

## 5. 参考資料/入手資料リスト

No.	資料名	形態	発行年	発行機関
1	Organization structure of Ministry of Education	印刷物	2018年	Ministry of Education
2	Budget of Ministry of Education from 2015/16 to 2018 (6 Month)	印刷物	2018年	Ministry of Education
3	Organization structure of Department of TVET	電子コピー	2018年	Ministry of Education
4	Numbers of DG, DDG, Officers and Staffs of Department of TVET	電子コピー	2018年	Ministry of Education
5	Job description of Department of TVET and	印刷物	2018年	Ministry of Education
6	Budget of Department of TVET from 2015/16 to 2017/18 (6 Month)	電子コピー	2018年	Ministry of Education
7	Organization structure of GTI	電子コピー	2018年	Ministry of Education
8	Total expenses of GTI from 2015/16 to 2017/18	電子コピー	2018年	Ministry of Education
9	Expenses of each GTI from 2015/16 to 2017/18	電子コピー	2018年	Ministry of Education
10	Number of Graduates from GTI to TU	電子コピー	2018年	Ministry of Education
11	Number of Students at each GTI from 2013/14 to 2017/18	電子コピー	2018年	Ministry of Education
12	Numbers of Certified and Dropout students from 2013/14 to 2018/19	印刷物	2018年	Ministry of Education
13	Numbers of Enrollees and Certified Students from 2013/14 to 2017/18	印刷物	2018年	Ministry of Education
14	GTI Teachers' academic background	電子コピー	2018年	Ministry of Education
15	MOU's with Development Partners	印刷物	2018年	Ministry of Education
16	TVET schools under DTVET supported by International Organizations	印刷物	2018年	Ministry of Education
17	Time schedule of Teachers' Training by DTVET	印刷物	2018年	Ministry of Education
18	Technical assistances by Development partners	印刷物	2018年	Ministry of Education
19	Introduction of GTI Insein	電子コピー	2018年	GTI Insein
20	Introduction of GTI Shwe Pyi Thar	電子コピー	2018年	GTI Shwe Pyi Thar
21	Timetable of Mechanical Engineering Department of GTI Shwe Pyi Thar	印刷物	2018年	GTI Shwe Pyi Thar
22	Timetable of Electric Power Engineering Department of GTI Shwe Pyi Thar	印刷物	2018年	GTI Shwe Pyi Thar
23	Syllabuses of Department of English	印刷物	2018年	GTI Shwe Pyi Thar
24	Syllabuses of Department of Mathematics	印刷物	2018年	GTI Shwe Pyi Thar

No.	資料名	形態	発行年	発行機関
25	Syllabuses of Department of Chemistry	印刷物	2018 年	GTI Shwe Pyi Thar
26	Syllabuses of Department of Physics	印刷物	2018 年	GTI Shwe Pyi Thar
27	Syllabuses of Department of Electrical Power Engineering	印刷物	2018 年	GTI Shwe Pyi Thar
28	Syllabuses of Department of Mechanical Engineering	印刷物	2018 年	GTI Shwe Pyi Thar
29	Automotive Technology Semester I& II	印刷物	2018 年	Ministry of Education
30	Generation, Transmission and Distribution Semester I & II	印刷物	2018 年	Ministry of Education
31	Applied Electronics Semester I & II	印刷物	2018 年	Ministry of Education
32	Refrigeration and Air Conditioning Semester II	印刷物	2018 年	Ministry of Education
33	English Semester I & II	印刷物	2018 年	Ministry of Education
34	Engineering Science Semester I & II	印刷物	2018 年	Ministry of Education
35	Engineering Science (Engineering Physics) Semester I & II	印刷物	2018 年	Ministry of Education
36	Engineering Science (Engineering Chemistry) Semester I & II	印刷物	2018 年	Ministry of Education
37	Engineering Mathematics Semester I & II	印刷物	2018 年	Ministry of Education
38	Technical High School (Aung San) land registration	電子コピー	2018 年	YCDC
39	Water Pipeline Map of Insein (Hydrants map)	電子コピー	2018 年	YCDC
40	Chemical Analysis report on water sample	電子コピー	2018 年	YCDC
41	Myanmar National Building Code (MNBC)	電子コピー	2016 年	Ministry of Construction
42	Singapore Fire Code	電子コピー	2013 年	Singapore Civil Defense Force
43	Singapore Fire Code Handbook	電子コピー	2013 年	Singapore Civil Defense Force
44	Singapore and Myanmar Vocational Training Centre, Brochure	印刷物		SMVTI, MOE
45	SMVTI Block Layout Plan	印刷物	2018 年	SMVTI, MOE
46	Government Technical Institute (Shwe Pyi Thar )	電子コピー		GTI Shwe Pyi Thar
47	Government Technical Institute (Insein )	電子コピー		GTI Insein

## 6. その他資料

6-1. 地盤調査報告書（第一回）

6-2. 測量マップ

6-3. 地下水調査報告書

6-4. 環境管理計画

**REPORT  
ON  
  
GEOLOGICAL SURVEY  
FOR  
  
THE PROJECT FOR THE ESTABLISHMENT OF JAPAN-MYANMAR  
VOCATIONAL TRAINING INSTITUTE (AUNG SAN)  
LOWER MINGALARDON ROAD, SINGU WARD, AUNG SAN, INSEIN  
TOWNSHIP, YANGON REGION**

**MATSUDA CONSULTANTS INTERNATIONAL CO., LTD**

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**REPORT ON  
GEOLOGICAL SURVEY  
FOR  
THE ESTABLISHMENT OF JAPAN-MYANMAR VOCATIONAL TRAINING  
INSTITUTE (AUNG SAN)**

## **1.0 INTRODUCTION**

### **1.1 General**

Geotechnical investigations are needed to take up to ascertain the nature and properties of soils available at different locations of site at various depths, safe bearing capacity and settlements of soil. It's also carried out for designing a right type of foundation safely and economically, a designer must possess sufficient information about the physical properties and the arrangement of underlying materials. The field and laboratory investigations required to get this essential information is known as soil exploration.

Geo-Friends Engineering and Construction Co., Ltd was assigned to investigate (2) boreholes of project area in order to know physical and mechanical properties of soil in the proposed area which is situated in Lower Mingalardon Road, Singu Ward, Aung San, Insein township, Yangon Region.

### **1.2 Objective of Investigation**

The objectives of investigation includes,

- To identify the soil type, thickness and distribution of soil strata on project area.
- To evaluate the penetration resistant, permeability and fluctuation of groundwater table at project area.
- To recognize the physical and mechanical properties of soil layers.
- To evaluate the geotechnical design parameters from the results of field and laboratory test.
- To evaluate the appropriate foundation design and construction methods, such as excavation and dewatering, in terms of safe and economy.

### **1.3 Scope of work**

Site investigation, including boring work, field tests were carried out in accordance with ASTM D 1586-99 "Code of Practice for Site Investigations".

#### **➔ Soil Boring and Sampling**

Setting out the borehole points and site clearance works were carried out by the contractor at each borehole location to avoid delayed work prior to commencement of the site investigation work. The boreholes were drilled by rotary method. Drilling fluid was pumped down via the hollow drilling rods and cutting bit to wash out the soil remnants. The diameter of the borehole is 100mm.

Disturbed soil samples were taken by using spoon sampler (50mm OD & 35mm ID), driven by hammer with standard weight (63.5kg) from the constant height (760mm), kept inside two containers and the rest were kept inside plastic bags. Undisturbed soil samples were recovered by using thin-walled sampler (for soft soil).

#### **➔ Standard Penetration Test**

Standard penetration tests were carried out at intervals of 1.0 m up to 6.0 m thereafter followed by 1.5 m or instructed by the Client and the tests were carried out in accordance with ASTM D 1586-99. The SPT tests service for the following purposes:

- a) To determine the relative density or consistency of soils, and
- b) To recover disturbed samples for visual inspection

This empirical dynamic penetration test determines the resistance of soils to the penetration of a split barrel sampler (spoon sampler) of 50mm external diameter, driven by a 63.5 kg automatic drop hammer, allowing a free fall of 760mm. The penetration resistance, N value is the number of blows required to effect a 300mm penetration below the seating drive which was, for this project, taken as completed at 150mm penetration. Standard penetration tests were also conducted when the sub-soils were too stiff for undisturbed soil samples to be obtained.

The values of N obtained in this investigation are recorded in borehole logs attached in the Appendices of BOREHOLE LOGS.

#### **➔ Ground water investigation**

Groundwater level measurement was made in all boreholes during the soil investigation work, usually before commencing of work in the morning and after completion of work in the evening.

#### **➔ Installation and monitoring of water standpipe**

Water standpipes were installed in selected completed boreholes and at depths specified by the contractor. A slotted 55 mm diameter PVC pipe was installed at the installation depth followed by backfilling of borehole with clean sand.

The water standpipe was monitored for ten days. The water standpipe installation diagram and monitoring results are shown in the Appendices.

#### **➔ Description of laboratory testing**

The laboratory test program is essential to evaluate the soil properties as scheduled by the Client. All soil tests were carried out at Contractor's own laboratory. Generally, the following tests are carried out in accordance with ASTM Standard "Methods of Test for Civil Engineering Purposes" if samples are enough for laboratory testing.

➤ Physical property tests

1. Moisture Content, Bulk & Dry Density Determination(ASTM D2216)
2. Index Test (Atterberg Limits)(ASTM D4318)

**3. Particle Size Distribution (Sieve & Hydrometer)(ASTM C136 & D422)**

➤ **Strength and Compressibility Properties**

1. Direct Shear Test (UU) (ASTM D 3080)
2. Triaxial Test (UU) (ASTM D 2850)
3. Triaxial Test (CU) (ASTM D 4767)
4. Unconfined Compression Test (ASTM D 2166)
5. One Dimensional Consolidation Test (ASTM D 2435)

➔ **Submission of final report**

**1.4 Project Location**

Project area is located in Lower Mingalardon Road, Singu Ward, Aung San, Insein township, Yangon Region. The Google earth view is presented in Figure – 1.1.



Figure - 1.1 : Google earth view of the project area

**1.5 Project Duration**

Geo-Friends Engineering and Construction Co., Ltd conducted soil investigation work at the designated area of Lower Mingalardon Road, Singu Ward, Aung San, Insein township, Yangon Region. The field soil investigation work was started from 27<sup>th</sup> May 2018 and finished at 31<sup>st</sup> May 2018. All field and laboratory works were undertaken by Geo-friends Engineering & Construction Co. Ltd. under the supervision of our supervisors.

The executed detailed working schedule is illustrated in Table – 1.1.

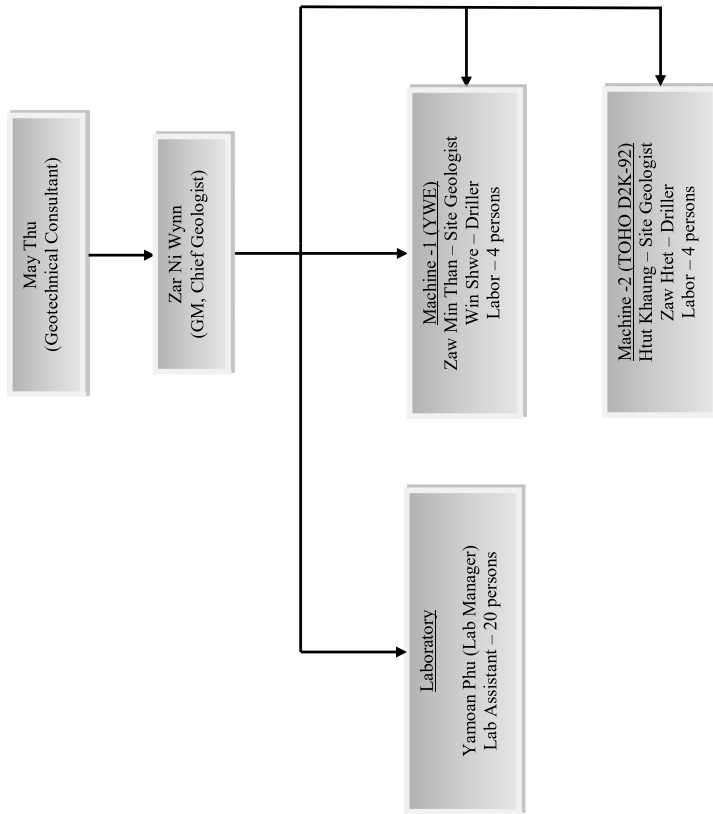


Photo - 1.1 : Mobilization of Equipment

Table - 1.1 : Actual Working Schedule of Geotechnical Investigation Works

Working schedule of Geological Survey for the Project for the Establishment of Japan-Myanmar Vocational Training Institute (Aung San)		Jun-18																								
		1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25
Sr.No	Description	Drilling Depth (m)																								
1	Setting the drilling stage	26	27	28	29	30	31	01	02	03	04	05	06	07	08	09	10	11	12	13	14	15	16	17	18	19
2	Soil Investigation at BH-1																									
3	Soil Investigation at BH-2																									
4	Demobilization to Store																									
5	Lab testing																									
6	Report Preparation																									
7	Submission of final report																									
		Total Boreholes = 2																								
		74.40																								

**1.6 Project Team Organization chart**



**1.7 Equipment Applied in the Project**

**1.7.1 Boring Equipment**

One number of boring equipment of TOHO D2K-92 and one number of YWE were applied in field investigation works to study general condition of soil layers under planned area for future construction. The photos of boring equipment were presented in following photo – 1.2.



Photo - 1.2 : Boring Equipments



### 1.7.2 Laboratory Instruments

The principal instruments applied for soil laboratory tests are as shown in the following table.

Table - 1.2 : Applied Laboratory Instruments

Instrument Name	Manufacturer and Type
1. Electric Balance	ELE, Made in UK
2. Atterberg's Limit Test Apparatus	ELE, Made in UK
3. One dimensional consolidation test machine	ELE, Made in UK
4. Direct shear test machine	ELE, Made in UK
5. Triaxial	ELE, Made in UK
6. UCS	NL, Made in Malaysia
7. Triaxial (CU/CD)	HUMBOLDT, Made in USA

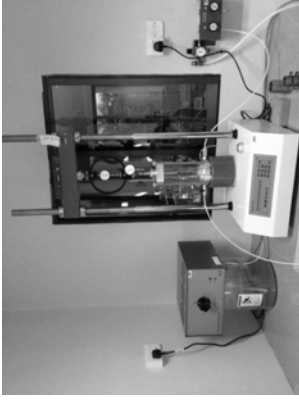


Photo - 1.3 : Triaxial Test

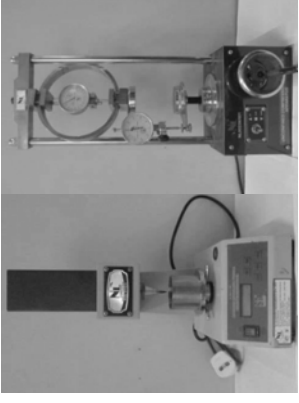


Photo - 1.4 : Liquid Limit Test and UCS

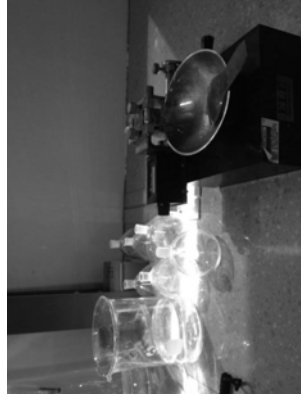


Photo - 1.5 : Liquid Limit Test



Photo - 1.6 : Direct Shear Test



Photo - 1.7 : Triaxial Test (CU)



Photo - 1.8 : Consolidation Test

## 2.0 TOPOGRAPHY

The overburden soil of the project site is mainly composed of clay, silt and sand.



Photo - 2.1 : Views of the project area

## 2.1 General Geology

### 2.1.1 Introduction

Yangon is geomorphically situated between Sittaung River in the east and Ayeyarwaddy River in the west and it is also the southern extensional rolling region of Bago Yoma. Yangon City is generally bounded by Hlaing River in the west, Yangon River in the south and Bago River in the east respectively. The approximate length of Yangon City is 20 miles in N-S and 15 miles in E-W which made aerial extent about 300 square miles.

### 2.1.2 Physiography of Yangon

The dominant physical feature of Yangon is the Yangon-Mingaladon Ridge, an anticlinal ridge which morphologically looks like as a homoclinal ridge. At the north, the elevation is greater than 150 feet and the regional slope is towards the south. The base of the Shwedagon Pagoda is more than 100 feet with respect to the sea level. The other small ridge called the Thingangyun Ridge may be considered as the northern continuation of the Thanlyin-Kyauktan Ridge.

Yangon area can be topographically recognized as; (i) *rolling and hilly area* in the central part with the ridges; (ii) *flat rolling area* especially within the Mingaladone and along the eastern and western limbs of Thanlyin-Kyauktan Ridge; (iii) *lake area* occupied by Hlawga Lake, Inya Lake and Kandawgyi Lake which lies nearly N-S that parallel to the trend of ridges and regional geological structures; (iv) *swampy* area occupied at Dala and Thilawa in the south, at the vicinity of Panhlaing River in the southwest, around PazundaungChaung in the southeast and tidal influent area of Nagmoeyeik Chaung; and (v) *alluvial area* covered at west of Yangon-Mingaladon Ridge and Hlaing River, and at some parts in Insein, Gyogon, Kamayut, Kyeemyindine, Alone.

The most common drainage pattern through Yangon area is dendritic and the modification and variation of the pattern sometimes due to structural controls. All drainage texture as

coarse, medium and fine are readily observed in Yangon area. Fine texture drainage can also be expected when the precipitation in an area is intense and the water table is not near the ground surface. This reveals that the amount or intensity of precipitation and the relative elevation of the water table also affect the drainage texture. The drainage system in Yangon area can be observed as: (i) *headwater channels or gullies* developed especially in the area between Mingaladon and Aungsanmyo and at the north of Hlawga Lake but poorly seen on the eastern flank of the Yangon-Mingaladon Ridge; (ii) *tributaries* observed as about 16 streams within Yangon area; and (iii) *the major streams* such as Hlaing-Yangon River, Panhlaing River and Pazundaung-NgamoeyeikChaung.

### 2.1.3 Regional Geological Setting

Yangon and its surrounding region include ridges and deltaic low lands and also extensional rolling region of Bago Yoma anticlinorium. The area is located in a N-S trending sedimentary basin containing a thick Tertiary and Quaternary deposits. Tertiary deposits belong to the Hlawga shale of lower Pegu Group, Thadugan sandstone (lower) and Besapet alternation (upper) of upper Pegu Group, and Arzanigon sandrock (lower) and Danyingon clay (upper) of Irrawaddy Formation. Quaternary sediments of older and younger alluvium deposits are widely distributed throughout the Yangon area.

Table - 2.1 : Regional Lithostratigraphic Units of Yangon Area and Bago Yoma

System	Series	Yangon Area	Bago Yoma
Quaternary	Recent	Young Alluvium	Alluvium
	Pleistocene	Valley-fill Deposit	
Tertiary	Pliocene	Danyingon Clay	Irrawaddy Formation
		Arzanigon Sandrock	
	Miocene	Besapet Alternation	Obogon Formation
		Thadugan Sandstone	Kyaukk Formation
Oligocene (?)	Hlawga Shale	?	

The regional dip is toward the east having a low to moderate dip angle and the western dip slope is very narrow often covered by the younger alluvium. Yangon area is complicated by numerous folding resulting in a characteristic an echelon folding system of rocks of Bago Yoma, regarded as Hlawga anticline, Yangon-Mingaladon anticline, Thingangyun-Thanlyinancline and Twante anticline. These folded structures were strongly cut across by numerous faults trending nearly E-W to ENE-WSW.

The Yangon-Mingaladon ridge is a long narrow anticlinal ridge of an anticlinal fold plunging definitely towards the north and the physiographic evidence of the nose of the anticline is observed at Danyingon. This Yangon-Mingaladon anticline is an asymmetrical rather than a symmetrical one. At its northern extremity, this anticlinal ridge extends toward north as the western flank of a regional syncline trending west of Hlegu, through Htaukkyant. The anticlinal structure of the ridge becomes distinct and identifiable at Danyingon and at the west of Mingaladon airport.

The Sagaing Fault, a recently active dextral strike-slip fault is regional recognized as the possible marker for the Neogene structural development feature. It is a recently active dextral strike-slip fault and actually cross cutting the eastern central basin. The largest fault in Yangon area named as Mingaladon Fault is observed as a lineament in the paddy field east of Mingaladon airport. This fault is considered as a normal fault and the fault plane is estimated to be dipping in a southeast to east direction. Another distinct fault namely Danyingon Fault is recognized as a lithologic boundary between rocks of Upper Pegu Group and Irrawaddy Formation within the Hlawga anticline. This fault can be easily observed in the field due to the juxtaposition of two different rock units especially around Hlawga Lake.

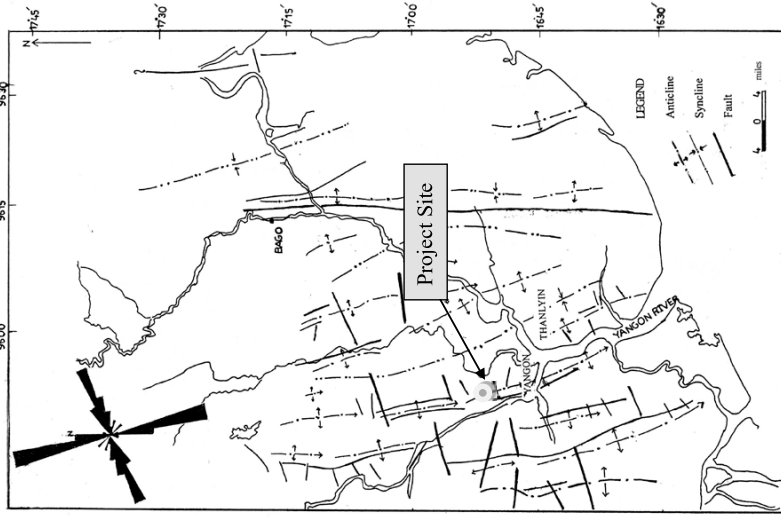


Figure - 2.1 : Regional structural map of the Yangon area (MOGE)

According to Win Naing (1972), the general succession of rocks underlying at the Yangon area is as follows:

Formation	Age
Younger Alluvium	Recent
Valley Filled Deposits	Pleistocene
	Unconformity
Danyingon Clay	Pliocene
Arzanigone Sandrocks	Pliocene
	- Irrawaddy Formation
	- Quaternary
	- Quaternary

## 2.2 Sediments of the Tertiary Age

### 2.2.1 Pegu Group

Pegu Group includes Besapet alternation, Thardugan sandstone and Hlawga shale.

