

Project: Port Moresby Sewerage System Upgrading
 Location: Kila Police Rep # 7 - Port Moresby, NCD
 Client: BDA

Job: NGTS01011
 Date: March 2010

PARTICLE SIZE DISTRIBUTION & LIQUEFACTION POTENTIAL

Client: njs - JICA

Project number: NGTS01011

Project: Port Moresby Sewerage System

Location: Kila Police Rep, Port Moresby - NCD

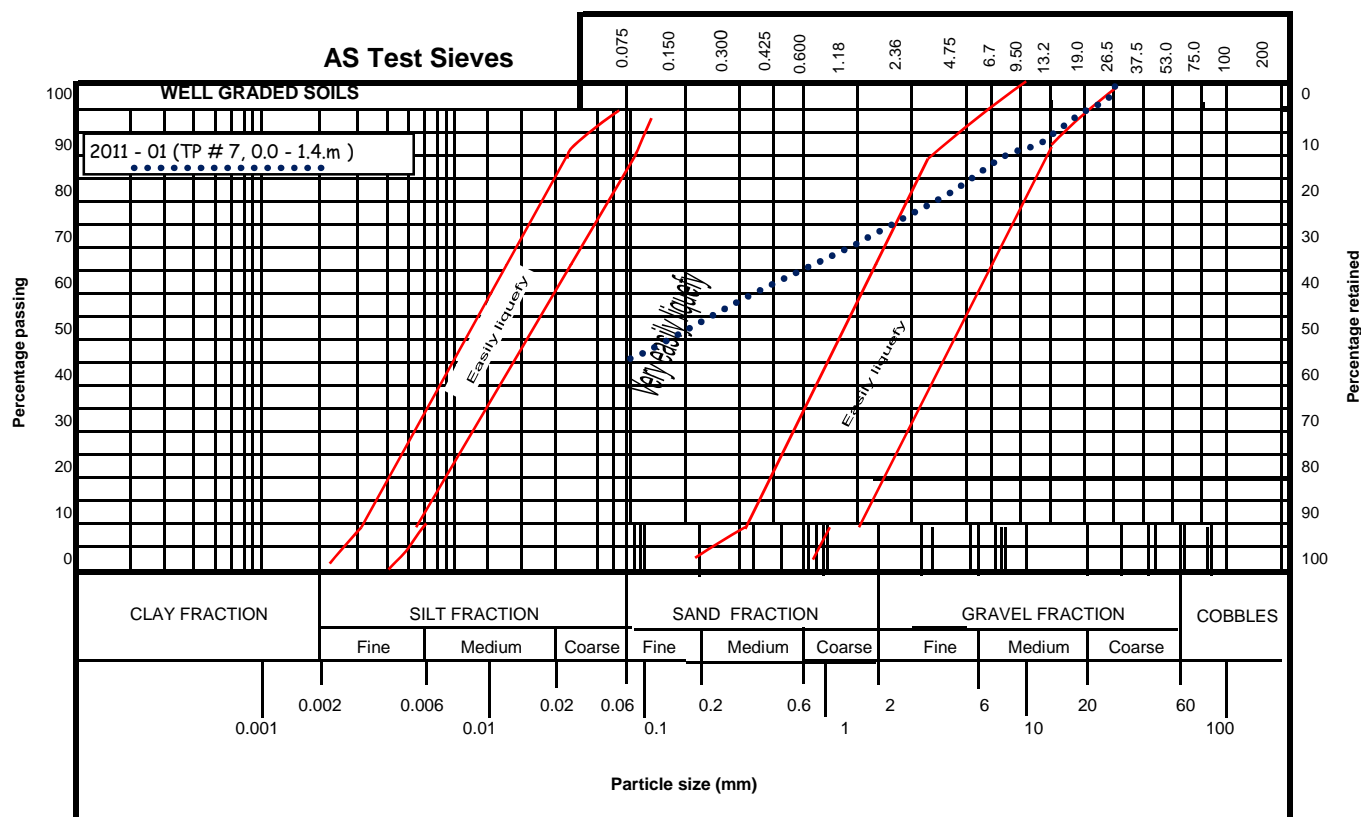


Figure 16 - GRADING CURVE FOR SAMPLES FROM TEST PIT # 7



Project: Port Moresby Sewerage System Upgrading

Location: Konebada New Pump Station # 8 - Port Moresby

Job No: NGTS01011

Client: njs - JICA

Date: April, 2011

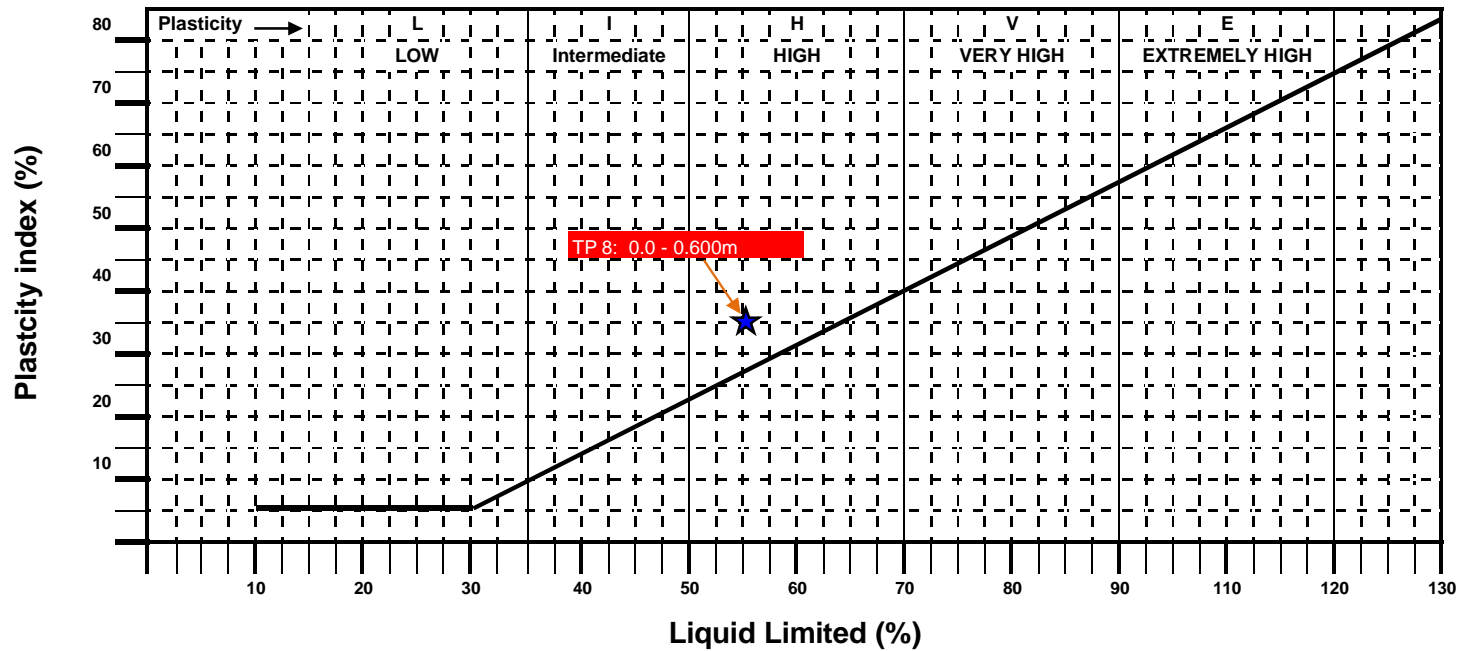


Figure 17. CASAGRANDE PLOT FOR SUBSOIL MATERIAL FROM TEST PIT # 8

Project: Port Moresby Sewerage System Upgrading
 Location: Konebad # 8 - Port Moresby, NCD
 Client: njs - JICA

