

## **15. 地形地質測量結果報告書（現地再委託）**

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REPUBLIC OF RWANDA  
MINISTRY OF INFRASTRUCTURE

*Execution Agency :*



Japan International Cooperation Agency

*Buyer :*



*Consultant :*



*Project :*

IMPROVEMENT OF SUBSTATIONS AND DISTRIBUTION NETWORK, PHASE 3, IN THE REPUBLIC OF RWANDA

**SOIL INVESTIGATIONS**

FILE 1 / 2



KIGALI, August 2017

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## I. METHODOLOGY

These soil studies relate to five areas defined by the project "Improvement of substations and distribution network, phase 3, in the Republic of Rwanda.

These areas are presented, each, by the enclosed plans :

- New substation construction site at Gasogi where 6 survey points were dug : Sip1 , Sip2, Sip3, Sip4, Sip5, Sip6
- Branch point line 110 kV, on the new substation where 1 survey point were dug : Sip7
- 15 kV Distribution line Gasogi substation - Nyagasambu, where 15 survey points were dug on : Sip1 , Sip2, Sip3, Sip4, Sip5, Sip6, Sip7 , Sip9, Sip10, Sip11, Sip12, Sip13, Sip15, Sip16 & Sip17 ; corresponding to the corner points of the projected electrical line.
- 15 kV Distribution line Gasogi substation - Masaka hospital, where 15 survey points were dug on : Sip1 , Sip3, Sip4, Sip5, Sip6, Sip8, Sip9, Sip10, Sip12, Sip13, Sip15, Sip16, Sip17, Sip18 & Sip19 ; corresponding to the corner points of the projected electrical line.
- Reconstruction of transmission 110 kV line Jabana - Birembo, where 13 survey points were dug on : Sip176 , Sip177, Sip178, Sip180, Sip181, Sip182, Sip184, Sip186, Sip187, Sip189, Sip190, Sip195, Sip196 ; corresponding to the location of the existing pylons on the current electrical line.

The points considered are those which are the most solicited and the most representative, from the geotechnical point of view, for different soils.

So, all soil investigation points have been tested according to the details described below.

### 1 ) Field tests and levy of samples

This phase comprises :

- Observations and analysis of soils texture on the walls of the test pits, 4 meters for maximum depth ;
- Levy of undisturbed samples ;
- Levy of samples overhauled ;
- Backfilling of the test pits ;
- Penetration test in natural ground, performed with a static penetrometer 50 kN, up to more than 10 m deep, 3 meters around the soil investigation point.

The bearing capacity =  $R_p \times 0.01$ ;  $R_p$  is the resistance to the tip and 0.01 is the coefficient taking into account the presence of clay ;  $R_t$  is the total resistance taking into account friction (see penetrometer tests below).

### 2 ) Laboratory tests

On the selected samples from test pits, physical, mechanical and chemical tests analysis were performed by the following tests :

- Physical tests : Natural water content, Specific gravity, Sieve analysis, Liquid limit, Plastic limit.
- Mechanical tests : Modified proctor, Shear test, unconfined compression test.
- Chemical tests : the degree of alkalinity in soil expressed as a pH, the concentration of sulphate (as  $SO_4^{2-}$ ) and chlorites ( as  $Cl^-$  )

### 3 ) Results and recommendations for foundation

Bringing together the results of field tests and laboratory tests, soil represented by each point is identified, in its form and in its quality.

This identification, facilitating the rational use of these soils, allows the expert to give recommendations for better land use, such as materials and support.

All the work of geotechnical studies, as described above, is presented in two volumes :

- volume 1/2 : own report of the expert ;
- volume 2/2 : test report.

For reasons of speed and ease, separate laboratories were used simultaneously :

- Rincent BTP Rwanda Ltd,
- Geotechnical laboratory and laboratory of agronomy, University of Rwanda, Huye campus .

## II. STANDARDS REFERENCES

In large part, we use the French standardization ( NF ) and, in lesser extent, British Standards ( BSCS ) and American Standards ( AASHTO ) & ASTM.

- Sampling ..... - Standard NF P94-202
- Moisture content ..... - Standard NF P94-050
- Specific gravity ..... - Standard NF P94-053
- Sieve analysis ..... - Standard NF P94-056
- Atterberg limits .....- Standard NF P94-051
- Modified proctor .....- Standard NF P94-093
- Direct shear box .....- Standard NF P94-071-2
- Unconfined compression strength .....- Standard AASHTO T134
- British Soil Classification System .....- BSCS
- Chemical analysis (sulfate & chloride) ..... - Standard : ASTM D 4327
- Chemical analysis ( pH ) ..... - Standard : ASTM D 4972
- In-Situ static penetration tests performed using a GeoMIL 50 kN SPT.

III. ILLUSTRATION OF STUDIES ON SOME PHASES

1 ) Excavation of test pit



2 ) Levy of sample



3 ) Penetration tests:

Setting ink needles for the base of the penetrometer



Penetrometer in action : measuring applied forces



4 ) Laboratory works

a. Liquid limit



b. Sieve analysis



c. Laboratory Technician at work



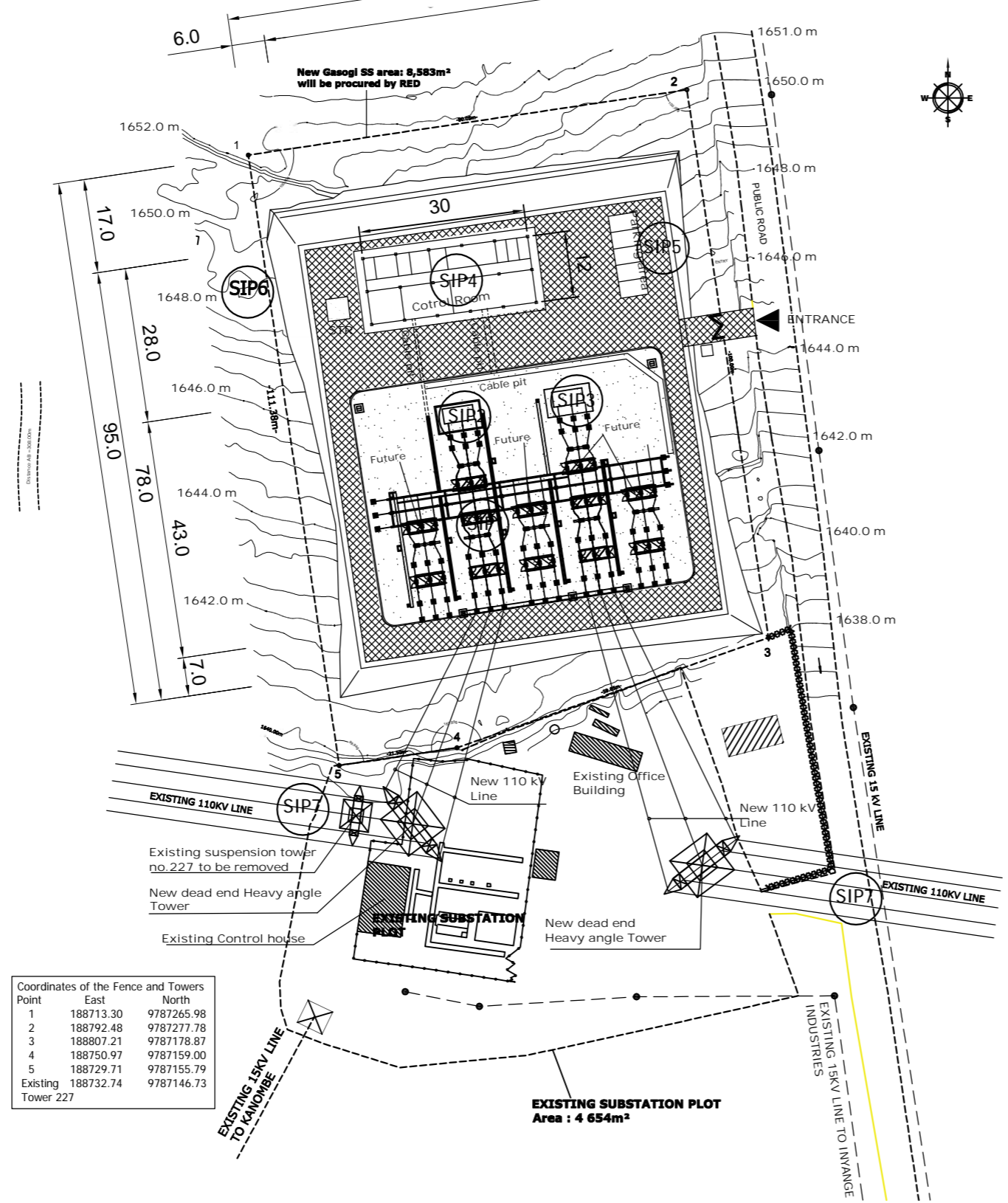


IV. EXPOSED OBSERVATIONS ON EACH TEST PIT

Legend :

- $I_p$  : Plasticity Index      -  $I_c$  : Consistency Index      -  $W_{optm} (\%)$  : Optimum water content
- $\gamma_{d\ optm}$  : Optimum dry specific gravity -  $\gamma_n$  : Natural specific gravity -  $W(N)$  : Natural Moisture content
- $C_{uu}$  : Cohesion unconsolidated - undrained      -  $\psi_{uu}$  : Angle of internal friction

A. New substation construction site & Branch point line 110 kV



Point	East	North
1	188713.30	9787265.98
2	188792.48	9787277.78
3	188807.21	9787178.87
4	188750.97	9787159.00
5	188729.71	9787155.79
Existing Tower 227	188732.74	9787146.73

FENCES COORDINATES AND TOWERS		
POINT	EAST	NORTH
1	188713.30	9787265.98
2	188792.48	9787277.78
3	188807.21	9787178.87
4	188750.97	9787159.00
5	188729.71	9787155.79
6	188736.95	9787259.40
7	188793.96	9787267.89
8	188801.19	9787217.31
9	188744.48	9787208.82
New tower A	188405.95	9787193.78
New tower B	188713.18	9787215.01
New tower C	188871.40	9787274.67
New tower D	189063.92	9787099.38
Exis. Tower no. 226	188379.36	9787197.16
Exis. Tower no. 227	188732.74	9787146.73
Exis. Tower no. 228	188896.99	9787123.16
Exis. Tower no. 229	189095.09	9787091.72

- LEGEND**
- Proposed 15 kV line
  - Existing 110kV OHL
  - Existing 15kV OHL rerouted
  - Existing 15kV OHL pole
  - SIP Soil Investigation Point

REV.	DATE	DESCRIPTION	ISSUED	CHECKED
1	March 2015	New Gasogi Substation Layout	REG LTD	REG LTD


**REPUBLIC OF RWANDA**


**RWANDA ENERGY GROUP**

Project : **JICA PROJECT PHASE III**

Title : **NEW GASOGI SUBSTATION LAYOUT**

Name	Fonction	Approva by Client	Date	Signature & stamp

PAPER SIZE	SCALE	DOCUMENT NO.	DATE	DRAWING STATUS
A1	1 / 500		March 2015	FEASIBILITY <input checked="" type="checkbox"/> INFO <input type="checkbox"/> FABRICATION <input type="checkbox"/> ERECTION <input type="checkbox"/>

Drawing n°S/JPP11

Soil investigation point	Extraction depth of the sample	Classification according USCS	Observation	ENGINEERING PROPERTIES		EXCAVATION CONDITION	GEOLOGICAL UNIT SIP 1 Light penetrometer ( GeoMIL 50 kN SPT ), test without soil coring for sample taking					
							DESCRIPTION OF THE PIT	Depth (below existing ground level) cm	Soil Stratigraphy (observed in the well)	GEOTECHNICAL BEARING CAPACITIES		ULS * (Mpa)
(Mpa)	(Kpa)											
<b>SIP 1</b> (new subst. construction site)	2.0 - 3.50 m	Silty gravel poorly graded (GM)	Lateritic zone	- IP : /  - Consistency $I_c > 1$ : very consistent ground  - Compaction Backfill : $\rho_{dOPM} = 1.95 \text{ t/m}^3$ $W_{OPM} = 14.68 \%$  - Natural soil : $\rho_h = 1.68 \text{ t/m}^3$ $W_{(N)} = 14.5 \%$ $\rho_s = 2.85 \text{ t/m}^3$	- Permissible bearing pressure (from 2.0 m deep) = 200 kPa  - Shear test : $C_{uu} = 26 \text{ kN/m}^2$ $\psi_{uu} = 23.99^\circ$  - Chemical analysis : pH = 7.3 $SO_4^2 = 31 \text{ mg/L}$ $Cl^- = 85 \text{ mg/L}$	Picks and shovels can be used.	Vegetable soil	0	0	0	0	0
							0.00 - 0.40 m	20	0.25	250	25	45
								40	0.30	300	30	50
								60	0.25	250	25	40
							Black Silty gravel poorly graded	80	0.63	630	63	80
								100	0.70	700	70	90
							0.40 - 1.40 m	120	0.90	900	90	250
								140	1.00	1000	100	370
								160	1.30	1300	130	420
								180	1.50	1500	150	500
								200				
								220				
								240				
							Red Silty gravel poorly graded	260				
								280				
							1.40 - 4.00 m	300				
								320				
								340				
								360				
								380				
								400				
								420				
								440				
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	760											
	780											
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	820											
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\* ULS : Ultimate Limit State

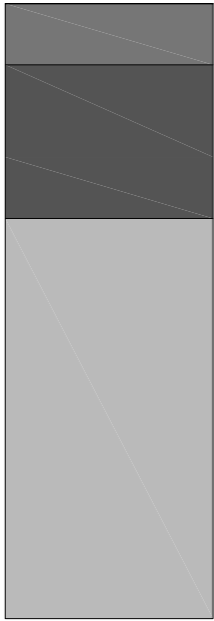


EXCAVATION ( test pit )

EXCAVATED SOIL

FOUNDATION TYPE



REFER TO THE DESIGN OF THE STRUCTURE

Soil investigation point	Extraction depth of the sample	Classification according USCS	Observation	ENGINEERING PROPERTIES		EXCAVATION CONDITION	GEOLOGICAL UNIT (Without penetrometer test )
SIP 2 (new subst. construction site )	3.70 - 4.00 m	Silty gravel poorly graded ( GM )	Semi rock soil	- IP : / - Consistency $I_c > 1$ : very consistent ground - Compaction Backfill : $\rho_{dOPM} = 1.95 \text{ t/m}^3$ $W_{OPM} = 15.10 \%$ - Natural soil : $\rho_h = 1.94 \text{ t/m}^3$ $W_{(N)} = 17.7 \%$ $\rho_s = 2.90 \text{ t/m}^3$	- Shear test : $C_{uu} = 14 \text{ kN/m}^2$ $\psi_{uu} = 26.57^\circ$ - Chemical analysis : $pH = 6.55$ $SO_4^2 = 11.7 \text{ mg/L}$ $Cl^- = 191 \text{ mg/L}$	Picks and shovels can be used.	Texture on the section of the test pit  <ul style="list-style-type: none"> <li>vegetable soil (30 cm)</li> <li>black Silty gravel poorly graded ( 2.0 m)</li> <li>light reddish gray Silty gravel poorly graded ( ± 1.7 m)</li> </ul>
EXCAVATION ( test pit )						FOUNDATION TYPE	
						EXCAVATED SOIL  <p>REFER TO THE DESIGN OF THE STRUCTURE</p>	

Soil investigation point	Extraction depth of the sample	Classification according USCS	Observation	ENGINEERING PROPERTIES		EXCAVATION CONDITION	GEOLOGICAL UNIT																																																																																																																																																																																																																																																																																				
							Light penetrometer ( GeoMIL 50 kN SPT ), test without soil coring for sample taking																																																																																																																																																																																																																																																																																				
SIP 3 (new subst. construction site)	3.50 - 4.00 m	red Clayey gravel poorly graded ( GC )	Lateritic zone	- IP : 14.8 % - Consistency $I_c = 1.52 > 1$ : very consistent ground - Compaction Backfill : . $d_{OPM} = 1.94 \text{ t/m}^3$ . $W_{OPM} = 13.80 \%$ - Natural soil : . $\rho_h = 1.93 \text{ t/m}^3$ . $W_{(N)} = 14.6 \%$ . $\rho_s = 2.80 \text{ t/m}^3$	- Permissible bearing pressure (from 1.60 m deep) = 50 Mpa - Shear test : . $C_{uu} = 14 \text{ kN/m}^2$ . $\psi_{uu} = 26.34^\circ$ - Chemical analysis : . pH = 7.02 . $SO_4^2 = 16.5 \text{ mg/L}$ . $Cl^- = 177 \text{ mg/L}$	Picks and shovels can be used.	<table border="1"> <thead> <tr> <th rowspan="2">DESCRIPTION OF THE PIT</th> <th rowspan="2">Depth (below existing ground level) / cm</th> <th rowspan="2">Soil Stratigraphy (observed in the well)</th> <th colspan="3">GEOTECHNICAL BEARING CAPACITIES</th> </tr> <tr> <th>Allowable (Mpa)</th> <th>(Kpa)</th> <th>ULS * (Mpa)</th> <th>Ultimate (Mpa)</th> </tr> </thead> <tbody> <tr> <td>Vegetable soil</td> <td>0</td> <td></td> <td>0</td> <td>0</td> <td>0</td> <td>0</td> </tr> <tr> <td rowspan="5">0.00 - 0.40 m</td> <td>20</td> <td></td> <td>0.20</td> <td>200</td> <td>20</td> <td>40</td> </tr> <tr> <td>40</td> <td></td> <td>0.30</td> <td>300</td> <td>30</td> <td>60</td> </tr> <tr> <td>60</td> <td></td> <td>0.25</td> <td>250</td> <td>25</td> <td>45</td> </tr> <tr> <td>80</td> <td></td> <td>0.20</td> <td>200</td> <td>20</td> <td>40</td> </tr> <tr> <td>100</td> <td></td> <td>0.80</td> <td>800</td> <td>80</td> <td>130</td> </tr> <tr> <td rowspan="6">Black Clayey gravel poorly graded</td> <td>120</td> <td></td> <td>1.20</td> <td>1200</td> <td>120</td> <td>350</td> </tr> <tr> <td>140</td> <td></td> <td>1.40</td> <td>1400</td> <td>140</td> <td>420</td> </tr> <tr> <td>160</td> <td></td> <td>1.50</td> <td>1500</td> <td>150</td> <td>500</td> </tr> <tr> <td>180</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>200</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>220</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td rowspan="12">Red Clayey gravel poorly graded</td> <td>240</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>260</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>280</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>300</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>320</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>340</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>360</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>380</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>400</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>420</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>440</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>460</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>480</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>500</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>520</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>540</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>560</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>580</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>600</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>620</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>640</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>660</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>680</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>700</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>720</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>740</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>760</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>780</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>800</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>820</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>840</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> </tbody> </table>					DESCRIPTION OF THE PIT	Depth (below existing ground level) / cm	Soil Stratigraphy (observed in the well)	GEOTECHNICAL BEARING CAPACITIES			Allowable (Mpa)	(Kpa)	ULS * (Mpa)	Ultimate (Mpa)	Vegetable soil	0		0	0	0	0	0.00 - 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\* ULS : Ultimate Limit State

EXCAVATION ( test pit )

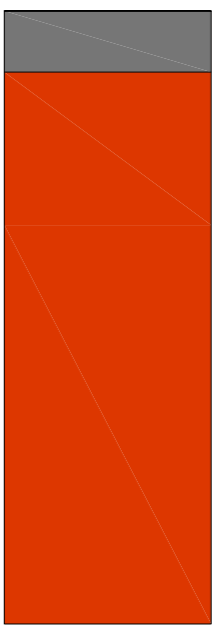




EXCAVATED SOIL





FOUNDATION TYPE

REFER TO THE DESIGN OF THE STRUCTURE

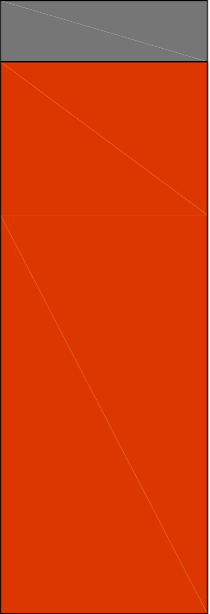


Soil investigation point	Extraction depth of the sample	Classification according USCS	Observation	ENGINEERING PROPERTIES		EXCAVATION CONDITION	GEOLOGICAL UNIT (Without penetrometer test )
<b>SIP 4</b> (new subst. construction site )	3.50 - 4.00 m	Clayey gravel poorly graded ( GC )	Semi rock soil	- IP : 15.2 % - Consistency $I_c = 1.34 > 1$ : very consistent ground - Compaction Backfill : . $d_{OPM} = 2.01 \text{ t/m}^3$ . $W_{OPM} = 12.80 \%$ - Natural soil : . $\rho_h = 1.94 \text{ t/m}^3$ . $W_{(N)} = 18.0 \%$ . $\rho_s = 2.80 \text{ t/m}^3$	- Shear test : . $C_{uu} = 10 \text{ kN/m}^2$ . $\psi_{uu} = 27.25^\circ$ - Chemical analysis : . pH = 6.63 . $SO_4^2 = 19 \text{ mg/L}$ . $Cl^- = 197 \text{ mg/L}$	Picks and shovels can be used.	Texture on the section of the test pit  vegetable soil (20 cm)  Red Clayey gravel poorly graded ( ± 3.8 m)
EXCAVATION ( test pit )						FOUNDATION TYPE	
						REFER TO THE DESIGN OF THE STRUCTURE	
EXCAVATED SOIL							

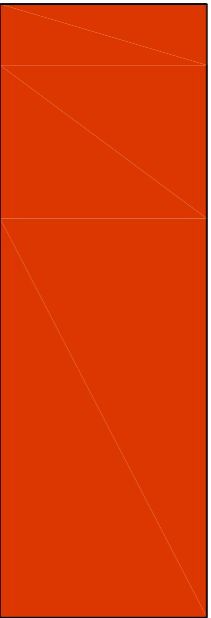


Soil investigation point	Extraction depth of the sample	Classification according USCS	Observation	ENGINEERING PROPERTIES		EXCAVATION CONDITION	GEOLOGICAL UNIT Light penetrometer ( GeoMIL 50 kN SPT ), test without soil coring for sample taking				
							DESCRIPTION OF THE PIT	Depth (below existing ground level) / cm	Soil Stratigraphy (observed in the well)	GEOTECHNICAL BEARING CAPACITIES	
Allowable		ULS *	Ultimate								
		( Mpa )	( Kpa )	( Mpa )	( Mpa )						
<b>SIP 5</b> (new subst. construction site)	3.50 - 4.00 m	Clayey of intermediate plasticity ( CI )	Semi rock soil	- IP : 18.1 % - Consistency $I_c = 1.30 > 1$ : very consistent ground - Compaction Backfill : $\rho_{dOPM} = 1.88 \text{ t/m}^3$ $W_{OPM} = 14.40 \%$ - Natural soil : $\rho_h = 1.84 \text{ t/m}^3$ $W_{(N)} = 16 \%$ $\rho_s = 2.80 \text{ t/m}^3$	- Shear test : $c_{uu} = 37 \text{ kN/m}^2$ $\psi_{uu} = 19.80^\circ$ - Chemical analysis : $pH = 6.67$ $SO_4^2 = 21 \text{ mg/L}$ $Cl^- = 193 \text{ mg/L}$	Picks and shovels can be used.	Vegetable soil 0.00 - 0.30 m	0	0	0	0
							20	0.20	200	35	55
							40	0.30	300	40	65
							60	0.25	250	30	55
							80	0.20	200	42	75
							100	0.80	800	80	130
							120	1.20	1200	130	420
							140	1.50	1500	150	500
							160				
							180				
							200				
							220				
							240				
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\* ULS : Ultimate Limit State

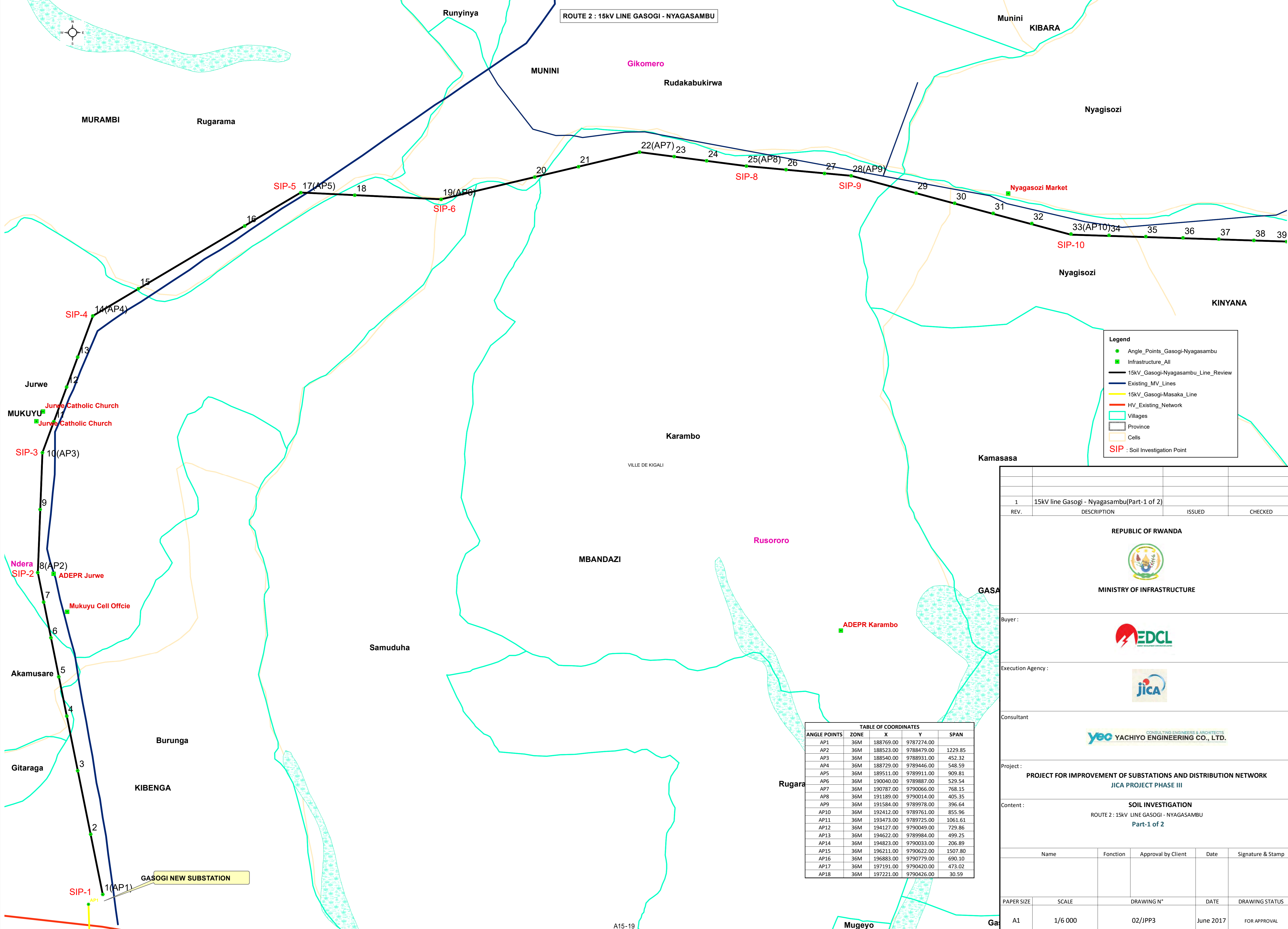
EXCAVATION ( test pit )	EXCAVATED SOIL	FOUNDATION TYPE
		REFER TO THE DESIGN OF THE STRUCTURE



Soil investigation point	Extraction depth of the sample	Classification according USCS	Observation	ENGINEERING PROPERTIES		EXCAVATION CONDITION	GEOLOGICAL UNIT (Without penetrometer test)
SIP 6 (new subst. construction site)	3.50 - 4.00 m	Clayey of intermediate plasticity (CI)	Lateritic zone	- IP : 18.8 % - Consistency $I_c = 1.25 > 1$ : very consistent ground - Compaction Backfill : . $d_{OPM} = 1.82 \text{ t/m}^3$ . $W_{OPM} = 16.98 \%$ - Natural soil : . $\rho_h = 1.71 \text{ t/m}^3$ . $W_{(N)} = 18 \%$ . $\rho_s = 2.77 \text{ t/m}^3$	- Shear test : . $C_{uu} = 26 \text{ kN/m}^2$ . $\psi_{uu} = 20.30^\circ$ - Chemical analysis : . pH = 5.4 . $SO_4 = 11.2 \text{ mg/L}$ . $Cl^- = 78.8 \text{ mg/L}$	Picks and shovels can be used.	<p style="text-align: center;"><u>Texture on the section of the test pit</u></p> 
<u>EXCAVATION ( test pit )</u>				<u>EXCAVATED SOIL</u>		FOUNDATION TYPE	
						REFER TO THE DESIGN OF THE STRUCTURE	

Soil investigation point	Extraction depth of the sample	Classification according USCS	Observation	ENGINEERING PROPERTIES		EXCAVATION CONDITION	GEOLOGICAL UNIT (Without penetrometer test )
SIP 7 (new subst. construction site )	3.50 - 4.00 m	Silty sand poorly graded ( SM )	Lateritic zone	- IP : 16 % - Consistency $I_c = 1.24 > 1$ : very consistent ground - Compaction Backfill : . $d_{OPM} = 1.96 \text{ t/m}^3$ . $W_{OPM} = 14.69 \%$ - Natural soil : . $\rho_h = 1.93 \text{ t/m}^3$ . $W_{(N)} = 18.3 \%$ . $\rho_s = 2.86 \text{ t/m}^3$	- Shear test : . $C_{uu} = 25 \text{ kN/m}^2$ . $\psi_{uu} = 23.75^\circ$ - Chemical analysis : . pH = 6.31 . $SO_4^{2-} = 37 \text{ mg/L}$ . $Cl^- = 134 \text{ mg/L}$	Picks and shovels can be used.	Texture on the section of the test pit  Silty sand poorly graded ( ± 4.0 m)
<u>EXCAVATION ( test pit )</u>				<u>EXCAVATED SOIL</u>		FOUNDATION TYPE	
						REFER TO THE DESIGN OF THE STRUCTURE	

B. 15 kV Distribution line Gasogi substation-Nyagasambu



ROUTE 2 : 15kV LINE GASOGI - NYAGASAMBU

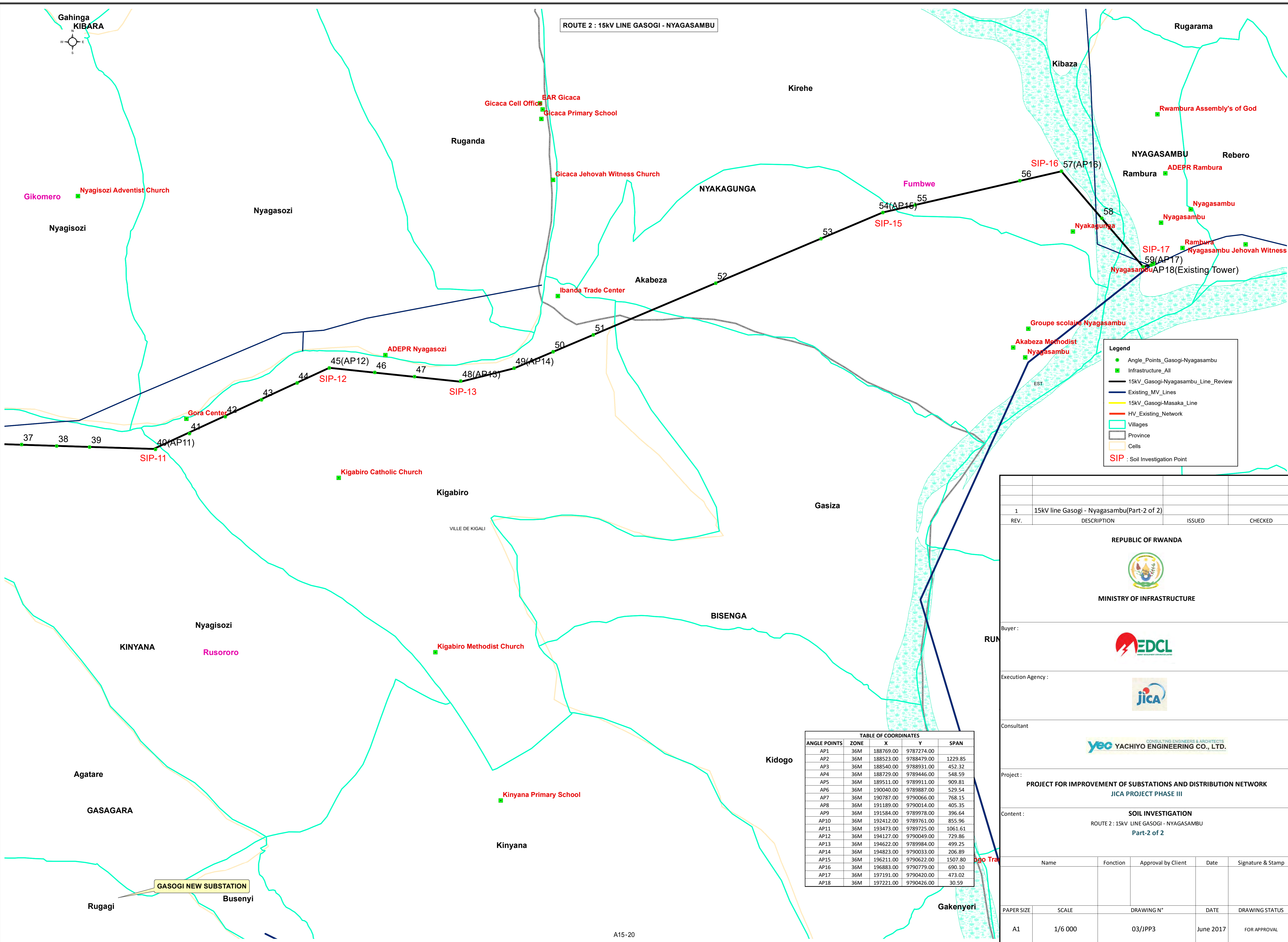
**Legend**

- Angle\_Points\_Gasogi-Nyagasambu
- Infrastructure\_All
- 15kV\_Gasogi-Nyagasambu\_Line\_Review
- Existing\_MV\_Lines
- 15kV\_Gasogi-Masaka\_Line
- HV\_Existing\_Network
- Villages
- Province
- Cells
- SIP : Soil Investigation Point

**TABLE OF COORDINATES**

ANGLE POINTS	ZONE	X	Y	SPAN
AP1	36M	188769.00	9787274.00	
AP2	36M	188523.00	9788479.00	1229.85
AP3	36M	188540.00	9788931.00	452.32
AP4	36M	188729.00	9789446.00	548.59
AP5	36M	189511.00	9789911.00	909.81
AP6	36M	190040.00	9789887.00	529.54
AP7	36M	190787.00	9790066.00	768.15
AP8	36M	191189.00	9790014.00	405.35
AP9	36M	191584.00	9789978.00	396.64
AP10	36M	192412.00	9789761.00	855.96
AP11	36M	193473.00	9789725.00	1061.61
AP12	36M	194127.00	9790049.00	729.86
AP13	36M	194622.00	9789984.00	499.25
AP14	36M	194823.00	9790033.00	206.89
AP15	36M	196211.00	9790622.00	1507.80
AP16	36M	196883.00	9790779.00	690.10
AP17	36M	197191.00	9790420.00	473.02
AP18	36M	197221.00	9790426.00	30.59

1	15kV line Gasogi - Nyagasambu(Part-1 of 2)			
REV.	DESCRIPTION	ISSUED	CHECKED	
<b>REPUBLIC OF RWANDA</b>  <b>MINISTRY OF INFRASTRUCTURE</b>				
Buyer :				
Execution Agency :				
Consultant				
Project :	<b>PROJECT FOR IMPROVEMENT OF SUBSTATIONS AND DISTRIBUTION NETWORK JICA PROJECT PHASE III</b>			
Content :	<b>SOIL INVESTIGATION ROUTE 2 : 15kV LINE GASOGI - NYAGASAMBU Part-1 of 2</b>			
Name	Fonction	Approval by Client	Date	Signature & Stamp
PAPER SIZE	SCALE	DRAWING N°	DATE	DRAWING STATUS
A1	1/6 000	02/JPP3	June 2017	FOR APPROVAL



ROUTE 2 : 15kV LINE GASOGI - NYAGASAMBU

**Legend**

- Angle\_Points\_Gasogi-Nyagasambu
- Infrastructure\_All
- 15kV\_Gasogi-Nyagasambu\_Line\_Review
- Existing\_MV\_Lines
- 15kV\_Gasogi-Masaka\_Line
- HV\_Existing\_Network
- Villages
- Province
- Cells
- SIP : Soil Investigation Point

**TABLE OF COORDINATES**

ANGLE POINTS	ZONE	X	Y	SPAN
AP1	36M	188769.00	9787274.00	
AP2	36M	188523.00	9788479.00	1229.85
AP3	36M	188540.00	9788931.00	452.32
AP4	36M	188729.00	9789446.00	548.59
AP5	36M	189511.00	9789911.00	909.81
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AP9	36M	191584.00	9789978.00	396.64
AP10	36M	192412.00	9789761.00	855.96
AP11	36M	193473.00	9789725.00	1061.61
AP12	36M	194127.00	9790049.00	729.86
AP13	36M	194622.00	9789984.00	499.25
AP14	36M	194823.00	9790033.00	206.89
AP15	36M	196211.00	9790622.00	1507.80
AP16	36M	196883.00	9790779.00	690.10
AP17	36M	197191.00	9790420.00	473.02
AP18	36M	197221.00	9790426.00	30.59

	1	15kV line Gasogi - Nyagasambu(Part-2 of 2)			
REV.	DESCRIPTION	ISSUED	CHECKED		
<b>REPUBLIC OF RWANDA</b>  <b>MINISTRY OF INFRASTRUCTURE</b>					
Buyer :					
Execution Agency :					
Consultant					
Project :	<b>PROJECT FOR IMPROVEMENT OF SUBSTATIONS AND DISTRIBUTION NETWORK</b> <b>JICA PROJECT PHASE III</b>				
Content :	<b>SOIL INVESTIGATION</b> ROUTE 2 : 15kV LINE GASOGI - NYAGASAMBU Part-2 of 2				
	Name	Fonction	Approval by Client	Date	Signature & Stamp
PAPER SIZE	SCALE	DRAWING N°	DATE	DRAWING STATUS	
A1	1/6 000	03/JPP3	June 2017	FOR APPROVAL	

Soil investigation point	Extraction depth of the sample	Classification according USCS	Observation	ENGINEERING PROPERTIES		EXCAVATION CONDITION	GEOLOGICAL UNIT Light penetrometer ( GeoMIL 50 kN SPT ), test without soil coring for sample taking					
				- IP : / - Consistency $I_c > 1$ : very consistent ground - Compaction Backfill : $d_{OPM} = 1.89 \text{ t/m}^3$ $W_{OPM} = 12.35 \%$	- Shear test : $C_{uu} = 31 \text{ kN/m}^2$ $\psi_{uu} = 23.03^\circ$		DESCRIPTION OF THE PIT	Depth (below existing ground level) (cm)	Soil Stratigraphy (observed in the well)	GEOTECHNICAL BEARING CAPACITIES		
Allowable		ULS *	Ultimate									
				( Mpa )	( Kpa )	( Mpa )	( Mpa )					
<b>SIP 1</b> (15 kV line Gasogi - Nyagasambu)	3.70 - 4.00 m	Silty gravel poorly graded ( GM )	Semi rock soil	- Natural soil : $\rho_h = 1.64 \text{ t/m}^3$ $W_{(N)} = 16.3 \%$ $\rho_s = 2.60 \text{ t/m}^3$ - Chemical analysis : $pH = 5.9$ $SO_4^2 = 7.7 \text{ mg/L}$ $Cl^- = 92 \text{ mg/L}$	Picks and shovels can be used.	Vegetable soil	0	0	0	0	0	
						0.00 - 0.30 m	20	0.12	120	12	45	
						Black Silty gravel poorly graded	0.30 - 2.30 m	40	0.18	180	18	65
								60	0.60	600	60	94
								80	1.20	1200	120	160
								100	0.30	300	30	85
								120	0.33	330	33	90
								140	0.25	250	25	70
								160	0.50	500	50	110
								180	0.53	530	53	200
								200	0.95	950	95	280
								220	1.00	1000	100	390
						240	1.50	1500	150	500		
						Light reddish gray Silty gravel poorly graded	2.30 - 4.00 m	260				
								280				
								300				
								320				
								340				
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EXCAVATION ( test pit )

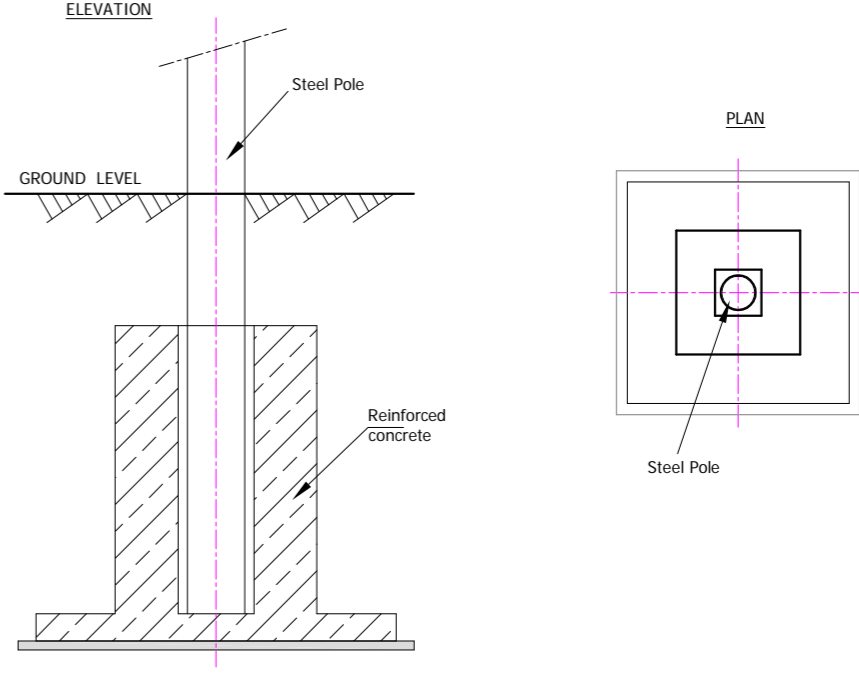


EXCAVATED SOIL



20

FOUNDATION TYPE



THE FOUNDATION DIMENSIONS COMPLY WITH THE STATIC BALANCE OF POST FIXING .

Soil investigation point	Extraction depth of the sample	Classification according USCS	Observation	ENGINEERING PROPERTIES	EXCAVATION CONDITION	GEOLOGICAL UNIT Light penetrometer ( GeoMIL 50 kN SPT ), test without soil coring for sample taking					
						DESCRIPTION OF THE PIT	Depth (below existing ground level) (m)	Soil Stratigraphy (observed in the well)	GEOTECHNICAL BEARING CAPACITIES		ULS * (Mpa)
(Mpa)	(Kpa)										
<b>SIP 2</b> (15 kV line Gasogi - Nyagasambu)	1.00 - 2.00 m	Silty sand poorly graded ( SM )	Semi rock soil	- IP : 16.8 - Consistency $I_c = 1.4 > 1$ : very consistent ground - Compaction Backfill : $\rho_{OPM} = 1.92 \text{ t/m}^3$ $W_{OPM} = 13.40 \%$ - Natural soil : $\rho_h = 1.72 \text{ t/m}^3$ $W_{(N)} = 14.6 \%$ $\rho_s = 2.68 \text{ t/m}^3$ - Shear test : $c_{Cu} = 28 \text{ kN/m}^2$ $\psi_{uu} = 25.41^\circ$ - Chemical analysis : $pH = 4.62$ $SO_4^2 = 6.7 \text{ mg/L}$ $Cl^- = 113.4 \text{ mg/L}$	Pneumatique hammer	Vegetable soil	0		0	0	0
						0.00 - 0.30 m	20	0.10	100	10	20
							40	0.20	200	20	40
							60	0.10	100	10	30
							80	0.75	750	75	100
							100	1.05	1050	105	160
							120	1.30	1300	130	340
							140	1.50	1500	150	500
							160				
							180				
							200				
							220				
							240				
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\* ULS : Ultimate Limit State

EXCAVATION ( test pit )

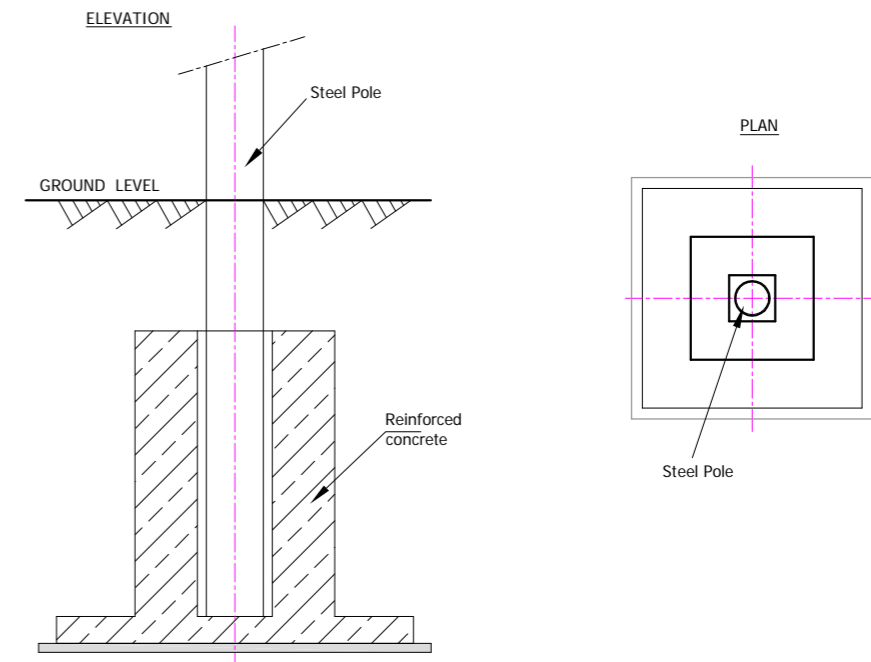


EXCAVATED SOIL



21

FOUNDATION TYPE



THE FOUNDATION DIMENSIONS COMPLY WITH THE STATIC BALANCE OF POST FIXING .

Soil investigation point	Extraction depth of the sample	Classification according USCS	Observation	ENGINEERING PROPERTIES	EXCAVATION CONDITION	GEOLOGICAL UNIT																																																																																																																																																																																																																																																																																																																													
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SIP 3 (15 kV line Gasogi - Nyagasambu)	3.60 - 4.00 m	Silt of low plasticity ( ML )	Semi rock soil	- IP : /  - Consistency $I_c > 1$ : very consistent ground  - Compaction Backfill : $d_{OPM} = 1.83 \text{ t/m}^3$ $W_{OPM} = 14.20 \%$  - Natural soil : $d_h = 1.63 \text{ t/m}^3$ $W_{(N)} = 17.2 \%$ $d_s = 2.60 \text{ t/m}^3$	Pneumatique hammer	<table border="1"> <thead> <tr> <th rowspan="3">DESCRIPTION OF THE PIT</th> <th rowspan="3">Depth (below existing ground level) / cm</th> <th rowspan="3">Soil Stratigraphy (observed in the well)</th> <th colspan="4">GEO TECHNICAL BEARING CAPACITIES</th> </tr> <tr> <th colspan="2">Allowable</th> <th rowspan="2">ULS *</th> <th rowspan="2">Ultimate</th> </tr> <tr> <th>( Mpa )</th> <th>( Kpa )</th> <th>( Mpa )</th> <th>( Mpa )</th> </tr> </thead> <tbody> <tr><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td></tr> <tr><td>Vegetable soil</td><td>0.00 - 0.30 m</td><td></td><td></td><td></td><td></td></tr> <tr><td>20</td><td></td><td></td><td>0.12</td><td>120</td><td>12</td><td>25</td></tr> <tr><td>40</td><td></td><td></td><td>0.05</td><td>50</td><td>5</td><td>50</td></tr> <tr><td>60</td><td></td><td></td><td>0.30</td><td>300</td><td>30</td><td>45</td></tr> <tr><td>80</td><td></td><td></td><td>0.20</td><td>200</td><td>20</td><td>55</td></tr> <tr><td>100</td><td></td><td></td><td>0.15</td><td>150</td><td>15</td><td>50</td></tr> <tr><td>120</td><td></td><td></td><td>0.20</td><td>200</td><td>20</td><td>48</td></tr> <tr><td>140</td><td></td><td></td><td>0.15</td><td>150</td><td>15</td><td>46</td></tr> <tr><td>160</td><td></td><td></td><td>0.30</td><td>300</td><td>30</td><td>48</td></tr> <tr><td>180</td><td></td><td></td><td>0.65</td><td>650</td><td>65</td><td>95</td></tr> <tr><td>200</td><td></td><td></td><td>0.70</td><td>700</td><td>70</td><td>140</td></tr> <tr><td>220</td><td></td><td></td><td>0.65</td><td>650</td><td>65</td><td>160</td></tr> <tr><td>240</td><td></td><td></td><td>0.50</td><td>500</td><td>50</td><td>155</td></tr> <tr><td>260</td><td></td><td></td><td>0.45</td><td>450</td><td>45</td><td>150</td></tr> <tr><td>280</td><td></td><td></td><td>0.50</td><td>500</td><td>50</td><td>180</td></tr> <tr><td>300</td><td></td><td></td><td>0.45</td><td>450</td><td>45</td><td>215</td></tr> <tr><td>320</td><td></td><td></td><td>0.50</td><td>500</td><td>50</td><td>220</td></tr> <tr><td>340</td><td></td><td></td><td>0.60</td><td>600</td><td>60</td><td>260</td></tr> <tr><td>360</td><td></td><td></td><td>0.65</td><td>650</td><td>65</td><td>300</td></tr> <tr><td>380</td><td></td><td></td><td>0.70</td><td>700</td><td>70</td><td>310</td></tr> <tr><td>400</td><td></td><td></td><td>0.70</td><td>700</td><td>70</td><td>350</td></tr> <tr><td>420</td><td></td><td></td><td>0.70</td><td>700</td><td>70</td><td>420</td></tr> <tr><td>440</td><td></td><td></td><td>0.75</td><td>750</td><td>75</td><td>500</td></tr> <tr><td>460</td><td></td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>480</td><td></td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>500</td><td></td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>520</td><td></td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>540</td><td></td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>560</td><td></td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>580</td><td></td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>620</td><td></td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>640</td><td></td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>660</td><td></td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>680</td><td></td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>700</td><td></td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>720</td><td></td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>740</td><td></td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>760</td><td></td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>780</td><td></td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>800</td><td></td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>820</td><td></td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>840</td><td></td><td></td><td></td><td></td><td></td><td></td></tr> </tbody> </table>				DESCRIPTION OF THE PIT	Depth (below existing ground level) / cm	Soil Stratigraphy (observed in the well)	GEO TECHNICAL BEARING CAPACITIES				Allowable		ULS *	Ultimate	( Mpa )	( Kpa )	( Mpa )	( Mpa )	0	0	0	0	0	0	Vegetable soil	0.00 - 0.30 m					20			0.12	120	12	25	40			0.05	50	5	50	60			0.30	300	30	45	80			0.20	200	20	55	100			0.15	150	15	50	120			0.20	200	20	48	140			0.15	150	15	46	160			0.30	300	30	48	180			0.65	650	65	95	200			0.70	700	70	140	220			0.65	650	65	160	240			0.50	500	50	155	260			0.45	450	45	150	280			0.50	500	50	180	300			0.45	450	45	215	320			0.50	500	50	220	340			0.60	600	60	260	360			0.65	650	65	300	380			0.70	700	70	310	400			0.70	700	70	350	420			0.70	700	70	420	440			0.75	750	75	500	460							480							500							520							540							560							580							620							640							660							680							700							720							740							760							780							800							820							840						
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\* ULS : Ultimate Limit State

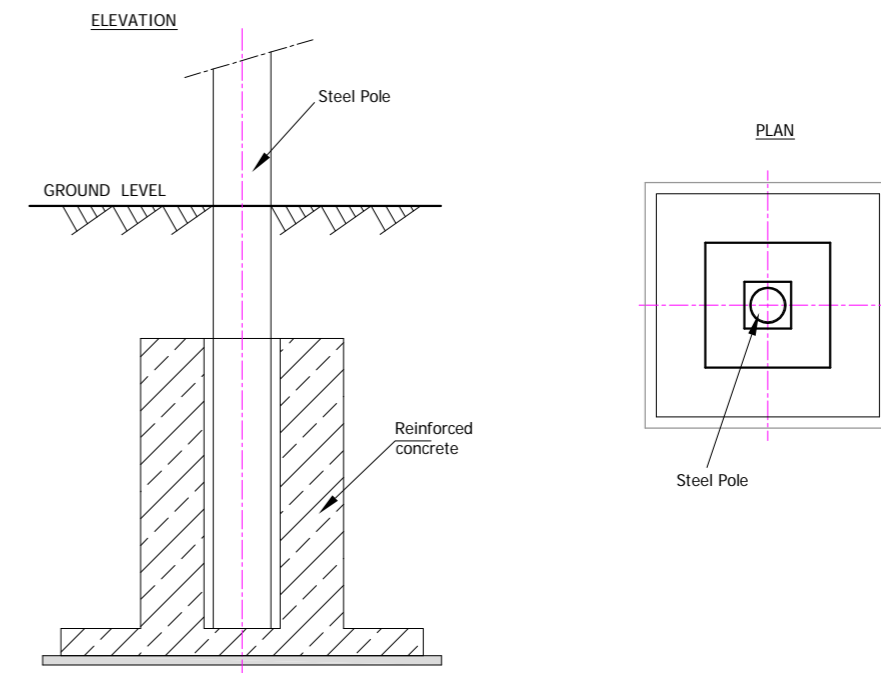
EXCAVATION ( test pit )



EXCAVATED SOIL



FOUNDATION TYPE



THE FOUNDATION DIMENSIONS COMPLY WITH THE STATIC BALANCE OF POST FIXING .



Soil investigation point	Extraction depth of the sample	Classification according USCS	Observation	ENGINEERING PROPERTIES	EXCAVATION CONDITION	GEOLOGICAL UNIT Light penetrometer ( GeoMIL 50 kN SPT ), test without soil coring for sample taking						
						DESCRIPTION OF THE PIT		GEOTECHNICAL BEARING CAPACITIES		ULS *		Ultimate
						Allowable		( Mpa )		( Mpa )		
						( Mpa )	( Kpa )	( Mpa )	( Mpa )			
<b>SIP 4</b> (15 kV line Gasogi - Nyagasambu)	3.60 - 4.00 m	Clay of intermediate plasticity ( CI )	Sandy soil	- IP : 21  - Consistency $I_c = 1.3 > 1$ : very consistent ground  - Compaction Backfill : $\rho_{dOPM} = 1.90 \text{ t/m}^3$ $W_{OPM} = 14.20 \%$  - Natural soil : $\rho_h = 1.64 \text{ t/m}^3$ $W_{(N)} = 20.0 \%$ $\rho_s = 2.72 \text{ t/m}^3$	- Shear test : $C_{uu} = 31 \text{ kN/m}^2$ $\psi_{uu} = 26.57^\circ$  - Chemical analysis : $pH = 6.98$ $SO_4^2 = 22 \text{ mg/L}$ $Cl^- = 159.3 \text{ mg/L}$	Picks and shovels can be used.	Vegetable soil 0.00 - 0.30 m	0 20 40 60 80 100 120 140 160 180 200 220 240 260 280 300 320 340 360 380 400 420 440 460 480 500 520 540 560 580 600 620 640 660 680 700 720 740 760 780 800 820 840	0 0.10 0.50 0.35 0.25 0.20 0.13 0.15 0.25 0.20 0.25 0.60 0.20 0.20 0.18 0.20 0.22 0.25 0.15 0.30 0.35 0.38 0.70 0.75 0.80 1.50	0 100 500 350 250 200 130 150 250 200 600 200 200 180 200 220 250 150 300 350 380 700 750 800 1500	0 10 50 35 25 20 13 15 25 20 60 20 20 18 20 22 25 15 30 35 38 70 75 80 150	0 30 65 55 58 43 42 55 55 62 70 80 90 100 100 110 103 130 155 180 213 240 265 350 400 500

\* ULS : Ultimate Limit State

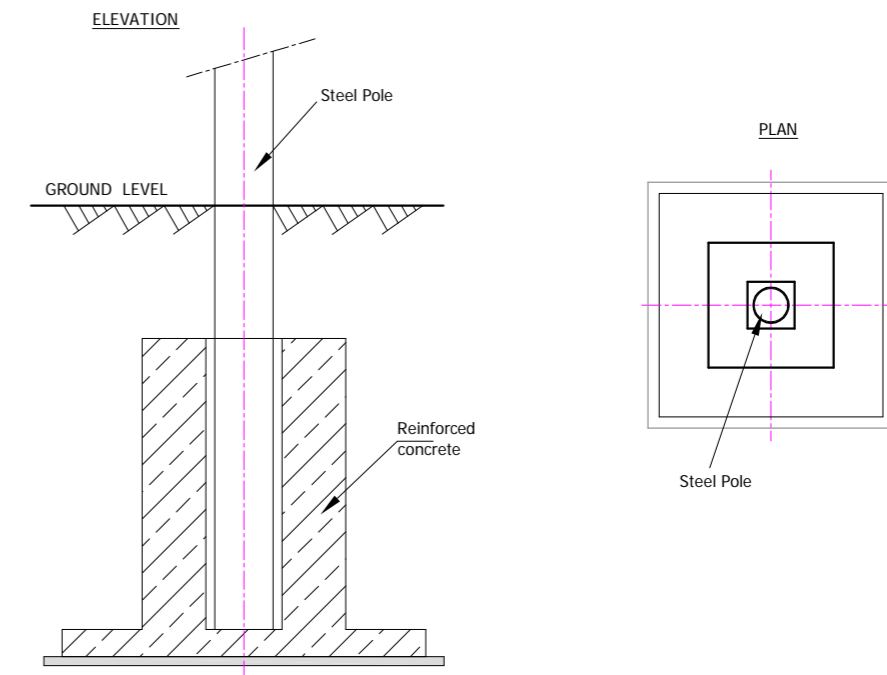
EXCAVATION ( test pit )



EXCAVATED SOIL



FOUNDATION TYPE



THE FOUNDATION DIMENSIONS COMPLY WITH THE STATIC BALANCE OF POST FIXING .

Soil investigation point	Extraction depth of the sample	Classification according USCS	Observation	ENGINEERING PROPERTIES		EXCAVATION CONDITION	GEOLOGICAL UNIT Light penetrometer ( GeoMIL 50 kN SPT ), test without soil coring for sample taking					
							DESCRIPTION OF THE PIT	Depth (below existing ground level) (cm)	Soil Stratigraphy (observed in the well)	Allowable		ULS *
(Mpa)	(Kpa)	(Mpa)	(Mpa)									
<b>SIP 5</b> (15 kV line Gasogi - Nyagasambu)	1.00 - 2.00 m	Clay of intermediate plasticity (CI)	Lateritic zone	- IP : 17.5 - Consistency $I_c = 1.32 > 1$ : very consistent ground - Compaction Backfill : $\rho_{dOPM} = 2.02 \text{ t/m}^3$ $W_{OPM} = 13.80 \%$ - Natural soil : $\rho_h = 1.65 \text{ t/m}^3$ $W_{(N)} = 15.0 \%$ $\rho_s = 2.90 \text{ t/m}^3$	- Shear test : $C_{uu} = 18 \text{ kN/m}^2$ $\psi_{uu} = 29.03^\circ$ - Chemical analysis : $pH = 5.4$ $SO_4^{2-} = 16.6 \text{ mg/L}$ $Cl^- = 77.8 \text{ mg/L}$	Picks and shovels can be used.	Vegetable soil	0	0	0	0	0
							0.00 - 0.30 m	20	0.10	100	10	30
							Black clay of intermediate plasticity	40	0.50	500	50	65
								60	0.35	350	35	55
								80	0.25	250	25	58
								100	0.20	200	20	43
								120	0.13	130	13	42
								140	0.15	150	15	55
							Red Clay of intermediate plasticity	160	0.25	250	25	55
								180	0.20	200	20	62
								200	0.25	250	25	70
								220	0.60	600	60	80
								240	0.20	200	20	90
								260	0.20	200	20	100
								280	0.18	180	18	100
								300	0.20	200	20	110
								320	0.22	220	22	103
								340	0.25	250	25	130
								360	0.15	150	15	155
								380	0.30	300	30	180
400	0.35	350	35	213								
420	0.38	380	38	240								
440	0.70	700	70	265								
460	0.75	750	75	350								
480	0.80	800	80	400								
500	1.50	1500	150	500								
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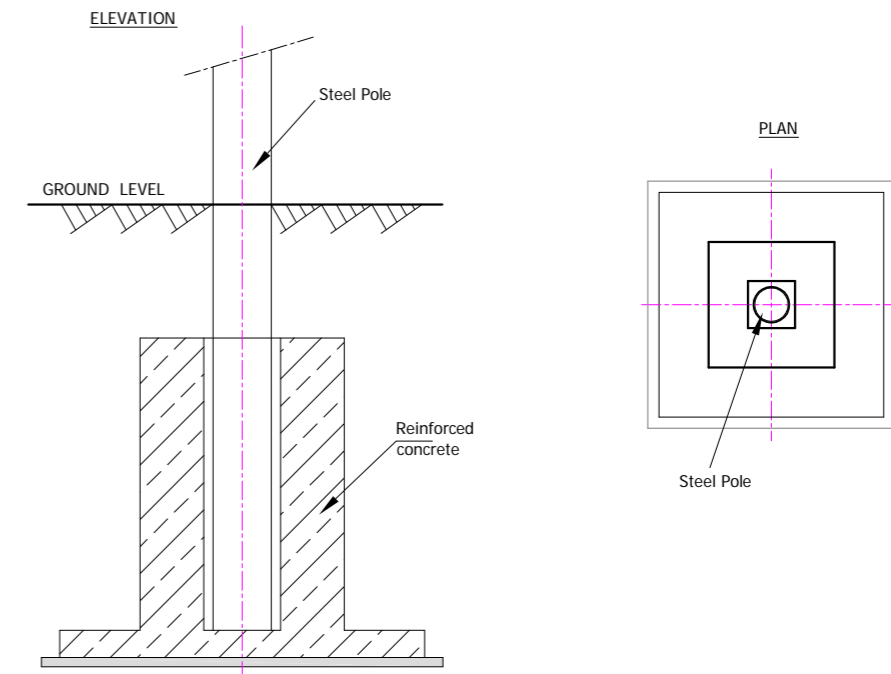
EXCAVATION ( test pit )



EXCAVATED SOIL



FOUNDATION TYPE



THE FOUNDATION DIMENSIONS COMPLY WITH THE STATIC BALANCE OF POST FIXING .

