

## Appendix 3

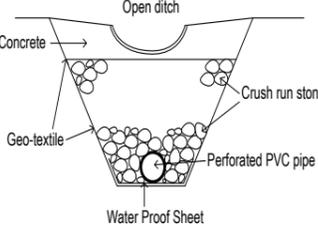
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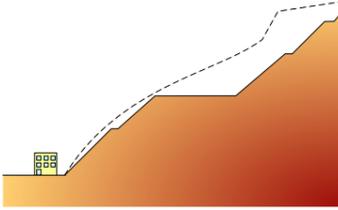
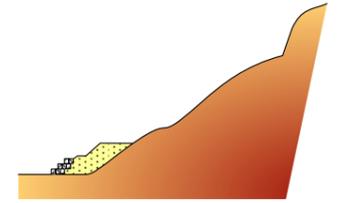
### *Catalog of Countermeasure Work for Slope Disaster*



CATALOG OF COUNTERMEASURE WORK FOR SLOPE DISASTER

LANDSLIDE						
Item	Photo/Drawing	Purpose	Availability	Limitation	Feasibility in Pakistan	Remarks
Surface Drainage (Open ditch)		To collect surface water and to drain them out from landslide area properly	This is the one of the simplest countermeasure works for landslide. The work can be expected to avoid infiltration of rainfall into the landslide block.  Generally, maintenance of the work also will not be difficult.	The drainage may be required flexible function to follow a moving of ground surface due to landslide activity, otherwise the drainage will be damaged by the ground moving, and then the water will penetrate into landslide from the damaged points.	It will be feasible in Pakistan; even any contractors can conduct the work unless the special material such as corrugated steel pipe is required for the ditch. Construction.	Generally, the effect of the work is difficult to be assumed in design stage.
Open-Blind Ditch (French drain)		To collect and to discharge the surface water and shallow groundwater in landslide area properly	In case that groundwater level is shown shallower in landslide area, the work will be effective to drain the groundwater and surface water.	If the groundwater level is deeper than 2m from ground surface, the blind ditch (conduit) part will not function.	It will be feasible in Pakistan; even any local contractors can conduct the work unless the special material such as corrugated steel pipe is required for the surface ditch part.	
Horizontal Drainage		To collect groundwater to draw down the groundwater level in landslide area	This is the one of the general countermeasure works for landslide. The ordinal drilling machine can be used for the work.  Since the work is not required large scale preparation, it can be applied as emergency countermeasure.	In case of the target groundwater level is deep, the work shall not be applied, and maximum length of the drainage shall be less than 50m  The work will be difficult to apply if it can be considered that the landslide consists of material with a lot of boulder.	The percussion drilling machine for anchor work is recommended for the work. However even rotary drilling machine which can drill in horizontal direction can be adopted for the work.	Maintenance of the drainage pipes is required regularly after completion of the work.
Drainage Well		To collect deep groundwater to draw down the groundwater level in landslide area.	The well can be adopted in case that the landslide block is too huge and deep to collect groundwater by horizontal drainage.	Generally, the work will be required specific machinery and material such as small excavator, cylinder liner plate, and small drilling machine and so on.	The work will be able to be conducted by Joint Venture of a local contractor and contractor from other countries such as India.	
Drainage Tunnel		To collect deep groundwater to draw down the groundwater level in landslide area.	The tunnel can be adopted of large scale landslide with thick sliding mass. Since the tunnel is installed under/out of landslide block, risk of damaging tunnel by landslide activity is low.	The tunnel shall be made on stable ground under the slip surface of landslide. The cost of the work will become high depending on the site condition.	The work might not be able to be conducted by even Joint Venture of a local contractor and contractor from other countries.	

## CATALOG OF COUNTERMEASURE WORK FOR SLOPE DISASTER

LANDSLIDE						
Item	Photo/Drawing	Purpose	Availability	Limitation	Feasibility in Pakistan	Remarks
Earth Removal		To reduce sliding force of landslide by removing head part of landslide block	This is the one of the simplest countermeasure works for landslide. The work can be expected direct effect for stability of landslide. The work can be used as emergency countermeasure work.	Depending on the shape of slip surface, the work may not contribute to make stable condition of the slope.	It will be feasible in Pakistan; even any local contractors can conduct the work	The work may be trigger of occurrence of another landslide which is located above of target landslide.
Counterweight Fill		To increase resisting force against sliding force of landslide.	This is the one of the simplest countermeasure works for landslide. The work can be expected direct effect for stability of landslide. The work can be used as emergency countermeasure work.	Depending on a location of fill (embankment) on the slope, the work may not contribute to make stable condition of the slope.	It will be feasible in Pakistan; even any local contractors can conduct the work	The work may be trigger of occurrence of another landslide which is located down of target landslide.
Steel Pile work		To increase resisting force against sliding force of landslide by shear strength of a piles.	This work is to stop a landslide moving by structural strength. Therefore, the work can be expected direct effect for stability of landslide.	The pile work will not work properly under the following conditions; <ul style="list-style-type: none"> <li>• The location that shows steep slip surface or,</li> <li>• The location on an active landslide</li> <li>• The location which consists of loose material</li> </ul>	The work will be able to be conducted by Joint Venture of a local contractor and contractor from other countries such as India.	For landslide countermeasure, pile shall not be driven pile but be installed in a boreholes.
Cast-in place concrete Shaft (Caisson)		To increase resisting force against sliding force of landslide by shear strength of a shaft piles.	This work is to stop a landslide moving by structural strength. Therefore, the work can be expected direct effect for stability of landslide.  If the restraint force of steel pile is not enough for the required force to stop the landslide, the shaft work can be adopted.  The shaft work can be conducted by manual to dig the borehole to cast the caisson depending on the site condition.	The pile work will not work properly under the following conditions; <ul style="list-style-type: none"> <li>• The location that shows steep slip surface or,</li> <li>• The location on an active landslide</li> <li>• The location which consists of loose material</li> <li>• The shaft work is required a firm ground foundation for the structure.</li> </ul>	The work will be able to be conducted by Joint Venture of a local contractor and contractor from other countries such as India.  The work will be costly.	
Ground Anchor		To fix a landslide body by transferring tension stress of structure to a firm ground	In the following cases, the work can be used for, <ul style="list-style-type: none"> <li>• No ground reaction force can be expect to obtain at steep slope,</li> <li>• immediate effect is required for emergency , and</li> <li>• Stabilization measure will be required at toe part of landslide against partially collapse.</li> </ul>	The work required specific machinery, equipment and materials.  In case of length of anchor (free length of anchor) is required more than 20m, effect of the work may not obtained as planned.	The work will be able to be conducted by Joint Venture of a local contractor and contractor from other countries such as India.  The work will be costly.	It is required to conduct lift-off test to check whether the anchor is keeping planned tension stress or not.

## CATALOG OF COUNTERMEASURE WORK FOR SLOPE DISASTER

DEBRIS SLOPE FAILURE						
Item	Photo/Drawing	Purpose	Availability	Limitation	Feasibility in Pakistan	Remarks
Surface Drainage		To collect surface water and to drain them out of slope properly	This is the one of the simplest countermeasure works. The work can be expected to avoid infiltration of rainfall into slope.  Generally, maintenance of the work also will not be difficult.	Nothing special	It will be feasible in Pakistan; even any contractors can conduct the work.  It is the work which is one of the common slope counter-measure works in Pakistan	Regular maintenance is required.  The work shall be adopted on benching on slope
Re-vegetation		To support stability of a slope surface.	The work can be recommended to apply on all cut slopes if possible.  Advantages of the work are as follows, • Cheap • Easy to implement • Good for environment and landscape	The work is not expected good effect on a loose condition of slope surface.  If the work will be adopted on a loose surface slope, wicker fence or crib work shall be combined with the work to keep plant staying on slope.	It will be feasible in Pakistan; even any contractors can conduct the work.  It is the work which is one of the common slope counter-measure works in Pakistan	The plant for the work shall be used local species to avoid a changing environment around the site.
Wicker Fence		To keep stability of a slope surface until growing of plants of the re-vegetation work	The work can be adopted on a loose surface slope to keep material of slope surface.  The fence can be made by wood and wooden branch.  No machinery is required to implement the work.	The work will be difficult to be adopted on hard rock slope.	It will be feasible in Pakistan; even any contractors can conduct the work.	
Wooden Log Crib		To keep stability of a slope surface until growing of plants of the re-vegetation work	The work has almost same function as wicker fence.  The main material for the work will be wood and stone.	The work will be difficult to be adopted on hard rock slope.	It will be feasible in Pakistan; even any contractors can conduct the work.	
Stone Pitching		To avoid erosion of slope surface and to keep stability of a slope surface	The work can be good effect for slope, especially sediments and weathered rock slope. The work is to avoid erosion and weathering on surface of slope.  Required material for the work is Boulder and concrete.	The stone pitching wall shall not be expected function of support of slope stability as retaining wall.  The wall shall not be adopted on a slope which many water seepages are found without any drainage work for the water.	It will be feasible in Pakistan; even any contractors can conduct the work.  It is the work which is one of the common slope counter-measure works in Pakistan	Weep-hole shall be put on the pitching wall to discharge water from behind of the wall.

## CATALOG OF COUNTERMEASURE WORK FOR SLOPE DISASTER

DEBRIS SLOPE FAILURE						
Item	Photo/Drawing	Purpose	Availability	Limitation	Feasibility in Pakistan	Remarks
Retaining Wall		To support stability of a slope surface and avoid erosion of slope surface	The work can be adopted in case that slope cannot be secured the appropriate/standard angle due to the limitation of site or topography.  The work can be applied to various slope conditions such as a slope with earth pressure or failure-prone slope by water seepage.	The work shall not be installed at the slope where excavation of toe part of the slope for installation of the wall will make unstable condition of the slope.	It will be feasible in Pakistan; even any contractors can conduct the work.  It is the work which is one of the common slope counter-measure works in Pakistan, especially Gabion and Masonry wall	Weep-hole shall be put on the pitching wall to discharge water from behind of the wall. The retaining wall can be made by Gabion, Masonry or Reinforced Concrete.
Barrier Wall		To avoid failed debris or fallen rocks to reach a road	In case that countermeasure work is difficult to apply on the slope directly, the work can be installed as a prevention measure.	The enough space to catch failed debris or rocks shall be required between slope and the wall.	It is the work which is one of the common slope counter-measure works in Pakistan, especially Gabion and Masonry wall	Rock protection fence will be attached on top of the wall generally.  Regular maintenance will be required to secure a open space behind the wall.
Re-shaping slope with Benching		To make a stable condition of slope with appropriate/standard angle	The work gives a direct effect to slope stability.  It is recommended to consider adoption of the work as the first step.	Since a space for machinery like a excavator is required, the work will be difficult to be adopted at steep slope.	The work is just simple earth work. It work is feasible in Pakistan.	When the work is planned, it shall be studied whether the excavation work will disturb stability of neighbor slopes.  Surface drainage shall be installed on benching
Concrete Crib		To retain slope stability and to avoid erosion and weathering to the slope	The work has 2 functions as retaining of slope and protection from erosion and weathering.  The work can be adopted various figure of slope.  Re-vegetation work can be combined with the work	Since the work is basically, conducted by cast-in-place method, certain work space as stage of work or compressor will be required.	The work will be able to be conducted by Joint Venture of a local contractor and contractor from other countries such as India.	The ground anchor or nailing work is used in combination with the work depending on the site condition.
Shotcrete		To avoid erosion, weathering and infiltration of water to the slope	The work can be adopted various figure of slope.	Since the work has not function of retaining, the work shall not apply on slope where many unstable rocks are found.	The work will be able to be conducted by Joint Venture of a local contractor and contractor from other countries such as India.	In case that many water seepage can be found on slope, weep hole shall be put on shotcrete surface frequently.
Ground Anchor		To fix a slope surface by transferring tension stress of structure to a firm ground	In the following cases, the work can be used for. <ul style="list-style-type: none"> <li>• No ground reaction force can be expect to obtain at steep slope,</li> <li>• immediate effect is required for emergency</li> </ul>	The work required specific machinery, equipments and materials.  In case of length of anchor (free length of anchor) is required more than 20m, effect of the work may not obtained as planned.	The work will be able to be conducted by Joint Venture of a local contractor and contractor from other countries such as India.  The work will be costly.	It is required to conduct lift-off test to check whether the anchor is keeping planned tension stress or not.

## CATALOG OF COUNTERMEASURE WORK FOR SLOPE DISASTER

ROCK SLOPE FAILURE						
Item	Photo/Drawing	Purpose	Availability	Limitation	Feasibility in Pakistan	Remarks
Rock Removal		To remove problematic part or unstable rock on slope	The work gives a direct effect to hazard of rock slope failure or rockfall. The work shall be the first option to consider for rock slope failure measure.	The following slope conditions may not be good for implementation of the work. • hard and massive rock • Steep / overhanging slope • No access for the machinery for excavation	The work will be feasible in Pakistan depending on site condition. Knowledge and experience for work on steep slope will be required.	
Retaining Wall		To support stability of a slope and avoid erosion and weathering of slope surface	The work can be adopted in case that slope cannot be secured the appropriate/standard angle due to the limitation of site or topography. The work can be applied to various slope conditions such as failure-prone slope by water seepage.	The work shall not be installed at the slope where excavation of toe part of the slope for installation of the wall will make unstable condition of the slope.	It will be feasible in Pakistan; even any contractors can conduct the work. It is the work which is one of the common slope countermeasure works in Pakistan, especially Gabion and Masonry wall	Weep-hole shall be put on the pitching wall to discharge water from behind of the wall. The retaining wall can be made by Gabion, Masonry or Reinforced Concrete.
Barrier Wall		To avoid fallen rocks to reach a road	In case that countermeasure work is difficult to apply on the slope directly as rock removal or protection rock net, the work can be installed as a prevention measure.	The enough space to catch failed debris or rocks shall be required between slope and the wall.	It is the work which is one of the common slope countermeasure works in Pakistan, especially Gabion and Masonry wall	Rock protection fence will be attached on top of the wall generally. Regular maintenance will be required to secure a open space behind the wall.
Protection Rock Net		To fix unstable rocks at original position	The work shall be adopted in case that the unstable rocks cannot be removed because of the site conditions.	Since the net shall be fixed by anchor bolt on the slope, fractured or weathered rock slope is not recommended to be applied the work	The work will be able to be conducted by Joint Venture of a local contractor and contractor from other countries such as India.	Generally, stage for the work will be required.
Rock Bolt (Nailing)/Anchor		To fix loose rock slope or unstable rocks at original position	The work shall be adopted for unstable rocks or unstable rock slope	Fractured or weathered rock slope is not recommended to be applied the work	The work will be able to be conducted by Joint Venture of a local contractor and contractor from other countries such as India.	Generally, the work is adopted in combination with shotcrete or concrete crib work.

## CATALOG OF COUNTERMEASURE WORK FOR SLOPE DISASTER

DEBRIS FLOW						
Item	Photo/Drawing	Purpose	Availability	Limitation	Feasibility in Pakistan	Remarks
Sabo Dam		To catch a debris, big boulders or flood woods of debris flow, and to discharge only water or with in some small amount of debris	The work can be adopted in the following cases. <ul style="list-style-type: none"> <li>Expected volume of debris is huge</li> <li>The river is deep</li> </ul>	The firm ground is required for foundation of the dam	The work will be able to be conducted by Joint Venture of a local contractor and contractor from other countries such as India. The work will be costly.	
Check Dam		To break speed of debris or water flow, and to catch some debris, boulder or flood wood.	The work can be applied on small tributary valley o gully as well. The work can be adopted on various gradient of valley	The dam may not work properly on the valley where many debris deposit on.	It will be feasible for Pakistan, especially it is made by Gabion and Masonry wall	
French Cascade		To break speed of debris or water flow	The work can be applied on small tributary valley o gully as well. The work shall be adopted on gentle gradient of valley.	It is not adopted on steep gradient of valley	It will be feasible for Pakistan, especially it is made by Gabion and Masonry wall	
Culvert		To discharge river water without influence to the road	The work can be adopted in the following cases. <ul style="list-style-type: none"> <li>Width of river is small</li> <li>expected volume of debris flow is small</li> </ul> The culvert can be made by RC, Gabion or Mason	Expected volume of debris flow is huge, bridge shall be adopted to cross the river.	This is a common structure to discharge river water under the road. It is feasible in Pakistan	
Buffer Forest		To break energy of debris flow	The work can be adopted in the gentle ground. Advantages of the work are as follows, <ul style="list-style-type: none"> <li>Cheep and Easy to implement</li> <li>Good for environment and landscape</li> </ul>	The work cannot be adopted in the following cases. <ul style="list-style-type: none"> <li>Steep slope</li> <li>Rocky slope</li> </ul>	It will be feasible for Pakistan,	
Shed Work		To discharge debris flow or river water without influence to the road	The work can be adopted in the following cases, <ul style="list-style-type: none"> <li>The estimated volume of debris flow is too huge to stop by Sabo dam, or</li> <li>the height of river bed is higher than the road.</li> </ul>	If height of river bed is lower than the road, the work cannot be applied.	The work will not be able to be conducted by Joint Venture of a local contractor and contractor from other countries. The work will be costly.	The width and inclination of shed shall be designed based on the river width and gradient

## CATALOG OF COUNTERMEASURE WORK FOR SLOPE DISASTER

ROCK FALL						
Item	Photo/Drawing	Purpose	Availability	Limitation	Feasibility in Pakistan	Remarks
Rock Removal		To remove unstable rock on slope	The work gives a direct effect to hazard of rockfall. The work shall be the first option to consider for rockfall measure.	The following slope conditions may not be good for implementation of the work. • The place is too high to work it. • No access for the machinery for excavation	The work will be feasible in Pakistan depending on site condition. Knowledge and experience for work on steep slope will be required.	
Protection Wall	 	To avoid fallen rocks to reach a road	In case that countermeasure work is difficult to apply on the slope directly as rock removal or protection rock net, the work can be installed as a prevention measure.	The enough space to catch failed debris or rocks shall be required between slope and the wall.	It is the work which is one of the common slope countermeasure works in Pakistan, especially, Gabion or Mason wall.	Rock protection fence will be attached on top of the concrete wall generally. Regular maintenance will be required to secure a open space behind the wall.
Rock Catch Net		To avoid fallen rocks to reach a road by catching a fallen rocks from slope	The work can be adopted on the slope where unstable rocks distributed extensively on.	The net shall not be expected to deal with large energy of fallen rock. The target rock shall therefore, not be big rock or rock at high slope.	The work will be able to be conducted by Joint Venture of a local contractor and contractor from other countries such as India.	Regular maintenance will be required to secure a space behind the net.
Fixing Work by shotcrete		To fix unstable rocks at original position	The work shall be adopted in case that the unstable rocks cannot be removed because of the site conditions.	Since the work has not function of retaining, the work shall not apply on slope where many or huge unstable rocks are found.	The work will be able to be conducted by Joint Venture of a local contractor and contractor from other countries such as India.	Generally, stage for the work will be required.
Shed Work		To protect road from fallen rocks	The work can be adopted for the slope where many unstable rocks which are difficult to deal by other measures are found.	The shed work has a capacity of bearing against falling rock energy. Estimated falling rock energy shall be examined in design stage of the work.	The work will not be able to be conducted by Joint Venture of a local contractor and contractor from other countries. The work will be costly.	

## CATALOG OF COUNTERMEASURE WORK FOR SLOPE DISASTER

OTHERS						
Item	Photo/Drawing	Purpose	Availability	Limitation	Feasibility in Pakistan	Remarks
Tunnel -Route Shift-		To avoid problematic road section by shifting route	The work can be adopted for the road section which cannot be dealt by other countermeasure because of site condition or economical comparison.	The availability shall be determined according to not only topographical and geological conditions, but economical or political planning also.	The work will not be able to be conducted by Joint Venture of a local contractor and contractor from other countries depending on the site condition. The work will be costly.	
Bridge -Route Shift-		To avoid problematic road section by shifting route	The work can be adopted for the road section which cannot be dealt by other countermeasure because of site condition or economical comparison.	The availability shall be determined according to not only topographical and geological conditions, but economical or political planning also.	The work will not be able to be conducted by Joint Venture of a local contractor and contractor from other countries depending on the site condition. The work will be costly.	
Advanced Traffic Control (Early Warning System)		To secure safety for road user in advance by road block based on weather condition.	The work can be adopted for the road section where slope disaster occurs frequently and its occurrence has close relation between rainfall.	Generally detour will be required during road closing. Value of criteria for road closing or opening shall be determined based on the rainfall data for long period.	The method will be able to apply in Pakistan. However operation system shall be established with relative organizations.	This is non-structural countermeasure.