- **Besapet Alternation:** It consists of shale and thinly laminated sandstones, which are exposed in the vicinity of Besapet Lake. They are characterized by bluish grey to greenish grey, bedded to non-bedded, silty shale with very thin parting of micaceous sandstones, and yellowish brown, fine to medium grained, soft micaceous and carbonaceous sandstones with calcareous concretions in places. A few fossiliferous beds are noted in this formation.
- **Thardugan Sandstone:** It consists of bluish grey to brownish grey, fine to medium grained micaceous and argillaceous sandstones with ferruginous band along the bedding planes. These sandstone sometimes contain nodules of silty and pot hole are common in calcareous sandstone due to leaching away of nodules from these sandstones. Poorly observed fossils are found in this formation.
- **Hlawga Shale:** Shales and laminated clays of this formation are considered to be the core of the Hlawga anticline.

### 2.2.2 Irrawaddy Formation

This formation includes two lithostratigraphic units. They are Danyingon clays and Arzarnigon sand rocks.

- **Danyingone Clay:** It consists of blue clays, siltstones, with interbedded sand rocks. The clays bands show current bedding, well recognized in Htanbington section. Fossil woods have been found. It clays layers are thinly laminated. Remarkable lateritization occurs on these sediments.
- **Arzarnigon Sandrocks:** These formations are loose to very dense and generally unconsolidate to consolidate. These sand rocks may appear clear or contain admixture of silt, clay and fine gravel at various percentage. These sand rocks are slightly pervious to pervious. Bluish grey colored, thinly laminated and thick bedded clay with ferrugeneous thin band and interbedded with buff to brownish colored soft and very fine grained sand rocks are well exposed.

### 2.2.3 Sediments of the Quaternary Age

- **Valley-fill deposits:** It consists of a thick sequence of loose, highly pervious, interbedded sand and fine to very coarse gravels.
- **Younger Alluvium:** This formation was deposited in recent time and thus, it blanketed the areas which are affected by tidal action. It is estimated to be about 20 m to 70 m with variation according to depositional environments. This formation consists essentially of yellowish grey, bluish grey, brownish grey silts and clays.

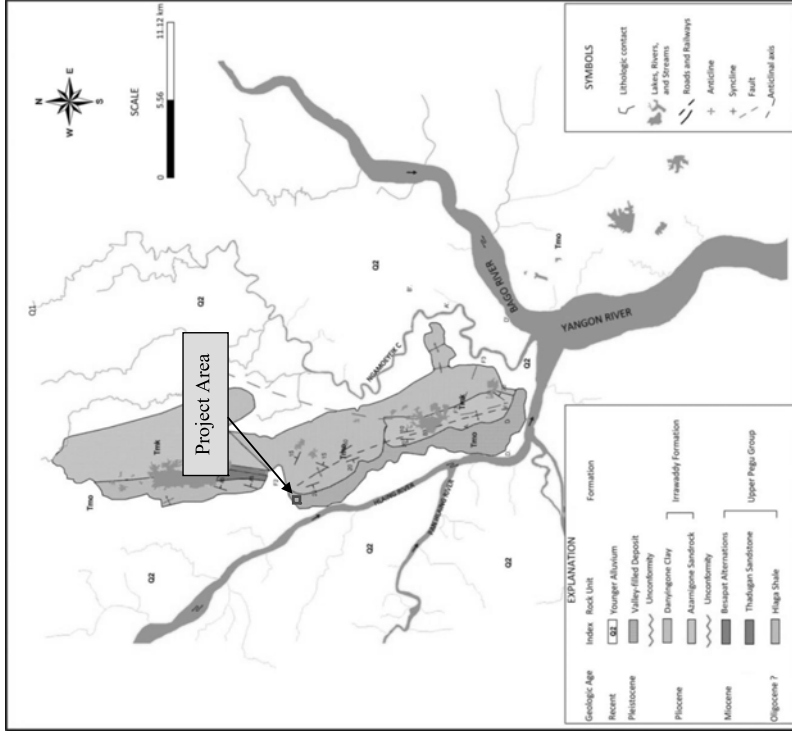


Figure - 2.2 : Geological Map of the Greater Yangon (Win Naing, 1972)

### 2.3 Location of Boring Points

The locations, levels and coordinates of investigation points of boring points were designated by the client. The numbers of borehole and termination depth are also decided by the client side. The locations of boreholes are presented in Figure - 2.3.

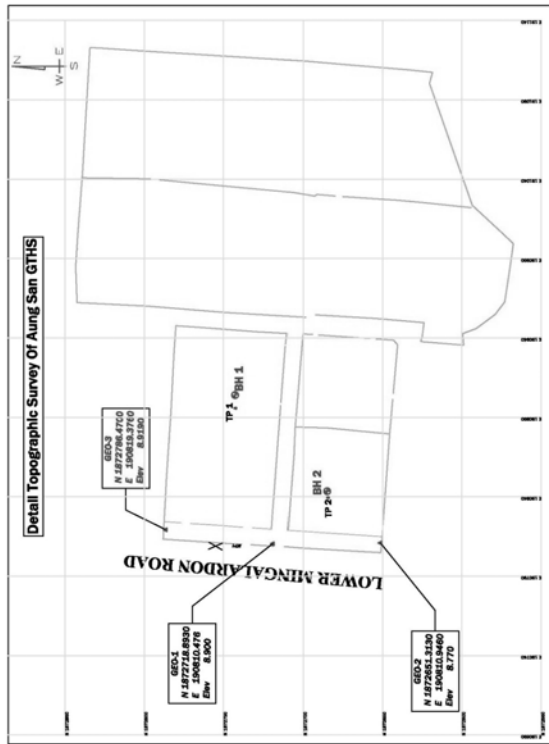


Figure - 2.3 : Plan Map of Investigation Boring Points

Table - 2.2 : Coordinates of Borehole Points

Borehole	Coordinates		Elevation (m)
	E	N	
BH-1	190904.100	1872742.629	8.639
BH-2	190843.559	1872684.984	8.512

### 3.0 LABORATORY TEST RESULTS

There were two numbers of investigation boring points at project site. Some selected numbers of disturbed and undisturbed samples were sent to office's laboratory to test physical and mechanical properties of soil in consulting with expert's discretion. **The detailed laboratory results are expressed in Appendices.** The entire tests were carried out in accordance with the ASTM standard.

The physical properties tests include the following items.

- Natural Moisture Content Test
- Specific Gravity Test
- Particle Size Analysis Test
  - Grain Size Distribution Test
  - Hydrometer Test
- Atterberg's Limits Test
  - Liquid Limit Test
  - Plastic Limit Test

The mechanical properties tests includes-

- Triaxial Test (UU)
- Direct Shear
- One dimensional consolidation test

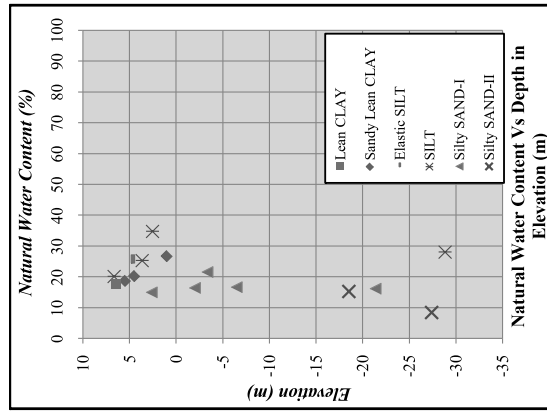
### 3.1 Property of Soil

Physical property tests and mechanical property tests were done for investigation. The detailed laboratory test results are illustrated in Appendix.

#### 3.1.1 Natural Moisture Content Test

Moisture content, bulk & dry density of representative soil samples were determined for the identification, classification and correlation of the soil types encountered.

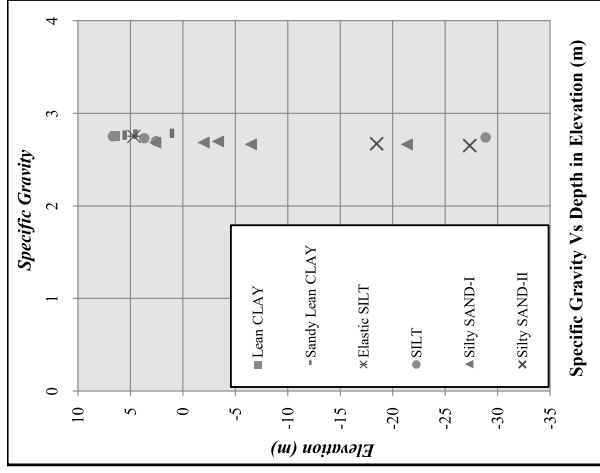
Natural moisture content tests are carried out on soil samples for required different soil layers at office's laboratory in accordance with ASTM D2216 and the variation of water content with depth in elevation with reference to project bench mark can be seen in Graph – 3.1.



Graph - 3.1 : Nature Water Content vs. Depth in Elevation (m)

#### 3.1.2 Specific Gravity Test

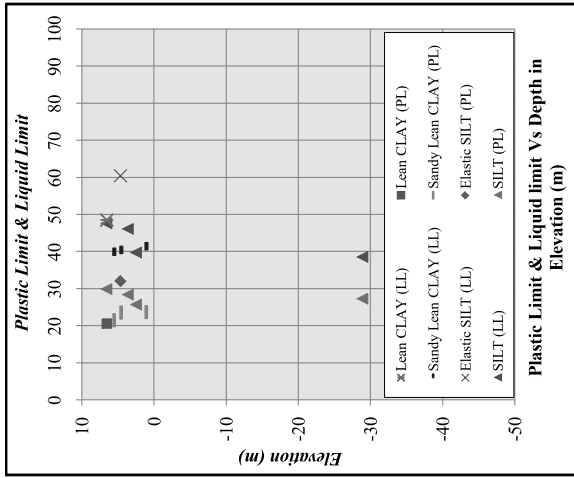
The specific gravity tests in this project were carried out in accordance with ASTM C 127 at office's laboratory. The relationship between specific gravity and depth in elevation of each soil layer is shown in Graph-3.2.



Graph - 3.2 : Specific Gravity vs. Depth in Elevation (m)

### 3.1.3 Atterberg's Limit Test

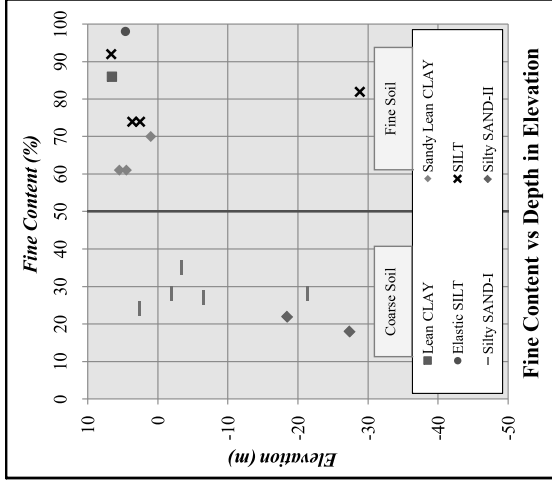
The Atterberg's Limit tests can be performed from SPT disturbed and undisturbed samples by ASTM D 4318 at office's laboratory. Graph-3.3 illustrates the Plastic Limit and Liquid Limit of each soil layer versus depth in elevation.



Graph - 3.3 : Plastic Limit & Liquid limit vs. Depth in Elevation (m)

### 3.1.4 Grain Size Analysis Test

After completion of Atterberg's Limit Test, grain size distribution tests were done by ASTM C 136 and D422. Graph-3.4 is illustrated the grain size distribution of each soil layer versus depth in elevation.



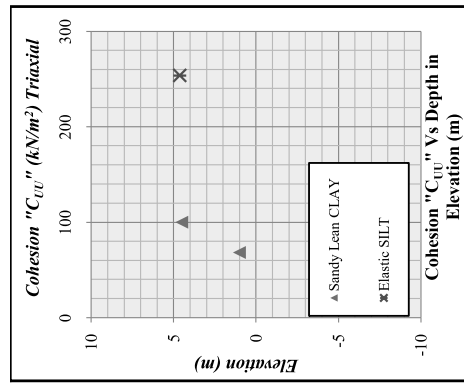
Graph - 3.4 : Fine Content vs. Depth in Elevation (m)

### 3.2 Mechanical Properties of Soil

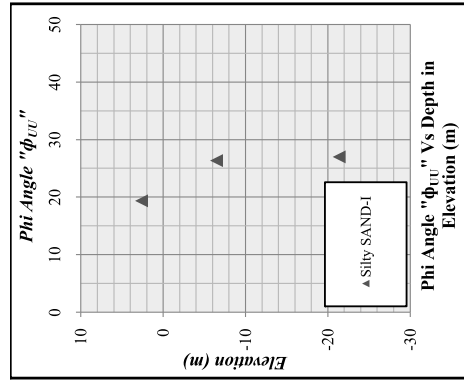
In order to obtain the mechanical properties for engineering analysis, triaxial tests (UU), direct shear tests and one dimensional consolidation tests were performed with undisturbed and disturbed samples at office laboratory.

#### 3.2.1 Triaxial test (UU) and Direct Shear Test

Triaxial test (UU) and Direct shear tests were performed with undisturbed and disturbed samples according to ASTM D 2850 and ASTM D 3080 standard. Graph-3.5 and Graph-3.6 indicate the relationship between cohesion (C) and ( $\phi$ ) versus their elevations at investigation area.



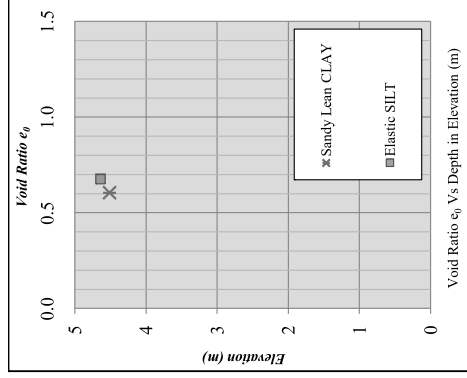
Graph - 3.5 : Cohesion "C<sub>UU</sub>" vs. Depth in Elevation (m)



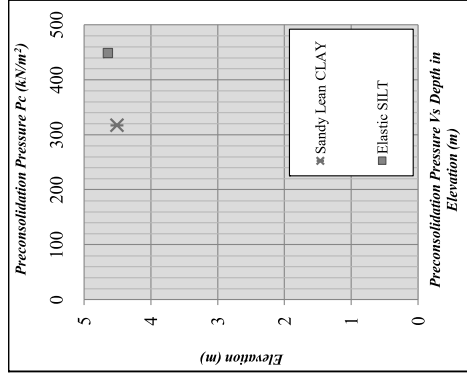
Graph - 3.6 : Phi "φ<sub>UU</sub>" vs. Depth in Elevation (m)

#### 3.2.2 One Dimensional Consolidation Test

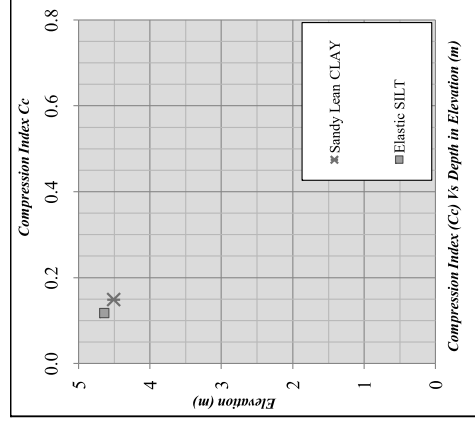
The one dimensional consolidation tests were carried out with undisturbed samples. These tests were carried out in accordance with standard ASTM D 2435. Graph-3.7 to Graph-3.9 indicate the relationship between ( $e_0$ ), ( $P_c$ ) and ( $C_c$ ) versus their depth in elevation at investigation area.



Graph - 3.7 : Void Ratio  $e_0$  vs. Depth in Elevation (m)



Graph - 3.8 : Preconsolidation pressure ( $P_c$ ) vs. Depth in Elevation (m)



Graph - 3.9 : Compression Index ( $C_c$ ) vs. Depth in Elevation (m)



**4.0 GEOTECHNICAL ENGINEERING CONSIDERATIONS**

**4.1 Characteristics of Soil Strata Relying on Field Test**

There have been (2) numbers of boreholes; depths of boreholes are of varying from 30.45 m to 43.95 m with the performance of Standard Penetration Tests. In this operation, six numbers of layers have been recognized. The soil layers are classified in accordance with their physical properties and/or their relative density or consistency. The boring logs are attached at Appendix. The six different layers observed in project area are described from top to bottom as follows.

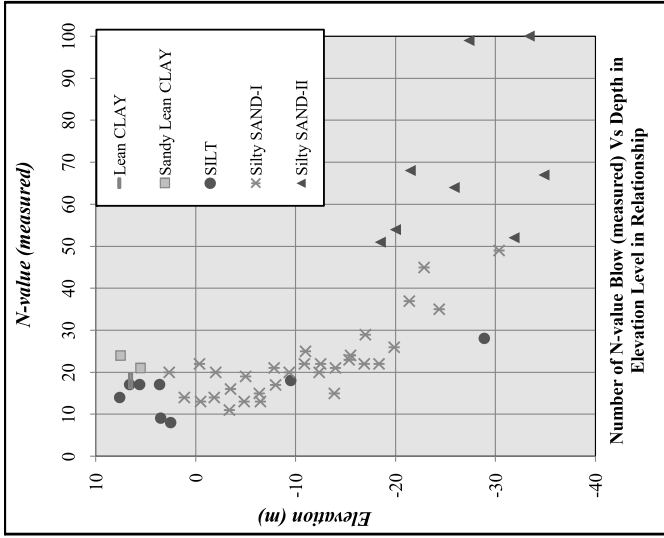
1. SILT
2. Elastic SILT
3. Sandy Lean CLAY
4. Lean CLAY
5. Silty SAND-I
6. Silty SAND-II

**4.2 Bearing stratum**

According to investigation results, six different layers have been identified in this area. The soil layers that can be used as reliable bearing layer should generally have N-value of round about 10 in light load structures (<50kN/m<sup>2</sup>) and that of N-value over 30 in heavy load structures (>50kN/m<sup>2</sup>). The range of N-value for each layer is illustrated in Table-4.1 and N-value measured in the field Vs elevation is also presented in Graph – 4.1.

Table - 4.1 : Range of N-value (Measured) from Different Soil Layers in Project Area

Sr. No.	Soil layer	N value (Measured)						Minimum	Maximum	Average
		10	20	30	40	50 & > 50				
1	SILT						8	28	16	
2	Elastic SILT							UID		
3	Sandy Lean CLAY						21	24	22	
4	Lean CLAY						18	18	18	
5	Silty SAND-I						11	49	22	
6	Silty SAND-II						51	100	69	



Graph - 4.1 : Number of N-Value (measured) vs Depth in elevation

**4.3 Soil Profile**

The present site investigation report is mainly based on the Code of practice for site investigations, ASTM D 1586-99. In the boring logs, the consistency/ density of the subsoils are based on SPT N-values and laboratory test results of soils.

Based on the borehole data and in-situ tests results obtained from the boreholes, the underlying subsoils can be sub-divided into the following layers:

- Top Soil Layer (0 to ~ 1.5 m) and
- Younger Alluvium (~ 1.5 m to ~6.0 m)
- Valley Filled Deposit (~6.0 m to termination of boreholes)

**4.3.1 Younger Alluvium and Valley Filled Deposit**

Younger Alluvium is observed from about 1.5 m to ~6.0 m both boreholes. It is mainly composed of very stiff to hard CLAY of low plasticity and firm to very stiff SILT of low plasticity to high plasticity.

Valley Filled Deposit is mainly observed from ~6.0 m to termination of both boreholes. It is chiefly composed of medium dense to very dense Silty SAND.

**4.4 List of Geological Cross-Section**

Please see the following soil profile (cross section) in Appendix.

SECTION	BOREHOLES
1	BH-1 ~ BH-2

**5.0 CONSIDERATION OF DESIGN PARAMETER AND BEARING CAPACITY**

**5.1 Consideration of Design Parameter**

The geotechnical parameters can be evaluated from many ways such as field in situ testing, laboratory testing and so on. Some of the design parameters cannot be evaluated directly neither from field tests nor laboratory tests due to the unfavorable nature of deposit or investigation methods. However, some parameters would be derived from the other instrumental testing of past events and some mechanical and physical properties obtained from field and laboratory tests. For evaluating the stability of ground, the shear strength parameters are significant. The geotechnical design parameters required for foundation design analysis are listed as below-

- $C_u$  Cohesion of soil (kN/m<sup>2</sup>)
- $\phi$  Friction angle of soil (angle of internal friction in degree)
- $\gamma_d$  Dry unit weight of soil (kN/m<sup>3</sup>)
- $\gamma_{sat}$  Saturated unit weight of soil (kN/m<sup>3</sup>)
- $\gamma'$  Effective unit weight of soil below water table (kN/m<sup>3</sup>)
- $E$  Modulus of elasticity of soil (kN/m<sup>2</sup>)

**5.1.1 Cohesion ( $C_u$ )**

The cohesive strength also known as un-drained shear strength of cohesive soil is normally evaluated from the unconfined compression test. The cohesive strength  $C_u$  can be derived from—

$$C_u = \frac{q_u}{2}$$

Where-  $C_u$  = cohesive strength (kN/m<sup>2</sup>)  
 $q_u$  = unconfined compressive strength (kN/m<sup>2</sup>)

$C_u$  also can be considered from SPT-N values as follows,

$$C_u = (20/3) * N \text{ (According to Japanese code)}$$

$$C_u = 6.25 * N \text{ (According to Terzaghi code)}$$

Hence, the cohesive strength ( $C_u$ ) is evaluated from SPT N-Value and compared with the laboratory results and the table of general relation of consistency and unconfined compressive strength of clay (Table – 5.1). For Sandy soil layer, the value of “ $C_u$ ” is taken as ‘0’.

Table - 5.1 : General Relationship of Consistency and Unconfined Compression Strength of Clays

Consistency	q <sub>u</sub>	
	kN/m <sup>2</sup>	ton/ft <sup>2</sup>
Very soft	0-25	0-0.25
Soft	25-50	0.25-0.5
Medium stiff (Firm)	50-100	0.5-1
Stiff	100-200	1-2
Very Stiff	200-400	2-4
Hard	>400	>4

**5.1.2 Friction angle (φ)**

The friction angle of the granular soil can be directly evaluated from the SPT N-value and also compared with the friction angle of granular soils which is evaluated from their average SPT N-value, in accordance with Table – 5.2. For the cohesive soil layers, their friction angles can be taken as ‘0’.

According to Hataakanda and Uchida (1996)

$$\phi = 3.5 * (N)^{1/2} + 22.3$$

Where;

φ = friction angle of the soil

N = SPT-N value

This equation ignores particle size. Most tests are done on medium to coarse sands. SPT-N values for fine sand will have a lower friction angle while coarse sands will have a larger friction angle.

