OF BH1 AND BH2

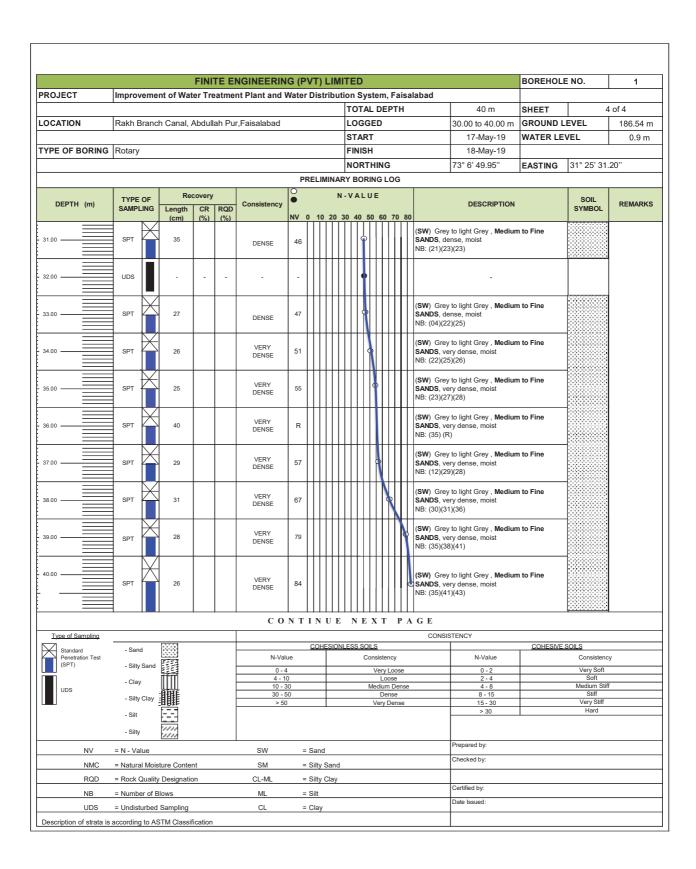


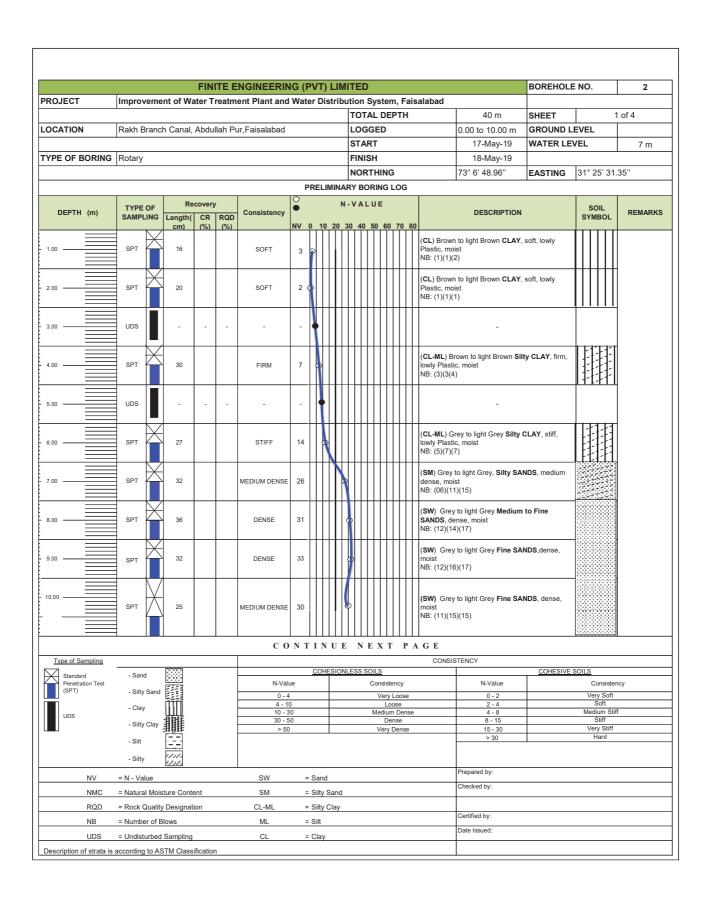
LOCATION OF BH3 AND BH4

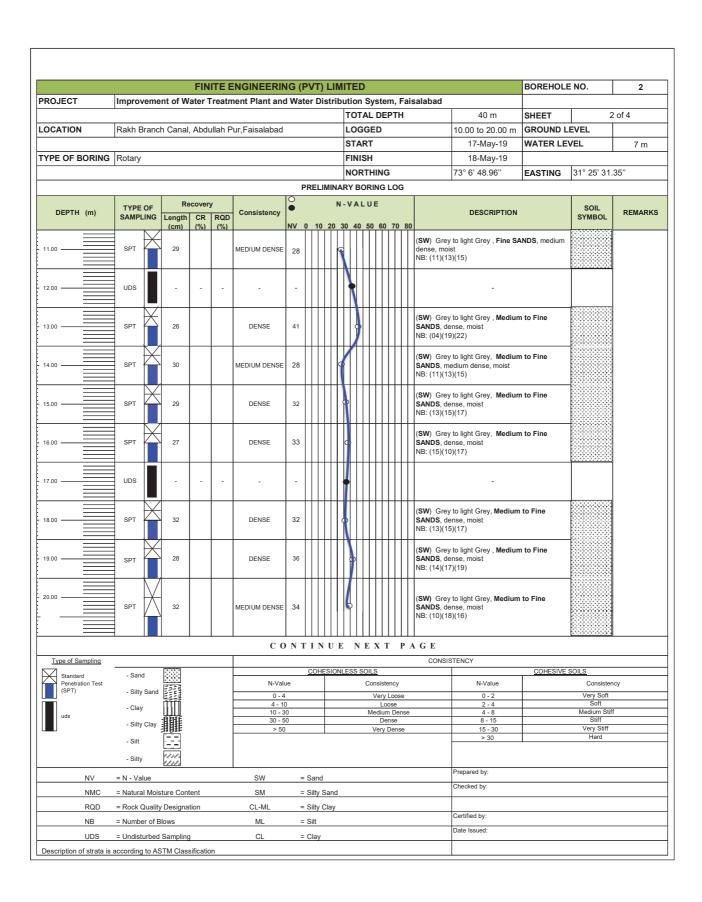


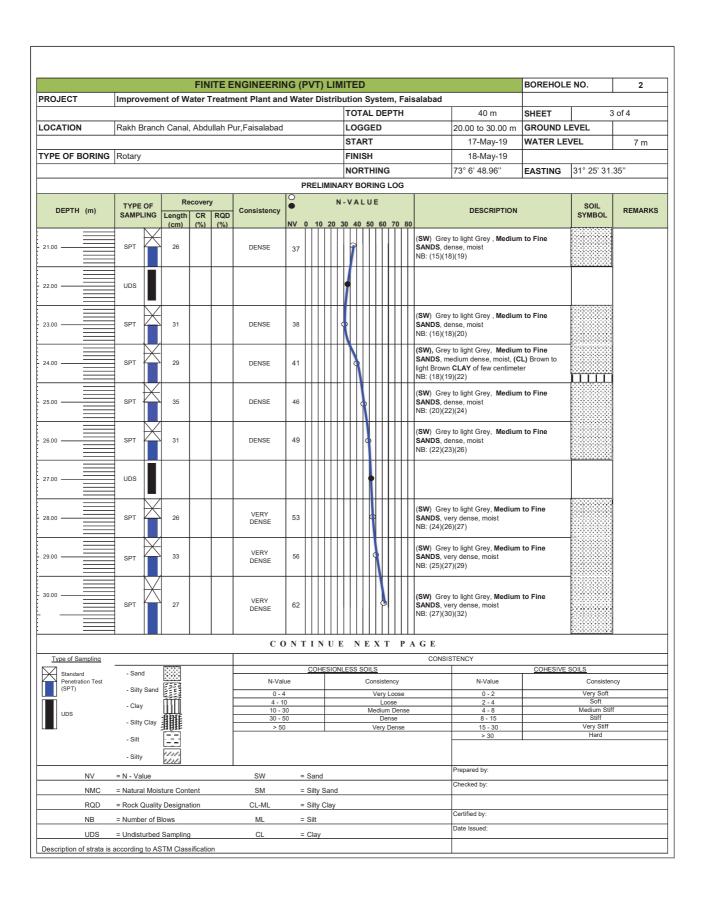


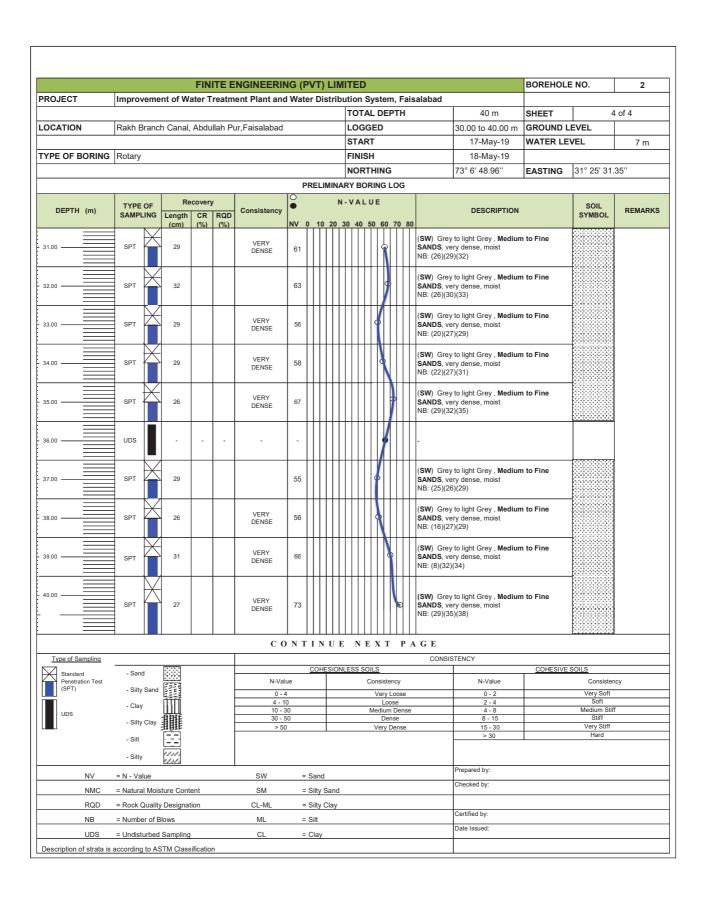


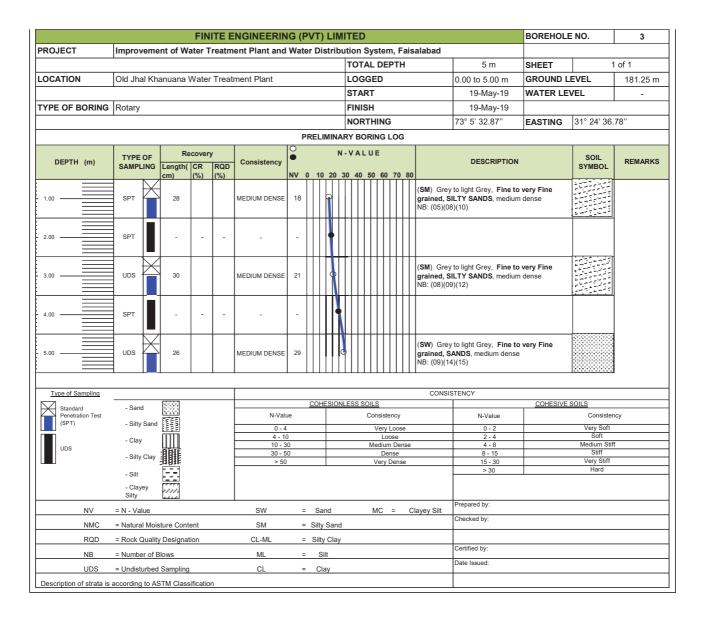


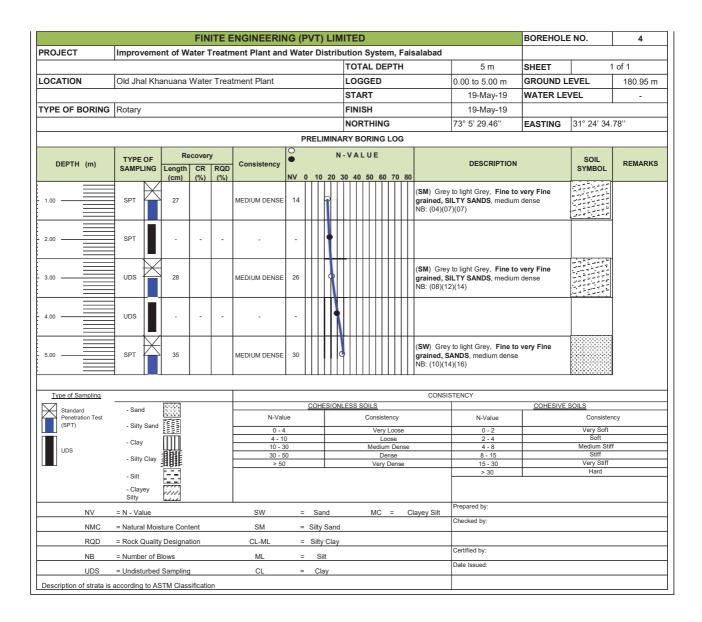










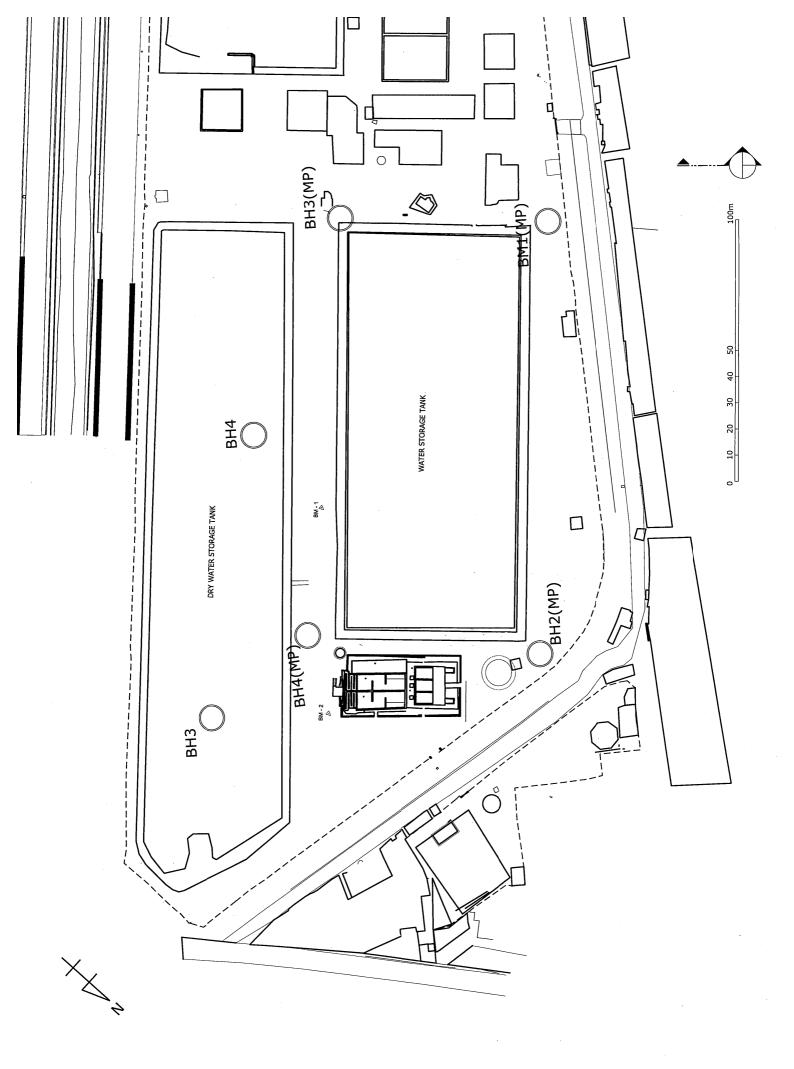


Project for Improvement of Water Treatment Plant and Water Distribution System, Faisalabad



