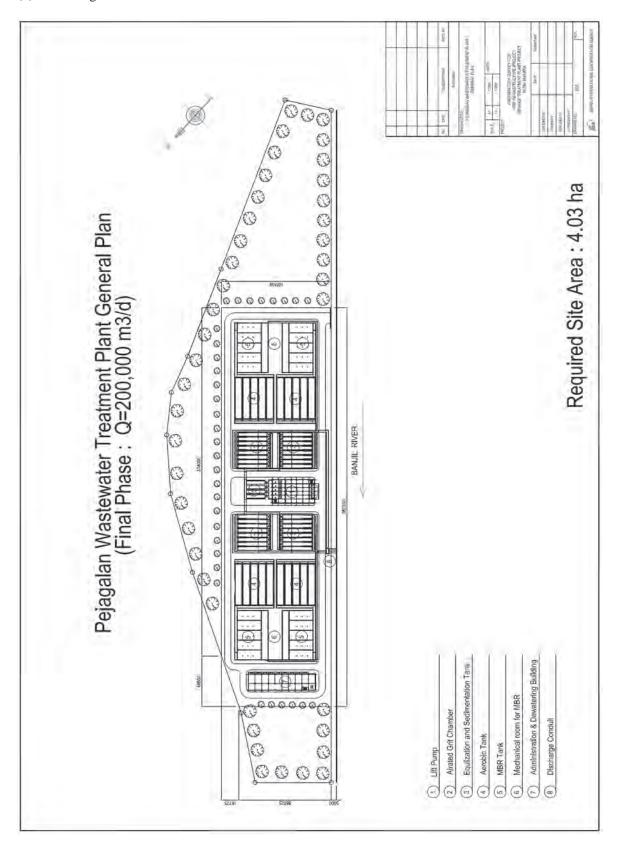
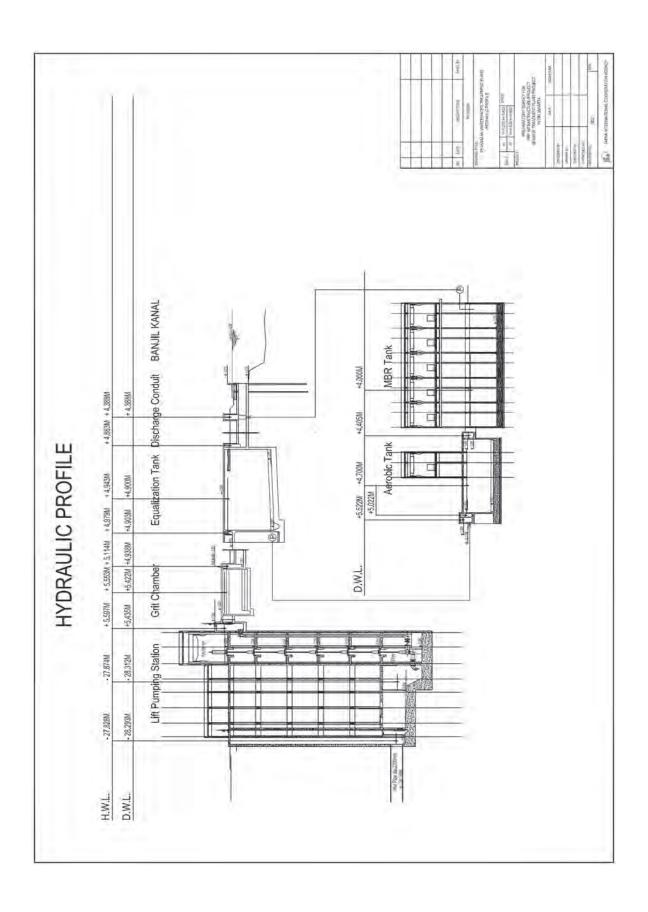
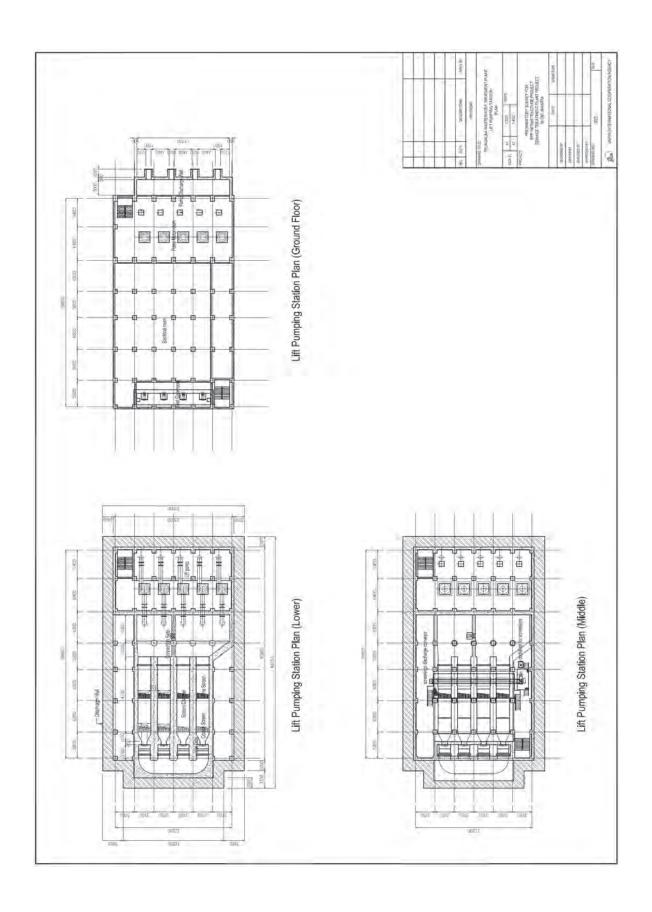
# 2. Conceptual Design of STP

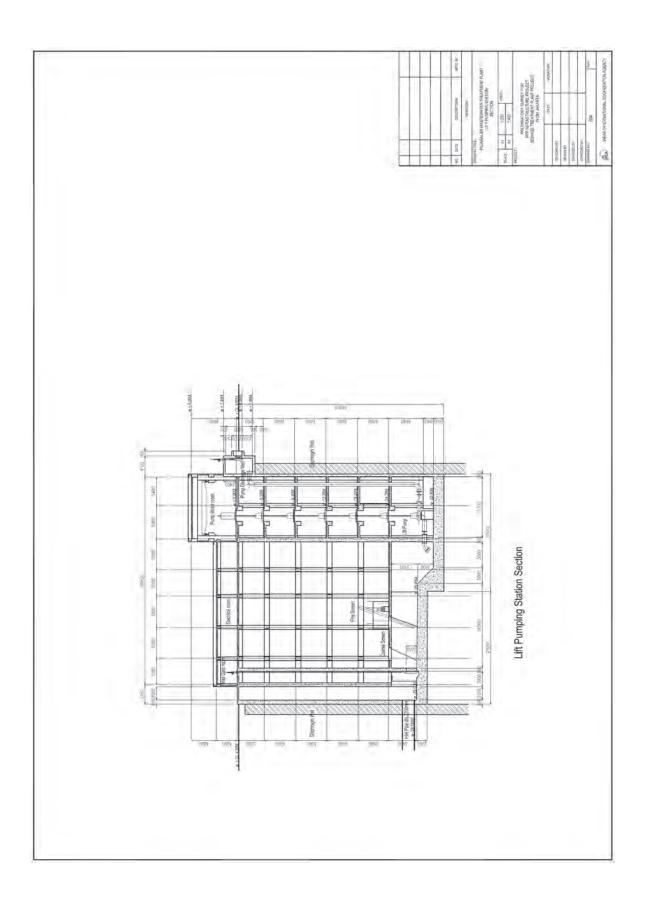
- (1) Drawings
- (2) Capacity Calculation
- (3) Main Equipment List
- (4) Hydraulic Calculation

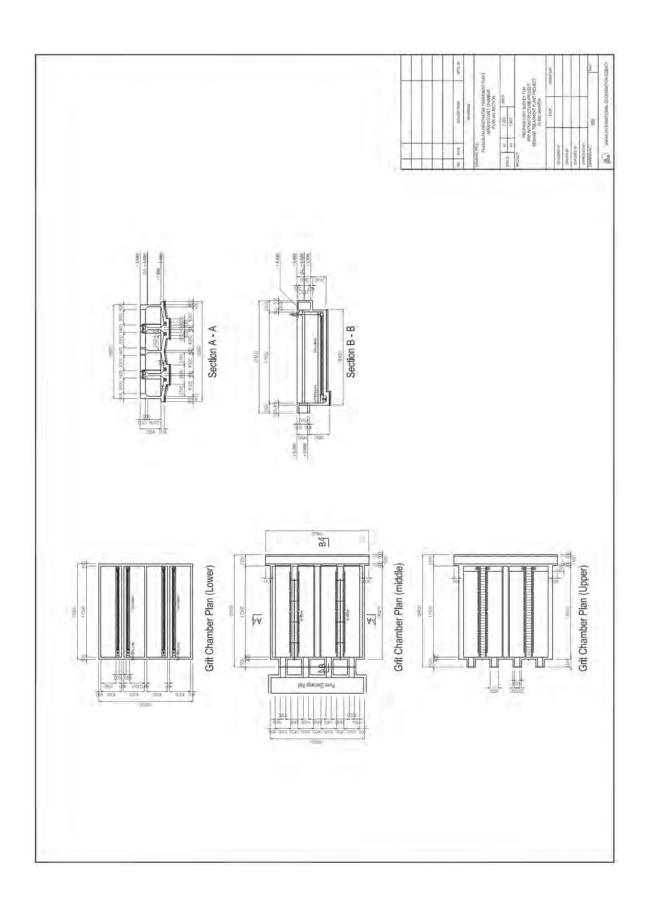
## (1) Drawings

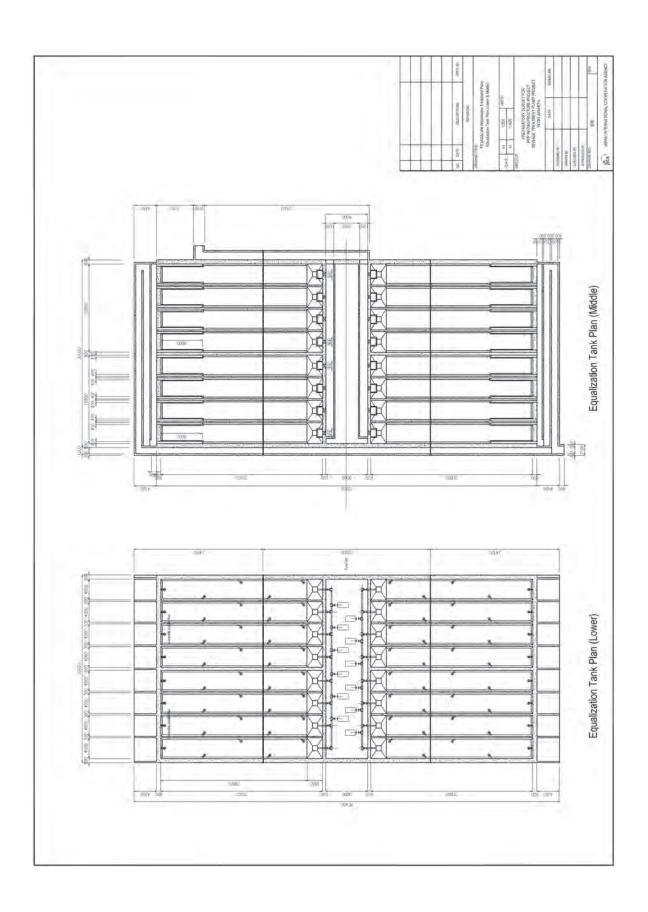


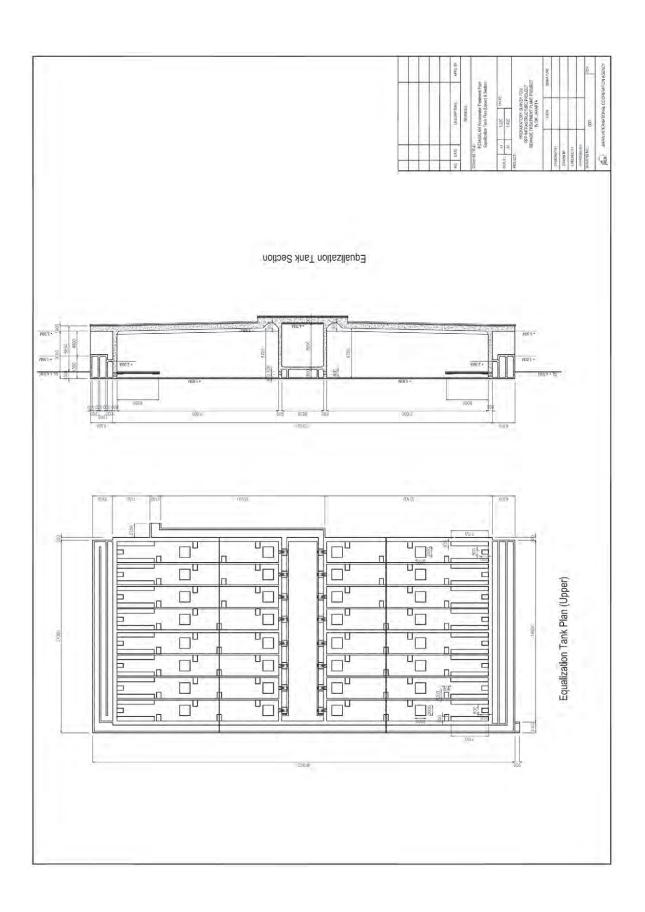


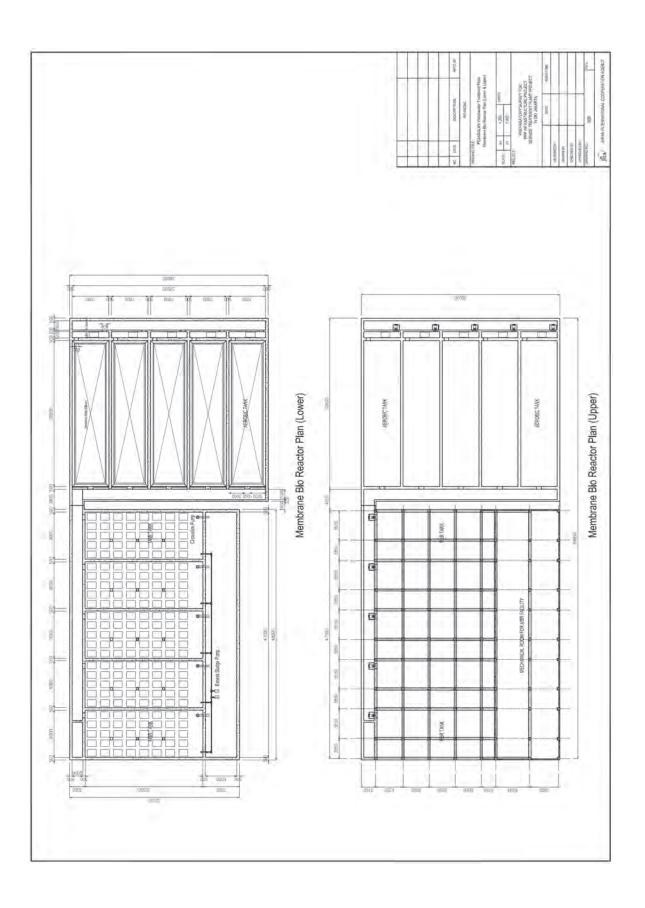


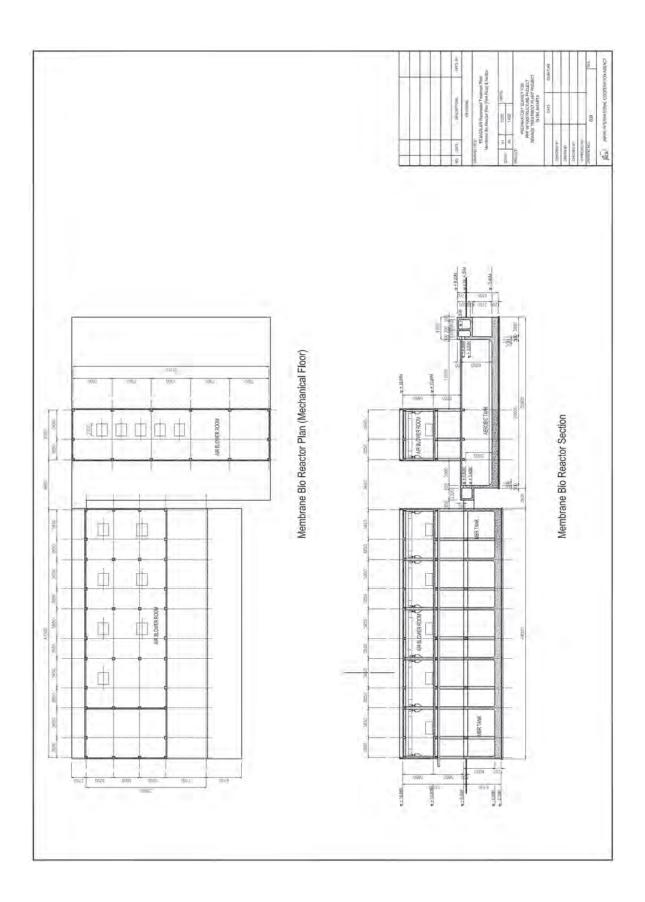


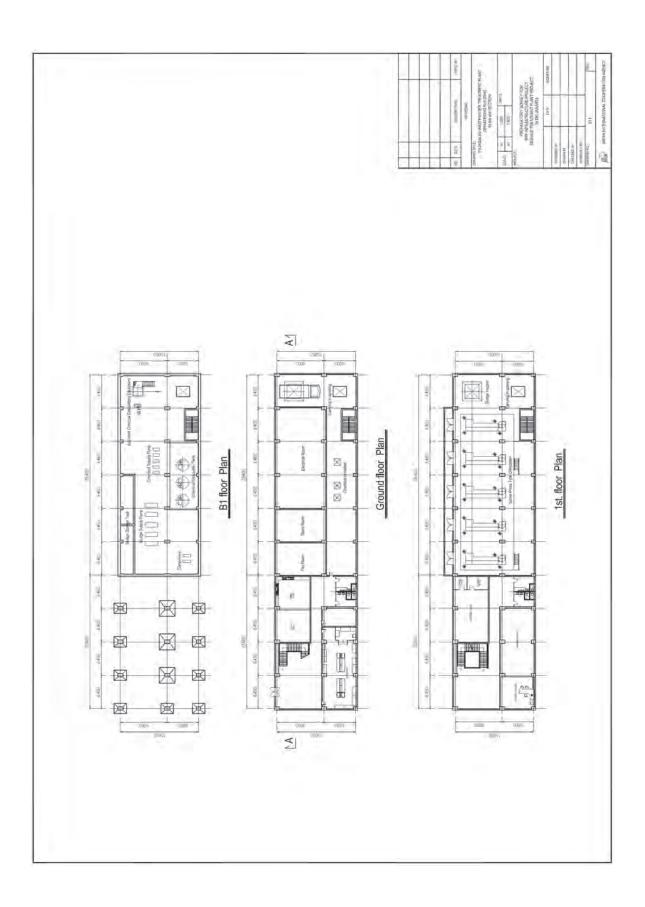


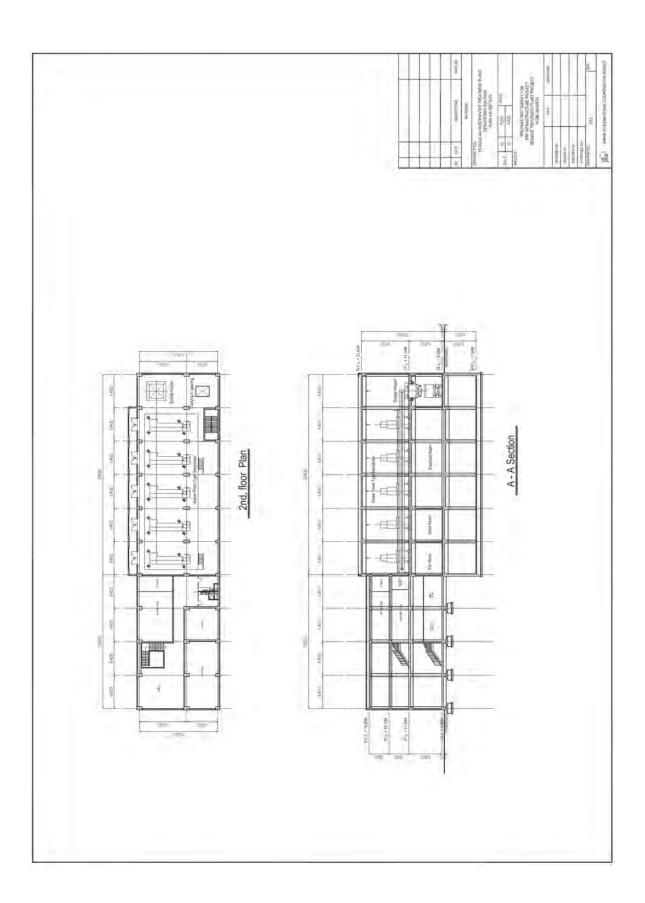












#### (2) Capacity Calculation

## Pejagalan Wastewater Treatment Plant Design Concept

- 1. Basic Design Condition
  - (1) Inflow Waste water Quantity

Inflow rate based on Revised Master Plan are as follows.

 $\begin{array}{lll} \mbox{Average Daily Flow rate} & 198,000 \ \mbox{m}^3/\mbox{d} \\ \mbox{Maximum Daily Flow rate} & 264,000 \ \mbox{m}^3/\mbox{d} \\ \mbox{Maximum Hourly Flow rate} & 396,000 \ \mbox{m}^3/\mbox{d} \end{array}$ 

Inflow rate for our design will be determined the same value as Revised Master Plan.

According to Pipe construction plan, phased inflow rate are determined as follows

	Final Phase	Phase 1
Average Daily Flow rate	200,000	100,000
Maximum Daily Flow rate	264,000	132,000
Maximum Hourly Flow rate	400,000	200,000

(2) Inflow Waste water Quality

The inflow wastewater qualities are determined by actual data in JAKARTA.

BOD	120	mg/L
TSS	120	mg/L

#### (3) Effluent Quality

Effluent qualities are in accordance with Effluent Quality Standards established in INDONESIA.

Source: Quality standard of liquid waste (DKI Jakarta Governor decree No.122, 2005)

BOD	< 50	mg/L
TSS	< 50	mg/L
Ammonia	<10	mø/I

Target treated water qualities are set as follows

BOD	<10	mg/L
TSS	<10	mg/L
Ammonia	<5	mg/L

<sup>\*</sup> Denitrification process is carried out in the future, does not perform immediate.

In the future T-N <15 mg/L

#### (4) Wastewater Treatment Process

As No 1 priority of the site constraints, we choose the Treatment Process. As a result, to adopt the following treatment process.

#### MBR (Membrane Bioreactor) Process

(5) Sludge Treatment Process

Since adopting the MBR process, the thickening process is omitted. Therefore only the Dewatering process is established.

#### Excess sludge ---- Dewatering ---- to Dump site

(6) Inflow Conduit

Dimension diameter 2200 mm

Bottom Elevation about PP - 29.20 M (GL - 33.70m)

(7) Discharge facility

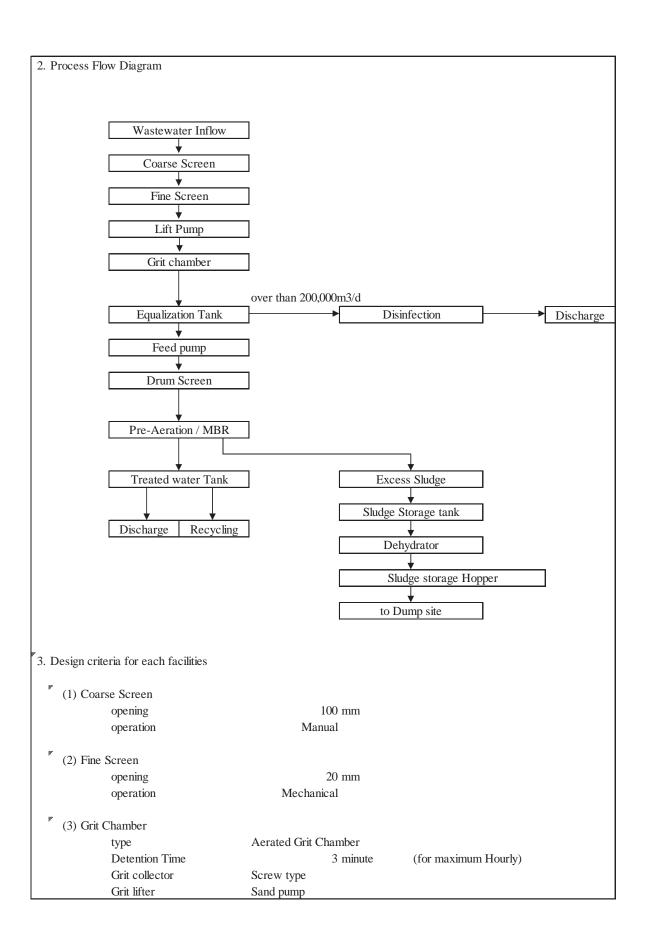
Discharge stream to BANJIL KANAL

Conduit Dimension 2,000mm<sup>H</sup> x 1,500mm<sup>W</sup>

Highest water level at Discharge point PP + 4.30 M

(8) Site Ground Elevation

Ground Elevation PP + 4.50 M



(4) Lift Pump

type Volute type mixed flow pump

capacity No.1 Diamater 700 mm 70m3/min

Pump head about 35 m (assumed value)

(5) Drum Screen

opening 1 mm

(6) Equalization Tank

Retention Time 4 hr Having a function of spillway and disinfection

(7) Membrane Bioreactor Tank

a) In the immediate

Membrane placement inside the activated sludge reactor

MLSS in reactor tank 9,000 mg/L Design Flux  $0.5\sim0.6~\text{m}^3/\text{m}^2/\text{d}$ 

Excess Sludge production rate 70 %

 $\begin{array}{lll} BOD \ removal \ rate & 0.12 \ kg-BOD/kg-SS/d \\ BOD \ removal \ rate \ per \ unit \ volume & 1.44 \ kg-BOD/m^3/d \\ Nitrification \ rate & 0.025 \ kg-N/kg-SS/d \\ Nitrification \ rate \ per \ unit \ volume & 0.3 \ kg-N/m^3/d \\ \end{array}$ 

(8) Oxygen requirement

a) for BOD removal

Required unit oxygen for BOD removal 0.5 kg-O<sub>2</sub>/kg-BOD

b) for Nitrification

Required unit oxygen for nitrification 64/14 kg-O<sub>2</sub>/kg-N

c) for endogenous respiration

Required unit oxygen for endogenous respiration 0.12 kg-O<sub>2</sub>/kg-VSS

MLVSS/MLSS 0.7~0.75

(9) Treated water Tank

a) for Back wash

about  $400 \text{ m}^3$ 

b) for recycling (in the future)

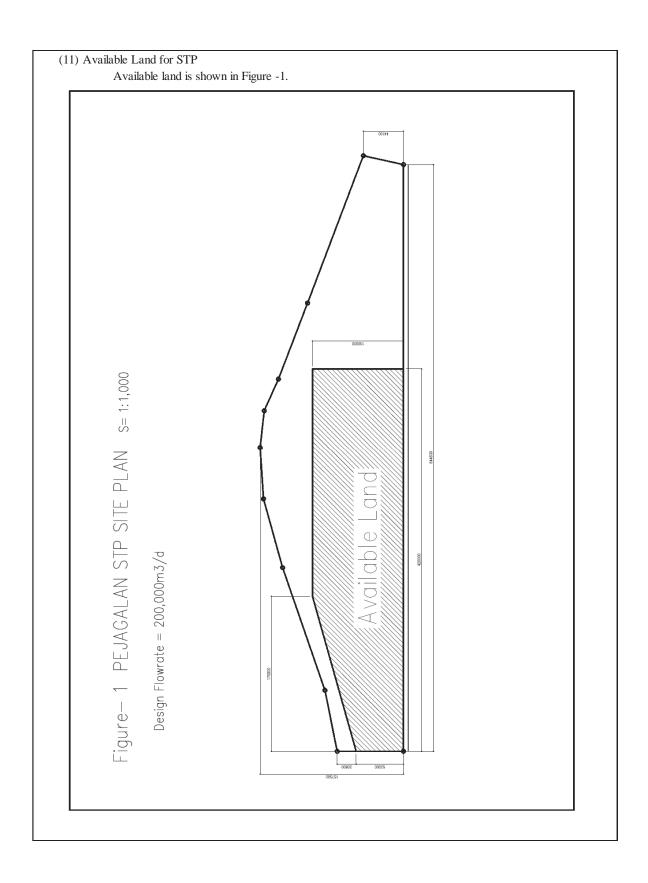
 $20.000 \text{m}^3 / \text{d} * 8 \text{hr}$  6,700 \text{ m}^3

(10) Dewatering Facility

Dehydrator Type Pressing Rotary Outer Cylinder-Type Screw Press

Influent Sludge Concentration 0.9 %
Dewatered Sludge Concentration 83 %
Operation time 24 hr

capability of Screw Press



12.231t/d Ord 1,359m3/d Rd 472mg/L 11.619t/d 1,291m3/d Return Wastewater Dewatering Conc. Odin Qdout Conc. Dsdo Dsdi Conc. Dsr Or 100,000m3/d 12.000t/d 124mg/L 0.9% 0.378t/d 4mg/L 120mg/L 124mg/L 99,932m3/d 12.609t/d 97.0% 101,291m3/d 12.609t/d 101,291m3/d 1,359m3/d 12.231t/d 99,932m3/d 12.609t/d Excess sludge Qout Qout Conc. Conc. Conc. Conc. Conc. Qin Qin Dsf R.E. Oef Dse Dsi Dsi Dsi Ds 0 Equalization MBR Tank <Phase 1> Pump well Discharge Influent tank 1.225t/d 2,582m3/d 95.0% Dewatering effluent 0.9% Dsrd 24.470t/d Qrd 2,719m3/d Rd 475mg/L 1.227t/d 23.247t/d 137m3/d 17.0% 2,581m3/c Return Wastewater Dewatering Conc. Conc. Qdout Odin Dsdo Dsdi Conc. Dsr Or Sludge Mass Balance Calculation Sheet 24.000t/d 4mg/L 125mg/L 97.0% 120mg/L 0.9% 200,000m3/d 202,581m3/d 25.227t/d 125mg/L 202,581m3/d 199,863m3/d 25.227t/d 2,719m3/d 24.470t/d 199,863m3/d 25.227t/d 0.757t/d **Excess sludge** Qout Equalization Qout Conc. Conc. Conc. Conc. Dsf Conc. Qef Dse Qin R.E. Qin Dsi Dsi Dsi Ds 0 <Final Phase> Pump well MBR Tank Discharge Influent tank

0.609t/d

0.611t/d 1,291m3/d

0.9% Dsrd

68m3/d

17.0%

Dewatering effluent

A2-17

Item	Final Phase Calculation	Phase 1 Calculation
1. In let Pipe		
1.1 Pipe Condition Design Flow rate		
Avegrage Daily Flow rate	$200,000 \text{ m}^3/\text{d} = 2.315$	$100,000 \text{ m}^3/\text{d} = 1.157$
Maximum Daily Flow rate Maximum hourly Flow rate	$= 264,000 \text{ m}/d = 5.030 \text{ m}/s$ $= 400,000 \text{ m}^3/d = 4.63 \text{ m}^3/s$	m /d = m 3/d =
Pipe Diameter	2,200 mm	2,200 mm
Pipe Gradient		= 1.1 permillage
Invert Level		= -29.199 M
Manning's "n" value	0.013	
Full Flow rate	7.602	7.602
Full Flow Velocity	= 2.000 m/s	= 2.000 m/s
Water Depth		
Avegrage Daily Flow rate		= 0.580 m
Maximum Daily Flow rate	= 1.060 m	= 0.669 m
Maximum hourly Flow rate	= 1.371 m	= 0.833 m
Water Level (above sea level)		
Avegrage Daily Flow rate	+ 0.906 $=$ -28.293	+ 0.580 =
Maximum Daily Flow rate	+ 1.060 $=$ -28.139	+ 0.669 $=$ -28.530
Maximum hourly Flow rate	= -29.199 + 1.371 $=$ -27.828 M	= -29.199 + 0.833 $=$ -28.366 M
1.2 Inlet Chamber		
Invert Hevation	= -30,000 M (above sea level)	= -30,000 M (ahove sea level)
Water denth at unstream of inlet Gate		
Avegrage Daily Flow rate	= 1.707 m	= 1.381 m
Maximum hourly Flow rate	= 2.172 m	= 1.634 m
2. Inlet Gate		
Invert Elevation	$= -30.000 \qquad M \text{ (above sea level)}$	= -30.000 M (above sea level)
Gate Width		= 0.800 m
Gate Height	= 2.000 m	.2
Number of Gate	= 4 Nos.	= 2 Nos.
Passing Velocity		
Avegrage Daily Flow rate	4) = 0.424	$1.157 / (0.80 \times 1.381 \times 2) = 0.524$
Maximum nouny riow rate	+.050 //	$= (7 \times 1.03 \times 2)$