## Appendix 4

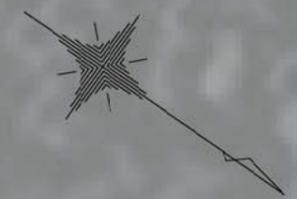
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*Drawings of Landslide Countermeasure for  
Shortlist Slopes*



①Slope Cutting, ②Grating Crib Works,  
 ③Rockbolt (L=2.0m, Interval 2.0m×2.0m, Bearing Plate 1000mm×1000mm)  
 ④Mortar Spraying, ⑥Ground Anchor (Average Anchor length L=12.0m, Interval 2.0m×2.0m)  
 Total Length L=300m



④Mortar Spraying  
 ③Rockbolt  
 ⑥Ground Anchor

⑦Drainage Works

⑤Gravity Retaining Wall

⑪Culvert

Buffer zone

To Islamabad

⑧Drainage (Pipe  $\phi$  75), ⑨Revetment (Slope Protection Molds)  
 ⑩Pavement Works & Soil Improvement (Base course), Total Length L=320m



APPROVED
CHECKED
DRAWN

NOTE  
 Google Earth  
 Image © 2018 CNES / Airbus

PROJECT NO.  
 DATE: June/ 2018

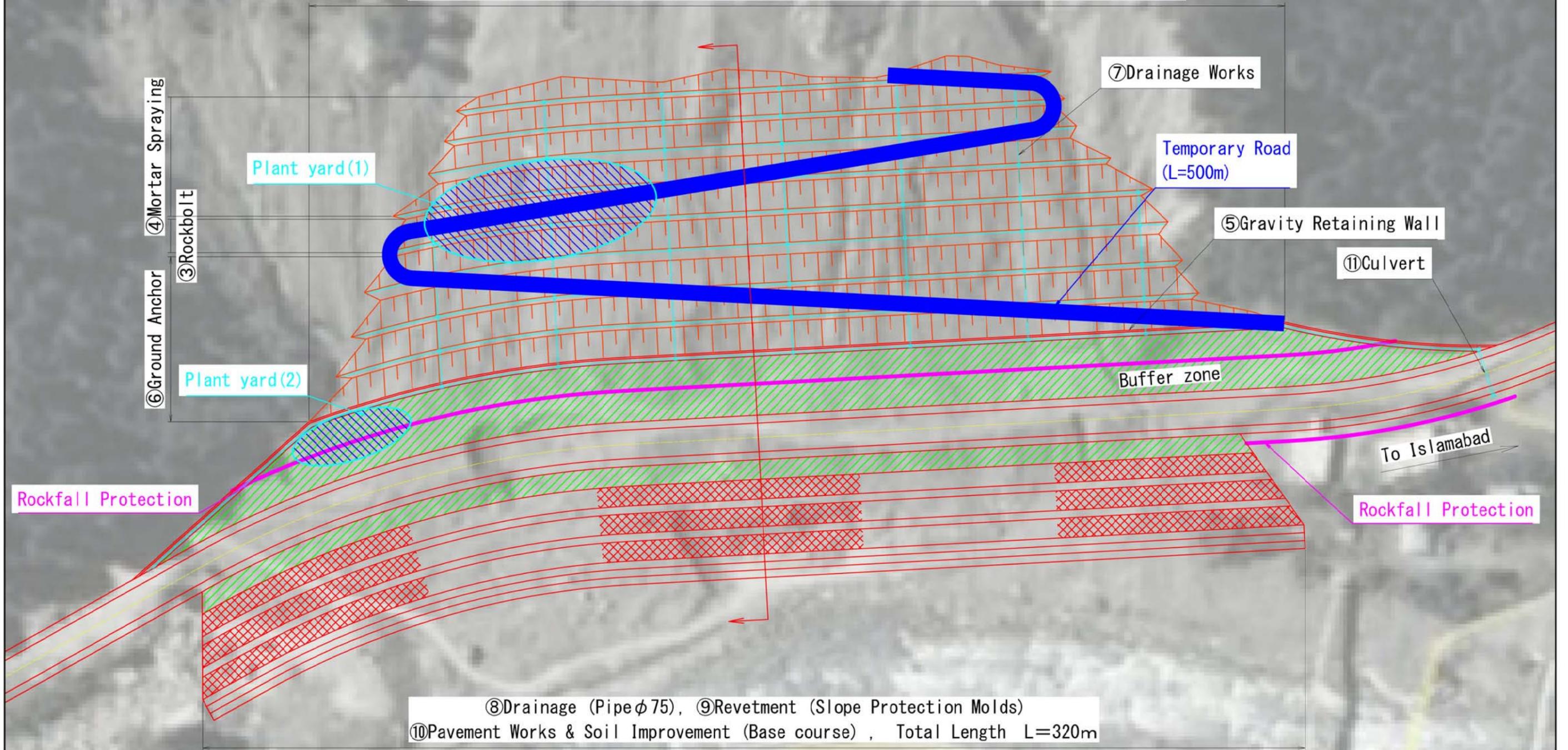
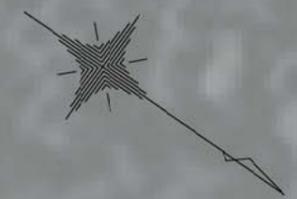
PROJECT NAME  
 Data Collection Survey  
 on Road Landslide Measures in Pakistan

TITLE  
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 Plan  
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 SCALE 1:500—(A1)  
 1:1000—(A3)

SHEET. NO.  
 1



①Slope Cutting, ②Grating Crib Works,  
 ③Rockbolt (L=2.0m, Interval 2.0m×2.0m, Bearing Plate 1000mm×1000mm)  
 ④Mortar Spraying, ⑥Ground Anchor (Average Anchor length L=12.0m, Interval 2.0m×2.0m)  
 Total Length L=300m

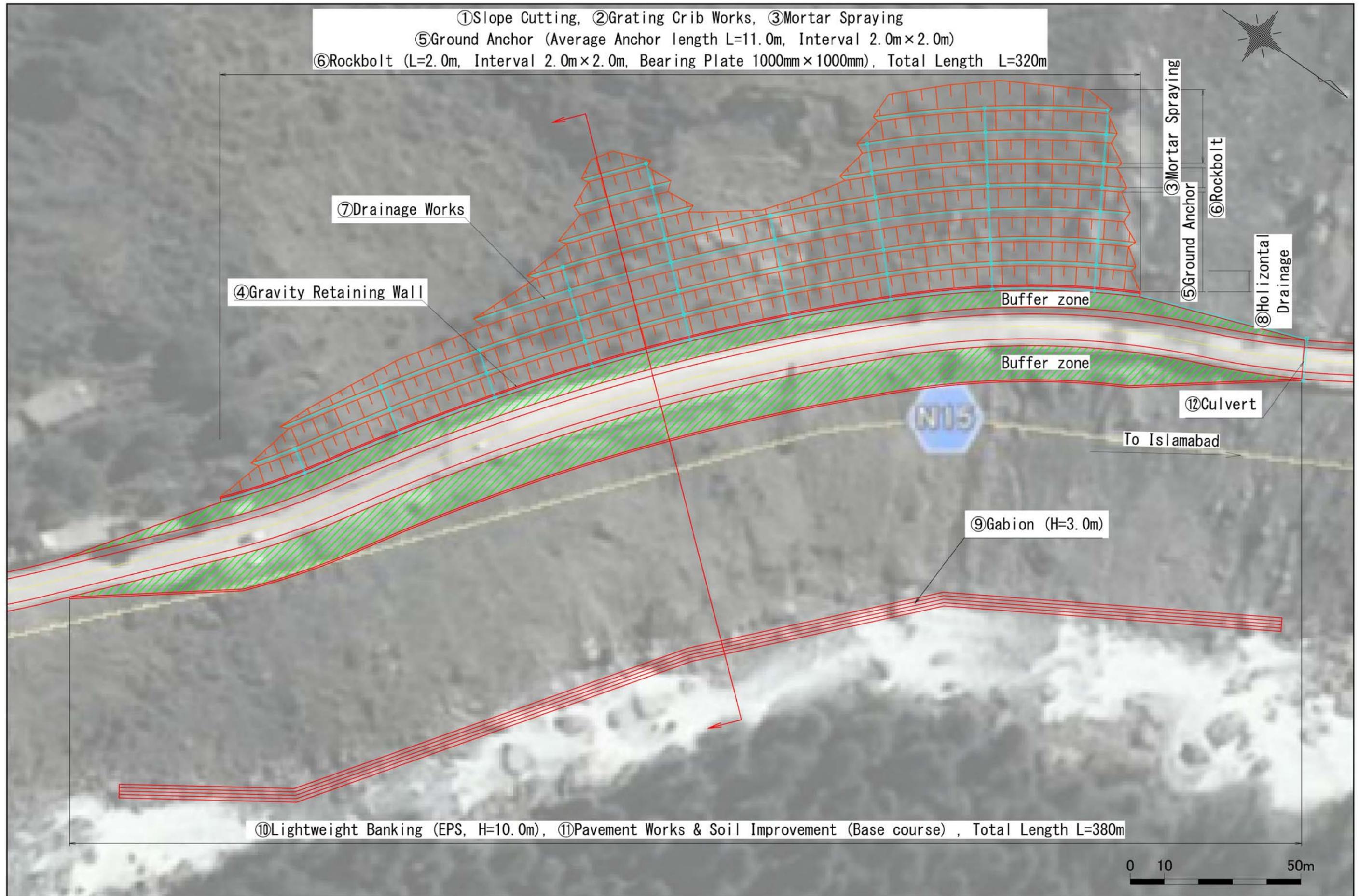
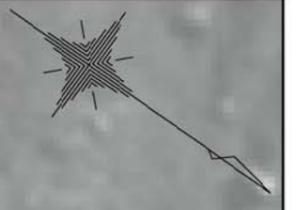


⑧Drainage (Pipe  $\phi$  75), ⑨Revetment (Slope Protection Molds)  
 ⑩Pavement Works & Soil Improvement (Base course), Total Length L=320m



APPROVED	NOTE	PROJECT NO.	PROJECT NAME	TITLE	SHEET. NO.
CHECKED	Google Earth Image © 2018 CNES / Airbus	DATE: June/ 2018	Data Collection Survey on Road Landslide Measures in Pakistan	N15-No. 3 Plan (Temporary work)	3
DRAWN				DRAWING No. N15-No.3-3	SCALE 1:500—(A1) 1:1000—(A3)

- ① Slope Cutting, ② Grating Crib Works, ③ Mortar Spraying
- ⑤ Ground Anchor (Average Anchor length L=11.0m, Interval 2.0m×2.0m)
- ⑥ Rockbolt (L=2.0m, Interval 2.0m×2.0m, Bearing Plate 1000mm×1000mm), Total Length L=320m



⑦ Drainage Works

④ Gravity Retaining Wall

Buffer zone

Buffer zone

③ Mortar Spraying

⑥ Rockbolt

⑤ Ground Anchor

⑧ Horizontal Drainage

⑫ Culvert

To Islamabad

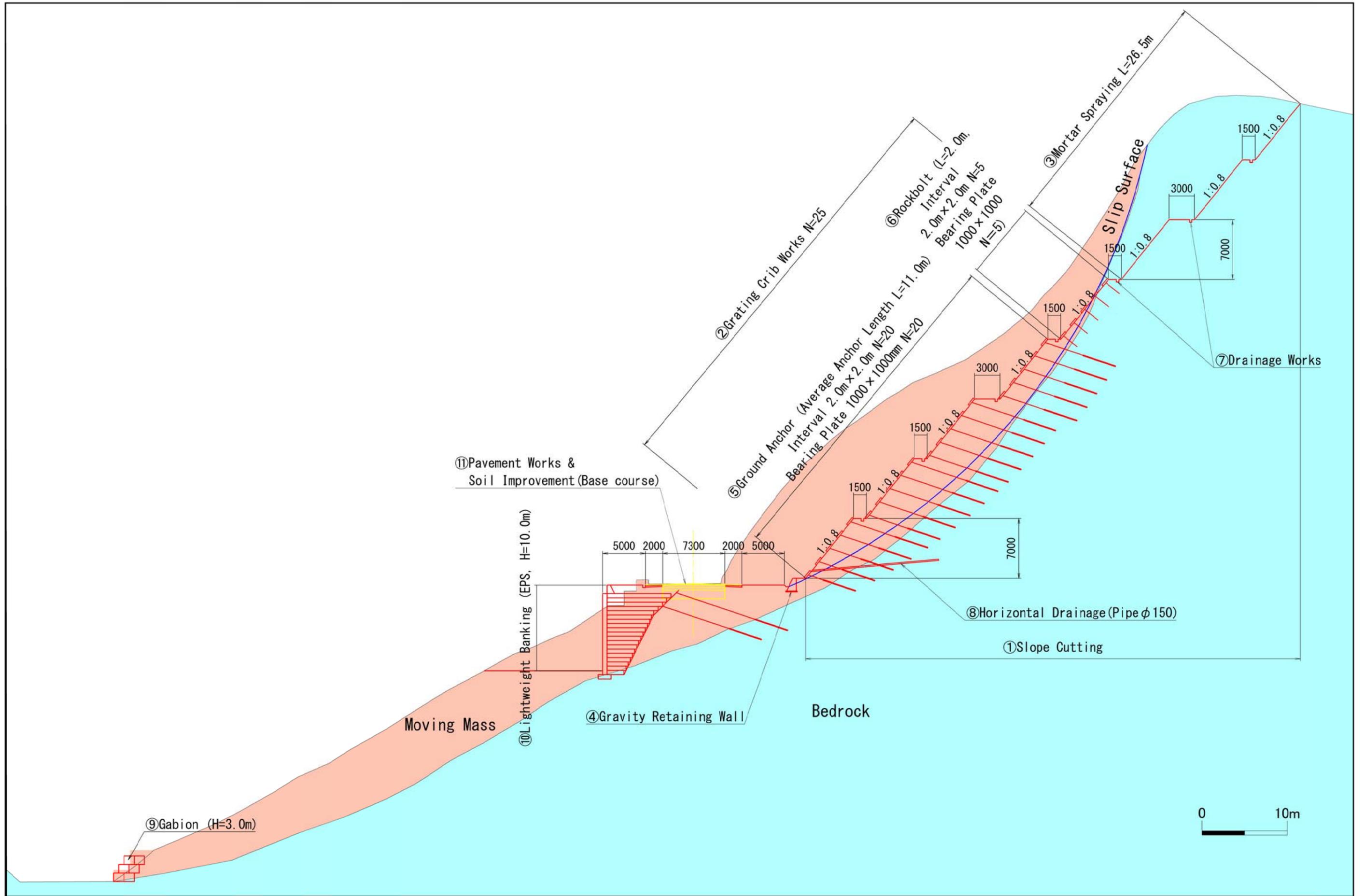
⑨ Gabion (H=3.0m)

⑩ Lightweight Banking (EPS, H=10.0m), ⑪ Pavement Works & Soil Improvement (Base course), Total Length L=380m

0 10 50m

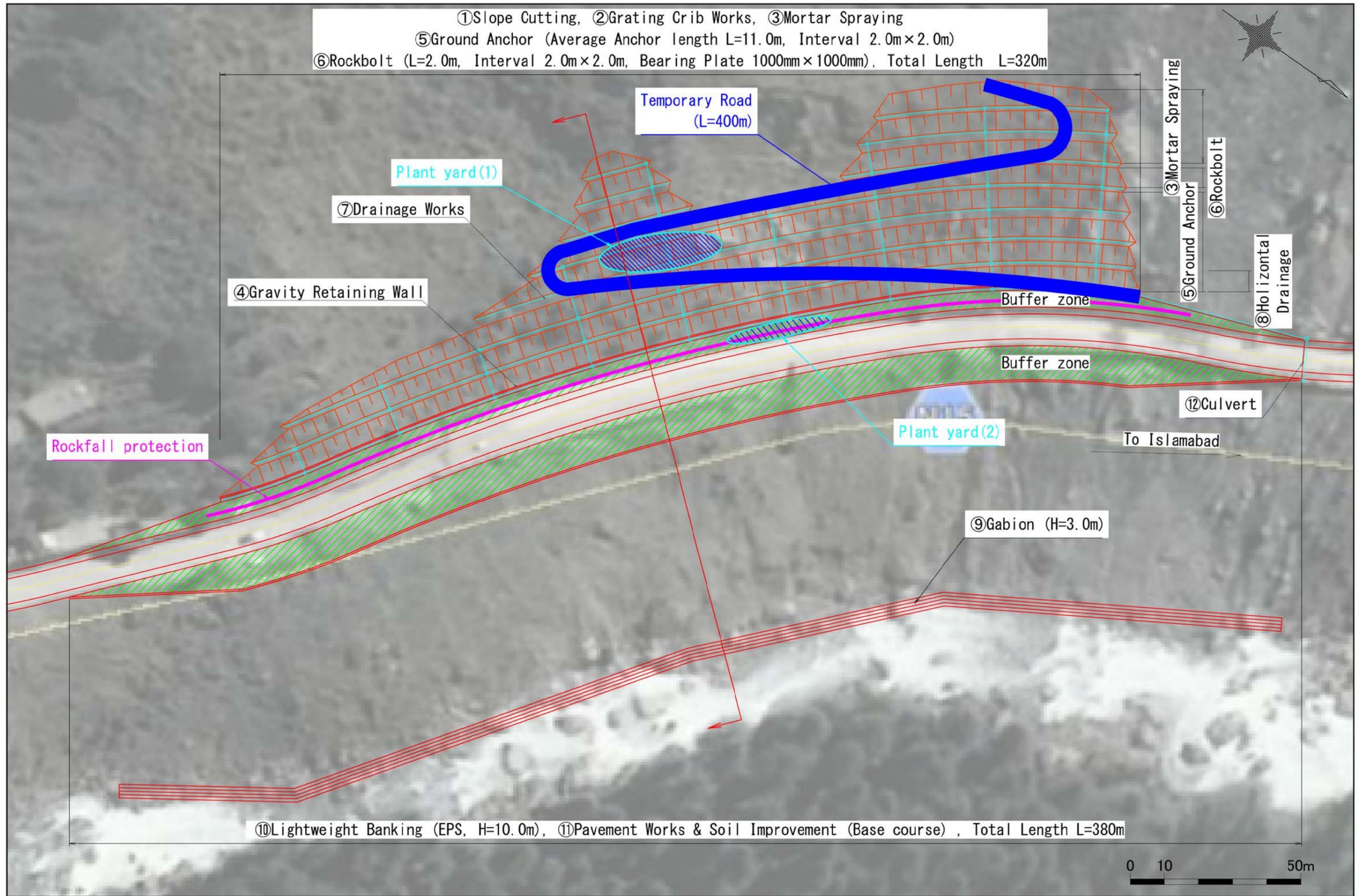
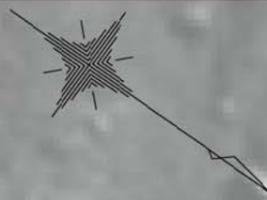
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CHECKED		DATE: June / 2018	Data Collection Survey on Road Landslide Measures in Pakistan	N15-No. 6 Plan	4
DRAWN				DRAWING No. N15-No.6-1	SCALE 1:500—(A1) 1:1000—(A3)

Google Earth  
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APPROVED CHECKED DRAWN	NOTE	PROJECT NO.	PROJECT NAME	TITLE	SHEET. NO.
		DATE: June / 2018	Data Collection Survey on Road Landslide Measures in Pakistan	N15-No. 6 Typical Corss Section	5
				DRAWING No. N15-No.6-2 SCALE 1:200—(A1) 1:400—(A3)	

- ① Slope Cutting, ② Grating Crib Works, ③ Mortar Spraying
- ⑤ Ground Anchor (Average Anchor length L=11.0m, Interval 2.0m×2.0m)
- ⑥ Rockbolt (L=2.0m, Interval 2.0m×2.0m, Bearing Plate 1000mm×1000mm), Total Length L=320m



Rockfall protection

Plant yard(1)

⑦ Drainage Works

④ Gravity Retaining Wall

Temporary Road  
(L=400m)

Plant yard(2)

⑨ Gabion (H=3.0m)

⑩ Lightweight Banking (EPS, H=10.0m), ⑪ Pavement Works & Soil Improvement (Base course) , Total Length L=380m