Job: NGTS001011
 Date: March 2011

PARTICLE SIZE DISTRIBUTION & LIQUEFACTION POTENTIAL

Client: njs - JICA

Project number: NGTS01011

Project: Port Moresby sewerage System upgrading

Location: Konebada # 8, Port Moresby - NCD

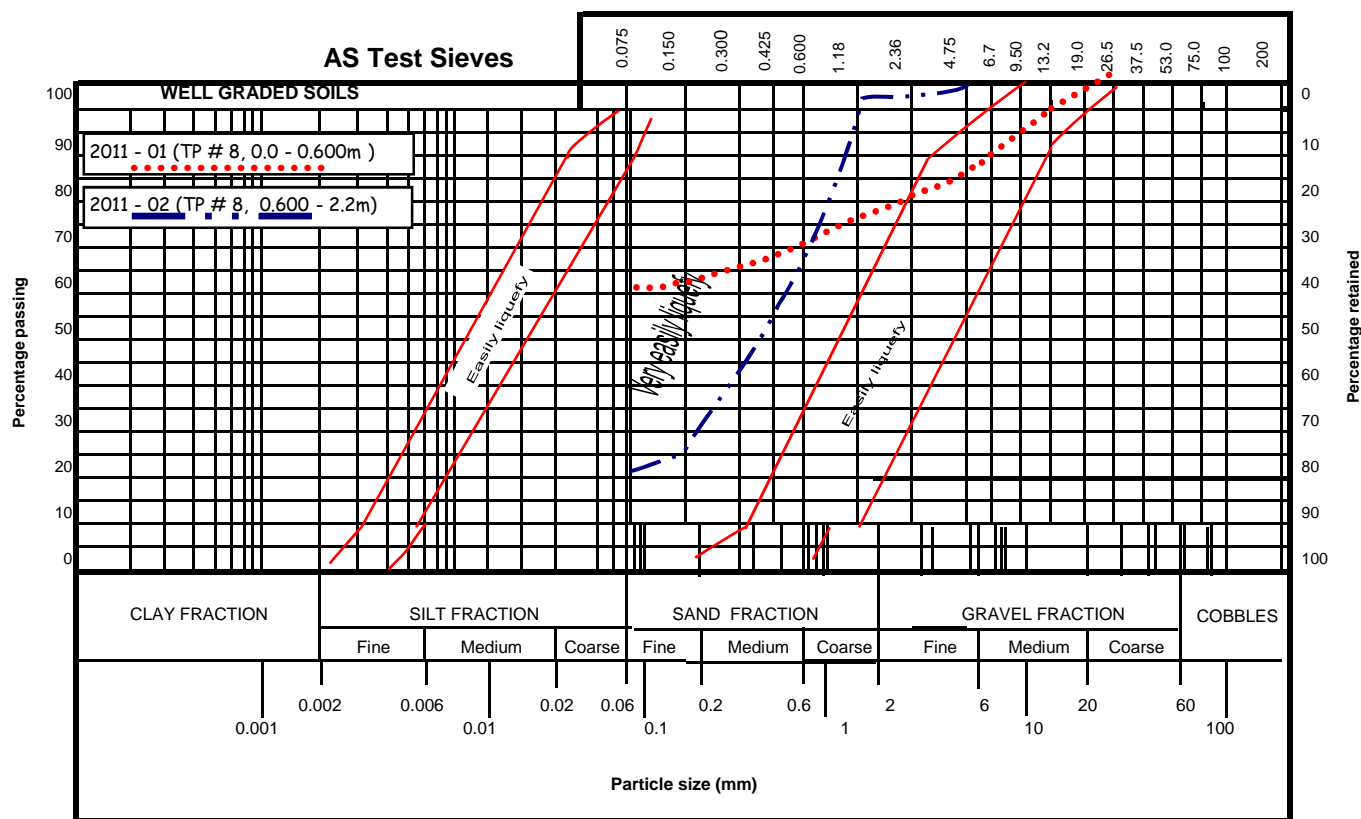


Figure 18 - GRADING CURVE FOR SAMPLES FROM TEST PIT # 8



Project: Port Moresby Sewerage System Upgrading

Location: Gabutu New Pump Station # 9 - Port Moresby

Client: njs - JICA

Job No: NGTS01011

Date: April, 2011

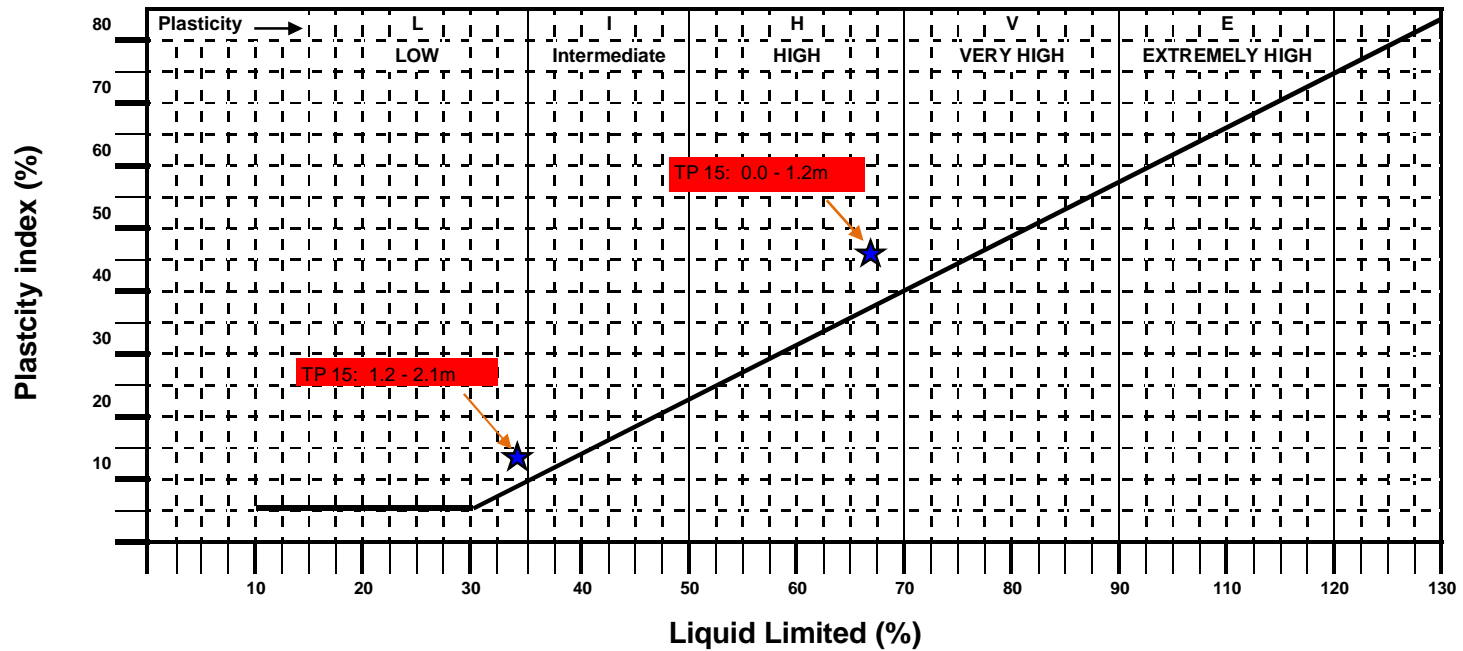
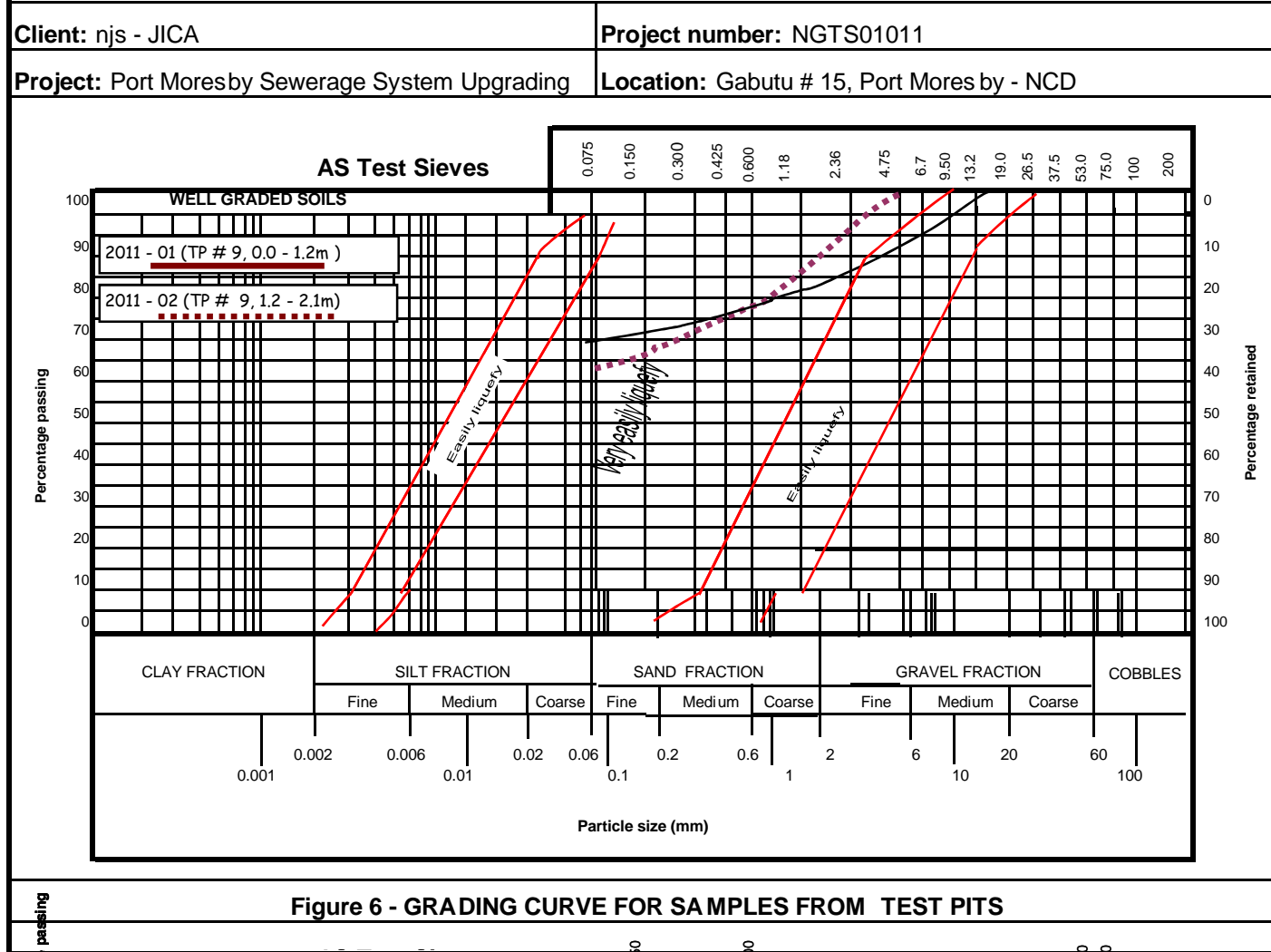