Item			Final Ph	Final Phase Calculation	ation				Phase 1 Calculation
Headloss at Inlet Gate									
Avegrage Daily Flow rate	П		0.424 ^2/	^2 / 19.6	= 0.0	0.014 m		П	$x = 0.524  ^{2}/19.6 =$
Maximum hourly Flow rate	П	1.5 x	0.666 ^2/	^2 / 19.6	= 0.0	0.034 m		П	$1.5 \text{ x} = 0.885 ^{2}/19.6 = 0.060 \text{ m}$
Water Level at downs tream of inlet Gate									
Avegrage Daily Flow rate	П		- 0.014	П		M		П	- 0.021 = $-28.640$
Maximum hourly Flow rate	II	-27.828	- 0.034	II	-27.862	M		П	-28.366 - 0.06 = -28.426 M
3. Screen									
3.1 Coarse Screen									
Openning	П	100	mm					П	100 mm
Type	П	Manual Rake	9					П	Manual Rake
Bottom Elevation	П	-30.050 M	М					П	-30.050 M
Channel width	П	2.000 r	ш					П	2.000 m
Number of Screen	II	4	Nos.					П	2 Nos.
Approch water depth									
Avegrage Daily Flow rate	П		30.050	П		ш		П	-28.64030.050 = 1.410  m
Maximum hourly Flow rate	II	-27.862	30.050	П	2.188	m		П	-28.426 - $-30.050$ = 1.624 m
Approch velocity									
Avegrage Daily Flow rate	П	2.315	/( 2.00 x	1.743	x 4)		m/s	П	/ ( 2.00 x 1.410 x 2 )=
Maximum hourly Flow rate	П	4.630	/( 2.00 x	2.188	x 4)	= 0.265	m/s	П	2.315 / ( $2.00 \times 1.624 \times 2$ )= 0.356 m/s
Headloss at coarse screen	II	neglect						II	neglect
3.2 Fine Screen									
Openning	II	15 r	mm					П	15 mm
Type	П	Mechanical Rake	Rake					П	Mechanical Rake
Bottom Elevation	П	_	M					П	-30.150 M
Channel width	П	2.000 r	m					П	2.000 m
Number of Screen	П	4	Nos.					П	2 Nos.
Approch water depth									
Avegrage Daily Flow rate	П	-28.307	30.150	П	1.843	ш		П	II
Maximum hourly Flow rate	П	-27.862	30.150	П	2.288	m		П	-28.426 - $-30.150$ = $1.724$ m
Approch velocity									
Avegrage Daily Flow rate	П		/( 2.00 x	1.843	x 4)	= 0.157	m/s	П	/ ( 2.00 x 1.510 x 2 )=
Maximum hourly Flow rate	П	4.630	/( 2.00 x	2.288	x 4)	= 0.253	m/s	П	2.315 / ( 2.00 x 1.724 x 2 )= 0.336 m/s

Item	Final Phase Calculation	Phase 1 Calculation
Headloss at fine screen Avegrage Daily Flow rate Maximum hourly Flow rate	$= 2.34 \times \sin 70 \times (9/15)^{4} \times (4/3) \times (0.157 \times 2)^{2} / 19.6 = 0.005$ $= 2.34 \times \sin 70 \times (9/15)^{4} \times (4/3) \times (0.253 \times 2)^{2} / 19.6 = 0.012$	= 2.34 x sin70 x (9/15) $^{4}$ (4/3) x( 0.192 x 2) $^{4}$ 2 / 19.6 = 0.007 = 2.34 x sin70 x (9/15) $^{4}$ (4/3) x( 0.336 x 2) $^{4}$ 2   19.6 = 0.021
water Leverat downstream of the screen Avegrage Daily Flow rate Maximum hourly Flow rate	= -28.307 - 0.005 = -28.312 M = -27.862 - 0.012 = -27.874 M	= -28.640 - 0.007 = -28.647 M = -28.426 - 0.021 = -28.447 M
4. Lift Pump Type	Vertical shaft Volute type mixed flow pump	
Design Flow rate Average Daily Flow rate	$m^3/d = 139$	$m^3/d = 69$
Maximum Daily Flow rate Maximum hourly Flow rate	= 264,000   m3/d = 183   m3/min $= 400,000   m3/d = 278   m3/min$	= 132,000   m3/d = 92   m3/min $= 200,000   m3/d = 139   m3/min$
Pump Capacity Number of Pump	= 4 nos	= 2 nos
Pump Capacity	$=$ 278.0 / 4 $=$ 70 $\text{m}^3/\text{min}$	$=$ 139.0 / 2 $=$ 70 $\text{m}^3/\text{min}$
Pump Diameter	$= 146 \times (70.0 / 3)^{0.5} = 705$ $= 700 \text{ mm}$	$= 146 \times (70.0 / 3) \times 0.5 = 705$ $= 700 \text{ nm}$
Pump Head Discharge water level Suction water level Actual Pump Head Others head loss	= 5.500 M = -27.500 M = 5.50027.500 m = 2.000 m (assumed)	= 5.500 M = -27.800 M = 5.50027.800 m (assumed)
Total Pump Head	= 33.000 + 2.000 $=$ 35.000 m	= 33.300 + 2.000 = 35.300 m
Motor output	$P = \frac{\rho \times g \times Q \times H}{60 \times 1000 \times \eta} \times (-1 + \alpha)$	

Item	Final Phase Calculation	Phase 1 Calculation
	$ ho$ : Water Density 1,000 kg/m3 g : Acceleration of Gravity 9.8 m/sec2 Q : Flow rate 70 m3/min H : Pump head 35.00 m $\eta$ : Pump Efficiency 0.8 $\alpha$ : Surplus ratio 0.15	
	$= \frac{1,000 \times 9.8 \times 70.0 \times 35.0}{60 \times 1,000 \times 0.8} \times (1 + 0.15)$	= Same left
	$= 575.2 \longrightarrow 580 \text{ kW}$	= same left
Lift Pimp Specification	Type Vertical shaft Volute type mixed flow pump  Diamater 700 mm  Capacity 70 m3/min  Pump Head 35.00 m  Motor output 580 kW  Numbers 5 nos (Include 1 stand-by)	Type Vertical shaft Volute type mixed flow pump  Diamater 700 mm  Capacity 70 m3/min  Pump Head 35.30 m  Motor output 580 kW  Numbers 3 nos (Include 1 stand-by)
5. Git Chamber Type Dasian Flour este	Aerated Grit Chamber	Aerated Grit Chamber
Avegrage Daily Flow rate Maximum hourly Flow rate	= 264,000   m3/d $= 400,000   m3/d$	= 132,000   m3/d $= 200,000   m3/d$
Detention Time (For Maximum Hourly Flow rate)	= 3 min	= 3 min
Required Volume	$= 400,000 \div 1,440 \times 3 = 834 \text{ m}^3$	$= 200,000 \div 1,440 \times 3 = 417 \text{ m}^3$
No. of basin	= 4 basins	= 2 basins
Required Chamber Volume (each)	$=$ 834 $\div$ 4 $=$ 209 $\text{m}^3$	$= 417 \div 2 = 209 \text{ m}^3$

Item	Fin al Phase Calculation	Phase 1 Calculation
Depth	= 3.0 m	= 3.0 m
Width	= 4.0 m	= 4.0 m
Length	= 17.5   m	= 17.5 m
Actual Volume	= $4.0 \text{ mW} \times 3.0 \text{ mH} \times 17.5 \text{ mL} = 210 \text{ m}^3$	= $4.0 \text{ mW} \times 3.0 \text{ mH} \times 17.5 \text{ mL} = 210 \text{ m}^3$
Required Air (each)	$=$ 17.5 m $\times$ 0.3 m <sup>3</sup> /min/m $=$ 5.3 m <sup>3</sup> /min	$=$ 17.5 m $\times$ 0.3 m <sup>3</sup> /min/m $=$ 5.3 m <sup>3</sup> /min
Total Air supply required	$=$ 5.3 $\times$ 4 $=$ 21.2 m <sup>3</sup> /min	$=$ 5.3 $\times$ 2 $=$ 10.6 $m^3/min$
Airation Blower Number of Blower	= 2 units	= 1 unit
Blower capacity		= 11 m <sup>3</sup> /min
Discharge Pressure		
Diffuser Depth	= 3,000 mmAq	= 3,000 mmAq
Loss of Diffuser	750	750
Other loss		
I otal Pressure Blower Specification		
Type	= Rotary Roots Blower	= Rotary Roots Blower
Diameter	= 150  mm	= 150 mm
Capacity	$= 11  m^3/min$	$= 11   m^3/min$
Discharge Pressure		
Motor Output	= 22 kW	= 22 kW
Qantity	= 3 units (including stand-by 1 unit)	= 2 units (including stand-by 1 unit)
Quantity of Grit at peak flow	Assume a value of 0.05m3/1000m3 at peak flow	Assume a value of 0.05m3/1000m3 at peak flow
	$= 400,000 \times 0.05 \div 1,000 = 20.0 \text{ m}^3/\text{d}$	$= 200,000 \times 0.05 \div 1,000 = 10.0 \text{ m}^3/\text{d}$

= 4 hour = 200,000 / 24 × 4 = 8 tanks = 33,333 / 8 = 50 m³/m²/d = 200,000 / 50 = = 16.0 m = 16.0 m = 8.5 m = 8.5 m = 8.5 m = 8.5 m = 16.0 × 31.0 × 8.5 Install Five (5) Pumps per twe (2) tanks = 200,000 / 20 / 1,440 = 146 × √ (6.9 / 2)	= 33,333 m <sup>3</sup>	
$ \begin{array}{rclrcl} &=&4 \text{ hour} \\ &=&8 \text{ tanks} \\ &=&33,333 &/ 8 \\ &=&50 & \text{m}^3/\text{m}^2/\text{d} \\ &=&200,000 &/ 50 &=\\ &=&16.0 & \text{m} \\ &=&31.0 & \text{m} \\ &=&8.5 & \text{m} \\ &=&16.0 \times 31.0 \times 8 \\ &=&16.$		
Length $= 16.0 \text{ m}$ $= 31.0 \text{ m}$ $= 8.5 \text{ m}$ Number $= 8.5 \text{ m}$ $= 16.0 \times 31.0 \times 8$ $= 3.968 \times 8.5$ Install Five (5) Pumps per twe (2) tanks $= 200,000 / 20 / 1,440$ $= 146 \times \sqrt{(69 / 2.5)}$ Nonclogging pump  Actual Pump head $= 10 \text{ m}$ Piping head loss $= 4 \text{ m}$	$= 4.167 \text{ m}^{3}$ $4,000 \text{ m}^{2}$ $8.30 \text{ m}$	= 4 hour = 100,000 / 24 × 4 = 16,667 m <sup>3</sup> = 4 tanks = 16,667 / 4 = 4,167 m <sup>3</sup> = 50 m <sup>3</sup> /m <sup>2</sup> /d = 2,000 m <sup>2</sup> = 100,000 / 50 = 2,000 m <sup>2</sup> = 16,667 / 2,000 = 8.30 m
$= 16.0 \times 31.0 \times 8$ $= 3.968 \times 8.5$ Install Five (5) Pumps per twe (2) tanks $= 200,000 / 20 / 1,440$ $= 146 \times \sqrt{ (6.9 / 2.4)}$ Nonclogging pump $= 10 \text{ m}$ Piping head loss = 10 m		= 16.0 m = 31.0 m = 8.5 m = 4 basins
Install Five (5) Pumps per twe (2) tanks $= 200,000 / 20 / 1,440$ $= 146 \times \sqrt{ (6.9 / 2.4)}$ Nonclogging pump $Actual Pump head = 10 \text{ m}$ Piping head loss = 4 m	= 3,968   m2 $= 33,728   m3$	$= 16.0 \times 31.0 \times 4 = 1,984  \text{m}^{2}$ $= 1,984  \times 8.5 = 16,864  \text{m}^{3}$
= 200,000 / 20 / 1,440 = 146 ×√ ( 6.9 / 2.4 Nonclogging pump  Actual Pump head = 10 m  Piping head loss = 4 m		
II II	= 6.9 m <sup>3</sup> /min ) = 243 mm = 250 mm	$= 100,000 / 8 / 1,440 = 8.7 m3/min$ $= 146 \times \sqrt{(8.7 / 3)} = 249 mm$ $= 250 mm$
Total Pump head = 14 m		
$\mathbf{P} = \frac{\rho \times \mathbf{g} \times \mathbf{Q} \times \mathbf{H}}{60 \times 1000 \times \eta} \times (1 + \alpha)$		
$= \frac{1,000 \times 9.8 \times 6.9 \times 6.9 \times 6.0 \times 1,000 \times 0.6}{60.0 \times 1,000 \times 0.6}$	14 × ( 1 + 0.2 )	

Item	Final Phase Calculation	Phase 1 Calculation
Pump Specification Type Diameter Capacity Head Motor output	= 31.6 → 37 kW  = Nonclogging pump = 250 mm = 6.9 m³/min = 14 m = 37 kW = 24 nos. (including 4 standby)	= Nonclogging pump = 250 mm = 8.7 m³/min = 14 m = 37 kW = 12 nos. (including 2 standby)
Mixer for antisettling  Type Propeller Diameter Motor output Numbers	= Submersible propeller Mixer = 300 mm = 2.8 kW = 5 nos./each channel × 4 × 8 = 160 nos.	= Submersible propeller Mixer = 300 = 2.8 = 5 nos./each channel × 4 × 4 = 80 nos.
6. Disinfection Tank Design Flow (maximum) Retention Time	$= 200,000   m^3/d$ $= 5   min$	$= 100,000   m^{3}/d$ $= 5   min$
Required Tank volume  Tank size  Length  Depth  number  Actual tank volume	= 200,000 / 1,440 × 5 = 694 m <sup>3</sup> = 1.20 m = 70.0 m = 2.10 m = 4 tanks = 706 m <sup>3</sup>	$= 100,000 / 1,440 \times 5 = 347 \text{ m}^{3}$ $= 1.20 \text{ m}$ $= 70.0 \text{ m}$ $= 2.10 \text{ m}$ $= 2 \text{ tanks}$ $= 353 \text{ m}^{3}$
Disinfection chemicals Effective chlorine density (β) Specific gravity Dosing ratio (α)	= Sodium hypochlorite = 10 % = 1.1 10% density = 10 mg/L	

Item	Final Phase Calculation	Phase 1 Calculation
Dosage Capacity Maximum consunption q	$= Q \times \alpha \times 10^{-6} \times (100 / \beta) \times (11 / \gamma)$ $= 200,000 \times 0.00001 \times (100 / 10) \times (11 / 1.1)$ $= 18.18  m^{3}/d = 12.6  L/min$	$= Q \times \alpha \times 10^{-6} \times ( 100 / \beta ) \times ( 11 / \gamma )$ $= 100,000 \times 0.00001 \times (100 / 10 ) \times (1 / 1.1 )$ $= 9.09 \text{ m}^{3}/\text{d} = 6.3 \text{ L/min}$
Storage tank capacity Required tank capacity	2 daays storage, and Itrain I tank $= 18.18 \times 2 / 4 = 9.1 \text{ m}^3$	$= 9.09 \times 2 / 2 = 9.1 \text{ m}^{3}$
Number of pump	1 train 1pump and 1 stanby	
Required pump capacity	= 12.6 / 4 $=$ 3.15 L/min	= 6.3 / 2 $=$ 3.15 L/min

Item	Final Phase Calculation	Phase 1 Calculation
6. Membrane Bioreactor Tank (MBR Tank)		
6.1 Tank Volume 1) MBR tank Volume		
Design flow rate Design Flux	$= 200,000   m^3/d$ $= 0.42   m^3/m^2/d$	$= 100,000   m^{3}/d$ $= 0.42   m^{3}/m^{2}/d$
Required Area of Membrane	_	_
Membrane Area per unit	$=$ 600 $m^2/unit$	= 600 m <sup>2</sup> /unit
Required number of Membrane unit		= 238,095 / 600 = 397 nos
Number of Basin	= 20 basins - 794 / 20 - 40 unite	= 20 basins - 397 / 20 - 20 mits
Membrane unit size		
Length	= 1,288 mm	= 1,288 mm
Width	= 1,687 mm	
Depth	= 2,852 mm	= 2,852 mm
	$\begin{array}{c c} & & & & & \\ \hline & & & & \\ \hline & & \\ \hline$	
	400   1.687   400	
Membrane tank size		
Length Width	= 20.0 m (2.50m x 8units = 20.00m) = 9.0 m (1.80m x 5units = 9.00m)	= 20.0 m (2.50m x 8units = 20.00m) = 90 m (1.80m x 5units = 9.00m)
Depth	5.0 m	5.0 m
Numbers		10
Membrane tank volume per unit	$=$ $900 \text{ m}^3$	$=$ $900 \text{ m}^3$
Membrane tank volume	$= 900 \times 20 = 18,000 \text{ m}^3$	$=$ 900 $\times$ 10 $=$ 9,000 $m^3$
MLSS in Membrane Tank	= 9,000 mg/L	= 9,000 mg/L

Item	Fin al Phase Calculation	Phase 1 Calculation
3) Aerobic tank volume for BOD removal 3)-1 BOD removal in Aerobic tank		
Design flow rate	$= 200,000 \text{ m}^3/d$	$= 100.000  \text{m}^{3}/\text{d}$
Influent BOD concentration	120	120
Influent SS concentration	= 120  mg/L	= 120  mg/L
BOD load	$= 200,000 \times 120 / 1,000 = 24,000 \text{ kg/d}$	$= 100,000 \times 120 / 1,000 = 12,000 \text{ kg/d}$
BOD removal rate	= 0.14 kg-BOD/kg-SS/d (25 degrees C)	= 0.14  kg-BOD/kg-SS/d (25  degrees C)
MLSS in MBR tank	= 9,000  mg/L	= 9,000  mg/L
BOD removal rate per unit volume	$= 0.14 \times 9,000 / 1,000 = 1.26 \text{ kg-BOD/m}^3$	$= 0.14 \times 9,000 / 1,000 = 1.26 \text{ kg-BOD/m}^3$
Required Aerobic tank volume for BOD	$=$ 24,000 / 1.26 $=$ 19,048 $m^3$	$= 12,000 / 1.26 = 9,524 \text{ m}^3$
3)-2 Nitrification in Aerobic tank		
Design flow rate	$= 200,000   m^3/d$	$= 100,000 \text{ m}^3/\text{d}$
Influent BOD concentration	= 120  mg/L	= 120 mg/L
Influent SS concentration	= 120  mg/L	= 120  mg/L
Influent T-N concentration	= 40 mg/L	= 40 mg/L
MLSS in Aerobic tank	T/gm 000,6 =	
BOD load	× 120 / 1,000 =	
T-N load	$= 200,000 \times 40 / 1,000 = 8,000 \text{ kg/d}$	$= 100,000 \times 40 / 1,000 = 4,000 \text{ kg/d}$
Excess sludge production rate	100	= 100 %
Nitrogen concentration in sludge	% 9 =	% 9 =
Nitrogen in excess sludge	$0 \times 120 \times 0.06 / 1,00$	$\times$ 120 $\times$ 0.06 / 1,00
T-N load for nitrification	= 8,000 - 1,440 = 6,560  kg/d	= 4,000 - 720 $=$ 3,280 kg/d
Nitrification rate	= 0.037  kg-N/kg-SS/d  (25  degrees C)	= 0.037  kg-N/kg-SS/d  (25  degrees C)
MLSS in MBR tank	mg/L	mg/L
Nitrification rate per unit volume	$= 0.037 \times 9,000 / 1,000 = 0.33 \text{ kg-N/m}^3$	$= 0.037 \times 9,000 / 1,000 = 0.33 \text{ kg-N/m}^3$
Nitrification in MBR tank	$= 19,600 \times 0.33 = 6,527 \text{ kg-N/d} - \text{OK}$	$= 9,000 \times 0.33 = 2,997 \text{ kg-N/d}$
Aerobic tank size		
Length	= 28.0 m	= 28.0  m
Width	= 7.0 m	= 7.0 m
Depth	= 5.0 m	= 5.0 m
Numbers		= 10 nos
Pre-aeration tank volume per unit	$= 980 \text{ m}^3$	= 980 m <sup>3</sup>
Pre-aeration tank volume	$= 980 \times 20 = 19,600 \text{ m}^3$	$= 980 \times 10 = 9,800 \text{ m}^3$

Item	Final Phase Calculation	Phase 1 Calculation
6.2 Required Air Volume  1) Oxygen for biological treatment  1)-1 Required Oxygen for BOD removal  Design Flow rate Influent BOD concentration  BOD load Required unit Oxygen for BOD removal  Required Oxygen for BOD removal	= 200,000   m3/d $= 120   mg/L$ $= 24,000   kg-BOD/d$ $= 0.5   kg-O2/kg-BOD$ $= 24,000   × 0.5   = 12,000   kg-O2/d$	= 100,000 mg/L = 120 mg/L = 12,000 kg-BOD/d = 0.5 kg-O <sub>2</sub> /kg-BOD = 12,000 × 0.5 = 6,000 kg-O <sub>2</sub> /d
1)-2 Required Oxygen for nitrification T-N load for nitrification Required unit Oxygen for nitrification Required Oxygen for nitrification	$= 6.560   kg-N/d$ $= 64/14   kg-O_2/kg-N$ $= 6.560   64/14   29,989   kg-O_2/d$	= $3,280$ kg-N/d = $64/14$ kg-O <sub>2</sub> /kg-N = $3,280 \times 64/14$ = $14,994$ kg-O <sub>2</sub> /d
1)-3 Required Oxygen for endogenous respiration Pre-aeration tank Volume Total tank Volume MLSS in tank MLVSS/MLSS Quantity of MLVSS Required unit Oxygen for endogenous respiration Required Oxygen for endogenous respiration (AOR)  1)-5 Standard Oxygen Requirement (SOR)	$= 19,600 \text{ m}^{3}$ $= 19,600 \text{ m}^{3}$ $= 9,000 \text{ mg/L}$ $= 9,000 \times 19,600 \times 0.750 / 1,000$ $= 132,300 \text{ kg/d}$ $= 0.12 \text{ kg-O}_{2}\text{kg-VSS}$ $= 132,300 \times 0.12 = 15,876 \text{ kg-O}_{2}\text{/d}$ $= 12,000 + 29,989 + 15,876 = 57,865 \text{ kg-O}_{2}\text{/d}$ $= AOR \times C_{SW \times IX} 760$	$\begin{array}{llllllllllllllllllllllllllllllllllll$

Phase 1 Calculation	= 35.292 kg as O <sub>2</sub> /d	$\begin{array}{c} 35.5 \\ 28 \times 1.29 \\ 458.654 \text{ Nm} \\ \hline 319 \text{ Nm} \\ 32 \text{ Nm} \end{array}$
Final Phase Calculation	C <sub>Sw</sub> ; water at 20 Celsius = 8.84 mg/L  Ca; Average DO = 1.5 mg/L  Oxygen saturation concentration in clean water at 25 Celsius = 8.39 mg/l  T; water temperature = 25 degees C  r; Coefficient of water depth  r = 1 + (H/2)/10.332 = 1.23  H; Average depth of air diffuser = 4.80 m  α ; 0.97  P ; 760	$ \begin{array}{cccccccccccccccccccccccccccccccccccc$
	SOR	
Item		1)-6 Required Air Volime (Gs)

Item	Final Phase Calculation	Phase 1 Calculation
<ol> <li>Air volume for Membrane air scrubbing         Air volume of one(1) membrane unit         Number of unit         Total Air volume for membrane</li> </ol>	0.18 3.6 800 800 ×	0.18 3.6 400 400 ×
7. Aeration Blowers	= 144 Nm³/min/each tank	= 144 Nm³/min/each tank
7.1 Required Air Volume per each Tank		
for Aeration Tank for MBR Tank	consider about 10% allowance = $32 \times 1.1 = 35 \text{ Nm}^3/\text{min/each tank}$ = $144 \times 1.1 = 158 \text{ Nm}^3/\text{min/each tank}$	= 32 $\times$ 1.1 = 35 Nm <sup>3</sup> /min/each tank = 144 $\times$ 1.1 = 158 Nm <sup>3</sup> /min/each tank
7.2 Number of Actual working Blower per each tank		
for Aeration Tank N for MBR Tank N	= 1 unit = 4 unit	= 1 unit = 4 unit
ank	32 / 1 = 32	32 / 1 = 32
for MBK Tank G	= 144 / 4 = 30 m/min	= 144 / 4 = 30 m'/min
7.4 Discharge pressure 1) for Aeration Tank		
Diffuser Depth	= 4,800 mmAq	= 4,800 mmAq
Loss of Diffuser	= 1,100 mmAq	
Other loss		
1 otal Fressure 2) for MBR Tank	= 6,500 mmAq = 68 kpa	= 0,900 mmAq = 08 kpa
Diffuser Depth	= 4,800 mmAq	= 4,800 mmAq
Loss of Diffuser	= 750 mmAq	
Other loss	1,000 mmAq	1,000 mmAq
Total Pressure	= 6,550 mmAq = 64 kpa	= 6,550 mmAq = 64 kpa

Item	Final Phase Calculation	Phase 1 Calculation
7.5 Blower Specification  1) for Aeration Tank Type Diameter Capacity Discharge Pressure Motor Output Qantity 2) for MBR Tank Type Diameter	Blov	Blo
Capacity Discharge Pressure Motor Output Qantity	245 m³/min 5000 mmAq 450 kW 14 units (including stand-by 2 units)	245 m³/min 5000 mmAq 450 kW 7 units (including stand-by 1 units)
7.6 Excess Sludge pump Drawing Sludge Volume Number of Pump	$= 2,719   m^3/d = 1.9   m^3/min$ 1 pump per 1 train (5 Tanks)	$= 1,359   m^3/d = 0.9   m^3/min$
Type Diameter Capacity Discharge Pressure Motor Output Qantity	Non-clog type sludge pump  100 mm  1.0 m³/min  12 m  11 kW  8 nos. Including 4 standby	Non-clog type sludge pump  100 m³/min  1.0 m³/min  11 kW  4 nos. Including 2 standby

Item	Final Phase Calculation	Phase 1 Calculation
8. Dewatering Facility		
8.1 Design Condition		
Input Sludge Solids quantity Input Sludge Solids Concentration Input Sludge Volume Operation time Dehydrater Type Screen Diameter Filtration rate 8.3 Dehydrater Specifications Type Screen Diameter Input Sludge Concentration Filtration rate Operation time Dosage type Dosage type Dosage rate Total Motor power Numbers	= 24.462 t/d = 0.9 % = 2.718 m³/d = Pressing Rotary Outer Cylinder-type Screw Press (Hybrid type) = 800 mm = 24.462 / 0.32 / 23 = 3.3 = 4 nos = 800 mm = 24.462 / 0.32 / 23 = 3.3 = 4 nos = 320 kg-DS/hr = 320 kg-DS/hr = 320 kg-DS/hr = 1.7 kW/1 unit = 0rganic Polymer (Cation) = 1.7 kW/1 unit = 5 nos. including 1 standby	= 12.237 t/d = 0.9 % = 0.9 % = 2.3 hr/d = Pres sing Rotary Outer Cylinder-type Screw Press (Hybrid type) = 800 nm = 320 kg-DS/hr = 12.237 / 0.32 / 23 = 1.7 = 2 nos = 800 nm = 320 kg-DS/hr = 2.3 hr/d = 320 kg-DS/hr = 17.7 % = 17.