③ Mortar Spraying

⑤ Ground Anchor

⑥ Rockbolt

⑧ Horizontal Drainage

Buffer zone

Buffer zone

⑫ Culvert

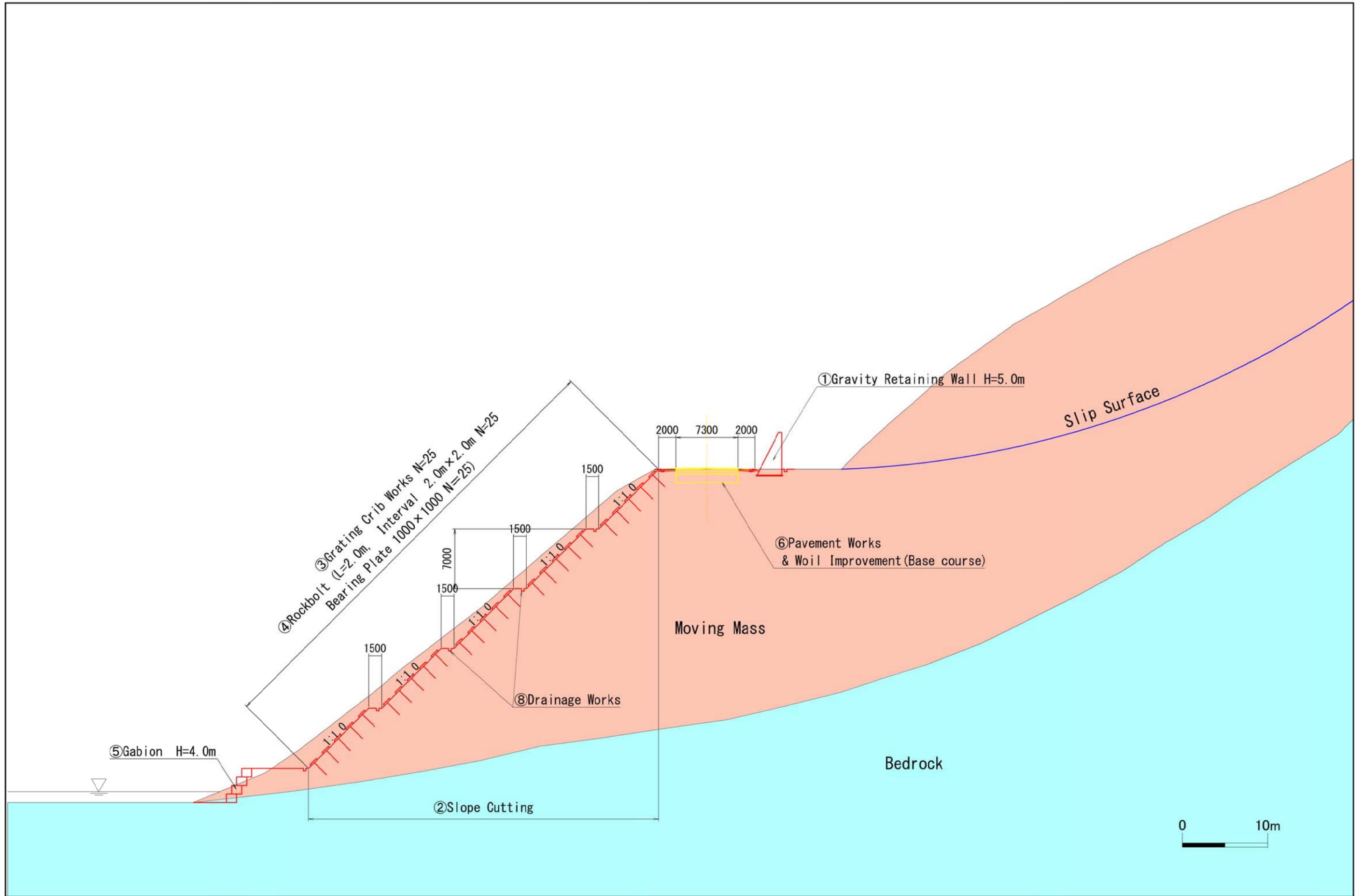
To Islamabad

0 10 50m

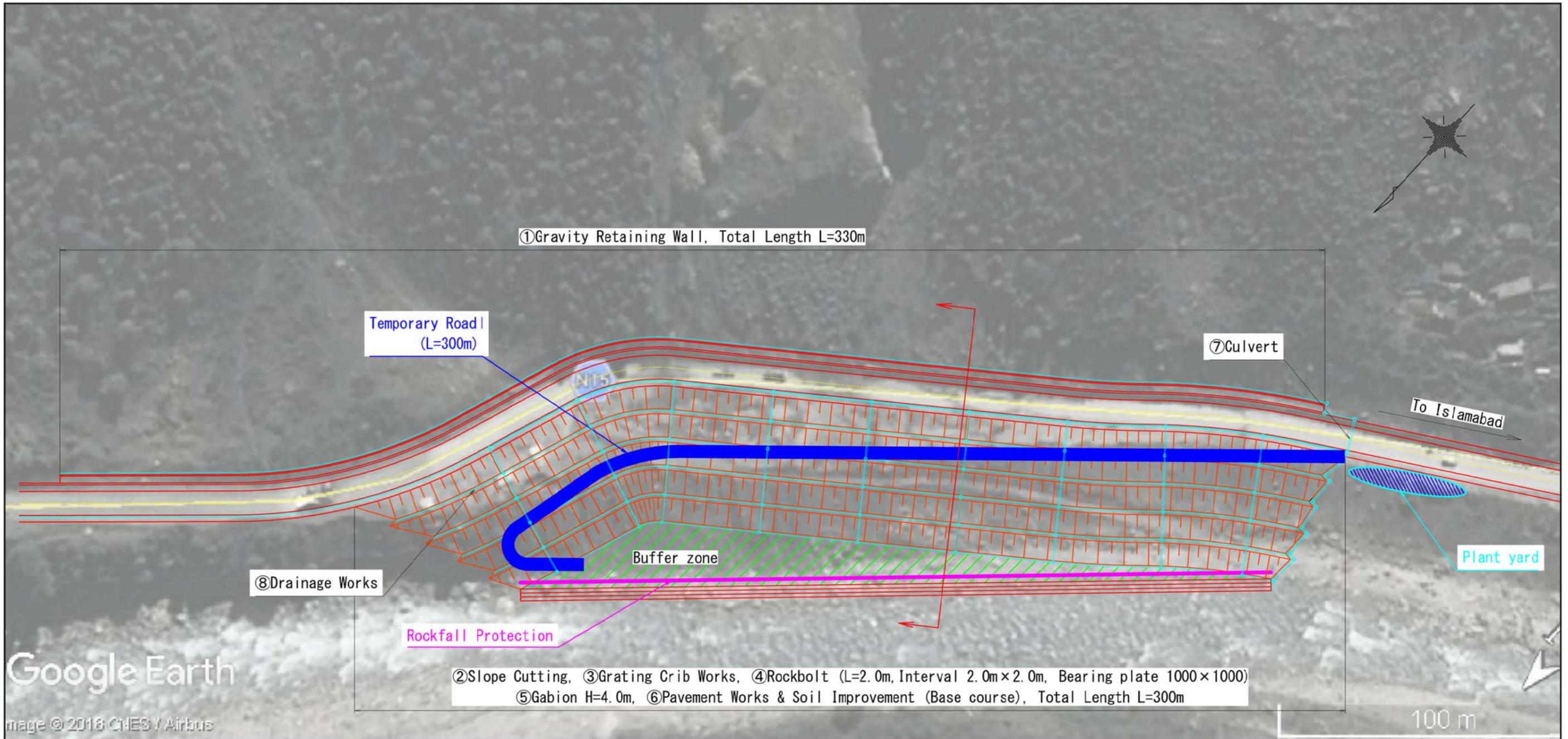
APPROVED	NOTE	PROJECT NO.	PROJECT NAME	TITLE	SHEET. NO.
CHECKED	Google Earth Image © 2018 CNES / Airbus	DATE: June / 2018	Data Collection Survey on Road Landslide Measures in Pakistan	N15-No. 6 Plan (Temporary work)	6
DRAWN				DRAWING No. N15-No.6-3	SCALE 1:500—(A1) 1:1000—(A3)



APPROVED	NOTE	PROJECT NO.	PROJECT NAME	TITLE	SHEET. NO.					
						Google Earth Image © 2018 CNES / Airbus	DATE: June / 2018	Data Collection Survey on Road Landslide Measures in Pakistan	N15-No. 8 Plan	7
						CHECKED	DATE: June / 2018	DRAWING No. N15-No.8-1	SCALE 1:600—(A1) 1:1200—(A3)	
DRAWN										



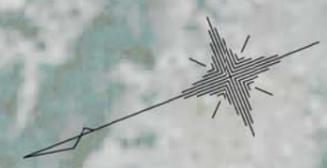
APPROVED CHECKED DRAWN	NOTE	PROJECT NO.	PROJECT NAME	TITLE	SHEET. NO.
		DATE: June / 2018	Data Collection Survey on Road Landslide Measures in Pakistan	N15-No. 8 Typical Corss Section	8
				DRAWING No. N15-No.8-2	SCALE 1:200 —(A1) 1:400 —(A3)



Google Earth

Image © 2018 CNES / Airbus

APPROVED	NOTE	PROJECT NO.	PROJECT NAME	TITLE	SHEET. NO.
CHECKED	Google Earth Image © 2018 CNES / Airbus	DATE: June / 2018	Data Collection Survey on Road Landslide Measures in Pakistan	N15-No. 8 Plan (Temporary work)	9
DRAWN				DRAWING No. N15-No.8-3	SCALE 1:600—(A1) 1:1200—(A3)



① Slope Cutting, ② Grating crib works, ③ Ground Anchor (Average Anchor length  $L=10.0\text{m}$ , Interval  $2.0\text{m} \times 2.0\text{m}$ )  
 ④ Rockbolt ( $L=3.5\text{m}$ , Interval  $2.0\text{m} \times 2.0\text{m}$ , Bearing Plate  $1000\text{mm} \times 1000\text{mm}$ ), Total Length  $L=130\text{m}$

⑥ Drainage Works

④ Rockbolt

③ Ground Anchor

⑤ Gravity Retaining Wall

⑧ Culvert

To Islamabad

⑦ Pavement Works & Soil Improvement (Base course), Total Length  $L=300\text{m}$

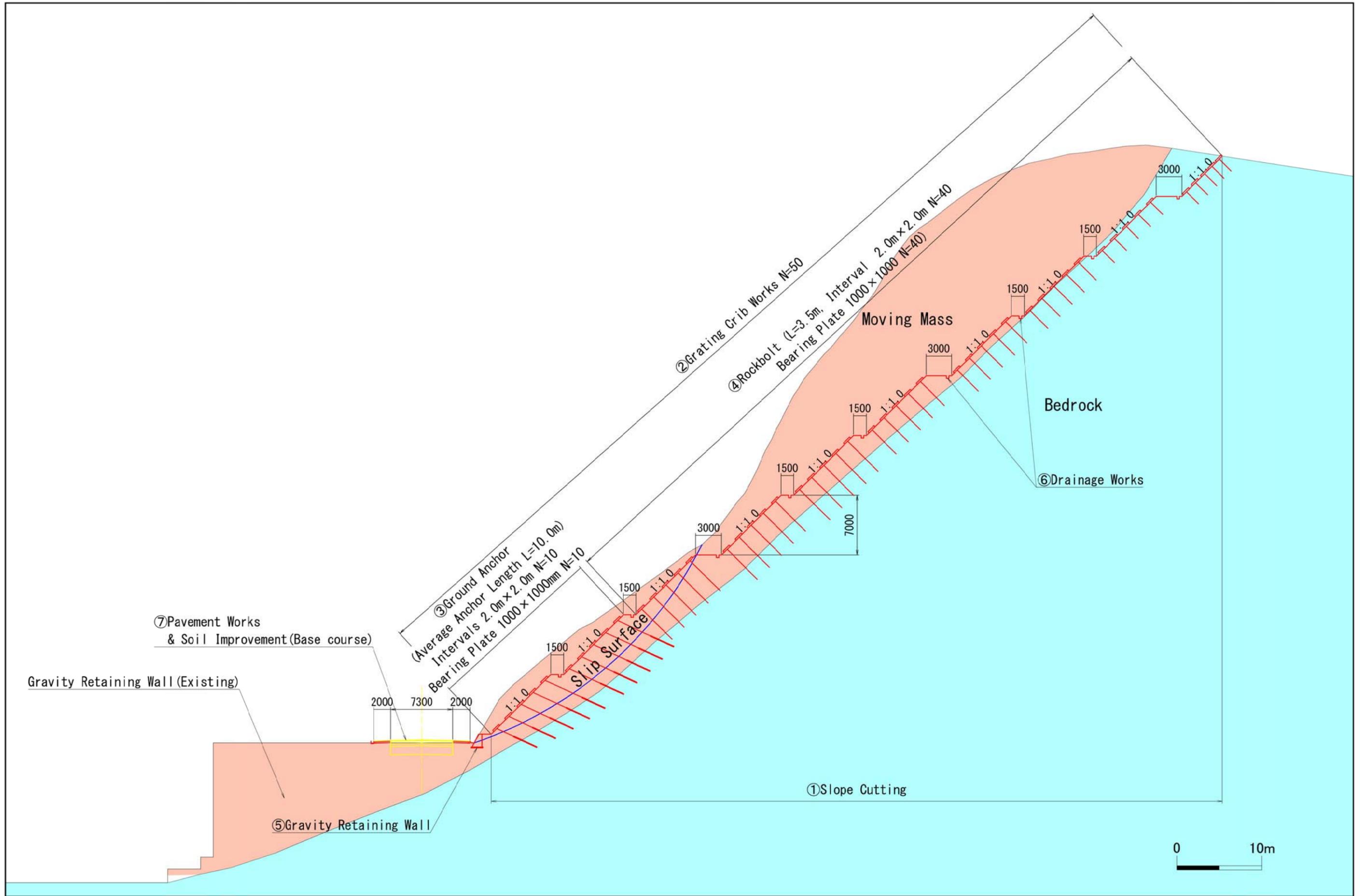
Google Earth

Image © 2018 CNES / Airbus

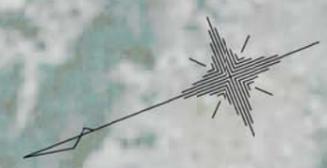


100 m

APPROVED	NOTE	PROJECT NO.	PROJECT NAME	TITLE	SHEET. NO.
CHECKED	Google Earth Image © 2018 CNES / Airbus	DATE: June / 2018	Data Collection Survey on Road Landslide Measures in Pakistan	N15-No. 9 Plan	10
DRAWN				DRAWING No. N15-No.9-1	SCALE 1:500 —(A1) 1:1000 —(A3)



APPROVED CHECKED DRAWN	NOTE	PROJECT NO.	PROJECT NAME	TITLE	SHEET. NO.
		DATE: June / 2018	Data Collection Survey on Road Landslide Measures in Pakistan	N15-No. 9 Typical Corss Section	11
				DRAWING No. N15-No.9-2	SCALE 1:200 —(A1) 1:400 —(A3)



①Slope Cutting, ②Grating crib works, ③Ground Anchor (Average Anchor length L=10.0m, Interval 2.0m×2.0m)  
 ④Rockbolt (L=3.5m, Interval 2.0m×2.0m, Bearing Plate 1000mm×1000mm), Total Length L=130m

⑥Drainage Works

Temporary Road  
(L=400m)

Plant yard(1)

Plant yard(2)

④Rockbolt

③Ground Anchor

⑤Gravity Retaining Wall

Plant yard(3)

Rockfall Protection

⑧Culvert

To Islamabad

⑦Pavement Works & Soil Improvement (Base course), Total Length L=300m



Google Earth  
 Image © 2018 CNES / Airbus

APPROVED	NOTE	PROJECT NO.	PROJECT NAME	TITLE	SHEET. NO.
CHECKED	Google Earth Image © 2018 CNES / Airbus	DATE: June / 2018	Data Collection Survey on Road Landslide Measures in Pakistan	N15-No. 9 Plan (Temporary work)	12
DRAWN				DRAWING No. N15-No.9-3	SCALE 1:500 —(A1) 1:1000 —(A3)

①Slope Cutting, ②Grating Crib Works, ③Mortar Spraying,  
 ④Rockbolt (L=4.0m, Interval 2.0m×2.0m, Bearing Plate 1000mm×1000mm),  
 Total Length L=150m

⑥Drainage Works

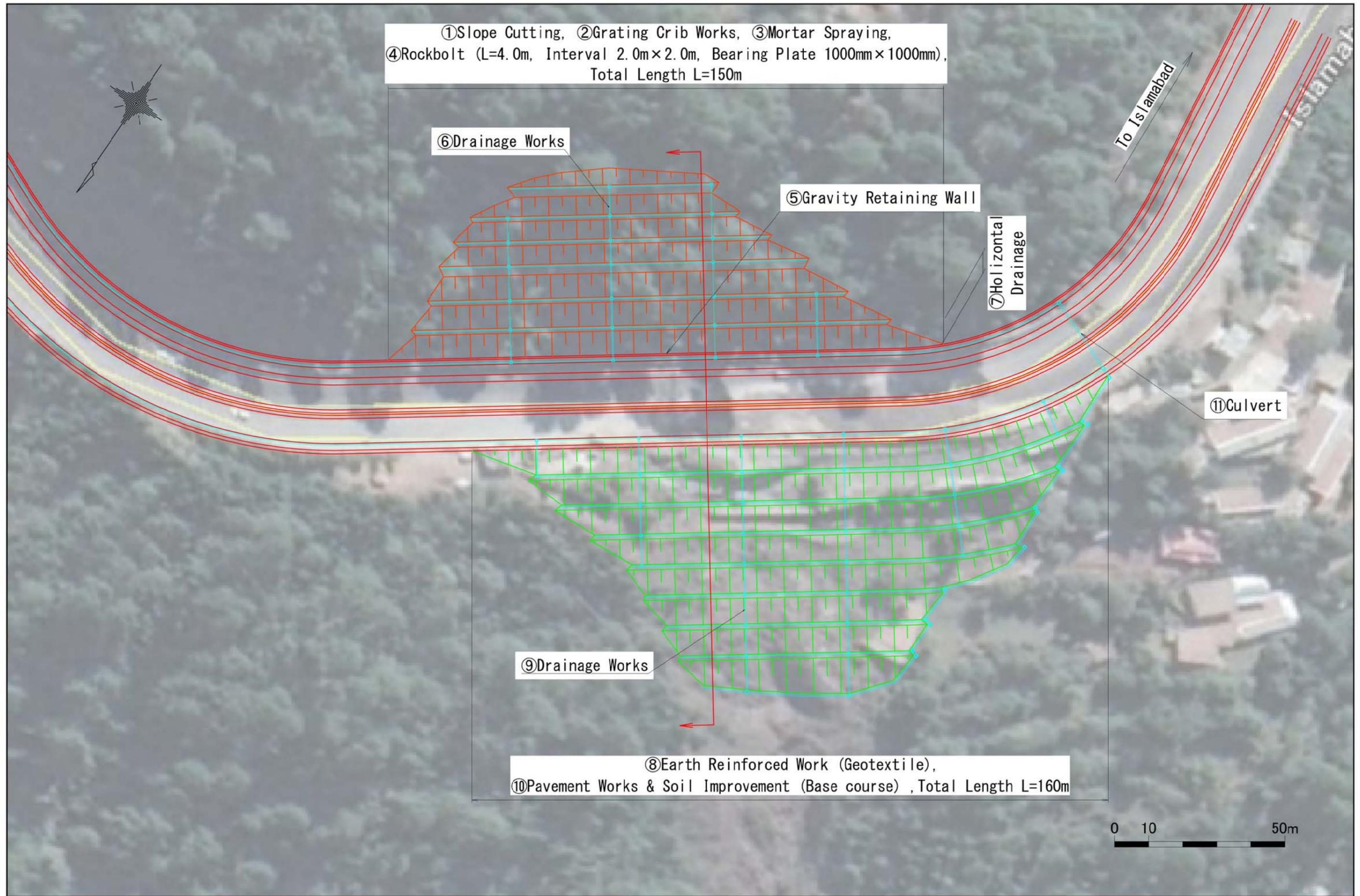
⑤Gravity Retaining Wall

⑦Horizontal  
 Drainage

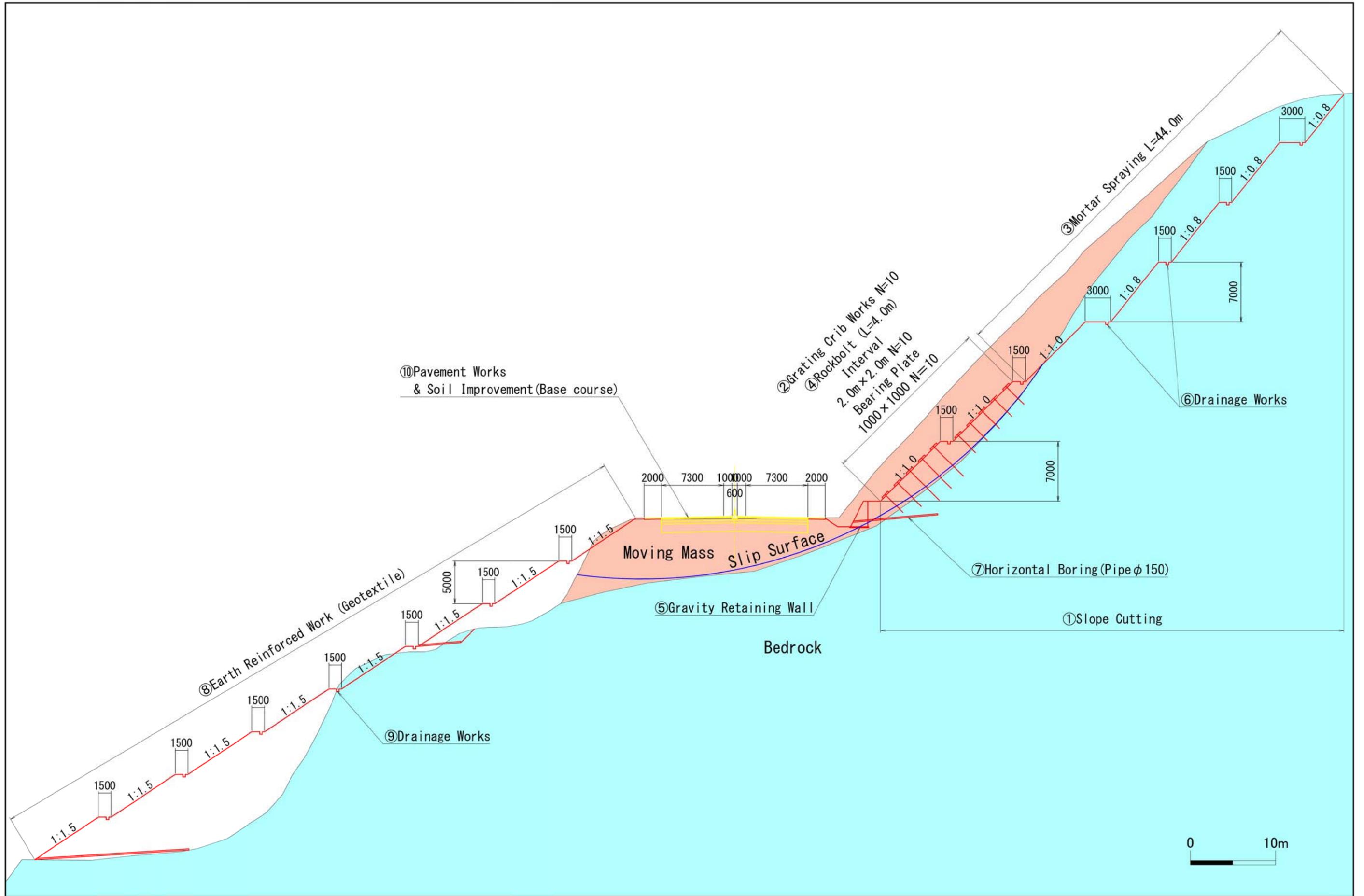
⑪Culvert

⑨Drainage Works

⑧Earth Reinforced Work (Geotextile),  
 ⑩Pavement Works & Soil Improvement (Base course) , Total Length L=160m

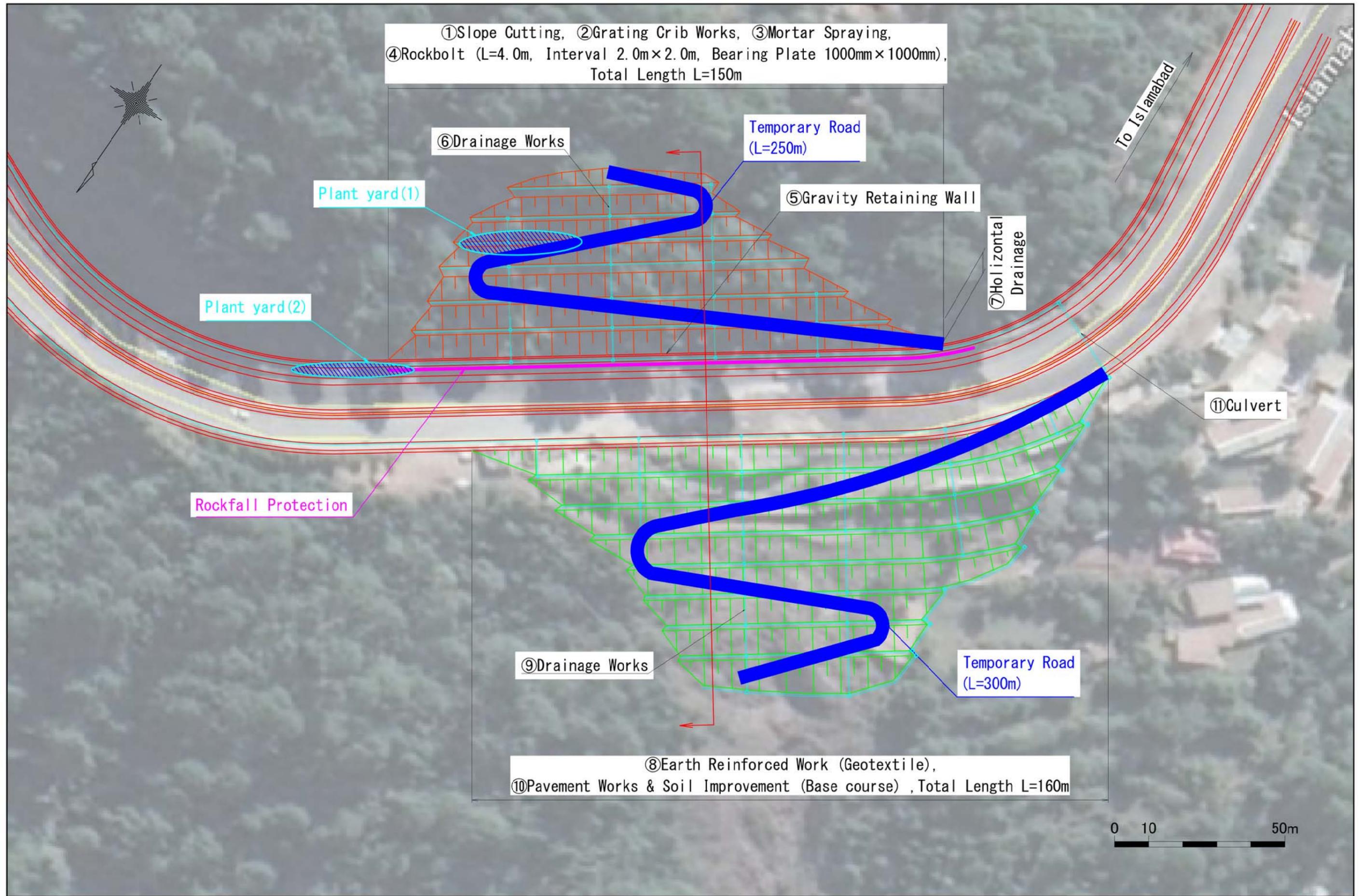


APPROVED	NOTE	PROJECT NO.	PROJECT NAME	TITLE	SHEET. NO.
CHECKED	Google Earth Image © 2018 CNES / Airbus	DATE: June / 2018	Data Collection Survey on Road Landslide Measures in Pakistan	N75-No. 2 Plan	13
DRAWN				DRAWING No. N75-No.2-1	SCALE 1:500—(A1) 1:1000—(A3)