Soil investigation point	Extraction depth of the sample	Classification according USCS	Observation	ENGINEERING PROPERTIES	EXCAVATION CONDITION	GEOLOGICAL UNIT																																																																																																																																																																																																																																																																													
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SIP 6 (15 kV line Gasogi - Nyagasambu)	3.60 - 4.00 m	Clayey of gravel poorly graded ( GC )	Lateritic zone	- IP : 15.7 - Consistency $I_c = 1.60 > 1$ : very consistent ground - Compaction Backfill : $d_{OPM} = 2.0 \text{ t/m}^3$ $W_{OPM} = 11.20 \%$ - Natural soil : $\rho_h = 1.82 \text{ t/m}^3$ $W_{(N)} = 14.2 \%$ $\rho_s = 2.68 \text{ t/m}^3$	- Shear test : $C_{uu} = 6 \text{ kN/m}^2$ $\psi_{uu} = 38.48^\circ$ - Chemical analysis : $pH = 4.34$ $SO_4^2 = 6.7 \text{ mg/L}$ $Cl^- = 85 \text{ mg/L}$	Picks and shovels can be used.	<table border="1"> <thead> <tr> <th rowspan="3">DESCRIPTION OF THE PIT</th> <th rowspan="3">Depth (below existing ground level) (cm)</th> <th rowspan="3">Soil Straightness (observed in the well)</th> <th colspan="4">GEO TECHNICAL BEARING CAPACITIES</th> </tr> <tr> <th colspan="2">Allowable</th> <th rowspan="2">ULS * (Mpa)</th> <th rowspan="2">Ultimate (Mpa)</th> </tr> <tr> <th>(Mpa)</th> <th>(Kpa)</th> </tr> </thead> <tbody> <tr> <td>Vegetable soil</td> <td>0</td> <td></td> <td>0</td> <td>0</td> <td>0</td> <td>0</td> </tr> <tr> <td rowspan="16">Red Clayey of gravel poorly graded</td> <td>20</td> <td></td> <td>0.05</td> <td>50</td> <td>5</td> <td>35</td> </tr> <tr> <td>40</td> <td></td> <td>0.15</td> <td>150</td> <td>15</td> <td>42</td> </tr> <tr> <td>60</td> <td></td> <td>0.30</td> <td>300</td> <td>30</td> <td>70</td> </tr> <tr> <td>80</td> <td></td> <td>0.80</td> <td>800</td> <td>80</td> <td>105</td> </tr> <tr> <td>100</td> <td></td> <td>0.80</td> <td>800</td> <td>80</td> <td>270</td> </tr> <tr> <td>120</td> <td></td> <td>1.00</td> <td>1000</td> <td>100</td> <td>380</td> </tr> <tr> <td>140</td> <td></td> <td>1.50</td> <td>1500</td> <td>150</td> <td>500</td> </tr> <tr> <td>160</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>180</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>200</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>220</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>240</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>260</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>280</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>300</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>320</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>340</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>360</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>380</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>400</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>420</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>440</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>460</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>480</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>500</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>520</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>540</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>560</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>580</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>620</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>640</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>660</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>680</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>700</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>720</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>740</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>760</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>780</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>800</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>820</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>840</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> </tbody> </table>	DESCRIPTION OF THE PIT	Depth (below existing ground level) (cm)	Soil Straightness (observed in the well)	GEO TECHNICAL BEARING CAPACITIES				Allowable		ULS * (Mpa)	Ultimate (Mpa)	(Mpa)	(Kpa)	Vegetable soil	0		0	0	0	0	Red Clayey of gravel poorly graded	20		0.05	50	5	35	40		0.15	150	15	42	60		0.30	300	30	70	80		0.80	800	80	105	100		0.80	800	80	270	120		1.00	1000	100	380	140		1.50	1500	150	500	160						180						200						220						240						260						280						300						320						340						360						380						400						420						440						460						480						500						520						540						560						580						620						640						660						680						700						720						740						760						780						800						820						840						* ULS : Ultimate Limit State
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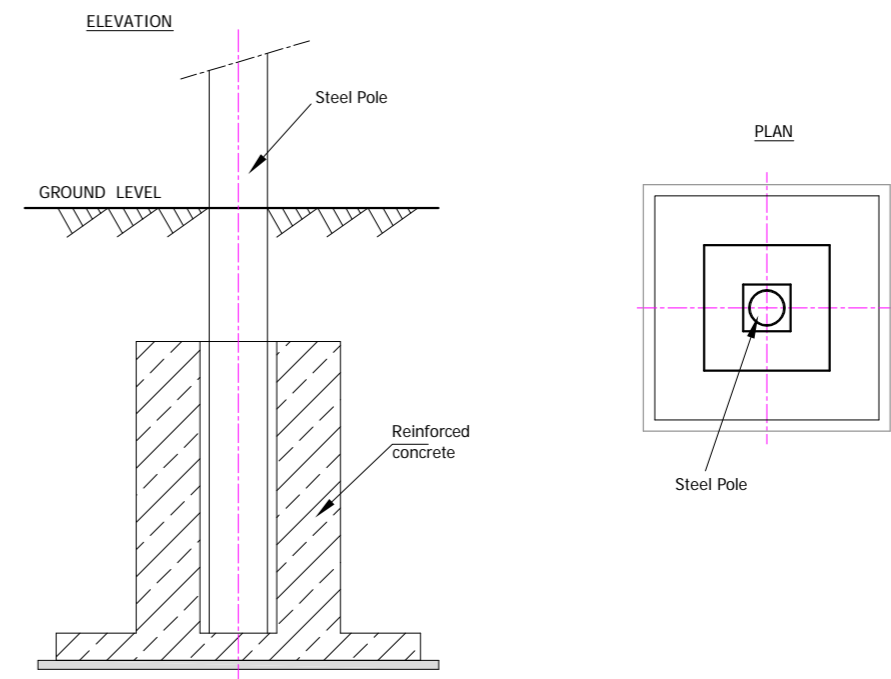
EXCAVATION ( test pit )



EXCAVATED SOIL



FOUNDATION TYPE



THE FOUNDATION DIMENSIONS COMPLY WITH THE STATIC BALANCE OF POST FIXING .

Soil investigation point	Extraction depth of the sample	Classification according USCS	Observation	ENGINEERING PROPERTIES		EXCAVATION CONDITION	GEOLOGICAL UNIT Light penetrometer ( GeoMIL 50 kN SPT ), test without soil coring for sample taking							
				- IP : 15.4 - Consistency $I_c = 1.67 > 1$ : very consistent ground - Compaction Backfill : . $d_{OPM} = 2.03 \text{ t/m}^3$ . $W_{OPM} = 10.0 \%$	- Shear test : . $C_{uu} = 14 \text{ kN/m}^2$ . $\psi_{uu} = 24.23^\circ$		DESCRIPTION OF THE PIT	Depth (below existing ground level) cm	Soil Stratigraphy (observed in the well)	GEOTECHNICAL BEARING CAPACITIES		ULS *	Ultimate	
( Mpa )	( Kpa )	( Mpa )	( Mpa )											
<b>SIP 7</b> (15 kV line Gasogi - Nyagasambu)	3.60 - 4.00 m	Clayey of gravel poorly graded ( GC )	Lateritic zone	- Natural soil : . $\gamma_h = 1.84 \text{ t/m}^3$ . $W_{(N)} = 14.3 \%$ . $\gamma_s = 2.65 \text{ t/m}^3$ - Chemical analysis : . pH = 4.34 . $SO_4^2 = 6.7 \text{ mg/L}$ . $Cl^- = 85 \text{ mg/L}$	- Shear test : . $C_{uu} = 14 \text{ kN/m}^2$ . $\psi_{uu} = 24.23^\circ$	Picks and shovels can be used.	Vegetable soil 0.00 - 0.30 m	0	0	0	0	0	0	* ULS : Ultimate Limit State
							20	0.15	150	15	23			
							40	0.30	300	30	45			
							60	0.60	600	60	80			
							80	0.25	250	25	50			
							100	0.70	700	70	100			
							120	0.80	800	80	120			
							140	0.90	900	90	200			
							160	1.05	1050	105	280			
							180	1.10	1100	110	325			
							200	1.20	1200	120	360			
							220	1.35	1350	135	400			
							240	1.50	1500	150	500			
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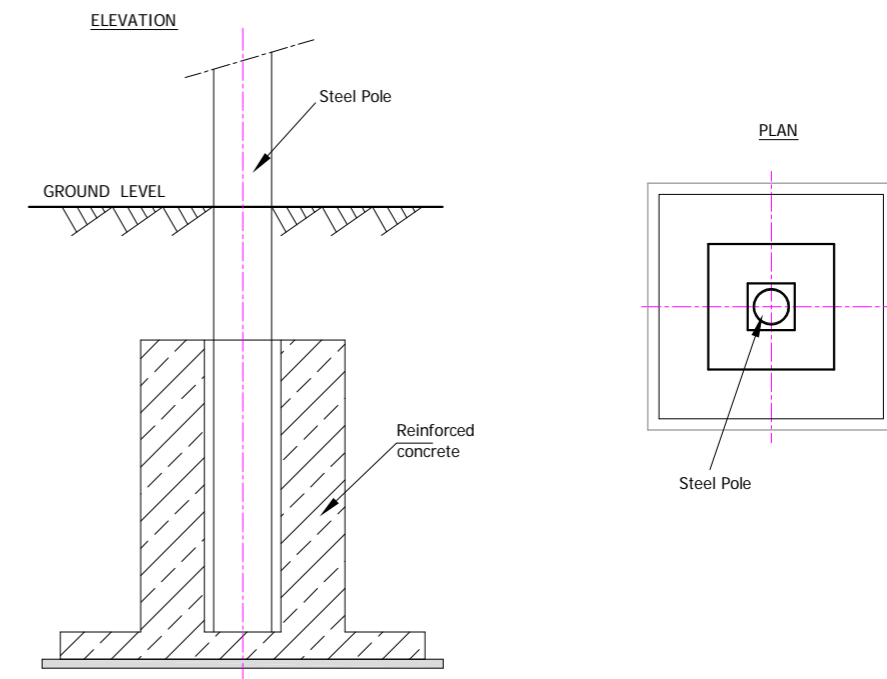
EXCAVATION ( test pit )



EXCAVATED SOIL



FOUNDATION TYPE



THE FOUNDATION DIMENSIONS COMPLY WITH THE STATIC BALANCE OF POST FIXING .

Soil investigation point	Extraction depth of the sample	Classification according USCS	Observation	ENGINEERING PROPERTIES	EXCAVATION CONDITION	GEOLOGICAL UNIT																																																																																																																																																																																																																																																																														
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SIP 9 (15 kV line Gasogi - Nyagasambu)	1.50 - 2.00 m	Clay of intermediate plasticity (CI)	Lateritic zone	- IP : 17.7 - Consistency $I_c = 1.29 > 1$ : very consistent ground - Compaction Backfill : . $d_{OPM} = 1.9 \text{ t/m}^3$ . $W_{OPM} = 14.20 \%$ - Natural soil : . $\rho_h = 1.78 \text{ t/m}^3$ . $W_{(N)} = 15.0 \%$ . $\rho_s = 2.72 \text{ t/m}^3$	- Shear test : . $C_{uu} = 25 \text{ kN/m}^2$ . $\psi_{uu} = 29.90^\circ$ - Chemical analysis : . pH = 7.03 . $SO_4^2 = 12 \text{ mg/L}$ . $Cl^- = 226.9 \text{ mg/L}$	Picks and shovels can be used.	<table border="1"> <thead> <tr> <th rowspan="3">DESCRIPTION OF THE PIT</th> <th rowspan="3">Depth (below existing ground level) cm</th> <th rowspan="3">Soil Stratigraphy (observed in the well)</th> <th colspan="4">GEOTECHNICAL BEARING CAPACITIES</th> </tr> <tr> <th colspan="2">Allowable</th> <th rowspan="2">ULS *</th> <th rowspan="2">Ultimate</th> </tr> <tr> <th>(Mpa)</th> <th>(Kpa)</th> <th>(Mpa)</th> <th>(Mpa)</th> </tr> </thead> <tbody> <tr><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td></tr> <tr><td rowspan="5">Vegetable soil 0.00 - 0.50 m</td><td>20</td><td></td><td>0.20</td><td>200</td><td>20</td><td>30</td></tr> <tr><td>40</td><td></td><td>0.10</td><td>100</td><td>10</td><td>20</td></tr> <tr><td>60</td><td></td><td>0.55</td><td>550</td><td>55</td><td>90</td></tr> <tr><td>80</td><td></td><td>0.80</td><td>800</td><td>80</td><td>190</td></tr> <tr><td>100</td><td></td><td>0.90</td><td>900</td><td>90</td><td>240</td></tr> <tr><td rowspan="14">Red Clayey of gravel poorly graded 0.50 - 4.00 m</td><td>120</td><td></td><td>1.00</td><td>1000</td><td>100</td><td>360</td></tr> <tr><td>140</td><td></td><td>1.50</td><td>1500</td><td>150</td><td>500</td></tr> <tr><td>160</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>180</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>200</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>220</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>240</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>260</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>280</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>300</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>320</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>340</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>360</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>380</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>400</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>420</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>440</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>460</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>480</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>500</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>520</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>540</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>560</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>580</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>620</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>640</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>660</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>700</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>720</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>740</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>760</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>780</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>800</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>820</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>840</td><td></td><td></td><td></td><td></td><td></td></tr> </tbody> </table>						DESCRIPTION OF THE PIT	Depth (below existing ground level) cm	Soil Stratigraphy (observed in the well)	GEOTECHNICAL BEARING CAPACITIES				Allowable		ULS *	Ultimate	(Mpa)	(Kpa)	(Mpa)	(Mpa)	0	0	0	0	0	0	0	Vegetable soil 0.00 - 0.50 m	20		0.20	200	20	30	40		0.10	100	10	20	60		0.55	550	55	90	80		0.80	800	80	190	100		0.90	900	90	240	Red Clayey of gravel poorly graded 0.50 - 4.00 m	120		1.00	1000	100	360	140		1.50	1500	150	500	160						180						200						220						240						260						280						300						320						340						360						380						400						420						440						460						480						500						520						540						560						580						620						640						660						700						720						740						760						780						800						820						840					
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\* ULS : Ultimate Limit State

EXCAVATION ( test pit )

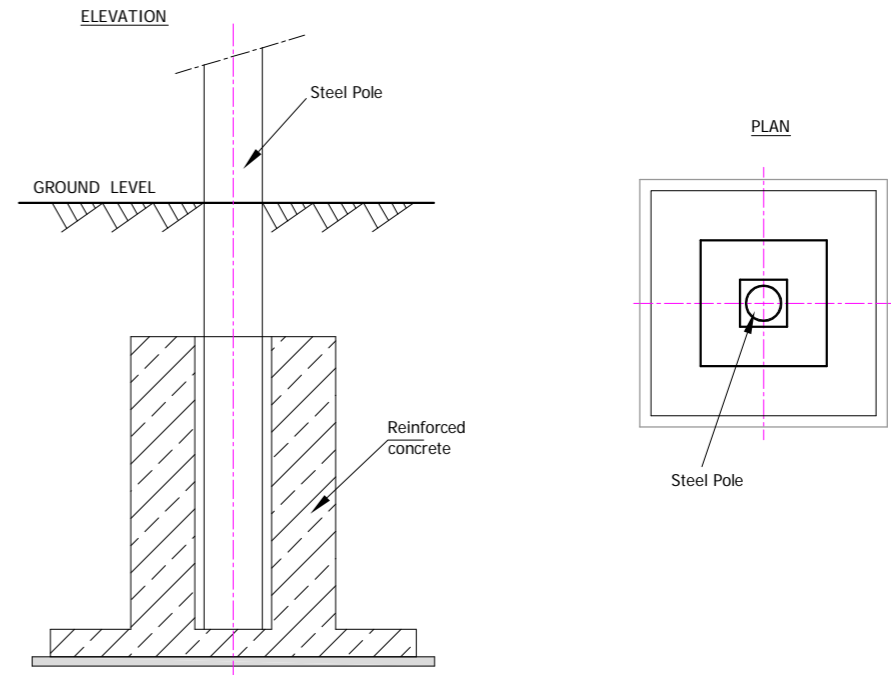


EXCAVATED SOIL



27

FOUNDATION TYPE



THE FOUNDATION DIMENSIONS COMPLY WITH THE STATIC BALANCE OF POST FIXING .

Soil investigation point	Extraction depth of the sample	Classification according USCS	Observation	ENGINEERING PROPERTIES	EXCAVATION CONDITION	GEOLOGICAL UNIT																													
						Light penetrometer ( GeoMIL 50 kN SPT ), test without soil coring for sample taking																													
SIP 10 (15 kV line Gasogi - Nyagasambu)	3.00 - 3.50 m	Brown clay of intermediate plasticity ( CI )	Lateritic zone	- IP : 19.1 - Consistency $I_c = 1.21 > 1$ : very consistent ground - Compaction Backfill : . $d_{OPM} = 1.94 \text{ t/m}^3$ . $W_{OPM} = 13.20 \%$ - Natural soil : . $\rho_h = 1.61 \text{ t/m}^3$ . $W_{(N)} = 15.0 \%$ . $\rho_s = 2.65 \text{ t/m}^3$	- Shear test : . $C_{uu} = 17 \text{ kN/m}^2$ . $\psi_{uu} = 24.90^\circ$ - Chemical analysis : . pH = 6.71 . $SO_4^2 = 14.7 \text{ mg/L}$ . $Cl^- = 196.7 \text{ mg/L}$	Picks and shovels can be used.	<table border="1"> <thead> <tr> <th rowspan="3">DESCRIPTION OF THE PIT</th> <th rowspan="3">Depth (below existing ground level) (m)</th> <th rowspan="3">Soil Stratigraphy (observed in the well)</th> <th colspan="4">GEOTECHNICAL BEARING CAPACITIES</th> </tr> <tr> <th colspan="2">Allowable</th> <th rowspan="2">ULS * (Mpa)</th> <th rowspan="2">Ultimate (Mpa)</th> </tr> <tr> <th>(Mpa)</th> <th>(Kpa)</th> </tr> </thead> <tbody> <tr> <td>Vegetable soil 0.00 - 0.30 m</td> <td>0 20 40 60 80 100 120 140 160</td> <td></td> <td>0 0.20 0.20 0.50 0.85 1.05 1.20 1.50</td> <td>0 200 200 500 850 1050 1200 1500</td> <td>0 20 20 50 85 105 120 150</td> <td>0 25 30 80 130 260 340 500</td> </tr> <tr> <td>Brown Clayey of gravel poorly graded 0.30 - 4.00 m</td> <td>180 200 220 240 260 280 300 320 340 360 380 400 420 440 460 480 500 520 540 560 580 620 640 660 680 700 720 740 760 780 800 820 840</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> </tbody> </table>		DESCRIPTION OF THE PIT	Depth (below existing ground level) (m)	Soil Stratigraphy (observed in the well)	GEOTECHNICAL BEARING CAPACITIES				Allowable		ULS * (Mpa)	Ultimate (Mpa)	(Mpa)	(Kpa)	Vegetable soil 0.00 - 0.30 m	0 20 40 60 80 100 120 140 160		0 0.20 0.20 0.50 0.85 1.05 1.20 1.50	0 200 200 500 850 1050 1200 1500	0 20 20 50 85 105 120 150	0 25 30 80 130 260 340 500	Brown Clayey of gravel poorly graded 0.30 - 4.00 m	180 200 220 240 260 280 300 320 340 360 380 400 420 440 460 480 500 520 540 560 580 620 640 660 680 700 720 740 760 780 800 820 840					
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Allowable		ULS * (Mpa)	Ultimate (Mpa)																																
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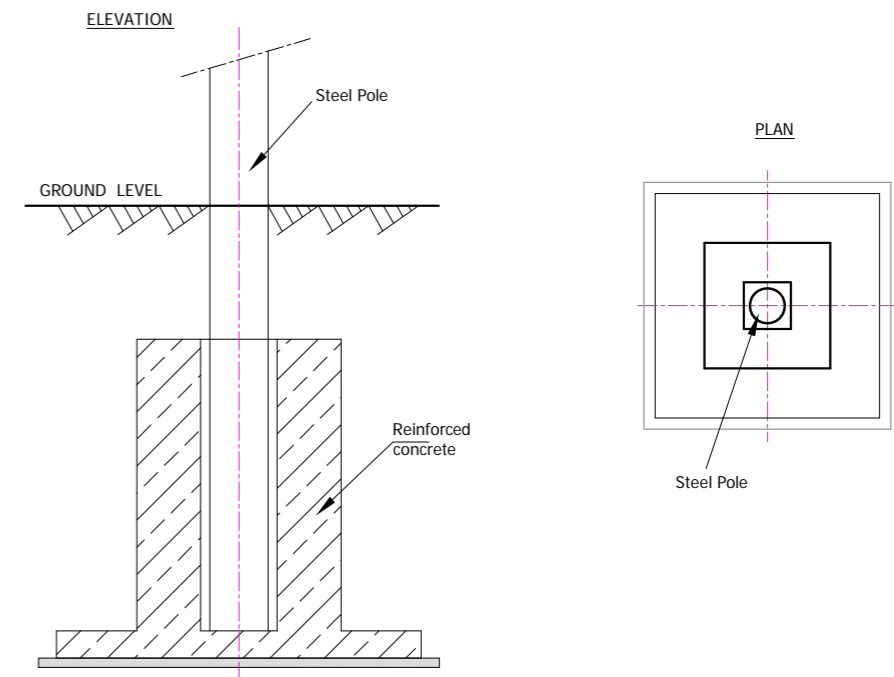
EXCAVATION ( test pit )



EXCAVATED SOIL



FOUNDATION TYPE



THE FOUNDATION DIMENSIONS COMPLY WITH THE STATIC BALANCE OF POST FIXING .

Soil investigation point	Extraction depth of the sample	Classification according USCS	Observation	ENGINEERING PROPERTIES		EXCAVATION CONDITION	GEOLOGICAL UNIT Light penetrometer ( GeoMIL 50 kN SPT ), test without soil coring for sample taking				
							GEOTECHNICAL BEARING CAPACITIES				
								Allowable	ULS *	Ultimate	
						(Mpa)	(Kpa)	(Mpa)	(Mpa)		
<b>SIP 11</b> (15 kV line Gasogi - Nyagasambu)	3.50 - 4.00 m	Clayey of gravel poorly graded (GC)	Lateritic zone	- IP : 15.7	- Shear test : . Cuu = 21 kN/m <sup>2</sup> . Ψ <sub>uu</sub> = 24.94°	Picks and shovels can be used.	Vegetable soil	0	0	0	0
				0.00 - 0.30 m			20	0.10	100	10	15
				Black Clayey of intermediate plasticity			40	0.45	450	45	55
							60	0.70	700	70	100
							80	0.60	600	60	140
							100	0.85	850	85	200
							120	0.60	600	60	230
							140	0.90	900	90	280
				Red Clayey gravel poorly graded			160	0.80	800	80	340
							180	0.60	600	60	340
							200	0.70	700	70	360
							220	0.65	650	65	380
							240	1.00	1000	100	390
							260	1.50	1500	150	500
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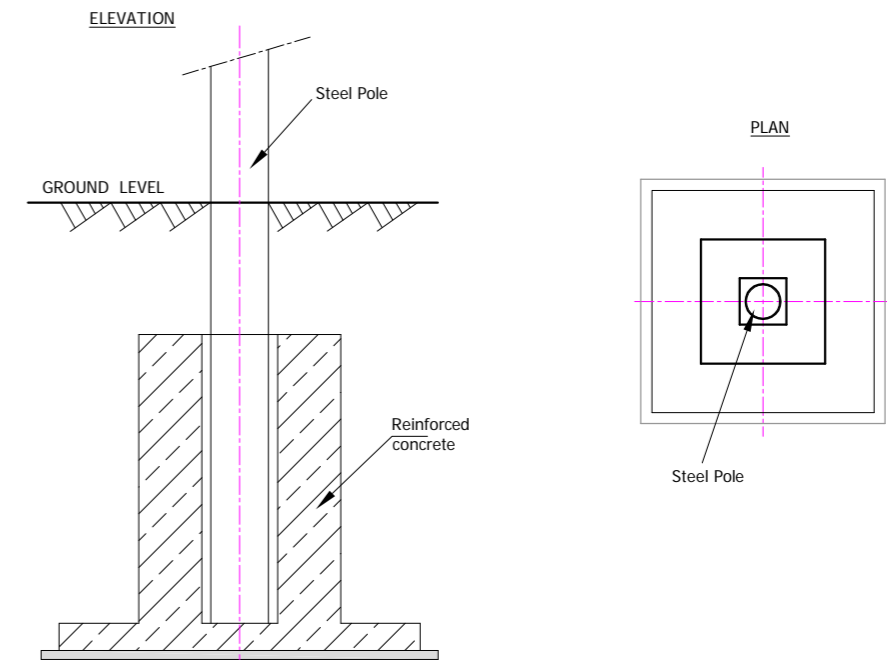
EXCAVATION ( test pit )



EXCAVATED SOIL



FOUNDATION TYPE



THE FOUNDATION DIMENSIONS COMPLY WITH THE STATIC BALANCE OF POST FIXING .

Soil investigation point	Extraction depth of the sample	Classification according USCS	Observation	ENGINEERING PROPERTIES	EXCAVATION CONDITION	GEOLOGICAL UNIT																																																																																																																																																																																																																																																																																													
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SIP 12 (15 kV line Gasogi - Nyagasambu)	3.60 - 4.00 m	Clayey of gravel poorly graded (GC)	Lateritic zone	- IP : 14.7  - Consistency $I_c = 1.63 > 1$ : very consistent ground  - Compaction Backfill : . $d_{OPM} = 2.0 \text{ t/m}^3$ . $W_{OPM} = 10.40 \%$  - Natural soil : . $\rho_h = 1.81 \text{ t/m}^3$ . $W_{(N)} = 13.2 \%$ . $\rho_s = 2.65 \text{ t/m}^3$	- Shear test : . $C_{uu} = 1 \text{ kN/m}^2$ . $\psi_{uu} = 39.18^\circ$  - Chemical analysis : . $pH = 6.09$ . $SO_4^2 = 12.2 \text{ mg/L}$ . $Cl^- = 178 \text{ mg/L}$	Picks and shovels can be used.	<table border="1"> <thead> <tr> <th rowspan="3">DESCRIPTION OF THE PIT</th> <th rowspan="3">Depth (below existing ground level) cm</th> <th rowspan="3">Soil Stratigraphy (observed in the well)</th> <th colspan="3">GEO TECHNICAL BEARING CAPACITIES</th> </tr> <tr> <th colspan="2">Allowable</th> <th rowspan="2">ULS *</th> <th rowspan="2">Ultimate</th> </tr> <tr> <th>(Mpa)</th> <th>(Kpa)</th> <th>(Mpa)</th> <th>(Mpa)</th> </tr> </thead> <tbody> <tr><td>Vegetable soil 0.00 - 0.50 m</td><td>0</td><td></td><td>0</td><td>0</td><td>0</td><td>0</td></tr> <tr><td></td><td>20</td><td></td><td>0.1</td><td>100</td><td>10</td><td>20</td></tr> <tr><td></td><td>40</td><td></td><td>0.25</td><td>250</td><td>25</td><td>45</td></tr> <tr><td></td><td>60</td><td></td><td>0.55</td><td>550</td><td>55</td><td>80</td></tr> <tr><td></td><td>80</td><td></td><td>0.70</td><td>700</td><td>70</td><td>105</td></tr> <tr><td rowspan="7">Brown Clayey of intermediate plasticity 0.50 - 2.00 m</td><td>100</td><td></td><td>0.45</td><td>450</td><td>45</td><td>130</td></tr> <tr><td>120</td><td></td><td>0.60</td><td>600</td><td>60</td><td>180</td></tr> <tr><td>140</td><td></td><td>0.60</td><td>600</td><td>60</td><td>240</td></tr> <tr><td>160</td><td></td><td>0.80</td><td>800</td><td>80</td><td>220</td></tr> <tr><td>180</td><td></td><td>0.85</td><td>850</td><td>85</td><td>360</td></tr> <tr><td>200</td><td></td><td>1.00</td><td>1000</td><td>100</td><td>390</td></tr> <tr><td>220</td><td></td><td>1.10</td><td>1100</td><td>110</td><td>400</td></tr> <tr><td rowspan="14">Red Clayey gravel poorly graded 2.00 - 4.00 m</td><td>240</td><td></td><td>1.50</td><td>1500</td><td>150</td><td>500</td></tr> <tr><td>260</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>280</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>300</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>320</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>340</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>360</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>380</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>400</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>420</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>440</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>460</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>480</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>500</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>520</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>540</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>560</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>580</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>600</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>620</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>640</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>660</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>680</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>700</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>720</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>740</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>760</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>780</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>800</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>820</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>840</td><td></td><td></td><td></td><td></td><td></td></tr> </tbody> </table>						DESCRIPTION OF THE PIT	Depth (below existing ground level) cm	Soil Stratigraphy (observed in the well)	GEO TECHNICAL BEARING CAPACITIES			Allowable		ULS *	Ultimate	(Mpa)	(Kpa)	(Mpa)	(Mpa)	Vegetable soil 0.00 - 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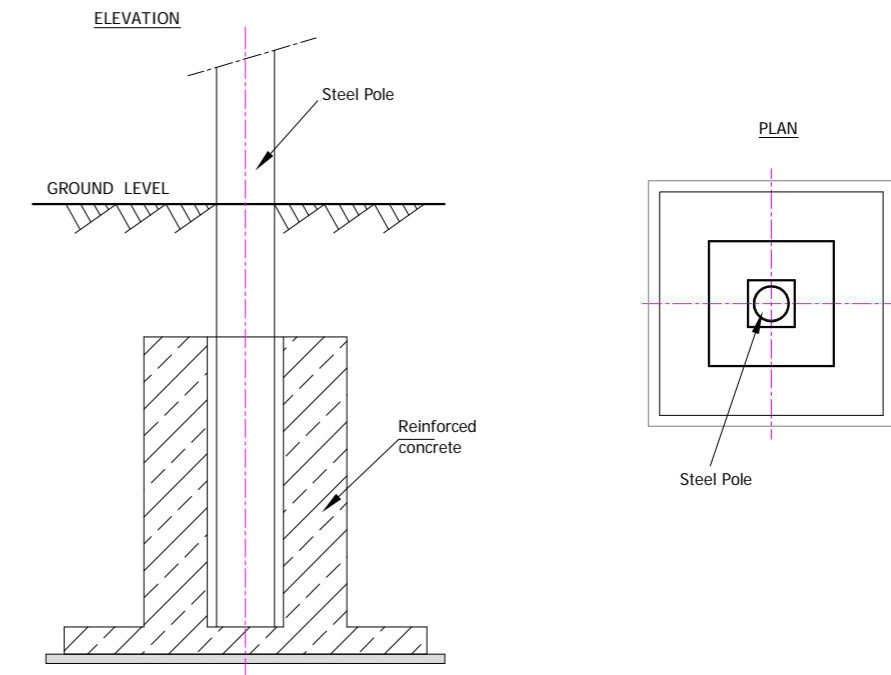
EXCAVATION ( test pit )



EXCAVATED SOIL



FOUNDATION TYPE



THE FOUNDATION DIMENSIONS COMPLY WITH THE STATIC BALANCE OF POST FIXING .



Soil investigation point	Extraction depth of the sample	Classification according USCS	Observation	ENGINEERING PROPERTIES	EXCAVATION CONDITION	GEOLOGICAL UNIT																																																																																																																																																																																																																																																																																						
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SIP 13 (15 kV line Gasogi - Nyagasambu)	3.60 - 4.00 m	Clay of intermediate plasticity ( CI )	Clayey soil	- IP : 21  - Consistency $I_c = 1.57 > 1$ : very consistent ground  - Compaction Backfill : $\rho_{dOPM} = 1.77 \text{ t/m}^3$ $W_{OPM} = 16.40 \%$  - Natural soil : $\rho_h = 1.62 \text{ t/m}^3$ $W_{(N)} = 13.3 \%$ $\rho_s = 2.60 \text{ t/m}^3$	- Shear test : $C_{uu} = 30 \text{ kN/m}^2$ $\psi_{uu} = 25.64^\circ$  - Chemical analysis : $pH = 6.43$ $SO_4^2 = 14.4 \text{ mg/L}$ $Cl^- = 200.3 \text{ mg/L}$	Picks and shovels can be used.	<table border="1"> <thead> <tr> <th rowspan="3">DESCRIPTION OF THE PIT</th> <th rowspan="3">Depth (below existing ground level) (cm)</th> <th rowspan="3">Soil Stratigraphy (observed in the well)</th> <th colspan="3">GEO TECHNICAL BEARING CAPACITIES</th> </tr> <tr> <th colspan="2">Allowable</th> <th rowspan="2">ULS *</th> <th rowspan="2">Ultimate</th> </tr> <tr> <th>(Mpa)</th> <th>(Kpa)</th> <th>(Mpa)</th> <th>(Mpa)</th> </tr> </thead> <tbody> <tr><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td></tr> <tr><td rowspan="6">Vegetable soil 0.00 - 0.50 m</td><td>20</td><td></td><td>0.05</td><td>50</td><td>5</td><td>13</td></tr> <tr><td>40</td><td></td><td>0.15</td><td>150</td><td>15</td><td>30</td></tr> <tr><td>60</td><td></td><td>0.45</td><td>450</td><td>45</td><td>60</td></tr> <tr><td>80</td><td></td><td>0.75</td><td>750</td><td>75</td><td>100</td></tr> <tr><td>100</td><td></td><td>0.45</td><td>450</td><td>45</td><td>135</td></tr> <tr><td>120</td><td></td><td>0.90</td><td>900</td><td>90</td><td>200</td></tr> <tr><td rowspan="14">Clayey of intermediate plasticity 0.50 - 4.00 m</td><td>140</td><td></td><td>0.60</td><td>600</td><td>60</td><td>220</td></tr> <tr><td>160</td><td></td><td>0.80</td><td>800</td><td>80</td><td>275</td></tr> <tr><td>180</td><td></td><td>0.75</td><td>750</td><td>75</td><td>330</td></tr> <tr><td>200</td><td></td><td>0.80</td><td>800</td><td>80</td><td>340</td></tr> <tr><td>220</td><td></td><td>1.00</td><td>1000</td><td>100</td><td>390</td></tr> <tr><td>240</td><td></td><td>1.25</td><td>1250</td><td>125</td><td>420</td></tr> <tr><td>260</td><td></td><td>1.50</td><td>1500</td><td>150</td><td>500</td></tr> <tr><td>280</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>300</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>320</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>340</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>360</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>380</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>400</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>420</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>440</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>460</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>480</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>500</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>520</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>540</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>560</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>580</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>600</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>620</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>640</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>660</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>680</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>700</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>720</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>740</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>760</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>780</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>800</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>820</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>840</td><td></td><td></td><td></td><td></td><td></td></tr> </tbody> </table>				DESCRIPTION OF THE PIT	Depth (below existing ground level) (cm)	Soil Stratigraphy (observed in the well)	GEO TECHNICAL BEARING CAPACITIES			Allowable		ULS *	Ultimate	(Mpa)	(Kpa)	(Mpa)	(Mpa)	0	0	0	0	0	0	Vegetable soil 0.00 - 0.50 m	20		0.05	50	5	13	40		0.15	150	15	30	60		0.45	450	45	60	80		0.75	750	75	100	100		0.45	450	45	135	120		0.90	900	90	200	Clayey of intermediate plasticity 0.50 - 4.00 m	140		0.60	600	60	220	160		0.80	800	80	275	180		0.75	750	75	330	200		0.80	800	80	340	220		1.00	1000	100	390	240		1.25	1250	125	420	260		1.50	1500	150	500	280						300						320						340						360						380						400						420						440						460						480						500						520						540						560						580						600						620						640						660						680						700						720						740						760						780						800						820						840					
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\* ULS : Ultimate Limit State

EXCAVATION ( test pit )

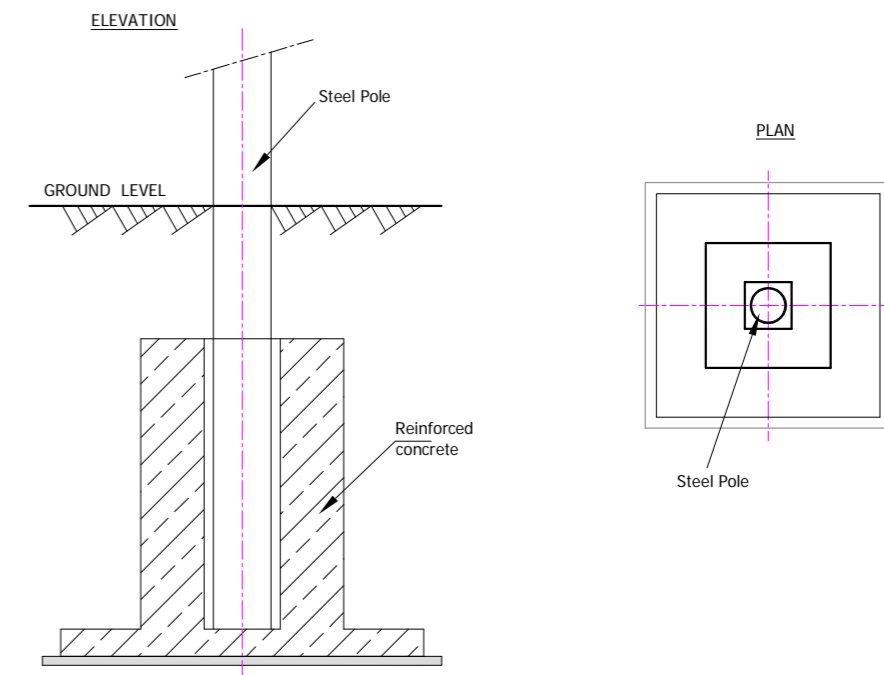


EXCAVATED SOIL



31

FOUNDATION TYPE



THE FOUNDATION DIMENSIONS COMPLY WITH THE STATIC BALANCE OF POST FIXING .

Soil investigation point	Extraction depth of the sample	Classification according USCS	Observation	ENGINEERING PROPERTIES	EXCAVATION CONDITION	GEOLOGICAL UNIT																																																																																																																																																																																																																																																																																			
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SIP 15 (15 kV line Gasogi - Nyagasambu)	3.60 - 4.00 m	Clay of intermediate plasticity ( CI )	Clayey soil	- IP : 16.3 - Consistency $I_c = 1.42 > 1$ : very consistent ground - Compaction Backfill : $\rho_{dOPM} = 2.01 \text{ t/m}^3$ $W_{OPM} = 12.40 \%$ - Natural soil : $\rho_h = 1.78 \text{ t/m}^3$ $W_{(N)} = 13.3 \%$ $\rho_s = 2.75 \text{ t/m}^3$	- Shear test : $c_{uu} = 12 \text{ kN/m}^2$ $\psi_{uu} = 30.54^\circ$ - Chemical analysis : $pH = 7.1$ $SO_4^{2-} = 19.4 \text{ mg/L}$ $Cl^- = 81 \text{ mg/L}$	Picks and shovels can be used.	<table border="1"> <thead> <tr> <th rowspan="3">DESCRIPTION OF THE PIT</th> <th rowspan="3">Depth (below existing ground level) cm</th> <th rowspan="3">Soil Stratigraphy (observed in the well)</th> <th colspan="4">GEO TECHNICAL BEARING CAPACITIES</th> </tr> <tr> <th colspan="2">Allowable</th> <th rowspan="2">ULS *</th> <th rowspan="2">Ultimate</th> </tr> <tr> <th>( Mpa )</th> <th>( Kpa )</th> <th>( Mpa )</th> <th>( Mpa )</th> </tr> </thead> <tbody> <tr><td>Vegetable soil</td><td>0</td><td></td><td>0</td><td>0</td><td>0</td><td>0</td></tr> <tr><td rowspan="16">Clayey of intermediate plasticity</td><td>20</td><td></td><td>0.15</td><td>150</td><td>15</td><td>25</td></tr> <tr><td>40</td><td></td><td>0.30</td><td>300</td><td>30</td><td>43</td></tr> <tr><td>60</td><td></td><td>0.45</td><td>450</td><td>45</td><td>70</td></tr> <tr><td>80</td><td></td><td>0.90</td><td>900</td><td>90</td><td>230</td></tr> <tr><td>100</td><td></td><td>1.25</td><td>1250</td><td>125</td><td>390</td></tr> <tr><td>120</td><td></td><td>1.50</td><td>1500</td><td>150</td><td>500</td></tr> <tr><td>140</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>160</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>180</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>200</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>220</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>240</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>260</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>280</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>300</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>320</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>340</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>360</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>380</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>400</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>420</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>440</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>460</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>480</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>500</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>520</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>540</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>560</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>580</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>620</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>640</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>660</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>680</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>700</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>720</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>740</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>760</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>780</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>800</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>820</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>840</td><td></td><td></td><td></td><td></td><td></td></tr> </tbody> </table>						DESCRIPTION OF THE PIT	Depth (below existing ground level) cm	Soil Stratigraphy (observed in the well)	GEO TECHNICAL BEARING CAPACITIES				Allowable		ULS *	Ultimate	( Mpa )	( Kpa )	( Mpa )	( Mpa )	Vegetable soil	0		0	0	0	0	Clayey of intermediate plasticity	20		0.15	150	15	25	40		0.30	300	30	43	60		0.45	450	45	70	80		0.90	900	90	230	100		1.25	1250	125	390	120		1.50	1500	150	500	140						160						180						200						220						240						260						280						300						320						340						360						380						400						420						440						460						480						500						520						540						560						580						620						640						660						680						700						720						740						760						780						800						820						840					
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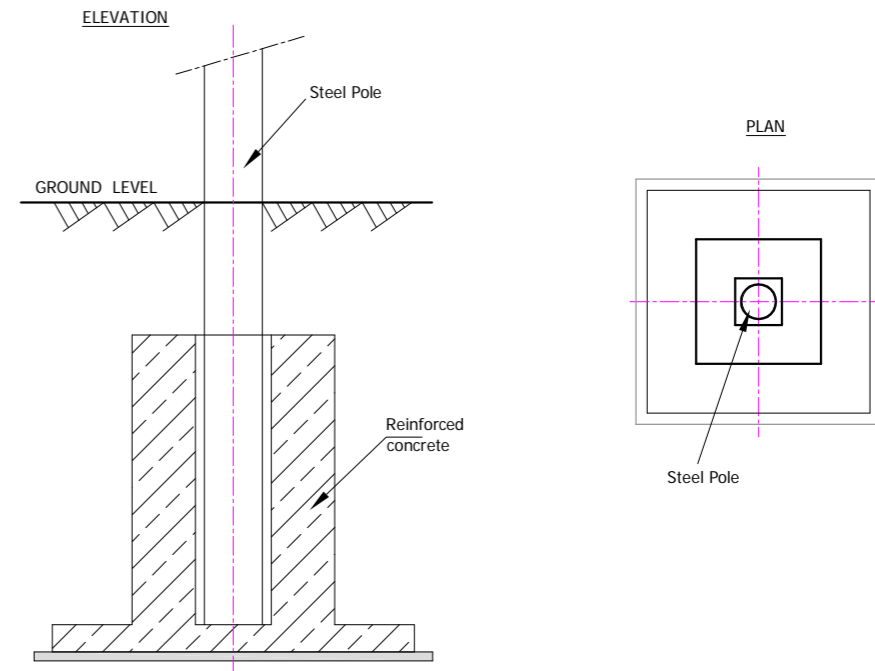
EXCAVATION ( test pit )



EXCAVATED SOIL



FOUNDATION TYPE



THE FOUNDATION DIMENSIONS COMPLY WITH THE STATIC BALANCE OF POST FIXING .

Soil investigation point	Extraction depth of the sample	Classification according USCS	Observation	ENGINEERING PROPERTIES	EXCAVATION CONDITION	GEOLOGICAL UNIT																																	
						Light penetrometer ( GeoMIL 50 kN SPT ), test without soil coring for sample taking																																	
SIP 16 (15 kV line Gasogi - Nyagasambu)	3.40 - 4.00 m	Clayey of gravel poorly graded ( GC )	Lateritic zone	- IP : 15.2 - Consistency $I_c = 1.63 > 1$ : very consistent ground - Compaction Backfill : $\rho_{dOPM} = 1.73 \text{ t/m}^3$ $W_{OPM} = 20.60 \%$ - Natural soil : $\rho_h = 1.93 \text{ t/m}^3$ $W_{(N)} = 18 \%$ $\rho_s = 2.80 \text{ t/m}^3$	- Shear test : $C_{uu} = 17 \text{ kN/m}^2$ $\psi_{uu} = 21.80^\circ$ - Chemical analysis : $\text{pH} = 4.73$ $\text{SO}_4^{2-} = 62.6 \text{ mg/L}$ $\text{Cl}^- = 213 \text{ mg/L}$	Picks and shovels can be used; preferably use pneumatic hammer.	<table border="1"> <thead> <tr> <th rowspan="3">DESCRIPTION OF THE PIT</th> <th rowspan="3">Depth (below existing ground level) (m)</th> <th rowspan="3">Soil Stratigraphy (observed in the well)</th> <th colspan="4">GEOTECHNICAL BEARING CAPACITIES</th> </tr> <tr> <th colspan="2">Allowable</th> <th rowspan="2">ULS *</th> <th rowspan="2">Ultimate</th> </tr> <tr> <th>(Mpa)</th> <th>(Kpa)</th> <th>(Mpa)</th> <th>(Mpa)</th> </tr> </thead> <tbody> <tr> <td>Vegetable soil 0.00 - 0.50 m</td> <td>0 20 40 60 80 100 120 140</td> <td></td> <td>0 0.20 0.15 0.80 1.00 1.20 1.50</td> <td>0 200 150 800 1000 1200 1500</td> <td>0 20 15 80 100 120 150</td> <td>0 40 30 120 350 400 500</td> </tr> <tr> <td>Clayey of intermediate plasticity 0.50 - 4.00 m</td> <td>160 180 200 220 240 260 280 300 320 340 360 380 400 420 440 460 480 500 520 540 560 580 620 640 660 680 700 720 740 760 780 800 820 840</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> </tbody> </table>				DESCRIPTION OF THE PIT	Depth (below existing ground level) (m)	Soil Stratigraphy (observed in the well)	GEOTECHNICAL BEARING CAPACITIES				Allowable		ULS *	Ultimate	(Mpa)	(Kpa)	(Mpa)	(Mpa)	Vegetable soil 0.00 - 0.50 m	0 20 40 60 80 100 120 140		0 0.20 0.15 0.80 1.00 1.20 1.50	0 200 150 800 1000 1200 1500	0 20 15 80 100 120 150	0 40 30 120 350 400 500	Clayey of intermediate plasticity 0.50 - 4.00 m	160 180 200 220 240 260 280 300 320 340 360 380 400 420 440 460 480 500 520 540 560 580 620 640 660 680 700 720 740 760 780 800 820 840					
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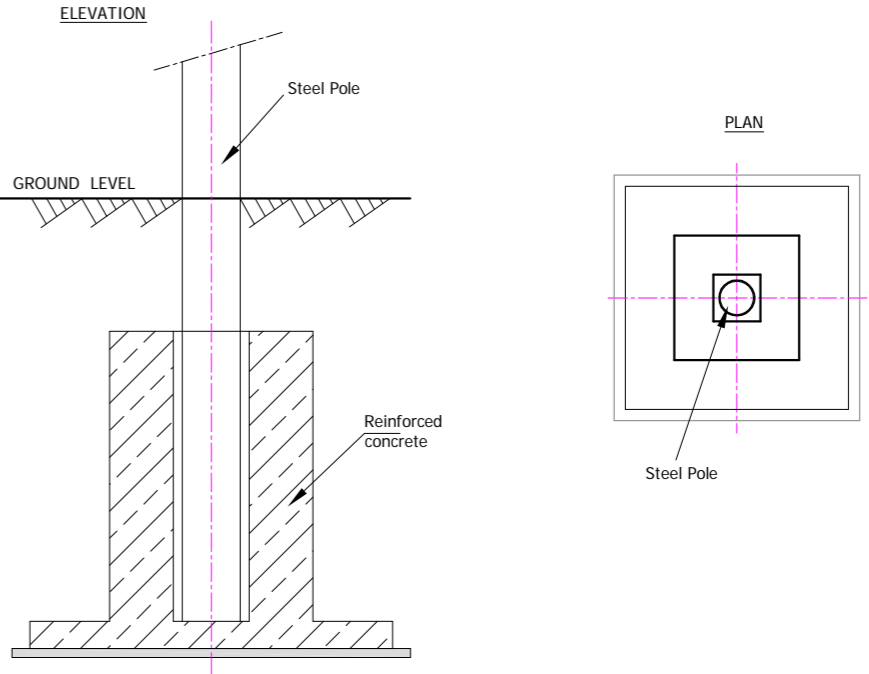
EXCAVATION ( test pit )



EXCAVATED SOIL



FOUNDATION TYPE



THE FOUNDATION DIMENSIONS COMPLY WITH THE STATIC BALANCE OF POST FIXING .

Soil investigation point	Extraction depth of the sample	Classification according USCS	Observation	ENGINEERING PROPERTIES	EXCAVATION CONDITION	GEOLOGICAL UNIT																																																																																																																																																																																																																																																																																		
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SIP 17 (15 kV line Gasogi - Nyagasambu)	0.50 - 1.00 m	Silt of intermediate plasticity ( MI )	Clayey soil	- IP : 20.1 - Consistency $I_c = 1.31 > 1$ : very consistent ground - Compaction Backfill : $\rho_{OPM} = 1.78 \text{ t/m}^3$ $W_{OPM} = 19.68 \%$ - Natural soil : $\rho_h = 1.76 \text{ t/m}^3$ $W_{(N)} = 19 \%$ $\rho_s = 2.84 \text{ t/m}^3$	- Shear test : $C_{uu} = 20 \text{ kN/m}^2$ $\psi_{uu} = 26.57^\circ$ - Chemical analysis : $pH = 6.81$ $SO_4^2 = 12.4 \text{ mg/L}$ $Cl^- = 186.8 \text{ mg/L}$	Picks and shovels can be used.	<table border="1"> <thead> <tr> <th rowspan="3">DESCRIPTION OF THE PIT</th> <th rowspan="3">Depth (below existing ground level) / cm</th> <th rowspan="3">Soil Stratigraphy (observed in the well)</th> <th colspan="4">GEOTECHNICAL BEARING CAPACITIES</th> </tr> <tr> <th colspan="2">Allowable</th> <th rowspan="2">ULS *</th> <th rowspan="2">Ultimate</th> </tr> <tr> <th>( Mpa )</th> <th>( Kpa )</th> <th>( Mpa )</th> <th>( Mpa )</th> </tr> </thead> <tbody> <tr> <td>Vegetable soil 0.00 - 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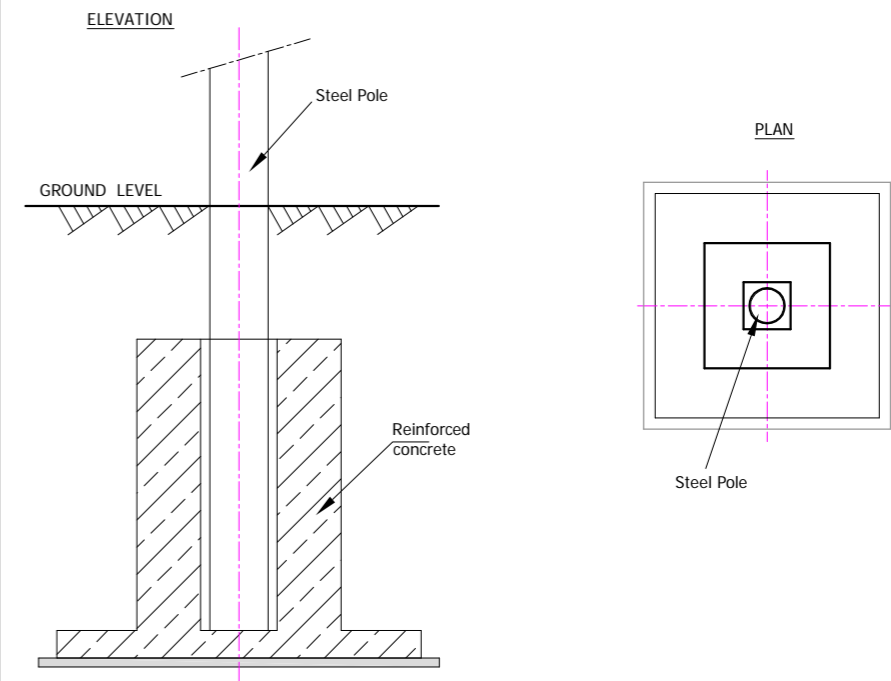
EXCAVATION ( test pit )



EXCAVATED SOIL

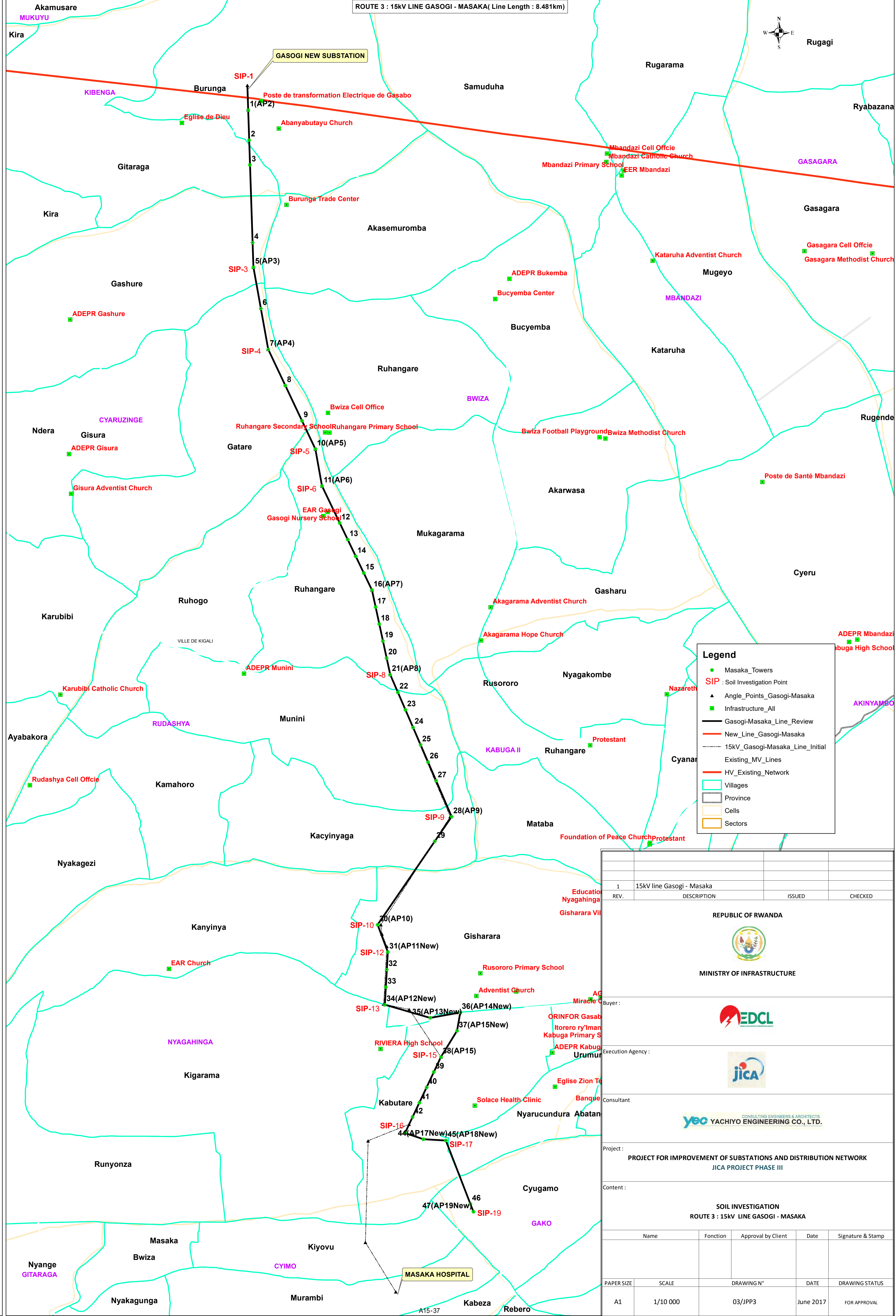
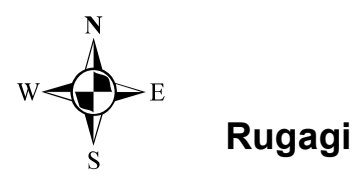


FOUNDATION TYPE



THE FOUNDATION DIMENSIONS COMPLY WITH THE STATIC BALANCE OF POST FIXING .