Modified equations are described as below;

For Fine Sand;  $\phi = 3.5 * (N)^{1/2} + 20$

For Medium Sand;  $\phi = 3.5 * (N)^{1/2} + 21$

For Coarse Sand;  $\phi = 3.5 * (N)^{1/2} + 22$

Typical values of φ are presented in Table – 5.2.

Table - 5.2 : Typical values for internal friction angles

Soil	Type of test	
	Unconsolidated undrained UU	Consolidated undrained CU / Consolidated drained CD
Gravel		
Medium size Sandy	40-45° 35-40°	40-55° 35-50°
Sand		
Loose dry	28-34°	
Loose saturated	28-34°	
Dense dry	35-46°	43-50° 43-50°
Dense saturated	1-2° less than dry sand	
Silt or silty sand		
Loose	20-22°	27-30°
Dense	25-30°	30-35°
Clay	0° if saturated	3-20° 20-42°

**5.1.3 Saturated Unit Weight of Soil (γ<sub>sat</sub>)**

The saturated soil defines as the soil located below the water table. The saturated unit weight of soil can be evaluated directly from the field density test or equation.-

$$\gamma_{sat} = \frac{G_s \gamma_w + e \gamma_w}{1 + e}$$

Where- γ<sub>sat</sub> = saturated unit weight of soil (kN/m<sup>3</sup>)

γ<sub>w</sub> = saturated unit weight of water (kN/m<sup>3</sup>)

G<sub>s</sub> = specific gravity of soil

e = void ratio of soil (e = wG<sub>s</sub> for saturated soil)

The G<sub>s</sub> and w can be resulted from laboratory tests of collected “Disturbed Samples”.

**5.1.4 Effective Unit Weight of Soil (γ')**

The effective unit weight of soil under water table can be evaluated from the equation-

$$\gamma' = \gamma_{sat} - \gamma_w$$

Where- γ' = effective unit weight of soil (kN/m<sup>3</sup>)

γ<sub>sat</sub> = saturated unit weight of soil (kN/m<sup>3</sup>)

γ<sub>w</sub> = unit weight of water (kN/m<sup>3</sup>)

The unit weight of water in SI unit is 9.81kN/m<sup>3</sup> (or) 10 kN/m<sup>3</sup>.

### 5.1.5 Modulus of Elasticity of Soil ( $E_s$ )

The modulus of elasticity or Young's modulus of a soil is an elastic soil parameter most commonly used in the estimation of settlement from static loads. Young's soil modulus,  $E_s$ , may be estimated from empirical correlations, laboratory test results on undisturbed specimens and results of field tests. Laboratory tests that may be used to estimate the soil modulus are the triaxial unconsolidated undrained compression or the triaxial consolidated undrained compression tests. Field tests include the plate load test, cone penetration test, standard penetration test (SPT) and the pressuremeter test.

$E_s$  from SPT-N value correlations,

For Sand,

$$E_s = P_a \times 8 \times N_{60} \quad (\text{Schmertmann 1970})$$

$$E_s = 700 \times N \quad (\text{kN/m}^2) \quad (\text{Japanese code})$$

For Normally consolidated Clay,

$$E_s = 250 C_u \text{ to } 500 C_u (\text{kN/m}^2) \quad (\text{Schmertmann 1970})$$

For Over consolidated Clay,

$$E_s = 750 C_u \text{ to } 1000 C_u (\text{kN/m}^2) \quad (\text{Schmertmann 1970})$$

Table - 5.3 : Typical  $E_s$  values

Soil	$E_s$ ( $\text{kN/m}^2$ )
Loose sand	10500-24000
Medium dense sand	17250-27600
Dense sand	34500-55200
Silty sand	10350-17250
Sand and gravel	69000-172500
Soft clay	4100-20700
Medium clay	20700-41400
Stiff clay	41400-96600

**\*\* According to USCS soil classification system, if sand % is greater than 50%, it can be said that clayey sand or silty sand. If clayey sand has sand % of between 50% and 60%, it can be taken as undisturbed sample (UD) during field investigation and UCS tests or triaxial (UU) tests can be done and cohesion ( $C_u$ ) value can also be obtained from these tests. So, careful judgment should be made for clayey sand which might have cohesion property as well as friction angle.**

If the designer would like to get effective design parameter such as  $c'$  and  $\phi'$ , please take reference to Table – 5.2 for granular soil and Table – 5.4 for cohesive soil.

Table - 5.4 :  $\phi'_{crit}$  for clayey soil (BS 8002-1994)

Plasticity index (PI)	$\phi'$ (degree)
15	30
30	25
50	20
80	15

In the absence of reliable laboratory test data, the conservative values of  $\phi'$  given in above mentioned table may be used with  $c'=0$ .

### 5.2 Determination of bearing capacity of shallow footing

The bearing capacity of shallow foundation is generally calculated based on the proposed building plan and structural loads that are indicated by client. The client do not propose the structural plan of the proposed project. So, the dimension of footing is considered as unit meter squared (1m x 1m) and the depth of footing ( $D_f$ ) is calculated for various depth of 1.5 to 6.0 m. So, the designer can use easily this bearing capacity ( $\text{kN/m}^2$ ) or ( $\text{ton/ft}^2$ ) values for various depths. The detailed calculation of shallow foundation is described in Appendix.

If the design parameters can be got by using neither correlation from SPT-N value nor laboratory test results, the estimated bearing capacity for shallow footing can be consider by using "Modified Terzaghi's Formula".

The Terzaghi bearing capacity equation is given below,

$$q_{ult} = (c \times N_c S_c) + (q \times N_q) + (0.5 \times B \times N_r \times \gamma \times S_r)$$

Where

$q_{ult}$  = the ultimate bearing capacity

$c$  = cohesion of the soil

$N_c$  = Terzaghi bearing capacity factor (obtained from Table – 5.5)

$S_c$  = Shape factor (obtained from Table – 5.6)

$q$  = effective stress at the bottom of the footing ( $\gamma \times d$ )

$d$  = distance from ground surface to the bottom of the footing

= effective density of soil

$B$  = width of the shorter dimension of the footing

$N_r$  = Terzaghi bearing capacity factor (obtained from Table – 5.5)

$N_q$  = Terzaghi bearing capacity factor (obtained from Table – 5.5)

$\gamma$  = effective density of soil

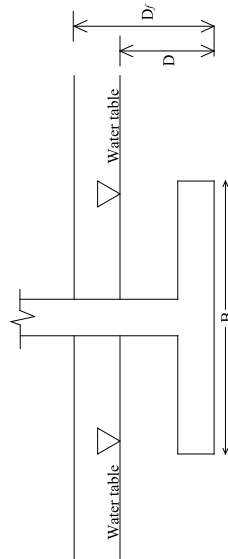
$S_r$  = shape factor (obtained from Table – 5.5)

### 5.2.1 Effect of water table on bearing capacity

The predicting formulations for bearing capacity were based on the assumption;

#### Case I – Water table above bottom of footing ( $0 < D < D_f$ )

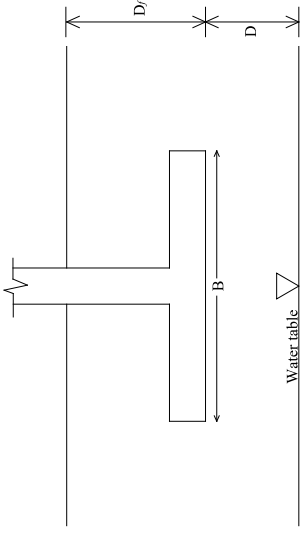
When the water table is above the bottom of the footing, the second and third term of the bearing capacity equation is needed to be modified as following.



$$q_{ult} = cN_c + [\gamma(D_f - D) + (\gamma_{sat} - \gamma_w)D]N_q + \frac{1}{2}(\gamma_{sat} - \gamma_w)BN_r$$

#### Case II – Water table at or below bottom of footing

When the water table is below the bottom of the footing, the last term of the bearing capacity equation is modified by replacing the unit weight of the soil with an average value as shown as below.



$$q_{ult} = cN_c + \gamma D_f N_q + \frac{1}{2} \gamma_{avg} B N_r$$

$$\gamma_{avg} = \frac{\gamma D + (\gamma - \gamma_w)(B - D)}{B} \quad D \leq B$$

$$= \gamma \quad D > B$$

Table - 5.5 : Bearing Capacity Factors

$\phi$	$N_c$	$N_q$	$N_r$
0°	5.7	1.0	0.0
5°	7.3	1.6	0.5
10°	9.6	2.7	1.2
15°	12.9	4.4	2.5
20°	17.7	7.4	5.0
25°	25.1	12.7	9.7
30°	37.2	22.5	19.7
35°	57.8	41.4	42.4
40°	95.7	81.3	100.4
45°	172.3	173.3	297.5
50°	347.5	415.1	415.1

Table - 5.6 : Shape Factors for Terzaghi bearing capacity equation

	$S_c$	$S_r$
Square footings	1.3	0.8
Strip footings (Wall)	1.0	1.0
Round footings	1.3	0.6

### 5.3 Liquefaction potential evaluation and analysis

Soil liquefaction occurs in loose, saturated and cohesionless soil (sands and silts) and sensitive clays when a sudden loss of strength and loss of stiffness is experienced, sometimes resulting in large, permanent displacement of the ground. Even thin lenses of loose saturated silts and sand may cause an overlying sloping soil mass to slide laterally along the liquefied layer during earthquakes.

According to the National Center for Earthquake Engineering Research (NCEER), the liquefaction can occur under the following conditions;

- Geological age and origin
- Lower fine contents and plasticity index
- Degree of saturation (location of water table)
- Soil penetration resistance

#### 5.3.1 Criteria for Liquefaction Potential

1. *Geologic age and origin:* If a soil layer is a fluvial, lacustrine or Aeolian deposit of Holocene age, a greater potential for liquefaction for liquefaction exists than for till, residual deposits or older deposits.
2. *Fines content and plasticity index:* Liquefaction potential in a soil layer increases with decreasing fines content and plasticity of the soil. In general, soils with a significant plasticity are not susceptible to liquefaction. Based on the Chinese findings, (Seed and Idriss, 1982), recommended that soils with significant **plastic fines should be evaluated for possible liquefaction based on the Atterberg limits.**
  - Percent finer than 0.005 mm ≤ 20%
  - (Liquid Limit +1%) ≤ 35%
  - (Water content +2%) ≥ 0.9\*(Liquid Limit+1%)

*A soil with plastic fines should be considered vulnerable to significant loss of strength or liquefaction in an earthquake if the measured index properties fall within these bounds.*

3. *Saturation:* Although low water content soils have been reported to liquefy, at least 80 to 85 % saturation is generally deemed to be a necessary condition for soil liquefaction. The highest anticipated temporal phreatic surface elevations should be considered when evaluating saturation.
4. *Depth below ground surface:* If a soil layer is within 20 m (~60 ft) from the ground surface, it is more likely to liquefy than deeper layers.

### 5.3.2 Procedures for Liquefaction Analysis

Liquefaction analysis is performed by the method of the National Center for Earthquake Engineering Research (NCEER)'s guideline as follows;

#### 1) Calculation of cyclic resistance ratio

For  $(N_1)_{60} \leq 30$

$$100 * CRR_{M=7.5} = \frac{95}{34 - (N_1)_{60}} + \frac{(N_1)_{60}}{1.3} - \frac{1}{2} \quad \text{①}$$

Where;

$CRR_{M=7.5}$  = the cyclic resistance ratio for a  $M_w=7.5$  earthquake

$(N_1)_{60}$  = the corrected SPT blow count

The value of  $CRR_{M=7.5}$  must be adjusted for the magnitude of the earthquake under consideration. This is done with a magnitude scaling factor (MSF);

$$CRR = CRR_{M=7.5} * MSF \quad \text{②}$$

Where,

MSF = Magnitude scaling factor

- For  $M_w < 7.0$ :  $MSF = 10^3 * M_w^{-3.46}$
- For  $M_w \geq 7.0$ :  $MSF = 10^{2.24} * M_w^{-2.56}$

For the corrected SPT blow count  $(N_1)_{60}$

$$(N_1)_{60} = N_{SPT} * C_N * C_E * C_B * C_S * C_R \quad \text{③}$$

Where;

$N_{SPT}$  = the measured blow count in the field

$C_N$  = correction factor for atmosphere

$$C_N = \sqrt{\frac{P_a}{\sigma'_{v0}}} \leq 2.0$$

$C_E$  = correction for energy delivered by the SPT hammer and can be estimated from the average values given by Seed et al. (1985)

Country	Hammer type	Hammer release	CE
United states	Safety	Rope and pulley	1.0
United states	Donut	Rope and pulley	0.75
Japan	Donut	Rope and pulley, Special throw release	1.12
Japan	Donut	Free fall	1.3

$C_B$  = correction for the borehole diameters

Diameter of boreholes	$C_B$
65 mm to 115 mm (2.5 to 4.5 in)	1.00
150 mm (6 in)	1.05
200 mm (8 in)	1.15

$C_S$  = correction for the sampler

$C_S = 1.2$  for split spoon sampler and 1.0 for a standard sampler

$C_R$  = correction for the loss of energy through reflection in short lengths

- For  $z \leq 3$  m:  $C_R = 0.75$
- For  $3 < z < 9$  m:  $C_R = (1.5+z)/24$
- For  $z \geq 9$  m:  $C_R = 1.0$  where  $z$  is the length of drill rod in meter

Next step is needed to compute  $(N_1)_{60}$

$$(N_1)_{60}' = (N_1)_{60} + \Delta(N_1)_{60}$$

- For  $F_c \leq 5\%$   $\Delta(N_1)_{60} = 0.0$
- For  $5 < F_c < 35\%$   $\Delta(N_1)_{60} = 7 * (F_c - 5) / 30$
- For  $F_c \geq 35\%$   $\Delta(N_1)_{60} = 7.0$

(Where  $F_c$  is percent finer than 0.075 mm)

## 2) Calculation of Cyclic Stress Ratio Induced by Earthquake

$$CSR = 0.65 * \frac{\sigma_{v0}'}{g} * \frac{\sigma_{v0}'}{\sigma_{v0}'} * \gamma_d$$

CSR = cyclic stress ratio

$\sigma_{v0}'$  = maximum peak horizontal acceleration

$g$  = acceleration due to gravity ( $m/s^2$ )

- $\sigma_{v0}'$  = total overburden stress
- $\sigma_{v0}$  = effective overburden stress
- $\gamma_d$  = stress reduction factor

$$\gamma_d = 1.0 + 1.6 * 10^{-6} (z^4 - 42 * z^3 + 105z^2 + 4200z)$$

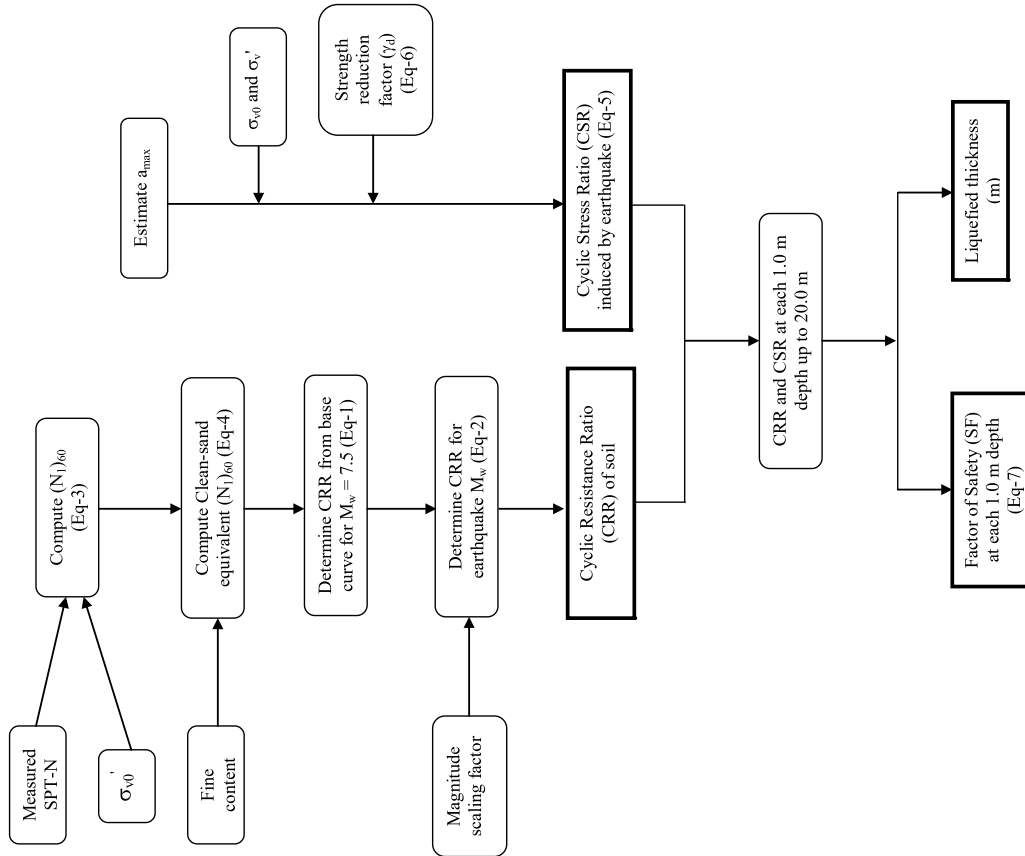
Where  $z$  is the depth below the ground surface in meters.

## 3) Prediction of Liquefied thickness

The factor of safety against liquefaction (FS) is defined with (Ishihara 1993; Seed and Harder 1990) as follow;

$$FS = \frac{CRR}{CSR}$$

Using a factor of safety less than 1.0 ( $FS < 1.0$ ) against liquefaction is not considered a sound engineering practice. This is because a factor of safety less than 1.00 indicates failure is likely to occur.



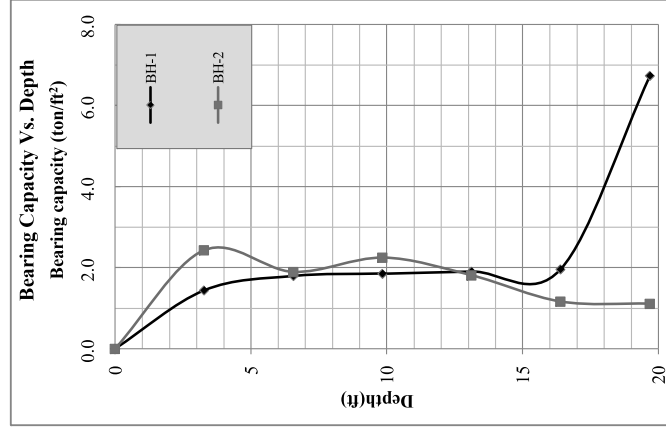
Flow chart - 5.1: Liquefaction Analysis

## 6.0 CONCLUSION AND RECOMMENDATION

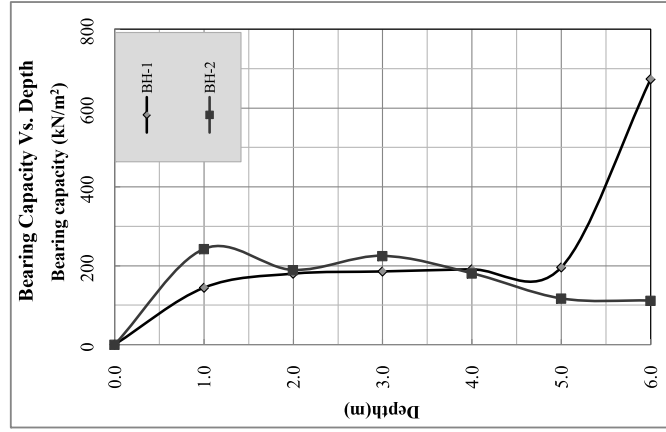
### 6.1 Conclusion

#### 6.1.1 Bearing Capacity of shallow foundation

By using modified Terzaghi's bearing capacity equation, the bearing capacity of shallow foundation for each depth can be described as Graph -6.1 and 6.2. The calculation sheet of bearing capacity is presented in Appendix. For depths which are deeper than 6.0 m (~20ft) should not be considered for shallow footing for proposed project. If the client wants to use driven pile foundation for proposed project, these deeper depths can be calculated with suitable pile size for respective bearing capacity (end bearing + skin friction).



Graph - 6.1 : Allowable bearing capacity (ton/ft<sup>2</sup>) vs. depth (ft)



Graph - 6.2 : Allowable bearing capacity (kN/m<sup>2</sup>) vs. depth (m)



## 6.2 General Suggestions

When evaluating the bearing capacity of proposed area of respective boreholes, laboratory test results as well as the results derived from SPT-N values correlation were also considered. The estimated allowable, average and minimum bearing capacity for each depth is described in Table – 6.1.

Table - 6.1 : Allowable, average and minimum bearing capacity

Depth (m)	BH-1 (ton/ft <sup>2</sup> )	BH-2 (ton/ft <sup>2</sup> )	Average allowable bearing capacity (ton/ft <sup>2</sup> )	Minimum allowable bearing capacity (ton/ft <sup>2</sup> )
0	0.00	0.00	0.00	0.00
1	1.44	2.43	1.93	1.44
2	1.80	1.89	1.84	1.80
3	1.86	2.22	2.04	1.86
4	3.20	4.14	3.67	3.20
5	1.96	1.10	1.53	1.10
6	6.73	1.03	3.88	1.03

- For shallow footing, Structural engineer can adjust and choose the various depths to meet the suitable bearing capacity requirement for the proposed building. From safety point of view, minimum allowable bearing capacity value should be used for different depths. **Nevertheless, the structural engineer should keep in mind that the bearing pressure under footing base will effect up to 1~2B (B= footing width).**
- Soil liquefaction is a major cause of damage during earthquakes. Modern engineering treatment of liquefaction related issues evolved initially in the wake of the two devastating earthquakes of 1964, the 1964 Niigata and 1964 Great Alaska earthquakes, in which seismically-induced liquefaction produced spectacular and devastating effects. The possibility of liquefaction (liquefaction potential) is high in loose sand deposits of sand. In contrast, liquefaction is unlikely to occur in dense sandy deposits. *Liquefaction is not possible when there is no ground water. Loose sandy deposit with high ground water table is mostly found in land reclamation (relative density being around 40%), young age of sand means no significant cementation and few experience of strong earthquake shaking. Liquefaction mostly occurs along the abandon of channels of rivers and their alluvial plain.* (“Geotechnical Earthquake Engineer” 2008, Chapter 18.11, Ikuro Towhata,

Professor of Geotechnical Engineering, Department of Civil Engineering, University of Tokyo).