Table 6: Summary of Laboratory Results

	Depth		NMC	Specific	Bulk	Partic	Particle Size Distribution	ution	Particle	Particle Size Distribution	bution	Direct	Direct Share Test
BH No.	(m)	Sample	(%)	ڻ ن	(kN/m³)	Gravel (%)	Sand	Silt & Clay	(%) T.T	P.L (%)	P.I	Cohesion	Angle of internal
	,	(Sec.)	0 0 7	9,0		c	(/0)	(0/)				(NI a)	
	Ţ	1 (DS)	5.61	2.60	' (0	11	60	' (' 6	. (-	1
	3	29 (UDS)	27.3	2.67	19.3	0	6	9.1	29	20	6	•	•
	2	30 (UDS)	22	2.68	20	0	10	06	27	20	7	•	1
	10	2 (DS)	14	2.65		0	96	4	1	-	1	-	-
	12	15 (UDS)	-	-	17.3	-	-	-	-	•	-	1.4	29
BH-1	17	16 (UDS)	-	-	17.9	-	-	-	-	-	1	8.0	30
	20	3 (DS)	10.6	2.66	1	0	62	3	1	1	1	-	1
	22	17 (UDS)	1		16.7	-	-	-	,	1	ı	1	31
	27	18 (UDS)	1	1	18.1	-	-	-	-	1	-	6.0	32
	32	19 (UDS)	-	-	17.2	-	-	-	-	-	-	1.1	32
	40	4 (DS)	28.3	2.67	1	0	62	3	-	-	-	-	1
	1	5 (DS)	17.3	2.68	-	26	23	51	-	-	-	-	1
	3	31 (UDS)	17	2.67	18.4	0	12	88	25	20	5	-	-
	5	32 (UDS)	14.1	2.69	19.6	12	7	81	28	20	8	-	
	10	6 (DS)	13.6	2.67	1	0	96	4	-	-	-	-	1
	12	20 (UDS)	-	1	17.3	1	-		-	-	1	0.4	29
BH-2	17	21 (UDS)	1	-	16.2	ı	-	1	-	-	-	0.8	30
	20	7 (DS)	22.7	2.65	1	0	97	3	-	-	-	-	-
	22	22 (UDS)	1	1	17.6	-	1	-	1	1	1	1.1	32
	27	23 (UDS)	1	-	16.6	-	-	-	-	-	-	0.4	32
	36	24 (UDS)	-	-	16.9	-	-	-	-	-	-	1.2	33
	40	8 (DS)	20.1	2.66	1	12	68	20		-	1		1
	1	9 (DS)	4.7	2.69	-	0	93	7	1	-	1	-	1
	2	25 (UDS)	1	1	16	1	_	-	1	1	-	1.8	27
BH-3	3	10 (DS)	5.8	2.65	1	0	89	11	-	-	-	-	-
	4	26 (UDS)	-	-	16.6	-	-	-	-	-	-	1.6	29
	5	11 (DS)	4.8	2.66	1	0	93	7		-	1	-	1
	1	12 (DS)	10.1	2.67	1	0	92	8	-	-	-	-	-
	2	27 (UDS)	1	-	16.1	1	-	1	-	-	-	1.6	28
BH-4	3	13 (DS)	6.1	2.66	1	0	95	5	1	1	1	-	1
	4	86 (UDS)	1	1	16.1	1	-	1	1	1	•	1.4	29
	5	14 (DS)	4.7	2.67	_	1	92	7	'	,	'	-	1



App 7(2)-13

			ECO	OS L	td.;	GEC	TEC	CHNI	CAL	L SERVICES			
		Loca	tion: WT							Project: WASA Master Plan			
		Bore	Hole No	.: 01						Fig No.			
B	ORE HOLE LOG	Туре	of Borin	g: Ro	tary					Date Started: 30-11-17			
			nination							Date Completed: 02-12-17			
		Grou	ınd Wate	er Tak	ole: 3					Logger: Umer			
		loq					etrat			Ro	cove	ery	
Depth(m)	Sample Description	Classification Symbol	Legend	Sample Type	Moisture	150		150 mm	N-Values	N- Profile (B) Lds	CR %	RQD %	Remarks
2	clay	CL		DS		1	1	1	2	29			
4	Silty clay	CL-N	1L	DS		1	1	2	3	27			
6	Silty sand	SM		DS		5	9	9	18				
8	Fine graind sand	SW		DS		8	11	12	23	23			
10	do	SW		DS		8	10	14	24	10 36			
12	do	SW		DS		9	11	12	23	12 33			
14	do	SW		DS		10	14	19	33	14 34			
16	do	SW		DS		18	22	23	45	16			
18	do	SW		DS		10	12	15	27	18 40			
20	do	sw		DS		17	20	18	38	20 38			
22	do	SW		DS		12	10	10	20	22 40			
24	do	SW		DS		12	12	21	33	24 40			
	Silty clay	CL-N	1L	DS		9	13	27	40	26			
	Silty sand	SM		DS		10	14	27	41	28			
	Medium graind sand	SW		DS		11	19	20	39	30			
32	do	SW		DS		11	26	45	71	32 27			
	do	SW		DS		13	27	50	77	34			
	Med-course sand	SW		DS		14	28	50	78	36 28			
38		SW		DS		20	26	38	64	38			
40	do	SW		DS		30	36	50	86	40			
Che	cked By:												

			ECC	OS L	td.;	GEC	TEC	CHN	ICAI	L SERVICES				
			tion: WT		al					Project: WASA Master Plan				
			Hole No							Fig No.				
B	ORE HOLE LOG		of Borin							Date Started: 03-12-2017				
			nination							Date Completed: 04-12-17				
			ınd Wate	er Tak	ole: 1		etrat	tion I		Logger: Umer				
		nbo					/alue				Re	cove	ry	
Depth(m)	Sample Description	Classification Symbol	puəßəŢ	Sample Type	Moisture	150	150 mm	150	N-Values	N- Profile 0 20 40 60 80 100	SPT (cm)	CR %	RQD %	Remarks
2	clay	CL		DS		5	5	7	12	0 2	39			
4	Silty clay	CL-N	1L	DS		5	6	8	14		33			
6	Silty sand	SM		DS		6	7	11	18		36			
8	Fine graind sand	SW		DS		8	13	14	27	8	30			
10	do	SW		DS		9	11	13	24	10	29			
12	do	sw		DS		9	11	16	28	12	30			
14	do	SW		DS		10	14	16	30	14	31			
16	do	sw		DS		25	30	31	61	16	35			
18	do	sw		DS		20	24	25	49	18	34			
20	do	sw		DS		17	25	29	54	20	30			
22	do	sw		DS		15	16	16	32	22	22			
24	do	SW		DS		16	18	20	38	24	25			
26	do	sw		DS		8	17	25	42	26	32			
28	Clay	CL		DS		15	14	31	45	28	27			
l	Medium graind sand	sw		DS		16	19	26	45	30	39			
32	do	sw		DS		18	21	24	45	32	28			
34		SW		DS		22	26	28	54	34	38			
	Med-course sand	SW		DS		25	30	35	65	36	28			
38		SW		DS		30	45	40	75	38	25			
40		SW		DS		40	40	50	90	40	14			
	cked By:													

			ECO	OS L	td.;	GEC	TEC	CHN	CAL	L SERVICES	
		Loca	tion: WT							Project: WASA Master Plan	
		Bore	Hole No	.: 03						Fig No.	
B	ORE HOLE LOG	Туре	of Borin	g: Ro	tary					Date Started: 05-12-2017	
		Tern	nination	Dept	h: 40	m				Date Completed: 06-12-2017	
		Grou	ınd Wate	er Tak	ole: 2					Logger: Umer	
		pol					etrat			Recovery	
<u></u>		Classification Symbol	_	Sample Type	ė	_ \	/alue	S	S		S
Depth(m)	Sample Description	ion	Legend	le T	Moisture				N-Values	N- Profile	кетагкѕ
)e		cat	Fee	m p	Noi	150	150	150	۷-۷	N- Profile SPT (cm) RQD % SPT (cm) R	ב ב
		ssif		Sa	_	mm	mm	mm			
		Cla								0 20 40 60 80 100	
2	clay	CL		DS		2	3	4	7		
4	Silty Sand	SM		DS		2	6	10	16	20	
6	Silty sand	SM		DS		8	11	12	23	6 32	
8	Fine graind sand	SW		DS		10	12	15	27	8 29	
10	do	SW		DS		12	18	20	38	10 27	
12	do	SW		DS		12	17	22	39	12 29	
14	do	SW		DS		22	32	35	67	14 35	
16	do	SW		DS		17	17	29	37	16 35	
18	do	SW		DS		11	14	18	32	18	
20	do	SW		DS		22	26	23	39	20 32	
22	do	SW		DS		15	18	21	39	22 32	
24	do	SW		DS		15	19	21	40	24 30	
	do	SW		DS		15	20	35	55	26 28	
l	Silty sand	SM		DS		23	30	32	62	28 35	
	Medium graind sand	SW		DS		19	29	38	67	30 33	
32	do	SW		DS		14	38	50	88	32 30	
	do	SW		DS		20	29	30	59	1 	
	Med-course sand	SW		DS		11	36	50	88	 	
38		SW		DS		20	39	50	89	1	
40	do	SW		DS		12	30	50	80	40 38	
Che	cked By:										

			ECC	OS L	td.;	GEC	OTEC	HNI	CAL	L SERVICES
		Loca	tion: WT							Project: WASA Master Plan
		Bore	Hole No	.: 04						Fig No.
В	ORE HOLE LOG	Турє	of Borin	g: Ro	tary					Date Started: 07-12-2017
		Tern	nination	Dept	h: 40	m				Date Completed: 08-12-17
		Grou	ınd Wate	er Tak	ole: 2					Logger: Umer
		pol					etrat			Recovery
Depth(m)	Sample Description	Classification Symbol	Legend	Sample Type	Moisture	150	150 mm	150	N-Values	N- Profile SPT (cm) SPT (cm) R@D % 0 00 00 00 00 00 00 00 00 00 00 00 00
2	clay	CL		DS		4	9	10	19	35
4	Silty clay	CL		DS		5	9	12	21	39
6	Silty sand	SM		DS		10	10	13	23	33
8	Fine graind sand	SW		DS		7	11	12	23	30
10	do	SW		DS		9	14	16	30	33
12	do	SW		DS		11	14	15	29	33
14	do	SW		DS		9	16	19	35	28
16	do	SW		DS		13	15	15	30	34
18	do	SW		DS		11	13	16	29	38
20	do	sw		DS		17	17	15	32	28
22	do	sw		DS		11	14	16	30	32
24	do	sw		DS		18	21	23	44	35
26	Silty clay	CL		DS		10	35	34	69	31
28	Silty sand	SM		DS		24	27	36	63	32
30	Medium graind sand	sw		DS		25	34	39	73	28
32		SW		DS		11	27	35	62	25
34	do	SW		DS		25	29	37	66	33
36	Med-course sand	SW		DS		27	30	28	58	25
38	do	SW		DS		25	30	31	61	33
40	do	SW		DS		19	18	24	42	34
Che	cked By:									

			ECC	OS L	td.;	GEC	TEC	CHN	ICAI	SERVICES				
			tion: Abo		n Pur	OHR	1			Project: WASA Master Plan				
			Hole No							Fig No.				
B	ORE HOLE LOG		of Borin							Date Started: 10-12-2017				
			nination							Date Completed: 11-12-2017				
			ınd Wate	eriak	oie: 1		etrat	ion I		Logger: Umer				
		gr					/alue				Re	cove	ry	
Depth(m)	Sample Description	Classification Symbol	Legend	Sample Type	Moisture	150	150 mm	150	N-Values	N- Profile 0 20 40 60 80 100	SPT (cm)	CR %	RQD %	Remarks
2	Silty clay	CL-N	1L	DS		3	2	4	6		30			
4	Silty Sand	SM		DS		5	6	8	14		34			
6	Silty sand	SM		DS		8	12	14	26		32			
8	Fine graind sand	sw		DS		10	13	16	29		34			
10	do	sw		DS		11	13	12	25		30			
12	do	sw		DS		13	15	19	34		29			
14	do	sw		DS		20	22	23	45		30			
16	Claye Silt	ML		DS		16	19	21	40		35			
18	Fine graind sand	sw		DS		10	17	19	36		35			
20	do	sw		DS		7	14	27	41		33			
22	do	sw		DS		9	15	30	45		22			
24	do	sw		DS		14	22	16	38		38			
26	Silty clay	CL-N	1L	DS		4	15	26	41		25			
	Silty sand	SM		DS		18	20	24	44		34			
30	Medium graind sand	SW		DS		14	15	22	37		29			
32	do	SW		DS		13	40	50	90		25			
١	do	SW		DS		10	20	39	59		29			
36	Med-course sand	SW		DS		12	33	29	62		32			
l	do	SW		DS		19	26	33	63		28			
40	do	SW		DS		30	45	50	95		32			
	cked By:													