C. 15 kV Distribution line Gasogi substation-Masaka hospital




**Legend**

- Masaka\_Towers
- SIP : Soil Investigation Point
- ▲ Angle\_Points\_Gasogi-Masaka
- Infrastructure\_All
- Gasogi-Masaka\_Line\_Review
- New\_Line\_Gasogi-Masaka
- 15kV\_Gasogi-Masaka\_Line\_Initial
- Existing\_MV\_Lines
- HV\_Existing\_Network
- Villages
- Province
- Cells
- Sectors


1	15kV line Gasogi - Masaka			
REV.	DESCRIPTION	ISSUED	CHECKED	
REPUBLIC OF RWANDA  MINISTRY OF INFRASTRUCTURE				
Buyer : 				
Execution Agency : 				
Consultant: 				
Project : PROJECT FOR IMPROVEMENT OF SUBSTATIONS AND DISTRIBUTION NETWORK JICA PROJECT PHASE III				
Content : SOIL INVESTIGATION ROUTE 3 : 15kV LINE GASOGI - MASAKA				
Name	Fonction	Approval by Client	Date	Signature & Stamp
PAPER SIZE	SCALE	DRAWING N°	DATE	DRAWING STATUS
A1	1/10 000	03/JPP3	June 2017	FOR APPROVAL

Soil investigation point	Extraction depth of the sample	Classification according USCS	Observation	ENGINEERING PROPERTIES	EXCAVATION CONDITION	GEOLOGICAL UNIT																																																																																																																																																																																																																																																																																																																																
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SIP 1 (15 kV line Gasogi - Masaka)	3.80 - 4.00 m	Clay of intermediate plasticity ( CI )	Lateritic zone	- IP : 18.6  - Consistency $I_c = 1.05 > 1$ : very consistent ground  - Compaction Backfill : $d_{OPM} = 1.83 \text{ t/m}^3$ $W_{OPM} = 17.20 \%$  - Natural soil : $\rho_h = 1.90 \text{ t/m}^3$ $W(N) = 20.7 \%$ $\rho_s = 2.75 \text{ t/m}^3$	- Shear test : $C_{uu} = 28 \text{ kN/m}^2$ $\psi_{uu} = 23.51^\circ$  - Chemical analysis : $pH = 5.11$ $SO_4^2 = 13 \text{ mg/L}$ $Cl^- = 122 \text{ mg/L}$	Picks and shovels can be used.	<table border="1"> <thead> <tr> <th rowspan="3">DESCRIPTION OF THE PIT</th> <th rowspan="3">Depth (below existing ground level) cm</th> <th rowspan="3">Soil Stratigraphy (observed in the well)</th> <th colspan="4">GEOTECHNICAL BEARING CAPACITIES</th> </tr> <tr> <th colspan="2">Allowable</th> <th rowspan="2">ULS *</th> <th rowspan="2">Ultimate</th> </tr> <tr> <th>(Mpa)</th> <th>(Kpa)</th> <th>(Mpa)</th> <th>(Mpa)</th> </tr> </thead> <tbody> <tr><td>Vegetable soil</td><td>0.00 - 0.40 m</td><td></td><td>0</td><td>0</td><td>0</td><td>0</td></tr> <tr><td></td><td>20</td><td></td><td>0.10</td><td>100</td><td>10</td><td>35</td></tr> <tr><td></td><td>40</td><td></td><td>0.30</td><td>300</td><td>30</td><td>55</td></tr> <tr><td></td><td>60</td><td></td><td>0.25</td><td>250</td><td>25</td><td>60</td></tr> <tr><td></td><td>80</td><td></td><td>0.38</td><td>380</td><td>38</td><td>65</td></tr> <tr><td></td><td>100</td><td></td><td>1.00</td><td>1000</td><td>100</td><td>150</td></tr> <tr><td></td><td>120</td><td></td><td>1.25</td><td>1250</td><td>125</td><td>240</td></tr> <tr><td></td><td>140</td><td></td><td>0.90</td><td>900</td><td>90</td><td>130</td></tr> <tr><td></td><td>160</td><td></td><td>1.20</td><td>1200</td><td>120</td><td>180</td></tr> <tr><td></td><td>180</td><td></td><td>1.30</td><td>1300</td><td>130</td><td>230</td></tr> <tr><td></td><td>200</td><td></td><td>1.40</td><td>1400</td><td>140</td><td>250</td></tr> <tr><td></td><td>220</td><td></td><td>1.35</td><td>1350</td><td>135</td><td>350</td></tr> <tr><td></td><td>240</td><td></td><td>1.20</td><td>1200</td><td>120</td><td>380</td></tr> <tr><td></td><td>260</td><td></td><td>1.00</td><td>1000</td><td>100</td><td>340</td></tr> <tr><td></td><td>280</td><td></td><td>1.50</td><td>1500</td><td>150</td><td>500</td></tr> <tr><td></td><td>300</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td></td><td>320</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td></td><td>340</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td></td><td>360</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td></td><td>380</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td></td><td>400</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td></td><td>420</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td></td><td>440</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td></td><td>460</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td></td><td>480</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td></td><td>500</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td></td><td>520</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td></td><td>540</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td></td><td>560</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td></td><td>580</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td></td><td>600</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td></td><td>620</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td></td><td>640</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td></td><td>660</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td></td><td>680</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td></td><td>700</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td></td><td>720</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td></td><td>740</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td></td><td>760</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td></td><td>780</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td></td><td>800</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td></td><td>820</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td></td><td>840</td><td></td><td></td><td></td><td></td><td></td></tr> </tbody> </table>				DESCRIPTION OF THE PIT	Depth (below existing ground level) cm	Soil Stratigraphy (observed in the well)	GEOTECHNICAL BEARING CAPACITIES				Allowable		ULS *	Ultimate	(Mpa)	(Kpa)	(Mpa)	(Mpa)	Vegetable soil	0.00 - 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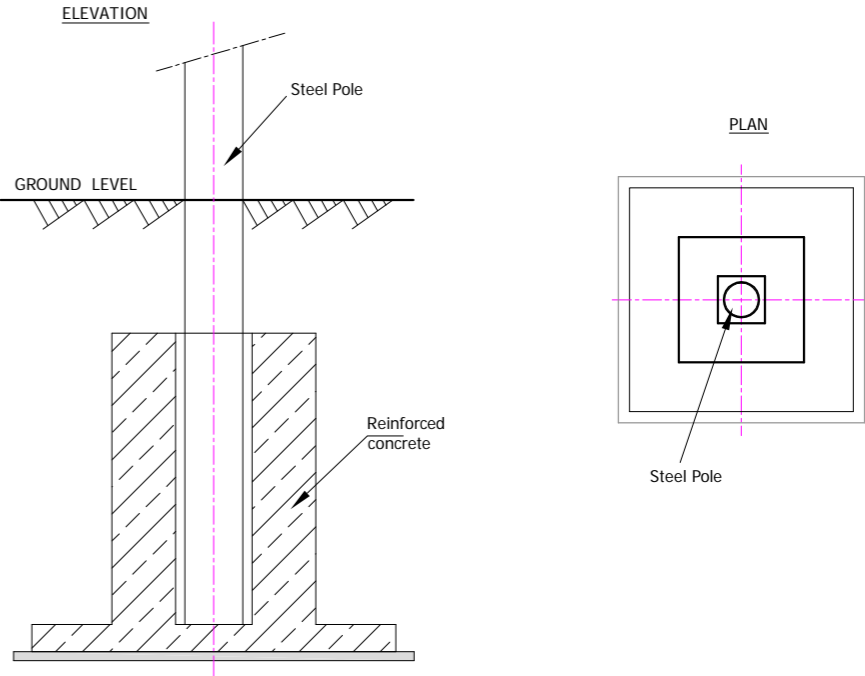
EXCAVATION ( test pit )



EXCAVATED SOIL



FOUNDATION TYPE



THE FOUNDATION DIMENSIONS COMPLY WITH THE STATIC BALANCE OF POST FIXING .

Soil investigation point	Extraction depth of the sample	Classification according USCS	Observation	ENGINEERING PROPERTIES		EXCAVATION CONDITION	GEOLOGICAL UNIT																																																																																																																																																																																																																																																																																																																										
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SIP 3 (15 kV line Gasogi - Masaka)	3.75 - 4.00 m	Silty sand poorly graded ( SM )	Lateritic zone	- IP : /  - Consistency $I_c > 1$ very consistent ground  - Compaction Backfill : $\rho_{OPM} = 1.68 \text{ t/m}^3$ $W_{OPM} = 22.40 \%$  - Natural soil : $\rho_h = 1.70 \text{ t/m}^3$ $W_{(N)} = 20.60 \%$ $\rho_s = 2.80 \text{ t/m}^3$	- Shear test : $C_{uu} = 9 \text{ kN/m}^2$ $\psi_{uu} = 30.54^\circ$  - Chemical analysis : $pH = 6.26$ $SO_4^2 = 59.2 \text{ mg/L}$ $Cl^- = 99.3 \text{ mg/L}$	Picks and shovels can be used.	<table border="1"> <thead> <tr> <th rowspan="2">DESCRIPTION OF THE PIT</th> <th rowspan="2">Depth (below existing ground level) cm</th> <th rowspan="2">Soil Stratigraphy (observed in the well)</th> <th colspan="4">GEOTECHNICAL BEARING CAPACITIES</th> </tr> <tr> <th colspan="2">Allowable</th> <th>Uls *</th> <th>Ultimate</th> </tr> <tr> <th>( Mpa )</th> <th>( Kpa )</th> <th>( Mpa )</th> <th colspan="2">( Mpa )</th> </tr> </thead> <tbody> <tr> <td>Vegetable soil 0.00 - 0.30 m</td> <td>0</td> <td></td> <td>0</td> <td>0</td> <td>0</td> <td>0</td> </tr> <tr> <td></td> <td>20</td> <td></td> <td>0.70</td> <td>700</td> <td>70</td> <td>100</td> </tr> <tr> <td></td> <td>40</td> <td></td> <td>1.23</td> <td>1230</td> <td>123</td> <td>160</td> </tr> <tr> <td></td> <td>60</td> <td></td> <td>0.70</td> <td>700</td> <td>70</td> <td>85</td> </tr> <tr> <td></td> <td>80</td> <td></td> <td>1.05</td> <td>1050</td> <td>105</td> <td>180</td> </tr> <tr> <td></td> <td>100</td> <td></td> <td>1.25</td> <td>1250</td> <td>125</td> <td>420</td> </tr> <tr> <td></td> <td>120</td> <td></td> <td>1.50</td> <td>1500</td> <td>150</td> <td>500</td> </tr> <tr> <td></td> <td>140</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td></td> <td>160</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td></td> <td>180</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td></td> <td>200</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>Silty sand poorly graded 0.30 - 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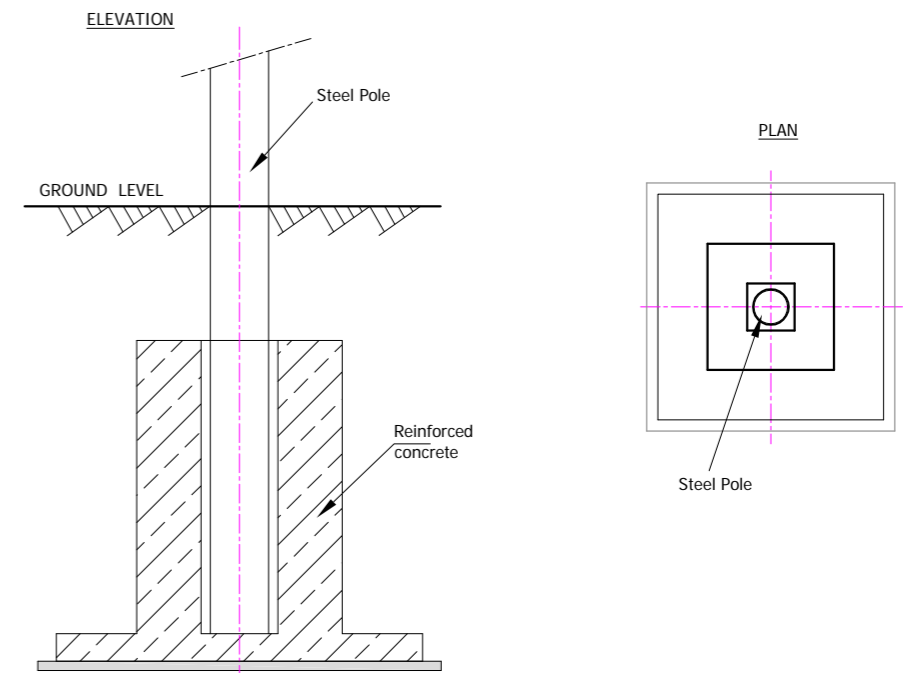
EXCAVATION ( test pit )



EXCAVATED SOIL



FOUNDATION TYPE



THE FOUNDATION DIMENSIONS COMPLY WITH THE STATIC BALANCE OF POST FIXING .



Soil investigation point	Extraction depth of the sample	Classification according USCS	Observation	ENGINEERING PROPERTIES	EXCAVATION CONDITION	GEOLOGICAL UNIT																																														
						Light penetrometer ( GeoMIL 50 kN SPT ), test without soil coring for sample taking																																														
SIP 4 (15 kV line Gasogi - Masaka)	3.50 - 4.00 m	Clayey gravel poorly graded ( GC )	Semi rock soil	- IP : 16.4 - Consistency $I_c = 1.40 > 1$ very consistent ground - Compaction Backfill : . $d_{OPM} = 2.02 \text{ t/m}^3$ . $W_{OPM} = 12.40 \%$ - Natural soil : . $\rho_h = 1.98 \text{ t/m}^3$ . $W_{(N)} = 16.30 \%$ . $\rho_s = 2.78 \text{ t/m}^3$	- Shear test : . $C_{uu} = 10 \text{ kN/m}^2$ . $\psi_{uu} = 26.10^\circ$ - Chemical analysis : . $pH = 6.15$ . $SO_4^2 = 18 \text{ mg/L}$ . $Cl^- = 79 \text{ mg/L}$	Picks and shovels can be used; preferably use pneumatic hammer.	<table border="1"> <thead> <tr> <th rowspan="3">DESCRIPTION OF THE PIT</th> <th rowspan="3">Depth (below existing ground level ) cm</th> <th rowspan="3">Soil Stratigraphy (observed in the well )</th> <th colspan="4">GEOTECHNICAL BEARING CAPACITIES</th> </tr> <tr> <th colspan="2">Allowable</th> <th rowspan="2">ULS *</th> <th rowspan="2">Ultimate</th> </tr> <tr> <th>( Mpa )</th> <th>( Kpa )</th> <th>( Mpa )</th> <th>( Mpa )</th> </tr> </thead> <tbody> <tr> <td>Vegetable soil 0.00 - 0.80 m</td> <td>0 20 40 60 80</td> <td rowspan="2">[Soil Stratigraphy]</td> <td>0</td> <td>0</td> <td>0</td> <td>0</td> </tr> <tr> <td></td> <td>100 120 140 160</td> <td>1.05 0.85 0.60 1.00</td> <td>1050 850 600 1000</td> <td>105 85 60 100</td> <td>130 100 80 120</td> </tr> <tr> <td>Clayey gravel poorly graded 0.80 - 4.00 m</td> <td>180 200 220 240 260 280 300 320 340 360 380 400</td> <td></td> <td>1.40 1.35 1.50</td> <td>1400 1350 1500</td> <td>140 135 150</td> <td>260 380 500</td> </tr> <tr> <td></td> <td>420 440 460 480 500 520 540 560 580 620 640 660 680 700 720 740 760 780 800 820 840</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> </tbody> </table>				DESCRIPTION OF THE PIT	Depth (below existing ground level ) cm	Soil Stratigraphy (observed in the well )	GEOTECHNICAL BEARING CAPACITIES				Allowable		ULS *	Ultimate	( Mpa )	( Kpa )	( Mpa )	( Mpa )	Vegetable soil 0.00 - 0.80 m	0 20 40 60 80	[Soil Stratigraphy]	0	0	0	0		100 120 140 160	1.05 0.85 0.60 1.00	1050 850 600 1000	105 85 60 100	130 100 80 120	Clayey gravel poorly graded 0.80 - 4.00 m	180 200 220 240 260 280 300 320 340 360 380 400		1.40 1.35 1.50	1400 1350 1500	140 135 150	260 380 500		420 440 460 480 500 520 540 560 580 620 640 660 680 700 720 740 760 780 800 820 840					
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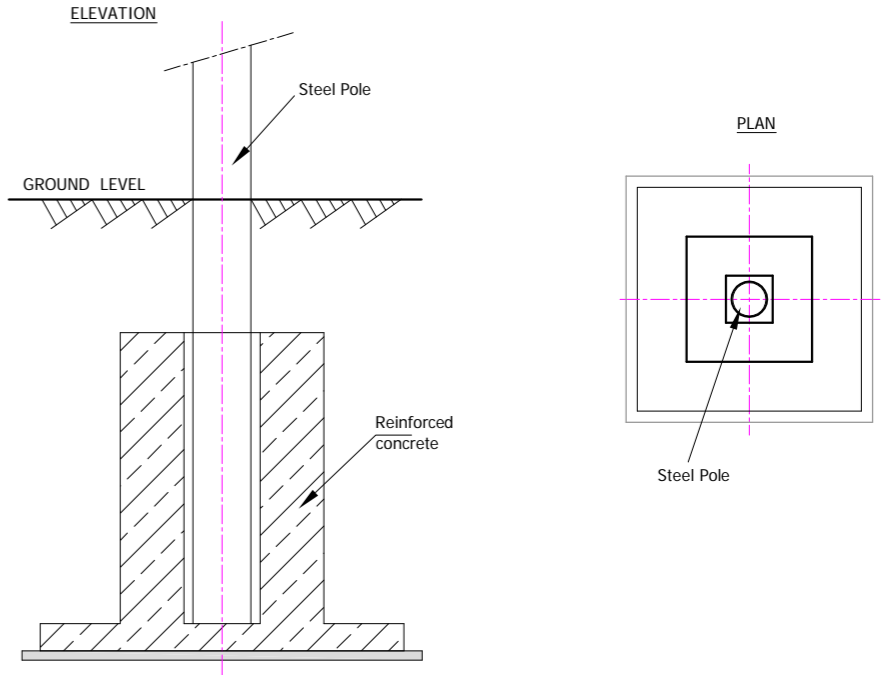
EXCAVATION ( test pit )



EXCAVATED SOIL



FOUNDATION TYPE



THE FOUNDATION DIMENSIONS COMPLY WITH THE STATIC BALANCE OF POST FIXING .

Soil investigation point	Extraction depth of the sample	Classification according USCS	Observation	ENGINEERING PROPERTIES		EXCAVATION CONDITION	GEOLOGICAL UNIT																																																																																																																																																																																																																																																																																			
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SIP 5 (15 kV line Gasogi - Masaka)	3.70 - 4.00 m	Silty gravel poorly graded ( GM )	Semi rock soil	- IP : /  - Consistency $I_c > 1$ very consistent ground  - Compaction Backfill : $d_{OPM} = 2.03 \text{ t/m}^3$ $W_{OPM} = 10.62 \%$  - Natural soil : $\rho_h = 1.72 \text{ t/m}^3$ $W_{(N)} = 14 \%$ $\rho_s = 2.70 \text{ t/m}^3$	- Shear test : $C_{uu} = 4 \text{ kN/m}^2$ $\psi_{uu} = 27.02^\circ$  - Chemical analysis : $pH = 5.79$ $SO_4^2 = 7.6 \text{ mg/L}$ $Cl^- = 78 \text{ mg/L}$	Picks and shovels can be used; preferably use pneumatic hammer.	<table border="1"> <thead> <tr> <th rowspan="2">DESCRIPTION OF THE PIT</th> <th rowspan="2">Depth (below existing ground level) / m</th> <th rowspan="2">Soil Stratigraphy (observed in the well)</th> <th colspan="3">GEOTECHNICAL BEARING CAPACITIES</th> </tr> <tr> <th>Allowable (Mpa)</th> <th>(Kpa)</th> <th>ULS * (Mpa)</th> <th>Ultimate (Mpa)</th> </tr> </thead> <tbody> <tr><td>Vegetable soil</td><td>0</td><td></td><td>0</td><td>0</td><td>0</td><td>0</td></tr> <tr><td rowspan="6">0.00 - 0.50 m</td><td>20</td><td></td><td>0.30</td><td>300</td><td>30</td><td>45</td></tr> <tr><td>40</td><td></td><td>0.35</td><td>350</td><td>35</td><td>48</td></tr> <tr><td>60</td><td></td><td>0.35</td><td>350</td><td>35</td><td>45</td></tr> <tr><td>80</td><td></td><td>0.33</td><td>330</td><td>33</td><td>45</td></tr> <tr><td>100</td><td></td><td>0.65</td><td>650</td><td>65</td><td>110</td></tr> <tr><td>120</td><td></td><td>0.75</td><td>750</td><td>75</td><td>120</td></tr> <tr><td rowspan="10">Clayey gravel poorly graded 0.50 - 3.50 m</td><td>140</td><td></td><td>1.00</td><td>1000</td><td>100</td><td>240</td></tr> <tr><td>160</td><td></td><td>1.23</td><td>1230</td><td>123</td><td>300</td></tr> <tr><td>180</td><td></td><td>0.80</td><td>800</td><td>80</td><td>170</td></tr> <tr><td>200</td><td></td><td>1.30</td><td>1300</td><td>130</td><td>250</td></tr> <tr><td>220</td><td></td><td>1.40</td><td>1400</td><td>140</td><td>400</td></tr> <tr><td>240</td><td></td><td>1.50</td><td>1500</td><td>150</td><td>500</td></tr> <tr><td>260</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>280</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>300</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>320</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>340</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>360</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>380</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>400</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>420</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>440</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>460</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>480</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>500</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>520</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>540</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>560</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>580</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>600</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>620</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>640</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>660</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>680</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>700</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>720</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>740</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>760</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>780</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>800</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>820</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>840</td><td></td><td></td><td></td><td></td><td></td></tr> </tbody> </table>					DESCRIPTION OF THE PIT	Depth (below existing ground level) / m	Soil Stratigraphy (observed in the well)	GEOTECHNICAL BEARING CAPACITIES			Allowable (Mpa)	(Kpa)	ULS * (Mpa)	Ultimate (Mpa)	Vegetable soil	0		0	0	0	0	0.00 - 0.50 m	20		0.30	300	30	45	40		0.35	350	35	48	60		0.35	350	35	45	80		0.33	330	33	45	100		0.65	650	65	110	120		0.75	750	75	120	Clayey gravel poorly graded 0.50 - 3.50 m	140		1.00	1000	100	240	160		1.23	1230	123	300	180		0.80	800	80	170	200		1.30	1300	130	250	220		1.40	1400	140	400	240		1.50	1500	150	500	260						280						300						320						340						360						380						400						420						440						460						480						500						520						540						560						580						600						620						640						660						680						700						720						740						760						780						800						820						840					
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EXCAVATION ( test pit )

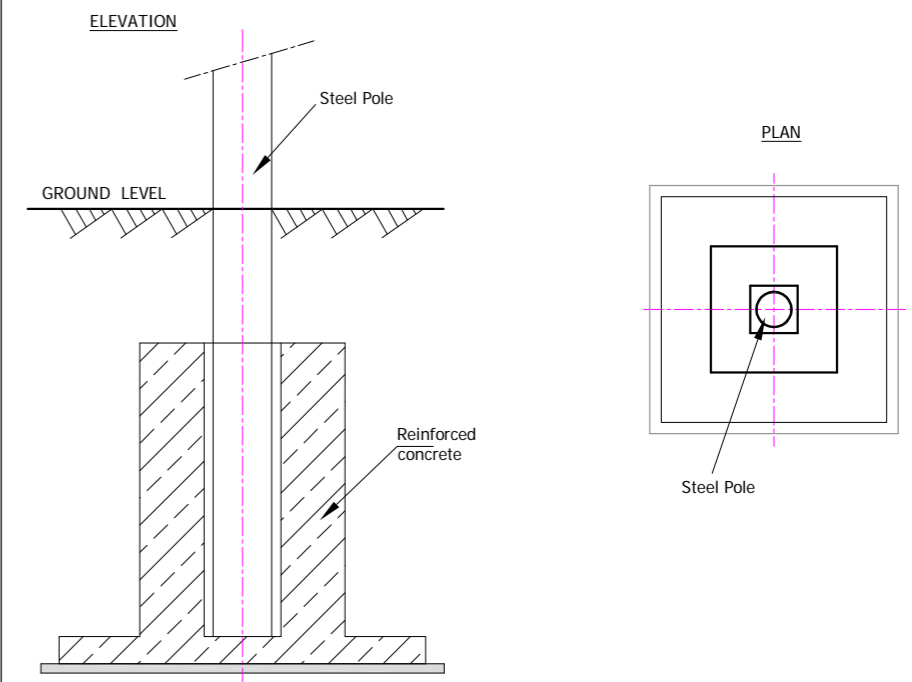


EXCAVATED SOIL



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FOUNDATION TYPE



THE FOUNDATION DIMENSIONS COMPLY WITH THE STATIC BALANCE OF POST FIXING .

Soil investigation point	Extraction depth of the sample	Classification according USCS	Observation	ENGINEERING PROPERTIES	EXCAVATION CONDITION
SIP 6 (15 kV line Gasogi - Masaka)	3.60 - 4.00 m	Silty gravel poorly graded (GM)	Semi rock soil	<p>- IP : 15.2</p> <p>- Consistency <math>I_c = 1.38 &gt; 1</math> very consistent ground</p> <p>- Compaction Backfill :</p> <p>. <math>d_{OPM} = 2.02 \text{ t/m}^3</math></p> <p>. <math>W_{OPM} = 11.36 \%</math></p> <p>- Natural soil :</p> <p>. <math>\rho_h = 1.90 \text{ t/m}^3</math></p> <p>. <math>W_{(N)} = 18 \%</math></p> <p>. <math>\rho_s = 2.72 \text{ t/m}^3</math></p> <p>- Shear test :</p> <p>. <math>C_{uu} = 14 \text{ kN/m}^2</math></p> <p>. <math>\psi_{uu} = 25.41^\circ</math></p> <p>- Chemical analysis :</p> <p>. <math>pH = 7.1</math></p> <p>. <math>SO_4^2 = 9.4 \text{ mg/L}</math></p> <p>. <math>Cl^- = 201.7 \text{ mg/L}</math></p>	Picks and shovels can be used.

GEOLOGICAL UNIT						
Light penetrometer ( GeoMIL 50 kN SPT ), test without soil coring for sample taking						
DESCRIPTION OF THE PIT	Depth (below existing ground level) cm	Soil Stratigraphy (observed in the well)	GEOTECHNICAL BEARING CAPACITIES			
			Allowable (Mpa)	(Kpa)	ULS * (Mpa)	Ultimate (Mpa)
Vegetable soil 0.00 - 0.50 m	0		0	0	0	0
	20		0.10	100	10	20
	40		0.25	250	25	55
	60		0.27	270	27	60
	80		0.25	250	25	62
	100		0.25	250	25	64
Clayey gravel poorly graded 0.50 - 3.00 m	120		0.22	220	22	72
	140		0.17	170	17	80
	160		0.25	250	25	90
	180		0.45	450	45	90
	200		0.45	450	45	100
	220		0.30	300	30	140
	240		0.40	400	40	180
	260		0.45	450	45	200
	280		0.30	300	30	200
	300		0.25	250	25	150
Silty gravel poorly graded 3.00 - 4.00 m	320		0.70	700	70	280
	340		1.30	1300	130	350
	360		1.50	1500	150	500
	380					
	400					
	420					
	440					
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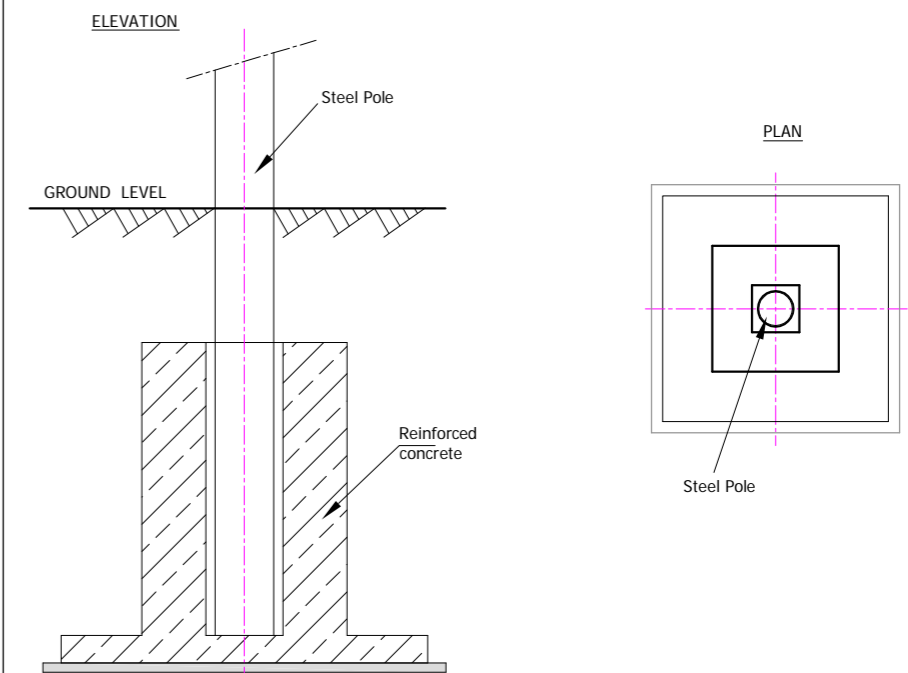
EXCAVATION ( test pit )



EXCAVATED SOIL



FOUNDATION TYPE



THE FOUNDATION DIMENSIONS COMPLY WITH THE STATIC BALANCE OF POST FIXING .

Soil investigation point	Extraction depth of the sample	Classification according USCS	Observation	ENGINEERING PROPERTIES	EXCAVATION CONDITION	GEOLOGICAL UNIT																																																																																																																																																																																																																																																																																																																													
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SIP 8 (15 kV line Gasogi - Masaka)	3.80 - 4.00 m	Silty gravel poorly graded ( GM )	Sandy soil	- IP : / - Consistency $I_c > 1$ very consistent ground - Compaction Backfill : $\rho_{OPM} = 2.06 \text{ t/m}^3$ $W_{OPM} = 10.40 \%$ - Natural soil : $\rho_h = 1.63 \text{ t/m}^3$ $W_{(N)} = 18.3 \%$ $\rho_s = 2.77 \text{ t/m}^3$	- Shear test : $C_{uu} = 4 \text{ kN/m}^2$ $\psi_{uu} = 25.70^\circ$ - Chemical analysis : $pH = 5.22$ $SO_4^2 = 5 \text{ mg/L}$ $Cl^- = 85 \text{ mg/L}$	Picks and shovels can be used.	<table border="1"> <thead> <tr> <th rowspan="2">DESCRIPTION OF THE PIT</th> <th rowspan="2">Depth (below existing ground level) cm</th> <th rowspan="2">Soil Stratigraphy (observed in the well)</th> <th colspan="4">GEOTECHNICAL BEARING CAPACITIES</th> </tr> <tr> <th colspan="2">Allowable</th> <th>ULS *</th> <th>Ultimate</th> </tr> <tr> <th></th> <th></th> <th></th> <th>( Mpa )</th> <th>( Kpa )</th> <th>( Mpa )</th> <th>( Mpa )</th> </tr> </thead> <tbody> <tr><td>Vegetable soil</td><td>0</td><td></td><td>0</td><td>0</td><td>0</td><td>0</td></tr> <tr><td></td><td>20</td><td></td><td>0.15</td><td>150</td><td>15</td><td>20</td></tr> <tr><td></td><td>40</td><td></td><td>0.85</td><td>850</td><td>85</td><td>100</td></tr> <tr><td></td><td>60</td><td></td><td>0.55</td><td>550</td><td>55</td><td>80</td></tr> <tr><td></td><td>80</td><td></td><td>1.10</td><td>1100</td><td>110</td><td>290</td></tr> <tr><td></td><td>100</td><td></td><td>1.30</td><td>1300</td><td>130</td><td>360</td></tr> <tr><td></td><td>120</td><td></td><td>1.50</td><td>1500</td><td>150</td><td>500</td></tr> <tr><td></td><td>140</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td></td><td>160</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td></td><td>180</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td></td><td>200</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td></td><td>220</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td></td><td>240</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td></td><td>260</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td></td><td>280</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td></td><td>300</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td></td><td>320</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td></td><td>340</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td></td><td>360</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td></td><td>380</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td></td><td>400</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td></td><td>420</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td></td><td>440</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td></td><td>460</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td></td><td>480</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td></td><td>500</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td></td><td>520</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td></td><td>540</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td></td><td>560</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td></td><td>580</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td></td><td>620</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td></td><td>640</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td></td><td>660</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td></td><td>680</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td></td><td>700</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td></td><td>720</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td></td><td>740</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td></td><td>760</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td></td><td>780</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td></td><td>800</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td></td><td>820</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td></td><td>840</td><td></td><td></td><td></td><td></td><td></td></tr> </tbody> </table>					DESCRIPTION OF THE PIT	Depth (below existing ground level) cm	Soil Stratigraphy (observed in the well)	GEOTECHNICAL BEARING CAPACITIES				Allowable		ULS *	Ultimate				( Mpa )	( Kpa )	( Mpa )	( Mpa )	Vegetable soil	0		0	0	0	0		20		0.15	150	15	20		40		0.85	850	85	100		60		0.55	550	55	80		80		1.10	1100	110	290		100		1.30	1300	130	360		120		1.50	1500	150	500		140							160							180							200							220							240							260							280							300							320							340							360							380							400							420							440							460							480							500							520							540							560							580							620							640							660							680							700							720							740							760							780							800							820							840					
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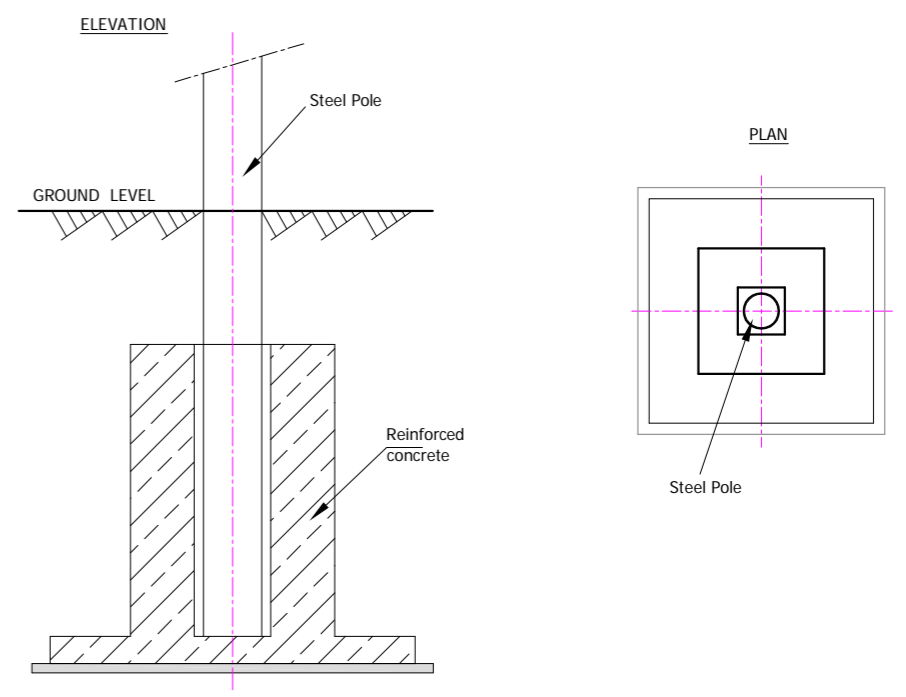
EXCAVATION ( test pit )



EXCAVATED SOIL



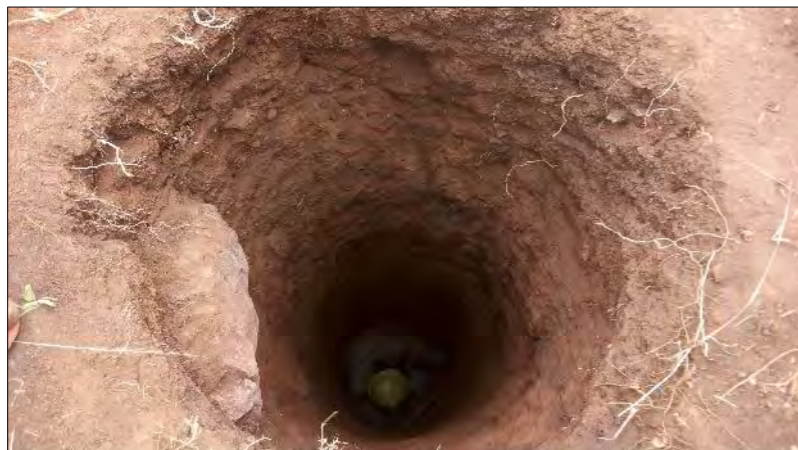
FOUNDATION TYPE



THE FOUNDATION DIMENSIONS COMPLY WITH THE STATIC BALANCE OF POST FIXING

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SIP 9 (15 kV line Gasogi - Masaka)	3.70 - 4.00 m	Clayey gravel poorly graded ( GC )	Semi rock soil	- IP : 16.5 - Consistency $I_c = 1.38 > 1$ very consistent ground - Compaction Backfill : $\rho_{OPM} = 1.94 \text{ t/m}^3$ $W_{OPM} = 15.20 \%$ - Natural soil : $\rho_h = 1.88 \text{ t/m}^3$ $W_{(N)} = 17.3 \%$ $\rho_s = 2.88 \text{ t/m}^3$	- Shear test : $C_{uu} = 20 \text{ kN/m}^2$ $\psi_{uu} = 23.75^\circ$ - Chemical analysis : $pH = 6.75$ $SO_4^2 = 9.2 \text{ mg/L}$ $Cl^- = 226.9 \text{ mg/L}$	Picks and shovels can be used.	<table border="1"> <thead> <tr> <th rowspan="3">DESCRIPTION OF THE PIT</th> <th rowspan="3">Depth (below existing ground level ) cm</th> <th rowspan="3">Soil Stratigraphy (observed in the well )</th> <th colspan="3">GEO TECHNICAL BEARING CAPACITIES</th> </tr> <tr> <th colspan="2">Allowable</th> <th rowspan="2">ULS *</th> <th rowspan="2">Ultimate</th> </tr> <tr> <th>( Mpa )</th> <th>( Kpa )</th> <th>( Mpa )</th> <th>( Mpa )</th> </tr> </thead> <tbody> <tr> <td>Vegetable soil 0.00 - 0.30 m</td> <td>0 20 40</td> <td></td> <td>0 0.20 0.18</td> <td>0 200 180</td> <td>0 20 18</td> <td>0 45 40</td> </tr> <tr> <td rowspan="14">Clayey gravel poorly graded 0.30 - 4.00 m</td> <td>40</td> <td></td> <td>0.70</td> <td>700</td> <td>70</td> <td>105</td> </tr> <tr> <td>60</td> <td></td> <td>1.10</td> <td>1100</td> <td>110</td> <td>205</td> </tr> <tr> <td>80</td> <td></td> <td>1.35</td> <td>1350</td> <td>135</td> <td>350</td> </tr> <tr> <td>100</td> <td></td> <td>1.40</td> <td>1400</td> <td>140</td> <td>400</td> </tr> <tr> <td>120</td> <td></td> <td>1.50</td> <td>1500</td> <td>150</td> <td>500</td> </tr> <tr> <td>140</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>160</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>180</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>200</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>220</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>240</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>260</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>280</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>300</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>320</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>340</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>360</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>380</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>400</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>420</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>440</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>460</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>480</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>500</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>520</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>540</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>560</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>580</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>600</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>620</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>640</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>660</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>680</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>700</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>720</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>740</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>760</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>780</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>800</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>820</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>840</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> </tbody> </table>					DESCRIPTION OF THE PIT	Depth (below existing ground level ) cm	Soil Stratigraphy (observed in the well )	GEO TECHNICAL BEARING CAPACITIES			Allowable		ULS *	Ultimate	( Mpa )	( Kpa )	( Mpa )	( Mpa )	Vegetable soil 0.00 - 0.30 m	0 20 40		0 0.20 0.18	0 200 180	0 20 18	0 45 40	Clayey gravel poorly graded 0.30 - 4.00 m	40		0.70	700	70	105	60		1.10	1100	110	205	80		1.35	1350	135	350	100		1.40	1400	140	400	120		1.50	1500	150	500	140						160						180						200						220						240						260						280						300						320						340						360						380						400						420						440						460						480						500						520						540						560						580						600						620						640						660						680						700						720						740						760						780						800						820						840						* ULS : Ultimate Limit State
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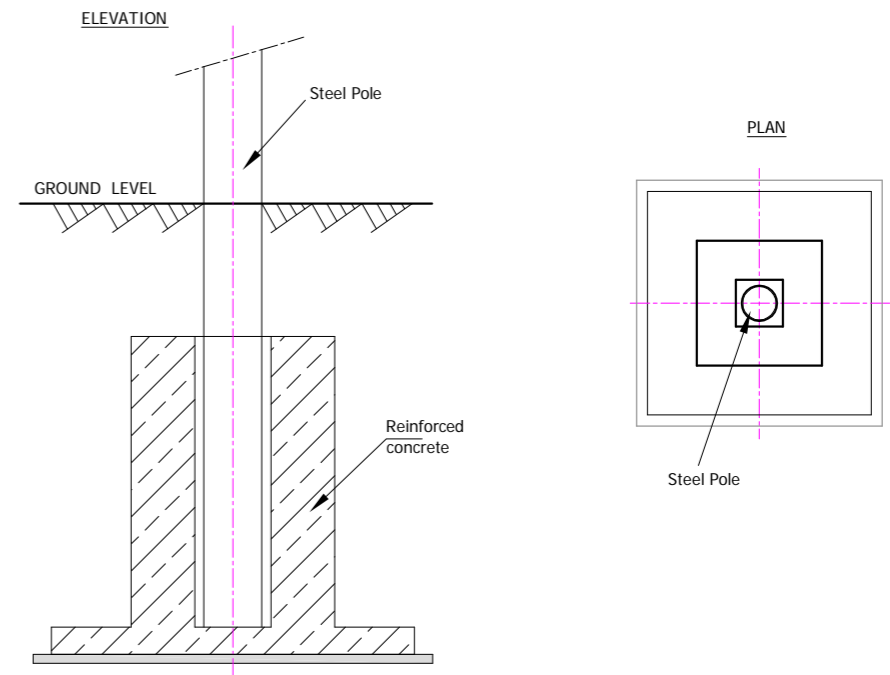
EXCAVATION ( test pit )



EXCAVATED SOIL



FOUNDATION TYPE



THE FOUNDATION DIMENSIONS COMPLY WITH THE STATIC BALANCE OF POST FIXING .

Soil investigation point	Extraction depth of the sample	Classification according USCS	Observation	ENGINEERING PROPERTIES	EXCAVATION CONDITION
SIP 10 (15 kV line Gasogi - Masaka)	3.70 - 4.00 m	Clay of intermediate plasticity (CI)	Lateritic zone	<p>- IP : 19.9</p> <p>- Consistency <math>I_c = 1.11 &gt; 1</math> very consistent ground</p> <p>- Compaction Backfill :</p> <p>    <math>d_{OPM} = 1.82 \text{ t/m}^3</math></p> <p>    <math>W_{OPM} = 18.40 \%</math></p> <p>- Natural soil :</p> <p>    <math>\rho_h = 1.87 \text{ t/m}^3</math></p> <p>    <math>W_{(N)} = 22.3 \%</math></p> <p>    <math>\rho_s = 2.78 \text{ t/m}^3</math></p> <p>- Shear test :</p> <p>    <math>c_{uu} = 38 \text{ kN/m}^2</math></p> <p>    <math>\psi_{uu} = 21.31^\circ</math></p> <p>- Chemical analysis :</p> <p>    pH = 6.83</p> <p>    <math>SO_4^2 = 8.8 \text{ mg/L}</math></p> <p>    <math>Cl^- = 199.4 \text{ mg/L}</math></p>	Picks and shovels can be used.

GEOLOGICAL UNIT						
Light penetrometer ( GeoMIL 50 kN SPT ), test without soil coring for sample taking						
DESCRIPTION OF THE PIT	Depth (below existing ground level) (cm)	Soil Stratigraphy (observed in the well)	GEO TECHNICAL BEARING CAPACITIES			
			Allowable		ULS * (Mpa)	Ultimate (Mpa)
			(Mpa)	(Kpa)		
Vegetable soil 0.00 - 0.30 m	0 20 40 60 80 100 120 140 160 180		0 0.20 0.40 0.55 0.50	0 200 400 550 500	0 20 40 55 50	0 50 60 80 100
Clay of intermediate plasticity 0.30 - 4.00 m	200 220 240 260 280 300 320 340 360 380 400 420 440 460 480 500 520 540 560 580 620 640 660 680 700 720 740 760 780 800 820 840		0.25 0.35 0.35 0.30 0.25 0.23 0.22 0.28 0.28 0.30 0.35 0.60 0.55 0.40 0.90 0.55 0.55 0.73 1.38 1.43 1.50	250 350 350 300 250 230 220 280 280 300 350 600 550 400 900 550 550 730 1380 1430 1500	25 35 35 30 25 23 22 28 28 30 35 60 55 40 90 55 55 73 138 143 150	70 70 90 110 130 125 135 150 135 180 185 250 300 280 330 310 290 330 340 400 500

\* ULS : Ultimate Limit State

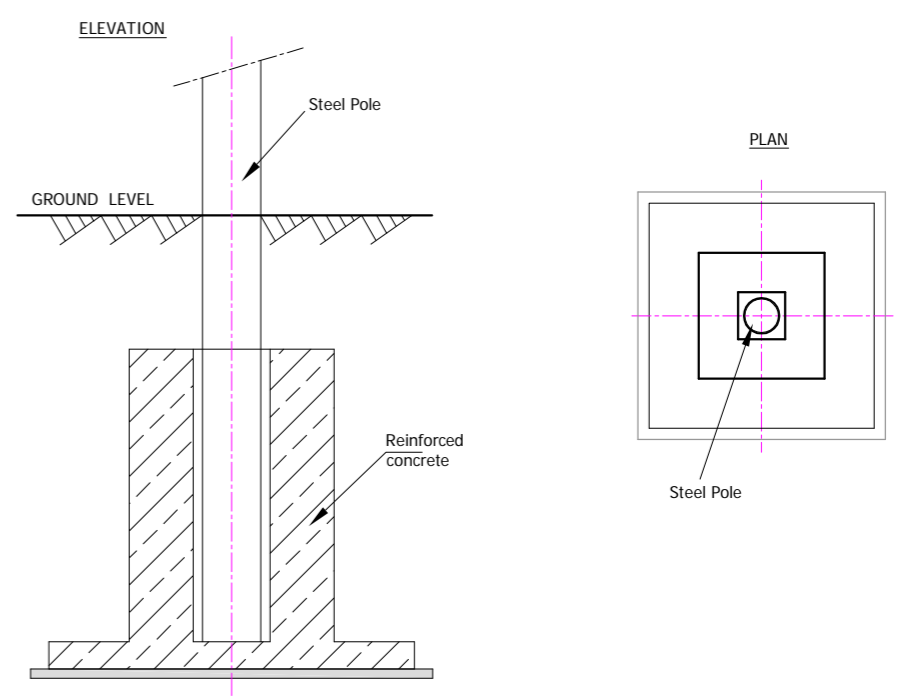
EXCAVATION ( test pit )



EXCAVATED SOIL



FOUNDATION TYPE



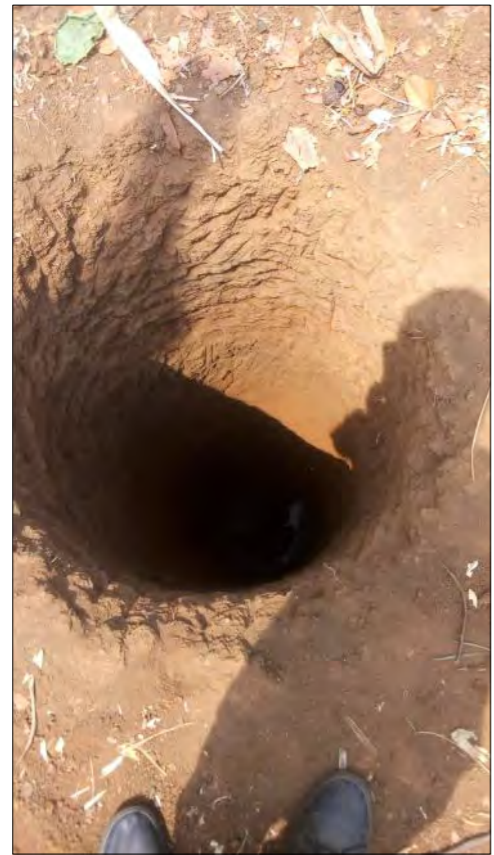
THE FOUNDATION DIMENSIONS COMPLY WITH THE STATIC BALANCE OF POST FIXING .

Soil investigation point	Extraction depth of the sample	Classification according USCS	Observation	ENGINEERING PROPERTIES	EXCAVATION CONDITION
SIP 12 (15 kV line Gasogi - Masaka)	3.50 - 4.00 m	Clay of intermediate plasticity (CI)	Lateritic zone	<p>- IP : 19.9</p> <p>- Consistency <math>I_c = 1.11 &gt; 1</math> very consistent ground</p> <p>- Compaction Backfill :</p> <p><math>\rho_{dOPM} = 1.82 \text{ t/m}^3</math></p> <p><math>W_{OPM} = 18.40 \%</math></p> <p>- Natural soil :</p> <p><math>\rho_h = 1.87 \text{ t/m}^3</math></p> <p><math>W_{(N)} = 22.3 \%</math></p> <p><math>\rho_s = 2.78 \text{ t/m}^3</math></p> <p>- Shear test :</p> <p><math>C_{uu} = 38 \text{ kN/m}^2</math></p> <p><math>\psi_{uu} = 21.31^\circ</math></p> <p>- Chemical analysis :</p> <p><math>\text{pH} = 6.83</math></p> <p><math>\text{SO}_4^2 = 8.8 \text{ mg/L}</math></p> <p><math>\text{Cl}^- = 199.4 \text{ mg/L}</math></p>	Picks and shovels can be used.

GEOLOGICAL UNIT						
Light penetrometer ( GeoMIL 50 kN SPT ), test without soil coring for sample taking						
DESCRIPTION OF THE PIT	Depth below existing ground level (cm)	Soil Stratigraphy (observed in the well)	GEOTECHNICAL BEARING CAPACITIES			
			Allowable		ULS *	Ultimate
			(Mpa)	(Kpa)		
Vegetable soil 0.00 - 0.50 m	0		0	0	0	0
	20		0.10	100	10	30
	40		0.50	500	50	60
	60		0.45	450	45	80
	80		0.55	550	55	80
	100		0.60	600	60	130
	120		0.90	900	90	200
	140		0.95	950	95	250
	160		0.90	900	90	280
	180		1.00	1000	100	360
	200		1.45	1450	145	400
Clay of intermediate plasticity 0.50 - 4.00 m	220		1.50	1500	150	500
	240					
	260					
	280					
	300					
	320					
	340					
	360					
	380					
	400					
	420					
	440					
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\* ULS : Ultimate Limit State

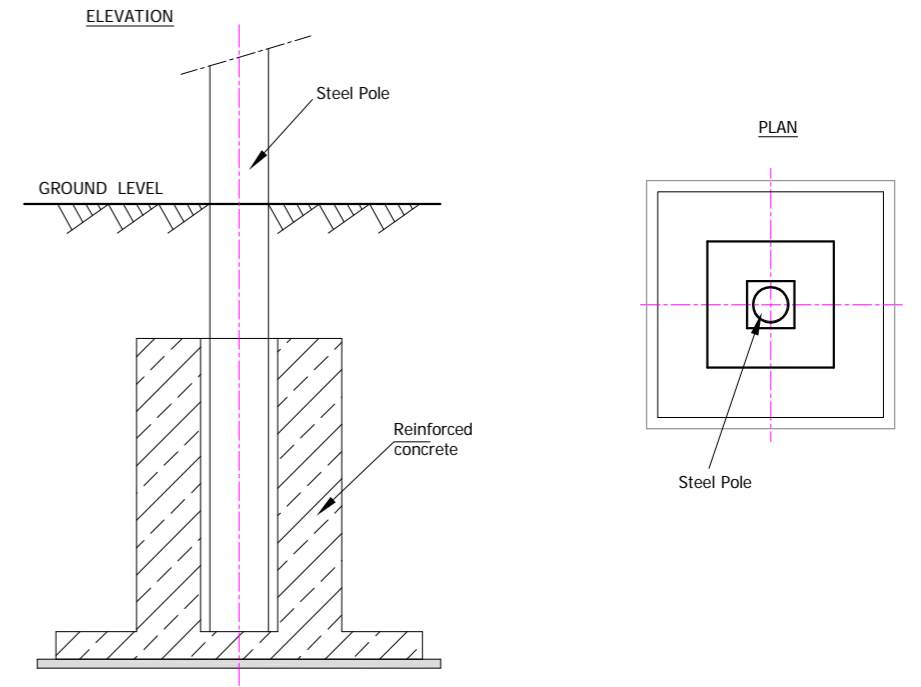
EXCAVATION ( test pit )



EXCAVATED SOIL



FOUNDATION TYPE



THE FOUNDATION DIMENSIONS COMPLY WITH THE STATIC BALANCE OF POST FIXING .

Soil investigation point	Extraction depth of the sample	Classification according USCS	Observation	ENGINEERING PROPERTIES	EXCAVATION CONDITION
SIP 13 (15 kV line Gasogi - Masaka)	3.60 - 4.00 m	Clay of intermediate plasticity (CI)	Clayey soil	<p>- IP : 20.7</p> <p>- Consistency <math>I_c = 1.12 &gt; 1</math> very consistent ground</p> <p>- Compaction Backfill :</p> <p>    . <math>d_{OPM} = 1.73 \text{ t/m}^3</math></p> <p>    . <math>W_{OPM} = 20.0 \%</math></p> <p>- Natural soil :</p> <p>    . <math>\rho_h = 1.70 \text{ t/m}^3</math></p> <p>    . <math>W_{(N)} = 21.0 \%</math></p> <p>    . <math>\rho_s = 2.77 \text{ t/m}^3</math></p> <p>- Shear test :</p> <p>    . <math>C_{uu} = 45 \text{ kN/m}^2</math></p> <p>    . <math>\psi_{uu} = 20.30^\circ</math></p> <p>- Chemical analysis :</p> <p>    . <math>pH = 6.64</math></p> <p>    . <math>SO_4^2 = 12.2 \text{ mg/L}</math></p> <p>    . <math>Cl^- = 213.1 \text{ mg/L}</math></p>	Picks and shovels can be used.

GEOLOGICAL UNIT						
Light penetrometer ( GeoMIL 50 kN SPT ), test without soil coring for sample taking						
DESCRIPTION OF THE PIT	Depth below existing ground level (m)	Soil Stratigraphy (observed in the well)	GEO TECHNICAL BEARING CAPACITIES			
			Allowable		ULS * (Mpa)	Ultimate (Mpa)
			(Mpa)	(Kpa)		
Vegetable soil 0.00 - 0.30 m	0		0	0	0	0
Clay of intermediate plasticity 0.30 - 4.00 m	20		0.60	600	60	80
	40		0.85	850	85	100
	60		0.95	950	95	130
	80		1.00	1000	100	160
	100		0.40	400	40	90
	120		0.30	300	30	70
	140		0.25	250	25	65
	160		0.28	280	28	80
	180		0.30	300	30	95
	200		0.25	250	25	100
	220		0.30	300	30	140
	240		0.30	300	30	155
	260		0.40	400	40	180
	280		0.70	700	70	260
	300		0.75	750	75	300
	320		0.40	400	40	280
	340		0.40	400	40	290
	360		0.45	450	45	313
	380		0.85	850	85	330
	400		0.75	750	75	360
420		0.70	700	70	365	
440		0.85	850	85	340	
460		0.95	950	95	350	
480		1.20	1200	120	300	
500		1.23	1230	123	350	
520		1.30	1300	130	405	
540		1.50	1500	150	500	
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\* ULS : Ultimate Limit State

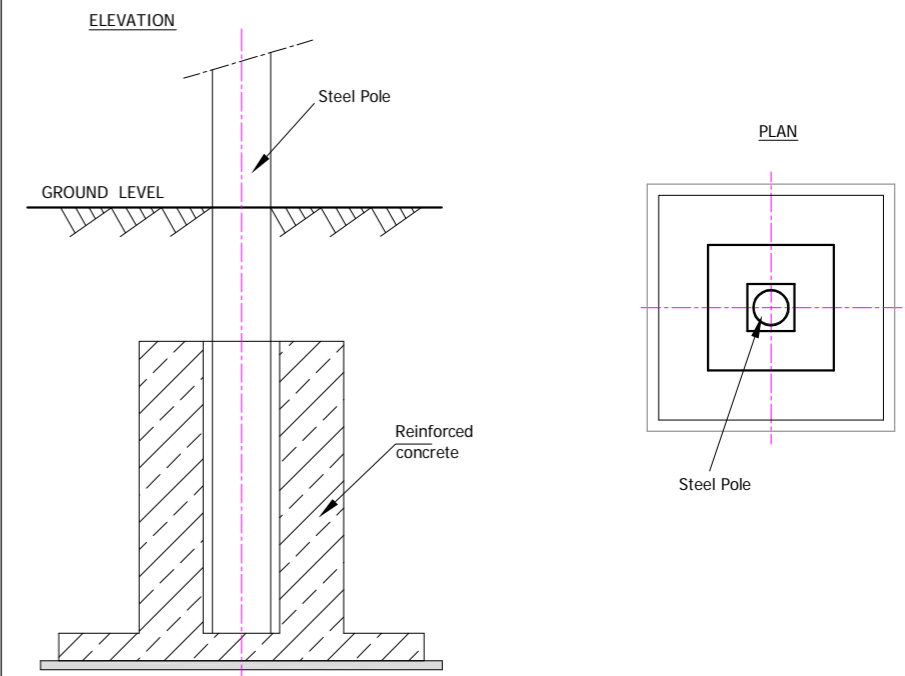
EXCAVATION ( test pit )



EXCAVATED SOIL



FOUNDATION TYPE



THE FOUNDATION DIMENSIONS COMPLY WITH THE STATIC BALANCE OF POST FIXING .



Soil investigation point	Extraction depth of the sample	Classification according USCS	Observation	ENGINEERING PROPERTIES		EXCAVATION CONDITION	GEOLOGICAL UNIT																																																																																																																																																																																																																																																																																																																						
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SIP 15 (15 kV line Gasogi - Masaka)	3.80 - 4.00 m	Clay of intermediate plasticity (CI)	Clayey soil (with laterit)	- IP : 20.7 - Consistency $I_c = 1.45 > 1$ very consistent ground - Compaction Backfill : $\rho_{OPM} = 1.78 \text{ t/m}^3$ $W_{OPM} = 18.0 \%$ - Natural soil : $\rho_h = 1.61 \text{ t/m}^3$ $W_{(N)} = 14.9 \%$ $\rho_s = 2.75 \text{ t/m}^3$	- Shear test : $C_{uu} = 41 \text{ kN/m}^2$ $\psi_{uu} = 26.10^\circ$ - Chemical analysis : $pH = 6.55$ $SO_4^2 = 9.7 \text{ mg/L}$ $Cl^- = 218.5 \text{ mg/L}$	Picks and shovels can be used.	<table border="1"> <thead> <tr> <th rowspan="2">DESCRIPTION OF THE PIT</th> <th rowspan="2">Depth (below existing ground level) cm</th> <th rowspan="2">Soil Stratigraphy (observed in the well)</th> <th colspan="3">GEOTECHNICAL BEARING CAPACITIES</th> </tr> <tr> <th>Allowable (Mpa)</th> <th>(Kpa)</th> <th>ULS * (Mpa)</th> <th>Ultimate (Mpa)</th> </tr> </thead> <tbody> <tr> <td rowspan="6">Vegetable soil 0.00 - 0.60 m</td> <td>0</td> <td></td> <td>0</td> <td>0</td> <td>0</td> <td>0</td> </tr> <tr> <td>20</td> <td></td> <td>0.55</td> <td>550</td> <td>55</td> <td>80</td> </tr> <tr> <td>40</td> <td></td> <td>0.45</td> <td>450</td> <td>45</td> <td>125</td> </tr> <tr> <td>60</td> <td></td> <td>1.00</td> <td>1000</td> <td>100</td> <td>300</td> </tr> <tr> <td>80</td> <td></td> <td>1.20</td> <td>1200</td> <td>120</td> <td>370</td> </tr> <tr> <td>100</td> <td></td> <td>1.40</td> <td>1400</td> <td>140</td> <td>400</td> </tr> <tr> <td rowspan="14">Clay of intermediate plasticity 0.60 - 4.00 m</td> <td>120</td> <td></td> <td>1.50</td> <td>1500</td> <td>150</td> <td>500</td> </tr> <tr><td>140</td><td></td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>160</td><td></td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>180</td><td></td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>200</td><td></td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>220</td><td></td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>240</td><td></td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>260</td><td></td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>280</td><td></td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>300</td><td></td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>320</td><td></td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>340</td><td></td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>360</td><td></td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>380</td><td></td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>400</td><td></td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>420</td><td></td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>440</td><td></td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>460</td><td></td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>480</td><td></td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>500</td><td></td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>520</td><td></td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>540</td><td></td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>560</td><td></td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>580</td><td></td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>600</td><td></td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>620</td><td></td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>640</td><td></td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>660</td><td></td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>680</td><td></td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>700</td><td></td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>720</td><td></td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>740</td><td></td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>760</td><td></td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>780</td><td></td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>800</td><td></td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>820</td><td></td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>840</td><td></td><td></td><td></td><td></td><td></td><td></td></tr> </tbody> </table>					DESCRIPTION OF THE PIT	Depth (below existing ground level) cm	Soil Stratigraphy (observed in the well)	GEOTECHNICAL BEARING CAPACITIES			Allowable (Mpa)	(Kpa)	ULS * (Mpa)	Ultimate (Mpa)	Vegetable soil 0.00 - 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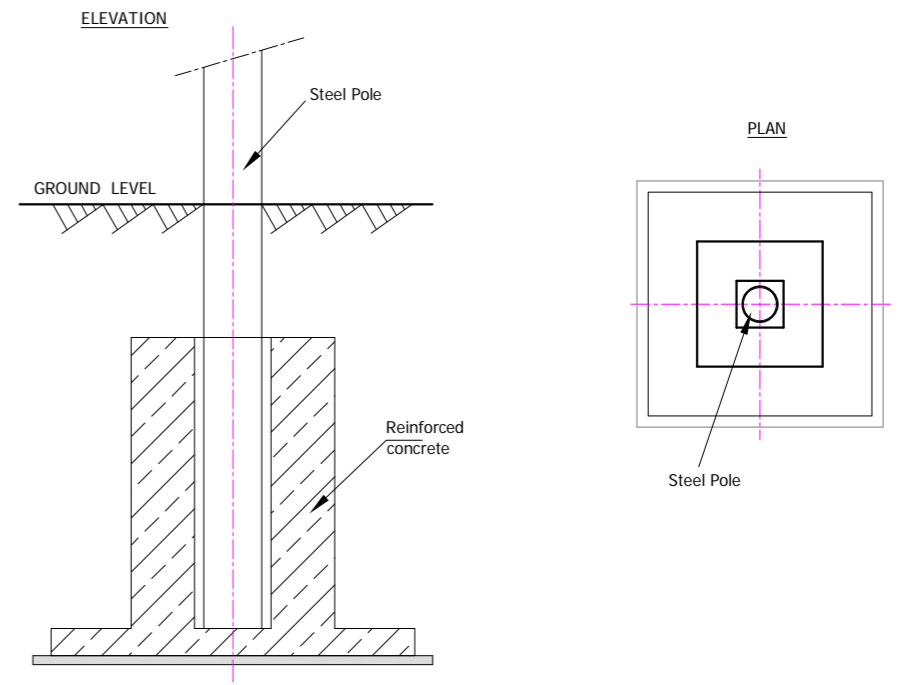
EXCAVATION ( test pit )



EXCAVATED SOIL



FOUNDATION TYPE



THE FOUNDATION DIMENSIONS COMPLY WITH THE STATIC BALANCE OF POST FIXING .