- ➔ **For the present site**, Liquefaction potential is “LOW” in both boreholes. The liquefaction potential is evaluated according to NCEER method as described in Section 5.3 and the calculation sheets are presented in Appendix. The maximum peak ground acceleration for Yangon region by CQHP (Committee for Quality Control of High-rise Building Project) is 0.2 g.
- ➔ To prevent earthquake effect, structural engineer should also consider above mentioned earthquake intensity for this proposed project. The Modified Mercalli (MM) Scale of 7.5 would also be suggested for design purpose for proposed structure.

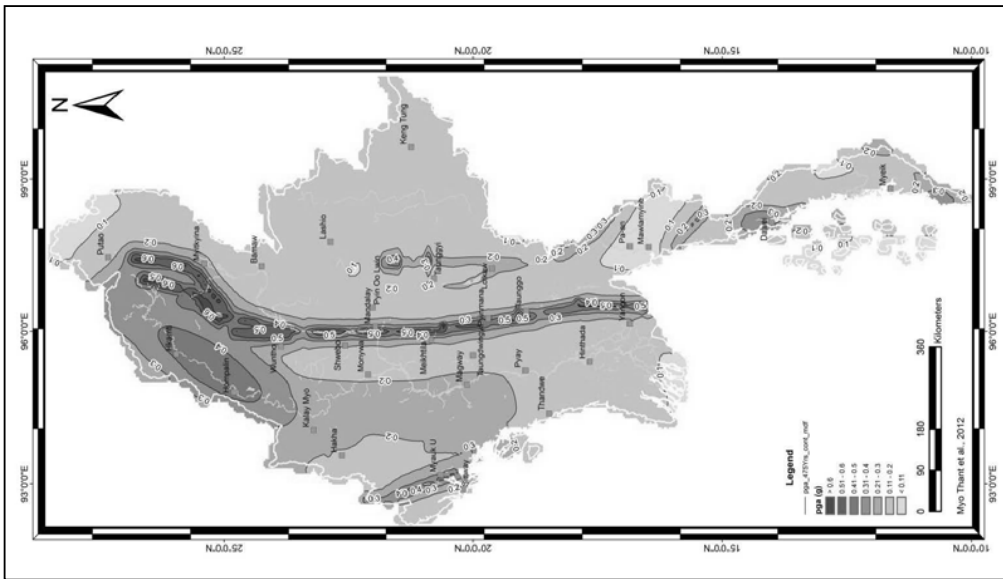


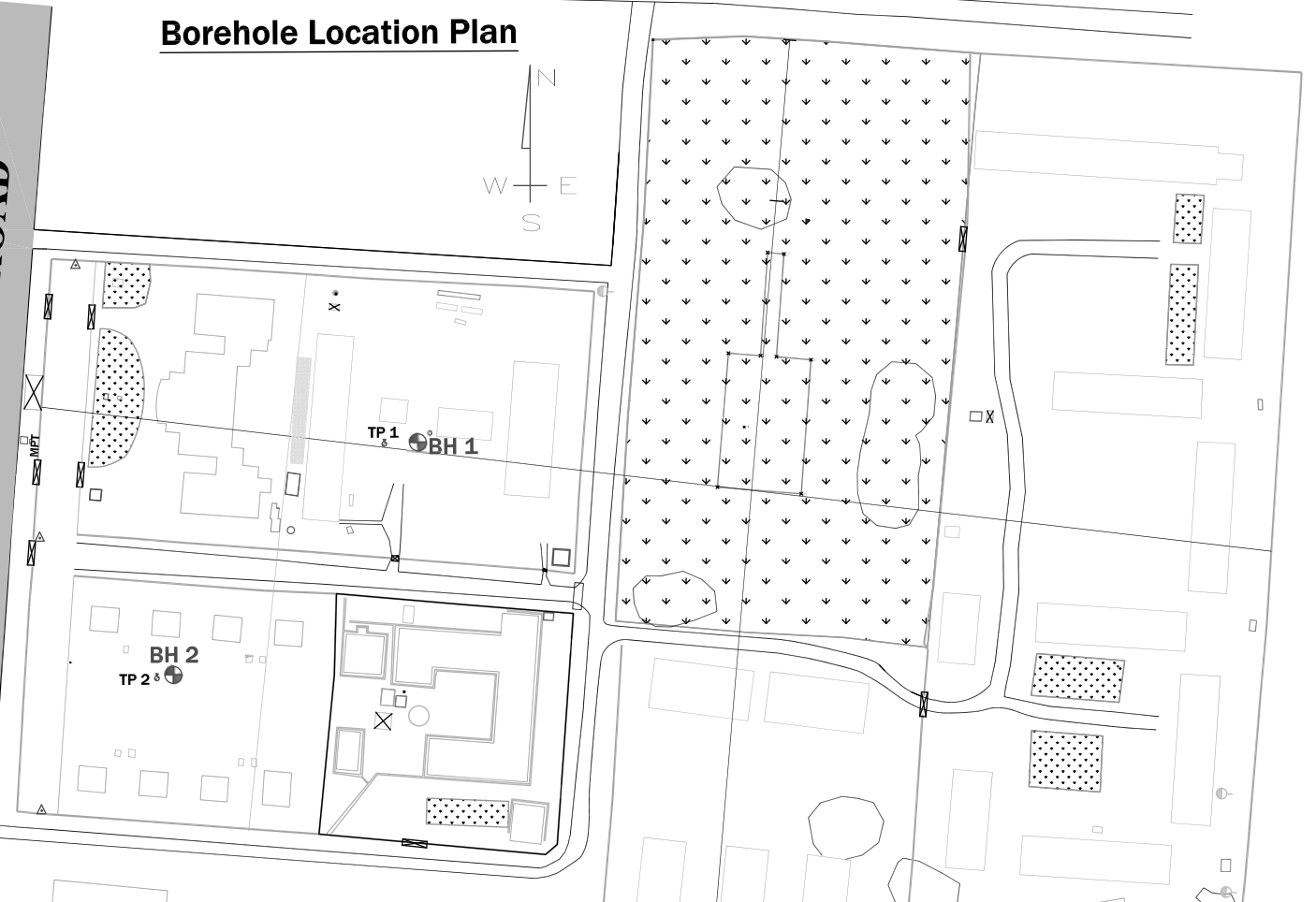
Figure - 6.1 : Probabilistic seismic hazard map of Myanmar (Myo Thant et al., 2012)

May Thu  
 B.E (Civil) YU  
 M.E (Civil) Geotechnical (Tokyo University)  
 PE (Geotechnical) MEng.C

# APPENDICES

# LOWER MINGALARDON ROAD

## Borehole Location Plan



## BORE LOG (FOR DESIGN PARAMETERS CONSIDERATION)

**BORE HOLE No. BH-1**      SHEET No. / OF 2  
 PROJECT NAME: Geotechnical Works for the Establishment of Apartment/Vacation/Retirement Homes (along with Boring Equipment)      DATE: 25/06/18  
 LOCATION: Lower Mingaldon Road, Sharnah Ward, Aung San, Insein Township, Yangon Region      BORING METHOD: Rotary Drilling Method      LOGGED BY: Zaw Min Than  
 GROUND LEVEL: 8.639 m      ORIENTATION: Vertical      DATE: 25/06/18  
 COORDINATE: N: 1827242.629 E: 1099064.100 DEPTH: 43.95 m      GROUND WATER LEVEL: 10.56 m      MATSUDA Consultants International Co., Ltd.

SCALE (m)	ELEVATION (m)	DEPTH (m)	THICKNESS (m)	DIAGRAM	COLOR	RELATIVE DENSITY (%)	SOIL NAME	SOIL DESCRIPTION	DATE & DEPTH (m)	CASTING DEPTH (m) & (SPT NO.)	WATER DEPTH (m)	DEPTH (m)	BRSS (mm)	SCALE (mm)	SAMPLE (SPT No.)	DEPTH (m)	TCR (%)	SCR (%)	RCP (%)	SCALE (m)
1	2.639	1.00	1.00	X X	Yellowish brown to grey	Soft to very soft	SILT	Top soil layer, Yellowish brown, Sandy SILT with pieces of bricks.		1.00	4.20	1.0	SPT-1	1.0	4.3				1	
2		2.00		X X	Yellowish brown to red reddish brown	Stiff to very stiff	SILT with trace of sand.	Stiff to very stiff, Yellowish brown, mottled reddish brown, Low plasticity, SILT with trace of sand.		2.00	7.30	2	SPT-2	2.0	7.5				2	
3		3.00		X X	Yellowish brown to very stiff					3.00	7.30	3	SPT-3	3.0	7.5				3	
4	4.639	4.00	3.00	X X	Yellowish brown			Yellowish brown mottled reddish brown, High plasticity, Elastic SILT.		4.00	9.40	4	SPT-4	4.0	9.4				4	
5	3.639	5.00	1.00	X X	Grey	Very stiff		Very stiff, Grey mottled yellowish brown, Low plasticity, SILT with sand.		5.00	7.30	5	SPT-5	5.0	7.5				5	
6	2.639	6.00	1.00	X X	Yellowish brown					6.00	20.30	6	SPT-6	6.0	6.5				6	
7		7.00		X X	Yellowish brown					7.00	14.30	7	SPT-7	7.0	7.0				7	
8		8.00		X X	Grey					8.00	14.30	8	SPT-8	8.0	8.0				8	
9		9.00		X X						9.00	22.30	9	SPT-9	9.0	9.4				9	
10		10.00		X X						10.00	14.30	10	SPT-10	10.0	10.0				10	
11		11.00		X X						11.00	14.30	11	SPT-11	11.0	10.5				11	
12		12.00		X X						12.00	17.30	12	SPT-12	12.0	11.5				12	
13		13.00		X X						13.00	17.30	13	SPT-13	13.0	12.5				13	
14		14.00		X X						14.00	13.30	14	SPT-14	14.0	13.0				14	
15		15.00		X X						15.00	5.30	15	SPT-15	15.0	14.5				15	
16		16.00		X X						16.00	21.30	16	SPT-16	16.0	15.5				16	
17		17.00		X X						17.00	21.30	17	SPT-17	17.0	16.5				17	
18		18.00		X X						18.00	20.30	18	SPT-18	18.0	17.5				18	
19		19.00		X X						19.00	22.30	19	SPT-19	19.0	18.5				19	
20		20.00		X X						20.00	22.30	20	SPT-20	20.0	19.5				20	
21		21.00		X X						21.00	20.30	21	SPT-21	21.0	20.5				21	
22		22.00		X X						22.00	22.30	22	SPT-22	22.0	21.5				22	
23		23.00		X X						23.00	15.30	23	SPT-23	23.0	22.5				23	
24		24.00		X X						24.00	23.30	24	SPT-24	24.0	23.0				24	
25		25.00		X X						25.00	23.30	25	SPT-25	25.0	23.5				25	
26		26.00		X X						26.00	22.30	26	SPT-26	26.0	25.0				26	
27		27.00		X X						27.00	22.30	27	SPT-27	27.0	26.5				27	
28		28.00		X X						28.00	3.630	28	SPT-28	28.0	27.5				28	
29		29.00		X X						29.00	3.630	29	SPT-29	29.0	28.5				29	
30		30.00		X X						30.00	3.630	30	SPT-30	30.0	29.5				30	
				X X							3.630									
				X X						4.10										

**NOTES**  
 Relative density description: Very loose (< 10), Loose (10-30), Medium dense (30-50), Dense (50-75), Very dense (> 75)  
 Consistency description: Very soft (< 25), Soft (25-40), Stiff (40-60), Very stiff (60-80), Hard (> 80)  
 SPT No./Unit (Blow): 0-10, 10-20, 20-30, 30-40, 40-50, 50-60, 60-70, 70-80, 80-90, 90-100, 100-110, 110-120, 120-130, 130-140, 140-150, 150-160, 160-170, 170-180, 180-190, 190-200, 200-210, 210-220, 220-230, 230-240, 240-250, 250-260, 260-270, 270-280, 280-290, 290-300, 300-310, 310-320, 320-330, 330-340, 340-350, 350-360, 360-370, 370-380, 380-390, 390-400, 400-410, 410-420, 420-430, 430-440, 440-450, 450-460, 460-470, 470-480, 480-490, 490-500, 500-510, 510-520, 520-530, 530-540, 540-550, 550-560, 560-570, 570-580, 580-590, 590-600, 600-610, 610-620, 620-630, 630-640, 640-650, 650-660, 660-670, 670-680, 680-690, 690-700, 700-710, 710-720, 720-730, 730-740, 740-750, 750-760, 760-770, 770-780, 780-790, 790-800, 800-810, 810-820, 820-830, 830-840, 840-850, 850-860, 860-870, 870-880, 880-890, 890-900, 900-910, 910-920, 920-930, 930-940, 940-950, 950-960, 960-970, 970-980, 980-990, 990-1000



**GROUND WATER SURVEY REPORT**

**GROUNDWATER CAPACITY MEASURING TEST FOR THE PROJECT  
FOR THE ESTABLISHMENT OF JAPAN-MYANMAR VOCATIONAL  
TRAINING INSTITUTE (AUNG SAN)**

@

**LOWER MINGALADON ROAD, SINGUU WARD, AUNG SAN, INSEIN  
TOWNSHIP, YANGON REGION**

**MATSUDA CONSULTANTS INTERNATIONAL CO., LTD.**

**(REVISED)**

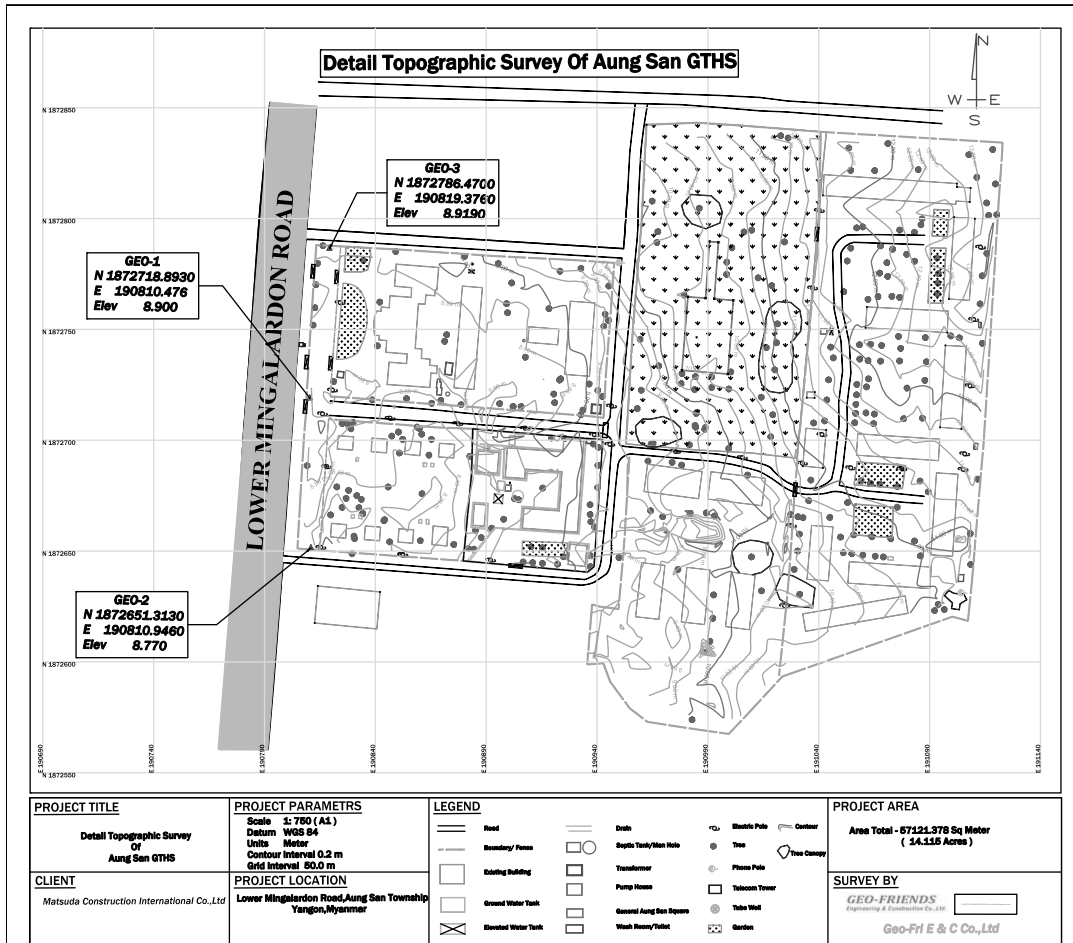
**18<sup>th</sup> AUGUST 2018**

Submitted by



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**Ø6" TUBE WELL DRILLING WORK,  
AQUIFER AND WELL TEST REPORT  
GTHS COMPOUND, AUNG SAN,  
INSEIN TOWNSHIP**



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## “ Revised Summary Short Report ”

### 1. Introduction

At the Japan-Myanmar Vocational Training Institute (Aung San), the  $\phi$  6” tubewell one number drilling work, pumping test and water sample chemical analysis of recent well & existing well are done in July 2018.

### 2. $\phi$ 6” well drilling

The  $\phi$  6” pvc well one number were drilled at near the north-east fencing 16° 55' 05.16" N, 96° 05' 55.74" E.

The drilling work with skit mounted drill machine water jetting reverse circulating method were drill tend to 500 ft depth. (Well log attached)

The coarse sand aquifer two layers were found at the depth 120'-200' Alluvial valley filled deposit yellow sand layer and the second at the depth 400'-480' Older alluvial blue sand layer.

These blue sand layer water quality may be high iron content and these medium sand water yield may be less than coarse sand.

The first yellow sand layer is good water quality and high yield quantity. So that  $\phi$  6” well were constructed by  $\phi$  6” pvc hand slotted screen 40' (160'-200') with casing 160' & sand trap 100'. The well developing were made water jet, serge, air lifting method with high yield pump and high compressor. (Well design, well data, water sample chemical test attached.

### 3. The pumping test

The 100 mm $\phi$  submersible pump 5.5 HP were installed with 2” pvc 100' pipe. The test row one week were made water yield 3000 gph to 6000 gph. The pumping test 4 days were made.

According to pumping test data

- (a)  $\phi$  6” well maximum yield is 6000 gph (27.27 m<sup>3</sup> ph)
- (b) Maximum water level is 11.80 mtr to 14.30 mtr (from ground level 0.0 mtr). It can be say (-11.80 m level) to (-14.30 m level)
- (c) Water level recovery is nearly static level within 24 hr from 24 hr pumping drawdown level.

### 4. Water quality

The water quality is good potable water of 200' depth level yellow sand layer. The chemical lab test result of recent well & existing tubewell be attached.

### 5. Conclusion

- (a) Recent  $\phi$  6” well water quality and water yield may be enough for school.

It may be get 3000 gph x 8 hr running = 24000 gal per day.

- (b) The next standby  $\phi$  6” well one number will be need.



### Water Well Design

Collected the sample of the formation penetrated at each 3 meters interval. Describe the detail information were thoroughly checked and the detail logs of the well was presented and water well design of the investigation well is shown in Figure (6.1)

Table (6.1) Summary of Lithological Log of  $\phi$  6” PVC Tube Well No (1)

Depth(ft)	Thickness	Lithology	Remark
0	10	Overburden, soil, sandy clay, yellow	
10	20	Sand, fine, yellow, clayey	
20	40	Sand, fine, yellow	
40	60	Sand, fine, yellow	
60	80	Sand, fine to medium, yellow	
80	100	Sand, medium to coarse, yellow	
100	120	Sand, coarse, dark yellow	
120	140	Sand, coarse, yellow	
140	160	Sand, coarse, yellow	
160	180	Sand, coarse, fine gravel, yellow	
180	200	Sand, coarse, yellow	
200	220	Sand, fine, dark yellow	
220	240	Sand, fine, dark yellow, bluish	
240	260	Sand, fine, dark grey	
260	280	Sand, medium, dark grey	
280	300	Sand, medium, blue	
300	320	Sand, fine, blue, slaty layer	
320	340	Clay, blue, sandy	
340	360	Clay, blue, sandy	
360	380	Sand, fine, dark blue	
380	400	Sand, fine, blue	
400	420	Sand, medium, blue	
420	440	Sand, medium, blue, slaty layer interbedded	
440	460	Sand, medium to coarse, dark blue	
460	480	Sand, medium, blue	
480	500	Sand, fine, blue, slaty layer interbedded	

Appendix (A)

**Ø6" PVC TUBE WELL (Investigation Well)**

**Well Data**

1. Well No - Ø6" Well No (1)
2. Location - GTHS Compound, Aung san, Insein Township, Northern District, Yangon, Myanmar.
3. Client - MATSUDA COSULANTS INTERNATIONAL INC
4. Well size - Diameter 6"
5. Well depth - 300'
6. Bore hole depth - 500'
7. Bore hole size - Ø12 "
8. Well casing pipe - Ø6" PVC pipe 13.5 class, thickness 1mm
9. Well screen pipe - Ø6" PVC pipe 13.5 class, hand slotted
10. Well profile - Top to bottom
  - Ø6" PVC pipe casing pipe - 160 ft
  - Ø6" PVC screen - 40 ft
  - Ø6" PVC sand trap - 100 ft
11. Static Water Level - 40 ft
12. Pumping Water Level - 60ft
13. Water Yield - 6000 gph
14. Suitable pump - Ø4" submersible pump, HST, KSB Cora 18/9, 4HP, 18-20 mH 4000-5000 gph, setting depth 100 ft with 2" Ø PVC discharge pipe

**1. Introduction**

To determine the aquifer characteristics for groundwater capacity measurement, the following tests are done in the site of Japan-Myanmar Vocational Training Institute (Aung San).