Figure 19. CASAGRANDE PLOT FOR SUBSOIL MATERIAL FROM TEST PIT # 9

PARTICLE SIZE DISTRIBUTION & LIQUEFACTION POTENTIAL





Project: Port Moresby Sewerage System Upgrading

Location: KilaKila New Pump Station # 10 - Port Moresby

Client: njs - JICA

Job No: NGTS01011

Date: April, 2011

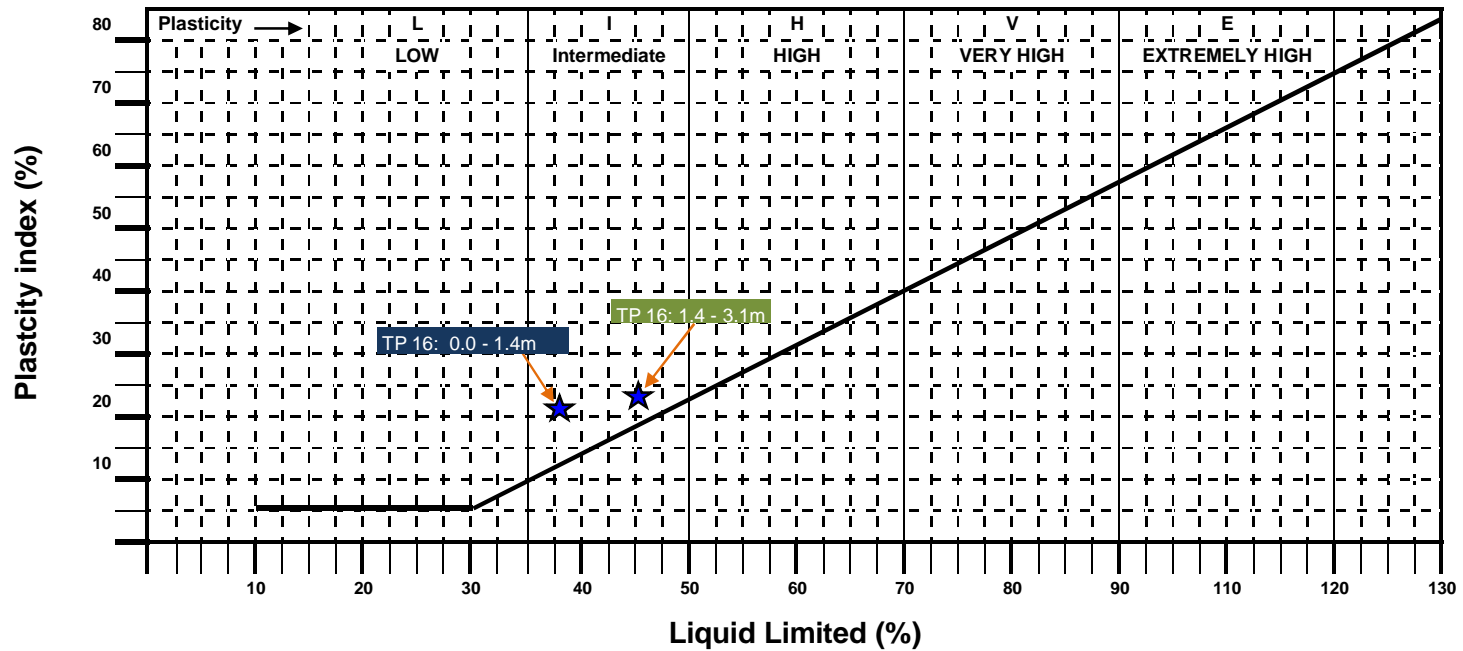


Figure 21. CASAGRANDE PLOT FOR SUBSOIL MATERIAL FROM TEST PIT # 10

Project: Port Moresby Sewerage System Upgrading
 Location: KilaKila # 10 - Port Moresby, NCD
 Client: njs - njs - JICA

