MINISTRY OF ROADS AND HIGHWAYS GHANA HIGHWAY AUTHORITY

PREPARATORY SURVEY ON THE PROJECT FOR THE IMPROVEMENT OF THE TEMA MOTORWAY ROUNDABOUT IN THE REPUBLIC OF GHANA

FINAL REPORT

February 2017

JAPAN INTERNATIONAL COOPERATION AGENCY (JICA) CTI ENGINEERING INTERNATIONAL CO., LTD.

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PREFACE

In response to the request from the government of Ghana, the government of Japan decided to conduct the "Preparatory Survey on the Project for the Improvement of the Tema Motorway Roundabout in the Republic of Ghana" and entrusted the survey to the Japan International Cooperation Agency (JICA).

JICA selected and dispatched a survey team headed by Mr. Ryohei Watanabe of CTI Engineering International Co., LTD. from March 2015 to March 2017.

The survey team held a series of discussions with the officials concerned of the Republic of Ghana, and conducted field studies associated with various engineering surveys. The results and findings of the field studies and the analysis results carried out in Japan are compiled in this report.

I hope that this report will contribute to the promotion of the project and to the enhancement of friendly relations between our two countries.

Finally, I wish to express my sincere appreciation to the officials concerned of the Republic of Kiribati for their close cooperation extended to the survey team.

February, 2017

Akira NAKAMURA Director General Infrastructure and Peacebuilding Department Japan International Cooperation Agency

SUMMARY

1. Situation of Republic of Ghana (hereafter referred to as Ghana)

Although located along the Gulf of Guinea and Atlantic Ocean in the subregion of West Africa ninetyfive percent (95%) of the country's transport is dependent on road transport. Ghana has been making remarkable progress recently in improving the road network. However, only half of the entire arterial roads is paved and 38% of the paved surface is severely deteriorated.

The Government of Ghana (GOG) formulated the National Transport Policy in 2008 and has been expanding and upgrading the international corridors. In particular, the policy puts high importance in the development of road network in the Greater Accra Region (GAR) that has a population of about 4 million as of 2010 and lying at the nodal point of three international transit corridors; i) Abidjan-Lagos Corridor, ii) Central Corridor, and iii) Eastern Corridor of Ghana with an aim to not only secure smooth flow of international logistics but also to resolve traffic issues of the region.

The Tema Motorway Roundabout is a 5-leg roundabout where traffic from Accra and Tema Port converge, causing heavy traffic congestion during rush hours. The congestion and speed retardation at the roundabout is becoming an impediment to smooth and safe movement of people and traffic. In addition, the number of cargo vessels at Tema Port has been increasing in an annual rate of 12% between year 2000 and 2012. The GOG is planning expansion of the port to correspond to the increase. The is expansion plan is anticipated to contribute to significant increase of traffic volume in this region and at the roundabout as well.

2. Requested Scope

The request made by the GOG is a phase wise improvement of Tema Intersection. The scopes of each phase are as follows;

- Phase-1: Construction of 1st tier flyover including detailed design and construction supervision
- Phase-2: Construction of 2nd tier flyover including detailed design and construction supervision

3. Survey Schedule

Survey schedule is shown in Table-1.

| | č |
|---|---|
| Surveys (Site Survey etc.) | Period |
| 1 st Site Survey | April 7 th 2015 ~ May 22 nd 2015 |
| Interim report Explanation | October 19th 2015 ~ October 31st 2015 |
| 2 nd Site Survey | November 11 th 2015 ~ December 20 th 2015 |
| Design Review and Traffic Safety by GHA | June 23 rd 2016 ~ July 15 th 2016 |
| Draft Final Report Explanation | November 2 nd 2016 ~ November 14 th 2016 |

Table-1 Survey Schedule

4. Project Scope

A preliminary comparison of improvement methods, which was based on conceptual images and limited was carried out and explained to the GOG during the IC/R meetings. The comparison was substantiated and application was justified by reflecting actual figures and data obtained from various engineering surveys and hearings conducted during the site surveys.

The project scope was then determined, discussed with JICA, explained to GHA and sought GHA's agreement. The agreed scope of the project is shown in Table-2.

| Requested Scopes | Target |
|--|--|
| Phase-1Construction of 1stlevel flyover including improvementof the objective roundabout, the detailed design of a gradedintersection and construction supervision | O (to be undertaken by this project) |
| <u>Phase -2</u> Construction of 2 nd level flyover including, the detailed design of a flyover and construction supervision | _ |

| Table-2 I Toject Scope | Table-2 | Project | Scope |
|------------------------|---------|---------|-------|
|------------------------|---------|---------|-------|

5. Project Outline

The outline of the for Phase 1 is shown in Table 3.

| Items | | Specification and rough quantities | | |
|--|--|---|------------------------------------|---|
| gth | | East - West | Total Length | 2,100m |
| | Length | Direction (Motorway – Aflao Road) | Depress Section | 730m (Box culvert : 190m, Depress section : 540m) |
| | Ler | | outh Direction - Akosombo Road) | 1,900m |
| | | L | amps | 7,000m |
| | | Serv | ice Road | 3,500m |
| isign | | Design Spe | ed | 100km/h (Harbor Road : 80km/h) |
| Outline Design ars of age Road Width th | Motorway | STA.0+0~8+20 | 31.9m | |
| | Depress Section | STA.8+20~10+10 | 31.9m | |
| | Vidt | Aflao Road | STA.10+10~16+0 | 31.9m |
| | Harbor Road | STA.0+0~7+0 | 11.0m | |
| | Ro | Akosombo Road | STA.7+0~14+20 | 11.0m |
| | Serv | ice Road | 6.0m | |
| | Motorway / Depres | ss Section / Aflao Road | 3 Lanes / 3.65m (3.65m×3=10.95m) | |
| | Numbers of Carriage way and Width | Akosombo Ro | oad / Harbor Road | 2 Lanes / 3.65m (3.65m×2=7.3m) |
| z | | Serv | ice Road | 1 Lane / 3.0m (3.0m×1=3.0m) |

Table-3Outline of Project for Phase 1

| | Items | | Specification and rough quantities |
|-----------|--|------------------------------|------------------------------------|
| | r er | Motorway | 10.0m / 3.0m |
| | Center Median and Shoulder | Depress Section / Aflao Road | 4.0m / 2.5m |
| | C M Sha | Akosombo Road /Harbor Road | 4.0m / 2.5m |
| | Asphaltic Concrete Pavement | | t=11~15cm, 136,300m ² |
| 0 | Box Culvert | | L=190m |
| Structure | Retaking wall | | H=1.4m~11.0m, L=640m |
| Stru | Pedestrian Bridge | | 4 Numbers |
| | At Grade Intersection controlled by Traffic Signal | | 1 Location |
| | Road Lighting | | Box culvert section :L=190m |

6. Project Implement Schedule and Project Cost

The implementation period of the Project is estimated to be 8 month for detailed engineering design, and 36 months for construction work.

The project cost to be borne by the recipient country is estimated to be about 6.6 million Ghana Cedi (213 million Japanese Yen, GHC1.0=JPY32.26).

The cost to be borne by the Japan' Grant Aid is not shown in this report due to the confidentiality.

7. Project Evaluation

(1) Relevance of the Project

Implementation of the Project under Japanese Grant Aid has been verified from the following viewpoints:

• Large Scale of Beneficiaries

The Project area is located in Tema City area and number of beneficially is estimated approximately 500 thousand residences and approximately 90 million users in total.

• The Effect of the Project

The Project would contribute to easing traffic congestion and smoothing transportation of people and freight. These would consequently contribute to the improvement of transportation of network development for logistics throughout the nation and entire Western Africa Region.

Human Security

The Project would contribute to the securing stability of people's lives, development of their daily lives, and securing BHN (Basic Human Needs).

• Operation and maintenance of Facilities:

As excessive advanced technique is not necessary, the Ghana side would be able to carry out adequate operation and maintenance work in planned consistent way

• Consistency with Long Development Plan

This Project is consistent with the Policy and target described in the Government National Development Master Plan and would help in achieving enhancement of national trunk road network improvement.

• Impact to Environment

There is no significant or permanent negative impact to environment and social condition.

• Necessity and Superiority of Japanese Technology

Japanese most advanced and cutting-edge construction technology, such as time control, safety control, and quality control, is required to execute the Project under Japan's Grant Aid scheme without any serious issues.

(2) Effectiveness

1) Quantitative Effect

The expected quantitative effect to be produced through performing the works of target Project for economic cooperation is shown in the following Table 3. The values of base year before conducting the Project and target year in three (3) years over completion of the Project are respectively set up.

| Index | Basic Value | Target Value in 2023 |
|--------------------------------------|------------------------------|---------------------------|
| Index | (Actual Record of 2015 year) | (Project Completion Year) |
| Passenger Volume | 86.6(Million)/year | 156.8(Million)/year |
| Freight(Cargo) Volume | 44.3(Million)/year | 74.3(Million)/year |
| Travel Time per minute | 8.2 minute | 2.0 minute |
| (AM peak hour at Accra-Aflao 2.0 km) | 0.2 IIIIIute | 2.0 mmute |

Table 3 Quantitative Effects

2) Qualitative Effect

The prospective effects are;

- To moderate traffic congestion and to provide uninterrupted traffic flows.
- To facilitate and grade up transit system in the West African Region.

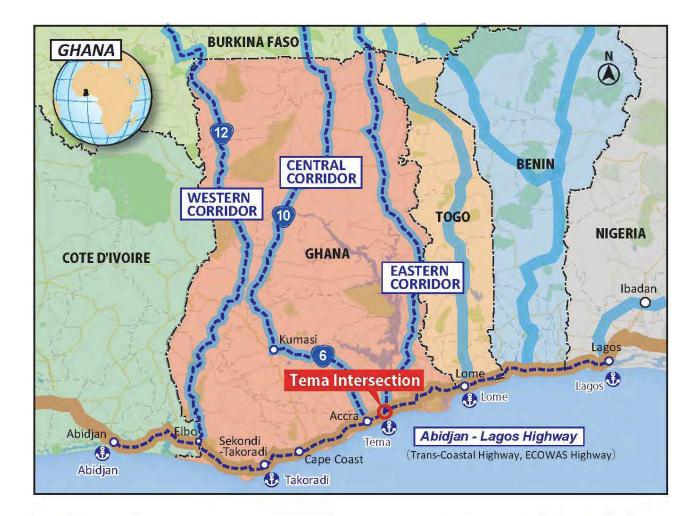
- a) Stable carrier-transit time and effectiveness in logistics leaving and arriving at the Tema Port will be improved.
- b) Traffic safety of pedestrians and vehicles at the intersections will be improved.
- c) Future emission of transport related greenhouse gases will be reduced.
- d) Connecting links between Coastal Highways (Coastal Corridor) and East Highways (East Corridor) will be enhanced.

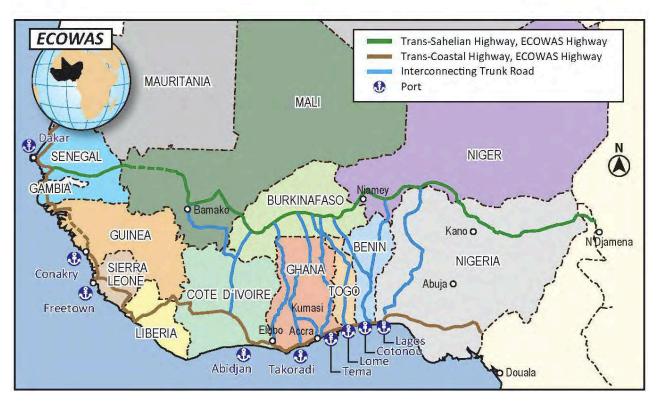
| Preface | |
|--|----|
| Summary | |
| Location Map/Perspective | |
| Contents | |
| List of Figures & Tables | |
| Abbreviations | |
| Chapter1 Background of the project | 1 |
| 1.1 Introduction | 1 |
| 1.2 Requested Scope | 2 |
| 1.3 Current Situation and Issues of the Objective Roundabout | 2 |
| 1.4 Natural Condition | 3 |
| 1.4.1 Geography and Geology | 3 |
| 1.4.2 Climate and Temperature | 4 |
| 1.4.3 Earthquake | 5 |
| 1.5 Environmental and Social Considerations | 5 |
| 1.5.1 Project Components | 5 |
| 1.5.2 Environmental Impact Assessment System | 5 |
| 1.5.3 Baseline of the Environmental and Social Condition | 6 |
| 1.5.3.1 Natural Environment | 6 |
| 1.5.3.2 Social Environment | 6 |
| 1.5.3.3 Pollution | 7 |
| 1.5.4 Scoping Result | 8 |
| 1.5.5 Main Mitigation Measures | 11 |
| 1.5.6 Environmental Monitoring Plan | 11 |
| 1.5.7 Stakeholder Meeting | 14 |
| 1.5.8 Land Acquisition and Resettlement | 14 |
| 1.5.8.1 Range of Impact | 14 |
| 1.5.8.2 Contents of Compensation and Assistance | 15 |
| 1.5.8.3 Monitoring Plan | 16 |
| 1.5.9 Environmental Checklist | 18 |
| Chapter2 Contents of the Project | 25 |
| 2.1 Basic Concept of the Project | 25 |
| 2.1.1 Requested Scope | 25 |
| 2.1.2 Survey Objective | 25 |
| 2.1.3 Overall Goal, Targets and Expected Outcomes | 25 |

| 2.2 Outline Design of the Requested Japanese Assistance | |
|--|-----|
| 2.2.1 Design Policy | |
| 2.2.1.1 Policies for Requested Scopes | |
| 2.2.1.2 Policies for Overall Improvement | |
| 2.2.1.3 Policies for Engineering Surveys | |
| 2.2.1.4 Policy for Land Use | 55 |
| 2.2.1.5 Procurement Policies | 58 |
| 2.2.1.6 Policy for Traffic Demand Forecast | 59 |
| 2.2.1.7 Policies for Intersection Improvement Plan | 65 |
| 2.2.2 Basic Construction Plan | 68 |
| 2.2.2.1 Intersection Improvement Plan | 68 |
| 2.2.2.2 Road Plan | 80 |
| 2.2.2.3 Structure Plan | |
| 2.2.2.4 Pavement Plan | |
| 2.2.2.5 Drainage Plan | 110 |
| 2.2.2.6 Street Lightning Plan | 113 |
| 2.2.2.7 Traffic Safety Facility Plan | 116 |
| 2.2.2.8 Considerations for Pedestrians (Pre-improvement Function Recovery) | 118 |
| 2.2.3 Outline Design Drawing | 121 |
| 2.2.4 Implementation Plan | 123 |
| 2.2.4.1 Implementation Policies | 123 |
| 2.2.4.2 Implementation Condition | 123 |
| 2.2.4.3 Scope of Works | 125 |
| 2.2.4.4 Procurement and Construction Supervision | 125 |
| 2.2.4.5 Quality Control Plan | |
| 2.2.4.6 Procurement Plan | |
| 2.2.4.7 Implementation Schedule | 131 |
| 2.3 Obligations of Recipient Country | |
| 2.4 Project Operation Plan | |
| 2.4.1 Organization for Operation and Maintenance | |
| 2.4.1.1 Organizational Framework | |
| 2.4.1.2 Road Sector Personnel in GHA | |
| 2.4.2 Contents of Operation and Maintenance | |
| 2.4.3 Points to Consider in Road Operation and Maintenance | |
| 2.5 Project Cost Estimate | |
| 2.5.1 Initial Cost Estimation | |
| 2.5.1.1 Japan's Contribution | |
| 2.5.1.2 Ghana's Contribution | |
| 2.5.1.3 Cost Estimation Condition | |

| 2.5.2 Operation and Maintenance Cost | |
|--|--|
| Chapter3 Project Evaluation | |
| 3.1 Preconditions to implement the project | |
| 3.2 Necessary input by recipient country | |
| 3.3 Important assumptions | |
| 3.4 Evaluation | |
| 3.4.1 Relevance | |
| 3.4.2 Effectiveness | |
| | |

- Appendix 1: Member List of the Study Team
- Appendix 2: Study Schedule
- Appendix 3: List of Parties Concerned in the Recipient Country
- Appendix 4: Minutes of Discussion (17 April, 2015)
- Appendix 5: 1st Technical Note (20 May, 2015)
- Appendix 6: 2nd Technical Note (18 December, 2015)
- Appendix 7: 3rd Technical Note (14 July, 2016)
- Appendix 8: Minutes of Discussion (8 November, 2016)
- Appendix 9: Drainage Calculations
- Appendix 10: Outline Design Drawings





LOCATION MAP

PERSPECTIVE

Current Condition



After Completion



PHOTOS: Activities during First Field Visit (1/2) (April 7, 2015 – May 22, 2015)



PHOTOS: Activities during First Field Visit (2/2) (April 7, 2015 – May 22, 2015)



PHOTOS: Activities during Second Field Visit (October 18, 2015 – November 1, 2015)



PHOTOS: Activities during Third Field Visit (November 11, 2015 – December 20, 2015)



PHOTOS: Activities during Fourth Field Visit (June 22, 2016 – July 16, 2016)



PHOTOS: Activities during Fifth Field Visit (November 1, 2016 – November 15, 2016)



Signing the MD between JICA and MRH

Signing the MD between JICA and MRH

LIST OF FIGURE

| FIGURE 1.4.2-1 MONTHLY AVERAGE TEMPERATURE | |
|--|----|
| FIGURE 1.4.2-2 MONTHLY RAINFALL | |
| FIGURE 1.4.3-1 EARTHQUAKE ZONING MAP | 5 |
| FIGURE 1.5.3-1 LAND USE LAYOUT OF STUDY AREA | 7 |
| FIGURE 1.5.8-1 DISTRIBUTION OF AFFECTED STRUCTURES | 15 |
| FIGURE 2.2.1-1 FLOW OF THE SURVEY | 27 |
| FIGURE 2.2.1-2 AREA AND SCOPE OF TOPOGRAPHIC SURVEY | 31 |
| FIGURE 2.2.1-3 LOCATION OF THE BENCHMARKS | 33 |
| FIGURE 2.2.1-4 LOCATION OF DRILLING POINTS AND CBR TEST | 35 |
| FIGURE 2.2.1-5 PROFILE OF BOREHOLES | 37 |
| FIGURE 2.2.1-6 LOCATION OF OBSERVATION WELL | 40 |
| FIGURE 2.2.1-7 DETAIL OF OBSERVATION WELL | 41 |
| FIGURE 2.2.1-8 UNDERGROUND FACILITY MAP | 44 |
| FIGURE 2.2.1-9 LOCATION OF TRAFFIC SURVEY STATIONS | 45 |
| FIGURE 2.2.1-10 EXISTING DRAINAGE FLOWS / SYSTEMS | 53 |
| FIGURE 2.2.1-11 FLOODED AREA | 54 |
| FIGURE 2.2.1-12 LAND USE AROUND TEMA ROUNDABOUT | 55 |
| FIGURE 2.2.1-13 HEAVY INDUSTRIAL AREA | 56 |
| FIGURE 2.2.1-14 TRENDS IN CONTAINER CARGO VOLUMES IN TEMA PORT | 57 |
| FIGURE 2.2.1-15 FUTURE TRAFFIC VOLUME ACCORDING TO DIRECTION AT PEAK HOURS (2020, 2035) | 62 |
| FIGURE 2.2.1-16 STUDY FLOW OF IMPROVEMENT PLAN | 66 |
| FIGURE 2.2.2-1 LANES AND LOS OF SIGNALIZED INTERSECTION OF PARTIAL FREE-FLOW TYPE IN PHASE-2 | 70 |
| FIGURE 2.2.2-2 3-TIER WITH AT-GRADE SIGNAL CONTROLLED INTERSECTION (COMPACT DIAMOND TYPE) | |
| FIGURE 2.2.2-3 LANES AND LOS OF SIGNALIZED INTERSECTION OF PARTIAL FREE-FLOW TYPE | 74 |
| FIGURE 2.2.2-4 BASIC CONSIDERATION ON STAGE CONSTRUCTION | 74 |
| FIGURE 2.2.2-5 TYPICAL LAYOUT OF EXISTING TRAFFIC SIGNALS ON NATIONAL ROADS IN GHANA | |
| FIGURE 2.2.2-6 CURRENT SITUATION (1) | 76 |
| FIGURE 2.2.2-7 CURRENT SITUATION (2) | 77 |
| FIGURE 2.2.2-8 TRAFFIC SIGNAL WIRING SYSTEM IN PHASE-1 | |
| FIGURE 2.2.2-9 TRAFFIC SIGNAL LAYOUT PLAN IN PHASE-1 | 78 |
| FIGURE 2.2.2-10 INTERSECTION PLAN IN PHASE-1 | 79 |
| FIGURE 2.2.2-11 EXISTING ROAD RESERVE (ROW) OF EACH ROAD AT TEMA ROUNDABOUT | 81 |
| FIGURE 2.2.2-12 HORIZONTAL ALIGNMENT OF MAIN CARRIAGEWAY AND RAMPS | 83 |
| FIGURE 2.2.2-13 PLAN OF MAIN CARRIAGEWAY AND RAMPS | 83 |
| FIGURE 2.2.2-14 PROFILE IN THE EAST-WEST DIRECTION | 84 |
| FIGURE 2.2.2-15 PROFILE IN THE NORTH-SOUTH DIRECTION | 84 |

| FIGURE 2.2.2-16 DESIGN VEHICLE | 86 |
|---|-------------|
| FIGURE 2.2.2-17 EXAMPLES OF LOS | 87 |
| FIGURE 2.2.2-18 CONCEPTUAL IMAGE OF RAMPS | 89 |
| FIGURE 2.2.2-19 MOTORWAY-AFLAO ROAD: TYPICAL CROSS SECTION (1) | |
| FIGURE 2.2.2-20 MOTORWAY-AFLAO ROAD: TYPICAL CROSS SECTION (2) | |
| FIGURE 2.2.2-21 HARBOUR ROAD - AKOSOMBO ROAD: TYPICAL CROSS SECTION | |
| FIGURE 2.2.2-22 RAMP: TYPICAL CROSS SECTION | |
| FIGURE 2.2.2-23 SCOPE OF STRUCTURES | |
| FIGURE 2.2.2-24 TYPICAL CROSS SECTION OF BOX CULVERT | |
| FIGURE 2.2.2-25 GENERAL MEASURE AGAINST WATER LEAKAGE | |
| FIGURE 2.2.2-26 GROUNDWATER LEVEL (WITHIN BOX CULVERT SECTION) | |
| FIGURE 2.2.2-27 APPLICATION AREAS OF ADDITIONAL MEASURES FOR PREVENTING GROUNDWATER | Leakage. 99 |
| FIGURE 2.2.2-28 EXISTING PAVEMENT SURVEY BY SOIL COMPACTION TESTER | 100 |
| FIGURE 2.2.2-29 LOCATION OF FIELD CBR TEST | 105 |
| FIGURE 2.2.2-30 AREA TO BE CONSIDERED COUNTERMEASURES | 108 |
| FIGURE 2.2.2-31 UNDERGROUND DRAINAGE PIPE | 109 |
| FIGURE 2.2.2-32 UNDERGROUND DRAINAGE PIPE LAYOUT IN DEPRESSED SECTION | 109 |
| FIGURE 2.2.2-33 TYPICAL CROSS SECTION IN DEPRESSED SECTION | 109 |
| FIGURE 2.2.2-34 UNDERGROUND DRAINAGE PIPE LAYOUT IN BOX CULVERT SECTION | 110 |
| FIGURE 2.2.2-35 TYPICAL CROSS SECTION IN DEPRESSED SECTION | 110 |
| FIGURE 2.2.2-36 DESIGN FLOW | 112 |
| FIGURE 2.2.2-37 CALCULATING FORMULA OF RUN-OFF | 113 |
| FIGURE 2.2.2-38 DRAINAGE PLAN | 113 |
| FIGURE 2.2.2-39 ILLUMINANCE DISTRIBUTION | 115 |
| FIGURE 2.2.2-40 STREET LIGHTING LAYOUT PLAN | 116 |
| FIGURE 2.2.2-41 SERVICE ROAD PLAN | 119 |
| FIGURE 2.2.2-42 FOOTBRIDGE LAYOUT | 121 |
| FIGURE 2.2.3-1 PLAN OVERVIEW | 121 |
| FIGURE 2.2.4-1 DETOUR PLAN | 125 |
| FIGURE 2.4.1-1 ORGANIZATIONAL FRAMEWORK | 134 |
| FIGURE 2.4.1-2 DEPARTMENT IN CHARGE OF MAINTENANCE IN GHA | |
| FIGURE 2.4.1-3 ORGANIZATION OF REGIONAL HEAD | |

LIST OF TABLES

| TABLE1.4.1-1 H | PEAK HOUR CONGESTION LENGTH AT ENTRY POINT | 2 |
|----------------|--|----|
| TABLE 1.5.4-1 | SCOPING RESULT | 8 |
| TABLE 1.5.6-1 | ENVIRONMENTAL MONITORING PLAN | 11 |
| TABLE 1.5.6-2 | DRAFT ENVIRONMENTAL MONITORING FORM | 13 |
| TABLE 1.5.8-1 | AFFECTED STRUCTURES AND PRIVATE PROPERTIES IN PRW | 14 |
| TABLE 1.5.8-2 | ENTITLEMENT MATRIX | 16 |
| TABLE 1.5.8-3 | DRAFT MONITORING FORM OF LAND ACQUISITION AND RESETTLEMENT | 17 |
| TABLE 1.5.9-1 | ENVIRONMENTAL CHECKLIST | 18 |
| TABLE 2.1.3-1 | OUTLINE OF THE PROJECT | 25 |
| TABLE 2.2.1-1 | PROJECT SCOPE | 26 |
| TABLE 2.2.1-2 | GENERAL DEFINITIONS OF LEVELS OF SERVICE | 28 |
| TABLE 2.2.1-3 | ENGINEERING SURVEYS | 30 |
| TABLE 2.2.1-4 | DETAILS OF BENCHMARKS INSTALLED | 32 |
| TABLE 2.2.1-5 | INVESTIGATION ITEMS AND ITS SCOPES | 34 |
| TABLE 2.2.1-6 | SUMMARY OF DRILLING RESULTS | 38 |
| TABLE 2.2.1-7 | INSTALLATION OF OBSERVATION WELL | 41 |
| TABLE 2.2.1-8 | OBSERVATION RESULTS | 41 |
| TABLE 2.2.1-9 | INVENTORY ITEMS | 42 |
| TABLE 2.2.1-10 |) Survey Items | 42 |
| TABLE 2.2.1-11 | SUMMARY OF TRAFFIC SURVEY | 45 |
| TABLE 2.2.1-12 | CLASSIFICATION OF VEHICLES IN THE SURVEY | 46 |
| TABLE 2.2.1-13 | ROAD SECTION TRAFFIC COUNT FOR EACH LOCATION | 46 |
| TABLE 2.2.1-14 | MEASURED TRAFFIC VOLUME AND PEAK PERCENTAGES IN THE MORNING | 47 |
| TABLE 2.2.1-15 | MEASURED TRAFFIC VOLUME AND PEAK PERCENTAGES IN THE EVENING | 47 |
| TABLE 2.2.1-16 | PEAK TRAFFIC VOLUME AND PERCENTAGES AGAINST THE TOTAL TRAFFIC VOLUME | 48 |
| TABLE 2.2.1-17 | 7 TRAFFIC CONGESTION LENGTH DURING PEAK HOURS (UNITS: M) | 49 |
| TABLE 2.2.1-18 | 3 TRAVELING TIME RESULTS DURING PEAK HOURS | 49 |
| TABLE 2.2.1-19 | PANNUAL AVERAGE DAILY TRAFFIC VOLUME (AADT) | 50 |
| TABLE 2.2.1-20 |) ANNUAL AVERAGE TRAFFIC VOLUME ACCORDING TO | 51 |
| TABLE 2.2.1-21 | ANNUAL AVERAGE TRAFFIC VOLUME ACCORDING TO | 52 |
| TABLE 2.2.1-22 | 2 TRENDS IN CONTAINER CARGO VOLUMES IN TEMA PORT | 56 |
| TABLE 2.2.1-23 | 3 TEMA PORT EXPANSION PROJECT LIST | 57 |
| TABLE 2.2.1-24 | PERCENTAGE INCREASE IN TRAFFIC VOLUME FROM PAST SURVEYS | 60 |
| TABLE 2.2.1-25 | POPULATION FRAMEWORK FOR THE DISTRICT AROUND TEMA ROUNDABOUT | 60 |
| TABLE 2.2.1-26 | 5 Volumes Handled Framework at Tema Port | 61 |
| TABLE 2.2.1-27 | ⁷ Revenue of Toll Gate | 61 |

| TABLE 2.2.1-28 GROWTH RATES IN TRAFFIC VOLUME | 62 |
|--|------|
| TABLE 2.2.1-29 TRAFFIC VOLUME ACCORDING TO DIRECTION AT PEAK HOURS IN 2020 (VEHICLES PER HOUR) | 63 |
| TABLE 2.2.1-30 TRAFFIC VOLUME ACCORDING TO DIRECTION AT PEAK HOURS IN 2035 (VEHICLES PER HOUR) | 64 |
| TABLE 2.2.1-31 CLASSIFICATION OF ROADS AND RESPONSIBLE AUTHORITIES | 66 |
| TABLE 2.2.1-32 DESIGN SPEEDS | 68 |
| TABLE 2.2.2-1 STUDIED / VERIFIED ITEMS FOR INTERSECTION IMPROVEMENT PLAN | 69 |
| TABLE 2.2.2-2 TRAFFIC CONTROL CAPACITY OF ROUNDABOUT | 69 |
| TABLE 2.2.3 CAPACITY ANALYSIS OF SIGNALIZED INTERSECTION IN PHASE-2 | 70 |
| TABLE 2.2.2-4 ALTERNATIVES FOR COMPARISON AND REASONS FOR ITS SELECTION | 71 |
| TABLE 2.2.5 COMPARATIVE STUDY OF INTERSECTION CONFIGURATION | 72 |
| TABLE 2.2.6 CAPACITY LIMIT IN PHASE-1 | 74 |
| TABLE 2.2.7 ANALYSIS RESULT OF SIGNALIZED INTERSECTION IN PHASE-1 | 75 |
| TABLE 2.2.2-8 CONTROL POINTS | 80 |
| TABLE 2.2.9 CLASSIFICATION OF ROADS | 81 |
| TABLE 2.2.2-10 GEOMETRIC CONDITIONS | 82 |
| TABLE 2.2.2-11 PROPOSED CROSS SECTION ELEMENTS | 84 |
| TABLE 2.2.2-12 REFERENCE POINT IN ROAD DESIGN | 85 |
| TABLE 2.2.2-13 REQUIRED NUMBER OF LANES ON ROAD SECTIONS | 88 |
| TABLE 2.2.2-14 REQUIRED NUMBER OF LANES ON FLYOVERS (GRADED SECTION) | 88 |
| TABLE 2.2.15 REQUIRED NUMBER OF LANES ON RAMP SECTION | 89 |
| TABLE 2.2.2-16 SUMMARY OF REQUIRED NUMBER OF LANES FOR ALL SECTIONS | 90 |
| TABLE 2.2.2-17 DESIGN CONDITIONS OF RETAINING WALL. | 95 |
| TABLE 2.2.18 DESIGN CONDITION FOR BOX CULVERT | 95 |
| TABLE 2.2.2-19 MATERIAL CONDITION | 96 |
| TABLE 2.2.2-20 COMPARISON OF RETAINING WALL TYPES | 97 |
| TABLE 2.2.2-21 COMPARISON OF MEASURES AGAINST GROUNDWATER LEAKAGE | 99 |
| TABLE 2.2.2-22 DESIGN PARAMETERS (INPUTS) OF PAVEMENT DESIGN | 103 |
| TABLE 2.2.2-23 DESIGN TRAFFIC VOLUME | 104 |
| TABLE 2.2.2-24 DESIGN TRAFFIC VOLUME | 104 |
| TABLE 2.2.2-25 DESIGN TRAFFIC VOLUME | 105 |
| TABLE 2.2.2-26 LABORATORY TEST RESULT. | 105 |
| TABLE 2.2.2-27 PAVEMENT STRUCTURE | 106 |
| TABLE 2.2.2-28 TYPICAL DEFECTS | 107 |
| TABLE 2.2.2-29 SURFACE TEMPERATURE SURVEY RESULT | 108 |
| TABLE 2.2.30 DRAINAGE OUTLETS | .111 |
| TABLE 2.2.31 TYPE OF LAMP AND CHARACTERISTICS | .114 |
| TABLE 2.2.32 DESIGN CONDITIONS | .115 |
| TABLE 2.2.33 CALCULATION RESULT | .115 |
| TABLE 2.2.34 APPLIED TRAFFIC SAFETY FACILITY | .117 |

| TABLE 2.2.2-35 DESIGN CONDITIONS OF SERVICE ROAD | 118 |
|--|-----|
| TABLE 2.2.2-36 FOOTBRIDGE OUTLINE | 120 |
| TABLE 2.2.3-1 TABLE OF CONTENTS | 122 |
| TABLE 2.2.4-1 RESPONSIBILITY OF EACH GOVERNMENT | 125 |
| TABLE 2.2.4-2 QUALITY CONTROL PLAN OF CONCRETE WORKS | 127 |
| TABLE 2.2.4-3 QUALITY MANAGEMENT PLAN FOR EARTHWORK AND PAVEMENT WORK | 127 |
| TABLE 2.2.4-4 PROCUREMENT AREA OF MAJOR CONSTRUCTION MATERIALS | 129 |
| TABLE 2.2.4-5 MAJOR CONSTRUCTION EQUIPMENT TO BE PROCURED | |
| TABLE 2.2.4-6 IMPLEMENTATION SCHEDULE | |
| TABLE 2.4.1-1 REGIONALS HEADS OF MAINTENANCE DEPARTMENT | |
| TABLE 2.4.1-2 GHA ROAD AREAS | 136 |
| TABLE 2.4.1-3 ROAD SECTOR PERSONNEL IN GHA | 136 |
| TABLE 2.5.1-1 PROJECT COST BORNE BY GHANA GOVERNMENT | 137 |
| TABLE 2.5.2-1 MAINTENANCE ITEM TO BE CHECKED AND ANNUAL MAINTENANCE COST | 138 |
| TABLE 3.4.2-1TABLE-4-4.1 QUANTITATIVE EFFECTS | |

| A-RAP | : | Abbreviated Resettlement Action Plan |
|--------|---|--|
| AADT | : | Annual Average Daily Traffic |
| AASHTO | : | American Association of State Highway Transportation Officials |
| CBR | : | California Bearing Ratio |
| D/D | : | Detailed Design |
| DHV | : | Design Hourly Volume |
| DF/R | : | Draft Final Report |
| DUR | : | Department of Urban Roads |
| EA | : | Environmental Assessment |
| ECOWAS | : | Economic Community of West African States |
| EIA | : | Environmental Impact Assessment |
| E/N | : | Exchange of Notes |
| EPA | : | Environmental Protection Agency |
| GHA | : | Ghana Highway Authority |
| GPHA | : | Ghana Ports and Harbour Authority |
| GRA | : | Greater Accra Region |
| GOG | : | Government of Ghana |
| GOJ | : | Government of Japan |
| IC/R | : | Inception Report |
| IEE | : | Initial Environmental Examination |
| IT/R | : | Interim Report |
| JICA | : | Japan International Cooperation Agency |
| JIS | : | Japanese Industrial Standard |
| LOS | : | Level of Service |
| MRH | : | Ministry of Roads & Highway |
| MTTU | : | Motor Transport and Traffic Unit |
| NTP | : | National Transport Policy |
| O/D | : | Outline Design |
| ODA | : | Official Development Assistance |
| PAPs | : | Project Affected Persons |
| PCU | : | Passenger Car Unit |
| PPP | : | Public Private Partnership |
| RAP | : | Resettlement Action Plan |
| R/D | : | Record of Discussion |
| ROW | : | Right of Way |
| SHRP | : | Strategic Highway Research Program |
| ТАН | : | Trans African Highway |
| TDC | : | Tema Development Corporation |
| TMA | : | Tema Metropolitan Assembly |
| T/R | : | Technical Review |
| | | |

ABBREVIATIONS

Chapter1 Background of the project

1.1 Introduction

Republic of Ghana is located along the Gulf of Guinea and Atlantic Ocean in the subregion of West Africa and shares borders with Togo in the east, Ivory Coast in the west and Burikina Faso in the north. Although all land, sea and air modes of transport is accomplished, ninety-five percent (95%) of the country's transport is dependent on road transport. Ghana has been making remarkable progress recently in improving the road network. However, the development still has a long way to go as only half of the entire arterial roads is paved. To make the situation worse, even the paved section is in poor condition as 38% of the paved surface is severely deteriorated where driving comfort is significantly low.

The Government of Ghana (GOG) formulated the National Transport Policy in 2008 and has been expanding and upgrading the international corridors to achieve integrated, efficient, and sustainable transport system and strengthen the function of the country as the transport hub of the West African region. In particular, the policy puts high importance in the development of road network in the Greater Accra Region (GAR) that has a population of about 4 million as of 2010 and lying at the nodal point of three international transit corridors; i) Abidjan-Lagos Corridor, a part of the Dakar to Lagos Trans African Highway (TAH) No.7 of the Economic Community of Western African States (ECOWAS), ii) Central Corridor, and iii) Eastern Corridor of Ghana that links Tema Port with Burkina Faso, Mali and Niger, to not only secure smooth flow of international logistics but also to resolve traffic issues of the region.

The Tema Motorway Roundabout is a 5-leg roundabout where traffic from Accra and Tema Port converge, causing heavy traffic congestion during rush hours. Speed at these times retard to below 10 km/h. The congestion and speed retardation at the roundabout is becoming an impediment to smooth and safe movement of people and traffic. In addition, the number of cargo vessels at Tema Port has been showing an annual increase of approximately 10% between year 2000 and 2012. The GOG is planning expansion of the port to correspond to the increase. The expansion of the port is anticipated to increase cargo vehicles to and from the port. This is expected to contribute to significant increase of traffic volume in this region and at the roundabout as well. In order to remedy such situation and to enhance smooth and safe traffic flow (logistics) both within the country and the West African Region, it is utterly and urgently important to undertake the improvement of the roundabout.

Under such circumstances, the GOG made a request in 2013 to the Government of Japan (GOJ) for a Grant Aid Assistance to implement "the Project for the Improvement of the Tema Motorway Roundabout (Project). The GOJ entrusted the survey to Japan International Cooperation Agency (JICA), the official agency to implement the Japanese Government's technical assistance and expediting proper execution of Japan's Grant Aid. JICA dispatched a consultant to carry out the survey.

This report compiles the results of the outline design drawing carried out reflecting the results of various engineering surveys undertaken in Ghana and information gathered from discussions and hearings conducted with the Ghana Highway Authority (GHA) and other relevant agencies of Ghana.

1.2 Requested Scope

The request made by the GOG is a phase wise improvement of Tema Intersection. The scopes of each phase are as follows;

- Phase-1: Construction of 1st tier flyover, including the detailed design and construction supervision
- Phase-2: Construction of 2nd tier flyover, including the detailed design and construction supervision

1.3 Current Situation and Issues of the Objective Roundabout

The Tema Motorway Roundabout is a major intersection located in the urban area of Tema Town on the Abidjan-Lagos highway. The Roundabout has five (5) legs. From the west in anti-clockwise direction, they are; (i) Accra-Tema Motorway, (ii) Tema-Hospital Road, (iii) Tema-Harbour Road, (iv) Tema-Aflao Road and (v) Tema-Akosombo Road. The roundabout has a 120m diameter mounted central island and an inscribed circle diameter of about 140m and a 2-lane circulatory roadway. Despite being relatively big, the roundabout is currently operating beyond its capacity and is experiencing heavy traffic congestion causing severe delays and lengthy travel time. The congestion is not only prevalent during peak hours but throughout the day time.

The traffic volume count results, as shown in **Table1.4.1-1**, indicate that during morning hours, congestion length on all legs excluding Hospital Road exceeds 100m, while the longest congestion is along Tema-Akosombo Road where the length is 1,100m. Although the congestion on Tema-Harbour Road and Tema-Hospital Road is comparatively less in the morning peak hour, it exceeds to several times the morning congestion length during evening hours.

| Direction | Peak Hour Congestion Length (m) | | |
|----------------------------------|---------------------------------|---------|--|
| Direction | Morning | Evening | |
| Accra-Tema Motorway (East Bound) | 500 | 500 | |
| Tema-Hospital Road (North Bound) | 20 | 170 | |
| Tema-Harbour Road (North Bound) | 120 | 700 | |
| Tema-Aflao Road (West Bound) | 600 | 450 | |
| Tema-Akosombo Road (South Bound) | 1,100 | 600 | |

 Table1.4.1-1 Peak Hour Congestion Length at Entry Point

The roundabout used to be controlled by traffic signals. Traffic volume had so much increased that use of these traffic signals was no more effective and the traffic control by signals was abandonment. Currently, congestion during peak hours is so severe (the traffic volume is so high) management within the roundabout is done with the help of traffic police from the Motor Transport and Traffic Unit (MTTU) of the Ghana Police Service. There are many factors that contribute to the congestion. Some of the major ones are briefly outlined below.

(1) Lack of Alternatives

Tema Roundabout is the only intersection where traffic converges from all directions. In fact, there are three

arterial roads, including the Accra-Tema Motorway running east-west, which connects Accra and its environs to the port of Tema. The other two roads are located south of the Motorway and run almost parallel to the Motorway. Both these roads end at the Tema-Harbour Road. Therefore, any vehicle bound beyond Tema, to the east or the north, must pass through the Tema Roundabout.

(2) Location and Geometry of the Roundabout

Generally, roundabouts operate effectively on collector or feeder roads, where traffic volume is relatively low. It is most effective on a typical four-legged intersection with similar order of traffic volumes. Tema Motorway Roundabout is a 5-legged roundabout located on an international corridor and the roads coming to it consist of different classifications. Where Accra-Tema Motorway, Tema-Harbour Road and Tema-Aflao Road have two lanes on both the approach and exit ways, Tema-Hospital Road and Tema-Akosombo Road have only one lane. Vehicles have to merge into a single lane and when the volume is high the speed of the merging vehicle is significantly reduced causing obstruction to the flow behind and ultimately creating heaving congestion.

(3) High Ratio of Heavy Vehicles

The result of the traffic volume count indicates that the rate of heavy vehicles account to almost 10% and is predicted to show constant increase in the years ahead. The heavy vehicles use the entire circulatory road width (both lanes) of the roundabout causing significant decrease of traffic capacity inside the roundabout.

(4) Disorderly Use of Open Space

The open spaces, particularly between the Tema-Aflao Road and Tema-Akosomobo Road, are full of vendors/small shops. People disorderly cross the road, mini buses frequently stop at any location to load or off load passengers. All these are some of the causes of severe obstruction to the flow of the vehicles.

(5) Vendors and Jay Walkers

There are many commercial street vendors who sell around the roundabout. Although, the vendors come onto the road only when the vehicles stop, they obstruct the vehicle flow when crossing from one side to the other. Also, there is no designated location to cross the road, so people cross the roads at their own convenient locations causing disruption to traffic flow.

1.4 Natural Condition

1.4.1 Geography and Geology

Republic of Ghana is located along the Gulf of Guinea and Atlantic Ocean in the sub-region of West Africa and shares its border with Togo in the east, Ivory Coast in the west and Burkina Faso in the north. The topography of Ghana is relatively flat with many swampy areas along the coastline and tropical forest that is distributed from the west coast to the central part of the country. The project area is located at Tema, in the Greater Accra Region of Ghana, approximately 19 kilometers east of Accra and approx. 5 kilometers north of Tema Port.

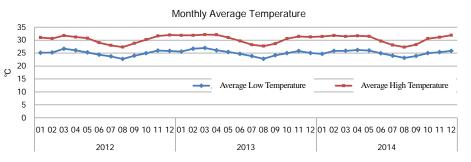
The geology of the area forms part of the Dahomeyan Series. The rocks of this geological system occur essentially as alternating belts of basic and acidic gneisses. Rocks of the series consist generally of muscovite – bitotite gneiss, quartz feldspar gneiss, augen gneiss and minor amphibolites. The area is known to exhibit little or no structural instability in its geology, the presence of faults and lineaments are not likely. Previous investigation as well as the present one has established that the area is underlain by GNEISS rock which has weathered to varying degrees.

1.4.2 Climate and Temperature

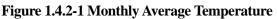
Climate of Ghana changes from a tropical humid climate to a Savanna climate from the southern plain to the central and northern basin.

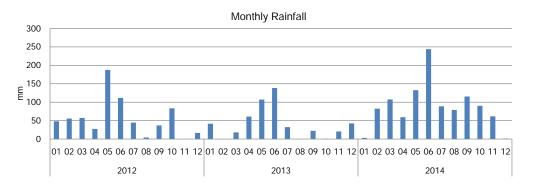
Tema, where the project area lies have dry equatorial climate and normally experiences two climate seasons in the year; the wet and dry season. The wet season usually extends from the middle of April to the end of October. The period between November and February is normally dry when the area is subjected to the North-East trade winds known as Harmattan winds. Monthly average temperatures and rainfall data for the last three consecutive years (2012 - 2014) are shown in **Figure 1.4.2-1** and **Figure 1.4.2-2** respectively. Mean temperatures are normally around 27 degrees Celsius. The peak levels are normally recorded during the dry season where temperatures rise to the range between 33 - 35 degrees. Minimum temperatures of 15 - 18 degrees are recorded during July and August.

Monsoon lies between March and October. The precipitation time varies but day-long rainfall is very rare. The maximum rainfall is seen between May and June where the monthly volume is observed at about 100mm.