- I. Step-drawdown Test or Well Performance Test
- II. Recovery of Step Test
- III. Constant Discharge Test and Recovery Test
- IV. *Step-draw down Test and Recovery of Step Test*

Start date	5.7.18
Finished date	5.7.18

*Constant Discharge Test*

- |               |        |
|---------------|--------|
| Start date    | 6.7.18 |
| Finished date | 7.7.18 |
- Recovery Test*
- |               |        |
|---------------|--------|
| Start date    | 7.7.18 |
| Finished date | 8.7.18 |





### 2. Location

The investigation site of Japan-Myanmar Vocational Training Institute, (Aung San) in Insein, Township, which lies in the northern parts of Yangon. The site lies between 16 °55'05.16"N and 96 ° 05'55.74"E. Show in figure (2.1)

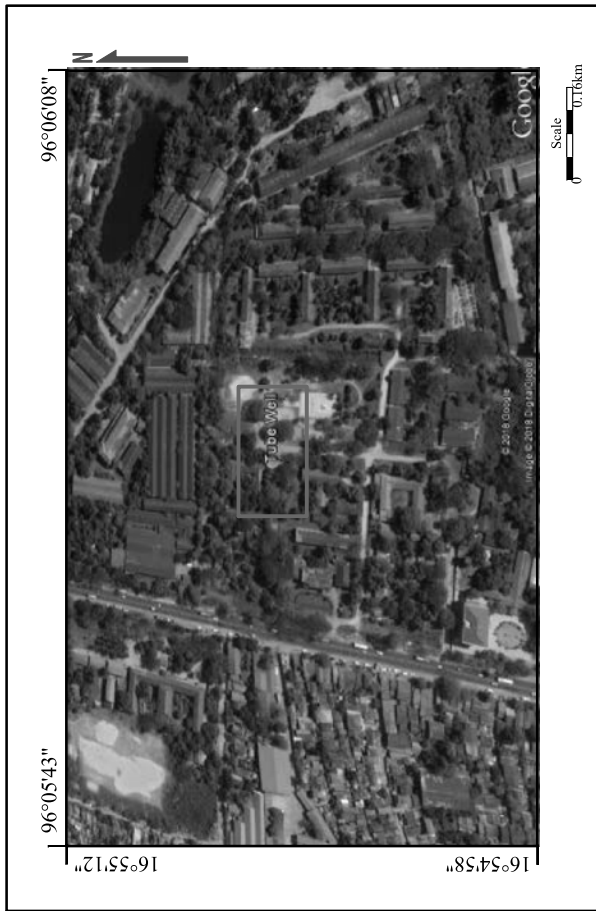


Figure (2.1) Location Map of the Investigation Site Area

### 3. Topography and Drainage

The site lies at the western part of Shwedagon-Mingalardon Anticlinal ridge. It is a low flat area. Elevation is about 30 ft above sea level. Hlaing River is situated in the west of the site. Drainage pattern is generally dendritic pattern. Streams are flowing from east to west into Hlaing River. It is shown in figure (3.1).

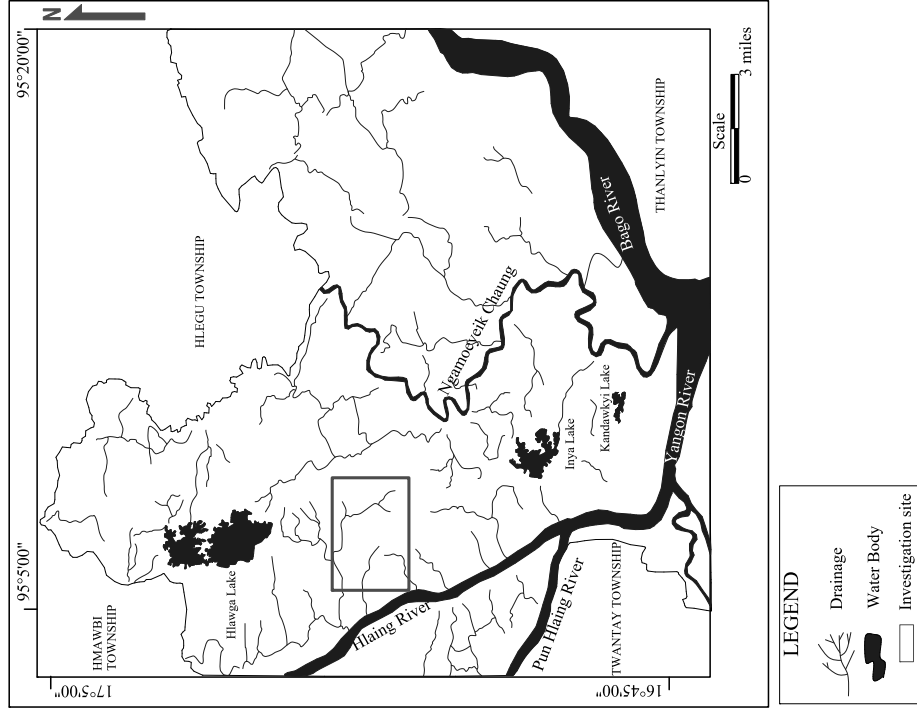


Figure (3.1) Drainage Map of Yangon Area

#### 4. Climate

Generally, the climate condition of Yangon is tropical monsoons climate with two seasons, the wet and dry. The wet condition occurs from mid May to mid October. The annual rainfall in this area is about 110 inches.

#### 5. Regional Geologic Setting

Yangon area includes deltaic land being southern continuation of Bago Yoma of north-south trending basin containing thick Tertiary and Quaternary deposits.

##### 5.1. Pegu Group (Tertiary)

Marine sandstone and shale deposits of Oligocene and Miocene belong to Pegu Group. The age of marine sandstones exposed at Thadugan was found to be Miocene (Win Naing 1972). This group includes three lithologic units, such as Hlawga shale, Thadugan sandstone and Besapet alteration. The Pegu group is exposed at the northern portion of Yangon area. Geologic map of the study area is shown in Figure (5.1).

##### 5.2. Irrawaddy Formation (Tertiary)

The continental and marine (?) deposits of the Pliocene belong to the Irrawaddy Formation. This formation includes two litho-stratigraphic units; they are Arzanigone sandstone and Danyingon clay.

##### 5.3. Arzanigone Sandstone

This formation is loose to very dense, generally unconsolidated to consolidated, sand rocks containing admixtures of silt, clay and fine gravel at various percentages. This formation gives moderate to fairly high yields of water. (Win Naing, 1972)

##### 5.4. Danyingon Clay

This formation is composed of clay with interbedded sand rock, exposed on the west side of Pyay road near Mingalardon Airport and at Mindama road. Well tapping this formation gives low yields because the aquifer is thin and permeability is low due to consolidation and admixture of silt and clay. (Win Naing, 1972)

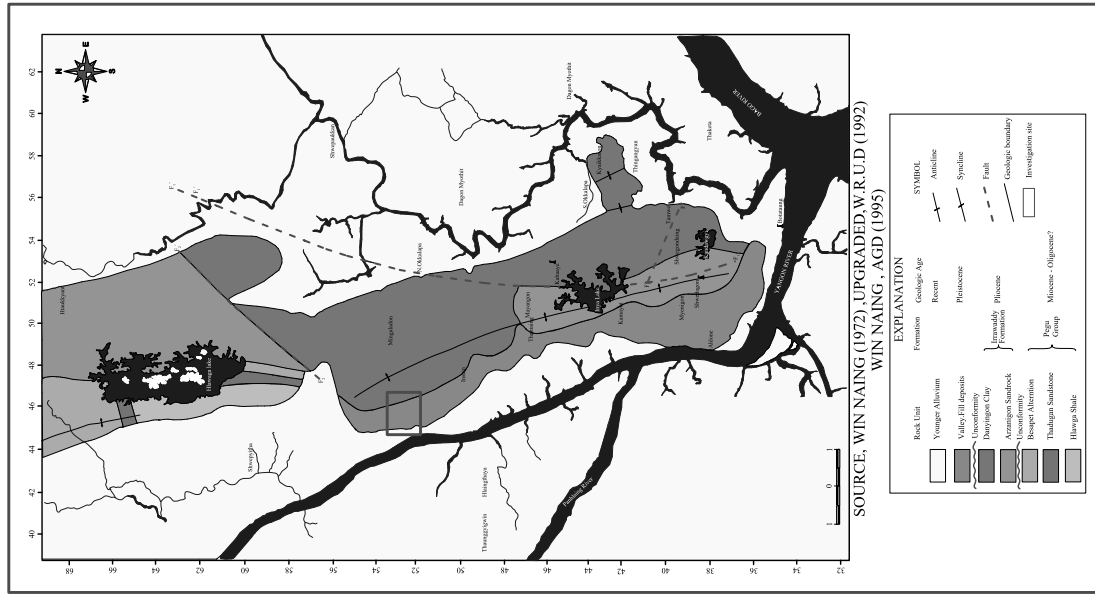


Figure (5.1) Regional Geology Map of the Yangon Area

## 6. Hydrogeologic Characteristics

### 6.1 Valley-Fill Deposit (Quaternary)

Valley-fill deposit occupies the synclinal valley west of Yangon ridge. They were probably deposited in Pleistocene and Sub-recent time as channel deposit. It consists of a thick sequence of loose, highly pervious, inter-bedded sand and fine to very coarse gravels. The Valley-fill deposits along Hlaing-Yangon river valley are the principal aquifer in the area.

Valley-fill deposits mainly consists of sands interbedded with fine to very coarse gravels and some clay and silt. It is a thick sequence of loose, highly pervious sediments.

At-least two water bearing horizons are found. Upper zone I is composed of yellow to light brown colour medium to coarse grained sand and gravel. The lower zone, zone II, is sand, silty sand and clayey sand. Light gray to light blue color clay unit separates these two water bearing horizons. Water bearing horizon depth (160ft to 200 ft or 48.8 m to 60.9m). Aquifer thickness is (12.2 m or 40ft).

## 6.2. Water Well Design

Collected the sample of the formation penetrated at each 3 meters interval. Describe the detail information were thoroughly checked and the detail logs of the well was presented and water well design of the investigation well is shown in Figure (6.1)

Table (6.1) Summary of Lithological Log of Ø 6" PVC Tube Well No (1)

Depth(ft)	Thickness		Lithology	Remark
	From	To		
0	10	10	Overburden, soil, sandy clay, yellow	
10	20	10	Sand, fine, yellow, clayey	
20	40	20	Sand, fine, yellow	
40	60	20	Sand, fine, yellow	
60	80	20	Sand, fine to medium, yellow	
80	100	20	Sand, medium to coarse, yellow	
100	120	20	Sand, coarse, dark yellow	
120	140	20	Sand, coarse, yellow	
140	160	20	Sand, coarse, yellow	
160	180	20	Sand, coarse, fine gravel, yellow	
180	200	20	Sand, coarse, yellow	
200	220	20	Sand, fine, dark yellow	
220	240	20	Sand, fine, dark yellow, bluish	
240	260	20	Sand, fine, dark grey	
260	280	20	Sand, medium, dark grey	
280	300	20	Sand, medium, blue	
300	320	20	Sand, fine, blue, slaty layer	
320	340	20	Clay, blue, sandy	
340	360	20	Clay, blue, sandy	
360	380	20	Sand, fine, dark blue	
380	400	20	Sand, fine, blue	
400	420	20	Sand, medium, blue	
420	440	20	Sand, medium, blue, slaty layer interbedded	
440	460	20	Sand, medium to coarse, dark blue	
460	480	20	Sand, medium, blue	
480	500	20	Sand, fine, blue, slaty layer interbedded	

**7. Hydraulic Characteristics of Aquifer**

**7.1 Aquifer Test / Well Test or Pumping Test**

The principle of a pumping test is that if used pump water from a well and measure the discharge of the well and the drawdown in the well. Constant discharge test has been done at Ø6" PVC well by using Ø4" submersible pump, HST, KSB Cora 18/9, 4HP, 18-20mH. During on test, the discharge was measured by 5 gallon capacity open bucket frequently to maintain the constant discharge. After (360minutes or 6hours) since pumping started the steady state condition was observed. After waiting for (18 hours), the pumping was not changed, therefore, after 24 hours pumping test, the pump was stopped and 24 hours recovery measurement test was done. It is shown in figure (7.2) and (7.5).

Pumping water level and recovery were already measured by using electric water level indicator. The computed transmissivity and hydraulic conductivity values of constant discharge test and recovery test are following.

**7.1.1 Constant – Discharge Pumping Test**

$$T = KD = \frac{2.3Q}{4\pi\Delta s}$$

Where,

T = Transmissivity (m<sup>2</sup>/day)

1330m<sup>2</sup>/day

Q = Constant well discharge rate (m<sup>3</sup>/day)

654m<sup>3</sup>/day

Δs = Drawdown difference per log cycle, (m)

0.09m

K = Permeability or Hydraulic Conductivity (m/d)

109m/day

**6.3. Well Log and Well Design**

The well log and well design of investigation well is shown in figure (6.1). Testing well is accommodated 6"Ø PVC casing pipe, 2"Ø PVC raiser pipe and submersible pump.

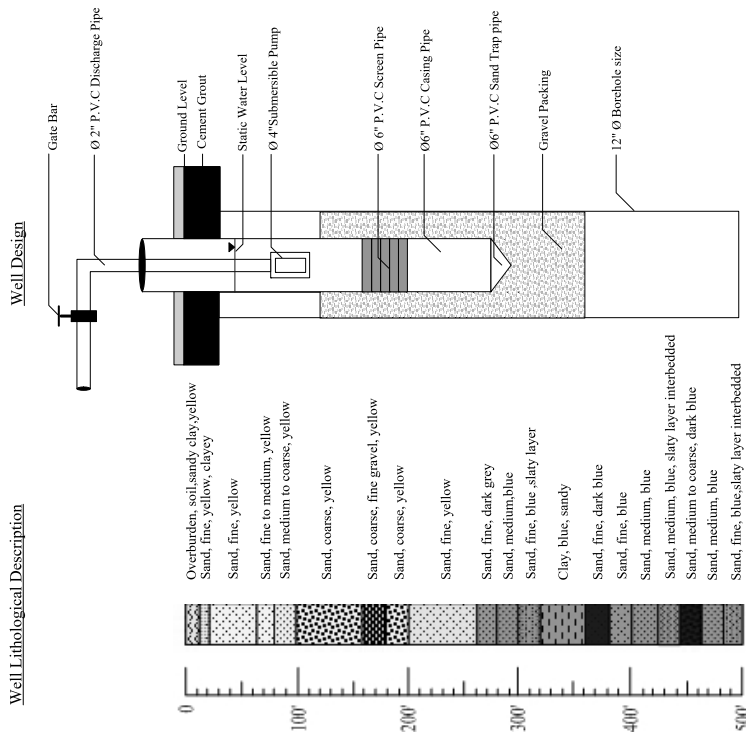


Figure (6.1) Well Log and Well Design of Investigation Site

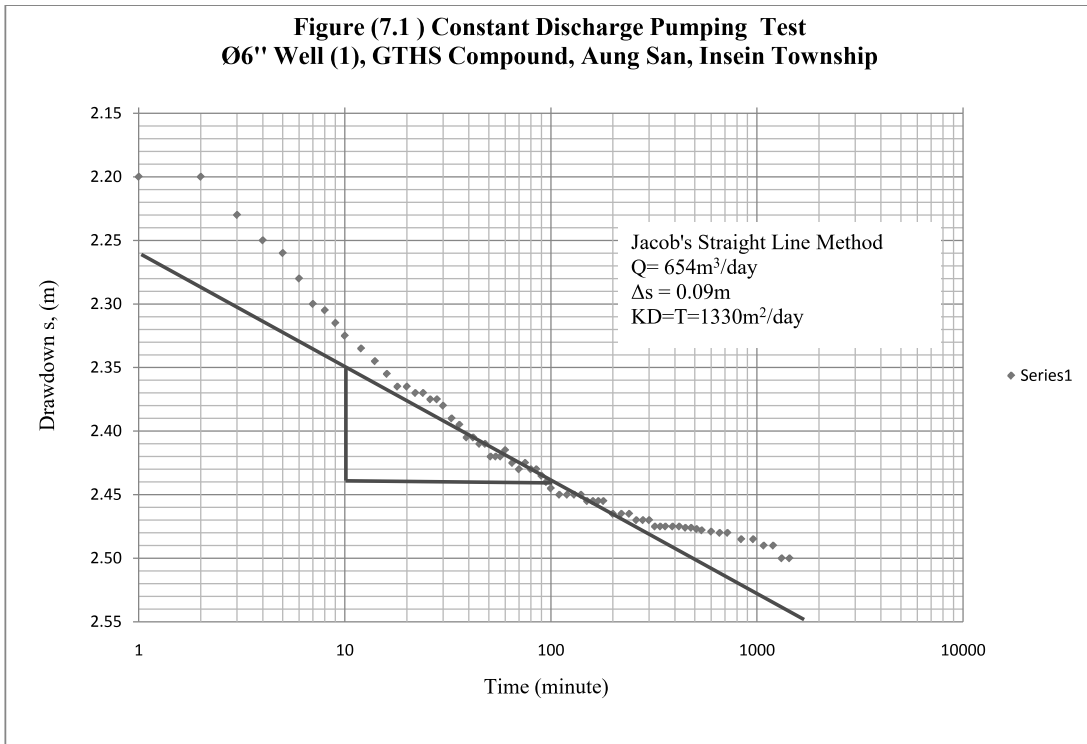


7.1.2 Constant Discharge Recovery Test

$$T = KD = \frac{2.3Q}{4\pi\Delta s'}$$

Where,

- T = Transmissivity (m<sup>2</sup>/day)  
1662m<sup>2</sup>/day
- Q = Constant well discharge rate (m<sup>3</sup>/day)  
654m<sup>3</sup>/day
- Δs' = Drawdown difference per log cycle, (m)  
0.072m
- K = Permeability or Hydraulic Conductivity (m/d)  
136 m/day



**7.2 Step-drawdown Test or Well Performance Test**

A step drawdown test is a single well test in which the well is pumped at a low constant discharge rate until the drawdown within the well stabilized. The pumping rate is then increased to a higher constant discharge rate and the well is pumped until the drawdown stabilized one more. This process repeated through at least three steps, which should be of equal duration, say 100 minutes. Drawdown is measured for successively increasing values of discharge such as 50%, 75% and 100% of maximum or 3000 gph, 4500 gph and 6000 gph of maximum.

The drawdown in pumped well consisting of two components, the aquifer loss and well loss. A step drawdown test is conducted to determine these losses. Excessive losses are caused by damaging of the aquifer during drilling and completion of the well. This test also shows the information regarding the relation between discharge and drawdown of the well, which is useful for selecting of optimum pump and depth of pumping. During pumping the water level measured by electric water level indicator. Figure (7.9)

Step drawdown test makes it possible to evaluate the parameter B (Aquifer loss) and C (Well loss). The well drawdown can be expressed as

$$S_w = BQ + CQ^2$$

Where,

$S_w$  = Drawdown in well

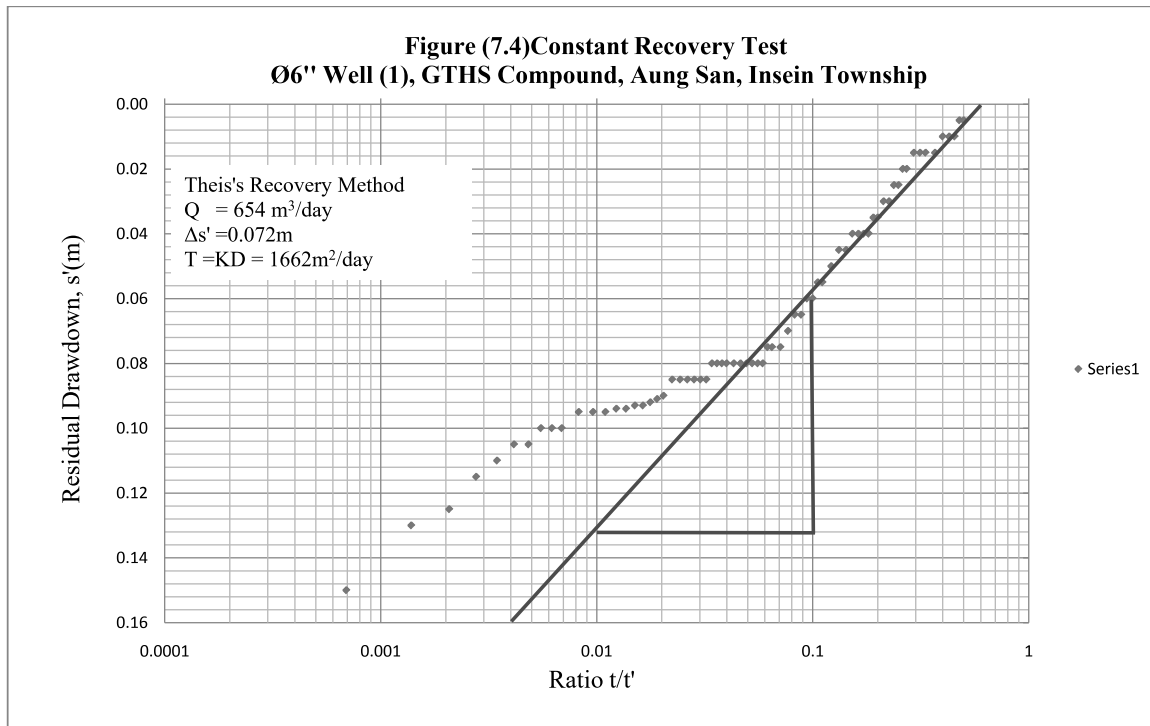
$Q$  = Well discharge

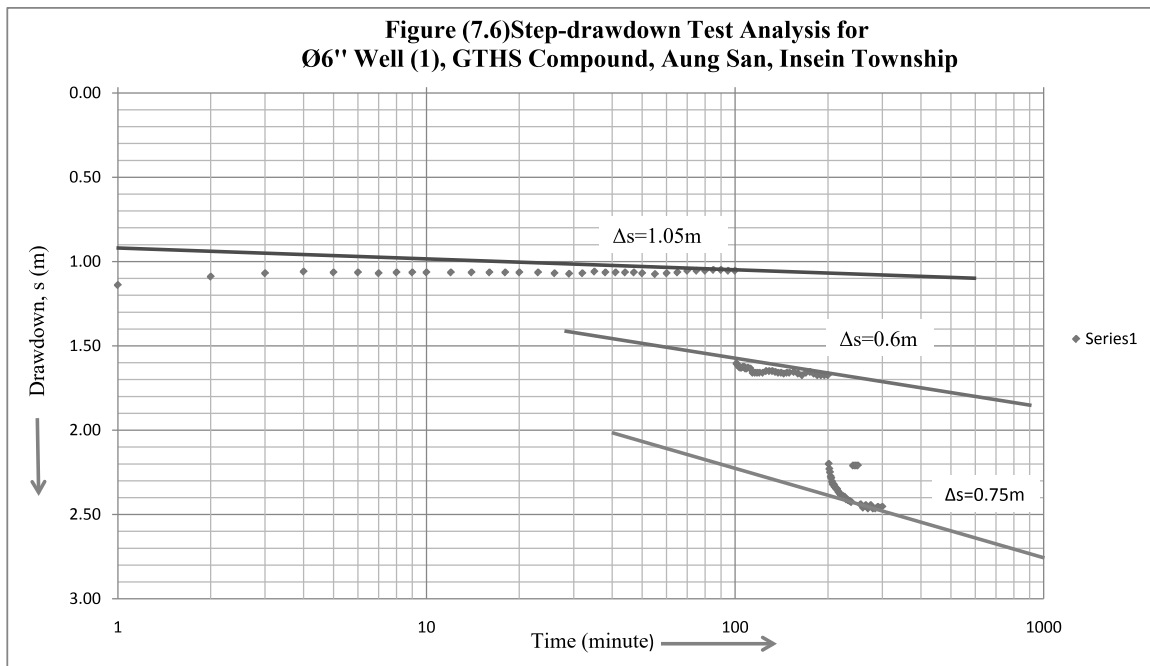
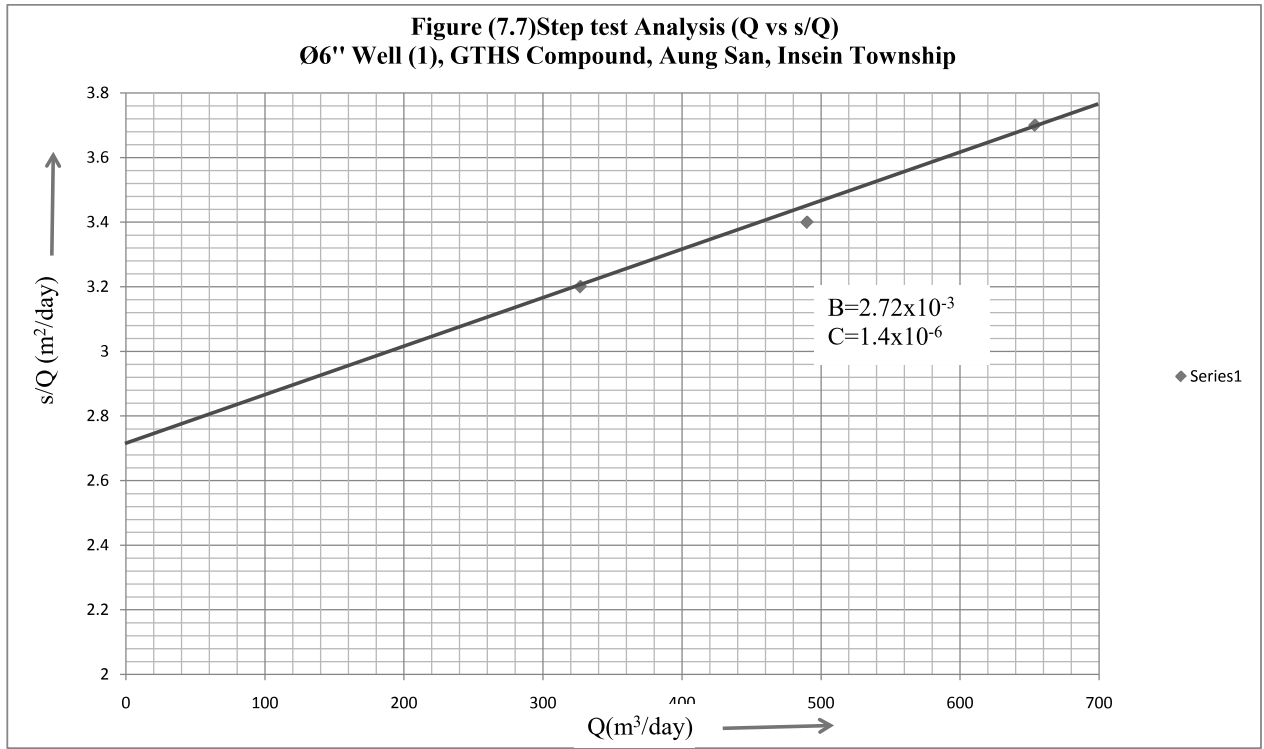
$B$  = Aquifer loss or Formation loss