# (3) Main Equipment List

Facility: Lift Pump and Grit Chamber Facility

				Quantity	У	Motor	
No.	Equipment	Brief Spec.	Phase	Final	Phase	output	Remarks
140.	Equipment	Blief Spec.	I	Expan- sion	Total	(kW)	Remarks
1	Inflow gate	Electrical motor operation cast iron sluice gate 1.0mW x 2.0mH	2	2	4	3.7	
2	Coarse screen	Flat bar screen (manual raking) Spacing 100mm ( 2.5mW x4.8mH, 65 deg installation)	2	2	4	-	
3	Fine Screen	Single rake automatic Screen Spacing 15mm ( 2.5mW x 4.8mH, 70 deg installation)	2	2	4	2.2	
4	Lift pump	Vertical shaft Volute type mixed flow pump 700mm x 70.0m3/min x 34.5mH	3	2	5	570	Include 1 standby
5	Flow meter	Ultrasonic type Diameter 700mm	3	2	5		
6	Hoist	Electric motor operation hoist lifting capacity: 10 ton	1	0	1	5.0	(hoisting)
						0.85	(traveling)
8	Pump up well connection gate	Manual operation cast iron sluice gate 1.5 mW x 1.5 mH	1	0	1	-	
9	Outflow gate	Electrical motor operation cast iron sluice gate 1.0 mW x 1.2 mH	4	0	4	3.7	
10	Grit collector	Screw type collector screw diameter 300mm, length 15.0m	2	2	4	5.5	
11	Grit lifting Pump	Submersible sludge pump 80mm x 0.5m3/min x 10m	2	2	4	5.5	
12	Air Blower	Rotary Roots Blower 150mm x 11m3/min x 42kpa	2	1	3	22	Including 1 standby

Facility : Equalization Tank Facility

				Ç	uantity		Motor	
١,	No.	Equipment	Brief Spec.			Phase	output	Remarks
1	10.	Equipment	Bier spee.	Phase I	Expan-	Total	(kW)	TOTRIKS
					sion		(1111)	
	1	Inflow weir gate	Manual operation cast iron weir type gate	16	16	32		
			0.6 mW x 0.6 mH					
	2	Constant rate pump	Nonclogging pump	12	12	24	44	Phase1 including 4 standby
			250mm x 8.7m3/min x 14m					Final phase including 4 standby
	3	Mixer for antisettling	Submersible propeller type	80	80	160	2.8	
			propeller diameter 300mm					

Facility : Membrane Reactor process Facility

No.	Equipment	Brief Spec.	Phase I	Quantity Final Expansion	Phase Total	Motor output (kW)	Remarks
1	Ultra fine screen	Drum Screen Spacing 1mm capacity 420m3/h or more	10	10	20	0.4	
2							
3	Aerobic tank Air diffuser	Membrane panel type Whole floor aeration	10	10	20	-	
4	Aeration Blower for BOD removal	Turbo Blower (single stage oilless) 120m3/min x 68kps	5	5	10	220	Phase1 including 1 standby Final phase including 2 standby
5	Membrane unit	Hollow fiber membrane Membrane area 600m2/unit	20	20	40		
6	Permeate pump	centrifugal pump 11m3/min x 7m	10	10	20	22	
7	Aeration Blower for membrane scrubbing	Turbo Blower (single stage oilless) 245m3/min x 64kps	7	7	14	450	Phase1 including 11 standby Final phase including 2 standby
9	Excess Sludge Pump	Non-clog type Sludge Pump	4	4	8	11	Phase1 including 2 standby Final phase including 2 standby

Facility : Disinfection Facility

No.	Equipment	Brief Spec.	Phase	Quantity Final Expan- sion	Phase	Motor output (kW)	Remarks
2	Sodium hypochlorite storage tank Sodium hypochlorite dosing pump	Fiberglass plastic construction cylindrical tank 10m3  Diaphrag m pump 15mm dia. x 3.2L/min x 0.2MPa	2	4	8	0.4	Phase1 including 2 standby Final phase including 2 standby

#### Facility: Dewatering Facility

No.	Equipment	Brief Spec.	Phase		Phase	Motor output	Remarks
110.	Equiphent	Biol Spec.	I	Expan- sion	Total	(kW)	Remarks
1	Excess sludge mixer	Vertical shaft paddle mixer	2	0	2	11	
2	Excess sludge feed pump	Progressive cabity pump 125mm x 4.3 - 35m3/h	3	2	5	11	including 1 standby
3	Dehydrater	Pressing Rotary Outer Cylinder -type Screw Press (Hybrid type) Screen diameter 800 mm	3	2	5	1.7	including 1 standby

#### Facility: Electrical Facility

	Equipment	Brief Spec.	Quantity			Motor	
No.			Phase I	Final Phase		output	Remarks
				Expan- sion	Total	(kW)	
1	Transfformer	20kV/380V 50Hz 3,200kVA	2	2	4		
2	Generater	380V 50Hz 3,700kVA	1	1	2		
		Gas turbine engine 4,000Ps or more					

# (4) Hydraulic Calculation

#### 1. Basic Conditions

- (1) Name of WWTP JKT Central treatment Plant
- (2) Wastewater Treatment Process Conventional Activated Sludge
- (3) Design Flowrate

Desire Florente	Final 1	Phase	First Phase		
Design Flowrate	$m^3/d$	$m^3/s$	$m^3/d$	$m^3/s$	
Average Daily Flowrate	200,000	2.315	100,000	1.157	
Maximum Daily Flowrate	264,000	3.056	132,000	1.528	
	,,,,,,		,,,,,,		
Maximum Hourly Flowrate	400,000	4.630	200,000	2.315	

(4) Inflow Pipe

Pipe Diameter φ2,200mm

(5) Discharge

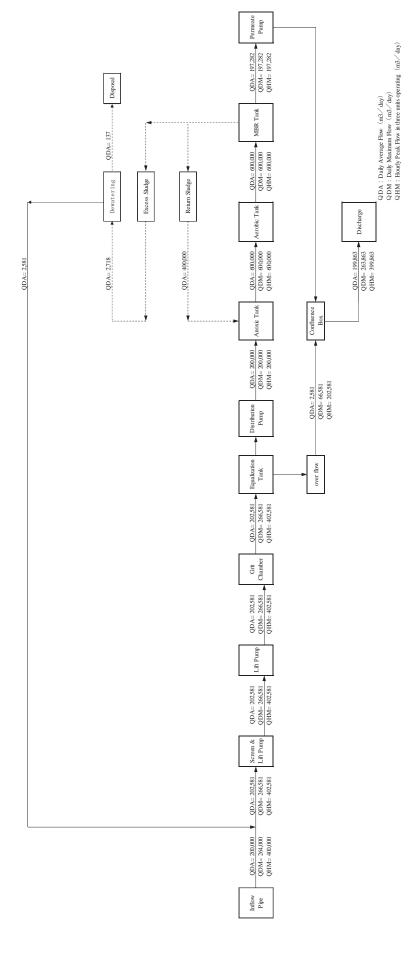
River BANJIL KANAL

Design High Water Level +4.30 M

(6) Planned Ground Level

Existing Ground Level  $+4.30~\mathrm{M} \sim$   $+4.50~\mathrm{M}$ 

Planned Ground Level +4.50 M



2. Water Mass Balance

		1			Formula:Manning
Content		Symbol		(	Case
		Byllicor	QDM	QHM	
Flowrate	$(m^3/d)$	Q	200,000	400,000	
riowrate	$(m^3/s)$	Q	2.315	4.630	
Number of pipes	(pipe)	n	1	2	
Flowrate/pipe	$(m^3/s)$	q	2.315	2.315	
Schematic			Box Culvert 2 seris	ses	
Diagram			Width	1	.5 m
			Depth	1	.5 m
Length	(m)	L	9	n=0.014	
Invert Elevation	(M)	FH	+1.500	11-0.014	
mvert Elevation	(1V1)	III	+1.300		
Water Depth	(m)	H'	1.500	1.500	Ho-FH
Cross Section	(m <sup>2</sup> )	A	2.25	2.25	110 111
Wetted Perimeter	(m)	P	6.00	6.00	
Hydraulic Radius	(m)	R	0.375	0.375	A/P
Velocity	(m/s)	V	1.029	1.029	q/A
Hydraulic Gradient	(‰)	I	0.767	0.767	$(n*V/R^{2/3})^2$
Down stream WL	(M)	Но	+4.300	+4.300	
Bown stream WE	(111)	110	11.300	11.200	
H Outflow	(m)		0.054	0.054	$1.0*(V^2/2g)*(Nm+1)$
E					
A Friction	(m)		0.007	0.007	I*L
L					
O S Inflow	(m)		0.027	0.027	$0.5*(V^2/2g)*(Nm+1)$
S	(111)		5.527	0.027	(1/25) (1/11/1)
Total	(m)	h	0.088	0.088	
Water Level	(M)	Н	+4.388	+4.388	Ho+h

	Formula:Manning
e	

				I		Formula:Manning
	Contents		Symbol		Ca	se
	Concents		2,111001	QDM	QHM	
	Flowrate	$(m^3/d)$	Q	100,000	200,000	
	Tiowrate	$(m^3/s)$	<u> </u>	1.157	2.315	
	Number of pipes	(pipe)	n	1	1	
	Flowrate/pipe	$(m^3/s)$	q	1.157	2.315	
Sch	ematic			Box Culvert 1 seris	es	
Dia	gram			Width	1.	5 m
				Depth	1.	5 m
	Length	(m)	L	130.00	n=0.014	
	Invert Elevation	(M)	FH	+1.500		
Wa	ter Depth	(m)	H'	1.500	1.500	Ho-FH
Cro	ss Section	(m <sup>2</sup> )	A	2.25	2.25	B*H'
We	tted Perimeter	(m)	P	6.00	6.00	2*H'+B
Нус	draulic Radius	(m)	R	0.375	0.375	A/P
X 7 1	•	3,	* 7	0.514	1.000	/^
	ocity	(m <sup>3</sup> /s)	V	0.514	1.029	p/A
Нус	draulic Gradient	(‰)	I	0.191	0.767	$(n*V/R^{2/3})^2$
Dov	wn stream WL	(M)	Но	+4.388	+4.388	
		• •				
H E	Outflow	(m)		0.013	0.054	$1.0*(V^2/2g)*1$
A	n			0.025	0.100	TANK
D	Friction	(m)		0.025	0.100	I*L
	Refraction (90°)	(m)		0.013	0.053	$0.99*(V^2/2g)*1$
L O	(50)	(***)		0.015	0.000	
S	Inflow	(m)		0.007	0.027	$0.5*(V^2/2g)*(Nm+1)$
S						
	Total	(m)	h	0.058	0.234	
	Water Level	(M)	Н	+4.446	+4.622	Ho+h
L	vv ater Level	(1/1)	п	<del>+-1.44</del> 0	T <b>4.</b> U∠∠	110 TII

	1 3 /	
Hormu	19.1/191	nnına
Formu	ia.iviai	шші

Contents   Symbol   Case   QDM   QHM
Compare   Comp
Number of pipes
Number of pipes
Flowrate/pipe (m³/s) q 0.000 0.579
Channel 1 serises   Width   1.0 m   Depth   2.0 m
Diagram   Width   1.0 m   Depth   2.0 m
Length
Depth   2.0 m
Length       (m)       L       73.80       n=0.014         Invert Elevation       (M)       FH       +2.500         Water Depth       (m)       H'       1.946       2.122       Ho-FH         Cross Section       (m²)       A       1.946       2.122       B*H'         Wetted Perimeter       (m)       P       4.89       5.24       2*H'+B         Hydraulic Radius       (m)       R       0.398       0.405       A/P
Invert Elevation         (M)         FH         +2.500           Water Depth         (m)         H'         1.946         2.122         Ho-FH           Cross Section         (m²)         A         1.946         2.122         B*H'           Wetted Perimeter         (m)         P         4.89         5.24         2*H'+B           Hydraulic Radius         (m)         R         0.398         0.405         A/P
Invert Elevation         (M)         FH         +2.500           Water Depth         (m)         H'         1.946         2.122         Ho-FH           Cross Section         (m²)         A         1.946         2.122         B*H'           Wetted Perimeter         (m)         P         4.89         5.24         2*H'+B           Hydraulic Radius         (m)         R         0.398         0.405         A/P
Invert Elevation         (M)         FH         +2.500           Water Depth         (m)         H'         1.946         2.122         Ho-FH           Cross Section         (m²)         A         1.946         2.122         B*H'           Wetted Perimeter         (m)         P         4.89         5.24         2*H'+B           Hydraulic Radius         (m)         R         0.398         0.405         A/P
Invert Elevation         (M)         FH         +2.500           Water Depth         (m)         H'         1.946         2.122         Ho-FH           Cross Section         (m²)         A         1.946         2.122         B*H'           Wetted Perimeter         (m)         P         4.89         5.24         2*H'+B           Hydraulic Radius         (m)         R         0.398         0.405         A/P
Invert Elevation         (M)         FH         +2.500           Water Depth         (m)         H'         1.946         2.122         Ho-FH           Cross Section         (m²)         A         1.946         2.122         B*H'           Wetted Perimeter         (m)         P         4.89         5.24         2*H'+B           Hydraulic Radius         (m)         R         0.398         0.405         A/P
Invert Elevation         (M)         FH         +2.500           Water Depth         (m)         H'         1.946         2.122         Ho-FH           Cross Section         (m²)         A         1.946         2.122         B*H'           Wetted Perimeter         (m)         P         4.89         5.24         2*H'+B           Hydraulic Radius         (m)         R         0.398         0.405         A/P
Invert Elevation         (M)         FH         +2.500           Water Depth         (m)         H'         1.946         2.122         Ho-FH           Cross Section         (m²)         A         1.946         2.122         B*H'           Wetted Perimeter         (m)         P         4.89         5.24         2*H'+B           Hydraulic Radius         (m)         R         0.398         0.405         A/P
Invert Elevation         (M)         FH         +2.500           Water Depth         (m)         H'         1.946         2.122         Ho-FH           Cross Section         (m²)         A         1.946         2.122         B*H'           Wetted Perimeter         (m)         P         4.89         5.24         2*H'+B           Hydraulic Radius         (m)         R         0.398         0.405         A/P
Invert Elevation         (M)         FH         +2.500           Water Depth         (m)         H'         1.946         2.122         Ho-FH           Cross Section         (m²)         A         1.946         2.122         B*H'           Wetted Perimeter         (m)         P         4.89         5.24         2*H'+B           Hydraulic Radius         (m)         R         0.398         0.405         A/P
Water Depth         (m)         H'         1.946         2.122         Ho-FH           Cross Section         (m²)         A         1.946         2.122         B*H'           Wetted Perimeter         (m)         P         4.89         5.24         2*H'+B           Hydraulic Radius         (m)         R         0.398         0.405         A/P
Cross Section         (m²)         A         1.946         2.122         B*H'           Wetted Perimeter         (m)         P         4.89         5.24         2*H'+B           Hydraulic Radius         (m)         R         0.398         0.405         A/P
Cross Section         (m²)         A         1.946         2.122         B*H'           Wetted Perimeter         (m)         P         4.89         5.24         2*H'+B           Hydraulic Radius         (m)         R         0.398         0.405         A/P
Wetted Perimeter         (m)         P         4.89         5.24         2*H'+B           Hydraulic Radius         (m)         R         0.398         0.405         A/P
Hydraulic Radius (m) R 0.398 0.405 A/P
3
Velocity $(m^3/s)$ V 0 0.273 p/A
Hydraulic Gradient (‰) I 0.000 0.049 (n*V/R <sup>2/3</sup> ) <sup>2</sup>
Down stream WL (M) Ho +4.446 +4.622
H Outflow (m) $0.000$ $0.004$ $1.0*(V^2/2g)*1$
E Outrow (III) 0.000 0.004 1.0 (V/2g) 1
A Friction (m) 0.000 0.004 I*L
D 116.000 0.000 1 E
L
S Inflow (m) 0.000 0.002 $0.5*(V^2/2g)*(Nm+1)$
S Total (m) h 0 001
Total (m) h 0 0.01
Water Level (M) H +4.446 +4.632 Ho+h

						Formula:Manning
	Contents		Symbol		Case	2
	Contents		Symbol	QDM	QHM	
	Flowrate	$(m^3/d)$		0	50,000	
	riowrate	$(m^3/s)$	Q	0	0.579	
	Number of pipes	(pipe)	n	1	1	
	Flowrate/pipe	$(m^3/s)$	q	0.000	0.579	
Scł	nematic			Channel 1 serises		
Dia	gram			Width	1.0	m
				Depth	2.0	m
	Length	(m)	L	17.00	n=0.014	
	Invert Elevation	(M)	FH	+2.500	11-0.014	
	mvert Elevation	(1V1)	I'II	+2.300		
Wa	ter Depth	(m)	H'	1.946	2.132	Но-FH
	oss Section	$(m^2)$	A	1.946	2.132	B*H'
	etted Perimeter	(m)	P	4.89	5.26	2*H'+B
	draulic Radius	(m)	R	0.398	0.405	A/P
113	aradic radias		- 10	0.370	0.102	
Ve	locity	$(m^3/s)$	V	0	0.272	p/A
Ну	draulic Gradient	(‰)	I	0.000	0.048	$(n*V/R^{2/3})^2$
D	XXI	$\Delta \Delta$	***	. 4.446	. 4. 622	
Do	wn stream WL	(M)	Но	+4.446	+4.632	
Н	Outflow	(m)		0.000	0.004	$1.0*(V^2/2g)*1$
Е				212.22		
A D	Friction	(m)		0.000	0.001	I*L
ע						
L						
О	T C	(- \ \		0.000	0.002	0.5*01210.1*01
S S	Inflow	(m)		0.000	0.002	$0.5*(V^2/2g)*(Nm+1)$
3	Total	(m)	h	0	0.007	
	1.5001	(211)	11	Ŭ	0.007	
	Water Level	(M)	Н	+4.446	+4.639	Ho+h

		I	I		Formula:Manning
Content	S	Symbol		Ca	ise
			QDM	QHM	
Flowrate	$(m^3/d)$	Q	0	25,000	
	$(m^3/s)$		0	0.289	
Number of pipes	(pipe)	n	1	1	
Flowrate/pipe	$(m^3/s)$	q	0.000	0.289	
la 1			Cl. 11		
Schematic			Channel 1 serises	1	0
Diagram			Width		0 m
			Depth	2.	0 m
Length	(m)	L	17.00	n=0.014	
				11=0.014	
Invert Elevation	(M)	FH	+2.500		
Water Depth	(m)	H'	1.946	2.139	Ho-FH
Cross Section	(m <sup>2</sup> )	A	1.946	2.139	B*H'
Wetted Perimeter	(m)	P	4.89	5.28	2*H'+B
Hydraulic Radius	(m)	R	0.398	0.405	A/P
Velocity	$(m^3/s)$	V	0	0.135	p/A
Hydraulic Gradient	(‰)	I	0.000	0.012	$(n*V/R^{2/3})^2$
Down stream WL	(M)	Но	+4.446	+4.639	
H Outflow	( )		0.000	0.001	1.04(1/2/2.)41
H Outflow	(m)		0.000	0.001	$1.0*(V^2/2g)*1$
A Friction	(m)		0.000	0.000	I*L
D Friction	(111)		0.000	0.000	I L
_					
L O					
S Inflow	(m)		0.000	0.000	$0.5*(V^2/2g)*(Nm+1)$
S					, , , ,
Total	(m)	h	0	0.001	
Water Level	(M)	Н	+4.446	+4.640	Ho+h

Point [Equalization Tank] Outflow Though

_					Formula: Thomas Gump		
Contents		Symbol	Case				
Contents		Symoor	QDM	QHM			
Flowrate	$(m^3/d)$	Q	0	50,000			
Towner	$(m^3/s)$	V	0.000	0.579			
Number of pipes	(pipe)	n	16	16			
Flowrate/pipe	$(m^3/s)$	q	0	0.0361875			
Schematic Diagram				Outlet Tr	rough		
			Outlet Pit		J		
			<u>⊉</u> H	∇FH	ho		
				L			
Length Width Invert Elevation	(m) (m) (M)	L B FH	8.000 0.400 +4.700	m			
Critical depth	(m)	hc	0	0.094	$(q^2/9.8*B^2)^{1/3}$		
Upper Water Depth	(m)	ho	0.000	0.163	$\sqrt{3}$ *hc		
Lower Water Level	(M)	Но	+4.446	+4.640			
H E A D L O S							
S Water Level	(M)	Н	+4.700	+4.863	FH+ho		

Point [Equalization Tank] Outflow V-notch

	,	T		Formula: Thompson	
Contents	Symbol	Case			
	Symbol	QDM	QHM		
Flowrate (m <sup>3</sup> /d)	Q	0	50,000		
$(m^3/s)$	<u> </u>	0	0.579		
Number of pipes (pipe)	n	1040	1040		
Flowrate/pipe (m <sup>3</sup> /s)	q	$0.000 \times 10^{-5}$	55.673 ×10-	5	
Schematic Diagram		₹ Ho	Final Sedimentat		
Length (m) Number of Notch  Weir Elevation (M)	D n FH	128.00 1040 +4.900	m (8.00 × 16) (8.0m / 0.125m =	- 64)	
Lower Water Level (M)	Но	+4.700	+4.863		
Complete Overflow  W D A E T P E T R H	h	0	0.043	(q/1.42)^(2/5)	
Water Level (M)	Н	+4.900	+4.943	FH+h	
Equalization Tank Base Level			100	H-8.00	

Point [Equalization Tank ] Inflow gate

					Formula:Manning
Contents		Symbol -			Case
Contents		Symbol	QDM	QHM	
Flowrate	$(m^3/d)$	Q	50,000	100,000	
Piowrate	$(m^3/s)$	V	0.579	1.157	
Number of pipes	(pipe)	n	8	8	
Flowrate/pipe	$(m^3/s)$	q	0.072	0.145	
Schematic					
Diagram					
Gate Width	(M)	В	0.400	m	
Bottom Elevation	(M)	FH	+4.400		
Water Depth	(m)	H'	0.500	0.543	Ho-FH
Cross Section	$(m^2)$	A	0.2	0.2172	B*H'
Velocity	$(m^3/s)$	V	0.362	0.666	q/A
D	(M)	11.	. 4.000	. 4.042	
Down stream WL	(M)	Но	+4.900	+4.943	
H Outflow	(m)		0.007	0.023	$1.0*(V^2/2g)*1$
E					
A					
D					
L					
O					
S					
S Inflow	(m)		0.003	0.011	$0.5*(V^2/2g)*(Nm+1)$
Total	(m)	h	0.003	0.034	
Water I1	(M)	11	. 4.002	. 4.077	ETT 15
Water Level	(M)	Н	+4.903	+4.977	FH+h

	3 6	
Formu	la ·M/La	nnıng
1 Offilia	ia.ivia	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,

						Formula:Manning	
Contents		Symbol	Case				
				Q DM Q HM			
Flowrate $(m^3/d)$ $(m^3/s)$		$(m^3/d)$	Q	25,000	50,000		
		$(m^3/s)$	~	0.289	0.579		
	Number of pipes	(pipe)	n	1	1		
	Flowrate/pipe	$(m^3/s)$	q	0.289	0.579		
Sch	ematic			Channel 1 serises			
Dia	gram			Width	1.5	m	
				Depth	2.0	m	
				· ·			
	Length	(m)	L	17.00	n=0.014		
	Invert Elevation	(M)	FH	+4.000			
						1	
Wat	ter Depth	(m)	H'	0.903	0.977	Ho-FH	
Cro	ss Section	(m <sup>2</sup> )	A	1.3545	1.4655	B*H'	
Wei	tted Perimeter	(m)	P	3.31	3.45	2*H'+B	
Нус	Iraulic Radius	(m)	R	0.410	0.424	A/P	
X7.1	9	(3/-)	* 7	0.212	0.205	/A	
	ocity	$(m^3/s)$	V	0.213	0.395	p/A	
Нус	Iraulic Gradient	(‰)	I	0.029	0.096	$(n*V/R^{2/3})^2$	
Dov	vn stream WL	(M)	Но	+4.903	+4.977		
		. ,		.,,,,,			
Н							
E A							
D	Friction	(m)		0.000	0.002	I*L	
L							
O							
S S							
5	Total	(m)	h	0	0.002		
		` /		-			
	Water Level	(M)	Н	+4.903	+4.979	Ho+h	

 $Point \ \ \big[ \ Equalization \ Tank \ \big] \ \ Distribution \ Channel \ (2)$ 

					Formula:Manning		
Contents		Symbol	Case				
Contents		Symbol	QDM	QHM			
Flowrate $(m^3/d)$ $(m^3/s)$		0	50,000	100,000			
		Q	0.579	1.157			
Number of pipes	(pipe)	n	1	1			
Flowrate/pipe	$(m^3/s)$	q	0.579	1.157			
Schematic			Channel 1 serises				
Diagram			Width	1.5	m		
			Depth	2.0	m		
Length	(m)	L	17.00	n=0.014			
Invert Elevation	(M)	FH	+4.000				
Water Depth	(m)	H'	0.903	0.979	Но-FH		
Cross Section	$(m^2)$	A	1.3545	1.4685	B*H'		
Wetted Perimeter	(m)	P	3.31	3.46	2*H'+B		
Hydraulic Radius	(m)	R	0.410	0.425	A/P		
	. 3.		0.425	0.500			
Velocity	(m <sup>3</sup> /s)	V	0.427	0.788	p/A		
Hydraulic Gradient	(‰)	I	0.117	0.381	$(n*V/R^{2/3})^2$		
Down stream WL	(M)	Но	+4.903	+4.979			
Н							
E A Existing							
D Friction	(m)		0.002	0.006	I*L		
L							
O S Inflow	(m)		0.005	0.016	$0.5*(V^2/2g)*(Nm+1)$		
S	. /						
Total	(m)	h	0.007	0.022			
***	/3 E		. 4.010	5.001	XX . 1		
Water Level	(M)	Н	+4.910	+5.001	Ho+h		

Point [Equalization Tank to Grit chamer] Connection conduit (1)

						Formula:Manning		
	Contents		Symbol	Case				
	Contents		Byllioor	Q DM Q HM				
Flowrate (m <sup>3</sup> /d)		Q	100,000	200,000				
	Nowrate	$(m^3/s)$	Q	1.157	2.315			
	Number of pipes	(pipe)	n	1	1			
	Flowrate/pipe	$(m^3/s)$	q	1.157	2.315			
Sch	ematic			Channel 1 serises				
Dia	gram			Width	1.5	5 m		
				Depth	1.5	5 m		
	Length	(m)	L	42.00	n=0.014			
	Invert Elevation	(M)	FH	+1.500	11-0.011			
	Invert Lie vation	(171)		11.500				
Wa	ter Depth	(m)	H'	1.500	1.500	Но-FН		
Cro	ss Section	(m <sup>2</sup> )	A	2.25	2.25	B*H'		
We	tted Perimeter	(m)	P	6.00	6.00	2*H'+B		
Нус	draulic Radius	(m)	R	0.375	0.375	A/P		
		. 3.		0.711	1.000			
	ocity	(m <sup>3</sup> /s)	V	0.514	1.029	p/A		
Нус	draulic Gradient	(‰)	I	0.191	0.767	$(n*V/R^{2/3})^2$		
Dov	wn stream WL	(M)	Но	+4.910	+5.001			
Н	Outflow	(m)		0.013	0.054	$1.0*(V^2/2g)*(Nm+1)$		
E A								
D	Friction	(m)		0.008	0.032	I*L		
L								
0	Inflow	(m)		0.007	0.027	$0.5*(V^2/2g)*(Nm+1)$		
S S	11111044	(111)		0.007	0.027	0.5 (* /25) (14III+1)		
Ĺ	Total	(m)	h	0.028	0.113			
Water Level (M)		Н	+4.938	+5.114	Ho+h			

Point [Grit Chamber] Outfall weir

						Formula: Francis		
	Contents		Symbol	Case				
			Symbol	Q DM Q HM				
	Flowrate $(m^3/d)$ $(m^3/s)$		Q	200,000	400,000			
			Q	2.315	4.63			
Nu	mber of pipes	(pipe)	n	4	4			
Fle	owrate/pipe	$(m^3/s)$	q	0.579	1.158			
Schem		(m) (M)	B FH	3.00 +5.200	♥ H  ▼ FH	▼ H		
Lower								
Water	1	(M)	Но	+4.938	+5.114			
W D A E T P E T R H		(m)	h	0.222	0.353	(q/1.84*L)^(2/3)		
W	Vater Level	(M)	Н	+5.422	+5.553	FH+h		
	- · · · · · ·	. /						

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Formu	ia.iviai	шші

					Formula:Manning	
	Contents		Symbol		Case	) I
				QDM	QHM	
	Flowrate	$(m^3/d)$	Q	200,000	400,000	
	$(m^3/s)$		*	2.315	4.630	
	Number of pipes	(pipe)	n	4	4	
	Flowrate/pipe	$(m^3/s)$	q	0.579	1.158	
Sch	ematic			Channel 1 serises		
	gram			Width	1.0	m
	8			Depth		
				Бериг	2.0	
	Length	(m)	L	4.50	n=0.014	
	Invert Elevation	(M)	FH	+4.000		
Wa	ter Depth	(m)	H'	1.422	1.553	Ho-FH
Cro	oss Section	(m <sup>2</sup> )	A	1.422	1.553	B*H'
	tted Perimeter	(m)	P	4.34	4.61	2*H'+B
	draulic Radius	(m)	R	0.327	0.337	A/P
11)	artune rtudius	(111)		0.327	0.337	1 1/1
Vel	locity	$(m^3/s)$	V	0.407	0.746	p/A
Нус	draulic Gradient	(‰)	I	0.144	0.465	$(n*V/R^{2/3})^2$
Do	wn stream WL	(M)	Но	+5.422	+5.553	
Н	Outflour	(100)		0.000	0.020	1.0*(N/ <sup>2</sup> /2~)*(N/·····1)
E	Outflow	(m)		0.008	0.028	1.0*(V <sup>2</sup> /2g)*(Nm+1)
A	Friction	(m)		0.001	0.002	I*L
D	TICTOIL	(111)		0.001	0.002	1 2
L						
O						
S	Inflow	(m)		0.004	0.014	$0.5*(V^2/2g)*(Nm+1)$
S						
	Total	(m)	h	0.013	0.044	
Pu	mp Discharge Well	(3. f.)	**	.5.425	. 5 505	TT . 1
Water Level (M)		Н	+5.435	+5.597	Ho+h	

Point [Grit Chamber] Inflow Channel

					Formula: Francis		
Contents		Symbol	Case				
Contents			QDM	QHM			
Flowrate	$(m^3/d)$	Q	150,000	200,000	QDM=Q*1/4, QHM=Q*1/3		
(m <sup>3</sup> /s)		V	1.736	2.315			
Number of pipes	(pipe)	n	2	2			
Flowrate/pipe	$(m^3/s)$	q	0.868	1.158			
Schematic Diagram			₩ Ho	y H √FH	D H D FH h		
Weir Width	(m)	L	0.50				
Weir Height	(M)	FH	+4.000				
Lower							
Water Level	(M)	Но	+5.422	+5.553			
Complete							
W D Overflow	(m)	h	0.962	1.165	(q/1.84*L)^(2/3)		
A E							
T P E T							
R H							
Water Level	(M)	Н	+4.962	+5.165	FH+h		
Base Level			+4.	000			

Results of Survey for Affordability and WTP of Household 3.

# [Outline of Affordability Survey]

Affordability: To evaluate affordability by examining the cost of relevant consumables in household expenditure as same value as exploited by water environment improvement.

# [Outline of WTP, Willingness to Pay, Survey]

#### **Background of WTP and CVM**

CVM (Contingent Valuation Method): to valuate extensive role of infrastructure, value of which is not prevalent to commercial market.

Role of Sewerage System: to improve sanitation and public health, to mitigate storm water inundation, to conserve water environment and water resource, and to create aesthetic urban view and amenity spot

WTP (Willingness to Pay): to evaluate the willingness to pay examined by statistical analysis of contingent valuation, CVM questionnaire survey

#### Study Scheme and Procedure of Survey

RDD (Random Digit Detecting) extracts recipients and questionnaire is mailed.

Survey area: Planned Central Sewerage Served Area in Jakarta

Sampling number: 60 samples, which is equivalent to 87% of reproducibility

Questionnaire is designed to "Double-Bounded Dichotomous Choice" and elaborated by pre-test.

Contingent situation of "Assumption of water environment" is carefully explained to Questionee since "Relation of water environment and role of sewerage" or "Before/after of sewerage system" is sometimes difficult to be understood.

Table-1 Presented Amount to Double-Bounded Dichotomous Choice of CVM Survey

Bound		Presented Amount (IDR. /month/household)										
1st bound	5,0	000	10,	000	15	,000	20,	000	30,	000	50	,000
2 <sup>nd</sup> bound	No	Yes	No	Yes	No	Yes	No	Yes	No	Yes	No	Yes
2 bound	2,500	7,500	5,000	15,000	10,000	20,000	15,000	25,000	20,000	40,000	30,000	70,000
Sample Number	1	0	1	0		10	1	0	1	0		10

## (Reference)

Table-2 Sample Number of household of PD PAL Jaya customers classified in monthly income.

Group	Type A	Type B	Type C	Type D	Total
Electricity consumption	450 <b>~</b> 900VA	900 <b>~</b> 1,300VA	1,300 <b>~</b> 2,200VA	2,200VA~	-
Number of Customer of PD Pal Jaya	896 (79%)	196 (17%)	20 (2%)	25 (2%)	1,137
Income Level	Low	Middle	High	h	-

Sample number of 60 is allocated to three income levels as same rate as PD Pal Jaya customer Type A through Type D.

(Reference No.	_)
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# Questionnaire "Affordability & WTP" Survey on

# Wastewater Management in DKI Jakarta

# **Part-1 General Information of Questionee**

Address	Kec. Kel.
No. of Family Member:	persons Area of Floor: m <sup>2</sup>
Monthly Household Inco	ome <u>IDR/month</u>
Monthly Household Inco	ome <u>IDR/month</u>
Immovable property tax	(Asset tax) <u>IDR/year</u>
Electricity Consumption	n ( 🗸 )
Type-A	Type-B Type-C Type-D
( ) 450-900 VA	( ) 900-1,300 VA ( ) 1,300-2,200 VA ( ) 2,200 VA ~
<b>)</b> -1	
Which is your water sup	oply? ( ) Piped(Tap) water ( ) Well ( ) Both
How much do you pay f	for water supply? <u>IDR/month</u>
How many and how mu	ich pay of 5 gallon-bottled water? <u>bottles</u> <u>IDR/month</u>
110 W many and now mu	but by or 5 gamon bottled water.
Tow many and now mu	bottles in pay of 3 gainst bottled water.
2-2	ion pay of 3 gailon bottled water.
<b>)-</b> 2	pollution caused by wastewater discharge of household and mur
<b>)-2</b>	
<b>)-2</b> Do you know water p	
<b>)-2</b> Do you know water p	pollution caused by wastewater discharge of household and mur
<b>)-2</b> Do you know water p	pollution caused by wastewater discharge of household and mur
Do you know water pactivities?	pollution caused by wastewater discharge of household and mur
Do you know water pactivities?	pollution caused by wastewater discharge of household and mur  Yes( ) No( )
Do you know water pactivities?	pollution caused by wastewater discharge of household and mur  Yes( ) No( )  f sewerage system as followings?  Yes( ) No( )
Do you know water pactivities?  Do you know the role of Roles of sewerage systems.	pollution caused by wastewater discharge of household and mur  Yes( ) No( )  f sewerage system as followings?  Yes( ) No( )
Do you know water pactivities?  Do you know the role of Roles of sewerage synto improve sanita	pollution caused by wastewater discharge of household and mur  Yes( ) No( )  f sewerage system as followings?  Yes( ) No( )
Do you know water pactivities?  2-3 Do you know the role of  Roles of sewerage system improve sanitation mitigate storm	pollution caused by wastewater discharge of household and mur  Yes( ) No( )  f sewerage system as followings?  Yes( ) No( )  yetem:  ation and public health,

### Part-2 Affordability Survey and WTP (Willingness to Pay) Survey

Please answer to Q-4 and Q-5&6, on the assumption that water environment is polluted and sewerage system restores as follows.