Job: NGTS01011
 Date: March 2011

PARTICLE SIZE DISTRIBUTION & LIQUEFACTION POTENTIAL

Client: njs - JICA

Project number: NGTS01011

Project: Port Moresby Sewerage System Upgrading

Location: KilaKila # 10, Port Moresby - NCD

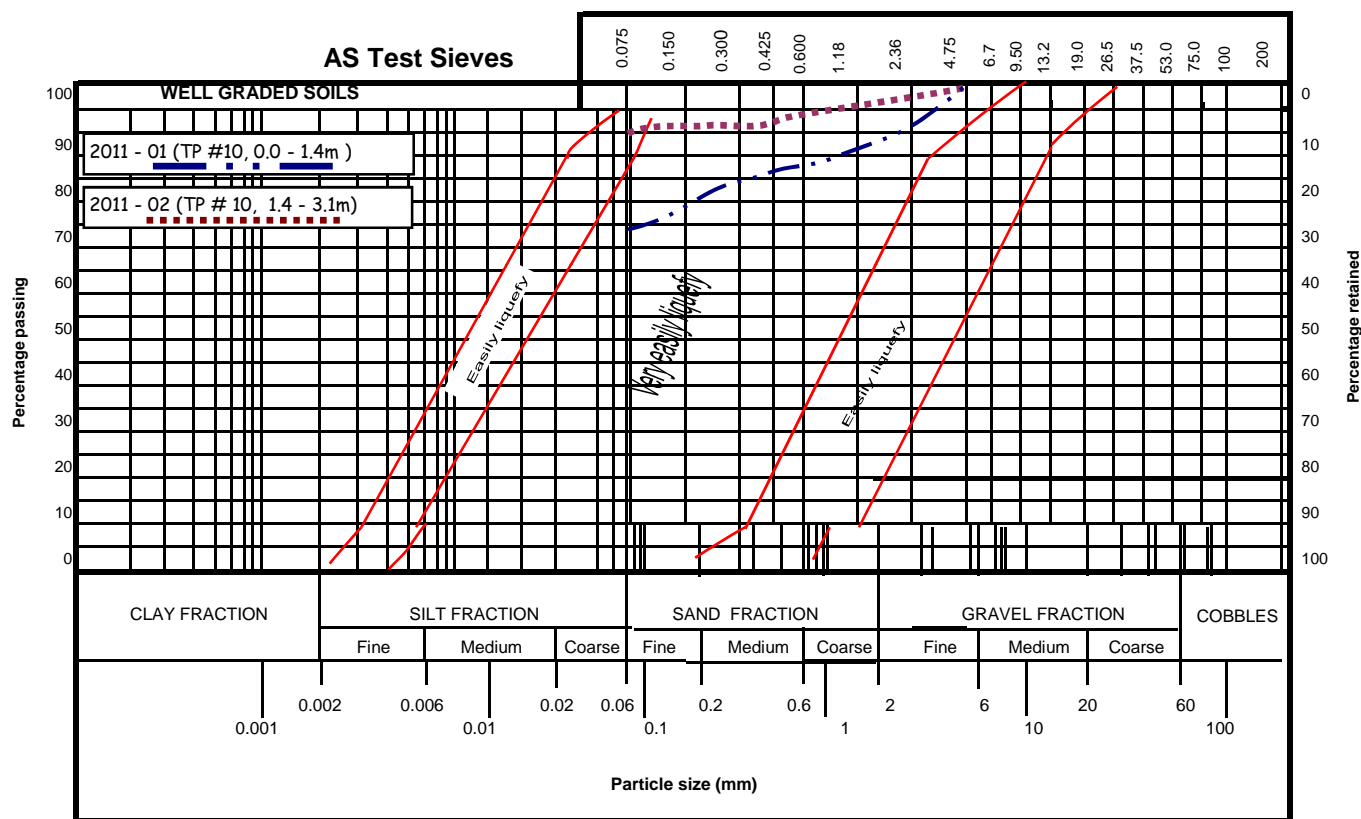


Figure 22 - GRADING CURVE FOR SAMPLES FROM TEST PIT # 10



Project: Port Moresby Sewerage System Upgrading

Location: KilaKila STP - Port Moresby, NCD

Client: njs - JICA

Job No: NGTS01011

Date: March, 2011

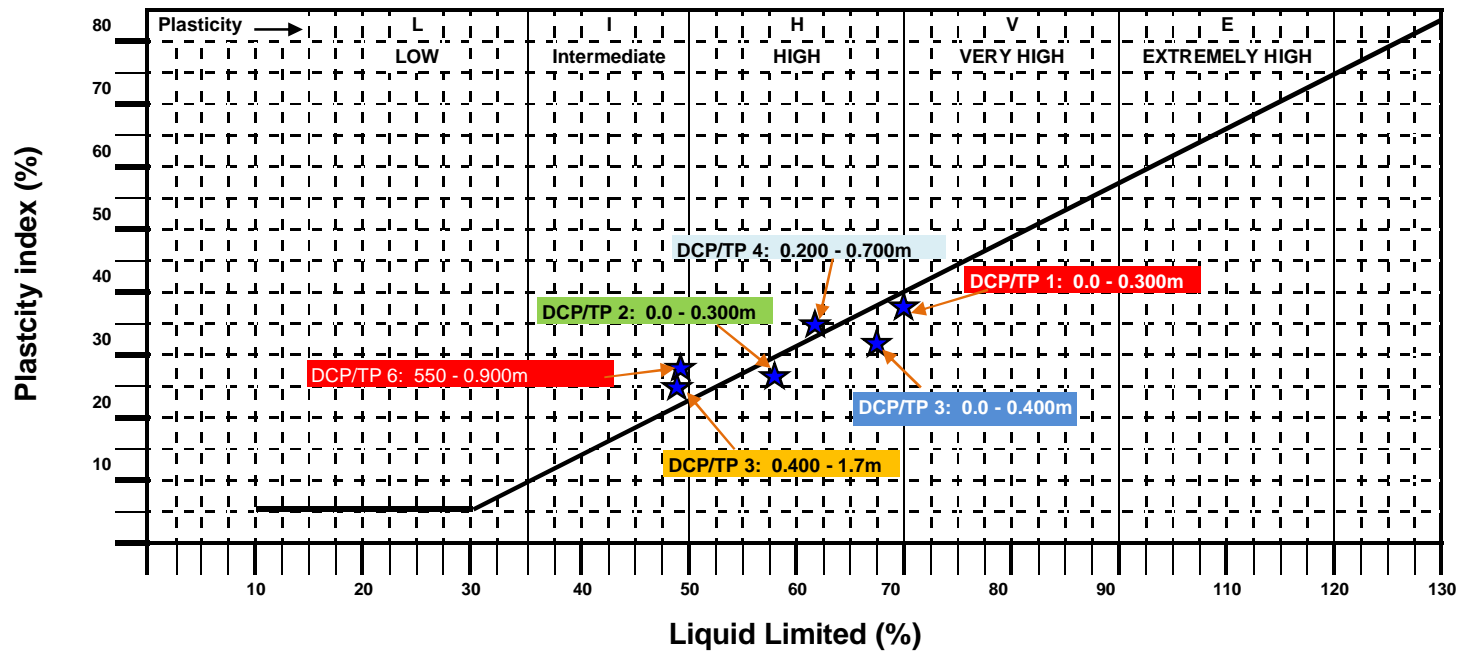


Figure 23. CASAGRANDE PLOT FOR SUBSOIL MATERIAL FROM TEST PIT AT KILAKILA

Project: Port Moresby Sewerage System Upgrading
 Location: KilaKila - Port Moresby, NCD
 Client: njs - JICA