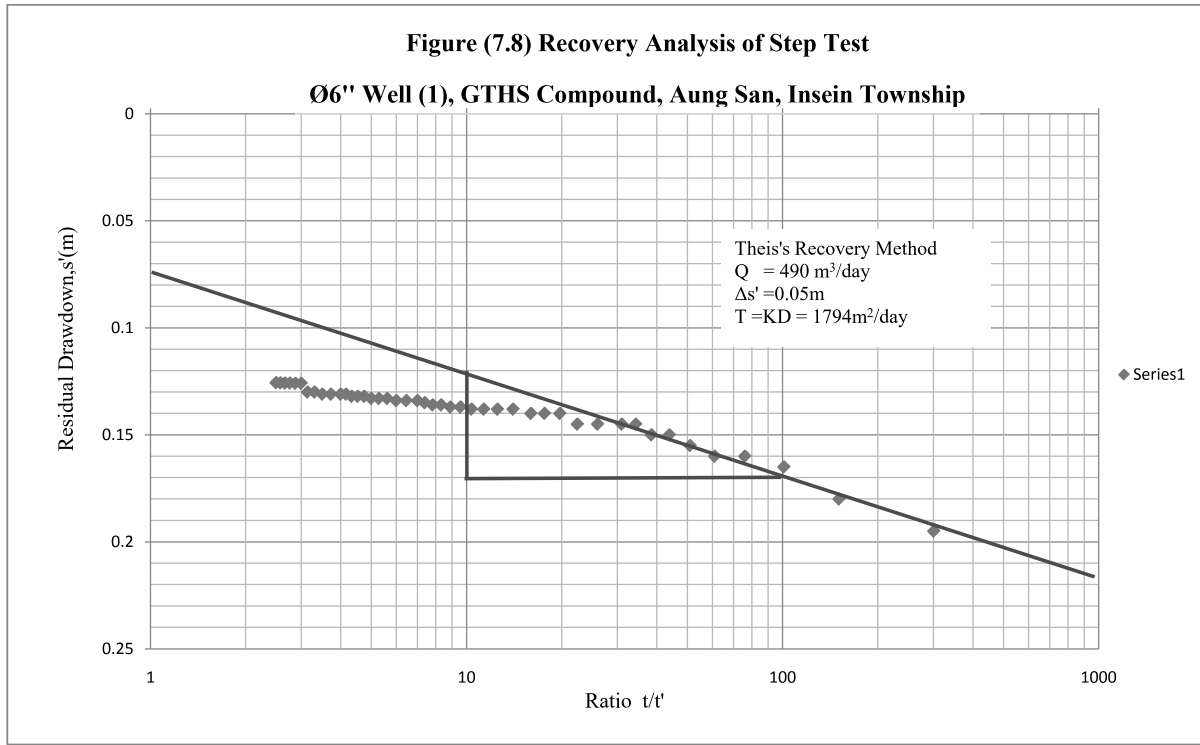
$C$  = Well loss

Table (7.1) Specific Drawdown Determined with the Hantush-BierschenkMethod: Step-drawdown Test

Step	Q (m <sup>3</sup> /d)	sw(m)	sw/Q
I	327	1.05	3.2x10 <sup>-3</sup>
II	490	1.65	3.4x10 <sup>-3</sup>
III	654	2.4	3.7x10 <sup>-3</sup>







**7.2.1 Recovery of Step Test**

$$T = KD = \frac{2.3Q}{4\pi\Delta s'}$$

Where,

T = Transmissivity (m<sup>2</sup>/day)

1794m<sup>2</sup>/day

Q = Constant well discharge rate (m<sup>3</sup>/day)

490m<sup>3</sup>/day

Δs' = Drawdown difference per log cycle, (m)

0.05m

K = Permeability or Hydraulic Conductivity (m/d)

147 m/day





Figure (7.9) Measurement step-drawdown Test at Investigation Site

**7.3 Discharge Rate with Various Step Test**

Shown in table (7.2)

Step	Discharge (Q)					
	g/hr	g/day	m <sup>3</sup> /hr	m <sup>3</sup> /day	L/hr	L/day
I	3000	72000	13	327	13638	327312
II	4500	108000	20	490	20457	490968
III	6000	144000	27	654	27276	654624

**7.4 Transmissivity and Permeability Determination from Various Tests.**

It is shown in table (7.3) and (7.4).

Table (7.3) Transmissivity Determination from Various Tests

Step	Q		Δs	T (m <sup>2</sup> /day)
	m <sup>3</sup> /hr	m <sup>3</sup> /d		
I	13	327	1.05	57.02
II	20	490	0.6	149.54
III	27	654	0.75	159.66
Average	20	490	0.8	122.07

Table (7.4) Transmissivity and Permeability Determination from Various Tests

No	Type of test	T (m <sup>2</sup> /sec)	K (m/sec)
1	Constant-discharge test	1.5x10 <sup>-2</sup>	1.22x10 <sup>-3</sup>
2	Recovery test (Constant-discharge)	1.8x10 <sup>-2</sup>	1.57x10 <sup>-3</sup>
3	Step I	6.27x10 <sup>-4</sup>	5.13x10 <sup>-5</sup>
	Step II	1.69x10 <sup>-3</sup>	1.38x10 <sup>-4</sup>
	Step III	1.83x10 <sup>-3</sup>	1.5x10 <sup>-4</sup>
4	Recovery test (step)	2.1x10 <sup>-2</sup>	1.72x10 <sup>-3</sup>
Average		9.69x10 <sup>-3</sup>	4.84x10 <sup>-3</sup>

$$T = KDK = T/D$$

Where:

T = Transmissivity (m<sup>2</sup>/sec)

K = Permeability (m/sec)

D = Water bearing horizon or aquifer thickness (m)

As a result the representative transmissivity and permeability ranges are from 6.27x10<sup>-4</sup> m<sup>2</sup>/sec to 2.1x10<sup>-2</sup> m<sup>2</sup>/sec and from 1.22x10<sup>-3</sup> m/sec to 5.13x10<sup>-5</sup> m/sec respectively. The average value of permeability is 4.84x10<sup>-3</sup> m/sec. It is indicated that the aquifer material is coarse sand. (Reference: Domenico and Schwartz 1990).

### 7.5 Well Efficiency

Express the relationship between drawdown and discharge as the specific capacity of a well,  $Q/sw$ , which describes the productivity of both the aquifer and the well. The specific capacity is not a constant but decreases as pumping continues ( $Q$ ) is constant and also decreased with measuring ( $Q$ ). The well efficiency,  $E_w$ , can be expressed as

$$E_w = \frac{BQ}{BQ+CQ^2} \times 100\%$$

$E_w$  = Well efficiency

$B$  = Aquifer loss coefficient

$C$  = Well loss coefficient

Table (7.5) Well Efficiency Determination from Various Steps

Step	$\Delta sw$ (m)	$sw$ (m)	$Q$ ( $m^3/d$ )	$sw/Q$	$BQ$	$CQ^2$	Well Efficiency(%)	Remark
I	1.05	1.05	327	$3.2 \times 10^{-3}$	0.889	0.149	85%	
II	0.6	1.65	490	$3.4 \times 10^{-3}$	1.332	1.336	79%	
III	0.75	2.4	654	$3.7 \times 10^{-3}$	1.778	0.598	74%	
						Average	79%	

Aquifer loss coefficient,  $B = 2.72 \times 10^{-3}$

Well loss coefficient,  $C = 1.4 \times 10^{-6}$

### 7.6 Pump Setting Depth

According to the step test analysis, the pump setting depth and its discharge are shown in the following table.

Table (7.6) Pump Setting Depth

Sr.no	Q		Well drawdown $sw=BQ+CQ^2$		Pump setting depth $P.S.D=sw+S.W.L$		Seasonal Fluctuation		Recommended P.S.D (m)
	$m^3/hr$	$m^3/d$	m	ft	m	ft	m	ft	
1	13	327	1.039	3.4	13.569	44	10	32	23.569
2	20	490	1.668	5.47	14.198	46	10	32	24.198
3	27	654	2.377	7.79	14.91	48	10	32	24.91
	S.W.L=12.53m from TOC		Average		14.23	47			24.22

Pump setting depth should be place (23.569 m or 77ft) depth from ground while the discharge is  $327m^3/day$  or 3000 gph.

8. Chemical Characteristics of Groundwater

Water sample was collected and sent to ISO Tech Laboratory. Lab results were presented as follow:

**ISO TECH LABORATORY**  
 Laboratory Technical Consultant: U Saw Christopher Mawng  
 B.Sc. Engg. (Civil), Dip. S.E. (Dist.) Lecturer of YIT (Recd), Consultant (Y.C.D.C.) LWSE 001.  
 Former Member (UNICEF, Water quality monitoring & Surveillance Myanmar)

**WTL-RE-001**  
 Issue Date - 01-12-2012  
 Effective Date - 01-12-2012  
 Issue No - 1.0 Page 1 of 2

**W0718 078**

**WATER QUALITY TEST RESULTS FORM**

Client: Geo Friend  
 Nature of Water: Tube Well Water (6 inches)  
 Location: GTHS (Insein)  
 Date and Time of collection: 4.7.2018  
 Date and Time of arrival at Laboratory: 4.7.2018  
 Date and Time of commencing examination: 5.7.2018  
 Date and Time of completing: 7.7.2018

**Results of Water Analysis**  
 (Geneva - 1993)

		WHO Drinking Water Guideline (Geneva - 1993)
pH	7.1	6.5 - 8.5
Colour (True)	20	15 TCU
Turbidity	42	5 NTU
Conductivity	micro S/cm	
Total Hardness	22	500 mg/l as CaCO <sub>3</sub>
Calcium Hardness	mg/l as CaCO <sub>3</sub>	
Magnesium Hardness	mg/l as CaCO <sub>3</sub>	
Total Alkalinity	mg/l as CaCO <sub>3</sub>	
Phenolphthalein Alkalinity	mg/l as CaCO <sub>3</sub>	
Carbonate (CaCO <sub>3</sub> )	mg/l as CaCO <sub>3</sub>	
Bicarbonate (HCO <sub>3</sub> )	mg/l as CaCO <sub>3</sub>	
Iron	0.73	0.3 mg/l
Chloride (as Cl)	9	250 mg/l
Sulphate (as SO <sub>4</sub> )	20	200 mg/l
Total Solids	mg/l	1500 mg/l
Suspended Solids	mg/l	
Dissolved Solids	81	1000 mg/l
Manganese	Nil	0.05 mg/l
Phosphate	mg/l	
Phenolphthalein Acidity	mg/l	
Methyl Orange Acidity	mg/l	
Salinity	ppt	

Remark: This certificate is issued only for the receipt of the test sample.

Tested by: Zaw Hein Oo  
 Signature: [Signature]  
 Name: B.Sc. (Chemistry)  
 ST. Chemist  
 ISO TECH LABORATORY

Approved by: [Signature]  
 Signature: [Signature]  
 Name: Soc Thit  
 B.E. (Civil) 1980  
 Technical Officer  
 ISO TECH LABORATORY

(a division of WEG Co., Ltd.)  
 No. 18, Lamit Road, Northgong Quarter, Insein Township, Yangon, Myanmar.  
 Ph: 01-640955, 09-7325175, 09-73242162, Fax: 01-644506, E-mail: isotechlaboratory@gmail.com, Website: weg-myanmar.com

**ISO TECH LABORATORY**  
 Laboratory Technical Consultant: U Saw Christopher Mawng  
 B.Sc. Engg. (Civil), Dip. S.E. (Dist.) Lecturer of YIT (Recd), Consultant (Y.C.D.C.) LWSE 001.  
 Former Member (UNICEF, Water quality monitoring & Surveillance Myanmar)

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 Issue Date - 01-12-2012  
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 Date and Time of commencing examination: 5.7.2018  
 Date and Time of completing: 7.7.2018

**Results of Water Analysis**  
 (Geneva - 1993)

		WHO Drinking Water Guideline (Geneva - 1993)
Temperature (°C)		°C
Fluoride (F)	0.4	mg/l 1.5 mg/l
Lead (as Pb)	Nil	mg/l 0.01 mg/l
Arsenic (As)	Nil	mg/l 0.01 mg/l
Nitrate (N.NO <sub>3</sub> )	0.3	mg/l 50 mg/l
Chlorine (Residual)		mg/l
Ammonia (NH <sub>3</sub> )		mg/l
Ammonium (NH <sub>4</sub> )		mg/l
Dissolved Oxygen (DO)		mg/l
Chemical Oxygen Demand (COD)		mg/l
Biochemical Oxygen Demand (BOD) (5 days at 20 °C)		mg/l
Cyanide (CN)	Nil	mg/l 0.07 mg/l
Zinc (Zn)	Nil	mg/l 3 mg/l
Copper (Cu)	Nil	mg/l 2 mg/l
Silica (Si)		mg/l

Remark: This certificate is issued only for the receipt of the test sample.

Tested by: Zaw Hein Oo  
 Signature: [Signature]  
 Name: B.Sc. (Chemistry)  
 ST. Chemist  
 ISO TECH LABORATORY

Approved by: [Signature]  
 Signature: [Signature]  
 Name: Soc Thit  
 B.E. (Civil) 1980  
 Technical Officer  
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(a division of WEG Co., Ltd.)  
 No. 18, Lamit Road, Northgong Quarter, Insein Township, Yangon, Myanmar.  
 Ph: 01-640955, 09-7325175, 09-73242162, Fax: 01-644506, E-mail: isotechlaboratory@gmail.com, Website: weg-myanmar.com

### 8.1. Water Quality

According to the Laboratory result, water quality of the presumed depth showed good condition for drinking purpose except of iron. The measuring of pH, EC and TDS during the step test period is shown in table (8.1).

Table(8.1)The Measured(pH, EC and TDS) Result in Pumping Period.

Step	EC(µmho/cm)	TDS(mg/l)	pH	Temperature(°C)
I	150	75	6.8	27
II	94	47	6.8	27
III	75	37	6.8	27

The classification of the water type based upon the chemical analysis of the water from the well. According to the results the major water type is  $Ca^{2+}$ - $Mg^{2+}$ -Cl type,  $Ca^{2+}$ - $Mg^{2+}$  dominant Cl<sup>-</sup> type, or Cl<sup>-</sup> dominant  $Ca^{2+}$ - $Mg^{2+}$  type waters. (Reference: D.K.Chada (1999)). Shown in table (8.2). Compare with Groundwater in the investigation site and W.H.O Drinking Water Quality Standard, (2011) is shown in table (8.3)

Table (8.2) Chemical Analysis Result of Investigation Site

Well	TDS	EC	pH	TH	Major Cations					Major Anions			Unit	
					Na	K	Ca	Mg	Fe	Cl	SO <sub>4</sub>	HCO <sub>3</sub>		CO <sub>3</sub>
Insein	81	40.5	7.1	22	-	-	15.72	10.68	0.73	2.3	19.81	26	ND	ppm
					-	-	0.79	0.89	0.02	0.65	0.41	0.43	ND	Epm

Table (8.3) Compare with Groundwater in the Investigation Site and W.H.O Drinking Water Quality Standard, (2011)

Characteristics	Guideline Values		The Obtained value of ground water in investigation site
	Max Permissible		
Calcium	200 mg/l		15.72mg/l
Magnesium	200 mg/l		10.68mg/l
Sodium	200 mg/l		-
Potassium	200 mg/l		-
Sulphate	600 mg/l		19.81mg/l
Bicarbonate	600 mg/l		26mg/l
Chloride	600 mg/l		2.3mg/l
Iron	2mg/l		0.73mg/l
pH	6.5-9.2		7.1
Hardness (CaCO <sub>3</sub> )	500 mg/l		22mg/l
EC	1500 µmhos/cm		40.5 µmhos/cm
TDS	1500 mg/l		81mg/l

### 9. Monitoring Program

Regular monitoring of table well is conducted with a view o detect any decrease in well performance and pump efficiency.

The follow basic observations have to be made for proper well monitoring.

1. Operating hour
2. Power consumption
3. Discharge rate
4. Static water level
5. Pumping water level
6. Water quality
7. Sand content

This regular monitoring can enhance to a prolonged well life.

### 10. Recommendation and Conclusion

If a person need 20 gallons per day of water the daily requirement for the training center, which have 240 peoples may need 4800 gallon per day. Maximum yield of recent well is 6000 gallon per day, when 3 hours pumping is done with the rate of 3000 gallon per day, above the requirement will be fulfilled. Maximum drawdown in the well is occurs 15.03m or 49ft of 24 hours pumping.

As a result the representative transmissivity and permeability ranges are from  $6.27 \times 10^{-4} \text{m}^2/\text{sec}$  to  $2.1 \times 10^{-2} \text{m}^2/\text{sec}$  and from  $1.22 \times 10^{-3} \text{m}/\text{sec}$  to  $5.13 \times 10^{-5} \text{m}/\text{sec}$  respectively. The average value of permeability is  $4.84 \times 10^{-3} \text{m}/\text{sec}$ . It is indicated that the aquifer material is coarse sand. (Reference: Domenico and Schwartz 1990).

According to the chemical analysis water from valley filled are chemically portable for drinking purpose by slightly high iron content is noticed.

The fluctuation may be due to the influence of pumping of the nearby wells.

**Ministry of Education  
The Republic of Myanmar**

**QUALITY IMPROVEMENT IN TVET PROGRAM IN MYANMAR**

**Japan -Myanmar  
(Aung San)  
Vocational Training Institute**

**ENVIRONMENTAL MANAGEMENT PLAN**

**Yangon, April, 2019**



**Resource and Environment Myanmar Ltd.**  
B 702 Delta Plaza, Shwegondaing Rd., Bahan, Yangon, Myanmar  
Tel: (959) 73013448; Fax: (951) 552901; [admin@enviromyanmar.net](mailto:admin@enviromyanmar.net)

**DECLARATIONS**

DECLARATION - EIA Experts

**Resource & Environment Myanmar Co., Ltd. (REM);** a local environmental consultant firm, conducted environmental impact assessment and prepared EMP report for Japan-Myanmar Aung San Vocational Training Institute in compliance with EIA Procedure and other relevant laws/rules and formally submitted to the Environmental Conservation Department (ECD) for final approval.

We do state, to the best of our knowledge at the time of report preparation, that

- To our knowledge, all information contained in this report is accurate and a truthful representation of all findings as relating to the project, and;
- The EMP Report has been prepared in strict compliance with all applicable laws, rules regulations and procedure in force.

We also consulted to Japan-Myanmar Aung San Vocational Training Institute to undertake that;

Japan-Myanmar Aung San Vocational Training Institute in respect of the “**QUALITY IMPROVEMENT IN TVET PROGRAM in Myanmar**” will at all times comply fully with (1) any and all commitments and obligations as set forth in the EMP Report which has been reviewed by Review Team, and (2) any and all plans and the various components thereof, including without limitation, impact avoidance, mitigation, and remediation measures, and with respect to such commitments, obligations, plans and measures related to the development, construction, commissioning, operation and maintenance of the project, and any circumstance in which work done or to be done, or services performed or to be performed, in connection with the project’s development.

Signed: (Zaw Naing Oo) Date: 12 -04-2019

Director

For: **Resource & Environment Myanmar Co., Ltd. (REM)**



Date: 12<sup>th</sup> April, 2019

Director General  
Ministry of Natural Resources and Environmental Conservation  
Office No. (53), Otrathiri Township,  
Nay Pyi Taw, Myanmar.

We refer to the captioned EMP, which was prepared and finalized by third party, Resource and Environment Myanmar Company Limited (REM Co., Ltd.) in accordance with the Environmental Conservation Law, Rules and Procedures under the instructions of Ministry of Natural Resources and Environmental Conservation and formally submitted by Environmental Conservation Department to Ministry of Natural Resources and Environmental Conservation.