APPROVED CHECKED DRAWN	NOTE	PROJECT NO.	PROJECT NAME	TITLE	SHEET NO.
		DATE: June / 2018	Data Collection Survey on Road Landslide Measures in Pakistan	N75-No. 2 Typical Corss Section	14
				DRAWING No. N75-No.2-2	SCALE 1:200 —(A1) 1:400 —(A3)

①Slope Cutting, ②Grating Crib Works, ③Mortar Spraying,  
 ④Rockbolt (L=4.0m, Interval 2.0m×2.0m, Bearing Plate 1000mm×1000mm),  
 Total Length L=150m



Plant yard(1)

Plant yard(2)

Rockfall Protection

⑥Drainage Works

Temporary Road  
(L=250m)

⑤Gravity Retaining Wall

⑦Horizontal  
Drainage

⑪Culvert

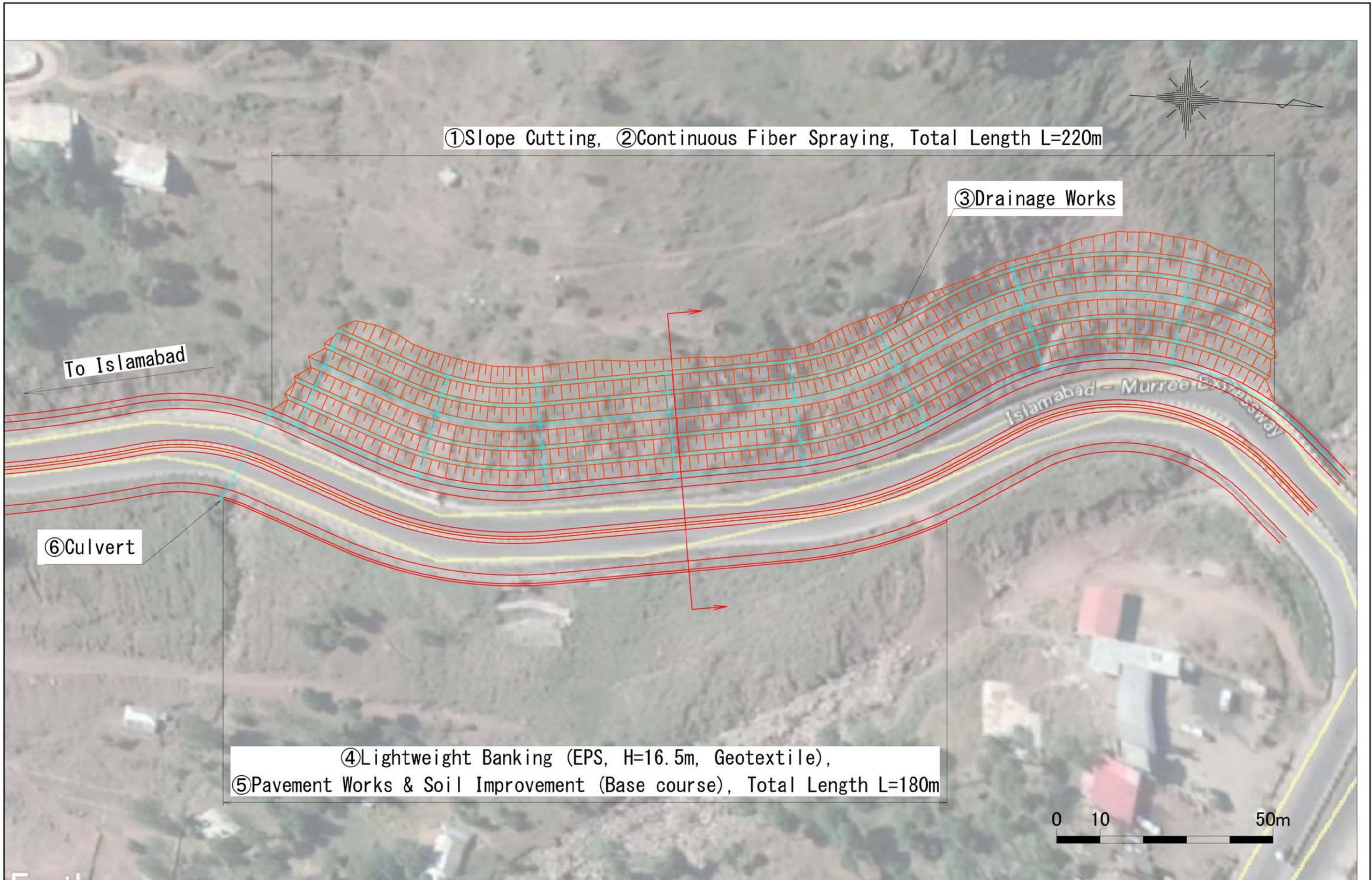
⑨Drainage Works

Temporary Road  
(L=300m)

⑧Earth Reinforced Work (Geotextile),  
 ⑩Pavement Works & Soil Improvement (Base course) , Total Length L=160m

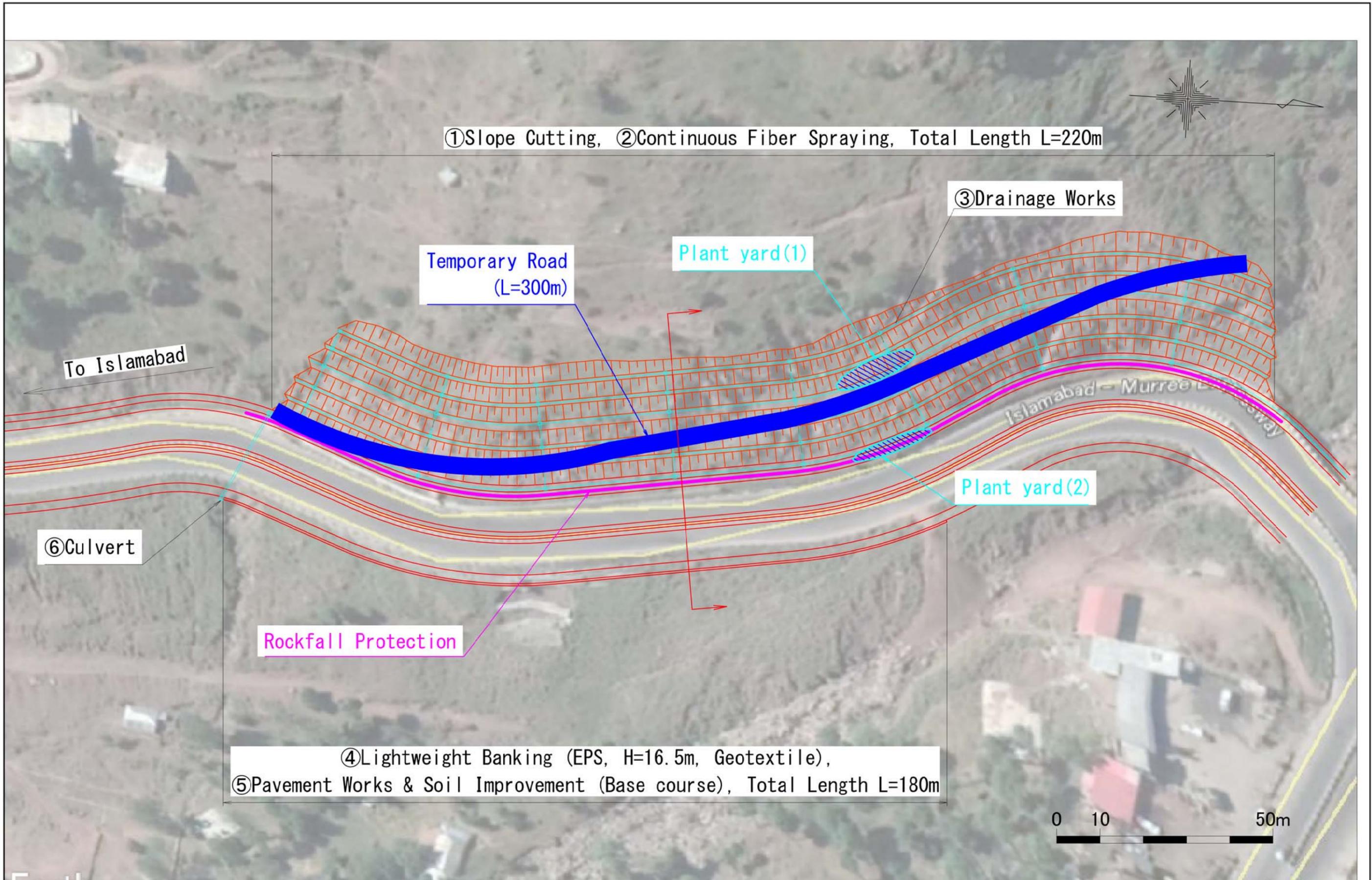
0 10 50m

APPROVED	NOTE	PROJECT NO.	PROJECT NAME	TITLE	SHEET. NO.
CHECKED	Google Earth Image © 2018 CNES / Airbus	DATE: June / 2018	Data Collection Survey on Road Landslide Measures in Pakistan	N75-No. 2 Plan (Temporary work)	15
DRAWN				DRAWING No. N75-No.2-3	SCALE 1:500—(A1) 1:1000—(A3)



APPROVED	NOTE	PROJECT NO.	PROJECT NAME	TITLE	SHEET. NO.
CHECKED	Google Earth Image © 2018 CNES / Airbus	DATE: June / 2018	Data Collection Survey on Road Landslide Measures in Pakistan	N75-No. 5 Plan	16
DRAWN				DRAWING No. N75-No.5-1	SCALE 1:400—(A1) 1:800—(A3)





APPROVED	NOTE	PROJECT NO.	PROJECT NAME	TITLE	SHEET. NO.
CHECKED	Google Earth Image © 2018 CNES / Airbus	DATE: June / 2018	Data Collection Survey on Road Landslide Measures in Pakistan	N75-No. 5 Plan (Temporary work)	18
DRAWN				DRAWING No. N75-No.5-3	SCALE 1:400—(A1) 1:800—(A3)

## Appendix 5

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### *Survey Result of Shortlist Slopes*



# **DATA COLLECTION SURVEY ON ROAD LANDSLIDE MEASURES IN PAKISTAN**

## **Part-I**

### ***ELECTRIC RESISTIVITY SURVEY DATA FOR THE SITES N15\_3, N15\_6, N15\_8 & N15\_9***

*Report submitted by:*

*Dr. Naveed Ahmad*

*Report submitted to:*

*Mr. Makoto ASHINO*

# 1. Introduction to Electrical Resistivity Survey

## 1.1 Principal:

Electrical current is transmitted into the earth and the response of the earth in the form of resistivity is measured. Ground contains various dissolved salts and it is conductive, this enables electric currents to flow into ground. As a result, by calculating the ground resistivity it gives the approximate number of layers and also the availability of water. This produces a one-dimensional profile of the resistivity beneath the midpoint of the survey. This technique is used to examine the vertical change in resistivity. This method involves progressively increasing the electrode spacing between measurements while the centre of the whole array is kept constant. As the electrode spacing is increased, the current permeates to greater depths (McDowell et. al., 2002). Therefore, a plot of apparent resistivity against electrode spacing produces an image of the variation of resistivity with depth.

## 1.2 Uses/Advantages of this method:

This Technique has been successfully used for:

- To delineate various lithological units
- To find bedrock depth or hard stratum
- Ground water depth estimation
- Interpretation of the data is rapid.
- It is a nondestructive test.

## 1.3 Limitation of this Method

This method alone does not resolve all ambiguities of the subsurface condition and some additional information such as borehole data is required to confirm the information obtained from this survey. A great experience is required to accurately interpret the data. Environmental condition of the area should be taken into account while interpreting the data of electrical resistivity survey. Properly integrated with other information, resistivity surveying is an effective method of obtaining subsurface information. Ambiguities in interpretation arising from equivalence (where two resistive layers carry nearly the same electric current if the products of their resistivity and thickness equal). Disadvantages of this method are stated below:

- Deep investigations require long cables and consume much field time.
- Interpretation of complex geologic structures is difficult and ambiguous.
- Presence of metal pipes, cables, fences and electrical grounds can complicate interpretation.
- Accuracy of depth determination is lower than with seismic techniques.
- Extremely resistive materials will prevent current injection into the ground.

## 1.4 Different method of Electrical Resistivity Survey:

This test can be conducted by making various arrangements of current and potential electrodes like Schlumberger array, wenner array, Dipole-dipole array etc. Every method have their own advantages like Schlumberger array can be use for deep investigation and wenner array can be used for shallow investigation.

### Schlumberger array:

This method consist of unequally spaced inline electrodes where  $AB > 5MN$ . Potential electrodes position is kept constant for some readings and current electrodes position are kept varying at constant position of potential electrodes.

The formula for calculating resistivity is given by:

$$R = \pi \frac{(AB/2)^2 - (MN/2)^2}{MN} \times \frac{V}{I}$$

Where “AB” is the distance between current electrodes “MN” is the distance between potential electrodes “V” is measured voltage and “I” is Current “K” =  $\pi \frac{(AB/2)^2 - (MN/2)^2}{MN}$  = Geometric factor

This method is use when investigation at greater depth is required.

### Wenner array:

Wenner array consist of equally spaced inline electrodes. The electrodes are position such that the distance between any two consecutive electrodes remains the same. The wenner array formula for calculating apparent resistivity can be given by:

$$R = 2\pi a \left(\frac{V}{I}\right) \quad \text{Where “a” is electrode spacing}$$

“V” is Measured voltage and “I” is Current

In wenner array the four electrodes are in a straight line and at a uniform spacing. The uniform spacing is designated “a”.

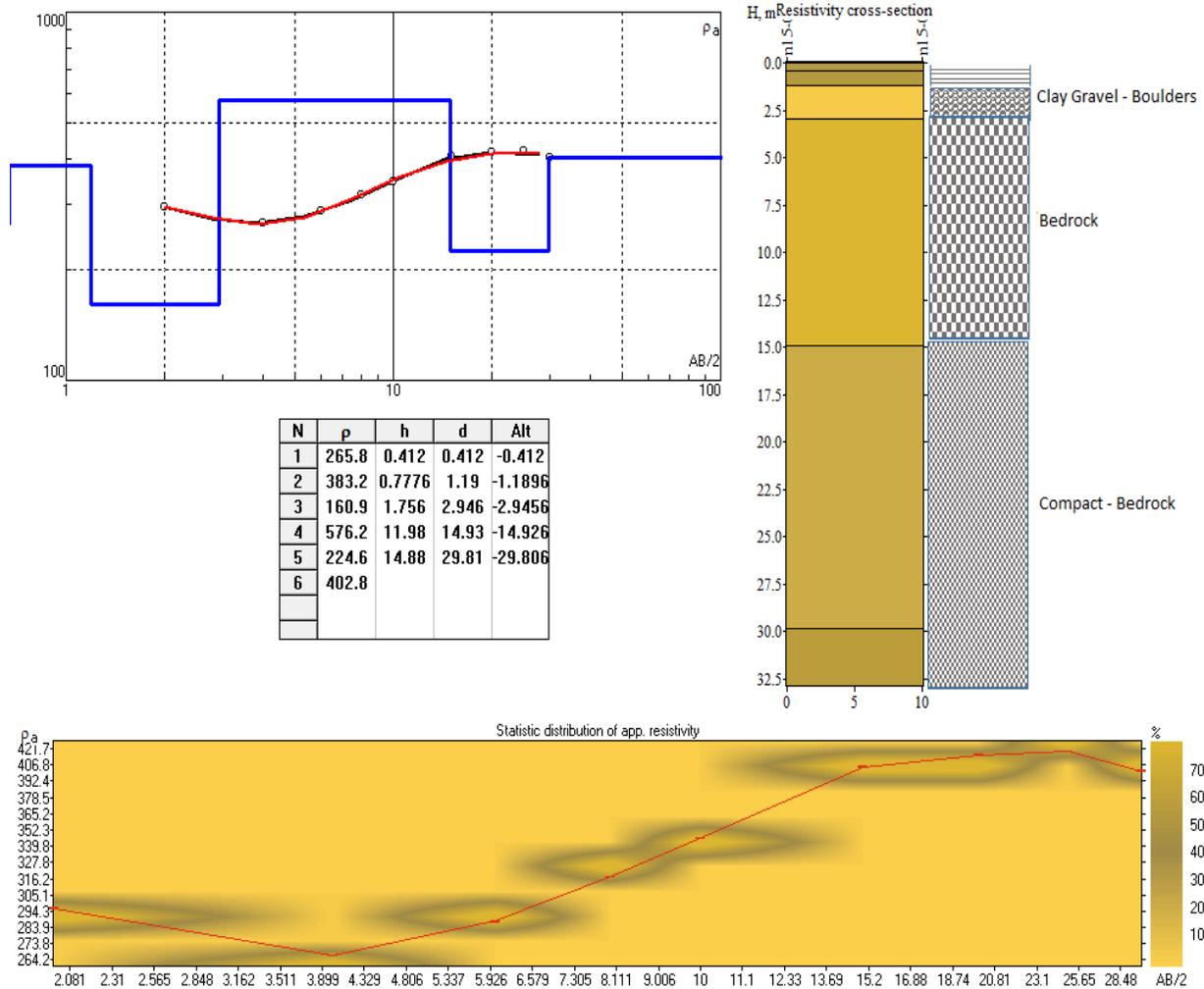
## 2. Site details for N15\_3

Table1: Site Details for N15\_3

<b>Area</b>	Northern Area
<b>Location</b>	N 15-03
<b>Client</b>	Mr. Makoto ASHINO
<b>Resistivity Parameter</b>	Schlumberger Array
<b>Purpose</b>	To investigate the bed rock depth.

Using Schlumberger, resistivity data was recorded from proposed three sites. For Schlumberger configuration the data up to spread “AB/2”=30 meter was recorded.

### 2.1 Resistivity Section



## 2.2 Data

Field Data Sheet					
Soil Resistivity Measurement					
Location	Area	Dept/Owner	Contractor	Consultant	
N15_3	Malakandi Kaghan				
No. Sounding 1 Date 02-06-2018 N 34 39 28.13365 E 073 30 0.79397 Direction EW Elev.: 1334 Meter		Field Inspector: Dr. Naveed Ahmad Field Staff: Inayat, Khalid, Amin Sub Soil Condition: Clay Gravel - boulders and Compact Bedrock			
AB/2	MN/2	R-Ohm/M	V	I	Density (gm/cm <sup>3</sup> )
2	0.5	296.22	39.77	1.59	1.68
4		256.58	51.07	9.85	1.71
6		288.13	35.2	13.73	2.41
8		322.2	22.16	20.48	2.51
10		326.44	21.34	20.35	2.59
10	2	346.42	93.46	2.13	2.61
15		406.94	4.99	2.94	2.68
20		394.95	3.73	17.98	2.74
25		346.41	12.77	2.75	2.79
30		325.5	2.28	113.44	2.83

## 2.3 Google Image



## 2.4 CONCLUSION:

Clay and gravel – boulders up to few meter, followed by compact bedrock. Bedrock in compact form can be explored on drilling around 20 - 30 ft.