Source: Ghana Meteorological Agency





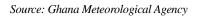
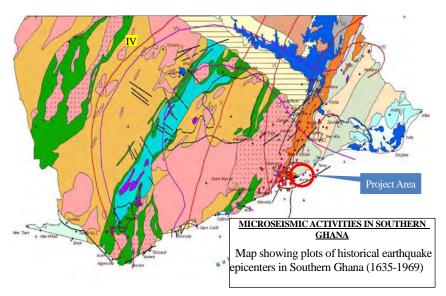


Figure 1.4.2-2 Monthly Rainfall

1.4.3 Earthquake

There are no major earthquake plates in Ghana, However, the southern part of the country occasionally

experiences seismic activities. Records of damaging earthquakes in Ghana date as far back as 1615. Historical data indicate that the most seismically active areas in Ghana are the west of Accra. According to the modified Building Code Part 3, 1988, the project area is classified into Zone 3 in terms of seismic activities. This means the horizontal ground acceleration is 'A', which is usually equal to or greater



Source: Ghana-German Technical Cooperation Project 2006 Figure 1.4.3-1 Earthquake Zoning Map

than 0.35g for this zone. The Code recommends that a minimum horizontal acceleration of 0.35g should be used in designing important structures in this zone.

1.5 Environmental and Social Considerations

1.5.1 Project Components

The components of the project are as following;

| • | Intersection Configuration | : 3-tier partial free-flow intersection (Phase 2) |
|---|----------------------------|--|
| • | Features | : Road improvement of approx. 2.1km on east & west side |
| | | 730m depressed section (Box culvert 190m, depressed 540m) |
| | | Road improvement o approx. 1.9km on north & south side |
| • | Ancillaries | : Service road approx. 3.5km |
| | | Ramps approx. 7.0km, signal controlled at-grade intersection, lighting |
| | | inside underpass, Drainage, safety measures, footbridges |
| • | Required Land area | : approx. 14.6 hector |
| • | Construction Period | : approx. 28 months (Phase 1) |
| | | |

1.5.2 Environmental Impact Assessment System

According to "Environmental Assessment Regulations 1999, LI 1652", development projects having potential environmental impacts must be registered with the Environmental Protection Agency (EPA) and

obtain an Environmental Permit. The proponents must conduct required procedure depending on SCHEDULE 1 and 2 in the Regulations. The proponents must submit an environmental registration form, project design and other required documents to EPA regional office in cases of projects listed in the SCHEDULE 1 or EPA central office in cases of projects listed in the SCHEDULE 2 respectively. All projects listed in the SCHEDULE 2 require implementation of Environmental Impact Assessment (EIA). Road construction projects require implementation of EIA as it is listed in SCHEDULE 2. This project is an improvement of a roundabout. However, implementation of EIA for this project is required as the improvement works include major construction works like that of new road construction projects.

1.5.3 Baseline of the Environmental and Social Condition

1.5.3.1 Natural Environment

(1) Fauna and Flora

There are no forests and wet lands and natural vegetation areas that are important for biodiversity in and around Tema intersection. According to field surveys conducted in December 2016, 3 species of reptiles (one is not identified), 4 breeds of birds and 8 species of trees existed in and around Tema intersection. All the identified species were common species.

(2) Protected Area and Forest Zone

Sakumo Lagoon (1,364 hectares) was designated as a Ramsar Site in 1992 and is a habitat for migratory birds and aquatic life in blackish water is located in 1.5 ~2 km west of Tema intersection. The area between Tema intersection and the Lagoon is mostly industrial and residential , and as such, pathways of surface or in water for wildlife migration are blocked. Rainwater in and around Tema intersection is not drained way to the Lagoon. Contamination of the lagoon at the estuary is very serious due to the fact that all domestic and industrial waste in and around Tema flows into the lagoon and the existence of a road along the coast does not allow a direct flow of the sea water into the lagoon, except at the estuary. There are no other protected areas or forest zones to serve as habitats for wildlife around Tema intersection.

1.5.3.2 Social Environment

(1) Social Economy

Tema intersection is a transportation hub of road networks connecting the nation's largest port. There are over 500 industries that produce chemicals, clothing, consumer electronics, electrical equipment, furniture, machinery, refined petroleum products and steel in the port zone, which also serves the "Free Economic Zone Areas". The free zone enterprises create employment in the factories. About 90 percent of the working population are employed in this zone. Unskilled women mainly work in service and sales sector derived from industry. Because agriculture, forestry and fishery are not thriving businesses in this area, the existence of these factories make a major contribution to poverty reduction measures in this area.

(2) Land use

The total land coverage of Tema Metropolitan Area is approximately 369 sq. km, about 45% of which has development plans. About 36% of the total land is used for residential, 7% for industrial and 4% for commercial purposes. Commercial activities are concentrated in the Central Business District and along some major radial roads including Tema intersection. Due to population growth, the current designated market areas in the Tema Metropolis are inadequate. The situation has led to the proliferation of commercial shops and stores in the form of simple structures, and shipping containers along the road corridors in Tema intersection.

The area around Tema intersection is within the Tema Development Corporation (TDC) Acquisition area managed by long-term lease hold, therefore problems associated with ownership of land is not anticipated to be high. However, there are some simple business stores within the Right of Way. The most prominent ones are facilities such as fast food joints, filling stations, bus terminals, sale of used cars and heavy machinery for hiring, and mechanic shops for car repairs that are related to transportation. The designated Tema Heavy Industry Area (about 50km2) lies southeast of Tema intersection. The open spaces, particularly between the Tema-Aflao Road and Tema-Akosomobo Road, are full of vendors/small shops.



Figure 1.5.3-1 Land Use Layout of Study Area

1.5.3.3 Pollution

(1) Noise Level

Measurement of noise level conducted at 5 points in and around Tema Roundabout under this survey

indicates that the noise levels except for the the measurement point at the center of the roundabout exceeded 75dB (as Aeq), that is the Permissible Noise Level in predominantly commercial areas.

(2) Air Quality

Air quality monitoring data around Tema Roundabout are not available. The air quality analysis conducted around Tema intersection in 2015 by a local consultant indicated that the PM10 and Total Suspended Particulate (TSP) levels (PM10 150 (μ g/m3), TSP 290 (μ g/m3)) exceed the Ambient Air Quality Standards in Ghana ((PM10 70 (μ g/m3), TSP 230 (μ g/m3)).

1.5.4 Scoping Result

Social impacts of the project are shown in the following Table as the scoping result.

| | | | Assessi | ment | |
|-----------|---|------------------------|---|--------------------|---|
| Category | No. | Impact Item | Pre- Construction Phase Construction Phase | Operation Phase | Reason / Remarks |
| | 1 | Air pollution | В- | B± | Construction Phase: Emission of dust and exhaust gas will increase due to construction equipment operations and traffic congestion in construction site. Operation Phase: In the future, the total amount of air pollutant caused by vehicle exhaust gas will increase. However, because of improved traffic efficiency, the amount may be reduced compared to current situation. |
| | 2 | | | D | Construction Phase: Turbid water will be generated during rainfall periods. However, because there are no water resource areas, rivers and lakes in and around the project site, impact of turbid water caused by construction works will be very limited. Operation Phase: Because water quality of drains will not change significantly, water pollution is unlikely to occur. |
| Po | 3 | Waste | B- | D | Construction Phase: Construction waste caused by construction and demolition works, and general waste from construction office will be generated. Operation Phase: Considerable generation of solid waste is |
| Pollution | 4 | Soil pollution | D | D | unlikely to occur. Because materials that may cause soil pollution such as heavy metal and toxic organic matter will not be used in the construction and maintenance works, soil pollution is unlikely to occur. |
| | 5 | Noise and vibration | B- | B± | Construction Phase: Construction equipment operation will cause noise and vibration. Operation Phase: Increased traffic volume and speed will elevate noise level. However, decrease in horning frequency, the noise level might be reduced compared to current situation. |
| | 6 Ground subsidence D D Because the ground is | | Because the ground is hard and groundwater withdrawal will not be included, ground subsidence is unlikely to occur. | | |
| | 7 | Offensive odors | D | D | Because materials and equipment that may cause offensive odors will not be used in construction and maintenance works, considerable offensive odors is unlikely to occur. |
| | 8 | Bottom sediment | D | D | Because there are no rivers and lakes near by the project site, considerable impacts of turbid water caused by construction works Or drainage form roads in operation phase on bottom sediment are unlikely to occur. |

 Table 1.5.4-1
 Scoping Result

| | | | Assessi | ment | |
|---------------------|-----------------|--|--|--------------------|---|
| Category | No. Impact Item | | Pre- Construction Phase Construction Phase | Operation Phase | Reason / Remarks |
| Natı | 9 | Protected areas | D | D | Sakumo Ramsar Site (1,364 hectares) is located in a lagoon of approximately 1.5 km southwest from Tema roundabout. However, because the zone between the lagoon and Tema roundabout comprise residential and industrial area and drainage from Tema roundabout does not flow into the lagoon, impact on ecosystem in the Ramsar site is unlikely to occur. |
| Natural Environment | 10 | Ecosystem | D | D | Felling of roadside trees including Neem, Rain Tree and Royal Poinciana will be required in construction phase. However, Impact on urban ecosystem created around Tema roundabout will be very limited. |
| ıment | 11 | Hydrology | D | D | Construction Phase: Considerable impact on ground water of pilling works is unlikely to occur. Operation Phase: Drainage system for rain water will not change significantly. |
| | 12 | Geographical features | D | D | Existing geographical features will not change considerably. Existing quarry site and borrow pit will be used for aggregate. |
| | 13 | Resettlement/ Land Acquisition | B- | D | Pre-Construction Phase: Because there are no living quarters around Tema roundabout, number of resettlement of dwellers will be small. However, 150 simple business stores including kiosk, container shops, parasol shops and stallkeeper openings around Tema roundabout will be removed temporarily or relocated. Moreover, removal of commercial or office buildings and gas stations may be required. Operation Phase: Additional resettlement and land acquisition will not be required. |
| Soc | 14 | Impoverished/Poor people | B± | D | Construction Phase: Disturbance in daily activities of street vendors which include impoverished people is likely to occur. Construction will create job opportunities to the poor as unskilled labor. Operation Phase: Considerable impact only on impoverished people is unlikely to occur. |
| cial Environment | 15 | Ethnic minorities and indigenous peoples | D | D | Because the project is located in developed areas, considerable impact on ethnic minorities or indigenous peoples is unlikely to occur. |
| mment | 16 | Local economies, such as employment, livelihood, etc. | B± | B+ | Construction Phase: Business activities of a gas station, several offices, shops and street vendors around Tema roundabout will be closed or suspended. Construction will create job opportunities to local people as unskilled labor. Operation Phase: Reduction of travel time by mitigated traffic jam will contribute to local economies. Adaptation of a new intersection configuration will change the land use plan. |
| | 17 | Land use and utilization of local resources | В- | B± | Construction Phase: Land acquisition will require change of land use such as from commercial area to road reserve. As a result, local resources will be partially lost. Operation Phase: Improved transportation will contribute to effective utilization of local resources. Adaptation of a new intersection configuration will change the land use plan. |
| | 18 | Water usage | D | D | Because there are no water resources around Tema Roundabout, considerable impact on water rights and its usage is unlikely to occur. |

| | | | Assess | ment | |
|----------|-----|---|--|--------------------|---|
| Category | No. | Impact Item | Pre- Construction Phase Construction Phase | Operation Phase | Reason / Remarks |
| | 19 | Existing social infrastructures and services | B- | B- | Pre-Construction Phase: Relocation or protection of utilities (service lines) such as water and sewer pipes, electric cable, telephone line and gas pipe will be required. Construction Phase: Temporary traffic congestion, shift of bus and taxi stations, and disturbance of access to roadside facilities will occurred. Operation Phase: Crossing of roads by pedestrians would only be allowed at designated places (footbridge). |
| | 20 | Social institutions such as social infrastructure and local decision- making institutions | D | D | Because the project is located in a developed area, considerable impact on social institutions is unlikely to occur. |
| | 21 | Misdistribution of benefits and damages | C- | C- | Because the project lies in a developed area, considerable misdistribution of benefit among local people is unlikely to occur. However, misdistribution of benefit between relocated and remaining business stores may occur. |
| | 22 | Local conflicts of interest | C- | C- | Because the project lies in a developed area, considerable impact due to local conflict is unlikely to occur. However, local conflict between relocated and remaining business stores may occur. |
| | 23 | Cultural heritage | D | D | There are no cultural heritages around Tema Roundabout. |
| | 24 | Landscape | B- | D | Construction Phase: Loss of vegetation and construction work will change the landscape. Operation Phase: Appearance of footbridges will change the landscape. However, as Tema roundabout is located in commercial and industrial areas, particular landscape conservation measures will not be required. |
| | 25 | Gender | D | D | Because the project is improvement works of existing roundabout, considerable impact only on gender is unlikely to occur. |
| | 26 | Children's rights | D | D | Because the project is improvement works of existing roundabout, considerable impact only on children's rights is unlikely to occur. |
| | 27 | Infectious diseases such as HIV/AIDS | D | D | Because the project site is located in a well developed urban area, new considerable influx of infected persons as construction worker will not occur. |
| | 28 | Working conditions (including occupational safety) | B- | D | Construction Phase: Impact of waste from construction workers on sanitary conditions around the construction site is likely to occur. Because construction will include works in heights, various accidents may occur. Operation Phase: Road operation will not have impacts on working conditions. |
| | 29 | Accidents | В- | B± | Construction Phase: Labor accident, including tumble accident may involve pedestrians and street vendors Operation Phase: Decrease of minor accidents in Tema Roundabout will be expected. On the other hand, traffic accident may increase in newly constructed intersection at the initial stage. |
| Other | 30 | Trans-boundary impacts or climate change | D | D | Trans-boundary impacts such as climate change are unlikely to occur. |

A+/-: Significant positive/negative impact is expected.

B+/-: Positive/negative impact is expected to some extent.

C+/-: Extent of positive/negative impact is unknown. (A further examination is needed, and the impact could be clarified as the study progresses)

D: No impact is expected

* Impact Items refer to "JICA Guidelines for Environmental and Social Considerations April 2010"

1.5.5 Main Mitigation Measures

• Resettlement :

Authorities concerned shall prepare and strictly implement a proper Abbreviated Resettlement Action Plan (ARAP).

• Environmental pollution during construction phase :

The contractor shall prepare and strictly implement mitigation measures against environmental pollution. The supervising consultant shall monitor the environmental conditions and complaints from the local people. If troubles of some sort occur, the supervising consultant shall direct the contractors to reconsider the construction technique and method.

• Impacts on local Society and Economy :

The detail design consultant shall provide relevant information including the final design layout of the intersection and traffic move forecasting for reconsideration of the plan for land use to Tema Development Corporation (TDC) necessarily or upon request by the TDC.

1.5.6 Environmental Monitoring Plan

The environmental monitoring plan and draft form are shown in the following Tables.

| Category | Environmental Item | Monitoring Item/ Parameter | Responsible Person and Organization | Location | Method | Frequency |
|-----------|-----------------------|---|---|---|---|---|
| | Air pollution | Construction Phase: • Dust | Supervising Consultant Contractor | Construction site | Visual observation and interview to pedestrians | Visual observation: Daily Interview: Monthly or as needed |
| | | • PM10、PM2.5、 NOx、SOx | | | Instrumental analysis | Instrumental analysis: Pre-Construction Phase 1 time Construction Phase 5 times Total 6 times |
| Pollution | | Operation Phase: • PM10、PM2.5、 NOx、SOx | GHA | Around Tema intersection. | Instrumental analysis | 1 time in dry season and 1 time in rainy season per year for 2 years after completion Total 4 times |
| | Water pollution | Construction Phase: • Turbid water and drainage conditions | Supervising Consultant Contractor | Construction site | Visual observation | During rainfall |
| | Waste | Construction Phase:: • Disposal methods of construction and general waste | Supervising Consultant Contractor | Construction site and disposal site | Visual observation and meeting with contractor | Visual observation: Daily Meeting: Monthly or as needed |

Table 1.5.6-1Environmental Monitoring Plan

| Category | Environmental Item | Monitoring Item/ Parameter | Responsible Person and Organization | Location | Method | Frequency |
|------------------|---|---|---|--|---|--|
| | Noise and vibration | Construction Phase: • Noise level • Vibration level | Supervising Consultant Contractor | Construction site | Interview to local residents and pedestrians | Interview: Monthly or as needed |
| | | | | | Instrumental measurement | Instrumental measurement: Pre-Construction Phase 1 time Construction Phase 5 times Total 6 times |
| | | Operation Phase:Noise levelVibration level | GHA | In and around Tema intersection | Instrumental measurement | 1 time per year for 2 years after completion Total 2 times |
| | Resettlement/ Land Acquisition | Pre-Construction Phase: • Progress of resettlement action plan | GHA | Around Tema intersection and relocation sites | Site survey and meeting with PAPs | Monthly or as needed |
| | Poor people Construction Phase: • Activity conditions of street venders | | Supervising Consultant Contractor | Construction site | Visual observation | Daily |
| | Local economies, such as employment, livelihood, etc. | Pre-Construction Phase: • Progress of resettlement action plan | GHA | Around Tema intersection and relocation sites | Site survey and meeting with PAPs | Monthly or as needed |
| Social Env | | Construction Phase: • Business activity around construction site • Employment situation of unskilled labor | Supervising Consultant Contractor | Construction site | Site survey and interview to local people and unskilled labors | Monthly or as needed |
| cial Environment | Land use and utilization of local resources | Pre-Construction Phase: • Progress of resettlement action plan | GHA | Around Tema intersection and relocation sites | Site survey and meeting with PAP | Monthly or as needed |
| | | Operation Phase: Condition of land use Condition of business activity | GHA TDC | In and around Tema intersection | Site survey and interview to local people | Monthly or as needed for 2 years after completion |
| | Existing social infrastructures and services | Pre-Construction Phase: • Relocation status of existing infrastructure facilities | GHA | In and around Tema intersection | Site survey and meeting with facility owners | Monthly or as needed |
| | | Construction Phase: • Condition of traffic congestion around construction site | Supervising Consultant Contractor | Construction site | Visual observation | Daily |

| Category | Environmental Item | Monitoring Item/ Parameter | Responsible Person and Organization | Location | Method | Frequency |
|----------|--|---|---|--|--|---|
| | | Operation Phase: • Crossing conditions of pedestrians | GHA | In and around Tema intersection | Site survey and interview to local people | Monthly or as needed for 2 years after completion |
| | Misdistributio n of benefits and damages | Pre-Construction Phase: • Progress of resettlement action plan | GHA | Around Tema intersection and relocation sites | Site survey and meeting with PAPs | Monthly or as needed |
| | | Operation Phase: • Living situations of Project Affected Persons (PAPs) | GHA | Around Tema intersection and relocation sites | Site survey and meeting with PAPs | Monthly or as needed for 2 years after relocation |
| | Local conflicts of interest | Pre-Construction Phase: • Progress of resettlement action plan | GHA | Around Tema intersection and relocation sites | Site survey and meeting with PAPs | Monthly or as needed |
| | | Operation Phase: • Living situations of Project Affected Persons (PAPs) | GHA | Around Tema intersection and relocation sites | Site survey and meeting with PAPs | Monthly or as needed for 2 years after relocation |
| | Landscape | Construction Phase: • Status of tree felling • Status of Planting works | Supervising Consultant Contractor | Construction site | Visual observation and meeting with contractor | Daily |
| | Working conditions (including occupational safety) | Construction Phase: • Workplace situations • Implementation status of accident prevention measures | Supervising Consultant Contractor | Construction site | Visual observation and meeting with contractor | Daily |
| Other | Accidents | Construction Phase: • Implementation status of accident prevention measures | Supervising Consultant Contractor | Construction site | Visual observation and meeting with contractor | Daily |
| | | Operation Phase::Number of traffic accident | GHA | In and around Tema intersection | Site survey and traffic accident data | Monthly or as needed for 2 years after completion |

Table 1.5.6-2 Draft Environmental Monitoring Form

| Item | Parameter | Location | Frequency | Responsible Agency | Result | | |
|---------------------|--------------------------------|-------------------|---------------------|--------------------------------------|--------|--|--|
| Construction Stage | | | | | | | |
| Air quality | PM10, PM2.5, NO, SOx | Construction site | 1 time/half year | Supervising Consultant Contractor | | | |
| Noise and Vibration | Noise level Vibration Level | Construction site | 1 time/half year | Supervising Consultant Contractor | | | |

| Item | Parameter | Location | Frequency | Responsible Agency | Result |
|---|----------------|-------------------|---------------|--------------------------------------|--------|
| Water Quality | Turbid water | Construction site | Rainfall time | Supervising Consultant Contractor | |
| Waste | Waste disposal | Construction site | Every day | Supervising Consultant Contractor | |
| Operation Stage | | | | | |
| Air qualityPM10, PM2.5, NO, SOxTema Intersection | | 2 times/year | GHA | | |
| Noise and VibrationNoise levelTemaVibration LevelIntersection | | 2 times/year | GHA | | |

1.5.7 Stakeholder Meeting

The first stakeholder meeting with relevant organizations and business enterprises was held on 11 December 2016. There were 39 participants including 3 women.

1.5.8 Land Acquisition and Resettlement

1.5.8.1 Range of Impact

Identified affected structures and private properties in the Provisional Road Width are shown in the following Table. These structures and properties will need resettlement, relocation and compensation. 14 structures of these structures are residence. The total number of residents are 59 persons.

| | Type of Structure | | | | | | | |
|--------------------|-------------------|--------------------|---------------------|-----------------|------|-----------------|-------|--|
| Road Name | Building | Metal Container | Wooden Structure | Wooden Kiosk | Shed | Wooden Table | Total | |
| Tema Harbour Road | 1 | - | - | 4 | - | - | 5 | |
| Tema-Aflao Road | 1 | 56 | 5 | 42 | 19 | 2 | 125 | |
| Tema-Akosombo Road | 3 | 12 | 10 | 1 | 2 | 2 | 30 | |
| Total | 5 | 68 | 15 | 47 | 21 | 4 | 160 | |

Table 1.5.8-1 Affected Structures and Private Properties in PRW

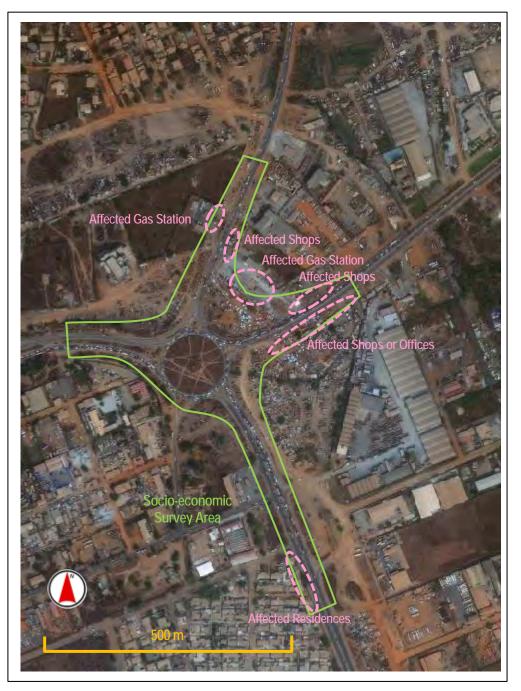


Figure 1.5.8-1 Distribution of Affected Structures

1.5.8.2 Contents of Compensation and Assistance

In Ghana, properties such as lands and structures lost by exploitation for public interests is generally subjected for monetary compensation. Project Affected Persons (PAPs) are categorized in Ghana into three types as follows;

- 1) Persons possessing official legal rights to own properties such as traditional lands and structures on the basis of custom admitted by laws and regulations
- 2) Persons possessing no official legal rights in national census, however identified ownership in social condition surveys
- 3) Persons unrecognized or not having legal rights, however claiming occupancy of properties

Because the area around Tema intersection has been managed by TDC, Most of the PAPs belong to the first category, . The entitlement matrix is shown in the following Table.

| | | Entitlements | | | | | |
|---|--|---|--|--|---|---|--|
| PAPs Type | Lost properties and rights | Compensation for loss of land (long-term lease hold right) and structures | Compensation for loss of assets | Compensation for loss of income | Moving allowance | Other assistance | |
| Business Owner (License holder from TDC) | Loss of land Loss of structure | Compensation at on-going market price Compensation at full replacement cost value 1) | Pay full cost of removal and fixing of removable | moval and fixing based on | | 10% of compensation cost estimated by Land Valuation Board for inconvenience and disturbance | |
| Business Tenant (No license holder from TDC) | No loss of land Loss of rental accommodation | No loss of structure so no compensation | - | period within which he/she relocates Implementation of income restoration program if needed | Cover full cost | 10% of compensation cost for inconvenience and disturbance | |
| Residence Owners (License holder from TDC) | Loss of land Loss of structure | Compensation at-going market price Compensation at full replacement cost value | - | Implementation of income restoration program if needed | - | 5% of compensation cost for inconvenience and disturbance | |
| Residence Tenant (No license holder from TDC) | No loss of land Loss of rental accommodation | Relocate to location of choice | Six months rents on the basis of present rents | Implementation of income restoration program if needed | Coverage of full cost of transport expenses | 5% of compensation cost for inconvenience and disturbance | |
| License squatters and Encroachers | Loss of use of land | - | - | Payments in lieu of wages while moving (About USD 50 uniformly. The actual payment price is estimated on the basis of the census survey in the next stage.) | Coverage of full cost of transport expenses | Full cost of disconnection and reconnection of utility services. Payment for loss of connectivity | |

| Table 1.5.8-2 | Entitlement Matrix |
|---------------|---------------------------|
| | |

1) Full replacement cost value: Valuation of assets that helps determine the amount sufficient to replace lost assets and cover transaction costs (including "Compensation for loss of assets" and "Moving allowance" in this Matrix)

1.5.8.3 Monitoring Plan

The draft monitoring form of land acquisition and resettlement are shown in the following Table.

Table 1.5.8-3 Draft Monitoring Form of Land Acquisition and Resettlement

Preparation of Resettlement Site

| No. | Explanation of the Site | Status Completed (date) or not) | Details | Expected Date of Completion |
|-----|-------------------------|------------------------------------|---------|-----------------------------|
| 1. | | | | |
| 2. | | | | |

| | Public Consultation | | | | | | |
|-----|---------------------|-------|--|--|--|--|--|
| No. | Date | Place | Contents of the construction / Main comments and answers | | | | |
| 1. | | | | | | | |
| 2. | | | | | | | |

| | D1 | | Prog | ress in Qu | antity | Progre | ss in % | Expected | D 11 |
|---|------------------|-----------------|--------------------------|-----------------------------|-------------------------|-----------------------------|-------------------------|-----------------------|-----------------------------|
| Resettlement Activity | Planned Total | Unit | During the Quarter | Till the Last Ouarter | Up to the Quarter | Till the Last Quarter | Up to the Quarter | Date of Completion | Responsible Organization |
| Preparation of ARAP* | | | | | | | | | GHA |
| Employment of Consultants | | Man- Month | | | | | | | |
| Implementation of Census Survey | | 1.101141 | | | | | | | |
| Approval of ARAP | | | Date of | Approval | : | | | L | ł |
| Finalization of PAPs List | | No. of PAPs* | | | | | | | |
| Progress of Compensation Payment (All Lots) | | No. of HHs* | | | | | | | |
| Lot 1 | | No. of HHs | | | | | | | |
| Lot 2 | | No. of HHs | | | | | | | |
| Progress of Land Acquisition (All Lots) | | ha | | | | | | | |
| Lot 1 | | ha | | | | | | | |
| Lot 2 | | ha | | | | | | | |
| Progress of Asset Replacement (All Lots) | | No. of HHs | | | | | | | |
| Lot 1 | | No. of HHs | | | | | | | |
| Lot 2 | | No. of HHs | | | | | | | |
| Progress of Relocation of People (All Lots) | | No. of HHs | | | | | | | |
| Lot 1 | | No. of HHs | | | | | | | |
| Lot 2 | | No. of HHs | | | | | | | |

*: ARAP: Abbreviated Resettlement Action Plan, PAPs: Project Affected Persons, HHs: Households

1.5.9 Environmental Checklist

Environmental checklist is shown in **Table 1.5.9-1**.

| | | Table 1.5.9-1 Environme | | |
|---------------------------|--|--|----------------------------------|--|
| Category | Environmental Item | Main Check Items | Yes: Y No: N | Confirmation of Environmental Considerations (Reasons, Mitigation Measures) |
| 1 Permits | (1) EIA and Environmental Permits | (a) Have EIA reports been already prepared in official process? (b) Have EIA reports been approved by authorities of the host country's government? (c) Have EIA reports been unconditionally approved? If conditions are imposed on the approval of EIA reports, are the conditions satisfied? (d) In addition to the above approvals, have other required environmental permits been obtained from the appropriate regulatory authorities of the host country's government? | (a) N (b) N (c) N (d) N | (a) The EIA report is preparing and will be submitted to Environmental Protection Agency in March, 2017. (b)(c) If the amendment of the EIA report is not required, the report will be approved within 50 days after the submission. (d) The other permissions related to environmental management are not required. |
| and Explanat ion | (2) Explanation to the Local Stakeholders | (a) Have contents of the project and the potential impacts been adequately explained to the Local stakeholders based on appropriate procedures, including information disclosure? Is understanding obtained from the Local stakeholders? (b) Have the comment from the stakeholders (such as local residents) been reflected to the project design? | (a) Y (b) Y | (a) Three stakeholder meetings were conducted in December, 2015 and the understanding has been obtained from local stakeholders. (b) The results of interview surveys to the local people and stakeholder meetings with the other relevant organizations were reflected in the design policy and mitigation measures for environmental impacts during constriction phase. |
| | (3) Examination of Alternatives | (a) Have alternative plans of the project been examined with social and environmental considerations? | (a) Y | (a) Several alternative plans on the structure of the intersection have been examined with social and environmental considerations at the preparatory study. |
| 2 Pollution Control | (1) Air Quality | (a) Is there a possibility that air pollutants emitted from the project related sources, such as vehicles traffic will affect ambient air quality? Does ambient air quality comply with the country's air quality standards? Are any mitigating measures taken? (b) Where industrial areas already exist near the route, is there a possibility that the project will make air pollution worse? | (a) - (b) - | (a)(b) Because the project site is located in industrial area, considerable air pollution is feared. However, continuous monitoring of the air quality is not conducted. It is unknown whether the air quality exceeds the environmental standards or not. In the future, total amount of air pollutant caused by vehicle exhaust gas will increase. However, because of improved traffic efficiency, the amount may be reduced compared to without project. |

Table 1.5.9-1 Environmental Checklist

| Category | Environmental Item | Main Check Items | Yes: Y No: N | Confirmation of Environmental Considerations (Reasons, Mitigation Measures) |
|---------------------------------|----------------------------|--|-------------------------|---|
| | (2) Water Quality | (a) Is there a possibility that soil runoff from the bare lands resulting from earthmoving activities, such as cutting and filling will cause water quality degradation in downstream water areas? (b) Is there a possibility that surface runoff from roads will contaminate water sources, such as groundwater? (c) Do effluents from various facilities, such as parking areas/service areas comply with the country's effluent standards and ambient water quality standards? Is there a possibility that the effluents will cause areas not to comply with the country's ambient water quality standards? | (a) N (b) N (c) N | (a) Turbid water will generate in the construction works. The turbid water will be disposed into existing drainage ditches along the roadside and not drain into the surrounding area. There are no intake facilities in and down the site. (b) Because drainage facilities have been constructed along the road, impact on water resources of runoff from road surface will not occur. (c) Development of parking or service areas which generate waste water in operation phase are not included in the project. |
| | (3) Wastes | (a) Are wastes generated from the project facilities, such as parking areas/service areas, properly treated and disposed of in accordance with the country's regulations? | (a) N | (a) Development of parking or service areas are not included in the project. |
| | (4) Noise and Vibration | (a) Do noise and vibrations from the vehicle and train traffic comply with the country's standards? | (a) N | (a) The noise level on the borderline of the right of way exceed the environmental standards at present. However, because the project site is located in commercial or industrial area, the impact on general population will not be serious. In the future, noise level caused by vehicle driving will increase. However, because flyover bridges will be installed in central part of the right of way as main driving lanes, the level on road side may be reduced compared to without project. To prevent increase in noise and vibration level, GHA should maintain favorable road surface condition. |
| 3 Natural Environ ment | (1) Protected Areas | (a) Is the project site located in protected areas designated by the country's laws or international treaties and conventions? Is there a possibility that the project will affect the protected areas? | (a) N | (a) There are no protected areas in the site and project affected areas. |

| | D 1 | | X7 X7 | Confirmation of Environmental |
|----------|-----------------------|---|-----------------|---|
| Category | Environmental Item | Main Check Items | Yes: Y No: N | Considerations |
| | nom | | | (Reasons, Mitigation Measures) |
| | | (a) Does the project site | (a) N | (a) There are no ecological valuable habitats |
| | | encompass primeval forests, | (b) N | in and around the site. |
| | | tropical rain forests, | (c) N | (b) The habitats of endangered species have not been identified in and down the site. |
| | | ecologically valuable habitats (e.g., coral reefs, mangroves, or | (d) N (e) N | (c) Significant ecological impact will not |
| | | tidal flats)? | (f) N | occur. |
| | | (b) Does the project site | (1) 1 | (d) Wild animals migrating through the site |
| | | encompass the protected | | have not been identified. |
| | | habitats of endangered species | | (e)(f) The project will not cause destruction |
| | | designated by the country's | | of forest and poaching because of |
| | | laws or international treaties | | construction works along existing road in |
| | | and conventions? | | urban area. |
| | | (c) If significant ecological | | |
| | | impacts are anticipated, are | | |
| | | adequate protection measures | | |
| | | taken to reduce the impacts on | | |
| | | the ecosystem? (d) Are adequate protection | | |
| | | measures taken to prevent | | |
| | | impacts, such as disruption of | | |
| | (2) E | migration routes, habitat | | |
| | (2) Ecosystem | fragmentation, and traffic | | |
| | | accident of wildlife and | | |
| | | livestock? | | |
| | | (e) Is there a possibility that | | |
| | | installation of roads will cause | | |
| | | impacts, such as destruction of | | |
| | | forest, poaching, desertification, reduction in | | |
| | | wetland areas, and disturbance | | |
| | | of ecosystems due to | | |
| | | introduction of exotic (non- | | |
| | | native invasive) species and | | |
| | | pests? Are adequate measures | | |
| | | for preventing such impacts | | |
| | | considered? | | |
| | | (f) In cases the project site is | | |
| | | located at undeveloped areas, is | | |
| | | there a possibility that the new development will result in | | |
| | | extensive loss of natural | | |
| | | environments? | | |
| | | (a) Is there a possibility that | (a) N | (a) Alteration of topographic features and |
| | | alteration of topographic | | tunnel construction are not included in the |
| | (3) Hydrology | features and installation of | | project. |
| | (5) Hydrology | structures, such as tunnels will | | |
| | | adversely affect surface water | | |
| | | and groundwater flows? | | $(a)(b)$ Creall and (a) (for $a \in 1$ Cir. |
| | | (a) Is there any soft ground on the route that may cause slope | (a) N (b) N | (a)(b) Small-scale cutting and filling works |
| | | the route that may cause slope failures or landslides? Are | (b) N (c) N | are included in the construction. However, there are no steep slope areas to occur slope |
| | | adequate measures considered | | failures or landslides in and around the site. |
| | (4) | to prevent slope failures or | | (c) Adequate cutting and filling prevent |
| | Topography | landslides, where needed? | | accidental and sufficient soil runoff. |
| | and Geology | (b) Is there a possibility that | | |
| | - <i>CJ</i> | civil works, such as cutting and | | |
| | | filling will cause slope failures | | |
| | | or landslides? Are adequate | | |
| | | measures considered to prevent | | |

| Category | Environmental Item | Main Check Items slope failures or landslides? (c) Is there a possibility that soil runoff will result from cut and fill areas, waste soil disposal sites, and borrow sites? Are adequate measures taken to prevent soil runoff? | Yes: Y No: N | Confirmation of Environmental Considerations (Reasons, Mitigation Measures) |
|-----------------------------|-----------------------|--|---|---|
| 4 Social Environ ment | (1) Resettlement | (a) Is involuntary resettlement caused by project implementation? If involuntary resettlement is caused, are efforts made to minimize the impacts caused by the resettlement?(b) Is adequate explanation on compensation and resettlement assistance given to affected people prior to resettlement?(c) Is the resettlement plan, including compensation with full replacement costs, restoration of livelihoods and living standards developed based on socioeconomic studies on resettlement?(d) Are the compensations going to be paid prior to the resettlement?(e) Are the compensation policies prepared in document?(f) Does the resettlement plan pay particular attention to vulnerable groups or people, including women, children, the elderly, people below the poverty line, ethnic minorities, and indigenous peoples?(g) Are agreements with the affected people obtained prior to resettlement?(h) Is the organizational framework established to properly implement resettlement?(i) Are any plans developed to monitor the impacts of resettlement?(j) Is the grievance redress mechanism established? | (a) Y(b) Y(c) Y(d) Y(f) Y(g) Y(h) Y(i) Y(j) Y | (a) The project is likely to require involuntary resettlement of about 14 households or 50 persons.(b) Explanations of basic policy on compensation and resettlement assistance have been conducted in the local stakeholder meetings.(c) The Abbreviated Resettlement Action Plan (ARAP) involves the result of socioeconomic survey including the inventory of losses owned by Project Affected Persons (PAPs), compensation with replacement costs and livelihood restoration plan.(d) The compensations will be paid prior to the resettlement.(e) The compensation policies are mentioned in the ARAP.(f) The ARAP considers particular attention to vulnerable people such as owners of small shops.(g) The agreement formation has been conducted in a series of the stakeholder meetings.(h) The organizational framework according to the ARAP has been established to properly implement resettlement. The capacity and budget will be secured to implement the plan.(i) The monitoring and evaluation are mentioned in the ARAP.(j) The grievance redress mechanism is mentioned in the ARAP. |

| Category | Environmental Item | Main Check Items | Yes: Y No: N | Confirmation of Environmental Considerations |
|-----------------------------|------------------------------|---|--|---|
| | Item | | | (Reasons, Mitigation Measures) |
| | (2) Living and Livelihood | (a) Where roads are newly installed, is there a possibility that the project will affect the existing means of transportation and the associated workers? Is there a possibility that the project will cause significant impacts, such as extensive alteration of existing land uses, changes in sources of livelihood, or unemployment? Are adequate measures considered for preventing these impacts? (b) Is there any possibility that the project will adversely affect the living conditions of the inhabitants other than the target population? Are adequate measures considered to reduce the impacts, if necessary? (c) Is there any possibility that diseases, including infectious diseases, such as HIV will be brought due to immigration of workers associated with the project? Are adequate considerations given to public health, if necessary? (d) Is there any possibility that the project will adversely affect road traffic in the surrounding areas (e.g., increase of traffic congestion and traffic accidents)? (e) Is there any possibility that troads will impede the movement of inhabitants? (f) Is there any possibility that structures associated with roads (such as bridges) will cause a sun shading and radio interference? | (a) N (b) Y (c) N (d) Y (e) Y (f) N | (a) Because of improvement project of existing arterial road in developed area, the project will not cause significant adverse changes and impacts on the livelihood of the local people and road traffic in operation phase. (b) The project is likely to require replacement of about 150 shops or offices. The adequate compensations will be paid to the affected owners. (c) Because of improvement project of existing arterial road in developed area, mass immigration from other areas is unlikely to occur. (d) Traffic congestion and control, and relocation of bus stops will be inevitable in construction phase. The proper construction planning and traffic management will mitigate the impact. (e) Due to the improvement of the intersection structure, the project is likely to impede the movement of local inhabitants. Installation of pedestrian bridges will be included in the project. (f) Because the distance between newly constructed bridges and road side is too long and there are no residents around the project site, impact on sun shading and radio interference will not occur. |
| 4 Social Environ ment | (3) Heritage | (a) Is there a possibility that the project will damage the local archeological, historical, cultural, and religious heritage? Are adequate measures considered to protect these sites in accordance with the country's laws? | (a) N | (a) There is no heritage in the site and project affected areas. |
| | (4) Landscape | (a) Is there a possibility that the project will adversely affect the local landscape? Are necessary measures taken? | (a) N | (a) There are no valuable landscape sites in and around the project sites. |

| Category | Environmental Item | Main Check Items | Yes: Y No: N | Confirmation of Environmental Considerations (Reasons, Mitigation Measures) |
|----------|---|--|----------------------------------|---|
| | (5) Ethnic Minorities and Indigenous Peoples | (a) Are considerations given to reduce impacts on the culture and lifestyle of ethnic minorities and indigenous peoples? (b) Are all of the rights of ethnic minorities and indigenous peoples in relation to land and resources to be respected? | (a) N (b) N | (a)(b) The project site is not area where ethnic minorities and indigenous people having unique culture and lifestyle are living |
| | (6) Working Conditions | (a) Is the project proponent not violating any laws and ordinances associated with the working conditions of the country which the project proponent should observe in the project? (b) Are tangible safety considerations in place for individuals involved in the project, such as the installation of safety equipment which prevents industrial accidents, and management of hazardous materials? (c) Are intangible measures being planned and implemented for individuals involved in the project, such as the establishment of a safety and health program, and safety training (including traffic safety and public health) for workers etc.? (d) Are appropriate measures being taken to ensure that security guards involved in the project not to violate safety of other individuals involved, or local residents? | (a) Y (b) Y (c) Y (d) Y | (a) Construction works will comply with the laws and ordinances associated with the working conditions. (b) Because construction works on higher ground are included, tangible safety considerations to prevent labor accidents will be involved in the project. (c)(d) Because the construction works are conducted along existing arterial road in urban area, health program and safety training to construction workers, and considerations to local residents will be included in the environmental management plan. |
| 5 Others | (1) Impacts during Construction | (a) Are adequate measures considered to reduce impacts during construction (e.g., noise, vibrations, turbid water, dust, exhaust gases, and wastes)? (b) If construction activities adversely affect the natural environment (ecosystem), are adequate measures considered to reduce impacts? (c) If construction activities adversely affect the social environment, are adequate measures considered to reduce impacts? | (a) Y (b) Y (c) Y | (a) The adequate mitigation measures including coordination of construction time and methods and monitoring plans to reduce impacts of pollution during the construction will be prepared. (b) The construction activities will not adversely affect the natural environment. (c) Because the construction works are conducted along existing arterial road in urban area, countermeasures against traffic jam will be included in the execution scheme |

| Category | Environmental Item | Main Check Items | Yes: Y No: N | Confirmation of Environmental Considerations (Reasons, Mitigation Measures) |
|----------|---|---|----------------------------------|--|
| | (2) Monitoring | (a) Does the proponent develop and implement monitoring program for the environmental items that are considered to have potential impacts? (b) What are the items, methods and frequencies of the monitoring program? (c) Does the proponent establish an adequate monitoring framework (organization, personnel, equipment, and adequate budget to sustain the monitoring framework)? (d) Are any regulatory requirements pertaining to the monitoring report system identified, such as the format and frequency of reports from the proponent to the regulatory authorities? | (a) Y (b) Y (c) Y (d) Y | (a) The monitoring plans mentioned in the EIA report will be implemented during the construction and operation phase. (b)(c)(d) Because the EIA report is in progress, the specific monitoring plans have not been prepared yet. JICA survey team has submitted the draft monitoring plan to Ghana Highway Authority. |
| 6 Note | Reference to Checklist of Other Sectors | (a) Where necessary, pertinent items described in the Forestry Projects checklist should also be checked (e.g., projects including large areas of deforestation).(b) Where necessary, pertinent items described in the Power Transmission and Distribution Lines checklist should also be checked (e.g., projects including installation of power transmission lines and/or electric distribution facilities). (a) If necessary, the impacts to transboundary or global issues should be confirmed, if | (a) N(b) N (a) N | (a) Deforestation is not included in the project.(b) Relocation of existing power transmission lines will be limited in the right of way and has no serious environmental impacts. (a) Impacts to transboundary or global environmental issues will not occur. |
| | Note on Using Environmental Checklist | necessary (e.g., the project includes factors that may cause problems, such as transboundary waste treatment, acid rain, destruction of the ozone layer, or global warming). | | |

 Regarding the term "Country's Standards" mentioned in the above table, in the event that environmental standards in the country where the project is located diverge significantly from international standards, appropriate environmental considerations are required to be made. In cases where local environmental regulations are yet to be established in some areas, considerations should be made based on comparisons with appropriate standards of other countries (including Japan's experience).

2) Environmental checklist provides general environmental items to be checked. It may be necessary to add or delete an item taking into account the characteristics of the project and the particular circumstances of the country and locality in which it is located.

Chapter2 Contents of the Project

2.1 Basic Concept of the Project

2.1.1 Requested Scope

The request made by the GOG is a phase wise improvement of Tema intersection. The scopes of each phase are as follows;

- Phase-1: Construction of 1st tier flyover, including the detailed design and construction supervision
- Phase-2: Construction of 2nd tier flyover, including the detailed design and construction supervision

2.1.2 Survey Objective

The objective of the Survey was to;

- Understand the background, purpose, and scope of project under the Japanese grant aid assistance,
- Study the feasibility of the project in terms of effectiveness, technical and economic justification,
- Conduct outline design for minimum but optimal scope and scale of the project,
- Estimate project cost, and
- Propose the contents, implementation and maintenance plan as well as critical points to be undertaken by the GOG to achieve the outcome and targets set for the project.

2.1.3 Overall Goal, Targets and Expected Outcomes

The overall goal of the project is to contribute to the growth of the economy of Ghana and that of the West African Region. The purposes of the project on the other hand are;

- To improve the capacity of the Tema Roundabout and its legs (approach roads)
- To improve safety and efficiency of transport in the Greater Accra Region; and
- To provide uninterrupted traffic flows to facilitate trade and transit in the West African Sub-Region.

Furthermore, the outline of the project is as summarized in Table 2.1.3-1.

| Targets | Ultimate: Enhancement of transport efficiency and logistics in GAR | | | | |
|------------------|---|---|--|--|--|
| | Immediate: | Improve traffic condition of traffics travelling along central Accra region and | | | |
| | | Tema Port | | | |
| Expected Outcome | Graded intersection of Tema Roundabout | | | | |
| Location | Greater Accra Region(GAR), Tema City | | | | |
| Beneficiaries | Direct beneficiaries: Users of Tema Roundabout and people in the vicinity | | | | |
| | Indirect bene | ficiaries: 4 million people of GAR, Eastern International Corridor users | | | |

Table 2.1.3-1 Outline of the Project

2.2 Outline Design of the Requested Japanese Assistance

2.2.1 Design Policy

2.2.1.1 Policies for Requested Scopes

A preliminary comparison of applicable configurations for the improvement of the existing roundabout was carried out and explained to the GOG during the IC/R meetings. However, the configurations compared were developed based on limited data and documents obtained prior to the commencement of the project. Therefore, to ensure effective and efficient improvement, comparison was repeated using data and information collected from various engineering surveys, discussions and hearings conducted with GHA and other stakeholders during the course of the first field survey. The factors such as cost effectiveness, urgency, social and environmental impacts were evaluated and the project's technical and economic viability and thus its justification for Japan's grant aid scheme was examined. The project scope was then determined, discussed with JICA, explained to GHA and sought GHA's agreement. The agreed scope of the project is shown in **Table 2.2.1-1**.