Job: NGTS01011
 Date: March 2011

PARTICLE SIZE DISTRIBUTION & LIQUEFACTION POTENTIAL

Client: njs - JICA

Project number: NGTS01011

Project: Port Moresby Sewerage System Upgrading

Location: KilaKila, Port Moresby - NCD

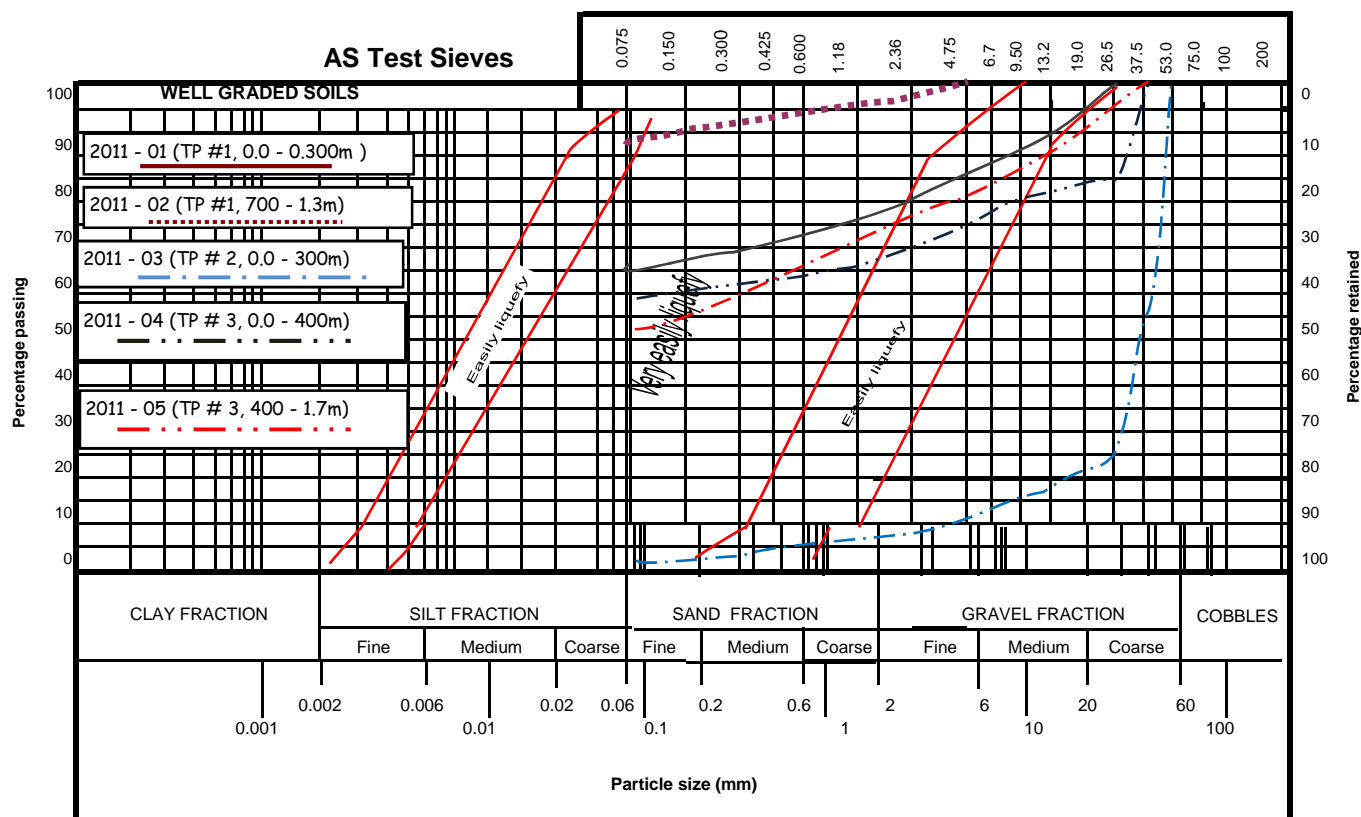


Figure 24 - GRADING CURVE FOR SAMPLES FROM TEST PIT AT KILAKILA

Project: Port Moresby Sewerage System Upgrading

Location: KilaKila - Port Moresby, NCD

Client: njs - JICA

Job: NGTS01011

Date: March 2011

PARTICLE SIZE DISTRIBUTION & LIQUEFACTION POTENTIAL

Client: njs - JICA

Project number: NGTS01011

Project: Port Moresby Sewerage System Upgrading

Location: KilaKila, Port Moresby - NCD

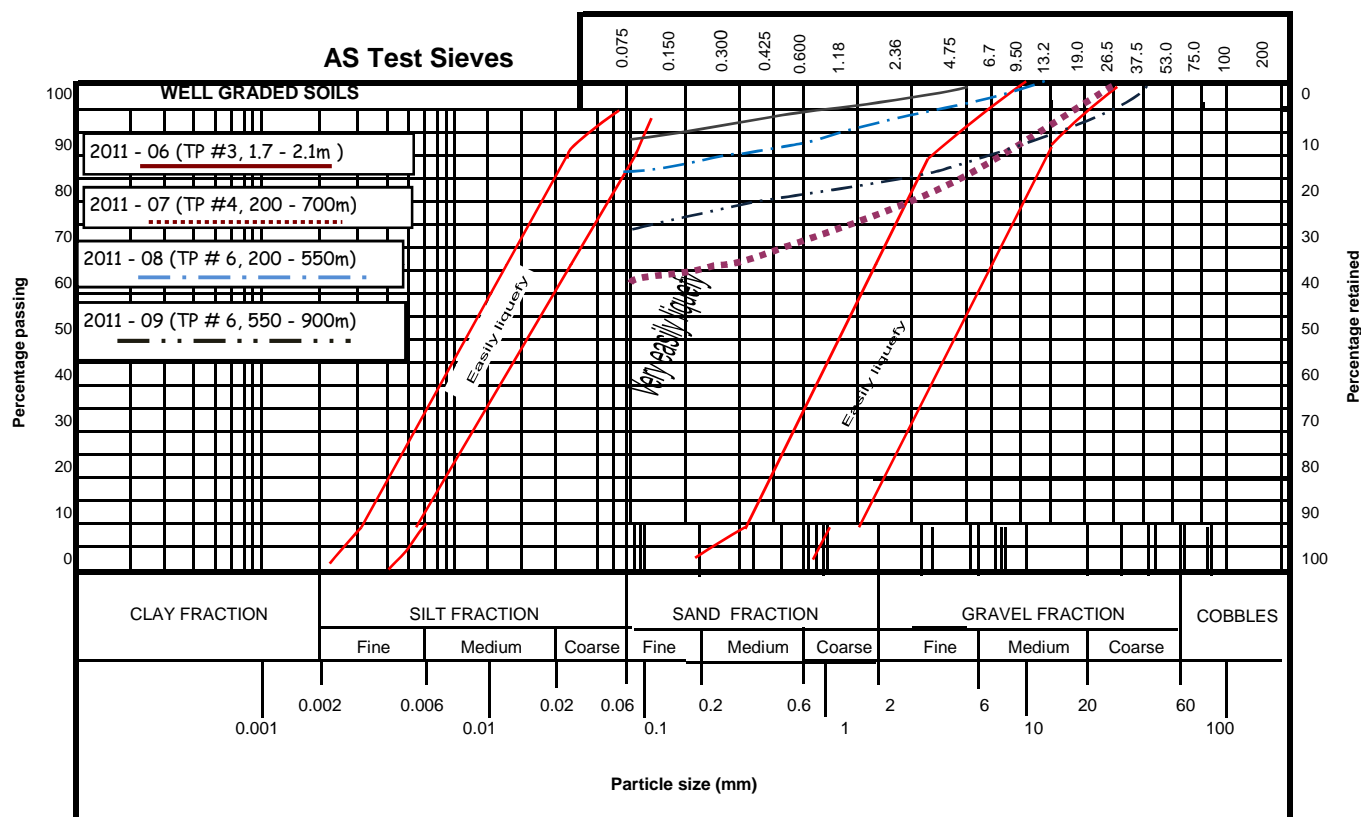


Figure 24a- GRADING CURVE FOR SAMPLES FROM TEST PIT AT KILAKILA



Project: Port Moresby Sewerage System Upgrading

Location: Morata - Port Moresby, NCD

Client: njs - JICA

Job No: NGTS01011

Date: March, 2011

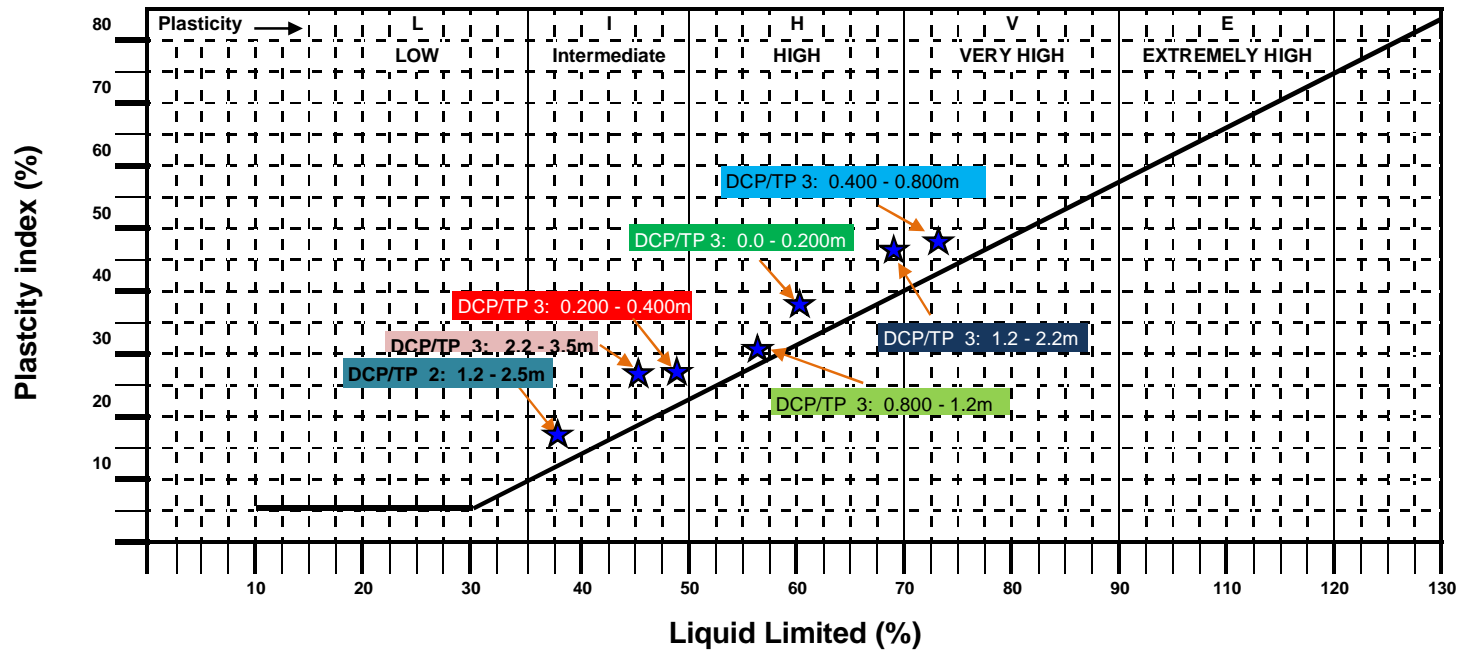


Figure 25. CASAGRANDE PLOT FOR SUBSOIL MATERIAL FROM TEST PIT AT MORATA

Project: Port Moresby Sewerage System Upgrading

Location: Morata - Port Moresby, NCD

Client: njs - JICA

Job: NGTS01011

Date: March 2011

PARTICLE SIZE DISTRIBUTION & LIQUEFACTION POTENTIAL

Client: njs - JICA

Project number: NGTS01011

Project: Port Moresby Sewerage System Upgrading

Location: Morata, Port Moresby - NCD

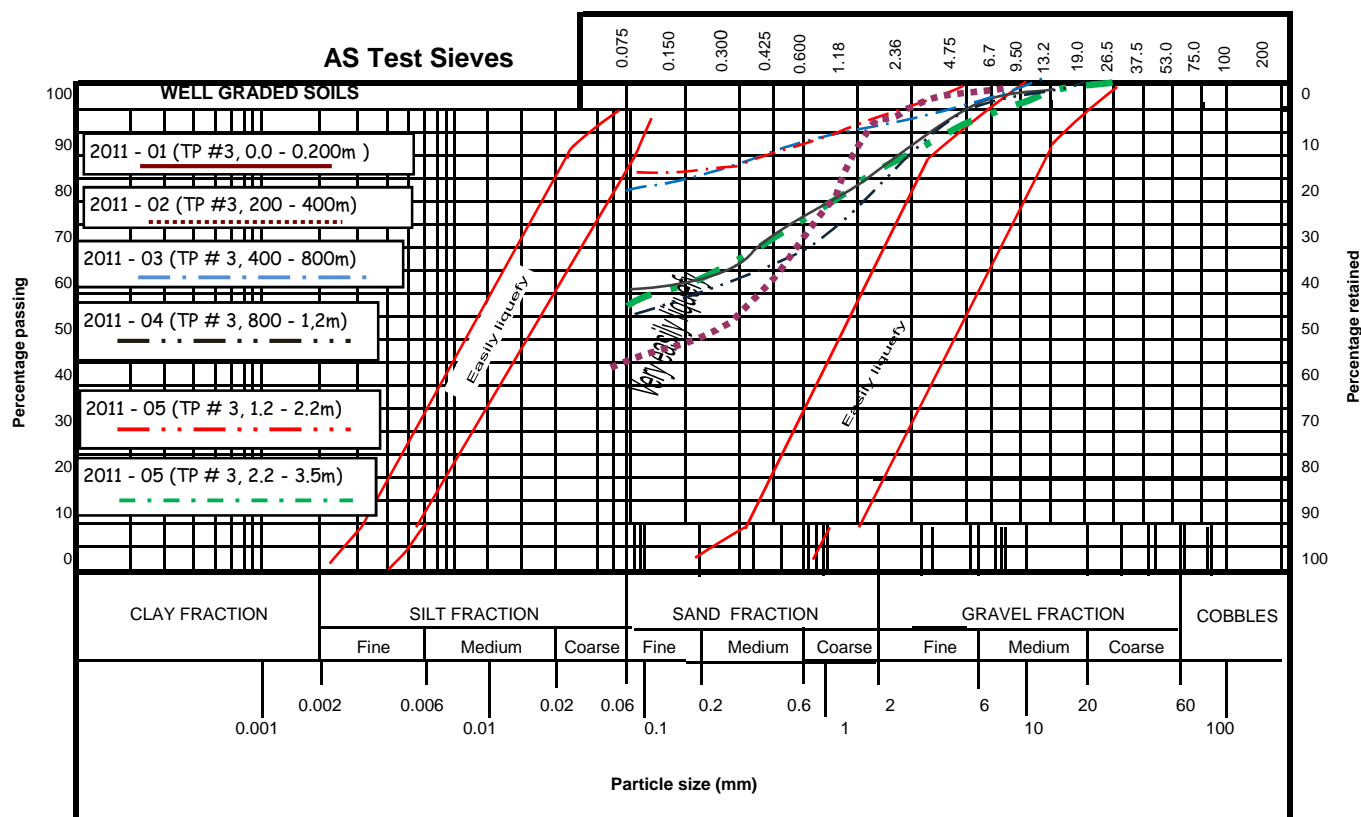


Figure 26 - GRADING CURVE FOR SAMPLES FROM TEST PIT AT MORATA

GENERAL

Soil and rock descriptions are generally in accordance with the recommendations of Australian Standard 1726 - 1981 and cover the following properties:

SOIL Colour Plasticity Grain Size Minor Components Moisture Consistency Origin & Structure Other Relevant Information	ROCK Colour Grain Size Structure Minor Components Weathering Strength Discontinuities
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Field tests have been used extensively to assess soil consistency, rock strength and grain size. Unless specifically stated otherwise these assessments have been transferred directly to the record sheets and not modified to coincide with laboratory test results. Field descriptions may therefore be used as an independent estimate of material properties which can be correlated with other data.

Descriptive terms used on the record sheets are explained on the succeeding pages.

TERMS COMMON TO SOIL AND ROCK:

STRUCTURE

BEDDING	SPACING (mm)	JOINTING	BLOCKINESS	BLOCK SHAPE
Very thickly bedded	>2000	Very widely jointed		
Thickly bedded	600 - 2000	Widely jointed		
Medium bedded	200 - 600	Medium jointed		
Thinly bedded	60 - 200	Closely jointed	Blocky Cloddy Nutty	Bulky (equally developed along three axes)
Very thinly bedded	20 - 60	Very closely jointed		
Laminated	6 - 20	Extremely closely jointed		
Thinly Laminated	<6	Intensely jointed		
	Dimension stated Generally <10		Prismatic Shattered	Elongated Lenticular platey or wedge-shaped

COLOUR

Individual assessment of colour has been used and no reference made to standard colour charts unless specifically stated.