Intending to be legally bound hereby and financially liable to the Ministry of Natural Resources and Environmental Conservation hereunder, we;

- a) Endorse and confirm to Ministry of Natural Resources and Environmental Conservation the accuracy and completeness of the EMP.
- b) Confirm and undertake to Ministry of Natural Resources and Environmental Conservation that the EMP has been prepared in strict compliance with applicable Environmental Conservation Law, Rules and Procedures and
- c) Confirm and undertake to Ministry of Natural Resources and Environmental Conservation that the project in established the (Japan-Myanmar Aung San Vocational Training Institute) in respect of the (Quality Improvement in TVET Program in Myanmar) shall at all times comply fully with: (i) any and all commitments and obligations as set forth in the EMP, and (ii) any and all plans and the various components thereof, including without limitation, impact avoidance, mitigation, and remediation measures, and with respect to both (i) and (ii), including but not limited to such commitments, obligations, plans and maintenance of the project, and any circumstance in which work done or to be done, or mine development, operation and closure of the project, and any circumstance in which work done or to be done, or services performed or to be performed, in connection with the project's development, operation and closure is carried out or intended or required to be carried out or intended or required to be carried out by any contractor, subcontractor or other party.

The issuance of this confirmation and undertaking has been duly authorized by all necessary corporate actions and a copy of the resolution of the Project Management Institution authorizing it and the power of attorney explicitly granting signing authorization to the individual who has signed below are attached as schedules hereto.

Dr. Yan Naing Tun  
Principal

Japan-Myanmar Aung San Vocational Training Institute  
DTVET, Ministry of Education (MOE), Myanmar

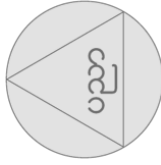


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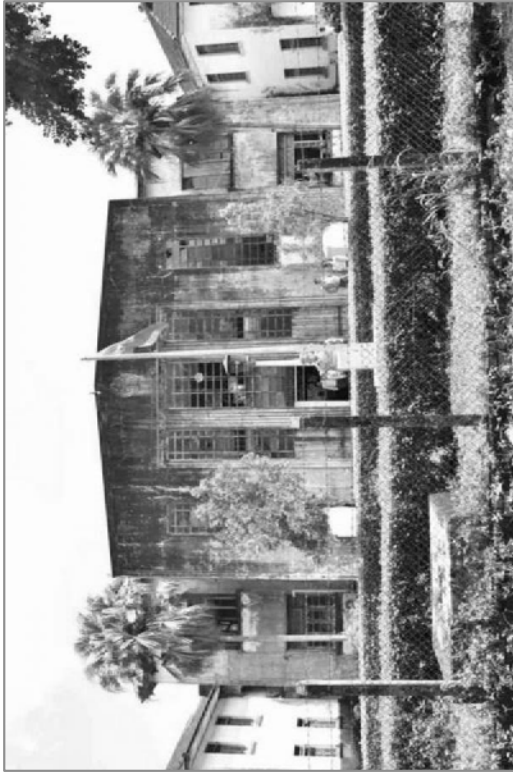
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## EXECUTIVE SUMMARY

This EMP report was prepared for the establishment of Japan-Myanmar Aung San Vocational Training Institute JMAYVTI project by Resource and Environment Myanmar (REM) consultant firm for the preparatory management plans of environmental and social impacts which can be emerged along with the construction and operation period.

The Japan International Cooperation Agency (JICA) has signed an agreement with the government to establish and fund the Japan-Myanmar Aung San Vocational Training Institute in December 2018 which will be operated under the operation of Department of Technical and Vocational Education and Training.



**Figure 1-1: Present Aung San Technical High School**

### 1.1 Project Structure

This project is planned to operate the 3 Years AGTI Diploma courses:

1. Automobile Technology (Maintenance)
2. Electrical Engineering (Industrial)

The overall project will include;

- a) Pre-Designation
- b) Submission of Layout Plan and Internal Approval
- c) Bidding of Demolition work
- d) EMP
- e) UXO Detection and Clearance Work
- f) Building Permit
- g) Relocation of Buildings

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## 1.2 Project Proponent

Department of Technical and Vocational Education and Training, Ministry of Education, the Republic of the Union of Myanmar.

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## 1.3 Consultant Profile

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Contact Person	: Mr. Thura Aung
Designation	: General Manager

## EMP Team Members

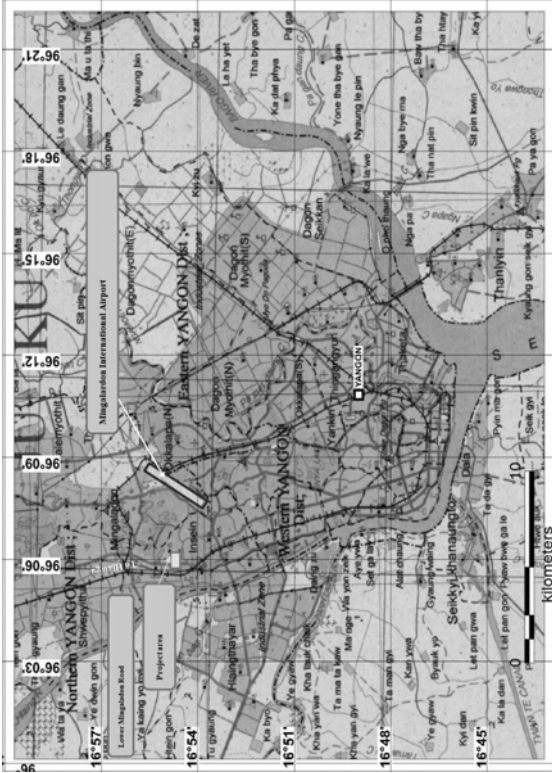
The EMP study team comprises members who have been involved in numerous environmental related studies. The personnel are well trained and qualified in their respective field. Please refer to Table 1.3-1 for details on members in the study team.

**Table 1.3-1: EMP Consultant Team**

No.	Name	Position	Responsibility
1.	U Zaw Naing Oo	Principal Consultant	Environmental Management Plan
2.	U Thura Aung	Principal Consultant	Physical Environment
3.	Daw Lai Lai Win	Environmental Consultant	Ecology, Environmental and Social Impact Assessment and Reporting
4.	U Kyaw Zin Win	Principal Consultant	GIS
5.	Daw Phyu Phyu Shein	Social Consultant	Socioeconomic
6.	Myat Ko Ko Hein	Junior Consultant	Ecology
7.	De Hlaing Zaw	Environmental Technician	Physical Environment

## 1.4 Project Layout

- 1) Establishing a model TVET institute
- 2) Opening of new diploma courses of Automobile Technology (Maintenance) and Electrical Engineering (Industrial)



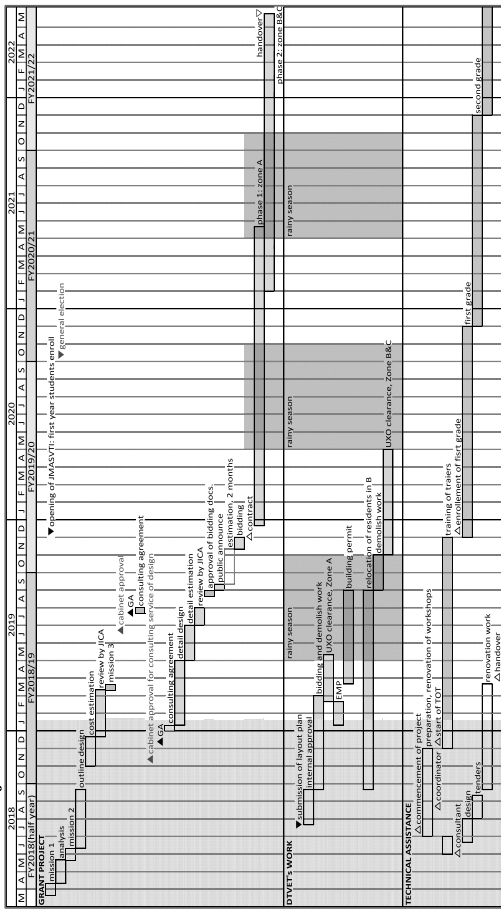
**Figure 1.4-1: Location Map of the Project Area**

The Japan-Myanmar Aung San Vocational Training Institute (JMASVTI) will be established on the area of 57121 m<sup>2</sup> (14.115 Acres) with the construction of institutional buildings and hostel for students. The institute is planned to operate with about the existing 240 students from the 1<sup>st</sup>, 2<sup>nd</sup> and 3<sup>rd</sup> academic years and 30 for teachers and administrative staffs. The total floor area is 9620 m<sup>2</sup> (103,548.8 ft<sup>2</sup>) in the building area of 6974 m<sup>2</sup>. The tentative project construction period is December 2019 to May 2022. The project is a Japan-Myanmar collaborative program and will be funded by the Japan International Cooperation Agency (JICA).

A number of buildings will be constructed at the proposed area as the following;

Building	Components	Story Type	Area (m <sup>2</sup> )
A	Main Entrance, Administration, Electrical, Workshop, Class Room	2 story Building	2,953
B	Assembly Hall, Administration, Library, Class room	2 story Building	2,020
C	Automobile Workshop	1 story Building	1,848
D	Male Hostel (80 persons)	2 story Building	678
E	Female Hostel (40 persons)	2 story Building	1,146
F	Canteen	1 story Building	654
G	Guard Room	1 story Building	20
H	Machine Room (Electrical)	1 story Building	143
I	Water Tower 1	1 story Building	69
J	Water Tower 2	1 story Building	79
K	Machine Room (for septic tank)	1 story Building	10
Other	Septic Tank	-	2 units

### Tentative Project Schedule



### 1.5 Baseline Studies

Physical and ecological baseline surveys were conducted in February 2019.

#### 1.5.1 Physical Environmental Survey: Ambient Air Quality

Ambient air quality and noise survey was conducted in two points in Aung San Technological High School.

Air Quality & Meteorology	Parameter	1) Nitrogen dioxide, 2) CO, 3) particulate Matter PM <sub>10</sub> , 4) Particulate Matter PM <sub>2.5</sub> Sulfur Dioxide, 6) Relative Humidity, 7) Temperature, 8) Wind Speed, and 9) Wind Direction
Period	Two points within 24 hours continuous	
Location	Aung San Technological High School	
Noise Level	Parameter	L <sub>Aeq</sub> (A-weighted loudness equivalent)
Period	One time at one location within one day	
Location	Within Aung San Technological High School	

The survey results for ambient air quality were presented in the following table.

Sampling No	Time	CO	NO	PM <sub>10</sub>	PM <sub>2.5</sub>	RH	SO <sub>2</sub>	Temp
AQN-1	24hrs	671.42	0.00	8.75	2.93	60.54	30.34	25.15
AQN-2	24hrs	641.40	0.00	8.29	2.55	62.18	24.11	25.97
WHO 24-hour (Interim Target 1)	24hrs	-	-	150	75	-	125	-
NEQG Standard (Myanmar)		-	-	50	25	-	20	-

### Hourly Nitrogen Dioxide Concentration

AQN-1	AQN-2	unit	NEQG Myanmar Standard (1 Hour)
141.85	3.76	µg/m <sup>3</sup>	200
182.84	3.76	µg/m <sup>3</sup>	
28.66	3.76	µg/m <sup>3</sup>	
43.37	14.61	µg/m <sup>3</sup>	
17.47	38.69	µg/m <sup>3</sup>	
47.94	77.95	µg/m <sup>3</sup>	
82.84	97.93	µg/m <sup>3</sup>	
85.98	95.73	µg/m <sup>3</sup>	
82.50	106.17	µg/m <sup>3</sup>	
76.98	104.39	µg/m <sup>3</sup>	
74.75	106.39	µg/m <sup>3</sup>	
74.88	107.40	µg/m <sup>3</sup>	
83.00	108.40	µg/m <sup>3</sup>	
73.94	103.60	µg/m <sup>3</sup>	
76.60	110.44	µg/m <sup>3</sup>	
80.55	96.80	µg/m <sup>3</sup>	
79.90	102.60	µg/m <sup>3</sup>	
77.67	105.11	µg/m <sup>3</sup>	
76.76	90.49	µg/m <sup>3</sup>	
69.14	14.80	µg/m <sup>3</sup>	
40.39	3.86	µg/m <sup>3</sup>	
3.76	3.76	µg/m <sup>3</sup>	
3.76	3.76	µg/m <sup>3</sup>	
3.76	3.76	µg/m <sup>3</sup>	

According to the survey results, the SO<sub>2</sub> level (24-hr) is slightly higher than the national emission guideline, NEQG. The reasons of higher SO<sub>2</sub> level would be associated with vehicular emissions due to the sampling points are closed to Lower Mingalardon road, which was fully operated with numerous vehicles all the time.

#### 1.5.2 Physical Environmental Survey: Noise

Ambient noise survey was also conducted at the same points of ambient air quality survey during day-time (7:00 – 22:00 hrs) and night-time (22:00 – 7:00 hrs). The following table showed the results of noise levels at the sampling points.

**Table 1.5-1: A-Weighted Loudness Equivalent Noise Level**

Sampling Points Result	N-1		N-2	
	Day time	Night time	Day time	Night time
NEQG standard (Residential and institutional area) WHO for specific environment, Industrial, commercial, shopping and traffic area, indoor and outdoor	67	57	55	45
	55	45	55	45
				70

recorded as endemic species only. During the survey period, species of herpetofauna and mammal were observed fewer than bird and butterfly species.

### 1.6 Impact Assessment and Mitigation Measures

The proposed project activities will be categorized into 3 main phases;

Phase-1(P1): Pre-construction

Phase-2(P2): Construction

Phase-3(P3): Operation

For the categorization of qualitative impact assessments, the impacts are classified as;

**A** : **Significant**

**B** : **Low**

**C** : **Insignificant**

**D** : **Negligible**

According to the survey results, the noise levels from both points are also above the national emission standard values of residential area. The proposed project area in current situation is likely as the industrial, commercial, shopping and traffic area, including with indoor and outdoor activities. World Health Organization (WHO) guideline value for community noise in specific environments is 70 dB for such kind of area with those activities.

### 1.5.3 Water Resources

The temporary disposal area might be impacted to the ground water quality nearby (a tube well), which was located at 16°55'05.16" N and 96° 05' 55.74" E.

According to the water quality results as shown in the following table (seen in Annex 5-1: Water Analysis Results), the tube well water is not suitable for drinking due to its high concentration of iron, color and turbidity, but suitable for general construction works.

**Table 1.5-2: Groundwater Quality Results**

Characteristics	WHO Guideline Values (Drinking Water)	Tube Well Water
Sulphate	200 mg/l	20 mg/l
Chloride	250 mg/l	9 mg/l
Iron	0.3 mg/l	0.73mg/l
Manganese	0.05 mg/l	Nil mg/l
pH	6.5-8.5	7.1
Total Hardness (CaCO3)	500 mg/l	22mg/l
EC	1500 $\mu$ mhos/cm	40.5 $\mu$ mhos/cm
TDS	1500 mg/l	81mg/l
Cyanide (Cn)	0.07 mg/l	Nil mg/l
Copper (Cu)	2 mg/l	Nil mg/l
Zinc (Zn)	3 mg/l	Nil mg/l
Lead	0.01 mg/l	Nil mg/l
Arsenic	0.01 mg/l	Nil mg/l
Dissolved solid	1000 mg/l	81 mg/l
Turbidity	5 NTU	42 NTU
Color	15 TCU	20 TCU
Fluoride	1.5 mg/l	0.4 mg/l
Nitrate	50 mg/l	0.3 mg/l

### 1.5.4 Ecological Baseline Survey

A targeted site reconnaissance and baseline data collection were conducted during February 2019. Aerial imagery was used to build a more complete spatial understanding of the pattern of vegetation communities and human uses on the site, and to map access routes and internal tracks.

A total of 37 plant species near fence (Northern and Southern Section), 39 plant species in the Area-A and 51 plant species in the Area-C, 4 species of Mammals, 4 species of Herpet, 17 species of Birds and 8 species of Butterflies were recorded during the survey period. In this survey, one Bird species

				<p>material transport routes, especially, the poor buildings which could be destroyed by vibration.</p> <ul style="list-style-type: none"> <li>- Long-term noise exposure will reduce hearing and labor productivity, and will cause fatigue, stress, and insomnia.</li> </ul>	<p>in action</p> <ul style="list-style-type: none"> <li>- Maintain machinery and equipment in good conditions</li> <li>- Maintain an active community consultation and positive relations with residents that will assist in alleviating concerns that might arise and resolve any potential noise complaints</li> <li>- Post warning signs within the vicinity of the impact and all personnel shall be provided with personal protective equipment. For example, workers operating equipment that generates noise should be equipped with the appropriate noise protection gear; and</li> <li>- Restrict the construction activities that will generate disturbing sounds to normal working hours</li> </ul>
<b>Water Resources and quality</b>	<b>C</b>	<b>C</b>	<b>B</b>	<ul style="list-style-type: none"> <li>- Degradation of water quality due to inappropriate management of construction wastes and domestic waste from camp site.</li> <li>- Impacts on groundwater quality as a result of construction activities such as deep foundation and piling works, and discharges.</li> <li>- Wastewater discharge from workshop activities during operation phase.</li> </ul>	<ul style="list-style-type: none"> <li>- No storage for fuel and lubricants/oil</li> <li>- regular maintenance and checking of all vehicles and machinery to minimize the risk of fuel or lubricant leakages</li> <li>- As construction activities typically generate disturbed soil, concrete fines, oils and other waste, on-site collection and settling of storm water, prohibition of equipment wash downs, and prevention of soil loss and toxic releases from the construction site are necessary to minimize water pollution; and</li> <li>- Training and equipping relevant staff in protected storage and handling practices, and rapid spill response and clean up techniques</li> <li>- Preparing proper sewage system/Use portable toilet for construction workers</li> </ul>
<b>Soil</b>	<b>B</b>	<b>B</b>	<b>D</b>	<ul style="list-style-type: none"> <li>- Clearance of Trees and UXO detection might affect soil erosion.</li> <li>- The accidental spillage of oil from vehicles used for transportation of construction material and accidental spillage from the building material used for construction purposes are also considered as soil contamination sources.</li> </ul>	<ul style="list-style-type: none"> <li>- Prevent soil contamination by oil or grease spills, leakages or releases, all manipulations of oil derivate in the process of construction and provision of fuel to the machines should be performed with maximum attention</li> <li>- Leak proof containers should be used for storage and transportation of oil/grease and wash off from the oil/grease handling area shall be drained through drains and treated properly before disposal</li> <li>- Construction waste and debris shall be collected on a regular basis,</li> </ul>

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**Table 1.6-1: Impact Assessment and Mitigations**

Impact Parameter	Evaluation of Impacts			Impacts/Sources	Mitigation Measures
	P1	P2	P3		
<b>Air Quality</b> <ul style="list-style-type: none"> <li>▪ PM<sub>10</sub></li> <li>▪ PM<sub>2.5</sub></li> <li>▪ SO<sub>2</sub></li> <li>▪ NO<sub>2</sub></li> </ul>	<b>C</b>	<b>B</b>	<b>B</b>	<ul style="list-style-type: none"> <li>- Increases in air pollutants caused by fugitive dust from foundation work, site excavation, and emissions from operation of vehicles and trucks and heavy construction equipment.</li> <li>- Occupational health concern for construction workers and community health lived in the closed surroundings of the construction site are expected.</li> </ul>	<ul style="list-style-type: none"> <li>- Contract with the license contractors for compliance of environmental management consistence with the concerned government authorized department</li> <li>- Sprinkling of water on dust generating areas</li> <li>- Restricting the speed limits of vehicles during movement on unpaved roads</li> <li>- Covering of vehicles carrying loose soil/construction material</li> <li>- Applying preventive maintenance system</li> <li>- Checking vehicle and equipment inspection daily</li> <li>- Stopping dust generating activities in high wind</li> <li>- Applying good site practice and housekeeping</li> <li>- Turning off the engine while not in use</li> <li>- Optimizing construction schedule to minimize time that vehicles are in operation</li> <li>- Covering load-carrying platform properly when carrying earth/sand</li> <li>- Vehicle engines and other machinery will be kept turned on only if necessary, avoiding any unnecessary emission</li> <li>- Activities will be conducted trying to use the minimum required number of means at the same time</li> <li>- Electric small-scale mechanization and technical tools will be used when available and feasible; and</li> <li>- Repair and maintenance of construction equipment and vehicles will be performed outside of the construction site by at specialized enterprises</li> </ul>
<b>Noise and Vibration</b>	<b>C</b>	<b>A</b>	<b>B</b>	<ul style="list-style-type: none"> <li>- Increase ambient noise level at the construction site, and communities near the</li> </ul>	<ul style="list-style-type: none"> <li>- Select adequate equipment (fit with noise mufflers)</li> <li>- Minimize machinery and equipment unused conditions with engines</li> </ul>

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				<ul style="list-style-type: none"> <li>- the clearing boundaries in the project site</li> <li>- Unnecessary cleaning the trees is to avoid</li> <li>- Environmental awareness training to be given to all workers for the preservation of local biodiversity species and induct the nature of the sensitivity of project area</li> <li>- Works areas in temporarily affected areas shall be reinstated with tree/shrub/ grass upon completion of the works</li> </ul>
<b>Traffic loads</b>	<b>D</b>	<b>B</b>	<b>C</b>	<ul style="list-style-type: none"> <li>- Heavy vehicle movements during construction phase are highly expected to transport construction machinery.</li> <li>- Definition of speed limits and make sure that they are respected by Project drivers (including contractors)</li> <li>- Adopt a Traffic Management Plan to ensure traffic safety, which should foresee safe drive trainings, regular alcohol and drug tests for drivers and driving restrictions during rush hours (especially close to schools)</li> </ul>
<b>Aesthetic view</b>	<b>A</b>	<b>B</b>	<b>D</b>	<ul style="list-style-type: none"> <li>- Clearance of Trees and demolition Activities during preconstruction phase and new buildings during construction phase can be unfamiliar and affect loss of aesthetic view to the surrounding community.</li> <li>- No introduce the vertical structures which can be overseen from various parts of the region</li> <li>- Adopt the control measures during the detailed design of the project such as building design, and growing vegetation, etc.</li> <li>- Color for project facilities should be carefully selected. Lighter color can be utilized to complement the surrounding areas. Where technically feasible, to decrease the visibility of facilities, plantation around the building should be planned</li> </ul>
<b>Occupational Health and Risk</b>	<b>C</b>	<b>B</b>	<b>D</b>	<ul style="list-style-type: none"> <li>- UXO detection works during pre-construction phase.</li> <li>- The construction dust and noise emissions will be affected to the construction workers.</li> <li>- Adopting and training all personnel (including contractor workers) in the use of Personal Protective Equipment (PPE) and chemical handling</li> <li>- Training in recognition of hazard symbols</li> <li>- Adoption of work site hazards signage in Myanmar language</li> <li>- Training of all personnel in health and safety risk prevention and protection</li> <li>- Regular noise surveys to ensure the on-site maximum levels are not exceeded</li> </ul>

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				<ul style="list-style-type: none"> <li>- Soil erosion during the construction phase is expected that can indirectly impact to natural landscape values.</li> <li>- covered by roof and disposed of at designated landfills</li> <li>- Prohibit to operate with equipment and vehicles outside the designated work areas and roads</li> <li>- Training and equipment will be in place to minimize the potential environmental impact in the case of accidents (for example using spill kits)</li> </ul>
<b>Solid wastes</b>	<b>B</b>	<b>B</b>	<b>B</b>	<ul style="list-style-type: none"> <li>- Demolition wastes and plant wastes from clearance of trees are expected at pre-construction phase.</li> <li>- Various types of construction solid wastes are likely to occur during the implementation stage of the project construction.</li> <li>- Domestic solid wastes, office wastes, workshop wastes are expected during operation phase.</li> <li>- A waste management plan shall be developed including requirements for separation, handling and disposal of all waste generated</li> <li>- All hazardous materials shall be stored in clearly labeled containers</li> <li>- Storage and handling of hazardous materials should be in accordance with national and local regulations appropriate to their hazard characteristics</li> <li>- Waste shall be separated on site and waste storage areas shall be roofed and bounded to prevent potential cross-contamination</li> <li>- Spent oils (including transformer oil) shall be recycled</li> <li>- Fire prevention systems and secondary containment shall be provided for storage facilities, where necessary, to prevent fires or releases of hazardous materials</li> <li>- All waste shall be disposed of in line with local requirements at a suitable and licensed waste disposal facility; and</li> <li>- Suitable disposal sites shall be identified with capacities for disposal for general and hazardous waste prior to the operation phase</li> </ul>
<b>Vegetation And Terrestrial fauna</b>	<b>A</b>	<b>C</b>	<b>D</b>	<ul style="list-style-type: none"> <li>- A removal of the tree and bushes in the construction area will be done so potential impact on vegetation was expected.</li> <li>- The regional fauna species are also expected to face loss of their habitats due to clearance of vegetation.</li> <li>- Routine checking of trenches (if any) and escape routes to minimize, if not prevent, entrapment of fauna</li> <li>- Reporting of any violation relating to hunting birds, snakes and trading activities</li> <li>- Implementing good housekeeping practices on the field and implementing good Solid Waste Management Plan in order to eliminate any source of hazard to the native fauna</li> <li>- Minimize vegetation clearance and habitat disturbance by demarcating</li> </ul>

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**1.7 Scope of EMP Work**

The EMP framework was Prepared by Resource and Environment Myanmar (REM) in compliance with the Environmental Management Plan in Section 63 (subsection: 8.0) of Environmental Impact Assessment Procedure 2015.