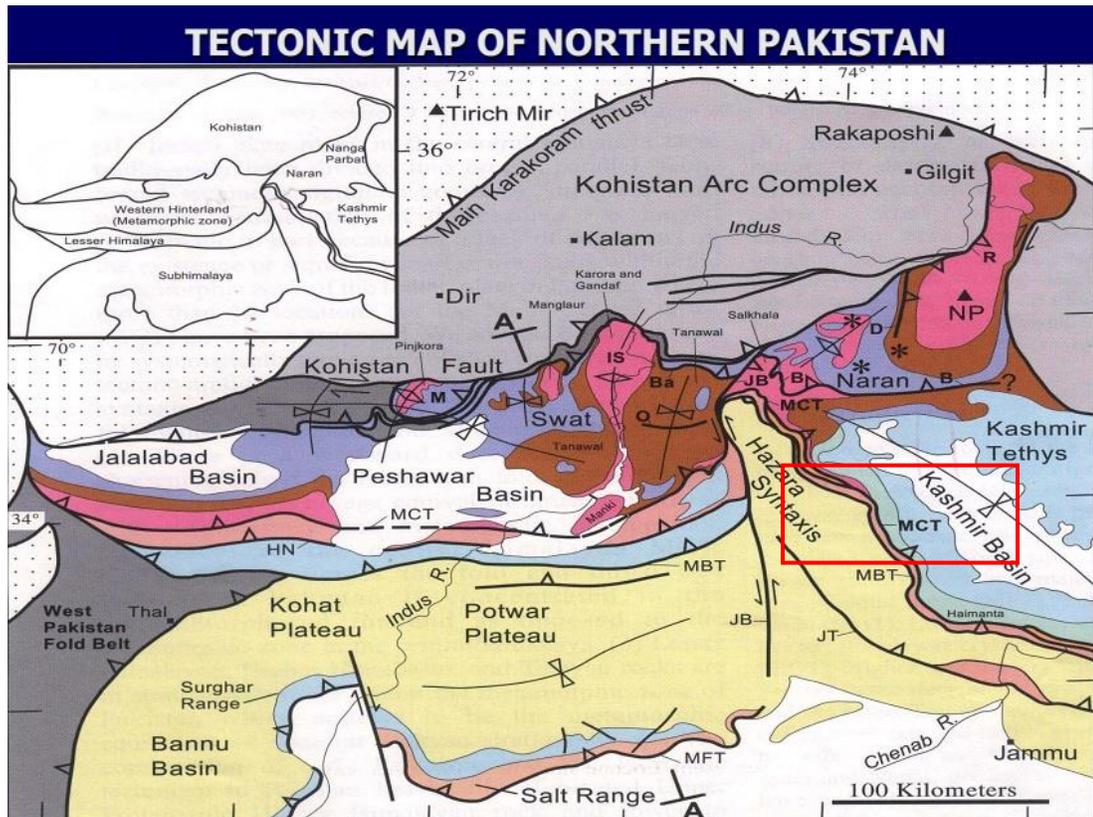
Sr. No.	Soil Condition	Depth (Meter)
1	Clay with Gravel – boulder and some sand	00 – 03
2	Weathered Bedrock	03 - 06
3	Hard Compact Bedrock	06 – Onward

“It is to be mentioned that selection of site has been made merely on current Resistivity data and is not supported by other hydro geological information in the area. **It is important to be aware of some limitations inherent in this method.**”

## 2.5 GENERAL GEOLOGY OF THE AREA/SITE:

### REGIONAL TECTONIC:

Kaghan valley, in north Pakistan, provides an unparalleled cross-section of the Indian Plate in the Himalaya with all of the tectonic units, structural features and metamorphic grades of the Indian Plate exposed in one valley. The Higher Himalaya Crystalline of the NW Himalaya, which crops out in the upper reaches of the valley, is a complex sequence of granites, gneisses, Calc-pelites, metapelites, marbles, amphibolites, quartzites and eclogites. It is bound by two major thrusts: The Main Central Thrust (MCT) to the south and the Main Mantle Thrust (MMT) to the north, which have subsequently led to the exposure of some of the deepest level rocks of the Indian plate during its collision with the Asian plate.



## 2.6 GEOLOGY OF THE SITE:

This site is mostly composed of dolomite. Dolomite is of dark to medium gray color in fresh surface and pale to light yellow in weathered surface. Also calcite veins and iron leaching has been found. The formation is highly weathered and crushed due to high tectonic activity in area.



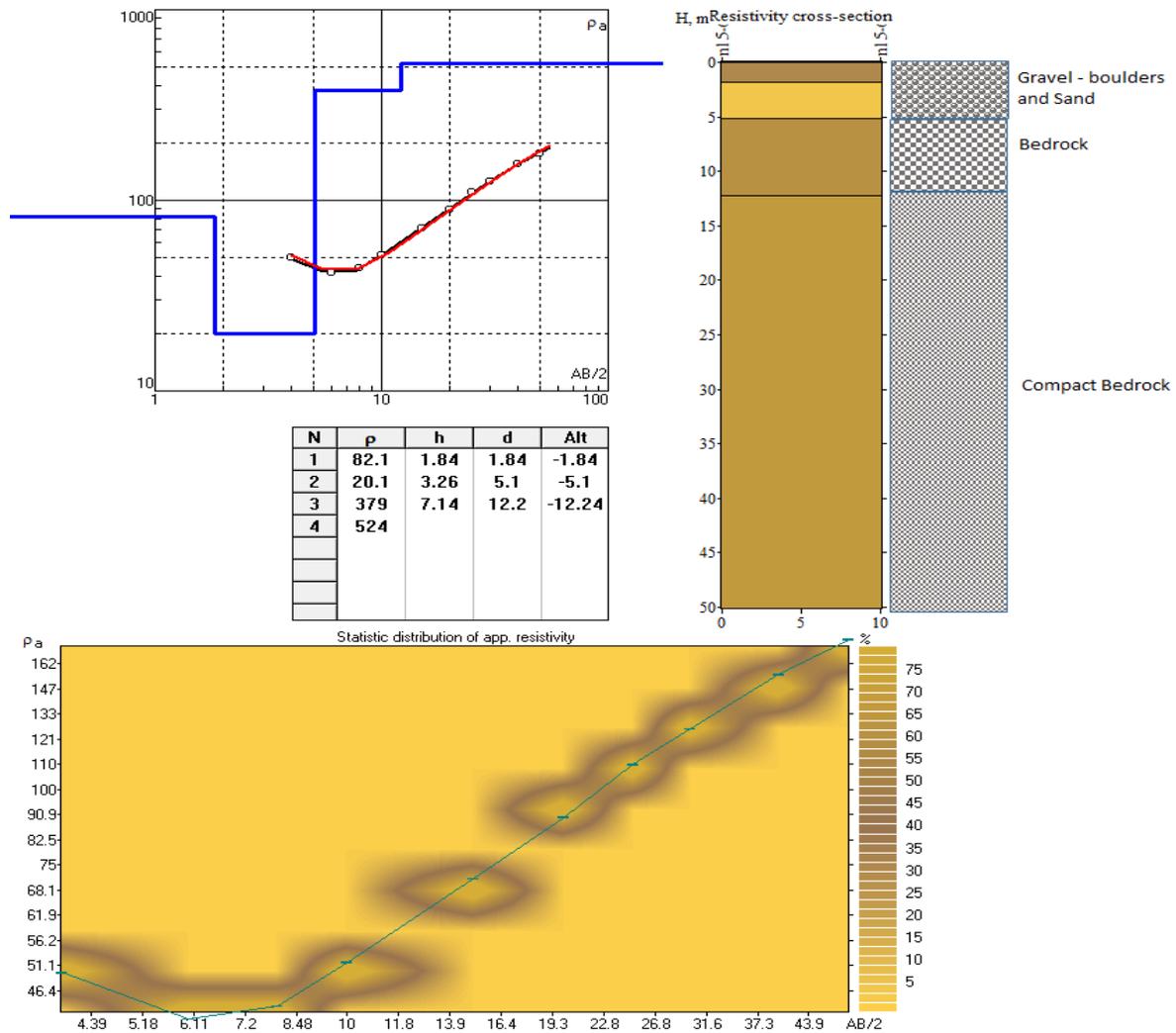
### 3. Site details for N15\_6

Table 2: Site details for N15\_6

<b>Area</b>	Northern Area
<b>Location</b>	N15_06
<b>Client</b>	Mr. Makoto ASHINO
<b>Resistivity Parameter</b>	Schlumberger Array
<b>Purpose</b>	To investigate the bed rock depth.

Using Schlumberger, resistivity data was recorded from proposed three sites. For Schlumberger configuration the data up to spread “AB/2”=50 meter was recorded.

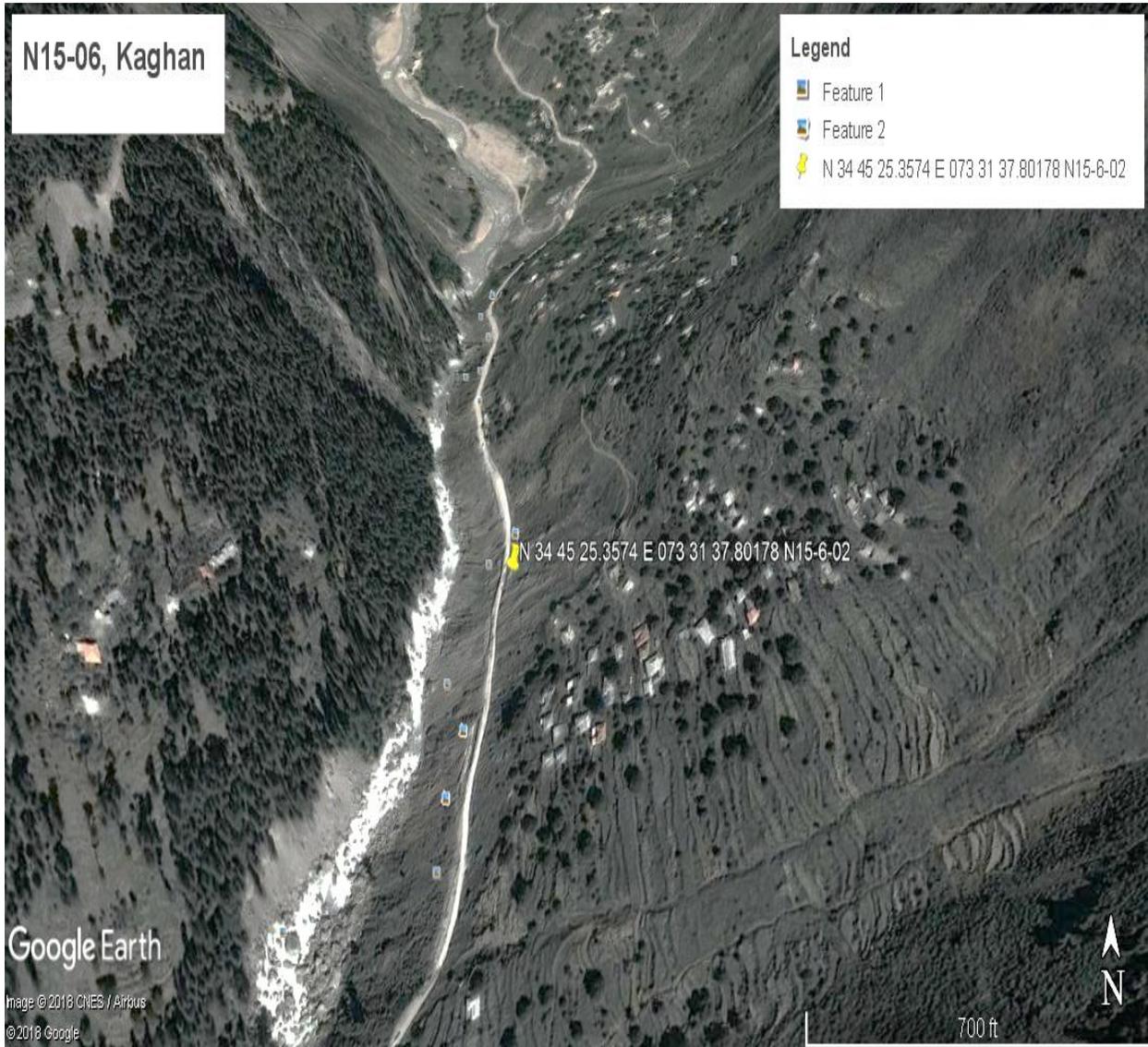
#### 3.1 Resistivity Section



### 3.2 Data

Field Data Sheet					
Soil Resistivity Measurement					
Location	Area	Dept/Owner	Contractor	Consultant	
N 15 -6	Kaghan				
No. Sounding 2 Date 02-06-2018 N 34 45 25.3574 E 073 31 37.80178 Direction EW Elev.: 1963 Meter		Field Inspector: Dr. Naveed Ahmad Filed Staff: Inayat, Khalid, Amin Sub Soil Condition: Clay Gravel - boulders and Compact Bedrock			
AB/2	MN/2	R-Ohm/M	V	I	Density (gm/cm3)
2	0.5	46.59	268.91	67.76	1.57
4		19.42	23.59	60.12	2.41
6		29.95	15.87	59.27	2.49
8		71.04	19.63	54.76	2.55
10		168.25	1.2	53.18	2.61
10	2	173.2	124.44	54.18	2.61
15		169.86	38.35	39.19	2.69
20		248.08	31.48	39.46	2.72
25		247.26	19.63	38.71	2.75
30		259.84	14.87	40.27	2.77
40		283.9	10.45	46.12	2.8
50		310.32	8.46	51.57	2.81

### 3.3 Google Image



### 3.4 CONCLUSION:

Clay and gravel – boulders up to few meter, followed by compact bedrock. Bedrock in compact form can be explored on drilling around 30 ft.

S#	Soil Condition	Depth (Meter)
1	Clay with Gravel – boulder and some sand	00 – 01
2	Large Boulders	01 - 05
3	Weathered Bedrock	05–12
4	Compact – Hard Bedrock	12 – Onward

“It is to be mentioned that selection of site has been made merely on current Resistivity data and is not supported by other hydro geological information in the area. **It is important to be aware of some limitations inherent in this method.**”

### 3.5 GEOLOGY OF THE SITE:

This site is mostly composed of metamorphosed rock mainly mica schist and quartz schist. Schist is a medium-grade metamorphic rock with medium to large, flat, sheet-like grains in a preferred orientation. Schist forms at a higher temperature and has larger grains flakes in a preferred sheet like orientation is called schistosity. The individual mineral grains in schist, drawn out into flaky scales by heat and pressure, can be seen with the naked eye. Schist is characteristically foliated, meaning that the individual mineral grains split off easily into flakes or slabs. Due to water it weak the bond between the layers and in result it swell and after some time split in sheets and cause erosion.



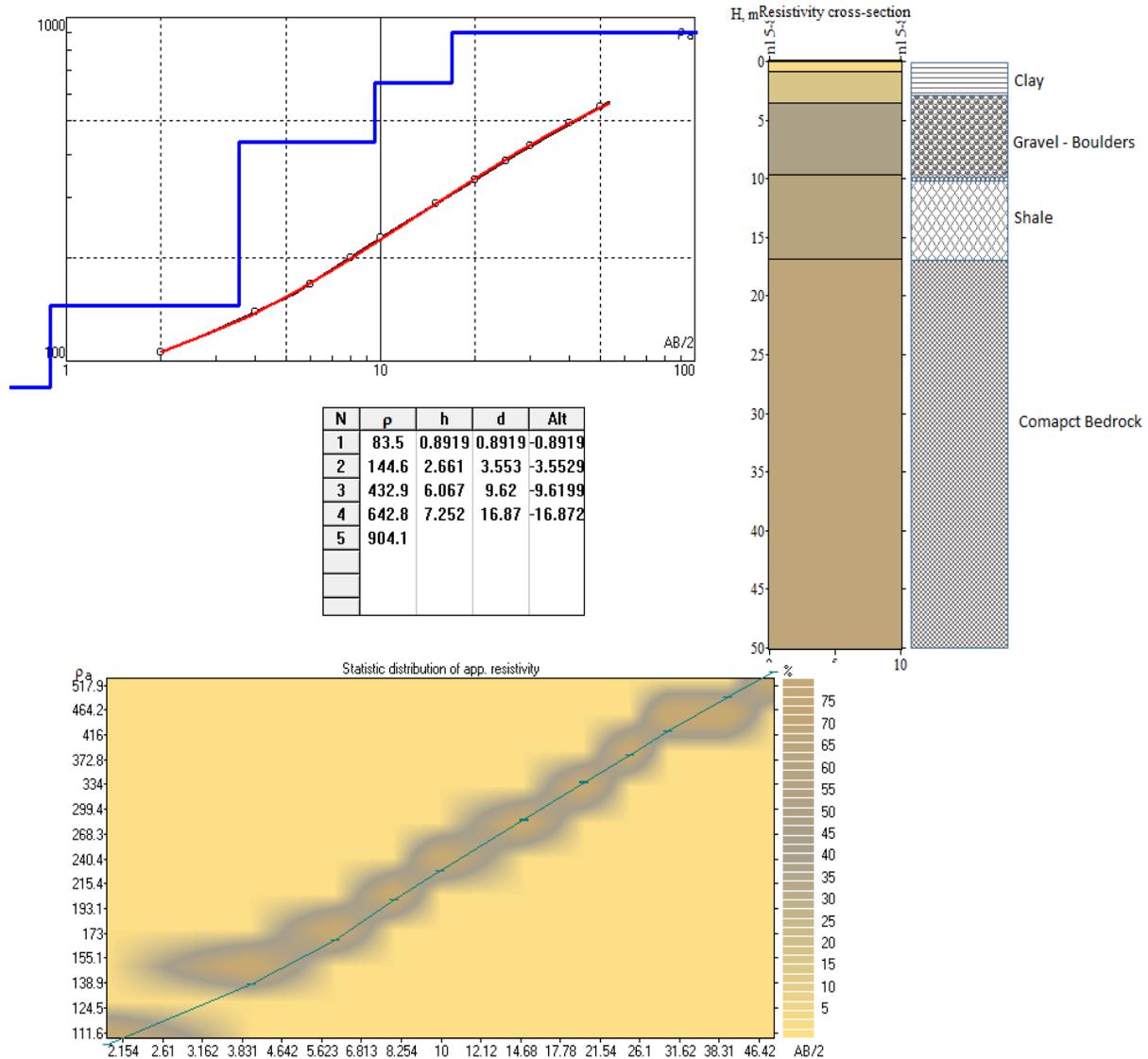
## 4. Site details for N15\_8

Table 3: Site details for N15\_8

<b>Area</b>	Northern Area
<b>Location</b>	N 15_08
<b>Client</b>	Mr. Makoto ASHINO
<b>Resistivity Parameter</b>	Schlumberger Array
<b>Purpose</b>	To investigate the bed rock depth.

Using Schlumberger, resistivity data was recorded from proposed three sites. For Schlumberger configuration the data up to spread “AB/2”=50 meter was recorded.

### 4.1 Resistivity Section



## 4.2 Data

Field Data Sheet					
Soil Resistivity Measurement					
Location	Area	Dept/Owner	Contractor	Consultant	
N 15 -8	Naran				
No. Sounding 3 Date 02-06-2018		Field Inspector: Dr. Naveed Ahmad			
N 34 51 21.47204 E 073 35 47.15335		Filed Staff: Inayat, Khalid, Amin			
Direction EW Elev.: 2232 Meter		Sub Soil Condition: Clay Gravel - boulders and Shale - Compact Bedrock			
AB/2	MN/2	R-Ohm/M	V	I	Density (gm/cm <sup>3</sup> )
2	0.5	104.35	95.43	10.78	1.62
4		136.18	12.3	4.47	1.63
6		165.15	10.47	7.12	2.51
8		183.01	7.28	7.96	2.56
10		196.91	6.53	10.38	2.52
10	2	227.95	31.72	10.49	2.57
15		319.08	12.59	6.85	2.7
20		364.74	13.61	11.61	2.71
25		457.32	10.46	11.16	2.71
30		400.14	39.95	19.81	2.73
40		463.81	20.34	12.05	2.73
50		618.03	7.55	6	2.77

### 4.3 Google Image



#### 4.4 CONCLUSION:

Clay and gravel – boulders up to few meter, followed by compact bedrock. Bedrock in compact form can be explored on drilling around 30 ft.

S#	Soil Condition	Depth (Meter)
1	Clay with Gravel – boulder and some sand	00 – 03
2	Large Boulders	03 - 09
3	Shale	09–16
4	Compact – Hard Bedrock	16 – Onward

“It is to be mentioned that selection of site has been made merely on current Resistivity data and is not supported by other hydro geological information in the area. **It is important to be aware of some limitations inherent in this method.**”

#### 4.5 GEOLOGY OF THE SITE:

This site is mainly composed of dolomite, slates and schist. Dolomite is of medium gray in color at fresh surface and pale to light yellow in weathered surface. Calcite veins and iron leaching has been found. Slates are dark gray to black in color and are highly folded and crushed due to tectonic activity. Schist is mostly mica schist and highly weathered.



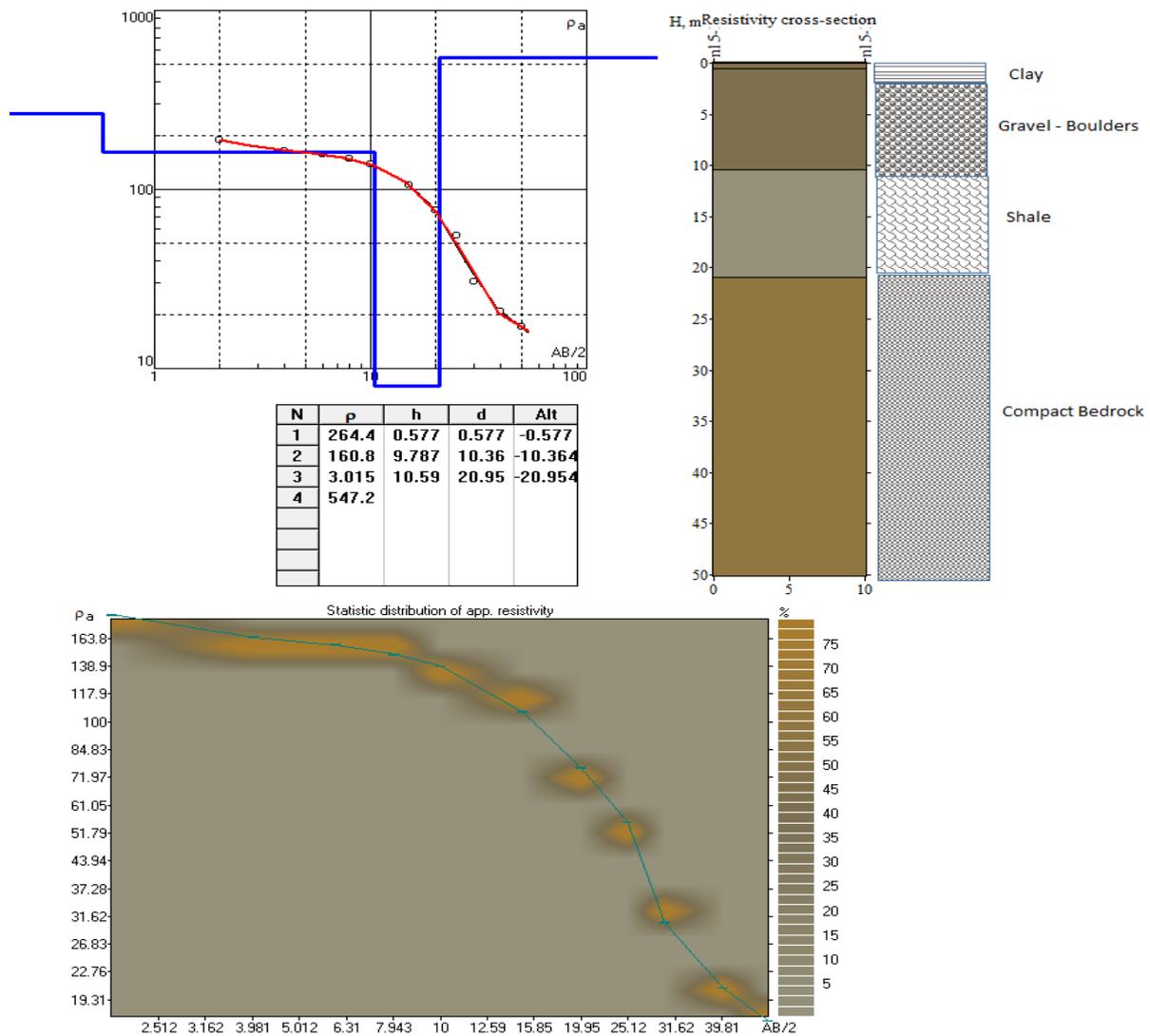
## 5. Site details for N15\_9

Table 4: Site details for N15\_9

<b>Area</b>		Northern Area
<b>Location</b>		N 15_09
<b>Client</b>		Mr. Makoto ASHINO
<b>Resistivity Parameter</b>		Schlumberger Array
<b>Purpose</b>		To investigate the bed rock depth.