PROPORTIONS OF MINOR COMPONENTS

a trace	up to 10 percent
a little	10 to 25 percent
some	25 to 40 percent
and	about 50 percent

EXPLANATORY NOTES

TYPICAL REPRESENTATION AND TERMS USED FOR SOILS

COARSE GRAINED SOILS (<50% passing .06mm sieve)		GROUP SYMBOL	GENERAL DESCRIPTION	FINE GRAINED SOILS (>50% passing .06mm sieve)		GROUP SYMBOL	GENERAL DESCRIPTION
			Fill			ML	Inorganic silts, very fine sands, rock flour, silty or clayey fine sand.
	Gravels	GW	Well graded gravel and gravel - sand mixtures - little or no fines.			CL	Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clays.
		GP	Poorly graded gravels and gravel - sand mixtures - little or no fines.			OL	Organic silts and organic clays of low plasticity.
	Gravelly Silts	GM	Silty gravels, gravel - sand - silt mixtures.			MH	Inorganic silts, micaceous or diatomaceous fine sands or silts.
		GC	Clayey gravels, gravel - sand - clay mixtures.			CH	Inorganic clays of high plasticity, fat clays.
	Sands	SW	Well graded sands and gravelly sands - little or no fines.			OH	Organic clays of medium to high plasticity.
		SP	Poorly graded sands and gravelly sands - little or no fines.			Pt	Peat, muck and other highly organic soil.
	Gravelly Sands	SM	Silty sands, sand - silt mixtures.				
	Sandy Silts	SC	Clayey sands, sand - clay mixtures.				

CONSISTENCY NON - COHESIVE SOILS

FIELD TEST	Easily excavated with a spade	Some resistance to a spade or penetration with a hand bar	Considerable resistance to spade or penetration with a hand bar	No penetration with a hand bar, requires pick for excavation	High resistance to a pick	
SPT 'N' VALUE (blows/300mm)	0	4	10	30	50	
DESIGNATION	Very loose (VL)	Loose (L)	Medium Dense (MD)	Dense (D)	Very Dense (VD)	
RELATIVE DENSITY %	0	15	35	65	85	100

CONSISTENCY COHESIVE SOILS

FIELD TEST	Penetrated by or exudes between fingers when squeezed	Easily penetrated by thumb or moulded with fingers	Penetrated by thumb with effort, moulded only by strong pressure of fingers	Indented by thumb cannot be moulded by fingers	Penetrated by thumb nail and to about 15mm with knife	
DESIGNATION	Very Soft (VS)	Soft (S)	Firm (F)	Stiff (ST)	Very Stiff (VST)	Hard (H)
UNDRAINED SHEAR STRENGTH (Cu) kPa	12	25	50	100	200	



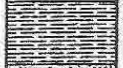




GRAIN SIZE

FIELD TEST	i Not visible with x10 lens ii Does not dilate on shaking iii Adheres to fingers when dry	i Particles >10µm visible with x10 lens ii Dilates on shaking iii Does not adhere to fingers when dry iv Feels gritty on teeth	i Particles >80µm visible to naked eye ii Fine sand feels gritty in fingers	Visual identification							
DESIGNATION	CLAY	SILT	SAND Fine (f) Medium (m) Coarse (c)			GRAVEL Fine (f) Medium (m) Coarse (c)			COBBLES	BOULDERS	
GRAIN SIZE	2			60	200	600	2	6	20	60	200
	Microns						Millimetres				



EXPLANATORY NOTES

TYPICAL REPRESENTATION AND TERMS USED FOR ROCK

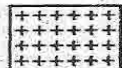



SEDIMENTARY

	Mudstone
	Shale
	Siltstone
	Sandstone
	Conglomerate
	Limestone
	Coal

METAMORPHIC

	Low grade: slate, phyllite, schist etc.
	High grade: quartzite, gneiss, marble etc.