Initially, the GOG had its original plan for the improvement. This plan applies a configuration that seems to be aimed not only for the improvement to ease congestion at the roundabout but for conversion of the intersection to a full-control multi-layer interchange with justification that all connecting roads are motorways. However, the GOG agreed to drop out this configuration following explanation from the Survey Team that adoption of the configuration under the grant aid of Japan is not viable as the scope is vast, requires significant land acquisition including large number of resettlement of project affected people, and relocation of service utilities.

| Requested Scope | Target |
|--|--|
| <u>Phase-1</u> Construction of 1 st level flyover including improvement of the objective roundabout, the detailed design of a graded intersection and construction supervision | O (to be undertaken by this project) |
| <u>Phase -2</u> Construction of 2 nd level flyover including, the detailed design of a flyover and construction supervision | _ |

 Table 2.2.1-1 Project Scope

2.2.1.2 Policies for Overall Improvement

(1) Consistency with Phase-2 Works

As aforementioned, the project scopes of this survey cover the components of Phase-1 only. However, planning of the component of Phase-2 is inevitable for the planning of Phase-1 components as the two phases need to be consistent with each other. Therefore, outline design of Phase-1 components is performed following preliminary design for the components of Phase-2. The design of Phase-1 components is carried out in such a manner that the project outcome is attained while construction efficiency of Phase-2 is also guaranteed.

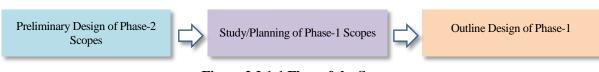


Figure 2.2.1-1 Flow of the Survey

(2) Target Year

In general, it is difficult to predict changes of regional economy, population and development plan of the target area beyond 20 years. Thus, target year for this Project is taken as 2035 that is 20 years from 2015. The same target year will be applied for future traffic volume, which is the governing factor of the project scale.

(3) Limits of Improvement Scope

The improvement of Tema Motorway Roundabout is expected to alleviate the traffic congestion in the project area significantly. However, this may not be able to eliminate the congestion totally. Elimination of the congestion is achievable only if a comprehensive improvement that will undertake simultaneous improvement of sections beyond those undertaken in this improvement or improvement of other roads, interchanges and various facilities is executed. For example, in the case of Tema-Akosombo Road, it might be necessary to improve the road all the way up to Ashaiman Roundabout (about 1.5 km north of Tema Motorway Roundabout) or even further as severe traffic congestion is prominent here. Therefore, it might be necessary to improve not only Tema roundabout but also Ashaiman Roundabout and the connecting road. The limits of improvement scope (start and end points) to be undertaken by the grant aid of Japan however, will be between the points that will be affected by the improvement plan of the existing roundabout. The GOG is requested to undertake the improvement of other sections at its own expenses considering the importance and necessities.

(4) Consistency with the Existing Plans

Information received by the Survey Team revealed that plans for improvement of the objective intersection exists. For example, Ghana Ports and Harbour Authority (GPHA) is said to be planning improvement/widening of the Tema Port access road (Hospital Road: linking the port and Ashaiman Interchange), which is scheduled to be carried out by Department of Urban Roads (DUR). On the other hand, GHA is also under the process of carrying out the improvement (widening) of Tema-Accra Motorway under the Public-Private-Partnership (PPP) Scheme, which envisages to widen the motorway to six-lane road in accordance with the requirement of ECOWAS. Furthermore, there are plans for widening of Harbour Road also. However, all the above-mentioned projects are either at a conceptual stage or are yet to kick off.

Therefore, the survey carried out assumed that there were no existing plans under progress that could potentially create conflicts with this plan and that the other to-be implemented improvement plans shall be made consistent with the output of this project.

(5) Consistency with National and Regional Plans

The Tema Roundabout lies on Abidjan-Lagos Highway, which is a part of the Dakar to Lagos Trans African

Highway (TAH) No. 7. The vision of the ECOWAS is to have 3-lane dual carriageway from Lagos to Abidjan in the short term. Therefore, it is important that the improvement plan should account for the design requirement for the Lagos-Abidjan Highway as well as initiatives for the Tema Port development, and be "fit for purpose" to realize long-term solution for streamlining the international and domestic traffic flows interrupted by the congestion at the roundabout. However, the existing Accra-Tema Motorway is currently a 2-lane dual carriageway road and the Aflao Road becomes 2-lane single carriage road from about a few kilometers beyond the Tema Roundabout. Under such situation, it may not be ideal to provide 3-lane dual carriageway if it is not required within the target year. In such a case, the improvement plan will provide number of lanes as derived from the traffic demand forecast. However, space for 3-lanes will be secured for widening by GOG in the future necessarily.

(6) Levels of Service

Level of Service (LOS) is defined as the operating conditions on the facility in terms of traffic performance measures related to speed and travel time, freedom to maneuver, traffic interruptions, and comfort and convenience. **Table 2.2.1-2** shows the general relation between LOS and the general operating conditions.

| Levels of Service General Operating Condi | | | |
|---|---------------------------|--|--|
| А | Free flow | | |
| В | Reasonable free flow | | |
| С | Stable flow | | |
| D | Approaching unstable flow | | |
| E | Unstable flow | | |
| F | Forced or breakdown flow | | |

 Table 2.2.1-2 General Definitions of Levels of Service

Source: Highway Capacity Manual

Technical Notes (T/N) compiled that served as agreement on basic technical conditions for the design was approved by GHA on 20th May and 18th December, 2015. In this T/N, LOS of each road is mentioned as 'B' or 'C'. Essentially, LOS becomes lower during peak traffic hours. Taking high LOS as targets for the design stage will result in an excessive design of roads and structures. To ensure technical and economic viability of the project to be implemented under Japan's grant aid scheme, the target LOS during morning hours for improvement is taken to be 'D' (Refer to **Figure 2.2.2-1**, page **70**).

On the other hand, it is noteworthy that the future traffic demand, which is fundamental for the improvement plan, has been calculated with an assumption that until year 2035 there are no development/improvement works of road network in the neighboring region of Tema Roundabout. This means, possibility of changes in traffic pattern (LOS improvement due to diversion of traffic currently using the roundabout to improved or newly developed roads in the area), has not been considered.

(7) Imposition of One-way Traffic

As aforementioned, the objective intersection is composed of 5-legs of which 4 roads are arterials while one road (the Tema-Hospital Road, also known locally as the Container Road) is an urban road. To make the intersection simple, safe and effective, it is preferable that the improved intersection is composed of same class roads (4 trunk road legs). Therefore, the improvement plan excludes the Tema-Hospital Road and treats it as one-way traffic flow road restricting traffics from entering the intersection. Road function of Tema-Hospital road shall be separately studied by planning a service road.

(8) Crossing Method

Motorway - Aflao Road runs in the east-west direction. Harbour Road - Akosombo Road runs in the northsouth direction. Intersection at different levels is required to separate these two directions. During phase-2, Motorway-Aflao Road is planned to run through an underpass (1st tier), turning ramps on the grade level (2nd tier) and Harbour Road-Akosombo Road over a flyover (3rd tier). The reasons for such type of consideration are;

- Motorway Aflao Road is a high priority international corridor and is a part of Abidjan-Lagos Highway where through traffics should be allowed without conflicts.
- Even though North-South direction will be improved by provision of a fly-over, it is not expected to be effective enough until one of the connecting roads (Akosombo Road), which is currently a two-lane single carriageway is widened.
- Project scale would have been much larger if expansion of Akosombo Road from a two-lane single carriageway to a two-lane dual-carriageway was included in this project and would have possibly lowered viability of the project to be implemented under the grant aid of Japan, in terms of urgency and economy.
- Construction of the flyover is easier if the road to be constructed prior to it is an underpass.

(9) Comparison of Improvement Methods

Several improvement methods were selected and compared based on the policies mentioned earlier. The optimum plan that contributes in achieving the following outcomes items the most was selected.

- Improvement of the capacity of Tema intersection, its approach roads and other roads in the area,
- · Enhance safety and efficiency of transport in the Greater Accra Region; and
- Provide uninterrupted traffic flows to facilitate trade and transit in the West African Sub-Region

2.2.1.3 Policies for Engineering Surveys

Engineering surveys were conducted with the purpose to collect higher level information, details and relevant data to be used for planning, design, and cost estimate for the improvement of the roundabout. Since the roundabout connects arterial roads in an urban area, it is necessary for studies to be conducted at each stage of the project, thus during design stage, construction stage and in-service stage. On the other hand, the higher volume of traffic currently being experienced at the roundabout as well as the existence of numerous

underground facilities means the project would not only affect residents living in the vicinity but road users also. Therefore, engineering survey needs to be comprehensively done.

After clarifying the issues and problems, various engineering surveys were carried out so that necessary data for designing can be sufficiently obtained.

The scope of works is shown in **Table 2.2.1-3** and the outline of the surveys conducted is briefly discussed thereafter.

| Surveys | Purposes | Locations | Scopes | Methodologies |
|-----------------------------------|---|--|--|--|
| 1. Climate/Hydrology | To understand as well as to collect the data for conducting suitable construction planning | In and around Tema Motorway Roundabout | Temperature, precipitation, water level of gullies, natural disaster, etc. | Collection of available data and site observation |
| 2. Topography Survey | To collect data and information on the features of the site to prepare the documents required for conducting the design works | In and around Tema Motorway Roundabout | Longitudinal survey Cross section survey Plane table survey Temp. bench mark | Subcontracting |
| 3. Geo-Technical Investigation | For collecting data to understand the soil properties and its distribution condition for conducting design of pavement and the facilities of the objective road | Areas along objective road and borrow pit | Site investigation Drilling, SPT Test Groundwater level Trial Pit and CBR test DCP test Fill material Base course material | Subcontracting |
| 4. Groundwater Observation | To collect data of groundwater level | In and around Tema Roundabout | Monthly groundwater level | Subcontracting |
| 5. Inventory Survey | To comprehend the existing condition of road incidental facilities so that it could be reflected in the design work | Areas along objective road | Existing Inventory Survey | Collection of available data and site observation |
| 6. Traffic Survey | To collect Traffic data for future traffic demand forecast | On all the roads connecting to Tema Roundabout | Counting during peak hour | Subcontracting |
| 7. Existing Drainage Survey | To comprehend the existing condition of road incidental facilities so that it could be reflected in the design work | Areas along objective road | Confirm the existing drainage systems/ trends Grasp the existing provision of drainage facilities Confirm the existing drainage downstream end | Collection of available data and site observation |
| 8. Other Surveys | To collect the information and data to consider for design | Areas along objective road | Existing pavement Current traffic operation Pedestrian volume Pavement surface condition Land use Gender and Population Economic & industrial survey | Collection of available data and site observation |

 Table 2.2.1-3 Engineering Surveys

(1) Climate and Hydrology

As mentioned in Section 1.4 , climate at Tema, where the objective roundabout is located have dry equatorial climate. The maximum temperature lies between 27 and 33 degrees Celsius and the minimum temperature between 22 and 26 degrees Celsius.

Monsoon lies between March and October. The precipitation time varies but day-long rainfall is very rare if not nil. The maximum rainfall is seen between May and June where the monthly volume is about 100mm.

(2) Topographic Survey

1) Survey Objective

Topographic survey was carried out to capture the terrain condition including features on the ground such as houses, buildings, trees and shapes and structures of the roundabout and the existing roads etc. to prepare the documents required for conducting the outline design.

2) Survey Scopes

The scopes of the survey are shown in **Figure 2.2.1-2**. The red line boundary is the limits of plane table survey. Scopes of longitudinal and cross section survey are highlighted in blue.

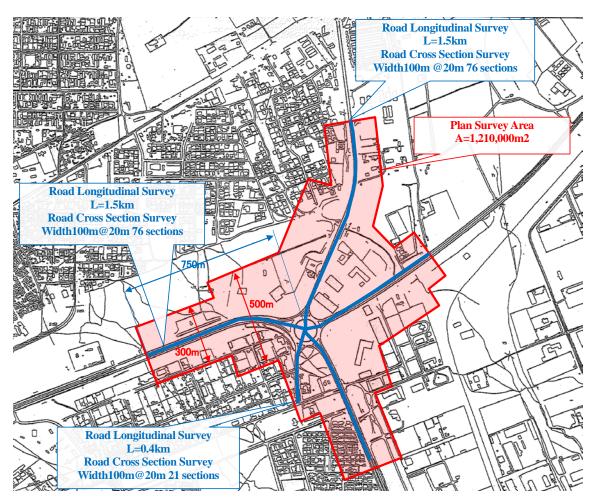


Figure 2.2.1-2 Area and Scope of Topographic Survey

3) Survey Period

The survey was conducted during the first field survey from April to May 2015. Additional survey to supplement the output of the previous survey was conducted between November and December 2015.

4) Survey Items

The survey included the following items;

5) Establishment of Bench Marks

Level survey was conducted to install benchmarks (concrete stakes) at 9 locations within the area shown in **Figure 2.2.1-2**. The coordinates and levels of these bench marks have been referred from the National Bench Mark (NBM). The coordinates in terms of both Ghana Grid and WGS84 and the elevation of the referred national benchmarks and the temporary benchmarks installed are shown in **Table 2.2.1-4**, its locations in **Figure 2.2.1-3**, and the type of these benchmarks are depicted in **Photo 2.2.1-1**.

| Bench marks | ID | Abbreviation on the map | Ghana Northing | Easting | WGS Northing | Easting | Elevation / Height (m) | Remarks |
|-------------------------|----------------|-------------------------|-------------------|------------|-----------------|-----------|---------------------------|--------------------|
| s | | | (m) | (m) | (m) | (m) | | |
| nal nark | SS66 | - | 101849.410 | 364862.714 | 618654.342 | 812175.92 | 51.452 | Existing |
| National Benchmarks | SG/11/01/GPS3 | GPS3 | 112354.665 | 383339.21 | 629233.633 | 830634.81 | 35.294 | |
| Ber N | GAMA/T/11/11/1 | GAMA1 | 112374.167 | 383399.017 | 629253.324 | 830694.53 | 35.524 | |
| | | | | | | | | |
| Ş | TMRP/T/04/15/1 | BM1 | 112195.225 | 382521.283 | 629071.14 | 829816.50 | 24.842 | Newly installed |
| Project Benchmarks | TMRP/T/04/15/2 | BM2 | 111779.614 | 383670.685 | 628659.08 | 830968.55 | 32.596 | |
| | TMRP/T/04/15/3 | BM3 | 112882.703 | 384116.274 | 629764.87 | 831410.78 | 28.546 | |
| | TMRP/T/04/15/4 | BM4 | 113288.864 | 383593.825 | 630169.65 | 830886.37 | 42.018 | |
| | | | | | | | | |
| Temporary Benchmarks | TBM/T/04/15/1 | TBM1 | 112029.785 | 383332.938 | 628908.34 | 830629.58 | 33.465 | |
| | TBM/T/04/15/2 | TBM2 | 111873.468 | 383607.266 | 628752.81 | 830904.73 | 32.186 | |
| | TBM/T/04/15/3 | ТВМЗ | 112864.716 | 384011.312 | 629746.50 | 831305.77 | 29.226 | Newly installed |
| | TBM/T/04/15/4 | TBM4 | 113206.962 | 383597.121 | 630087.67 | 830889.95 | 40.669 | |
| | TBM/T/04/15/5 | TBM5 | 112259.604 | 382672.034 | 629136.11 | 829967.19 | 24.522 | |

Table 2.2.1-4 Details of Benchmarks Installed

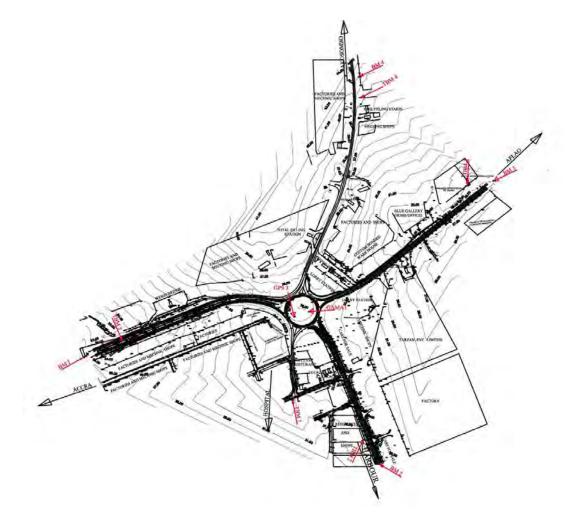


Figure 2.2.1-3 Location of the Benchmarks



Photo 2.2.1-1 Project Benchmarks

6) Plan Survey

The plan survey of the topography, land use and existing drainage facilities were carried out for an approximate area of 121 hectares as shown in **Figure 2.2.1-2**. Features such as houses, drainage facilities, fences, poles, plantations, trees etc. were also measured. All these features are reflected in the topographic survey map (drawing).

7) Road Alignment/Longitudinal Survey

Centerline (tentative centerline) survey was carried out followed by longitudinal survey along all the legs of the roundabout. The length covered on each leg, as shown in **Figure 2.2.1-2** is 850m each from the roundabout in the east-west direction, 1,000m to north direction (Akosombo Road) and 400m for Hospital Road. The survey results are incorporated in the profile drawing.

8) Cross Section Survey

Cross Section Survey at 20 m intervals was conducted along the centerline for all legs of the roundabout. The elevation of undulations on the cross section were surveyed and correlated with the TBM. The results were incorporated in the cross section drawings.

(3) Geo-technical Investigation

1) Objectives

The objectives of the investigations were;

- To provide specific site information to confirm and amplify the geotechnical and geomorphologic findings of the desk study;
- To obtain detailed knowledge of the soils to be encountered at the site and their likely behavior on substructures ;
- To foresee and provide against difficulties and delays that may arise during construction due to groundwater and other local conditions;
- To establish design parameters and present basis for the design of substructures.

2) Contents

The investigation consists of field work and laboratory work. The scopes of each work are shown in **Table 2.2.1-5**.

| Items | Scopes (works performed) | | |
|-----------------|--|--|--|
| 1 Castashrical | 1. Exploratory borings with standard penetration tests, sampling and | | |
| 1. Geotechnical | 2. confirmation of natural ground water level | | |
| | 3. Laboratory soil tests on samples collected from boreholes | | |
| | 4. CBR tests at the existing road and the planed road | | |
| | 5. Dynamic Cone Penetration test, and | | |

| Items | Scopes (works performed) | |
|-------------|--|--|
| 2. Material | 1. Laboratory tests on soil samples, and | |
| | 2. Laboratory tests on aggregate samples | |

3) Investigation Period

The investigation works commenced in the middle of April 2015 and completed in the middle of June 2015.

4) Results of Investigation

A) Geotechnical Investigation

The approach adopted for the geotechnical investigation consisted of:

- · Desk study and field reconnaissance (geological and geotechnical information on the project area)
- · Drilling and sample extraction (soil and rock (in case of weathered rocks) samples.
- Laboratory testing of selected soil and rock samples from the drilling holes.

The ground investigation was undertaken in accordance with the ASTM D220. For drilling and sample extraction, five (5) boreholes were drilled at the site. The location of the boreholes are indicated in **Figure 2.2.1-4**. In-situ Standard Penetration Test (SPT) was carried out at every 1m interval up to till refusal was recorded.

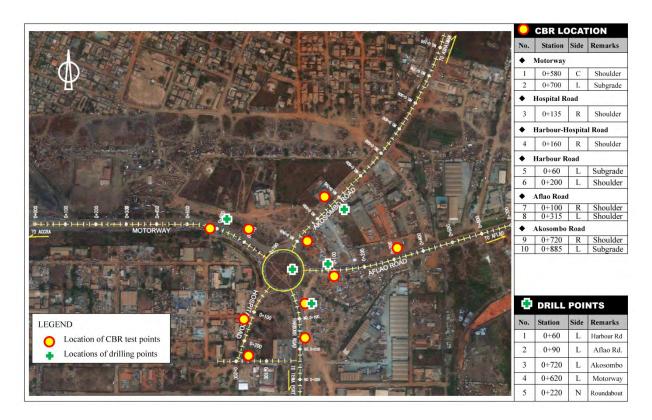


Figure 2.2.1-4 Location of Drilling Points and CBR Test

a) Laboratory Testing

The disturbed and SPT samples recovered from the boreholes have been tested at the laboratory for the following items;

- Moisture/water content (ASTM D2216)
- Unit weight (ASTM C29)
- Specific gravity (ASTM D854)
- Grading/Sieve analysis (sieving + Hydrometer test, ASTM D422)
- Liquid and plastic limits (ASTM D431)

b) Findings

The profiles of each bore holes are shown in **Figure 2.2.1-5**. The composition of the ground, soil distribution layer and its N-value and soil properties for all five (5) locations where drilling was conducted are summarized in **Table 2.2.1-6**. It was confirmed that weathered rock of which N value is greater than 50 exists in the ground about 10m deep. Therefore, this shall be regarded as foundation ground.

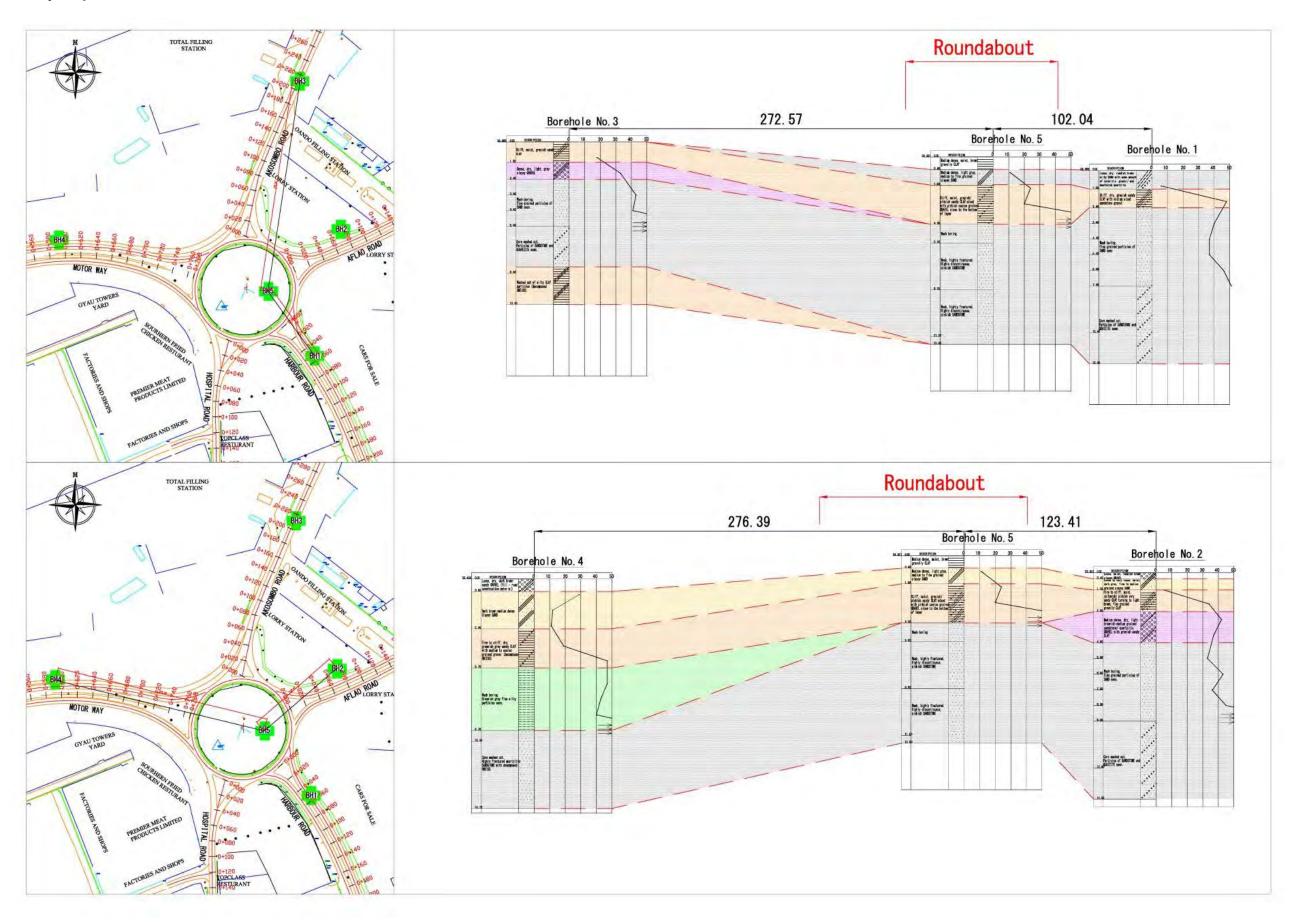


Figure 2.2.1-5 Profile of Boreholes

| No. | Depth (m) | Layer Thickness (m) | Soil Properties N-value (Nos.) | | Remarks | | |
|------|--------------|---------------------------|--|-------|--------------|--|--|
| | 1.2 | 1.2 | Loose, dry, reddish brown silty SAND | 6 | | | |
| BH1 | 2.4 | 1.2 | Stiff, dry, greyish sandy CLAY with medium sized sandstone particles. | 48 | Auger-boring | | |
| | 7.4 | 5.0 | SANDSTONE and QUARTZITE 33-51 Wash | | | | |
| | >12.4 | >5m | Rock | >50 | Refusal | | |
| BH2 | 0.4 | 0.4 | Loose, moist, reddish brown clayey GRAVEL (fill) | - | | | |
| | 1.1 | 0.7 | Loose to very loose, moist, dark grey,5fine to medium grained clayey SAND | | | | |
| | 2.5 | 1.4 | Firm to stiff, moist, yellowish brown sandy CLAY | 34 | Auger-boring | | |
| | 4.5 | 2.0 | Medium dense, dry, light brownish medium grained sandstone/ quarzitic GRAVEL with greyish sandy CLAY | 37-42 | | | |
| | 9.5 | 5.0 | Fine grained particles of SAND | 33-45 | Wash out | | |
| | >14.5 | 5m> | Fine grained particles of SAND | >50 | Refusal | | |
| внз | 1.3 | 1.3 | Stiff, moist, greyish sandy CLAY | - | Auger-boring | | |
| | 2.4 | 1.1 | Dense, dry, light grey clayey GRAVEL | 18 | | | |
| | 5.4 | 3.0 | Fine grained particles of SAND | | | | |
| | 8.4 | 3.0 | Particles of SANDSTONE and QUARTZITE | | | | |
| | >10.4 | >2.0 | silty CLAY (decomposed GNEISS) | >50 | Refusal | | |
| | 0.8 | 0.8 | Loose, dry, dark brown sandy GRAVEL(Fill Material) | - | | | |
| BH4 | 5.7 | 4.9 | Firm to stiff, dry, greenish grey sandy CLAY11-29AugerGreenish grey silty CLAY particles40-49 | | Auger-boring | | |
| DII4 | 9.7 | 4.0 | | | | | |
| | 12.7 | 3.0 | | 50 | Wash out | | |
| | >14.7 | >2.0 | Decomposed gneiss | >50 | Refusal | | |
| | 0.8 | 0.8 | Medium dense, moist, brown gravelly CLAY | - | | | |
| BH5 | 1.8 | 1.0 | Medium dense, light grey, medium to 11 fine grained clayey SAND | | Auger-boring | | |
| | 4.3 | 2.5 | Stiff, moist, sandy CLAY with coarse grained GRAVEL | 20-50 | | | |
| | 8.5 | 4.2 | Highly functioned Consideration | > 50 | Wash out | | |
| | >12.0 | >3.5 | Highly fractured Sandstone | >50 | Refusal | | |

Table 2.2.1-6 Summary of Drilling Results

c) Others

i) Earthquake Considerations

The range of horizontal ground acceleration, A, is usually equal to or greater than 0.36g for zones with these characteristics. The modified Ghana Building Code Part 3 (1988) classifies

the area as lying in zone III in terms of seismic activity. The range of horizontal ground acceleration, A, is usually equal to or greater than 0.35g for this zone. The code recommends that a minimum horizontal acceleration of 0.35g should be used in designing important structures in this zone.

ii) Groundwater

Drilling carried out for geo-technical investigation during first site survey could not confirm existence of groundwater. An alternative method was applied by providing observation wells at 5 locations within the project area and the groundwater level was recorded. Detail is provided in Section 2.2.1.3 (4) .

B) Material Investigation

a) Trial Pitting

Trial pitting was carried out manually using pick-axes and shovels at ten (10) locations along all the legs of the roundabout. All but three of the ten test pits were sunk at the road shoulders rather than the center-line to ensure that road usage was not interfered with during the period of fieldwork. The other three trial pits were sunk outside the existing roadway where the newly planned road layout is expected to be.

b) Dynamic Cone Penetration

To evaluate the integrity and strength characteristics of the existing pavement and the subgrade, DCP testing was carried out at ten (10) locations along all the 5 legs of roads connecting the motorway roundabout.

c) Laboratory Testing

The bulk samples taken from the trial pits were subjected to standard identification and classification tests for the determination of the following parameters:

- Soil Classification (USCS)
- Moisture/water content (ASTM D2216)
- Specific gravity (ASTM D854)
- Compaction tests (4.5kg rammer method, AASHTO T180)
- Soaked CBR test (AASHTO T193)

The laboratory-testing program was carried out using methods recommended in the relevant ASTM Standards.

d) Findings

Almost all the pavement structure on all the five-leg roads adjoining the Tema Motorway Roundabout is essentially of the same material quality. Except for Motorway leg, the pavement structures are constructed of crushed-stone base course overlying a well compacted, reddish brown, fine to medium coarse, lateritic gravel material- serving in almost all cases as both the sub-base and subgrade material. This lateritic type is reasonably well graded. It appears to have been taken from the only known soil pit near the project area - the TDC Kpone gravel pit which is now disused. On the hospital roads, however, the subgrades are made of moist, brownish grey, moderately dense clayey sand. All the in-situ CBR values obtained from the DPCTs were higher that their corresponding values obtained from the soaked 96 hr. submersion in water. This may well be explained by the fact that the investigations were carried out at a time of dry weather condition where the in-situ CBRs were expected to be their highest

(4) Groundwater Level Observation

1) Purpose

To study the countermeasures to be taken for structure and road design in the depressed section in the east-west direction, groundwater level observation has been conducted at the observation wells which were installed at 2 locations near Tema Roundabout.

2) Locations and Observation Period

Groundwater level observation was conducted at locations shown in **Figure 2.2.1-6**. Observation has been continued since November 2015 and will last for 1 year.

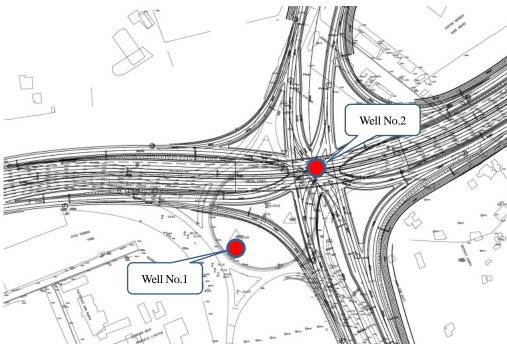


Figure 2.2.1-6 Location of Observation Well

3) Observation Method

Observation wells were installed to a depth of about 5 meter from the surface of foundation ground with N value greater than 50. Observation of groundwater was done twice every month from December 2015 till July 2016. A Well sounder equipped with detector was used as a measuring tool to monitor the groundwater level.

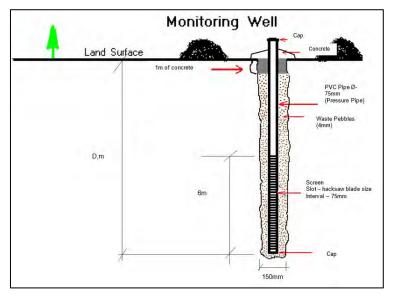
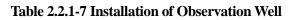


Figure 2.2.1-7 Detail of Observation Well





4) Results and Findings

Observation results are summarized in **Table 2.2.1-8**. There are not large differences between dry season (December) and rainy season (May). However, as existence of groundwater was confirmed, countermeasures to the groundwater shall be considered for structure design and pavement design.

(5) Road Inventory Survey

1) Objectives

The objectives of road inventory survey are;

- To know the current situation of the area that will be affected by this project in the future;
- To obtain detailed information to study drainage plan and service roads at the outline design stage;
- To check the topographic survey result.

 Table 2.2.1-8 Observation Results

| DATE | TIME | WELL 1 (m) | WELL 2 (m) |
|------------|---------|------------|------------|
| 2015/12/2 | 9:38am | 6.10 | 6.54 |
| 2015/12/15 | 14:00pm | 5.90 | 6.46 |
| 2015/12/17 | 1:45pm | 6.90 | 6.36 |
| 2015/12/23 | 1:15pm | 7.10 | 6.44 |
| 2016/1/7 | 10:00am | 7.25 | 6.57 |
| 2016/1/20 | 10:30am | 7.39 | 6.62 |
| 2016/2/3 | 12:10pm | 7.43 | 6.67 |
| 2016/2/17 | 10:35am | 7.50 | 6.75 |
| 2016/3/2 | 9:00am | 7.33 | 6.69 |
| 2016/3/17 | 9:45am | 7.34 | 6.69 |
| 2016/5/13 | 10:23am | 7.45 | 6.69 |
| 2016/6/3 | 11:30am | 7.47 | 6.69 |
| 2016/6/17 | 10:23am | 6.85 | 6.22 |
| 2016/7/7 | 11:15am | 6.76 | 6.2 |

2) Survey Period and Survey Items

Inventory survey of the roads was conducted from middle of April till middle of June 2015. Inventory items, its breakdown and inventory methods are listed in **Table 2.2.1-9**.

| Survey Items | Breakdown | Methodology | | | | |
|---------------------------------|--|---|--|--|--|--|
| 1. Road reserves | • ROW Stakes | Using optical distance measuring device | | | | |
| 2. Road parameters/condition | Pavement condition Carriage way Shoulder Median Curb Sidewalk | | | | | |
| 3. Drainage | Route Flow direction Shape and material of drainage structures | By using tape measure | | | | |
| 4. Access to abutting buildings | Private houses Gas station Bus station Shop | | | | | |

| Table 2.2.1-9 Inventory Item | S |
|------------------------------|---|
|------------------------------|---|

(6) Underground Facility Survey

1) Objectives

The objectives of the survey are;

- To obtain detailed information to study the improvement plan of Tema Motorway Roundabout, and
- To provide information to GHA and relevant agencies of the underground facilities that might be affected by/during the construction.

2) Survey Period

The Survey commenced in the middle of May 2015 and completed at the end of May 2015.

3) Survey Items

Survey items, responsible companies and methodology of the survey are summarized in Table 2.2.1-10.

| Survey Items | Company | Methodology |
|----------------------------|--|---|
| 1. Water pipe | • Ghana Water Company Limited (GWCL) | |
| 2.Communication pipe/cable | MTN Vodafone Milicom Ghana (TIGO) GLO | By collecting data or information from the companies and trial digging at site. |
| 3. Electricity pipe/cable | • Electricity Company of Ghana (ECG) | |

Table 2.2.1-10 Survey Items

4) Findings

The findings from this survey are as illustrated in **Figure 2.2.1-8**. As is apparent from the figure, existence of several utilities (service lines) was confirmed. Removal of these service lines that will be affected by the improvement plan must be done under the responsibility of the GOG.

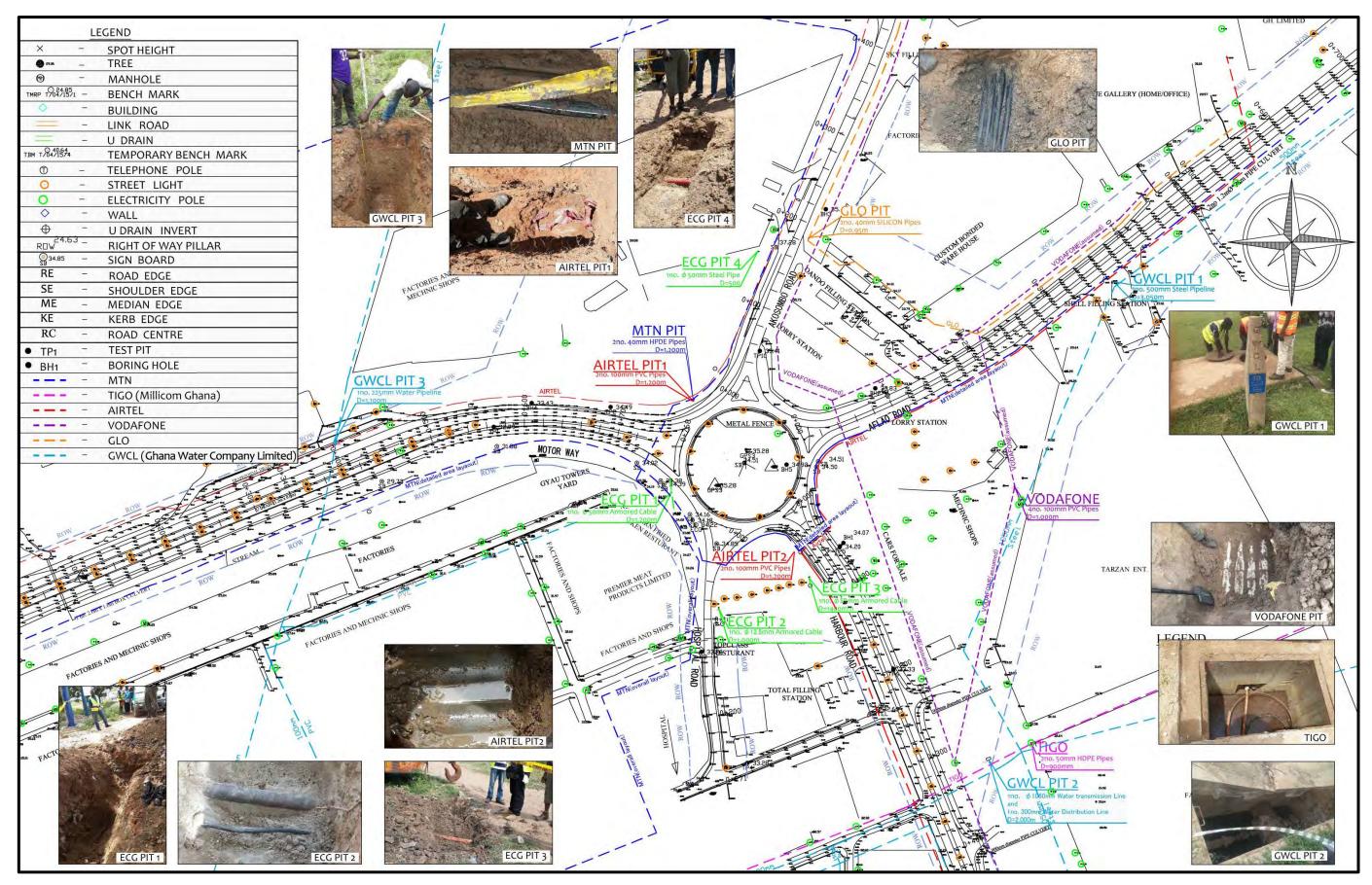


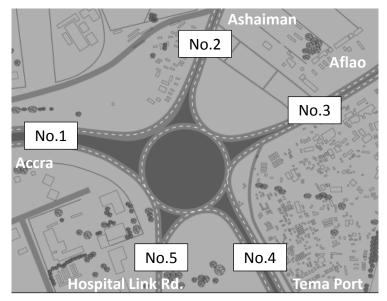
Figure 2.2.1-8 Underground Facility Map

(7) Traffic Survey

1) Implementation of Traffic Survey

This chapter summarizes the results of the traffic survey with an aim to provide basic data for formulating an improvement plan of Tema Motorway Roundabout along with the results of traffic demand forecast.

Traffic surveys were carried out on all 5 legs of the existing Tema roundabout shown in **Figure 2.2.1-9**. The survey consisted of four items as shown in **Table 2.2.1-11**, and the survey was subcontracted to a local consultant. The traffic survey was implemented from 14 to 17 April 2015.



Source: JICA Survey Team

Figure 2.2.1-9 Location of Traffic Survey Stations

| Survey Item | Survey Contents |
|----------------------------------|--|
| Traffic count survey | 1 weekday, 24 hours, by vehicle type, by direction |
| Turning volume count survey | 1 weekday, morning and evening peak 2 hours, by |
| | vehicle type, by direction |
| Traffic congestion length survey | 1 weekday, morning and evening peak time |
| Travel speed survey | 1 weekday, morning and evening peak time |

 Table 2.2.1-11 Summary of Traffic Survey

Source: JICA Study Team

The classification of vehicles in the survey was done in accordance with the classification of vehicles in the GHA pavement design manual. In this survey, the vehicle types were classified into eight (8) types, as shown in **Table 2.2.1-12**.

| Classification of vehicles in the | |
|-----------------------------------|--------------------------------|
| survey | GHA classification of vehicles |
| 1. Motor Cycle | Motor bike |
| | Car |
| 2. Passenger Car | Taxi |
| | Pick-up/Van/4WD vehicle |
| 3. Minibus | Small bus |
| 4. Bus | Medium bus/Mammy wagon |
| 4. Dus | Large bus |
| 5. Light Truck | Light truck |
| 6 Haarn Truck | Medium truck |
| 6. Heavy Truck | Heavy truck |
| 7. Trailer | Semi-trailer (Light, Heavy) |
| /. 11anci | Truck-trailer |
| 8. Others | Extra-large truck & others |
| Source: IICA Survey Team | |

Source: JICA Survey Team

2) Results of Traffic Volume Survey on Road Sections

Table 2.2.1-13 shows the results of the road section traffic survey for each leg of the roundabout. The maximum traffic volume was counted at station 3 on the Tema-Aflao Road, with 33,169 vehicles per day. This was followed by the total traffic volume at station 2 on the Tema-Akasombo Road with 29,505 vehicles per day, station 4 on the Tema-Harbour Road with 28,290 vehicles per day, and station 1 on the Accra-Tema Motorway of 23,854 vehicles per day. The traffic volume at station 5 was 12,855 vehicles per day. Looking at the composition of vehicle types, the highest proportion of large size vehicle was on Harbour Road at 15%. Next was station 1 on the Accra-Tema Motorway at 12%. The other locations had percentages values not exceeding 10%.

| Classification | No.1 Accra-Tema Motorway | | No.2 Tema-Akosombo Rd. | | No.3 Tema-Afla | o Rd. | No.4 Tema Harbo | ur Rd. | No.5 Tema-Hospital Rd. | |
|---------------------------|-----------------------------|------|---------------------------|------|-------------------|-------|--------------------|--------|---------------------------|------|
| | Volume | % | Volume | % | Volume | % | Volume | % | Volume | % |
| Motorcycle | 1,977 | 8% | 2,230 | 8% | 1531 | 5% | 1846 | 7% | 746 | 6% |
| Car & Taxi | 11,885 | 50% | 18,975 | 64% | 21875 | 66% | 16330 | 58% | 9750 | 76% |
| Minibus | 4,643 | 19% | 4,716 | 16% | 5379 | 16% | 4771 | 17% | 1252 | 10% |
| Bus | 177 | 1% | 607 | 2% | 329 | 1% | 983 | 3% | 81 | 1% |
| Light Truck | 2,393 | 10% | 1,369 | 5% | 1705 | 5% | 1232 | 4% | 520 | 4% |
| Heavy Truck | 1,218 | 5% | 951 | 3% | 993 | 3% | 1256 | 4% | 292 | 2% |
| Trailer | 1,158 | 5% | 484 | 2% | 1037 | 3% | 1387 | 5% | 55 | 0% |
| Others | 403 | 2% | 173 | 1% | 320 | 1% | 485 | 2% | 189 | 1% |
| Total (All type of Veh.) | 23,854 | 100% | 29,505 | 100% | 33,169 | 100% | 28,290 | 100% | 12,885 | 100% |
| Share of Large size Veh. | 2956 | 12% | 2215 | 8% | 2679 | 8% | 4111 | 15% | 617 | 5% |
| Share of Large size truck | 2,779 | 12% | 1608 | 5% | 2350 | 7% | 3128 | 11% | 536 | 4% |
| Total PCU | 28,579 | - | 31,947 | - | 37,292 | - | 33,966 | - | 13,633 | - |

 Table 2.2.1-13 Road Section Traffic Count for Each Location

Source: JICA Survey Team

Table 2.2.1-14 and **Table 2.2.1-15** show the measured traffic volume and the percentages during the peak hour. From the measured results, the peak hour in the morning is from 7:30 until 8:30. This applies to all locations. The peak percentage for the total of all the entering traffic volume was 7.5%. In terms of location, the peak percentage for Tema-Harbour Road was the highest at 9.1%. The evening peaks varying trends with respect to each location. The section total at the peak in the evening was from 17:30 to 18:30, but the percentage was 5.1%, so the peak was clearly not as high as that of the morning peak.

| C | | Duite | T ff (1- | (1) | Morn | ing Peak (Vel | n./hr) | M | orning Peak (%) | | |
|-------------------|---------------------|--------------------------|-----------|---------|-------|---------------|--------|-------------|-----------------|-------|--|
| Survey Station | Road Name | Daily Traffic (veh./day) | | | | 7:30 - 8:30 | | 7:30 - 8:30 | | | |
| Station | | Entry | Exit | Total | Entry | Exit | Total | Entry | Exit | Total | |
| No.1 | Accra-Tema Motorway | 11,180 | 12,674 | 23,854 | 874 | 826 | 1,700 | 7.8% | 6.5% | 7.1% | |
| No.2 | Tema-Akosombo Road | 15,442 | 14,063 | 29,505 | 1,234 | 790 | 2,024 | 8.0% | 5.6% | 6.9% | |
| No.3 | Tema-Aflao Road | 17,389 | 15,780 | 33,169 | 1,236 | 1,141 | 2,377 | 7.1% | 7.2% | 7.2% | |
| No.4 | Tema Harbour Road | 10,858 | 16,422 | 27,280 | 908 | 1,563 | 2,471 | 8.4% | 9.5% | 9.1% | |
| No.5 | Tema-Hospital Road | 8,953 | 3,932 | 12,885 | 527 | 403 | 930 | 5.9% | 10.2% | 7.2% | |
| Total | | 63,822 | 62,871 | 126,693 | 4,779 | 4,723 | 9,502 | 7.5% | 7.5% | 7.5% | |

Table 2.2.1-14 Measured Traffic Volume and Peak Percentages in the Morning

Source: JICA Survey Team

Table 2.2.1-15 Measured Traffic Volume and Peak Percentages in the Evening

| Common | | Deile | Troffic (uch | (day) | Even | ing Peak (Vel | h./hr) | Evening Peak (%) | | | |
|-------------------|---------------------|--------------------------|--------------|---------|-------|---------------|--------|------------------|------|-------|--|
| Survey Station | Road Name | Daily Traffic (veh./day) | | | | 17:30 -18:30 | | 17:30 -18:30 | | | |
| Station | | Entry | Exit | Total | Entry | Exit | Total | Entry | Exit | Total | |
| No.1 | Accra-Tema Motorway | 11,180 | 12,674 | 23,854 | 410 | 729 | 1,139 | 3.7% | 5.8% | 4.8% | |
| No.2 | Tema-Akosombo Road | 15,442 | 14,063 | 29,505 | 874 | 805 | 1,679 | 5.7% | 5.7% | 5.7% | |
| No.3 | Tema-Aflao Road | 17,389 | 15,780 | 33,169 | 891 | 1,143 | 2,034 | 5.1% | 7.2% | 6.1% | |
| No.4 | Tema Harbour Road | 10,858 | 16,422 | 27,280 | 379 | 523 | 902 | 3.5% | 3.2% | 3.3% | |
| No.5 | Tema-Hospital Road | 8,953 | 3,932 | 12,885 | 573 | 147 | 720 | 6.4% | 3.7% | 5.6% | |
| | Total | 63,822 | 62,871 | 126,693 | 3,127 | 3,347 | 6,474 | 4.9% | 5.3% | 5.1% | |

Source: JICA Survey Team

3) Results of Turning Volume Count Survey

Traffic volume by direction was calculated based on the results of roadside OD (Origin-Destination) interview survey conducted at each of the 5 legs of the Tema Roundabout. The survey of traffic volume by direction was conducted in the morning and evening peak hours. The effective number of samples obtained at each cross-section was greater than 30%. The samples obtained were converted into traffic volume by time, by direction and by vehicle type using the survey results. From above, the results of the traffic count by direction at Tema Roundabout are as shown in **Table 2.2.1-16**.