Using Schlumberger, resistivity data was recorded from proposed three sites. For Schlumberger configuration the data up to spread “AB/2”=50 meter was recorded.

### 5.1 Resistivity Section



## 5.2 Data

Field Data Sheet						
Soil Resistivity Measurement						
Location		Area	Dept/Owner	Contractor	Consultant	
N 15 – 09		Naran				
No. Sounding 4 Date 02-06-2018			Field Inspector: Dr. Naveed Ahmad			
N 34 51 47.611 E 073 36 11.24			Filed Staff: Inayat, Khalid, Amin			
Direction EW		Elev.: 2245 Meter	Sub Soil Condition: Clay Gravel - boulders and Shale - Compact Bedrock			
AB/2	MN/2		R-Ohm/M	V	I	Density (gm/cm <sup>3</sup> )
2	0.5		188.15	37.74	2.37	1.66
4			165.05	26.83	8.05	2.45
6			157.71	8.08	5.76	2.51
8			165.43	3.61	4.37	2.53
10			180.18	3.13	5.44	2.55
10	2		137.54	10.01	5.49	2.55
15			116.75	4.98	7.41	1.81
20			95.11	1.27	4.16	1.86
25			65.2	1.35	10.1	1.92
30			30.47	0.55	12.58	1.97
40			14.48	0.6	50.89	2.02
50			17.02	0.82	94.25	2.1

### 5.3 Google Image



## 5.4 CONCLUSION:

Clay and gravel – boulders up to few meter, followed by shale - compact bedrock. Bedrock in compact form can be explored on drilling around 30 ft.

S#	Soil Condition	Depth (Meter)
1	Clay with Gravel – boulder and some sand	00 – 01
2	Large Boulders	01 - 10
3	Shale	10–20
4	Compact – Hard Bedrock	20 – Onward

“It is to be mentioned that selection of site has been made merely on current Resistivity data and is not supported by other hydro geological information in the area. **It is important to be aware of some limitations inherent in this method.**”

## 5.5 GEOLOGY OF THE SITE:

This site is mainly composed of shale and slate. Shale is of dark gray to black in color and in mostly powdered form. Shale is low to medium carbonaceous. In resistivity high value of “I” indicate the shale in subsurface with low resistance value. Slates are also dark in color and are found in thin layers, highly folded and crushed in area.



## SPT N-values for different boreholes upto bored depth

Depth (m)	SPT N-values								
	N75_2			N15_3	N15_6			N15_8	N15_9
	1	2	3		1	2	3		
1	5	3	4		3	50/5cm	8	30	12
2	11	50/2.5cm	14		50/8cm (Seepage water intercepted)		4	24	5
2.5			50/2.5cm		50/2.5cm				
3	50/10cm						4	23	50/20cm
4							50/25cm	38	50/17cm
5							50/2.5cm	48	50/15cm
6								50/22cm	50/15cm
7								26	50/10cm
8								50/7.5cm	

Note please:

- 1- Due to the presence of gravels, SPT sampler was used with cone instead of split spoon sampler.
- 2- Diameter of boreholes advanced at N75\_2 is 12.5cm and for all other sites it is 15cm.
- 3- Seepage water was not intercepted in boreholes upto intercepted depths except for site N15\_6.
- 4- At N75\_2, hard stratum was intercepted at a shallow depth and borehole was not advancing any further, so two additional boreholes parallel to newly constructed retaining wall were advanced to verify that it is a bedrock.
- 5- At N15\_6, first two boreholes were drilled adjacent to road on hillside but due to unavailability of space and presence of Optical Fiber Cable, third borehole was advanced on the riverside adjacent to same road and opposite to first borehole.
- 6- Complete report showing vertical soil profile at borehole location, location of seepage/ground water table (if any), tentative location of bedrock, density of debris material and all the results of lab/field tests will be shared before 15th of June 2018.



## Electrical resistivity survey in soil science: a review .

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Richard

► **To cite this version:**

Anatja Samouëlian, Isabelle Cousin, Alain Tabbagh, Ary Bruand, Guy Richard. Electrical resistivity survey in soil science: a review .. Soil and Tillage Research, Elsevier, 2005, 83, pp.2, 173-193. <10.1016/j.still.2004.10.004>. <hal-00023493>

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# Electrical resistivity survey in soil science: a review

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## Abstract

Electrical resistivity of the soil can be considered as a proxy for the spatial and temporal variability of many other soil physical properties (i.e. structure, water content, or fluid composition). Because the method is non-destructive and very sensitive, it offers a very attractive tool for describing the subsurface properties without digging. It has been already applied in various contexts like : groundwater exploration, landfill and solute transfer delineation, agronomical management by identifying areas of excessive compaction or soil horizon thickness and bedrock depth, and at least assessing the soil hydrological properties. The surveys, depending on the areas heterogeneities can be performed in one-, two- or three-dimensions and also at different scales resolution from the centimetric scale to the regional scale. In this review, based on many electrical resistivity surveys, we expose the theory and the basic principles of the method, we overview the variation of electrical resistivity as a function of soil properties, we listed the main electrical device to performed one-, two- or three-dimensional surveys, and explain the basic principles of the data interpretation. At least, we discuss the main advantages and limits of the method.

## 1. Introduction

The changes caused on soil by intensive agricultural production are variable in space and time. As a consequence, a continuous and precise spatially and temporal follow-up of the soil physical and chemical properties is required. Geophysical methods have been applied to soil sciences for a considerable period. The general principle of geophysical exploration is to non intrusively collect data on the medium under investigation (Scollar et al., 1990). Among such methods, those based on the electric properties seem particularly promising because soil materials and properties are strongly correlated and can be quantified through the geoelectrical properties. Indeed, the flux of electrical charges through materials permits conductor materials like metal or electrolytes, where the conductivity is great, to be distinguished from insulating materials like air, ice and plastics, where it is small. Among the latter, soil materials exhibit intermediate electrical properties depending on their physical and chemical properties (texture, salinity or water content). Schlumberger in 1912 cited by Meyer de Stadelhofen (1991) introduced the idea of using electrical resistivity measurements to study subsurface rock bodies. This method was first adopted in geology by oil companies searching for petroleum reservoirs and delineating geological formations. In soil science, Bevan (2000) reported that the first known equipotential map was compiled by Malamphy in 1938 for archaeological research at the site of Williamsburg in USA. Since that early study, the interest in subsurface soil prospecting by electrical prospecting has steadily increased. In this paper, we review the literature dealing with the use of electrical resistivity applied to soil. We first present the basic concept of the method and the different array devices. Then, we discuss the sensitivity of the electrical measurements to the soil

properties. Finally, the advantages and limitations of electrical resistivity in soil survey and the application to agricultural management are discussed. 2. Theory and basic principles The purpose of electrical resistivity surveys is to determine the resistivity distribution of the sounding soil volume. Artificially generated electric currents are supplied to the soil and the resulting potential differences are measured. Potential difference patterns provide information on the form of subsurface heterogeneities and of their electrical properties (Kearey et al., 2002). The greater the electrical contrast between the soil matrix and heterogeneity, the easier is the detection. Electrical resistivity of the soil can be considered as a proxy for the variability of soil physical properties (Banton et al., 1997). The current flow line distributions depend on the medium under investigation; they are concentrated in conductive volumes. For a simple body, the resistivity  $\rho$  ( $\Omega$  m) is defined as follows :

$$\rho = R \left( \frac{S}{L} \right)$$

with  $R$  being the electrical resistance ( $\Omega$ ),  $L$  the length of the cylinder (m) and  $S$  is its cross-sectional area ( $m^2$ ). The electrical resistance of the cylindrical body  $R$  ( $\Omega$ ), is defined by the Ohm's law as follows:

$$R = \frac{V}{I}$$

with  $V$  being the potential (V) and  $I$  is the current (A). Electrical characteristic is also commonly described by the conductivity value  $\sigma$  ( $Sm^{-1}$ ), equal to the reciprocal of the soil resistivity. Thus :

$$\sigma = \frac{1}{\rho}$$

In a homogeneous and isotropic half-space, electrical equipotentials are hemispherical when the current electrodes are located at the soil surface as shown in Fig. 1 (Scollar et al., 1990; Kearey et al., 2002; Sharma, 1997; Reynolds, 1997). The current density  $J$  ( $A/m^2$ ) has then to be calculated for all the radial directions with :

$$J = \frac{I}{2\pi r^2}$$

where  $2\pi r^2$  is the surface of a hemispherical sphere of radius  $r$ . The potential  $V$  can then be expressed as follows:

$$V = \frac{\rho I}{2\pi r}$$

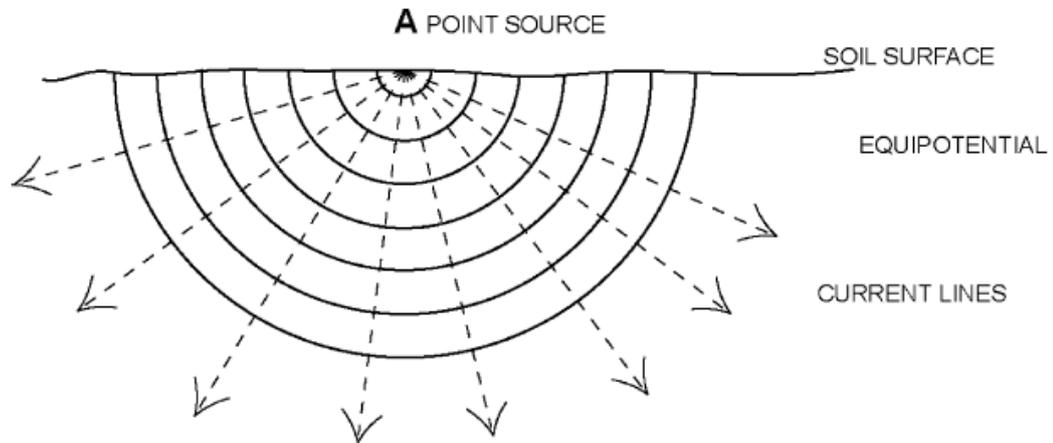


Fig. 1. Distribution of the current flow in a homogeneous soil.

Measurement of electrical resistivity usually requires four electrodes: two electrodes called A and B that are used to inject the current (“current electrodes”), and two other electrodes called M and N that are used to record the resulting potential difference (“potential electrodes”). The potential difference  $\Delta V$  measured between the electrodes M and N is given by the equation :

$$\Delta V = \frac{\rho I}{2\pi} \left[ \frac{1}{AM} - \frac{1}{BM} - \frac{1}{AN} + \frac{1}{BN} \right]$$

Where AM, BM, AN and BN represent the geometrical distance between the electrodes A and M, B and M, A and N, and B and N, respectively. The electrical resistivity is then calculated using :

$$\rho = \left[ \frac{2\pi}{(1/AM) - (1/BM) - (1/AN) + (1/BN)} \right] \frac{\Delta V}{I}$$

$$= K \frac{\Delta V}{I} \quad (7)$$

where  $K$  is a geometrical coefficient that depends on the arrangement of the four electrodes A, B, M and N. The current electrodes A, B, and the potential electrodes M and N can be placed in the field at the soil surface, or in boreholes. As compared with the surface methods, the cross-borehole methods present the advantage of a high resolution with depth (Slater et al., 2000). This technique requires nevertheless intrusion into the studied bodies for the insertion of the electrodes. At the laboratory scale this technique can also be applied by placing the electrodes around the soil sample at various depths (Olsen et al., 1999).

### 3. Variation of electrical resistivity as a function of soil properties

The electrical resistivity is a function of a number of soil properties, including the nature of the solid constituents (particle size distribution, mineralogy), arrangement of voids (porosity, pore size distribution, connectivity), degree of water saturation (water content), electrical resistivity of

the fluid (solute concentration) and temperature. The air medium is an insulator (i.e. infinitively resistive), the water solution resistivity is a function of the ionic concentration, and the resistivity of the solid grains is related to the electrical charges density at the surface of the constituents. These parameters affect the electrical resistivity, but in different ways and to different extents. Electrical resistivity experiments have been performed to establish relationships between the electrical resistivity and each of these soil characteristics.

### 3.1. Nature and arrangement of solid constituents

In the context of soil mapping, electrical resistivity exhibits a large range of values from 1  $\Omega$  m for saline soil to several  $10^5$   $\Omega$  m for dry soil overlaying crystalline rocks (Table 1). The electrical conductivity is related to the particle size by the electrical charge density at the surface of the solid constituents. In clay soil, the electrical charges located at the surface of the clay particles lead to greater electrical conductivity than in coarse-textured soils because of the magnitude of the specific surface (Fukue et al., 1999). The electrical resistivity recorded by Giao et al. (2003) on 25 clay samples collected worldwide ranged from 1 to 12  $\Omega$  m. Lamotte et al. (1994) studied two cultivated sandy soils of very similar composition but significantly different electrical resistivity: in the sandy soil showing the greatest resistivity, few clay microaggregates were juxtaposed to the sand grains, while in the other soil the sand grains were coated and bridged by clay leading to a great continuity of the clay phase. The geometry of the pores (void distribution and form) determines the proportion of air and water according to the water potential. Robain et al. (1996) linked resistivity variations with the structure of the pedological materials, identifying that high and low resistivity values were related to macro- and mesoporosity, respectively. This enabled the detection of badger burrows and the study of their network as demonstrated by Butler et al. (1994). This also enabled the study of the crack opening at the centimetric scale by Samouelian et al. (2003).

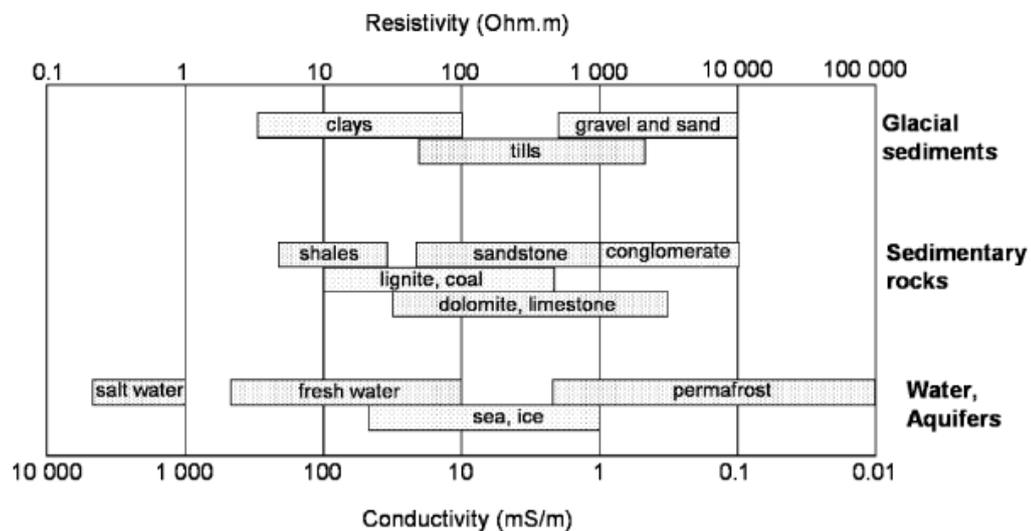


Table 1 : Typical ranges of electrical resistivities of earth materials (after modified Palacky, 1987)

The porosity can be obtained for the electrical property via the Archie's law, which for a saturated soil without clay is written as :

$$F = \frac{\rho}{\rho_w} = a\phi^{-m}$$

where the proportionality factor  $F$  is called the formation factor,  $a$  and  $m$  are constants related, respectively, to the coefficient of saturation and the cementation factor,  $r$  and  $r_w$  are, respectively, the resistivity of the formation and the resistivity of the pore-water,  $f$  is the porosity. The factor  $F$  depends then on the pore geometry. Knowing the pore-water resistivity and the  $a$  and  $m$  constants the porosity can be calculated from the resistivity value. The calculated porosity should be considered as an "apparent" porosity values because Archie's law assumes that all the void space is filled with water excluding the possibility of the gas presence.

### 3.2. Water content

Electrical current in soils is mainly electrolytic, i.e. based on the displacement of ions in pore-water, and is therefore greater with the presence of dissolved salts. Thus, electrical current in soils depends on the amount of water in the pores and on its quality. In most studies concerning the water content, the electrical conductivity of the solution is assumed to remain relatively constant to be neglected against its variation related to water content variation. Prior to field surveys, preliminary calibration of the volumetric water content related to the electrical resistivity is usually performed in the laboratory. Fig. 2 shows examples of laboratory calibration between the electrical resistivity and the volumetric water content (McCarter, 1984; Michot et al., 2000; Fukue et al., 1999). The electrical resistivity decreases when the water content increases. It can also be seen that for water content <15%, the electrical resistivity rapidly decreases with increasing water content. The relationship between the electrical resistivity (or its reciprocal, the conductivity, expressed in Siemens/m noted mho/m) and the water content has firstly been studied by authors mainly in the field of petroleum research. Archie (1942) proposed an empirical relationship based on laboratory measurements of clean sandstone samples. This relationship was a modified form of the previous equation (8), taking into account that the porosity can be filled by another medium as water, for example air or petroleum. The water saturation was expressed in function of the formation factor  $F$ , of the formation resistivity  $\rho$  and of the water resistivity  $\rho_w$  :

$$S^n = \frac{F\rho_w}{\rho}$$

by combining with the Eq. (8) we obtain :

$$S^n = \frac{a\rho_w}{\phi^m\rho}$$

where  $S$  is the saturation degree and  $n$  is a parameter related to the saturation degree. Eq. (10) was established to be valid for medium to coarse-grained soils. It assumes that the characteristic of the solid phase does not influence the electrical current conduction. Frohlich and Parke (1989) reported that the great practical success of Archie's law was related to the assumed validity of the determined constants on a large range of soils except for clayey soil. Indeed, this relationship was

successfully used for water content estimation in numerous studies (Binley et al., 2002; Zhou et al., 2001). An empirical linear relationship between the resistivity and the water content was proposed by Goyal et al. (1996) and Gupta and Hanks (1972) as follows :

$$\rho_{(z,t)} = a + b\theta_{(z,t)}$$

where  $a$  and  $b$  are empirical constants implicitly containing the soil and water characteristics (i.e. porosity, temperature, salinity) and assumed to be invariant with time. Temporal variations in the soil moisture profile are estimated by using electrical resistivity sounding data acquired at different times (Aaltonen, 2001; Michot et al., 2003).

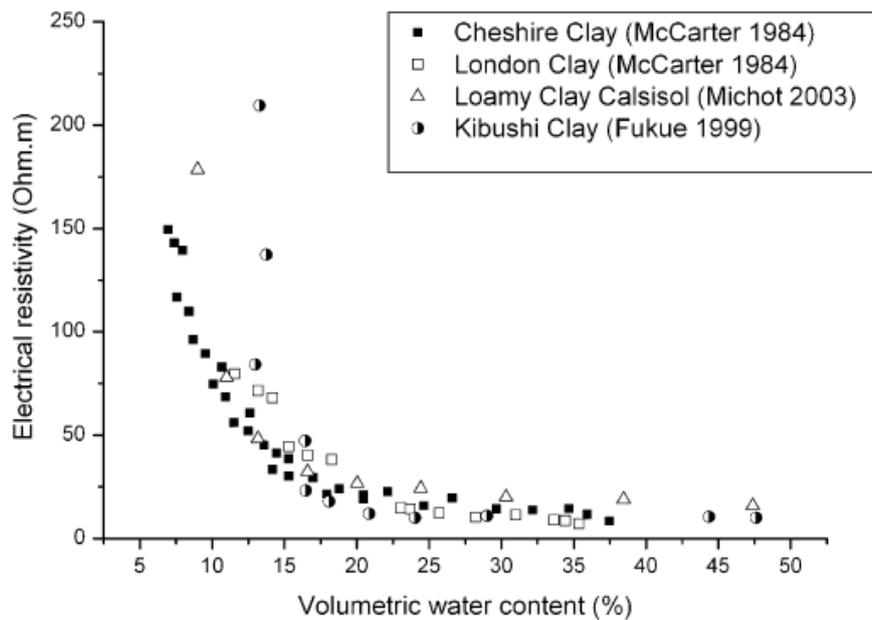


Fig. 2. Relationship between the volumetric water content and the electrical resistivity for different soil types (values issues from Fukue et al., 1999; Michot et al., 2003; McCarter, 1984).

$$\frac{1}{\rho} = \frac{1}{\rho_w}(a\theta^2 + b\theta) + \frac{1}{\rho_s}$$

where  $\rho_s$  and  $\rho_w$  represent the solid matrix and the pore-water resistivity, respectively,  $a$  and  $b$  are coefficients depending on the solid phase characteristics, related to the texture and mineralogy, and  $\theta$  is the volumetric water content ( $\text{cm}^3 \text{cm}^{-3}$ ). By using Eq. (12), Kalinski and Kelly (1993) predicted the volumetric water content with a standard error of 0.009 for water contents ranging from 0.20 to 0.50 in soil containing 20% clay.