			ECC	OS L	td.;	GEC	TEC	CHN	ICAI	L SERVICES				
			tion: Ma		Tow	n OH	IR NC).2		Project: WASA Master Plan				
_			Hole No							Fig No.				
B	ORE HOLE LOG		of Borin							Date Started: 13-12-2017				
			nination and Wate							Date Completed: 14-12-2017				
			ing wate	eriak	oie: 1		etrat	ion		Logger: Umer				
		Jan		ا ا			/alue				Re	cove	ry	
Depth(m)	Sample Description	Classification Symbol	Pegend	Sample Type	Moisture		150 mm		N-Values	N- Profile 0 20 40 60 80 100	SPT (cm)	CR %	RQD %	Remarks
2	clay	CL		DS		4	5	7	12		25			
4	Clayey Silt	ML		DS		5	7	10	17		30			
6	Silty sand	SM		DS		11	14	16	30		35			
8	Silty sand	SM		DS		10	16	17	33		22			
10	Fine graind sand	sw		DS		9	10	12	22		31			
12	do	sw		DS		15	15	21	36		33			
14	do	sw		DS		11	14	18	32		28			
16	do	sw		DS		15	10	22	32		30			
18	do	sw		DS		15	17	21	38		30			
20	do	sw		DS		13	17	18	35		28			
22	do	sw		DS		9	29	45	74		27			
24	do	SW		DS		22	34	35	69		33			
26	do	SW		DS		12	18	35	53		27			
28	do	SW		DS		18	28	37	65		27			
30	Medium graind sand	SW		DS		20	31	42	73		35			
32	do	SW		DS		30	41	50	91		35			
34	do	SW		DS		29	42	47	89		48			
36	Med-course sand	SW		DS		30	37	49	86		28			
38		SW		DS		30	33	35	68		26			
١	do	SW		DS		32	35	40	75		22			
Che	cked By:													



University of Engineering & Technology, Lahore Department of Civil Engineering Geotechnical Engineering Laboratory

SUMMARY OF THE TEST RESULTS

Geotechnical Investigation for WASA Master Plan, Faisalabad

Project:

M/S ECOS Ltd

Client:

BH/ TP No.	Sample No	Depth (m)	NMC (%)	Bulk Density (kN/m³)	Specific Gravity G _s
	1 (UDS)	-	21.68	19.21	2.7
	2 (UDS)	7	25.05	18.64	2.67
	3 (NDS)	14	17.96	14.55	2.65
3	4 (UDS)	19	18.27	16.08	2.67
.O-H9	s (UDS)	25	24.24	20.55	2.65
	e (UDS)	30	23.88	15.74	2.66
	7 (UDS)	35	25.17	13.48	2.69
	8 (NDS)	40	17.67	15.90	2.68
	(SQN) 6	က	9.61	20.48	2.7
	10 (UDS)	2	7.50	18.93	2.67
	11 (UDS)	15	7.32	16.97	2.67
0	12 (UDS)	20	4.32	16.31	2.66
20-U2	13 (UDS)	25	17.34	19.79	2.65
	14 (UDS)	30	27.66	18.30	2.67
	15 (UDS)	35	14.41	12.48	2.67
	16 (UDS)	40	19.27	18.44	2.65





University of Engineering & Technology, Lahore 2 Department of Civil Engineering Geotechnical Engineering Laboratory

SUMMARY OF THE TEST RESULTS

Geotechnical Investigation for WASA Master Plan, Faisalabad

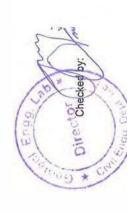
Client:

Project:

M/S ECOS Ltd

BH/ TP No.	Sample No	Depth (m)	NMC (%)	Bulk Density (KN/m³)	Specific Gravity G _s
	17 (UDS)	5	2.37	16.54	2.66
	18 (UDS)	11	7.22	20.71	2.67
	19 (UDS)	15	9.19	17.99	2.67
-	20 (UDS)	21	20.48	17.94	2.66
BH-03	21 (UDS)	25	21.92	20.86	2.67
	22 (UDS)	31	26.52	16.14	2.66
	23 (UDS)	35	24.92	20.47	2.65
	24 (SPT)	40	16.53		2.65
	25 (UDS)	2	4.32	16.20	2.67
	26 (UDS)	11	6.41	17.78	2.67
	27 (UDS)	15	12.36	17.35	2.67
70	28 (UDS)	21	9.82	26.16	2.66
5	29 (UDS)	25	17.35	19.44	2.65
	30 (NDS)	31	22.79	18.65	2.67
	31 (SPT)	36	32.67		2.66
	32 (SPT)	40	22.86		2.65





University of Engineering & Technology, Lahore Geotechnical Engineering Laboratory

SUMMARY OF THE TEST RESULTS

Geotechnical Investigation for WASA Master Plan, Faisalabad

Client:

Project:

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BH/ TP No.	Sample No	Depth (m)	NMC (%)	Bulk Density (kN/m³)	Specific Gravity G _s	
	33 (NDS)	2	9.23	17.74	2.65	_
	34 (UDS)	11	7.28	16.47	2.65	_
	35 (UDS)	15	15.10	16.46	2.66	
-	36 (UDS)	21	23.43	18.48	2.67	
BH-03	37 (UDS)	25	24.64	17.79	2.67	
	38 (UDS)	31	23.30	17.50	2.67	
	39 (SPT)	36	19.62		2.67	
	40 (SPT)	40	20.11		2.67	
	41 (UDS)	1.5	20.39	17.17	2.70	
	42 (UDS)	2	21.26	16.61	2.65	77
	43 (UDS)	11	7.78	15.98	2.67	
90 70	44 (UDS)	15	9.63	18.34	2.68	
5	45 (UDS)	21	6.79	16.12	2.67	
	46 (UDS)	25	18.72	18.05	2.66	
	47 (SPT)	32	20.46		2.65	
	48 (SPT)	36	22.78		2.67	



Appendix7 References

(3) Results on Excavation and Underground Infrastructure Observation Survey

Survey List (Excavation and underground structure observation survey)

				derground structure observ		I	I	
Sr.#	MAP#	Pit/Chamber Name	Excavation date	Pipe to be identified	Actual identified pipe	Road	Pavement	Notes
1	6	⑤ −II−1−1	27-Jun	Installation point of a valve on Ex. Distribution Main (DN250)	PVC DN100, DP=1.8m PVC DN100, DP=1.9m	Main Rd (Susan Rd)	No pavement (green belt)	
2	3	⑥-PM-7	28-Jun	Intersection point of Ex. Distribution Main (DN300) and proposed Distribution Primary Main (DN450)	CIP DN250, DP=1.43m CIP DN250, DP=1.43m	Main Rd (Susan Rd)	No pavement (green belt)	
3	9	⑥-EX-01	28-Jun	Intersection pointof Ex.Distribution Main (DN600) and proposed Interconnecting Main.	AC DN600, DP=1.65m	Inside Old JK WTP	No pavement	
4	9	⑥-EX-02	29-Jun	Intersection point of Ex.Distribution Main (DN600) and proposed Interconnecting Main.	No pipe	Inside Old JK WTP	No pavement	
5	7	⑥-TM-3	1-Jul	Intersection point of Ex. Distribution Main (DN250) and proposed Transmission Main (DN450)	AC DN250, DP=1.52m	Main Rd (Green Belt Rd)	No pavement (shoulder)	
6	7	⑥-PM-9	1-Jul	Intersection point of Ex. Distribution Main (DN250) and proposed Distribution Primary Main (DN450)	AC DN250, DP=1.18m	Main Rd (Green Belt Rd)	Brick (shoulder)	
7	11	②-II-3-1	2-Jul	Point of cutting or installation of a valve on Ex. Distribution Main (DN150)	AC DN200, DP=1.1m	Town Rd (Madina Town)	No pavement (shoulder)	
8	12	⑥-РМ-13	2-Jul	Intersection point of Ex. Distribution Main (DN200) and proposed Distribution Primary Main (DN300)	AC DN200, DP=1.43m CIP DN75, DP=1.43m	Town Rd (Madina Town)	Asphalt	
9	15	⑥-РМ-12	3-Jul	Intersection point of Ex. Distribution Main (DN300) and proposed Distribution Primary Main (DN300)	Steel DN300, DP=0.23m Steel DN200, DP=0.23m	Main Rd (Jaranwala Rd)	Asphalt	
10	3	⑥−TM−2	4-Jul	Intersection point of Ex. Distribution Main (DN600) and proposed Transmission Main (DN450)	No pipe	Main Rd (Service road of Canal Expy)	Asphalt	
11	11	①-II-3-1	13-Jul	Connecting point of Ex. Distribution Main (DN100) and proposed Distribution Secondary Main (DN200)	No pipe	Town Rd (Madina Town)	No pavement (shoulder)	
12	11	①-II-3-1A	13-Jul	Connecting point of Ex. Distribution Main (DN100) and proposed Distribution Secondary Main (DN200)	AC DN150, DP=1.25m	Town Rd (Madina Town)	No pavement (shoulder)	
13	2	①-I-1-1	14-Jul	Connecting point of Ex. Distribution Main (DN75) and proposed Distribution Secondary Main (DN200)	AC DN100, DP=1.50m	Town Rd (Abdullar Pur)	Concrete	
14	2	①-I-2-2	15-Jul	Connecting point of Ex. Distribution Main (DN100) and proposed Distribution Secondary Main (DN150)	AC DN100, DP=1.27m	Town Rd (Abdullar Pur)	Asphalt	
15	2	④ −I−1−2	15-Jul	Cutting point of Ex. Distribution Main (DN150)	No pipe	Town Rd (Abdullar Pur)	Asphalt	
16	7	①-II-1-6	16-Jul	Connecting point of Ex. Distribution Main (DN75) and proposed Distribution Secondary Main (DN150)	AC DN250, DP=1.75m CIP DN75, DP=1.75m	Main Rd (Susan Rd)	No pavement (green belt)	
17	7	①-II-1-5	16-Jul	Connecting point of Ex. Distribution Main (DN75) and proposed Distribution Secondary Main (DN150)	AC DN250, DP=1.24m CIP DN150, DP=1.24m	Main Rd (Susan Rd)	No pavement (green belt)	
18	6	①-II-1-1	17-Jul	Connecting point of Ex. Distribution Main (DN100) and proposed Distribution Secondary Main (DN150)	AC DN100, DP=1.5m (new) AC DN50, DP=1.2m (old)	Town Rd (Madina Town)	Tile, Asphalt	
19	6	①-II-1-2	17-Jul	Connecting point of Ex. Distribution Main (DN100) and proposed Distribution Secondary Main (DN150)	AC DN100, DP=0.57m AC DN100, DP=0.57m	Town Rd (Madina Town)	Asphalt	
20	11	⑤ -II-4-1	18-Jul	Installation point of a valve to Ex. Distribution Main (DN200)	AC DN200, DP=1.74m	Town Rd (Madina Town)	Asphalt	
21	12	①-II-4-3	18-Jul	Connecting point of Ex. Distribution Main (DN75) and proposed Distribution Secondary Main (DN150)	PVC DN75, DP=1.10m	Town Rd (Madina Town)	Asphalt	
22	7	①-II-3-2	19-Jul	Connecting point of Ex. Distribution Main (DN75) and proposed Distribution Secondary Main (DN150)	PVC DN75, DP=0.70m	Town Rd (Madina Town)	Asphalt	
23	3	①-II-1-8	19-Jul	Connecting point of Ex. Distribution Main (DN75) and proposed Distribution Secondary Main (DN150)	PVC DN100, DP=1.00m PVC DN75, DP=1.00m	Town Rd (Madina Town)	Asphalt	
24	3	①-II-1-7	19-Jul	Connecting point of Ex. Distribution Main (DN75) and proposed Distribution Secondary Main (DN150)	PVC DN75, DP=0.90m PVC DN75, DP=0.90m	Town Rd (Madina Town)	Asphalt	