Traffic volume from 07:00 to 08:00 hours from inflow point 2 to outflow point 1 was the highest at 633 vehicles per hour or 13% of the total. Next, from inflow point 3 to outflow point 4 the traffic volume was 483 vehicles per hour or 10%. From 8:00 to 9:00 hours the traffic volume from inflow point 3 to outflow point 4 was the largest at 746 vehicles per hour or 15% of the total. Next, from inflow point 2 to outflow point 4 the traffic volume was 440 vehicles per hour or 9%.

| Morning | time 7 a.m. | | | | | | Veh./hr | Morning | time 8 a.m. | | | | | | Veh./hr |
|---------|--------------|-----------------|----------------|----------------|----------------|-----|------------|---------|--------------|-------|----------|----------|----------|----------|-----------|
| Turnin | g Volume | | | Outf | low | | | Turnin | g Volume | | | Outf | low | | |
| runni | gvolume | 1 | 2 | 3 | 4 | 5 | Total | Turrin | gvolume | 1 | 2 | 3 | 4 | 5 | Total |
| Inflow | 1 | 13 | 235 | 258 | 301 | 118 | 926 | | 1 | 29 | 308 | 282 | 213 | 34 | 865 |
| | 2 | 633 | 29 | 146 | 396 | 126 | 1,330 | | 2 | 432 | 41 | 111 | 440 | 182 | 1,207 |
| Inflow | 3 | 386 | 139 | 13 | 483 | 198 | 1,220 | Inflow | 3 | 370 | 90 | 6 | 746 | 85 | 1,297 |
| millow | 4 | 208 | 398 | 292 | 3 | 17 | 918 | millow | 4 | 164 | 406 | 277 | - | 27 | 873 |
| | 5 | 22 | 144 | 246 | 54 | 2 | 469 | | 5 | 46 | 167 | 295 | 67 | - | 576 |
| | Total | 1,262 | 947 | 956 | 1,238 | 461 | 4,863 | | Total | 1,041 | 1,012 | 972 | 1,466 | 327 | 4,818 |
| | | | | Outf | low | | | | | | | Out | low | | |
| Volun | ne Share | 1 | 2 | 3 | 4 | 5 | Total | Volur | ne Share | 1 | 2 | 3 | 4 | 5 | Total |
| | 1 | 0% | 5% | 5% | 6% | 2% | 19% | | 1 | 1% | 6% | 6% | 4% | 1% | 18% |
| | 2 | 13% | 1% | 3% | 8% | 3% | 27% | | 2 | 9% | 1% | 2% | 9% | 4% | 25% |
| | 3 | 8% | 3% | 0% | 10% | 4% | 25% | | 3 | 8% | 2% | 0% | 15% | 2% | 27% |
| Inflow | 4 | 4% | 8% | 6% | 0% | 0% | 19% | Inflow | 4 | 3% | 8% | 6% | 0% | 1% | 18% |
| | 5 | 0% | 3% | 5% | 1% | 0% | 10% | | 5 | 1% | 3% | 6% | 1% | 0% | 12% |
| | Total | 26% | 19% | 20% | 25% | 9% | 100% | | Total | 22% | 21% | 20% | 30% | 7% | 100% |
| Evening | time 17 p.m. | | | | | | Veh./hr | Evening | time 18 p.m. | | | | | | Veh./hr |
| | | | | Outf | low | | veni, m | | | | | Outf | low | | 1011,111 |
| Turnin | g Volume | 1 | 2 | 3 | 4 | 5 | Total | Turnin | g Volume | 1 | 2 | 3 | 4 | 5 | Total |
| | 1 | 3 | 180 | 158 | 67 | 35 | 443 | | 1 | 21 | 170 | 190 | 38 | 27 | 446 |
| | 2 | 400 | 52 | 128 | 162 | 83 | 824 | | 2 | 386 | 87 | 115 | 171 | 83 | 842 |
| Inflow | 3 | 448 | 74 | 20 | 267 | 60 | 868 | Inflow | 3 | 434 | 67 | 6 | 240 | 53 | 799 |
| mnow | 4 | 81 | 239 | 41 | 1 | 4 | 366 | mnow | 4 | 92 | 58 | 36 | - | 24 | 210 |
| | 5 | 51 | 213 | 228 | 78 | - | 571 | | 5 | 106 | 204 | 158 | 37 | - | 505 |
| | Total | 983 | 758 | 574 | 575 | 182 | 3,072 | | Total | 1,039 | 585 | 505 | 486 | 187 | 2,802 |
| | | | | Outf | law | | | | | | | Outf | low | | |
| Volun | ne Share | 1 | 2 | 3 | 4 | 5 | Total | Volur | ne Share | 1 | 2 | 3 | 4 | 5 | Total |
| | 1 | 1 0% | 2 6% | 5% | 4 2% | 1% | 10(a) | | 1 | 1% | 2 6% | 5 7% | 4 1% | 1% | 10(a) |
| | 2 | 13% | 2% | 3% 4% | 5% | 3% | 27% | | 2 | 1/% | 3% | 4% | 1% | 3% | 30% |
| | | 1.570 | | - | 9% | 2% | 27% | | 3 | 14% | 2% | 4% | 9% | 2% | 29% |
| | | 15% | 2% | 1% | | | | Inflow | | | | | | | |
| Inflow | 3 | 15% 3% | 2% 8% | 1% 1% | | | | Inflow | | | 2% | 1% | 0% | | 7% |
| Inflow | | 15% 3% 2% | 2% 8% 7% | 1% 1% 7% | 9% 0% 3% | 0% | 12% 19% | Inflow | 4 | 3% | 2% 7% | 1% 6% | 0% 1% | 1% 0% | 7% 18% |

Table 2.2.1-16 Peak Traffic Volume and Percentages against the Total Traffic Volume

Source: JICA Survey Team

Also, an OD survey was additionally implemented on Tema-Hospital Road to confirm the status of traffic detouring from the Tema Roundabout. Specifically, it was to investigate by-passing traffic from Accra towards Ashaiman (survey road-station 2) and Aflao (survey road-station 3) going to outflow point 2 and 3 via Hospital Link Road. The results showed that traffic volume equivalent to about 10% of the total traffic volume according to the direction on Tema-Hospital Road were detouring

4) Results of the traffic Congestion Length Survey

Longest traffic congestion with respect to the time period was obtained from the traffic survey. The length of congestion is the length of the line of vehicles from the point of intersection with the roundabout. The result is shown in

Table 2.2.1-17. The maximum length of congestion measured in the morning was 1,100 m, which occurred on the Tema-Akasombo Road. On the Accra-Tema Motorway 500 m was measured on the Tema-Aflao Road 600 m was measured and on the Tema-Harbour Road and the Tema-Hospital Road 120 m length of traffic congestion was measured. All these were measured between 8:00 hours and 9:00 hours.

In the evening the maximum length of congestion was 700 m measured on the Tema-Harbour Road. On the Accra-Tema Motorway 500 m was measured between 18:00 and 19:00 hours. On the Tema-Akasombo Road 600 m was measured between 17:00 and 18:00 hours. On the Tema-Aflao Road 450 m was measured. On the Tema-Hospital Road traffic congestion of length 170 m was measured.

| | nuore 222117 manie Congestion Dength during Fear Mours (units, m) | | | | | | | | | | | | | |
|------|---|-------|-------|-------|-------|-------|-------|-------|-------|--|--|--|--|--|
| No. | Road | 7:15 | 7:30 | 7:45 | 8:00 | 8:15 | 8:30 | 8:45 | 9:00 | | | | | |
| No.1 | Accra-Tema Motorway | 215 | 200 | 300 | 500 | 500 | 500 | 450 | 400 | | | | | |
| No.2 | Tema-Akosombo Rd. | 170 | 1,000 | 1,000 | 1,100 | 1,100 | 1,100 | 800 | 800 | | | | | |
| No.3 | Tema-Aflao Rd. | 400 | 300 | 450 | 590 | 350 | 500 | 450 | 600 | | | | | |
| No.4 | Tema-Harbor Rd. | - | 60 | 40 | 100 | 100 | 120 | 90 | 120 | | | | | |
| No.5 | Tema-Hospital Rd. | 24 | - | 52 | 100 | 100 | 120 | 120 | 120 | | | | | |
| No. | Road | 17:15 | 17:30 | 17:45 | 18:00 | 18:15 | 18:30 | 18:45 | 19:00 | | | | | |
| No.1 | Accra-Tema Motorway | 250 | 350 | 400 | 400 | 500 | 500 | 400 | 300 | | | | | |
| No.2 | Tema-Akosombo Rd. | 600 | 600 | 400 | 240 | 240 | 300 | 141 | 200 | | | | | |
| No.3 | Tema-Aflao Rd. | 450 | 450 | 450 | 450 | 450 | 400 | 50 | 50 | | | | | |
| No.4 | Tema-Harbor Rd. | 270 | 300 | 330 | 340 | 320 | 550 | 650 | 700 | | | | | |
| No.5 | Tema-Hospital Rd. | 120 | 120 | 120 | 170 | 170 | 170 | 170 | 170 | | | | | |

Table 2.2.1-17 Traffic Congestion Length during Peak Hours (units: m)

Source: JICA Survey Team

5) Results of Traffic Speed Survey

A travel speed survey was carried out on sections between each inflow point to the Tema Roundabout. The traveling speed results of the peak times are shown in **Table 2.2.1-18**. On the Tema-Akasombo Road the traveling speed was less than 5 km/h. The delay time to arrive at the intersection was 13 minutes. Apart from Tema-Harbour Road, the driving speeds for all the inflow points were less than 10 km/h.

| Summore | Survoy | | Observe | ed Result | Doloy Timo |
|-------------------|---------------------|---------------|----------------------|----------------------|---------------------|
| Survey Station | Road Name | Length (m) | Travel Time (min) | Travel Speed (km) | Delay Time (min) |
| No.1 | Accra-Tema Motorway | 1,000 | 6.9 | 8.7 | No.1 |
| No.2 | Tema-Akosombo Rd. | 1,000 | 14.3 | 4.2 | No.2 |
| No.3 | Tema-Aflao Rd. | 1,000 | 10.7 | 5.6 | No.3 |
| No.4 | Tema-Harbour Rd. | 1,000 | 4.5 | 13.3 | No.4 |
| No.5 | Tema-Hospital Rd. | 300 | 2.3 | 7.8 | No.5 |

 Table 2.2.1-18 Traveling Time Results during Peak Hours

Source: JICA Survey Team

6) Calculation of Annual Average Traffic Volume

A) Basic Considerations

Based on the traffic survey results the annual average traffic volume was calculated. The annual average traffic volume was calculated by applying a day of the week correction and a month correction to the measured traffic volume results. The values of each of the corrections used were obtained from the results of study in previous years by GHA. The day of the week correction was 0.81, and the month correction was 1.20. Note that the annual average traffic demands at the intersection were obtained taking into consideration the length of congestion.

B) Traffic Volume on Road Section

Table 2.2.1-19 shows the annual average daily traffic volume (AADT) separately for inflow, outflow and section at the Tema Roundabout. The table shows AADT was greatest on the Tema-Aflao Road at 32,240 vehicles per day. Next was Tema-Akosombo Road at 28,679 vehicles per day, Tema-Harbour Road at 26,516 vehicles per day, and the Accra-Tema Motorway at 23,186 vehicles per day. For Tema-Hospital Road the result was 12,524 vehicles per day. Note that the results for Accra - Tema Motorway were lower than those for the other sections, but this was due to a separate route to access the Tema Port area before the Team Roundabout.

| | | Daily Traffic (veh./day) | | | Morni | ing Peak (Ve | h./hr) | Evening Peak (Veh./hr) | | |
|-------------------|---------------------|--------------------------|-------------|---------|-------|--------------|--------|------------------------|-------|-------|
| Survey Station | Road Name | Dally | Tranic (ven | i./day) | | 7:30 - 8:30 | | 17:30 -18:30 | | |
| | | Entry | Exit | Total | Entry | Exit | Total | Entry | Exit | Total |
| No.1 | Accra-Tema Motorway | 10,867 | 12,319 | 23,186 | 1,005 | 896 | 1,900 | 554 | 821 | 1,374 |
| No.2 | Tema-Akosombo Road | 15,010 | 13,669 | 28,679 | 1,371 | 857 | 2,228 | 896 | 906 | 1,803 |
| No.3 | Tema-Aflao Road | 16,902 | 15,338 | 32,240 | 1,358 | 1,237 | 2,595 | 991 | 1,287 | 2,278 |
| No.4 | Tema Harbour Road | 10,554 | 15,962 | 26,516 | 912 | 1,695 | 2,607 | 529 | 589 | 1,118 |
| No.5 | Tema-Hospital Road | 8,702 | 3,822 | 12,524 | 531 | 437 | 968 | 584 | 165 | 749 |

 Table 2.2.1-19Annual Average Daily Traffic Volume (AADT)

Source: JICA Survey Team

C) Directional Traffic Volume

Table 2.2.1-20 shows the directional annual average traffic volume at the peak hours on Tema Roundabout. The values here would be the traffic volume in the case that the detour traffic determined in the additional survey travelled via the Tema Roundabout without detouring. Table 2.2.1-21 shows the traffic volume in terms of PCU. The above results are examined in the next section to obtain the future traffic demand for a standard planning year.

Table 2.2.1-20 Annual Average Traffic Volume according to

| | | ction at Pe | | Outfl | | | |
|-------------|-------|-------------|------------|------------|---------|-----|-------|
| МОТ | OR | 1 | 2 | 3 | 4 | 5 | Total |
| MOI | 1 | 3 | 13 | 22 | 4 28 | 18 | 8 |
| | 2 | 64 | 3 | 12 | 76 | 13 | 16 |
| | - | | | | | | |
| Inflow | 3 | 11 | 11 | 5 | 14 | 12 | 5 |
| | 4 | 16 | 27 | 27 | 0 | 0 | 7 |
| | 5 | 2 | 18 | 11 | 2 | 0 | 3 |
| | Total | 96 | 72 | 77 | 120 | 44 | 40 |
| CA | R | 1 | 2 | 3 | 4 | 5 | Total |
| | 1 | 8 | 127 | 293 | 136 | 42 | 60 |
| | 2 | 221 | 14 | 121 | 385 | 144 | 88 |
| | 3 | 147 | 80 | 9 | 563 | 111 | 91 |
| Inflow | 4 | 52 | 156 | 224 | 2 | 18 | 45 |
| | 5 | 14 | 83 | 170 | 42 | 1 | 31 |
| | Total | 442 | 460 | 817 | 1128 | 316 | 316 |
| MINI | | | | | | | |
| MINI | | 1 | 2 | 3 | 4 | 5 | Total |
| | 1 | 0 | 92 | 67 | 98 | 7 | 26 |
| | 2 | 91 | 12 | 31 | 66 | 6 | 20 |
| Inflow | 3 | 70 | 15 | 1 | 147 | 11 | 24 |
| mnow | 4 | 51 | 149 | 86 | 0 | 1 | 28 |
| | 5 | 3 | 9 | 29 | 14 | 0 | 5 |
| | Total | 215 | 277 | 214 | 325 | 25 | 105 |
| LARGE | | 1 | 2 | 3 | 4 | 5 | Total |
| LANCE | | 1 0 | 2 | 5 | 4 2 | 0 | rotal |
| | 1 | | | | | | |
| | 2 | 4 | 3 | 4 | 5 | 0 | 1 |
| Inflow | 3 | 1 | 0 | 0 | 4 | 0 | |
| miow | 4 | 2 | 4 | 1 | 0 | 0 | |
| | 5 | 0 | 1 | 1 | 0 | 0 | |
| | Total | 7 | 10 | 7 | 11 | 0 | 3 |
| LIGHT | TRUCK | 1 | 2 | 3 | 4 | 5 | Total |
| LIGHT | 1 | 6 | - 6 | 24 | - 13 | 2 | 5 |
| | 2 | 23 | 0 | | | 3 | |
| | - | | | 3 | 6 | | 3 |
| Inflow | 3 | 28 | 2 | 0 | 13 | 6 | 4 |
| | 4 | 14 | 7 | 12 | 0 | 0 | 3 |
| | 5 | 2 | 2 | 9 | 2 | 0 | 1 |
| | Total | 73 | 17 | 48 | 34 | 11 | 18 |
| TRU | CK | 1 | 2 | 3 | 4 | 5 | Total |
| | 1 | 1 | 10 | 16 | 10 | 0 | 3 |
| | 2 | 11 | 1 | 10 | 18 | 7 | 4 |
| | 3 | 18 | 1 | 10 | 10 | 0 | 3 |
| Inflow | 4 | 6 | 1 | 9 | 0 | 1 | 1 |
| | - | | | | | | |
| | 5 | 2 | 4 | 8 | 5 | 0 | 1 |
| | Total | 38 | 17 | 44 | 47 | 8 | 15 |
| TRAI | LER | 1 | 2 | 3 | 4 | 5 | Total |
| | 1 | 0 | 3 | 3 | 4 | 0 | 1 |
| | 2 | 4 | 0 | 0 | 3 | 0 | |
| T CI | 3 | 16 | 6 | 0 | 10 | 1 | 3 |
| Inflow | 4 | 4 | 5 | 11 | 0 | 0 | 2 |
| | 5 | 0 | 0 | 0 | 0 | 0 | |
| | Total | 24 | 14 | 14 | 17 | 1 | 7 |
| OTH | | 1 | 2 | | 4 | 5 | Total |
| OTH | 1 | | | 3 | | | |
| | 1 | 0 | 0 | 14 | 12 | 20 | 4 |
| | 2 | 0 | 0 | 1 | 1 | 1 | |
| Inflow | 3 | 8 | 0 | 0 | 10 | 4 | 2 |
| | 4 | 0 | 9 | 0 | 0 | 0 | |
| | 5 | 0 | 0 | 5 | 1 | 0 | |
| | Total | 8 | 9 | 20 | 24 | 25 | 8 |
| All v | | 1 | 2 | 3 | 4 | 5 | Total |
| | 1 | 18 | 253 | 440 | 303 | 89 | 110 |
| | 2 | | 33 | | | | |
| | | 418 | | 182 | 560 | 175 | 136 |
| | 3 | 299 | 115 | 16 | 775 | 145 | 135 |
| Inflow | | | | | | 20 | 89 |
| Inflow | 4 | 145 | 358 | 370 | 2 | 20 | |
| Inflow | 4 5 | 145 23 | 358 117 | 370 233 | 2 66 | 20 | 44 |

Direction at Peak Hours (vehicles per hour)

Source: JICA Survey Team

| MOT | OR | 1 | 2 | Outf 3 | 4 | 5 | Tota |
|--------|-------|-----|-----|-----------|------|--------|-------|
| | 1 | 2 | - 7 | 11 | 4 14 | 9 | 1014 |
| | 2 | 32 | 2 | 6 | 38 | 9 7 | |
| | 3 | 6 | 6 | 3 | | 6 | |
| Inflow | 4 | 8 | | | 0 | 0 | |
| | | | 14 | 14 | | - | |
| | 5 | 1 | 9 | 6 | 1 | 0 | |
| | Total | 49 | 38 | 40 | 60 | 22 | |
| CA | R | 1 | 2 | 3 | 4 | 5 | Tota |
| | 1 | 8 | 127 | 293 | 136 | 42 | |
| | 2 | 221 | 14 | 121 | 385 | 144 | |
| Inflow | 3 | 147 | 80 | 9 | 563 | 111 | |
| Inflow | 4 | 52 | 156 | 224 | 2 | 18 | |
| | 5 | 14 | 83 | 170 | 42 | 1 | |
| | Total | 442 | 460 | 817 | 1128 | 316 | , |
| MINI | BUS | 1 | 2 | 3 | 4 | 5 | Tota |
| | 1 | 0 | 92 | 67 | 98 | 7 | |
| | 2 | 91 | 12 | 31 | 66 | 6 | |
| | 3 | 70 | 12 | 1 | 147 | 11 | |
| Inflow | 4 | 51 | 13 | 86 | 0 | 11 | |
| | - | | 9 | | | | |
| | 5 | 3 | | 29 | 14 | 0 | |
| | Total | 215 | 277 | 214 | 325 | 25 | |
| LARGE | BUS | 1 | 2 | 3 | 4 | 5 | Tota |
| | 1 | 0 | 4 | 2 | 4 | 0 | |
| | 2 | 8 | 6 | 8 | 10 | 0 | |
| Inflow | 3 | 2 | 0 | 0 | 8 | 0 | |
| millow | 4 | 4 | 8 | 2 | 0 | 0 | |
| | 5 | 0 | 2 | 2 | 0 | 0 | |
| | Total | 14 | 20 | 14 | 22 | 0 | |
| LIGHT | TRUCK | 1 | 2 | 3 | 4 | 5 | Tota |
| | 1 | 6 | 6 | 24 | 13 | 2 | |
| | 2 | 23 | 0 | 3 | 6 | 3 | |
| | 3 | 28 | 2 | 0 | 13 | 6 | |
| Inflow | 4 | 14 | 7 | 12 | 0 | 0 | |
| | 5 | 2 | 2 | 9 | 2 | 0 | |
| | Total | 73 | 17 | 48 | 34 | 11 | |
| TRU | | | 2 | | 4 | | Teres |
| IKU | | 1 | | 3 | | 5 | Tota |
| | 1 | 2 | 20 | 32 | 20 | 0 | |
| | 2 | 22 | 2 | 20 | 36 | 14 | |
| Inflow | 3 | 36 | 2 | 2 | 28 | 0 | |
| | 4 | 12 | 2 | 18 | 0 | 2 | |
| | 5 | 4 | 8 | 16 | 10 | 0 | |
| | Total | 76 | 34 | 88 | 94 | 16 | |
| TRAI | LER | 1 | 2 | 3 | 4 | 5 | Tota |
| | 1 | 0 | 9 | 9 | 12 | 0 | |
| | 2 | 12 | 0 | 0 | 9 | 0 | |
| | 3 | 48 | 18 | 0 | 30 | 3 | |
| Inflow | 4 | 12 | 15 | 33 | 0 | 0 | |
| | 5 | 0 | 0 | 0 | 0 | 0 | |
| | Total | 72 | 42 | 42 | 51 | 3 | |
| OTH | | 1 | 2 | 3 | 4 | 5 | Tota |
| OIII | 1 | 0 | 2 0 | 42 | 4 36 | 60 | 1014 |
| | 2 | 0 | 0 | | 30 | 3 | |
| | | | | 3 | | | |
| Inflow | 3 | 24 | 0 | 0 | 30 | 12 | |
| | 4 | 0 | 27 | 0 | 0 | 0 | 1 |
| | 5 | 0 | 0 | 15 | 3 | 0 | |
| | Total | 24 | 27 | 60 | 72 | 75 | |
| All v | eh. | 1 | 2 | 3 | 4 | 5 | Tota |
| | 1 | 18 | 265 | 480 | 333 | 120 | |
| | 2 | 409 | 36 | 192 | 553 | 177 | |
| Infle | 3 | 361 | 123 | 15 | 826 | 149 | |
| Inflow | 4 | 153 | 378 | 389 | 2 | 21 | |
| | - | | | | | | |
| | 5 | 24 | 113 | 247 | 72 | 1 | |

Table 2.2.1-21 Annual Average Traffic Volume according to

Direction at Peak Hours (PCU per hour)

Source: JICA Survey Team

(8) Survey of Existing Drainage System

1) Objectives

The objectives of the Survey are;

- To confirm the existing drainage systems
- · To grasp the dimensions and layout of existing provision of drainage facilities
- To confirm the locations of drainage outlets

2) Survey Period

The survey was conducted between 1st May, 2015 and 16th May, 2015 (16 days).

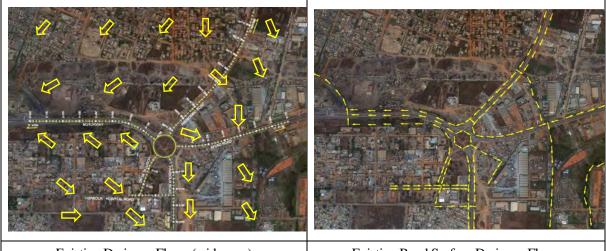
3) Survey Items

Major items surveyed are as follows:

- Flow pattern/directions
- · Dimensions and sizes of drainage facilities
- · Condition of drainage outlets

4) Survey Result

A) Drainage Flows / Systems



Existing Drainage Flows (wide area)

Existing Road Surface Drainage Flows

Figure 2.2.1-10 Existing Drainage Flows / Systems

B) Waterlogging Area after Rainfall

Poor drains, undulations in topography and lack of timely and proper maintenance are causing waterlogging in the project area. Hearings were carried out with residents to identify the locations. **Figure 2.2.1-11** indicates the result of the hearings (waterlogging areas).

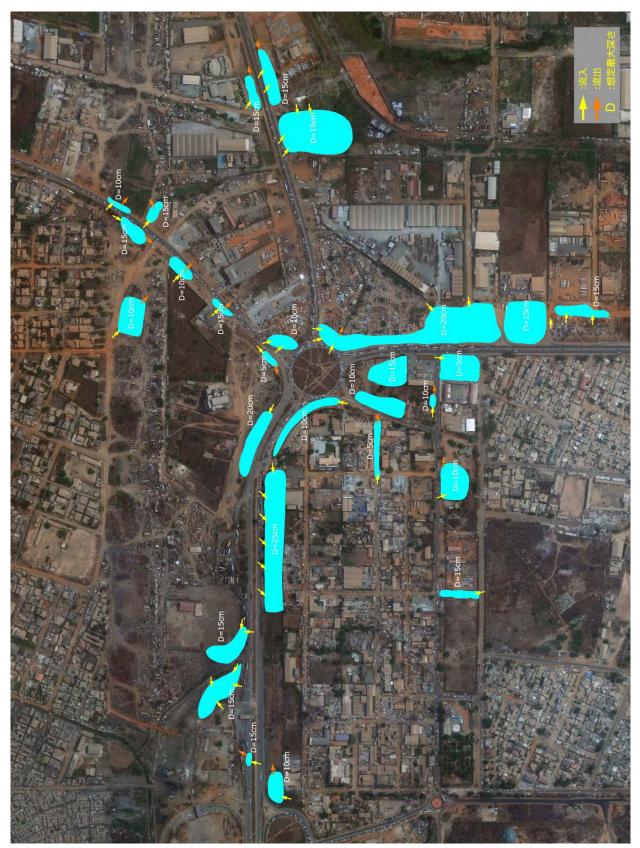


Figure 2.2.1-11 Flooded Area

2.2.1.4 Policy for Land Use

(1) Current Situation

1) Land Use of Tema

There are commercial facilities (hotels, restaurants, shops and sales of heavy machines), stock yard for containers and bus terminals. Residential facilities and schools are also densely located in southwest of Tema roundabout. In this area, many car repair shops exist. It's about 36% of the total land use of the land.



Figure 2.2.1-12 Land Use around Tema Roundabout

2) Heavy Industrial Area

Southeast of Tema roundabout is designated as Tema Heavy Industrial Area (about 50km2) and functions as a trading hub.



Figure 2.2.1-13 Heavy Industrial Area

3) Industries in Tema City

The main industrial activities of Tema City are aluminum processing, oil refining, food processing, cement manufacturing and construction material production. In addition, five quarry sites are located north at an approximate distance of 20km from Tema Roundabout.

4) Statistics of Tema Port

Axle load regulation was enhanced in 2009, and handling freight flows decreased at one time, but operative improvement has been performed since 2008. As a result, it has been increasing since 2010. Below are the trends between 2000 and 2013.

| TEMA | PORT PERFORM | ANCE 2000-2013 | | | | | | | |
|-------|---------------------|---------------------|---|---------------|---------------|----------------|------------------------|-----------------------------|---|
| Years | Vessel call (units) | Total cargo traffic | Growth Rate of Total cargo traffic (year-on-year) | Export tonnes | Import tonnes | Transit tonnes | Transhipment tonnes | Container traffic (TEUs) | Growth Rate of Container traffic (year-on-year) |
| 2000 | 1,163 | 6,219,517 | - | 910,779 | 5,083,439 | 144,973 | 17715 | 166,963 | - |
| 2001 | 1,169 | 6,314,968 | 2% | 932,931 | 5,13,07 | 261,251 | 38165 | 178,342 | 7% |
| 2002 | 1,272 | 6,841,481 | 8% | 821,042 | 5,186,690 | 627,773 | 151233 | 223,377 | 25% |
| 2003 | 1,172 | 7,391,268 | 8% | 809,589 | 5,490,893 | 885,093 | 138,520 | 305,868 | 37% |
| 2004 | 1,381 | 8,447,655 | 14% | 1,072,006 | 6,403,422 | 764,128 | 71,082 | 342,882 | 12% |
| 2005 | 1,643 | 9,249,977 | 9% | 1,182,469 | 6,936,688 | 875,325 | 155,815 | 392,761 | 15% |
| 2006 | 1,994 | 8,046,838 | -13% | 955,084 | 5,675,027 | 887,589 | 339,841 | 425,408 | 8% |
| 2007 | 1,672 | 8,378,682 | 4% | 1,099,094 | 6,120,583 | 843,656 | 119,209 | 489,147 | 15% |
| 2008 | 1,568 | 8,727,049 | 4% | 1,305,451 | 6,120,583 | 864,307 | 195,326 | 555,009 | 13% |
| 2009 | 1,634 | 7,406,490 | -15% | 981,075 | 5,694,280 | 509,124 | 192,565 | 525,694 | -5% |
| 2010 | 1,787 | 8,696,951 | 17% | 1,154,826 | 6,823,488 | 447,071 | 236,615 | 590,147 | 12% |
| 2011 | 1667 | 10,748,943 | 24% | 1,532,139 | 8,431,531 | 614,078 | 171,195 | 756,899 | 28% |
| 2012 | 1,521 | 11,468,962 | 7% | 1,477,390 | 9,383,462 | 530,457 | 50,403 | 824,238 | 9% |
| 2013 | 1,553 | 12,180,615 | 6% | 1,493,956 | 10,014,243 | 620,668 | 51,748 | 841,989 | 2% |

Source: Ghana Ports & Harbours Authority



Refer to "TRENDS IN TRANSPORT AND LOGISTICS ON THE TEMA-OUAGADOUGOU-BAMAKO CORRIDOR, West Africa Trade Hub Technical Report #51, USAID, May 2013".

Figure 2.2.1-14 Trends in Container Cargo Volumes in Tema Port

5) Tema Port Expansion Project

According to the "PORT OF TEMA PROJECTS: 2012 -2018" being conducted by GPHA, many projects as shown in **Table 2.2.1-23**, are completed or on-going. These projects are considered to promote much heavy traffic in near future in Tema Roundabout.

| No. | Project Title | Duration |
|-----|---|----------------|
| 1. | Construction of Bulk Cargo Handling Jetty – Tema Port | 2013 - 2014 |
| 2. | Dredging of Canoe Basin and Wreck Removal – Tema Fishing Hbr. | Completed-2013 |
| 3. | Reconstruction of Net Mending Wharf – Tema Fishing Hbr | 2012 - 2013 |
| 4. | Installation of Optical Character Recognition System (OCR), Tema | 2013 - 2013 |
| 5. | Purchase of Multipurpose Tug Boat & Security Patrol Boat, Tema | Arrived |
| 6. | Purchase of Container Handling Equipment, Tema | Arrived |
| 7. | Construction of 130-bed Maritime Hospital Tema | 2012 - 2015 |
| 8. | Rehabilitation of Loading Arms at Oil Berth, Tema | Completed-2013 |
| 9. | Consultancy Services for Design of Satelite Truck Village, Ashaiman | 2013 - 2013 |
| 10. | Tema Port Upgrade and Expansion | 2013 - 2018 |
| 11. | Master Terminal Software Installation | 2013 - 2014 |
| 12. | Coastal Fishing Ports and Fish Landing Sites | 2014 - 2016 |
| 13. | Transit Truck Park (World Bank Project) | 2013 - 2015 |
| 14. | Re-Modeling of Office Building for Tema Port Head Office | 2013 - 2013 |
| 15. | Bathymetric and Geotechnical Investigations, Tema | 2013 - 2013 |
| 16. | Building and delivering of Pilot Launch for the Port of Tema | 2013 - 2013 |

Table 2.2.1-23 Tema Port Expansion Project List

Source: GPHA HP

(2) Basic Policy

Basic policy for the improvement of the Tema Roundabout were established based on above conditions. The project location is surrounded by residential and industrial facilities. Thus, implementation of the project shall make sure influence of the project on these facilities is minimum.

The extension of Tema Port in future is expected to increase heavy or freight vehicles. For traffic demand forecast, growth ratio shall be considered as one of the calculation parameters.

In the case that the functional points on traffic will be caused by implementation of the project, functional compensation plan shall be made by planning service roads or footbridges.

2.2.1.5 Procurement Policies

(1) **Procurement of Labors**

1) Labor Environment

The study on the labors required for the construction work affirmed the followings;

- There are about 10 companies in Ghana that has experience of depressed road and road improvement.
- Specialized labors for management of construction site or operation of equipment could be procured in Tema or Accra.

2) Working Conditions

The working conditions in Ghana are regulated by the labor law and are summarized as follows;

A) Basic work time

The working time in a day shift should be 8 hours per day and below 48 hours per week. The employee working in a day shift would work during 6 A.M. to 8 P.M. and in a night shift would work from 8 P.M. to 6 A.M the next day.

B) Overtime

In the case of overtime or working on an off day, 100% allowance must be added to the basic salary. (200% in total).

C) Salary Insurance:

Under the law, salary insurance for labors is settled as "The payment for labors must prior to other debt. The payment for labors will not be affected by tender, bankruptcy and succession. On top of that, salaries must be paid immediately."

D) Bonus (13th month salary)

All workers are eligible for receiving bonus (double pay) on the 12th month after working for a year. This bonus is also refered to as the 13th month salary. If within a year they work for more than a month, workers are eligible for receiving extra salary agreed upon in the contract of employment in accordance with the length of their working period.

E) Severance Wage

In the case of termination of appointment of an employee after one year of employment, employer ought to pay one month's worth of salary as a severance wage for them

3) Conclusion

From above speculations, labors including engineering work execution supervisors, skilled workers and construction equipment operators could be recruited in Tema or Accra. In addition, it is possible to undertake construction work by subletting the work to local contractors. Although contract for the construction works for the project will be made with the Japanese contractor, local resource shall be utilized by considering their capacity or experiences.

(2) Procurement of Construction Materials

Most of the material required can be locally procured excluding special materials such as bearings for approach cushion slab, admixtures for modified asphalt and water proof material.

Other material not readily available in the local markets will be procured from Japan.

(3) Procurement of Construction Equipment

Construction equipment will generally be procured locally. Market survey conducted under this project shows that most of the construction equipment that would be used are possessed by the contractors in Accra. These equipment, particularly the typical ones such as the backhoes and bulldozers are available for lease. Cranes are also readily available but extra-heavy ones are difficult to be procured.

2.2.1.6 Policy for Traffic Demand Forecast

(1) **Basic Considerations**

The future traffic demand was predicted by the method of multiplying the current traffic volume by the growth rate of traffic volume. The percentage increase in traffic volume was calculated separately. **Table 2.2.1-24** shows the percentage increase in traffic volume estimated in various surveys in Ghana. However, the target of estimation was the whole country, and the basis of the calculations is not clear. For Tema Roundabout it was necessary to take into consideration the change in traffic volume near the Tema Roundabout in recent years and the volume of cargo handled by Tema Port in the future. Therefore, it was decided that the numbers below be used as reference only, and to carry out a separate calculation in this survey. In calculating the growth rate in the traffic volume, first a demand forecast model was built for two type of vehicle using the toll revenue of the Accra - Tema Motorway as an explained variable. The explanatory variables were the population of the Tema region and the cargo volume handled in the port. From the demand forecast model, the traffic growth rate in the toll revenue was calculated, and this was applied as the percentage increase in the traffic volume. Note that it was considered that a more sophisticated study based on the TDC Development Plan and comprehensive plan for the roads was necessary, but as usable data was not available, a macroscopic approach was undertaken in this survey.

Also, it is anticipated that in the future cross-border traffic, mainly trucks, will increase. However, congestion at Tema Roundabout is mainly caused by traffic during the morning peak hours. During this period, the cross-border traffic is limited. In road design, the analysis is carried out for the peak hours. Therefore, in this demand prediction of cross-border traffic was not particularly taken into consideration.

| No. | Name of Study | Year of Estimation | Estimation Method |
|-----|--|-----------------------|---|
| 1 | Highway Network Master Plan2020(JICA, GHA,2000) | 2020 | Separate calculation 7.2% |
| 2 | Integrated Transport Plan for Ghana (EU, Ministry of Finance and Economic Planning, 2010.6) | 2035 | Separate calculation States only that the percentage increase in passengers will be 8.7%. |
| 3 | Final Report: Detailed Design of Nkwanta Yendi Road and Two Interchanges at Tema and Ashaiman Roundabout (2011?) | 2035 | Separate calculation (p54) 3 scenarios set. Percentage increase for each vehicle type provided for each 5 year. Passenger cars 5%-6%, Freight vehicles 3.6%-4.4% |
| 4 | Preparatory Survey on Eastern Corridor Development Project in the Republic of GHANA (JICA, MRH,2013) | 2036 | Separate calculation Provides estimated values for each year at representative cross-sections. Each 10 year: 1.19%, 1.51%, 1.75% |
| 5 | PPP study: Takoradi – Accra (2013) | 2025 | Separate calculation Passenger cars 7%, goods vehicles 7.2% |

Source: JICA Survey Team

(2) Socio-economic Framework

In calculating the future traffic demand, first the socio-economic framework was set based on the available data. The set framework included the local population, and the volume of cargo handled at Tema Port. The considerations behind calculation of each of the indices were as outlined below. Note that demand forecast model using the number of vehicle registrations was also investigated, but because the correlation to the population was high, the number of vehicle registrations was not applied, but was represented by the population.

1) Population

The population framework was set using the population of the Greater Accra Region, excluding the Ga West District. This was because it was judged that the traffic volume at the Tema Roundabout has a high correlation with this region. The values for the framework as are shown in **Table 2.2.1-25**, which were set taking into consideration the values from past population census in Ghana, and the urban population of Ghana according to UN estimates.

| | Table 2.2.1 25 1 optimition 1 fame work for the District around Tenia Roundabout | | | | | | | |
|------|--|-----------|----------------|----------------|-----------|--|--|--|
| Year | Accra Metropolitan | Tema | Dangme West | Dangme East | Total | | | |
| 2000 | 1,658,937 | 506,400 | 96,809 | 93,112 | 2,355,258 | | | |
| 2010 | 2,076,546 | 671,824 | 122,836 | 130,795 | 3,002,001 | | | |
| 2011 | 2,142,129 | 693,042 | 126,716 | 134,926 | 3,096,813 | | | |
| 2012 | 2,209,784 | 714,930 | 130,718 | 139,187 | 3,194,619 | | | |
| 2013 | 2,279,575 | 737,510 | 134,846 | 143,583 | 3,295,515 | | | |
| 2014 | 2,351,571 | 760,803 | 139,105 | 148,118 | 3,399,597 | | | |
| 2015 | 2,425,841 | 784,831 | 143,498 | 152,796 | 3,506,966 | | | |
| 2020 | 2,728,839 | 902,985 | 210,846 | 204,475 | 4,047,145 | | | |
| 2025 | 3,011,449 | 1,024,140 | 309,802 | 273,634 | 4,619,025 | | | |
| 2030 | 3,262,379 | 1,148,298 | 455,200 | 366,184 | 5,232,062 | | | |
| 2035 | 3,447,302 | 1,272,783 | 668,839 | 490,037 | 5,878,961 | | | |

 Table 2.2.1-25 Population Framework for the District around Tema Roundabout

Source: JICA Survey Team based on population census in Ghana

2) Transaction Volume at Tema Harbour Port

The central predicted values from the Ghana Master Ports Development Plan -Cargo Forecast- were adopted for the volumes of cargo that would be handled at Tema Port in the future. Marine transshipment volume, which does not affect the land traffic, was excluded. Dry bulk cargos, general cargos, container cargos, and liquid cargos were on the other hand included. **Table 2.2.1-26** shows the cargo volume handled in the past and cargos predicted to be handled in the future at the Tema Harbour Port.

| Framework at Tema Port | | | | | | |
|------------------------|----------------|--|--|--|--|--|
| Vaar | Volume Handled | | | | | |
| Year | (kilo tons) | | | | | |
| 2010 | 8,460 | | | | | |
| 2011 | 10,578 | | | | | |
| 2012 | 11,419 | | | | | |
| 2013 | 12,129 | | | | | |
| 2014 | 15,370 | | | | | |
| 2015 | 18,610 | | | | | |
| 2020 | 26,350 | | | | | |
| 2025 | 35,480 | | | | | |
| 2030 | 44,800 | | | | | |
| 2035 | 55,130 | | | | | |

Table 2.2.1-26 Volumes Handled Framework at Tema Port

Source: Statistic data and Forecast by GPHA

(3) Building the Demand Forecast Model

In calculating the growth rate in the traffic volume, first a demand forecast model was built for two types of vehicles using the toll revenue of the Accra - Tema Motorway as an explained variable. The explanatory variables were the population of the Tema region, and the volume of cargos handled at Tema Port. From the demand prediction model the percentage increase in the toll revenue was calculated, and this was applied as the growth rate in the traffic volume. There were 2 types of prediction model, a passenger car model and a freight vehicle model. The model equation was a linear regression equation. **Table 2.2.1-26** shows the model parameters. For both models the coefficient of determination was high, so the applicability was good.

 Table 2.2.1-26 Model Parameters

| Passenger car | Coefficient (population) | Constant term ('000) | Coefficient of determination | |
|---------------|---------------------------------------|-------------------------|------------------------------|--|
| model | 0.74 | -1481 | 0.99 | |
| Goods vehicle | Coefficient (cargo volume at port) | Constant term ('000) | Coefficient of determination | |
| model | 1.10 | -1365 | 0.90 | |

Source: JICA Survey Team

| Year | Revenue of Toll gate ('000 Cedi) |
|------|----------------------------------|
| 2010 | 8,300 |
| 2011 | 9,600 |
| 2012 | 11,000 |
| 2013 | 12,600 |
| 2014 | 14,400 |

Source: Ghana Road Fund

(4) Estimated Growth Rate of Traffic Volume

The future toll revenue was estimated based on the model prepared in the previous section. The average annual percentage increase for each 5 year obtained from the estimation results are shown in **Table 2.2.1-28**. Note that the values for light truck were taken to be the average of the values for passenger cars and heavy trucks, because light trucks have characteristics intermediate between the two types.

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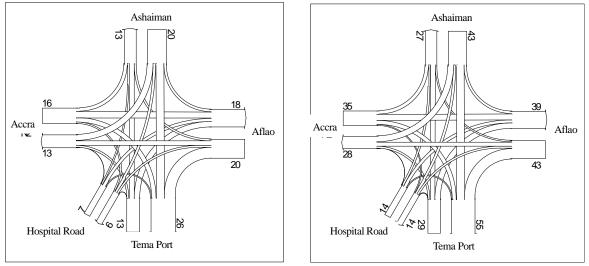
| Period of growth rate | Passenger cars | Light trucks | Heavy trucks |
|-----------------------|----------------|--------------|--------------|
| 2015-2020 | 8.52% | 8.08% | 7.65% |
| 2020-2025 | 6.27% | 6.33% | 6.40% |
| 2025-2030 | 5.08% | 5.00% | 4.93% |
| 2030-2035 | 4.25% | 4.30% | 4.35% |

| Table 2.2.1-28 Growth | Rates in | Traffic | Volume |
|-----------------------|----------|---------|--------|
|-----------------------|----------|---------|--------|

Source: JICA Survey Team

(5) Results of Traffic Demand Forecast

The traffic demand was forecasted by multiplying the growth rate of the future traffic volume by the present traffic volume. The traffic volume for each direction at the peak hours was calculated as the input condition for the intersection design. Estimates were produced for 2 points in time: for the operation target year (2020), and for the planning target year (2035). The traffic volume for the total number of vehicles by each direction is shown in **Figure 2.2.1-15**. Also, traffic volumes for each vehicle type for each year are shown in **Table 2.2.1-29** and **Table 2.2.1-30**.