Soil investigation point	Extraction depth of the sample	Classification according USCS	Observation	ENGINEERING PROPERTIES		EXCAVATION CONDITION
SIP 16 (15 kV line Gasogi - Masaka)	3.40 - 4.00 m	Clay of intermediate plasticity (CI)	Lateritic zone	- IP : 19.2 - Consistency $I_c = 1.15 > 1$ very consistent ground - Compaction Backfill : $d_{OPM} = 1.77 \text{ t/m}^3$ $W_{OPM} = 20.40 \%$ - Natural soil : $\rho_h = 1.69 \text{ t/m}^3$ $W_{(N)} = 20.3 \%$ $\rho_s = 2.84 \text{ t/m}^3$	- Shear test : $C_{uu} = 39 \text{ kN/m}^2$ $\psi_{uu} = 22.29^\circ$ - Chemical analysis : $pH = 4.91$ $SO_4 = 6.9 \text{ mg/L}$ $Cl^- = 113.3 \text{ mg/L}$	Picks and shovels can be used.

GEOLOGICAL UNIT						
Light penetrometer ( GeoMIL 50 kN SPT ), test without soil coring for sample taking						
DESCRIPTION OF THE PIT	Depth (below existing ground level) cm	Soil Stratigraphy (observed in the well)	GEOTECHNICAL BEARING CAPACITIES			
			Allowable		ULS *	Ultimate
			(Mpa)	(Kpa)	(Mpa)	(Mpa)
Vegetable soil 0.00 - 0.25 m	0		0	0	0	0
	20		0.60	600	60	90
	40		0.43	430	43	120
	60		0.75	750	75	140
	80		0.93	930	93	260
	100		1.10	1100	110	380
	120		1.20	1200	120	420
	140		1.50	1500	150	500
	160					
	180					
Clay of intermediate plasticity 0.25 - 4.00 m	200					
	220					
	240					
	260					
	280					
	300					
	320					
	340					
	360					
	380					
	400					
	420					
	440					
	460					
	480					
	500					
	520					
	540					
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	740					
	760					
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	800					
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\* ULS : Ultimate Limit State

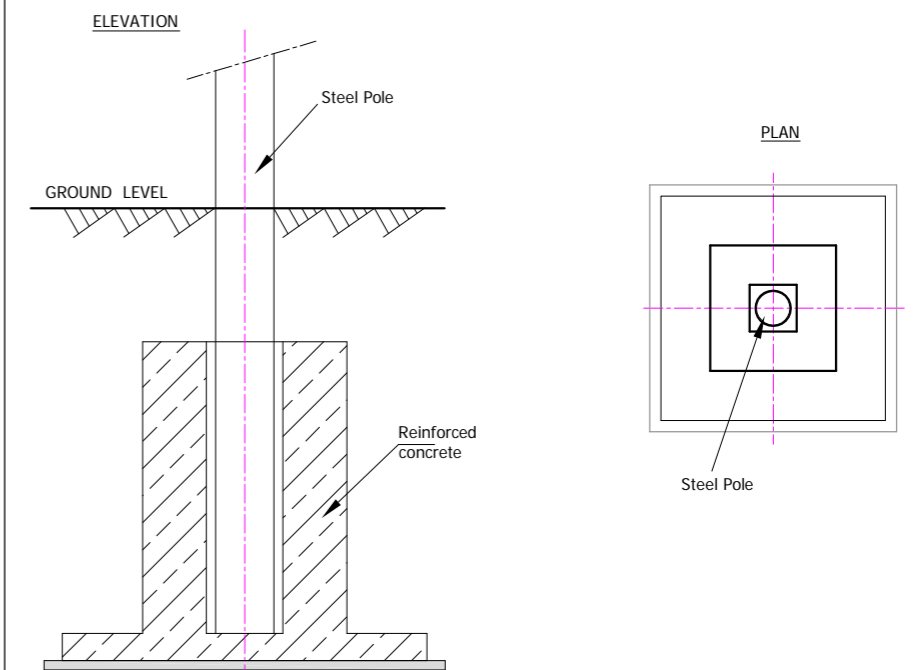
EXCAVATION ( test pit )



EXCAVATED SOIL



FOUNDATION TYPE



THE FOUNDATION DIMENSIONS COMPLY WITH THE STATIC BALANCE OF POST FIXING .

Soil investigation point	Extraction depth of the sample	Classification according USCS	Observation	ENGINEERING PROPERTIES	EXCAVATION CONDITION
SIP 17 (15 kV line Gasogi - Masaka)	2.50 - 3.50 m	Clay of intermediate plasticity (CI)	Clayey soil	<p>- IP : 15.7</p> <p>- Consistency <math>I_c = 1.55 &gt; 1</math> very consistent ground</p> <p>- Compaction Backfill :</p> <p>. <math>d_{OPM} = 1.87 \text{ t/m}^3</math></p> <p>. <math>W_{OPM} = 15.65 \%</math></p> <p>- Natural soil :</p> <p>. <math>\rho_h = 1.63 \text{ t/m}^3</math></p> <p>. <math>W_{(N)} = 13.5 \%</math></p> <p>. <math>\rho_s = 2.75 \text{ t/m}^3</math></p> <p>- Shear test :</p> <p>. <math>C_{uu} = 19 \text{ kN/m}^2</math></p> <p>. <math>\psi_{uu} = 31.59^\circ</math></p> <p>- Chemical analysis :</p> <p>. <math>pH = 6.49</math></p> <p>. <math>SO_4^2 = 11.7 \text{ mg/L}</math></p> <p>. <math>Cl^- = 208.4 \text{ mg/L}</math></p>	Picks and shovels can be used.

GEOLOGICAL UNIT					
Light penetrometer ( GeoMIL 50 kN SPT ), test without soil coring for sample taking					
DESCRIPTION OF THE PIT	Depth (below existing ground level) / cm	Soil Stratigraphy (observed in the well)	GEO TECHNICAL BEARING CAPACITIES		
			Allowable		Ultimate (Mpa)
			(Mpa)	(Kpa)	
Vegetable soil 0.00 - 0.25 m	0		0	0	0
Clay of intermediate plasticity 0.40 - 4.00 m	20		0.55	550	55
	40		0.50	500	50
	60		0.45	450	45
	80		0.50	500	50
	100		0.40	400	40
	120		0.30	300	30
	140		0.35	350	35
	160		0.30	300	30
	180		0.25	250	25
	200		0.30	300	30
	220		0.40	400	40
	240		0.50	500	50
	260		0.60	600	60
	280		0.65	650	65
	300		0.65	650	65
	320		0.70	700	70
	340		0.50	500	50
	360		0.55	550	55
	380		0.65	650	65
	400		0.60	600	60
420		0.63	630	63	
440		0.60	600	60	
460		0.70	700	70	
480		0.83	830	83	
500		1.00	1000	100	
520		1.15	1150	115	
540		1.50	1500	150	
560					
580					
620					
640					
660					
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\* ULS : Ultimate Limit State

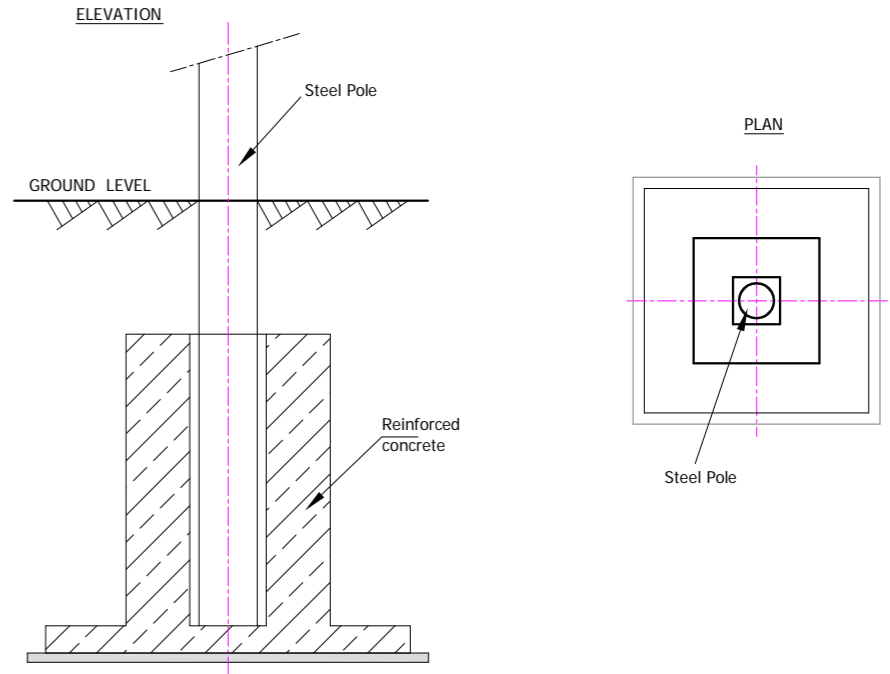
EXCAVATION ( test pit )



EXCAVATED SOIL




FOUNDATION TYPE



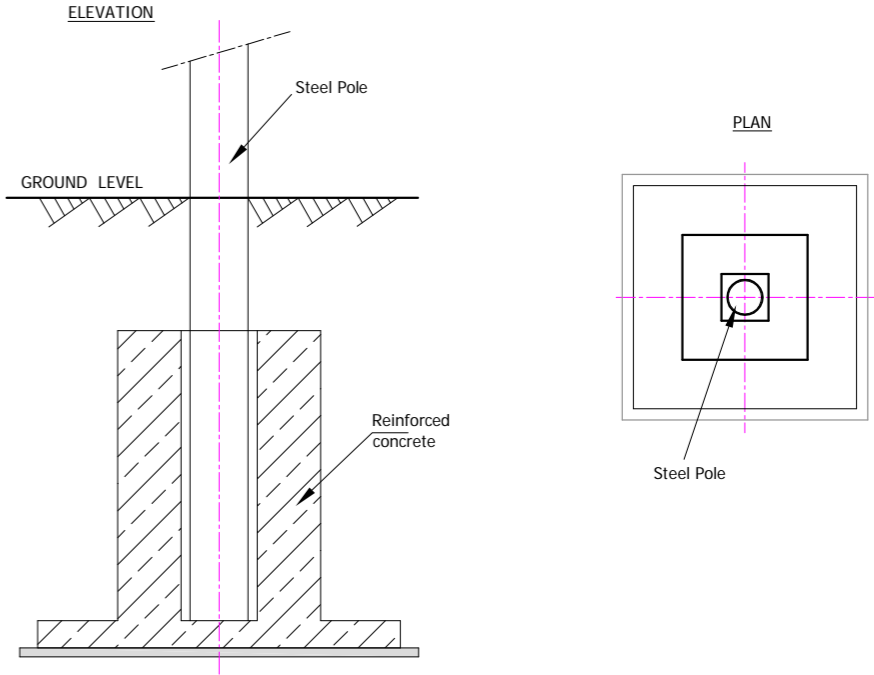
THE FOUNDATION DIMENSIONS COMPLY WITH THE STATIC BALANCE OF POST FIXING .

Soil investigation point	Extraction depth of the sample	Classification according USCS	Observation	ENGINEERING PROPERTIES	EXCAVATION CONDITION	GEOLOGICAL UNIT																																																																																																																																																																																																																																																																																					
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SIP 18 (15 kV line Gasogi - Masaka)	Visual observation	Clay of intermediate plasticity ( CI )	Clayey soil :  Soil of the same physical & mechanical nature as the soil of the Sip 17	- IP : 16.4  - Consistency $I_c = 1.55 > 1$ very consistent ground  - Compaction Backfill : $d_{OPM} = 1.84 \text{ t/m}^3$ $W_{OPM} = 15.20 \%$  - Natural soil : $\rho_h = 1.73 \text{ t/m}^3$ $W_{(N)} = 15 \%$ $\rho_s = 2.70 \text{ t/m}^3$	- Shear test : $C_{uu} = 21 \text{ kN/m}^2$ $\psi_{uu} = 26.79^\circ$  - Chemical analysis : $pH = 4.94$ $SO_4^{2-} = 65.2 \text{ mg/L}$ $Cl^- = 102.2 \text{ mg/L}$	Picks and shovels can be used.	<table border="1"> <thead> <tr> <th rowspan="2">DESCRIPTION OF THE PIT</th> <th rowspan="2">Depth (below existing ground level) cm</th> <th rowspan="2">Soil Stratigraphy (observed in the well)</th> <th colspan="4">GEOTECHNICAL BEARING CAPACITIES</th> </tr> <tr> <th colspan="2">Allowable</th> <th>ULS *</th> <th>Ultimate</th> </tr> <tr> <th></th> <th></th> <th></th> <th>( Mpa )</th> <th>( Kpa )</th> <th>( Mpa )</th> <th>( Mpa )</th> </tr> </thead> <tbody> <tr> <td rowspan="2">Vegetable soil 0.00 - 0.25 m</td> <td>0</td> <td></td> <td>0</td> <td>0</td> <td>0</td> <td>0</td> </tr> <tr> <td>20</td> <td></td> <td>0.50</td> <td>500</td> <td>50</td> <td>65</td> </tr> <tr> <td rowspan="20">Clay of intermediate plasticity 0.25 - 4.00 m</td> <td>40</td> <td></td> <td>0.25</td> <td>250</td> <td>25</td> <td>62</td> </tr> <tr> <td>60</td> <td></td> <td>0.20</td> <td>200</td> <td>20</td> <td>56</td> </tr> <tr> <td>80</td> <td></td> <td>0.20</td> <td>200</td> <td>20</td> <td>42</td> </tr> <tr> <td>100</td> <td></td> <td>0.25</td> <td>250</td> <td>25</td> <td>60</td> </tr> <tr> <td>120</td> <td></td> <td>0.30</td> <td>300</td> <td>30</td> <td>60</td> </tr> <tr> <td>140</td> <td></td> <td>0.20</td> <td>200</td> <td>20</td> <td>45</td> </tr> <tr> <td>160</td> <td></td> <td>0.30</td> <td>300</td> <td>30</td> <td>63</td> </tr> <tr> <td>180</td> <td></td> <td>0.40</td> <td>400</td> <td>40</td> <td>50</td> </tr> <tr> <td>200</td> <td></td> <td>0.40</td> <td>400</td> <td>40</td> <td>80</td> </tr> <tr> <td>220</td> <td></td> <td>0.35</td> <td>350</td> <td>35</td> <td>80</td> </tr> <tr> <td>240</td> <td></td> <td>0.50</td> <td>500</td> <td>50</td> <td>100</td> </tr> <tr> <td>260</td> <td></td> <td>0.65</td> <td>650</td> <td>65</td> <td>125</td> </tr> <tr> <td>280</td> <td></td> <td>0.55</td> <td>550</td> <td>55</td> <td>150</td> </tr> <tr> <td>300</td> <td></td> <td>0.70</td> <td>700</td> <td>70</td> <td>180</td> </tr> <tr> <td>320</td> <td></td> <td>0.63</td> <td>630</td> <td>63</td> <td>180</td> </tr> <tr> <td>340</td> <td></td> <td>0.70</td> <td>700</td> <td>70</td> <td>240</td> </tr> <tr> <td>360</td> <td></td> <td>0.65</td> <td>650</td> <td>65</td> <td>235</td> </tr> <tr> <td>380</td> <td></td> <td>0.35</td> <td>350</td> <td>35</td> <td>245</td> </tr> <tr> <td>400</td> <td></td> <td>0.65</td> <td>650</td> <td>65</td> <td>270</td> </tr> <tr> <td>420</td> <td></td> <td>0.62</td> <td>620</td> <td>62</td> <td>280</td> </tr> <tr> <td>440</td> <td></td> <td>0.65</td> <td>650</td> <td>65</td> <td>310</td> </tr> <tr> <td>460</td> <td></td> <td>0.70</td> <td>700</td> <td>70</td> <td>320</td> </tr> <tr> <td>480</td> <td></td> <td>0.80</td> <td>800</td> <td>80</td> <td>330</td> </tr> <tr> <td>500</td> <td></td> <td>1.00</td> <td>1000</td> <td>100</td> <td>400</td> </tr> <tr> <td>520</td> <td></td> <td>1.50</td> <td>1500</td> <td>150</td> <td>500</td> </tr> <tr> <td>540</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>560</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>580</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>620</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>640</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>660</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>680</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>700</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>720</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>740</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>760</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>780</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>800</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>820</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>840</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> </tbody> </table>					DESCRIPTION OF THE PIT	Depth (below existing ground level) cm	Soil Stratigraphy (observed in the well)	GEOTECHNICAL BEARING CAPACITIES				Allowable		ULS *	Ultimate				( Mpa )	( Kpa )	( Mpa )	( Mpa )	Vegetable soil 0.00 - 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EXCAVATED SOIL



FOUNDATION TYPE



THE FOUNDATION DIMENSIONS COMPLY WITH THE STATIC BALANCE OF POST FIXING .

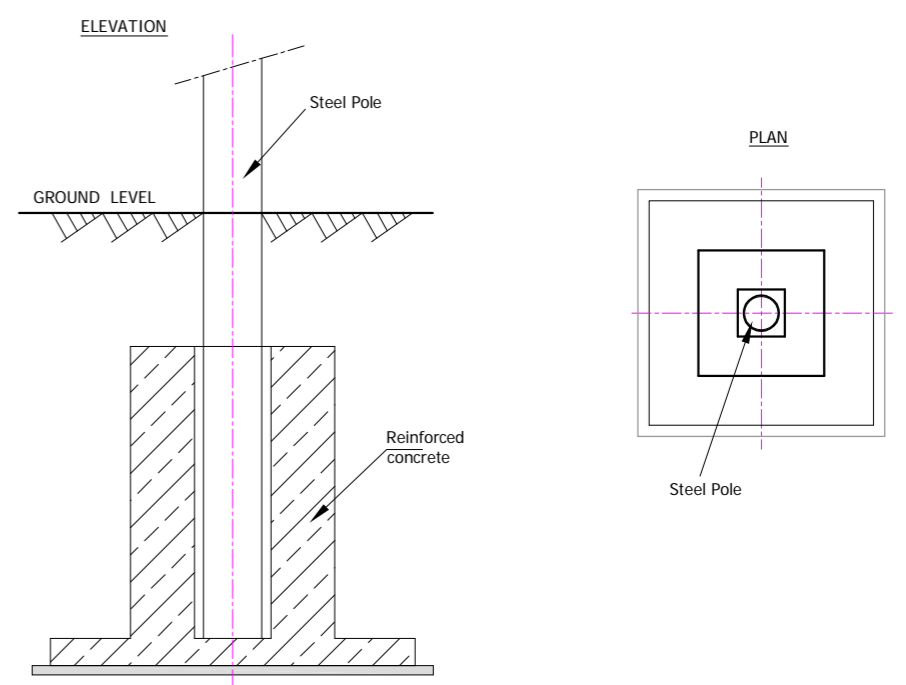
Soil investigation point	Extraction depth of the sample	Classification according USCS	Observation	ENGINEERING PROPERTIES	EXCAVATION CONDITION	GEOLOGICAL UNIT																																																																																																																																																																																																																																																																																																																										
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\* ULS : Ultimate Limit State

EXCAVATED SOIL

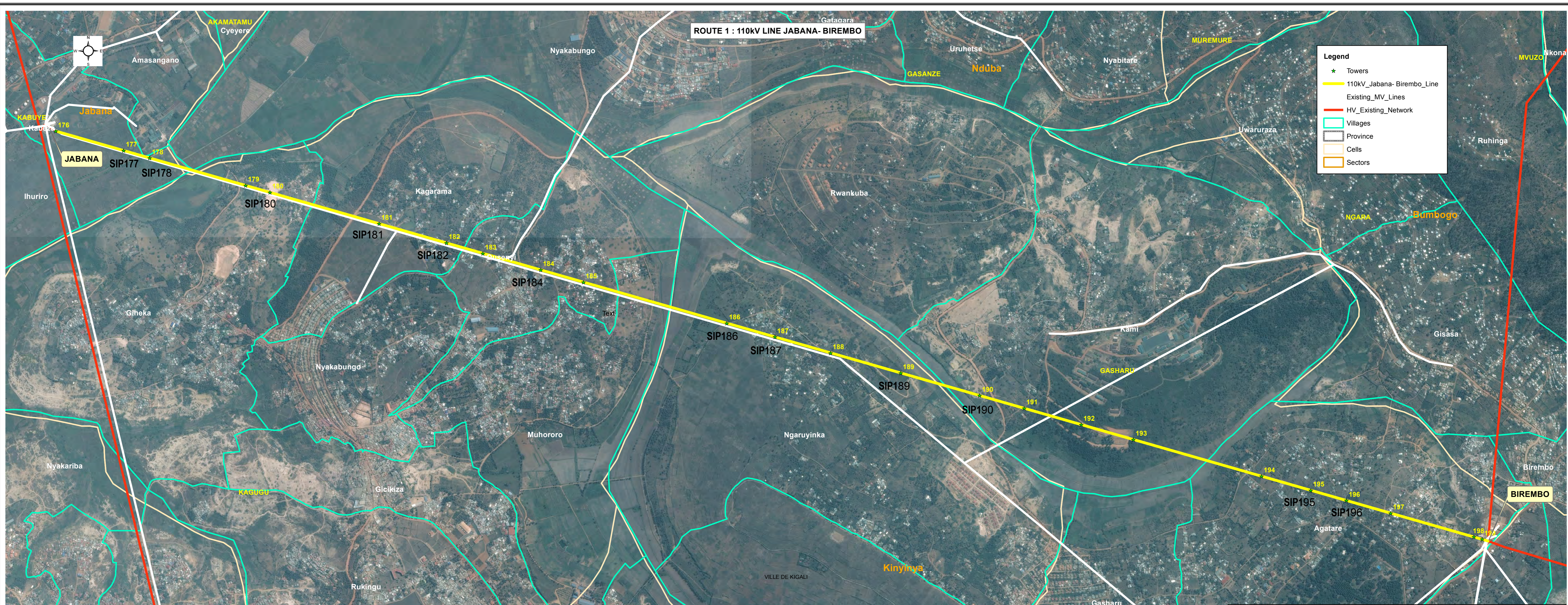


FOUNDATION TYPE



THE FOUNDATION DIMENSIONS COMPLY WITH THE STATIC BALANCE OF POST FIXING .

D. Reconstruction of transmission 110 kV line Jabana-Birembo



**ROUTE 1 : 110kV LINE JABANA- BIREMBO**

**Legend**

- \* Towers
- 110kV\_Jabana- Birembo\_Line
- Existing\_MV\_Lines
- HV\_Existing\_Network
- Villages
- Province
- Cells
- Sectors

Towers	X	Y
178	507828.9251	4790828.881
179	508274.0097	4790701.441
180	508386.4321	4790669.252
181	508892.5636	4790524.332
182	509205.1044	4790434.843
183	509373.4015	4790386.655
184	509641.556	4790309.876
185	509840.0016	4790253.055
186	510504.7781	4790062.712
187	510727.7962	4789998.856
188	510985.4621	4789925.079
189	511310.6545	4789831.967
190	511677.1473	4789727.031
191	511883.0435	4789668.077
192	512147.7947	4789592.271
193	512389.7326	4789522.998
194	512984.0216	4789352.837
195	513212.087	4789287.536
196	513377.4711	4789240.182
197	513582.5021	4789181.476
198	513968.7414	4789070.885
176	507393.4434	4790953.571
177	507708.2626	4790863.43
199	514006.3213	4789060.125

1	110kV line Jabana - Birembo													
REV.	DESCRIPTION	ISSUED	CHECKED											
<b>REPUBLIC OF RWANDA</b>  <b>MINISTRY OF INFRASTRUCTURE</b>														
Buyer :														
Execution Agency :														
Consultant														
Project :														
<b>PROJECT FOR IMPROVEMENT OF SUBSTATIONS AND DISTRIBUTION NETWORK</b> <b>JICA PROJECT PHASE III</b>														
Content :														
<b>SOIL INVESTIGATION</b> <b>ROUTE 1 : 110kV LINE JABANA - BIREMBO</b>														
Name	Fonction	Approval by Client	Date	Signature & Stamp										
<table border="1"> <tr> <th>PAPER SIZE</th> <th>SCALE</th> <th>DRAWING N°</th> <th>DATE</th> <th>DRAWING STATUS</th> </tr> <tr> <td>A1</td> <td>1/9 000</td> <td>01/JPP3</td> <td>June 2017</td> <td>FOR APPROVAL</td> </tr> </table>					PAPER SIZE	SCALE	DRAWING N°	DATE	DRAWING STATUS	A1	1/9 000	01/JPP3	June 2017	FOR APPROVAL
PAPER SIZE	SCALE	DRAWING N°	DATE	DRAWING STATUS										
A1	1/9 000	01/JPP3	June 2017	FOR APPROVAL										

Soil investigation point	Extraction depth of the sample	Classification according USCS	Observation	ENGINEERING PROPERTIES	EXCAVATION CONDITION	GEOLOGICAL UNIT																																																																																																																																																																																																																																																																																			
						Light penetrometer ( GeoMIL 50 kN SPT ), test without soil coring for sample taking																																																																																																																																																																																																																																																																																			
SIP 176 (110 kV line Jabana - Birembo)	0.50 - 1.00 m	Clay of intermediate plasticity ( CI )	Swampy area	- IP : 19.3 - Consistency $I_c = 1.16 > 1$ : very consistent ground - Compaction Backfill : $d_{OPM} = 1.83 \text{ t/m}^3$ $W_{OPM} = 17.32 \%$ - Natural soil : $\rho_h = 1.71 \text{ t/m}^3$ $W_{(N)} = 19.7 \%$ $\rho_s = 2.81 \text{ t/m}^3$	Picks and shovels can be used.	<table border="1"> <thead> <tr> <th rowspan="2">DESCRIPTION OF THE PIT</th> <th rowspan="2">Depth (below existing ground level) cm</th> <th rowspan="2">Soil Stratigraphy (observed in the well)</th> <th colspan="4">GEO TECHNICAL BEARING CAPACITIES</th> </tr> <tr> <th colspan="2">Allowable</th> <th>ULS *</th> <th>Ultimate</th> </tr> <tr> <th></th> <th></th> <th></th> <th>(Mpa)</th> <th>(Kpa)</th> <th>(Mpa)</th> <th>(Mpa)</th> </tr> </thead> <tbody> <tr> <td rowspan="10">Vegetable soil 0.00 - 0.30 m</td> <td>0</td> <td></td> <td>0</td> <td>0</td> <td>0</td> <td>0</td> </tr> <tr> <td>20</td> <td></td> <td>0.06</td> <td>60</td> <td>12</td> <td>20</td> </tr> <tr> <td>40</td> <td></td> <td>0.06</td> <td>60</td> <td>12</td> <td>20</td> </tr> <tr> <td>60</td> <td></td> <td>0.07</td> <td>70</td> <td>14</td> <td>23</td> </tr> <tr> <td>80</td> <td></td> <td>0.09</td> <td>90</td> <td>18</td> <td>25</td> </tr> <tr> <td>100</td> <td></td> <td>0.10</td> <td>100</td> <td>20</td> <td>27</td> </tr> <tr> <td>120</td> <td></td> <td>0.09</td> <td>90</td> <td>18</td> <td>27</td> </tr> <tr> <td>140</td> <td></td> <td>0.10</td> <td>100</td> <td>20</td> <td>33</td> </tr> <tr> <td>160</td> <td></td> <td>0.12</td> <td>120</td> <td>25</td> <td>35</td> </tr> <tr> <td>180</td> <td></td> <td>0.17</td> <td>170</td> <td>35</td> <td>45</td> </tr> <tr> <td rowspan="10">Clayey of intermediate plasticity 0.30 - 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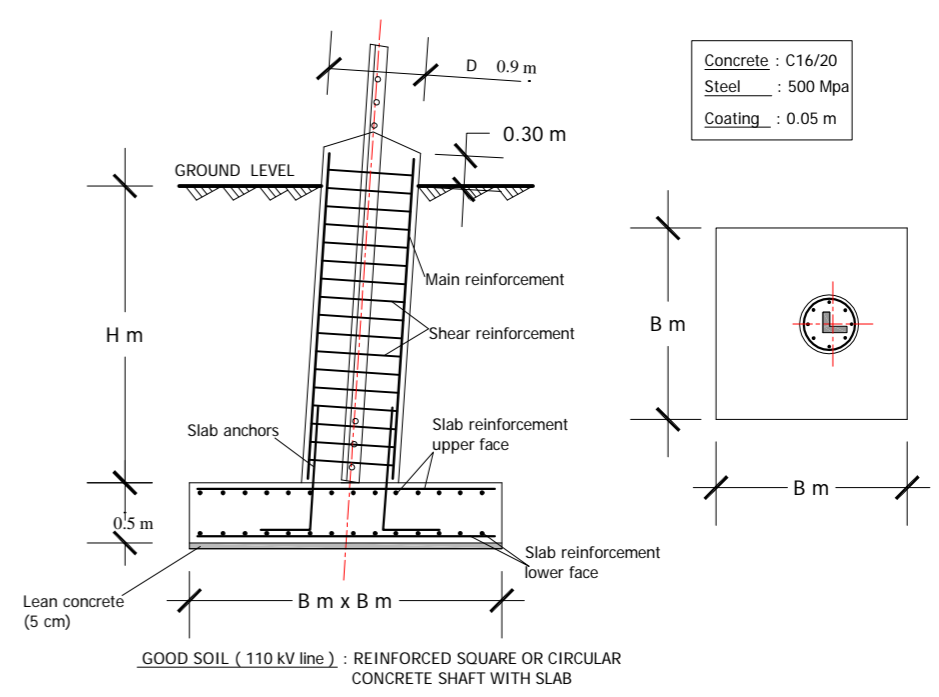
EXCAVATION ( test pit )



EXCAVATED SOIL



FOUNDATION TYPE



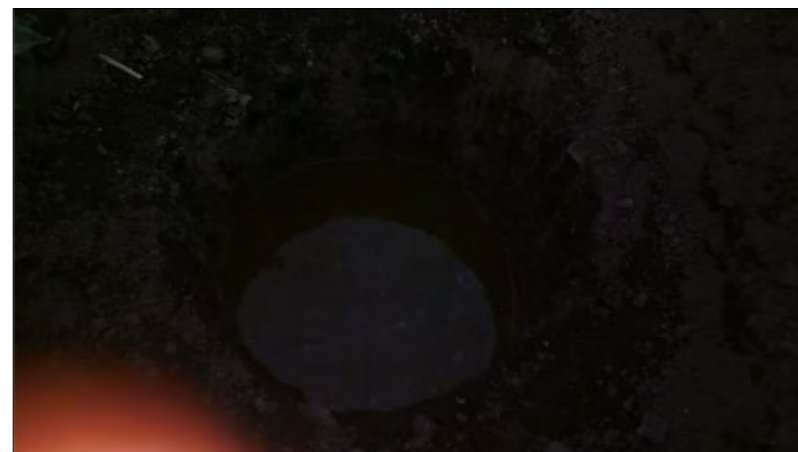
GOOD SOIL ( 110 kV line ) : REINFORCED SQUARE OR CIRCULAR CONCRETE SHAFT WITH SLAB

THE FOUNDATION DIMENSIONS COMPLY WITH THE STATIC BALANCE OF TOWER FIXING .



Soil investigation point	Extraction depth of the sample	Classification according USCS	Observation	ENGINEERING PROPERTIES	EXCAVATION CONDITION	GEOLOGICAL UNIT																																																																																																																																																																																																																																																																																																																									
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SIP 177 (110 kV line Jabana - Birembo)	0.50 - 1.00 m	Clay of intermediate plasticity ( CI )	Swampy area	- IP : 20.7  - Consistency $I_c = 1.33 > 1$ : very consistent ground  - Compaction Backfill : $\rho_d^{OPM} = 1.81 \text{ t/m}^3$ $W_{OPM} = 18.98 \%$	Picks and shovels can be used.	<table border="1"> <thead> <tr> <th rowspan="2">DESCRIPTION OF THE PIT</th> <th rowspan="2">Depth (below existing ground level) cm</th> <th rowspan="2">Soil Stratigraphy (soil type in the well)</th> <th colspan="4">GEOTECHNICAL BEARING CAPACITIES</th> </tr> <tr> <th colspan="2">Allowable</th> <th>ULS *</th> <th>Ultimate</th> </tr> <tr> <th></th> <th></th> <th></th> <th>(Mpa)</th> <th>(Kpa)</th> <th>(Mpa)</th> <th>(Mpa)</th> </tr> </thead> <tbody> <tr><td>Vegetable soil</td><td>0</td><td></td><td>0</td><td>0</td><td>0</td><td>0</td></tr> <tr><td></td><td>20</td><td></td><td>0.10</td><td>100</td><td>20</td><td>35</td></tr> <tr><td></td><td>40</td><td></td><td>0.09</td><td>90</td><td>19</td><td>33</td></tr> <tr><td></td><td>60</td><td></td><td>0.09</td><td>90</td><td>18</td><td>35</td></tr> <tr><td></td><td>80</td><td></td><td>0.10</td><td>100</td><td>21</td><td>38</td></tr> <tr><td></td><td>100</td><td></td><td>0.10</td><td>100</td><td>20</td><td>38</td></tr> <tr><td></td><td>120</td><td></td><td>0.09</td><td>90</td><td>19</td><td>38</td></tr> <tr><td></td><td>140</td><td></td><td>0.10</td><td>100</td><td>21</td><td>39</td></tr> <tr><td></td><td>160</td><td></td><td>0.11</td><td>110</td><td>22</td><td>40</td></tr> <tr><td></td><td>180</td><td></td><td>0.11</td><td>110</td><td>22</td><td>42</td></tr> <tr><td></td><td>200</td><td></td><td>0.12</td><td>120</td><td>25</td><td>42</td></tr> <tr><td></td><td>220</td><td></td><td>0.13</td><td>130</td><td>27</td><td>45</td></tr> <tr><td></td><td>240</td><td></td><td>0.16</td><td>160</td><td>32</td><td>50</td></tr> <tr><td></td><td>260</td><td></td><td>0.17</td><td>170</td><td>35</td><td>54</td></tr> <tr><td></td><td>280</td><td></td><td>0.18</td><td>180</td><td>37</td><td>55</td></tr> <tr><td></td><td>300</td><td></td><td>0.23</td><td>230</td><td>47</td><td>70</td></tr> <tr><td></td><td>320</td><td></td><td>0.24</td><td>240</td><td>49</td><td>68</td></tr> <tr><td></td><td>340</td><td></td><td>0.26</td><td>260</td><td>52</td><td>80</td></tr> <tr><td></td><td>360</td><td></td><td>0.28</td><td>280</td><td>57</td><td>92</td></tr> <tr><td></td><td>380</td><td></td><td>0.33</td><td>330</td><td>67</td><td>100</td></tr> <tr><td></td><td>400</td><td></td><td>0.28</td><td>280</td><td>57</td><td>100</td></tr> <tr><td></td><td>420</td><td></td><td>0.23</td><td>230</td><td>47</td><td>95</td></tr> <tr><td></td><td>440</td><td></td><td>0.26</td><td>260</td><td>52</td><td>80</td></tr> <tr><td></td><td>460</td><td></td><td>0.26</td><td>260</td><td>52</td><td>90</td></tr> <tr><td></td><td>480</td><td></td><td>0.25</td><td>250</td><td>50</td><td>90</td></tr> <tr><td></td><td>500</td><td></td><td>0.28</td><td>280</td><td>57</td><td>95</td></tr> <tr><td></td><td>520</td><td></td><td>0.31</td><td>310</td><td>62</td><td>100</td></tr> <tr><td></td><td>540</td><td></td><td>0.26</td><td>260</td><td>53</td><td>140</td></tr> <tr><td></td><td>560</td><td></td><td>0.30</td><td>300</td><td>60</td><td>150</td></tr> <tr><td></td><td>580</td><td></td><td>0.25</td><td>250</td><td>50</td><td>150</td></tr> <tr><td></td><td>600</td><td></td><td>0.35</td><td>350</td><td>70</td><td>200</td></tr> <tr><td></td><td>620</td><td></td><td>0.40</td><td>400</td><td>80</td><td>230</td></tr> <tr><td></td><td>640</td><td></td><td>0.42</td><td>420</td><td>85</td><td>255</td></tr> <tr><td></td><td>660</td><td></td><td>0.45</td><td>450</td><td>90</td><td>270</td></tr> <tr><td></td><td>680</td><td></td><td>0.40</td><td>400</td><td>80</td><td>290</td></tr> <tr><td></td><td>700</td><td></td><td>0.45</td><td>450</td><td>90</td><td>380</td></tr> <tr><td></td><td>720</td><td></td><td>0.50</td><td>500</td><td>100</td><td>420</td></tr> <tr><td></td><td>740</td><td></td><td>0.75</td><td>750</td><td>150</td><td>500</td></tr> <tr><td></td><td>760</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td></td><td>780</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td></td><td>800</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td></td><td>820</td><td></td><td></td><td></td><td></td><td></td></tr> </tbody> </table>	DESCRIPTION OF THE PIT	Depth (below existing ground level) cm	Soil Stratigraphy (soil type in the well)	GEOTECHNICAL BEARING CAPACITIES				Allowable		ULS *	Ultimate				(Mpa)	(Kpa)	(Mpa)	(Mpa)	Vegetable soil	0		0	0	0	0		20		0.10	100	20	35		40		0.09	90	19	33		60		0.09	90	18	35		80		0.10	100	21	38		100		0.10	100	20	38		120		0.09	90	19	38		140		0.10	100	21	39		160		0.11	110	22	40		180		0.11	110	22	42		200		0.12	120	25	42		220		0.13	130	27	45		240		0.16	160	32	50		260		0.17	170	35	54		280		0.18	180	37	55		300		0.23	230	47	70		320		0.24	240	49	68		340		0.26	260	52	80		360		0.28	280	57	92		380		0.33	330	67	100		400		0.28	280	57	100		420		0.23	230	47	95		440		0.26	260	52	80		460		0.26	260	52	90		480		0.25	250	50	90		500		0.28	280	57	95		520		0.31	310	62	100		540		0.26	260	53	140		560		0.30	300	60	150		580		0.25	250	50	150		600		0.35	350	70	200		620		0.40	400	80	230		640		0.42	420	85	255		660		0.45	450	90	270		680		0.40	400	80	290		700		0.45	450	90	380		720		0.50	500	100	420		740		0.75	750	150	500		760							780							800							820						
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				- Shear test : $C_{uu} = 41 \text{ kN/m}^2$ $\psi_{uu} = 19.03^\circ$																																																																																																																																																																																																																																																																																																																											
				- Natural soil : $\rho_h = 1.80 \text{ t/m}^3$ $W_{(N)} = 15.7 \%$ $\rho_s = 2.82 \text{ t/m}^3$																																																																																																																																																																																																																																																																																																																											
				- Chemical analysis : $pH = 6.52$ $SO_4^2 = 125 \text{ mg/L}$ $Cl^- = 283.6 \text{ mg/L}$																																																																																																																																																																																																																																																																																																																											

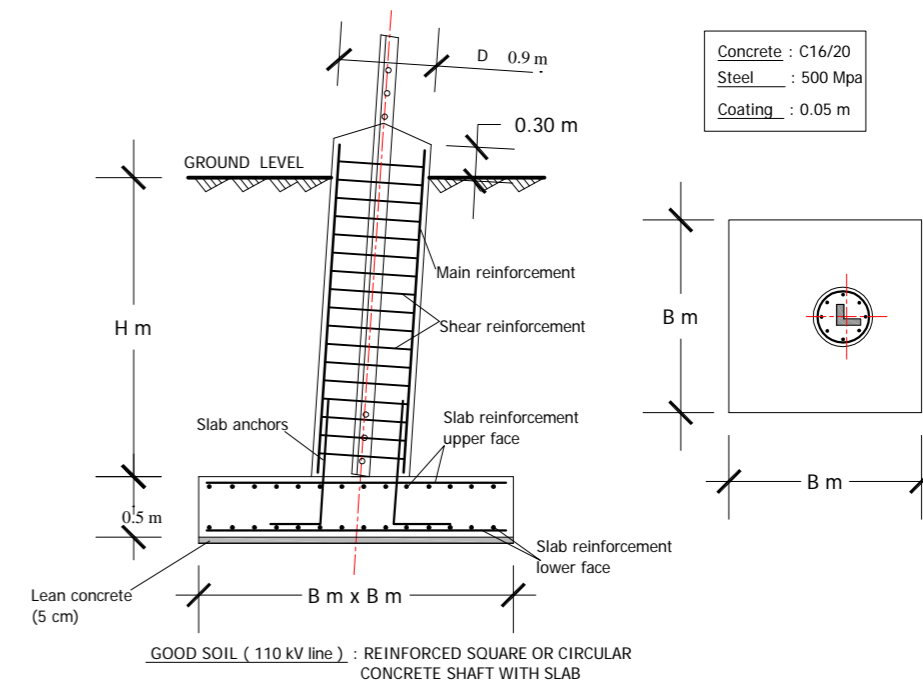
EXCAVATION ( test pit )



EXCAVATED SOIL




FOUNDATION TYPE




THE FOUNDATION DIMENSIONS COMPLY WITH THE STATIC BALANCE OF TOWER FIXING .

Soil investigation point	Extraction depth of the sample	Classification according USCS	Observation	ENGINEERING PROPERTIES	EXCAVATION CONDITION	GEOLOGICAL UNIT																																																																																																																																																																																																																																	
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SIP 178 (110 kV line Jabana - Birembo)	1.50 - 2.00 m	Silt of low plasticity ( ML )	Swampy area	- IP : / - Consistency $I_c < 1$ : soft soil - Compaction Backfill : $\rho_{d}^{OPM} = 1.69 \text{ t/m}^3$ $W_{OPM} = 22.0 \%$ - Natural soil : $\rho_h = 1.62 \text{ t/m}^3$ $W_{(N)} = 17.4 \%$ $\rho_s = 2.77 \text{ t/m}^3$	- Shear test : $C_{uu} = 10 \text{ kN/m}^2$ $\psi_{uu} = 26.79^\circ$ - Chemical analysis : $pH = 7.04$ $SO_4^{2-} = 70 \text{ mg/L}$ $Cl^- = 85 \text{ mg/L}$	Picks and shovels can be used.	<table border="1"> <thead> <tr> <th rowspan="2">DESCRIPTION OF THE PIT</th> <th rowspan="2">Depth (below existing ground level) cm</th> <th rowspan="2">Soil Stratigraphy (observed in the well)</th> <th colspan="3">GEOLOGICAL BEARING CAPACITIES</th> </tr> <tr> <th>Allowable (Mpa)</th> <th>(Kpa)</th> <th>Ultimate (Mpa)</th> </tr> </thead> <tbody> <tr> <td rowspan="2">Vegetable soil 0.00 - 0.30 m</td> <td>0</td> <td></td> <td>0</td> <td>0</td> <td>0</td> </tr> <tr> <td>20</td> <td></td> <td>0.03</td> <td>30</td> <td>6</td> </tr> <tr> <td rowspan="24">Silt of low plasticity 0.30 - 4.00 m  ( -3.00 m )</td> <td>40</td> <td></td> <td>0.05</td> <td>50</td> <td>10</td> </tr> <tr> <td>60</td> <td></td> <td>0.04</td> <td>40</td> <td>8</td> </tr> <tr> <td>80</td> <td></td> <td>0.07</td> <td>70</td> <td>15</td> </tr> <tr> <td>100</td> <td></td> <td>0.10</td> <td>100</td> <td>20</td> </tr> <tr> <td>120</td> <td></td> <td>0.12</td> <td>120</td> <td>25</td> </tr> <tr> <td>140</td> <td></td> <td>0.07</td> <td>70</td> <td>15</td> </tr> <tr> <td>160</td> <td></td> <td>0.10</td> <td>100</td> <td>20</td> </tr> <tr> <td>180</td> <td></td> <td>0.12</td> <td>120</td> <td>25</td> </tr> <tr> <td>200</td> <td></td> <td>0.06</td> <td>60</td> <td>13</td> </tr> <tr> <td>220</td> <td></td> <td>0.10</td> <td>100</td> <td>20</td> </tr> <tr> <td>240</td> <td></td> <td>0.10</td> <td>100</td> <td>20</td> </tr> <tr> <td>260</td> <td></td> <td>0.07</td> <td>70</td> <td>15</td> </tr> <tr> <td>280</td> <td></td> <td>0.10</td> <td>100</td> <td>20</td> </tr> <tr> <td>300</td> <td></td> <td>0.15</td> <td>150</td> <td>30</td> </tr> <tr> <td>320</td> <td></td> <td>0.20</td> <td>200</td> <td>40</td> </tr> <tr> <td>340</td> <td></td> <td>0.20</td> <td>200</td> <td>40</td> </tr> <tr> <td>360</td> <td></td> <td>0.20</td> <td>200</td> <td>40</td> </tr> <tr> <td>380</td> <td></td> <td>0.20</td> <td>200</td> <td>40</td> </tr> <tr> <td>400</td> <td></td> <td>0.14</td> <td>140</td> <td>38</td> </tr> <tr> <td>420</td> <td></td> <td>0.20</td> <td>200</td> <td>40</td> </tr> <tr> <td>440</td> <td></td> <td>0.24</td> <td>240</td> <td>48</td> </tr> <tr> <td>460</td> <td></td> <td>0.22</td> <td>220</td> <td>45</td> </tr> <tr> <td>480</td> <td></td> <td>0.25</td> <td>250</td> <td>50</td> </tr> <tr> <td>500</td> <td></td> <td>0.27</td> <td>270</td> <td>55</td> </tr> <tr> <td>520</td> <td></td> <td>0.30</td> <td>300</td> <td>60</td> </tr> <tr> <td>540</td> <td></td> <td>0.32</td> <td>320</td> <td>65</td> </tr> <tr> <td>560</td> <td></td> <td>0.30</td> <td>300</td> <td>60</td> </tr> <tr> <td>580</td> <td></td> <td>0.30</td> <td>300</td> <td>60</td> </tr> <tr> <td>600</td> <td></td> <td>0.35</td> <td>350</td> <td>70</td> </tr> <tr> <td>620</td> <td></td> <td>0.15</td> <td>150</td> <td>30</td> </tr> <tr> <td>640</td> <td></td> <td>0.05</td> <td>50</td> <td>10</td> </tr> <tr> <td>660</td> <td></td> <td>0.07</td> <td>70</td> <td>15</td> </tr> <tr> <td>680</td> <td></td> <td>0.10</td> <td>100</td> <td>20</td> </tr> <tr> <td>700</td> <td></td> <td>0.15</td> <td>150</td> <td>30</td> </tr> <tr> <td>720</td> <td></td> <td>0.15</td> <td>150</td> <td>30</td> </tr> <tr> <td>740</td> <td></td> <td>0.20</td> <td>200</td> <td>40</td> </tr> <tr> <td>760</td> <td></td> <td>0.50</td> <td>500</td> <td>100</td> </tr> <tr> <td>780</td> <td></td> <td>0.70</td> <td>700</td> <td>140</td> </tr> <tr> <td>800</td> <td></td> <td>0.75</td> <td>750</td> <td>150</td> </tr> <tr> <td>820</td> <td></td> <td></td> <td></td> <td></td> </tr> </tbody> </table>				DESCRIPTION OF THE PIT	Depth (below existing ground level) cm	Soil Stratigraphy (observed in the well)	GEOLOGICAL BEARING CAPACITIES			Allowable (Mpa)	(Kpa)	Ultimate (Mpa)	Vegetable soil 0.00 - 0.30 m	0		0	0	0	20		0.03	30	6	Silt of low plasticity 0.30 - 4.00 m  ( -3.00 m )	40		0.05	50	10	60		0.04	40	8	80		0.07	70	15	100		0.10	100	20	120		0.12	120	25	140		0.07	70	15	160		0.10	100	20	180		0.12	120	25	200		0.06	60	13	220		0.10	100	20	240		0.10	100	20	260		0.07	70	15	280		0.10	100	20	300		0.15	150	30	320		0.20	200	40	340		0.20	200	40	360		0.20	200	40	380		0.20	200	40	400		0.14	140	38	420		0.20	200	40	440		0.24	240	48	460		0.22	220	45	480		0.25	250	50	500		0.27	270	55	520		0.30	300	60	540		0.32	320	65	560		0.30	300	60	580		0.30	300	60	600		0.35	350	70	620		0.15	150	30	640		0.05	50	10	660		0.07	70	15	680		0.10	100	20	700		0.15	150	30	720		0.15	150	30	740		0.20	200	40	760		0.50	500	100	780		0.70	700	140	800		0.75	750	150	820				
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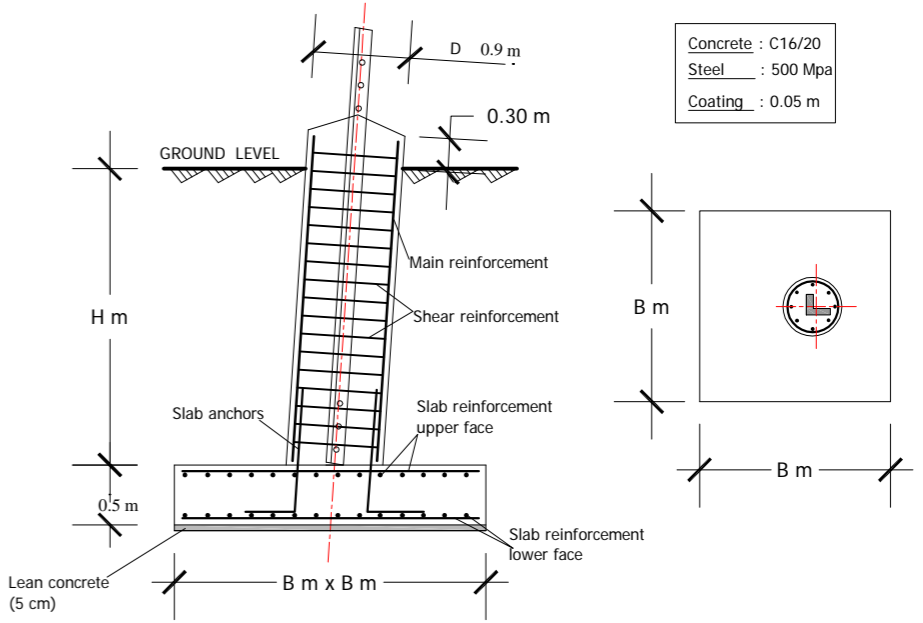
EXCAVATION ( test pit )



EXCAVATED SOIL



FOUNDATION TYPE

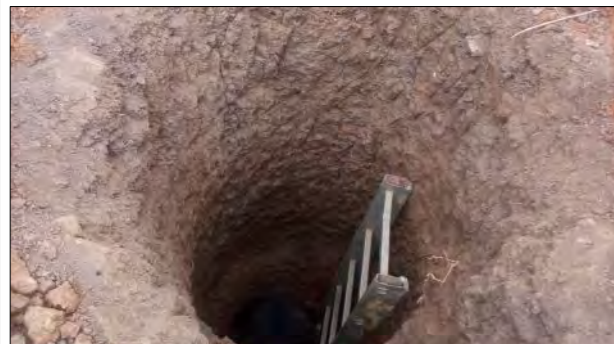


THE FOUNDATION DIMENSIONS COMPLY WITH THE STATIC BALANCE OF TOWER FIXING .

Soil investigation point	Extraction depth of the sample	Classification according USCS	Observation	ENGINEERING PROPERTIES	EXCAVATION CONDITION	GEOLOGICAL UNIT Light penetrometer ( GeoMIL 50 kN SPT ), test without soil coring for sample taking																																																																																																																																																																																																																																																																													
SIP 180 (110 kV line Jabana - Birembo)	3.70 - 4.00 m	Silt of intermediate plasticity ( MI )	Sandy soil	- IP : 17.3 - Consistency $I_c = 1.51 > 1$ : very consistent ground - Compaction Backfill : $d_{OPM} = 1.87 \text{ t/m}^3$ $W_{OPM} = 14.36 \%$ - Natural soil : $\rho_h = 1.80 \text{ t/m}^3$ $W_{(N)} = 16 \%$ $\rho_s = 2.80 \text{ t/m}^3$ - Shear test : $c_{uu} = 20 \text{ kN/m}^2$ $\psi_{uu} = 24.23^\circ$ - Chemical analysis : $pH = 6.2$ $SO_4^2 = 8.4 \text{ mg/L}$ $Cl^- = 81 \text{ mg/L}$	Picks and shovels can be used.	<table border="1"> <thead> <tr> <th rowspan="3">DESCRIPTION OF THE PIT</th> <th rowspan="3">Depth (below existing ground level) cm</th> <th rowspan="3">Soil Stratigraphy (observed in the well)</th> <th colspan="4">GEOTECHNICAL BEARING CAPACITIES</th> </tr> <tr> <th colspan="2">Allowable</th> <th rowspan="2">ULS *</th> <th rowspan="2">Ultimate</th> </tr> <tr> <th>( Mpa )</th> <th>( Kpa )</th> <th>( Mpa )</th> <th>( Mpa )</th> </tr> </thead> <tbody> <tr><td>Vegetable soil</td><td>0</td><td></td><td>0</td><td>0</td><td>0</td><td>0</td></tr> <tr><td rowspan="16">Silt of intermediate plasticity</td><td>20</td><td></td><td>0.20</td><td>200</td><td>20</td><td>40</td></tr> <tr><td>40</td><td></td><td>0.65</td><td>650</td><td>65</td><td>80</td></tr> <tr><td>60</td><td></td><td>0.40</td><td>400</td><td>40</td><td>65</td></tr> <tr><td>80</td><td></td><td>1.00</td><td>1000</td><td>100</td><td>180</td></tr> <tr><td>100</td><td></td><td>1.20</td><td>1200</td><td>120</td><td>360</td></tr> <tr><td>120</td><td></td><td>1.30</td><td>1300</td><td>130</td><td>400</td></tr> <tr><td>140</td><td></td><td>1.50</td><td>1500</td><td>150</td><td>500</td></tr> <tr><td>160</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>180</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>200</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>220</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>240</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>260</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>280</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>300</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>320</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>340</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>360</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>380</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>400</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>420</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>440</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>460</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>480</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>500</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>520</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>540</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>560</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>580</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>600</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>620</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>640</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>660</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>680</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>700</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>720</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>740</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>760</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>780</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>800</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>820</td><td></td><td></td><td></td><td></td><td></td></tr> </tbody> </table>	DESCRIPTION OF THE PIT	Depth (below existing ground level) cm	Soil Stratigraphy (observed in the well)	GEOTECHNICAL BEARING CAPACITIES				Allowable		ULS *	Ultimate	( Mpa )	( Kpa )	( Mpa )	( Mpa )	Vegetable soil	0		0	0	0	0	Silt of intermediate plasticity	20		0.20	200	20	40	40		0.65	650	65	80	60		0.40	400	40	65	80		1.00	1000	100	180	100		1.20	1200	120	360	120		1.30	1300	130	400	140		1.50	1500	150	500	160						180						200						220						240						260						280						300						320						340						360						380						400						420						440						460						480						500						520						540						560						580						600						620						640						660						680						700						720						740						760						780						800						820					
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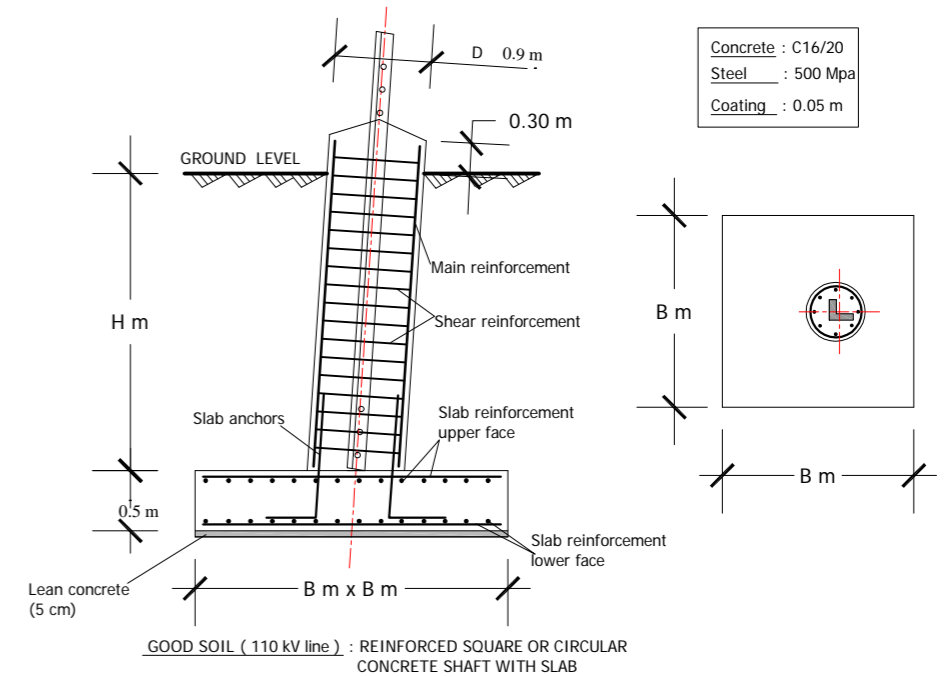
EXCAVATION ( test pit )



EXCAVATED SOIL



FOUNDATION TYPE



THE FOUNDATION DIMENSIONS COMPLY WITH THE STATIC BALANCE OF TOWER FIXING .

Soil investigation point	Extraction depth of the sample	Classification according USCS	Observation	ENGINEERING PROPERTIES	EXCAVATION CONDITION
SIP 181 (110 kV line Jabana - Birembo)	3.40 - 4.00 m	Clay of intermediate plasticity (CI)	Sandy soil	- IP : 21.1 - Consistency $I_c = 1.33 > 1$ : very consistent ground  - Compaction Backfill : $d_{OPM} = 1.94 \text{ t/m}^3$ $W_{OPM} = 15.20 \%$  - Natural soil : $\rho_h = 1.68 \text{ t/m}^3$ $W_{(N)} = 16 \%$ $\rho_s = 2.64 \text{ t/m}^3$	- Shear test : $C_{uu} = 31 \text{ kN/m}^2$ $\psi_{uu} = 23.75^\circ$  - Chemical analysis : $pH = 6.55$ $SO_4^{2-} = 34 \text{ mg/L}$ $Cl^- = 145 \text{ mg/L}$

GEOLOGICAL UNIT					
Light penetrometer ( GeoMIL 50 kN SPT ), test without soil coring for sample taking					
DESCRIPTION OF THE PIT	Depth (below existing ground level) cm	Soil Stratigraphy (observed in the well)	GEO TECHNICAL BEARING CAPACITIES		
			Allowable		Ultimate (Mpa)
			(Mpa)	(Kpa)	
Vegetable soil 0.00 - 0.40 m	0 20 40		0 0.25 0.30	0 250 300	0 25 30
Clay of intermediate plasticity 0.40 - 4.00 m	60		0.80	800	80
	80		0.90	900	90
	100		1.00	1000	100
	120		1.20	1200	120
	140		1.40	1400	140
	160		1.50	1500	150
	180				
	200				
	220				
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\* ULS : Ultimate Limit State

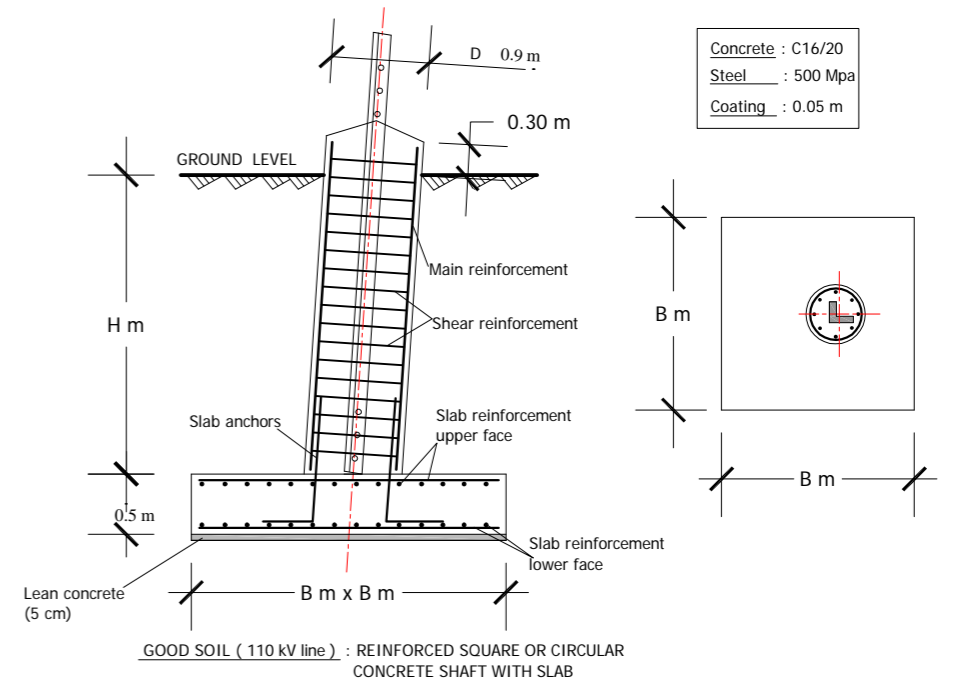
EXCAVATION ( test pit )



EXCAVATED SOIL




FOUNDATION TYPE




THE FOUNDATION DIMENSIONS COMPLY WITH THE STATIC BALANCE OF TOWER FIXING .