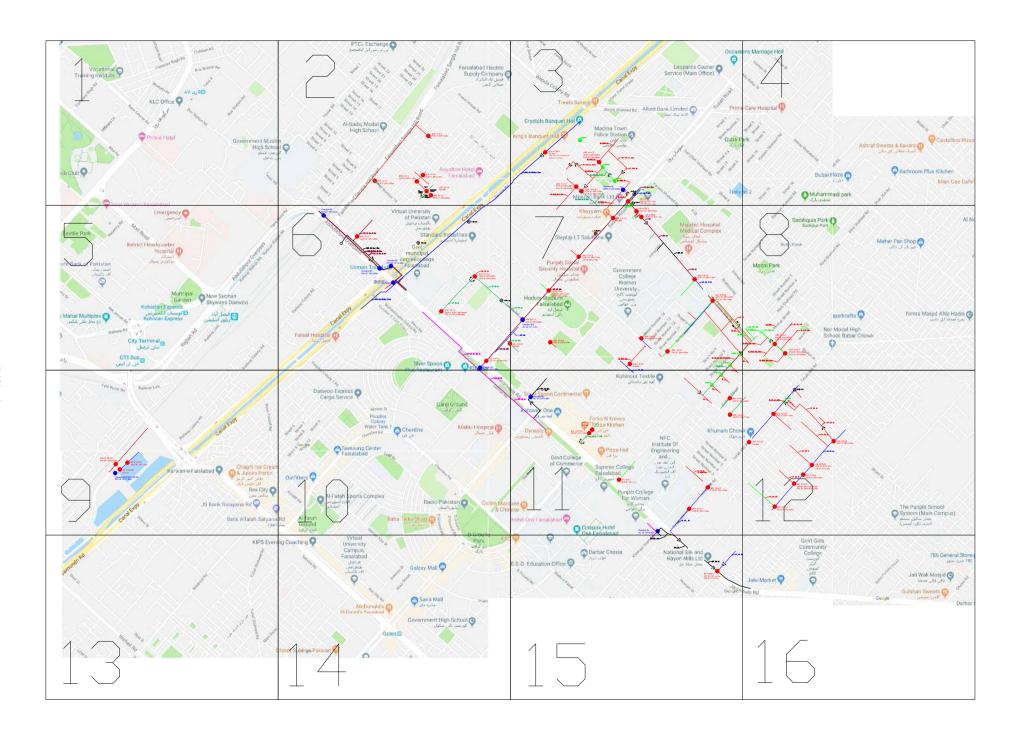
Sr.#	MAP#	Pit/Chamber Name	Excavation date	Pipe to be identified	Actual identified pipe	Road	Pavement	Notes
25	2	_	21-Jul	Connecting point of Ex. Distribution Main (DN75) and proposed Distribution Secondary Main (DN150)	PVC DN75, DP=1.05m	Town Rd (Abdullar Pur)	Concrete, Asphalt	
26	2	④ −I−1−3	21-Jul	Cutting point of Ex. Distribution Main (DN75)	ACP DN75, DP=0.96m	Town Rd (Abdullar Pur)	Concrete, Asphalt	
27	12	①-II-4-4	22-Jul	Connecting point of Ex. Distribution Main (DN75) and proposed Distribution Secondary Main (DN200)	ACP DN200, DP=1.45m	Town Rd (Madina Town)	Asphalt	
28	11	①-II-4-2	22-Jul	Connecting point of Ex. Distribution Main (DN75) and proposed Distribution Secondary Main (DN150)	ACP DN100, DP=1.75m	Town Rd (Madina Town)	Tile (shoulder)	
29	7	③-II-1-1	23-Jul	Cutting point of Ex. Distribution Main (DN100)	PVC DN150, DP=0.80m	Main Rd (Susan Rd)	No pavement (green belt)	Count as 2 pits because of more than 3m2 of excavation area
30	7	③-II-1-3	23-Jul	Cutting point of Ex. Distribution Main (DN75)	No pipe	Main Rd (Susan Rd)	No pavement (green belt)	
31	11	①-II-4-1	24-Jul	Connecting point of Ex. Distribution Main (DN75) and proposed Distribution Secondary Main (DN150)	ACP DN100, DP=1.45m	Town Rd (Madina Town)	Asphalt	
32	7	①-II-2-1	24-Jul	Connecting point of Ex. Distribution Main (DN100) and proposed Distribution Secondary Main (DN150)	PVC DN150, DP=0.93m	Town Rd (Madina Town)	Asphalt	
33	3	4 -II-1-2	26-Jul	Cutting point of Ex. Distribution Main (DN75)	ACP DN100, DP=0.68m PVC DN75, DP0.68m	Town Rd (Madina Town)	Asphalt	
34	7	⑥−PM−8	26-Jul	Intersection point of Ex. Distribution Main (DN250) and proposed Distribution Primary Main (DN400)	ACP DN250, DP=1.30m PVC DN150, DP01.30m	Main Rd (Green Belt Rd)	No pavement (shoulder)	
35	9	⑥-EX-03	27-Jul	Connecting point of Ex.Arterial Main (DN800) and proposed Interconnecting Main.	DIP DN800, DP=2.30m	Town Rd (Jhal)	Asphalt	
36	7	①-II-3-3	27-Jul	Connecting point of Ex. Distribution Main (DN75) and proposed Distribution Secondary Main (DN150)	No pipe	Town Rd (Madina Town)	Tile	
37	8	4 -II-4-2	28-Jul	Cutting point of Ex. Distribution Main (DN75)	PVC DN150, DP=0.66m PVC DN75, DP=0.66m	Town Rd (Madina Town)	Asphalt	
38	8	4 -II-4-1	28-Jul	Cutting point of Ex. Distribution Main (DN75)	PVC DN150, DP=0.67m PVC DN75, DP=0.67m	Town Rd (Madina Town)	Asphalt	
39	7	③-II-2-2	29-Jul	Cutting point of Ex. Distribution Main (DN150)	ACP DN250, DP=1.70m	Main Rd (Green Belt Rd)	No pavement (shoulder)	
40	7	③-II-2-3	29-Jul	Cutting point of Ex. Distribution Main (DN150)	ACP DN100, DP=1.45m	Town Rd (Madina Town)	Brick (shoulder)	Count as 2 pits because of more than 3m2 of excavation area
41	7	③-II-3-1	30-Jul	Cutting point of Ex. Distribution Main (DN200)	No pipe	Main Rd (Susan Rd)	Tile, Asphalt	
42	11	③-II-4-1	31-Jul	Cutting point of Ex. Distribution Main (DN75)	PVC DN200, DP=1.40m	Town Rd (Madina Town)	Asphalt	
43	12	③-II-4-6	31-Jul	Cutting point of Ex. Distribution Main (DN75)	ACP DN200, DP=1.43m	Town Rd (Madina Town)	Asphalt	
44	12	③-II-4-2	1-Aug	Cutting point of Ex. Distribution Main (DN75)	No pipe	Town Rd (Madina Town)	Asphalt	
45	12	③-II-4-4	1-Aug	Cutting point of Ex. Distribution Main (DN150)	PVC DN200, DP=0.60m	Town Rd (Madina Town)	Asphalt	
46	12	③-II-4-5	1-Aug	Cutting point of Ex. Distribution Main (DN75)	ACP DN200, DP=1.30m (OLD) PVC DN200, DP=0.50m (NEW)	Town Rd (Madina Town)	Asphalt	
47	6	②-I-1-1	2-Aug	Installation point of a valve to Ex. Distribution Main (DN250)	ACP DN250, DP=1.45m	Main Rd (Faisalabad Sangla Hill Rd)	Asphalt	
48	2	④ −I−1−1	2-Aug	Cutting point of Ex. Distribution Main (DN100)	ACP DN150, DP=1.13m	Town Rd (Abdullar Pur)	Concrete, Asphalt	
Valve chambers surveyed								
Chamber # 1	6	⑤-II-1-1	-	Installation point of a valve on Ex. Distribution Main (DN250)	AC DN250, DP=1.98m	Main Rd (Susan Rd)	-	
Chamber # 2	11	⑥-РМ-10	-	Intersection point of Ex.Distribution Main (DN300) and proposed Distribution Primary Main (DN400)	DN250, DP=1.98m	Main Rd (Jaranwala Rd)	-	
Chamber # 3	11	⑥-РМ-11	-	Intersection point of Ex.Arterial Main (DN500) and proposed Distribution Primary Main (DN300)	AC DN300, DP=0.92m	Main Rd (Jaranwala Rd)	-	

Sr.#	MAP #	Pit/Chamber Name	Excavation date	Pipe to be identified	Actual identified pipe	Road	Pavement	Notes
Chamber # 4	3	4)-II-1-5	-	Cutting point of Ex. Distribution Main (DN75)	DN75, DP=0.92m	Town Rd (Madina Town)	-	
Chamber # 5	6	⑥-PM-4	-	Intersection point of Ex.Arterial Main (DN800) and proposed Distribution Primary Main (DN300)	DIP DN800, DP=3.66m	Main Rd (Canal Expy)	-	
Chamber # 6	6	⑥-PM-5	-	Intersection point of Ex.Arterial Main (DN600) and proposed Distribution Primary Main (DN300)	DIP DN800, DP=4.58m	Main Rd (Canal Expy)	-	
Chamber # 7	6	⑥-РМ-6	-	Intersection point of Ex.Arterial Main (DN800) and proposed Distribution Primary Main (DN300)	DIP DN800, DP=3.66m	Main Rd (Jaranwala Rd)	-	
Chamber # 8	6	⑥-TM-1 / ⑥-PM-1	_	Intersection point of Ex.Arterial Main (DN800) and proposed Transmission Main (DN450) and proposed Distribution Primary Main (DN300)	DIP DN800, DP=2.44m	Main Rd (Faisalabad Sangla Hill Rd)	-	
Chamber # 9	9	⑥-EX-01 / ⑥-EX-02	-	Intersection point of Ex.Distribution Main (DN600) and proposed Interconnecting Main.	DIP DN600, DP=1.98m	Inside Old JK WTP	-	
Chamber # 10	7	③-II-1-1	_	Cutting point of Ex. Distribution Main (DN100)	AC DN250, DP=1.52m AC DN150, DP=1.52m	Main Rd (Susan Rd)	-	
No need	to be ex	cavated						
-	3	⑥-PM-2	-	Intersection point of Ex. Distribution Main (DN250) and proposed Distribution Primary Main (DN400)	-	Main Rd (Susan Rd)	-	
-	3	⑥-РМ-3	-	Intersection point of Ex. Distribution Main (DN300) and proposed Distribution Primary Main (DN400)	-	Main Rd (Susan Rd)	-	
-	12	⑤-II-4-2	-	Installation point of a valve to Ex. Distribution Main (DN200)	-	Town Rd (Madina Town)	-	
-	6	①-I-1-2	-	Connecting point of Ex. Distribution Main (DN75) and proposed Distribution Secondary Main (DN200)	-	Main Rd (Canal Expy)	-	
-	6	①-II-1-3	-	Connecting point of Ex. Distribution Main (DN75) and proposed Distribution Secondary Main (DN150)	-	Town Rd (Madina Town)	-	
-	6	①-II-1-4	-	Connecting point of Ex. Distribution Main (DN75) and proposed Distribution Secondary Main (DN150)	-	Town Rd (Madina Town)	-	
Cannot b	Cannot be excavated							
-	3	4 -II-1-1	-	Cutting point of Ex. Distribution Main (DN75)	-	Town Rd (Madina Town)	-	
-	3	4 -II-1-3	-	Cutting point of Ex. Distribution Main (DN75)	-	Town Rd (Madina Town)	-	
-	3	4 -II-1-4	-	Cutting point of Ex. Distribution Main (DN75)	-	Town Rd (Madina Town)	-	
-	7	③-II-1-2	-	Cutting point of Ex. Distribution Main (DN75)	-	Main Rd (Susan Rd)	-	
-	3	③-II-2-1	-	Cutting point of Ex. Distribution Main (DN200)	-	Main Rd (Susan Rd)	-	
-	11	4 -II-2-1	-	Cutting point of Ex. Distribution Main (DN75)	-	Town Rd (Madina Town)	-	
-	11	④ -II-2-2	-	Cutting point of Ex. Distribution Main (DN75)	-	Town Rd (Madina Town)	-	
-	7	④ -II-2-3	-	Cutting point of Ex. Distribution Main (DN75)	-	Town Rd (Madina Town)	-	
-	7	4 -II-2-4	-	Cutting point of Ex. Distribution Main (DN75)	-	Town Rd (Madina Town)	-	
-	12	③-II-4-3	-	Cutting point of Ex. Distribution Main (DN75)	-	Town Rd (Madina Town)	-	

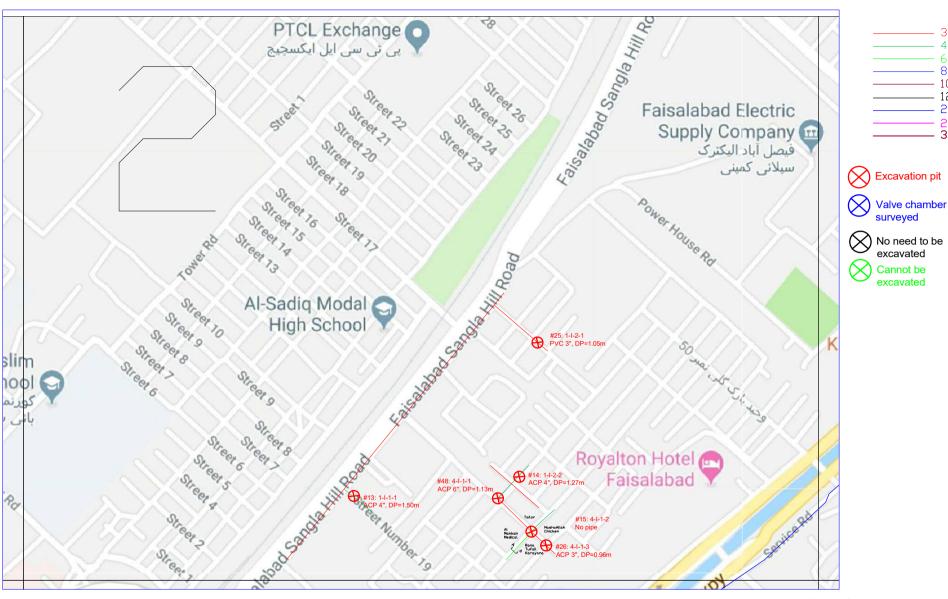
Notes:

- 1 $To identify \ actual \ connecting \ point \ of \ new \ distribution \ secondary \ main \ and \ existing \ distribution \ tertiary \ main.$
- To identify actual isolation point (installing valve or cutting & plugging) between existing distribution primary main (to be used) and existing distribution secondary main (not to be used). 2
- 3 4 5 To identify actual isolation point (installing valve or cutting & plugging) between existing distribution secondary main and existing distribution tertiary main. To identify actual isolation point (installing valve or cutting & plugging) of existing distribution tertiary main at boundary of DMA.

 To identify actual valve installation point on existing primary or secondary main to isolate distribution area.
- To identify actual location (alignment and depth) of existing underground infrastructure (big water supply pipe such as arterial main and primary main, sewerage pipe, drainage channel, commutation cable, etc.) in order to define alignment and depth of new transmission main and new distribution primary main.