Unit: 100veh./h Source: JICA Survey Team

Figure 2.2.1-15 Future Traffic Volume according to Direction at Peak Hours (2020, 2035)

Table 2.2.1-29 Traffic Volume according to Direction at Peak Hours in 2020 (vehicles per hour)

| | | | | Outf | low | | |
|---|---|------|------|------|------|-----|-------------|
| MO | TOR | 1 | 2 | 3 | 4 | 5 | Total |
| | 1 | 5 | 20 | 33 | 42 | 27 | 127 |
| | 2 | 96 | 5 | 18 | 114 | 21 | 254 |
| T C | 3 | 17 | 17 | 8 | 21 | 18 | 81 |
| Inflow | Inflow 1 5 20 33 42 22 1af 17 17 8 21 11 4 24 41 41 0 11 5 3 27 17 3 Total 145 110 117 180 66 2 333 21 182 579 21 1 12 191 441 205 66 3 221 120 14 847 10 3 121 125 256 63 17 1 0 138 101 147 1 17 2 137 18 47 99 1 | 0 | 106 | | | | |
| | | 0 | 50 | | | | |
| | MOTOR 1 2 3 4 5 7 1 5 20 33 42 27 2 96 5 18 114 21 18 3 17 17 8 21 18 4 24 44 44 0 0 Total 145 110 117 180 66 CAR 1 2 3 4 5 7 3 221 120 14 847 167 4 78 235 337 77 7 5 21 125 256 63 2 7 7 7 24 20 167 746 7 4 77 224 129 0 2 17 4 77 224 129 0 2 17 1 0 3 2 3 | | | | 618 | | |
| C | | 1 | 2 | 3 | 4 | 5 | Total |
| | 1 | 12 | | - | 205 | - | 912 |
| | - | | | | | | 1332 |
| | | | | | | | 1369 |
| Inflow | MOTOR 1 2 3 4 5 7 a 1 5 20 33 42 27 a 2 96 5 18 114 21 18 3 17 17 8 21 18 1 4 24 41 41 0 0 0 5 3 27 17 3 0 0 Total 145 110 117 180 66 0 2 333 21 182 579 217 0 3 221 120 14 847 167 2 4 77 23 4 5 7 1 1 0 138 101 11 1 1 2 137 18 47 99 9 2 a 1 1 2 3 4 | 680 | | | | | |
| | MOTOR 1 2 3 4 5 7 1 5 20 33 42 27 3 17 17 8 21 18 3 17 17 8 21 18 4 24 41 41 0 0 5 3 27 17 3 0 Total 145 110 117 180 66 2 333 21 182 579 217 3 221 120 14 847 167 4 78 23 37 3 27 5 21 125 256 63 2 17 1 0 138 101 147 11 17 2 137 18 47 99 9 2 10 138 01 14 21 0 2 < | | 467 | | | | |
| | | | | | | | 4760 |
| | | | | | | | Total |
| | | | | | - | | |
| | - | | | | | | 397 |
| | | | | | | | 310 |
| Inflow | - | | | | | | 368 |
| | | | | | | | 432 |
| | | | | | | - | 84 |
| | | | | | | | 1591 |
| LARGE | | | | | | | Total |
| | | | | | | | 8 |
| | | | | | | | 25 |
| Inflow | 3 | 2 | 0 | 0 | 6 | 0 | 8 |
| mnow | 4 | 3 | 6 | 2 | 0 | 0 | 11 |
| | 5 | 0 | 2 | 2 | 0 | 0 | 4 |
| | Total | 11 | 16 | 12 | 17 | 0 | 56 |
| LIGHT | TRUCK | 1 | 2 | 3 | 4 | 5 | Total |
| | 1 | 9 | 9 | 35 | 19 | 3 | 75 |
| | 2 | 34 | 0 | 4 | 9 | 4 | 51 |
| | 3 | 41 | 3 | 0 | 19 | 9 | 72 |
| Inflow | | | | | | 0 | 49 |
| | 5 | 3 | 3 | 13 | 3 | 0 | 22 |
| | | | | | | | 269 |
| TRI | | | | | | | Total |
| | 1 | | | | | | 52 |
| | | | | - | | | 67 |
| | | | | | | | 48 |
| Inflow | | | | | | - | 24 |
| | | | | - | | | |
| | - | | | | | | 28 |
| | | | | | | | 219 |
| TRA | | | | | | | Total |
| | | | | | | | 14 |
| | | | | | | | 10 |
| Inflow | - | | | | | | 47 |
| | | | | | | | 29 |
| | | | | | | | 0 |
| $ \begin{tabular}{ c c c c c c c c c c c c c c c c c c c$ | | | | | | 100 | |
| OTH | IERS | 1 | 2 | 3 | 4 | 5 | Total |
| | 1 | 0 | 0 | 20 | 17 | 29 | 66 |
| | 2 | 0 | 0 | 1 | 1 | 1 | 3 |
| T. Cl. | 3 | 12 | 0 | 0 | 14 | 6 | 32 |
| Inflow | 4 | 0 | 13 | 0 | 0 | 0 | 13 |
| | 5 | | | | | | 8 |
| | | | | | | | 122 |
| | | | | | | | Total |
| | | | | | | | 1651 |
| | | | | | | | 2052 |
| | - | | | | | | 2032 |
| Inflow | | | | | | | 1344 |
| | | | | | | | |
| | - | | | | | | 663 7725 |
| | Total | 1335 | 1316 | 1863 | 2006 | 045 | 7735 |

Source: JICA Survey Team

Table 2.2.1-30 Traffic Volume according to Direction at Peak Hours in 2035 (vehicles per hour)

| | | | | Outf | flow | | |
|--|---|--|--------------------------------------|--|-------------------------------|------------------------------|-------------------------------|
| MO | TOR | 1 | 2 | 3 | 4 | 5 | Total |
| | 1 | 10 | 42 | 71 | 90 | 58 | 271 |
| | 2 | 206 | 10 | 39 | 245 | 45 | 545 |
| | 3 | 35 | 35 | 16 | 45 | 39 | 170 |
| Inflow | $ \begin{split} \hline \text{MOTOR} & 1 & 2 & 3 & 4 & 5 \\ \hline 1 & 10 & 42 & 71 & 90 & 5 \\ \hline 2 & 206 & 10 & 39 & 245 & 4 \\ \hline 3 & 35 & 35 & 16 & 45 & 33 \\ \hline 4 & 51 & 87 & 87 & 0 & 0 \\ \hline 5 & 6 & 58 & 35 & 6 & 0 \\ \hline Total & 308 & 232 & 248 & 386 & 14 \\ \hline CAR & 1 & 2 & 3 & 4 & 5 \\ \hline 1 & 26 & 409 & 943 & 438 & 13 & 3 \\ \hline 2 & 711 & 45 & 389 & 1239 & 46 & 0 \\ \hline 3 & 473 & 257 & 29 & 1811 & 35 & 0 & 0 \\ \hline 5 & 45 & 267 & 547 & 135 & 0 & 0 \\ \hline 1 & 122 & 1480 & 2629 & 3629 & 101 \\ \hline MINIBUS & 1 & 2 & 3 & 4 & 5 \\ \hline Total & 1422 & 1480 & 2629 & 3629 & 101 \\ \hline MINIBUS & 1 & 2 & 3 & 4 & 5 & 0 \\ \hline 1 & 0 & 296 & 216 & 315 & 2 & 0 & 0 \\ \hline MINIBUS & 1 & 2 & 3 & 4 & 5 & 0 \\ \hline 1 & 0 & 296 & 216 & 36 & 0 & 0 & 0 \\ \hline 1 & 142 & 1480 & 479 & 277 & 0 & 0 & 0 & 0 & 0 \\ \hline 1 & 144 & 164 & 479 & 277 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ \hline 1 & 1 & 0 & 6 & 3 & 6 & 0 & 0 & 0 & 0 & 0 & 0 \\ \hline 1 & 1 & 0 & 6 & 3 & 6 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0$ | 0 | 225 | | | | |
| 1111123333126Total308C112613126131261312613111211110 <td></td> <td></td> <td></td> <td></td> <td>0</td> <td>105</td> | | | | | 0 | 105 | |
| | - | | | 42 71 90 58 10 39 245 445 35 16 45 39 87 87 0 0 58 35 6 0 32 248 386 142 3 4 5 0 32 248 386 142 3 4 5 0 99 943 438 135 45 389 1239 463 57 29 1811 357 607 547 135 3 80 2629 3629 1016 3 4 5 19 96 216 315 23 39 100 212 19 48 3 473 35 6 3 6 0 9 9277 0 3 10 13 16 0 13 3 0 0 13 <td>1316</td> | | 1316 | |
| C | | 1 | | 3 | | | Total |
| | 1 | 26 | 409 | 943 | 438 | 135 | 1951 |
| | | | | | | | 2847 |
| | 3 | 473 | 257 | 29 | 1811 | 357 | 2927 |
| Inflow | - | 167 | | 721 | | | 1454 |
| | | | | | | | 997 |
| | Total | 1422 | 1480 | 2629 | 3629 | 1016 | 10176 |
| MIN | | 1 | | | | | Total |
| | 1 | | | | | | 850 |
| | | - | | | | | 663 |
| | | | | | | | 784 |
| Inflow | | - | - | | | | 923 |
| | | | | | | | 177 |
| | | | | | - | | 3397 |
| LARG | | | | | | | Total |
| LANU | 1 | | | | - | | 10121 |
| | | | | | | | 52 |
| | | | - | | | - | 16 |
| Inflow | | | | | - | | 22 |
| | | | | | | | 6 |
| | | - | | | - | - | 111 |
| | | | | | | - | |
| | | | | | | | Total |
| | | | | | | | 161 |
| | | | | | | | 110 |
| Inflow | | | | | | | 154 |
| | | | | | | | 104 46 |
| | | | | - | | - | 40 575 |
| TDI | | | | | | | |
| IRU | 1 | | | | | | Total |
| | | | | | | | 115 |
| | | | | | | | 146 |
| Inflow | | | | | | | 105 |
| | | | | | | | 53 |
| | | | | | | | 59 |
| | | | - | | | | 478 |
| TRUCK 1 2 3 4 5 Total TRAILER 1 2 3 | | | | | | | Total |
| | | | | | | | 30 |
| | | | | | - | | 21 |
| Inflow | - | | | | | | 103 |
| | | | | | | | 62 |
| | | | | | | | 0 |
| ~ ~ ~ | | | | | | | 216 |
| OTH | 1 | | | | | | Total |
| | | | | | | | 142 |
| | | - | | | | | 9 |
| Inflow | | | | | | | 68 |
| | | 0 | 28 | | | | 28 |
| | | | | 16 | 3 | 0 | 19 |
| | 5 | 0 | | | _ | _ | |
| | 5 Total | 0 25 | 28 | 62 | | | 266 |
| All | 5 Total veh. | 0 25 1 | 28 2 | 62 3 | 4 | 5 | Total |
| All | 5 Total veh. 1 | 0 25 1 58 | 28 2 812 | 62 3 1411 | 4 970 | 5 284 | Total 3535 |
| All | 5 Total veh. 1 2 | 0 25 1 58 1342 | 28 2 812 107 | 62 3 1411 584 | 4 970 1799 | 5 284 561 | Total 3535 4393 |
| | 5 Total veh. 2 3 | 0 25 1 58 1342 955 | 28 2 812 107 368 | 62 3 1411 584 51 | 4 970 1799 2488 | 5 284 561 465 | Total 3535 4393 4327 |
| | 5 Total veh. 2 3 4 | 0 25 1 58 1342 955 463 | 28 2 812 107 368 1150 | 62 3 1411 584 51 1188 | 4 970 1799 2488 6 | 5 284 561 465 64 | Total 3535 4393 |
| | $\begin{array}{c c c c c c c c c c c c c c c c c c c $ | | | | | | |

Source: JICA Survey Team

2.2.1.7 Policies for Intersection Improvement Plan

(1) **Preconditions**

Preconditions for Improvement plan mentioned below have been discussed and agreed with GHA in the T/N signed in July 2016.

- Phase-1 will only include the depressed section of the east-west direction, the ramps and at-grade intersections. However, the final stage of the improvement (Phase-2) shall be considered so that implementation of Phase-2 can be made consistent with minimal recovery works.
- To accomplish the project objectives, it is necessary that the roads, nearby interchanges and other facilities be improved simultaneously with the improvement of the Tema Motorway Roundabout. For example, in the case of Tema-Akosombo Road, it might be necessary to improve the road up to Ashaiman Roundabout (about 1.5 km north of Tema Motorway Roundabout). However, the limits of improvement scope (start point and end point) will be between the points that will have tangible effect from the improvement plan.
- The objective intersection (roundabout) is composed of 5 roads. This includes the Tema-Harbour Road and Tema-Hospital Road that lie under the jurisdiction of the DUR. It is therefore necessary to explain and obtain views of the DUR on the improvement plan. Also, there are several utilities (water pipes, electricity & telecommunication cables) belonging to different companies buried within the area. Timely and proper coordination with relevant agencies will be ensured by GHA for relocation of the utilities (service lines) such that it will not affect the proposed plan.
- One way will be imposed at Tema-Hospital Road. GHA will coordinate with the DUR for the imposition.

(2) Policies of Improvement Plan

Following policies apply to the improvement of the existing roundabout.

- The fundamental type to be applied for the improvement will be a grade separated intersection,
- The plan will to the possible extent be within the existing right-of-way to minimize the area to be acquired,
- The east-west direction will be depressed and north-south direction graded. Connections will be provided through ramps,
- The facilities that are deemed difficult to be relocated will be avoided to the possible extent (minimization of relocation and resettlement),
- Grade separation will be avoided for traffics plying between Accra and Tema Harbour as the loaded trailers will have difficulty negotiating the vertical grades on the graded ramps,
- Direct access from buildings/stores will be allowed only through service roads,
- The scale of the project will be viable to be implemented under the grant assistance of Japan, in terms of technology, cost and environmental and social considerations.

(3) Methodology of the Improvement Plan

Figure 2.2.1-16 shows the procedural flow of the Study on the improvement plan.

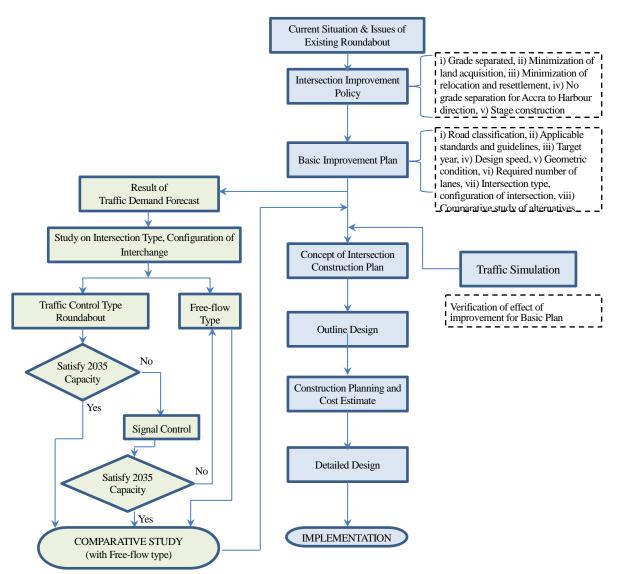


Figure 2.2.1-16 Study Flow of Improvement Plan

1) Classification of Roads

Roads from five directions merge at the objective roundabout. These roads have different classifications and the authorities responsible also vary. The functional classification of these roads in accordance with the Ghana Road Design Guideline 1991 and its responsible authority is given in **Table 2.2.1-31**. There will be no change in the classification of the roads after the improvement.

| Objective Roads | Functional Classification | Responsible Authority | | | |
|---|------------------------------|--------------------------|--|--|--|
| Accra-Tema Road | Motorway | GHA | | | |
| Tema-Aflao Road* Tema-Akosombo Road* | National Highway | GHA | | | |
| Tema-Harbour Road Tema-Hospital Road | Urban Road | DUR | | | |

Table 2.2.1-31 Classification of Roads and Responsible Authorities

| Objective Roads | Functional | Authority Authority assified into motorway. | Responsible | | | | | | |
|---|---------------------------|---|-------------|--|--|--|--|--|--|
| Objective Roads | Classification | Authority | | | | | | | |
| *: According to GHA, these roads are also classified into motorway. | | | | | | | | | |
| However, these roads are neit | ther access controlled no | r its alignment meet | | | | | | | |
| the requirement. | | - | | | | | | | |

2) Applicable Standards and Guidelines

The improvement plan of the objective roundabout will fundamentally follow the prevailing standards and guidelines of Ghana. However, for items not covered in these standards and guidelines, other international standards and guidelines, as listed below will be referred to. This will also apply to the urban roads including bridges that the DUR is responsible for.

The above policy regarding application of standards and guidelines was discussed and agreed upon with GHA.

- Standards/Guidelines for Planning and Design of Road / Intersection
 - i) Road Design Guide (GHA, March 1991)
 - ii) A Policy on Design of Highways and Streets (American Association of State Highway and Transportation Officials: AASHTO, 2001)
 - iii) Highway Capacity Manual (Transportation Research Board, 2010)
 - iv) Japan Road Structure Ordinance June, 2015
- Standards/Guidelines for Planning and Design of Structures (Retaining Walls, Culverts etc.)
 - i) Design Guideline for Retaining Wall (Japan Road Association, 2010)
 - ii) Design Guideline for Culvert (Japan Road Association, 2010)
- Standards/Guidelines/Manuals for Pavement Design
 - i) Pavement Design Manual (GHA, 1998)
 - ii) Guide for Design of Pavement Structure (American Association of State Highway and Transportation Officials: AASHTO, 1993)
 - iii) Pavement Design Guidelines (Japan Road Association, 2006)
- Guide/Guidelines for Drainage Design
 - i) Road Design Guide (GHA, March 1991)
 - ii) Guidelines for Drainage Design (Japan Road Association, 2006)
- Standards/Guidelines for Street Lighting Design
 - i) Road Lightning Installation Guidelines and Explanation (Japan Road Association, 2007)
 - ii) LED Road and Lightning Installation Guidelines [Draft] (Ministry of Land, Infrastructure, Transport and Tourism, 2011)

3) Target Year

AASHTO suggests that prediction of traffic forecast and potential development of the vicinity beyond 20 years cannot be done with any degree of accuracy due to probable changes in the general regional economy, population and land development in the area. Accordingly, the target year for design is set for 20 years.

4) Design Speeds

Design speeds of the roads that compose the roundabout based on the Ghana Road Design Guideline are as listed in **Table 2.2.1-32**.

| Existing Road | Design Speed (km/h) | Remarks |
|---------------------|---------------------|----------------------------------|
| Accra-Tema Motorway | 100 | For through traffics and not for |
| Tema-Aflao Road | 100 | those on turning lanes or ramps |
| Tema-Akosombo Road | 100 | |
| Tema-Harbour Road | 80 | |
| Tema-Hospital Road | 40 | |
| Ramps | 50 | |

Table 2.2.1-32 Design Speeds

2.2.2 Basic Construction Plan

2.2.2.1 Intersection Improvement Plan

(1) Selection of Optimum Configuration

In general, two types of improvement methods can be applied for the improvement of the existing roundabout; a partial free-flow type (traffic control type) or an absolute free-flow type (junction type).

A clover type and a deformed loop type (initial concept of the GOG), the two simplest configurations among the free-flow types and a compact diamond type from partial free-flow type was selected followed by a preliminary comparative study. The result of the comparison, which recommended partial free-flow type as the optimum alternative, was explained to GHA during the IC/R Meeting. However, the preliminary comparison was based on conceptual images and limited data. GHA insisted for a more detailed comparison using actual conditions, figures including findings and information collected from discussions conducted with various stakeholders. The Survey Team repeated the comparison using the results of various engineering surveys carried out during the first and second site surveys and justified the results of the preliminary comparative study. The intersection type and the selected configuration was verified in the manner shown in **Table 2.2.2-1**.

| No. | Studied / Verified Items | | Description |
|-----|---|-------------------|---|
| 1 | Intersection Plan for Phase-2 | i) ii) iii) | Capacity of the roundabout type will be calculated to check if it can manage the traffic of the target year Capacity of the signal control type will be calculated to check if it can resolve the congestion problem Detailed comparative study of configuration types that will have sufficient capacity from the above two types (traffic control type) including free-flow types will be conducted to select the optimum alternative. |
| 2 | Check of Improvement Effect by Traffic Flow Simulation | i) ii) | Reproduction of present situation Simulation result for each alternative |
| 3 | Intersection Plan for Phase-1 | i) ii) iii) | Capacity limit in Phase-1 Target construction area in Phase-1 Traffic signal layout |

 Table 2.2.2-1 Studied / Verified Items for Intersection Improvement Plan

(2) Intersection Improvement Plan for Phase-2

1) Traffic Control Capacity of Roundabout

The traffic control capacity of a roundabout for the case of partial free-flow type was calculated based on the method of Highway Capacity Manual 2010. The capacity was calculated for target periods 2035, 2028 and 2025. The results are shown in **Table 2.2.2-2**. The result indicates that the LOS at the roundabout will fall to F (heavily congested) as early as in the year 2020, meaning that the traffic will experience extreme congestion by 2020 and beyond. This implies provision of a roundabout will not be appropriate in resolving the issue of congestion.

| | Case | | Simulation-01 | Simulation-02 | Simulation-03 |
|-------|-------------------------|--------------|---------------|---------------|---------------|
| | Completion year | | 2035 | 2028 | 2020 |
| | | Eastbound | 19.96 | 6.19 | 2.17 |
| | Degree of saturation | Westbound | 11.47 | 5.79 | 3.11 |
| | | Northbound | 2.47 | 1.23 | 0.69 |
| | Southbound | 15.52 | 5.11 | 2.14 | |
| Chock | | Eastbound | F | F | F |
| CHECK | Check | Westbound | F | F | F |
| | | Northbound | F | F | D |
| | | Southbound | | F | F |
| | | Intersection | F | F | F |

Table 2.2.2-2 Traffic Control Capacity of Roundabout

2) Traffic Control Capacity of Signalized Intersection

Next, traffic control capacity of a signalized intersection for the partial free-flow type was analyzed, in the similar manner as to the roundabout. Analysis results are summarized in **Table 2.2.2-3**. The row highlighted in red is the LOS at the intersection. The results indicate that the LOS B during off-peak hours and LOS D during peak hours can be achieved in the year 2035 if the number of turning lanes as shown in **Figure 2.2.2-1** is provided and the effective green light time of the traffic signal is set in the manner shown in **Table 2.2.2-3**.

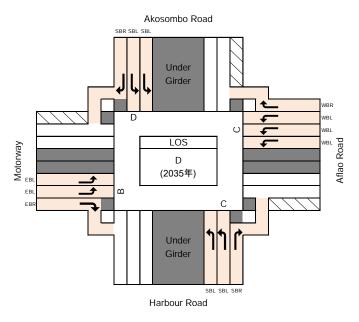


Figure 2.2.2-1 Lanes and LOS of Signalized Intersection of Partial Free-Flow Type in Phase-2

| Demand Inform | nation | | | | EB | | | WB | | | NB | | | SB | |
|-------------------------|------------|------------------------|-----|--------|-----|-------|-------|-------|------|-------|------|-------|-------|-----|------|
| Approach Move | ement | | | L | T | R | L | T | R | L | Т | R | L | Т | R |
| Demand (v), ve | h/h | | | 812 | | 0 | 2953 | 0 | | 536 | | 0 | 584 | | 0 |
| Signal Informa | tion | | | | | 1 | 1. | 1 | | | | | | - | L |
| Cycle, s | 75.0 | Reference Phase | 2 | 1 | ž | 7 e | R | 2 | 7 5 | | 1 | | C - | 2 | 2 |
| Offset, s | 0 | Reference Point | End | Green | 0.0 | 41.0 | 0.0 | 0.0 | 22.0 | 0.0 | - | - | ¥ 2 4 | - 3 | |
| Uncoordinated | Yes | Simult. Gap E/W | On | Yellow | | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | | 1 | 7 | | |
| Force Mode | Fixed | Simult. Gap N/S | On | Red | 0.0 | 2.0 | 1.0 | 1.0 | 2.0 | 1.0 | | â | - | - 1 | Y |
| Timer Results | a | _ | | EBL | | EBT | WB | | WBT | NBL | | NBT | SBL | | SBT |
| Assigned Phas | e | | | 5 | | 2 | 1 | | 6 | 3 | | 8 | 7 | | 4 |
| Case Number | | | | 1.3 | | 3.0 | 1.2 | | 4.0 | 1.2 | | 3.0 | 1.3 | | 3.0 |
| Phase Duration | , s | | | 47.0 |) | 0.0 | 47.0 |) | 0.0 | 28.0 | | 0.0 | 28.0 | | 0.0 |
| Change Period | (Y+Rc) | , s | | 6.0 | | 6.0 | 6.0 | | 0.0 | 6.0 | | 5.0 | 5.0 | | 5.0 |
| Max Allow Hea | dway (N | IAH), s | | 2.1 | 1 | 0.0 | 2.1 | | 0.0 | 2.1 | | 0.0 | 2.1 | | 0.0 |
| Queue Clearan | ce Time | e (gs), s | | 14.0 | | | 43.0 |) | | 14.0 | - | | 10.4 | | |
| Green Extensio | n Time | (ge), s | | 0.9 | | 0.0 | 0.0 | | 0.0 | 0.5 | | 0.0 | 0.6 | | 0.0 |
| Phase Call Pro | bability | | | 1.00 |) | | 1.00 |) | | 1.00 | | | 1.00 | | |
| Max Out Proba | bility | | - | 0.00 | | 1.00 | | 0.00 | 0.00 | | 0.00 | | | | |
| Movement Gro | oup Res | sults | | EB | | WB | | NB | | | SB | | | | |
| Approach Move | ement | | | L | Т | R | L | Т | R | L | Т | R | L | Т | R |
| Assigned Move | ment | | | 5 | | 12 | 1 | 6 | | 3 | | 18 | 7 | | 14 |
| Adjusted Flow I | Rate (v) | , veh/h | | 829 | | 0 | 3013 | 0 | | 547 | | 0 | 596 | | 0 |
| Adjusted Satura | ation Flo | ow Rate (s), veh/h/ln | | 1658 | | 1438 | 1673 | 1900 | | 1627 | | 1519 | 1627 | | 154 |
| Queue Service | Time (g | (s), S | | 12.0 | 1 | 0.0 | 41.0 | 0.0 | | 12.0 | i | 0.0 | 8.4 | | 0.0 |
| Cycle Queue C | learance | e Time (g₀), s | | 12.0 | | 0.0 | 41.0 | 0.0 | | 12.0 | | 0.0 | 8.4 | | 0.0 |
| Capacity (c), ve | eh/h | 12 C | | 2004 | | 423 | 3029 | 3 | | 1146 | 1 | 832 | 1190 | | 848 |
| Volume-to-Cap | acity Ra | tio (X) | | 0.413 | | 0.000 | 0.995 | 0.000 | | 0.477 | | 0.000 | 0.501 | | 0.00 |
| Available Capa | city (Ca), | , veh/h | | 9377 | | 3313 | 3029 | 1316 | 1.1 | 1320 | | 6738 | 6304 | _ | 352 |
| Back of Queue | (Q), veh | n/In (95th percentile) | | 11.5 | | 0.0 | 32.2 | 0.0 | | 8.9 | | 0.0 | 9.0 | | 0.0 |
| Overflow Queu | e (Q3), v | /eh/ln | | 0.0 | | 0.0 | 0.0 | 0.0 | | 0.0 | | 0.0 | 0.0 | | 0.0 |
| Queue Storage | Ratio (| RQ) (95th percentile |) | 1.83 | | 0.00 | 5.10 | 0.00 | | 1.44 | | 0.00 | 1.46 | - | 0.00 |
| Uniform Delay | (d1), s/v | eh | | 17.4 | | 0.0 | 32.4 | 0.0 | | 29.3 | | 0.0 | 25.8 | | 0.0 |
| Incremental De | lay (d2), | s/veh | | 0.1 | | 0.0 | 15.1 | 0.0 | | 0.1 | | 0.0 | 0.1 | - | 0.0 |
| Initial Queue De | elay (d3) |), s/veh | | 0.0 | | 0.0 | 0.0 | 0.0 | | 0.0 | | 0.0 | 0.0 | _ | 0.0 |
| Control Delay (| d), s/veł | 1 | | 17.5 | | 0.0 | 47.4 | 0.0 | | 29.4 | | 0.0 | 25.9 | | 0.0 |
| Level of Service | e (LOS) | | | В | | | D | 1.1 | | C | | | C | | |
| Approach Delay | v. s/veh | /LOS | | 17.5 | i l | В | 47.4 | | D | 29.4 | | С | 25.9 | | С |
| Intersection De | lay, s/ve | h/LOS | | | | 37 | 7.9 | | | | | | D | | _ |
| Multimodal Re | sults | | | - | EB | | 1 | WB | | | NB | | | SB | |
| Pedestrian LOS | S Score | /LOS | | 3.2 | | С | 2.8 | | С | 2.9 | | С | 2.9 | | С |
| Bicycle LOS Score / LOS | | - | | | F | 5.5 | | F | 1 | | F | | | F | |

3) Comparative Study of Alternatives

The alternatives undertaken for the comparative study are as mentioned in Table 2.2.2-4.

| No. | Alternatives | Configuration | Selection Reason | | | | | | |
|-----|--|---|--|--|--|--|--|--|--|
| 1 | Existing Plan (GOG Free-Flow Type) | Type) Deformed Loop It is the configuration type proposed in the Study by a third country consultant and we by GHA. e Clover-leaf It is the only possible free-flow configuration layer grade separated alternative It is the configuration type proposed in the study by a third country consultant and we by GHA. It is the only possible free-flow configuration layer grade separated alternative It is the configuration that guarantees the reconfiguration while considerations while considerations to cost end the study of | | | | | | | |
| 2 | Free-Flow Type | Clover-leaf | It is the only possible free-flow configuration for a two- layer grade separated alternative | | | | | | |
| 3 | Consultant Recommended Type Compact Diamond | | It is the configuration that guarantees the required traffi functions while considerations to cost, environmenta impact and stage construction is given. | | | | | | |

 Table 2.2.2-4 Alternatives for Comparison and Reasons for its Selection

The comparative study of the above alternatives is summarized in **Table 2.2.2-5**. Based on the result and after discussion with and agreement of GHA, the Compact Diamond Type (consultant recommended type) was selected and applied in the project.

| | | | Table 2.2.2-5 | Comparative Study of Intersection Configuration | | | | | | | |
|------------------|---|--|---------------------------------------|---|--|---|--|--|--|--|--|
| | Evaluation Criteria | EXISTING PLAN (GOG PLAN) Deformed Loop Type | | FREE-FLOW TYPE Clover Leaf Type | | | | | | | |
| | PLAN OUTLINE | | | A CONTRACTOR | | | | | | | |
| | LEGEND Flyover Under-pass Control Point | | | | | | | | | | |
| | Dutline of East-west Direction bidjan-Lagos Corridor) | | | <u> </u> | | _ | | | | | |
| ц | 1. Configuration Outline | Free-flow 3-tier grade separated Both vertical and adjacent loops are non-symmetrical and are generally suita where traffic volume is high. Small curves, many conflict points steep vertical a acquisition, high construction cost etc. It is also confusing as the intended direction is different as well as combined weaving of merging and diverging tra | grades, huge land and actual drive | Free-flow 2-tier grade separated Typical type for a 4-leg graded intersection and is suitable for interchanges of highways. Weaving lengths provided separately for merging and diverging traffics, driving type and thus drivability is high | Partial free-flow 3-ti Typical type for a 4- highways. Left turning vehicle symmetrical and enline | | | | | | |
| raffic Function | 2. Travel Length/Time | The loops and other ramps have small radius of curvature and are lengthy. Grad are high for being a 3-tier structure and may cause travel difficulty for heavy very | | The loops and other ramps have small radius of curvature and are the long grades of loop ramps are moderate for being a 2-tier structure | interchanges of high standard • Partial free-flow 3-ti interchanges of high standard • Operational under complete interchanges of high standard • Partial free-flow 3-ti is short. Provision is possible • Operational under complete • Conflict of ramps exertification is possible • Similar to 'Deforme • Controllable by propersed Section 1 • 2-tier graded section 2 • Controllable by propersed Section 1 • 2-tier graded section 2 • Controllable by propersed Section 1 • Controllable by propersed Section 1 • Controllable by propersed section 2 • Controllable by propersed section 3 • V is difficult. Low • Acquisition of land i V is difficult. Low • | | | | | | |
| Traffic | 3. Level of Service (LOS) | • Level of service is high as there is no conflict of ramps. | | • Level of service is high as there is no conflict of ramps. | Conflict of ramps ex effectively, efficient | | | | | | |
| | 4. Access to Abutting Properties | Utilize substantially huge land and to secure proper connectivity with the existi and access from abutting properties, large scale improvement will be required | | Access from abutting at all quarters can be maintained but improvement will b securing connectivity with existing road network also. | Utilize limited land a with the existing roa | | | | | | |
| | 5. Pedestrian Convenience | The interval of cross points is long as the graded section is long. Walking distan the crossings are provided underneath the graded section. | ice is long even if | The interval of cross points is short as the span of graded section is short. Provis inside the ramp also. | | | | | | | |
| | 1. Facility Scale/Cost | (2 nd tier : 950m, 3 nd tier : 1,950m) • Ramp Length : 5,723 m | Estimated Cost : 16.0 Billion JPY | • Ramp Length : 7,760 m 7.0 | | 2-tier graded section Ramp Length: 5,15 | | | | | |
| omic | 2. Traffic Obstacles during Construction | Controllable by providing a detour and its shifting. Requires long period Difficulty adapting stage construction | | Shifting of traffic is easy and the period is short Difficulty adapting stage construction | Adaptation of stage | | | | | | |
| Economic | 3. Land Acquisition | Land Acquisition :297,840m2Outside ROW : 5,890m2 | Large | Land Acquisition : 751,020 m2 Outside ROW : 292,900 m2 | Extreme | Outside ROW: 0m | | | | | |
| | 4. Removal of Obstacles | Gas Station: 3 nos., Major facilities: 1 no., Houses: approx. 120 nos. Obstacles (5,050m) | Plenty | Gas Station: 3 nos., Major Facilities: 5 nos., Houses: approx. 200 nos. Obstacles (15,000m) | Plenty | Gas Station: 2 nos., Obstacles (2,700m) | | | | | |
| | 5. O & M Cost | • 2 nd tier and 3 rd tier graded section and long distance ramps | Huge | 2 nd tier graded section and long distance ramps | Small | Depressed section a | | | | | |
| ron at | 1. Land Acquisition, Resettlement | • Huge land, large buildings/structures and access from abutting properties are issues but possible within the existing ROW | Fairly low | Requires huge land and construction within existing ROW is difficult. Therefore high land acquisition cost | Low | Acquisition of land i | | | | | |
| Environ ment | 2. Traffic Limitations during Construction | Conflict occurs during construction of loop ramps | -L | Conflict occurs during construction of loop ramps | Traffic control during | | | | | | |
| Construct ion | 1. Adaptability of Stage wise Construction | • Complete construction desirable as it functions only after all direction traffic as it has a 2-tier graded structure in combination with loop ratio | | Temporary operation by providing at-grade intersection at connecting points of However, it will have two close signalized intersection, which is not appropria point of view, Therefore, complete construction is desirable. | As the configuration is possible. The outc completed. | | | | | | |
| | 2. period | 35 months | | 28 months | 30 | | | | | | |
| | ation/Feasibility to be aken by Japanese Grant Aid ae | Requires huge land acquisition and removal of large numbers of obstacles and big scale structures. In addition construction cost is highest. Not feasible under Japanese Grant Aid Scheme | × | Requires huge land acquisition and removal of large numbers of obstacles and lots of big scale structures. Not feasible under Japanese Grant Aid Scheme. | Δ | Land acquisition is min existing ROW. Numb Japanese Grant Aid Sch | | | | | |

| CONSULTANT RECOMMENDED TYPE Compact Diamond Type | | | | | | | |
|--|--------------|--|--|--|--|--|--|
| | | | | | | | |
| | | | | | | | |
| -tier grade separated | | | | | | | |
| 4-leg graded intersection and is suitable for interchanges of hi | gh standard | | | | | | |
| cles converge at one point at the intersection making the nhance easy, comfortable and safe driving. | intersection | | | | | | |
| conventional driving, symmetrical and travel distance is least. | | | | | | | |
| exists and thus LOS is less than other alternatives. However, tr ntly and safely controlled using traffic signals d and large scale improvement is not required to secure proper of bad network and access from abutting properties ned Loop Type' | | | | | | | |
| n Length : 640 m Estimated Cost : 9.0 Billio | n JPY | | | | | | |
| on Length : 550 m 152 m (Phase 1:6.0 Bil. Phase 2: 3 | 3.0 Bil.) | | | | | | |
| roviding a detour and its shifting. Requires long period ge construction is possible | | | | | | | |
| : 146,140m2 m2 | Medium | | | | | | |
| s., Major Facilities: 0, Houses: approx. 30nos. n) | Medium | | | | | | |
| n and 2 nd tier graded section and signalized intersection | Medium | | | | | | |
| d is minimum and construction can be within existing ROW | High | | | | | | |
| ing construction done by provision of roundabout within existing ROW | | | | | | | |
| on consists of an at-grade intersection, stage construction in each direction atcome (functionality) can be expected even when a particular direction is | | | | | | | |
| 30 months (Phase1:26 months, Phase 2:24 months) | | | | | | | |
| inimum and the plan is feasible to be constructed within the | | | | | | | |
| nber of obstacles to be removed is least. Feasible under Scheme, given that stage construction is applied | 0 | | | | | | |

(3) Traffic Flow Simulation

1) Basic Policy

Traffic behavior after improvement of the intersection may be complicated because the turning traffics will meet at the intersection using the on/off ramps. Whether the improvement meets the expected project outcome needs to be checked and verified accordingly. A traffic flow simulation will be carried out to examine the outcome of the project.

2) Objective and Methodology

The objective of traffic flow simulation is to visually check the effect of improvement in the future. For traffic flow simulation, specific computer software is used to simulate dynamic traffic behavior. Result of the traffic survey and future traffic demand is used as input for the analysis.

3) Simulation Result

Simulation results indicate that the intersection type (compacted diamond type) applied was totally capable to manage the vehicles in the intersection during single green light phase even in year 2035. The LOS calculated for year 2035 was 'D', which means the intersection will experience gradual congestion after year 2035.



Figure 2.2.2-2 3-tier with At-grade Signal Controlled Intersection (Compact Diamond Type)

(4) Intersection Plan for Phase-1

1) Traffic Control Capacity of the Intersection

Capacity analysis of the intersection during Phase-1 was analyzed for partial free-flow type based on the method recommended in the Highway Capacity Manual 2010 in the similar manner to that of Phase-2. The results indicate that LOS 'D' during off-peak hours can be achieved if the number of turning lanes shown in **Figure 2.2.2-3** and the effective green light time in **Table 2.2.2-6** are provided.

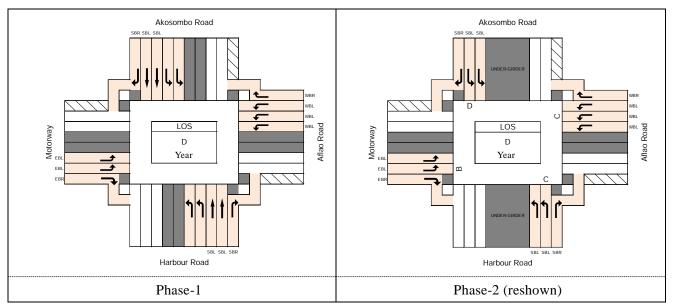


Figure 2.2.2-3 Lanes and LOS of Signalized Intersection of Partial Free-Flow Type

| Year | Hourly volume to be operated in Phase-1 during peak hour (Veh/h) | LOS |
|------|---|-----|
| 2020 | 3953 | С |
| 2023 | 4785 | D |
| 2024 | 5063 | Е |
| 2025 | 5340 | F |

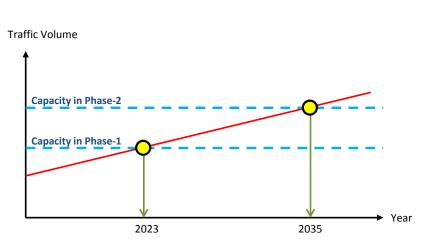


Table 2.2.2-6 Capacity Limit in Phase-1

Figure 2.2.2-4 Basic Consideration on Stage Construction

| Demand Information Approach Movement | | | | EB | | | WB | | | | NB | | | SB | |
|---|---------------------------------|---|------|--------|------|---------|---------|-------|---------|----------|-------|---------|------------|---------------|------|
| | | | | - L - | T | R | L | Т | R | L | T | R | L | T | F |
| Demand (v), veh/h | | | 459 | | 0 | 1670 | | 0 | 303 | 687 | 1 | 327 | 1339 | | |
| Signal Informa | tion | | - | 1 | 1.5 | 5 | | 1.1 | 1.10 | | | | | 1 1 | |
| Cycle, s | 105.5 | Reference Phase | 2 | 1 | 7 8 | × | 3 E | 1. | 1 1 1 | | | | 7 | 5 | 1 |
| Offset, s | 0 | Reference Point | End | Green | 174 | 9.6 | 0.0 | 24.1 | 15.4 | 9.0 | _ | 6 | X = | 3 | |
| Uncoordinated | Yes | Simult. Gap E/W | On | Yellow | | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | - | | > | ~ | |
| Force Mode | Fixed | Simult. Gap N/S | On | Red | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | | 3 | ł. | 7 | - |
| Timer Results | 1 | | _ | EBL | | EBT | WBL | 1 | WBT | NB | | NBT | SB | | SBT |
| Assigned Phase | e | | | 5 | | 2 | 1 | | 6 | 7 | | 4 | 3 | | 8 |
| Case Number | - | | | 1.1 | | 3.0 | 1.1 | | 3.0 | 2.0 | La | 4.0 | | | 4.0 |
| Phase Duration | , s | | | 23.4 | | 0.0 | 39.0 | 11.1 | 15.6 | 15.0 | 1 | 30.1 | | | 51.5 |
| Change Period, | (Y+Rc) | l, s | | 6.0 | | 6.0 | 6.0 6.0 | | 6.0 | 6.0 6.0 | | 6.0 | | | 6.0 |
| Max Allow Head | 1 / | | | 2.1 | | | 2.1 | | 0.0 2.1 | | 111 | 2.0 | | 2.1 | |
| Queue Clearan | the second second second second | and the second se | | 16.8 | 3 | - | 33.4 | | 11.0 | |) | 23.3 | | | 43.8 |
| Green Extensio | | | | | | 0.0 | 0.0 | | 0.0 | 0.0 | _ | 0.7 | | | 1.5 |
| Phase Call Probability | | | 1.00 | | | 1.00 | | | 1.00 | 210 | | 1.00 | | 1.00 | |
| Max Out Probability | | | 0.00 | 0.00 | | 1.00 | | 1.00 | 0.00 | | 1.00 | 1.00 0. | | | |
| Movement Gro | up Res | sults | | 1 | EB | | - | WB | | - | NB | | 1 | SB | |
| Approach Move | ement | | | 1 (L.) | Т | R | L | T | R | L | T | R | . L | 15 T . | F |
| Assigned Move | ment | | | 5 | | 12 | 1 | - | 16 | 7 | 4 | - | 3 | 8 | |
| Adjusted Flow F | Rate (v) | , veh/h | | 468 | | 0 | 1704 | | 0 | 309 | 701 | 1 | 334 | 1366 | |
| Adjusted Satura | ation Flo | ow Rate <i>(s</i>), veh/h/ln | | 1658 | | 1438 | 1673 | | 1519 | 1627 | 1723 | 1 | 1627 | 1706 | 1 |
| Queue Service | Time (g | 1s), S | | 14.8 | | 0.0 | 31.4 | | 0.0 | 9.0 | 21.3 | | 10.1 | 41.8 | |
| Cycle Queue C | learanc | e Time <i>(g₀</i>), s | | 14.8 | | 0.0 | 31.4 | | 0.0 | 9.0 | 21.3 | | 10.1 | 41.8 | |
| Capacity <i>(c</i>), ve | h/h | | | 686 | - | 1 | 1771 | | 137 | 277 | 789 | - | 939 | 1476 | |
| Volume-to-Cap | acity Ra | atio (X) | | 0.683 | | 0.000 | 0.962 | | 0.000 | 1.116 | 0.888 | | 0.355 | 0.926 | |
| Available Capa | city (ca) | , veh/h | | 5124 | | 381 | 1771 | | 201 | 277 | 3128 | | 939 | 3099 | |
| Back of Queue (Q), veh/In (95th percentile) | | | 10.8 | | 0.0 | 22.8 | | 0.0 | 12.2 | 15.2 | | 7.8 | 26.5 | | |
| Overflow Queue (Q3), veh/In | | | 0.0 | | 0.0 | 0.0 | | 0.0 | 0.0 | 0.0 | | 0.0 | 0.0 | | |
| Queue Storage Ratio (RQ) (95th percentile) | | | 1.73 | | 0.00 | 3.61 | | 0.00 | 1.98 | 2.41 | | 1.27 | 4.24 | | |
| Uniform Delay (d1), s/veh | | 48.6 | | 0.0 | 37.4 | | 0.0 | 51.4 | 47.5 | | 38.6 | 43.6 | - | | |
| Incremental Delay (d2), s/veh | | 0.5 | | 0.0 | 13.4 | | 0.0 | 89.0 | 1.4 | | 0.1 | 1.2 | | | |
| Initial Queue Delay (d3), s/veh | | 0.0 | | 0.0 | 0.0 | - | 0.0 | 0.0 | - 0.0 - | - | 0.0 | 0.0 | | | |
| Control Delay (d), s/veh | | 49.1 | | 0.0 | 50.9 | | 0.0 | 140.4 | 48.9 | | 38.7 | 44.7 | | | |
| Level of Service | e (LOS) | | | D | | 1.1.1.1 | D | | | F | D | | D | D | 1 |
| Approach Delay, s/veh / LOS | | 49.1 | | D | 50.9 | | D | 76.9 | 9 | Е | 43.6 | 3 | D | | |
| Intersection De | lay, s/ve | eh / LOS | | | | 53 | 8.5 | | | _ | | | D | | |
| Multimodal Re | sults | | | | EB | | WB | | | NB | | | SB | | |
| Pedestrian LOS Score / LOS | | | 3.3 | - | С | 3.0 | | С | 3.0 | AC 1 1 1 | С | 2.9 | | С | |
| Fedestrian LOC | | | | | | | | | | | | | | | |

2) Target Construction Area in Phase-1

In accordance with the planning policies mentioned in the preceding section, target construction area in phase-1 shall be determined in the following manner.