IGNEOUS

	Plutonic (generally coarse grained): granite, gabbro etc.
	Hypabyssal (generally medium grained): micro – granite, dolerite etc.
	Volcanic (generally fine grained): rhyolite, andesite, basalt etc.
	Pyroclastic: pumice, tuff etc.

WEATHERING BASED ON VISUAL IDENTIFICATION

FIELD APPEARANCE	Shows chemical and physical alteration to fabric caused by temperature, pressure or injection of other material	Has soil properties and often shows complete change in appearance. Rock fabric not visible	Shows considerable change in appearance and loss in strength. Material is still rock but normally very low strength	Visible change in appearance and significant loss in strength	Visible change in appearance but no significant loss in strength	No weathering effects visible to naked eye
DESIGNATION	Altered (A)	Completely weathered (CW)	Highly weathered (HW)	Moderately weathered (MW)	Slightly weathered (SW)	Fresh (F)

STRENGTH BASED ON POINT LOAD STRENGTH INDEX, CORRECTED TO 50mm DIAMETER

FIELD TEST	Indented by thumb nail with difficulty	Scratched by thumb nail	Easily broken by hand or pared with knife	Broken by hand or scraped with knife	Broken in hand with firm hammer blows. Cannot be scraped with a knife	Broken against solid object with single hammer blow	Difficult to break against solid object with several hammer blows
POINT LOAD STRENGTH INDEX $I_s(50)$ MPa	0.03		0.1	0.3	1	3	10
DESIGNATION	Extremely low (EL)	Very low (VL)	Low (L)	Medium (M)	High (H)	Very high (VH)	Extremely high (EH)
UNCONFINED COMPRESSIVE STRENGTH (q_u) MPa	The unconfined compressive strength is typically about 20 x $I_s(50)$. The ratio may vary widely for different rock types.						

STRUCTURE & DISCONTINUITY SPACING BASED ON VISUAL IDENTIFICATION

STRUCTURE DESIGNATION	Thinly laminated	Laminated	Very thinly bedded	Thinly bedded	Medium bedded	Thickly bedded	Very thickly bedded
SPACING mm	6		20	60	200	600	2000
DISCONTINUITY DESIGNATION	Intense	Extremely close	Very close	Close	Medium	Wide	Very wide

EXPLANATORY NOTES

SYMBOLS & ABBREVIATIONS

DRILLING

METHOD

AD	Auger drilling [drilled to depths shown (m)]
V bit	Steel 'V' bit
TC	Tungsten carbide bit
RR	Tricone (rock roller) bit
W	Washboring
NMLC,BMLC	Triple tube rotary core drilling (52 mm, 35 mm dia.)
NQ,HQ	Wireline core drilling
D	Diatube coring

SUPPORT

W	Water
M	Mud
C	Casing
T	Timbering
U	Unsupported

SAMPLE & FIELD TESTING

D	Disturbed sample
U(x)	Undisturbed sample x mm diameter
U(x) +	U(x) attempted, little or no recovery
PT	Pressuremeter test
PL	Point load test (● axial, ○ diametral test)
Is(50)	Point load strength index (MPa)
qc	Cone resistance (from CPT)
qp,PP	Unconfined compressive strength estimated from pocket penetrometer (kPa)
RQD	Rock quality designation expressed as: <u>sum of lengths of sound core pieces > 100 mm</u> / total length of core section considered
D/DD	Dip/dip direction of rock discontinuity (degrees)
CPT	Cone penetration test
SPT	Standard penetration test

N	SPT blow count (blows/300 mm)
R	SPT refusal
V	In situ vane test [showing peak/residual value (kPa)]
W	Water sample
B	Bulk sample

WATER - MOISTURE

W	Wet
M	Moist
D	Dry
S	Standpipe installed to depth shown
P	Piezometer installed at depth shown
→	Inflow
←	Outflow (loss)
↕	Level (date)
↔	Partial loss

SOIL PROPERTIES

CBR	California Bearing Ratio
Cc	Compression index
Cu	Undrained shear strength
Cv	Coefficient of consolidation
C _α	Coefficient of secondary compression
c'	Effective shear strength
DD	Dry density
Dr	Dry density expressed as a percentage of MDD
E	Elastic modulus
e	Void ratio
G	Shear modulus
Gs	Specific gravity
k	Permeability
MDD	Maximum dry density obtained in compaction test
mv	Coefficient of volume compressibility

NDD	Natural dry density
NMC	Natural moisture content
OMC	Optimum moisture content obtained in compaction test
LI	Liquidity index
LL	Liquid limit
LS	Linear shrinkage
PI	Plasticity index
PL	Plastic limit
qu,UCS	Unconfined compressive strength
w	Moisture content
γ _b	Bulk density
γ _d	Dry density
γ _w	Density of water
ν	Poisson's ratio
φ _u	Apparent angle of friction from quick undrained triaxial test
φ _u , φ _{u'}	Effective angles of friction in drained and undrained conditions

DESIGN PARAMETERS


Ab	Footing or pile base area
B-	Footing or pile width or diameter
D	Footing or pile depth
d	Diameter of pile (m)
K	Coefficient of earth pressure
Ka	Coefficient of active pressure
Ko	Coefficient of earth pressure at rest
Kp	Coefficient of passive pressure
L	Footing length
Nc,Nq,Nγ	Bearing capacity factors
NSF	Negative skin friction
P	Load
Pa	Total active force
Pb	Pile base load
Pp	Total passive force
Ps	Pile shaft load

pa	Active earth pressure
pb	Pile base pressure
pp	Passive earth pressure
ps	Pile shaft adhesion
S	Settlement
Tv	Dimensionless time factor
t	Time
U	Degree of consolidation
u	Pore water pressure
α	Shaft adhesion factor
δ	Angle of friction between soil and structure
σ	Total normal stress
σ'	Effective normal stress
τ	Shear stress

Subscript all allowable or working
 Subscript h horizontal
 Subscript r residual

Subscript ult ultimate
 Subscript v vertical

APPENDIX A


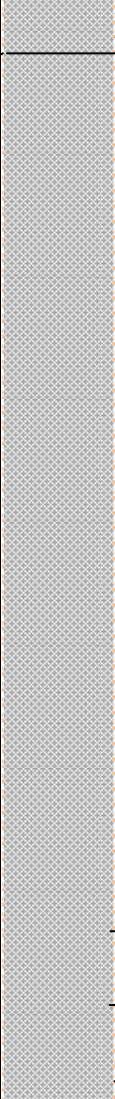
 NIAWANG GEOTECH LIMITED Suite # 6, NCD LikLik Bisnis Centre. Portion 2401. Stores Road. 4 Mile PO Box 6298, BOROKO. NCD Fax/Tel: (675) 323 1836 Mobile: 76986593 / 73277732 or 72101529 Email: niawanggeotech@daltron.com.pg	Job No: NGTS/0111
	Borehole and Test Pit No: 1
	Sheet No: 1 of 1

Equipment : Backhoe, Drill Rig, DCP & Hand Auger	Location: KANUDI PS : 1
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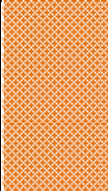
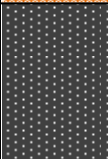



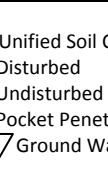

CLIENT : njs - JICA (eda ranu)

PROJECT : Port Moresby Sewerage System Upgrading




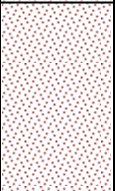

Ground Level: 1.057	GPS Coordinates 109221.660 N 103717.567 E	Program Duration	
		Commence : 01/04/11	Completed: 01/04/11