Section A-A





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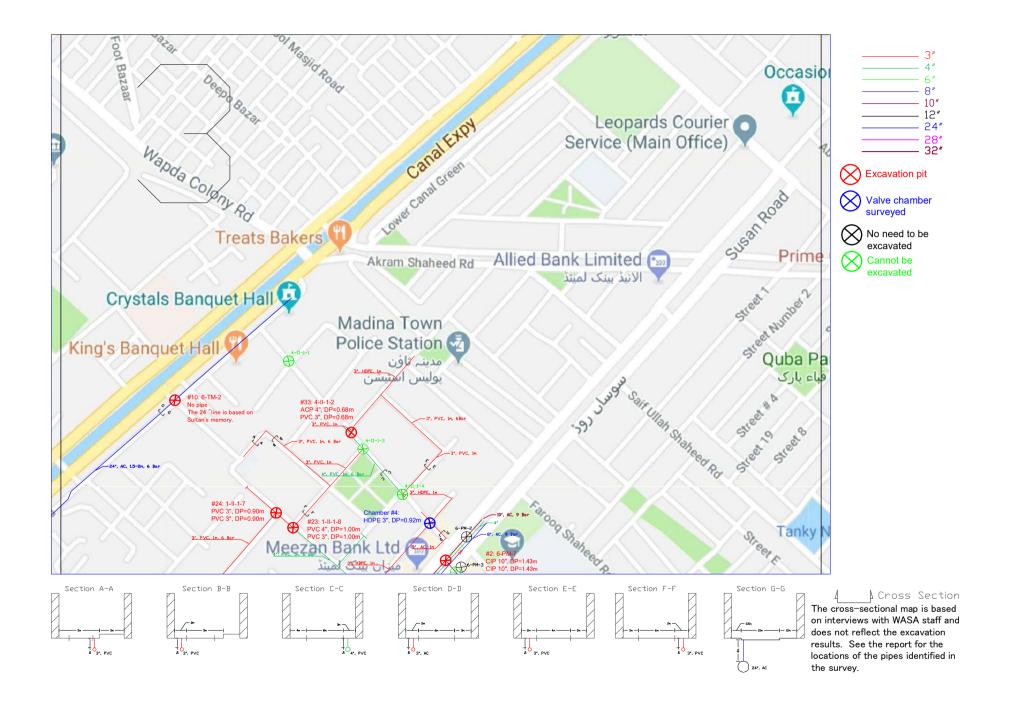
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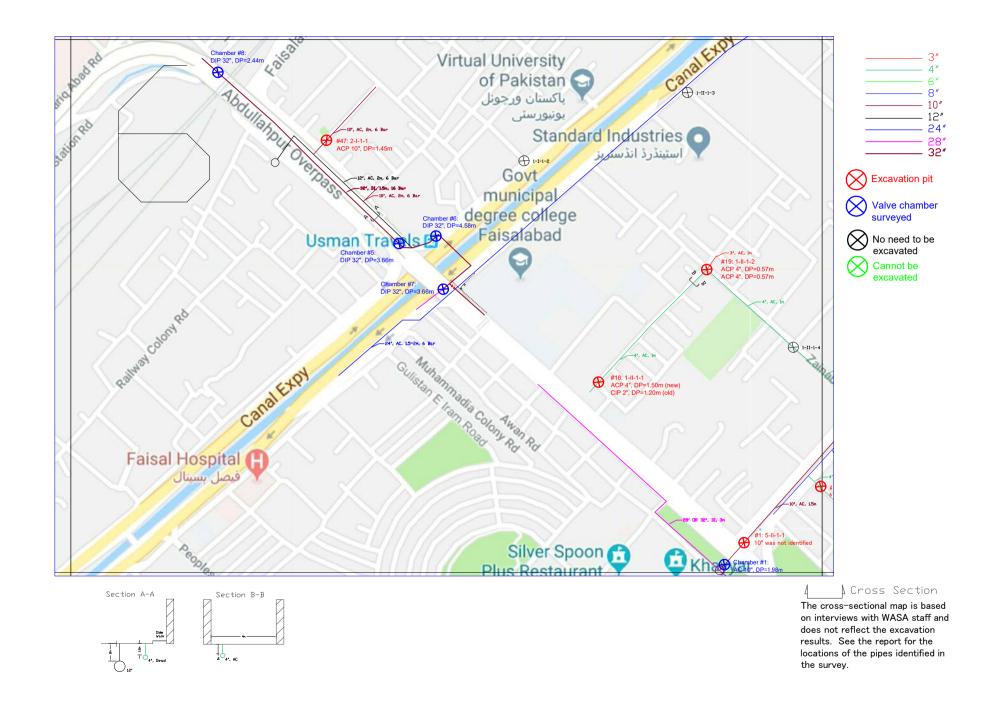
24"

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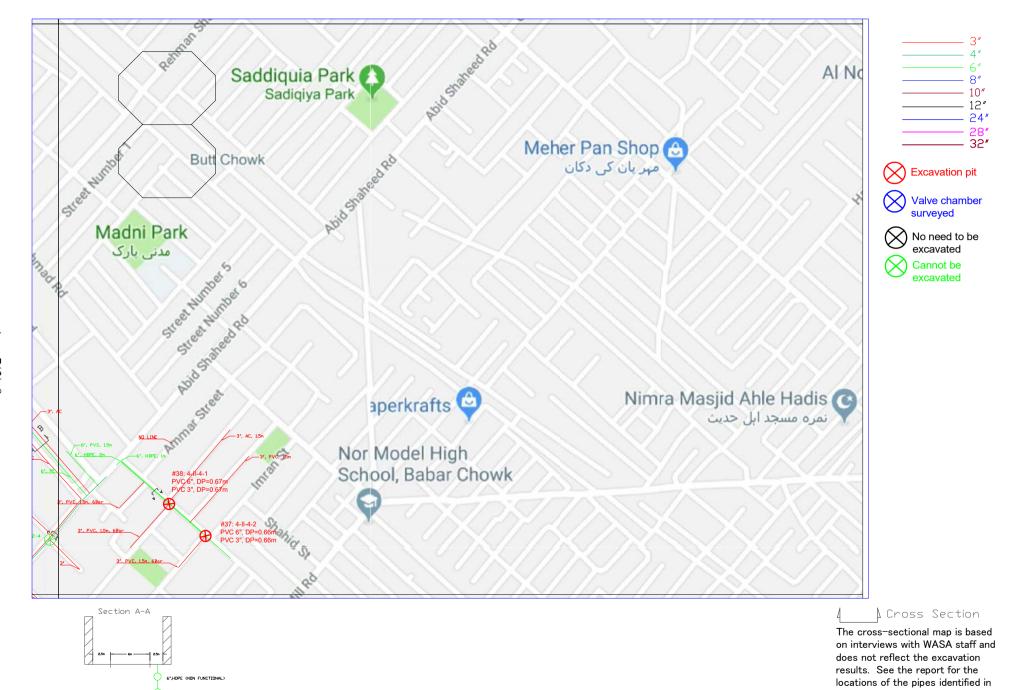
32"

The cross-sectional map is based on interviews with WASA staff an does not reflect the excavation results. See the report for the locations of the pipes identified in the survey.

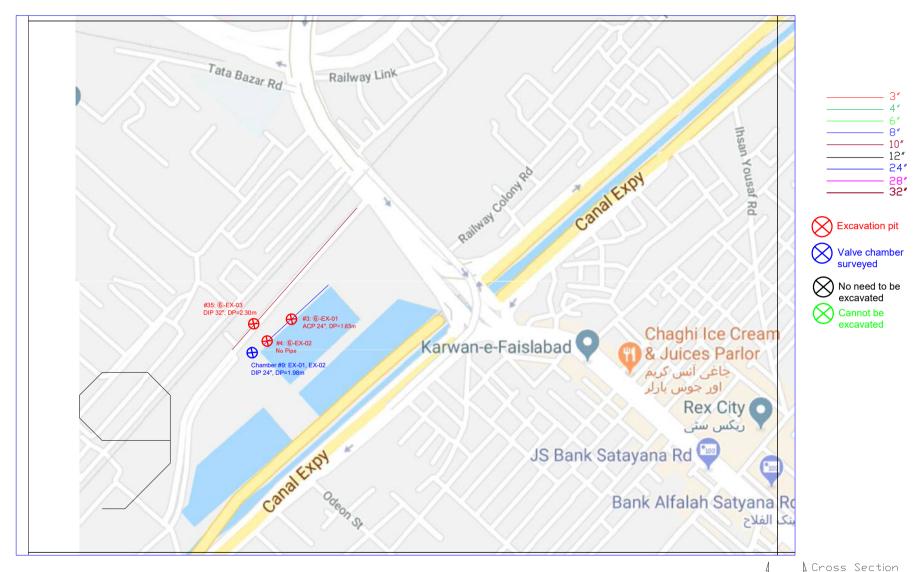








the survey.



The cross-sectional map is based on interviews with WASA staff and does not reflect the excavation results. See the report for the locations of the pipes identified in the survey.



Cross Section

The cross-sectional map is based on interviews with WASA staff and does not reflect the excavation results. See the report for the locations of the pipes identified in the survey.

8"

10" - 12"

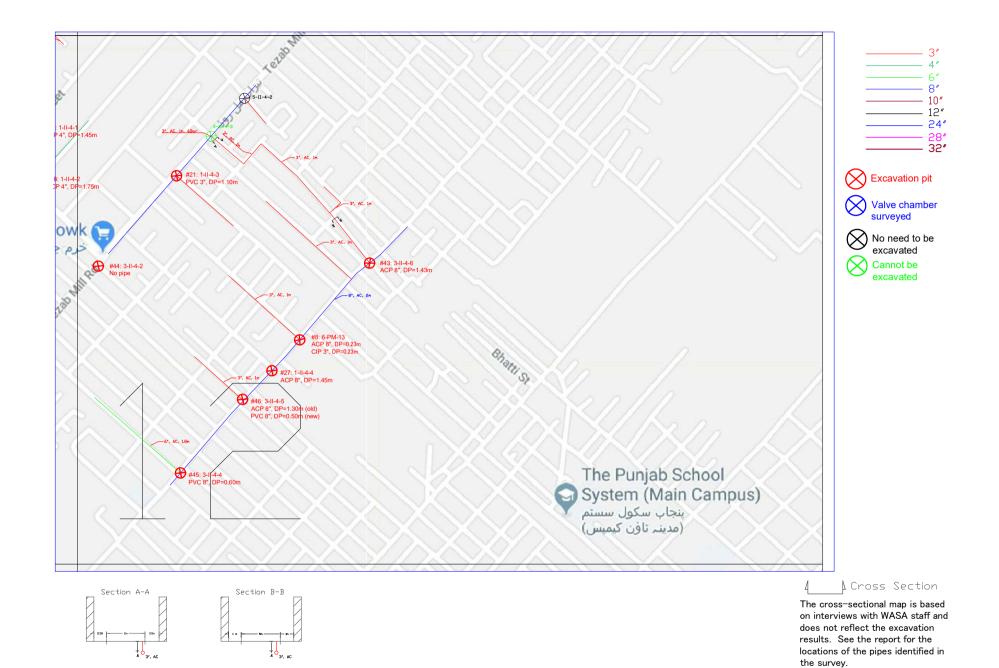
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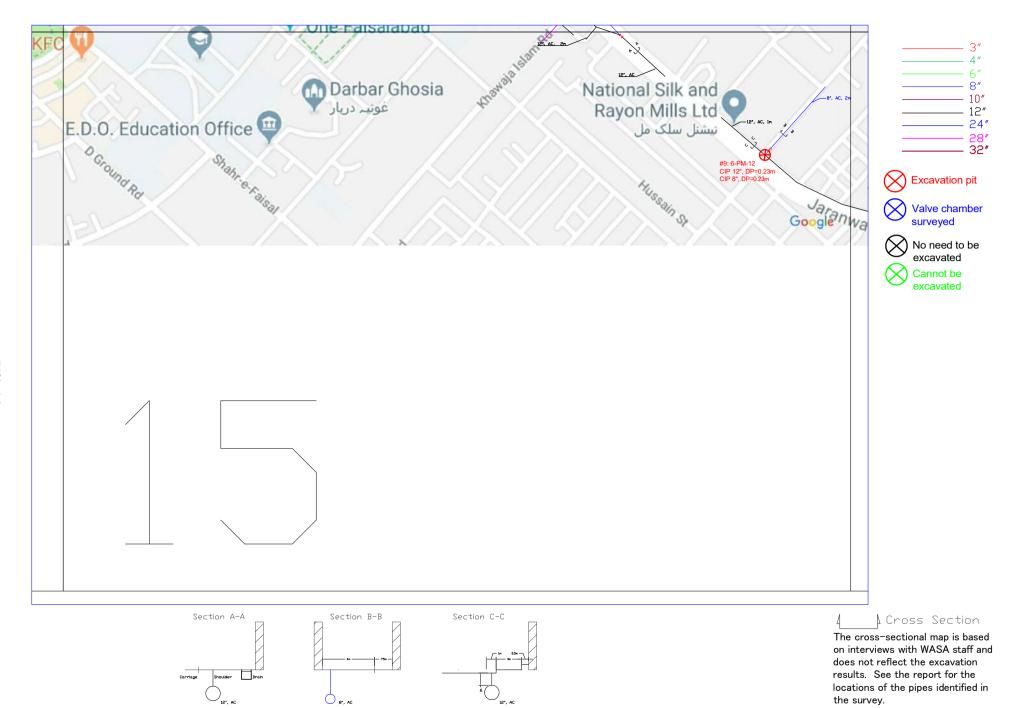
- 32"

surveyed

excavated







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Preliminary Design

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1. Technical Parameter of Intake and Treatment Facilities

1. Technical Parameter of Intake and Treatment Facilities

1.1 Intake and Raw Water Transmission Facilities

(1) Planned Intake Capacity

Production capacity is set at 45,500 m3/d (10 mgd).

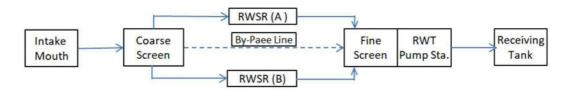
Intake capacity is designed at 5% of production capacity.

Therefore, design capacity of intake becomes 47,900 m³/d (45,500 x 1/(1-5%).

note: Water right from the Irrigation Department is 20 cfs (48,900 m³/d) which exceeds above design capacity of intake.

(2) Flow Diagram of Intake and Raw Water Transmission

Water of the irrigation canal (RBC: Rahk Branch Canal) is transmitted through coarse screen to the existing raw water storage reservoirs (RWSR A and B). Clarified water in RWSR is transmitted to the raw water transmission pump station after screening by fine screen, then it is transferred to the Receiving tank for water purification process. The by-pass line is provided for direct transmission to the raw water transmission pump station during winter season when raw water turbidity becomes low in the irrigation canal. The flow diagram of intake and transmission is shown in flow chart below.



(3) Intake and Raw Water Transmission Facilities

1) Intake Mouth

Water is taken from RBC at its most down stream, where its cross section is about 12m width bottom with no lining and concrete lining at the both sides with slope of 1:1.5. Water depth is measured at a.1.54 m where water level and bottom level is measured as + 184.86 and + 183.32 respectively, thus area of cross section becomes about 22 m2. According to the irrigation canal authority, the design flow there is 11. 27 m3/s and velocity is calculated at 0.51 m/s. Right after the intake point, water is diverted to Dijkot Disty (distribution canal). According to the information by a operator in OJK WTP, water level of the canal is stable within a small change.

Dimensions of Intake Mouth

Main road runs along RBC at the intake point, therefore limited space is available for construction of intake mouth (about 3.3 m between road edge and canal shoulder).