- Shape and ROW for intersection shall not be changed from Phase-1 to Phase-2 to minimize rework by removal or relocation of road facilities.
- Since the signalized intersection in Phase-1 is not capable of controlling the right-turn vehicle, exclusive right-turn lane(s) for Phase-2 shall be constructed and used during Phase-1 stage of the project.
- Construction of ramps of Phase-2 is required to be done during Phase-1 of the project because the ramps have an important function to connect the two crossing roads. Therefore, all ramps shall be completed during Phase-1.

3) Traffic Signal Layout Plan

A) Typical Layout of Traffic Signal on National Roads in Ghana

In general, traffic signals on national roads of Ghana are provided with a LED (Light Emitting Diode) type lamps. **Figure 2.2.2-5** shows typical layout of traffic signals on national roads. Generally, standing type traffic signals (single pole) are installed in inflow (entry) side and lamps with overhang beam are installed in outflow (exit) side.

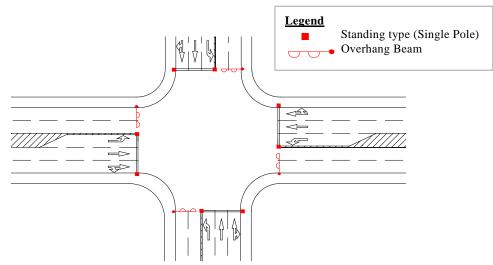


Figure 2.2.2-5 Typical Layout of Existing Traffic Signals on National Roads in Ghana



Figure 2.2.2-6 Current Situation (1)

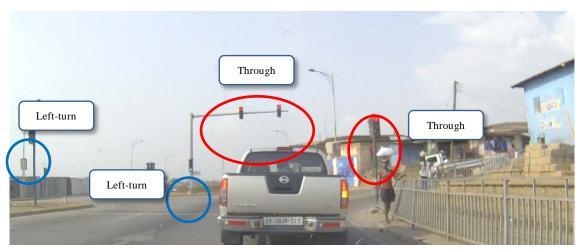


Figure 2.2.2-7 Current Situation (2)

B) Policies on Traffic Signal Layout

Traffic signal layout shall correspond to typical layout on national roads in Ghana. In the case of installation of traffic signal on a wide road, gate (gantry) type is generally used instead of single pole overhang beam (cantilever) type. Although ROW of intersection remains unchanged regardless of the phases, removal and relocation will be required because traffic operation patterns during Phase-1 and Phase-2 will vary and not be similar. To minimize the removal and relocation work, single pole shall be basically applied as much as possible instead of gate (gantry) type.

C) Traffic Signal Wiring System

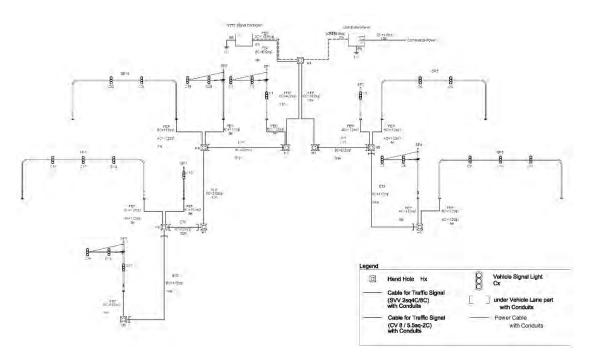


Figure 2.2.2-8 Traffic Signal Wiring System in Phase-1

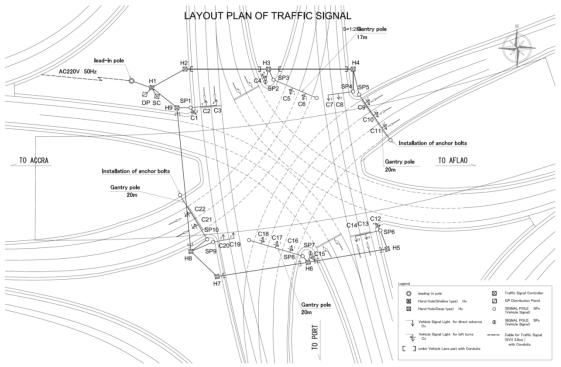


Figure 2.2.2-9 Traffic Signal Layout Plan in Phase-1

(5) Intersection Improvement Plan

Figure 2.2.2-10 shows the intersection improvement plan for Phase-1.

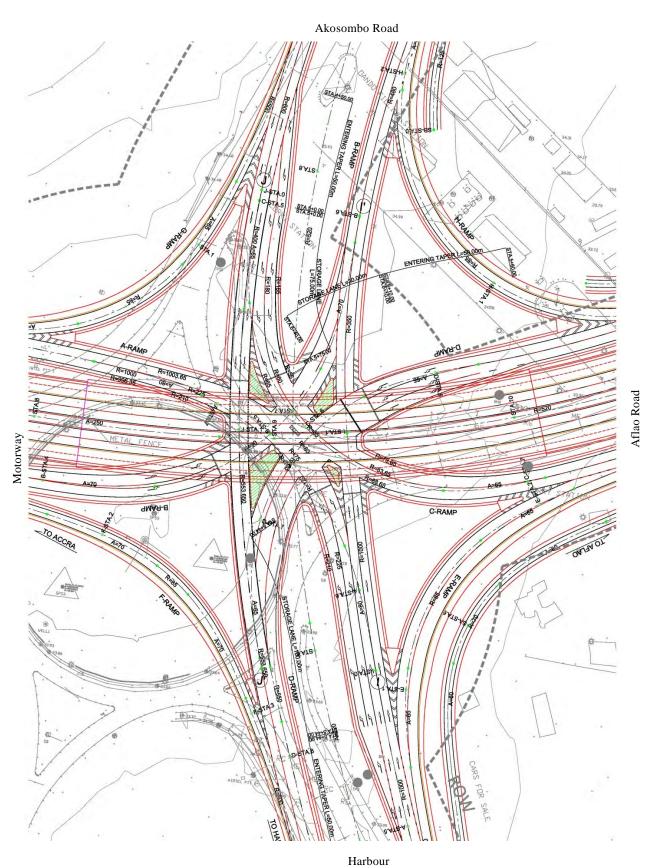


Figure 2.2.2-10 Intersection Plan in Phase-1

2.2.2.2 Road Plan

(1) **Control Points**

Control points taken into consideration for delineation of horizontal and vertical alignments are shown in the aerial photo, while the measures taken for each of these control points are listed up in **Table 2.2.2-8**.

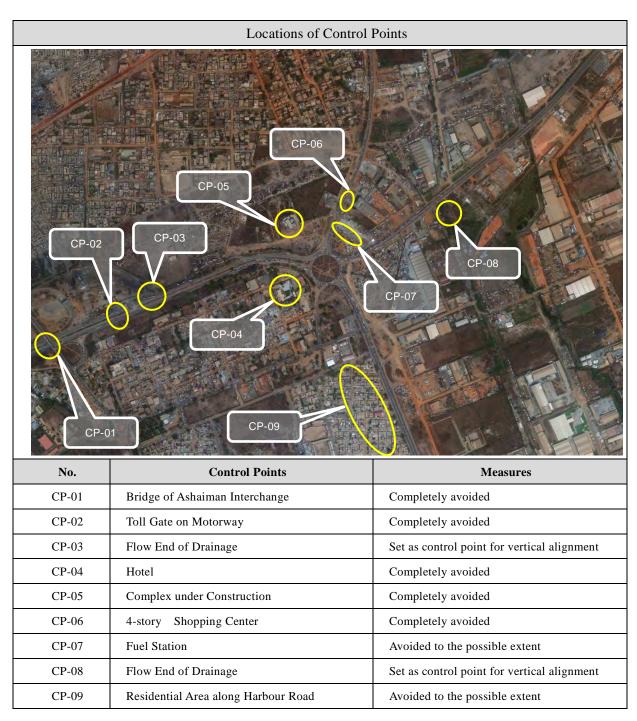


 Table 2.2.2-8 Control Points

(2) Right of Way (ROW)

Figure 2.2.2-11 illustrates the ROW of each road at Tema Roundabout in accordance with the information made available by GHA and DUR.



Figure 2.2.2-11 Existing Road Reserve (ROW) of Each Road at Tema Roundabout

(3) Geometric Conditions/Parameters

The classifications of roads are shown in **Table 2.2.2-9** and the geometric conditions to be applied for each objective road are summarized in **Table 2.2.2-10**. It has been agreed with GHA that excepting the Harbour Road and Tema-Hospital Road, the design speed of all other roads will be 100km/h. The design speed of the Tema- Harbour Road and Tema-Hospital Road will be 80km/ and 50 km/h respectively.

| Objective Roads | Functional Classification |
|---|---------------------------|
| Accra-Tema Road | Motorway |
| Tema-Aflao Road* Tema-Akosombo Road* | National Highway |
| Tema-Harbour Road Tema-Hospital Road | Urban Road |

Table 2.2.2-9 Classification of Roads

Table 2.2.2-10 Geometric Conditions

| | Standards a | and Geometric | Conditions | s (Ghana Ro | oads Design | Guideline) | |
|------------------|---------------------------------------|-----------------------|-------------------|-------------|----------------|--------------------|-----------|
| HI | GHWAY / ROAI | O CLASSIFICATI | ON | Motorway | National | Inter-regional | Urban |
| GENERAL | | | | | | | |
| Min. Design | Flat Terrain (insid | e bracket for urban a | rea) | 120 (100) | 100 (80) | 80 (60) | 80 (60) |
| Speed (km/hr) | | nside bracket for urb | | 100 (80) | 80 (60) | 60 (40) | 60 (40) |
| | · · · · · · · · · · · · · · · · · · · | de bracket for urban | area) | 80 (60) | 60 (40) | 50 (30) | 50 (30) |
| Level of Service | | | | В | С | C | D |
| CROSS-SEC | TION ELEMENT | | | | | | |
| | ROW Width (m |) | Urban | 90 | 90 | 60 | - |
| | Min. Median (m | 1) | Rural Urban | 10 2-4 | 10 2-4 | 2-4 2-4 | 4 2-4 |
| | Median Shoulde | er | Cibai | 0.5-0.75 | 0.3 - 0.5 | 0.3 - 0.5 | 0.3 - 0.5 |
| Road Cross | | | Flat/Rolling | 3.65 | 3.65-3.25 | 3.65 - 3.25 | |
| Section Width | Vehicle Lane | | Mountenous | 3.50 | 3.5-3.25 | 3.5 - 3.0 | 3.65-3.25 |
| | Ramp width (m) | I | | 3.65 - 3.5 | | - | |
| | Right Shoulder | (m) | Flat/Rolling | 3.00 | 2.50 | 2.50 | 2.0 - 3.0 |
| | | | Mountenous | 3.00 | 3.00 | 2.00 | - |
| VERTICAL I | ELEVATION CO | ONTROLS | | | | | |
| Minimum Vorti | cal Clearance (m) | Carriag | eway | 5.5 | 5.5 | 5.5 | 5.5 |
| | | Sidew | alk | 2.5 | 2.5 | 2.5 | 2.5 |
| TRAFFIC VO | DLUME | | | | | | |
| Design Traffic V | Volume (ADT) | | | >10,000 | >10,000 | 3,000-10,000 | <150 |
| STRUCTURI | E LOADING | | | | | | |
| Structure Live I | Loading (Minimum) |) | | BS | 5400 or Equiva | alent (B-load Japa | an) |
| PAVEMENT | STRUCTURE | | | | | | |
| Pavement | Surface Type | | | | Flexible | Pavement | |
| ravement | Crossfall (%) | | | 1.5 - 2.0 | 1.5 - 2.5 | 1.5 - 2.5 | 1.5 - 2.5 |
| GEOMETRIC | C CONDITIONS | | | | | | |
| | | | | | Desigr | Speed | |
| HORIZONTA | L ALIGNMENT | | | 120 | 100 | 80 | 40 |
| | | Desirable (5% SE) | m | 1030 | 700 | 420 | 100 |
| Min. Horizo | ontal Curvature | Minimum (9%SE) | m | 540 | 370 | 230 | 50 |
| Maximum Su | perelevation | | % | | | le 5% ,Acceptab | |
| Min. Curvatu | | | m | 200 | 170 | 140 | 70 |
| | ion Curve Length | | m | 67 | 56 | 44 | 22 |
| | equiring Transition | Curve | m | 1310 | 910 | 580 | 150 |
| | ~ | | 6% | 996 | 694 | 441 | 174 |
| Value of C | | | 5% | 1206 | 849 | 540 | 212 |
| to Radius of | perelevation with | respect | 4% | 1527 | 1091 | 674 | 273 |
| to Radius of | Curvature | | 3% | 1910 | 1348 | 880 | 347 |
| | | | Reverse (2%) | 3510 | 2560 | 1710 | 525 |
| | not requiring Supe | erelevation | m | 7500 | 5000 | 3500 | 800 |
| Superelevation | | | | 1/200 | 1/175 | 1/150 | 1/100 |
| VERTICAL A | LIGNMENT | | | | | | |
| | | Standard | % | 2 | 3 | 4 | 7 |
| Max. Gradier | nt | Gradient with | % (m) | 3% (800m) | 4% (700m) | 5%(600m) | 8% (400m) |
| mun. Graule | | limitations | % (m) | 4% (500m) | 5% (500m) | 6% (500m) | 9% (300m) |
| | | | % (m) | 5% (400m) | 6% (400m) | 7% (400m) | 10% (200m |
| Sight Distanc | re. | Stopping | m | 210 | 160 | 110 | 40 |
| | ~ | Passing | m | 780 | 620 | 500 | 210 |
| Min. Radius | Crest Curve | K-Va | lue | 111 | 64 | 30 | 4 |
| | Crost Curve | Radius | m | 11000 | 6400 | 3000 | 400 |
| Min. Radius | Sag Curve | K-Va | lue | 40 | 28 | 18 | 5 |
| | - | Radius | m | 4000 | 3000 | 2000 | 500 |
| Min. Vertical | l Curve Length | | m | 100 | 85 | 70 | 35 |
| | | | | | | | |

Standards and Geometric Conditions (Ghana Roads Design Guideline)

Note: Those not provided in GRDG is referred from AASHTO's Standard as shown by the italic figures

(4) Horizontal Alignment

Horizontal alignment that meets the geometric requirements was planned taking into consideration the measures against the control points as mentioned in Table 2.2.2-8 and safety and comfort of driving at a high speed. Horizontal curve radii that don't require widening was applied with an aim to keep the scale of the box culvert at the underpass as small as possible. For better understanding of the plan, the ramps were tentatively named

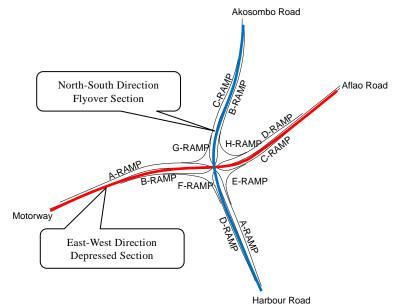


Figure 2.2.2-12 Horizontal Alignment of Main Carriageway and Ramps

for design purpose as shown in **Figure 2.2.2-12**.

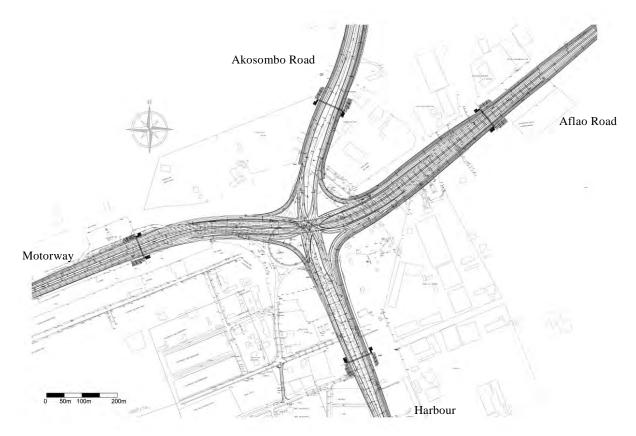


Figure 2.2.2-13 Plan of Main Carriageway and Ramps

(5) Vertical Alignment

Vertical alignment was planned in a manner that not only satisfies the geometric requirements but also addresses the measures to be considered for the control points. A minimum vertical grade of 0.4% was applied such that the road surface water collected can be naturally conveyed/transported to the end flow (outlets) of the drainage. The outlets taken into consideration in the plan are the streams that are currently serving as outlets.

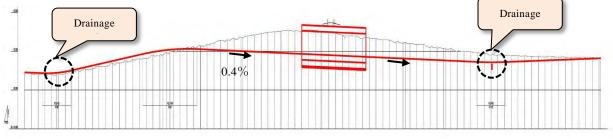


Figure 2.2.2-14 Profile in the East-West Direction

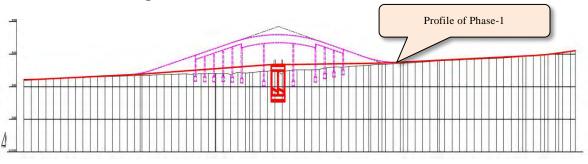


Figure 2.2.2-15 Profile in the North-South Direction

(6) Cross Section Elements

Table 2.2.2-11 indicates the proposed cross section elements of each leg. ROW and lane widths of the objective roads are based on the Ghana Road Design Guideline or in accordance with the existing parameter.

| Existing Roads | Median Width | Shoulder (m) Inner Outer* | | Median Width Shoulder (m) La | | Lane(s) | ROW |
|--|-----------------------|------------------------------|----------------|------------------------------|-----|---------|-----|
| Existing Roads | (m) | | | (m) | Row | | |
| Accra-Tema Motorway | 10 | 0.5 | 3.0* | 3.65 | 45 | | |
| Tema-Hospital Road | - | - | 1.5 | 3.65 | 33 | | |
| Tema-Harbour Road | 4.0 | 0.5 | 2.0 | 3.65 | 45 | | |
| Tema-Aflao Road | 4.0 | 0.5 | 2.5 | 3.65 | 45 | | |
| Tema-Akosombo Road 4.0 0.5 2.5 3.65 45 | | | | | | | |
| *: The width of outer shoulders provided can be reduced to a minimum of 0.6m along structures (flyover). | | | | | | | |
| **: 3.0m wide shoulder | consists of 0.5m wide | e unpaved sho | ulder (verge). | - | - | | |

Table 2.2.2-11 Proposed Cross Section Elements

(7) Reference Points in Road Design

There are two methods in determining the reference points (hereinafter, RP) of a road for the design of vertical alignment and cross section on divided highways. These methods are described in **Table 2.2.2-12**. As the objective roads are wide (4-10m median strip with dual or triple lane double carriageway) and has a superelevated stretch of about 800m, the effect on the abutting houses/residences

or facilities as well as to the sectional area of the box culvert at the underpass is less if the reference point is taken in the middle of carriageways. Therefore, Type A is applied for the alignments in this project.

| Items | Type A : Middle of Carriageway | Type B : Edge of Median |
|-----------|---|--|
| Image | A B' B' | RP A A' (B) |
| Feature | It is applicable to a section with superelevation. Elevation difference between right edge and left edge of the road can be smaller | Since RP is close to center of the road, it is simple to design according to design standard. |
| Condition | Having median and being large in width | none in particular |
| Applied | 0 | - |

 Table 2.2.2-12 Reference Point in Road Design

(8) Design Vehicle

As agreed with the GHA during the technical note meeting, the vehicle to be considered for the design will be WB-20 defined by AASHTO. The basic dimension of the vehicle is illustrated in **Figure 2.2.2-16**.

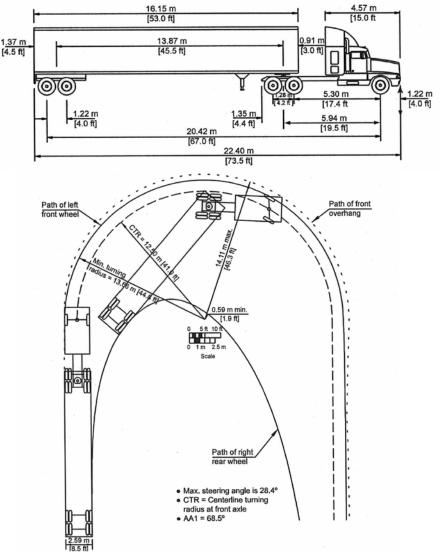


Figure 2.2.2-16 Design Vehicle

(9) Required Number of Lanes

1) Conditions for Calculation of Required Number of Lanes

Following conditions will apply for calculation of required number of lanes;

- The east-west direction will be depressed and north-south direction graded. Connections between these roads will be provided through ramps,
- The required number of lanes will be calculated based on the method recommended by the Highway Capacity Manual 2010. Design Hourly Traffic Volume (DHV) and traffic capacity will be calculated with respect to the target LOS,
- DHV is calculated by converting the Average Annual daily Traffic (AADT) to hourly traffic volume using the time factor (K) and directional factor (D). However, parameters required for Ghanaian formula is not available. Therefore, DHV will be calculated with reference to the method applied in previous JICA study,
- · Access limit will be imposed on Accra-Tema Motorway, Tema-Aflao Road and Tema-

Akosombo Road. Tema-Harbour Road will have an imposition of o. Access from abutting properties will be through service lanes,

- As aforementioned in Section 2-1-6, traffics from Hospital Road will not be allowed in the intersection,
- The LOS agreed in the T/N is 'B' for Tema-Motorway, Tema-Aflao Road and Tema-Akosombo Road and 'C' for Tema-Harbour Road. However, securing the agreed LOS even during peak hours is uneconomical and apparently unrealistic. Therefore, the number of lanes has been calculated taking the LOS of the target year as 'D' (stable to unstable flow), meaning the saturation (ratio of volume by capacity). **Figure 2.2.2-17** shows the condition of flow at each level of service.



Source: Highway Capacity Manual 2010 Figure 2.2.2-17 Examples of LOS

2) Calculation/Result of Required Number of Lanes

Number of lanes was calculated for all legs of the existing Tema Roundabout, the ramps, and the legs of graded intersection (for graded intersection type configuration). The results are summarized hereunder.

A) Road Section

Road section means the section excluding ramps, graded roads and section within the intersection for partial free-flow configuration. The result of required number of lanes on road sections of each leg is shown in **Table 2.2.2-13**. The results show that for the conditions provided, 4-lanes are sufficient on Accra-Tema Motorway. However, Tema-Aflao Road needs 6-lanes. Therefore, in order to maintain continuity with Tema-Aflao Road, as well as taking the Ghanaian PPP scheme (which has plans to widen the Motorway to 6-lanes) into

consideration, it is decided that the Motorway will be planned to have the road space for 6lanes. However, pavement will be applied on the inner 4-lanes only.

| Road | | Accra-Tema Motorway | Tema-Akosombo Road | Tema-Aflao Road | Tema Harbour Road | Tema Hospital Road | |
|------------------------------|-----------------|---------------------|--------------------|-----------------|-------------------|--------------------|-----------------|
| | Type of highway | | Freeway | Freeway | Freeway | Multi lane | Two lane |
| Access | | | Full control | Full control | Full control | Partial control | Partial control |
| | Studied len | gth (m) | 760 | 940 | 800 | 600 | 270 |
| | | Passenger car | 2498 | 2540 | 3593 | 4974 | 1201 |
| | Inbound | Heavy vehicle | 239 | 156 | 264 | 307 | 105 |
| Peak hour | [| Sub total | 2737 | 2696 | 3857 | 5281 | 1306 |
| traffic | | Passenger car | 3098 | 3893 | 3950 | 2594 | 1273 |
| volume | Outbound | Heavy vehicle | 302 | 228 | 292 | 165 | 84 |
| (veh/h) | | Sub total | 3400 | 4121 | 4242 | 2759 | 1357 |
| | Higher value | Passenger car | 3098 | 3893 | 3950 | 4974 | 1273 |
| | righer value | Heavy vehicle | 302 | 228 | 292 | 307 | 105 |
| | Demand volun | ne (veh/h) | 3400 | 4121 | 4242 | 5281 | 1378 |
| EES (Eroo | -flow speed) | km/h | 100 | 100 | 100 | 80 | 40 |
| FFS (FIGE | -now speed) | mi/h | 62.5 | 62.5 | 62.5 | 50 | 25 |
| Lon | e width | m | 3.65 | 3.65 | 3.65 | 3.65 | 3.65 |
| Lain | ewidin | ft | 12.0 | 12.0 | 12.0 | 12.0 | 12.0 |
| Latoral | Clearance | m | 2.5 | 2.5 | 2.5 | 2 | 1.5 |
| Laterar | Clearance | ft | 8.2 | 8.2 | 8.2 | 6.6 | 4.9 |
| Total Ra | mp Density | ramps/mi | 0.33 | 0.17 | 0.17 | - | - |
| Peak H | our Factor | - | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 |
| Terrain | Segments | Grade (%) | 2 | 1 | 1 | level | level |
| | | Inbound | 8.9 | 5.5 | 6.9 | 6.0 | 6.2 |
| % of Hea | avy vehicle | Outbound | 8.7 | 5.8 | 6.8 | 5.8 | 8.0 |
| | Γ | Applied value | 8.9 | 5.8 | 6.9 | 6.0 | 8.0 |
| Pas | ssenger-car equ | ivalent (PCE) | 2.5 | 2.5 | 2.5 | 2.5 | 2.5 |
| | Target L | .OS | D | D | D | D | D |
| Access-Point Density Inbound | | Inbound | - | - | - | 8 | 14 |
| (access points/mi) Outbound | | | - | - | 8 | 14 | |
| Numbe | r of lanes | Inbound | 2 | 3 | 3 | 3 | 1 |
| Rec | quired | Outbound | 2 | 3 | 3 | 3 | 1 |

Table 2.2.2-13 Required Number of Lanes on Road Sections

B) Flyover (Graded Section)

The sections that are elevated (flyover) and accommodate through traffics (vehicles going straight) are subjected to flyover. That is the flyover section of north-south bound (Akosombo-Tema Harbour) or east-west bound (Accra-Aflao). The results of the required number of lanes on these flyover sections are presented in **Table 2.2.2-14**. The result indicates that 4-lanes are sufficient for both Motorway-Aflao Road and Akosombo-Harbour Road for both cases. However, for the reasons mentioned above, the Motorway will be planned to have a space for 6-lanes whose 4-lanes (inner side) will be paved.

 Table 2.2.2-14 Required Number of Lanes on Flyovers (Graded Section)

| Road | | Motorway | / - Aflao Road | Akosombo - | Harbour Road |
|--------------------------------|---------------|----------|----------------|------------|--------------|
| Direction | | EB | WB | SB | NB |
| Type of high | way | Freeway | Freeway | Freeway | Multi lane |
| Peak hour traffic volume | Passenger car | 1368 | 930 | 2360 | 1186 |
| (veh/h) | Heavy vehicle | 43 | 25 | 6 | 28 |
| Demand volume | (veh/h) | 1411 | 955 | 2366 | 1214 |
| FFS (Free-flow speed) | km/h | 100 | 100 | 100 | 80 |
| FFS (Free-now speed) | mi/h | 62.5 | 62.5 | 62.5 | 50 |
| Laws with the | m | 3.65 | 3.65 | 3.65 | 3.65 |
| Lane width | ft. | 12.0 | 12.0 | 12.0 | 12.0 |
| Lateral Clearance | m | 2.5 | 2.5 | 2.5 | 2 |
| Lateral Clearance | ft. | 8.2 | 8.2 | 8.2 | 6.6 |
| Total Ramp Density | ramps/mi | 0.33 | 0.17 | 0.17 | - |
| Peak Hour Factor | - | 0.98 | 0.98 | 0.98 | 0.98 |
| Terrain Segments | Grade (%) | 3.0% | 3.0% | 3.0% | 3.0% |
| % Heavy tra | ick | 3.0% | 2.6% | 0.3% | 2.3% |
| Passenger-car equivalent (PCE) | | 2.5 | 2.5 | 2.5 | 2.5 |
| LOS as of 20 | 035 | В | A | С | A |
| Number of lanes Required | | 2 | 2 | 2 | 2 |

C) Ramps

The concept of on-ramp and off-ramp is shown in **Figure 2.2.2-18**. Vehicles diverting from the main road and going to another main road (ahead of the intersection) are off-ramp traffic, and vehicles merging into the main road after diverting from another main road (beyond the intersection) are on-ramp vehicles.

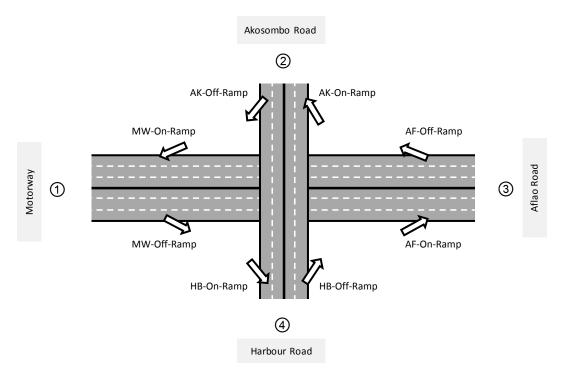


Figure 2.2.2-18 Conceptual Image of Ramps

| No. | | 1 | | 2 | | 3 | | (4) | | |
|------------------|-----------|---------|--------|-------|----------|--------|-------|-------|---------|-------|
| | Road | | Motory | vay | Akosombo | o Road | Aflao | Road | Harbour | Road |
| R | amp Type | | On | Off | On | Off | On | Off | On | Off |
| Volur | 20 | Freeway | 955 | 1411 | 1214 | 2366 | 1411 | 955 | 2366 | 1214 |
| Volui | ne | Ramp | 1346 | 2066 | 378 | 1926 | 1939 | 3321 | 1288 | 2471 |
| | Freeway | km/h | 100 | 100 | 100 | 100 | 100 | 100 | 80 | 80 |
| FFS (Free- | гтеемау | mi/h | 62.5 | 62.5 | 62.5 | 62.5 | 62.5 | 62.5 | 50 | 50 |
| flow speed) | Ramp | km/h | 50 | 50 | 50 | 50 | 50 | 50 | 50 | 50 |
| | | mi/h | 31.25 | 31.25 | 31.25 | 31.25 | 31.25 | 31.25 | 31.25 | 31.25 |
| Ave. % of Hv | Free | eway | 3% | 3% | 2% | 0% | 3% | 3% | 0% | 2% |
| Ave. // ULHV | Ra | mp | 6% | 9% | 6% | 6% | 7% | 5% | 8% | 7% |
| Grade | | | Level | Level | Level | Level | Level | Level | Level | Level |
| Peak Hour Factor | | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 | 0.98 | |
| Number o | f lanes R | equired | 1 | 2 | 1 | 2 | 1 | 2 | 1 | 2 |
| LOS | | В | В | В | В | С | В | С | Α | |

Table 2.2.2-15 Required Number of Lanes on Ramp Section

D) Summary

The result of number of lanes to be provided for each section under this improvement plan is indicated in **Table 2.2.2-15**. The results come from the calculation described above as well as consideration taken in view to secure consistency with future plans of GOG and ECOWAS.

| Sections | | Accra-Tema Motorway | Tema-Aflao Road | Tema- Akosombo Road | Tema- Harbour Road | Tema-Hospital Road |
|--|-----|-------------------------|-------------------------|---------------------------|--------------------------|-----------------------|
| Road Section (both sides combined) | | 6 (4-lanes paved) | 6 | 6 | 6 | 2 |
| Flyover section (both sides combined) | | 6 (4-lanes paved) | 6 (4-lanes paved) | 4 | 4 | - |
| Demas | OFF | 2 | 2 | 2 | 2 | Ramps |
| Ramps | ON | 1 | 1 | 1 | 1 | |

 Table 2.2.2-16 Summary of Required Number of Lanes for All Sections

(10) Typical Cross Section

Following drawings shows typical cross section.

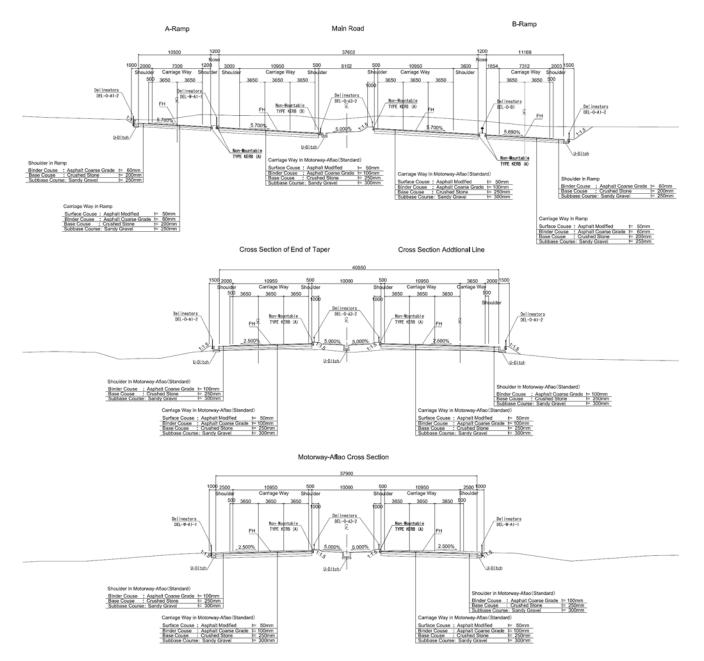
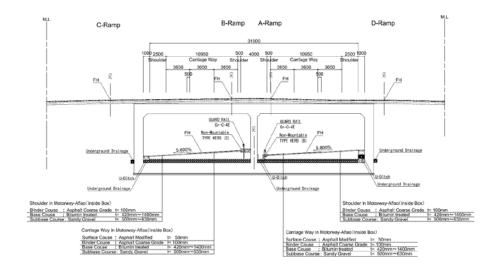


Figure 2.2.2-19 Motorway-Aflao Road: Typical Cross Section (1)

Cross Section at Underpass (Box CUlvert)



Cross Section at Depressed Section (Outside Box Culvert)

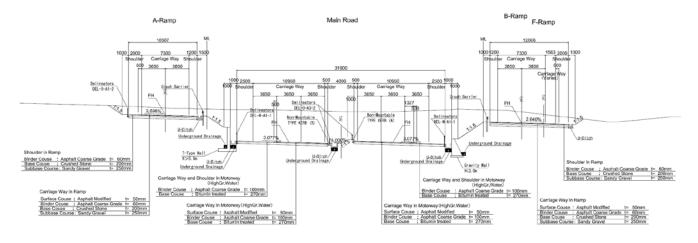


Figure 2.2.2-20 Motorway-Aflao Road: Typical Cross Section (2)

11000

7300

1200 1000

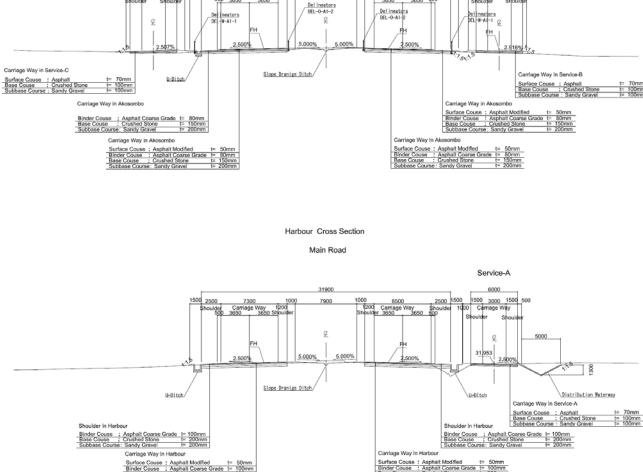
Service-C

6000

1500

500 1500 3000

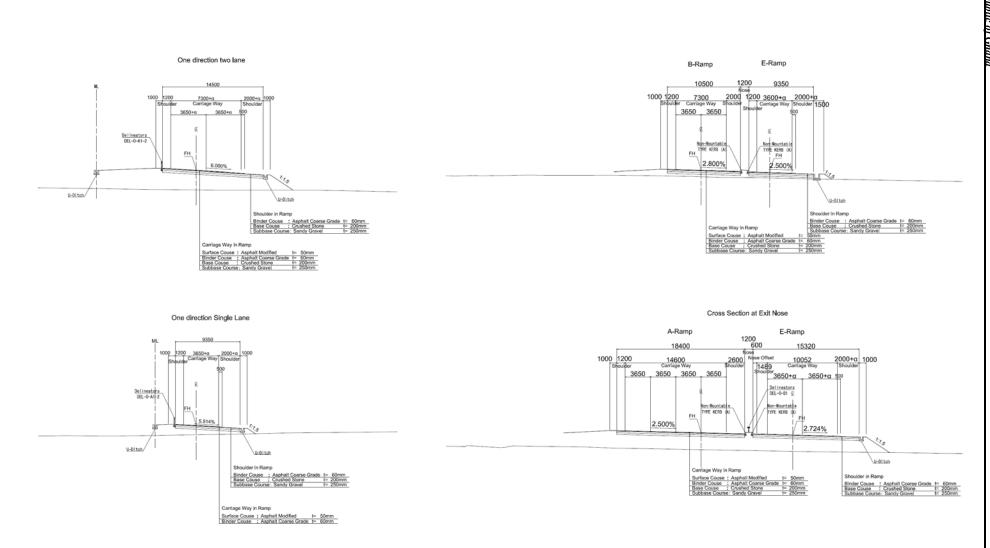
Final Report



9900

Figure 2.2.2-21 Harbour Road - Akosombo Road: Typical Cross Section

Grad



- 94 -

2.2.2.3 Structure Plan

(1) **Outline**

In the East-West direction, proposed height of the road goes below the existing ground level. Thereby, this section will be depressed with an underpass at the intersecting point with the north-south direction. **Photo 2.2.2-1** shows an example of an underpass. As can be understood from the photo, this type of underpass will require provision of retaining walls along the depresses section and a culvert or a bridge at the underpass.



Photo 2.2.2-1 Example of Depressed and Underpass Section

(2) Basic Policy

Basic policies for planning of structures are;

- Retaining wall shall be applied instead of earth cut in order to minimize influence to the surrounding environment.
- The structures applicable for retaining purpose will be selected from conceivable alternatives.
- A box culvert is planned for underpass structure at the intersection point of east-west and northsouth direction roads.

(3) **Design Condition**

Design conditions of retaining walls are as summarized below.

| Type of retaining wall | | Inverted-T Type | | | |
|------------------------|---|----------------------------|--|--|--|
| Maxim | um wall height | H=10.0m | | | |
| Upper Load | Design live load (Weight of design vehicle) | 250kN | | | |
| LUau | Unit weight of backfilling soil | $\gamma = 19 \text{kN/m3}$ | | | |
| | Table 2.2.2-18 Design condition for BOX Culvert | | | | |
| Inner se | ection of BOX Culvert | B (16.45+16.45) m x H 7.5m | | | |
| Max. v | ertical gradient | i=0.4% | | | |
| Width o | of carriageway | W=13.95m | | | |
| Upper Load | Design live load (Weight of design vehicle) | 250kN | | | |
| | Pavement | t=80mm | | | |

Table 2.2.2-17 Design conditions of Retaining Wall

| Design strength of concrete | $\alpha k=24 \text{ kN/m3}$ |
|--|-----------------------------|
| Allowable bending compressive stress of | ∞k=8.0 kN/mm2 |
| concrete | |
| Allowable shearing stress of concrete | τa=0.39 N/mm2 |
| Allowable adhesion stress of concrete | τa=1.6 N/mm2 |
| Allowable tensile stress of rebar | csa=180 N/mm2 |
| Used diameter of rebar | 12mm, 16mm, 20mm, 25mm |
| Allowable bearing capacity of foundation | 600 kN/m2 |
| ground | |

Table 2.2.2-19 Material Condition

(4) Scope of Structures

The sections where structures (retaining walls and box culvert) are planned are shown in Figure 2.2.2-23.

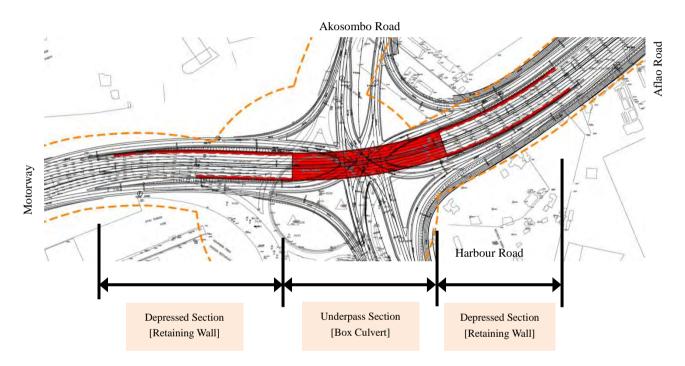


Figure 2.2.2-23 Scope of Structures

(5) Selection of Retaining Wall Types

Table 2.2.2-20 shows the types of retaining walls that are commonly applicable along the depressed section. The comparison results show that inverted-T Type and Gravity Type are recommendable for application in this section. The former is recommended where the height exceeds 3m and the latter for heights lesser than 3m. Both the types are not only comparatively economical but are also effective in minimizing the rear side excavation area. Minimizing the excavation area at the rear side of the retaining walls is important to ensure that the detour for maintaining the existing traffic flow during construction can be provided within the existing ROW.

| r | Гуре | Gravity Type | Inverted-T Type | L-Type (Cast in place) | |
|---------------|-------------------------|---|---|------------------------|--|
| Image | | | | | |
| F | eature | Resistant against earth pressure by self weight Plain concrete structure | Resistant against earth pressure by self weight and soil weight the slab Wall and slab are resittant as cantilever beam against external force | | |
| Covera | age Height | Lower than around 5m | 3~10m | | |
| | e of Rear excavation | Middle | Middle | Large | |
| | 3.0m | 1.0 | 0.6 | 0.7 | |
| Cost | 4.0m | 1.3 | 0.9 | 1.0 | |
| Ratio of Cost | 5.0m | 2.0 | 1.2 | 1.3 | |
| tio | 6.0m | | 1.8 | 1.9 | |
| Rat | 7.0m | | 2.4 | | |
| | 8.0m | - | 3.3 | - | |
| Eva | aluation | Recommendable | Recommendable | Not Recommendable | |
| A | pplied | Section where wall height is lower than 3m. | Section where wall height is higher than 3m. | Not applicable | |

Table 2.2.2-20 Comparison of Retaining Wall Types

(6) Box Culvert

Typical cross section at the underpass (box culvert) is shown in **Figure 2.2.2-24**. The underpass lies in a curved alignment and has a maximum superelevation of 5.8%.

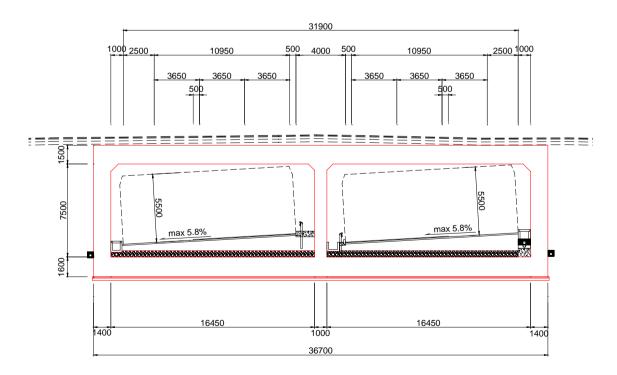


Figure 2.2.2-24 Typical Cross Section of Box Culvert

(7) Measures against Groundwater Leakage

1) Basic Policy

The groundwater observation has confirmed existence of groundwater at the project area. The level of groundwater is at about 6 to 7 meters from the ground surface. In particular, observation well No.2 which is located close to the planned box culvert recorded the highest groundwater level at an approximate depth of 6 meters from the surface. As shown in **Figure 2.2.2-26**, this level is slightly higher than the bottom slab of the box culvert.

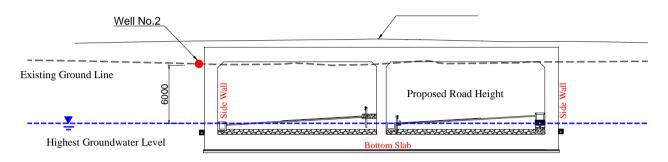


Figure 2.2.2-26 Groundwater Level (within box culvert section)

Joints or openings in the side walls and within the bottom slab of the box culvert could be outlets

where the groundwater could be leaked, especially in the future when there are drastic changes in climatic conditions.

In general, measures as shown in **Figure 2.2.2-26** is applied at joints of all walls in a box culvert, but this is mainly for preventing leakage caused by rainfall. In the presence of groundwater this may not be functionally sufficient. Therefore, additional measure was applied to

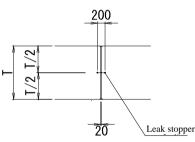


Figure 2.2.2-25 General Measure against Water Leakage

make the structure more resistant to the leakage of water. The measure applied was selected following a comparison of various options, which is discussed in the next section.