Soil investigation point	Extraction depth of the sample	Classification according USCS	Observation	ENGINEERING PROPERTIES		EXCAVATION CONDITION	GEOLOGICAL UNIT																																																																																																																																																																																																																																																																														
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SIP 182 (110 kV line Jabana - Birembo)	3.80 - 4.00 m	Clay of intermediate plasticity (CI)	Sandy soil	- IP : 21.5	- Shear test : . Cuu = 38 kN/m <sup>2</sup> . ψ <sub>uu</sub> = 23.03°	Picks and shovels can be used.	<table border="1"> <thead> <tr> <th rowspan="3">DESCRIPTION OF THE PIT</th> <th rowspan="3">Depth (below existing ground level) cm</th> <th rowspan="3">Soil Stratigraphy (observed in the well)</th> <th colspan="4">GEOTECHNICAL BEARING CAPACITIES</th> </tr> <tr> <th colspan="2">Allowable</th> <th rowspan="2">ULS *</th> <th rowspan="2">Ultimate</th> </tr> <tr> <th>(Mpa)</th> <th>(Kpa)</th> <th>(Mpa)</th> <th>(Mpa)</th> </tr> </thead> <tbody> <tr><td>Vegetable soil</td><td>0</td><td></td><td>0</td><td>0</td><td>0</td><td>0</td></tr> <tr><td rowspan="12">Clay of intermediate plasticity</td><td>20</td><td></td><td>0.10</td><td>100</td><td>10</td><td>20</td></tr> <tr><td>40</td><td></td><td>0.35</td><td>350</td><td>35</td><td>65</td></tr> <tr><td>60</td><td></td><td>0.50</td><td>500</td><td>50</td><td>90</td></tr> <tr><td>80</td><td></td><td>0.45</td><td>450</td><td>45</td><td>70</td></tr> <tr><td>100</td><td></td><td>0.80</td><td>800</td><td>80</td><td>245</td></tr> <tr><td>120</td><td></td><td>0.90</td><td>900</td><td>90</td><td>290</td></tr> <tr><td>140</td><td></td><td>1.00</td><td>1000</td><td>100</td><td>350</td></tr> <tr><td>160</td><td></td><td>1.30</td><td>1300</td><td>130</td><td>420</td></tr> <tr><td>180</td><td></td><td>1.50</td><td>1500</td><td>150</td><td>500</td></tr> <tr><td>200</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>220</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>240</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>260</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>280</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>300</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>320</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>340</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>360</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>380</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>400</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>420</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>440</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>460</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>480</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>500</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>520</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>540</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>560</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>580</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>600</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>620</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>640</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>660</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>680</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>700</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>720</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>740</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>760</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>780</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>800</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>820</td><td></td><td></td><td></td><td></td><td></td></tr> </tbody> </table>	DESCRIPTION OF THE PIT	Depth (below existing ground level) cm	Soil Stratigraphy (observed in the well)	GEOTECHNICAL BEARING CAPACITIES				Allowable		ULS *	Ultimate	(Mpa)	(Kpa)	(Mpa)	(Mpa)	Vegetable soil	0		0	0	0	0	Clay of intermediate plasticity	20		0.10	100	10	20	40		0.35	350	35	65	60		0.50	500	50	90	80		0.45	450	45	70	100		0.80	800	80	245	120		0.90	900	90	290	140		1.00	1000	100	350	160		1.30	1300	130	420	180		1.50	1500	150	500	200						220						240						260						280						300						320						340						360						380						400						420						440						460						480						500						520						540						560						580						600						620						640						660						680						700						720						740						760						780						800						820						
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				- Natural soil : . ρ <sub>h</sub> = 1.86 t/m <sup>3</sup> . W <sub>(N)</sub> = 20.7 % . ρ <sub>s</sub> = 2.67 t/m <sup>3</sup>	- Chemical analysis : . pH = 6.13 . SO <sub>4</sub> <sup>2-</sup> = 13.7 mg/L . Cl <sup>-</sup> = 101 mg/L																																																																																																																																																																																																																																																																																

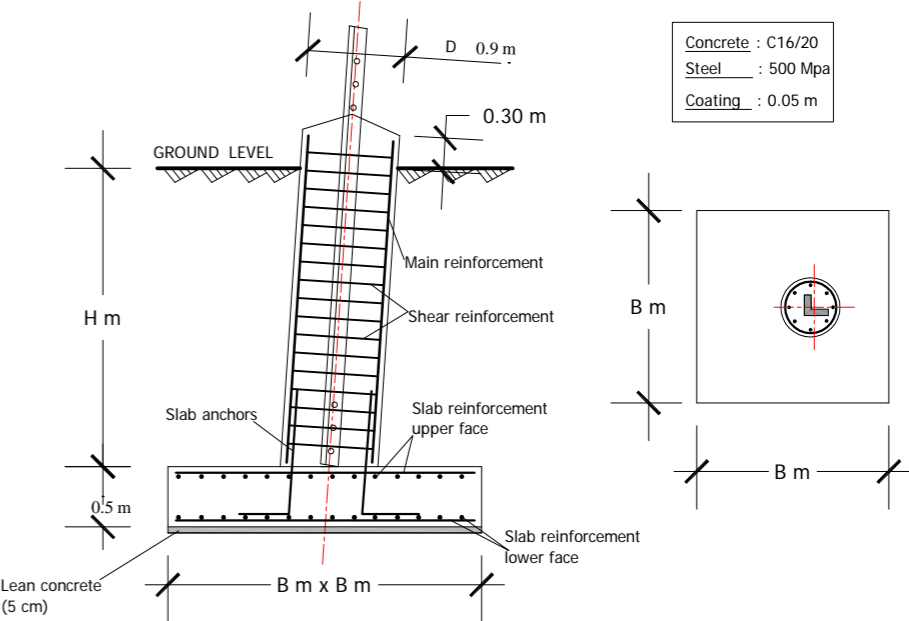
EXCAVATION ( test pit )



EXCAVATED SOIL



FOUNDATION TYPE




Concrete : C16/20  
Steel : 500 Mpa  
Coating : 0.05 m

GOOD SOIL ( 110 kV line ) : REINFORCED SQUARE OR CIRCULAR CONCRETE SHAFT WITH SLAB


THE FOUNDATION DIMENSIONS COMPLY WITH THE STATIC BALANCE OF TOWER FIXING .

Soil investigation point	Extraction depth of the sample	Classification according USCS	Observation	ENGINEERING PROPERTIES	EXCAVATION CONDITION	GEOLOGICAL UNIT																																																																																																																																																																																																																																																																																																																								
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SIP 184 (110 kV line Jabana - Birembo)	3.50 - 4.00 m	Clay of intermediate plasticity ( CI )	Clayey soil	- IP : 19.1 - Consistency $I_c = 0.97 < 1$ : consistent ground - Compaction Backfill : $\rho_{dOPM} = 1.79 \text{ t/m}^3$ $W_{OPM} = 19.0 \%$ - Natural soil : $\rho_h = 1.82 \text{ t/m}^3$ $W_{(N)} = 21.5 \%$ $\rho_s = 2.79 \text{ t/m}^3$	Picks and shovels can be used.	<table border="1"> <thead> <tr> <th rowspan="3">DESCRIPTION OF THE PIT</th> <th rowspan="3">Depth below existing ground level ( cm )</th> <th rowspan="3">Soil Stiffness (observed in the well)</th> <th colspan="4">GEO TECHNICAL BEARING CAPACITIES</th> </tr> <tr> <th colspan="2">Allowable</th> <th rowspan="2">ULS *</th> <th rowspan="2">Ultimate</th> </tr> <tr> <th>( Mpa )</th> <th>( Kpa )</th> <th>( Mpa )</th> <th>( Mpa )</th> </tr> </thead> <tbody> <tr><td>Vegetable soil</td><td>0</td><td></td><td>0</td><td>0</td><td>0</td><td>0</td></tr> <tr><td></td><td>20</td><td></td><td>0.85</td><td>850</td><td>85</td><td>100</td></tr> <tr><td></td><td>40</td><td></td><td>0.90</td><td>900</td><td>90</td><td>125</td></tr> <tr><td></td><td>60</td><td></td><td>0.95</td><td>950</td><td>95</td><td>140</td></tr> <tr><td></td><td>80</td><td></td><td>1.00</td><td>1000</td><td>100</td><td>160</td></tr> <tr><td></td><td>100</td><td></td><td>0.30</td><td>300</td><td>30</td><td>60</td></tr> <tr><td></td><td>120</td><td></td><td>0.25</td><td>250</td><td>25</td><td>50</td></tr> <tr><td></td><td>140</td><td></td><td>0.20</td><td>200</td><td>20</td><td>45</td></tr> <tr><td></td><td>160</td><td></td><td>0.30</td><td>300</td><td>30</td><td>50</td></tr> <tr><td></td><td>180</td><td></td><td>0.30</td><td>300</td><td>30</td><td>45</td></tr> <tr><td></td><td>200</td><td></td><td>0.80</td><td>800</td><td>80</td><td>100</td></tr> <tr><td></td><td>220</td><td></td><td>0.80</td><td>800</td><td>80</td><td>123</td></tr> 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soil	0		0	0	0	0		20		0.85	850	85	100		40		0.90	900	90	125		60		0.95	950	95	140		80		1.00	1000	100	160		100		0.30	300	30	60		120		0.25	250	25	50		140		0.20	200	20	45		160		0.30	300	30	50		180		0.30	300	30	45		200		0.80	800	80	100		220		0.80	800	80	123		240		0.75	750	75	150		260		0.85	850	85	180		280		0.90	900	90	220		300		1.00	1000	100	250		320		0.95	950	95	280		340		1.00	1000	100	320		360		1.15	1150	115	380		380		1.50	1500	150	500		400							420							440							460							480							500							520							540							560							580							600							620							640							660							680							700							720							740							760							780							800							820					
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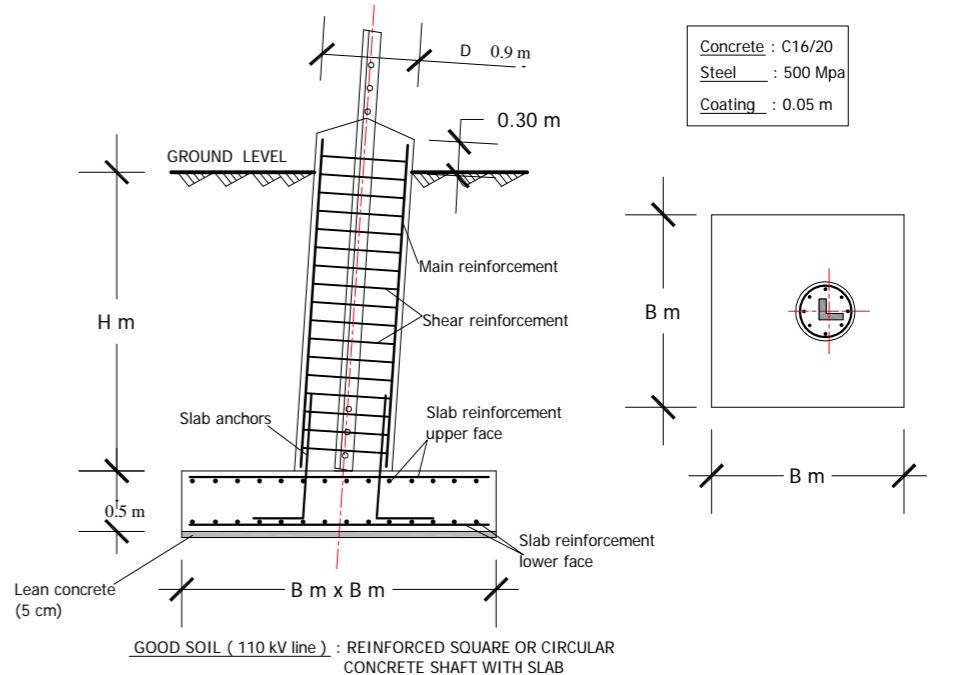
EXCAVATION ( test pit )



EXCAVATED SOIL



FOUNDATION TYPE



THE FOUNDATION DIMENSIONS COMPLY WITH THE STATIC BALANCE OF TOWER FIXING .

Soil investigation point	Extraction depth of the sample	Classification according USCS	Observation	ENGINEERING PROPERTIES	EXCAVATION CONDITION
SIP 186 (110 kV line Jabana - Birembo)	3.70 - 4.00 m	Clayey gravel poorly graded (GC)	Lateritic zone	<p>- IP : 16.4</p> <p>- Consistency <math>I_c = 1.52 &gt; 1</math> : very consistent ground</p> <p>- Compaction Backfill :</p> <p>    . <math>d_{OPM} = 1.93 \text{ t/m}^3</math></p> <p>    . <math>W_{OPM} = 15.88 \%</math></p> <p>- Natural soil :</p> <p>    . <math>\rho_h = 1.90 \text{ t/m}^3</math></p> <p>    . <math>W_{(N)} = 18.4 \%</math></p> <p>    . <math>\rho_s = 2.90 \text{ t/m}^3</math></p> <p>- Shear test :</p> <p>    . <math>C_{uu} = 23 \text{ kN/m}^2</math></p> <p>    . <math>\psi_{uu} = 26.57^\circ</math></p> <p>- Chemical analysis :</p> <p>    . pH = 6.44</p> <p>    . <math>SO_4^2 = 68.4 \text{ mg/L}</math></p> <p>    . <math>Cl^- = 113.4 \text{ mg/L}</math></p>	Picks and shovels can be used.

**GEOLOGICAL UNIT**  
Light penetrometer ( GeoMIL 50 kN SPT ), test without soil coring for sample taking

DESCRIPTION OF THE PIT	Depth (below existing ground level) cm	Soil Stratigraphy (observed in the well)	GEOTECHNICAL BEARING CAPACITIES			
			Allowable		ULS * (Mpa)	Ultimate (Mpa)
			(Mpa)	(Kpa)		
Vegetable soil 0.00 - 0.50 m	0 20 40 60 80		0 0.10 0.20 0.45 0.30	0 100 200 450 300	0 10 20 45 30	0 25 35 70 55
Clayey gravel poorly graded 0.50 - 4.00 m	100 120 140 160 180 200 220 240 260 280 300 320 340 360 380 400		1.05 1.35 1.50	1050 1350 1500	105 135 150	290 420 500
	420					
	440					
	460					
	480					
	500					
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	580					
	600					
	620					
	640					
	660					
	680					
	700					
	720					
	740					
	760					
	780					
	800					
	820					

\* ULS : Ultimate Limit State

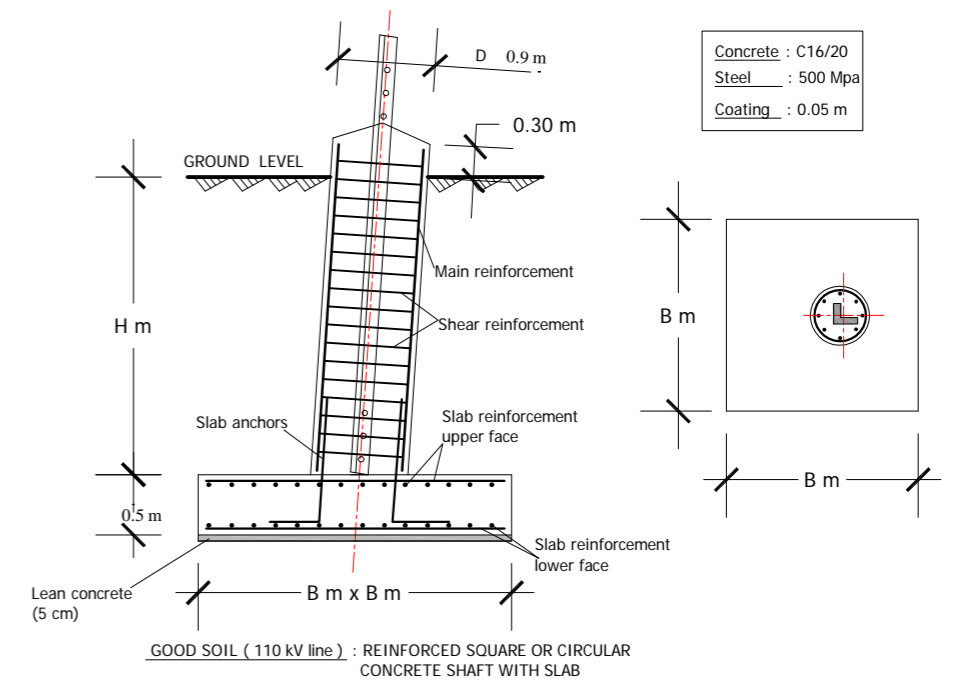
EXCAVATION ( test pit )



EXCAVATED SOIL




FOUNDATION TYPE




THE FOUNDATION DIMENSIONS COMPLY WITH THE STATIC BALANCE OF TOWER FIXING .

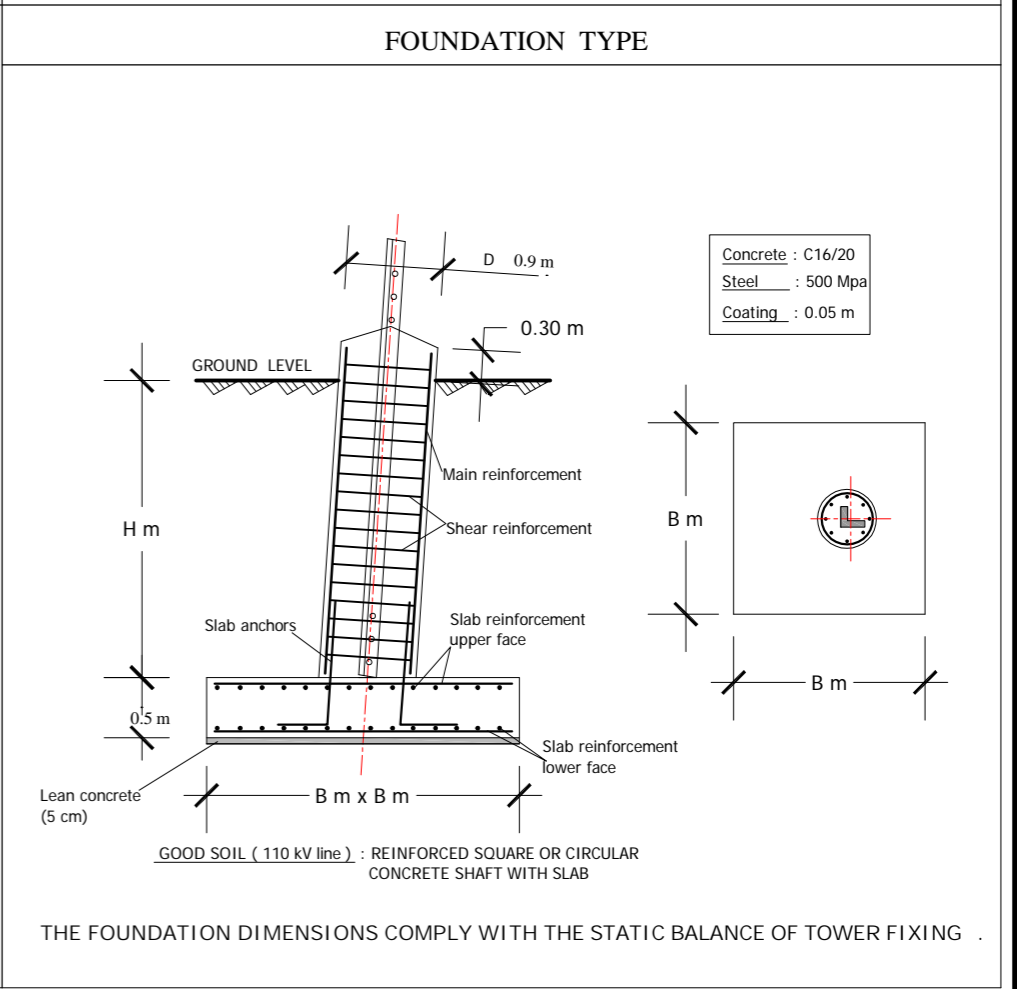
Soil investigation point	Extraction depth of the sample	Classification according USCS	Observation	ENGINEERING PROPERTIES	EXCAVATION CONDITION	GEOLOGICAL UNIT																																																																																																																																																																																																																																																																											
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SIP 187 (110 kV line Jabana - Birembo)	3.60 - 4.00 m	Clay of intermediate plasticity ( CI )	Clayey soil	- IP : 19.6 - Consistency $I_c = 1.28 > 1$ : very consistent ground  - Compaction Backfill : $\rho_{dOPM} = 1.87 \text{ t/m}^3$ $W_{OPM} = 16.54 \%$  - Natural soil : $\rho_h = 1.72 \text{ t/m}^3$ $W_{(N)} = 19.0 \%$ $\rho_s = 2.85 \text{ t/m}^3$	Picks and shovels can be used.	<table border="1"> <thead> <tr> <th rowspan="3">DESCRIPTION OF THE PIT</th> <th rowspan="3">Depth (below existing ground level ) cm</th> <th rowspan="3">Soil Stratigraphy (observed in the well )</th> <th colspan="4">GEOTECHNICAL BEARING CAPACITIES</th> </tr> <tr> <th colspan="2">Allowable</th> <th rowspan="2">ULS * (Mpa)</th> <th rowspan="2">Ultimate (Mpa)</th> </tr> <tr> <th>(Mpa)</th> <th>(Kpa)</th> </tr> </thead> <tbody> <tr><td>Vegetable soil</td><td>0</td><td></td><td>0</td><td>0</td><td>0</td><td>0</td></tr> <tr><td rowspan="14">Clay of intermediate plasticity</td><td>20</td><td></td><td>0.15</td><td>150</td><td>15</td><td>20</td></tr> <tr><td>40</td><td></td><td>0.25</td><td>250</td><td>25</td><td>45</td></tr> <tr><td>60</td><td></td><td>0.65</td><td>650</td><td>65</td><td>110</td></tr> <tr><td>80</td><td></td><td>1.00</td><td>1000</td><td>100</td><td>270</td></tr> <tr><td>100</td><td></td><td>1.05</td><td>1050</td><td>105</td><td>380</td></tr> <tr><td>120</td><td></td><td>1.30</td><td>1300</td><td>130</td><td>410</td></tr> <tr><td>140</td><td></td><td>1.50</td><td>1500</td><td>150</td><td>500</td></tr> <tr><td>160</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>180</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>200</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>220</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>240</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>260</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>280</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>300</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>320</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>340</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>360</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>380</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>400</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>420</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>440</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>460</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>480</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>500</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>520</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>540</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>560</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>580</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>600</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>620</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>640</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>660</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>680</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>700</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>720</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>740</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>760</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>780</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>800</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>820</td><td></td><td></td><td></td><td></td><td></td></tr> </tbody> </table>	DESCRIPTION OF THE PIT	Depth (below existing ground level ) cm	Soil Stratigraphy (observed in the well )	GEOTECHNICAL BEARING CAPACITIES				Allowable		ULS * (Mpa)	Ultimate (Mpa)	(Mpa)	(Kpa)	Vegetable soil	0		0	0	0	0	Clay of intermediate plasticity	20		0.15	150	15	20	40		0.25	250	25	45	60		0.65	650	65	110	80		1.00	1000	100	270	100		1.05	1050	105	380	120		1.30	1300	130	410	140		1.50	1500	150	500	160						180						200						220						240						260						280						300						320						340						360						380						400						420						440						460						480						500						520						540						560						580						600						620						640						660						680						700						720						740						760						780						800						820					
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EXCAVATION ( test pit )



EXCAVATED SOIL







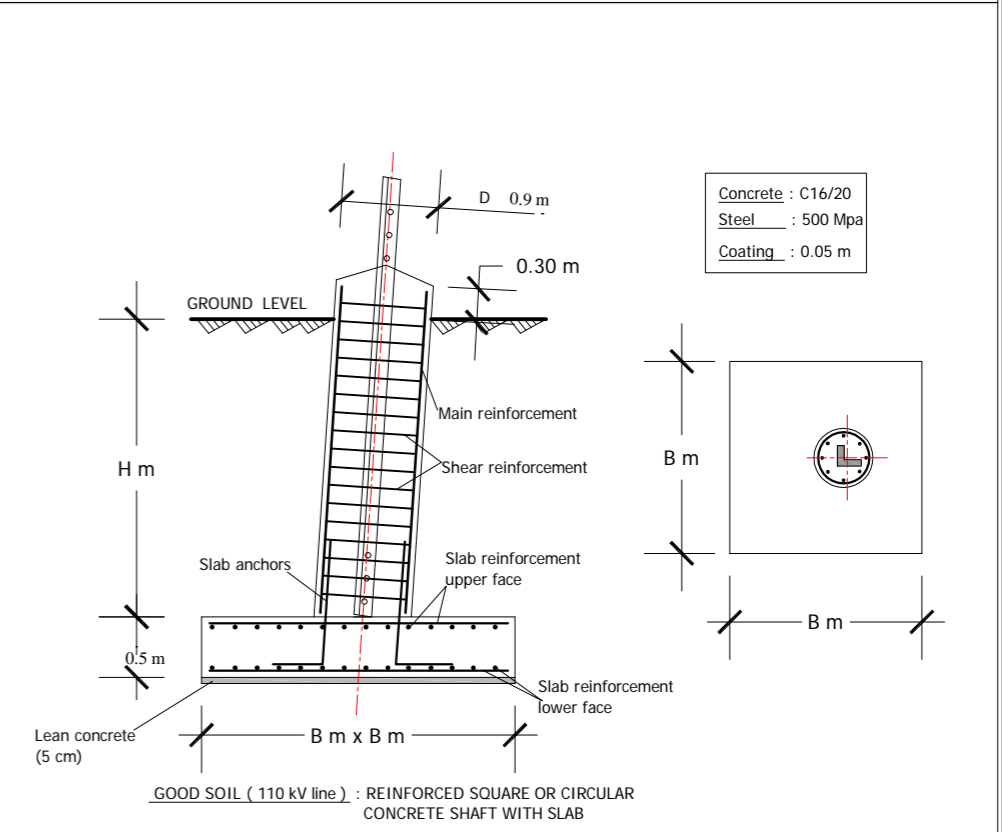
Soil investigation point	Extraction depth of the sample	Classification according USCS	Observation	ENGINEERING PROPERTIES	EXCAVATION CONDITION
SIP 189 (110 kV line Jabana - Birembo)	3.50 - 4.00 m	Clayey sand poorly graded (SC)	Sandy soil & Swampy area	<p>- IP : 17.2</p> <p>- Consistency <math>I_c = 1.28 &gt; 1</math> : very consistent ground</p> <p>- Compaction Backfill :</p> <p>. <math>d_{OPM} = 1.97 \text{ t/m}^3</math></p> <p>. <math>W_{OPM} = 14.40 \%</math></p> <p>- Natural soil :</p> <p>. <math>\rho_h = 1.80 \text{ t/m}^3</math></p> <p>. <math>W_{(N)} = 16.0 \%</math></p> <p>. <math>\rho_s = 2.85 \text{ t/m}^3</math></p> <p>- Shear test :</p> <p>. <math>C_{uu} = 37 \text{ kN/m}^2</math></p> <p>. <math>\psi_{uu} = 22.29^\circ</math></p> <p>- Chemical analysis :</p> <p>. pH = 7.24</p> <p>. <math>SO_4^2 = 65.9 \text{ mg/L}</math></p> <p>. <math>Cl^- = 70.9 \text{ mg/L}</math></p>	Picks and shovels can be used.

**GEOLOGICAL UNIT**  
Light penetrometer ( GeoMIL 50 kN SPT ), test without soil coring for sample taking

DESCRIPTION OF THE PIT	Depth (below existing ground level) (m)	Soil Stratigraphy (observed in the well)	GEOTECHNICAL BEARING CAPACITIES			
			Allowable		ULS *	Ultimate
			(Mpa)	(Kpa)		
Vegetable soil 0.00 - 0.30 m	0		0	0	0	0
Clayey sand poorly graded 0.30 - 4.00 m	20		0.22	220	42	63
	40		0.23	230	47	73
	60		0.21	210	42	68
	80		0.18	180	37	63
	100		0.13	130	27	46
	120		0.18	180	37	48
	140		0.16	160	32	53
	160		0.13	130	27	46
	180		0.16	160	32	53
	200		0.13	130	27	48
	220		0.36	360	72	93
	240		0.28	280	57	100
	260		0.36	360	72	130
	280		0.38	380	77	95
	300		0.33	330	67	130
320		0.38	380	77	95	
340		0.26	260	52	100	
360		0.23	230	47	130	
380		0.41	410	82	150	
400		0.65	650	130	250	
420		0.75	750	150	500	
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\* ULS : Ultimate Limit State

**FOUNDATION TYPE**



THE FOUNDATION DIMENSIONS COMPLY WITH THE STATIC BALANCE OF TOWER FIXING .

EXCAVATION ( test pit )



EXCAVATED SOIL



Soil investigation point	Extraction depth of the sample	Classification according USCS	Observation	ENGINEERING PROPERTIES	EXCAVATION CONDITION	GEOLOGICAL UNIT																																																																																																																																																																																																																																																																																																								
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SIP 190 (110 kV line Jabana - Birembo)	3.00 - 3.50 m	Clayey sand poorly graded ( SC )	Sandy soil & Swampy area	- IP : 18.2 - Consistency $I_c = 1.30 > 1$ : very consistent ground  - Compaction Backfill : $d_{OPM} = 1.88 \text{ t/m}^3$ $W_{OPM} = 17.01 \%$  - Natural soil : $\rho_h = 2.01 \text{ t/m}^3$ $W_{(N)} = 16.0 \%$ $\rho_s = 2.88 \text{ t/m}^3$	- Shear test : $C_{uu} = 22 \text{ kN/m}^2$ $\psi_{uu} = 25.64^\circ$  - Chemical analysis : $pH = 4.81$ $SO_4^2 = 62.6 \text{ mg/L}$ $Cl^- = 99.3 \text{ mg/L}$	Picks and shovels can be used.	<table border="1"> <thead> <tr> <th rowspan="3">DESCRIPTION OF THE PIT</th> <th rowspan="3">Depth (below existing ground level) cm</th> <th rowspan="3">Soil Stratigraphy (observed in the well)</th> <th colspan="3">GEOTECHNICAL BEARING CAPACITIES</th> </tr> <tr> <th colspan="2">Allowable</th> <th rowspan="2">ULS *</th> <th rowspan="2">Ultimate</th> </tr> <tr> <th>(Mpa)</th> <th>(Kpa)</th> <th>(Mpa)</th> <th>(Mpa)</th> </tr> </thead> <tbody> <tr> <td>Vegetable soil</td> <td>0</td> <td></td> <td>0</td> <td>0</td> <td>0</td> <td>0</td> </tr> <tr> <td rowspan="12">Clayey sand poorly graded</td> <td>20</td> <td></td> <td>0.23</td> <td>230</td> <td>46</td> <td>62</td> </tr> <tr> <td>40</td> <td></td> <td>0.19</td> <td>190</td> <td>39</td> <td>52</td> </tr> <tr> <td>60</td> <td></td> <td>0.18</td> <td>180</td> <td>36</td> <td>47</td> </tr> <tr> <td>80</td> <td></td> <td>0.13</td> <td>130</td> <td>26</td> <td>45</td> </tr> <tr> <td>100</td> <td></td> <td>0.10</td> <td>100</td> <td>21</td> <td>42</td> </tr> <tr> <td>120</td> <td></td> <td>0.21</td> <td>210</td> <td>43</td> <td>62</td> </tr> <tr> <td>140</td> <td></td> <td>0.15</td> <td>150</td> <td>31</td> <td>55</td> </tr> <tr> <td>160</td> <td></td> <td>0.30</td> <td>300</td> <td>61</td> <td>82</td> </tr> <tr> <td>180</td> <td></td> <td>0.30</td> <td>300</td> <td>71</td> <td>97</td> </tr> <tr> <td>200</td> <td></td> <td>0.35</td> <td>350</td> <td>71</td> <td>102</td> </tr> <tr> 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graded	20		0.23	230	46	62	40		0.19	190	39	52	60		0.18	180	36	47	80		0.13	130	26	45	100		0.10	100	21	42	120		0.21	210	43	62	140		0.15	150	31	55	160		0.30	300	61	82	180		0.30	300	71	97	200		0.35	350	71	102	220		0.40	400	81	110	240		0.45	450	91	125	260		0.50	500	101	180	280		0.35	350	71	200	300		0.45	450	91	220	320		0.45	450	91	235	340		0.47	470	95	260	360		0.52	520	105	380	380		0.75	750	150	500	400							420							440							460							480							500							520							540							560							580							600							620							640							660							680							700							720							740							760							780							800							820						
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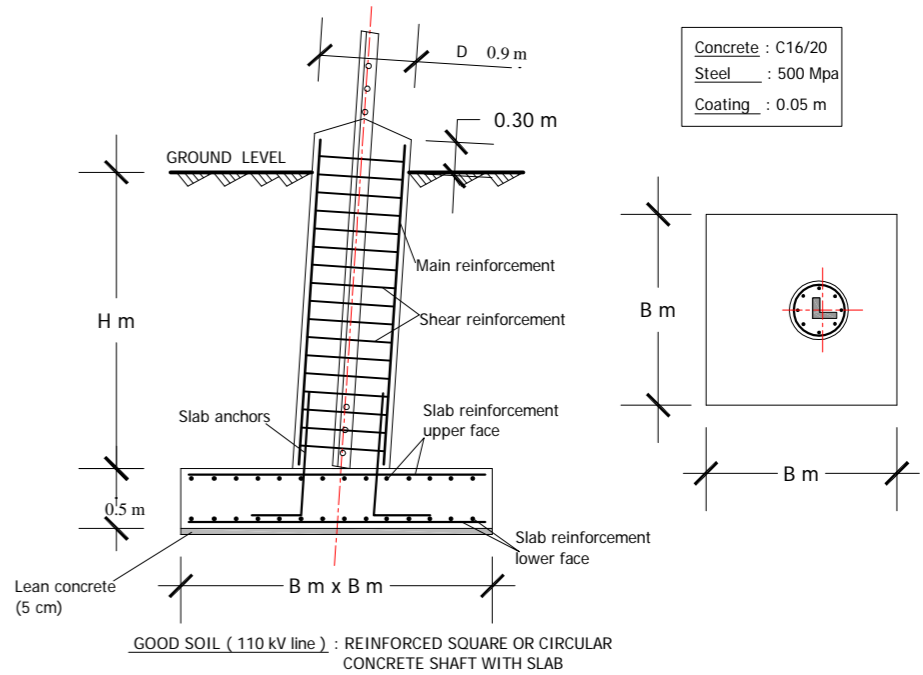
EXCAVATION ( test pit )



EXCAVATED SOIL



FOUNDATION TYPE




THE FOUNDATION DIMENSIONS COMPLY WITH THE STATIC BALANCE OF TOWER FIXING .


Soil investigation point	Extraction depth of the sample	Classification according USCS	Observation	ENGINEERING PROPERTIES		EXCAVATION CONDITION	GEOLOGICAL UNIT Light penetrometer ( GeoMIL 50 kN SPT ), test without soil coring for sample taking					
							DESCRIPTION OF THE PIT	Depth (below existing ground level) cm	Soil Stratigraphy (observed in the well)	GEO TECHNICAL BEARING CAPACITIES		
						Allowable			ULS *	Ultimate		
						( Mpa )	( Kpa )	( Mpa )	( Mpa )	( Mpa )		
<b>SIP 195</b> (110 kV line Jabana - Birembo)	3.60 - 4.00 m	Clayey sand poorly graded ( SC )	Sandy soil & Swampy area	- IP : 16.5 - Consistency $I_c = 1.44 > 1$ : very consistent ground  - Compaction Backfill : . $d_{OPM} = 1.91 \text{ t/m}^3$ . $W_{OPM} = 16.40 \%$  - Natural soil : . $\rho_h = 1.97 \text{ t/m}^3$ . $W_{(N)} = 16.6 \%$ . $\rho_s = 2.88 \text{ t/m}^3$	- Shear test : . $C_{uu} = 18 \text{ kN/m}^2$ . $\psi_{uu} = 27.70^\circ$  - Chemical analysis : . pH = 4.81 . $SO_4^2 = 62.6 \text{ mg/L}$ . $Cl^- = 99.3 \text{ mg/L}$	Picks and shovels can be used.	Vegetable soil 0.00 - 0.30 m	0		0	0	0
								20	0.25	250	25	35
								40	0.20	200	20	45
								60	0.15	150	15	30
								80	0.90	900	90	125
								100	1.00	1000	100	160
								120	1.20	1200	120	420
								140	1.50	1500	150	500
								160				
								180				
								200				
								220				
								240				
								260				
								280				
								300				
								320				
								340				
								360				
								380				
								400				
								420				
								440				
								460				
								480				
	500											
	520											
	540											
	560											
	580											
	600											
	620											
	640											
	660											
	680											
	700											
	720											
	740											
	760											
	780											
	800											
	820											

\* ULS : Ultimate Limit State

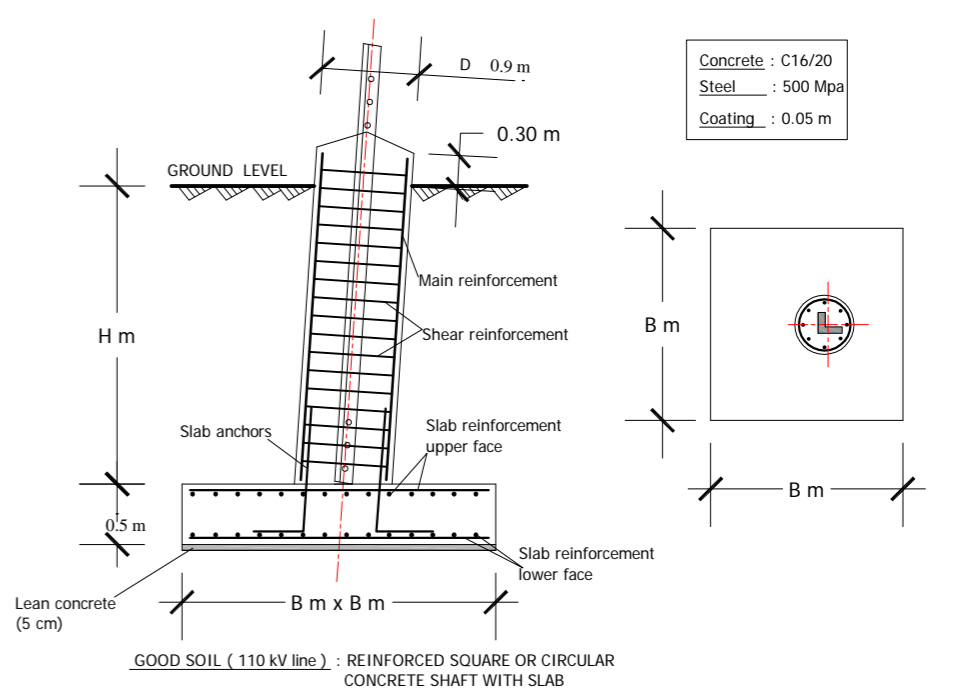
EXCAVATION ( test pit )



EXCAVATED SOIL



**FOUNDATION TYPE**




Concrete : C16/20  
Steel : 500 Mpa  
Coating : 0.05 m

GOOD SOIL ( 110 kV line ) : REINFORCED SQUARE OR CIRCULAR CONCRETE SHAFT WITH SLAB


THE FOUNDATION DIMENSIONS COMPLY WITH THE STATIC BALANCE OF TOWER FIXING .

Soil investigation point	Extraction depth of the sample	Classification according USCS	Observation	ENGINEERING PROPERTIES		EXCAVATION CONDITION	GEOLOGICAL UNIT Light penetrometer ( GeoMIL 50 kN SPT ), test without soil coring for sample taking					
				- IP : 20.26 - Consistency $I_c = 1.13 > 1$ : very consistent ground - Compaction Backfill : $\rho_{OPM} = 1.89 \text{ t/m}^3$ $W_{OPM} = 16.21 \%$ - Natural soil : $\rho_h = 1.89 \text{ t/m}^3$ $W_{(N)} = 18.8 \%$ $\rho_s = 2.81 \text{ t/m}^3$	- Shear test : $c_{uu} = 30 \text{ kN/m}^2$ $\psi_{uu} = 25.41^\circ$ - Chemical analysis : $pH = 6.11$ $SO_4^2 = 24 \text{ mg/L}$ $Cl^- = 202.4 \text{ mg/L}$		DESCRIPTION OF THE PIT	Depth (below existing ground level) (cm)	Soil Strength (shear stress in the soil)	GEO TECHNICAL BEARING CAPACITIES		
Allowable (Mpa)	(Kpa)	ULS * (Mpa)	Ultimate (Mpa)									
SIP 196 (110 kV line Jabana - Birembo)	3.70 - 4.00 m	Clay of intermediate plasticity (CI)	Sandy soil			Picks and shovels can be used.	Vegetable soil 0.00 - 0.50 m	0	0	0	0	0
							20	0.30	300	30	40	
							40	0.40	400	40	65	
							60	0.25	250	25	45	
							80	0.75	750	75	120	
							100	0.95	950	95	200	
							120	1.00	1000	100	300	
							140	1.10	1100	110	380	
							160	1.50	1500	150	500	
							180					
							200					
							220					
							240					
							260					
							280					
							300					
							320					
							340					
							360					
							380					
							400					
							420					
440												
460												
480												
500												
520												
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560												
580												
600												
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700												
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740												
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780												
800												
820												

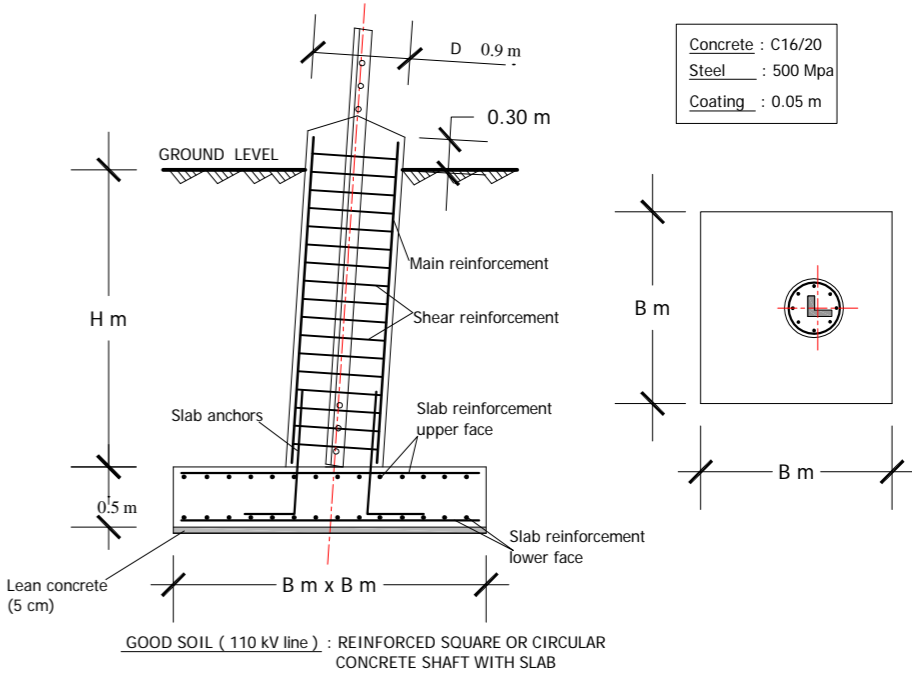
EXCAVATION ( test pit )



EXCAVATED SOIL



**FOUNDATION TYPE**



Concrete : C16/20  
Steel : 500 Mpa  
Coating : 0.05 m

GOOD SOIL ( 110 kV line ) : REINFORCED SQUARE OR CIRCULAR CONCRETE SHAFT WITH SLAB

THE FOUNDATION DIMENSIONS COMPLY WITH THE STATIC BALANCE OF TOWER FIXING .

## V. GENERAL CONCLUSION

### 1 ) Soil as support structure.

The presence of clay being general in all the soil structures we have studied ; caution is importante in defining the bearing capacity of each lot.

Instead of the safety coefficient of 0.33 used by the laboratory ; we propose a coefficient of 0.01 which takes account of this harmful presence, in a tropical environment.

Indeed, if these studies were carried out during the dry season, the works will be carried out in all seasons and any earthwork will be confronted to a presence of water large enough in the rainy season.

It is strictly prudent to protect the works from any field sensitivity, as the index of plasticity varies from 14.8 % to 21.9 % , although the cohesion is acceptable ( the internal friction angle sometimes exceeding 30° ).

Apart a few cases requiring special treatment (as unconfined compressive strenght ), from a depth of 100 cm, a bearing capacity of 200 kN / m<sup>2</sup> can be , safety, envisaged for the entire project ; with the exception of flood areas where the bearing capacity is lower as indicated above.

This would be in the interest of the project whose foundations, for posts and for towers, are laid at more than 2.00 m depth.

( Nota / penetration test sheet :

Rt : total resistance, Rp : resistance to the tip and Qadm : bearing capacity ).

### 2 ) Soil as building material.

Set up by backfill, soils must protect the works of all lifting and overturn.

The characteristics data from these studies attribute those qualities to examined soils ; indeed :

- The dry density  $d_{OPM} = 1.67 \text{ t/m}^3$
- The internal friction angle is greater than 20 ° for the majority of points ( the shear strength is proportional to soil consolidation).
- The unconfined compressive strength gives good resistances ( 1.48 Mpa ) to the compression pressure and good density ( 1.71 t / m<sup>3</sup> ).
- No soil presents no aggression against cement ( 4.62 pH 7.3 ).

3) All the results of the tests imposed on soils allows to classify them into three categories :

#### a. Good soil, including :

- Clay of Intermediate plasticity ,
- Silty Sand ,
- Clayey Sand .

#### b. Soft rock, including :

- weathered rock,
- silty gravel,
- clayey gravel,
- semi rock soil.

#### c. Poor soils with water (or not) :

These soils are located in swampies areas or in any other areas which can be flooded .

### 4 ) Remarque

Any anomalies observed in the performance of the results given above must be reported to an expert for advices and interpretation.



REPUBLIC OF RWANDA  
MINISTRY OF INFRASTRUCTURE

Execution Agency :



Japan International Cooperation Agency

Buyer :



Consultant :



Project :

IMPROVEMENT OF SUBSTATIONS AND DISTRIBUTION NETWORK, PHASE 3, IN THE REPUBLIC OF RWANDA

**SOIL INVESTIGATIONS**

FILE 2 / 2



KIGALI, August 2017

IMPROVEMENT OF SUBSTATIONS AND DISTRIBUTION NETWORK, PHASE 3, JICA PROJECT.

**SOIL INVESTIGATION** ( File 2 / 2 : Field & laboratory tests )

**CONTENT**

I. LIGHT STATIC PENETRATION TESTS ( on 46 points )

II. CHIMICAL ANALYSIS (for 50 points)

III. PHISICAL & MECHANICAL TESTS (for 50 points)

( with unconfined compressive strength : 36 points )

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IMPROVEMENT OF SUBSTATIONS AND DISTRIBUTION NETWORK, PHASE 3, JICA PROJECT.

SOIL INVESTIGATION ( File 2 / 2 : Field & laboratory tests )

I. LIGHT STATIC PENETRATION TESTS ( on 46 points )

- New substation construction site & branch point line 110 kV at Gasogi ( 3 points ) : Sip1 , Sip3, Sip5
- 15 kV Distribution line Gasogi substation - Nyagasambu (15 points ) : Sip1 , Sip2, Sip3, Sip4, Sip5, Sip6, Sip Sip 9, Sip10, Sip11, Sip12, Sip13, Sip15, Sip16 & Sip17 .
- 15 kV Distribution line Gasogi substation - Masaka hospital (15 points ) : Sip1 , Sip3, Sip4, Sip5, Sip6, Sip8, Sip 9, Sip10, Sip12, Sip13, Sip15, Sip16, Sip17, Sip18 & Sip19.
- Reconstruction of transmission 110 kV line Jabana - Birembo ( 13 points ) : Sip176 , Sip177, Sip178, Sip180, Sip181, Sip182, Sip184, Sip186, Sip187, Sip189, Sip190, Sip195, Sip196 .



**PROJECT: IMPROVEMENT OF SUBSTATION & DISTRIBUTION NETWORK - PHASE III**

**SUB PROJECT: New Substation Construction Site**

**Client:** PITRAD IBAMBA LTD.

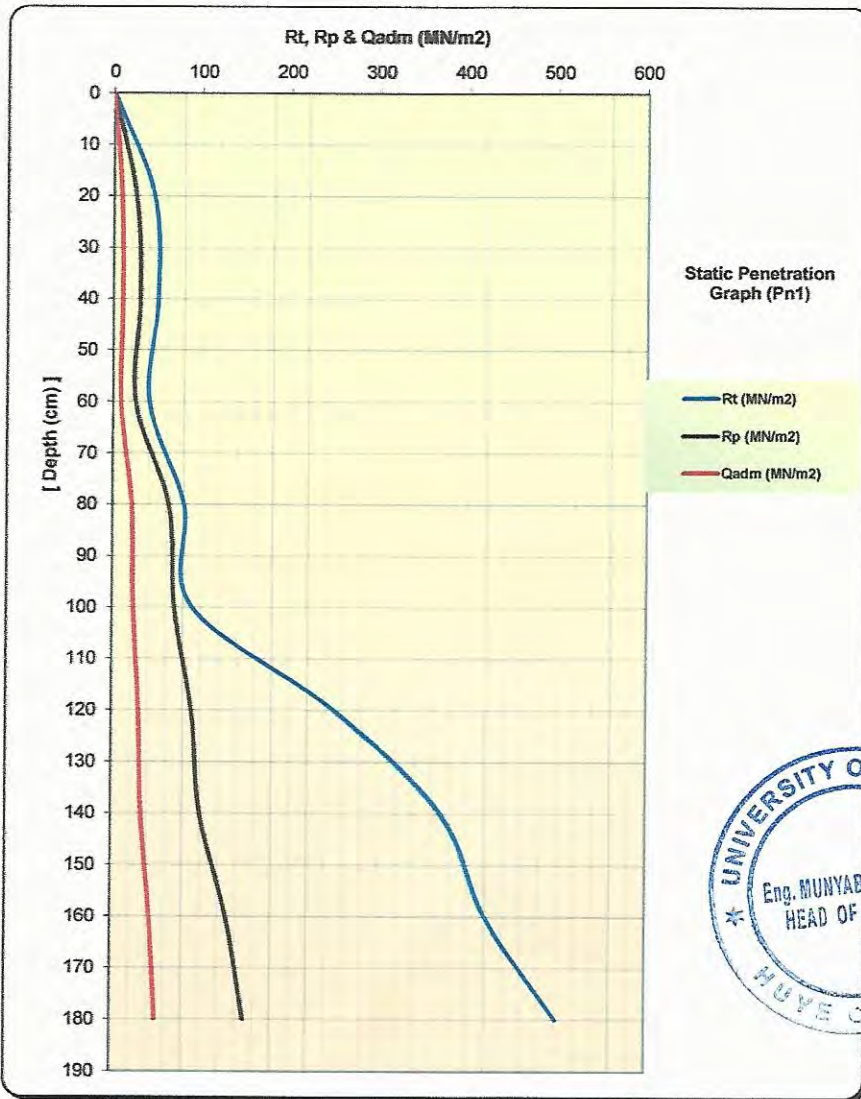
Operated by: Eng. Venerand GWIZA

Verified by: Eng. Vivien MUNYABURANGA (MSc)

Date: July, 2017

Penetration Test **N° S 1**

Depth (cm)	Rt (MN/m <sup>2</sup> )	Rp (MN/m <sup>2</sup> )	Qadm (MN/m <sup>2</sup> )	Qadm (kg/cm <sup>2</sup> )
0	0	0	0.00	0.00
20	45	25	8.33	84.98
40	50	30	10.00	101.97
60	40	25	8.33	84.98
80	80	63	21.00	214.14
100	90	70	23.33	237.93
120	250	90	30.00	305.92
140	370	100	33.33	339.91
160	420	130	43.33	441.88
180	500	150	50.00	509.86
200				



**PROJECT: IMPROVEMENT OF SUBSTATION & DISTRIBUTION NETWORK - PHASE III**

**SUB PROJECT: New Substation Construction Site**

**Client:** PITRAD IBAMBA LTD.

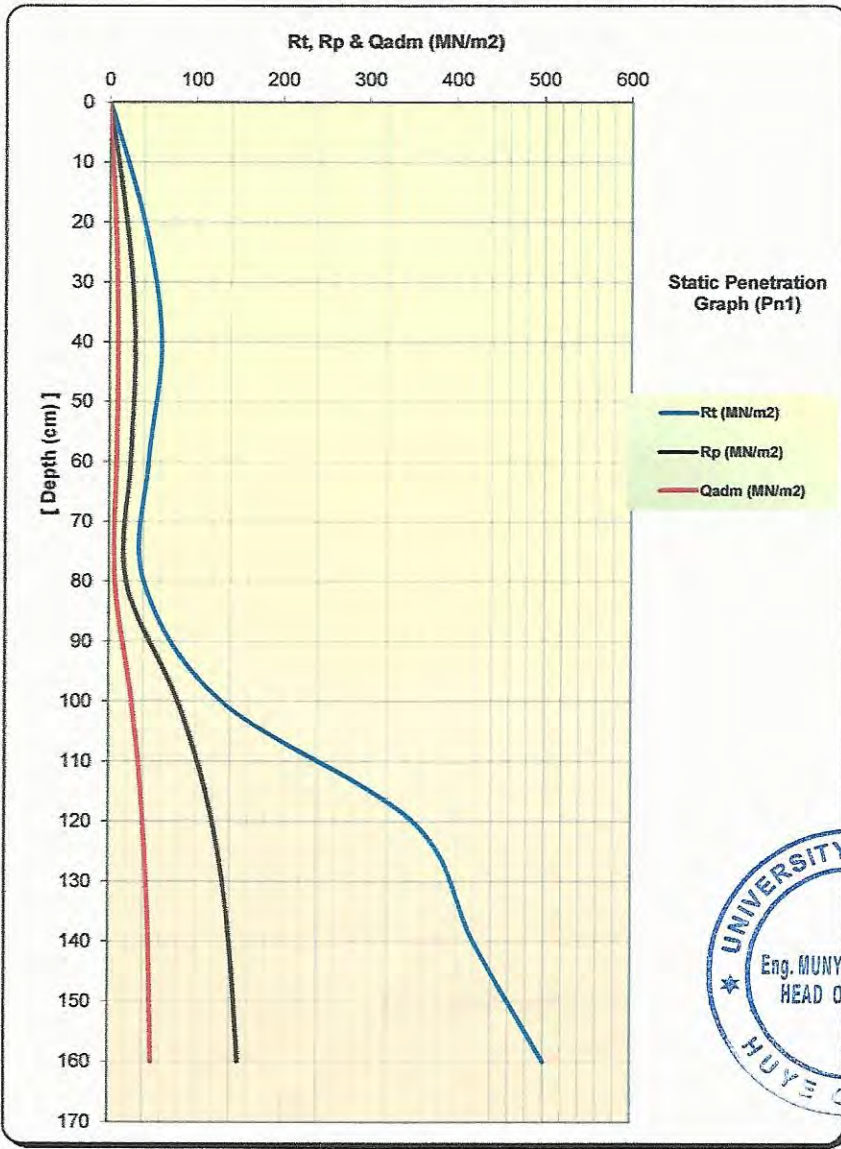
Operated by: Eng. Venerand GWIZA

Verified by: Eng. Vivien MUNYABURANGA (MSc)

Date: July, 2017

**Penetration Test N° S 3**

Depth (cm)	Rt (MN/m <sup>2</sup> )	Rp (MN/m <sup>2</sup> )	Qadm (MN/m <sup>2</sup> )	Qadm (kg/cm <sup>2</sup> )
0	0	0	0.00	0.00
20	40	20	6.67	67.98
40	60	30	10.00	101.97
60	45	25	8.33	84.98
80	40	20	6.67	67.98
100	130	80	26.67	271.93
120	350	120	40.00	407.89
140	420	140	46.67	475.87
160	500	150	50.00	509.86
180				



**PROJECT: IMPROVEMENT OF SUBSTATION & DISTRIBUTION NETWORK - PHASE III**

**SUB PROJECT: New Substation Construction Site**

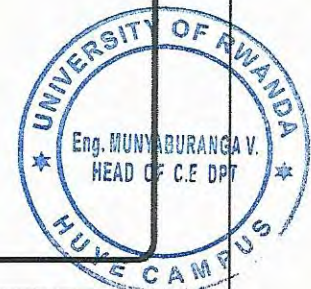
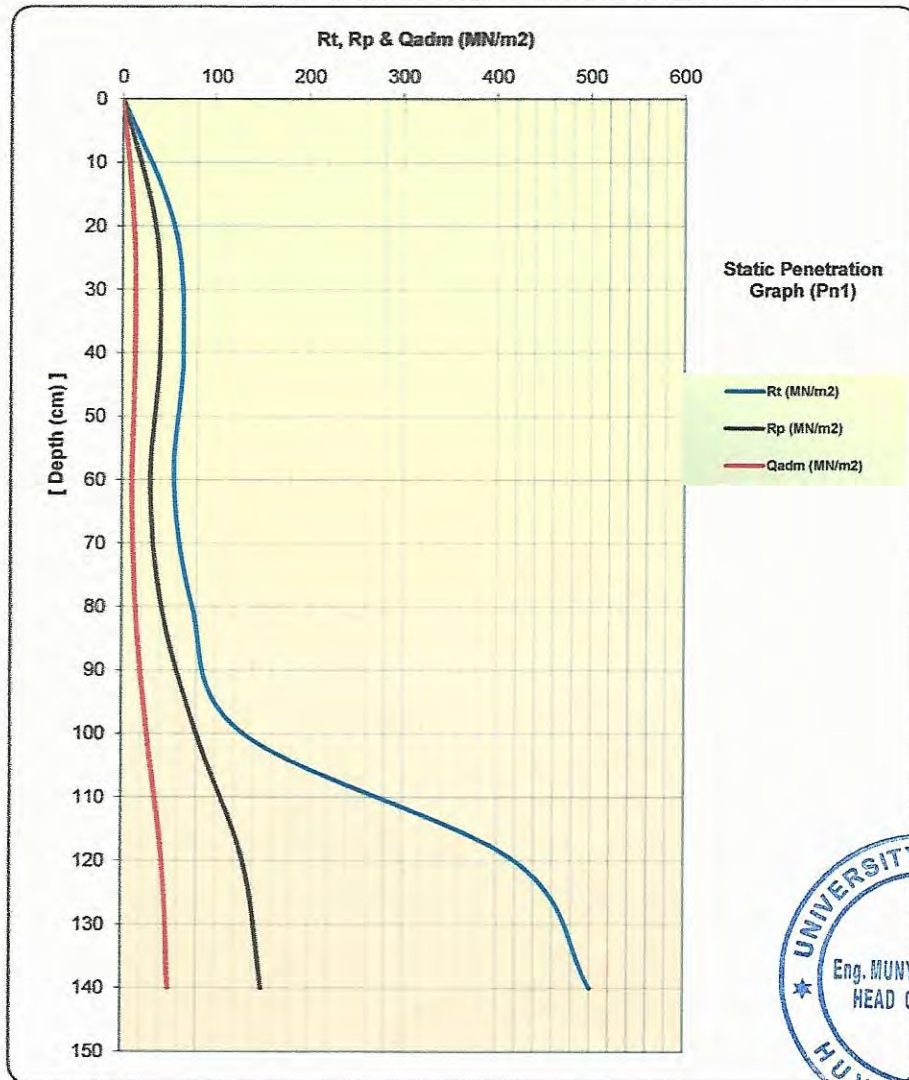
**Client:** PITRAD IBAMBA LTD.