### *3.3. Pore fluid composition*

As outlined above, the electrical conductivity is related to the mobility of the ions present in the fluid filling the pores. Conductivity depends on the concentration and the viscosity of the water (Scollar et al., 1990). The estimation of the water content by resistivity measurements requires a knowledge of the concentration of dissolved ions. Early studies dealing with the determination of the soil water content were confronted with the problem of estimating the soil salinity variation (Rhoades et al., 1977). Since salts have to be in an ionized form to conduct the current, the amount of water in soil governs the available paths of conduction. Shea and Luthin (1961) found a close linear relationship between electrical resistivity and salinity for a soil water content ranging from saturation to -3 kPa water potential. Thus, estimation of the soil salinity by electrical resistivity requires measurements made at the same water content. The soil salinity is usually measured at saturation, as this is considered as a standardized condition. Kalinski and Kelly (1993) estimated the volumetric water content using Eq. (12) and with pore solution resistivity ( $\rho_w$ ) of 1, 2 and 3 mmho/cm. They found that at a given water content, the electrical resistivity decreases when the water conductivity increases (Fig. 3). Moreover, the different ions present in the solution ( $H^+$ ,  $OH^-$ ,  $SO_4^{2-}$ ,  $Na^+$ ,  $Cl^-$ , . . .) do not affect the conductivity in the same way because of differences in ion mobility. This explains why soil solutions at the same concentration but having different ionic compositions, may have different electrical conductivities. This results in a large range of possible electrical conductivities because of concentration and ionic composition variations in different areas of the soil. This property was also used by Bernstone et al. (1998) to delineate landfill structure. The large resistivity contrast between salt water- and fresh water-saturated zones was used by several investigators to study salt water intrusion into coastal areas (Nowroozi et al., 1999; Acworth, 1999; Yaramanci, 2000). Van Dam and Meulenlamp (1967) considered the soil resistivity values of 40, 12 and 3  $\Omega$  m as representative of fresh, brackish and saline water, respectively.

### *3.4. Temperature*

Ion agitation increases with temperature when the viscosity of a fluid decreases. Thus, the electrical resistivity decreases when the temperature increases. Comparisons of electrical resistivity measurements require the expression of the electrical resistivity at a standardized temperature. By conducting laboratory experiments on 30 samples of saline and alkaline soils, Campbell et al. (1948) showed that conductivity increased by 2.02% per  $^{\circ}C$  between 15 and 35  $^{\circ}C$ .

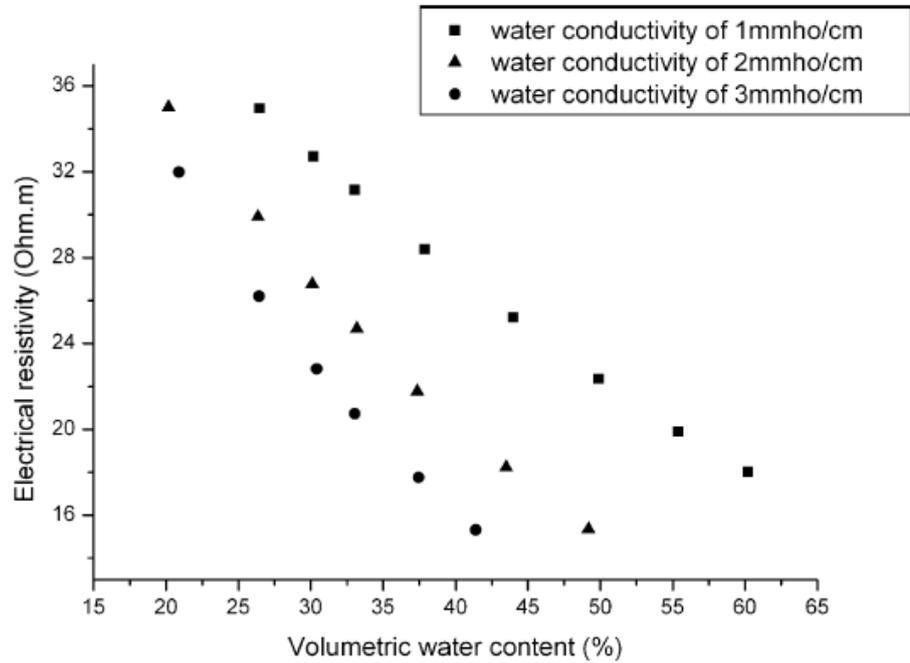


Fig. 3. Relationship between the volumetric water content and resistivity for different values of pore-water conductivity. (values issues from Kalinski and Kelly, 1993).

Corrections can be then calculated to express the electrical conductivity at the standardized temperature of 25 °C as follows :

$$\sigma_t = \sigma_{25^\circ\text{C}}[1 + \alpha(T - 25^\circ\text{C})]$$

where  $\sigma_t$  is the conductivity at the experiment temperature,  $\sigma_{25^\circ\text{C}}$  the conductivity at 25 °C, and  $\alpha$  is the correction factor equal to 2.02%.

Colman and Hendrix (1949) discussed the validity of the Campbell's equation using 13 soils showing a wide range of texture. These results are in agreement with Campbell and corresponded also to the references formula used to correct the temperature effect in the log interpretation chart of Schlumberg (1989). In soils, temperature variation during a year occurs at two temporal scales, day and season. In studies where the temperature effect is not corrected, an assumption is made that temperature remains stable mostly because measurements are done every day at the same time over a short period (Bottraud et al., 1984b). At the annual scale, it is not possible to avoid the effect of temperature on electrical field resistivity measurements. Usually, the greatest resistivity values are recorded from September to November (in the Northern hemisphere), while the smallest resistivity values are recorded from June to July. Aaltonen (2001) also reported that coarse-grained materials presented a wider range in seasonal resistivity variation than clayey soil. Thus, a knowledge of the seasonal variation of the temperature and its consequences on the electrical resistivity is essential to avoid misinterpretation of field measurements when comparing resistivity acquisition at the same place but on different dates.

## 4. One-, two- and three-dimensional surveys

### 4.1. One-dimensional survey

One-dimensional arrays using four-electrodes cells A, B, M, N are commonly used in the laboratory for electrical resistivity calibration (Rhoades and van Schilfgaarde, 1976a), and in the field for vertical electrical sounding (VES). The latter consists of electrical measurements where distances between the electrodes are successively increased. At each step, the depth and volume of soil investigated increase and the measurement displays the variation of soil resistivity with depth without taking into account the horizontal variation (Loke, 2001). For VES data interpretation, it is usually assumed that the subsurface consists of several horizontal layers (Pozdnyakova et al., 2001). Bottraud et al. (1984a) emphasized the interest in soil science to record information about the vertical discontinuity associated with different soil horizons. For more complex formations with lateral changes over short distances, 2D and 3D surveys display more realistic results.

### 4.2. Two-dimensional survey

Two-dimensional multi-electrode arrays provide a two-dimensional vertical picture of the sounding medium. The current and potential electrodes are maintained at a regular fixed distance from each other and are progressively moved along a line at the soil surface. At each step, one measurement is recorded. The set of all these measurements at this first inter electrode spacing gives a profile of resistivity values. The inter-electrode spacing is increased then by a factor  $n = 2$ , and a second measurement line is done. This process (increasing the factor  $n$ ) is repeated until the maximum spacing between electrodes is reached. One can notice that the larger the  $n$ -values, the greater the depths of investigation (Fig. 4). As the distribution of the current also depends on the resistivity contrasts of the medium, the depth of investigation deduced from the spacing is called the “pseudo-depth”. The data are then arranged in a 2D “pseudo-section” plot that gives a simultaneous display of both horizontal and vertical variations in resistivity (Edwards, 1977). The conventional graphic representation, introduced by Hallof (1957), puts each measured value at the intersection of two  $45^\circ$  lines through the centres of the quadripole. Each horizontal line is then associated with a specific value of  $n$ , and gives a pseudo-depth of investigation.

Depending on the respective position of the potential electrodes and on the current electrodes, several array configurations can be defined : Wenner, Wenner–Schlumberger, dipole–dipole pole–pole or pole–dipole arrays are the most commonly used (Table 2). The latter configuration is an asymmetrical array, in which two directions (forward and reversed) are considered. Depending on the array configuration, the geometrical factor  $K$  differs; Seaton and Burbey (2002) reported that the array configuration has a substantial influence on the resolution, sensitivity and depth of investigation. Table 3 summarizes for different 2D array configurations and compares the following characteristics for all the arrays: (i) the sensitivity of the array to horizontal and vertical heterogeneities, (ii) the depth of investigation, (iii) the horizontal data coverage and (iv) the signal strength (Loke, 2001; Griffiths and Barker, 1993).

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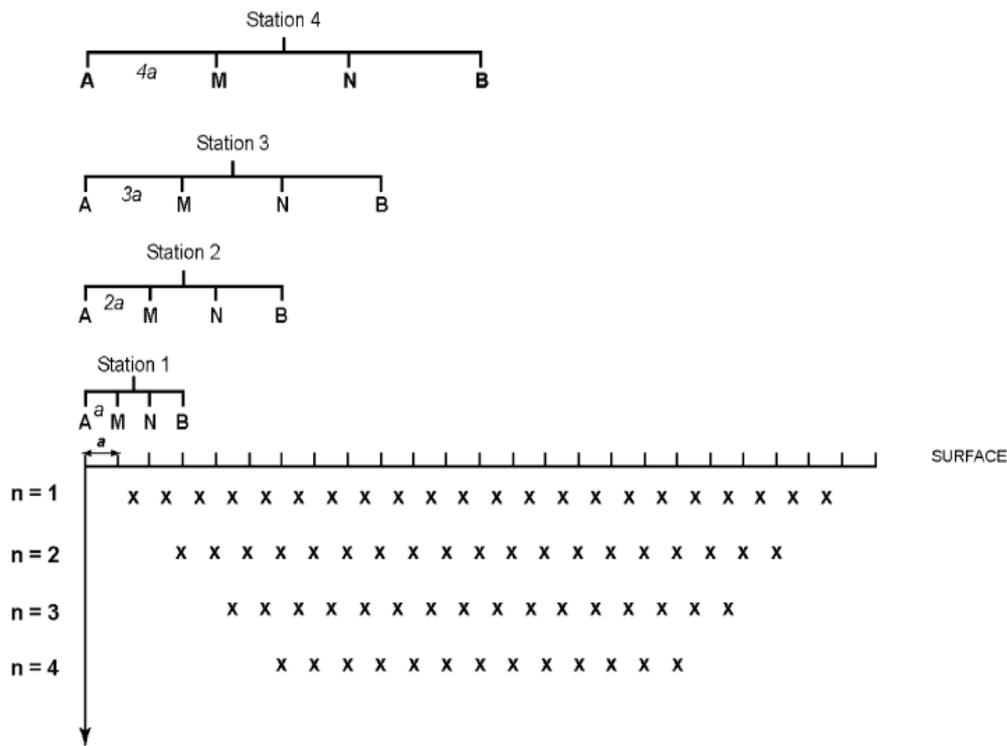


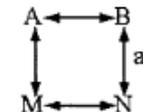
Fig. 4. Establishment of a 2D electrical resistivity pseudo-section.

Table 3 summarizes for different 2D array configurations and compares the following characteristics for all the arrays: (i) the sensitivity of the array to horizontal and vertical heterogeneities, (ii) the depth of investigation, (iii) the horizontal data coverage and (iv) the signal strength (Loke, 2001; Griffiths and Barker, 1993). The different orientations of heterogeneity can be vertical for heterogeneities such as dykes, cavities, preferential flow, or horizontal such as sedimentary layers. The depth of investigation is determined for homogeneous ground, but gives an a priori indication of the depth of investigation in heterogeneous ground. The horizontal data coverage is related to the electrode array configuration. The signal strength is related to the joint signal-response of the measurement. It is inversely proportional to the geometric factor  $K$  and is an important factor if the survey is carried out in areas with high background noise. All the different array types have specific advantages and limitations. The choice of the array configuration then depends on the type of heterogeneity to be mapped and also on the background noise level; the characteristics of an array have to be taken into account. Hesse et al. (1986) emphasized that in specific cases the use of multiple configurations can improve the chances of reading different features of the subsurface and leads to a better interpretation.

#### 4.3. Three-dimensional survey

Two methods can be used to obtain a three dimensional electrical resistivity acquisition. The first method consists of building a three dimensional electrical picture by the reconstruction of a two-dimensional network of parallel pseudo-sections (al Hagrey et al., 1999; Chambers et al., 1999,

2002; Oglivy et al., 1999; Zhou et al., 2001). An accurate three-dimensional electrical picture is thus recorded if electrical anomalies are preferentially oriented and if the in-line measurement electrodes are perpendicular to the orientation of the anomalies. Chambers et al. (2002) outlined that electrical surveys using co-linear arrays carried out on sites with heterogeneous subsurface conditions should include measurements using electrode configurations oriented in at least two directions perpendicular to one another. Zhou et al. (2001) mapped 3D electrical resistivity with several 2D parallel pseudo-sections along the X, Y, XY and -XY directions, enabling the study of an anisotropic media. The second method consists of using a square array of four electrodes (Table 2). Habberjam and Watkins (1967) showed that such an array provides a measure of resistivity less orientation-dependent than that given by an in-line array. Senos Matias (2002) emphasizes too that the data were orientationally stable, so there was no need for prior knowledge of the electrical heterogeneity orientation. He recommended then to use this array over anisotropic ground.

		Electrodes array	K
2D	Wenner	A $\longleftrightarrow$ M $\longleftrightarrow$ N $\longleftrightarrow$ B a          a          a	$2\pi a$
	Wenner-Schlumberger	A $\longleftrightarrow$ M $\longleftrightarrow$ N $\longleftrightarrow$ B na          a          na	$\pi n(n+1)a$
	Dipole-Dipole	A $\longleftrightarrow$ B $\longleftrightarrow$ M $\longleftrightarrow$ N a          na          a	$\pi n(n+1)(n+2)a$
	Pole-Pole	B $\longleftrightarrow$ x $\longleftrightarrow$ A $\longleftrightarrow$ M $\longleftrightarrow$ x $\longleftrightarrow$ N x          a          x	$2\pi a$
	Pole-Dipole <i>Forward</i>	A $\longleftrightarrow$ M $\longleftrightarrow$ N na          a	$2\pi n(n+1)a$
	<i>Reversed</i>	M $\longleftrightarrow$ N $\longleftrightarrow$ A na          a	
3D	Square		$\frac{2\pi a}{2 - \sqrt{2}}$

A and B current electrodes, M and N potential electrodes  
A: spacing between electrodes used in a particular measurement  
n: spacing factor (integer values 1-6)  
x: distance to "infinite electrodes" in pole-pole array

Table 2 Example of 2D in-line electrodes array configuration, and 3D electrode device

	Wenner	Wenner–Schlumberger	Dipole–dipole	Pole–pole	Pole–dipole
Sensitivity of the array horizontal structures	++++	++	+	++	++
Sensitivity of the array vertical structures	+	++	++++	++	+
Depth of investigation	+	++	+++	++++	+++
Horizontal data coverage	+	++	+++	++++	+++
Signal Strength	++++	+++	+	++++	++

The labels are classified from (+) to (++++), equivalent at poor sensitivity to high sensitivity for the different array configurations.

Table 3 Characteristics of different 2D arrays configurations types

Detection and characterisation of cracking in concrete with the square array quadripole was investigated by Lataste et al. (2002). Apparent resistivity measurements and the establishment of an anisotropic index provided a useful indication of the presence and localization of cracks. Moreover, the presence of anisotropies resulting in a strong orientational effect, they were commonly detected by measuring the azimuthal resistivity (Pettinen et al., 1999; Busby, 2000). This method consists of rotating the electrical device from a fixed centre point and then measuring the resistivity as a function of the azimuth, i.e. the angle of rotation. The measured apparent resistivity changed with the orientation of the square array; indeed the square device is more reliable linked to anisotropy than more commonly used devices, i.e. Schlumberger or Wenner array (Lane et al., 1995). To avoid preferential directions in data recording that would be introduced by the linear electrode layout, Brunner et al. (1999) measured electrical resistivity by the way of a pole–pole electrode array arranged in concentric circles at the surface. Panissod et al. (1997b) underlined the need for research into new array geometries describing better the heterogeneities in order to optimize the apparent resistivity image before data interpretation. Through the combined results of theoretical studies and of field surveys, one of their conclusions pointed out the advantage of using a circular array configuration in the detection of heterogeneities in an archaeological context. Apparent anisotropy effects also provide useful information related to the direction of electrical anomalies for 3D electrical survey (Habberjam and Watkins, 1967; Senos Matias, 2002), or in-line arrays (Meheni et al., 1996).

## 5. Recent technological improvements

Electrical resistivity prospecting has recently been improved with respect to measurement time. The improvement of computer-controlled multi-channel resistivity-meters using multi-electrode arrays has led to an important development of electrical imaging. Switching units allow any combination of four electrodes to be connected to the resistivity-meter at any time. The electrical data measurement is then fully automated and acquisition can be rapid (Binley et al., 1996). These technological improvements in data acquisition allow efficient and complex arrays to be used, facilitating research that was inconceivable using manual methods in 2D and 3D electrical prospecting. Beside these 1D, 2D and 3D methods that allow the characterisation of electrical resistivity at different scales but on relatively few points, some specific experiments have been developed for surveys on large areas and with continuous measurements. Hesse et al. (1986) introduced a new electrical device called the ‘Résistivimètre Attelé à Enregistrement AUtomatique’ (‘RATEAU’ the English equivalent terms is harrow), where the improvement in data capture time permits large scale soil prospecting (from 1 to 10 ha in a day). The use of this device (Bourennane et al., 1998) allows the representation of the three-dimensional organisation of the near surface materials, after interpreting the electrical resistivity data. Christensen and Sorensen (1998) introduced a ‘‘Pulled Array Continuous Electrical Profiling’’ (PACEP) for spatially dense measurements over large areas. In this case, the electrode array is towed across

the field behind a small vehicle. Panissod et al. (1997a) used eight rolling electrodes corresponding to three depths of investigation (one pair of electrodes injects the current and three pairs record potential differences). This system named Multidepth Continuous Electrical Profiling (MUCEP) is shown in Fig. 5. It can be towed by a tractor or a quad, the positioning of the data points being done by a differential GPS system and with an on-board Doppler radar system. If we consider geophysical techniques in soil mapping, the application of a mobile electrical quadripole allows a considerable increase in the size of potentially surveyed areas, whilst maintaining a high spatial resolution.

## **6. Interpretation of electrical resistivity variation**

### *6.1. Apparent resistivity*

When the soil is uniform, the resistivity calculated using Eq. (5) is constant and independent of both the electrode spacing and surface location. When heterogeneities exist in the subsurface, the resistivity varies according to the relative position of the electrodes (Bertrand, 1967). The calculated resistivity is then known as the “apparent” resistivity. The latter enables a qualitative estimation of the electrical parameters of the medium (Meheni et al., 1996) but does not give the true resistivity and shapes of the anomalies. Each data acquisition corresponds to a volumetric measurement and constitutes qualitative information that is plotted against a pseudo-depth. Thus, the apparent resistivity values in a pseudo-section distort the real subsurface model picture and are closely dependent on the type of electrode array configuration (Andrews et al., 1995). For a given study, the resulting apparent resistivity distribution depends on the sensitivity of the electrode array. For example, a vertical resistant heterogeneity in a homogeneous medium shows a pseudo-section with a reversed V-shape of greater resistivity values enclosing lower resistivity values when recorded by a dipole–dipole array. Conversely, when the same sample is measured using a Wenner array, the pseudo-section displays a reversed V-shape with smaller resistivity values enclosing higher resistivity values at the location of the resistant body (Fig. 6). Thus, comparisons between apparent resistivity pseudo-sections are possible only for a given electrode array configuration. In order to restrict misinterpretation due to graphic representation, Lile et al. (1994) proposed to move the data points according to an a priori knowledge of the sounding body, while Ritz et al. (1999) proposed to use a filtering process. Nevertheless, comparisons between surveys done by different arrays are feasible if we are able to assign a “real” resistivity value to each point in the survey area. We can then calculate this “real” resistivity value by means of an inversion program.



Fig. 5. Multidepth Continuous Electrical Profiling (MUCEP).

### 6.2. *Interpreted resistivity*

Quantitative information on resistivity distribution requires a mathematical inversion of the apparent resistivity measurement into interpreted resistivity. The inversion consists of converting the volumetric apparent resistivity into inverted resistivity data that represents the resistivity at the effective depth of investigation, as opposed to the pseudo-depth.

#### 6.2.1. *Effective depth evaluation*

The differences of depth investigation with the different array configurations is related to the  $a$  and  $n$  values (Table 2). By increasing the  $a$  and  $n$  factors, the effective investigation depth is increased. It is commonly assumed that the depth of investigation corresponds to the distances between the A and M electrodes. So, the effective depth of investigation depends on the relative positions of both current and potential electrodes. However, the effective investigation depth is also related to subsurface layering, as a conductive surface layer will reduce the investigation depth. For these reasons, there are as many investigation depths as there are possible layered structures or as there are A–M spacing.

#### 6.2.2. *Inversion process*

In the case of vertical electrical sounding (one dimensional), the data are plotted on a graph (curves) expressing the variation of the apparent resistivity with the increasing electrode spacing. These curves represent, the variation of the resistivity with depth in a qualitative way. In relatively simple cases (succession of two or three horizontal strates), the estimation of the depth of the layer can be made by comparing the field data with theoretical apparent resistivity curves.

This technique provides a coarse interpretation in the absence of computer facilities or enables to derive an approximate model that is required as a starting point for one of the iterative modelling schemes. If the ground is not one-dimensional (succession of horizontal layers), the two or three-dimensional data treatment requires numerical modelling inversion procedure. Different numerical solutions have been developed, including finite-difference approximation (Dey and Morrison, 1979; Loke and Barker, 1996; Oldenburg and Li, 1994) and moment-method modelling, applied first to electromagnetic surveys (Tabbagh, 1985) and then to electrical prospecting (Dabas et al., 1994).