Present water environment is (D-level)

Polluted and offensive odor

Debris deposits

Unhygienic and caused habitual diarrhea

After sewerage system improves water quality level to:

Boatable (C)

Fishable (B)



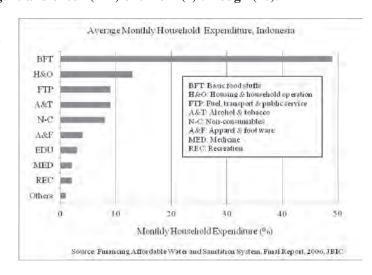
Water cleaned up to "Boatable"

	Relation between water quality level and water life							
→ Polluted	D - level Polluted Offensive odor	A WAY						
_	C - level Raise to boatable/ fishable level							
Water quality level	B - level  Raise to playing in water /bio- diversity level							
Clean ←	A - level Raise to swimmable							

## [Affordability Survey]

#### Q-4: How much do you pay? Please refer Figure and check ( $\checkmark$ ) one from (1) through (10).

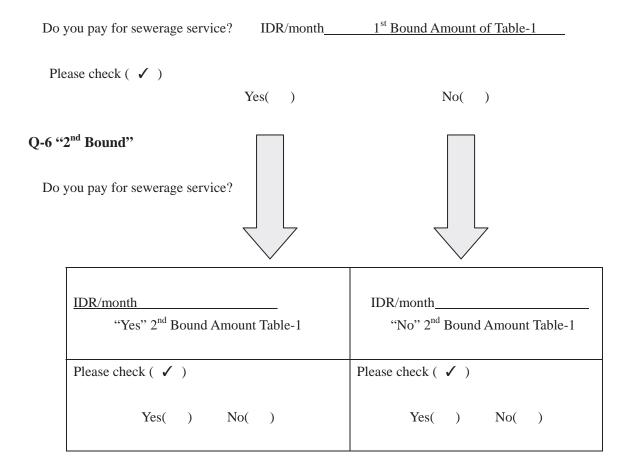
- (1) BFT: Basic food stuffs
- (2) H&O: Housing & household operation
- (3) FTP: Fuel, transport & public service
- (4) A&T: Alcohol & tobacco
- (5) N-C: Non-consumables
- (6) A&F: Apparel & foot ware
- (7) MED: Medicine
- (8) REC: Recreation
- (9) Others
- (10) No



# [WTP, Willingness to Pay, Survey]

# How much do you pay for sewerage service?

# Q-5 "1st Bound"



No.	Kecamatan	Kelurahan	Family Member	Floor Area (m²)	Household Income (IDR/month)	Household Expenditure (IDR/month)	Asset Tax (IDR/year)	
1	Tanah Abang	Kebon Kacang	13	300	3,000,000	1,000,000	2,000,000	
2	Sawah Besar	Kartini	5	42	3,000,000	2,400,000	100,000	
3	Sawah Besar	Kartini	8 4	40	1,000,000	800,000	100,000 N/ <i>A</i>	
<u>4</u> 5	Tambora Taman Sari	Tambora Glodok	6	6 18	1,500,000 1,000,000	1,300,000 1,000,000	50,000	
6	Gambir	Petoio Selatan	15	105	4,400,000	4,400,000	N/A	
7	Penjaringan	Pejagalan	6	60	900,000	900,000	137,500	
8	Menteng	Kebon Sirih	5	42	3,000,000	2,500,000	70,000	
9	Menteng	Kebon Sirih	5	72	1,000,000	1,000,000	150,000	
10	Menteng	Kebon Sirih	4 2	56	1,500,000	1,300,000	220,000	
11	Penjaringan Gambir	Pejagalan Petojo Utara	7	60 90	800,000 4,000,000	800,000 3,500,000	205,000 90,000	
13	Taman Sari	Glodok	3	20	1,500,000	1,000,000	90,000 N//	
14	Tambora	Tambora	7	42	3,000,000	2,000,000	100,000	
15	Sawah Besar	Kartini	3	60	2,000,000	1,500,000	400,000	
16	Sawah Besar	Kartini	4	20	2,000,000	2,000,000	100,000	
17	Penjaringan	Pejagalan	4	60	2,000,000	2,000,000	160,000	
18 19	Menteng Menteng	Cikini Cikini	5 10	20 28	1,500,000 4,500,000	1,500,000 3,000,000	120,000 120,000	
20	Menteng	Cikini	6	60	2.000.000	2,000,000	200,000	
21	Menteng	Cikini	5	28	900,000	900,000	50,000	
22	Tanah Abang	Kebon Kacang	3	50	5,000,000	4,500,000	100,000	
23	Tambora	Tambora	2	6	450,000	450,000	N/A	
24	Taman Sari	Glodok	2	16	600,000	600,000	N/A	
25	Sawah Besar	Kartini	7	60	5,200,000	4,600,000	150,000	
26 27	Sawah Besar Gambir	Kartini Petojo Selatan	3 6	24 50	1,000,000 3,000,000	1,000,000 1,500,000	60,000 60,000	
28	Penjaringan	Pejagalan	8	75	3,000,000	2,700,000	00,000 N/A	
29	Penjaringan	Pejagalan	4	75	3,000,000	2,000,000	300,000	
30	Penjaringan	Pejagalan	4	175	1,500,000	1,500,000	N/A	
31	Menteng	Cikini	9	56	2,000,000	2,000,000	185,000	
32	Menteng	Cikini	8	90	2,000,000	2,000,000	300,000	
33	Tanah Abang Tambora	Kebon Kacang	5	60	2,000,000	1,500,000	N/A	
34 35	Tambora Taman Sari	Tambora Glodok	4	10 12	700,000 600,000	500,000 600,000	160,000 N/A	
36	Sawah Besar	Kartini	11	30	1,000,000	1,000,000	60,000	
37	Sawah Besar	Kartini	5	9	1,000,000	900,000	30,000	
38	Gambir	Petojo Utara	5	54	3,000,000	3,000,000	80,000	
39	Penjaringan	Pejagalan	7	195	4,500,000	4,000,000	240,000	
40	Menteng	Cikini	12	100	9,000,000	8,000,000	200,000	
41 42	Menteng Penjaringan	Cikini Pejagalan	6 5	50 60	1,500,000 1,500,000	1,500,000 1,200,000	135,000 200,000	
43	Penjaringan	Pejagalan	3	78	1,500,000	1,000,000	265,000	
44	Tambora	Tambora	3	9	1,200,000	1,200,000	N/A	
45	Taman Sari	Glodok	8	16	1,800,000	1,800,000	N/A	
46	Sawah Besar	Kartini	4	9	800,000	600,000	N/A	
47	Tanah Abang	Kebon Kacang	4	60	1,500,000	1,000,000	200,000	
48 49	Sawah Besar Gambir	Kartini Petojo Utara	5 5	36 120	2,000,000 3,000,000	1,800,000 2,500,000	250,000 320,000	
50	Penjaringan	Pejagalan	4	48	3,000,000	3,000,000	200,000	
51	Penjaringan	Pejagalan	9	48	4,000,000	3.800.000	250,000	
52	Menteng	Cikini	6	200	2,500,000	2,500,000	1,000,000	
53	Penjaringan	Pejagalan	5	35	3,000,000	2,500,000	N/A	
54	Menteng	Kebon Sirih	9	46	3,000,000	3,000,000	125,000	
55	Tambora	Tambora	4	6	950,000	800,000	N/A	
56 57	Tanah Abang Taman Sari	Kebon Kacang Glodok	5 2	160 30	1,500,000 3,000,000	1,500,000 3,000,000	400,000 50,000	
58	Sawah Besar	Kartini	5	18	1,500,000	1,300,000	60,000	
59	Sawah Besar	Karang Anyar	4	24	800,000	800,000	40,000	
60	Gambir	Petojo Utara	6	60	3,000,000	2,000,000	150,000	
61	Menteng	Kebon Sirih	5	45	6,000,000	4,000,000	190,000	
62	Penjaringan	Pejagalan	6	90	7,000,000	6,000,000	400,000	
63	Penjaringan	Pejagalan	4	140	10,000,000	7,000,000	400,000	
64 65	Penjaringan Menteng	Pejagalan	8 5	120	8,000,000	6,000,000 20,000,000	560,000 300,000	
(17.7)	uvientens	Cikini	1 3	200	20,000,000	ZU.UUU.UUUI	300.0	

	E1.	ctricity C	oneumn+	on I			-	Q-1		
	Ele	ectricity C	onsumpti	on I	14/	iter Source			D.A.L.	I W-4
No.	450-	900-	1300-	>2200vA	Piped			Water supply cost	Quantity	Water Cost
	900vA	1300vA	2200vA	/ 2200 (//	water	Well	Both	(IDR/month)	(gallon/month)	(IDR/month)
1			1			1		0	15	60,00
2	1					1		0	4	52,00
3		1					1	180,000	0	
4	1					1		0	4	16,00
5	1				1		·····	60,000	0	
6			1		1		····	100,000	12	144,00
7	1				1			80,000	0	
8	1		***************************************				1	70,000	6	72,0
9	1						1	12,000	0	
10	1		1		1		1	120,000	0	
11			1		1			17,000 200,000	0 15	100.0
12 13	1					1		200,000	8	180,00 96,00
14			1	***************************************	1	I		100,000	2	22,0
15	1		I				1	46,000	3	36,0
16	1					1		0	10	110,0
17			1				1	65,000	0	110,0
18		1	<u> </u>				1	35,000	6	78.0
19		1				1	>>>>=>>>=>>>>>>>>>>>>>>>>>>>>>>>>>>>>>	0	8	104,0
20	•	1				1	***************************************	0	10	35,0
21	•		1			1		0	0	
22		1			1		00000B000000000B000000000000	225,000	8	96,0
23	1				1			9,000	3	36,0
24	1					1		0	8	28,0
25	1					1		0	3	13,5
26	1						1	50,000	0	
27		1	***************************************		1		00000000000000000000000000000000000000	50,000	0	
28	***************************************	1		***************************************	1		***********************	150,000	0	
29				1	1		·	120,000	0	
30			1		1			45,000	0	
31		1					1	60,000	9	99,0
32			1				1	50,000	10	120,0
33	1.				1		***************************************	200,000	8	24,0
34			1				1	15,000	2	24,0
35	1					1		0	0	
36		1			1			50,000	0	
37	1	4					1	30,000	0	000
38		1			1			25,000	8	96,0
39 40		1	1				1	150,000	8	92,0
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41 42			1			l l	1	17,000	3 0	30,0
43		***************************************	1				1			
44	1	***************************************	I				1	20,000	4	16,0
45	1					1		20,000	4	48,0
46	1						1	90,000	8	25,5
47		1	***************************************		1	***************************************		250,000	3	26,0 36,0
48	1					1		230,000	10	120,0
49		1			1	······································		100,000	6	72,0
50			1		1		a	100,000	10	120,0
51		***************************************	1		1			200,000	0	
52				1		1		0	5	60,0
53			1		1			100,000	5	60,0
54		1					1	45,000	2	26,0
55	1					1		0	4	16,0
56		1			1		***************************************	300,000	12	48,0
57		1					1	30,000	2	24,0
58		1					1	30,000	0	
59	1						1	70,000	20	70,0
60	1				1			200,000	16	192,0
61			1				1	100,000	8	88,0
62				1	1			150,000	10	110,0
63			***************************************	1	1			300,000	12	132,0
64		000000000000000000000000000000000000000	200200200200200200200200200200200200200	1	1		000000000000000000000000000000000000000	200,000	15	82,5
65				1	1			125,000	10	120,0

	Q-	2	Q-3						Q-	-4				
No.	Yes	No	Yes	No	BFT	н&о	FTP	А&Т	N-C	A&F	MED	REC	OTHER S	NO
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2		1	1											
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6 7	1	E0000000000000000000000000000000000000	1	4							1			
	1			1							4		1	
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11	1	***************************************	1							1				
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32	1		1					1						
33	1		1			1								
34	1		1			,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,						,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,		
35	1		1											
36	1	***************************************	1										1	
37	1		1									1		
38		1	1			1								
39		1		1							1			
40	1			1								1		
41	1		1					1						
42	1	***************************************		1		***************************************						***************************************		
43	1	•	1										1	
44	1	4	1			***************************************							4	>>>>>>>>
45	4	1	-1	1									1	
46	1	***************************************	1								4			
47 48	1		1					1			1			
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51	1	***************************************	1	I										
52	1		1										1	**********
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56	-	<u>.</u>	1								<b></b>		1	
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58	1		1			1								
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60		1	1					***************************************			1	***************************************		
61	1			1		1								
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63	1			1		1								
64	1	***************************************		1									1	
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			(4/4)						
	Q-5		Q-	6a	Q-	6b			
No.	Yes	No	Yes	No	Yes	No	1 stbound		
1	1		1				5,000		
2		1				1	5,000		
3	1		1				5,000		
4	1		1				5,000		
5 6	1 1		1 1				5,000 5,000		
7	1	20000000E000000000E0	1				5,000		
8	1		1				5,000		
9	1		1				5,000		
10	1		1				5,000		
11	1		1				5,000		
12 13	1	1	1			1	10,000 10,000		
14	1	<u> </u>	1				10,000		
15		1				1	10,000		
16	1		1				10,000		
17	1			1			10,000		
18	1	000000000000000000000000000000000000000	1		000000000000000000000000000000000000000		10,000		
19	1		1				10,000		
20	1 1			1 1			10,000		
21 22	1		1		***************************************		10,000 15,000		
23		1			***************************************	1	15,000		
24		1			***************************************	1	15,000		
25	1		1				15,000		
26	1	oao	1			•••••	15,000		
27	1			1			15,000		
28 29	1		1	1			15,000		
30	1			1	***************************************		15,000 15,000		
31		1				1	15,000		
32	1		1		***************************************	***************************************	15,000		
33	1		1				20,000		
34		1				1	20,000		
35	-	1	4			1	20,000		
36 37	1 1		1 1				20,000 20,000		
38	1		1				20,000		
39	1			1			20,000		
40	1			1			20,000		
41	1			1			20,000		
42		1				1	20,000		
43	1	4		1		4	20,000		
44 45	1	1	***************************************	1		1	30,000 30,000		
46		1				1	30,000		
47	1		***************************************	1	***************************************		30,000		
48		1				1	30,000		
49		1				1	30,000		
50		1				1	30,000		
51		1				1	30,000		
52 53	1 1		1	1			30,000 30,000		
54	1		1				30,000		
55	1	***************************************	1	***************************************	***************************************		50,000		
56		1				1	50,000		
57		1				1	50,000		
58		1			1		50,000		
59		1				1	50,000		
60 61	1	1		1		1	50,000		
62	1		***************************************	1			50,000 50,000		
63	1		1	<u>.</u>			50,000		
64	1			1			50,000		
65	1			1			50,000		

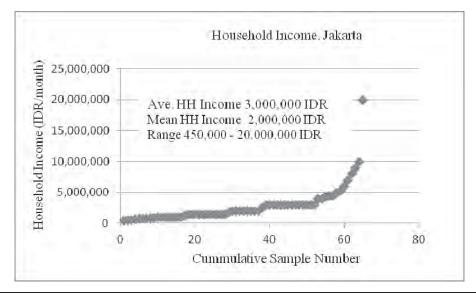
#### **Household Income**

Average: 3,000,000 IDR/HH/month

Mean Value: 2,000,000 IDR/HH/month

 $75\ \%$  of Existing Survey "5,295 USD  $\ (2008,Mizuho\ Research$  Institute." which does not include

apartment



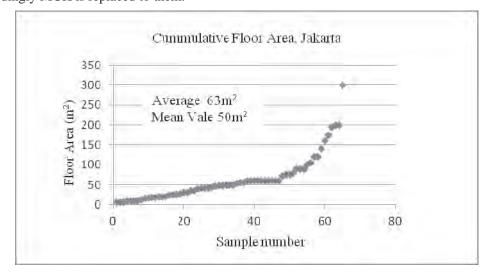
#### **House Floor Area**

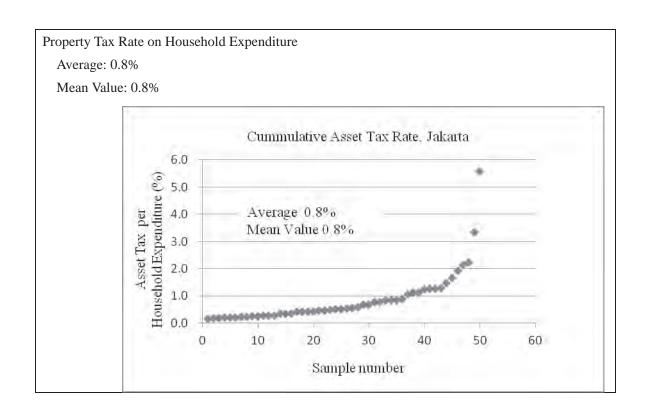
Average: 63 m<sup>2</sup>/HH

Mean Value:  $50\text{m}^2/\text{HH}$ 

Modest household lives in single room house which does not equip toilet, bath room or laundry.

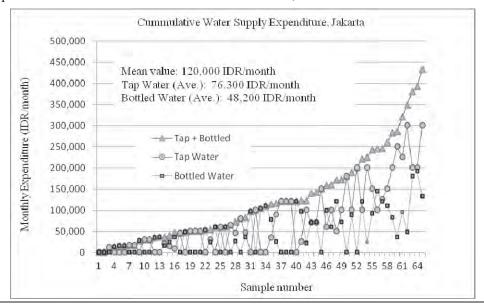
Accordingly MCK is replaced to them.





### Water Supply Cost of Tap water and Bottled water

Average monthly cost: 124,500 IDR/HH/month, Mean value: 124,000 IDR/HH/month Tap water: 76.300 IDR/HH/month Bottled water: 48,200 IDR/HH/month

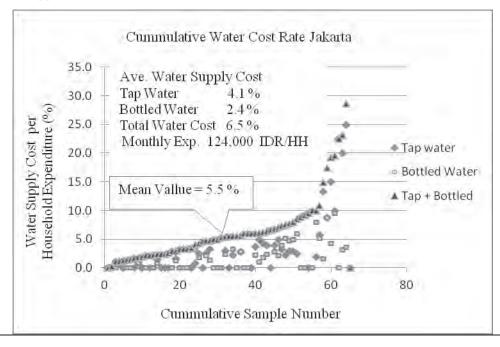


#### Rate of Water supply cost of Tap water & Bottled water on Household expenditure

Average: 6.5% (Tap water 4.1 % and Bottled water 2.4 %)

Mean vale: 5.5%

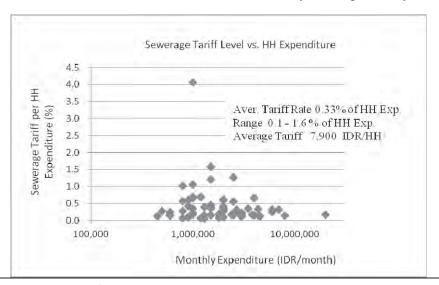
Water supply cost (Average) is 124,000 IDR/HH/month



#### Sewerage tariff rate per Household expenditure

Average: 0.33%×HH Exp. Range: 0.1~1.6 %×HH Exp.

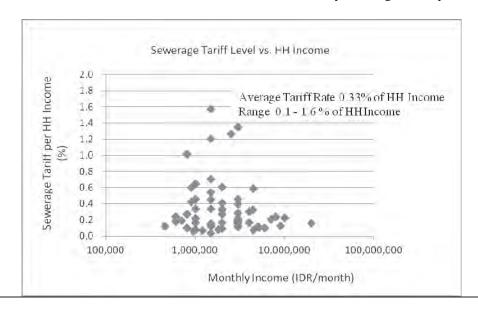
(Estimated by Existing Tariff System)



### Sewerage tariff rate per Household income

Average: 0.33%×HH Income Range: 0.1~1.6%×HH Income

(Estimated by Existing Tariff System)

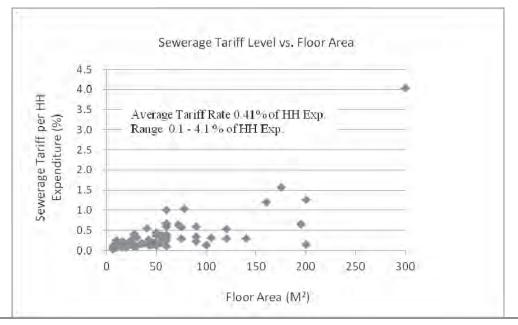


### Sewerage tariff rate per Household expenditure classified by floor area

Average: 0.41%×HH Exp.

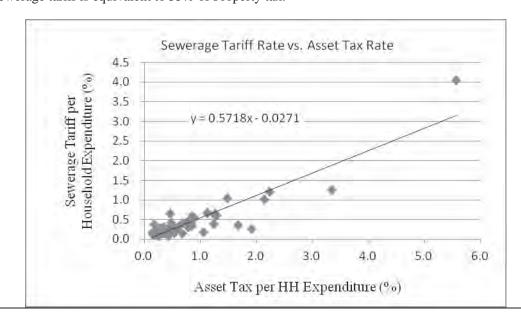
Less than average floor area ( $63\,\mathrm{m}^2$ ): 1.0% ×HH Exp.

Less than mean value (50m<sup>2</sup>): 0.5%×HH Exp.



### Sewerage tariff rate per Property tax

Sewerage tariff is equivalent to 55% of Property tax.



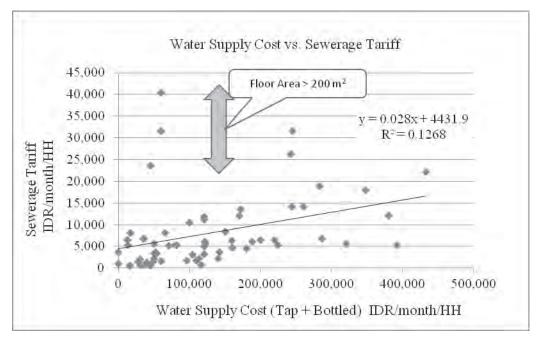
#### Sewerage tariff rate vs. Water supply cost

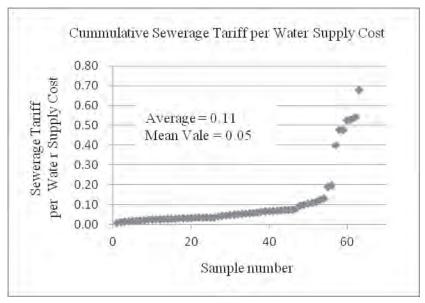
Sewerage tariff rate is equivalent to 2.8% of water supply cost plus 4,400 IDR.

Sewerage tariff per water supply : Average 11% Mean value 5%

High rate tariff household: House owner of large house area more than 200m<sup>2</sup>.

Only shallow well user (2HH of surveyed 65HH): Sewerage tariff is newly levied, accordingly poverty mitigation is required.

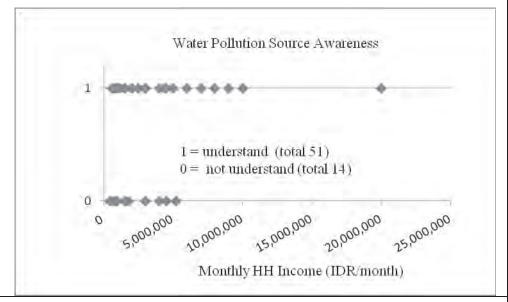




#### **Understanding on Water pollution source**

Yes "understanding": 78% No "not understanding": 22%

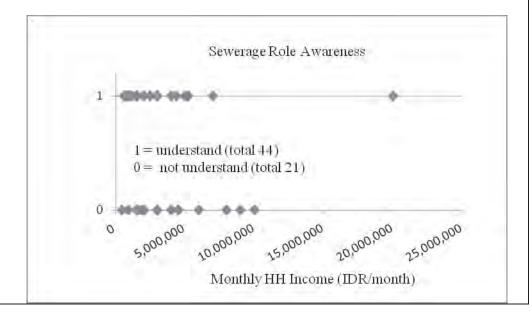
Respondents range over low income to high income.

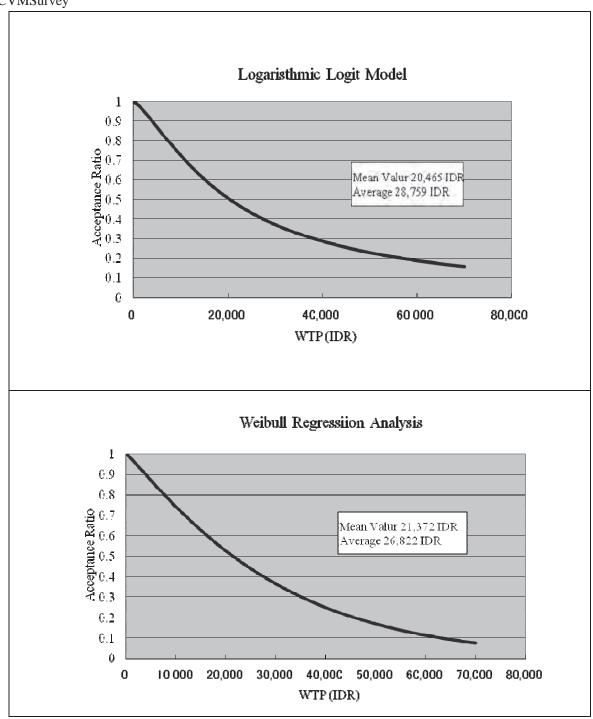


### **Understanding on Sewerage**

Yes "understanding": 68% No "not understanding": 32%

Respondents range over low income to high income.





#### **Affordability Survey**

Among respondents of 65 HH,

Yes "have willingness": 78.5% of Alcohol & tobacco, medicine and recreation.

Most frequent response is "Others".

No "have not willingness": 21.5%

AFT: Approx. 1.0-1.5% of household expenditure.

	Q-4 (ATP)													
Responded Nos.& %	BFT	Н&О	FTP	A&T	N-C	A&F	MED	REC	OTHERS	NO				
Total = 65	0	13	1	4	3	3	5	4	18	14				
%	0	0.2	0.015	0.062	0.046	0.046	0.077	0.062	0.277	0.215				

#### Questionnaire

(1) BFT: Basic food stuffs

(2) H&O: Housing & household operation

(3) FTP: Fuel, transport & public service

(4) A&T: Alcohol & tobacco

(5) N-C: Non-consumables

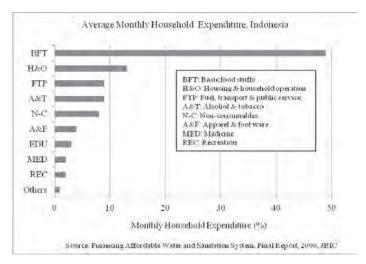
(6) A&F: Apparel & foot ware

(7) MED: Medicine

(8) REC: Recreation

(9) Others

(10) No



#### Affordability (Conclusion)

- Sewerage tariff (Ave. 7,900 IDR/HH) is 2.4 % of water supply cost (Tap water & Bottled water) of 124,000 IDR. Water supply cost is 6.5% of HH expenditure.
- · AFT & WTP is estimated to apprx. 1% of HH expenditure through existing surveies in overseas.
- · No willingness is approx. 20% of respondents.
- Sewerage tariff level is estimated to 0.41% of household expenditure and less than 1.6% for 99% of responents. Sewerage tariff levy requires poverty mitigation.

# 4. Results of Survey for Buildings

- (1) Buildings in Central Sewerage Area
- (2) Building Sewer Management Survey-1
- (3) Building Sewer Management Survey-2

# (1) Buildings in Central Sewerage Area

# (A) Building List

Exemplati Residences   I. M.H. Thumin   Jalanta Pasat   55	No.	Building Name	Location	City	Floor
2   SCA Tower	1	Kempinski Residences	II MH Thamrin	Jakarta Pusat	57
4 The Plaza Tower					
S Plaza BII Tower	3	UOB Plaza	Jl. MH. Thamrin	Jakarta Pusat	42
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59 The Regatta Acapulco Tower Jl. Pantai Mutiara Jakarta Utara 24					
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No.	Building Name	Location	City	Floor
61	The Regatta New York Tower	Jl. Pantai Mutiara	Jakarta Utara	24
62	The Regatta Shanhai Tower	Jl. Pantai Mutiara	Jakarta Utara	24
63	Graha Reformed Millenium	Jl. Kemayoran	Jakarta Pusat	24
64	Star City Apartment Tower 1	Jl. Gajahmada No 188	Jakarta Utara	24
65	Star City Apartment Tower 2	Jl. Gajahmada No 188	Jakarta Utara	24
66	Laguna Apartment	Pluit	Jakarta Utara	24
	Menara Salemba Batavia Tower 1	Jl. Matraman	Jakarta Pusat	22
	Menara Salemba Batavia Tower 2	Jl. Matraman	Jakarta Pusat	22
	Menara Salemba Batavia Tower 3	Jl. Matraman	Jakarta Pusat	22
70	Allson Residence I	Jl. Senen Raya No. 135-137	Jakarta Pusat	21
	Allson Residence 2	Jl. Senen Raya No. 135-137	Jakarta Pusat	21
_	Allson Residence 3	Jl. Senen Raya No. 135-137	Jakarta Pusat	21
	Hayam Wuruk Plaza Tower	Jl. Hayam Wuruk No. 8, Kebon Kelapa Village	Jakarta Pusat	21
	DKI Jakarta Government Office	Jl. Merdeka Selatan	Jakarta Pusat	21
	Wisma Kosgoro	Jl. MH Thamrin	Jakarta Pusat	20
	Gedung Sapta Pesona	Jl. Medan Merdeka Barat No. 17	Jakarta Pusat	20
	Sheraton Media Hotel & Towers	Jl. Gunung Sahari Raya No. 3	Jakarta Utara	19
	Wisma Antara	Jl. Medan Merdeka Selatan. No. 17, Gambir	Jakarta Pusat	18
	BCA Wisma Asia II	Jl. S Parman Kav 79	Jakarta Pusat	18
	Borobudur Jakarta Hotel	Jl. Lapangan Banteng Selatan No.1	Jakarta Pusat	18
		Pluit	Jakarta Utara	17
	Emporium Pluit			
	Hayam Wuruk Residence	Jl. Hayam Wuruk	Jakarta Pusat	17
	Mediterania Lagoon Residences Tower 2	Jl. Benyamin Sueb St. Pademangan	Jakarta Pusat	16
	Dusit Mangga Dua Mall	Jl. Mangadua	Jakarta Utara	16
	Mahkamah Konstitusi	Jl. Medan Merdeka Barat No. 6	Jakarta Pusat	16
	Menteng Park Apartment Tower 1	Jl. Pegangsaan Barat Kav. 6-12 Menteng	Jakarta Pusat	16
	Menteng Park Apartment Tower 2	Jl. Pegangsaan Barat Kav. 6-12 Menteng	Jakarta Pusat	16
	Menteng Park Apartment Tower 3	Jl. Pegangsaan Barat Kav. 6-12 Menteng	Jakarta Pusat	16
89	Menteng Park Apartment Tower 4	Jl. Pegangsaan Barat Kav. 6-12 Menteng	Jakarta Pusat	16
90	Sahid Sahirman Memorial Hospital	Sahid Puri Kencana Superblock	Jakarta Pusat	15
91	Dinas Teknis Abdul Muis	Jl. Abdul Muis No. 66, Petojo Selatan Village,	Jakarta Pusat	15
92	Surya Building	Jl. MH. Thamrin, Kav. 9, Menteng	Jakarta Pusat	15
93	CBD Pluit Office Tower 1	Jl. Pluit Selatan Raya	Jakarta Utara	15
94	CBD Pluit Office Tower 2	Jl. Pluit Selatan Raya	Jakarta Utara	15
	Dusit Mangga Dua Hotel	Jl. Arteri Mangga Dua Raya	Jakarta Utara	14
96	PT. Pelayaran Nasional Indonesia	Jl. Gajah Mada. No.14, Gambir	Jakarta Pusat	14
97	Ratu Prabu	Jl. Jend. Sudirman. Kav.9, Tanah Abang	Jakarta Pusat	14
98	Wisma Hayam Wuruk	Jl. Hayam Wuruk. No. 8, Gambir	Jakarta Pusat	14
99	Apartemen Mangga Dua Court I	Jl. Mangga Dua Raya,	Jakarta Utara	14
100	Apartemen Mangga Dua Court 2	Jl. Mangga Dua Raya,	Jakarta Utara	14
101	Hotel Sari Pan Pacific	Jl. MH Thamrin	Jakarta Pusat	13
102	Wisma BII	Jl. MH Thamrin	Jakarta Pusat	13
103	Menteng I	Kecamatan Menteng,	Jakarta Pusat	12
104	Menteng II	Kecamatan Menteng,	Jakarta Pusat	12
	Menteng III	Kecamatan Menteng,	Jakarta Pusat	12
	Wisma BSG	Jl. Abdul Muis Kavling 40	Jakarta Pusat	12
	Departemen Luar Negeri		Jakarta Pusat	12
	Exim Melati	Jl. MH. Thamrin, Kav. 8-9, Kebon Melati Villa	Jakarta Pusat	12
	Jaya Building	Jl. MH. Thamrin, No.12, Menteng	Jakarta Pusat	12
	Plaza BII Tower II	Jl. MH. Thamrin	Jakarta Pusat	12
	Lion Air Tower	Jl. Gajah Mada, No. 7	Jakarta Pusat	12
	Komite Olahraga Nasional Indonesia (KONI) Pusa		Jakarta Pusat	12
	Graha Adira	Jl. Raya Menteng, No. 21, Menteng	Jakarta Pusat	12
1131	Graha Adira			