Front Yard front yard level (50 cm higher than canal bed) Inflow velocity				183.82 m 0.6 m/s
Cross section	2 intake mouths are provided each having dimensions	width water depth cross section area		0.6 m 1.01 m 0.606 m ²
	inflow velocity	cross section area		0.46 m/s < 0.6
Appurtenant Facilities	Stop log at mouth screen is provided in the water treats the site of intake mouth for operation	•	space	e available at

2) Raw Water Transmission Main (Intake mouth ~ Coarse screen)

Pipe Materials	Ductile Cast Iron
Diameter	800 mm
Velocity	1.10 m/s

3) Coarse Screen and Branch Valve Chamber

Coarse Screen Number 1 unit

Type Manual operation bar screen width 1.6 m height 3.0 m

bar spacing 5.0 m

Branch Valve Number to RWSR A & B and By-pass to RWS Pump Sta. 3 units

Type Short body butterfly valve Diameter 800 mm

4) Inflow and Outflow of RWSR

Inflow Transmission pipe is installed to prevent short cut flow from inflow to outflow

and to utilized capacity and surface area of RWSR effectively.

Pipe Materials Reinforced concrete pipe Diameter 900 mm

Outflow pit Provide two weirs placed at the pit for each RWSR for surface water intake, where

submerged weir is used

Overflow rate per one weir $0.277 \text{ m}^3/\text{s}$ Weir width 1.0 mOverflow height 0.5 m

During canal stoppage period, a gate is provide at the bottom of the pit to intake stored

canal water in RWSR

Number 1 unit Size 600 x 600 mm

Outlet Pipe (RWSR ~ RWT Pump Sta.)

Pipe Materials Ductile Cast Iron
Diameter 800 mm
Velocity 1.10 m/s

5) Raw Water Transmission Pump Station

Receiving Chamber Fine screen is provided in channels before pump well of RWT Pump Sta.

The dimensions of channel are

 Width
 1.15 m

 Length
 2.9 m

 Height
 4.3 m

Install a gate at inflow and outflow of the channel respectively for maintenance of fine

screen

Number at each channel 2 units Size 600 x 600 mm

Fine Screen Type Automatic vertical mesh screen

Number 2 units width 1 m height 5.4 m mesh 12 mm

Pump Well Dimensions of pump well is determined to fit those pump room (length and height)

Width3.6 mLength19.6 mWater depth4.3 mEffective capacity 303 m^3 Detention time9 min

Pump Room Dimensions of pump room is determined based on capacity and number of pumps

with appurtenant equipment (flow meter, control valve, etc.)

Width 4.2 m Length 20.0 m Height below beam soffit 6.6 m

Electric Room Dimensions of electric room is determined to meet the spaces required for

electric panels for power, pumps and control together with hatch room, etc. Width $$4.2\ m$$ Length $$20.0\ m$$

Height below beam soffit 3.5 m

(6) Raw Water Transmission Main

Dimensions of a flow meter and a control valve which are installed in the pump room.

Flow meter Type Electric magnetic flow meter
Diameter 500 mm

Flow control valve Type 500 mm
Butterfly valve

Diameter 500 mm

Transmission Main (Raw Water Transmission Pump Sta. ~ Receiving Well)

Pipe Materials Ductile Cast Iron
Diameter 700 mm

1.2 Water Treatment Plant Facilities

(1) Pre-Treatment Facilities (Flash Mixing, Flocculation and Sedimentation Tanks)

	Production Capacity						$45,500 \text{ m}^3/6$	d
	Treatment Capacity	処理過程の	ロス	5%	i		47,900 m ³ /c	d
							$2,000 \text{ m}^3/1$	h
							33.3 m ³ /1	min
							0.554 m ³ /s	S
1)	Receiving Tank							
	Detention time						3 min	
	Number of comparts						2 unit	S
	Dimensions per	Width					3.0 m	
	compartment	Length					3.0 m	
		Water depth					5.5 m	
		Capacity	(per tank	()			49.5 m^3	
	Appurtenant	Inlet pipe		Diameter	700 m	nm	1 no.	
		Inflow gate		Size	600 x 600 m	ım	2 unit	S
		By-pass gate	2	Size	500 x 500 m		el 1 nuit	S
		Outlet pipe		Diameter	600 m	ım	2 nos	
		Drain pipe		Diameter	150 m	nm	2 nos	
2)	Flash Mixing Tank							
	Mixing Method					Hydrauli	ic Mixing (Water	
	Mixing Intensity						500 sec	-1
	Number of Tanks						2 tank	7.7
	Treatment Capacity	per Tank				23,950 m ³ /d	$0.277 \text{ m}^3/\text{s}$	S
					a	t Inflow	at Outflow (M	(ixing)
	Detention Time					146 sec	24 sec	
	Dimensions	V	Vidth			3.0 m	3.0 m	
		L	ength			3.0 m	0.8 m	
		V	Vater dep	oth		4.5 m	2.8 m	
		C	apacity			40.5 m^3	6.72 m^3	
	Mixing Intensity	V	Vater tem	perature			15 °C	
		N	lixing int	tensity		0 0 0	497 sec	-1

Mixing Intensity		
$G = (1/\mu x) (\rho x g x)$	$q \times hf / V)^{0.5} =$	497 sec ⁻¹
2210.	μ: Viscosity	0.00098 kg/m/s
	ρ : Specific gravity of water	$1,000 \text{ kg/m}^3$
	g: Gravity acceleration	9.8 m/sec^2
	q : Flow rate	$0.277 \text{ m}^3/\text{s}$
	hf: Head loss (freefall depth)	0.60 m
	V : Volume	6.72 m^3

3) Flocculation Tank

Method Up-and-Down Flow Number of Tanks 4 tanks Treatment Capacity per Tank 11,975 m³/d $0.139 \text{ m}^3/\text{s}$ Mixing Intensity about $20 \sim 60 \text{ sec}^{-1}$ Number of Channels 4 列 Dimensions per Channel Width 1.85 m Length 9.75 m Water depth 3.5 ~ 3.8 m Detention Time about 30 min Energy of Dissipation (GT-value) about 80,000

$G = (1/\mu^* (\rho^*))$	g * q * h	f / V))^0.5	
where,	μ:	viscosity (15°C)	0.00098 kg/m/s
	ρ:	specific gravity of water	$1,000 \text{ kg/m}^3$
	g:	gravity acceleration	9.8 m/s^2
	q :	flow rate	variable m ³ /s
	V:	volume	variable m ³

Energy Dissipation by Phases

a. Designed Treatment Capacity (Day Maximum Demand in 2038) per Tank 0.1386 m³/s

4			Channel	Number		
Dimensions	unit	No.1	No.2	No.3	No.4	Total
Baffle Wall	nos.	5	5	5	5	20
Baffle Plate	nos.	3	4	5	6	18
slit dia. and layout	mm		^w 300 x	^h 80 x ⁿ 4		
area of slit	m ²	0.288	0.384	0.480	0.576	-
Velocity at Slit	m/s	0.481	0.361	0.289	0.241	-
Head Loss at Slit	m	0.164	0.092	0.059	0.041	0.356
Volume of Channel	m ³	68.1	65.2	63.5	62.4	259.2
width	m	1.85	1.85	1.85	1.85	-
length	m	9.7	9.7	9.7	9.7	-
water depth	m	3.80	3.63	3.54	3.48	3.61
Detention time of Channel	sec	491	470	458	450	1,869
Mixing Intensity	sec-1	57.8	44.2	35.9	30.2	43.6
GT-value	-	28,400	20,800	16,400	13,600	79,200

b. Designed Treatment Capacity (Day Minimum Demand in 2038) per Tank $0.1048 \text{ m}^3/\text{s}$

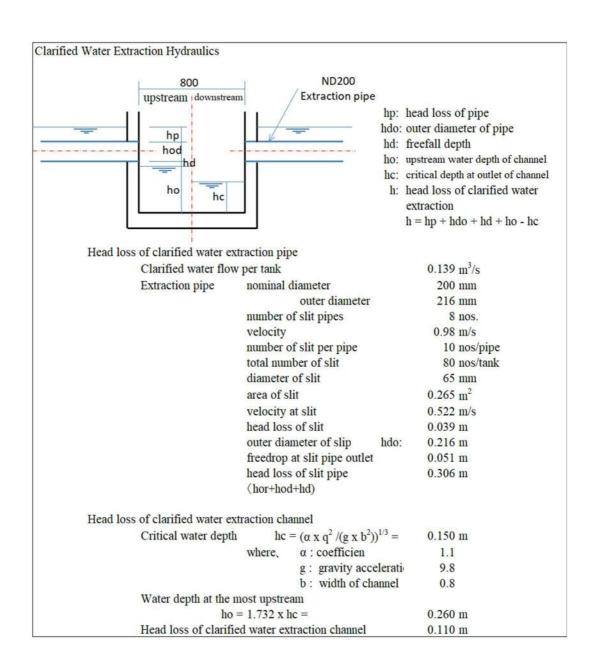
			Channel	Number		
Dimensions	unit	No.1	No.2	No.3	No.4	Total
Baffle Wall	nos.	5	5	5	5	20
Baffle Plate	nos.	3	4	5	6	18
slit dia. and layout	mm		^w 300 x	^h 80 x ⁿ 4		
area of slit	m ²	0.288	0.384	0.480	0.576	-
Velocity at Slit	m/s	0.364	0.273	0.218	0.182	-
Head Loss at Slit	m	0.094	0.053	0.034	0.023	0.204
Volume of Channel	m ³	65.5	63.8	62.9	62.3	255
width	m	1.85	1.85	1.85	1.85	-
length	m	9.7	9.7	9.7	9.7	-
water depth	m	3.65	3.56	3.50	3.47	3.55
Detention time of Channel	sec	625	609	600	594	2,428
Mixing Intensity	sec-1	44.6	33.9	27.4	22.6	33.3
GT-value	-	27,900	20,600	16,400	13,400	78,300

			水路ナ	ンバー		
Dimensions	unit	No.1	No.2	No.3	No.4	Total
Baffle Wall	nos.	5	5	5	5	20
Baffle Plate	nos.	3	4	5	6	18
slit dia. and layout	mm		^w 300 x	^h 75 x ⁿ 4		
area of slit	m ²	0.317	0.384	0.480	0.576	-
Velocity at Slit	m/s	0.198	0.163	0.130	0.109	-
Head Loss at Slit	m	0.028	0.019	0.012	0.008	0.067
Volume of Channel	m ³	63.0	62.5	62.1	61.9	250
width	m	1.85	1.85	1.85	1.85	-
length	m	9.7	9.7	9.7	9.7	-
water depth	m	3.51	3.48	3.46	3.45	
Detention time of Channel	sec	1,007	999	992	989	3,987
Mixing Intensity	sec-1	24.8	20.5	16.4	13.4	19.3
GT-value	-	25,000	20,500	16,300	13,300	75,100

)	Sedimentation Tank						
	Metod				I	nclining Tul	be
	Number of Tank						tanks
	Treatment Capacity	per Tank			$12,000 \text{ m}^3/\text{d}$	500	m ³ /h
	Surface Loading					1	$m^3/h/m^2$
	Efficiency					80	%
	Inclining Tube	Size of tul	oe .			80 x 80	mm
		Installation	n height			1.0	m
		Installation	n angle to horizontal			60	deg.
		Effective a	area			0.577	m^2/m
		Module of	f inclining tube (1.0	0 x 1.0m)		7.22	m ² /module
		Effective a	area of tank			625	m ² /tank
		Number o	f modules			>87	units/tank
	Dimension of Tank	Width	Number of compar	tments		2	compartments
			Number of module	s per compartme	ent	4	units
			Distance between n			10	cm
			Width of clarified wa	ter collecting char	nnel (incl.side wall)	1.3	m
			Width per tank			9.7	m
		Length	Number of module	s (87 x 1/8)		11	units
			Installation length of	modules (incl. spac	e between modules)	12.0	m
			Stilling zone length	(incl. wall) (1	.5 + 0.25)	1.75	m
			Length of tank			13.75	m
		Water depth	Water depth above	inclining tube		0.8	m
			Installation height of	of inclining tube	(incl. support)	1.2	m
			Height under the in	clining tube		1.5	m
			Free board			0.4	m
			Total water depth			3.5	m
	Clarified Water	Method				Pipes	
	Extraction		Extraction pipe	Diameter		200	mm
			Number of pipes			4	nos/compartment
			Weir loading			< 200	$m^3/d/m$
		Clarified v	water extraction char	nnel		Reinforce	concrete
			**			0.0	

0.8 m

Net width

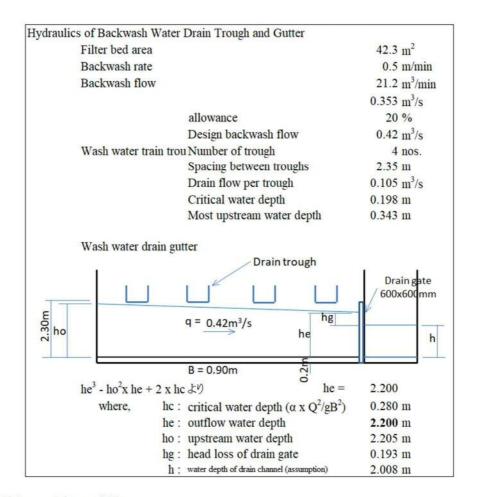


(2) Rapid Sand Filter and Clear Water Reservoir/Transmission Pump Station)

1) Rapid Sand Filter

a. Treatment Capacity				
Production				$45,500 \text{ m}^3/\text{d}$
Treatment capacity	loss	3 %		$46,900 \text{ m}^3/\text{d}$
b. Dimensions				
Type	Filter media			single layer)
	Filtration			istant rate
	Flow control		Equal	split at inlet
Filtration Rate				140 m/d
Filter Media	Filter sand Effect	tive size		0.9 mm
		rmity coefficient		1.4
		ness of sand layer		100 cm
	Supporting Numb			4 layers
		ness of each layer		5 cm
		e of particle size		$2 \sim 50 \text{ mm}$
	Thick	ness of gravel layer		20 cm
Underdrain	Type		nc	zzle type
	Head loss at back	cwashing	Maximum	70 cm
Number of Filters				8 filters
	Filtration flow pe	r filter		$5,860 \text{ m}^3/\text{d}$
Dimensions of	Width			4.5 m
Filter	Length			9.4 m
11101	Filter area			42.3 m ²
	Timer area	Filtration rate		139 m/d
	Height	Titration face		5.75 m
	Treight	Water depth above sand		1.25 m
		Clogging loss		1.30 m
		Thickness of Filter sand and grav	vel	1.2 m
		Underdrain		1.1 m
		Free board		0.9 m
		******		was a second
	Backwash	Width		0.9 m
	drain gutter	Length		9.4 m
		Water depth		4.5 m
	Operation/	Width		4 m
	Pipe gallery	Length		44.4 m