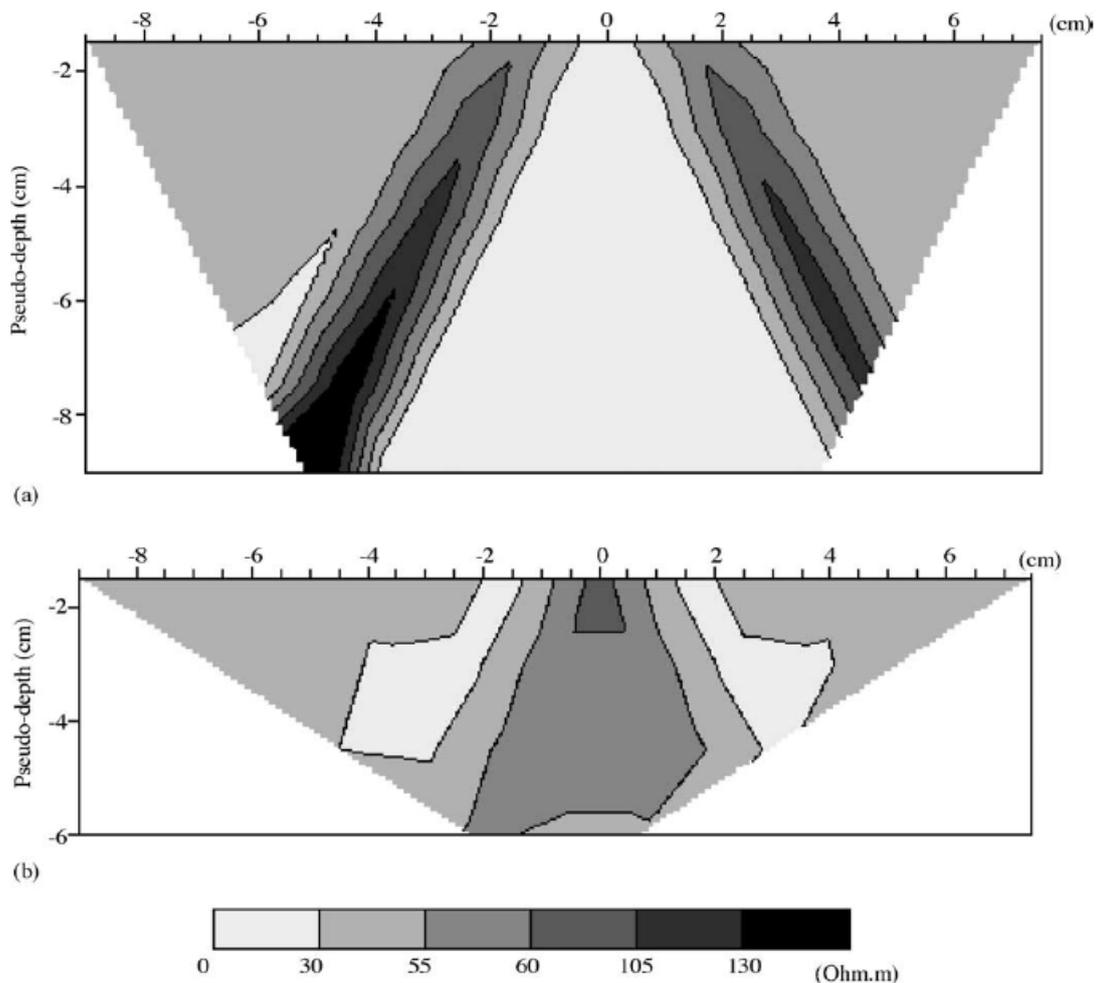


Fig. 6. Influence of the array device in electrical resistivity prospecting, simulation of a apparent resistivity pseudo-section over a vertical resistive structure of 4 mm width and 40 mm length located at  $x = 0$  in an homogeneous medium of  $40 \Omega \text{ m}$ , (a) dipole-dipole array, (b) Wenner array.

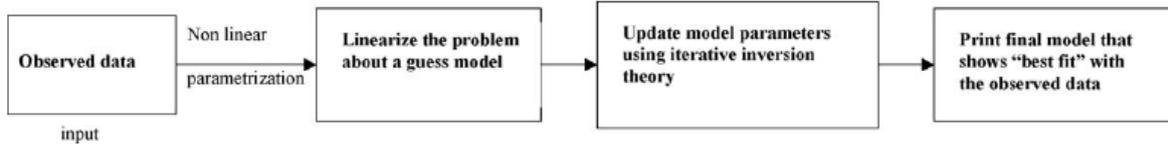


Fig. 7. Diagram of iterative inversion modeling scheme (after Sharma, 1997).

The inversion process is of non-linear type, and the resolution essentially starts with a guess model whose parameters are updated using a linearized iterative type adjustment procedure (Fig. 7). In order to be able to simulate complex distribution of electrical resistivity, the model of the medium must be discretized in many cells ( $j = 1$  to  $m$ ). The physical properties (resistivity) in each cell are fixed ( $m_j$ , with  $j = 1$  to  $m$ ). The electrical resistivity data (apparent resistivity) can be described as one mathematical function, which has relevant medium properties as variables :

$$d_i = F_i(m_1, m_2, \dots, m_m) + e_i$$

where  $d_i$  are the observed data ( $i = 1$  to  $n$ ),  $F_i$  the forward mapping operator which allows to calculate the model response for a given set of the earth model parameters  $m_j$  ( $j = 1$  to  $m$ ) at each of the data point  $I$  ( $i = 1$  to  $n$ ), and  $e_i$  are the errors in the observation data set. Solving the non-linear Eq. (14) requires at first to express this equation into a form that can be handled by methods used for linear inverse problems. A commonly procedure to “linearize” the problem is to start with an initial model with guessed values of the resistivity distribution (Sharma, 1997). The theoretical data  $c_i$  of the initial earth model with guessed resistivity parameters  $m_j$  is calculated at the data points ( $i = 1$  to  $n$ ).

$$c_i = F_i(m_1, m_2, \dots, m_m)$$

As expressed in the Eq. (16), the observed data  $d_i$  differ from the theoretical data  $c_i$  by the values  $e_i$ :

$$e_i = d_i - c_i$$

The objective is to find a set of update resistivities ( $m_j$ ), which minimizes the differences between the observed data and the computed data for all data points Eq. (16).

## 7. Advantages, limitations and new prospects for soil survey

### 7.1. Advantages

In the context of soil science, electrical resistivity permits the delineation of the main soil types and, when performed repeatedly over time, also provides information on soil functioning. The information collected is usually very useful for agronomists, soil scientists, waste management, civil and environmental engineers.

#### 7.1.1. Non-destructive mapping technique

The greatest advantage is that the prospecting disturbs neither the structure nor the functioning of the soil. Indeed, subsurface heterogeneities and transport properties can be determined non-invasively and in a dynamic manner. Moreover, one-, two- or three-dimensional prospecting can be done. Compared to other non-destructive techniques applied in soil science, like X-ray

tomography, Olsen et al. (1999) emphasized the possibility and advantage of monitoring resistivity variation both over long time and at decimetric scale. It can also be rapidly and easily carried out over several meters and thus applied to describe both horizontal and vertical variability of soil structure and properties at the scale of interest (Tabbagh et al., 2000). Park (1998) underlined that this method may show details of fluid migration that are unavailable with conventional hydrological monitoring technique. Preferred pathways corresponding to horizontal and vertical heterogeneities were also detected by this method and localised in the unsaturated soil zone (al Hagrey et al., 1999).

### 7.1.2. Temporal monitoring

This approach is very appealing for monitoring the temporal changes in soil water distribution. For instance, it permits the monitoring and resolving of relatively complex flow and transport mechanism. (Slater et al., 2002). Jackson et al. (2002) identified anomalies in a roadside embankment following repeated measurement of resistivity over an 18 months period, incorporating several wet and dry seasons. Even with resistivity data, temporal monitoring can be carried out. In temporal surveys, resistivity anomalies ( $\Delta\rho_t$ ) are computed by the Eq. (17) :

$$\Delta\rho_t = \frac{\rho_t - \rho_0}{\rho_0}$$

where  $\rho_0$ ,  $\rho_t$  are resistivities at the initial stage (0) and during the temporal monitoring (t), respectively. This index is easily calculated and provides useful information on temporal variations of electrical resistivity. The differences between successive resistivity measurements are more accurate than the absolute values of these measurements, because systematic errors are eliminated. Bottraud et al. (1984a,b) observed different patterns of water distribution related to variations in grape vine growth in a homogeneous sandy soil : they established a qualitative description of water transfer using the relative variation of apparent resistivity, during monitoring periods of 2.5 months. Samouelian et al. (2003) monitored artificial cracks as they deepened and observed an increasing apparent resistivity anomaly over time. This pattern is related to climatological variation affecting the ground watertable, precipitation, and temperature. In a 2 year long experiment, Binley et al. (2002) found a clear correlation between the net rainfall and the change of the electrical resistivity in the 0–0.82 m depth.

### 7.1.3. Various scales application

Electrical resistivity prospecting offers several advantages compared to other traditional soil prospecting (e.g. augering and excavation) and hydrological measurements (e.g. neutron probe, TDR, and tensiometry device). Classical techniques have restricted measurement scales that are usually incompatible with the subsurface variability. Electrical prospecting can be applied within a large range of scales by adjusting the inter-electrode spacing. Several different inter-electrode spacing could be applied at the same site. From the macroscopic to field scale, the measurements can be done without limitation and provide useful information. In this way, there is greater flexibility in the volume of soil that may be investigated. Jones (1995) selected this method for detecting the scaling properties of a fracture system at different resolutions (i.e. inter-electrode spacing), from 20 cm to 16 m. Depountis et al. (2001) reported a scale modelling experiment of a plume tracer by miniaturising the electrical device on a soil sample with dimensions of 75 cm x 22 cm x 12 cm, at a 3 cm resolution.

#### 7.1.4. Data acquisition facilities

The improvement of computer controlled multielectrodes arrays has led to an important development of electrical imaging. As a consequence of these improvements, electrical surveys can provide spatially dense and detailed measurements over large areas at low cost (Andrews et al., 1995; Frohlich et al., 1994). Direct modelling of the apparent resistivity over defined and synthetic subsurfaces would help in choosing the more appropriate electrical configuration in a specific context (Kampke, 1999; Mauriello et al., 1998; Seaton and Burbey, 2002).

#### 7.1.5. Large sensitivity of the measurement

As shown previously, the sensitivity of the electrical resistivity measurement is spread over a wide range depending on the soil physical properties. Choudhury et al. (2001) delineated the saline water contamination in soil at a district scale with electrodes 2 km apart. White (1988) used the small resistivity of a solution with a great salt content as a tracer because it guarantees a great electrical resistivity contrast. Frohlich et al. (1994) mapped the vulnerability of an aquifer by determining the extent and flow path of pollution by distinguishing the polluted from the clean water. Groundwater contamination has also been monitored (Abdelatif and Sulaiman, 2001; Gue´rin et al., 2002; Karlik and Kaya, 2001; Yoon and Park, 2001) in different regions. White (1994) identified and mapped the groundwater flow direction and seepage velocity using an electrical resistivity device.

#### 7.1.6. Numerical modelling advancement

The interpreted resistivity data also provide parameters for modelling water and solute transport. Frequently, groundwater flow models are restricted in their utilization because of the insufficient characterisation of the heterogeneous aquifer (Sandberg et al., 2002). Electrical resistivity measurements provide a powerful tool for detailed studies of vertical water movement in the unsaturated soil zone and therefore should help to assess the boundary conditions for infiltration modelling (Benderitter and Schott, 1999). Recent research in monitoring solute plumes during tracer tests, and attempts to estimate a quantitative assessment of the transport characteristics (breakthrough curves), gives a better understanding of advective-dispersive transport processes, and provides a promising tool to calibrate solute transport modelling (Binley et al., 1996; Kemna et al., 2002; Slater et al., 2000, 2002).

### 7.2. *Limitations*

As seen above, electrical resistivity survey can be affected by many different factors and they can act at the same time that leads the measurements more difficult to interpret.

#### 7.2.1. Contact between the soil and the electrodes

From the point of view of technical aspects, systematic errors due to poor electrode contact or noise averaging can be avoided by carrying out replicated and reciprocal (i.e. reversed the positive and negative current and potential electrodes) measurements, as recommended by several authors (Binley et al., 1996; Slater et al., 1997; Xu and Noel, 1993). Hesse et al. (1986) concluded that poor electrical contact was achieved in soil, under dry climatic conditions or on rocky ground. They proposed the use of a conductive liquid injected by a high-pressure jet. For example, the electrodes Cu/CuSO<sub>4</sub> permitted a wet point source electrical contact at the soil surface even for small electrode spacing at the laboratory scale (Samouelian et al., 2003) and also avoid electrode charge-up effects, i.e. polarization. This phenomenon is attributed to charge

build-up at the interface between the conducting metal of the electrode and the less conductive surrounding soil. As a consequence, one should avoid making a measurement of potential (electrodes M and N) with an electrode that has just been used to inject current (electrodes A and B) (Dahlin, 2000).

#### 7.2.2. Calibration

Field prospecting using electrical resistivity can be associated with laboratory studies (Shaaban and Shaaban, 2001). The purpose of these preliminary studies is to calibrate the resistivity within different soil units under controlled conditions. Field electrical measurements can be then used to estimate in situ properties for which laboratory calibrations have been made. This calibration approach is valid for all electrical surveys, i.e. pedological prospecting, water content monitoring and identifying specific anomalies such as in archaeology. Nevertheless, the calibration cannot usually be generalised to other soil types (Gupta and Hanks, 1972).

#### 7.2.3. Duration of the time of measurement

When soil function is studied, the technical time of electrical data acquisition (temporal resolution) with respect to the kinetics of the process studied is a crucial factor (Depountis et al., 2001; Slater et al., 2002). To maintain the advantages of non-destructive methods, one must choose the methods that allow a strictly reversible effect; that is the medium under study must be identical before and after the measurement (Tabbakh et al., 2000). This requirement is particularly important when monitoring transfer processes such as water or solute transport. The technical time of electrical resistivity acquisition has to be instantaneous compared to transport time. Michot et al. (2003) reported that deviation between the soil water content estimated by electrical resistivity and measured by a TDR probe was probably due to the time of electrical acquisition. Binley et al. (1996) used a high speed instrumentation in order to deal with the rapid process of water and solute transfer.

Robain et al. (2001) proposed combining a rapid electrical sounding technique called the “three-point method” with an assumption of the subsurface into only two layers. This model comprised three parameters: (i) a first resistive layer  $\rho_1$ , (ii) its thickness  $h$  corresponding to the unsaturated zone, and (iii) another second conductive layer  $\rho_2$  corresponding to the saturated zone. Thus, the model has to solve three equations containing three unknown quantities ( $\rho_1$ ,  $\rho_2$ ,  $h$ ) corresponding to three measurements at different inter-electrode spacings. This method is applicable when the two-layers soil model can be applied and when a priori values of thickness and the resistivity ratio are required to define an adequate inter-electrode spacing. It allows then a rapid field survey with only three independent measurements of resistivity instead of a complete vertical electrical sounding.

#### 7.2.4. Adequacy between heterogeneity and configuration

Using electrical resistivity, Bottraud et al. (1984a,b) attempted to map a soil with an increasing clay content with depth and a random stone distribution. They only detected a major discontinuity located between 5 and 10 m; the electrical resistivity contrast was not great enough to reveal smaller heterogeneities, and their electrical signature was very noisy. In such a case, a specific survey should consider the scale of soil heterogeneity. Because this scale may often be different down a soil profile, an adaptation of the electrode array configuration is one solution, if one keeps in mind that the sensitivity of the measurement decreases as the depth of investigation increases. Moreover, the quality of the data is strongly dependent on the contrast of the physical

properties between the juxtaposed soil volumes (Demanet et al., 2001). High data density is of fundamental importance for the resolution and interpretation of complex heterogeneities (Dahlin and Loke, 1998). Too sparse data can cause important features to be overlooked, or can create false subsurface images. Xu and Noel (1993) emphasized that Wenner pseudo-section surveys cannot resolve complex heterogeneities. Thus, acquisition of extra independent data may be important for resolving complex 2D subsurface formation. In practice, the final data density should be a balance between the total electrical time acquisition and accuracy needed. The usefulness of electrical surveys is determined by the reliability of data, the density of the measurements, and the quality of the interpretation (Christensen and Sorensen, 1998).

#### 7.2.5. Inversion process and non-uniqueness of the solution

The non-uniqueness of the solution in the inversion scheme can lead to ambiguity or misinterpretation. Thus, a basic knowledge of the medium under study is recommended before inversion. Olayinka and Yaramanci (2000) suggested that the inversion procedure cannot be very precise because: (i) the solution from the inverse problem is often not unique, (ii) the models determine a continuous function of the space variables, whereas the amount of data is usually finite in real experiments, (iii) when the solution is unique exact inversion techniques are often unstable and (iv) real data are often contaminated by noise. Inversion can only provide an approximate guide to the true geometry of the subsurface heterogeneities. Christensen and Sorensen (1998), Dannowski and Yaramanci (1999), Garambois et al. (2002) and Slater and Reeve (2002) recommend that to reduce the inversion ambiguity and so improve the quality of the interpretation, data from several different prospecting methods should be integrated.

#### 7.3. Recent improvement and future prospect

As presented previously, electrical resistivity surveys can provide information about soil physical properties. Recent technological improvements in electrical devices allow rapid resolution both in space and time.

Several depths (three for example, with the “MUCEP”) can be prospected over a wide surface area to characterize the spatial variability of soils at the metric scale. Observations obtained by electrical resistivity provide a useful contribution compared to conventional soil mapping by drilling and sampling. It allows the surveyor to distinguish and delineate different soil types. Likewise Dabas et al. (1989) identified areas of poor crop growth in a field by precise electrical soil mapping. Electrical prospecting allowed them to distinguish the sand layer in a limestone formation and to estimate the sand layer thickness. Poor crop growth was directly linked to the shallowest sand layer 0.25 m thickness compared to 1 m thickness in the other parts of the field. Since electrical measurements are also sensitive to water content and the ionic concentration, spatial and temporal variability of plant nutriment can also be estimated. This information can be useful in precision farming for the management of irrigated to both optimise production and to minimise nitrate leaching (Corwin and Lesch, 2003). It also provides an efficient, economical, and rapid way of (i) identifying areas of excessive soil compaction due to wheel traffic, (ii) identifying soil horizon thickness and bedrock depth and (iii) assessing soil hydrological properties over vast agricultural production regions (Freeland et al., 1998). Electrical variability is closely linked with the crop yield at the field scale (Zanolin, 2003). Besson et al. (2004) outlined the link between the soil structure and electrical resistivity values in an agricultural context. A two-dimensional electrical imaging (Fig. 8) was performed in the field perpendicularly to the traffic direction. It shows the efficiency of the electrical measurements to discriminate extreme structural states of the soil (i.e. compacted or porous). The wheel track located in

between the 0.40–0.95 m and 2.3–2.75 m in the first 0.15 m and the plough pan located in the 0.25–0.30 m were distinguished by their low value of resistivity and corresponded to compacted zones ( $1.53 \text{ Mg m}^{-3}$ ), as compared to the more porous zones ( $1.39 \text{ Mg m}^{-3}$ ). Below 0.30 m, the low resistivity values corresponded of the B horizon.

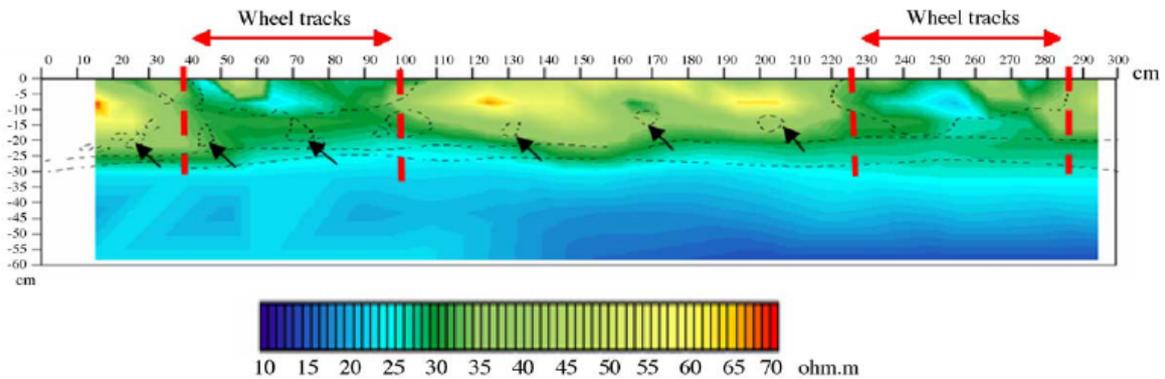


Fig. 8. Two-dimensional electrical resistivity image perpendicular in the traffic direction (after Besson et al., 2004).

Electrical resistivity devices and inversion softwares are currently available on the market. Nevertheless, the primary results obtained after electrical surveys give only indirect information. The method needs primary calibration at the laboratory and interpretations require a minimum knowledge of the medium under study. The use of this method as a routine operation for farmers would appear to be unlikely. The duration of data acquisition is shortened with newly available dataloggers and multiplexers. Such equipment is necessary to perform 3D measurements with a high spatial resolution or to apply electrical resistivity measurements to transfer processes. Landscape solute transport modelling can serve as a crucial component of precision agriculture by providing feedback concerning solute loading to ground-water or to drainage tile systems (Corwin and Lesch, 2003). Christensen and Sorensen (1998) estimated the vulnerability of an aquifer to leakage of a polluting substance from the soil surface, such as infiltration of nitrates from excess fertilisers. These studies constitute a useful step in the hydrological understanding of chemical pollutant transfers such as heavy chemical products released from agricultural and industrial practices. Indeed, salinity limits water uptake by plants by reducing the osmotic potential and thus the available water content for the plant growth. Real-time measurements obtained by electrical resistivity surveys may assist in our understanding of the transport phenomena of water solution and his spatial distribution. Model predictions may help identify management actions that will prevent the occurrence of detrimental conditions.

## 8. Conclusion

Electrical resistivity prospecting is a very attractive method for soil characterisation. Contrary to classical soil science measurements and observations which perturb the soil by random or by regular drilling and sampling, electrical resistivity is non-destructive and can provide continuous measurements over a large range of scales. In this way, temporal variables such as water and plant nutrient, depending on the internal soil structure, are monitored and quantified without altering the soil structure. The applications are numerous: (i) determination of soil horization

and specific heterogeneities, (ii) follow-up of the transport phenomena, (iii) monitoring of solute plume contamination in a saline or waste context. It enables the improvement of our understanding of the soil structure and its functioning in varying fields such as agronomy, pedology, geology, archaeology and civil engineering. Concerning agronomy, applications are present in precision farming surveys. Nevertheless, electrical measurements do not give a direct access to soil characteristics that interest the agronomist. Preliminary laboratory calibration and qualitative or quantitative data (i.e. after inversion) interpretations have to be done to link the electrical measurements with the soil characteristics and function.

**Acknowledgement** : The authors gratefully thank Setphen Cattle for improving the English.

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