Source: JICA PPP Study Team

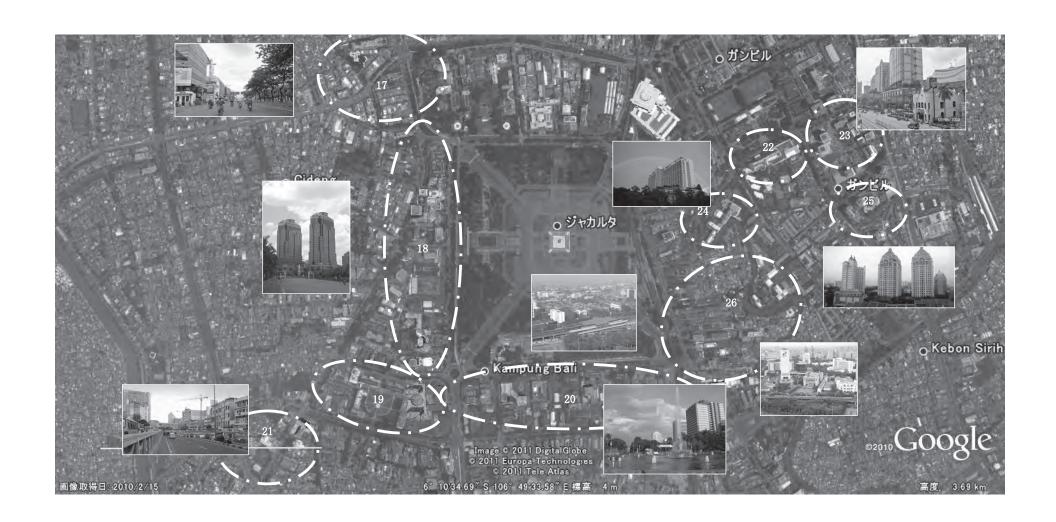
## (B) Number & Distribution of Buildings

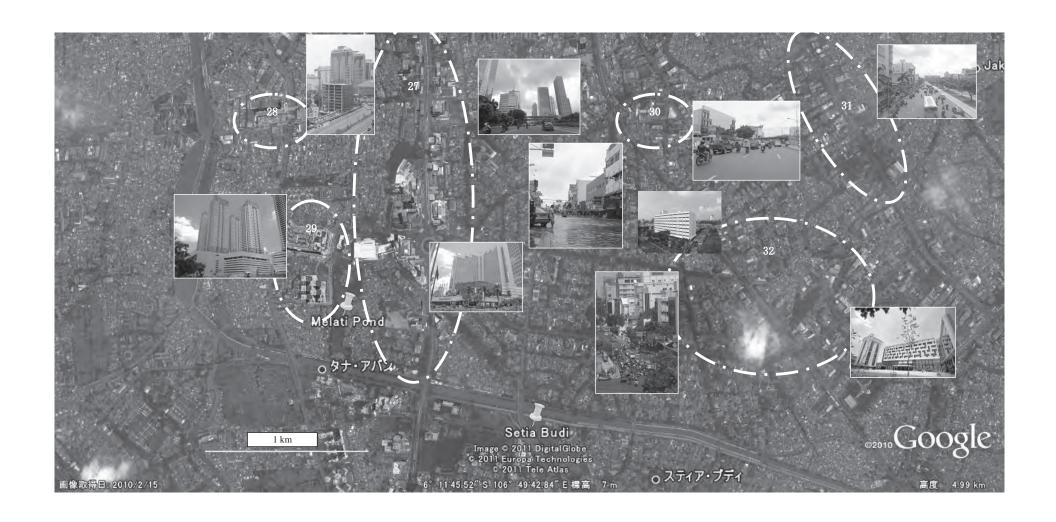
District	No. of Building	Building use	District	No. of Building	Building use
1	5	High rise Apartment & Hotel	21	4	Medium Commercial
2	4	High rise Apartment	22	2	Large Hotel & Government
3	4	High rise Apartment	23	8	High rise Apartment
4	1	Large Commercial	24	3	Large Hotel & High rise Apartment
5	14	Medium Commercial	25	6	High rise Apartment & Commercial
6	3	High rise Apartment & Hotel	26	5	Commercial & Hospital
7	2	High rise Apartment	27	30	Large Commercial & Hotel
8	2	High rise Apartment	28	2	Large Commercial
9	7	High rise Apartment	29	8	High rise Apartment & Commercial
10	2	Medium Commercial & Hotel	30	5	Medium Commercial & Government
11	7	Large Commercial	31	12	Medium Commercial
12	1	Large Commercial	32	8	Medium Apartment & Commercial
13	1	Large Commercial	33	13	High rise Apartment & Commercial
14	1	Large Commercial	Sub-tot al	225	
15	12	Large Commercial & Hotel			
16	2	High rise Apartment	34	1	Large Commercial & Government
17	13	Medium Commercial & Government	35	13	Large Commercial & Apartment
18	9	High rise Commercial & Hotel	Sub-tot al	13	
19	6	Large & Medium Commercial			
20	23	Medium Commercial	Total	238	











### (2) Building Sewer Management Survey-1

Questionnaire

### QUESTIONNAIRE ON BUILDING SEWERAGE COST ON WASTEWATER MANAGEMENT IN DKI JAKARTA

### PART-1 GENERAL INFORMATION OF QUESTIONEE

### SKYLINE BUILDING

Address Jl. MH. Thamrin No.9 Kel. Kebon Sirih Kec. Menteng

No. of Floors 18 Floors Area of Building Floor 21.195,97  $\,\mathrm{m}^2$ 

Immovable property tax (Asset tax) N/A IDR/year

Water Sourc : Piped-water

3.791 m<sup>3</sup>/month 47.729.620 IDR/month Monthly Water Consumption

### PART-2 INDIVIDUAL INFORNATION ON TREATMENT FACILITIES

Q1

### **Installation Cost of Wastewater Treatment Plant**

TOTAL	0 x 1.000.000 IDR	(no data)
ME	0 x 1.000.000 IDR	(no data)
CIVIL	0 x 1.000.000 IDR	(no data)

Installation Date : December 1975

100 m<sup>3</sup>/day **Treatment Capacity Type of Treatment Process**: Extended Aeration Process

Q2

### **Management Cost of Wastewater Treatment Plant**

Monthly O&M 2,700,000 IDR (excl. Labor Cost)

Number of O&M Staff 1 People

### History and Cost of Repair, Rehabilitation and/or Replacement

(month/year)	
Oct-87	38,730,000 IDR
2008	19,750,000 IDR
2009	18,150,000 IDR
2010	5,850,000 IDR
	2008 2009

Q3

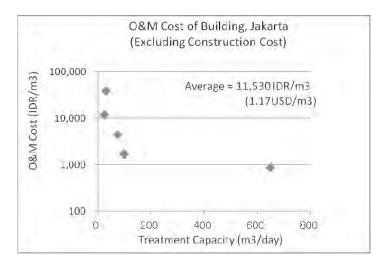
### Any Action in order to comply with the effluent standards

1 Ex	amination of water sample in the laboratory every month	115,000 IDR
2 Ex	amination of water sample in the laboratory every three months	300,000 IDR
3 En	nptying septic tank once a year	300,000 IDR

**Survey Results** 

	Building	SK	WBM	SPP	GR	GP	MB
No. of Floor	stories	18	6	21	5	6	23
Floor Area	m2	21,196	2,500	11,370	7,800	76,822	29,000
Property tax	IDR/year	N.A.	53,751,250	N.A.	65,000,000	2,320,000,000	373,000,000
Property Tax per Area & Month	IDR/m2/month		1,792		694	2,517	1,072
Monthly Water Consumption	m3/month	3,791	200	24,000	200	2,400	2,952
	IDR/month	47,729,620	2,500,000	301,200,000	6,500,000	30,120,000	37,000,000
	IDR/m3	12,590	12,500	12,550	13,000	12,550	12,534
Installation Cost (Total)		N.A.	N.A.	N.A.	19,000,000	240,000,000	N.A.
ME		N.A.	N.A.	N.A.	8,000,000	140,000,000	N.A.
Civil		N.A.	N.A.	N.A.	11,000,000	100,000,000	N.A.
Treatment Capacity	m3/day	100	32	059	26	22	70
Treatment Process		Extended Aeration	Septic Tank	Extended Aeration	Septic Tank	Extended Aeration	Activated Sludge
Monthly O&M excld. Labor Cost	IDR/month	2,700,000	150,000	2,000,000	1,000,000	1,500,000	N.A.
Number of Staff		1	3	4	2	3	2
Repair Cost	IDR/year	10,937,500	N.A.	85,000,000	N.A.	10,903,750	N.A.
Sampling Test	IDR/year	2,580,000	N.A.	360000	N.A.	913,000	N.A.
Desludging	IDR/year	1,200,000	1,000,000	20,000,000	N.A.	7,200,000	N.A.
O&M Total Cost	IDR/year	77,117,500	92,800,000	252,600,000	72,000,000	127,016,750	

O&M Cost of Building excluding Construction Cost



Wastewater Management Cost per Floor Area excluding Construction Cost



Wastewater Management Cost of Construction & OM per Floor Area



### Wastewater Management Cost per Property Tax excluding Construction Cost



# (3) Building Sewer Management Survey-2

# Questionnaire "Building Sewer Management" Survey on

# Wastewater Management in DKI Jakarta

# **Part-1 General Information of Questionee**

Address	Kec.	Kel.
No. of Floor: Floor	ors Area of Building Fl	loor: <u>m²</u>
Immovable property tax (As		
Water Source ( 🗸 )		
( ) Piped-water (	) Shallow well ( ) I	Deep well
Monthly Water Consumption	$m^3/mont$	th IDR/month
Part-2 Individual Inform	ation of Building Sewer	Management
	_	-
Q-1		
Wastewater Treatment Plant	( 🗸 )	
( ) Individual (	) Communal owned	
Treatment Capacity	m <sup>3</sup> /day	
Type of Treatment Proc	cess	
Q-2		
Cleansing & Desludging of	Treatment Plant	
times/ye	ear	m <sup>3</sup> tank truck /each time
Cleaning Cost		
		( ) Public (Dinas Kebersihan)
( , , (	,	( , , , , , , , , , , , , , , , , , , ,
Q-3		
What bothers you on wastev	vater disposal and facility ma	anagement?
· ·		anagement: , ( ) Expertise Staff, ( ) Floor Area
( ) Offensive Odol, (	) Manifellance, ( ) Cost,	, ( ) Expertise Stair, ( ) 1 1001 Area
(in detail)		
(III uctaii)		

V)-4
------

Do you know the role of sewerage system as followings?

Yes ( ) No ( )

### Q-5

Do you want to continue owned treatment plant or to connect to public sewer, if public sewerage system is developed?

Yes ( ) No ( ) depends on the Tariff ( IDR/month)

Present water environment is (D-level)

Polluted and offensive odor

Debris deposits

Unhygienic and caused habitual diarrhea

After sewerage system improves water quality level to:

Boatable (C)

Fishable (B)



Water cleaned up to "Boatable"

	Relation between wa	ater quality level and water life
Polluted	D - level Polluted	W.X.
P₀	Offensive odor	H
'	C - level	
Water quality level	Raise to boatable/ fishable level	
, dus	B - level	2.
Water	Raise to playing in water /bio- diversity level	
$\downarrow$	A - level	- w a/-
Clean	Raise to swimmable	Der I

Co	mmecial Buildi	Commecial Building Survey (1/3)										
					GE	NERAL IN	GENERAL INFORMATION	NC				
<u>0</u> 6	Kecamatan	Kelurahan	No. of Floor	Area of Building Floor (m²)	Asset Tax (IDR/year)	Piped	Shallow Deel		Recycled	Water volume (m³/month)	Water Consumption  Lume cost (IDR/month)	note
~	Menteng	Kebon Sirih		21195,97 (gross); 15137,93 (nett)	N/A	>				3,791	47,729,620	
7	Tanah Abang	Tanah Abang	26		33,000 1,000,000,000	>				5,000	65,000,000	water consumption cost is around Rp 65,000,000 60.000.000 - 70.000.000
က	Tanah Abang	Karet Tengsin	19 & 26	49,000	49,000 1,257,000,000	`		`		11,500	144,325,000	Volume of water consumption is around 60% from tap water 40% from ground 144,325,000 water. The answer just cover volume for tap water, because they didn;t know exactly volume for ground water usage
4	Menteng	Gondangdia	11 & 30	77,000	77,000 3,200,000,000	>			>	15,000	188,250,000	
5	Tanah Abang	Gelora	29	50,000	N/A	>			<b>,</b>	1,500	18,825,000	
9	Senen	Kenari	6	2,500	53,751,250	>		>		200	2,500,000	They use ground water just for back up (irregulary used)
7	Gambir	Petojo Utara	23	29,000	373,000,000	>		>		2,952	37,000,000	They use ground water just for back up (irregulary used)
8	Menteng	Kebon Sirih	21	11,370	Y/N	>		>		24,000	301,200,000	301,200,000 They use ground water just for back up (irregulary used)
6	Setiabudi	Kuningan Timur	5	7,800	65,000,000	>		>		200	6,500,000	They use ground water just for back up (irregulary used)
10	Taman Sari	Mangga Besar	6	76,822	2,320,000,000	>		>		2,400	30,120,000	They use ground water mostly. Supply 30,120,000 from tap water use for just 3rd & 4th floor

Con	Commecial Building Survey (2/3)	Iding Surv	ey (2/3)									
L				۵-1					Q-2			
ID no.	Indi	Maste Treatment Plant Individua Commun connec	dy t to age	Treatment capacity (m³)	Treatment capacity (m³/10,000m²)	Type of treatment process	note	Frequency of desludging (times/year)	Cleaning cost (IDR/year)	Cleaning method Private Public	method Public	note
~	>			100		extended aeration process		2	5,250,000		`	
Ν			>		0							
ю			>									
4	>			400		52 aeration & bioreactor		-	25,000,000		`	They do desludging every 9 months; cleaning cost is included in all STP operational&maintenance cost per month
22	>			1000		aeration & membran		12	36,000,000	>		
9	>			32		128 septic tank		2	1,200,000	>		
7	>			02	24	aeration process & active sludge method		1	2,000,000	>		
ω	>			029	572	extended aeration process & septic tank		3	25,000,000		>	
6	>			26		33 septic tank		7	1,500,000	>		
10	>			75		extended aeration process		0	2,000,000	>		

Co	Commecial Building Survey (3/3)	ilding Surv	vey (3/3)											
L						Q-3			Q-4			Q-5		
ID no.	Offensive Odor	Maintenance	Cost	Expertise staff	Floor area Don't have problem	Don't have problem	in detail	note	Yes	o <sub>N</sub>	Yes	No	Depends on tariff	note
~						<i>&gt;</i>			>		`			they think if using sewerage system will make operational&maintenance easier to handle.
7						<b>,</b>	Because of already connected to sewerage piped, so far they don't have any problem.		`		`			
ဇ						<i>&gt;</i>	Because of already connected to sewerage piped, so far they don't have any problem.		<b>&gt;</b>		`			
4		>	>	>					>			>		For now, they still using own facility. The reason is because high demand on water consumption while supply of tap water is vey low and unstable.
2		>					usually problem about machinery failure. Some broken equipments or blower		`		`			
9		^							<i>&gt;</i>		^			
7		<i>&gt;</i>					Not enough capacity of blower. So aeration process is not going well		>		>			
∞		>	>	>			Existing treatment facility is kind a out of dated, so there are some leaks that occur in the outlet pipe		>			>		They already had plan to build recycled treatment.
6		>							>		>			
10		>					Broken pump caused by too much solid waste flow into pump. It cause overflowed to aeration tank (so aeration didn't perform maximum process)		>		>			

## **Result of Commercial Building Survey**

		Water source
Piped water	10	Waster consumption $7 - 800 \text{ m}^3/\text{day}$
Shallow well	0	Small office: $10 - 20 \text{ m}^3/\text{day}$
Deep well	6	Large office: $100 - 300 \text{ m}^3/\text{day}$
Recycled water	2	Hotel: $500 - 800 \text{ m}^3/\text{day}$
		Water supply cost: 12,600 IDR/m <sup>3</sup>
		Estimated sewerage tariff cost: $25 - 60 \%$ of water supply

	Wastewater treatment	
Treatment process	Extended aeration	
	Trickling filter, Aeration & RO (filtration)	
	Septic tank	
Treatment capacity	$26 - 1,000 \text{ m}^3/\text{day}$	
	Mode: $70 - 100 \text{ m}^3/\text{day}$	
Sludge disposal	1 – 3 times/year, most frequent 12 times/year	
	Sludge volume: 5 - 50 m <sup>3</sup> /time	
	Cleansing Cost: 42,000 - 200,000 IDR/m <sup>3</sup>	
	Ave. Cost: 170,000 IDR/m <sup>3</sup>	

Problem on O&M	Yes	Detailed comment
Offensive odor	0	· Usually problem about machinery failure. Some broken
Maintenance	7	equipments or blower
Cost	2	· Not enough capacity of blower. So aeration process is not
Expertise staff	2	going well Existing treatment facility is kind a out of dated, so
Floor area	0	there are some leaks that occur in the outlet pipe
No problem	3	· Broken pump caused by too much solid waste flow into
		pump. It cause overflowed to aeration tank (so aeration didn't
		perform maximum process)

Awareness of Sewer	age		
	Yes	No	Detailed comment
Sewerage role	10	0	
Public sewerage	8	2	<ul> <li>They think if using sewerage system will make operational &amp; maintenance easier to handle.</li> <li>For now, they still using own facility. The reason is because high demand on water consumption while supply of tap water is very low and unstable.(Majority is miscellaneous use = low quality water)</li> <li>They already had plan to build recycled treatment.</li> </ul>

Asset Tax	$700 - 3,500 \text{ IDR/m}^2$
	Average: 2,200 IDR/m <sup>2</sup>

Estimated Sewerage Tariff Level	
Water Supply	25 – 60 % of water supply
Asset Tax	30 % of Asset Tax

# Additional Opinion on Commercial Building Survey

JICA PPP Study Team

Building Name	Opinion	Questioner impression
WBM	Actually they are willing to connect to sewerage pipe as long as it is indeed the terms set by the government. But if that is an option, it will be a consideration. They will connect as long as cost still makes sense. They think the waste water that has been produced is not a problem. Because office occupancy rate is low enough, then the domestic waste produced is also not much.	In my opinion, building owner was not aware that the existing system they use can pollute the environment especially ground water. They still use a conventional septic tank with absorption into the soil. It is automatically contaminate groundwater.  In addition, the government seems not strictly apply the rules. (probably it caused by location of the office not on a business or commercial district)
MBTN	They have a problem with broken existing equipment (machinery failure) and limited budget for the renovation. Because of it, they think sewerage pipe is a better solution. If they connect to sewerage pipe, building owner is no longer thinking about STP maintenance issue. Would be better is the tariff are cheaper than STP maintenance and operational costs.	Overall, the existing STP is on good condition. But somehow, their blower does not perform maximum capacity. They have 2 blowers and it run one by one (switch every 2 hours). They told me that blower capacity not enough to process all of wastewater. Sometimes, effluent water appear turbid and bit smelly.
SPPH	Basically building owner was still considering about connect to sewerage pipe. They had a plan to build recycled water treatment this year. The plant automatically requires a supply from STP output. They are willing to connect with condition that existing STP not to be closed. In other word, waste water that will be flowed to sewerage pipe are the rest of the recycled water treatment. Actually they think sewerage pipe is a good idea to manage general drainage condition in Jakarta. During this time, each building flow waste water onto city drainage pipe. The pipe often becomes clogged due to many solid wastes. When rainy season comes, the pipe becomes overloaded and cause flooding. This is caused waste water drain back onto building pipe. It is of course aesthetically disturbing and may cause sanitation problems. Then with the sewerage pipe is expected to be an alternative for better drainage system.	

<b>Building Name</b>	Opinion	Questioner impression
GR	For the building owner this plan is very welcomed. With the condition of old buildings, existing STP has been renovated several times. So in their opinion, sewerage pipe facilitate their needs. They are no longer deal with wastewater treatment problem because it has been handled by government. Besides they have a plan to expand building area, so the area should be allocated for STP can be used for other purposes given the relatively high land prices.	
GP	Sewerage pipe feel quite necessary to be realized. Building use as a shopping center then wastewater generated was quite a lot. Building owner find it difficult to deal with problems caused by STP. If they connect to sewerage pipe, then they would not provide expert staff to handle the problems.	Based on the info obtained, the building was often reprimanded by BPLHD because the quality of effluent water does out of standard. They have several problems of broken blower, pump, even leak pipe. STP is overloaded and does not have enough capacity to treat waste water. So, before waste water is properly processed in one tank, it will be overflow to the next tank.

5. Summary of Environmental Survey on Road Conditions

# SUMMARY OF ROAD CONDITION WHICH THAT WILL INSTALLING SEWERAGE PIPELINE

Saturday-Sunday, 1-2 July 2012 (Holiday) Ade Erfansyah Surveyed on Surveyed by

	Photo Doc				THE STATE OF THE S
	Underground Infrastructure	<ul> <li>Telephone Cable and</li> <li>Drinking water pipe (PAM)</li> </ul>	Telephone Cable     Drinking water pipe (PAM)	Telephone     Cable     Gas pipe     Drinking     water pipe     (PAM)     Electricity     cable	<ul><li>Telephone Cable</li><li>Electricity cable</li></ul>
Description of Road Condition	Majority Usage of Side Walk and Roadside	<ul> <li>Poles of street lighting, electricity, and billboard</li> <li>Roadside usage are Small shop, business and town house</li> </ul>	<ul> <li>Poles of street lighting, electricity, billboard, telephone, traffic sign, trees, bridge crossed</li> <li>Roadside usage are Small shop, business, school, gov't office and town house</li> </ul>	<ul> <li>Poles of street lighting, electricity, billboard, telephone, traffic sign, trees,</li> <li>Roadside usage are university and town house</li> </ul>	<ul> <li>Poles of street lighting and crossing bridge</li> <li>Roadside usage are retail, warong and town house</li> </ul>
	Traffic Condition and Public Trans	<ul> <li>Relatively dense on workday and smoothly on holyday.</li> <li>P-156, AC-60, P-116, Steady Safe 936 (Manggarai-Psr Baru),</li> <li>Steady Safe AC-47 (Manggarai-Senen)</li> </ul>	<ul> <li>Relatively dense on workday and smoothly on holyday.</li> <li>Mikrolet M-13 (Kalideres-Kota)</li> <li>Ojeg (free route)</li> </ul>	<ul> <li>Relatively dense on workday and smoothly on holyday.</li> <li>Trans Jkt coridor V (Kp. Melayu-Ancol)</li> <li>Bus Sinar Jaya P-156 (Kp. Rambutan – Psr Baru), Bus AC P-16 (Rawamangun – Lebak bulus), Mayasari 905 (Pulo Gadung – Dr Wahidin),</li> <li>Steady Safe 962 and AC-54 (Pulo Gdg-Kota),</li> <li>Metromini T-49 (Pulo Gadung – Manggarai),</li> </ul>	<ul> <li>Relatively dense on workday and smoothly on holyday.</li> <li>Mikrolet M-13 (Kalideres-Kota)</li> </ul>
	Geometric	7 20 m 14 m 7 m 0.8 m 0.5 m (C)	15 m 10 m 5 m 1 m 0.7 m (C)	7 15 m 10 m 5 m 1,5 m 0.8 m	6m 6 m - 1m 0.6m
	Ge	ROW TV TL SW DR	ROW TL SW DR	ROW TW TL SW DR SR	ROW TL SW DR SR
to ome (N) a cited of	Road Segment	Jin Tambak ( 2 way), Kelurahan Pegangsaan, Jkt Pusat	Jin Bandengan Utara (1 way), Penjaringan, Jkt Utara	Jin Proklamasi ( 1 way), Kelurahan Pegangsaan, Jkt Pusat	Jin. Bandengan Utara ( 1 way), Kelurahan Penjaringan, Jakarta Utara
	STA.	Ť.	2	Э	4

Photo Doc					
Underground	Gas pipe     Drinking     water pipe (PAM)	Drinking     water pipe     (PAM)	<ul> <li>Drinking         water pipe         (PAM)</li> <li>Electricity         cable</li> </ul>	<ul> <li>Drinking         water pipe         (PAM)</li> <li>Telephone         fiber optic         Cable</li> </ul>	<ul> <li>Drinking water pipe (PAM)</li> </ul>
Description of Road Condition Majority Usage of Side Walk and Roadside	<ul> <li>Parking on right sidewalk</li> <li>Roadside usage are offices, Apartment, Puskesmas.</li> <li>Sebrang jln: Taman, Station Kereta Cikini</li> </ul>	<ul> <li>Poles of street lighting, electricity, billboard, telephone, traffic sign, trees,</li> <li>Roadside usage is Elite settlement</li> </ul>	<ul> <li>Poles of street lighting, the big pots</li> <li>Roadside usage are Commercial building, and retail</li> </ul>	<ul> <li>Poles of street lighting, electricity, telephone, trees,</li> <li>Roadside usage are Elite settlement and embassy office</li> </ul>	<ul> <li>Poles of street lighting, electricity, telephone, trees,</li> <li>Roadside usage are townhouse, retail, office, warehouse, and traditional market (pasar pagi) includes parking and waste area</li> </ul>
Traffic Condition and Public Trans	<ul> <li>Relatively dense on workday and smoothly on holyday.</li> <li>U 07, T 502, U 13</li> </ul>	<ul> <li>Relatively dense on workday and smoothly on holyday</li> <li>Kopaja T-502 (Kp. Rambutan – Tanah Abang)</li> </ul>	<ul> <li>Relatively dense on workday and smoothly on holyday</li> <li>Mikrolet M-10 (Tanah Abang- Kota)</li> </ul>	<ul> <li>Relatively dense on workday and smoothly on holyday</li> <li>P73, P20, p158, P116, 52, 507, 906, P7, 507</li> </ul>	<ul> <li>Relatively dense on workday and holyday</li> <li>Mikrolet M-10 (Tanahabang-Kota)</li> <li>Many Ojeg in the traditional market area</li> </ul>
Geometric	TW 7 m TL 7 m SW 1m DR 1m	ROW TW 6m TL 6m SW 1m DR 2 m SR -	TW 6m TV 6m TL 6 m SW 0.8 m SR 6 m	TW 6m TL 6m SW 0.8 m SR 6m	TW 5 m TL 5 m SW 0.8 m SR 1 m
Location/Name of Road Segment	JI. Pegangsaan ROV Barat ( 2 way), TW Kelurahan TL Pegangsaan, SW Jakarta Pusat DR	Jln. KH. Moch. Yamin (1 way), Kelurahan TL Menteng, Jakarta SW Pusat SR	Jl. Pejalan ( 1 way), RON kecamatan Roa TW Malaka, Jakarta TL Utara SW DR	Jln. KH. Moch. Yamin (1 way), TW Kelurahan TL Menteng, Jakarta SW Pusat DR	Jl. Perniagaan RON Barat, kecamatan TW Roa Malaka, TL Jakarta Utara SW DR
		<u> </u>		I -	1

Photo Doc					
Underground Infrastructure	<ul> <li>Drinking         water pipe         (PAM)</li> <li>Telephone         fiber optic         Cable</li> </ul>	Drinking     water pipe (PAM)	Drinking     water pipe     (PAM)	Drinking     water pipe (PAM)	Telephone fiber optic Cable
Description of Road Condition Majority Usage of Side Walk and Roadside	<ul> <li>Poles of street lighting, electricity, telephone, trees,</li> <li>Roadside usage are Elite settlement and embassy office</li> </ul>	<ul> <li>Poles of street lighting, electricity, telephone, trees,</li> <li>Roadside usage are townhouse, retail, office, warehouse, and traditional market includes parking and school</li> </ul>	<ul> <li>Poles of street lighting, electricity, telephone, trees,</li> <li>Roadside usage are Elite settlement and embassy office</li> </ul>	<ul> <li>Poles of street lighting, electricity, telephone, trees,</li> <li>Roadside usage are townhouse, retail, office, warehouse, and traditional market includes parking and school</li> </ul>	<ul> <li>Poles of street lighting, electricity, telephone, trees,</li> <li>Roadside usage are Elite settlement and embassy office</li> </ul>
Traffic Condition and Public Trans	<ul> <li>Relatively dense on workday and smoothly on holyday</li> <li>P73, P20, p158, P116, 52, 507, 906, P7, 507</li> </ul>	Relatively dense on workday and smoothly on holyday	<ul> <li>Relatively smoothly on workday and holyday</li> <li>PPD 916 (Kp Melayu-Tanah Abang</li> </ul>	Relatively dense on workday and smoothly on holyday	Relatively smoothly on workday and holyday
Geometric	6 m 6 m 0.8 m 6m	10 m 10 m 5 m 3 m 0.8 m	10m 5m - 2 m	10 m 5 m 3 m 0.8 m	6 m 6 m 0.3 m 0.8 m
Geo	ROW TL TL SW DR	ROW TL SW DR SR	ROW TT TL SW SW SR SR	ROW TTL SW SW SR SR	ROW TL SW DR SR
Location/Name of Road Segment	Jin. KH. Moch. Yamin (1 way), Kelurahan Menteng, Jakarta Pusat	Jin. KH. Moch. Masyur (2 way), Kelurahan Tambora, Jakarta Barat	JI. H. Agus Salim (2 way), Kelurahan Menteng, Jakarta Pusat	Jln. KH. Moch. Masyur (2 way), Kelurahan Tambora, Jakarta Barat	Jl. Sumenep.(1 way), Kelurahan Menteng- Jakarta Pusat
STA.	10	11	12	13	14

Photo Doc				
Underground	Telephone fiber optic Cable	<ul> <li>Telephone fiber optic Cable</li> <li>Gas pipe</li> <li>Drinking water pipe (PAM)</li> <li>Electricity cable</li> </ul>	<ul> <li>Telephone fiber optic Cable</li> <li>Drinking water pipe (PAM)</li> </ul>	Drinking     water pipe (PAM)
Description of Road Condition Majority Usage of Side Walk	Poles of street lighting, electricity, telephone, trees,     Roadside usage are Elite settlement	<ul> <li>Poles of street lighting, electricity, telephone, trees,</li> <li>Roadside usage are townhouse, restaurant, pot and un-formal retail</li> </ul>	<ul> <li>Poles of street lighting, electricity, telephone, trees,</li> <li>Roadside usage are townhouse, restaurant, pot and un-formal retail</li> </ul>	<ul> <li>Poles of street lighting, electricity, telephone and, trees</li> <li>Roadside usage are townhouse, hotel, apartment, and mall</li> </ul>
Traffic Condition and Public Trans	Relatively smoothly on workday     and holyday	Relatively smoothly on workday and holyday	Relatively dense on workday and smoothly on holyday	Relatively dense on workday and smoothly on holyday
Geometric	TW 6 m TL 6 m SW 0.7 m DR 4 m SR 0.6 m	ROW TW 12 m TL 12 m SW DR 1 m SR 0.3 m	TW 8 m TL 4 m SW 0.7 m DR 1.5 m SR 0.8 m MD	KOW TW 12 m TL 7 m 0.3 SW cm DR 3 m SR 0.3 m
Location/Name of Road Segment	Ji. Purworejo .(1 Rway), Kelurahan TMenteng-Jakarta TPusat S	Ji. Blora .(1 way), R Kelurahan T Menteng-Jakarta T Pusat S D D	JI. Teluk Betung (2 Rway), Kelurahan TKebon Melati- TJakarta Pusat S	JI. Kebon Kacang R (2 way), Kelurahan T Kebon Melati- T Jakarta Pusat S S
STA.	15	16	17	18