2) Additional Measure against Groundwater Leakage

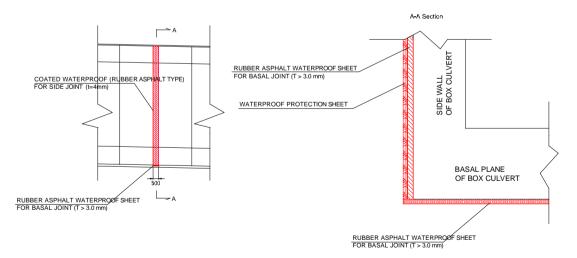
Table 2.2.2-21 shows the applicable additional measures for preventing leakage of groundwater. Taking into consideration the fact that the box culvert will be installed under the ground, application of water resistant sheet and liquid membrane are most suitable in terms of construction efficiency and cost. Application of concrete admixture is also effective and can be applied in the arch concrete above the bottom slab.

| | Installation of Waterproof | Application of Liquid | Application of Barrier | Application of Concrete |
|-----------------------------|---|---|--|--|
| Methods | Sheet | Membrane | Penetrant | Admixture |
| Outline | | | | 毘 |
| | Impermeable gum sheet is manually pasted with adhering agent. | Liquid material consisting primarily of "High- Polymerized Compound" is applied to surface of concrete to form the waterproofing layer. | Silane or Silicate will be impregnated into the concrete to fill up small voids and prevent water penetration. | Concrete admixture, which induces chemical reaction to generate silica component, creates watertight concrete with self-reactive healing function for crack repair. |
| Durability | Protection sheet is additionally required since the waterproof sheet is easily damaged by earth filling during the construction. The sheet is inferior in deformational flexibility to cracks in concrete. | Protection sheet is additionally required since the waterproof sheet is easily damaged by earth filling during the construction. The membrane is excellent in deformational flexibility to cracks in concrete. | Re-coating is required every 10 years since this type easily deteriorates. It is unsuitable for underground structures. | Durability is sustained as long as concrete keeps its soundness. There is no fear that damage will occur by the backfilling of embankment during construction. |
| Workability | Accomplished skills and accuracy in flatness of concrete surface are required in order to firmly adhere to the sheet to the concrete. | Construction work is comparatively easy at shorter time by using spray type. | Construction work is comparatively easy at shorter time by using spray type. | Work Time will be saved due to simple work such as mixing the concrete admixture at the time of concrete batching. |
| Cost Ratio | 1.80 | 1.20 | 0.80 | 1.00 |
| Evaluation (Application) | Applicable (bottom slab expansion joints) | Applicable (side wall construction joints) | Not Applicable | Applicable (Bottom slab/ arch Concrete) |

Table 2.2.2-21 Comparison of Measures against Groundwater Leakage

3) Applied Area

The areas where the additional measures for preventing leakage of groundwater are to be applied are as shown in **Figure 2.2.2-27**.





2.2.2.4 Pavement Plan

(1) Condition of Existing Pavement

The existing pavement on the Motorway is a rigid type (concrete) pavement. Other roads have flexible

(asphaltic) pavement. Although partial deterioration on the pavement surface was found, most of the pavement is still under the good to fair condition.

As shown in **Figure 2.2.2-28**, existing pavement survey was conducted by soil compaction tester for the purpose to roughly evaluate existing pavement condition and to check if the laboratory test result is appropriate. In the survey, an average of 3 separately measured CBR values at a point was taken as the field CBR at that point. For pavement design, CBR obtained from laboratory test was used.



Figure 2.2.2-28 Existing Pavement Survey by Soil Compaction Tester

| Thickness | Material | CBR Value | Photos |
|--|--------------------------------------|---|--------|
| Surface Course t=20cm(Carriage way) t=5cm (Shoulder) | 【Carriage way】 Cement Concrete | - | |
| | 【Shoulder】 Asphalt concrete | - | |
| Base Course/ Sub-base Course t=35cm | CLAY with Gravel | 188 | |
| Subgrade | Clayey Sand | 99 (Surface on subgrade) 23 (10cm down from surface) | |

1) Accra- Tema Motorway

2) Tema-Hospital Road

| Thickness | Material | CBR Value | Photos |
|--|---|-----------------------------------|--------|
| Surface Course t=8cm | [Carriage way] Asphalt concrete [Shoulder] Asphalt concrete | - | |
| Base Course/ Sub-base Course t=25cm | Crashed stone | 96 (Surface on base-course) | |
| Subgrade | Clayey Sand | 105 (Surface on subgrade) | |

3) Tema-Harbour Road

| Thickness | Material | CBR Value | Photos |
|--|---|------------------------------------|--------|
| Surface Course t=15cm(Carriage way)※1 t=5cm(Shoulder) | [Carriage way] Asphalt concrete [Shoulder] Asphalt concrete | - | |
| | | 202 (Surface on base-course) | |
| Base Course/ Sub-base Course t=30cm | CLAY with Gravel | 145 (15cm down from surface) | |
| | | 128 (25cm down from surface) | |
| Subgrade※3 | CLAY with Gravel | 39 (30cm down from subgrade) | |

※1. Height of pavement was assumed to be up to the height of the curb
※2. Existence of subgrade was not confirmed under 50 cm from the surface
※3. Unpaved section outside of carriage way was assumed to be subgrade.

4) Tema-Aflao Road

| Thickness | Material | CBR Value | Photos |
|---|---------------------------------------|---|--------|
| Surface Course t=15cm(Carriage way) | 【Carriage way】 Asphalt concrete | - | |
| t=5cm(Shoulder) | 【Shoulder】 Asphalt concrete | | |
| Base Course/ Sub-base Course t=15cm | Crashed stone | N/A (Surface on base-course) 113 (5cm down from surface) | |
| Subgrade | Sandy CLAY with Gravel | 99 (Surface on subgrade) | |

5) Tema-Akosombo Road

| Thickness | Material | CBR Value | Photos |
|---|---------------------------------------|---|--------|
| Surface Course t=10cm(Carriage way) | 【Carriage way】 Asphalt concrete | - | |
| t=10cm(Shoulder) | 【Shoulder】 Asphalt concrete | | |
| Base Course/ Sub-base Course t=10cm | Crashed stone | 82 (Surface on base-course) | |
| Subgrade | Sandy CLAY with Gravel | 111 (Surface on subgrade) 63 (10cm down from subgrade) | |

(2) Basic Policies

Basic policies for pavement design are as follows;

- Flexible pavement which is common in Ghana shall be applied.
- AASHTO Guide for Design of Pavement Structure, 1993 shall be adopted.
- Countermeasures for groundwater shall be considered.
- Minimum pavement thickness to be applied will be 10cm or more.
- The possible future increment of heavy traffic passing the objective intersection is anticipated.

At the intersection, drivers would have to negotiate curves at low driving speed. Against plastic deformation of wearing course of pavement, modified asphalt shall be applied to the wearing course. For the service roads and temporary detour, modified asphalt shall not be applied.

- As traffic volumes differ, pavement of main carriageway and ramp shall be designed separately.
- Pavement structure of temporary detour shall be designed for the year 2020 with an assumption that the project will be implemented in 2019 and complete by year 2020. Although pavement composition may vary due to difference of traffic volume and CBR values, the pavement structure that has the biggest layer values will be applied for all the roads.

(3) Pavement Design

1) Design Period

Design period shall be 15 years as stipulated in the Pavement Design Manual (GHA).

2) Design Parameters

Design parameters are summarized in **Table 2.2.2-23**. The Structural Number (SN) required for the asphalt pavement will be calculated from the following formula.

| Item | Description | Design Condition | Remarks |
|-------------------------|---|---|--|
| Design Period | The period of time that an initial pavement structure will last before it needs rehabilitation. | 15 years (2020 – 2034) | |
| Traffic Load | The traffic load is expressed by cumulative number of 18-kip equivalent single axle load (ESAL) applications (w_{18}) during the performance period and is calculated based on the future traffic volume which is converted to 18-kip ESALs applying the axle load equivalency factors given in the AASHTO Guide. | Calculated by traffic demand forecast | |
| Reliability | Means of incorporating some degree of certainty into the design process. | The level of reliability (R)=90% Standard normal deviate corresponding to level of reliability (Z_R) = -1.282 Combined standard error of the traffic prediction and performance prediction (S_o) = 0.44 | |
| Performance Criteria | The Present Serviceability Index (PSI) is used to represent pavement performance. The total change in PSI (\triangle PSI) is defined as the difference between initial serviceability index (p ₀ : value immediately after construction) and terminal serviceability index (p _t : lowest index that will tolerate before rehabilitation, resurfacing or reconstruction) | $p_0 = 4.2$ $p_t = 2.5$ | p₀=4.2 pt=2.5 △PSI=1.7 |
| Roadbed Soil | The resilient modulus (M_R) is used. The AASHTO Guide introduces the equation | CBR=9.6~20 | M _R =14,400psi ∼ 30,000 |

Table 2.2.2-22 Design Parameters (Inputs) of Pavement Design

| Item | Description | Design Condition | Remarks |
|---|--|---|---------|
| Property | estimating M_R from CBR as $M_R = 1,500 \times CBR$ (CBR is regarded as 20 in case of CBR more than 20) | (calculated based on CBR investigation) | |
| Pavement Layer Material Properties | The pavement strength is expressed by the structural number (SN) which is calculated as : $SN = a_1D_1 + a_2D_{2m_2} + a_3D_{3m_3}$ where $a_i=i^{th}$ layer coefficient $D_i=i^{th}$ layer thickness (inches) $m_i=i^{th}$ layer drainage coefficient | Asphalt concrete wearing course : a1=0.41 (EAC=425,000 psi) Asphalt concrete binder course : a2=0.39 (EAC=425,000 psi) Base : :a4=0.14 Subbase : a5=0.11 | |
| Drainage Condition | The factor to modify the SN considering the effects of drainage. | m ₄ =m ₅ =1.0 (water removed within 1 week, and pavement structure is exposed to moisture levels approaching saturation during 5% of the year) | |

3) Design Traffic Volume

 Table 2.2.2-23 shows design traffic volumes calculated using traffic survey data conducted in April 2015.

| | Design Traffic Volume (vehicle / day) | | | | | | | | | |
|------------------|---------------------------------------|-----|-------------|----------------|---------|--------|--|--|--|--|
| Road | Car, Taxi, Minibus | Bus | Light Truck | Heavy Truck | Trailer | Others | | | | |
| Motorway | 16,528 | 177 | 2,393 | 1,218 | 1,158 | 403 | | | | |
| Akosombo Road | 23,691 | 607 | 1,369 | 951 | 484 | 173 | | | | |
| Aflao Road | 27,254 | 329 | 1,705 | 993 | 1,037 | 320 | | | | |
| Harbour Road | 21,101 | 983 | 1,232 | 1,256 | 1,387 | 485 | | | | |

Table 2.2.2-23 Design Traffic Volume

Design traffic volume for ramps was calculated based on the directional traffic volume during peak hours.

| | | Conversion | Traffic Volume by Deirection (Vehicle/day) | | | | | | | |
|--|-----------------|------------|---|-----|-------------|-------------|---------|--------|--|--|
| Road | Section | factor | Car, Taxi, Minibus | Bus | Light Truck | Heavy Truck | Trailer | Others | | |
| | Right turn lane | 45.2% | 7,471 | 80 | 1,082 | 551 | 523 | 182 | | |
| Motorway | Left turn lane | 25.4% | 4,198 | 45 | 608 | 309 | 294 | 102 | | |
| | Common section | 70.6% | 11,669 | 125 | 1,689 | 860 | 818 | 285 | | |
| 41 1 | Right turn lane | 47.6% | 11,277 | 289 | 652 | 453 | 230 | 82 | | |
| | Left turn lane | 11.0% | 2,606 | 67 | 151 | 105 | 53 | 19 | | |
| Koau | Common section | 58.6% | 13,883 | 356 | 802 | 557 | 284 | 101 | | |
| | Right turn lane | 11.4% | 3,107 | 38 | 194 | 113 | 118 | 36 | | |
| Aflao Road | Left turn lane | 55.8% | 15,208 | 184 | 951 | 554 | 579 | 179 | | |
| | Common section | 67.2% | Car, Taxi, Minibus Bus Light Truck Heavy Truck 2% 7,471 80 1,082 551 4% 4,198 45 608 309 6% 11,669 125 1,689 860 6% 11,277 289 652 453 0% 2,606 67 151 105 6% 13,883 356 802 557 4% 3,107 38 194 113 8% 15,208 184 951 554 2% 18,315 221 1,146 667 8% 6,921 322 404 412 5% 5,170 241 302 308 | 697 | 215 | | | | | |
| | Right turn lane | 32.8% | 6,921 | 322 | 404 | 412 | 455 | 159 | | |
| Motorway Akosombo Road Aflao Road Harbour Road | Left turn lane | 24.5% | 5,170 | 241 | 302 | 308 | 340 | 119 | | |
| | Common section | 56.3% | 11,880 | 553 | 694 | 707 | 781 | 273 | | |

Table 2.2.2-24 Design Traffic Volume

Accurate forecasting of traffic volume on service roads from the traffic volume obtained from the traffic count survey is virtually not possible as presently there are no service roads in the project area. Therefore, the volume is determined assuming 3% of the total traffic will use the service road. The assumed traffic volumes are as shown in **Table 2.2.2-25**.

| Road | Design Traffic | Remarks | | |
|----------|--------------------|---------|-------|-----------------|
| | Car, Taxi, Minibus | Bus | Truck | Kellar K5 |
| Motorway | 496 | 5 | 72 | No service road |
| Akosombo | 711 | 18 | 41 | |
| Aflao | 818 | 10 | 51 | |
| Harbour | 633 | 20 | 37 | |

| Table | 2.2.2- | 25 Desig | gn Traffic | Volume |
|-------|--------|----------|------------|--------|
| Lanc | | | zn manne | volume |

Source: JICA Survey Team

4) Growth Rates of Traffic Volumes

Growth rates of traffic volumes were determined subject to the result of future traffic demand.

5) CBR of Subbases

Figure 2.2.2-29 illustrates locations of the field CBR tests, and **Table 2.2.2-26** shows the results of laboratory CBR tests. In pavement design, CBR obtained from laboratory test was applied.



Figure 2.2.2-29 Location of Field CBR Test

| SAMPLE ID | | REFERENCE | SOIL | NATURAL MOISTURE | SPECIFIC | | | COMPACTION | | CBR | |
|-------------|------------|-----------|----------------|---------------------|----------|-------------|--------------------------|------------|---------|---------|-----|
| | (KM) | | CLASSIFICATION | CONTENT (%) | GRAVITY | INDEX (%) | MDD (g/cm ³) | OMC (%) | 95% MDD | 98% MDD | % |
| M1C Lay.1 | | | GM | 2.90 | 2.62 | 10.1 | 2.26 | 4.8 | 75 | 84 | 95 |
| M1C Lay.2 | 1 - 0+580 | Shoulder | GW | 3.50 | 2.598 | 18.1 | 2.26 | 6.5 | 45 | 54 | 46 |
| M1C Lay.3 | | | GP | 3.40 | 2.569 | 9.2 | 2.175 | 6.0 | 35 | 42 | 44 |
| M2L Lay. 1 | | | SW | 4.00 | 2.685 | NP | 2.025 | 6.2 | 64 | 72 | 96 |
| M2L Lay. 2 | 2 - 0+700 | Subgrade | SM | 4.60 | 2.61 | NP | N.A | N.A | N.A | N.A | 129 |
| M2L Lay. 3 | | | СН | 5.20 | 2.564 | 31.5 | 2045 | 13.5 | 7 | 10 | 28 |
| H1R Lay-1 | | | GW | 4.40 | 2.85 | non-plastic | 2.335 | 7.2 | 65 | 93 | 75 |
| H1R Lay. 2 | 3 - 0+135 | Shoulder | GM | 6.10 | 2.685 | 7 | 2.227 | 7.8 | 52 | 70 | 73 |
| H1R Lay. 3 | | | GC | 8.60 | 2.672 | 12.8 | 1.871 | 13.2 | 21 | 26 | 40 |
| HH1R Lay 1 | | | GW | 2.50 | 2.753 | NP | 2.398 | 5.0 | 89 | 112 | 132 |
| HH1R Lay 2 | 4 - 0+160 | Shoulder | GM | 3.40 | 2.638 | 3.5 | 2.25 | 7.0 | 45 | 54 | 56 |
| HH1R Lay 3 | | | GM | 4.00 | 2.698 | NP | 2.32 | 5.0 | 28 | 35 | 32 |
| TH2L Lay.1 | 5 - 0+200 | Shoulder | GM | 2.50 | 2.653 | 9.5 | 2.198 | 7.4 | 75 | 90 | 153 |
| TH1L Lay. 1 | | | GM | 6.00 | 2.618 | 10.6 | 2.19 | 6.5 | 55 | 75 | 280 |
| TH1L Lay. 2 | 6 - 0+60 | Subgrade | GC | 5.50 | 2.637 | 10.5 | 2.2 | 6.7 | 50 | 65 | 47 |
| TH1L Lay. 3 | | | GM | 5.00 | 2.58 | 11.5 | 2.21 | 8.5 | 42 | 53 | 151 |
| A1R Lay.1 | 7 - 0+100 | Shoulder | GW | 3.60 | 2.658 | 3.2 | 2.378 | 5.2 | 85 | 109 | 111 |
| A1R Lay.2 | 7 - 0+100 | Shoulder | GM | 3.50 | 2.653 | 11.8 | 2.21 | 7.2 | 55 | 65 | 96 |
| A2L Lay. 1 | | | GM | 3.60 | 2.605 | 10.5 | 2.195 | 7.5 | 60 | 72 | 191 |
| A2L Lay. 2 | 8 - 0+315 | Shoulder | GW | 2.30 | 2.712 | NP | 2.39 | 5.5 | 78 | 103 | 211 |
| A2L Lay. 3 | | | GM | 3.00 | 2.615 | 9.5 | 2.26 | 7.5 | 67 | 78 | 94 |
| AS1R Lay. 1 | | | GW | 1.40 | 2.69 | NP | 2.39 | 5 | 88 | 112 | 85 |
| AS1R Lay. 2 | 9 - 0+720 | Shoulder | GM | 2.80 | 2.593 | 9.5 | 2.205 | 7.5 | 63 | 87 | 101 |
| AS1R Lay 3 | | | GM | 2.40 | 2.614 | 9.3 | 2.25 | 7.3 | 67 | 87 | 49 |
| AS2L Lay. 1 | | | SW | 3.20 | 2.658 | 4.2 | 2.253 | 6.2 | 60 | 73 | 702 |
| AS2L Lay. 2 | 10 - 0+885 | Subgrade | GM | 3.50 | 2.668 | 8.3 | 2.23 | 7 | 62 | 79 | 97 |
| AS2L Lay. 3 | | | GM | 4.20 | 2.605 | 9.1 | 2.2 | 8.2 | 54 | 72 | 21 |

Table 2.2.2-26 Laboratory Test Result

MDD - Maximum Dry Density

OMC - Optimum Moisture Content

CBR - California Bearing Ratio

From the above result, all CBR values were high, except for one point on the Motorway. In pavement design of AASHTO, an equation to obtain Resilient Modulus (MR) on the subbase is taken as 20% even if the actual value is bigger than that.

6) Result

Pavement structures (calculation results) of main carriageways, ramps and service roads are shown in **Table 2.2.2-27**.

| Pavement Composition (Material) | | Station | Surface Course | Binder Course | Base Course | | Subbase Course | | Total Thickness | Remarks | |
|------------------------------------|--------------------------------|--------------------------------------|-------------------|------------------|--------------------|------------------|-----------------|------|--------------------|---|--|
| | | Station | Asphalt | | Bitumin treated | Crushed Stone | Sandy Gravel | Sand | (mm) | Kentariks | |
| | Motorway (Standard Section) | 00+00 ~ 06+40 | 50 | 100 | - | 250 | 300 | - | 700 | Subgrade replacement t=350mm | |
| (IIII) | High Gr. Water Section | 06+40 ~ 12+00 | 50 | 100 | 270 | - | - | - | 420 | | |
| NOLI | Inside Box | 08+20 ~ 10+10 | 50 | 100 | 170 | - | - | - | 220-700 | Leveling layer 50-400mm, Drainage Layer t=100mm | |
| VEMENT COMPOSITION | Aflao (Standard Section) | 12+00 ~ 17+05 | 50 | 80 | | 200 | 200 | | | | |
| | | $\frac{8+65}{(7+06.090} \sim 14+95$ | 50 | 80 | - | 150 | 200 | - | 480 | Inside parenthesis is for Phase-1 (from box edge) | |
| | Harbour Road | $00+00 \sim \frac{4+65}{(6+70.197)}$ | 50 | 100 | - | 200 | 200 | - | 550 | Inside parenthesis is for Phase-1(to box edge) | |
| | Flyover | 4+65 ~ 8+65 | 80 | - | - | - | - | - | 80 | Including 40mm leveling layer | |
| PA | Motorway-Akosombo | torway-Akosombo | | 60 | - | 200 | 250 | - | 560 | | |
| | Intersection | ntersection | | 50 | - | 200 | 250 | - | 550 | ボックス上の舗装。橋梁同様8cm(表層+レベリング) | |
| | Service Road | | 70 | | - | 100 | 100 | - | 270 | | |

 Table 2.2.2-27 Pavement Structure

(4) Application of Modified Asphalt Concrete

1) Outline

Modified asphalt concrete is asphalt having high fluidity and abrasion produced by mixing straight asphalt with a polymer or rubber. In Japan, its commercial use begun in 1963.

In Ghana, PG 76 and PG 82 have been used recently. PG stands for Performance Grade, which is a standard defined by Superpave (Superior Performance Pavement) and proposed in SHRP (Strategic Highway Research Program). PG 76 means that maximum surface temperature to assure flexibility and stress-relaxation is 76 degrees Centigrade regardless of area or season.

2) Application in the Project

Modified asphalt concrete is planned to be provided on the wearing course at main carriageway and ramps including the intersection. However, this will not be provided on service roads.

3) Justification for Application of Modified Asphalt Concrete

A) Application at Intersection

Since the objective intersection is located on the international corridor, the intersection is expected to see significant rise in the number of heavy vehicles in the future. With the on-going expansion project at Tema Port, it is expected that freight vehicle to and from the port will also increase. The pavements on existing roads are seen to be in a deteriorated state even under the present traffic volume. Typical deteriorations found on the roads in an around Tema Roundabout are shown in **Table 2.2.2-28**. Major causes of these defects are rapid

acceleration/deceleration and curved path of heavy traffic at low speed.

Table 2.2.2-28 Typical Defects

| Alligator Crack | Deformation | Pothole |
|-----------------|-------------|---------|

Also, all the left-turning ramps and through lanes on north-south direction meet at the intersection which is located on top of the box culvert at the depressed section (east-west direction). As such, the pavement here is anticipated to experience distress, which could have adverse effect on the box culvert (top slab).

B) Application on Main Carriageway and Ramps

Due to improvement of the intersection, driving speed on Motorway – Aflao Road will drastically rise (it is high even at present). However, driving speed could occasionally reduce depending on the traffic conditions. Slow movement of vehicles inflicts bigger damages to the pavement surface possibly leading into rutting. Rutting can pose immense danger to vehicles driving at high speeds, especially, since the horizontal alignment of the depressed and underpass has a combination of opposite curves (S-shaped curve). Bad visibility in association with deformations caused by partial load applied due to superelevation could be adverse factors that affect traffic safety. Also, stagnation of rainfall water at rutted portions might also cause severe traffic accidents.

On the other hand, ramps are sections where rapid acceleration/deceleration is repeated and its horizontal curve radius is relatively small. In the curve section, deformation and rutting caused by partial load applied on the surface is likely to occur. Also, traffic congestions on ramps can easily occur because of its smaller width and could potentially become bottle necks. On the main carriageway also, there is the possibility of traffic accidents to occur due to rutting.

4) Availability and Applicability of Modified Asphalt Concrete

A) Production

For the application of modified asphalt concrete in this project, an admixture, which will be procured in Japan and transported to the site, is planned to be used for modifying the asphalt. The hearings conducted with the manufacturers regarding applicability have revealed the followings;

- · Local asphalt plants have experience of producing modified asphalt concrete.
- Local plants apply batch type plant (use of admixture for plant mix type is possible).

The nearest plant is in a distance of around 8kilometers from the project site.

B) Performance

Modified asphalt concrete types commonly used in Japan are classified into Performance Grade 64 (PG64) and PG 70. The figures indicate the maximum surface temperature where performance ability of the material is endorsed. The surface temperatures on the existing roads the Survey Team had measured are shown in **Table 2.2.2-29**. The temperatures measured are all below 64 degrees Centigrade and is favorable for application of modified asphalt concrete.

| | Motorway | | Aflao | Road | Harbou | r Road | Akosombo Road | |
|-------------|--------------------------|------------------------|--------------------------|---------|--------------------------|---------|--------------------------|---------|
| Time | Concrete (Roundabout) | Concrete (Standard) | Concrete (Roundabout) | Asphalt | Concrete (Roundabout) | Asphalt | Concrete (Roundabout) | Asphalt |
| 10:00-10:30 | 46.8°C | 47.4°C | 49.3°C | 49.3°C | 48.8°C | 49.9°C | 48.4°C | 48.2°C |
| 12:00-12:30 | 55.2°C | 55.2°C | 53.1°C | 53.9°C | 54.1°C | 56.1°C | 52.6°C | 56.0°C |
| 14:00-14:30 | 52.4°C | 53.0°C | 53.4°C | 53.2°C | 52.0°C | 56.0°C | 51.0°C | 53.1°C |
| 16:00-16:30 | 47.3°C | 46.3°C | 47.2°C | 48.4°C | 48.6°C | 48.5°C | 45.3°C | 46.4°C |

 Table 2.2.2-29 Surface Temperature Survey Result

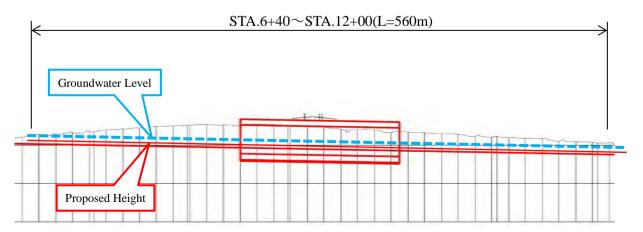
C) Quality Control Test

According to the information gathered from hearings conducted, there are no organizations or companies which are capable of doing a quality control test. Thus, the tests such as wheel tracking test for modified asphalt concrete is conducted in Denmark or South Africa by exporting asphaltic material. Since it is possible to transport the material overseas, the test can be done in Japan also.

5) Countermeasures to Groundwater

6) Outline

Groundwater can have adverse effects on the pavement in the depressed and underpass section. Although the pavement has a less chance of being affected by the groundwater from behind the retaining walls, risk of damages from the water infiltrating into the pavement due to piping phenomenon caused by the head water difference is relatively high. This issue should be properly addressed in order to mitigate or prevent the deterioration of the pavement.





7) Countermeasure Plan

A) Provision of Underground Drainage Pipe

In order to adequately drain the water seaping under the pavement due to pressurization caused by water head difference, underground drainage facility with perforated drain pipes having a diameter of 200mm and surrounded by crushed stones and non-woven fabrics as shown in **Figure 2.2.2-31** shall be provided. The locations to be provided are shown in **Figure 2.2.2-32** and **Figure 2.2.2-33**.

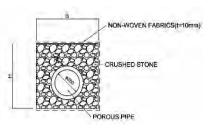


Figure 2.2.2-31 Underground Drainage Pipe

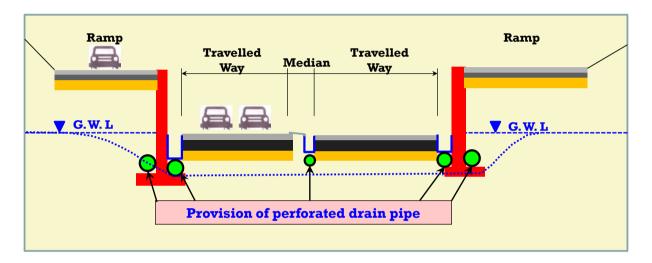


Figure 2.2.2-32 Underground Drainage Pipe Layout in Depressed Section

B) Application of Asphalt Stabilized Material

To prevent damage to the base course of the pavement from piping, asphalt stabilized material having high resistivity and perforated pipes will be applied. The locations where the perforated drain pipes are planned to be provided is shown in **Figure 2.2.2-33** or **Figure 2.2.2-35**.

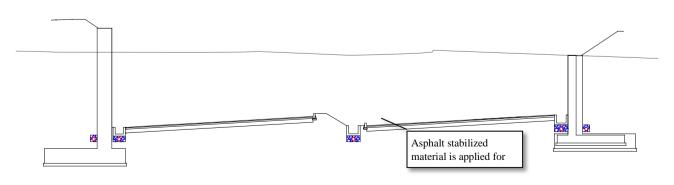


Figure 2.2.2-33 Typical Cross Section in Depressed Section

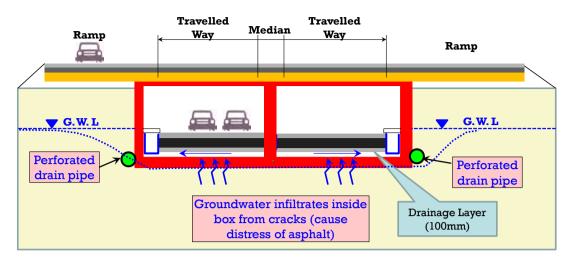


Figure 2.2.2-34 Underground Drainage Pipe Layout in BOX Culvert Section

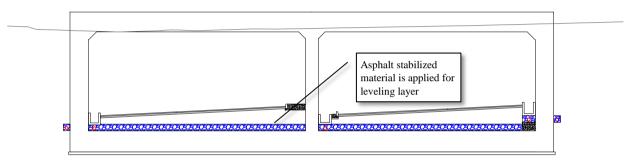


Figure 2.2.2-35 Typical Cross Section in Depressed Section

2.2.2.5 Drainage Plan

(1) **Basic Policies**

The following basic policies are set.

- Planned drainage system shall follow the existing system as much as possible, and drained at existing outlets (river or channel),
- Existing V-shape ditches and other drainage facilities that would be affected by the project will be replaced. Similar type with similar dimension will be provided,
- Catch basins shall be installed at connecting points of U-ditches and between U-ditches and crossdrainage facilities.

(2) Drainage Outlets

Surface water collected is transported and drained out in one of the outlet facilities listed in **Table 2.2.2-30**.

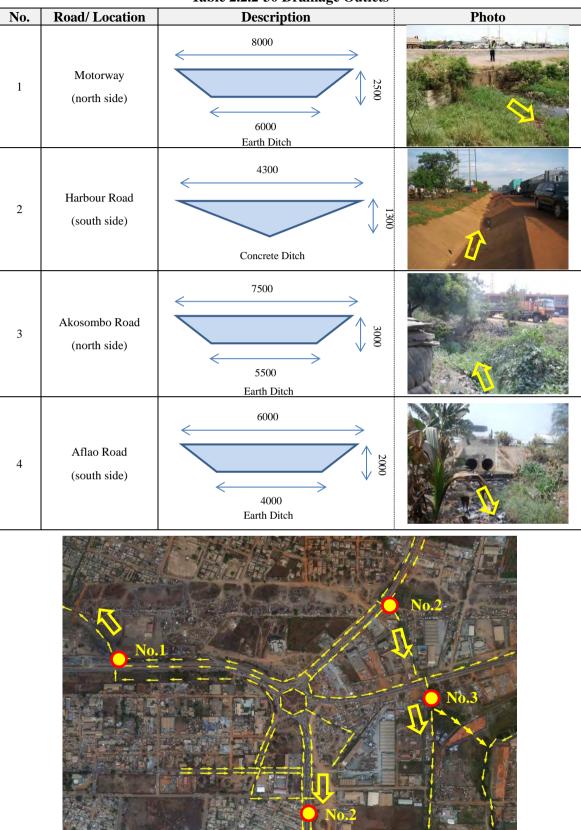


Table 2.2.2-30 Drainage Outlets

(3) Design Condition

1) Applied Parameters

Following parameters shall be applied in accordance with Road Design Guide (GHA, March

1991).

- Rainfall Intensity: 127mm/h
- Frequency of Occurrence: 5 years
- Run-off Coefficient: 0.9
- Coefficient of Roughness of Drainage Facility: 0.015

2) Design Flow

Design flow shall follow the method prescribed in Road Design Guide (GHA, March 1991) in **Figure 2.2.2-36** and **Figure 2.2.2-37**.

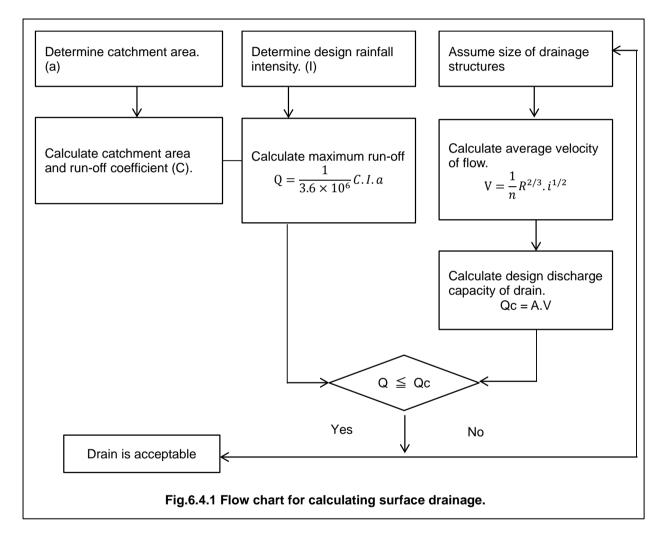


Figure 2.2.2-36 Design Flow

The maximum run-off is obtained from following equation.

$$Q = \frac{1}{3.6 \times 10^{6}} C.I.a$$

or
$$Q = \frac{1}{3.6} C.I.A$$

Q; Maximum run-off from the catchment area (m³/sec).

C; Run-off coefficient (a coefficient which represents ratio of run-off to rainfall).

I; Average rainfall intensity (mm/h).

Figure 2.2.2-37 Calculating Formula of Run-off

3) Drainage System

Figure 2.2.2-38 gives the general layout of the drainage system. Rainwater on the road surface is conveyed to one of the four outputs; one on each leg. The type of drainage facility and its dimension are calculated using the methods explained hereabove. The calculation and the results are attached in Appendix-5.

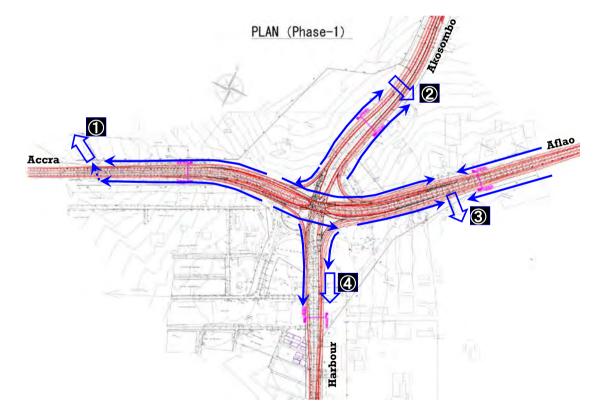


Figure 2.2.2-38 Drainage Plan

2.2.2.6 Street Lightning Plan

(1) **Basic Policies**

Provision of lights along the roads is not included in this project. However, lightning is planned to be

provided inside the box culvert at the underpass. The proposed box culvert section is about 190m long and being positioned in the S-curve segment, provision of street lightning is important to secure safety for traffic within that section.

The layout of the lights was determined following illuminance distribution calculation.

Provision of the facility including electric cables to connect to lead-in pole is covered by the project. However, connection to the grid and power supply to the distribution board would be the responsibility of GHA as agreed upon in T/N.

(2) Selection of Lamp

A list of lamp types is presented in **Table 2.2.2-31**. Use of LED (Light Emitting Diode) is recently common in Ghana. Therefore, although the initial cost is higher compared to other lamps, it is desirable to provide LED for as;

- The efficiency and ability to endurance is higher
- It is stable against temperature fluctuation
- It provides proper light color and color rendering properties
- It provides good visual performance and induction effect against car fumes and fog.

| Type of Lamp | | Color | Color Rendering | Influence Tempera | • | Dimming | Instant Re-Start |
|-------------------------------|--|----------------|--------------------|----------------------|-------|------------------------------------|---------------------|
| | | | Properties | Efficiency | Start | | Ke-Start |
| High-pressure sodium vapor | With incorporated starter | Pale yellow | Fair | No | No | Gradual dimming | No |
| lamp | With mouthpieces | White | Fall | INO | NO | Gradual dimming | Yes |
| | For exclusive used as high-frequency lighting, straight tube lump | White | Good | Yes | Yes | Continuous dimming | Yes |
| Fluorescent lamp | For exclusive use as high-frequency lighting, double tube lamp | White | Good | Yes | Yes | Continuous dimming | Yes |
| | For exclusive use as high-frequency lighting, electrode-less | White | Good | Yes | Yes | Gradual dimming | Yes |
| | For rapid-start lamp | White | Good | Yes | Yes | Continuous dimming | Yes |
| Metal halide lamp | | | Good | No | No | No | No |
| Ceramic metal h | alide lamp | White | Good | No | Yes | * | * |
| Fluorescent mercury lamp | | White | Good | No | No | Gradual dimming | No |
| Low pressure sodium lamp | | Orange | Bad | No | No | No | Yes |
| LED (Light Emitting Diode) | | White | Good | Yes | Yes | Gradual & continuous dimming | Yes |

 Table 2.2.2-31 Type of Lamp and Characteristics

* Metal halide lamp has both "Yes" and "No" in dimming and instant start.

Source: Road Lightning Installation Guidelines and Explanation, Japan Road Association

(3) Design Condition

Design conditions of lights inside the box culvert are shown in Table 2.2.2-32.

| | 8 |
|---|------------------|
| Design Speed | 100km/h |
| Light source of lightning for Snow Shed inside | LED |
| Pavement | Asphalt Concrete |
| Conversion factor of average illuminance | K=18(lx/cd/m2) |
| Road width | W=16.45(m) |
| Maintenance factor | M=0.65 |
| Average illuminance | 20 (lx) or more |

Table 2.2.2-32 Design Conditions

(4) Lamp Layout

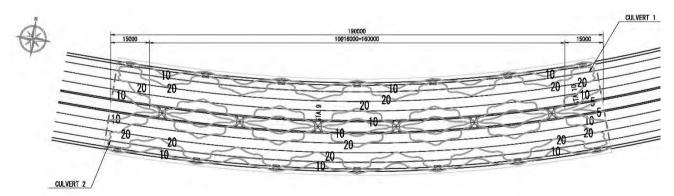
For cost-effectiveness, the lamps will be arranged in a staggered manner, and the lamps shall be installed on top of inner and side wall of the box culvert in 45 degree.

(5) Illuminance Distribution

Illuminance distribution was calculated. The results are shown in Table 2.2.2-33 and Figure 2.2.2-39.

Table 2.2.2-33 Calculation Result

| | CULVERT | 1 | CULVERT 2 | | | |
|------------------------------|-----------|------|-----------|------|--|--|
| AVERAGE ILLUMINANCE (Ix) | 21.2 > 20 | [OK] | 20.6 > 20 | [OK] | | |
| CONVERSION LUMINANCE (cd/m²) | 1. 17 | | 1.14 | | | |





(6) Layout Plan

Figure 2.2.2-40 illustrates the layout plan of the lamps.

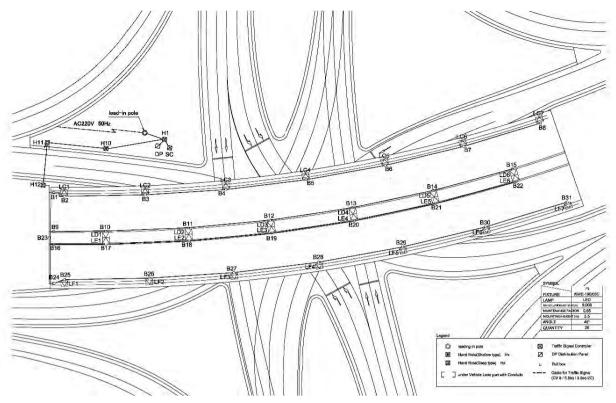


Figure 2.2.2-40 Street Lighting Layout Plan

2.2.2.7 Traffic Safety Facility Plan

(1) **Basic Policy**

Basic policies for planning traffic safety facilities are outlined below.

- To prevent traffic from erroneously approaching a median and for proper visibility of the median to drivers, a reflector shall be installed along the median.
- Reflector shall be installed where drainage exists outside of shoulder.
- Reflector shall be installed at merging and diverging sections of ramp terminal and along curve sections of ramps.
- Crash barrier shall be installed at sections where height differences exceed 3 meters.
- Crash barrier shall be installed at the inner shoulder of box culvert in order to prevent vehicles from crashing against the inner wall.
- Cushion drum shall be installed at diverging points of ramp nose in order for impact relaxation at the time of crashing.
- Guard fence shall be installed in borders between ramps and service roads.
- Traffic sign for attracting attention of a driver shall be installed accordingly.

(2) Applied Traffic Safety Facilities

The facilities that are to be provided along with its image and the sections to be provided are given in **Table 2.2.2-34**.

| No. | Facility | Image | Section to be Provided |
|-----|---------------|-------|---|
| 1 | Reflector | | Both side of Median Border between ramp and service road Merge and diverge section Curve section on ramp |
| 2 | Crash Barrier | | Over 3 meter height difference Inner wall in BOX culvert |
| 3 | Cushion Drum | | • Diverging Point at ramp nose |
| 4 | Guard Fence | | Border between a ramp and a service road |

Table 2.2.2-34 Applied Traffic Safety Facility

| No. | Facility | Image | Section to be Provided |
|-----|--------------|---|------------------------|
| 5 | Traffic Sign | 2 RIGHT BEND 9 CARRIAGEWAY NARROWS 25 TRAFFIC SIGNALS 34 MINOR ROAD FROM RIGHT | • Where appropriate |

2.2.2.8 Considerations for Pedestrians (Pre-improvement Function Recovery)

(1) **Basic Policies**

It is important that the existing facilities/services for use of pedestrian should be recovered/compensated after improvement. In this context, service roads and footbridges are planned to be provided. Basic policies for provision of these facilities are as follows;

- · Pedestrian shall not be allowed to cross the road through the intersection after improvement,
- Service roads to facilitate access to and from abutting properties shall be planned. Service roads shall be provided at locations agreed upon with GHA in the T/N,
- Footbridges shall be provided on all four legs of the intersection to facilitate crossing roads for pedestrians without interrupting the traffics.

(2) Service Roads

1) Design Conditions

Design conditions of service roads are shown in Table 2.2.2-35.

| Item | Applied | | | | | | |
|------------------------|--|--|--|--|--|--|--|
| Design Speed | 40km/h | | | | | | |
| Width of Carriageway | 3.0m | | | | | | |
| Width of Shoulders | 1.5m (Both side) | | | | | | |
| Crossfall | 2.5% | | | | | | |
| Max. Superelevation | 6.0% | | | | | | |
| Max. Vertical Gradient | 6.0% | | | | | | |
| Pavement Structure | Asphalt Concrete (See '2.2.2.4 Pavement Plan' about thickness) | | | | | | |

2) Provision Area

Service roads are planned at areas shown by red lines in **Figure 2.2.2-41**. The area or the scope of service roads was determined with discussions with GHA.

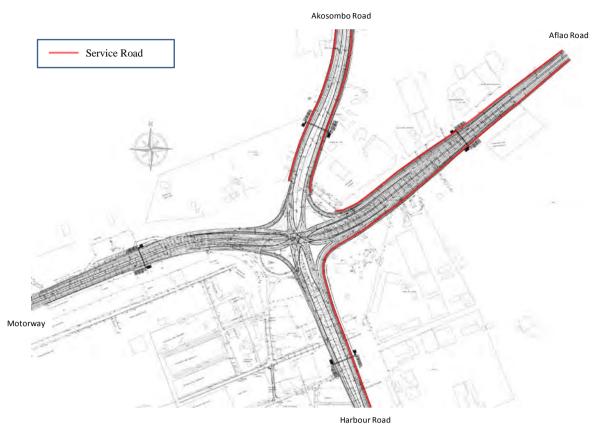


Figure 2.2.2-41 Service Road Plan

(3) Footbridges

1) Application of GHA Standard Drawing

Footbridges shall be designed in reference with the standard drawings of GHA. Footbridges are planned to be provided on all four legs of the intersection. However, the outline design is conducted only for the footbridge at Harbor Road (Sta. 10+40). The design for each footbridge will be carried out during the detailed design stage. The outline of the footbridge is summarized in **Table 2.2.2-36**.

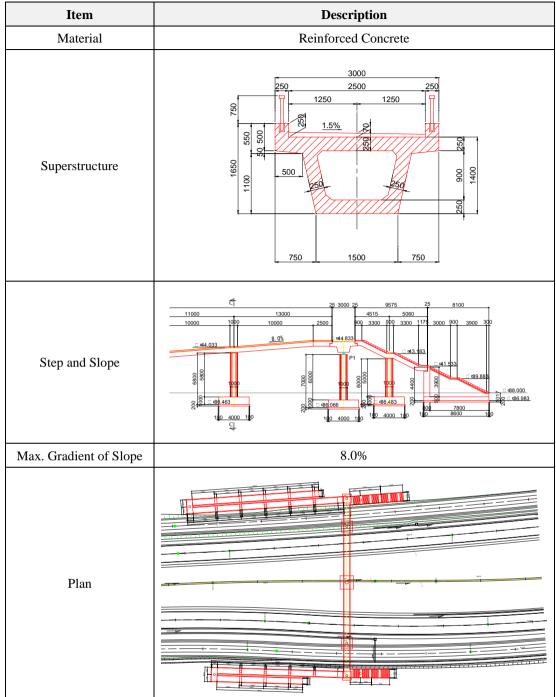


Table 2.2.2-36 Footbridge Outline

2) Locations

Figure 2.2.2-42 shows locations of footbridge to be provided. The design for footbridges on other legs of the intersection will be done individually during the D/D stage.

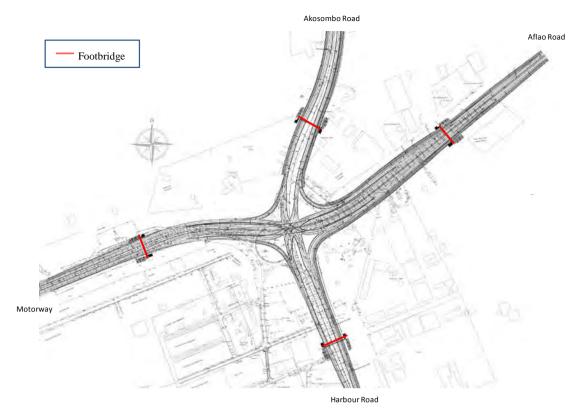


Figure 2.2.2-42 Footbridge Layout

2.2.3 Outline Design Drawing

(1) Plan Overview

Plan overview is shown in Figure 2.2.3-1.