Filter Washing	Method		Air + Backwash
	Air Scouring	Scouring rate	0.9 ~ 1.0 m/min
		Scouring time	10 min
	Backwash	Rate of initial backwash (Air+Water)	0.25 m/min
		Washing time	$2 \sim 3 \text{ min}$
		Backwasing (water only)	$8 \sim 10 \text{ min}$
	Backwash water	Number of (1 池当たり)	4 本
	drain trough	Width	40 cm
	0.00	Height	40 cm
		Length	4.65 m



Pipe, Valves and Gates of Filte

pipe· valve· gate	size (mm)	velocity (m/s)	pipe· valve· gate	size (mm)	velocity (m/s
Inlet gate	300 x 300	0.76	Backwash main	500	2.01
Wash drain gate	600 x 600	1.10	Air pipe	250	15.3
Filtered water pipe	250	1.51	Air main	300	7.8
Filtered water main	800	1.08	Filter drain pipe	150	-
Backwash pipe	450	2.48	Filter drain main	200	-

2) Clear Water Reservoir and Treated Water Transmission Pump Station

a. Clear Water Reservoir

Detention Time*		apprx.	1.2 h
Capacity			2,280 m ³
Number of Compartment			2 nos.
Dimensions per Compartment	Width Length Effective water depth Total volume Effective volume (approx. 98%) Free board Detention time		15.8 m 17.2 m 4.5 m 1,223 m 2,400 m3 0.65 m 1.27 hours

注* refer to Attached Table 1-1 volume of Clear Water Reservoir at Canal Stoppage

b. Treated Water Transmission Pump Station

ansimission F	rump Station	
		iber of
Width	including pipe gallery of punp suction and delivery pipes	9.4 m
Length		32.4 m
Height	below beam soffit	7.8 m
Dimensi	ons of electric room is determined to meet the spaces requir	ed for
electric	panels of power, pumps and controlst together with hatch roo	om, etc.
Width		4.2 m
Length		32.4 m
Height	below beam soffit	3.5 m
	Dimensi pumps v Width Length Height Dimensi electric Width Length	Length Height below beam soffit Dimensions of electric room is determined to meet the spaces require electric panels of power, pumps and control式 together with hatch roow Width Length

Volume of Clear Water Reservoir at Canal Stoppage Attached Table 1 – 1

1 In Year 2028

Water Demand Day Maximum Water Demand 17,860 m3/d

> Day Minimum Water Demand 13,500 m³/d $750 \text{ m}^3/\text{h}$

Supply hour at 18 hours (5 to 22)

Supply Treatment Plant (12 hours operation $3,700 \text{ m}^3/\text{d}$ $310 \text{ m}^3/\text{h}$

from Arterial Main

9,800 m³/d 1,640 m³/h

note: Receiving water from Arterial Main for 6 hours/day (3 times each 2 hours)

water demand is assumed as minimum demand, i.e., about 75% of Day Maximum demand

Canal close is for 21 days

Efective capacity of RWSRs of A and B is 78,300 m3 3,700 m3/d

1.1 Constant Transmission Flow

		Supp	ly		Demai	nd	Balance		Trans. Flow
hr	WTP	Arterial M.	Total	Σ	(Transmission)	Σ			Rate
								750	
1			0	0		0	0	750	
2			0	0		0	0	750	
3			0	0		0	0	750	
4			0	0		0	0	750	
5			0	0	750	750	-750	0	100%
6		1,640	1,640	1,640	750	1,500	140	890	100%
7	310	1,640	1,950	3,590	750	2,250	1,340	2,090	100%
8	310		310	3,900	750	3,000	900	1,650	100%
9	310		310	4,210	750	3,750	460	1,210	100%
10	310		310	4,520	750	4,500	20	770	100%
11	310		310	4,830	750	5,250	-420	330	100%
12	310	1,640	1,950	6,780	750	6,000	780	1,530	100%
13	310	1,640	1,950	8,730	750	6,750	1,980	2,730	100%
14	310		310	9,040	750	7,500	1,540	2,290	100%
15	310		310	9,350	750	8,250	1,100	1,850	100%
16	310		310	9,660	750	9,000	660	1,410	100%
17	310		310	9,970	750	9,750	220	970	100%
18	310	1,640	1,950	11,920	750	10,500	1,420	2,170	100%
19		1,640	1,640	13,560	750	11,250	2,310	3,060	100%
20			0	13,560	750	12,000	1,560	2,310	100%
21			0	13,560	750	12,750	810	1,560	100%
22			0	13,560	750	13,500	60	810	100%
23			0	13,560		13,500	60	810	100%
24			0	13,560		13,500	60	810	100%
	3,720	9,840	13,560		13,500				

1.2 Water Level Control (Transmission Flow Control)

		Supp	ly		Dema	nd	Balanc	:e	Trans. Flow
hr	WTP	Arterial M.	Total	Σ	(Transmission)	Σ			Rate
								600	
1			0	0	0	0	0	600	
2			0	0	0	0	0	600	
3			0	0	0	0	0	600	
4			0	0	0	0	0	600	
5			0	0	600	600	-600	0	80%
6		1,640	1,640	1,640	900	1,500	140	740	120%
7	310	1,640	1,950	3,590	975	2,475	1,115	1,715	130%
8	310		310	3,900	900	3,375	525	1,125	120%
9	310		310	4,210	750	4,125	85	685	100%
10	310		310	4,520	600	4,725	-205	395	80%
11	310		310	4,830	600	5,325	-495	105	80%
12	310	1,640	1,950	6,780	900	6,225	555	1,155	120%
13	310	1,640	1,950	8,730	975	7,200	1,530	2,130	130%
14	310		310	9,040	900	8,100	940	1,540	120%
15	310		310	9,350	750	8,850	500	1,100	100%
16	310		310	9,660	600	9,450	210	810	80%
17	310		310	9,970	600	10,050	-80	520	80%
18	310	1,640	1,950	11,920	750	10,800	1,120	1,720	100%
19		1,640	1,640	13,560	900	11,700	1,860	2,460	120%
20			0	13,560	600	12,300	1,260	1,860	80%
21			0	13,560	600	12,900	660	1,260	80%
22			0	13,560	600	13,500	60	660	80%
23			0	13,560	0	13,500	60	660	
24			0	13,560	0	13,500	60	660	
	3720	9,840	13,560		13,500				1.00

Dimensions of Reservoir

Constant Transmission Flow Water Level Control width (m) 31.6 (15.8m x 2 - 0.4) 31.6 (15.8m x 2 - 0.4) length (m) 21.6 (4.4m x 5span - 0.4) 17.2 (4.4m x 4span - 0.4) water depth (m) 4.5 4.5 Effective Area (m2) 683 (wall thick: 0.4m) 544 (wall thick: 0.4m) 3,072 2,446 volume (m3)

(3) Waste Water Treatment Facilities

Waste water treatment facilities are composed of /Sludge Buffer Tank/Waste Water Tank, Sludge Thickener and Sludge Drying Bed

1) Sludge Buffer Tank

a. Sludge Extraction of Sedimentation Tank

Treatment Flow	$47,900 \text{ m}^3/\text{d}$
	Maximum Turbidity

Sludge	Sludge extraction	(times of extraction)	4 times/d
	Maximum Turbidity	(RWST outflow)	200 NTU
	Clarified Water Turk	pidity	5 mg/l
	Alum Dosage rate (S	Solid 17%)	33 mg/l
	TS/Turbidity		1.0
	Sludge	Solid weight	9,710 kg/d
		Sludge content	1.0%
		Sludge flow	$970 \text{ m}^3/\text{d}$
		Allowance	20%
			$1,160 \text{ m}^3/\text{d}$

Sludge flow per extraction 290 m³/time

Two tanks of Sludge buffer tank is planned including one stand-by, each having volume of 250 m³ (refer to Attached Table 1-3a Volume of Sludge Buffer Tank)

b. Dimensions of Sludge Holding Tank

Number of Tank 2 tanks

Capacity (at Maximum turbidity,200 NTU) 250 m³

Time for Sludge Transfer to Sludge Thickener Continuous pump operation under water level control

Dimensions	Width	4.2 m
	Length	20.0 m
	Effective water depth	3.0 m
	Volume	504 m^3

c. Appurtenant Equipment

Mixer

Submersible mixers to prevent settlement of sludge in tank and transfer sludge with uniform sludge content as possible to sludge thickener. Two mixers per compartment is installed.

Sludge transfer pump

Sludge is transferred by submersible waste water pumps. Two units of pumps including one stand-by are installed in each tank.

Sludge is pumped to a chamber located on top slab of sludge holding tank, from where sludge is transferred to sludge thickener by gravity.

From the said sludge chamber, half of sludge is transferred to sludge thickener and remaining sludge is transferred directly to sludge drying bed by gravity to avoid large scale of sludge thickener when raw water turbidity becomes high.

High turbid raw water of 1,000 NTU or larger will occur only several times in wet sean in a year.

2) Waste Water Tank

a. Backwash Waste Water and Supernatant Water

Treatment Flow of Filter	$46,900 \text{ m}^3/\text{d}$
Number of Filter	8 filters

Filter Backwash Waste Water

Filter bed area	42.3 m^2

Filter bed area					42.3 m^2
	Washing	rate (m ³ /min) t	ime(min)	waste water
		initial	0.25	2~3	21
		Final	0.5	8~10	169
Filter run					48 h
Washing time per	day				4 times/d
Waste water volum	ne				190 m ³ /time
	Allowance				20%
					$910 \text{ m}^{3}/\text{d}$
	Inflow of Ba	nckwash was	te water p	er time	230 m ³ /time
Sllid weight	Inflow and o	outflow turbi	dity is 5 a	nd 1 respective	ely 190 kg/d
Solid content			SAFAR CLEASE SAFA		0.21%

Supernatant (24 hours continuous inflow)

850 m³/d from Sludge Thickener from Sludge Drying Bed 300 m³/d

Number of waste water tank is two tanks including one stand-by, each having volume of 210 m³ (refer to Attached Table 1-3b Volume of Waste Water Tank)

b. Dimensions of Waste Water Tank

Number of Tank 2 tanks

210 m³ Volume per tank

Time for Dewateringr Continuous pump operation under water level control

Dimensions	Width	4.2 m
	Length	20.0 m
	Effective water depth	2.5 m
	Volume	210 m3

c. Appurtenant Equipment

Submersible mixers to prevent settlement sludge in waste water in tank and transfer waste water Mixer with uniform sludge content as possible to receiving tank for recycle use. Two mixers per tank is installed.

Waste water transfer pump

Two submersible waste water pumps including one stand-by are installed in each compartment.

Pumps for Preventing Overflow

Because no appropriate waste water drain facilities is available around the water treatment plant, all waste water and overflows of plant inflow to the waste water tank.

Therefore, drain pumps are planned to be installed in the waste water tank to drain the RWSR in ordinal plant operation. As required, drain water is pumped to drain channel (former irrigation distribution canal which is not utilized at the present) located along north-west boundary wall of the plant.

4)	Sludge Thickener				
	Type			Gravity Ce	nter Feed Thickener
	Sludge (solid weight	t)			10,080 kg
	Sludge Loading				20 kg/d/m^2
	Number of Thicken	er			2 池
	Surface Area requir	ed			504 m^2
	111				252 m ² /thickener
	Dimensions	Diameter			12.8 m
		Effective	water depth		3.5 m
		Sludge	Depth		0.5 m
		Deposit	Sludge volume)		64 m^3
		Slope of			10%
		-	enter feed chamber	Dia.	2.5 m
		Size Slud	ge extraction pit	Dia.	2.2 m
	Sludge Extraction	Type of s		Ro	otating type
	100 To	Extraction	n pipe	Dia.	150 mm
5)	Thickened Sludge T	ransfer Dun	nn Station		
2)	Pump Room	Basement	• 1		60 m ²
	rump Room	Dascincii	Width		5 m
			Length		12 m
			Height		5.55 m
			Height		3.33 III
	Electric Room	Ground F	loor		60 m2
			Width		5 m
			Length		12 m
			Height	below beam soffit	3.5 m
6)	Sludeg Drying Bed				
	Sludge (Solid Weigl	nt)*1			911,400 kg/年
	Annual Sludge Load	ling			220 kg/m^2
	Floor Area of Sludg	e Drving B	ed		$4,140 \text{ m}^2$
	Number of Beds	, ,			7 beds
	Area				$600 \text{ m}^2/\text{bed}$
	Dimensions	Width			20 m
		Length			30 m
		Filter	Sand		30 cm
			Gravel		20 cm
		Water de	pth above sand		1.5 m
		Free boar			50 cm
	Appurtenant	Piping	Inlet Pipe	PE	150 mm
	Faculities and	10	Supernatant Drain	RCP	150 mm
	Equipment	Stop log	(at supernatant drain pit)	W x H (20cm x 8nos)	
	***	Ramp	(for dried sludge disposal)	W x L (3.0 x 7.5m)	1 pl/bed

Note *1: Sludge (Solid Weight)

	Q =	47,900	The state of the s		
Month	Turbidity	Alum	Sludge		Sludge by Seasons
	(average)		kg/d	kg/mon	Dry Season Wet Season Annual (kg)
Jan	21	29	1,091	33,800	33,800
Feb	16	27	830	23,200	23,200
Mar	32	34	1,674	51,900	51,900
Apr	46	42	2,435	73,100	73,100
May	44	41	2,328	72,200	72,200
Jun	67	50	3,530	105,900	105,900
Jul	121	59	6,218	192,800	192,800
Aug	114	58	5,871	182,000	182,000
Sep	52	44	2,744	82,300	82,300
Oct	22	30	1,151	35,700	35,700
Nov	19	28	984	29,500	29,500
Dec	18	28	937	29,000	29,000
Ave/Total	48	39	2,483	911,400	kg/year 203,100 708,300 911,400

Attached Table 1-2 Mass Balance

Maximum Turbidity of Canal Water: 1,000 NTU and Raw Water Storage Reservoir: 200 NTU