	Photo Doc				
	Underground Infrastructure	<ul> <li>Drinking water pipe (PAM)</li> </ul>	<ul> <li>Drinking water pipe (PAM)</li> </ul>	<ul> <li>Telephone fiber optic Cable</li> <li>Drinking water pipe (PAM)</li> </ul>	<ul> <li>Telephone fiber optic Cable</li> <li>Drinking water pipe (PAM)</li> </ul>
Description of Road Condition	Majority Usage of Side Walk and Roadside	<ul> <li>Poles of street lighting, electricity, telephone</li> <li>Roadside usage are townhouse, mall and Street parking (2 road side)</li> </ul>	<ul> <li>Poles of street lighting, electricity, telephone, trees,</li> <li>Roadside usage are townhouse, hotel, apartment, and mall</li> </ul>	<ul> <li>Poles of street lighting, electricity, telephone and, trees</li> <li>Roadside usage are townhouse, offices and retail /informal retail (kaki lima)</li> </ul>	<ul> <li>Poles of street lighting, electricity, telephone and, trees</li> <li>Roadside usage are townhouse, offices and retail /informal retail (kaki lima)</li> </ul>
	Traffic Condition and Public Trans	<ul> <li>Relatively dense on workday and smoothly on holyday</li> </ul>	<ul> <li>Relatively dense on workday and smoothly on holyday</li> </ul>	<ul> <li>Relatively dense on workday and smoothly on holyday</li> <li>Steady Safe P-133 (Grogol-Kelender)</li> <li>Kopaja S-608 (Blok M-Tanahabang), Kopaja S-615 (lebak Bulus-Tanahabang) and Mikrolet M-38 (Grogol-Tanahabang)</li> </ul>	<ul> <li>Relatively dense on workday and smoothly on holyday</li> <li>Mayasari Bakti 507 (Pulo Gadung-Tanahabang), Steady Safe 939 (Blok M-Veteran), Steady Safe AC-110 (Tj. Priok- Tanahabang) and Mikrolet M-08 (Tanahabang-Kota).</li> </ul>
	Geometric	ROW TW 6 m TL 6 m SW DR SR	ROW TW 7 m TL 7 m SW 0.3 m DR - SR 0.8 m	ROW TW 12 TL 6 SW - DR 1 m SR MD	TW 6 m TL 3 m SW 1 m SR 0.6 m
Jo 000 01N) 00 1+000 1	Road Segment	Noname Road (1 ROV way), Kelurahan TW Kebon Melati- TL Jakarta Pusat SW DR SW	JI. Kebon Kacang RO' (2 way), Kelurahan TW Kebon Melati- TL Jakarta Pusat SW DR	JI. KH. Mas Mansyur (2 way), TW Kelurahan Kebon TL Melati- Jakarta SW Pusat DR SR	JI. Abdul Muis (2 ROway), kelurangan TW Petojo Selatan, TL Jakarta Selatan BR SR
	STA.	19	20	21	22

way), kelurahan TW Gambir, Jakarta TL Pusat SW DR	MD	Veteran), PPD 12 (Blok M-Senen), PPD 11 (bekasi-Harmoni)  Relatively smooth on workday			一年在八二
MD  JI. Veteran (1  way), kelurahan  Pusat  Pusat  TL  Busw  ay)  SW  DR  1 m  SR  MD  JI. Teluk Gong (2  WAD  Way), kelrahan  Pejagalan, Jakarta  SR  MD  TL  14 m  ND  MD  MD  MD  MD  MD  MD  MD  MD  MD	ROW TW 7 m 7 m 7 m 7 m 1L (3 m ay) SW ay) SR - NMD ROW TL 14 m SW 7 m DR 1 m SW 7 m OR 1 m SW 7 m MD	PPD P1 (Bekasi-Harmoni), PPD P43 (Banteng Utara-Depok)     Relatively dense on workday and smoothly on holyday     M25 (Grogol –Kota), B01     (grogol-angke)	Poles of street lighting, electricity, telephone and, trees Roadside usage are Elite settlement on the side back of President Palace Poles of street lighting, electricity, telephone and, trees Roadside usage are townhouse, offices and settlement	Telephone fiber optic Cable Drinking water pipe (PAM)  Telephone fiber optic Cable Drinking water pipe (PAM)  Prinking water pipe (PAM)	

A5-6

# SUMMARY OF ROAD CONDITION WHICH THAT WILL INSTALLING SEWERAGE PIPELINE Surveyed on : Saturday -Sunday, 1-2 July 2012 (Holiday) Surveyed by : Randi Suhendi

	Photo Doc		LEGAL REST.		00 2008///
	Underground Infrastructure	<ul> <li>Telephone Cable and</li> <li>Drinking water pipe (PAM)</li> </ul>	<ul> <li>Telephone Cable and</li> <li>Drinking water pipe (PAM)</li> </ul>	Telephone Cable and     Drinking water pipe (PAM)	Telephone Cable and     Drinking     water pipe (PAM)
Description of Road Condition	Majority Usage of Side Walk and Road Side	<ul> <li>Poles of street lighting, electricity, and billboard</li> <li>Roadside usage are glassware market on ± 100 m corridor in the morning and vegetables in the evening (PasarJembatan Lima)</li> </ul>	<ul> <li>Poles of street lighting, electricity, and billboard</li> <li>Roadside usage are townhouse, office and retail</li> </ul>	<ul> <li>Poles of street lighting, electricity, and billboard</li> <li>Roadside usage are townhouse, office and retail</li> </ul>	<ul> <li>Poles of street lighting, electricity, and billboard</li> <li>Roadside usage are townhouse, retail and settlement along the river side</li> </ul>
	Traffic Condition and Public Transportation Route	<ul> <li>The traffic relatively dense on workday and smoothly on holyday.</li> <li>Metro mini No. 80 Kali Deres Jembatan Lima</li> <li>Mikrolet M 10 Tanah Abang-Kota</li> </ul>	<ul> <li>The traffic relatively dense on workday and smoothly on holyday.</li> <li>Busway corridor</li> <li>Mikrolet 25 and 01 Grogol-Muarakarang</li> </ul>	The traffic relatively dense on workday and smoothly on holyday.	<ul> <li>The traffic relatively dense on workday and smoothly on holyday.</li> <li>Metro mini 86 and 29, Mikrolet 02</li> </ul>
	Geometric	ROW m TW 6 m TL 2 m SR 1 m SW - DR 1.5 m	KOW m TW 16 m TL 4 m SR 6 m SW - DR 1 m	KOW m TW 10 m TL 4 m SR 3 m SW 5 m DR -	ROW m TW 8 m TL 2 m SR 1,5 m SW - DR 1 m
Location/Name of	Road Segment	JI. Moh Mansyur	Al. Latumenten Raya T	J. Bendungan Utara	JI. Tubagus Angke
į	SIA.	ਜ਼ਂ	7	m <sup>i</sup>	4

	Photo Doc				177/2012 17:24
	Underground Infrastructure	<ul> <li>Telephone         Cable and         Drinking         water pipe         (PAM)     </li> </ul>	Telephone     Cable and     Drinking     water pipe     (PAM)	Telephone Cable and     Drinking water pipe (PAM)	Telephone Cable and     Drinking     water pipe (PAM)
Description of Road Condition	Majority Usage of Side Walk and Road Side	<ul> <li>Poles of street lighting, electricity, and billboard and an existing train crossing</li> <li>Roadside usage are townhouse, retail and settlement</li> </ul>	<ul> <li>Poles of street lighting, electricity, and billboard</li> <li>Roadside usage are townhouse, retail and settlement</li> </ul>	<ul> <li>Poles of street lighting, electricity, and billboard</li> <li>Roadside usage are townhouse, retail and settlement, as well as informal retail (Kaki lima)</li> </ul>	<ul> <li>Poles of street lighting, electricity, and billboard</li> <li>Roadside usage are townhouse, retail and office</li> </ul>
	Traffic Condition and Public Transportation Route	<ul> <li>The traffic relatively dense on workday and smoothly on holyday.</li> <li>Metro mini 86 and 29, Mikrolet 02</li> </ul>	<ul> <li>The traffic relatively dense on workday and smoothly on holyday.</li> <li>Metro mini 83, and Mikrolet 25 Grogol-Kota</li> </ul>	<ul> <li>The traffic relatively dense on workday and smoothly on holyday.</li> <li>Metro mini 84 and Mikrolet M 25 Kota-Kapuk</li> </ul>	<ul> <li>The traffic relatively dense on workday and smoothly on holyday.</li> <li>Busway Pluit Pinang Ranti Corridor</li> <li>Metro Mini 84 (Kali Deres-Kota), No. 87 (Muara Baru-Kali Deres)</li> <li>Mikrolet 01 MuaraKarang – Grogol</li> </ul>
	Geometric	ROW m TW 5 m TL 2 m SR - SW 1 m DR 20 m	ROW m TW 12 m TL 3 m SR 1 m SW - DR 1 m	ROW m TW 19 m TL 3 m SR - SW 1,5 m DR 2 m	ROW m TW 15 m TL 5 m SR - SW 1,5 m DR 2 m
Location/Name of	Road Segment	Ji. Tubagus Angke Jembatan Lima	Ji. Jembatan 2 Raya	Jl. Gedong Panjang 2	Jl. Gedong Panjang 1
į	SIA.	.5.	.9	7.	∞

	Photo Doc				
	Underground Infrastructure	<ul> <li>Telephone Cable and</li> <li>Drinking water pipe (PAM)</li> </ul>	<ul> <li>Telephone Cable and</li> <li>Drinking water pipe (PAM)</li> </ul>	<ul> <li>Telephone         Cable         Drinking         water pipe         (PAM)         Electricity         cable     </li> </ul>	• Electricity cable
Description of Road Condition	Majority Usage of Side Walk and Road Side	<ul> <li>Poles of street lighting, electricity, and billboard</li> <li>Roadside usage are townhouse, retail and office</li> </ul>	<ul> <li>Poles of street lighting, electricity, and billboard</li> <li>Road side usage are townhouse, retail and office as well as informal retail (kaki lima)</li> </ul>	<ul> <li>Poles of street lighting, electricity, and billboard</li> <li>Road side usage are settlement and retail</li> </ul>	<ul> <li>Poles of street lighting, electricity</li> <li>Road side usage are housing t</li> </ul>
	Traffic Condition and Public Transportation Route	<ul> <li>The traffic relatively dense on workday and smoothly on holyday.</li> <li>Mikrolet 01 MuaraKarang – Grogol</li> </ul>	<ul> <li>The traffic relatively dense on workday and smoothly on holyday.</li> <li>100 m to crossing with traditional market and 200 m to overfly</li> </ul>	<ul> <li>The traffic relatively dense on workday and smoothly on holyday.</li> <li>Mikrolet 01 Grogol – Angke</li> </ul>	<ul> <li>The traffic relatively smooth on workday and on holyday.</li> <li>Ojeg</li> </ul>
	Geometric	ROW m TW 6 m TL 2 m SR 6 m SW - DR 1 m	ROW m TW 15 m TL 5 m SR - SW - DR 5 m	ROW m TW 12 m TL 2 m SR 4 m SW - DR 1 m	ROW m TW 6 m TL 2 m SR 2 m SW - DR 1 m
Location/Name of	Road Segment	JI. Teluk Gong Raya	Jl. Raya Jembatan 3	Jl. Pluit Utara Raya	Jl. Pantai Mutiara 1
	STA.	б	10.	11.	12.

	Photo Doc	80% 2100/1/1			
	Underground Infrastructure	<ul> <li>Telephone Cable</li> <li>Drinking water pipe (PAM)</li> <li>Electricity cable</li> </ul>	Telephone Cable     Drinking     water pipe (PAM)     Electricity cable	Drinking     water pipe     (PAM)	Telephone Cable     Drinking     water pipe (PAM)
Description of Road Condition	Majority Usage of Side Walk and Road Side	<ul> <li>Poles of street lighting, electricity</li> <li>Road side usage are houses complex</li> </ul>	<ul> <li>Poles of street lighting, electricity</li> <li>Road side usage are dock, market, and town-house</li> </ul>	<ul> <li>Poles of street lighting, electricity</li> <li>Road side usage are traditional market (Pasar Muara Angke) and fish storage ass well as town house</li> </ul>	<ul> <li>Poles of street lighting, electricity</li> <li>Road side usage are townhouse and settlement in a long the river corridor</li> </ul>
	Traffic Condition and Public Transportation Route	<ul> <li>The traffic relatively dense on workday and smoothly on holyday.</li> </ul>	The traffic relatively dense on workday and smoothly on holyday.	<ul> <li>The traffic relatively dense on workday and smoothly on holyday.</li> </ul>	<ul> <li>The traffic relatively dense on workday and smoothly on holyday.</li> </ul>
	Geometric	ROW m TW 8 m TL 2 m SR 2,5 m SW 1,5 m DR -	ROW m TW 8m TL 2m SR 2m SW - DR 1m	TW 7 m TL 2 m SR 1 m SW - DR 1 m	KOW m TW 20 m TL 4 m SR - SW - m DR - m
Location/Name of	Road Segment	JI. Pantai Mutiara	JI. Mandala Bahari	JI. Pasar Muara Angke	Jl. Kapau
	STA.	13.	14.	15.	16.

Photo Doc	CAVING CA	1000 BIOCALLA	THOU. SET	
Underground	Telephone     Cable     Drinking     water pipe (PAM)	Telephone Cable     Drinking     water pipe (PAM)	Telephone Cable     Drinking     water pipe (PAM)	Telephone Cable     Drinking     water pipe (PAM)
Description of Road Condition  Majority Usage of Side Walk	<ul> <li>and Koad Side</li> <li>Poles of street lighting, electricity</li> <li>Road side usage are retail and warehouse</li> </ul>	<ul> <li>Poles of street lighting, electricity and billboard</li> <li>Road side usage are townhouse, retail and housing complex Muara Angke</li> </ul>	<ul> <li>Poles of street lighting,</li> <li>electricity and billboard</li> <li>Road side usage are</li> <li>warehouse, apartment and</li> <li>settlement</li> </ul>	<ul> <li>Poles of street lighting and electricity</li> <li>Road side usage are Settlement on the right and left road corridor</li> </ul>
Traffic Condition and Public	<ul> <li>Iransportation Route</li> <li>The traffic relatively dense on workday and smoothly on holyday.</li> </ul>	<ul> <li>The traffic relatively dense on workday and smoothly on holyday.</li> <li>Mitro mini 02 (Senen- Muara Karang) and Mikrolet M 15</li> </ul>	<ul> <li>The traffic relatively dense on workday and smoothly on holyday.</li> <li>Mitro mini O2 (Senen- Muara Karang)</li> </ul>	<ul> <li>The traffic relatively dense on workday and smoothly on holyday.</li> <li>Mikrolet U 11 (Muara Baru – Muara Angke)</li> </ul>
Geometric	ROW m TW 20 m TL 4 m SR - SW - m DR - m	ROW m TW 20 m TL 40 m SR - SW DR 4 m	ROW m TW 10 m TL 2 m SR - SW 1,2m DR 1 m	ROW m TW 10 m TL 2 m SR - DR -
Location/Name of Road Segment	JI. Lodan Raya	Jl. Pakin	JI. Jalan Muara Baru (jalan raya)	JI. Muara Baru
STA.	17.	18.	19.	20.

	Photo Doc			E CL. STORY OF THE	
	Underground Infrastructure	<ul> <li>Telephone Cable</li> <li>Drinking water pipe (PAM)</li> </ul>	Telephone Cable     Drinking     water pipe (PAM)	Telephone Cable     Drinking     water pipe (PAM)	Telephone Cable     Drinking     water pipe (PAM)
Description of Road Condition	Majority Usage of Side Walk and Road Side	<ul> <li>Poles of street lighting and electricity</li> <li>Road side usage are Settlement on the right and left road corridor</li> </ul>	<ul> <li>Poles of street lighting and electricity</li> <li>Road side usage are Settlement on the right and left road corridor</li> </ul>	<ul> <li>Poles of street lighting and electricity</li> <li>Road side usage are Settlement on the right and left road corridor</li> </ul>	Poles of street lighting and electricity     Road side usage are townhouse, retail and settlement
	Traffic Condition and Public Transportation Route	<ul> <li>The traffic relatively dense on workday and smoothly on holyday.</li> <li>Metro mini 02 and Mikrolet U 11 (Muara Baru – Muara Angke)</li> </ul>	<ul> <li>The traffic relatively dense on workday and smoothly on holyday.</li> <li>Metro mini 02 and Mikrolet U 11 (Muara Baru – Muara Angke)</li> </ul>	The traffic relatively dense on workday and smoothly on holyday.	<ul> <li>The traffic relatively dense on workday and smoothly on holyday.</li> <li>Bis (kota-Muara Angke) and Mikrolet 01(Grogol – Muara Angke)</li> </ul>
	Geometric	M m 10 m 3 m 6 m 1,5 m	8 m 2 m 3 m 3,5 m	W m 10 m 2 m 1 m 1 m	W m 10 m 2 m 4 m 2 m
Jo		DR	TL SR SW DR	TV TL SR SW DR	ROW T L SR SW DR
Location/Name of	Road Segment	JI. Pluit Raya	JI. Pluit Raya	JI. Mandala Bahari	Jl. Pluit Karang Barat
	STA.	21.	22.	23.	24.

	Photo Doc	11/202 150			
	Underground Infrastructure	<ul> <li>Telephone         <ul> <li>Cable</li> <li>Drinking</li> <li>water pipe</li> <li>(PAM)</li> </ul> </li> <li>IPAL pipeline         <ul> <li>of Housing</li> </ul> </li> </ul>	Telephone Cable     Drinking     water pipe (PAM)	Telephone Cable     Drinking     water pipe (PAM)	Telephone     Cable     Drinking     water pipe     (PAM)
Description of Road Condition	Majority Usage of Side Walk and Road Side	<ul> <li>Poles of street lighting and electricity</li> <li>Road side usage are Housing Komplek of Karang Elok</li> </ul>	<ul> <li>Poles of street lighting and electricity</li> <li>Road side usage are Settlement on the right and left road corridor</li> </ul>	<ul> <li>Poles of street lighting and electricity</li> <li>Road side usage are settlement, retail and Mall</li> </ul>	<ul> <li>Poles of street lighting and electricity</li> <li>Road side usage are Settlement on the right and left road corridor</li> </ul>
	Traffic Condition and Public Transportation Route	<ul> <li>The traffic relatively dense on workday and smoothly on holyday.</li> </ul>	The traffic relatively dense on workday and smoothly on holyday.	The traffic relatively dense on workday and smoothly on holyday.	<ul> <li>The traffic relatively dense on workday and smoothly on holyday.</li> <li>Bus Way Corridor 9</li> <li>Metro mini 02 (Bendungan-Kota), and Mikrolet 01 (Grogol - Angke)</li> </ul>
	Geometric	ROW m TW 8m TL 2m SR 2m SW - DR 2m	ROW m TW 24 m TL 4 m SR 4 m SW - DR 2 m	ROW m TW 14 m TL 3 m SR 4 m SW - DR 1 m	ROW m TW 12 m TL 2 m SR 4 m SW - DR 1 m
Location/Name of	Road Segment	Ji. Komplek Karang Filok Tilok	JI. Muara Karang Utara Raya	Jl. Permai Raya	Jl. Pluit Utara Raya
	STA.	25.	26.	27.	28.

Photo Doc				
Underground	Telephone Cable Drinking water pipe (PAM)	Telephone Cable     Drinking     water pipe (PAM)	Telephone Cable     Drinking     water pipe (PAM)	Telephone Cable     Drinking     water pipe (PAM)
Majority Usage of Side Walk	Poles of street lighting and electricity     Road side usage are Settlement on the right and left road corridor	<ul> <li>Poles of street lighting and electricity</li> <li>Road side usage are Settlement on the right and left road corridor</li> </ul>	<ul> <li>Poles of street lighting and electricity</li> <li>Road side usage are Settlement on the right and left road corridor</li> </ul>	<ul> <li>Poles of street lighting and electricity</li> <li>Road side usage are Settlement on the right and left road corridor</li> </ul>
Traffic Condition and Public	<ul> <li>The traffic relatively dense on workday and smoothly on holyday.</li> <li>KWK 02 (Kota-Cengkareng)</li> </ul>	<ul> <li>The traffic relatively dense on workday and smoothly on holyday.</li> <li>Metromini M 41 Kota Grogol</li> </ul>	The traffic relatively dense on workday and smoothly on holyday.	<ul> <li>The traffic relatively dense on workday and smoothly on holyday.</li> </ul>
Geometric	KOW m TW 5,5 m TL 2 m SR 1 m SW 1,5 m DR 1,5 m	ROW m TW 6,5 m TL 2 m SR 1 m SW - DR 1 m	ROW m TW 6 m TL 2 m SR 1 m SW - DR 1 m	ROW m TW 6m TL 2m SR 1m SW - DR 1m
Location/Name of Road Segment	Jl. Tambora	Jl. Tanah Serial	Jl. Krendeng Raya	Jl. Krendeng Barat
STA.	29.	30.	31.	32.

	Photo Doc				
	Underground Infrastructure	<ul> <li>Telephone Cable</li> <li>Drinking water pipe (PAM)</li> </ul>	Telephone     Cable     Drinking     water pipe     (PAM)	Telephone Cable     Drinking     water pipe (PAM)	Telephone Cable     Drinking water pipe (PAM)
Description of Road Condition	Majority Usage of Side Walk and Road Side	<ul> <li>Poles of street lighting and electricity</li> <li>Road side usage are Settlement on the right and left road corridor</li> </ul>	<ul> <li>Poles of street lighting and electricity and billboard</li> <li>Road side usage are townhouse and retail</li> </ul>	<ul> <li>Poles of street lighting and electricity and billboard</li> <li>Road side usage are townhouse, retail and office</li> <li>Existing artificial lake, toll bridge and river</li> </ul>	<ul> <li>Poles of street lighting and electricity and billboard</li> <li>Road side usage are townhouse, retail and office</li> </ul>
	Traffic Condition and Public Transportation Route	<ul> <li>The traffic relatively dense on workday and smoothly on holyday.</li> <li>Mikrolet M 41 (Kota-Grogol)</li> </ul>	<ul> <li>The traffic relatively dense on workday and smoothly on holyday.</li> <li>Mikrolet M 41 (Kota-Grogol)</li> </ul>	<ul> <li>The traffic relatively dense on workday and smoothly on holyday.</li> <li>KWK 25 and 01, Bus Way (Muara Karang-Kampung Rambutan) and Kopaja 86</li> </ul>	<ul> <li>The traffic relatively dense on workday and smoothly on holyday.</li> <li>Angkot M 10 (Tanah Abang-Kota) and Metro mini 80 (Grogol-Kali Deres)</li> </ul>
	Geometric	7W m 6 m 2 m 2 m 2 m 2 m 2 m 2 m 2 m 2 m 2	10 m 2 m 1 m 1 m 7 1,5 m	14,5 m 4 m 4 m 4 m 7 3 m	. 2 m m
location/Name of	Road Segment	JJ. Jembatan Besi ROW awal TL TL SR SW SW DR	JJ. Jembatan Besi ROW akhir TW TL SR SR SW DR	Jl. Lutumenten ROW TW TL SR SR SW DR	JI. Mohammad ROW mansyur TW (tembusan Hasyim TL Akbar) SR SW DR
	STA.	33.	34.	35.	36.

	Photo Doc				08.0 - 2.02/L/3
	Underground Infrastructure	<ul> <li>Telephone         <ul> <li>Cable</li> <li>Drinking</li> <li>water pipe</li> </ul> </li> <li>(PAM)</li> </ul>	Telephone Cable     Drinking     water pipe (PAM)	Telephone Cable     Drinking     water pipe (PAM)	Telephone Cable     Drinking     water pipe (PAM)
Description of Road Condition	Majority Usage of Side Walk and Road Side	<ul> <li>Poles of street lighting and electricity</li> <li>Road side usage are Settlement on the right and left road corridor</li> </ul>	<ul> <li>Poles of street lighting and electricity and billboard</li> <li>Road side usage are townhouse, retail and office</li> </ul>	<ul> <li>Poles of street lighting and electricity and billboard</li> <li>Road side usage are townhouse, retail and office</li> </ul>	<ul> <li>Poles of street lighting and electricity and billboard</li> <li>Road side usage are townhouse, retail and office</li> <li>Existing train station</li> </ul>
	Traffic Condition and Public Transportation Route	The traffic relatively dense on workday and smoothly on holyday.	The traffic relatively dense on workday and smoothly on holyday.	<ul> <li>The traffic relatively dense on workday and smoothly on holyday.</li> <li>Metromini (Senen –Benhil) and Bus way (Blok M – Kali Deres and Priuk – Blok M)</li> </ul>	The traffic relatively dense on workday and smoothly on holyday.
	Geometric	ROW m TW 5,5 m TL 1 m SR 2 m SW - DR 1 m	ROW m TW 12 m TL 2 m SR 1 m SW - DR 1,5 m	KOW m TW 12 m TL 4 m SR 2 m SW 4 m DR 3 m	KOW m TW 8 m TL 3 m SR 1 m SW 1,5 m DR 2,5 m
Location/Name of		Jl. Veteran	Jl. Pecenongan	JI. POS	Jl. Taman Sahari
	STA.	37.	38.	39.	40.

	Photo Doc				FTT/2012 - 25.11
	Underground Infrastructure	<ul> <li>Telephone         Cable         Drinking         water pipe         (PAM)     </li> </ul>	Telephone Cable     Drinking     water pipe (PAM)	Telephone Cable     Drinking     water pipe (PAM)	Telephone Cable     Drinking water pipe (PAM)
Description of Road Condition	Majority Usage of Side Walk and Road Side	<ul> <li>Poles of street lighting and electricity and billboard</li> <li>Road side usage are townhouse, retail and office</li> </ul>	<ul> <li>Poles of street lighting and electricity and billboard</li> <li>Road side usage are townhouse, retail and office</li> </ul>	<ul> <li>Poles of street lighting and electricity</li> <li>Road side usage are setllement</li> </ul>	<ul> <li>Poles of street lighting and electricity and billboard</li> <li>Road side usage are townhouse, retail and office</li> </ul>
	Traffic Condition and Public Transportation Route	<ul> <li>The traffic relatively dense on workday and smoothly on holyday.</li> <li>Angkot 01 (Pasar Baru- Kota)</li> </ul>	<ul> <li>The traffic relatively dense on workday and smoothly on holyday.</li> <li>Angkot 01 (Pasar Baru Kota)</li> </ul>	The traffic relatively dense on workday and smoothly on holyday.	<ul> <li>The traffic relatively dense on workday and smoothly on holyday.</li> </ul>
	Geometric	ROW m TW 8,5 m TL 2 m SR 2 m SW - DR 1 m	ROW m TW 8,5 m TL 2 m SR 2 SW 3 m DR 1 m	ROW m TW 5,5 m TL 1 m SR - m SW - DR 1 m	TW 10 m TL 3 m SR 1 m SW - DR 1,5 m
Location/Name of	Road Segment	JI. Mangga Besar 5 RON TW TL SR SW SW DR	JJ. Buni ROV TW TW TL SR SW SW DR	JI. Manggis ROV TV TL SR SW DR	Jl. Jayakarta RON TW TL SR SR SW SW DR
	STA.	41.	42.	43.	44.

	Photo Doc		AFANTA CITA
	Underground Infrastructure	<ul> <li>Telephone         Cable         Drinking         water pipe         (PAM)     </li> </ul>	Telephone     Cable     Drinking     water pipe     (PAM)
Description of Road Condition	Majority Usage of Side Walk and Road Side	<ul> <li>Poles of street lighting and electricity and billboard</li> <li>Road side usage are townhouse, retail and office</li> </ul>	<ul> <li>Poles of street lighting and electricity and billboard</li> <li>Road side usage are townhouse, retail and office</li> </ul>
	Traffic Condition and Public Transportation Route	<ul> <li>The traffic relatively dense on workday and smoothly on holyday.</li> <li>Angkot 53 (pulo gadung-kota), Metro mini 02 (muara karang- senen) and bus way (Ancol - Kampung Melayu)</li> </ul>	<ul> <li>The traffic relatively dense on workday and smoothly on holyday. Angkot 53 pulo gadung kota</li> </ul>
	Geometric	m 10 m 4 m 2 m - m	8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8
	Gec	ROW TV TL SR SW DR	ROW TV TL SR SW DR
Location/Name of	Road Segment	JI. Gunung Sahari	JI. Mangga Besar Raya
į	STA.	45.	46.

Preparatory Survey for PPP Infrastructure Project Sewage Treatment Plant Project in DKI Jakarta

## JKT Sewerage Zone-1 PPP-FS Working Group Meeting

18 June 2012

#### AGENDA of Working Group #1

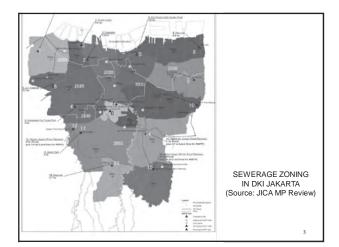
#### A. Technical: Zone-1 sewerage facilities

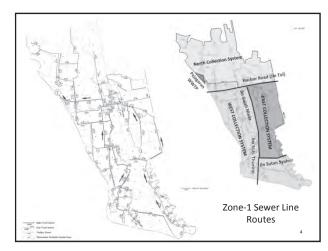
- A-1 Sewer line routes
- A-2 Prediction of sewage inflow based on sewer construction progress
- A-3 WWTP capacities to be balanced with sewage inflow
- A-4 Zone-1 development phasing plan

#### B. Financial:

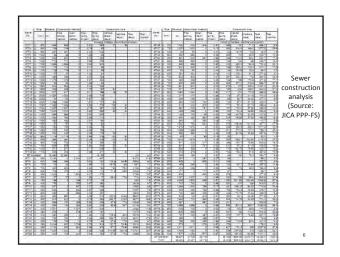
- B-1 Consideration for PPP model
- B-2 Construction cost and Service fee
- B-3 Preliminary Analysis on Cost sharing
- B-4 Key to Success of the Project

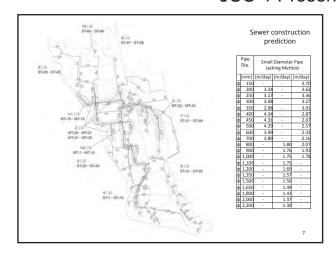
#### C. Subsequent PPP-FS schedule

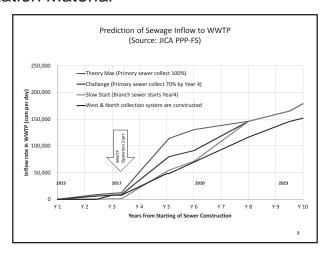


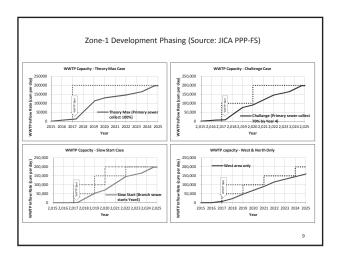


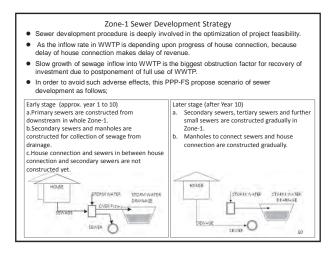
Pipe iamete	r Distance	Constructio	on Method	Pipe		Distan	ice of P	ipe Jack	ing Me	thod			nce of I			No	s of N	tanhol	e		Nos o
(mm)	(m)	Pipe jacking method	Open trench method	Slope (%)	3 m	5 m	7 m	9 m	10 m	>10m 5	ubTotal	1.5 m	3 m	5 m :	SubTotal	Type 1 T	Type 2	Type 3	Type 4 S	ubTotal	DC
15	168	0	168	3.0	0	0	0	0	0	0	0	168	0	0	168	2	0	0	0	2	
20	1,397	713	684	3.0	100	613	0	0	0	0	713	0	684	0	684	7	14	0	0	21	:
25	1,478	508	969	2.8	0	508	0	0	0	0	508	0	969	0	969	11	8	0	0	19	
30			4,082		0	1,891	0	0	0	0	1,891	2,471	1,610	0	4,082	42	31	0	0	73	3
35		2,996			800	2,196	0	0	0	0	2,996	588	4,247	0	4,835	51	49	0	0	100	
40		5,658			100	1,324	1,734	0	2,500	0	5,658	752	4,521	0	5,273	44	106	0	0	150	6
45		1,820			246	500	1,074	0	0	0	1,820	602	2,691	0	3,293	38	30	0	0	68	
50		3,441	3,663			665	0	1,220	814	742	3,441	2,434	1,228	0	3,663	33	53	0	0	86	
60		5,222	1,298			637	1,222	2,951	411	0	5,222	0	1,298	0	1,298	13	67	0	0	80	
70		10,850				560	2,537	2,558	803		10,850	65	0	0	65	0	0	139	0	139	
80		8,610	789			3,723	278	0	1,368	3,241	8,610	0	789	0	789	0	0	53	0	53	
90		4,371		2.0		704	1,116	0	0	2,552	4,371	0	0	0	0	0	0	18	0	18	
1,00				1.8		1,647	1,285	0	38	4,196	7,165	0	0	0	0	0	0	20	0	20	
1,10		3,283		1.6	0	0	629	0	0	2,654	3,283	0	0	0	0	0	0	11	0	11	
1,20		1,810		1.6	0	0	0	0	0	1,810	1,810	0	0	0	0	0	0	7	0	- 1	
1,35 1,50		1,130		1.5		0	-	-	0	1,130	1,130	0	0			0	-		-	1	
1,50 1,65		970		14	0	0	0	0	0	466 970	466 970	0		0	0	0	0	2	0	- 1	
p 1,65		970		12	0	0	0	0	0	970	970		0	0	,	0	0	0	0	1	
p 1,80 b 2.00		1.941		12	0	0	0	0	0	1 941	1.941		0	0	,	0	0	0	5	,	
2,20			0	1.1	0	0	0	0	0	1,423	1,423	0	0	0	0	0	0	0	5	5	
Tota	89,386	64,267	25,119		2,862	14,967	9,875	6,730	5,934	23,900	64,267	7,081	18,038	0	25,119	241	358	258	10	867	44
			1) Des	ign Flow	(daily a	averag	e & pe	ak)			m³/d			200	0,000		1				
			2) Des	ign Flow	(Timel	y peak	)				m³/d			300	0,000						
			Max	permiss	ible ov	erload	for me	chanic	al												
				reatmen						% of c	lesign	value		Not c	lecide	1				5	

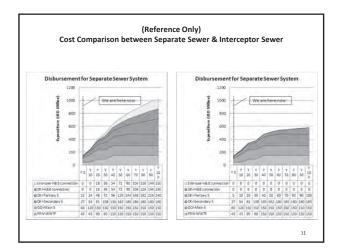












## Proposal on Business Model JICA PPP-FS Zone1

(for Pre-JCC1 Meeting)

- 1. Conditions and Constraints
- 2. Cases of Phasing Construction of WWTP
- 3. Points to be discussed in related to Cases of Phasing Construction

June , 2012 JICA PPP-FS Study Team

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### Conditions given by Indonesian sides in Zone 1 Sewerage Development

- A. Policy Target of Sewerage Service Coverage in DKI Jakarta shall be at 20% by 2020
  - ⇒Sewage Treatment Required in 2020
    - = 20% × Total discharge in JKT
    - = 403,462m3/d(this figure based on M/P Draft Final)
- B. Zone 1 contribution
  - = 198,000m3/average day
  - = 49 % of Sewage Treatment required in 2020
- C. WWTP Operation shall be started in 2017
- D. Treated Sewage Effluent Water Quality shall be complied with Environmental Regulations.