Operated by: Eng. Venerand GWIZA

Verified by: Eng. Vivien MUNYABURANGA (MSc)

Date: July, 2017 ; S<sub>5</sub>

Depth (cm)	Rt (MN/m <sup>2</sup> )	Rp (MN/m <sup>2</sup> )	Qadm (MN/m <sup>2</sup> )	Qadm (kg/cm <sup>2</sup> )
0	0	0	0.00	0.00
20	55	35	11.67	118.97
40	65	40	13.33	135.96
60	55	30	10.00	101.97
80	75	42	14.00	142.76
100	130	80	26.67	271.93
120	420	130	43.33	441.88
140	500	150	50.00	509.86
160				



**PROJECT:** IMPROVEMENT OF SUBSTATION & DISTRIBUTION NETWORK - PHASE III

**SUB PROJECT:** 15 KV Distribution line Gasogi Substation - Nyagasambu

**Client:** PITRAD IBAMBA LTD.

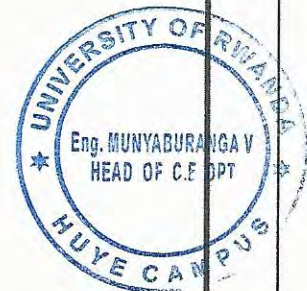
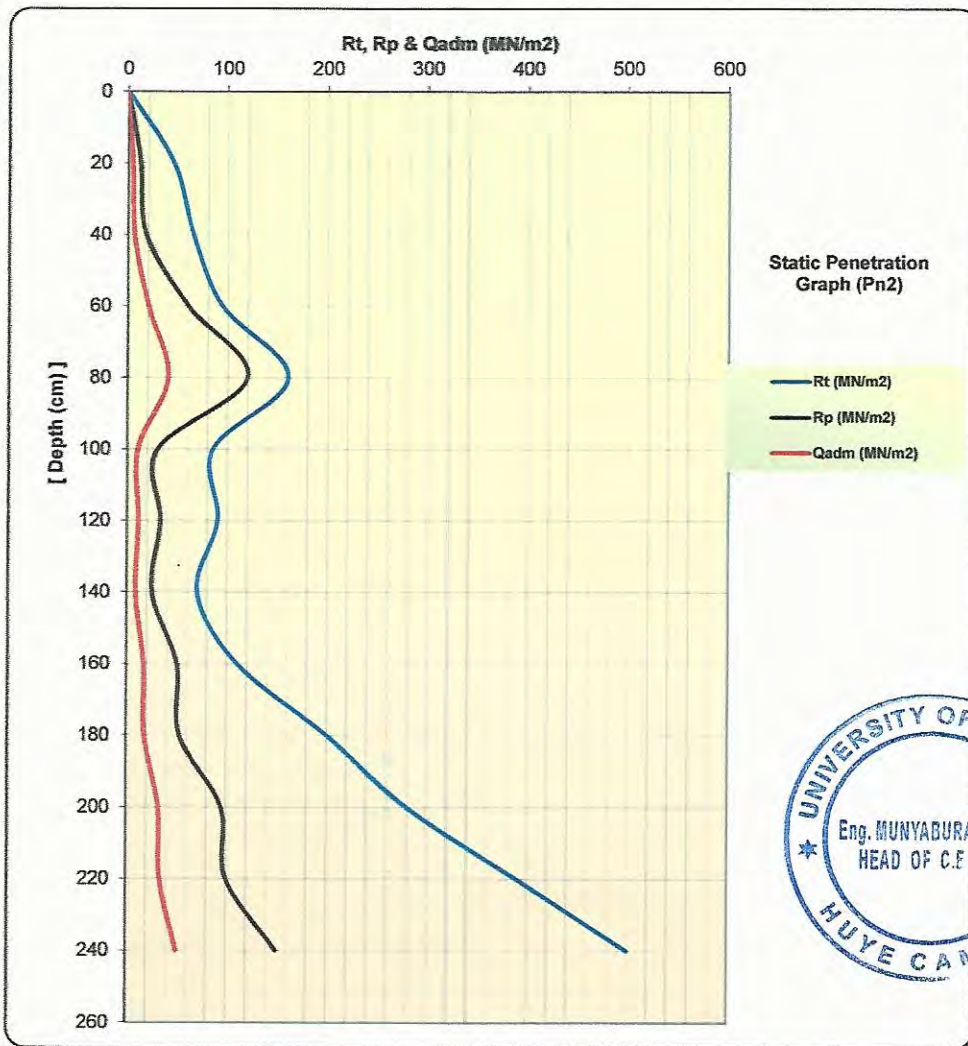
Operated by: Eng. Venerand GWIZA

Verified by: Eng. Vivien MUNYABURANGA (MSc)

Date: July, 2017

Penetration Test **N° AP 1**

Depth (cm)	Rt (MN/m <sup>2</sup> )	Rp (MN/m <sup>2</sup> )	Qadm (MN/m <sup>2</sup> )	Qadm (kg/cm <sup>2</sup> )
0	0	0	0	0.00
20	45	12	4.00	40.79
40	65	18	6.00	61.18
60	94	60	20.00	203.94
80	160	120	40.00	407.89
100	85	30	10.00	101.97
120	90	33	11.00	112.17
140	70	25	8.33	84.98
160	110	50	16.67	169.95
180	200	53	17.67	180.15
200	280	95	31.67	322.91
220	390	100	33.33	339.91
240	500	150	50.00	509.86
260				



**PROJECT:** IMPROVEMENT OF SUBSTATION & DISTRIBUTION NETWORK - PHASE III

**SUB PROJECT:** 15 KV Distribution line Gasogi Substation - Nyagasambu

**Client:** PITRAD IBAMBA LTD.

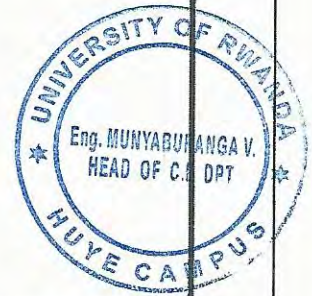
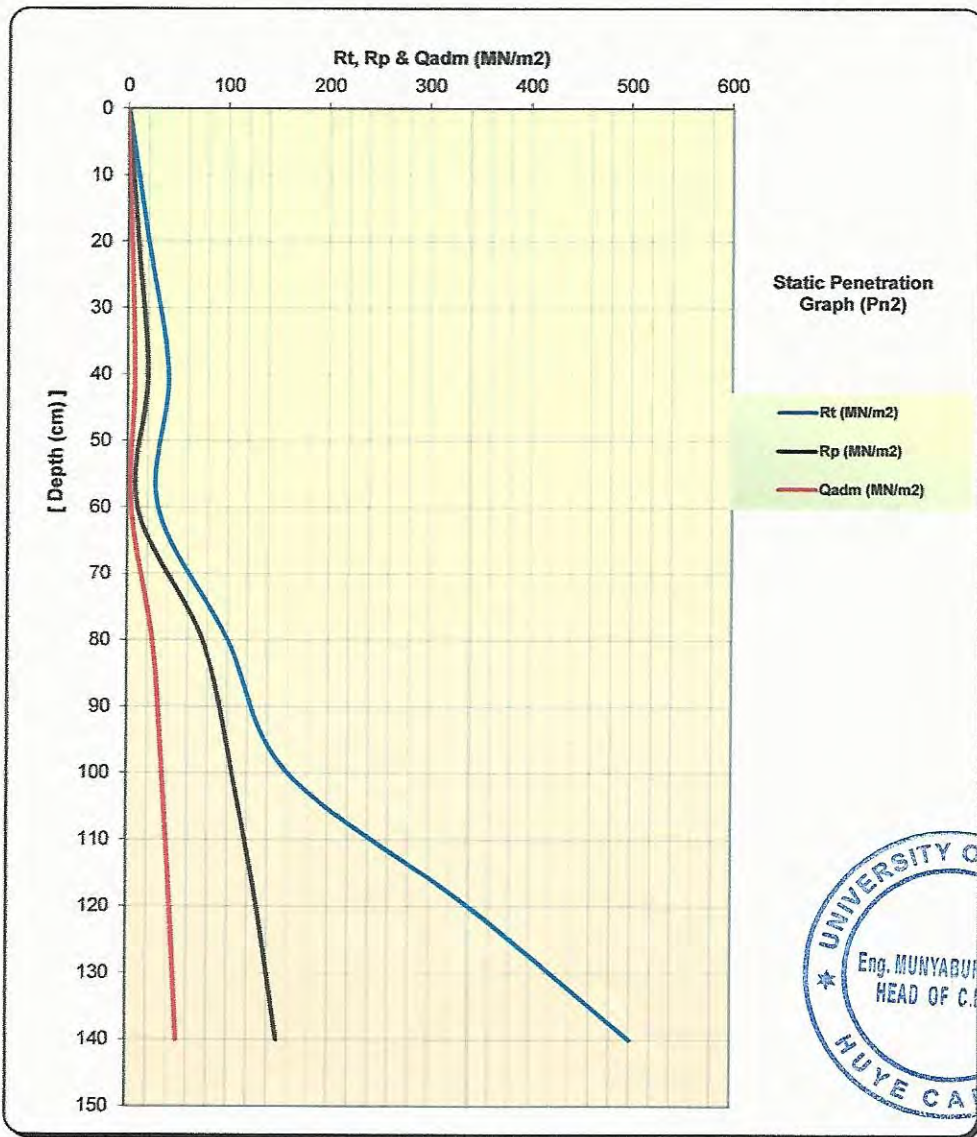
Operated by: Eng. Venerand GWIZA

Verified by: Eng. Vivien MUNYABURANGA (MSc)

Date: July, 2017

Penetration Test **N° AP 2**

Depth (cm)	Rt (MN/m <sup>2</sup> )	Rp (MN/m <sup>2</sup> )	Qadm (MN/m <sup>2</sup> )	Qadm (kg/cm <sup>2</sup> )
0	0	0	0	0.00
20	20	10	3.33	33.99
40	40	20	6.67	67.98
60	30	10	3.33	33.99
80	100	75	25.00	254.93
100	160	105	35.00	356.90
120	340	130	43.33	441.88
140	500	150	50.00	509.86
160				



**PROJECT:** IMPROVEMENT OF SUBSTATION & DISTRIBUTION NETWORK - PHASE III

**SUB PROJECT:** 15 KV Distribution line Gasogi Substation - Nyagasambu

**Client:** PITRAD IBAMBA LTD.

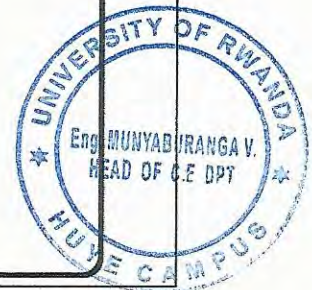
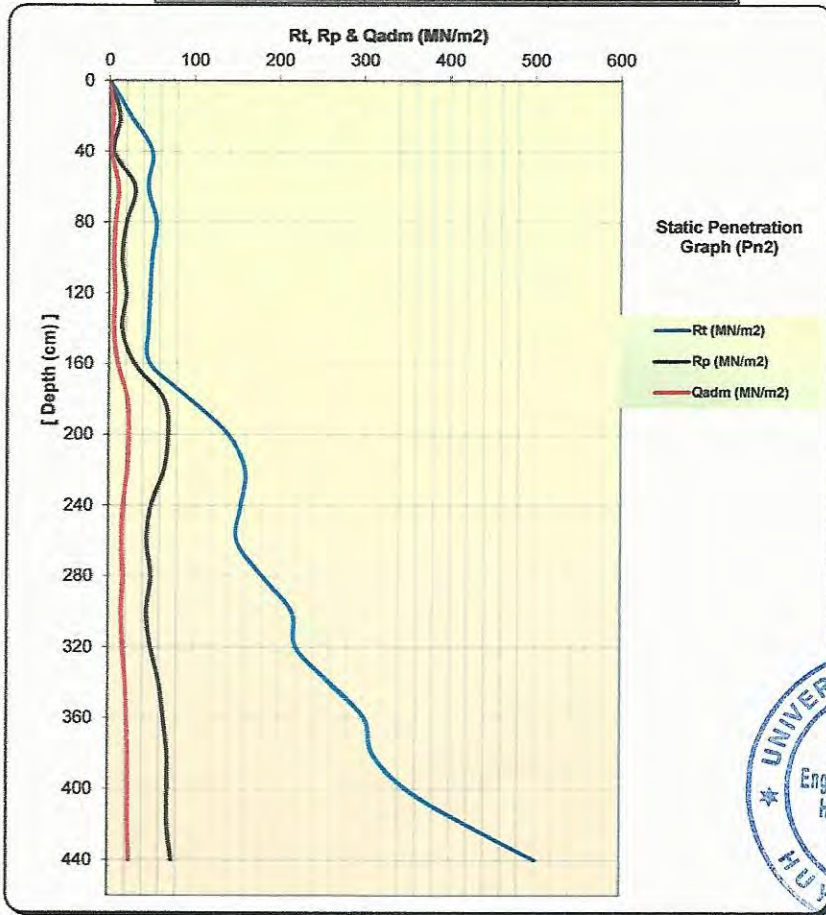
Operated by: Eng. Venerand GWIZA

Verified by: Eng. Vivien MUNYABURANGA (MSc)

Date: July, 2017

Penetration Test **N° AP 3**

Depth (cm)	Rt (MN/m <sup>2</sup> )	Rp (MN/m <sup>2</sup> )	Qadm (MN/m <sup>2</sup> )	Qadm (kg/cm <sup>2</sup> )
0	0	0	0	0.00
20	25	12	4.00	40.79
40	50	5	1.67	17.00
60	45	30	10.00	101.97
80	55	20	6.67	67.98
100	50	15	5.00	50.99
120	48	20	6.67	67.98
140	46	15	5.00	50.99
160	48	30	10.00	101.97
180	95	65	21.67	220.94
200	140	70	23.33	237.93
220	160	65	21.67	220.94
240	155	50	16.67	169.95
260	150	45	15.00	152.96
280	180	50	16.67	169.95
300	215	45	15.00	152.96
320	220	50	16.67	169.95
340	260	60	20.00	203.94
360	300	65	21.67	220.94
380	310	70	23.33	237.93
400	350	70	23.33	237.93
420	420	70	23.33	237.93
440	500	75	25.00	254.93
460				



**PROJECT:** IMPROVEMENT OF SUBSTATION & DISTRIBUTION NETWORK - PHASE III

**SUB PROJECT:** 15 KV Distribution line Gasogi Substation - Nyagasambu

**Client:** PITRAD IBAMBA LTD.

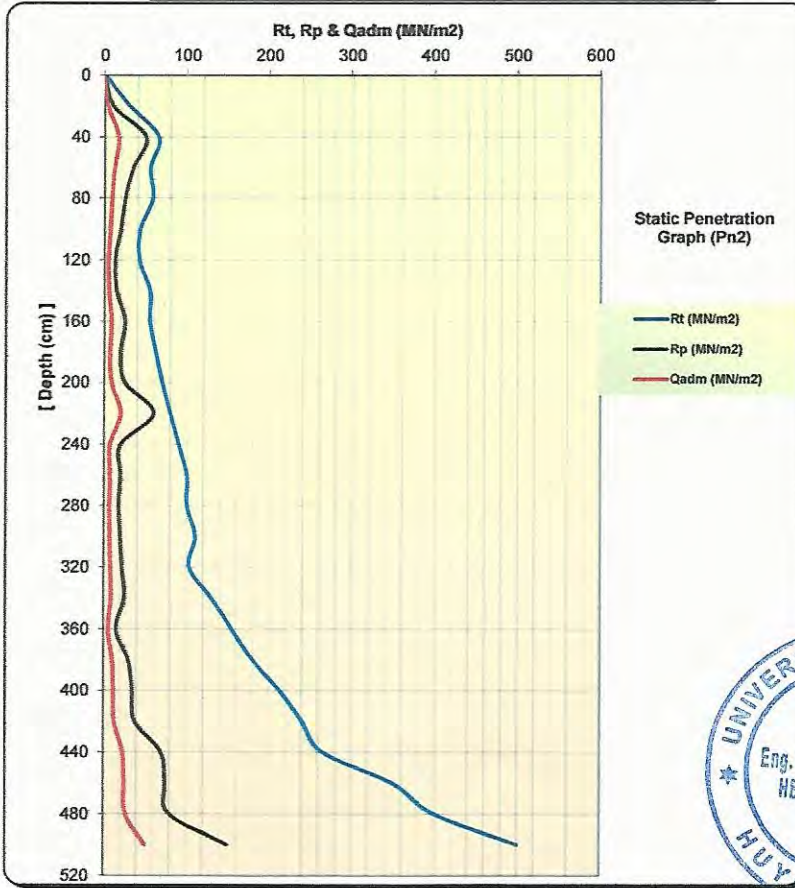
Operated by: Eng. Venerand GWIZA

Verified by: Eng. Vivien MUNYABURANGA (MSc)

Date: July, 2017

Penetration Test **N° AP 4**

Depth (cm)	Rt (MN/m <sup>2</sup> )	Rp (MN/m <sup>2</sup> )	Qadm (MN/m <sup>2</sup> )	Qadm (kg/cm <sup>2</sup> )
0	0	0	0.00	0.00
20	30	10	3.33	33.99
40	65	50	16.67	169.95
60	55	35	11.67	118.97
80	58	25	8.33	84.98
100	43	20	6.67	67.98
120	42	13	4.33	44.19
140	55	15	5.00	50.99
160	55	25	8.33	84.98
180	62	20	6.67	67.98
200	70	25	8.33	84.98
220	80	60	20.00	203.94
240	90	20	6.67	67.98
260	100	20	6.67	67.98
280	100	18	6.00	61.18
300	110	20	6.67	67.98
320	103	22	7.33	74.78
340	130	25	8.33	84.98
360	155	15	5.00	50.99
380	180	30	10.00	101.97
400	213	35	11.67	118.97
420	240	38	12.67	129.16
440	265	70	23.33	237.93
460	350	75	25.00	254.93
480	400	80	26.67	271.93
500	500	150	50.00	509.86
520				



**PROJECT:** IMPROVEMENT OF SUBSTATION & DISTRIBUTION NETWORK - PHASE III

**SUB PROJECT:** 15 KV Distribution line Gasogi Substation - Nyagasambu

**Client:** PITRAD IBAMBA LTD.

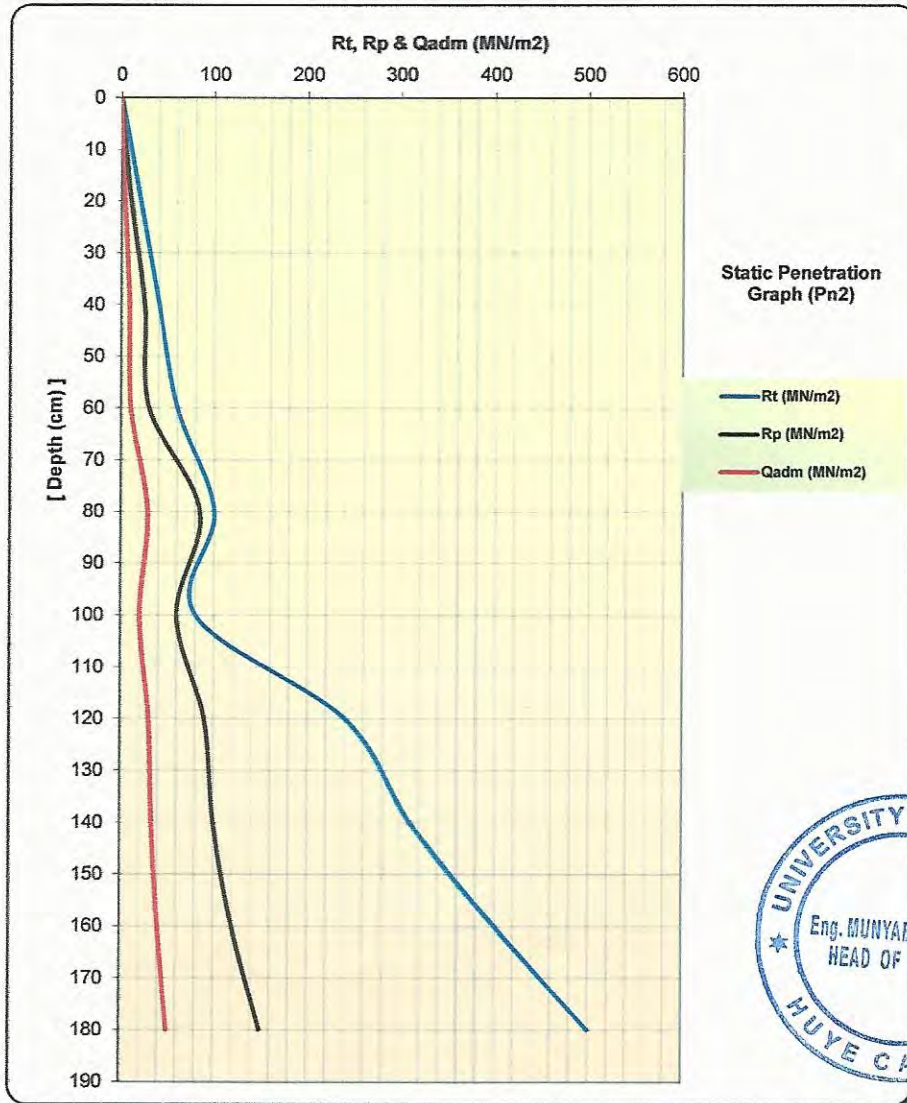
Operated by: Eng. Venerand GWIZA

Verified by: Eng. Vivien MUNYABURANGA (MSc)

Date: July, 2017

Penetration Test N° AP 5

Depth (cm)	Rt (MN/m <sup>2</sup> )	Rp (MN/m <sup>2</sup> )	Qadm (MN/m <sup>2</sup> )	Qadm (kg/cm <sup>2</sup> )
0	0	0	0.00	0.00
20	20	10	3.33	33.99
40	40	25	8.33	84.98
60	60	30	10.00	101.97
80	100	85	28.33	288.92
100	80	60	20.00	203.94
120	240	90	30.00	305.92
140	310	100	33.33	339.91
160	400	120	40.00	407.89
180	500	150	50.00	509.86
200				





**PROJECT:** IMPROVEMENT OF SUBSTATION & DISTRIBUTION NETWORK - PHASE III

**SUB PROJECT:** 15 KV Distribution line Gasogi Substation - Nyagasambu

**Client:** PITRAD IBAMBA LTD.

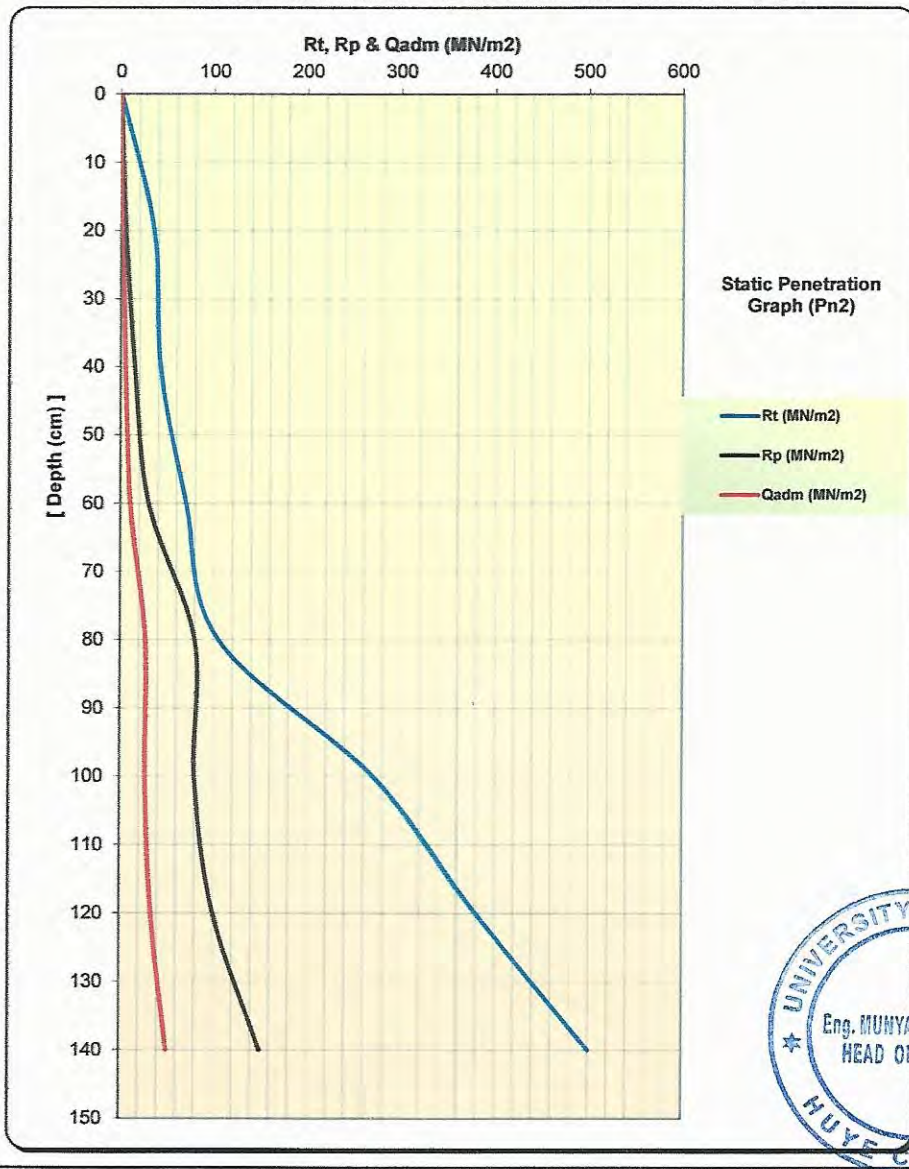
Operated by: Eng. Venerand GWIZA

Verified by: Eng. Vivien MUNYABURANGA (MSc)

Date: July, 2017

Penetration Test **N° AP 6**

Depth (cm)	Rt (MN/m <sup>2</sup> )	Rp (MN/m <sup>2</sup> )	Qadm (MN/m <sup>2</sup> )	Qadm (kg/cm <sup>2</sup> )
0	0	0	0	0.00
20	35	5	1.67	17.00
40	42	15	5.00	50.99
60	70	30	10.00	101.97
80	105	80	26.67	271.93
100	270	80	26.67	271.93
120	380	100	33.33	339.91
140	500	150	50.00	509.86
160				



**PROJECT:** IMPROVEMENT OF SUBSTATION & DISTRIBUTION NETWORK - PHASE III

**SUB PROJECT:** 15 KV Distribution line Gasogi Substation - Nyagasambu

**Client:** PITRAD IBAMBA LTD.

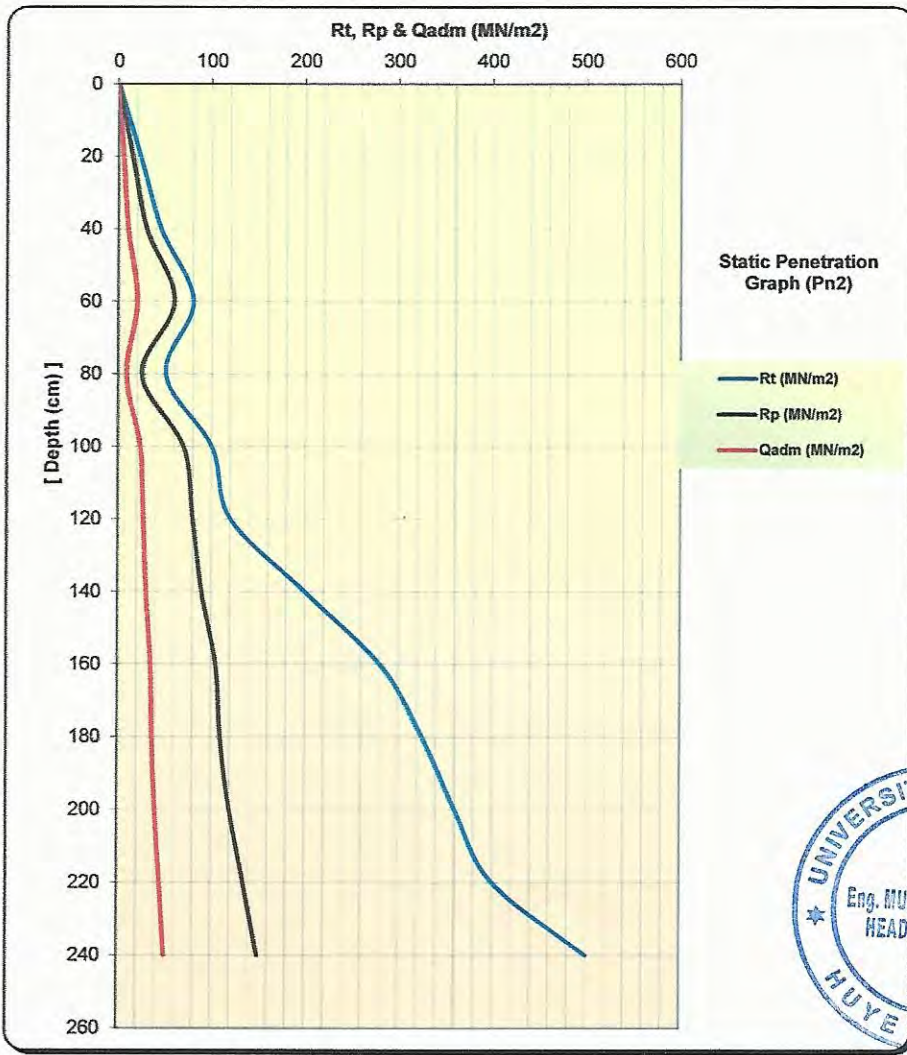
Operated by: Eng. Venerand GWIZA

Verified by: Eng. Vivien MUNYABURANGA (MSc)

Date: July, 2017

Penetration Test N° AP 7

Depth (cm)	Rt (MN/m <sup>2</sup> )	Rp (MN/m <sup>2</sup> )	Qadm (MN/m <sup>2</sup> )	Qadm (kg/cm <sup>2</sup> )
0	0	0	0.00	0.00
20	23	15	5.00	50.99
40	45	30	10.00	101.97
60	80	60	20.00	203.94
80	50	25	8.33	84.98
100	100	70	23.33	237.93
120	120	80	26.67	271.93
140	200	90	30.00	305.92
160	280	105	35.00	356.90
180	325	110	36.67	373.90
200	360	120	40.00	407.89
220	400	135	45.00	458.87
240	500	150	50.00	509.86
260				



**PROJECT:** IMPROVEMENT OF SUBSTATION & DISTRIBUTION NETWORK - PHASE III

**SUB PROJECT:** 15 KV Distribution line Gasogi Substation - Nyagasambu

**Client:** PITRAD IBAMBA LTD.

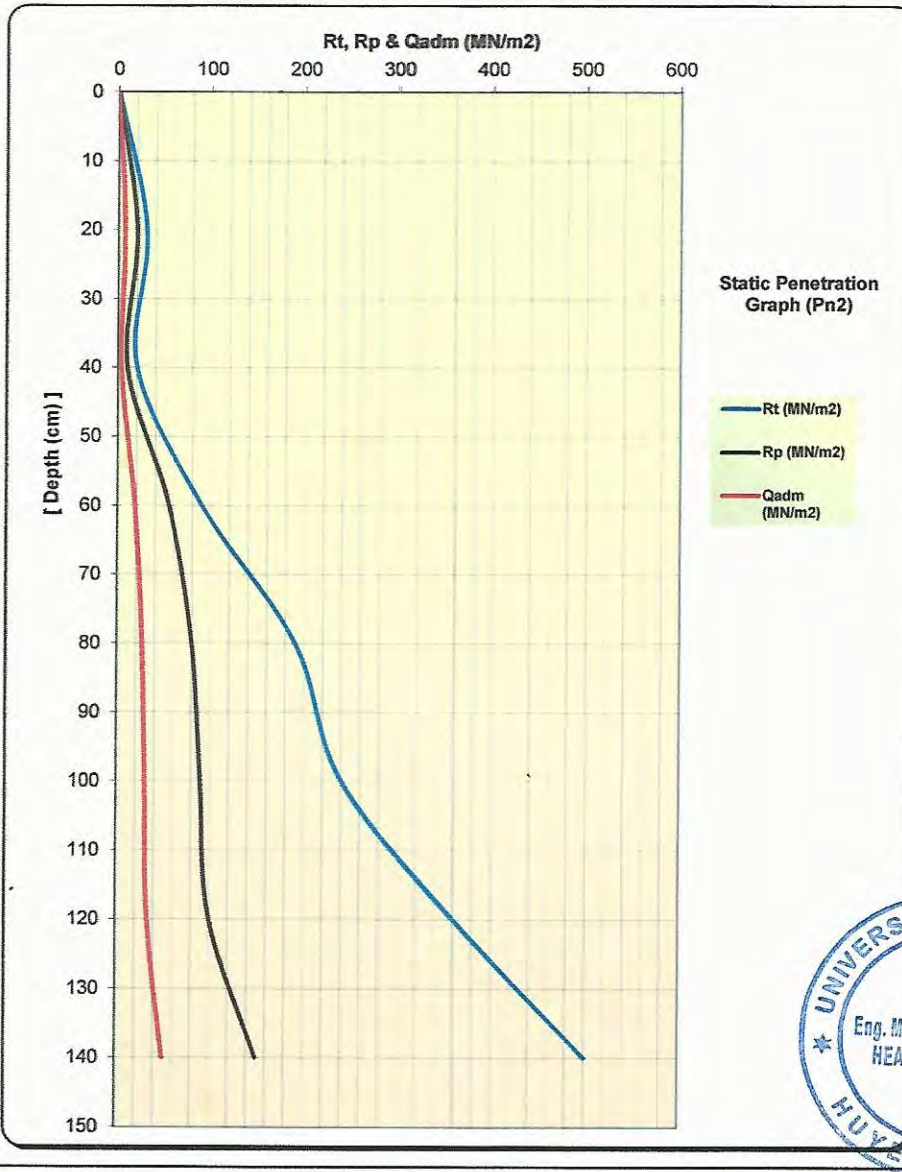
Operated by: Eng. Venerand GWIZA

Verified by: Eng. Vivien MUNYABURANGA (MSc)

Date: July, 2017

Penetration Test N° AP 9

Depth (cm)	Rt (MN/m <sup>2</sup> )	Rp (MN/m <sup>2</sup> )	Qadm (MN/m <sup>2</sup> )	Qadm (kg/cm <sup>2</sup> )
0	0	0	0.00	0.00
20	30	20	6.67	67.98
40	20	10	3.33	33.99
60	90	55	18.33	186.95
80	190	80	26.67	271.93
100	240	90	30.00	305.92
120	360	100	33.33	339.91
140	500	150	50.00	509.86
160				



**PROJECT:** IMPROVEMENT OF SUBSTATION & DISTRIBUTION NETWORK - PHASE III

**SUB PROJECT:** 15 KV Distribution line Gasogi Substation - Nyagasambu

**Client:** PITRAD IBAMBA LTD.

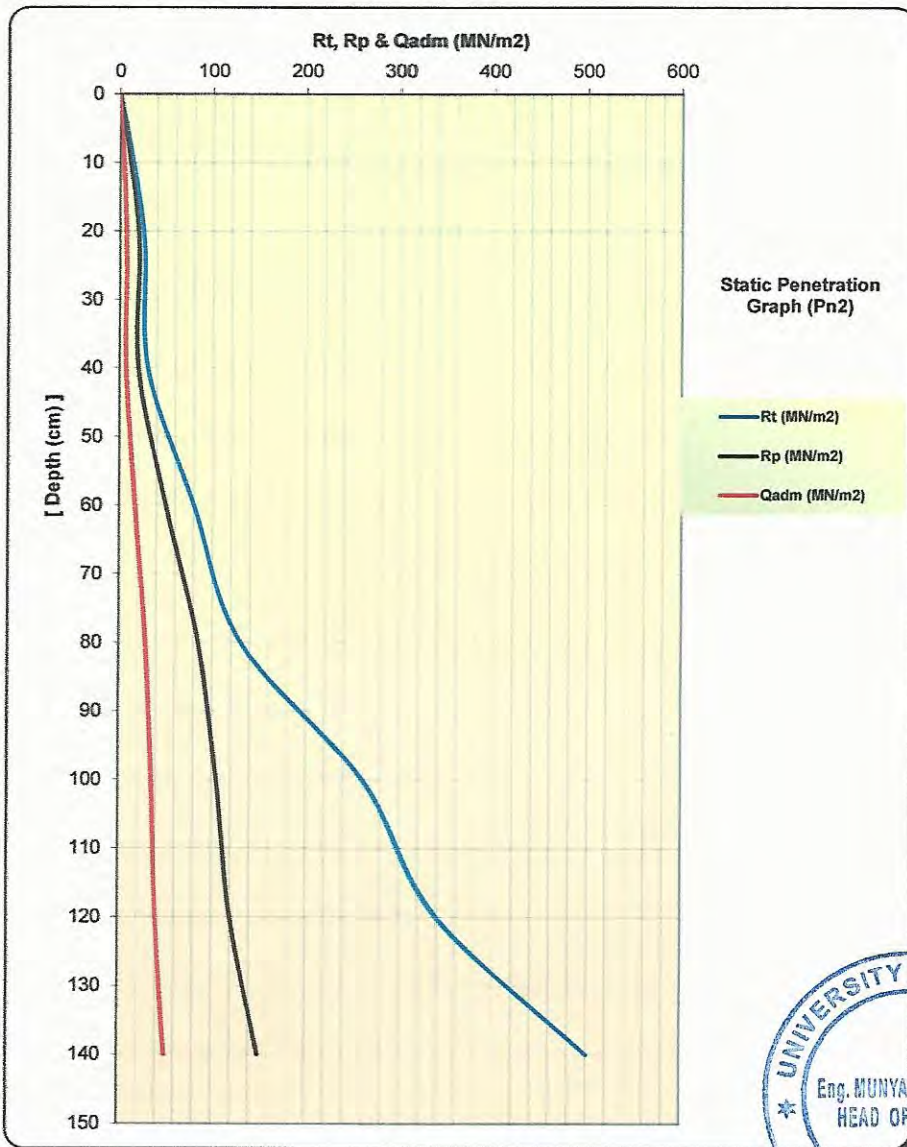
Operated by: Eng. Venerand GWIZA

Verified by: Eng. Vivien MUNYABURANGA (MSc)

Date: July, 2017

Penetration Test **N° AP 10**

Depth (cm)	Rt (MN/m <sup>2</sup> )	Rp (MN/m <sup>2</sup> )	Qadm (MN/m <sup>2</sup> )	Qadm (kg/cm <sup>2</sup> )
0	0	0	0	0.00
20	25	20	6.67	67.98
40	30	20	6.67	67.98
60	80	50	16.67	169.95
80	130	85	28.33	288.92
100	260	105	35.00	356.90
120	340	120	40.00	407.89
140	500	150	50.00	509.86
160				



**PROJECT: IMPROVEMENT OF SUBSTATION & DISTRIBUTION NETWORK - PHASE III**

**SUB PROJECT: 15 KV Distribution line Gasogi Substation - Nyagasambu**

**Client:** PITRAD IBAMBA LTD.

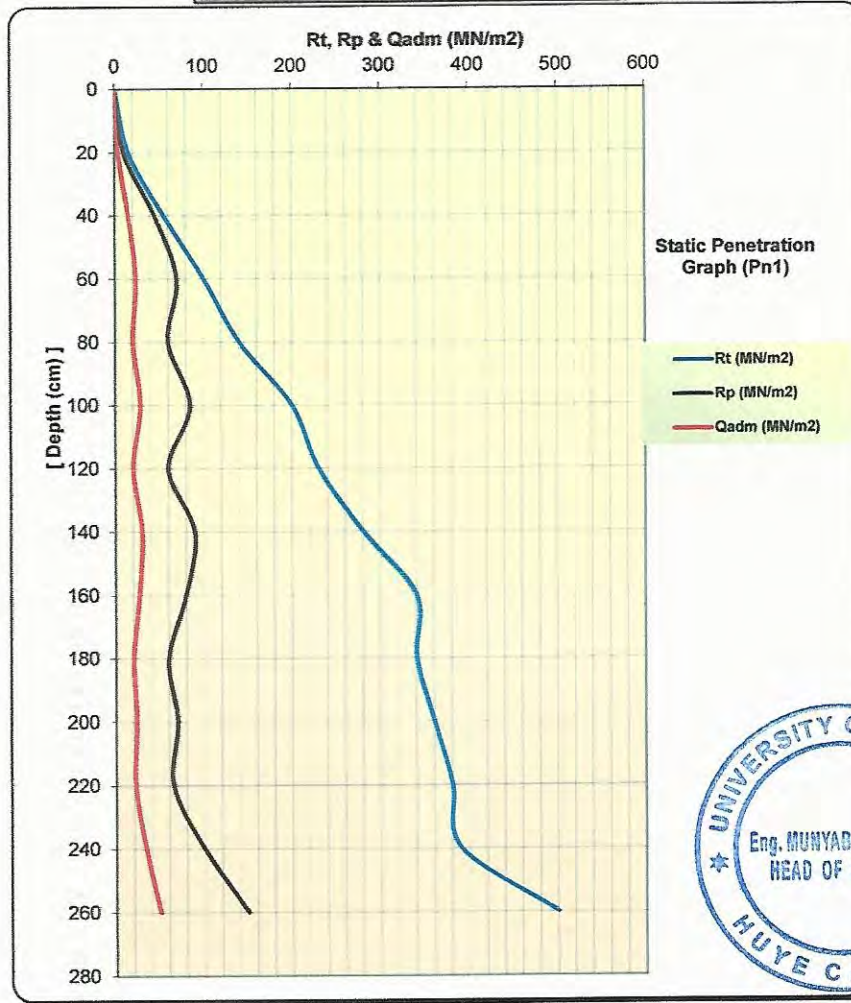
Operated by: Eng. Venerand GWIZA

Verified by: Eng. Vivien MUNYABURANGA (MSc)

Date: July, 2017

Penetration Test **N° AP 11**

Depth (cm)	Rt (MN/m <sup>2</sup> )	Rp (MN/m <sup>2</sup> )	Qadm (MN/m <sup>2</sup> )	Qadm (kg/cm <sup>2</sup> )
0	0	0	0.00	0.00
20	15	10	3.33	33.99
40	55	45	15.00	152.96
60	100	70	23.33	237.93
80	140	60	20.00	203.94
100	200	85	28.33	288.92
120	230	60	20.00	203.94
140	280	90	30.00	305.92
160	340	80	26.67	271.93
180	340	60	20.00	203.94
200	360	70	23.33	237.93
220	380	65	21.67	220.94
240	390	100	33.33	339.91
260	500	150	50.00	509.86
280				



**PROJECT: IMPROVEMENT OF SUBSTATION & DISTRIBUTION NETWORK - PHASE III**

**SUB PROJECT: 15 KV Distribution line Gasogi Substation - Nyagasambu**

**Client:** PITRAD IBAMBA LTD.

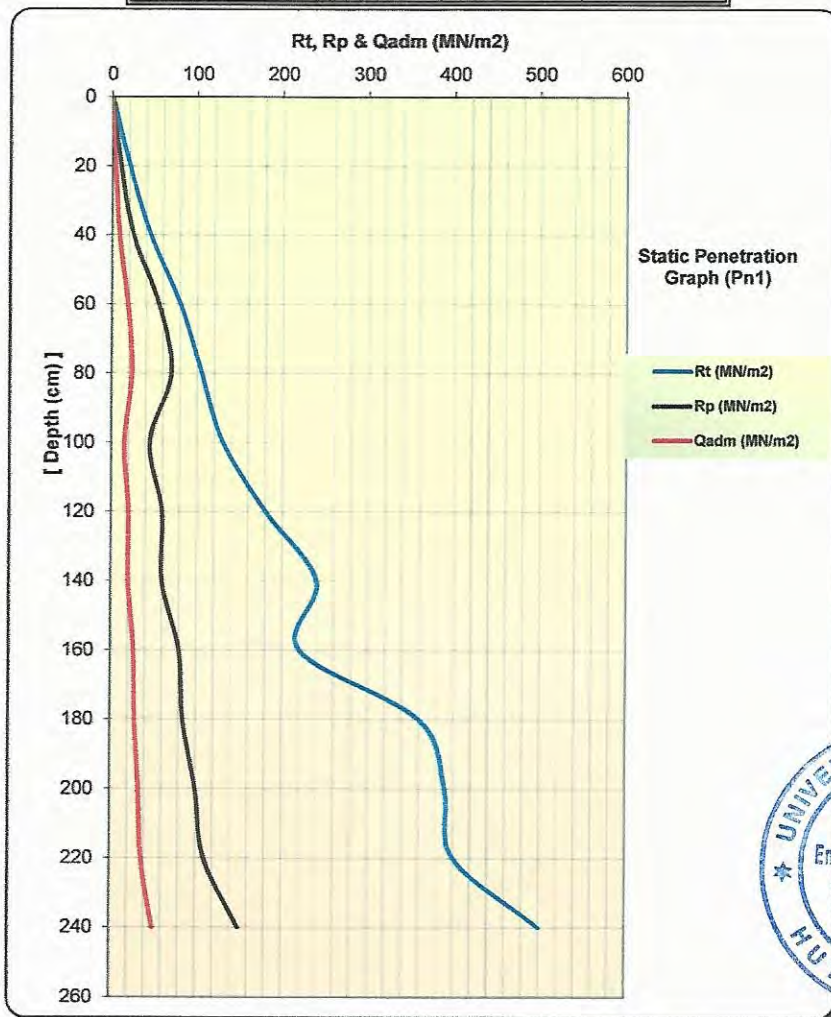
Operated by: Eng. Venerand GWIZA

Verified by: Eng. Vivien MUNYABURANGA (MSc)

Date: July, 2017

Penetration Test **N° AP 12**

Depth (cm)	Rt (MN/m2)	Rp (MN/m2)	Qadm (MN/m2)	Qadm (kg/cm2)
0	0	0	0.00	0.00
20	20	10	3.33	33.99
40	45	25	8.33	84.98
60	80	55	18.33	186.95
80	105	70	23.33	237.93
100	130	45	15.00	152.96
120	180	60	20.00	203.94
140	240	60	20.00	203.94
160	220	80	26.67	271.93
180	360	85	28.33	288.92
200	390	100	33.33	339.91
220	400	110	36.67	373.90
240	500	150	50.00	509.86
260				



**PROJECT: IMPROVEMENT OF SUBSTATION & DISTRIBUTION NETWORK - PHASE III**

**SUB PROJECT: 15 KV Distribution line Gasogi Substation - Nyagasambu**

**Client:** PITRAD IBAMBA LTD.

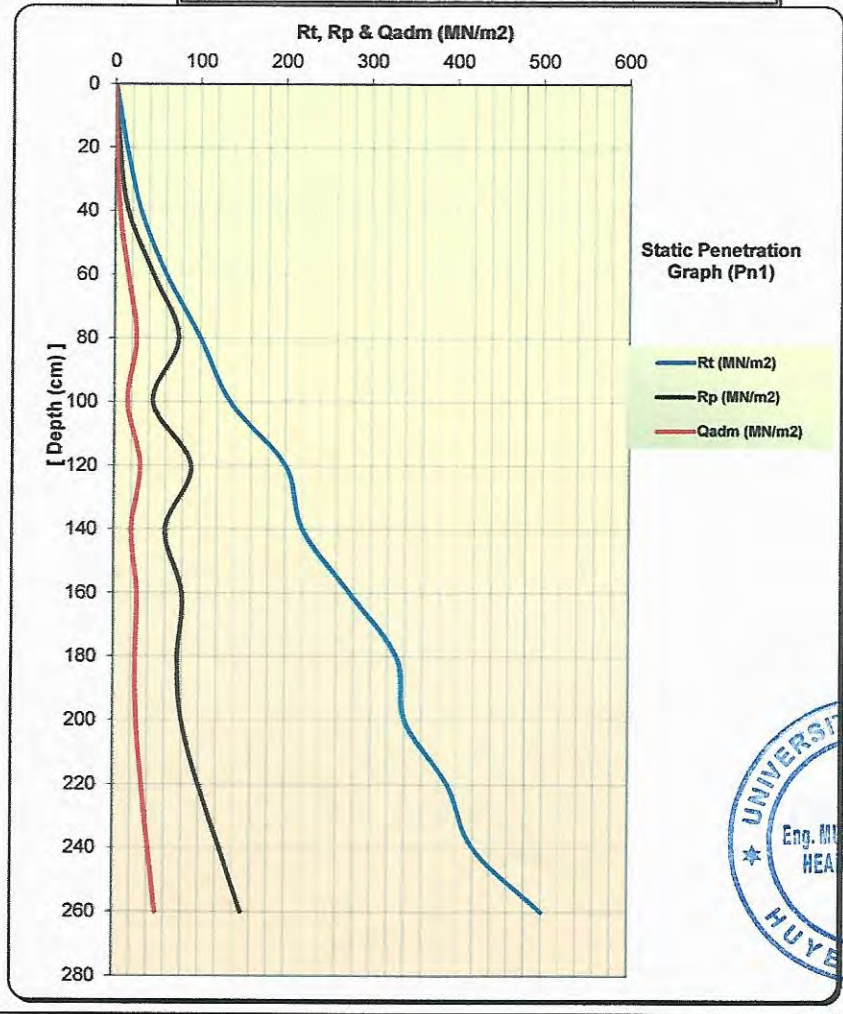
Operated by: Eng. Venerand GWIZA

Verified by: Eng. Vivien MUNYABURANGA (MSc)

Date: July, 2017

Penetration Test N° AP 13

Depth (cm)	Rt (MN/m <sup>2</sup> )	Rp (MN/m <sup>2</sup> )	Qadm (MN/m <sup>2</sup> )	Qadm (kg/cm <sup>2</sup> )
0	0	0	0.00	0.00
20	13	5	1.67	17.00
40	30	15	5.00	50.99
60	60	45	15.00	152.96
80	100	75	25.00	254.93
100	135	45	15.00	152.96
120	200	90	30.00	305.92
140	220	60	20.00	203.94
160	275	80	26.67	271.93
180	330	75	25.00	254.93
200	340	80	26.67	271.93
220	390	100	33.33	339.91
240	420	125	41.67	424.88
260	500	150	50.00	509.86
280				



**PROJECT: IMPROVEMENT OF SUBSTATION & DISTRIBUTION NETWORK - PHASE III**

**SUB PROJECT: 15 KV Distribution line Gasogi Substation - Nyagasambu**

**Client:** PITRAD IBAMBA LTD.

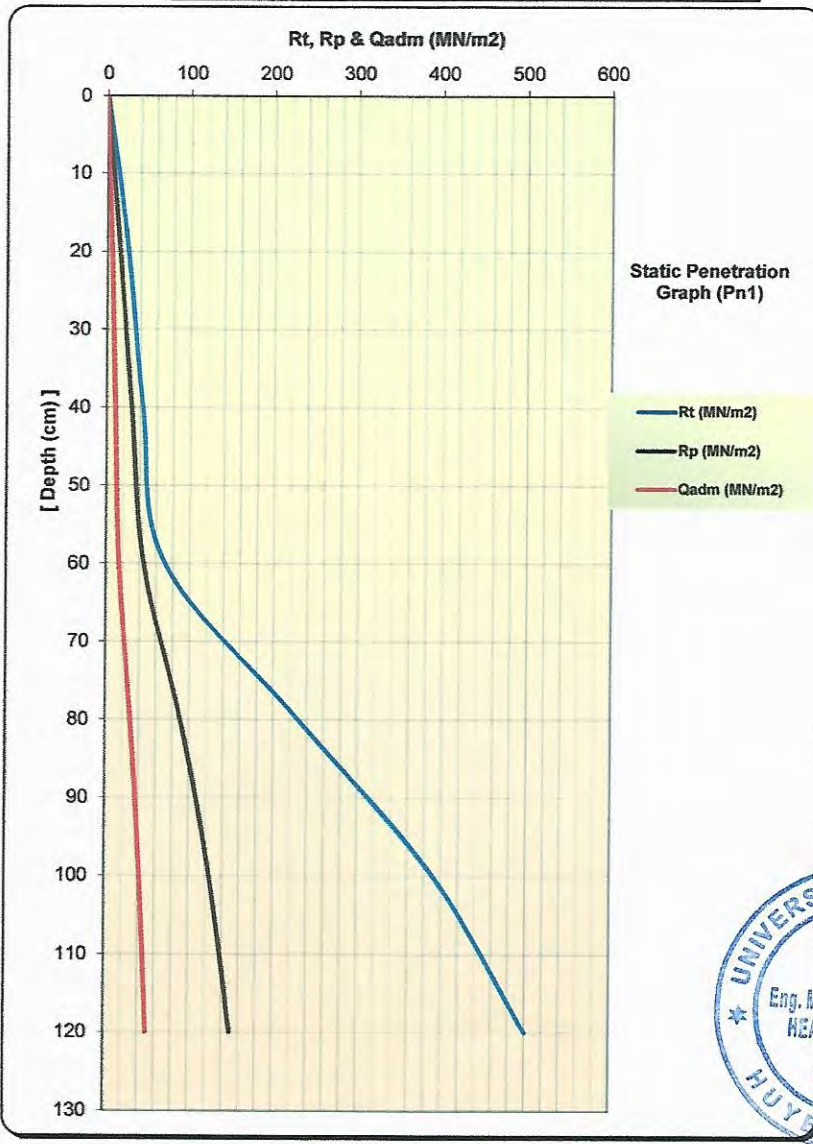
Operated by: Eng. Venerand GWIZA

Verified by: Eng. Vivien MUNYABURANGA (MSc)

Date: July, 2017

Penetration Test **N° AP 15**

Depth (cm)	Rt (MN/m <sup>2</sup> )	Rp (MN/m <sup>2</sup> )	Qadm (MN/m <sup>2</sup> )	Qadm (kg/cm <sup>2</sup> )
0	0	0	0.00	0.00
20	25	15	5.00	50.99
40	43	30	10.00	101.97
60	70	45	15.00	152.96
80	230	90	30.00	305.92
100	390	125	41.67	424.88
120	500	150	50.00	509.86
140				





**PROJECT: IMPROVEMENT OF SUBSTATION & DISTRIBUTION NETWORK - PHASE III**

**SUB PROJECT: 15 KV Distribution line Gasogi Substation - Nyagasambu**

**Client:** PITRAD IBAMBA LTD.

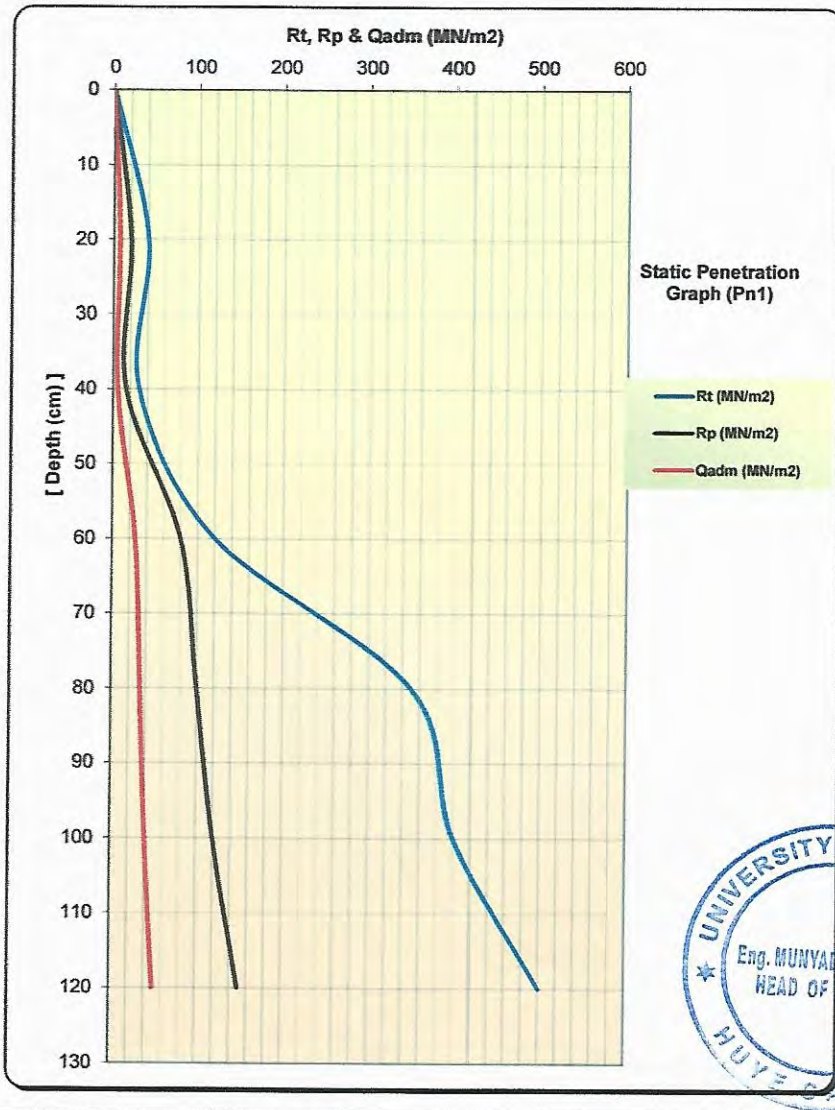
Operated by: Eng. Venerand GWIZA

Verified by: Eng. Vivien MUNYABURANGA (MSc)

Date: July, 2017

Penetration Test **N° AP 16**

Depth (cm)	Rt (MN/m <sup>2</sup> )	Rp (MN/m <sup>2</sup> )	Qadm (MN/m <sup>2</sup> )	Qadm (kg/cm <sup>2</sup> )
0	0	0	0.00	0.00
20	40	20	6.67	67.98
40	30	15	5.00	50.99
60	120	80	26.67	271.93
80	350	100	33.33	339.91
100	400	120	40.00	407.89
120	500	150	50.00	509.86
140				



**PROJECT: IMPROVEMENT OF SUBSTATION & DISTRIBUTION NETWORK - PHASE III**

**SUB PROJECT: 15 KV Distribution line Gasogi Substation - Nyagasambu**

**Client:** PITRAD IBAMBA LTD.

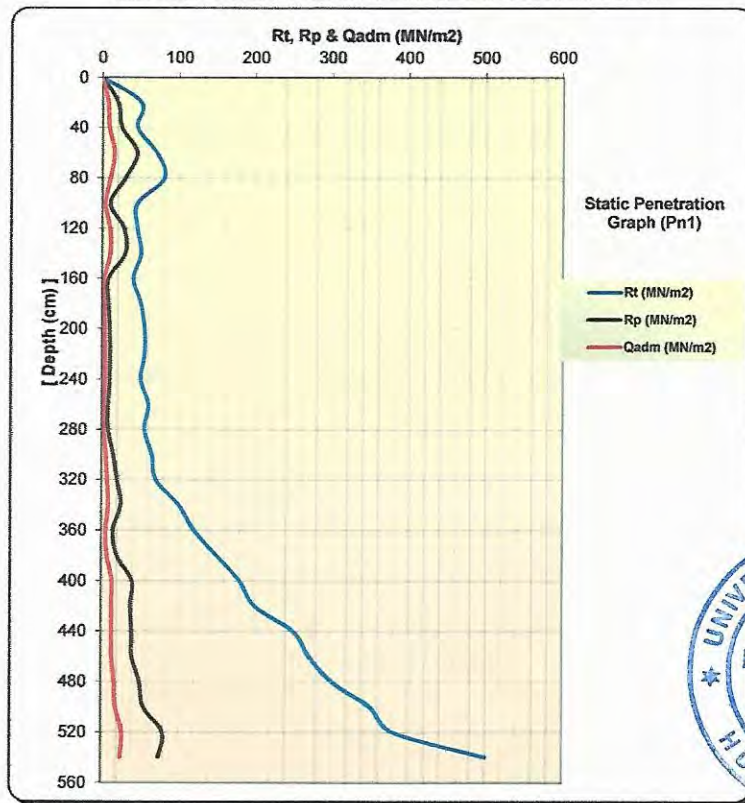
Operated by: Eng. Venerand GWIZA

Verified by: Eng. Vivien MUNYABURANGA (MSc)