The Environmental Management Plan consists of two parts.

- 1) Environmental mitigation plan: An EMP will be conducted in order to minimize and/or avoid negative impacts, and to strengthen positive impacts.
- 2) Environmental monitoring plan: An environmental monitoring plan will be carried out in order to determine the environmental condition, to ensure that the mitigation plan is effectively functioning and to specify adverse impacts before their expanding.

**1.7.1 Institutional Arrangement for EMP Implementation  
Environmental Management Responsibilities during Construction**

The project proponent (DTVET) and JMASVTI will outsource a contractor to implement the detailed design and construction during the pre-construction and construction phases. The outsourced contractor will establish a project office to undertake the implementation of the detailed design and construction works together with the environmental mitigation and management plan and the environmental monitoring, while the project proponent will supervise their works.

**Regular Operation Stages**

The implementation organization of the monitoring tasks during operation phase will be JMASVTI. During regular operation stage, DTVET shall implement environmental mitigation measures and submission of monitoring report biannually in accordance with the submitted EMP and shall receive the first inspection after 6 months from the start of vocational training school operation and additional inspection after the first inspection as necessary.

**1.8.1 Construction Phase: Air Quality Management Plan**

Objectives	This EMP relates principally to the control of dust emissions from demolition works, loading/unloading of construction materials and bulk materials transported by truck vehicles.
Legal Requirements	National Environmental Quality (Emission) Guidelines, 2015 Yangon City Development Committee Law, 2018
Implementation Schedule	Construction phase of the project
Management Action	<ul style="list-style-type: none"> <li>▪ Keeping construction equipment and generators in good operating condition</li> <li>▪ Keeping vehicles under good condition, with regular checking of vehicle condition to ensure compliance with national standards</li> <li>▪ Adopt machine and equipment that energy saving and create less pollution</li> <li>▪ Proper storage including covering of sand, gravel and other materials which are easily spread into the atmosphere</li> <li>▪ Watering unpaved /dusty roads ( at least twice a day)</li> <li>▪ Sprinkling and covering stockpiles</li> <li>▪ Cleaning of construction sites, especially near site entrance</li> <li>▪ Covering top of trucks carrying materials to the site and carrying construction debris away from the site</li> <li>▪ Protection of all works and materials by installing green net or other measures that</li> </ul>

				<ul style="list-style-type: none"> <li>- Development of inspection, testing and maintenance programs for machinery and equipment</li> <li>- Accident recording and investigation and prevention initiatives</li> <li>- Development of training in site emergency response plans both for the construction phase; and</li> <li>- Compliance to all international, national or local health safety standards that may exist</li> </ul>
<b>Community Health</b>	<b>D</b>	<b>B</b>	<b>D</b>	<ul style="list-style-type: none"> <li>- Guarantee proper vehicle maintenance to reduce noise and accidents</li> <li>- Maintain the Project roads to reduce the possibility of accidents, including clearing of vegetation on to improve sight distance and visibility</li> <li>- A series of traffic measures should be also considered: dust suppression measures, as vehicle speed restrictions, wheel washing area installed at all site access points, containment for dusty materials, and frequent watering or covering of exposed areas of ground, and prompt site restoration; installation of appropriate temporary road sign points on the roads used by Project traffic at bends, junctions, schools and populated areas</li> <li>- Engage with local communities through traffic safety awareness campaigns</li> </ul>

Responsibilities	<ul style="list-style-type: none"> <li>will prevent dust from spreading around</li> <li>Dust mask will be provided to the construction workers working in dusty areas</li> </ul> <p>Staff of Contractor's HSE Department under the guidance of DTIVET</p>
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#### 1.8.2 Construction Phase: Noise & Vibration Management Plan

Objectives	Activities have minimal adverse noise and vibration effects on surrounding environment and project site. To control the noise from operation of construction machinery will increase the level of noise generated, affecting household and sensitivity receptors (e.g. schools, hospitals and monastery)
Legal Requirements	National Environmental Quality (Emission) Guidelines, 2015 Yangon City Development Committee Law, 2018
Implementation Schedule	Construction phase of the project
Management Action	<ul style="list-style-type: none"> <li>Routine maintenance of vehicles and construction equipment</li> <li>Scheduling of deliveries during non-school hours and after regular working hours</li> <li>Installation of barrier fences during construction to reduce noise disturbance, especially near sensitive receptors (e.g. park and staff house)</li> <li>Development of working rules so as to, for example, avoid unnecessary use of air-horns, keep to the speed limit, turn off engines when not in operation and train drivers and operators to follow the rules</li> <li>Scheduling to avoid much equipment operating at the same time near sensitive receptors (e.g. park and staff house)</li> <li>Avoiding, as much as possible, construction equipment producing excessive noise during school hours</li> <li>Avoiding prolonged exposure to noise (produced by equipment) by workers</li> <li>Supply construction workers who will be operating noisy equipment with appropriate personal noise protection gear (e.g. ear muffs, ear plugs, etc.)</li> </ul>
Responsibilities	Staff of Contractor's HSE Department under the guidance of DTIVET

#### 1.8.3 Construction Phase: Soil Erosion and Drainage Management Plan

Objectives	To control and prevent surface water quality in case of soil erosion To prevent flash flooding in construction site during heavy rain
Legal Requirements	Environmental Conservation Law, 2012 Yangon City Development Committee Law, 2018
Implementation Schedule	Construction phase of the project
Management Action	<ul style="list-style-type: none"> <li>Ensure that the existing protected trees will not be damaged during the progress of construction works</li> <li>Adequate temporary drainage channels will be constructed to help facilitate the outflow of onsite runoff to existing drainage facilities. These temporary drainage channels will be constructed in such a manner that they, (a) feed into existing, offsite, natural/engineered drains and (b) do not result in compromise and overtopping of existing offsite drainage features</li> <li>Storm water should be controlled (channeled), before it enters the site, to ensure that the processing plant is not jeopardized during heavy rains.</li> </ul>

Responsibilities	Staff of Contractor's HSE Department under the guidance of DTIVET
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#### 1.8.4 Construction Phase: Solid Waste Management Plan

Objectives	Solid waste will increase by civil works such as large volume of sand waste especially the foundation works. To control and prevent groundwater quality in case of dumping of solid waste.
Legal Requirements	Environmental Conservation Law, 2012 Yangon City Development Committee Law, 2018
Implementation Schedule	Construction phase of the project
Management Action	<ul style="list-style-type: none"> <li>Complying with waste management rule under township development committee for disposing waste at disposal site.</li> <li>Preparation of a temporary waste dumping site during storage</li> <li>Installation of a signboard to prohibit waste dumping in inappropriate areas</li> <li>Collection of residues of oils, including lubricating oil</li> <li>Prohibition of placing materials (e.g. soil, gravel, and sand) on roadside or any other areas outside the project site</li> <li>Reuse of sand materials for road improvement and others</li> <li>Arrange garbage bin for general waste and hazardous waste separately</li> <li>General waste will be clean out once per week and hazardous waste will be clean out once per month</li> </ul>
Responsibilities	Staff of Contractor's HSE Department under the guidance of DTIVET

#### 1.8.5 Construction Phase: Domestic Wastewater Management Plan

Objectives	Domestic especially sanitary water
Legal Requirements	Environmental Conservation Law, 2012 Yangon City Development Committee Law, 2018
Implementation Schedule	Construction phase of the project
Management Action	<ul style="list-style-type: none"> <li>Regular maintenance and checking of all vehicles and machinery to minimize the risk of fuel or lubricant leakages</li> <li>As construction activities typically generate disturbed soil, concrete fines, oils and other waste, on-site collection and settling of storm water, prohibition of equipment wash downs, and prevention of soil loss and toxic releases from the construction site are necessary to minimize water pollution; and</li> <li>Training and equipping relevant staff in protected storage and handling practices, and rapid spill response and clean up techniques</li> <li>Preparing proper sewage system of existing toilets</li> <li>The contractor will procure portable toilets and locate them at the construction site if the existing toilets may not enough for construction workers</li> <li>Waste water generated from washing of concrete mixer and machines will be stored in the special storage (e.g. fiber drum) and send to the designated treated place after discussion with YCDC</li> </ul>
Responsibilities	Staff of Contractor's HSE Department under the guidance of DTIVET



### 1.8.6 Construction Phase: Traffic Safety Plan

Objectives	This Traffic Safety Plan relates principally to the road transport of materials and supplies to the Project site To control the project traffic to prevent traffic jam/disturb traffic flow in front of the school due to mobilization and loading/unloading of equipment and materials for civil works and the construction activities
Legal Requirements	NA
Implementation Schedule	Construction Phase
Management Action	<ul style="list-style-type: none"> <li>▪ To install traffic signs and warnings at the entrance and exit gates for vehicles and heavy equipment</li> <li>▪ To provide adequate parking lots at the construction site and to forbid parking vehicles on the roadside</li> <li>▪ To appoint a staff in charge of traffic control, especially lower Mingaladon Road in front of school</li> <li>▪ To arrange a schedule of mobilization of equipment to avoid increasing traffic congestion and to avoid busy hours</li> <li>▪ To coordinate with the traffic police to manage the traffic when traffic congestion period</li> <li>▪ Speed reduction to 10 km per hour within the school zones</li> </ul>
Responsibilities	Contractor (Monitoring by HSE section and/or third party)

### 1.8.8 Rehabilitation Plan

Objectives	Clearance of trees for construction of new buildings for office main buildings, hostels, workshops and other training facilities are defined and rehabilitation plan for the green area shall be developed. The flora species should be suitable for institutional area. The national flowering plants, Padauk (Myanmar) and Sakura (Japan) where possible, and seasonal flowering plants, Wild Himalayan Cherry ( <i>Prunus wallichii</i> ), Ceylon ironwood ( <i>Mesua ferrea</i> ), Peacock flower ( <i>Caesalpinia pulcherrima</i> ), Golden shower ( <i>Caesalpinia pulcherrima</i> ) etc. shall be planted along with the fence area.
Legal Requirements	Environmental Conservation Law, 2012
Implementation Schedule	Operation phase of the project
Management Action	<ul style="list-style-type: none"> <li>▪ Areas of exposed soil should be replanted with grass, as soon as possible, after site preparation and construction to help mitigate against flash flooding and soil erosion</li> <li>▪ Yangon City Development Committee (relevant department associated with park area) has responsibility for regrowth of trees in Insein Township</li> <li>▪ Therefore, JMASVTI will be requested to co-operate with playgrounds, parks &amp; gardens department for plant seedlings and replantation activities as part of ecological management.</li> </ul>
Responsibilities	DTVET and Township playgrounds, parks & gardens department

### 1.9 Environmental Monitoring Plan

Table 1.9-1: Environmental Monitoring Plan (Construction Phase)

Environmental Monitoring Plan (Construction Phase)				Target Value
Category	Item	Location	Frequency	Responsible Organization
Air emission (mainly dust)	<ul style="list-style-type: none"> <li>- Dust emissions during foundation works</li> <li>- Water spray to limit dust from vehicle movements.</li> <li>- Record of sprinkle water implementation</li> </ul>	Construction site (Within the school compound) and main gate	Daily	Contractor's Environmental Team (HSE)
Noise	<ul style="list-style-type: none"> <li>- Loud Noise by visual observation</li> <li>- Complaint from nearby communities</li> </ul>	Construction site (Within the school compound)	Daily	Contractor's Environmental Team (HSE)
Solid Waste	<ul style="list-style-type: none"> <li>- Amount of solid waste</li> <li>- Non-hazardous waste (office waste)</li> <li>- Hazardous waste (used lubricants)</li> <li>- Site observation.</li> <li>- Examination of records of type and volume of waste/ Proportion of recycling/reuse</li> </ul>	Construction Site	Daily/Weekly	Contractor's Environmental Team (HSE)
Traffic congestion and incident	Site observation Installation of traffic signals, compliance with traffic routes, etc.) hearing with the community contractors and employees	In front of the JMASVTI	Weekly	Contractor's Environmental Team (HSE)
Occupational Health and Safety	OHS training records for all contractors and employees	Construction site	Once a month	Contractor's Environmental Team (HSE)
Community Health and Safety	Training record and number of accidents,	Construction site	Twice a year	Contractor's Environmental Team (HSE)

**Table 1.9-3: Environment Monitoring Plan (Operation Phase)**

Environment Monitoring Plan (Operation Phase)					
Category	Item	Location	Frequency	Responsible Organization	Target Value
Waste water	Proper functioning of the water supply and sanitary sewage facilities to ensure full hygienic practices	Designated location	Once a month	Engineer from JMASVTI	YCDC Rules and Guidelines
Strict commitment to EHS requirements	Proper functioning of automobile workshop and electrical workshop	Designated location	Once a month	Engineer from JMASVTI	As required
Solid Waste	Nonhazardous waste (office waste)	Main office	Monthly	- Engineer from JMASVTI - YCDC	YCDC Guidelines
	Hazardous waste (used lubricants)	Storage area	Twice / year.		
Hazardous and Chemical Substances	Fuel storage, new and used lubricants area	Storage area	Monthly	- Engineer from JMASVTI	Community perception
Occupational Health and Safety	Training record for staff; loss time injury, and number of accidents	Entire boundary	Twice/year	- Engineer from JMASVTI	Community perception
Community Health and Safety	Training record and number of accidents	Surrounding	Twice/year	- Engineer from JMASVTI	Community perception

**1.10 Public Consultation Meeting and Disclosure of EMP**

Public consultation meeting was conducted to disclose the project information, the baseline study results and the estimation of potential impacts to the surrounding communities and the relevant government departments.

Total 26 participants were recorded in meeting attendant list, who are representatives of JMASVTI itself, the surrounding institutes of JMASVTI, relevant government officers from planning and ECD and consultant organizations. The following table shows the summary of Public Consultation Meeting and Disclosure of EMP. Detailed Public Consultation Meeting and Disclosure of EMP is provided in Chapter 10.

<b>Date and Time</b>	29-03-2019 10:00 – 11:30
<b>Venue</b>	Government Technical Institute (Insein)
<b>Consultation Method</b>	<ul style="list-style-type: none"> <li>• Presentation</li> <li>• Questions and Answers</li> <li>• Face to Face Discussion</li> </ul>
<b>Participants</b>	<ul style="list-style-type: none"> <li>• JMASVTI</li> <li>• Consultant (MICT)</li> <li>• REM Consultants</li> <li>• ECD (Yangon Region)</li> <li>• Planning Department (Yangon Region)</li> <li>• UHC Insein</li> <li>• THS (Ywar-Ma)</li> </ul>
<b>Meeting Organizers</b>	<ul style="list-style-type: none"> <li>• Principal, JMASVTI</li> <li>• REM Consultant</li> </ul>

<b>Outlines of Presentation</b>	<ul style="list-style-type: none"> <li>• Objectives of Public Consultation Meeting</li> <li>• Project Description</li> <li>• Aim and Objectives of the Project</li> <li>• Location and Project Components</li> <li>• building Layout Plan</li> <li>• Project Implementation Schedule</li> <li>• Environmental Baseline Results (Physical and Ecological Environments) (Water, Wind Direction, Air, Flora, Fauna)</li> <li>• Impact Assessments</li> <li>• Mitigation Measures</li> <li>• Institutional Arrangement of EMP Implementation Team</li> <li>• EMP for Construction Phase and Operation Phase</li> </ul>
<b>Meeting Results</b>	<ul style="list-style-type: none"> <li>• Clarification of project components</li> <li>• Baseline surveys</li> <li>• Electricity and water usage during construction and operation period</li> <li>• Wastewater and solid treatment units</li> <li>• Protection barrier for atmospheric and noise emissions</li> <li>• Rehabilitation of plant species which were cleared during preconstruction phase</li> <li>• Potential replantation plan during the operation period</li> </ul>

**1.11 Conclusion and Recommendations**

The Resource & Environment Myanmar Ltd. has been invited as the consultant to support and prepare the EMP report by Developer.

In support and approval of this EMP report, the Resource & Environment Myanmar Ltd. had collected and analyzed physical, biological and social data such as people's perceptions, concern, opinion, and expectation on the project for the approval of clean environment and guiltless society during and after the development of the project.

The proposed project is aimed to:

- develop qualified human resources through the graduates of JMASVTI, contributing their skill and knowledge that make fulfilling in the field of Automobile Technology (Maintenance) and Electrical Engineering (Industrial).
- establish a system based on local market needs in Automobile Technology (Maintenance) and Electrical Engineering (Industrial) field which in turn fulfill the education gaps.

Although the project involves some inevitable negative environmental impacts, such impacts as clearance of vegetation and demolition activities, are not served as to not undertake the project. Mitigation measures have been proposed to adequately minimize the significant impacts. Hence, the project is justifiable in the light of the socioeconomic conditions and anticipated benefits from the project which clearly and absolutely outweigh the negative environmental impacts upon completion.

## 1.0: INTRODUCTION

The Environmental Management Plan (EMP) of Japan-Myanmar Aung San Vocational Training Institute (JMASVTI) project is prepared by Resource & Environment Myanmar Company Ltd. The Environmental management plan deals with environmental protection measures, environmental management system and environmental monitoring during project construction and operation period, to ensure environmental protection measures and monitoring requirements can be effectively executed in subsequent stages of the project.

This environmental management plan is worked out during the preparation of project, all kinds of conditions and expected objectives for the project are put forward in the plan.

Once construction and operation schemes are finalized during the implementation of project, this environmental management plan shall be revised accordingly.

### 1.1 Scope of EMP

The project is expected to provide direct financing to "Project for Quality Improvement in TVET Program" to establish and fund the Japan-Myanmar Aung San Vocational Training Institute. The Japan International Cooperation Agency has pledged to provide financial and technical support for the establishment of the Japan-Myanmar Aung San Vocational Training Institute in Yangon.

This EMP is developed by Ministry of Education (MOE) based on the EIA Procedures 2015 to address potential impacts arising from project implementation and operation and in line with the relevant National and International Legislation.

The EMP lists the obligations and responsibilities of each party involved in the project, stipulates methods and procedures that will be followed, and outlines the environmental and social management actions that will be implemented.

### 1.2 Purpose and Objectives of the EMP

The present EMP has been prepared according to the letter dated on 27 June 2018 of Ministry of Natural Resources and Environmental Conservation.

This environmental management plan includes the following contents:

- 1) Executive Summary
- 2) Introduction
- 3) Project Description
- 4) Environmental institutional, policies and legislative framework applicable to the project
- 5) Environmental and Ecological Baseline
- 6) Role and Responsibility of Environmental Management
- 7) Summary of Impacts and Mitigation Measures
- 8) Environmental Management Plan
- 9) Environmental supervision and monitoring plan
- 10) Cost for Mitigation Measures and Monitoring
- 11) Public Consultation Meeting and Disclosure of EMP

## 1.2) Conclusion

### 1.3 Project Proponent

Department of Technical and Vocational Education and Training, Ministry of Education, the Republic of the Union of Myanmar.

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### 1.4 Consultant Profile

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Contact Person	:	Mr. Thura Aung
Designation	:	General Manager

### EMP Team Members

The EMP study team comprises members who have been involved in numerous environmental related studies. The personnel are well trained and qualified in their respective field. Please refer to Table 1.3-1 for details on members in the study team.

Table 1.3-1: EMP Consultant Team

No.	Name	Position	Responsibility
1.	U Zaw Naing Oo	Principal Consultant	Environmental Management Plan
2.	U Thura Aung	Principal Consultant	Physical Environment
3.	Daw Lai Lai Win	Environmental Consultant	Ecology, Environmental and Social Impact Assessment and Reporting
4.	U Kyaw Zin Win	Principal Consultant	GIS
5.	Daw Phyu Phyu Shein	Social Consultant	Socioeconomic
6.	Myat Ko Ko Hein	Junior Consultant	Ecology
7.	De Hlaing Zaw	Environmental Technician	Physical Environment