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#### Physical Constraints for PPP Driver in Zone 1 Sewerage Development

- (A) Taking into account JICA ODA Loan Standard Process, sewer pipe construction will be started in 2015.
- (B) Then the estimated Zone-1 WWTP inflow rate in the end of 2016 is estimated to be 6,039m3/average-day.
- (C) As well as the above (B), the estimated WWTP inflow rate in the end of 2018 is about 79,926m3/average-day at the completion of Trunk Sewers Construction.

14

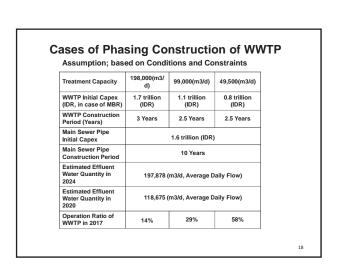
### Financial Constraints in Sewerage Development in Zone 1

- (A) Construction Period for Sewer Pipe; within 10 Years (Grace Period of JICA ODA Loan is Maximum 10 Years)
- (B) Shortening the Schedule for necessary Procedures for JICA ODA Loan is limited ex)EIA Procedure before Commencement of Construction

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## 

# Item of Cost, Type of Payment to SPC and Supplementation for Service Fee (in O&M Period) Item of Cost O&M Cost (O&M cost included Additional Capex) Type of Payment to SPC Supplementation for Service Fee from Local Govt Supplementation for Service Fee Characteristic of Sewerage Project> Initial and Additional capex... Huge Full cost recovery by Tariff...Impossible The financial gap should be supplemented in Construction Period and/or in O&M Period



#### Points to be discussed in related to Cases of Phasing Construction 1

Based on the current Pipe Network Plan, Estimated Effluent Water Quantity (Average Daily Flow) will much increase in 3~4 years.

in 2 Years(2016); 6,039m3/d in 3 Years(2017); 28,525m3/d in 4 Years(2018); 79,926m3/d

- In 2020 (after 6 Years since the Commencement of Pipe Network Construction by JICA ODA Loan), Estimated Effluent Water Quantity is 118,675m3/d.
- ·High Operation Ratio of WWTP is important, but if the number of Phasing Construction increase, we will have more Inflation Risk in the future.

Points to be discussed in related to Cases of Phasing Construction 2

- •For the increase of WWTP Operation Ratio, additional Construction for Sewer Pipe Networks is necessary.
- Shortening the Schedule for necessary Procedures for JICA ODA Loan is limited.
  - We want to ask DKI to priorly invest to Sewer Pipe Networks (for example, 2 Years Prior Invest)
- ·We want to discuss the extent of Fiscal Burden by GOI and DKI.

Possible Amount of Initial Capex Investment will determine Phasing Construction Pattern.

#### Thank you for your attention!!

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#### B. Financial: Zone-1 financial structures

- B-1 Consideration for PPP model
- B-2 Construction cost and Service fee
- B-3 Preliminary Analysis on Cost sharing
- B-4 Key to success of the Project

(for your references)

- > Finance for Sewerage in Japan and USA
- > Tariff level all over the World and Concept of Full cost recovery
- Development Strategy of Manila Water
- > Step-wised House Connection in JKT Sewerage

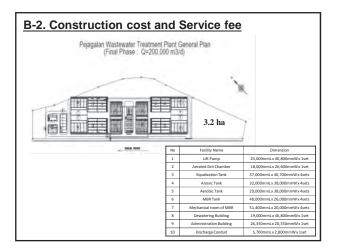
#### **B-1. Consideration for PPP model** Pipe network 290 million US\$ (61%) (39%)GOI GOJ Public SPC Typical PPP 185 million US\$ Impossible Note: GOI is very negative to increase "Foreign Debt". GOJ is very positive to accelerate "PPP by Japanese Companies".

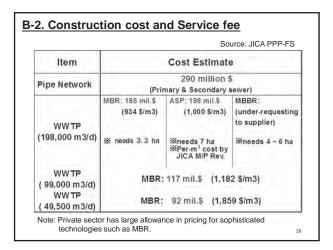
#### B-1. Consideration for PPP model

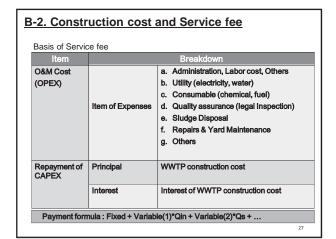
#### **Proposed Demarcation of Zone-1 PPP development**

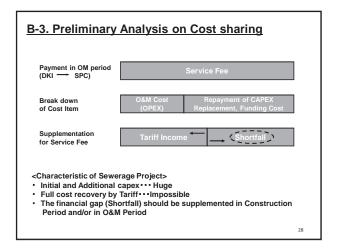
	Budgetary source	Management	Implementation	
Construction of WWTP	Private	DKI Jakarta	Private	
Construction of Sewers	ODA Loan	GOI / DKI Jakarta	GOI / DKI Jakarta	
O&M of WWTP	Tariff / Subsidy	DKI Jakarta	Private	
O&M of Sewers	Tariff / Subsidy	DKI Jakarta	DKI Jakarta	

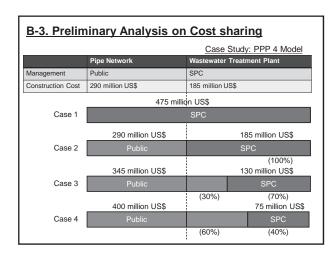
- 1. O&M Company will be set up jointly by DKI Jakarta and Japa 2. One agreement to cover EPC and O&M is applied to WWTP.

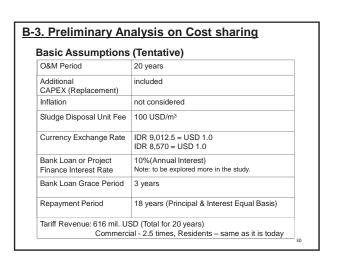


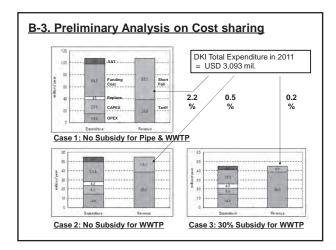


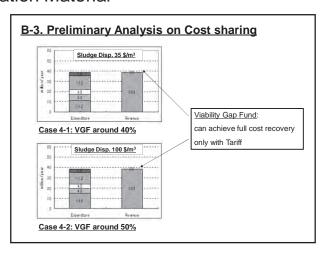




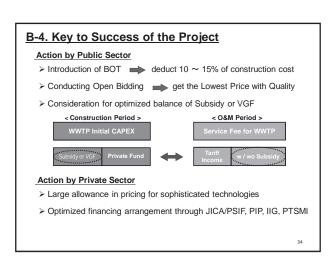


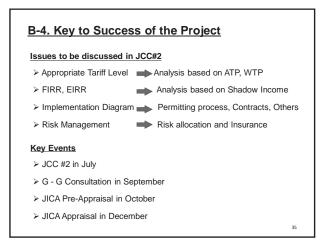






#### B-3. Preliminary Analysis on Cost sharing Summary on Service fee Variation for DKI Budget Interest of Sewer Private fund Investment Case 1 Private 10% Annu. Private 68.9 2.2% 0.5% 16.1 Case 2 Private -ditto-Case 3 70 % Private -ditto-6.0 0.2% 30 % Public 60 % Private Public 0.0 0.0% Case 4-1 -ditto-40 % VGF 50 % Private 50 % VGF Case 4-2 -ditto-Public 0.0 0.0%







Preparatory Survey for PPP Infrastructure Project Sewage Treatment Plant Project in DKI Jakarta

#### JKT Sewerage Zone-1 PPP-FS J C C Meeting 2 nd

26 July 2012



Japan International Cooperation Agency

#### AGENDA of JCC 2

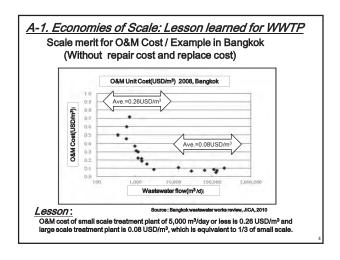
#### A. Standard of Sewerage: Lesson learned in other countries

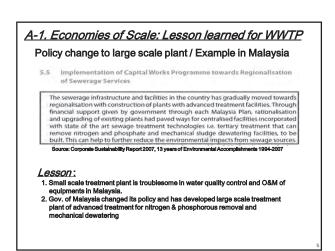
- A-1 Economies of Scale/Proper Volume & Sewer Type
- A-2 Financial Support for Sewerage Development/Subsidy or Charge
- A-3 Affordable Tariff Level and Cross-subsidy
- A-4 Practice and Issues of Other Cities in Indonesia

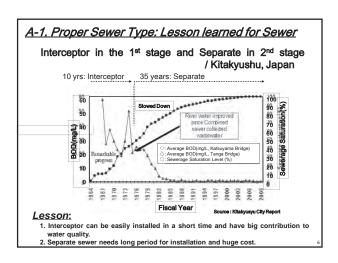
#### B. Financial Simulation: Affordable Tariff & Impact of Subsidy

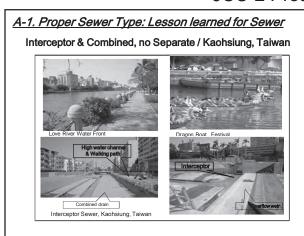
- B-1 Evaluation of Treatment Process for Pejagaran/MBR, MBBR, ASP
- B-2 Tariff Settings and Affordability
- B-3 Impact Analysis of Subsidy and VGF
- B-4 Evaluation of Cost Recovery by EIRR
- C. Implementation Structures and Short Term Schedule

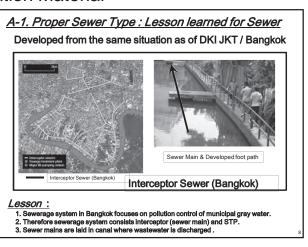
## A-1. Economies of Scale: Lesson learned for WWTP Scale merit of Sewerage system / Development in EU The European Water Association and The German Association for Water, Wastewater and Waste 46.% Lesson: Life cycle cost of per capita 50,000 PE is approximately 50 % of 1,000 PE. Catchment areas with bigger WWTPs are more efficient - both in terms of cost and pollution control

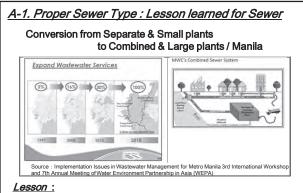






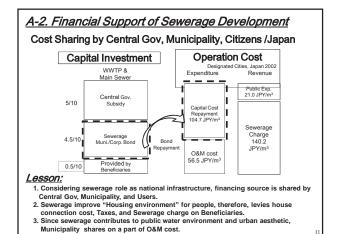


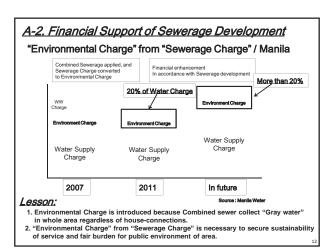


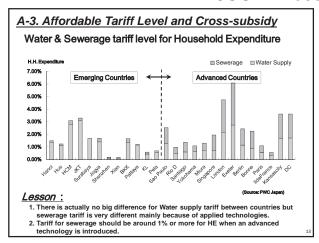


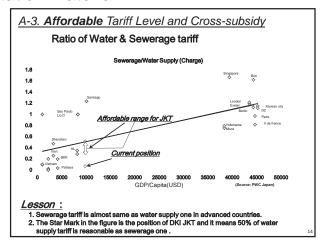
## Combined sewerage system has been replacing separate one due to provide 100 % collection of wastewater urgently and to reduce project cost. Combined sewerage system is defined to "a transitional provision for wastewater collection until separate systems are in place".

#### A-2. Financial Support of Sewerage Development Change of Financial Support of Federal Gov. / USA Federal Gov Subsidy Federal Gov Subsidy Subsidy 55% 55% 75% Municipal Gov Bond 25 % 1956 - 1971 Water Pollution Control Act Amendment Federal Water Pollution Control Act Lesson: Legal system is amended from wastewater treatment/pollution control to sound water environment. Subsidy system has enhanced municipality to accelerate WWTP installation.









## A-3. Affordable Tariff Level and Cross-subsidy Recommendation for DKI JKT Current Tariff level in DKI JKT: Water supply tariff is 3.2% of monthly HE. Sewerage tariff is merely 0.2% of monthly HE. Lesson learned: Sewerage tariff should be 1% to 3.3% for monthly HE. Cross-subsidy is applied from Commercial to residents in almost all countries. Analysis for DKI JKT: 1% to 1.6% of monthly HE is quite Affordable for residents. Sewerage tariff equivalent to 1 % is 5 times of current one. For commercial, 2.5 times of tariff increase is still affordable. Recommendation: For commercial facilities, it should be raised 2.5 times. For residents, it can be kept in the current level.

## 1. Treatment process Aerated lagoon in most cities.(Lagoon 10, RBS 1, and ASP 1) 2. Sewer type Sewer mains are constructed first and construction of house connection has been just started. 3. Finance STP and main sewers are constructed by APBN (Central government fund). Branch sewers are constructed by either APBN (Central government fund) or APBD (Local government fund). House connections are constructed by APBD. 4. Operation and maintenance O&M cost are owned by APBD. 5. Tariff system Flat rate or Per-floor rate Ratio of resident tariff / commercial tariff are various between 1 to 2.5 – 6.0. Ratio of resident tariff / industrial tariff are also various between 1 to 4 – 20.

A-4. Practice & Issues of Other Cities in Indonesia

#### A-Summary:

Lessons learned to Sewerage System in DKI JKT

#### Lessons learned from foreign countries

- 1) Bigger treatment plants are obviously efficient on any points.
- Interceptor and Combined Sewer is Standard in all Asian Cities.
- 3) Sewerage charge is collected without house-connection in all Asian Cities.
- 4) Central Gov. Subsidy is necessary and 55 75% for WWTP.
- 5) Affordable tariff and Cross-subsidy is key for sustainability.

#### A-Summary:

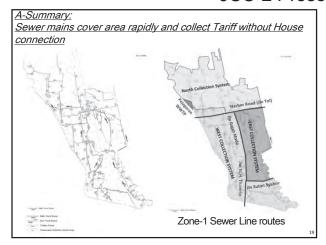
based on the GDP growth.

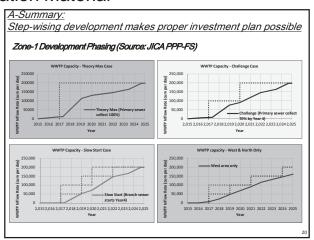
#### Recommendation to Sewerage System in DKI JKT

- \_ Appropriate development way of Sewerage System in DKI JKT
- a) Step-wising and sewer main construction in advance Sewer main collects wastewater (gray water), which is a principal pollution load, quickly and widely.
- b) Setting of Affordable tariff and Cross-subsidy
  Tariff for Commercial should be raised 2.5 times at the beginning
  of the project according to the installation of sewer mains and regularly

#### Collaboration with urban development project

Effluent quality of on-site systems should be monitored, then encourage Commercial to access to public sewer connection. Sewer main will provide public sewer connection to urban development & re-development projects (= Separate sewer).





#### AGENDA of JCC 2

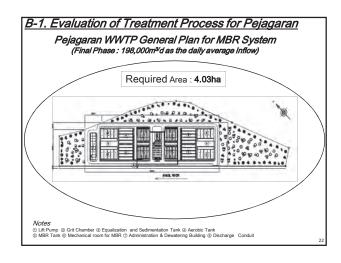
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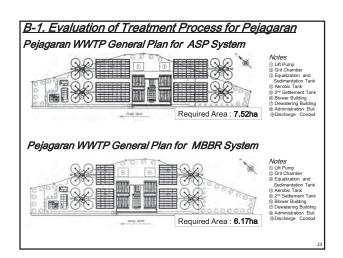
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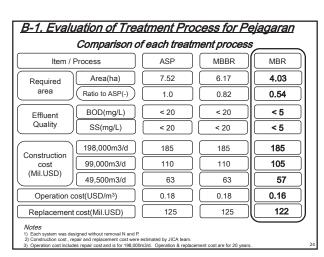
#### B. Financial Simulation: Affordable Tariff & Impact of Subsidy

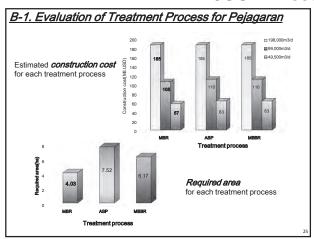
- B-1 Evaluation of Treatment Process for Pejagaran/MBR, MBBR, ASP
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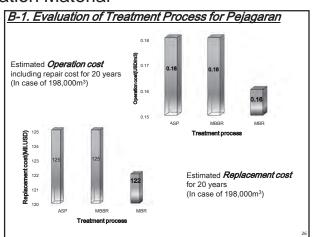
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#### B-2. Tariff Settings and Affordability

#### Recommendation from A-3:

For commercial facilities, it should be raised 2.5 times. For residents, it can be kept in the current level.

#### Common condition:

- 1. Interceptor is widely installed in Zone-1 in 2007.
- 2. Therefore, DKI JKT starts collection of tariff in 2007.
- 3. GDP increase of DKI JKT is the same as National average (6%).

#### For Commercial:

Tariff is raised 2.5 times from current one in 2017. Then, it is raised by cumulative GDP increase every 3 years. Commercial floors increase with GDP increase every year.

#### For Residents:

Tariff is the same as current one. (0.16% of monthly HE)
Then, it is raised by cumulative GDP increase every 3 years.
Burden is same or lower than current level through Project period.

#### B-2. Tariff Settings and Affordability

Comparison between MP review and PPP/FS

Unit(Mil. IDR)

2017	2018	2019	2020		2030	2034		2036
22,391	37,433	54,292	61,106	П	162,381	146,415		188,900
0	0	5,133	1,754		16,178	2,950		24,365
22,391	37,433	59,426	62,860		178,559	149,365		213,265
0	0	130,701	137,931		403,686	508,413		802,060
-22,391	-37,433	71,275	75,071		225,127	359,047		588,795
					37,892	37,892		45,130
					451,126	451,126		537,405
L	ess than .	half most	<u>ly</u>		→ 17,243	20,536		24,559
					386,444	487,876		777,600
	22,391 0 22,391 0 -22,391	22,391 37,433 0 0 22,391 37,433 0 0 -22,391 -37,433	22,391 37,433 54,292 0 0 5,133 22,391 37,433 59,426 0 0 130,701 -22,391 -37,433 71,275	22,391 37,433 54,292 61,106 0 0 5,133 1,754 22,391 37,433 59,426 62,860 0 0 130,701 137,931	22,391 37,433 54,292 61,106 0 0 5,133 1,754 22,391 37,433 59,426 62,860 0 0 130,701 137,931 -22,391 -37,433 71,275 75,071	22,391 37,433 54,292 61,106 162,381 0 0 5,133 1,754 16,178 22,391 37,433 59,426 62,860 178,559 0 0 130,701 137,931 403,686 -22,391 -37,433 71,275 75,071 225,127  237,892 451,126  Less than half mostly > 17,243	22,391 37,433 54,292 61,106 162,381 146,415 0 0 0 5,133 1,754 16,176 2,950 22,391 37,433 59,426 62,860 1778,559 149,365 0 0 130,701 137,931 403,686 508,413 -22,391 -37,433 71,275 75,071 225,127 359,047 37,892 37,892 451,126 451,126 451,126 -37,243 20,536	22,391 37,433 54,292 61,106 162,381 146,415 0 0 5,133 1,754 16,178 2,950 22,391 37,433 59,426 62,860 178,559 149,365 0 0 130,701 137,931 403,686 508,413 -22,391 -37,433 71,275 75,071 225,127 359,047  Less than half mostly 71,243 20,536

Less than Estimate of MP Review for 14 years

#### B-3. Impact Analysis of Subsidy and VGF : Review of JCC1

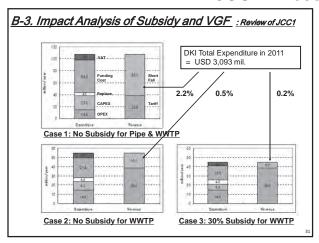
#### Case Study: PPP 4 Model

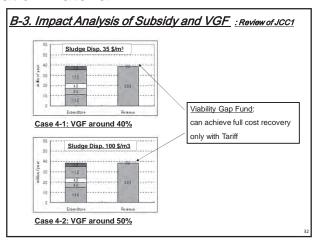
		Case 3	luuy	. FFF 4 WOULE			
Item	Pipe Network	Wastew	/ater T	reatment Plant			
Management	Public	SPC					
Construction Cost	290 million US\$	1	85 mil	lion US\$			
	475 mill	ion US\$					
Case 1		SPC					
_	290 million US\$		18	5 million US\$			
Case 2	Public		SPC				
	345 million US\$		13	(100%) 60 million US\$			
Case 3	Public			SPC			
	400 million US\$	(30%)		(70%) 75 million US\$			
Case 4	Public			SPC			
		(60%)		(40%)			

#### B-3. Impact Analysis of Subsidy and VGF : Review of JCC1

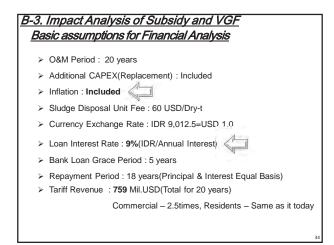
#### **Basic Assumptions (Tentative)**

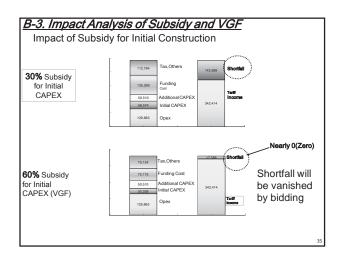
O&M Period	20 years
Additional CAPEX (Replacement)	included
Inflation	not considered
Sludge Disposal Unit Fee	100 USD/m <sup>3</sup>
Currency Exchange Rate	IDR 9,012.5 = USD 1.0 IDR 8,570 = USD 1.0
Bank Loan or Project Finance Interest Rate	10%(Annual Interest) Note: to be explored more in the study.
Bank Loan Grace Period	3 years
Repayment Period	18 years (Principal & Interest Equal Basis)
Tariff Revenue: 616 mil. US Commerci	SD (Total for 20 years) al - 2.5 times, Residents – same as it is today

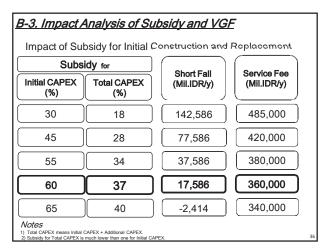


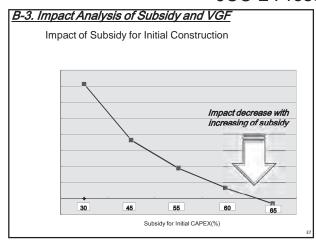


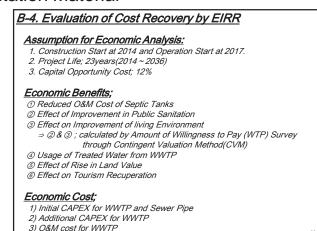
#### B-3. Impact Analysis of Subsidy and VGF : Review of JCC1 Summary on Service fee Variation Assumptions Subsidy in OM Period WWTP Interest of Sewer (milli.USD/y) **DKI Budget** investment Private fund Investment Private Case 1 10% Anu. Private 68.9 Private Case 2 -ditto-Public 16.1 0.5% 70 % Private -ditto-0.2% Public Case 3 6.0 30 % Public 60 % Private -ditto-Public 0.0 0.0% (40 % VGF) 50 % Private Case 4-2 50 % VGF Public 0.0% -ditto-0.0

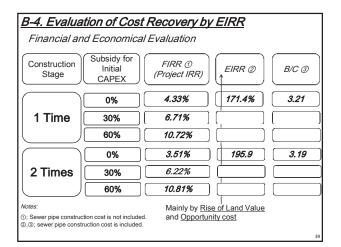


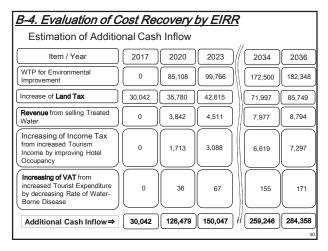


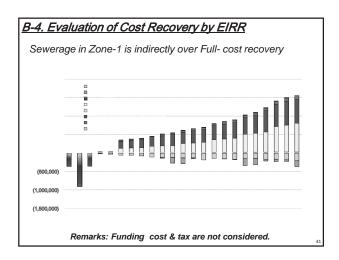


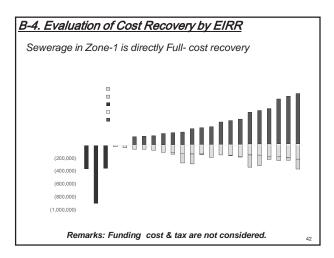


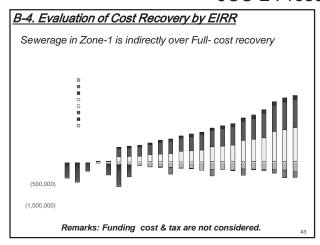


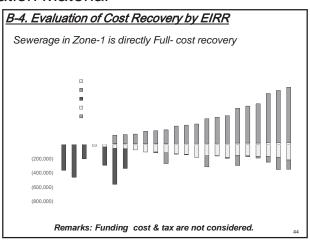












B-Summary:
Recommendation to Sewerage System in DKI JKT

Appropriate development way of Sewerage System in DKI JKT

a) Treatment Process for Pejagaran

1. MBR is the best from the points of Cost Performance, Area Constraints, and Easy O&M.

b) Subsidy and VGF

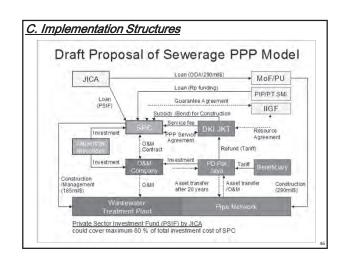
1. Impact of Subsidy decrease with its increasing volume.

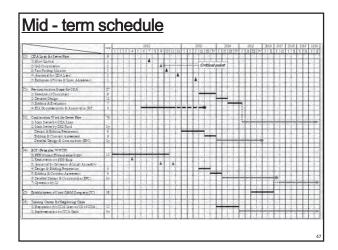
2. 35–40% Subsidy is enough for construction and replacement in Zone-1, which is equivalent to 60% subsidy of 1 time in BOT.