Date: July, 2017

Penetration Test N° AP 17

Depth (cm)	Rt (MN/m2)	Rp (MN/m2)	Qadm (MN/m2)	Qadm (kg/cm2)
0	0	0	0.00	0.00
20	50	20	6.67	67.98
40	45	25	8.33	84.98
60	70	45	15.00	152.96
80	80	30	10.00	101.97
100	45	10	3.33	33.99
120	45	28	9.33	95.17
140	50	30	10.00	101.97
160	40	8	2.67	27.19
180	50	8	2.67	27.19
200	55	10	3.33	33.99
220	55	10	3.33	33.99
240	50	10	3.33	33.99
260	60	8	2.67	27.19
280	55	8	2.67	27.19
300	65	15	5.00	50.99
320	70	20	6.67	67.98
340	100	25	8.33	84.98
360	120	15	5.00	50.99
380	150	20	6.67	67.98
400	180	40	13.33	135.96
420	200	38	12.67	129.16
440	250	40	13.33	135.96
460	270	40	13.33	135.96
480	300	50	16.67	169.95
500	350	55	18.33	186.95
520	380	80	26.67	271.93
540	500	75	25.00	254.93
560				



**PROJECT:** IMPROVEMENT OF SUBSTATION & DISTRIBUTION NETWORK - PHASE III

**SUB PROJECT:** 15 KV Distribution line Gasogi Substation - Masaka Hospital

**Client:** PITRAD IBAMBA LTD.

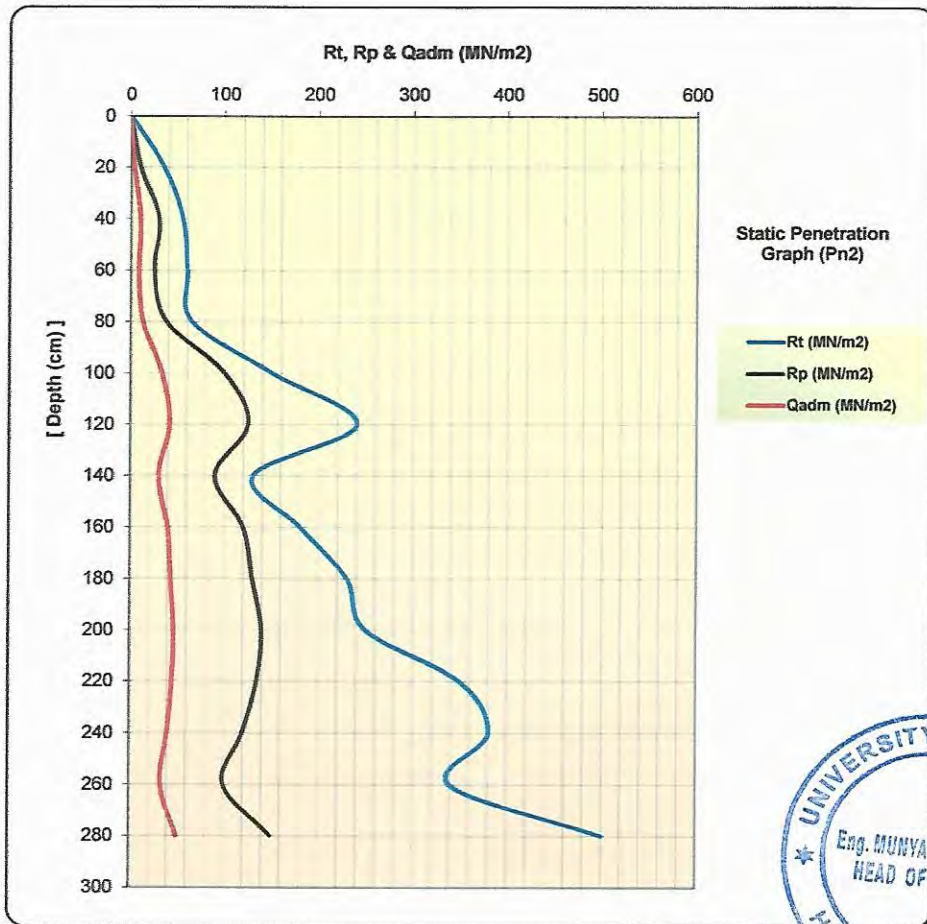
Operated by: Eng. Venerand GWIZA

Verified by: Eng. Vivien MUNYABURANGA (MSc)

Date: July, 2017

Penetration Test **N° AP 1**

Depth (cm)	Rt (MN/m <sup>2</sup> )	Rp (MN/m <sup>2</sup> )	Qadm (MN/m <sup>2</sup> )	Qadm (kg/cm <sup>2</sup> )
0	0	0	0	0.00
20	35	10	3.33	33.99
40	55	30	10.00	101.97
60	60	25	8.33	84.98
80	65	38	12.67	129.16
100	150	100	33.33	339.91
120	240	125	41.67	424.88
140	130	90	30.00	305.92
160	180	120	40.00	407.89
180	230	130	43.33	441.88
200	250	140	46.67	475.87
220	350	135	45.00	458.87
240	380	120	40.00	407.89
260	340	100	33.33	339.91
280	500	150	50.00	509.86
300				



**PROJECT:** IMPROVEMENT OF SUBSTATION & DISTRIBUTION NETWORK - PHASE III

**SUB PROJECT:** 15 KV Distribution line Gasogi Substation - Masaka Hospital

**Client:** PITRAD IBAMBA LTD.

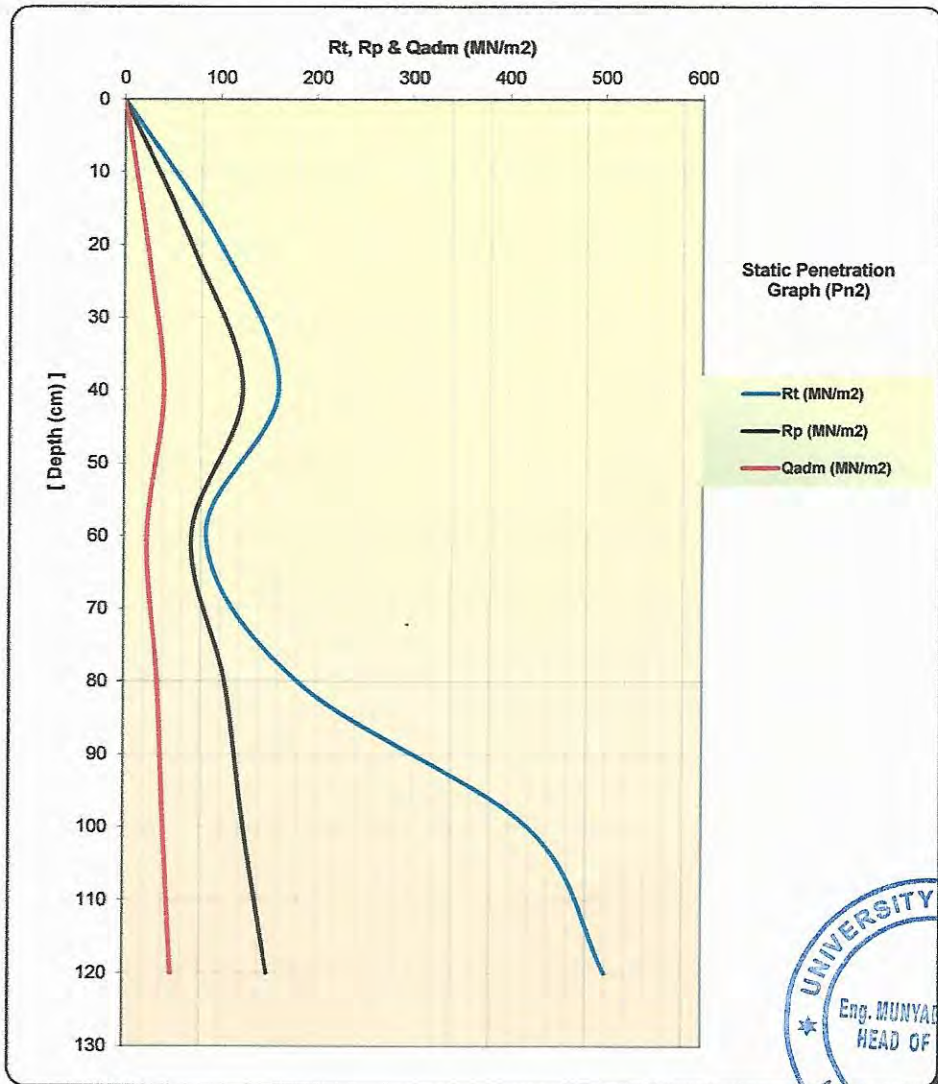
Operated by: Eng. Venerand GWIZA

Verified by: Eng. Vivien MUNYABURANGA (MSc)

Date: July, 2017

Penetration Test **N° AP 3**

Depth (cm)	Rt (MN/m <sup>2</sup> )	Rp (MN/m <sup>2</sup> )	Qadm (MN/m <sup>2</sup> )	Qadm (kg/cm <sup>2</sup> )
0	0	0	0	0.00
20	100	70	23.33	237.93
40	160	123	41.00	418.09
60	85	70	23.33	237.93
80	180	105	35.00	356.90
100	420	125	41.67	424.88
120	500	150	50.00	509.86
140				

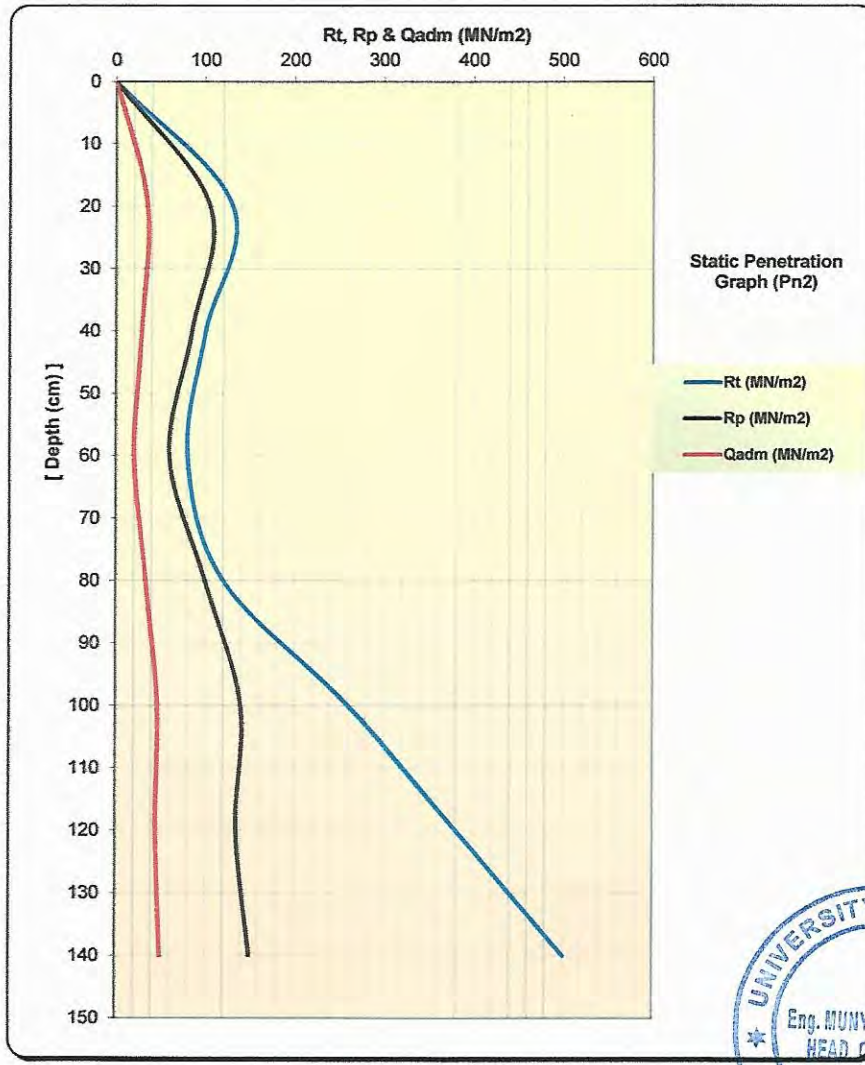


**PROJECT:** IMPROVEMENT OF SUBSTATION & DISTRIBUTION NETWORK - PHASE III  
**SUB PROJECT:** 15 KV Distribution line Gasogi Substation - Masaka Hospital

**Client:** PITRAD IBAMBA LTD.  
 Operated by: Eng. Venerand GWIZA  
 Verified by: Eng. Vivien MUNYABURANGA (MSc)  
 Date: July, 2017

Penetration Test **N° AP 4**

Depth (cm)	Rt (MN/m <sup>2</sup> )	Rp (MN/m <sup>2</sup> )	Qadm (MN/m <sup>2</sup> )	Qadm (kg/cm <sup>2</sup> )
0	0	0	0.00	0.00
20	130	105	35.00	356.90
40	100	85	28.33	288.92
60	80	60	20.00	203.94
80	120	100	33.33	339.91
100	260	140	46.67	475.87
120	380	135	45.00	458.87
140	500	150	50.00	509.86
160				

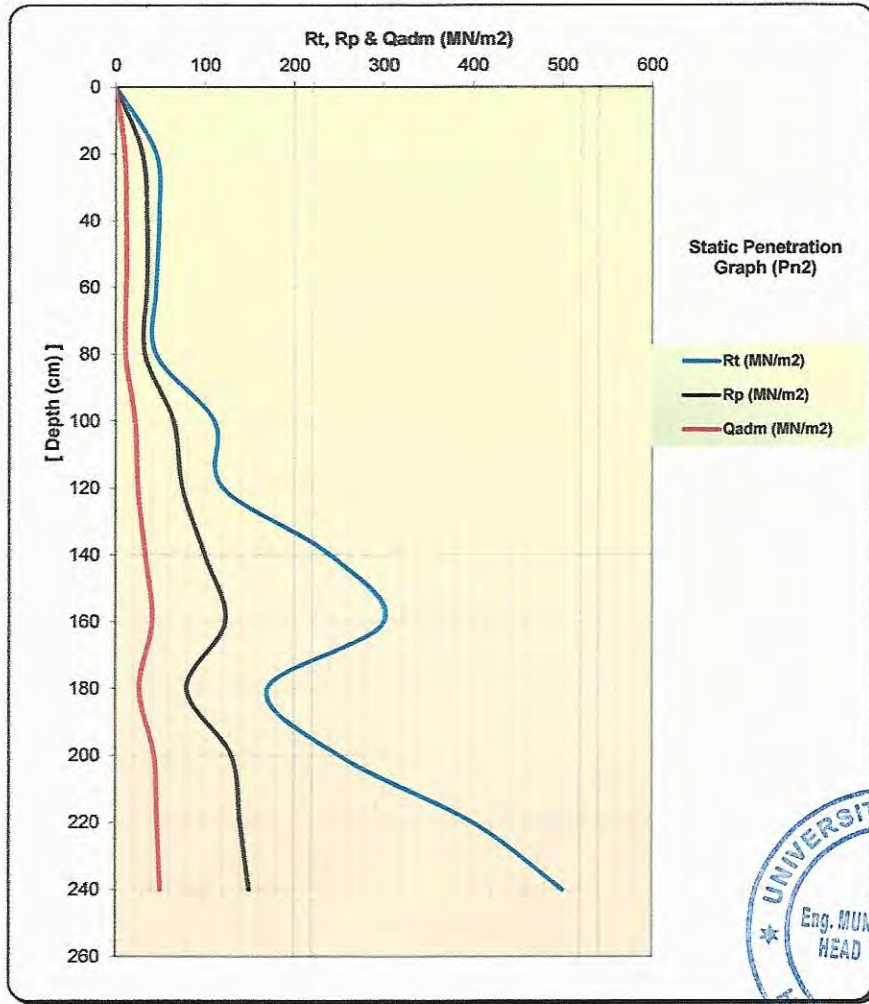


**PROJECT:** IMPROVEMENT OF SUBSTATION & DISTRIBUTION NETWORK - PHASE III  
**SUB PROJECT:** 15 KV Distribution line Gasogi Substation - Masaka Hospital

**Client:** PITRAD IBAMBA LTD.  
 Operated by: Eng. Venerand GWIZA  
 Verified by: Eng. Vivien MUNYABURANGA (MSc)  
 Date: July, 2017

Penetration Test **N° AP 5**

Depth (cm)	Rt (MN/m <sup>2</sup> )	Rp (MN/m <sup>2</sup> )	Qadm (MN/m <sup>2</sup> )	Qadm (kg/cm <sup>2</sup> )
0	0	0	0.00	0.00
20	45	30	10.00	101.97
40	48	35	11.67	118.97
60	45	35	11.67	118.97
80	45	33	11.00	112.17
100	110	65	21.67	220.94
120	120	75	25.00	254.93
140	240	100	33.33	339.91
160	300	123	41.00	418.09
180	170	80	26.67	271.93
200	250	130	43.33	441.88
220	400	140	46.67	475.87
240	500	150	50.00	509.86
320				



**PROJECT: IMPROVEMENT OF SUBSTATION & DISTRIBUTION NETWORK - PHASE III**

**SUB PROJECT: 15 KV Distribution line Gasogi Substation - Masaka Hospital**

**Client:** PITRAD IBAMBA LTD.

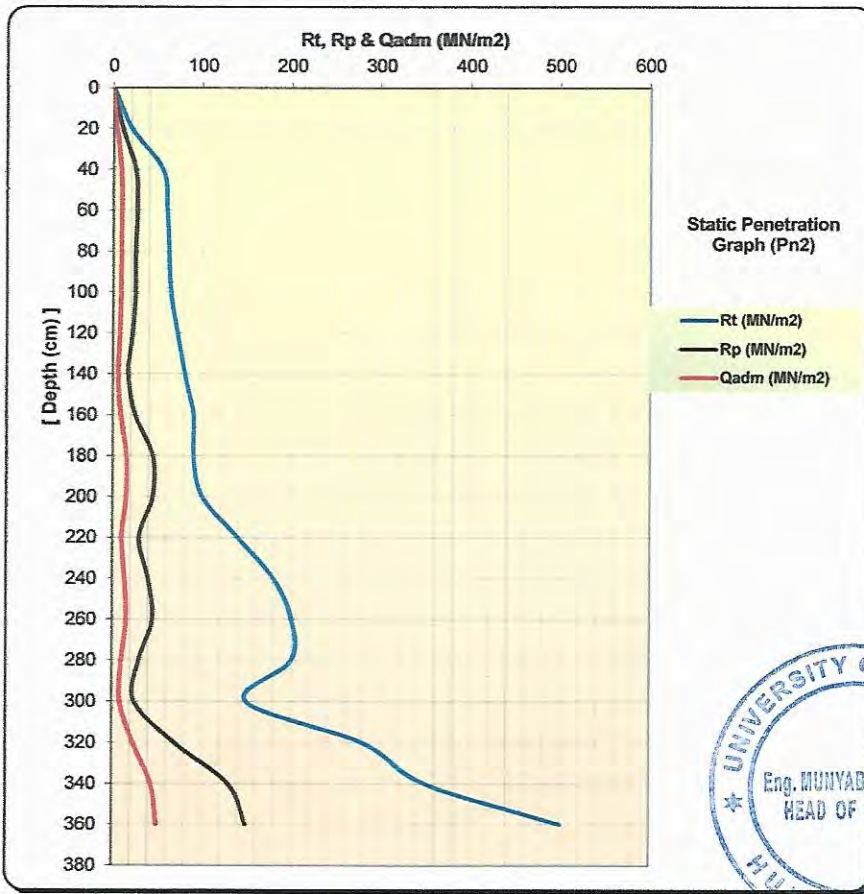
Operated by: Eng. Venerand GWIZA

Verified by: Eng. Vivien MUNYABURANGA (MSc)

Date: July, 2017

Penetration Test **N° AP 6**

Depth (cm)	Rt (MN/m <sup>2</sup> )	Rp (MN/m <sup>2</sup> )	Qadm (MN/m <sup>2</sup> )	Qadm (kg/cm <sup>2</sup> )
0	0	0	0.00	0.00
20	20	10	3.33	33.99
40	55	25	8.33	84.98
60	60	27	9.00	91.77
80	62	25	8.33	84.98
100	64	25	8.33	84.98
120	72	22	7.33	74.78
140	80	17	5.67	57.78
160	90	25	8.33	84.98
180	90	45	15.00	152.96
200	100	45	15.00	152.96
220	140	30	10.00	101.97
240	180	40	13.33	135.96
260	200	45	15.00	152.96
280	200	30	10.00	101.97
300	150	25	8.33	84.98
320	280	70	23.33	237.93
340	350	130	43.33	441.88
360	500	150	50.00	509.86
380				



**PROJECT:** IMPROVEMENT OF SUBSTATION & DISTRIBUTION NETWORK - PHASE III

**SUB PROJECT:** 15 KV Distribution line Gasogi Substation - Masaka Hospital

**Client:** PITRAD IBAMBA LTD.

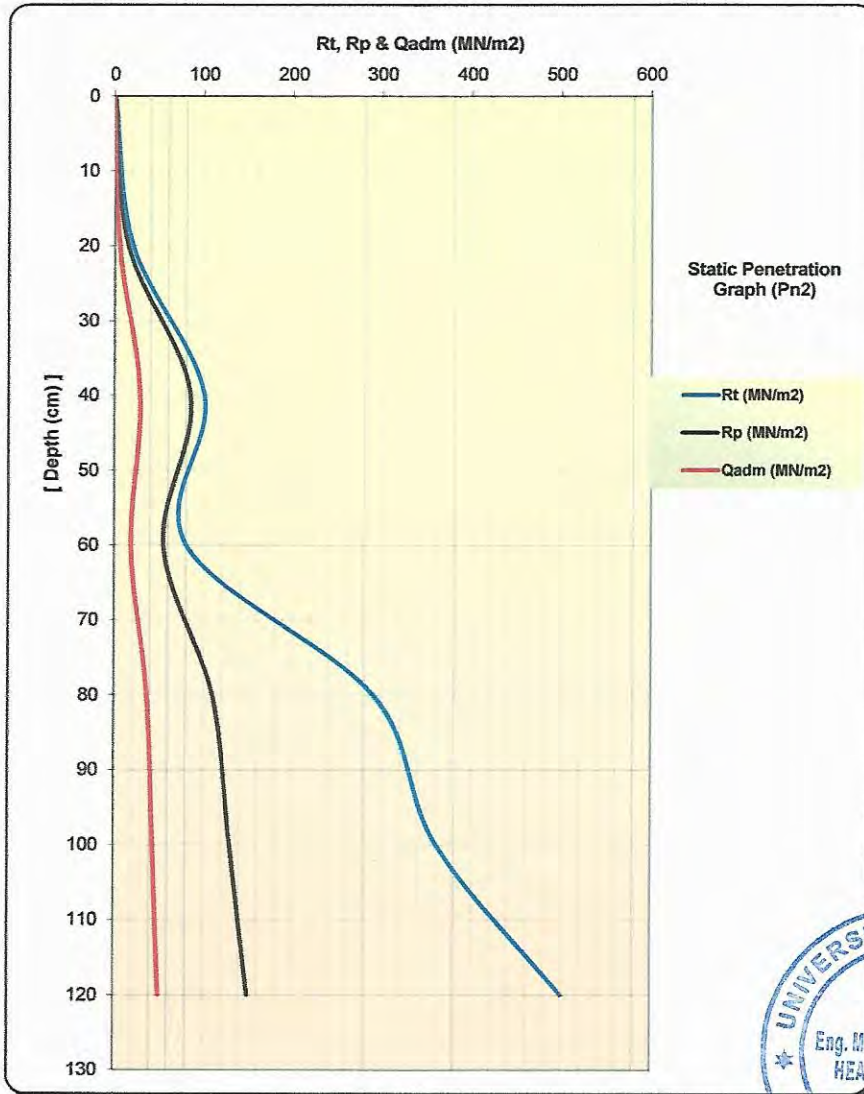
Operated by: Eng. Venerand GWIZA

Verified by: Eng. Vivien MUNYABURANGA (MSc)

Date: July, 2017

Penetration Test **N°8** - Pump Station

Depth (cm)	Rt (MN/m <sup>2</sup> )	Rp (MN/m <sup>2</sup> )	Qadm (MN/m <sup>2</sup> )	Qadm (kg/cm <sup>2</sup> )
0	0	0	0.00	0.00
20	20	15	5.00	50.99
40	100	85	28.33	288.92
60	80	55	18.33	186.95
80	290	110	36.67	373.90
100	360	130	43.33	441.88
120	500	150	50.00	509.86
140				





**PROJECT:** IMPROVEMENT OF SUBSTATION & DISTRIBUTION NETWORK - PHASE III

**SUB PROJECT:** 15 KV Distribution line Gasogi Substation - Masaka Hospital

**Client:** PITRAD IBAMBA LTD.

Operated by: Eng. Venerand GWIZA

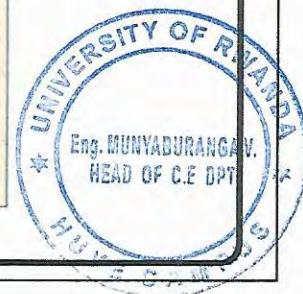
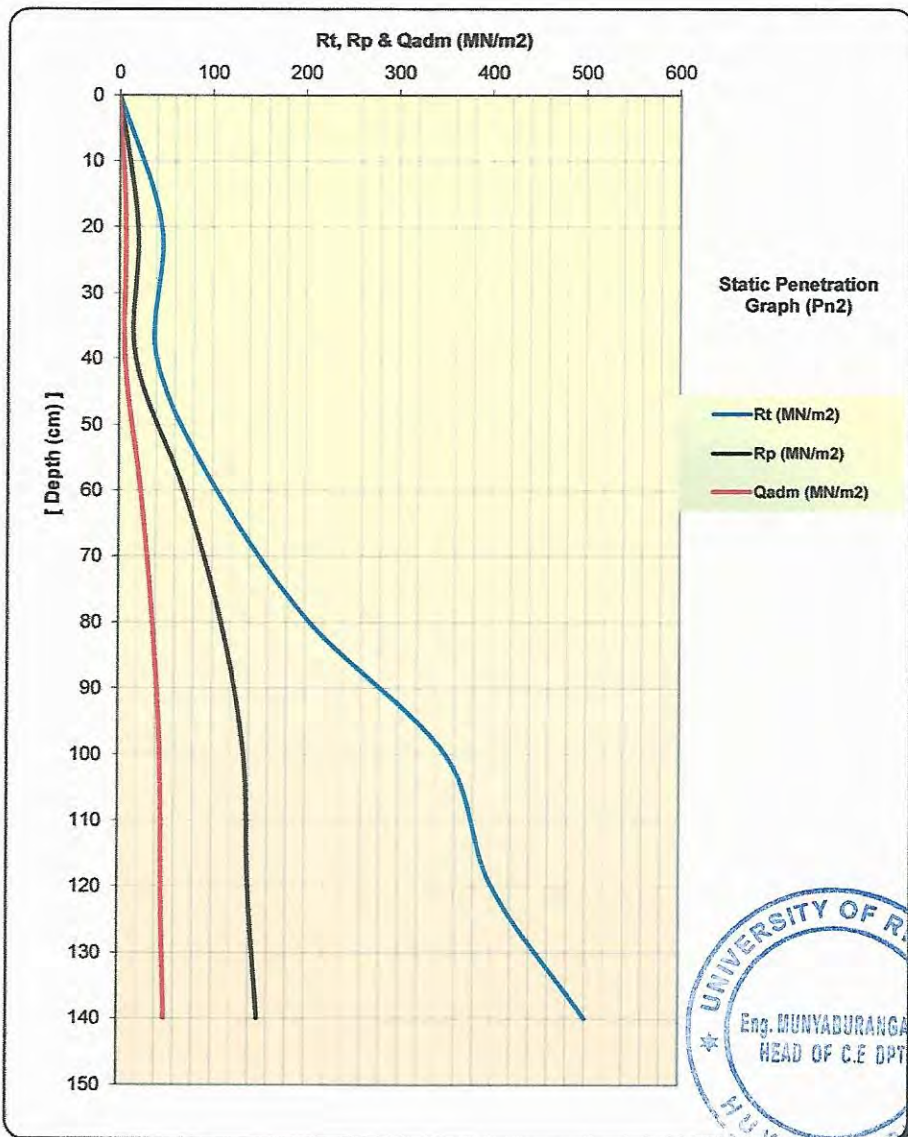
Verified by: Eng. Vivien MUNYABURANGA (MSc)

Date: July, 2017

Penetration Test **N° AP 9**

Date: 29-08 to 25-09-2013

Depth (cm)	Rt (MN/m <sup>2</sup> )	Rp (MN/m <sup>2</sup> )	Qadm (MN/m <sup>2</sup> )	Qadm (kg/cm <sup>2</sup> )
0	0	0	0.00	0.00
20	45	20	6.67	67.98
40	40	18	6.00	61.18
60	105	70	23.33	237.93
80	205	110	36.67	373.90
100	350	135	45.00	458.87
120	400	140	46.67	475.87
140	500	150	50.00	509.86
160				

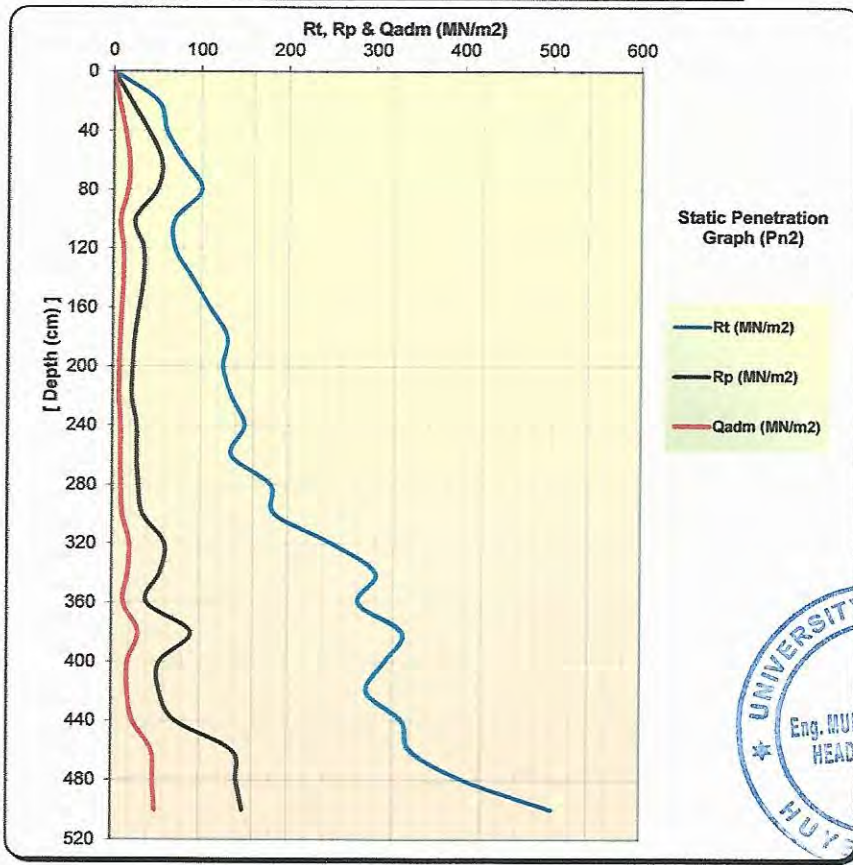


**PROJECT:** IMPROVEMENT OF SUBSTATION & DISTRIBUTION NETWORK - PHASE III  
**SUB PROJECT:** 15 KV Distribution line Gasogi Substation - Masaka Hospital

**Client:** PITRAD IBAMBA LTD.  
 Operated by: Eng. Venerand GWIZA  
 Verified by: Eng. Vivien MUNYABURANGA (MSc)  
 Date: July, 2017

Penetration Test **N° AP 10**

Depth (cm)	Rt (MN/m <sup>2</sup> )	Rp (MN/m <sup>2</sup> )	Qadm (MN/m <sup>2</sup> )	Qadm (kg/cm <sup>2</sup> )
0	0	0	0.00	0.00
20	50	20	6.67	67.98
40	60	40	13.33	135.96
60	80	55	18.33	186.95
80	100	50	16.67	169.95
100	70	25	8.33	84.98
120	70	35	11.67	118.97
140	90	35	11.67	118.97
160	110	30	10.00	101.97
180	130	25	8.33	84.98
200	125	23	7.67	78.18
220	135	22	7.33	74.78
240	150	28	9.33	95.17
260	135	28	9.33	95.17
280	180	30	10.00	101.97
300	185	35	11.67	118.97
320	250	60	20.00	203.94
340	300	55	18.33	186.95
360	280	40	13.33	135.96
380	330	90	30.00	305.92
400	310	55	18.33	186.95
420	290	55	18.33	186.95
440	330	73	24.33	248.13
460	340	138	46.00	469.07
480	400	143	47.67	486.07
500	500	150	50.00	509.86
520				



**PROJECT:** IMPROVEMENT OF SUBSTATION & DISTRIBUTION NETWORK - PHASE III

**SUB PROJECT:** 15 KV Distribution line Gasogi Substation - Masaka Hospital

**Client:** PITRAD IBAMBA LTD.

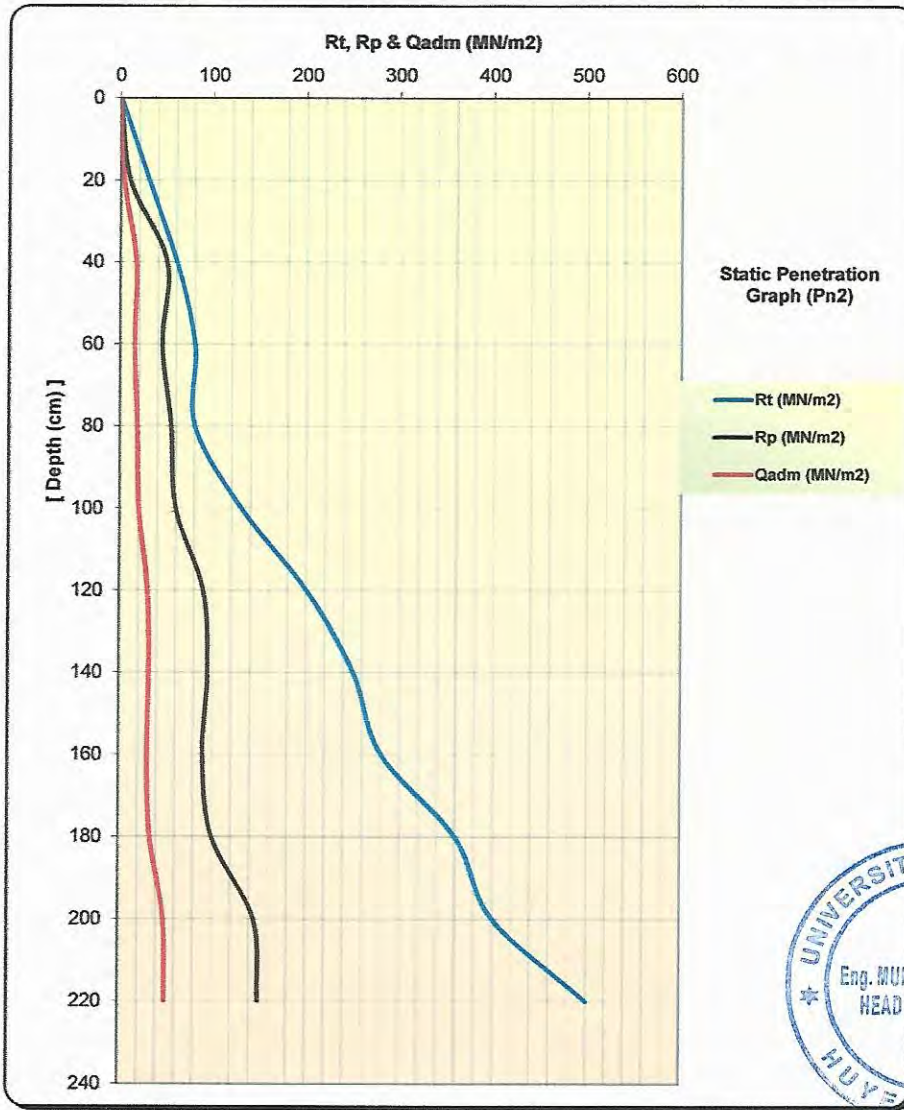
Operated by: Eng. Venerand GWIZA

Verified by: Eng. Vivien MUNYABURANGA (MSc)

Date: July, 2017

Penetration Test **N°AP 12**

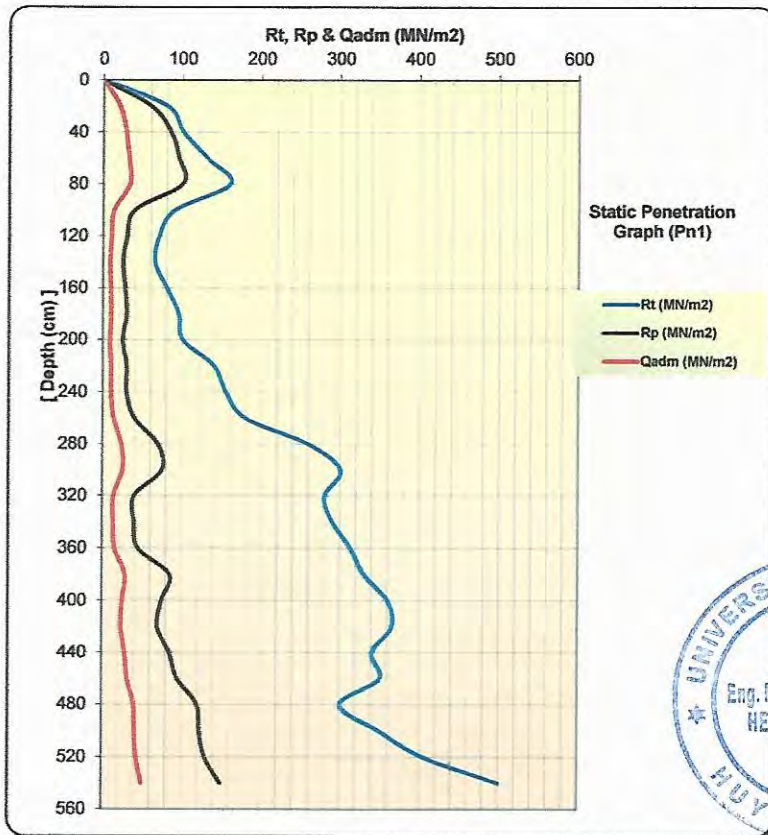
Depth (cm)	Rt (MN/m <sup>2</sup> )	Rp (MN/m <sup>2</sup> )	Qadm (MN/m <sup>2</sup> )	Qadm (kg/cm <sup>2</sup> )
0	0	0	0	0.00
20	30	10	3.33	33.99
40	60	50	16.67	169.95
60	80	45	15.00	152.96
80	80	55	18.33	186.95
100	130	60	20.00	203.94
120	200	90	30.00	305.92
140	250	95	31.67	322.91
160	280	90	30.00	305.92
180	360	100	33.33	339.91
200	400	145	48.33	492.86
220	500	150	50.00	509.86
240				



**PROJECT: IMPROVEMENT OF SUBSTATION & DISTRIBUTION NETWORK - PHASE III**  
**SUB PROJECT: 15 KV Distribution line Gasogi Substation - Masaka Hospital**

**Client:** PITRAD IBAMBA LTD.  
 Operated by: Eng. Venerand GWIZA  
 Verified by: Eng. Vivien MUNYABURANGA (MSc)  
 Date: July, 2017  
 Penetration Test **N° AP 13**

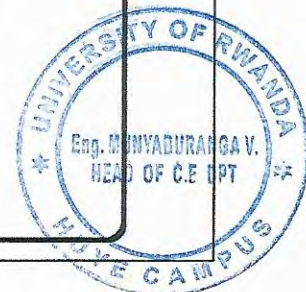
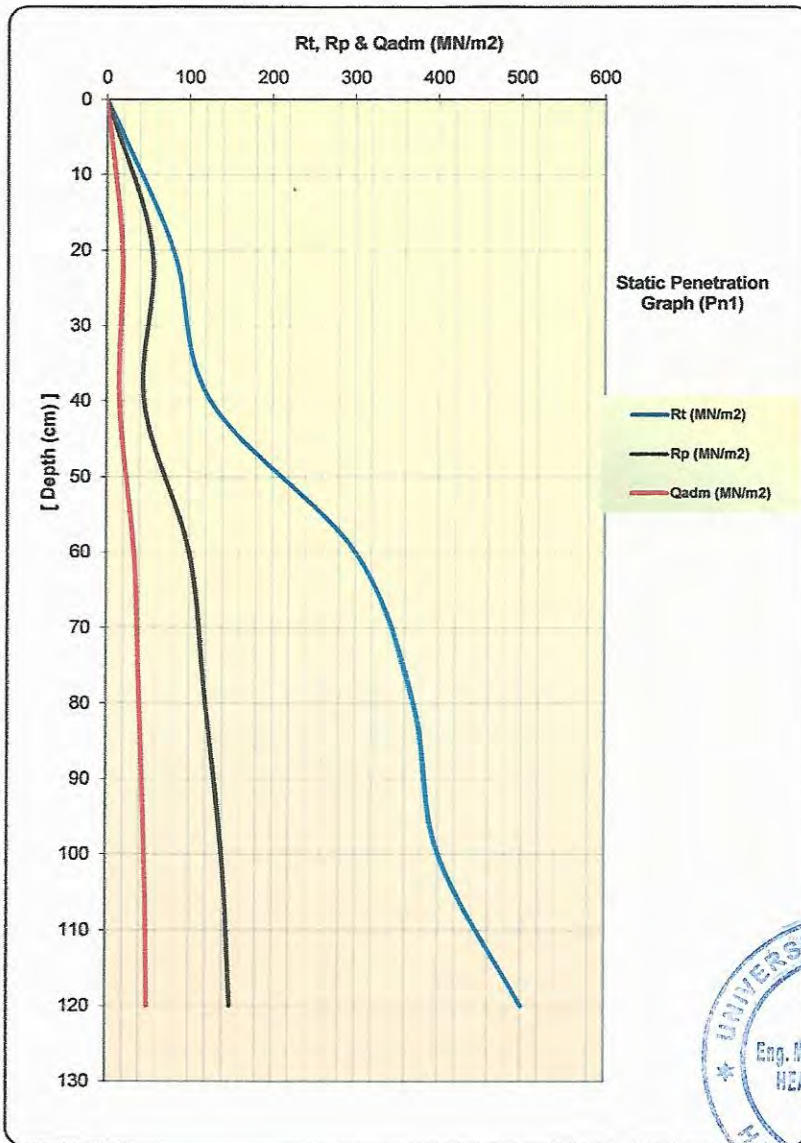
Depth (cm)	Rt (MN/m2)	Rp (MN/m2)	Qadm (MN/m2)	Qadm (kg/cm2)
0	0	0	0.00	0.00
20	80	60	20.00	203.94
40	100	85	28.33	288.92
60	130	95	31.67	322.91
80	160	100	33.33	339.91
100	90	40	13.33	135.96
120	70	30	10.00	101.97
140	65	25	8.33	84.98
160	80	28	9.33	95.17
180	95	30	10.00	101.97
200	100	25	8.33	84.98
220	140	30	10.00	101.97
240	155	30	10.00	101.97
260	180	40	13.33	135.96
280	260	70	23.33	237.93
300	300	75	25.00	254.93
320	280	40	13.33	135.96
340	290	40	13.33	135.96
360	313	45	15.00	152.96
380	330	85	28.33	288.92
400	360	75	25.00	254.93
420	365	70	23.33	237.93
440	340	85	28.33	288.92
460	350	95	31.67	322.91
480	300	120	40.00	407.89
500	350	123	41.00	418.09
520	405	130	43.33	441.88
540	500	150	50.00	509.86
560				



**PROJECT: IMPROVEMENT OF SUBSTATION & DISTRIBUTION NETWORK - PHASE III**  
**SUB PROJECT: 15 KV Distribution line Gasogi Substation - Masaka Hospital**

**Client:** PITRAD IBAMBA LTD.  
 Operated by: Eng. Venerand GWIZA  
 Verified by: Eng. Vivien MUNYABURANGA (MSc)  
 Date: July, 2017  
 Penetration Test **N° AP 15**

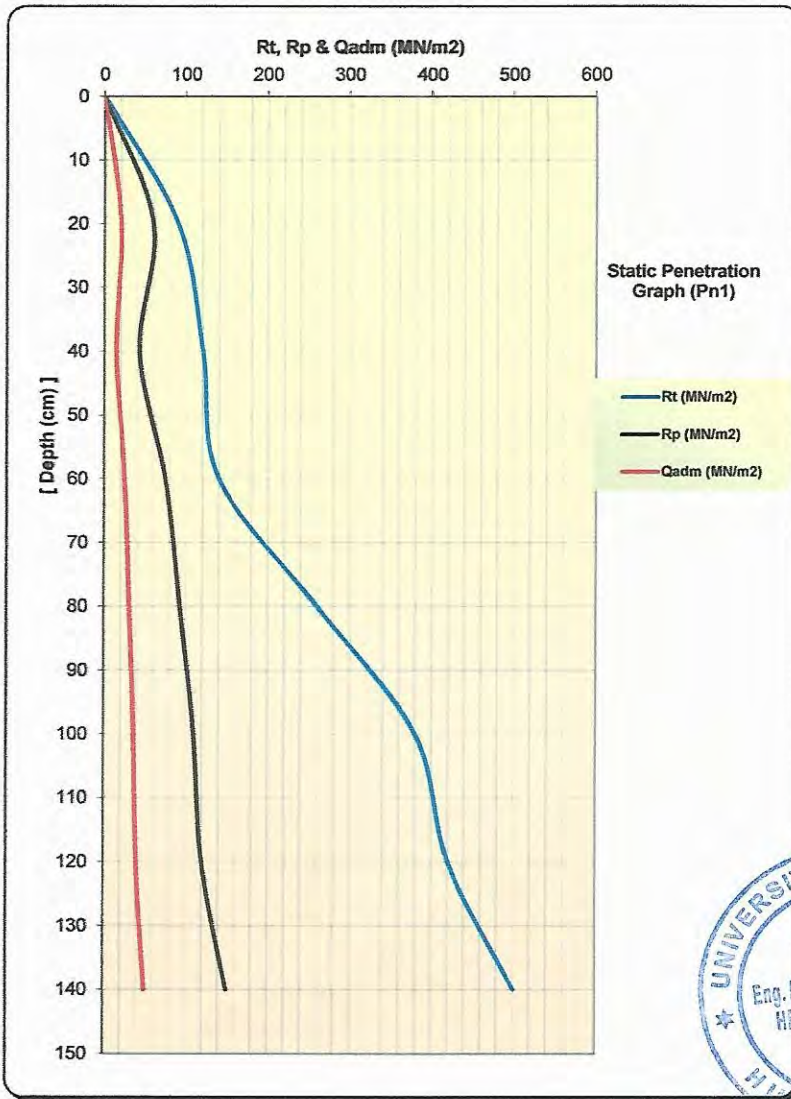
Depth (cm)	Rt (MN/m <sup>2</sup> )	Rp (MN/m <sup>2</sup> )	Qadm (MN/m <sup>2</sup> )	Qadm (kg/cm <sup>2</sup> )
0	0	0	0.00	0.00
20	80	55	18.33	186.95
40	125	45	15.00	152.96
60	300	100	33.33	339.91
80	370	120	40.00	407.89
100	400	140	46.67	475.87
120	500	150	50.00	509.86
140				



**PROJECT: IMPROVEMENT OF SUBSTATION & DISTRIBUTION NETWORK - PHASE III**  
**SUB PROJECT: 15 KV Distribution line Gasogi Substation - Masaka Hospital**

**Client:** PITRAD IBAMBA LTD.  
 Operated by: Eng. Venerand GWIZA  
 Verified by: Eng. Vivien MUNYABURANGA (MSc)  
 Date: July, 2017  
 Penetration Test N° AP 16

Depth (cm)	Rt (MN/m <sup>2</sup> )	Rp (MN/m <sup>2</sup> )	Qadm (MN/m <sup>2</sup> )	Qadm (kg/cm <sup>2</sup> )
0	0	0	0.00	0.00
20	90	60	20.00	203.94
40	120	43	14.33	146.16
60	140	75	25.00	254.93
80	260	93	31.00	316.11
100	380	110	36.67	373.90
120	420	120	40.00	407.89
140	500	150	50.00	509.86
160				



**PROJECT:** IMPROVEMENT OF SUBSTATION & DISTRIBUTION NETWORK - PHASE III  
**SUB PROJECT:** 15 KV Distribution line Gasogi Substation - Masaka Hospital

**Client:** PITRAD IBAMBA LTD.

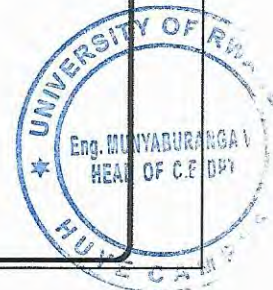
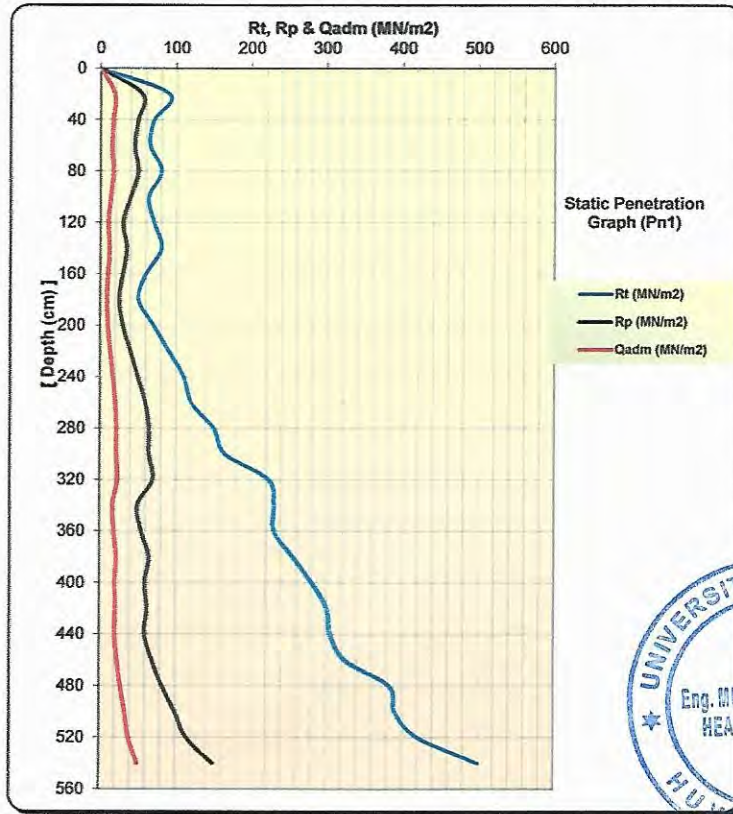
Operated by: Eng. Venerand GWIZA

Verified by: Eng. Vivien MUNYABURANGA (MSc)

Date: July, 2017

Penetration Test N° AP 17

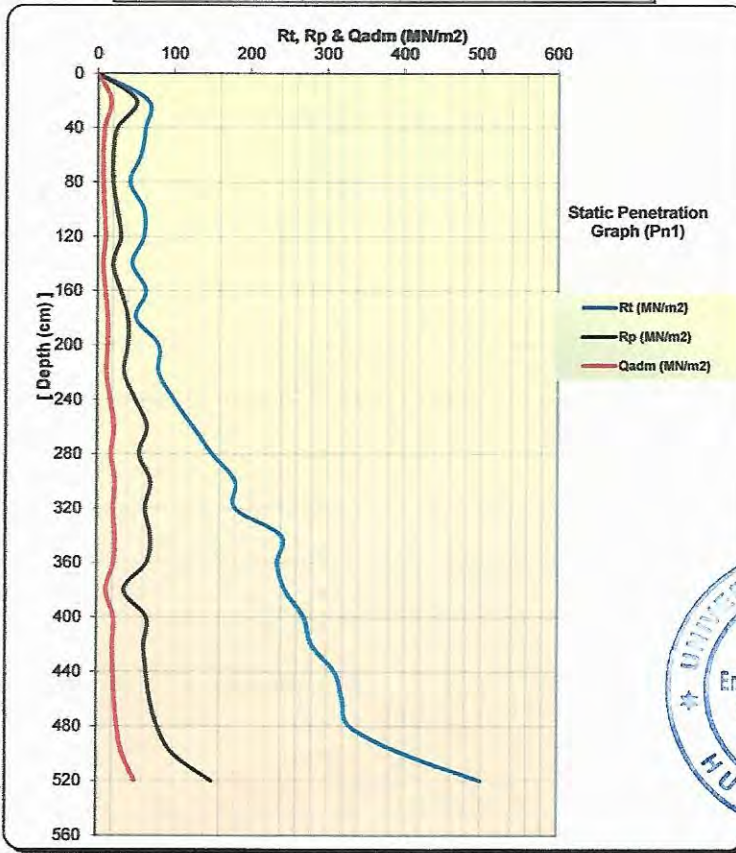
Depth (cm)	Rt (MN/m2)	Rp (MN/m2)	Qadm (MN/m2)	Qadm (kg/cm2)
0	0	0	0.00	0.00
20	90	55	18.33	186.95
40	70	50	16.67	169.95
60	85	45	15.00	152.96
80	80	50	16.67	169.95
100	63	40	13.33	135.96
120	70	30	10.00	101.97
140	80	35	11.67	118.97
160	60	30	10.00	101.97
180	50	25	8.33	84.98
200	70	30	10.00	101.97
220	90	40	13.33	135.96
240	110	50	16.67	169.95
260	120	60	20.00	203.94
280	150	65	21.67	220.94
300	165	65	21.67	220.94
320	223	70	23.33	237.93
340	230	50	16.67	169.95
360	230	55	18.33	186.95
380	255	65	21.67	220.94
400	280	60	20.00	203.94
420	300	63	21.00	214.14
440	305	60	20.00	203.94
460	325	70	23.33	237.93
480	383	83	27.67	282.12
500	390	100	33.33	339.91
520	420	115	38.33	390.89
540	500	150	50.00	509.86
560				



**PROJECT: IMPROVEMENT OF SUBSTATION & DISTRIBUTION NETWORK - PHASE III**  
**SUB PROJECT: 15 KV Distribution line Gasogi Substation - Masaka Hospital**

**Client:** PITRAD IBAMBA LTD.  
 Operated by: Eng. Venerand GWIZA  
 Verified by: Eng. Vivien MUNYABURANGA (MSc)  
 Date: July, 2017  
 Penetration Test **N° AP 18**

Depth (cm)	Rt (MN/m <sup>2</sup> )	Rp (MN/m <sup>2</sup> )	Qadm (MN/m <sup>2</sup> )	Qadm (kg/cm <sup>2</sup> )
0	0	0	0.00	0.00
20	65	50	16.67	169.95
40	62	25	8.33	84.98
60	56	20	6.67	67.98
80	42	20	6.67	67.98
100	60	25	8.33	84.98
120	60	30	10.00	101.97
140	45	20	6.67	67.98
160	63	30	10.00	101.97
180	50	40	13.33	135.96
200	80	40	13.33	135.96
220	80	35	11.67	118.97
240	100	50	16.67	169.95
260	125	65	21.67	220.94
280	150	55	18.33	186.95
300	180	70	23.33	237.93
320	180	63	21.00	214.14
340	240	70	23.33	237.93
360	235	65	21.67	220.94
380	245	35	11.67	118.97
400	270	65	21.67	220.94
420	280	62	20.67	210.74
440	310	65	21.67	220.94
460	320	70	23.33	237.93
480	330	80	26.67	271.93
500	400	100	33.33	339.91
520	500	150	50.00	509.86
540				





**PROJECT:** IMPROVEMENT OF SUBSTATION & DISTRIBUTION NETWORK - PHASE III  
**SUB PROJECT:** 15 KV Distribution line Gasogi Substation - Masaka Hospital

**Client:** PITRAD IBAMBA LTD.

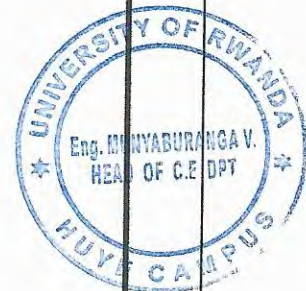
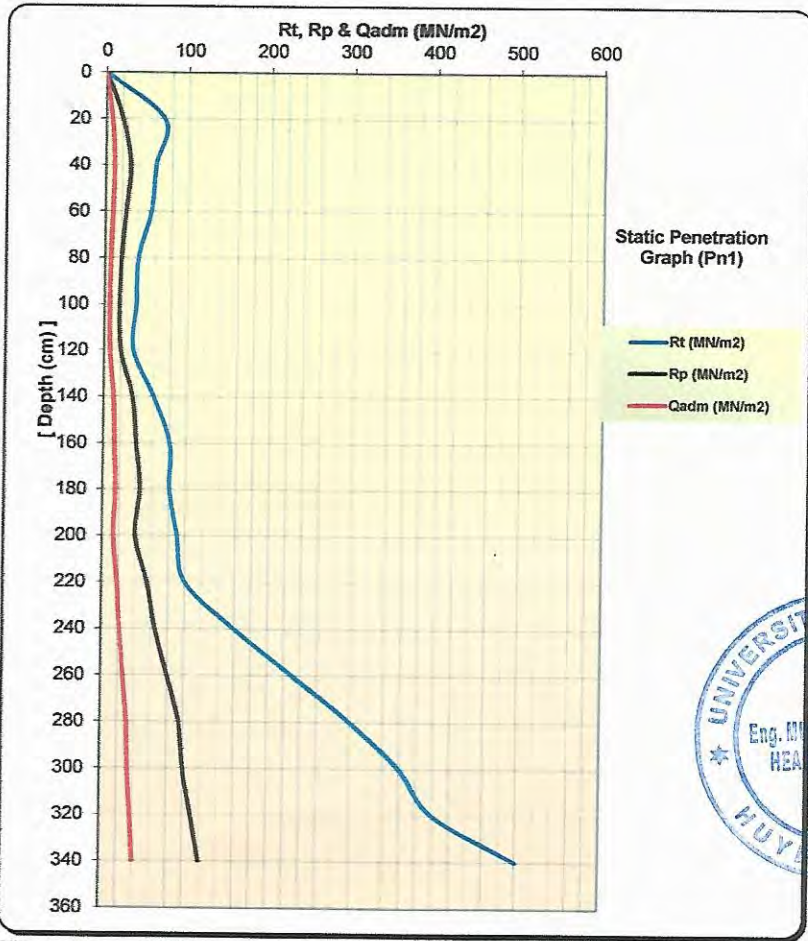
Operated by: Eng. Venerand GWIZA

Verified by: Eng. Vivien MUNYABURANGA (MSc)

Date: July, 2017

Penetration Test **N° AP 19**

Depth (cm)	Rt (MN/m2)	Rp (MN/m2)	Qadm (MN/m2)	Qadm (kg/cm2)
0	0	0	0.00	0.00
20	70	20	6.67	67.98
40	60	30	10.00	101.97
60	55	25	8.33	84.98
80	40	20	6.67	67.98
100	38	18	6.00	61.18
120	35	20	6.67	67.98
140	60	35	11.67	118.97
160	80	40	13.33	135.96
180	80	45	15.00	152.96
200	90	40	13.33	135.96
220	100	55	18.33	186.95
240	160	65	21.67	220.94
260	230	80	26.67	271.93
280	300	95	31.67	322.91
300	360	100	33.33	339.91
320	400	110	36.67	373.90
340	500	120	40.00	407.89
360				



UNIVERSITY OF RWANDA - COLLEGE OF SCIENCE AND TECHNOLOGY (Huye Campus)  
 SCHOOL OF ENGINEERING  
 CIVIL ENGINEERING DEPARTMENT  
 LABORATORY OF GEOTECHNICS