Water & Wast	re	RW	/SR	Treatment	Flow after	Waste wa	ater/Sludge	note		
Water Stream		Turbidity	Solid weight	Flow	Treatment	Flow	Sokid weight			
		NTU	mg/l	m ³ /d	m ³ /d	m^3/c	kg/d			
Treatment Pr Production	rocess			45,500						
Treatment Fl	ow	200	200		47,900	34	•	Loss of treatment (est TSS/NTU = 1	imated) .0	59
Sedimentation	Inflow	200	200	47,900			10,080	Alum dosage 66, sludg	ge concent	ration 0.85%
Tank	Outflow				46,720	1,180)	Loss of treatment		2.59
Rapid	Inflow	9.2	5	46,720			190	Washing per Filter		190 m³/filter
Sand Filter	Outflow		1		45,810	910)	washing time	4 filters/c	1 1.99
Clear Water	Inflow		1	45,810				treated water turbidity		1 mg/l
Resevoir	Outflow		1		45,620	190	50	other loses 19	$90 \text{ m}^3/\text{d}$	0.409

注: Refer to analysis in "losses in treatment process" below for figures shown in the above table

Waste Water Treatment

		Turbidity	Solid weight	Treatment flow	flow after treatment	Recycle Water	Solid weight	note	
		NTU	mg/l	m3/d	m3/d	m3/d	kg/d		
Sludge Buffer	inflow		8,500	1,180	-		10,080	Sludge content 0.85	%
Tank	outflow		8,500		1,180		10,080	Loss	2.5%
Sludge	inflow		8,500	1,180			10,080	Sludge content 0.85%	,
Thickener	outflow		30,000		330		9,900	outflow to sludge drying bed 3%	28.3%
	1900c		210		850	850	180	supernatant water (sludge content)	0.02%
Sludge	inflow		30,000	330			9,900	10 20	
Drying Bed	inflow		329,000		30		9,870	Dewatered sludge (sludge content)	35%
			100		300	300	30	supernatant water	
Waste Water	inflow		210			850	180	Inflow from sludge thickener	
Tank			100			300	30	Inflow from sludge drying bed	
			180			1,150	210	Total of supenatant water	
	outflow		180			1,150	210	Turbidity of recycle water 0.02%	(200mg/l)
						910	190	from waste water tank	
Recycling	inflow		1,150			1,150	210	from Recycling Sump	
			200			45,840	9,170	from Raw water	4.5%
			200			47,900	9,570	Total inflow	

Clarifier:	Treatment capacity	47,900 m3/d	Filter:	Treatment capacity	46,690 m3/d
	Turbidity	200 NTU		No. of filter for wash	4 filters/d
	Alum Dosage (8% liquid Alum)	66 mg/l		Filter area	42.3 m2
	Solid	10,320 kg/d		Washing rate & time	
	Sludge extraction			initial w/air	0.25 x 2min
	Effluent to filter	5 mg/l		backwashing	0.5 x 8min
	or 17,700 x 5/(350+80x0.234) =	236 kg/d		Wahing waste water	190 m3/filte
		3%			760 m3/d
	Sludge content	1%		Allowance	20%
	Sludge volume	1,010 m3/d			910 m3/d
	Allowance for design of	20%		Loss of Backwash waste water	1.9%
		1210 m3/d		Solid content in backwash waste water	
	Loss of sludge extraction	2.5%		Turbidity of filtered water	1 mg/l
				Solid	190 kg/d
				Solid content	173 mg/l
Clear Wa	ter Reservoir				
	Sludge content of filtered water	1 mg/l	Total los	s in treatment process	2,310 m3/s
	Minor water loss for plant operation	0.4%			5%
	Loss of water	190 m3/d			

Attached Table 1-3a Volume of Sludge Buffer Tank

Time	Inflow	Outflow	Balance	Cumulativ	e
				150	
1		50	-50	100	
2		50	-50	50	
3		50	-50	0	
4	300	50	250	250	Sludge
5		50	-50	200	
6		50	-50	150	
7		50	-50	100	
8		50	-50	50	
9		50	-50	0	A
10	300	50	250	250	Sludge
11		50	-50	200	
12		50	-50	150	
13		50	-50	100	
14		50	-50	50	
15		50	-50	0	
16	300	50	250	250	Sludge
17		50	-50	200	
18		50	-50	150	
19		50	-50	100	
20		50	-50	50	
21		50	-50	0	
22	300	50	250	250	Sludge
23		50	-50	200	
24		50	-50	150	
		1,200			

Volume of Sludge Holding Tank 250 m

Attached Table 1-3b Volume of Waste Water Tank

(filter wahing: 4 time/d, continuous inflow of supernatant)

Time	Infdlow				Outflow	Balance	Cumulative
	Backwash Waste Water	10.770	Spernatant 2	Total			228
1		35	13	48	86	-38	190
2		35	13	48	86	-38	152
3		35	13	48	86	-38	114
4		35	13	48	86	-38	76
5		35	13	48	86	-38	38
6		35	13	48	86	-38	0
7	228	35	13	275	86	190	190
8		35	13	48	86	-38	152
9		35	13	48	86	-38	114
10		35	13	48	86	-38	76
11	228	35	13	275	86	190	266
12		35	13	48	86	-38	228
13		35	13	48	86	-38	190
14		35	13	48	86	-38	152
15	228	35	13	275	86	190	342
16		35	13	48	86	-38	304
17		35	13	48	86	-38	266
18		35	13	48	86	-38	228
19	228	35	13	275	86	190	418
20		35	13	48	86	-38	380
21		35	13	48	86	-38	342
22		35	13	48	86	-38	304
23		35	13	48	86	-38	266
24		35	13	48	86	-38	228
	910	850	300	2,060	2,060	-0	

Volume of Waste Water Tank 418

Backwash waste water and supernatant inflow

(refer to Attached Table 1-2 Mass balance)

backwash waste water 910 m3/d 4times/d

Supernatant 1 Sludge Thickener: 850 m3/d 24hours continuous Supernatant 2 Sludge Drying Bed: 300 m3/d 24hours continuous

Attachment 1-1 Sludge Extraction of Sedimentation Tank

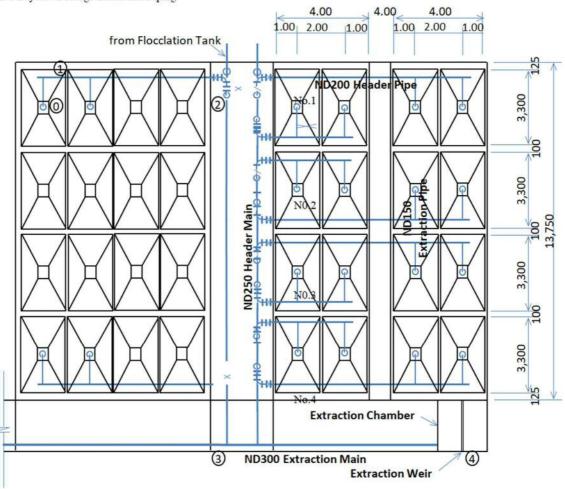
1 Sludge Volume

Turbidity	Maximum 7	Turbidity		Annual Ave	erage Turbid	ity
	Flow(m ³ /d)	Turbidity(NTU)	Sludge (kg)	Flow(m ³ /d)	Turbidity(NTU)	Sludge (kg)
	47,900	200	10,010	47,900	45	2,300
Flocculation Tank 5.0%			501			115
Settling Tank 95.0%			9,510			2,185
Sludge						
Sludge content (extraction):			1%			0.5%
Sludge volume (m3/d)			1,001			437
Flocculation Tank			50			22
Settling Tank			951			415

note: It is assumed that 5% of sludge is settled in the Floccutation Tank and remaining of 95% is in the Settling Tank.

2 Sludge Extraction

2.1 Layout of Sludge Extraction Piping



2.2 Sludge Extraction

Number of tank for Simultaneous Sludge Extraction	1 tank
Sludge Extraction per day	1 times/d
Interval of Sludge Extraction per Tank	6 h
per sludge hopper (simultaneous extraction from 2 hoppers)	45 min

2.3 Sludge Extraction Hydraulics

1) Levels

Water level of Sedimentation Tank (ST) + 187.32 m

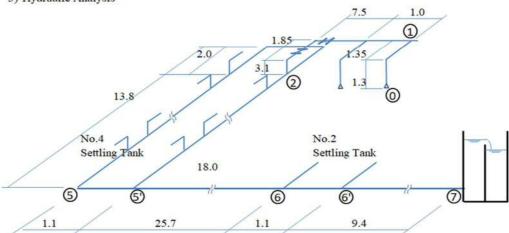
Overflow Weir Tip Level of Sludge Extraction Chamber (SEC) + 185.76 m

Level Deference Overflow height of weir 0.14 m
Water level of ST - Overflow height of SEC) 1.42 m

2) Sludge Extraction Flow and Extraction Time

Sludge extraction flow per a hopper 35.1 l/s Sludge extraction time per two hopper2 ホッパー当たり 7.1 min per tank 1.5 h

3) Hydraulic Analysis



(Max. Turbidity)

section 1 ~ 2:
$$Q = q \text{ I/s}$$
 ND: 200 mm length-1: 1 m
 $Q = 2q \text{ I/s}$ ND: 200 mm length-1: 12.45 m
 $h2 = f1 \times 11/d \times v1^2/2g + (f2 \times 12/d + fy + 2 \times fb90 + fy + fbe) \times v2^2/2$:

 $h2 = f1 \times 11/d \times v1^2/2g + (f2 \times 12/d + f\gamma + 2 \times fb90 + fv + fbe) \times v2^2/2g = 491.1 \text{ q}^2$

where, fl x l1/d = 0.15 f2 x l2/d = 1.91 f γ = 0.87 fb90 = 0.3 fv = 0.3 (B.V) fbe = 0.99

0.074 D0: 150 D
= 0.874 Dβ: 150 Dγ: 20

 ${\bf 2}$. Raw Water Quality and Chemical Dosage

2 Raw Water Quality and Chemical Dosage

2.1 Raw Water Quality

(1) Raw Water Quality Data

Existing old Jhal Khanuana water treatment plant (Old JK WTP: slow sand filtration plant) takes raw water from Rakh Branch Canal (RBC). High turbid raw water is reduced its turbidity in Raw Water Storage Reservoirs (RWSRs) for slow sand filtration. The planned rapid filtration plan will continues to intakes the same raw water source.

At the down stream of RBC, rapid sand filtration plant named New Jhal Kanuana Water Treatment Plant (New JK WTP) constructed under the financial aid of French Government is in operation using the same raw water. Water quality data from both water treatment plant, therefore available.

Important water quality parameters for plant design include water temperature, pH, turbidity, alkalinity. Inanition, water quality parameters of ammonium, iron and manganese are also required to grasp chlorine consumption.

Water quality Data of water temperature, pH and turbidity during $2012 \sim 14$ period is available from Old JK WTP. And from New JK WTP, daily data is also available for the same water quality parameters during $2016 \sim 18$ period. These data is presented in Attached Table 2-1 and 2-2.

Only limited water quality data is available for ammonium, iron and manganese. These water quality parameters are available from Master Plan Study as shown in Attached Table 2-3.

The present plan uses the existing RWSRs to reduce high turbidity of raw water for treatment. The effect of RWSR is able to estimate by comparing turbidities between raw water (canal water) and clarified water by RWSR. Data of Old JK WTP in 2012 \sim 14 show the reduction rate as 60 \sim 90% for raw water turbidity of 50 NTU or lower. For high turbidity raw water, the reduction rate is 80 \sim 90% against raw water turbidity of 50 \sim 500 NTU and very high rate of 96% \sim 97% for raw water turbidity of 500 \sim 800 NTU. These data may indicate large detention time of RWSR affect the above high turbidity reduction rate. However, these data varied widely as shown in Attached Table 2-1.

In the second site survey, settling test was carried out to grasp the effect of RWSR reduction of high turbidity of raw water. The rest results are shown in attached table 2-4 and summarized as follows:

- Existing two RWSRs are available, each having detention time as 24 hours against planed treatment capacity of 47,900 m³/d.
- The test results show the following reduction of turbidity ranging 80 ~ 800 NTU and settling time.

Turbidity	NTU	800	600	400	190	80
Settling tim	e (hour)					
6		320	116	185	58	47
12		185	71	93	<u>-</u>	-
24		76	41	63	16	24

 From the above test results, it is estimated that 90% of reduction rate for high turbid water of 500 NTU or more and 70% of reduction rate for turbidity of 100 NTU or less in 24 hours settling time.

Following reduction rate of raw water is estimated taking the settling efficiency of RWSR into account.

Raw Water Turbidity (NTU)	< 50	100	150	250	> 500
Reduction Rate	60%	65%	70%	75%	80%

The estimated turbidity reduced in RWSR for one year is presented in Attached Table 2-5 using annual data of turbidity in 2017/18 obtained from New JK WTP.

For other water quality parameters of alkalinity, ammonium, iron and manganese, the following test results are shown in below table (refer to attached table 2-3).

		Water Quality Parameters			
Data ource	Water Source	Alkalinity	Iron	manganese	Ammonium
		mg/l	mg/l	mg/l	mg/l
Panjab prvince (2009)	Cenab River*1	-	0.81	0.02	-
WSA-F (2013 ~ 16)	RBC*2	-	-	-	0.3
					$(0.1 \sim 0.5)$
ЛСА Теат (2016)	Cenab River	110 ~ 133 0	0.18 ~ 0.3	< 0.01	< 0.01
in Master Plan Study	RBC*2	70 ~ 120 0	0.36 ~ 0.8	< 0.01	< 0.01

Note*1: Water Source of RBC

(2) Estimated Water Quality

Raw water of RBS is planned to be transfer to RWSR and water is treated using clarified water in RWSR. Turbidity of raw water will be therefore reduced significantly, on the other hand little change is expected for such water quality parameters as water temperature, pH, alkalinity, ammonium, iron and manganese. Reduction of turbidity will affect largely for chemical consumption for coagulation. On the other hand chlorine consumption will not be affected due to little change of ammonium, iron and manganese RWSR which affect chlorine consumption.

Future water quality is estimated for chemical application plan as shown in below table.

Water Quality		Future Plan (2038)			
Parameter	unit	Maximum	Average	Minimum	
Water temperature	°C	30	33	11	
pН	-	8.7	8.2	7.6	
Turbidity	NTU	200	48	10	
Alkalinity	mg/l	120	90	70	
Ammonium	mg/l	0.05	0.03	0.01	
Iron	mg/l	0.8	0.5	0.3	
Manganese	mg/l	0.05	0.02	0.01	

Note: Past water quality data (2017 ~ 2019) of RBC shows that high turbidities (more than 1000 NTU) occurs 1 ~ 5 times in July ~ August period, where the maximum turbidity was 1400 NTU. In design of water treatment, 1000 NTU is used as the maximum turbidity for consideration of economy.

^{*2:} Rakh Branch Canal figures in paresis show minimum~maximum