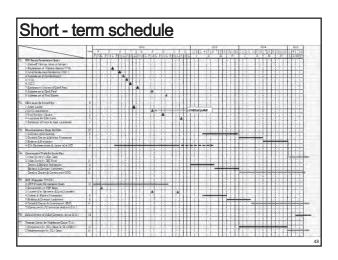
c) Cost Recovery

1. Sewerage in Zone-1 is viable because of positive EIRR.

2. Sewerage in Zone-1 is indirectly full-cost recovery project.



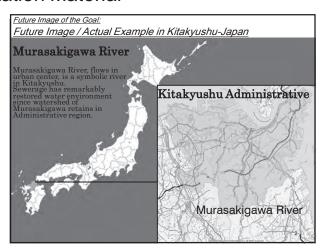


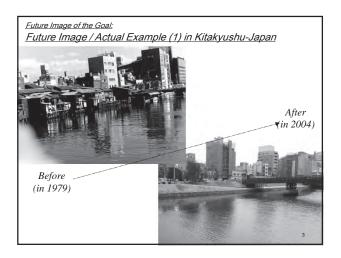


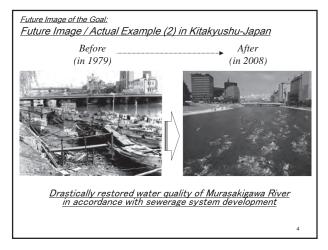
#### Sewerage PPP Project in DKI JKT

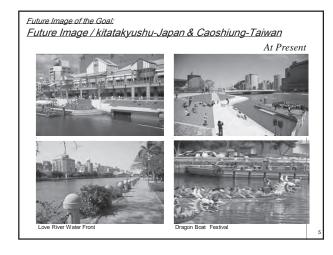
- > Future Image of the Goal
- Issues and Solutions
- > Cost, Revenue, and Fund Resource
- > Steps to the Initiation

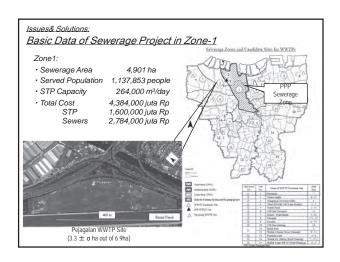
December 5, 2012 <u>JICA PPP Study Team</u>





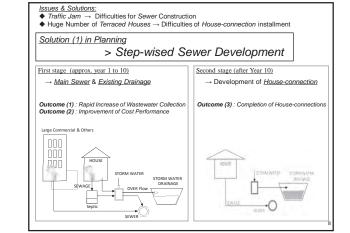


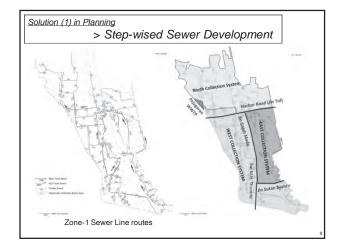


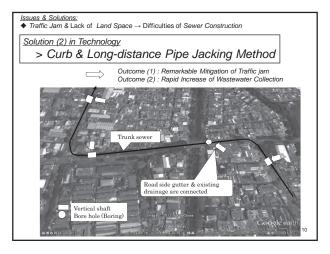


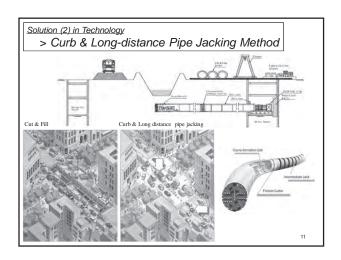
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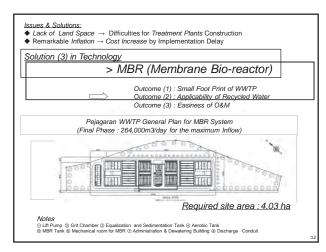
◆ Lack of Environmental Consciousness → Low Interest for Sewerage
 ◆ Huge Unbalance of Revenue → Difficulties of Tariff Setting

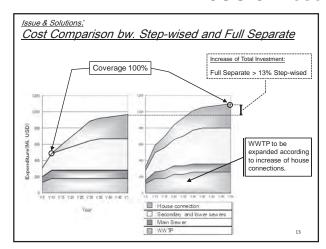


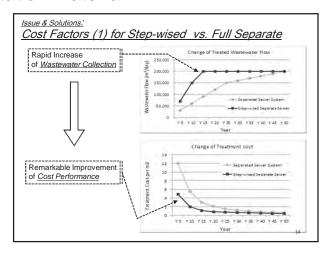


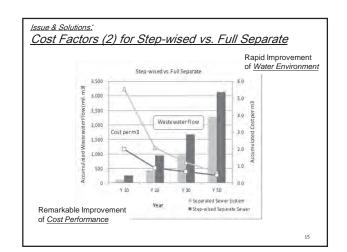


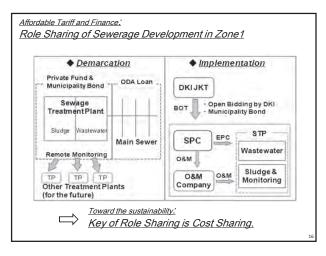




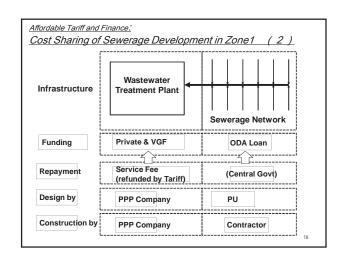


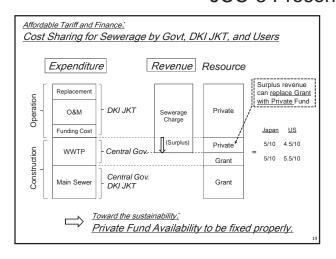


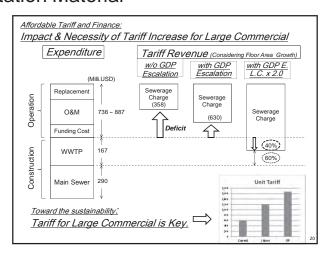


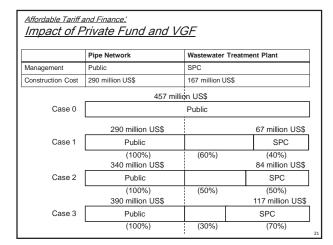


	Budgetary source	Management	Implementation
Construction of WWTP	Private / Public	GOI / DKI Jakarta	Private
Construction of Sewers	ODA Loan	GOI / DKI Jakarta	GOI / DKI Jakarta
O&M of WWTP	Tariff / Subsidy	DKI Jakarta	Private
O&M of Sewers	Tariff / Subsidy	DKI Jakarta	DKI Jakarta

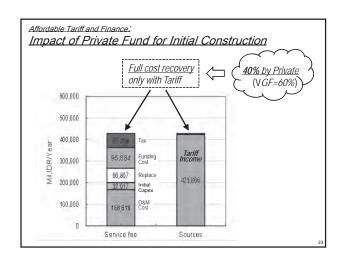


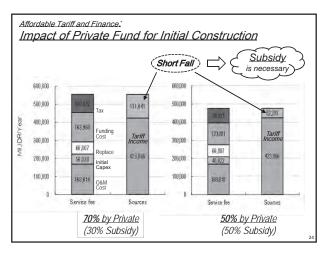


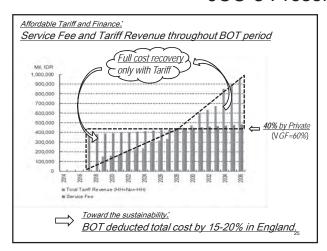


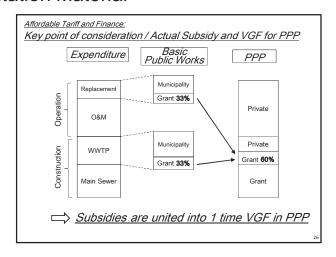


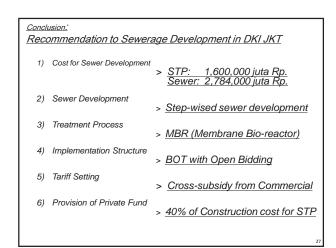
Basic Assumption of Financial Analysis > O&M Period: 20 years > Additional CAPEX(Replacement) : Included > Inflation : Included > Sludge Disposal Unit Fee : 60 USD/Drv-t > Currency Exchange Rate : IDR 9,012.5 = USD 1.0 ➤ Loan Interest Rate : 9% (IDR/Annual Interest) > Bank Loan Grace Period : 5 years > Repayment Period : 18 years > Tariff Revenue : 940 Mil.USD 1) Total for 20 years
2) New unit tariff adopted
3) GDP E & L.C. x 2.0
4) Floor Area Growth

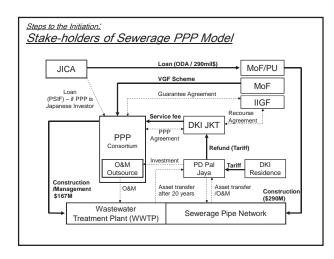


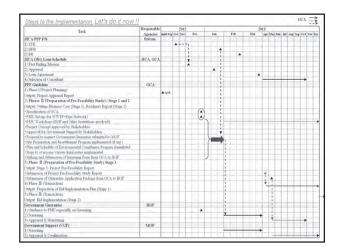


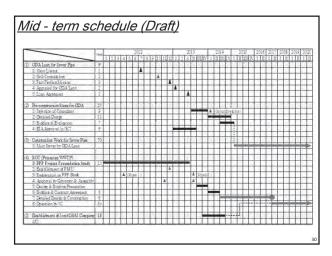


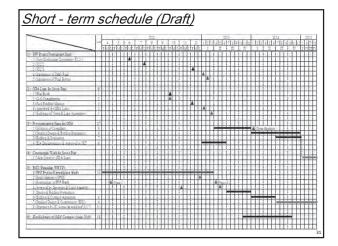












### Terima Kasih!

#### Solicited projects or Unsolicited

Whether the Project is classified as a solicited or unsolicited project under Indonesian rules and regulations?



The proposed project is considered a solicited project, given that it is contained a master plan for sewerage sector in Jakarta and listed in the 2012 PPP Book.



In case of solicited projects,

\*procurements for PPP services must be open bidding (public tender) by the Government

\*project companies may be granted government support.

#### Solicited projects or Unsolicited

#### [Details]

- The PPP Presidential Regulation acknowledges two types of PPP projects, namely those which have been indicated by the Government (solicited) and those who have not (unsolicited).
- One of the requirements for unsolicited project is "not included in the master plan of the relevant sector"
- The PPP Presidential Regulation states that Solicited projects are openly published and accessible. In the 2012 PPP Book, the Project is included as solicited project.

#### [Relevant Laws]

Presidential Regulation No. 67 of 2005 on Cooperation between the Government and Private Entities in Infrastructure Procurement as amended by Presidential Regulation No. 13 of 2010 and lastly amended by Presidential Regulation No. 56 of 2011

7. Memorandum of JCC

## MINUTES OF MEETING JOINT COORDINATION COMMITTEE

#### **JUNE 21, 2012**

#### SS1 BAPPENAS

**Chair of the Meeting:** Dr. Dedy S. Priatna, Deputy to the Minister of Bappenas

**Participants:** Members of the JCC (See Attached List of Participants)

**Presentation:** (1) Technical Aspect (Mr. Suzuki, See attachments)

(2) Financial Aspect (Mr. Yamamoto, See attachments)

#### **Comments and Discussion:**

#### Dr. Dedy S. Priatna, Deputy o the Minister - Bappenas:

- 1. On the Summary of Service fee Variation (page 33): In the case 4.1, it is stated that the subsidy required for service fee in the operation and maintenance period is 0 per year. This situation can be achieved by cost sharing in Capital Expenditure for WWTP between the private (SPC, 60 %) and public (DKI Jakarta Government, 40%). This option shows that the DKI Jakarta will choose providing "Government Support" for the construction of the WWTP at the initial stage instead of giving subsidy for the service fee to the SPC annually. Several questions can be raised as follows. Will the DKI Jakarta accept this option? Do we believe to the consultant? Is the Japanese standard quality applied in this case not too expensive?
- 2. In the scenario of the report, pipeline sewerage will be provided through the public fund. How about lessons learned from other countries such as Taiwan, Vietnam and others?
- 3. According to the consultant, the technical model applied for the proposed WWTP is a membrane system. How is the trade off between the cost and performance? What are models employed for other countries?
- 4. Options for staging in the construction period of the WWTP proposed by the consultant are very important. Which one should be chosen for optimizing the capacity of the plant and the cost which should be paid?
- 5. Comment on the assumption on flat tariff for the residential uses during the o & m period: A scenario for gradual increase on tariff should be employed in the calculation of affordability to pay of the Jakarta Government for service fee.

#### Mr. Syukrul Amien, Director - Ministry of Public Works

- 1. Prior to giving comments, Mr. Amien Reported to the Chair of the JCC regarding last week meeting between the MPW and the consultant.
- 2. Based on the scenario proposed by the consultant, the construction of sewerage pipelines will be conducted by the government through the public fund (either loan or APBN) due to various reasons. Although the consultant should be responsible to the JICA, the scope of the PPP project should include a particular section or coverage area of the sewerage pipeline.
- 3. Zone 1 is characterized by the domination of commercial activities and accordingly potential for high revenue for a PPP project. Sewerage zones in Jakarta are different from the Zone 1 and many of them are dominated by residential uses. Therefore, replication of the PPP approach employed in the Zone 1 to other zones should consider the characteristic of such zones.
- 4. Comment on the option of construction staging of the WWTP: To build a full capacity of the WWTP at the initial stage will be ineffective because there will be idle capacity of the facility. Accordingly, option no 4 (staging approach) is more appropriate.
- 5. Comment on membrane technology for the WWTP: Membrane technology is very good but expensive. If possible the construction cost should be minimized and staging approach to the construction should be applied.

#### Mrs. Driah Head of Physical Sub Directorate – Bappeda, DKI Jakarta

- Comment on Operation and Maintenance: Are the operation and maintenance proposed by
  the consultant is only limited to the WWTP or they include the sewerage pipeline? As a
  system, O & M suppose to be conducted by one provider, to ensure continues supply of
  waste water intake to the WWTP.
- 2. On tariff and its relation to the revenue: Based on current survey, an initial model on the relationship between tariff and potential revenue in some zones is available.
- 3. PPP project scheme requires involvement of various agencies in the management and implementation processes. Therefore, the FS report should clearly describe their roles, tasks and responsibilities.

#### Mr. Adi A. Head of Economic Bureau – Bappeda, DKI Jakarta

 Since the PPP project will deal with big asset and high value, therefore institutional arrangements for the project management is crucial and should be comprehensively included in the report.

#### Mr. Andono Warih BPLHD-DKI Jakarta

1. Subsidy from the Government (GOI and DKI Jakarta) should be minimized.

#### Mr. Fadly H. Tanjung Ministry of Environment

1. Consultant should take into account the relationship between Zona 1 and other Zones in the management processes including its environmental impacts.

#### Mr. I Wayan Sutana Ministry of Finance

1. Up till now there is no formal regulation on Viability of Gap Funding (VGF) from the MOF. RPMK (Rencana Peraturan Menteri Keuangan/ Draft Regulation of the MOF) will be issued within 2 -3 months later.

#### Mr Nugroho, Director of Settlements and Sanitation – Bappenas

- 1. Based on lessons learned (practices), up till today, there is no Sewerage Projects in the world, Financially Feasible (no subsidy).
- The sewerage system projects are based on principle of improving the welfare of the society or on social-economic benefit. In this case the government support or subsidy is categorized as sunk cost.
- 3. It should be emphasized here that the principle of matching grant will be employed in the case of grant from the Government to the local government.

#### Mr. Bastary, Director of PPP – Bappenas

1. Experiences in several cities in Indonesia show that provision of Sewerage System through merely public fund, does not work well due to the limited budget of the national as well as local governments.

2. The WWTP PPP DKI Jakarta project may be considered as a "good potential project" since it is not purely public fund but a combination of both the private and public funds.

#### Mr. Syukrul Amien, Director - Ministry of Public Works

- 1. Repeated his comments and messages in English language.
- 2. Emphasized that the Study is a grant from the JICA to the GOI. Therefore the project will be treated as a solicited project and will be delivered through a competitive open tender.

#### Conclussion: Mr Dedy S. Priatna, Deputy to the Minister – Bappenas

- 1. Possible options proposed by the consultant in the report will be carefully scrutinized and taken into account in the decision making process by the government of DKI Jakarta, Ministry of Public Works, Bappenas and other main stakeholders.
- 2. Suggestion to the government of DKI Jakarta: Annual subsidy to fulfill the gap between service fee (payment for SPC for Opex and Capex of the WWTP) and revenue from tariff should be avoided. Instead, DKI should provide initial Capex for the WWTP as a sunk cost).
- 3. Consultant should include in the report other countries experiences in providing and managing sewerage facilities.
- 4. Consultant should include a scenario on a package of PPP project in which WWTP and the sewerage pipeline (covers a certain area probably dominated by commercial uses) are bundled, utilizing private fund.
- 5. The DKI Jakarta government will explore the possibility of utilization of VGF from the Ministry of Finance.
- 6. Indonesian consultant, in this regard Mr. Windu is assigned to complete and submit the minutes of meeting.

#### Responses from Mr. Yamamoto

- 1. In responding to the technology employed in the project: China adopts the membrane system and there is no objection to such a method so far.
- 2. Further response will be conducted by the consultant after receiving the translation of MoM

3. The second JCC meeting will be conducted before Ramadhan

#### <u>Limited Discussion After the JCC</u>

- 1. Participants: Dr Dedy (Bappenas),(Mrs Yani (Bappeda DKI Jakarta), Mr. Windu H (Consultant), Kawik Sugiana (Consultant)
- 2. Result of the Discussion:
  - a. Comments from Ibu Yani:
    - Sewer Development Strategy, on early stage: rain water will go to the sewer pipe, therefore it will increase the volume of waste water flowing to the WWTP.
    - ii. The facilities and building should be shifted toward north direction within the site
  - b. Dr Dedy request Mr Windu to include the result of the limited discussion in the Minutes of Meeting of JCC.

Kawik Sugiana Ph.D

## MINUTES OF MEETING JOINT COORDINATION COMMITTEE 2

#### SS 1, Bappenas

Date: July 26, 2012
Agenda: See attachment

Chair: Dr. Dedy S. Priatna, Deputy to the Minister of Bappenas on Infrastructure Affairs

Participants: See attachment

**Reporter:** Kawik Sugiana Ph.D

#### **OPENING**

#### Opening Speech by Dr. Dedy S. Priatna, Bappenas

- Well come to the participants
- Introduced the JICA consultants responsible for formulating the FS of Sewerage Treatment Plants in Zone 1 and Zone 6, DKI-Jakarta
- Both teams were asked to present the progress of their studies in the JCC 2 meeting,
- The Zone 1 is implemented through PPP, while Zone 6 is managed through the ODA loan or Public-funding. Based on those two studies, the GOI will then determine, which approach should be more appropriate for the case of DKI Jakarta

#### Overview from representative of the JICA-Jakarta

- Several Studies have been conducted under the Master Plan of the DKI-Jakarta.
   Some of them will be implemented through PPP approach, including the WWTP of the Zone 1.
- Emphasized the different management approaches employed in Zone 1 and Zone 6.

#### **PRESENTATION**

Presentation of Zone 1: Mr. Kenichi Yamamoto, Orix

See attachment

Presentation of Zone 6: Yachio Consultant

See attachment

#### **DISCUSSION**

#### Dr. Dedy S. Priatna, Bappenas

- He was surprised with the time schedule proposed by the Zone 6 team. Why does the Zone 6 require longer period in the project completion (project formulation, procurement and construction) in comparison with the Zone 1 which adopts the PPP approach? The period of project completion in the case of Zone 6 was too long according to Dr.Dedy
- Two main financial alternatives proposed by the team of the Zone 1, they are: (1) go by loan/public fund and (2) some parts will be funded through private sector.
- Asked the representative of JICA-Jakarta to request the Zone 6 team to conduct financial-calculation similar to the Zone 1 team, to include both the project financing scenarios through public funding and PPP model.

#### Representative of the JICA-Jakarta

• Agreed with the request of Dr. Dedy; the Zone 1 team will include both the project financing scenarios through public funding and PPP model

#### Mr. Syukrul Amien, Director - Ministry of Public Works

Prior to give comments he reported the last week meeting and discussion with the two consultants. Since these projects concern with big investment, decision from related higher decision makers will be required.

- In the JCC1, an option for a step by step construction process was discussed, since there were constraints/problems related to house-connections. However, he had an objection if the construction of WWTP should be divided into 4 steps. A single step for constructing the WWTP is preferable; however, the scope of the water sewage treatment will be the grey water.
- Based on the proposal for the Zone 1, 60 % of the CAPEX of WWTP will come from the government and another 40 % from the SPC, and 100 % CAPEX of the pipe network will be provided by the government, if O % of subsidy is required in the Operation and Maintenance. In this scenario, therefore, around 400 + 110 million IDR is required from the government fund. Accordingly, careful consideration and decision should be conducted by the government of Indonesia, since the decision will concern with huge investment.

• The Zone 6 team offered the cost-sharing scheme between the central and the local governments.

#### Mrs. Yani, Head of the Bappeda DKI-Jakarta

- The government of Jakarta would be very glad with the initiative to solve the environmental matters.
- There are 15 zones in Jakarta. Therefore, the systems and technologies employed across those zones should be designed in such a way to maintain their harmony and consistency. The investment cost for Zone 1 is approximately 4.5 trillion IDR.
- As discussed in the previous limited meeting, the site-plan of the project should be modified towards the north-west direction. The project location is still the same with the initial design.
- In the case of piping network, the existing network and the new one are combined in order to reduce the cost.
- Further clarification was needed: (1) will the black water be channeled through special pipes? Whether user charge will be imposed on the grey water?
- Social study on social- behavior change is required in relation to the burden of tariff imposed on the house-holds. Currently, the tariff for house hold is only 10 15 thousand IDR per month and about 40% of costumers have not paid the bill yet.
- To keep the house hold's tariff similar with the existing charge as proposed by the consultant was inappropriate according to her, since this would not educate the people.
- The DKI-Jakarta government may not be able to solve its environmental problems. Accordingly, support from the central government is strongly required.

#### Dr. Dedy S. Priatna, Bappenas

• Emphasized the need for in-depth social study to influence the behavioral change.

#### Ms. Lily, PD IPAL DKI-Jakarta

• Needed further clarification regarding the technologies and systems applied in Zone 1 and Zone 6; one is using a combined system while the other using a separated system. In the case of the 15 Zones in Jakarta, variation of technologies and systems employed in each zone will complicate the operation of services.

- According to the Consultant, tariff for the commercial users will be increased 2,5 times from the current one. Two issues should be taken into account: (1) Currently, the government of DKI-Jakarta attempts to increase 15 % of the commercial tariff; many of commercial users oppose to such a policy; (2) The proposed tariff should be less than the cost for individual treatment. In this regard, individual septic tank is still allowed according to the environmental law
- Option not to increase the house hold tariff during the concession period should be further elaborated.

#### Dr. Dedy S. Priatna, Bappenas

 Option for keeping the tariff is absolutely wrong, because citizens should appreciate efforts to improve the quality of their environment which in turns will give benefit to them

#### Mr. Adi A, Head of Economic Bureau – DKI Jakarta

• In principal agreed with the proposed scheme presented by consultant.

#### Mrs. Tyas DKI-Jakarta

- Questioned on the Project Cost of Zone 1 and Zone 6: whether the cost includes cost for removal of the existing buildings and facilities.
- DKI-Jakarta government couldn't afford to bear the budget for its development by itself. As a Special Capital District, according to the law, DKI – Jakarta is eligible to get financial support from central government

#### Mr. Syukrul Amien, Ministry of Public Works

- Suggested consultants to handle gray water and black water in Zone 1 and Zone 6.
- To treat black water requires significant cost and intensive participation from society. Therefore black water will be treated through stages. In Stage 1, grey water will be treated. In the absence of black water treatment facility, septic tanks can be used instead. In the sub-sequent stage, black water treatment will be provided.
- Tariff across zones should be similar otherwise protest from consumers will arise.

#### Ms. Lily, PD IPAL DKI-Jakarta

• Back to the Spatial Plan of the DKI-Jakarta. The drainage (for water storm) should be separated from the sewerage. Shall we still consistent with that principle?

• If both of them should be combined, what kind of technology will be used?

#### Mrs. Yani, Head of the Bappeda DKI-Jakarta

- The case of other countries: At the initial stage, a combined system between drainage and sewerage is employed. Later the separated one will be used.
- If the staging approach is more expensive, why not to choose the separated system at the initial stage?

#### Mr. Nugroho, Director of Settlements and Sanitation – Bappenas:

- Explained that it would be ideal if we use the separated system. However, in practice the construction will be very very slow, unless for the new housing complex.
- There are 2 ways could be applied in the combined system:
  - 1. Install the interceptor at the end of the system
  - 2. Install the interceptor on each house, on the outlet system. This way costs less than the first one
- Population will also get other benefits in the Zone 6 method, such as better quality of environment, increasing land value etc.
- Prior to determine whether a system is cheap or expensive, see page 33 of the presentation ("Preparatory Survey for PPP Infrastructure Project Sewage Treatment Plant Project in DKI Jakarta. JKT Sewerage Zone-1 PPP-FS"). Several options of payment are shown: at the beginning, while construction/operation, or at the end of operation or concession period. The more we delay to provide the infrastructure, the more expensive the cost will be.
- Based on best practice in the developed countries, Wastewater tariff should be around 70%-80% of the clean water tariff.

#### Mrs. Yani, Head of the Bappeda DKI-Jakarta

• No other choice, but we have to start immediately.

#### Mr. Bastary, Director of PPP Bappenas

- PPP is not merely a budget oriented based approach, in which private parties will share the budget for infrastructure provision and services.
- Other objective for encouraging private sectors to participate in infrastructure provision is to improve the performance of public services.

•	Therefore, efforts to encourage PPP approach in the WTP should be highly appreciated. Of course the tariff is not fix yet, and further analysis will be required.

#### Mr. Kawik Sugiana, Consultant

- Stated that many other issues should be discussed and decided by the JCC members, in addition to technical, financial and environmental matters. If we take a look at page 43 from the presentation of "Preparatory Survey for PPP Infrastructure Project Sewage Treatment Plant Project in DKI Jakarta. JKT Sewerage Zone-1 PPP-FS", several questions may arise related to legal aspect, institutional aspect etc.
- When those matters will be discussed?

#### Dr. Dedy S. Priatna, Bappenas

• Consultant should provide options. The decision is on government's hands, including the payment method

#### Mr. Nugroho Director of Settlements and Sanitation – Bappenas

• Stated that unfortunately, we had only few days to read the report. At least 2 weeks prior to the discussion, report should be distributed to members of JCC. We need more time to learn, to produce a good decission.

#### Dr. Dedy S. Priatna, Bappenas

- Asked both consultants to take into account issues and decisions in the meeting in the study and to complete the reports
- If a combined system (a combination of drainage and sewerage) is applied, each consultant should collect information on people's opinion.
- Closing the discussion.

## MINUTES OF MEETING JOINT COORDINATION COMMITTEE 3 Sewerage Treatment Plant/ STP PPP Project in DKI Jakarta Akmani Hotel, Jakarta

Date: December 05, 2012 Agenda: See attachment

**Venue:** Lentini and Amalfi rooms,

Akmani Hotel, Jl. KH Wahid Hasyim 91, Jakarta

Chair: Dr. Dedy S. Priatna,

Deputy to the Minister of Bappenas on Infrastructure Affairs

**Participants:** See attachment **Reporter:** Kawik Sugiana Ph.D

#### **OPENING**

#### Opening Speech by Dr. Dedy S. Priatna, Bappenas

- Well come to the participants
- Team leader was asked to present the progress of the study in the JCC 3 meeting,
- The GOI expected that Project Ground Breaking would be conducted in the end of 2013

#### **PRESENTATION**

Presentation: Mr. Kenichi Yamamoto, Orix

- Material of presentation for the meeting included: Future Image of the Goal, Issues and solutions, Costs, Revenues and Funding resources, Steps to the initiation (see attachment)
- Because of the time limitation for the meeting, the presentation and discussion were focused on issues related to *the time schedule* and *finance*.

#### Schedule

- ✓ The Minister of Public Works and the new Governor of DKI Jakarta have agreed on the Sewerage PPP project in DKI Jakarta
- ✓ The construction period would take within 3 years started from the end of 2014 and completed in 2017 (see draft mid-term schedule, presentation 05 Dec, 2005 in the attachment))
- ✓ Based on the above time schedule for the construction, Mr. Yamamoto emphasized the importance of fulfilling the readiness criteria for the PPP project, i.e., clarification of the Government Contracting Agency /GCA, establishment of the Project Management Unit/PMU), formulation of the Pre-Feasibility Study, registration for the next 2013 PPP book (see also Steps to Implementation. Presentation 05 Dec, 2005 in the attachment).

#### Financial Issue

- ✓ Financial analysis is shown in page 18 to 27 of the presentation:
  - Cost for the STP would be funded through government (1 time VGF) and private sector. The proportion of the private sector would be further discussed and decided in the discussion (initial proposal VGF : SPC = 60% : 40%)
  - o PPP structure: BOT
  - o Tariff setting: Cross-subsidy from commercial
  - ✓ Clear and appropriate tariff setting was required and tariff adjustment should be conducted. Cross-subsidy approach with increasing tariff (2 x) for the large commercial business would be applied. The tariff should be lower than the cost for Independent Treatment Plant/ITP of large commercial (see page 18).

#### **DISCUSSION**

#### Dr. Dedy S. Priatna, Bappenas

- He was confused with the figures: He asked the team to clarify how much money should be the portion of the MPW and the DKI government respectively.
- He also requested further clarification on the basis of the tariff formulation
- With regard to the tariff escalation: whether the escalation would be based on annual escalation or certain period escalation.

#### Mr. Yamamoto

- Projection of tariff revenue was based on Floor Area growth of commercial areas.
- Three scenario of tariff escalation was presented by the team namely: (1) W/O GDP escalation; (2) With GDP escalation 6 % annually; (3) 2 X of tariff of large commercial with GDP escalation 6 % annually.

#### Dr. Dedy S. Priatna, Bappenas

- Tariff adjustment based on the escalation of GDP may be reasonable, however the
  impact of tariff adjustment on the subsidy provided by the DKI government should be
  further discussed with the new governor of DKI Jakarta. Escalation of subsidy every year
  would be difficult for the government of the DKI Jakarta.
- He suggested that the team should make a simulation for the investment cost; it should be provided within one time or per year payment and their impact to the DKI Jakarta Government. Again, the provision of investment cost per year would create problem to the DKI government since it has to deal with the Provincial Legislative Body.
- However, which option would be taken, the decision would be dependent on the Jakarta government itself.

#### Mr. Budiyuwono, DG-Ciptakarya, MPW

- For this STP PPP project, VGF could be provided by the central government in the form of loan from overseas.
- With regard to tariff adjustment, he was unsure whether the provincial government of Jakarta could easily increase the tariff.

#### Mrs Lily, PD IPAL DKI-Jakarta

- Principally, the government of DKI Jakarta intended to utilize Full-Separated system in the future for both the commercial buildings as well as residential areas.
- She also concerned about technological solutions to sewerage system on the using of interceptor (combined system) versus full separated system (page 8 – 14). The government of Jakarta should carefully review the options offered by the team prior to making the decision.
- Interceptor would be utilized in the high density populated residential areas.

#### Mr. Nugroho, Director of Settlements and Sanitation – Bappenas:

 Explained that it would be ideal if Full-Separated system would be applied. However, in practice the construction process would be very slow, unless for the new housing complex.

#### Dr. Dedy S. Priatna, Bappenas

- He urged the consultant to show the data, calculation, and simulation to the government of DKI Jakarta and MPW. This activity should be followed by presenting all the figures to the New Governor of DKI Jakarta
- Prior to do so, working group discussion should be conducted to elaborate the following issues and to achieve agreement:
  - ✓ Technological solutions including house connections and interceptors
  - ✓ Financial issue: the team should check the regulation on VGF with staff of the Ministry of Finance
  - ✓ Schedule

#### Mr. Bastary, Panji Indra, Director of PPP Bappenas

- Comment on the schedule: the team should indicate who would be responsible for activities described in the schedule
- The team should also clearly state the amount of VGF needed for the PPP project to the MoF
- He also asked about the tendering process in the construction of STP funded through VGF and Private: Will the tender be conducted through a simultaneous process or separated process?
- How the integration of the tendering process, construction and operation between STP and sewerage piping projects?

- He also reminded that according to the PPP guideline, the governor of DKI Jakarta should request VGF to the Minister of Finance
- Above all, the team should devote its efforts to convince the new governor of DKI
  Jakarta that PPP approach to infrastructure provision is beneficial for the DKI Jakarta
  government

#### Mr. Fajar, Dinas PU, DKI Jakarta

- Issues and materials presented in the meeting (i.e., reasons for adopting PPP approach, return of investment, tariff setting) have not been recognized and well understood by the majority of staff in Dinas PU DKI Jakarta as well as in other government agencies. Therefore, dissemination of such materials should be conducted.
- According to his opinion, subsidy that should be provided by the government of DKI
  every year could become a big burden. He also mentioned that some groups in the
  society might not receive the subsidy and the benefit. Therefore, the formulation of
  subsidies should be conducted carefully and wisely, to avoid social problems in the
  future.

#### Dr. Dedy S. Priatna, Bappenas

He invited other comments and inputs from the participants of the meeting

#### Mr. Arianto Wibowo, Indonesian Infrastructure Guarantee Fund (IIGF)

- He was interested in the project risks since these issues were very important beside the financial and technical issues. He asked about further elaboration of the risk profiles and how should they be mitigated.
- He also concerned with the project risk associated with "connectivity" between the STP and the sewerage piping system. It was critical because there would be two different parties involving in the construction as well as operation processes.
- For the purpose of providing the government guarantee the DKI Jakarta government should clarify who would be the CGA (e.g., Dinas Public Works Jakarta, Bappeda). He seemed to have objection if PD PAL Jaya would be assigned as the GCA.
- He recommended that since in the beginning the JICA team should continuously discuss and work together with IIGF particularly on issues related to compliance of the STP PPP project with the PPP guideline. A schedule on this collaboration (particularly with ibu Cintya) should immediately be made.

#### Assistant Deputy, Environmental Agency, DKI Jakarta Provincial Government

• Agreed with Dr. Dedy's recommendations that the JICA team should provide options and their consequences.

#### Dr. Dedy S. Priatna, Bappenas

- He wanted to discuss the schedule again and clarify the responsibility of the central government, the DKI Jakarta provincial government and the private sector.
- The central government would be responsible for the overseas loan.

- The DKI Jakarta provincial government should calculate its capacity in investment cost of the project based on the provincial tax/ non tax revenues and other possible sources.
- He reminded that the ground-breaking would be at the end of 2013 or the beginning of 2014.

#### Mr. Ogawa, Representative of JICA

- JICA would carry on the loan in the pipe-line of the projects.
- Fact findings would be conducted in next week.

#### Mr. Syukrul Amien, Director - Ministry of Public Works

- He expected that sources of the project funding and sharing would be clearly and fully understood by the respective parties.
- He asked whether the procurement process of DED for the construction of the sewerage system could be accelerated by JICA.

#### Dr. Dedy S. Priatna, Bappenas

- Based on the current "Blue Book", the amount of loan for this project was USD 144
   Million and the rest would be provided in Rupiah currency.
- To get more information about the VGF, the JICA team should contact Mr. Freddy Saragih at the Risk Management Unit/RMU-MOF.

#### Mr. Kobayasi, Representative of the JICA Team

- On behalf of the Team, he appreciated the participation of the stakeholders in the discussion and he believed that it was very productive.
- He also stated to maintain the compliance of the project to the PPP regulations and guideline and also the ODA requirements.
- The team would follow up the result of the discussion, and would start to have a working group meeting tomorrow.