Figure 2.2.3-1 Plan Overview

(2) Outline Design Drawing

Outline Design Drawing is attached in Appendix-6 and its table of contents is shown in **Table 2.2.3-1**. In the drawing, cross section except depressed / underpass section of main carriageway and profile / cross section of ramps are omitted.

| No. | DRAWING TITLE | SHEET NO. | | | No. of Sheets | | | |
|--|---|-----------|---|----|------------------|-----|-----|--|
| 1. | GENERAL | GN | - | 01 | ~ | 04 | 4 | |
| | PROJECT LOCATION MAP | GN | - | 01 | | | 1 | |
| | KEY PLAN | GN | - | 02 | ~ | 04 | 3 | |
| 2. | TYPICAL CROSS SECTIONS | TP | - | 01 | 2 | 04 | 4 | |
| 3. | PLAN | PL | - | 01 | ~ | 18 | 18 | |
| 4 | INTERSECTION PLAN | IP | - | 01 | | | 1 | |
| 5 | PROFILE | PR | - | 01 | 2 | 24 | 24 | |
| 6 | CROSS SECTION | CS | - | 01 | ~ | 105 | 105 | |
| 7 | OUTLINE OF STRUCTURES ALONG MOTORWAY-AFLAO ROAD (UNDERPASS) | US | - | 01 | ~ | 07 | 7 | |
| | GENERAL DRAWINGS | US | - | 01 | ~ | 03 | 3 | |
| | STRUCTURE OF CULVERT | US | - | 04 | ~ | 05 | 2 | |
| | STRUCTURE OF RETAINING WALLS | US | - | 06 | ~ | 07 | 2 | |
| 8 | OUTLINE OF PEDESTRIAN BRIDGES | PB | - | 01 | ~ | 3 | 3 | |
| 9. | OUTLINE OF PAVEMENT STRUCTURE | PS | - | 01 | | | 1 | |
| 10 | DRAINAGE PLAN | DP | - | 01 | ~ | 18 | 18 | |
| 11 | OUTLINE OF DRAINAGE STRUCTURES | DR | - | 01 | ~ | 04 | 4 | |
| | SIDE DITCH | DR | - | 01 | | | 1 | |
| | CROSS DRAINAGE | DR | - | 02 | | | 1 | |
| | CATCH BASIN AND UNDERGROUND DRAINAGE | DR | - | 03 | | | 1 | |
| | DISTRIBUTION WATERWAY | DR | - | 04 | | | 1 | |
| 12 | OUTLINE OF ROAD ANCILLARIES | RA | - | 01 | ~ | 36 | 38 | |
| | ANCILLARY PLAN | RA | - | 01 | ~ | 18 | 18 | |
| | LAYOUT OF REFLECTORS | RA | - | 19 | ~ | 25 | 7 | |
| | LAYOUT OF TRAFFIC SIGNS | RA | - | 26 | ~ | 32 | 7 | |
| | MEDIAN BLOCK, KERB AND EDGE BLOCK (FLUSH KERB) | RA | - | 33 | | | 1 | |
| | CRASH BARRIER AND REFLECCTORS | RA | - | 34 | | | 1 | |
| | CROSS PREVENTION FENCE | RA | - | 35 | | | 1 | |
| | TYPICAL TRAFFIC SIGNS | RA | - | 36 | | | 1 | |
| | PAVEMENT MARKINGS | RA | - | 37 | > | 38 | 2 | |
| 13 | TRAFFIC SAFETY FACILITIES | TS | - | 01 | ~ | 13 | 10 | |
| | LAYOUT OF TRAFFIC SIGNAL | TS | - | 01 | | | 1 | |
| | WIRING SYSTEM OF TRAFFIC SIGNAL | TS | - | 02 | | | 1 | |
| | DETAIL OF TRAFFIC SIGNAL | TS | - | 03 | ~ | 06 | 4 | |
| | LAYOUT OF STREET LIGHT (UNDERPASS) | TS | - | 07 | | | 1 | |
| | WIRING SYSTEM OF STREET LIGHT (UNDERPASS) | TS | - | 08 | | | 1 | |
| | DETAIL OF STREET LIGHT (UNDERPASS) | TS | - | 09 | | | 1 | |
| DETAIL OF CABLES AND HAND HOLE TS - 10 | | | | | | | | |
| | Subtotal (number of sheets) | | | | | | | |

| | < REFERENCE DRAWINGS > | | | | | | |
|----|--|----|---|----|---|----|-----|
| 1. | GENERAL DRAWING OF FLYOVER SECTION (PHASE-2) | RF | - | 01 | ~ | 03 | 3 |
| 2. | 2. INTERSECTION PLAN (PHASE-2) | | | | | 05 | 2 |
| 3. | 3. TEMPORARY DETOUR DURING CONSTRUCTION RF - 06 ~ 08 | | | | | | 3 |
| | Total number of sheets | | | | | | 245 |

2.2.4 Implementation Plan

2.2.4.1 Implementation Policies

The basic policies for implementation of the project are as follows:

- This project will be implemented under the Grant Aid Scheme of the Government of Japan (GOJ) in accordance with the Grant Agreement (G/A) and the Exchange of Notes (E/N) between the Republic of Ghana and the GOJ.
- The executing agency for the implementation of the project is the Ghana Highway Authority (GHA) of the Republic of Ghana.
- The consulting services including detailed design, tender-related works and construction supervision services, will be provided by a Japanese consulting firm in accordance with the consultancy contract that shall be signed with the Republic of Ghana.
- The construction works will be executed by a Japanese construction firm that shall be selected through pre-qualification and bidding, in accordance with the construction work contract that shall be signed between the said construction firm and the Republic of Ghana.
- The basic policies for the construction/procurement of this project are as follows:
 - The equipment, materials and labor for construction shall be, to the possible extent, procured locally. In cases where local procurement is not possible, they shall be procured either from a third country or from Japan where it is most economical provided the required quality and supply quantity are secured.
 - Construction methods and the construction processes shall be adequately determined taking the local climate, topography, geology and natural conditions including the characteristics of nearby rivers into consideration.
 - The contractor's site organization shall be planned to satisfy the established construction specifications and construction management standards set for this project. Likewise, the consultant's organization shall be based on such specified project management standards.
 - ➤ To ensure safety during construction, an appropriate traffic management plan including deployment of traffic personnel at vantage positions shall be considered.
 - In order to reduce the influence of construction works on the environment of the project site, appropriate/necessary environmental preservation measures, such as the selection of temporary garbage dumping sites specified by the Republic of Ghana shall be adopted.

2.2.4.2 Implementation Condition

(1) Points to Be Considered during Implementation

Approximately 23,000 to 32,000 vehicles ply the objective intersection daily and is therefore not reasonable to block the roads during construction. Therefore, construction methods which does not hinder existing traffic operations and in addition ensure the security of the traffic operations must be adopted.

(2) Detour Plan during Construction

1) Basic Policy

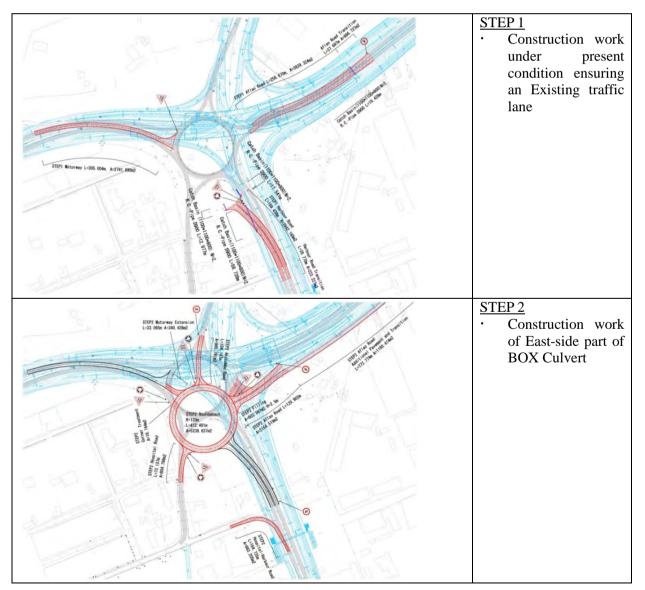
Basic policies are shown below.

- Number of carriage lanes and lane widths of the detour shall be equivalent to the number of the existing traffic lanes,
- · Size of temporary roundabout shall be equivalent to the size of the existing roundabout,
- Volume of rework shall be minimized by utilizing the permanent (completed) roads as the temporary detour,
- Detours shall be planned by considering soil volume balance in addition to above policies.

2) Conditions

- Design speed of the detours shall be 40km/h,
- · All geometric conditions (alignment requirements) will comply for the above design speed,

3) Detour Plan and Step-wise Construction



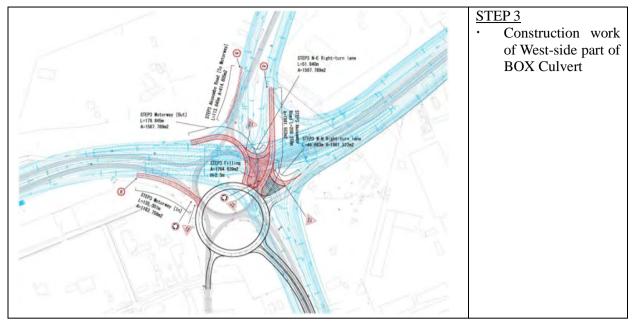


Figure 2.2.4-1 Detour Plan

2.2.4.3 Scope of Works

The responsibilities to be borne by Japan and the Republic of Ghana are summarized in Table 2.2.4-1.

| Items | Contents/Requirements | Responsible Country | | Remarks | | |
|---|---|------------------------|-------|---|--|--|
| | | Japan | Ghana | | | |
| Land acquisition | Procurement before construction | | 0 | | | |
| Weigh station | Relocation before construction | | 0 | | | |
| Electric power for traffic signal and street lighting | Power supply | | 0 | | | |
| Procurement | Materials and equipment | | 0 | Specific material will be procured from Japan. | | |
| | Customs clearance | 0 | 0 | | | |
| Land provision | Land Acquisition necessary for construction | | 0 | Project Office, Accommodation Construction Yard | | |
| | Other than above | 0 | | | | |
| Removal/Relocation of utilities | Relocation of Obstruction | | 0 | Water pipe, electric cable, communication cable, sign board | | |
| Main Construction | Road, box culvert, retaining wall and traffic safety facility | 0 | | | | |

 Table 2.2.4-1 Responsibility of Each Government

2.2.4.4 Procurement and Construction Supervision

Basically, the government of Ghana will select a Japanese Consultant for assisting the executing agency in the contractor bidding process and for rendering construction supervision service of the project.

(1) Detailed Design

The major works to be carried out by the Consultant during the detailed design stage include the followings:

• Undertake consultations with concerned authorities of Ghana and carry out field surveys,

- · Carry out detailed design and prepare design drawings
- Estimate project cost

The duration of the detailed design is about 5 months.

(2) Bidding Process

The major tasks to be undertaken between the time of inviting contractors to bid and the time for signing of contract for construction includes:

- Preparation of bid documents (in parallel with the detailed design).
- · Bid announcement
- Pre-qualification of bidders
- Bidding
- Evaluation of bid documents
- Preparation of Contract Agreement

The duration of the bid-related activities is about 4 months.

(3) Construction Supervision

The Consultant will supervise the Contractor's planning and implementation of the construction contract. The major tasks under this stage include:

- · Verification/Approval of related surveys and quantities
- Review/Approval construction plans
- Quality Control
- Process Control
- Work Output Control
- Safety Management
- Turnover Inspection and Acceptance

The duration of construction supervision is approximately 28 months. The construction supervision team shall consist of: 1-Resident/Chief Engineer (Japanese), 1-Site Inspectors (Local), 1-Clerk of Works (Local) and 1-Utility Personnel (Local). A construction supervision engineer would be dispatched at the time of construction of structural components and asphalt pavement.

A safety control officer is necessary to supervise, direct and cooperate with a contractor's safety manager so that occurrence of an accident may be prevented.

2.2.4.5 Quality Control Plan

The tasks to be carried out for quality control during the construction period are as follows:

- Concrete Works
- Reinforcing Bars and Formworks
- Earthwork
- Pavement Works

Based on the above, the quality control of main items for concrete works is presented in **Table 2.2.4-2** while the quality control of main items for pavement is presented in **Table 2.2.4-3**.

| Item | Test Items | Test Method | Test Frequency | | | | |
|-----------|---|---------------|--|--|--|--|--|
| Concrete | Cement Property/Physical Test | AASHTO M85 | Once before trial mix and once every 500m ³ batch of concrete; or once during production of cement (Mill sheet) | | | | |
| | Property/Physical Test | AASHTO M6 | Once before trial mix and once every 500m ³ batch of concrete; and every change of source/quarry location (check supplier data) | | | | |
| Aggregate | Property/Physical Test | AASHTO M80 | Once before trial mix and once every 500m ³ batch of concrete; and every change of source/quarry location (check supplier data) | | | | |
| | Sieve Analysis | AASHTO T27 | Once a month | | | | |
| | Alkali-silica Reactive Test(Mortar Bar Method) | ASTM C1260 | Once before trial mix and every change of source/quarry location (check supplier data) | | | | |
| | Mineral Composition Test | ASTM C295 | Once before trial mix and every change of source/quarry location (check supplier data) | | | | |
| Water | Water Quality Test | AASHTO T26 | Once before trial mix and when necessary | | | | |
| ADMIXTURE | Quality Test | ASTM C494 | Once before trial mix and when necessary (Mill Sheet) | | | | |
| Concrete | Slump Test | AASHTO T119 | Once every 75m ³ or per batch | | | | |
| | Air Content Test | AASHTO T121 | Once every 75m ³ or per batch | | | | |
| | Compressive Strength Test | AASHTO T22 | 6 Samples per batch or 6 samples for every 75m ³ concrete (3 samples each for 7-day strength and 28-d strength) | | | | |
| | Temperature | ASTM C1064 | Once every 75m ³ or per batch | | | | |

 Table 2.2.4-2 Quality Control Plan of Concrete Works

Table 2.2.4-3 Quality Management Plan for Earthwork and Pavement Work

| Item | Test Items | Test Method | Test Frequency |
|-------------|--------------------|-------------|--|
| Embankment | Density Test | AASHTO T191 | Every 500m ² |
| Lindankinem | (Compaction) | | Every soom |
| | Material Test | AASHTO T27 | Once before placing and once every 1,500m3 or |
| | (Sieve Analysis) | AASHIO 127 | change in source/quarry location. |
| | Material Test | AASHTO T193 | Once before placing and once every 1,500m3 or |
| Dava sama | (CBR Test) | AASHIO 1195 | change in source/quarry location. |
| Base course | Dry Density Test | AASHTO T180 | Once before placing and twice every 1,500m3 or |
| | (Compaction) | AASHIU 1180 | change in source/quarry location. |
| | Field Density Test | | From 500m ² |
| | (Compaction) | AASHTO T191 | Every 500m ² |
| Asphalt | Material Test | AASHTO | Once before placing and once every 1,500m3 or |
| paving | (Sieve Analysis) | M43,M80 | change in source/quarry location. |

| Item | Test Items | Test Method | Test Frequency | | | |
|------------------|--|---------------------------|---|--|--|--|
| | Material testing (density and percentage of absorption). | AASHTO T84 | | | | |
| | Density-in-situ examination. | AASHTO T209 | Every 200m | | | |
| | Temperature survey | | Every track | | | |
| Modified asphalt | Marshall stability test | ASTM D 1559-89 | Design stage: Five samples every a mix, three pieces = 15 times | | | |
| paving | | | Trial mix stage: : Three samples every a mix, three pieces= 9 times | | | |
| | | | Paving stage: Once before placing | | | |
| | Dynamic Stability Test | Plastic | At Trial Mix: Once per 1 mix At Construction : Once per paving asphalt of 1,000 ton | | | |
| | | Wheel Tracking Machine | | | | |
| | Other test | JIS | as may be necessary | | | |

2.2.4.6 Procurement Plan

The major construction materials and equipment to be procured, based on field research for procurement, are mentioned below.

(1) **Procurement of Major Construction Materials**

1) Procurement of Ready-mixed Concrete

Ready-mixed Concrete for the retaining wall and box culvert shall be procured from the local concrete plants (85m3 per hour) which are located in a distance of 19 km from construction site. shows the plant in production.

2) Procurement of Base and Subbase Materials

Base and subbase materials shall be procured from quarrying plant located at Shai Hill on Akosombo road.

Photo 2.2.4-2 shows the plant in production.



Photo 2.2.4-1 Concrete Plant in Production



Photo 2.2.4-2 Quarry Plant in Production

3) Procurement Area of Major Construction Materials

Procurement area of major construction materials are summarized into Table 2.2.4-4.

| Iter | Pro | curement A | Area | Procurement | Procurement Routes | |
|-----------------------------|-------------------------------|------------|-----------------|--------------|-----------------------|------------------------|
| Item Name | Description | Local | Third County | Item Name | Reason Description | Local |
| Materials for structures | | | | | | |
| Reinforcing bars | D13~D32 | 0 | | | | Near construction site |
| Aggregate for concrete | 20~25N/mm2 | 0 | | | | Near construction site |
| Macadam | 150 ~ 200mm | 0 | | | | Shai Hill |
| Base | Graded grain crushed stone | 0 | | | | Shai Hill |
| Subbase | Crushed stone | 0 | | | | Shai Hill |
| Embankment | | 0 | | | | Near construction site |
| Bitumen | | 0 | | | | Near construction site |
| Temporary Materials | | | | | | |
| Gasoline, Diesel oil | | 0 | | | | Near construction site |
| Wooden form | | 0 | | | | Accra |
| Plywood | | 0 | | | | Accra |
| Temporary Steel | H-steel | 0 | | | | Near construction site |
| Support | Supported frame | 0 | | | | Accra |

4) Procurement of Special Materials

Special materials which can not be procured in Ghana include an admixture for modified asphalt concrete, joint material for approach cushion slab and waterproofing materials. These materials shall be procured in Japan.

(2) Procurement of Major Construction Equipment

Photo 2.2.4-3 shows major construction equipment which can be procured from local contractors and leasing company.



Concrete pump vehicle

Concrete mixer vehicle



Bulldozer

Backhoe & road roller



Backhoe
Photo 2.2.4-3 Major Construction Equipment to be Procured

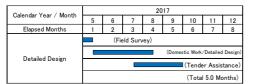
| Construction equipment to be procured is summarized in Table 2.2.4-5. |
|---|
|---|

| | | | | rement | Procurement | | |
|------------------|--------------------|-----------|------------------|-----------|-------------|--------------|------------------|
| Iten | n | | | Reason | | Routes | Item |
| Item Name | Description | Area/Rent | Local (Ghana) | Item Name | | Rent/ Buy | Local (Ghana) |
| Backhoe | 0.45m ³ | Rent | 0 | | | | Accra |
| Backhoe | 0.8m ³ | Rent | 0 | | | | Accra |
| Backhoe | 1.4m ³ | Rent | 0 | | | | Accra |
| Dump Track | 10t | Rent | 0 | | | | Accra |
| Dump Track | 4t | Rent | 0 | | | | Accra |
| Bulldozer | 21t | Rent | 0 | | | | Accra |
| Bulldozer | 15t | Rent | 0 | | | | Accra |
| Tire Roller | 8~20t | Rent | 0 | | | | Accra |
| Road Roller | 10 ~ 12t | Rent | 0 | | | | Accra |
| Motor Grader | W=3.1m | Rent | 0 | | | | Accra |
| Truck with Crane | 16~25t | Rent | 0 | | | | Accra |
| Concrete Breaker | 600 ~ 800kg | Rent | 0 | | | | Accra |
| Vibrator Roller | Riding Type 3~4t | Rent | 0 | | | | Accra |
| Water Pomp | φ100mm, 15kw | Rent | 0 | | | | Accra |
| Diesel Generator | 22KVA | Rent | 0 | | | | Accra |

2.2.4.7 Implementation Schedule

 Table 2.2.4-6 presents the implementation schedule of detail design and construction period.

Table 2.2.4-6 Implementation Schedule



| | | | | Calendar Year / | | | | | | 2 | 018 | | | | | | 1 | | | | | 20 |)19 | | | | | | 1 | 20 | 020 | 1 | |
|------|-------------------------------------|--|----------|-----------------|---|---|---|---|---|---|-----|---|---|----|----|----|----|----|----|----|----|----|-----|----|----|----|----|----|----|----|-----|----|---------|
| | | Item | | Month | 1 | 2 | 3 | 4 | 5 | | | 8 | 9 | 10 | 11 | 12 | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 | 11 | 12 | 1 | | 3 | 4 | Remarks |
| | | | | Elapsed Months | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 | 11 | 12 | 13 | 14 | 15 | 16 | 17 | 18 | 19 | 20 | 21 | 22 | 23 | 24 | 25 | 26 | 27 | 28 | |
| | Type of Work | Specification | Unit | Quantity | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| No. | Preparation | | | | | | ţ | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | Temporary work | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | Detour | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | Ramp | Excluding intersection | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | Service road | 4 routes | m | 3,318 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | Retaining wall No.2, left side | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| - | Gravitiy type retaining wall | Concrete volume | m3 | 58 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| STEP | Inverted T-type retaining wall | Concrete volume | m3 | 1,413 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| IS | Retaining wall No.2, right side | | | | | | | | 1 | | | 1 | | | 1 | | 1 | | | | | | | | | | | | | | | | - |
| 1 | Gravitiy type retaining wall | Concrete volume | m3 | 54 | | | | | 1 | | | | | | | 1 | 1 | | | | | | | | 1 | | | 1 | 1 | | | | |
| | inverted T-type retaining wall | Concrete volume | m3 | 664 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 1 | Retaining wall No.1, left side | | | | | | | | | | 1 | | | | l | | 1 | | | | 1 | | l | | 1 | 1 | | 1 | 1 | 1 | | | |
| | Inverted T-type retaining wall | Concrete volume | m3 | 730 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | Temporary roundabout, detuor | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 1 | BOX culvert on the east side | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| STEP | BOX culvert | Concrete volume | m3 | 18,268 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| IS | Approach cushion slab, back filling | Concrete volume | m3 | 522 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | Pavement for Aflao Road | Area of surface | m2 | 26,131 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | Detour | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | Retaining wall No.1, right side | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | inverted T-type retaining wall | Concrete volume | m3 | 1,021 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| EP 3 | BOX culvert on the west side | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| STEP | BOX culvert | Concrete volume | m3 | 9,061 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 1 | Approach cushion slab, back filling | Concrete volume | m3 | 153 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | Pavement for Akosombo Road | Area of surface | m2 | 10,898 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | Pavement for Motorway | Area of surface | m2 | 26,131 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | Pavement for Intersection | Area of surface | m2 | 3,916 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 1 | Pavement for Harbour Road | Area of surface | m2 | 8,543 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 4 | Traffic safety facility | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| STEP | Street lighting | Iluminat | Nos | 26 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| s | Trattic signat | Poles | Nos | 6 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 1 | Traffic sign | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | Ramp | Intersection | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | Footbridge | 4 Span Continuous RC Box Girder Bridge | Location | 4 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | Miscellaneous work | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |

2.3 Obligations of Recipient Country

The undertakings required from the GOG for the smooth execution of this project are as follows:

- To provide documents, data and information necessary for the execution of this project;
- To acquire land for construction yard;
- To secure land for construction yard, stock yard, disposal area for construction debris, site office yard, and detour routes;
- To secure borrow pits, spoil-banks, and industrial waste disposal areas;
- To obtain all necessary permits, to coordinate and share necessary information with concerned organizations regarding the method of road occupancy of the Motorway, procedure for allowing public vehicles, traffic restrictions, and day-time, night time works;
- To get the public informed and take necessary steps before hand regarding blockage of road for public vehicles during relocation of overhead facilities such as traffic signs;
- To coordinate with concerned organizations and agencies in charge of underground utilities pertaining to its protection, reinforcement / repair and to pre inform all road users and local inhabitants in case disruption of water and electricity are anticipated;
- To coordinate with concerned organizations and agencies in charge of street lights and electronic traffic signs regarding its protection and/or its relocation and to take necessary steps to inform road users beforehand in case disruption of electricity is anticipated;
- To obtain necessary permits that would allow the personnels engaged in the construction work such as the supervision engineer, construction workers etc. to access ROW;
- To obtain necessary permits that would allow the construction vehicles and equipment to enter and exit the ROW;
- To bear the cost of bank charges such as the Advising Commission and Payment Commission to the Japanese bank where an account related to the project is opened.
- To bear the value-added-tax related to the project;
- To bear all expenses required for 15 % of VAT (Value Added Tax)
- To bear all expenses required for 2.5 % of NHI (National Health Insurance)
- To assist in the process for exemption of materials imported for the construction work from taxation and Customs clearance in order to ensure smooth inland transportation;
- To assist in the process for exemption of Japanese nationals engaged in the construction work from Customs duties and other fiscal levies on products and services necessary for the execution of the project;
- To assist in the process for exemption of Japanese nationals from all legislation measures necessary for entering and staying in the Republic of Ghana;
- To ensure proper use and maintenance of the road after its construction;
- To cooperate in solving potential troubles with the local people or any third party in connection with the execution of the project;
- To bear all expenses required for the execution of the project, other than those borne by the Grant-Aid of Japan.

2.4 Project Operation Plan

2.4.1 Organization for Operation and Maintenance

2.4.1.1 Organizational Framework

Roads in Ghana are administered by the Ministry of Roads and Transport (MRT) through three agencies namely- Ghana Highway Authority (GHA), Department of Feeder Roads (DFR) and the Department of Urban Roads (DUR).

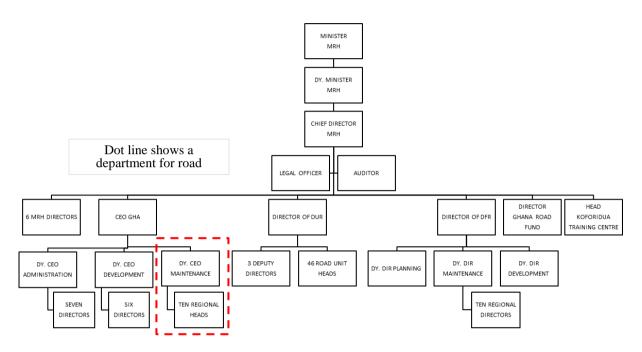


Figure 2.4.1-1 Organizational Framework

As shown in **Figure 2.4.1-1**, Maintenance Department which has 10 regional heads nationwide is responsible for road maintenance.

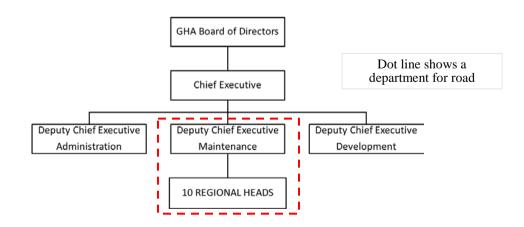


Figure 2.4.1-2 Department in Charge of Maintenance in GHA

| Upper East Region Upper West Region Northern Region Brong Ahofo Region Ashanti Region Eastern Region Central Region Western Region Great Accra Region Volta Region | Table 2.4 | 1.1-1 Regionals Heads of Maintenance Department |
|---|-----------|--|
| Ashanti Region Eastern Region Central Region Western Region Great Accra Region | | Upper East Region |
| Ashanti Region Eastern Region Central Region Western Region Great Accra Region | or ern | Upper West Region |
| Ashanti Region Eastern Region Central Region Western Region Great Accra Region | ecto | Northern Region |
| Ashanti Region Eastern Region Central Region Western Region Great Accra Region | ° No | Brong Ahofo Region |
| لق Central Region Western Region Great Accra Region | | Ashanti Region |
| Central Region Western Region Great Accra Region Volta Region | | Eastern Region |
| Western Region Great Accra Region Volta Region | or ern | Central Region |
| ි ඊ ඊ Great Accra Region Volta Region | ecto | Western Region |
| Volta Region | Š Š | Great Accra Region |
| | | Volta Region |

Regional Head s are operated as shown in Figure 2.4.1-3.

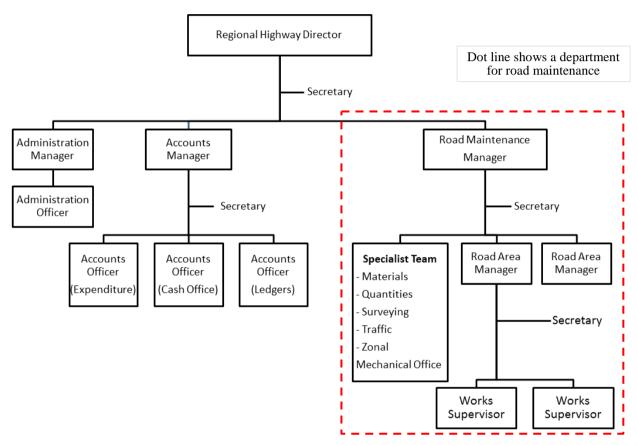


Figure 2.4.1-3 Organization of Regional Head

For purposes of maintenance in the regions, each region is divided into GHA Road Areas, the number and size of each depending on the number and geographical spread of projects in the region.

Road Areas shall have the three primary responsibilities of

- Collecting road data,
- Inspecting roads and •
- Supervising road maintenance

| Region | No. of Areas | Covered Areas |
|-------------------------|--------------|-----------------------------------|
| Ashanti Region | 3 | Kumasi, Mampong, Bekwai |
| Eastern Region | 4 | Koforidua, Oda, Nsawam, Nkawkaw |
| Volta Region | 3 | Ho, Hohoe, Keta |
| Central Region | 3 | Cape Coast, Dunkwa, Winneba |
| Western Region | 3 | Takoradi, Tarkwa, Wiawso |
| Greater Accra Region | 1 | Accra |
| Brong-Ahafo Region | 4 | Sunyani, Goaso, Kintampo, Atebubu |
| Northern Region | 4 | Tamale, Yendi, Gambaga, Saula |
| Upper East Region | 1 | Bolgatanga |
| Upper West Region | 2 | Wa, Tumu |
| Total No. of Road Areas | 28 | |

 Table 2.4.1-2 GHA Road Areas

Each Region has a Special Team made up of personnel with specialization in materials, topographic surveying, traffic and quantity surveying It can be deduced that this team is a pool of expertise. Road maintenance for this project is responsible for Greater Accra Region.

2.4.1.2 Road Sector Personnel in GHA

Table 2.4.1-2 shows road sector personnel in GHA by Staff Category as of 2015.

| Staff Category | Male | Female | Under 30 | 30-40 | 40-50 | 50-60 | Over 60 | Total |
|--------------------|------|--------|----------|-------|-------|-------|------------|-------|
| Directors | 30 | 2 | - | - | 7 | 25 | - | 32 |
| Engineers | 139 | 16 | 41 | 37 | 30 | 47 | - | 155 |
| Quantity Surveyors | 22 | 2 | 1 | 8 | 6 | 9 | - | 24 |
| Economists | 1 | 1 | - | 1 | - | 1 | - | 2 |
| Technicians | 155 | 11 | 18 | 48 | 17 | 83 | - | 166 |
| Planners (Valuers) | 4 | - | - | 2 | - | 2 | - | 4 |
| Accountants | 115 | 35 | 2 | 15 | 15 | 118 | - | 150 |
| Administrators | 28 | 69 | 7 | 10 | 11 | 69 | - | 97 |
| Drivers | 206 | 0 | 5 | 48 | 42 | 111 | - | 206 |
| Others | 801 | 215 | 173 | 182 | 128 | 533 | - | 1016 |
| Total by Age | 1501 | 351 | 247 | 351 | 256 | 998 | - | 1852 |

Table 2.4.1-3 Road Sector Personnel in GHA

2.4.2 Contents of Operation and Maintenance

Road maintenance work conducted by GHA is shown below. Most of the works are done by subcontracting.

- Pavement repair
- Road structure repair
- Cleaning on pavement surface and drainage
- Planting
- Traffic signal and street lighting
- Bridge inspection and repair

2.4.3 Points to Consider in Road Operation and Maintenance

To realize full benefits of the project and its facilities, and to sustain its operation and keep it in good

driving condition, it is important to improve its durability. In addition, the objective road is as the full access controlled motorway hence it is of sgreat ignificance to secure the highway characteristics which matter most to the driver which includes speed of travel, safety, comfort, and convenience, and also implementing facility maintenance related to road functional management, traffic management and safety management. The following needs should be noted:

Inspect the facility regularly to abreast with its condition at all times

Regular cleaning of road and incidental road facilities - especially drainage system

Implementing regular facility inspection, cleaning and maintenance related to road functional management, traffic management and safety management

Secure budget necessary for maintenance

With that in mind, it is important to allocate budget for operation and maintenance.

2.5 **Project Cost Estimate**

2.5.1 Initial Cost Estimation

2.5.1.1 Japan's Contribution

The cost borne by the Japan's Grant Aid is not shown in this report due to the confidentiality.

2.5.1.2 Ghana's Contribution

The cost to be borne by the Government of Republic of Ghana is estimated to be about 6.6 million Ghana Cedi (213 million Japanese Yen). Breakdown is presented in **Table 2.5.1-1**.

| | | 9 | | |
|-----------------|------|-------------------|------------|--------------------------------|
| | Ite | m | Cost (GHC) | Cost (million Japanese Yen) |
| (1) Bank Char | rges | | 392,374 | 12.7 |
| (2) | Land | Land acquisition | 2,050,000 | 66.1 |
| acquisition, | | Street lightning/ | 70,000 | 2.2 |
| Removal | / | electric poles | | |
| Relocation | of | Underground | 560,000 | 18.1 |
| Utilities, etc. | | facilities | | |
| | | Buildings, houses | 3,140,000 | 101.3 |
| | | and shops | , , | |
| | | Trees | 40,000 | 1.3 |
| | | Bill board | 350,000 | 11.3 |
| | | Sub total | 6,210,000 | 200.3 |
| | То | tal | 6,602,374 | 213.0 |

2.5.1.3 Cost Estimation Condition

| Cost Estimation Date Foreign Exchange Rate | : : | December 2015 GHC1.00=32.26 JPY US\$1.00=121.93 JPY |
|---|--------|---|
| Construction Period | : | Schedule of construction supervision is shown in the implementation schedule |
| Others | : | The project is carried out based on the Japanese Government's Grant Aid Scheme. |

2.5.2 Operation and Maintenance Cost

Table 2.5.2-1 represents operation and maintenance cost for the major works. Since operation and maintenance works are conducted by subcontracting, there are no technical problems on maintenance work.

| Category | Facility Name | Items to be checked | Frequency | Personnel | Equipment | Total Number | Cost (1,000 GHC) |
|------------------------|---------------------|--------------------------------|--------------|-----------|----------------------------|------------------|------------------------|
| | Pavement Crack etc. | Crack etc. | | | Scoop/Hammer/ | 40 persons/year | 2.66 |
| | Drainage | Sediment | | | Sickle/Barricade | | |
| | - | Deposition/Obstacle | | | | | |
| | BOX | Damage/Deformation/Peeling | | 2 persons | | | |
| Periodic Inspection | Culvert / | etc. | 4 times/year | | | | |
| | Retaining | | 5 days/time | | | | |
| | Wall | | | | | | |
| | Traffic | Reflector, Crash Barrier, etc. | | | Pickup | 20 vehicles/year | 12.58 |
| | Safety | | | | | | |
| | Facility | | | | | | |
| | Subtotal | | | | | | 15.24 |
| | Pavement | Cleaning | | | Scoop/Barricade | 200 persons/year | 24.09 |
| Routine | Drainage | Removal of Obstacle or | 4 times/year | 10 | | | |
| Maintenance | | Sediment | 5 days/time | persons | | | |
| Maintenance | Footbridge | Cleaning | | | Small Truck | 20 vehicles/year | 12.58 |
| | Subtotal | | | | | | 36.68 |
| | Pavement | Crack etc. | | | Worker | 84 persons/year | 3.44 |
| | Drainage | Damaged part etc. | | | Plate Compactor | 14 vehicles/year | 10.15 |
| | BOX | Damaged part etc. | | | Small Truck | 14 vehicles/year | 9.45 |
| | Culvert / | | | | | | |
| | Retaining | | | | | | |
| | Wall | | | | | | |
| Repair | Traffic | Damaged part etc. | | | | 50.0m3/ year | 3.7 |
| | Safety | | | | Roadbed Material | | |
| | Facility | | 2 times/year | | | | |
| | Median | Crack etc. | 7 days/time | 6 persons | Asphalt | 10.0t/ year | 7.52 |
| | Reflector | Crack etc. | 7 days/time | | Cement | 130 bags/ year | 4.72 |
| | Street | Exchange of Lamp | | | Cobbled Stone | 3.0m3/ year | 0.23 |
| | Lighting | | | | Cobbled Stolle | 5.0m5/ year | |
| | Footbridge | Damaged part etc. | | | Lane Marking | 50m/ year | 0.35 |
| | Traffic | Exchange of Lamp | | | Lamp for Traffic | 2 lamps/ year | 31.92 |
| | Signal | | | | Signal | | |
| | Traffic | Damaged part etc. | | | Lamp for Street | 4 lamps/ year | 50.29 |
| | Safety | | | | Lighting in BOX Culvert | | |
| | Facility | | | | | | |
| | Subtotal | | | | | | 121.78 |
| Total | | | | | | | 173.70 |

 Table 2.5.2-1 Maintenance Item to be checked and Annual Maintenance Cost

Chapter3 Project Evaluation

3.1 Preconditions to implement the project

The Project preconditions related to the required undertaking from the Republic of Ghana are as follows:

- i) Banking Arrangement (B/A) with a Bank in Japan shall be concluded within one (1) month after the signing date of Grant Agreement (G/A).
- ii) The payments will be made when payment requests are presented by the Bank under an Authorization to Pay (A/P). This A/P shall be issued to the Bank that concluded B/A within one (1) month after Consulting Service Agreement is made.
- Before the date of formal invitation to Pre-Qualification, such public utilities, like power poles, electric lines/cables, and communication network lines/pipes as will interrupt the Project works shall be all relocated to the locations where no hindrance and inconvenience shall occur.
- iv) It is essential that land for Project construction shall be acquired prior to public announcement of Pre-Qualification and no pending issues in relation to this shall be left behind.
- v) The Weigh Station close to the Tema Toll Gate at Accra-Tema Motorway shall be moved to the location to pose no troubles on work implementation.
- vi) Prior to commencement of the works, adequate and proper compensations and resettlement in accordance with ARAP shall be undertaken for the sake of the PAPs.
- vii) Any necessary actions for tax exemption shall be taken keeping to the regulations of E/N and G/A.
- viii) Quick and smooth customs declaration and import tax formalities shall be totally finished when the imported materials or goods arrive from Japan and any other third countries.
- ix) Approval of necessary EIA for the Project shall be obtained within one (1) month after G/A is formally signed. Throughout all the construction period, quarterly monitoring Report shall be prepared for submittal to JICA. This Report shall describe monitoring data about such potential natural environmental items as air quality, water pollution, etc. which are more likely to give impacts on the works during construction phase and even after it is completed.
- x) During construction period, eminent assistance shall be given to traffic handling and management plus safety control management.
- xi) The power pipes/lines and water supply pipes/lines necessary for the construction work at site shall be routed and installed.
- xii) As a means of dispute resolution while the works proceed, discussion and assistance in good earnest shall be made to reach agreement on disputes with people in the vicinity or any other third party around.

3.2 Necessary input by recipient country

To derive benefit from the entire Project and to make it sustainable, the necessary input by recipient country is shown as follows.

(1) Management and Maintenance

To secure the service life(durability) of pavement, approach road, structure, and other related appurtenant facilities, periodic maintenance inspection or patrol shall be firmly carried out under annual approximate maintenance cost of Ghana Cedis GHC 173,700 as shown in *****. This budget shall be secured.

It is necessary as well to get it repaired appropriately and instantly soon after the damage is discovered.

Routine maintenance operations such as cleaning, removal of sediment, debris and obstacles on the road pavement surface or at the drainage facilities shall be done.

This action will bring about further and bigger improvement of road services and more safer transport measures for the road users.

Bearing in mind that regular maintenance, inspection and safety patrol to check the road lights and traffic signals are an absolute must. In terms of sustainable safety performance, daily maintenance inspection shall be performed surely and steadily. It is highly suggested to take consideration into building up the traffic management system to deal with traffic control of left-turn vehicles on the road by way of directing traffic flow by the policeman in case of sudden black-out.

(2) Pedestrian Off-Limits at the Intersection

Since pedestrian is not allowed to go and come across the intersection after Project is completed, it is required to keep constant control and sufficient watch over pedestrian's stepping in at the intersection.

This action shall require full collaboration with the Police authority.

(3) Regulating Illegally Overloaded Vehicles

The fact remains that Government of Ghana have been conducting control of overloading and that they have been checking up on illegal overloads at the existing Weigh Station nearest to the Tema Roundabout.

It has been discovered that the Weigh Station is inconvenience and disturbance to the Project execution

program and thus its absolute relocation shall be instantly fulfilled by the Government of Ghana.

Judging from securing durability of pavement structures, it is required that this Weigh Station is surely removed. Its operation shall quickly start to continue to crack down on the vehicle overloading at the newly relocated Weigh Station.

3.3 Important assumptions

The important assumptions of the Project outcome are as described below:

(1) Consideration of building new bus terminal

There is large scale bus terminal building located North-East and South-East direction from the existing intersection. This bus terminal has occupied part of the inside area of Right of Way and, therefore, it is

not perfect and public governmental building property. However, a big number of people have got to use the bus services for their transportation including long distance bus service.

Taking full account of the benefit and convenience to the people in the vicinity, construction of new public bus terminal building shall be greatly needed in cordial co-operation with any concerned organizations.

(2) Increase of traffic lanes at Accra-Tema Motorway

Eastern and Western Highways passing through Tema Motorway roundabout is regarded to be a part of Accra-Tema Motorway. The Government of Ghana is planning to have 3-lane carriageway in each directions (both inbound lane and outbound lane) on Accra-Tema Motorway which is to be widened under PPP Scheme. Feasibility Study on its development is going on. This Motorway is taking up the position of a component of total development plan of ECOWAS, namely, Abidjan-Lagos Highway Development (Trans-Coastal Highway, ECOWAS Highway).

In the top-level governmental conference among the leaders from each concerned country, the idea that design of inbound and outbound 3-lane carriageways all through the distance in Abidjan-Lagos Highway is accepted.

In view of the above circumstances, the adopted structural design requirement for Eastern and Western Highways are to cope with the works for this 3-lane carriageway expansion. Earliest widening to 3-lanes both for inbound lane and for outbound lane on the Eastern and Western Highways shall be carried out to produce rapid effects of the Project.

(3) Improvement of Ashaiman Roundabout

Southern and Northern Highways are linked to Ashaiman Roundabout with 52-meter diameter, located in the distance of approximately 1.5 km north of Tema Motorway Roundabout.

Traffic congestion at Tema Motorway Roundabout comes partly from the current severe congestion at Ashaiman Roundabout. Early commencement of the Project is of importance since the Government of Ghana well understand this Ashaiman Roundabout improvement will turn out to be a good solution to inconvenient traffic flow at Tema Motorway Roundabout. The primary objective of improving heavy congestion at Ashaiman Roundabout is to moderate and ease traffic congestion at Tema Motorway Roundabout.

3.4 Evaluation

3.4.1 Relevance

- 506,000 residents of Tema City (calculated based on the survey result in 2000) are direct beneficiaries of the Project. On the other hand, an approximate 86.6 million people is assumed (2015 survey) users of Tema Motorway Roundabout.
- This Project will contribute to easing traffic congestion and smoothing transportation of people and freight as well. It is highly expected that this will eventually lead to a comprehensive transportation network development for logistics throughout the nation and entire West African Regions.

- Traffic capacity increase to possibly respond to Tema International Port development will be expected.
- Expansion and development of major national integrated arterials covering international transit road as a part of it will be brought about. This Project will be immediately and strongly needed to secure stability of people's lives, development of their daily lives, and securing BHN (Basic Human Needs).
- Excessively advanced technique is not necessary for this Project. The Government of Republic of Ghana can manage to run, operate and maintain the improved road, related structures, and facilities by using own fund, own local manpower and locally available technical skills.

· National integrated trunk road network system including

international transit road as a part will be equally consistent with the

policy and target described in the Government National Development Master Plan. This Project can help in achieving enhancement of national trunk road network improvement.

- There is little negative impact upon environmental and social condition.
- Performing the Project will justify the incentive as well as necessity to utilize Japanese most advanced and cutting-edge construction technology, such as time control, safety control, and quality control. Through the Japan's Grant Aid Scheme, the Project will be conducted adequately without any serious issues.

3.4.2 Effectiveness

(1) Quantitative Effect

The expected quantitative effect to be produced through performing the works of target Project for economic cooperation is shown in the following **Table 3.4.2-1**. The values of base year before conducting the Project and target year in three (3) years over completion of the Project are respectively set up.

| Index | Basic Value | Target Value in 2023 | |
|--------------------------------------|------------------------------|---------------------------|--|
| mdex | (Actual Record of 2015 year) | (Project Completion Year) | |
| Passenger Volume | 86.6(Million)/year | 156.8(Million)/year | |
| Freight(Cargo) Volume | 44.3(Million)/year | 74.3(Million)/year | |
| Travel Time per minute | 8.2 minute | 2.0 minute | |
| (AM peak hour at Accra-Aflao 2.0 km) | 8.2 minute | | |

 Table 3.4.2-1Table-4-4.1 Quantitative Effects

(2) Qualitative Effect

The prospective effects are;

• To moderate traffic congestion and to provide uninterrupted traffic flows.

- To facilitate and grade up transit system in the West African Region.
- Stable carrier-transit time and effectiveness in logistics leaving and arriving at the Tema Port will be improved.
- Traffic safety of pedestrians and vehicles at the intersections will be improved.
- Future emission of transport related greenhouse gases will be reduced.
- Connecting links between Coastal Highways (Coastal Corridor) and East Highways (East Corridor) will be enhanced.