**PROJECT: IMPROVEMENT OF SUBSTATION & DISTRIBUTION NETWORK - PHASE III**

**SUB PROJECT: Reconstruction of transmission 110 KV Line Jabana - Birembo**

**Client:** PITRAD IBAMBA LTD.

Operated by: Eng. Venerand GWIZA

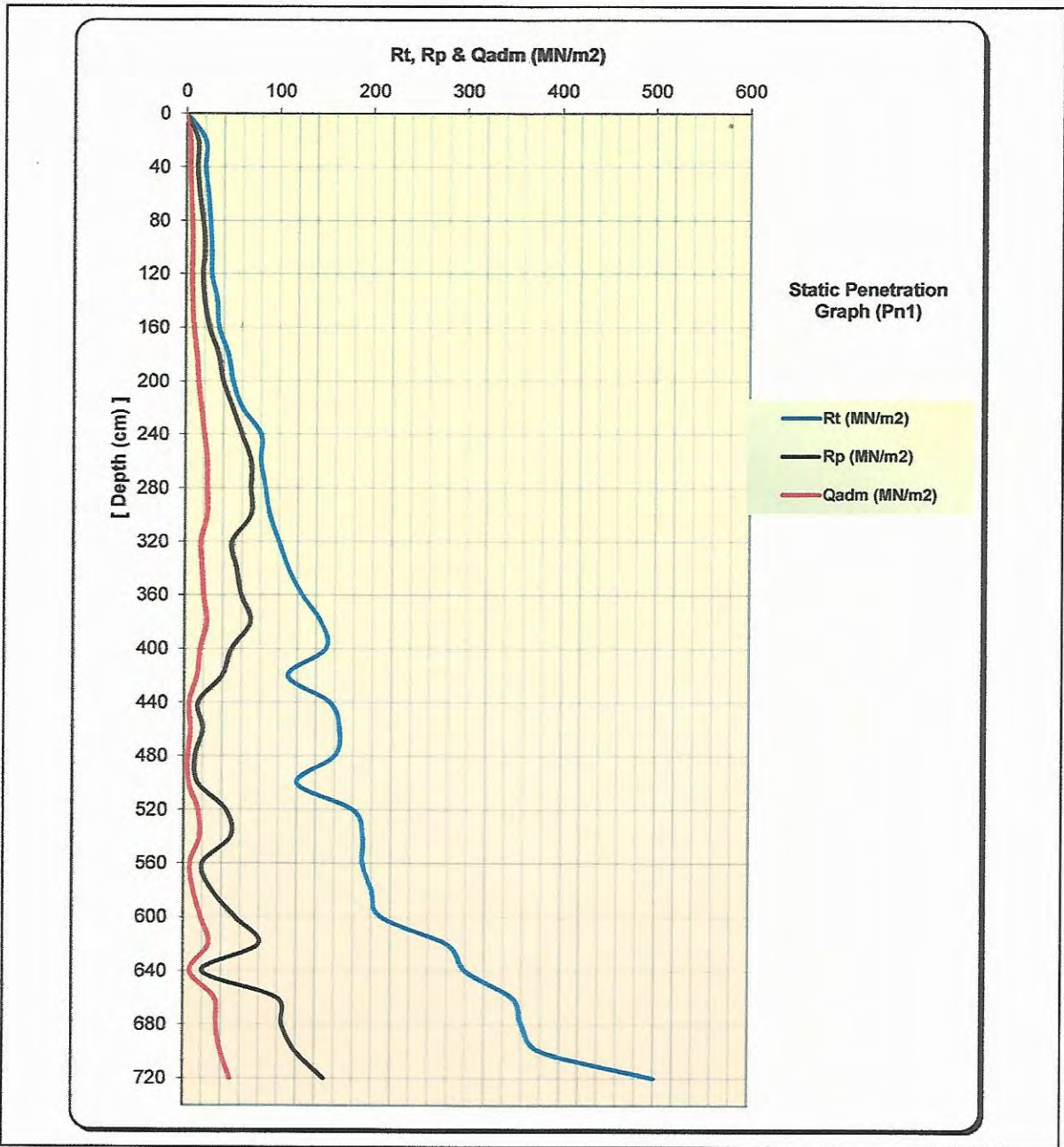
Verified by: Eng. Vivien MUNYABURANGA (MSc)

**Date:** July, 2017

Penetration Test **N° 176**

Depth (cm)	Rt (MN/m <sup>2</sup> )	Rp (MN/m <sup>2</sup> )	Qadm (MN/m <sup>2</sup> )	Qadm (kg/cm <sup>2</sup> )
0	0	0	0.00	0.00
20	20	12	4.00	40.79
40	20	12	4.00	40.79
60	23	14	4.67	47.59
80	25	18	6.00	61.18
100	27	20	6.67	67.98
120	27	18	6.00	61.18
140	33	20	6.67	67.98
160	35	25	8.33	84.98
180	45	35	11.67	118.97
200	50	40	13.33	135.96
220	60	50	16.67	169.95
240	80	60	20.00	203.94
260	80	70	23.33	237.93
280	85	70	23.33	237.93
300	90	70	23.33	237.93
320	100	50	16.67	169.95
340	110	55	18.33	186.95
360	125	60	20.00	203.94
380	145	70	23.33	237.93
400	150	50	16.67	169.95
420	110	40	13.33	135.96
440	155	15	5.00	50.99
460	165	20	6.67	67.98
480	160	12	4.00	40.79
500	120	15	5.00	50.99
520	180	45	15.00	152.96
540	190	50	16.67	169.95
560	190	20	6.67	67.98
580	200	30	10.00	101.97
600	210	55	18.33	186.95
620	280	80	26.67	271.93
640	300	20	6.67	67.98
660	350	100	33.33	339.91
680	360	105	35.00	356.90
700	380	120	40.00	407.89
720	500	150	50.00	509.86
740				





UNIVERSITY OF RWANDA - COLLEGE OF SCIENCE AND TECHNOLOGY (Huye Campus)  
 SCHOOL OF ENGINEERING  
 CIVIL ENGINEERING DEPARTMENT  
 LABORATORY OF GEOTECHNICS

**PROJECT:** IMPROVEMENT OF SUBSTATION & DISTRIBUTION NETWORK - PHASE III

**SUB PROJECT:** Reconstruction of transmission 110 KV Line Jabana - Birembo

**Client:** PITRAD IBAMBA LTD.

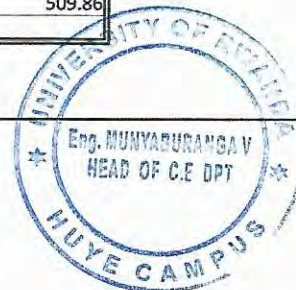
Operated by: Eng. Venerand GWIZA

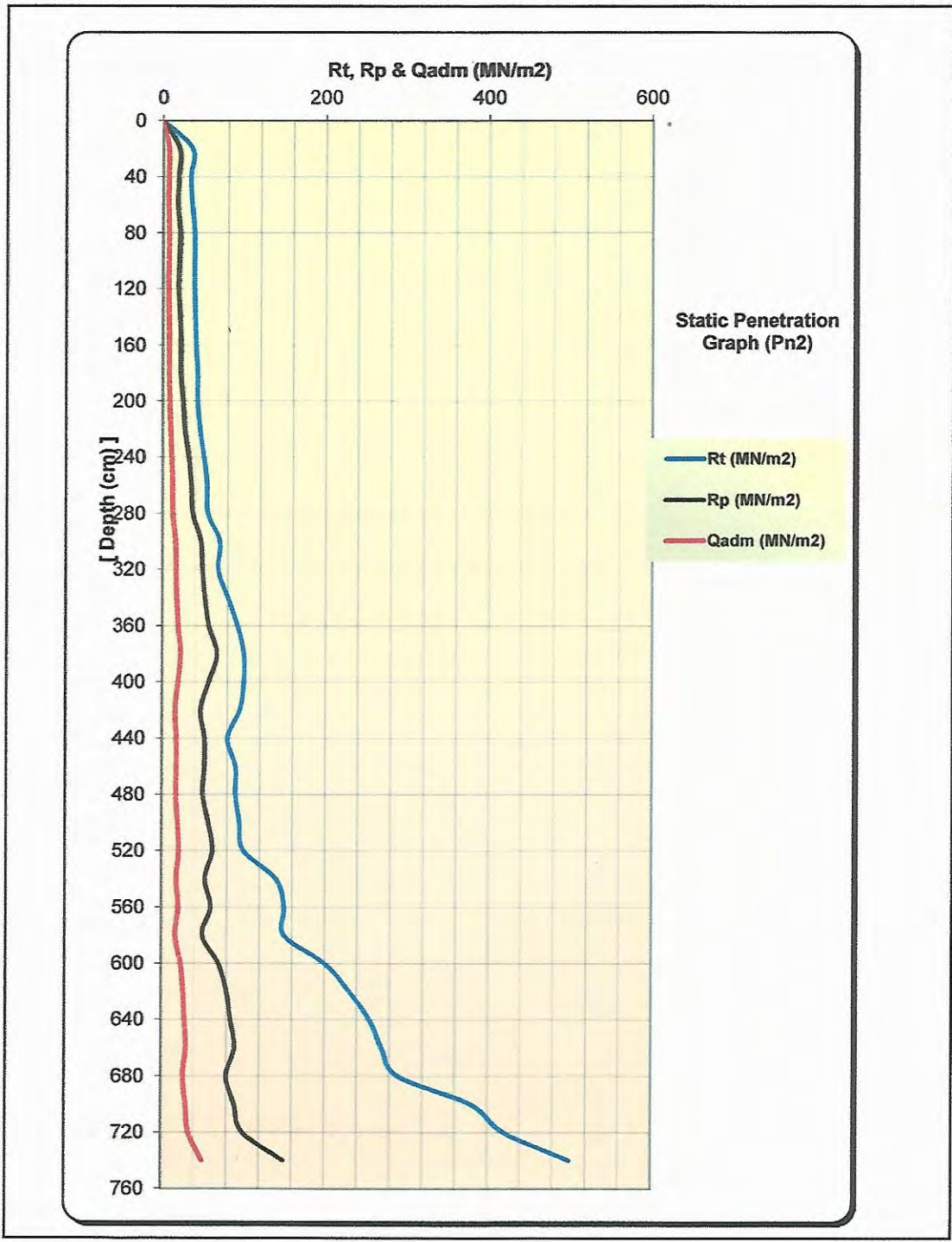
Verified by: Eng. Vivien MUNYABURANGA (MSc)

**Date:** July, 2017

Penetration Test **N° 177**

Depth (cm)	Rt (MN/m2)	Rp (MN/m2)	Qadm (MN/	Qadm (kg/cm2)
0	0	0	0.00	0.00
20	35	20	6.67	67.98
40	33	19	6.33	64.58
60	35	18	6.00	61.18
80	38	21	7.00	71.38
100	38	20	6.67	67.98
120	38	19	6.33	64.58
140	39	21	7.00	71.38
160	40	22	7.33	74.78
180	42	22	7.33	74.78
200	42	25	8.33	84.98
220	45	27	9.00	91.77
240	50	32	10.67	108.77
260	54	35	11.67	118.97
280	55	37	12.33	125.77
300	70	47	15.67	159.76
320	68	49	16.33	166.55
340	80	52	17.33	176.75
360	92	57	19.00	193.75
380	100	67	22.33	227.74
400	100	57	19.00	193.75
420	95	47	15.67	159.76
440	80	52	17.33	176.75
460	90	52	17.33	176.75
480	90	50	16.67	169.95
500	95	57	19.00	193.75
520	100	62	20.67	210.74
540	140	53	17.67	180.15
560	150	60	20.00	203.94
580	150	50	16.67	169.95
600	200	70	23.33	237.93
620	230	80	26.67	271.93
640	255	85	28.33	288.92
660	270	90	30.00	305.92
680	290	80	26.67	271.93
700	380	90	30.00	305.92
720	420	100	33.33	339.91
740	500	150	50.00	509.86
760				





UNIVERSITY OF RWANDA - COLLEGE OF SCIENCE AND TECHNOLOGY (Huye Campus)  
 SCHOOL OF ENGINEERING  
 CIVIL ENGINEERING DEPARTMENT  
 LABORATORY OF GEOTECHNICS

**PROJECT:** IMPROVEMENT OF SUBSTATION & DISTRIBUTION NETWORK - PHASE III

**SUB PROJECT:** Reconstruction of transmission 110 KV Line Jabana - Birembo

**Client:** PITRAD IBAMBA LTD.

Operated by: Eng. Venerand GWIZA

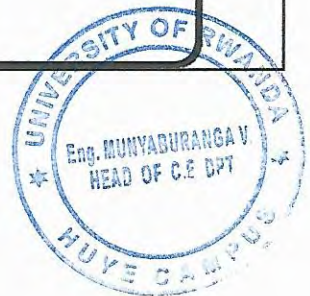
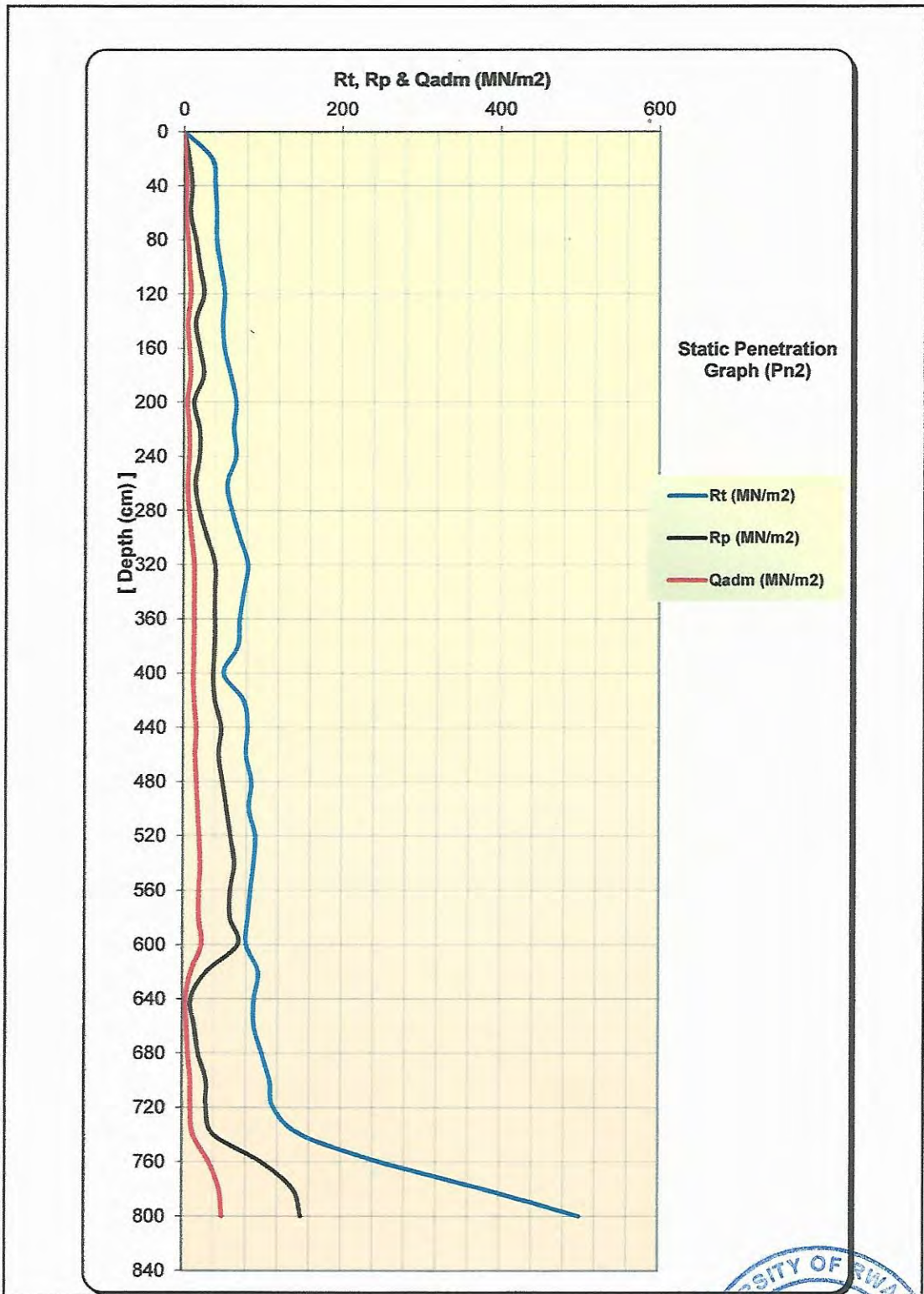
Verified by: Eng. Vivien MUNYABURANGA (MSc)

**Date:** July, 2017

Penetration Test **N° 178**

Depth (cm)	Rt (MN/m <sup>2</sup> )	Rp (MN/m <sup>2</sup> )	Qadm (MN/)	Qadm (kg/cm <sup>2</sup> )
0	0	0	0.00	0.00
20	35	6	2.00	20.39
40	39	10	3.33	33.99
60	41	8	2.67	27.19
80	41	15	5.00	50.99
100	46	20	6.67	67.98
120	51	25	8.33	84.98
140	49	15	5.00	50.99
160	51	20	6.67	67.98
180	59	25	8.33	84.98
200	66	13	4.33	44.19
220	63	20	6.67	67.98
240	66	20	6.67	67.98
260	55	15	5.00	50.99
280	61	20	6.67	67.98
300	71	30	10.00	101.97
320	81	40	13.33	135.96
340	76	40	13.33	135.96
360	71	40	13.33	135.96
380	69	40	13.33	135.96
400	51	38	12.67	129.16
420	76	40	13.33	135.96
440	81	48	16.00	163.16
460	79	45	15.00	152.96
480	86	50	16.67	169.95
500	83	55	18.33	186.95
520	91	60	20.00	203.94
540	89	65	21.67	220.94
560	85	60	20.00	203.94
580	82	60	20.00	203.94
600	80	70	23.33	237.93
620	95	30	10.00	101.97
640	90	10	3.33	33.99
660	90	15	5.00	50.99
680	100	20	6.67	67.98
700	110	30	10.00	101.97
720	115	30	10.00	101.97
740	150	40	13.33	135.96
760	250	100	33.33	339.91
780	380	140	46.67	475.87
800	500	150	50.00	509.86
820				





**PROJECT: IMPROVEMENT OF SUBSTATION & DISTRIBUTION NETWORK - PHASE III**

**SUB PROJECT: Reconstruction of transmission 110 KV Line Jabana - Birembo**

**Client:** PITRAD IBAMBA LTD.

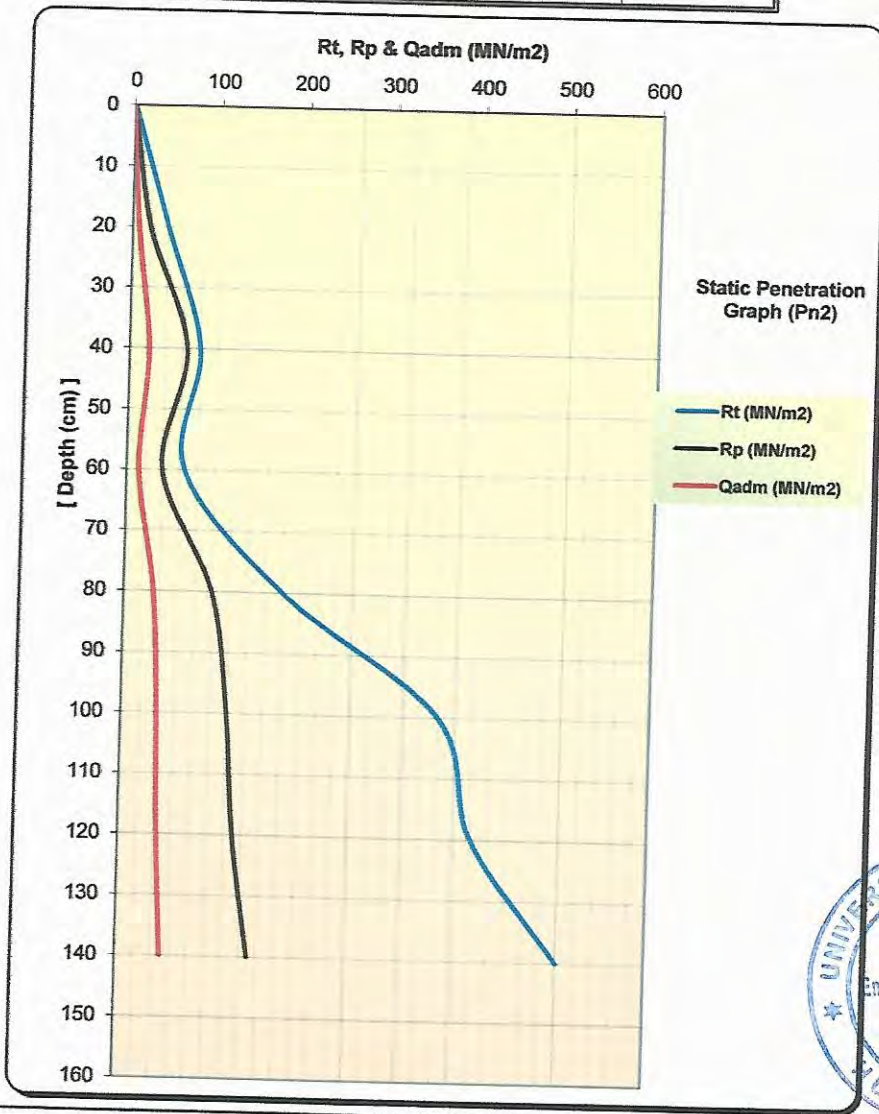
Operated by: Eng. Venerand GWIZA

Verified by: Eng. Vivien MUNYABURANGA (MSc)

**Date: July, 2017**

Penetration Test **N° 180**

Depth (cm)	Rt (MN/m <sup>2</sup> )	Rp (MN/m <sup>2</sup> )	Qadm (MN/m <sup>2</sup> )	Qadm (kg/cm <sup>2</sup> )
0	0	0	0	0.00
20	40	20	6.67	67.98
40	80	65	21.67	220.94
60	65	40	13.33	135.96
80	180	100	33.33	339.91
100	360	120	40.00	407.89
120	400	130	43.33	441.88
140	500	150	50.00	509.86
160				





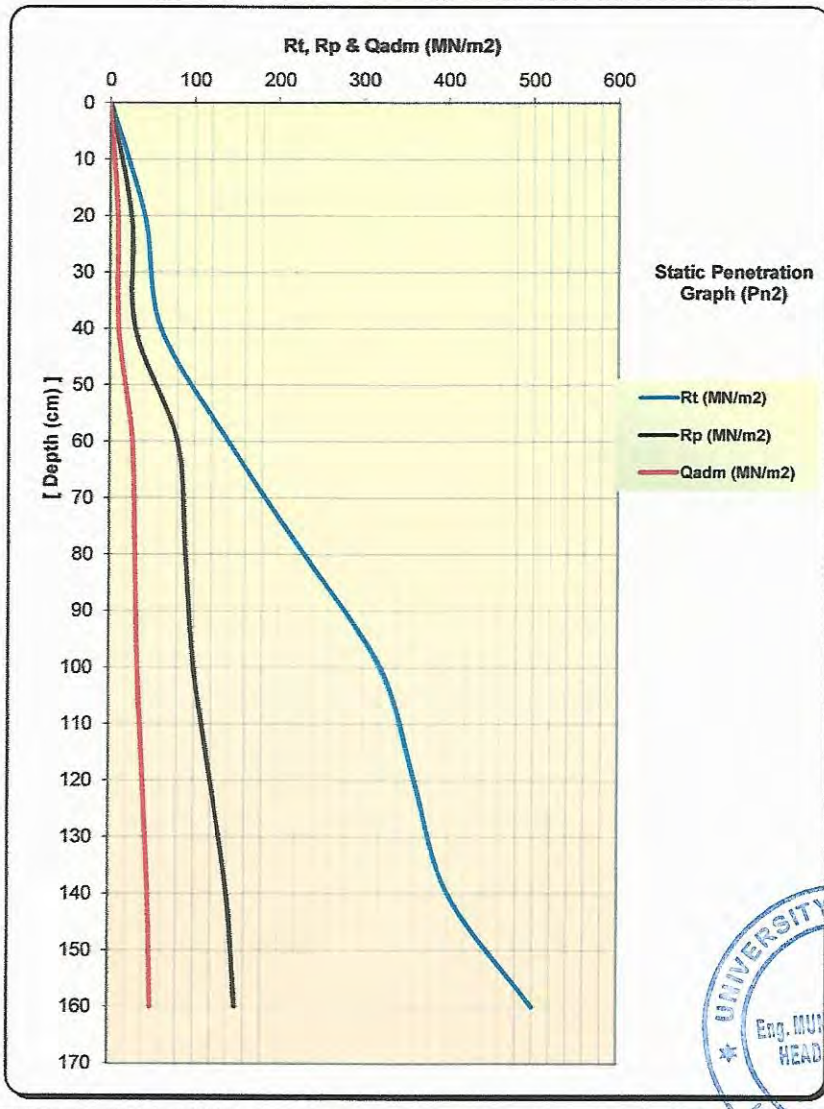
UNIVERSITY OF RWANDA - COLLEGE OF SCIENCE AND TECHNOLOGY (Huye Campus)  
 SCHOOL OF ENGINEERING  
 CIVIL ENGINEERING DEPARTMENT  
 LABORATORY OF GEOTECHNICS

**PROJECT:** IMPROVEMENT OF SUBSTATION & DISTRIBUTION NETWORK - PHASE III  
**SUB PROJECT:** Reconstruction of transmission 110 KV Line Jabana - Birembo

**Client:** PITRAD IBAMBA LTD.  
 Operated by: Eng. Venerand GWIZA  
 Verified by: Eng. Vivien MUNYABURANGA (MSc)  
 Date: July, 2017

Penetration Test N° 181

Depth (cm)	Rt (MN/m <sup>2</sup> )	Rp (MN/m <sup>2</sup> )	Qadm (MN/m <sup>2</sup> )	Qadm (kg/cm <sup>2</sup> )
0	0	0	0.00	0.00
20	40	25	8.33	84.98
40	60	30	10.00	101.97
60	140	80	26.67	271.93
80	230	90	30.00	305.92
100	320	100	33.33	339.91
120	360	120	40.00	407.89
140	400	140	46.67	475.87
160	500	150	50.00	509.86
180				



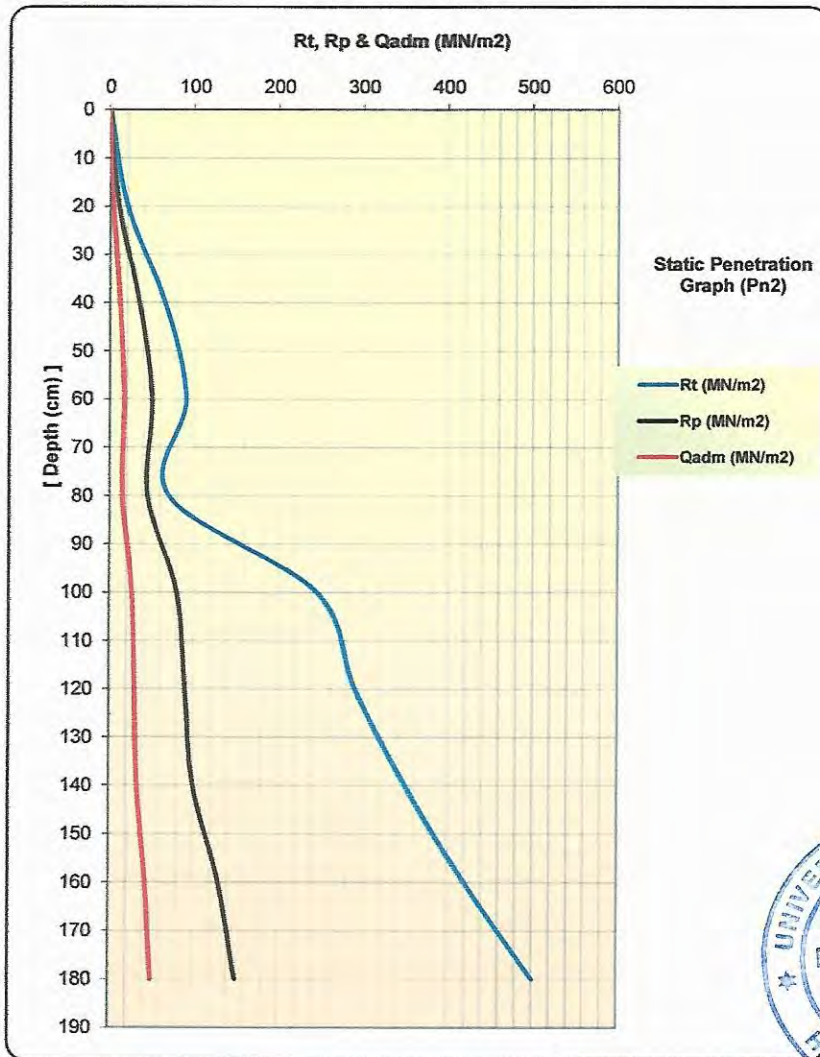
**PROJECT: IMPROVEMENT OF SUBSTATION & DISTRIBUTION NETWORK - PHASE III**  
**SUB PROJECT: Reconstruction of transmission 110 KV Line Jabana - Birembo**

**Client:** PITRAD IBAMBA LTD.  
 Operated by: Eng. Venerand GWIZA  
 Verified by: Eng. Vivien MUNYABURANGA (MSc)

**Date:** July, 2017

Penetration Test **N° 182**

Depth (cm)	Rt (MN/m2)	Rp (MN/m2)	Qadm (MN/m2)	Qadm (kg/cm2)
0	0	0	0.00	0.00
20	20	10	3.33	33.99
40	65	35	11.67	118.97
60	90	50	16.67	169.95
80	70	45	15.00	152.96
100	245	80	26.67	271.93
120	290	90	30.00	305.92
140	350	100	33.33	339.91
160	420	130	43.33	441.88
180	500	150	50.00	509.86
200				



UNIVERSITY OF RWANDA - COLLEGE OF SCIENCE AND TECHNOLOGY (Huye Campus)  
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 LABORATORY OF GEOTECHNICS

**PROJECT:** IMPROVEMENT OF SUBSTATION & DISTRIBUTION NETWORK - PHASE III

**SUB PROJECT:** Reconstruction of transmission 110 KV Line Jabana - Birembo

**Client:** PITRAD IBAMBA LTD.

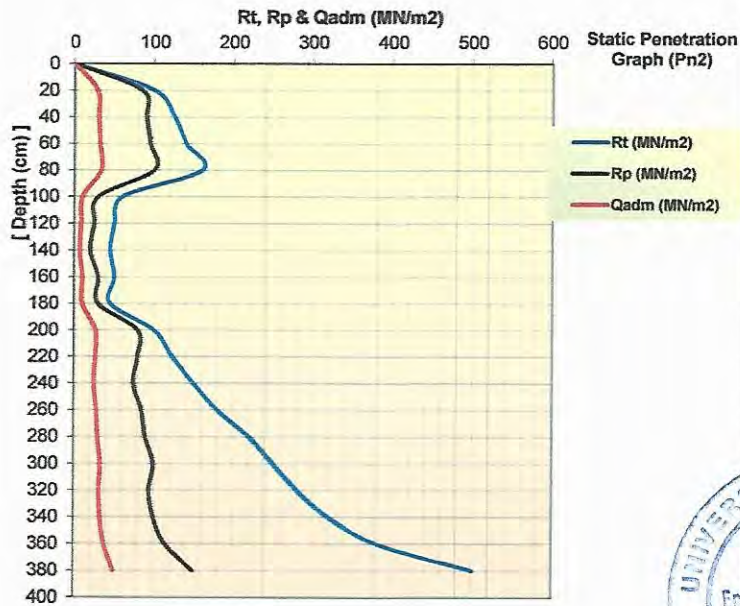
Operated by: Eng. Venerand GWIZA

Verified by: Eng. Vivien MUNYABURANGA (MSc)

Date: July, 2017

Penetration Test N° 184

Depth (cm)	Rt (MN/m2)	Rp (MN/m2)	Qadm (MN/m2)	Qadm (kg/cm2)
0	0	0	0.00	0.00
20	100	85	28.33	288.92
40	125	90	30.00	305.92
60	140	95	31.67	322.91
80	160	100	33.33	339.91
100	60	30	10.00	101.97
120	50	25	8.33	84.98
140	45	20	6.67	67.98
160	50	30	10.00	101.97
180	45	30	10.00	101.97
200	100	80	26.67	271.93
220	123	80	26.67	271.93
240	150	75	25.00	254.93
260	180	85	28.33	288.92
280	220	90	30.00	305.92
300	250	100	33.33	339.91
320	280	95	31.67	322.91
340	320	100	33.33	339.91
360	380	115	38.33	390.89
380	500	150	50.00	509.86
400				



**PROJECT: IMPROVEMENT OF SUBSTATION & DISTRIBUTION NETWORK - PHASE III**

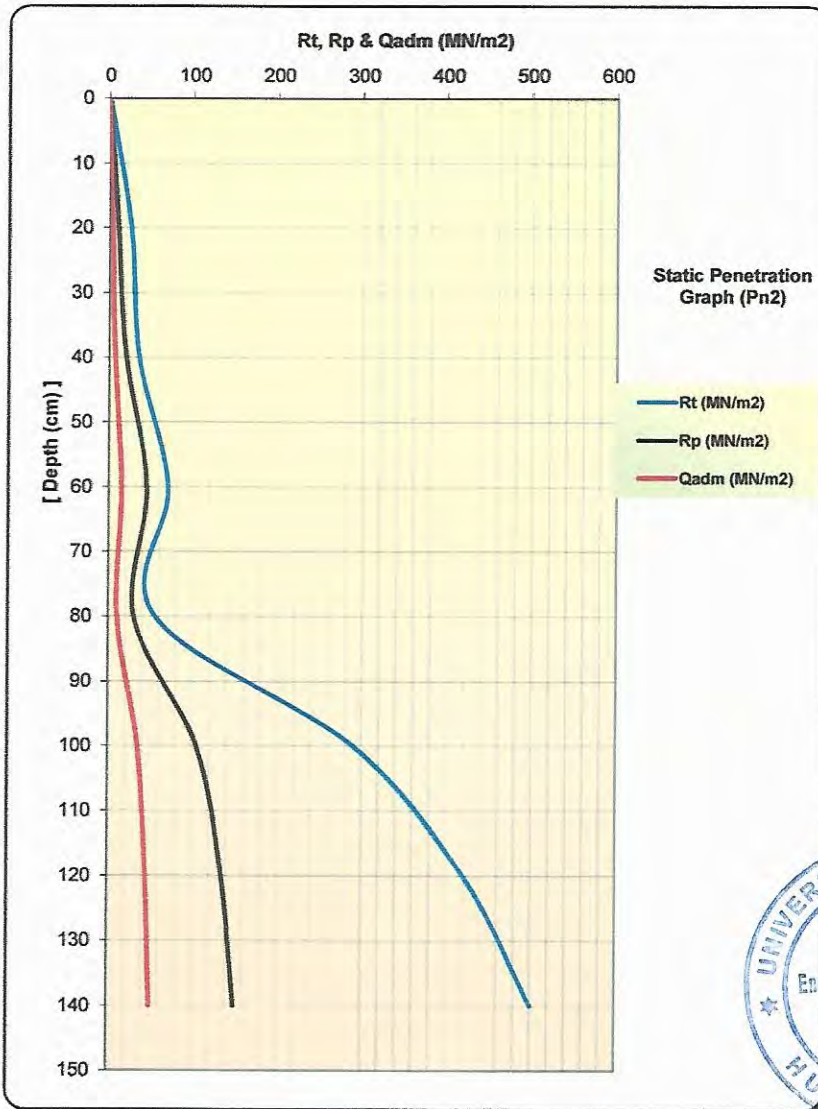
**SUB PROJECT: Reconstruction of transmission 110 KV Line Jabana - Birembo**

**Client:** PITRAD IBAMBA LTD.  
 Operated by: Eng. Venerand GWIZA  
 Verified by: Eng. Vivien MUNYABURANGA (MSc)

**Date:** July, 2017

Penetration Test **N° 186**

Depth (cm)	Rt (MN/m <sup>2</sup> )	Rp (MN/m <sup>2</sup> )	Qadm (MN/m <sup>2</sup> )	Qadm (kg/cm <sup>2</sup> )
0	0	0	0.00	0.00
20	25	10	3.33	33.99
40	35	20	6.67	67.98
60	70	45	15.00	152.96
80	55	30	10.00	101.97
100	290	105	35.00	356.90
120	420	135	45.00	458.87
140	500	150	50.00	509.86
160				



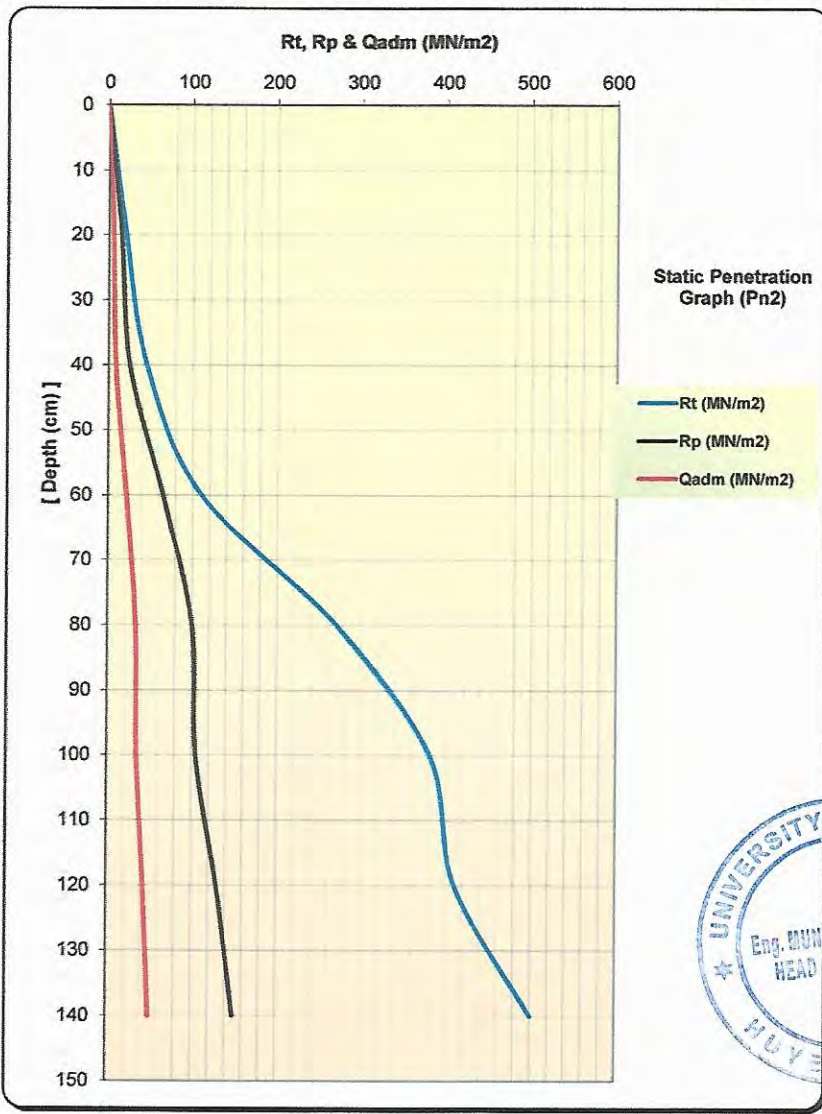
UNIVERSITY OF RWANDA - COLLEGE OF SCIENCE AND TECHNOLOGY (Huye Campus)  
 SCHOOL OF ENGINEERING  
 CIVIL ENGINEERING DEPARTMENT  
 LABORATORY OF GEOTECHNICS

PROJECT: IMPROVEMENT OF SUBSTATION & DISTRIBUTION NETWORK - PHASE III  
 SUB PROJECT: Reconstruction of transmission 110 KV Line Jabana - Birembo

Client: PITRAD IBAMBA LTD.  
 Operated by: Eng. Venerand GWIZA  
 Verified by: Eng. Vivien MUNYABURANGA (MSc)  
 Date: July, 2017

Penetration Test N° 187

Depth (cm)	Rt (MN/m <sup>2</sup> )	Rp (MN/m <sup>2</sup> )	Qadm (MN/m <sup>2</sup> )	Qadm (kg/cm <sup>2</sup> )
0	0	0	0.00	0.00
20	20	15	5.00	50.99
40	45	25	8.33	84.98
60	110	65	21.67	220.94
80	270	100	33.33	339.91
100	380	105	35.00	356.90
120	410	130	43.33	441.88
140	500	150	50.00	509.86
160				



**PROJECT: IMPROVEMENT OF SUBSTATION & DISTRIBUTION NETWORK - PHASE III**

**SUB PROJECT: Reconstruction of transmission 110 KV Line Jabana - Birembo**

**Client:** PITRAD IBAMBA LTD.

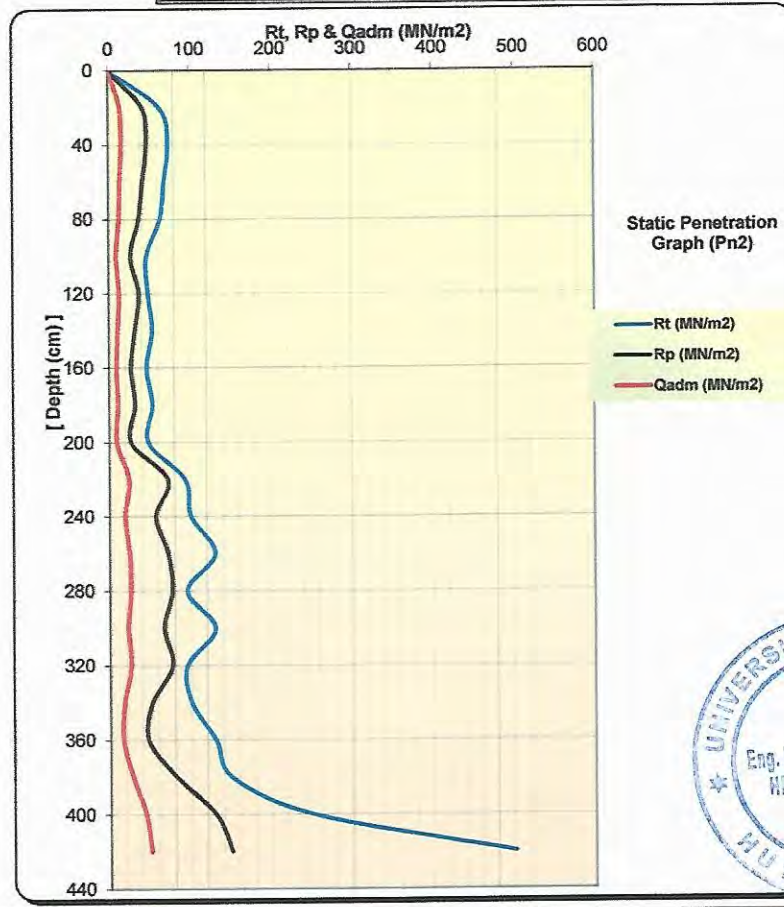
Operated by: Eng. Venerand GWIZA

Verified by: Eng. Vivien MUNYABURANGA (MSc)

Date: July, 2017

**Penetration Test N° 189**

Depth (cm)	Rt (MN/m <sup>2</sup> )	Rp (MN/m <sup>2</sup> )	Qadm (MN/m <sup>2</sup> )	Qadm (kg/cm <sup>2</sup> )
0	0	0	0.00	0.00
20	63	42	14.00	142.76
40	73	47	15.67	159.76
60	68	42	14.00	142.76
80	63	37	12.33	125.77
100	46	27	9.00	91.77
120	48	37	12.33	125.77
140	53	32	10.67	108.77
160	46	27	9.00	91.77
180	53	32	10.67	108.77
200	48	27	9.00	91.77
220	93	72	24.00	244.73
240	100	57	19.00	193.75
260	130	72	24.00	244.73
280	95	77	25.67	261.73
300	130	67	22.33	227.74
320	95	77	25.67	261.73
340	100	52	17.33	176.75
360	130	47	15.67	159.76
380	150	82	27.33	278.72
400	250	130	43.33	441.88
420	500	150	50.00	509.86
440				



**PROJECT: IMPROVEMENT OF SUBSTATION & DISTRIBUTION NETWORK - PHASE III**

**SUB PROJECT: Reconstruction of transmission 110 KV Line Jabana - Birembo**

**Client:** PITRAD IBAMBA LTD.

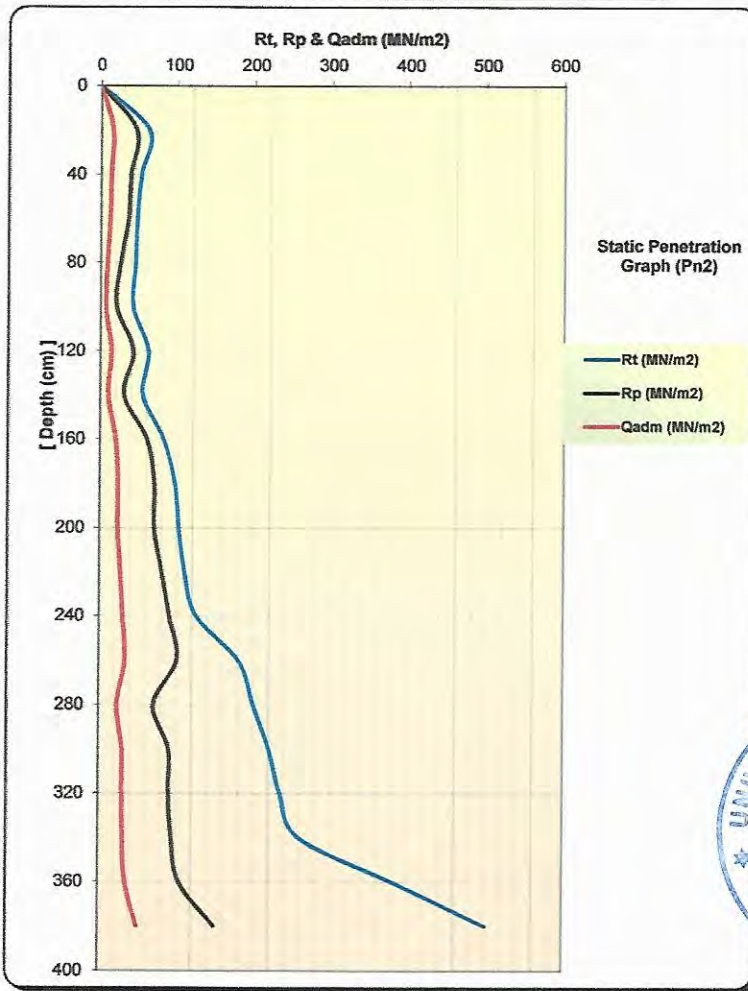
Operated by: Eng. Venerand GWIZA

Verified by: Eng. Vivien MUNYABURANGA (MSc)

**Date:** July, 2017

Penetration Test **N°190**

Depth (cm)	Rt (MN/m <sup>2</sup> )	Rp (MN/m <sup>2</sup> )	Qadm (MN/m <sup>2</sup> )	Qadm (kg/cm <sup>2</sup> )
0	0	0	0.00	0.00
20	62	46	15.33	156.36
40	52	39	13.00	132.56
60	47	36	12.00	122.37
80	45	26	8.67	88.38
100	42	21	7.00	71.38
120	62	43	14.33	146.16
140	55	31	10.33	105.37
160	82	61	20.33	207.34
180	97	71	23.67	241.33
200	102	71	23.67	241.33
220	110	81	27.00	275.32
240	125	91	30.33	309.32
260	180	101	33.67	343.31
280	200	71	23.67	241.33
300	220	91	30.33	309.32
320	235	91	30.33	309.32
340	260	95	31.67	322.91
360	380	105	35.00	356.90
380	500	150	50.00	509.86
400				



UNIVERSITY OF RWANDA - COLLEGE OF SCIENCE AND TECHNOLOGY (Huye Campus)  
 SCHOOL OF ENGINEERING  
 CIVIL ENGINEERING DEPARTMENT  
 LABORATORY OF GEOTECHNICS

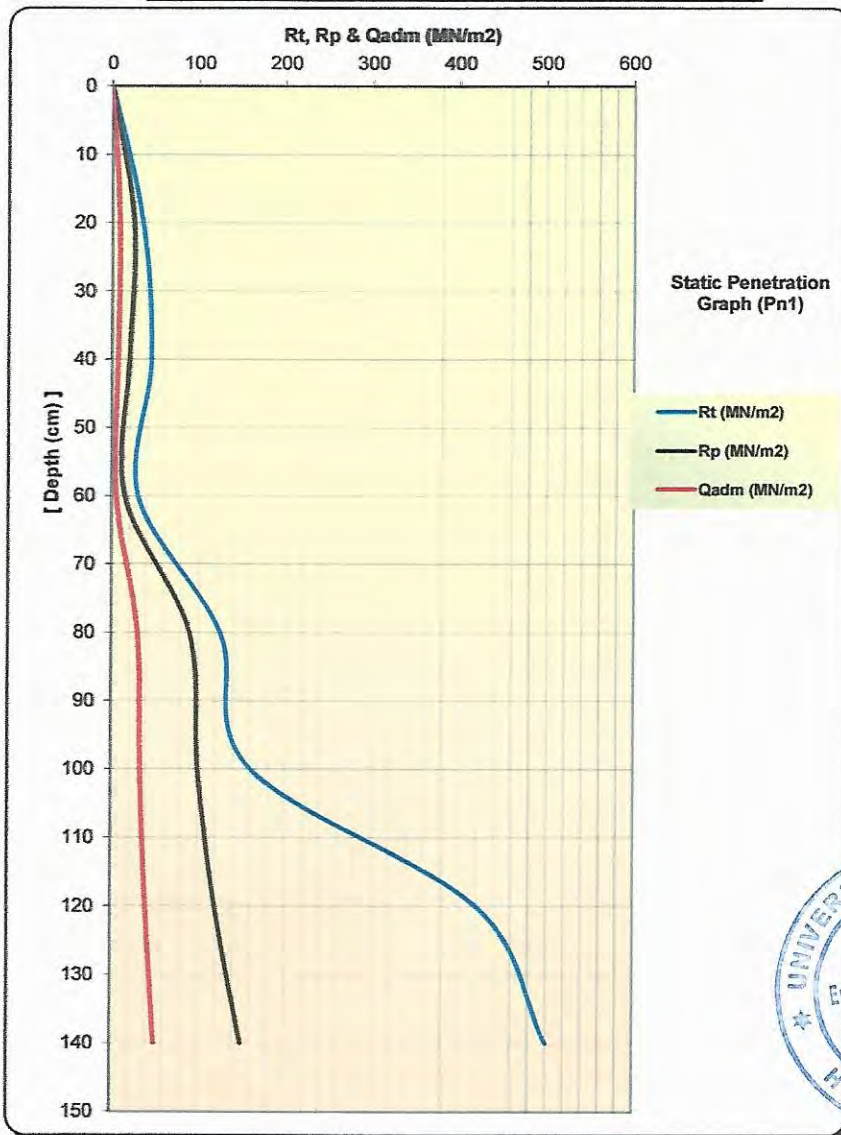
**PROJECT:** IMPROVEMENT OF SUBSTATION & DISTRIBUTION NETWORK - PHASE III  
**SUB PROJECT:** Reconstruction of transmission 110 KV Line Jabana - Biremba

**Client:** PITRAD IBAMBA LTD.  
 Operated by: Eng. Venerand GWIZA  
 Verified by: Eng. Vivien MUNYABURANGA (MSc)

**Date:** July, 2017

Penetration Test **N° 195**

Depth (cm)	Rt (MN/m <sup>2</sup> )	Rp (MN/m <sup>2</sup> )	Qadm (MN/m <sup>2</sup> )	Qadm (kg/cm <sup>2</sup> )
0	0	0	0.00	0.00
20	35	25	8.33	84.98
40	45	20	6.67	67.98
60	30	15	5.00	50.99
80	125	90	30.00	305.92
100	160	100	33.33	339.91
120	420	120	40.00	407.89
140	500	150	50.00	509.86
160				





**PROJECT:** IMPROVEMENT OF SUBSTATION & DISTRIBUTION NETWORK - PHASE III

**SUB PROJECT:** Reconstruction of transmission 110 KV Line Jabana - Birembo

**Client:** PITRAD IBAMBA LTD.

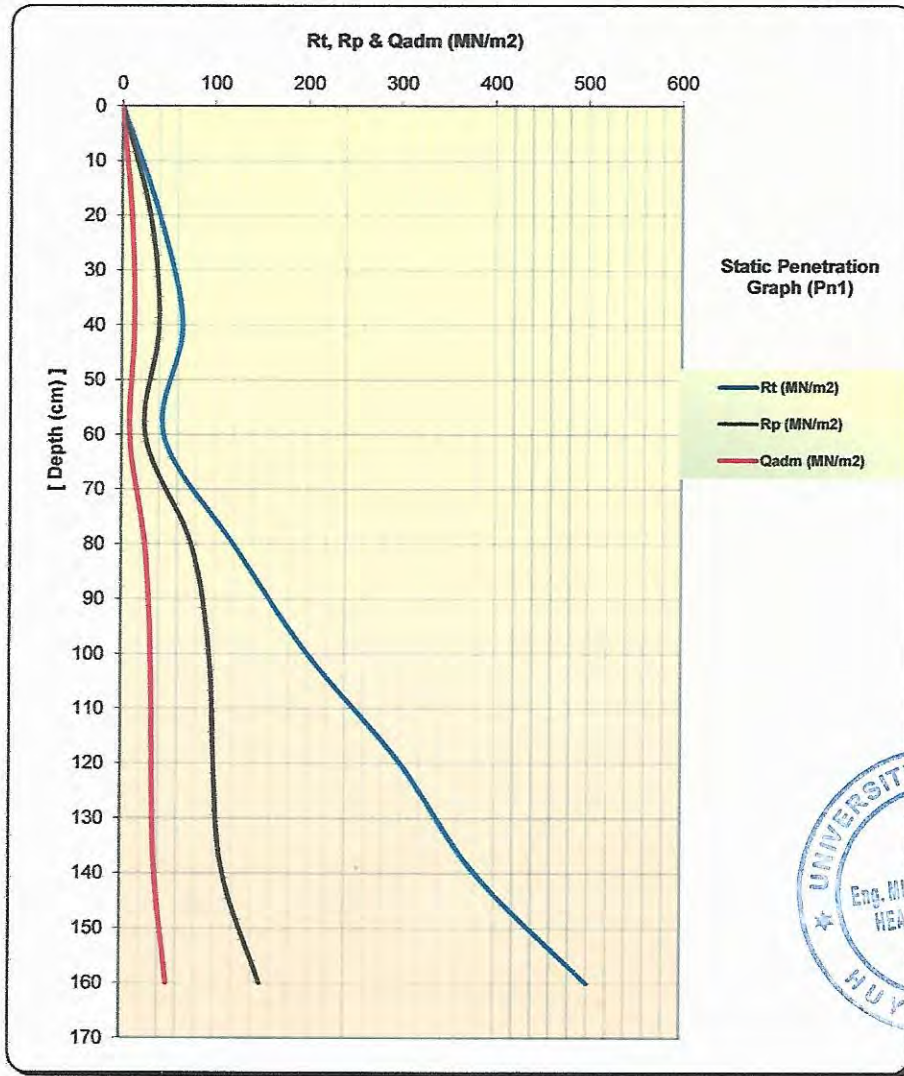
Operated by: Eng. Venerand GWIZA

Verified by: Eng. Vivien MUNYABURANGA (MSc)

**Date:** July, 2017

Penetration Test **N° 196**

Depth (cm)	Rt (MN/m <sup>2</sup> )	Rp (MN/m <sup>2</sup> )	Qadm (MN/m <sup>2</sup> )	Qadm (kg/cm <sup>2</sup> )
0	0	0	0.00	0.00
20	40	30	10.00	101.97
40	65	40	13.33	135.96
60	45	25	8.33	84.98
80	120	75	25.00	254.93
100	200	95	31.67	322.91
120	300	100	33.33	339.91
140	380	110	36.67	373.90
160	500	150	50.00	509.86
180				



SOIL INVESTIGATION ( File 2 / 2 : Field & laboratory tests )

II. CHEMICAL TESTS (for 50 points )

- New substation construction site & branch point line 110 kV at Gasogi ( 7 points ) : Sip1 , Sip2, Sip3, Sip4, Sip5, Sip7
- 15 kV Distribution line Gasogi substation - Nyagasambu (15 points ) : Sip1 , Sip2, Sip3, Sip4, Sip5, Sip6, Sip7, Sip9, Sip10, Sip11, Sip12, Sip13, Sip15, Sip16 & Sip17 .
- 15 kV Distribution line Gasogi substation - Masaka hospital (15 points ) : Sip1 , Sip3, Sip4, Sip5, Sip6, Sip8, Sip9, Sip10, Sip12, Sip13, Sip15, Sip16, Sip17, Sip18 & Sip19.
- Reconstruction of transmission 110 kV line Jabana - Birembo ( 13 points ) : Sip176 , Sip177, Sip178, Sip180, Sip181, Sip182, Sip184, Sip186, Sip187, Sip189, Sip190, Sip195, Sip196 .

**Chemical Lab Results with pH - Chloride & Sulfate**

**PROJECT:** IMPROVEMENT OF SUBSTATION & DISTRIBUTION NETWORK - PHASE III Soil Study

**CLIENT:** PITRAD

**Operator :** NZARAMBA Sylvanus

**Date:** July, 2017

**I. NEW SUBSTATION CONSTRUCTION SITE**

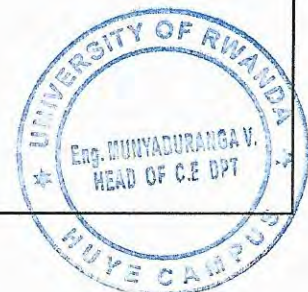
SN	Sample#	pH	Chloride (Cl-) (mg/L)	Sulfate (SO <sub>4</sub> <sup>2-</sup> ) (mg/L)
1	S1	7.3	85	81
2	S2	6.55	191	11.7
3	S3	7.02	177	16.5
4	S4	6.63	197	19
5	S5	6.67	193	21
6	S6	5.4	78.8	11.2

**II BRANCH POINT LINE 110 KV (new substation)**

SN	Sample#	pH	Chloride (Cl-) (mg/L)	Sulfate (SO <sub>4</sub> <sup>2-</sup> ) (mg/L)
7	S7	6.31	134	37

**III. 15 KV DISTRIBUTION LINE GASOGI SUBSTATION - NYAGASAMBU**

SN	Sample#	pH	Chloride (Cl-) (mg/L)	Sulfate (SO <sub>4</sub> <sup>2-</sup> ) (mg/L)
8	AP1	5.9	92	7.7
9	AP2	4.62	113.4	6.7
10	AP3	5.12	99.7	8.1
11	AP4	6.98	159.3	22
12	AP5	5.4	77.8	16.6
13	AP6	4.34	85	6.7
14	AP7	4.81	79.5	6.2
15	AP9	7.03	226.9	12
16	AP10	6.71	196.7	14.7
17	AP11	4.86	120.5	5.9
18	AP12	6.09	178	12.2
19	AP13	6.43	200.3	14.4
20	AP15	7.1	81	19.4
21	AP16	4.73	213	62.6
22				



**Chemical Lab Results with pH - Chloride & Sulfate**

**PROJECT: IMPROVEMENT OF SUBSTATION & DISTRIBUTION NETWORK - PHASE III** Soil Study

**CLIENT: PITRAD**

Operator : NZARAMBA Sylvanus

Date: July, 2017

**IV. 15 KV DISTRIBUTION LINE GASOGI SUBSTATION - MASAKA HOSPITAL**

SN	Sample#	pH	Chloride (Cl-) (mg/L)	Sulfate (SO <sub>4</sub> <sup>2-</sup> ) (mg/L)
23	AP1	5.11	122	13
24	AP3	6.26	99.3	59.2
25	AP4	6.15	79	18
26	AP5	5.79	78	7.6
27	AP6	7.1	201.7	9.4
28	AP8	5.22	85	5
29	AP9	6.75	226.9	9.2
30	AP10	6.83	199.4	8.8
31	AP12	6.15	209.9	10.1
32	AP13	6.64	213.1	12.2
33	AP15	6.55	218.5	9.7
34	AP16	4.91	113.3	6.9
35				
36	AP18	4.94	102.2	65.2
37	AP19	5.72	63.8	4.2

**V. RECOSTRUCTION OF TRANSMISSION 110 KV JABANA - BIREMBO**

SN	Sample#	pH	Chloride (Cl-) (mg/L)	Sulfate (SO <sub>4</sub> <sup>2-</sup> ) (mg/L)
38				
39				
40	178	7.04	85	70
41	180	6.2	81	8.4
42	181	6.55	145	34
43	182	6.13	101	13.7
44	184	6.22	151	28
45	186	6.44	113.4	68.4
46	187	5.42	92.2	10.9
47	189	7.24	70.9	65.9
48	190	4.81	99.3	62.6
49	195	5.88	176.9	12.2
50	196	6.11	202.4	24
51				



**University of Rwanda**  
**Department of Water & Environmental Engineering**

**Chemical Lab Results with pH - Chloride & Sulfate**

**PROJECT:** IMPROVEMENT OF SUBSTATION & DISTRIBUTION NETWORK - PHASE III Soil Study

**CLIENT:** PITRAD

**Operator :** NZARAMBA Sylvanus

**Date:** July, 2017

Legend of Soil Color Ramp	Total Number of Samples
REDDISH BROWN	20
REDDISH GRAY	2
LIGHT REDDISH GRAY	2
LIGHT YELLOWISH BROWN	3
YELLOW	1
LIGHT GRAY	1
LIGHT BROWN	5
YELLOWISH BROWN	3
RED	2
DARK BROWN	1
REDDISH BROWN AND SOME YELLOW	2
BROWN, TRACE GRAY	2
OLIVE	1
GRAY	4
GRAY, TRACE YELLOW	2