Description	USC	Graphic Log	Depth (m)	Samples / Tests				Field Records	
				Strata Thickness (m)	Type	Samples Depth (m)	SPT N' VALUE		ABC (kPa)
Topsoil mixture of root fibres and peat			0.1 0.2 0.3	0.3					
Sandy Silty GRAVEL soft to Medium dense, fairly graded coarse grained, low plasticity, grey.			0.4 0.5 0.6 0.7 0.8 0.9 1.0 1.1 1.2 1.3 1.4 1.5 1.6 1.7 1.8 1.9 2.0 2.1 2.2 2.3 2.4 2.5 2.6 2.7 2.8 2.9 3.0 3.1 3.2 3.3 3.4 3.5 3.6 3.7 3.8 3.9 4.0 4.1 4.2 4.3 4.4 4.5	▽GWT 0.5	DCP	0.5	2/4/3 N = 7	172	0.0 - 0.5m depth Loose/Firm
----As above but non plasticity.....					DCP	1.0	4/4/4 N = 8	190	0.5 - 1.0m depth Loose/Firm
					DCP	1.5	4/3/5 N = 8	190	1.0 - 1.5m depth Loose/Firm
					DCP	2.0	4/3/4 N = 7	172	1.5 - 2.0m depth Loose/Firm
					DCP	2.5	4/5/4 N = 9	207	2.0 - 2.5m depth Loose to medium dense
					DCP	3.0	5/5/5 N = 10	223	2.5 - 3.0m depth Loose to medium dense
					DCP	3.5	7/9/8 N = 16	316	3.0 - 3.5m depth Medium dense
					DCP	4.0	9/12/12 N = 24	425	3.5 - 4.0m depth Medium dense
					DCP	4.5	15/17/20 N = 37	574	4.0 - 4.5m depth medium dense to dense
Terminated at 4.5m depth			4.6 4.7						



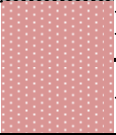
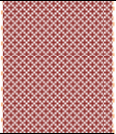
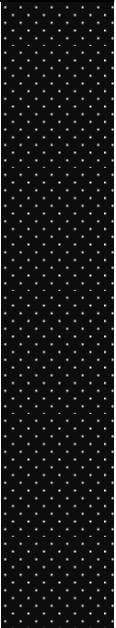
KEY: DCP Dynamic Cone Penetrometer SPT Standard Penetration Test VST Vane Shear Test CR Core Recovery (%) RQD Rock Quality Designation (%) ABC Allowable Bearing Capacity (kPa)	EGL Existing Ground Level USC Unified Soil Classification D Disturbed UD Undisturbed PP Pocket Penetrometer GWT Ground Water Table	Remarks: Further investigation will continue with drilling after the land issue with land owners is settled by EDA RANU.	Logged by: WKG Checked by: ME Drafted by : Scale:
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 NIAWANG GEOTECH LIMITED Suite # 6, NCD LikLik Bisnis Centre. Portion 2401. Stores Road. 4 Mile PO Box 6298, BOROKO. NCD Fax/Tel: (675) 323 1836 Mobile: 76986593 / 73277732 or 72101529 Email: niawanggeotech@daltron.com.pg				Job No: NGTS/0111					
				Borehole No: DCP/TP 2					
				Sheet No: 1 of 1					
Equipment : Backhoe, Drill Rig, DCP & Hand Auger		Location: IDUBADA PS : 2							
CLIENT : njs - JICA (eda ranu)									
PROJECT : Port Moresby Sewerage System Upgrading									
		Ground Level: 1.325		Coordinates GPS 108465.480 N 104182.277 E					
				Program Duration Commence : 01/04/11 Completed 01/04/11					
Description	USC	Graphic Log	Depth (m)	Samples / Tests				Field Records	
				Strata Thickness (m)	Type	Samples Depth (m)	SPT N' VALUE		ABC (kPa)
FILL: Red ridge gravelly CLAY, medium plasticity, red ridge Sandy Gravelly CLAY (FILL) 00 – 800mm thick			0.1						
			0.2						
			0.3						
			0.4						
			0.5		DCP	0.5	2/4/2 N = 6	153	0.0 – 0.5m depth Soft - firm
			0.6						
			0.7						
			0.8						
Gravelly Sandy CLAY black – grayish, high plasticity, (800 – 2.0m thickness)			0.9						
			1.0		DCP	1.0	1/2/2 N = 4	114	0.5 – 1.0m depth Soft
			1.1						
			1.2						
			1.3						
			1.4						
			1.5		DCP	1.5	3/4/3 N = 7	172	1.0 - 1.5m depth Soft - firm
			1.6						
			1.7						
			1.8						
 water table at 2.0m			1.9						
			2.0		DCP	2.0	2/1/2 N = 3	92	1.5 – 2.0m depth Firm
			2.1						
			2.2						
			2.3						
			2.4						
			2.5		DCP	2.5	2/2/2 N = 6	92	2.0 – 2.5m depth Soft
			2.6						
			2.7						
			2.8						
.....As above but firm to stiff.....			2.9						
			3.0		DCP	3.0	4/6/9 N = 15	301	2.5 – 3.0m depth Firm – stiff
			3.1						
			3.2						
			3.3						
			3.4						
			3.5		DCP	3.5	11/13/11 N = 24	425	3.0 – 3.m depth Stiff or hard
			3.6						
			3.7						
			3.8						
Terminated at 3.5m depth on bed rock			3.9						
			4.0		DCP	4.0	14/16/13 N = 29	496	3.5 – 4.0m depth Stiff or hard
			4.1						
			4.2						
			4.3						
			4.4						
			4.5		DCP	4.5	16/22 N = 38	584	4.0 – 4.5m depth Firm to Stiff or Hard weathered bed rock, very hard to excavate
			4.6						
	4.7								
KEY: DCP Dynamic Cone Penetrometer SPT Standard Penetration Test VST Vane Shear Test CR Core Recovery (%) RQD Rock Quality Designation (%) ABC Allowable Bearing Capacity		USC Unified Soil Classification D Disturbed UD Undisturbed PP Pocket Penetrometer GWT  Ground Water Table		Remarks: Ground Water Table was encountered at 2.0m and it was high tide during the time of the investigation.		Logged by: WKG Checked by: PE Drafted by : Scale:			










APPENDIX A

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						Borehole and Test Pit No: 3			
						Sheet No: 1 of 1			
Equipment : Backhoe, DCP & Hand Auger			Location: ELEVALA PS : 3						
CLIENT : njs - JICA (eda ranu)									
PROJECT : Port Moresby Sewerage System Upgrading									
		Ground Level: 6.007		GPS Coordinates 104926.283 E 107928.257 N		Program Duration Commence: 01/04/11 Completed: 01/04/11			
Description	USC	Graphic Log	Depth (m)	Samples / Tests				Field Records	
				Strata Thickness (m)	Type	Samples Depth (m)	SPT N' VALUE		ABC (kPa)
Topsoil: Black silty CLAY \geq PI (0.0 - 400mm thickness)			0.1 0.2 0.3 0.4	0.400				0.0 - 0.5m depth Soft	
Light brown, gravelly CLAY \geq PI (400 - 800mm thickness)			0.5 0.6 0.7 0.8	0.800	DCP	0.5	3/8/4 N = 12	255	0.5 - 1.0m depth Soft - Firm
Medium plasticity, dark brown , gravelly CLAY $mc \leq$ PI (800 – 1.5m thick)			0.9 1.0 1.1 1.2 1.3 1.4	1.5	DCP	1.0	7/14/7 N = 21	385	1.0 - 1.5m depth soft
high plasticity, soft to firm, moist, grey $mc \leq$ PI (1.5 – 2.5m thick)			1.5 1.6 1.7 1.8 1.9 2.0 2.1 2.2 2.3 2.4		DCP	1.5	6/6/6 N = 12	255	1.5 – 2.0m depth soft
			2.0 2.1 2.2 2.3 2.4		DCP	2.0	6/6/6 N = 12	255	2.0 – 2.5m depth firm
Terminated at 2.5m depth on bed rock			2.5 2.6 2.7 2.8 2.9 3.0 3.1 3.2 3.3 3.4 3.5 3.6 3.7 3.8 3.9 4.0 4.1 4.2 4.3 4.4 4.5 4.6 4.7						
KEY: DCP Dynamic Cone Penetrometer SPT Standard Penetration Test VST Vane Shear Test CR Core Recovery (%) RQD Rock Quality Designation (%) ABC Allowable Bearing Capacity (kPa)		EGL Existing Ground Level USC Unified Soil Classification D Disturbed UD Undisturbed PP Pocket Penetrometer GWT Ground Water Table		Remarks: No Ground Water Table Encountered, dry on completion				Logged by: WG/PE Checked by: ME Drafted by : Scale:	


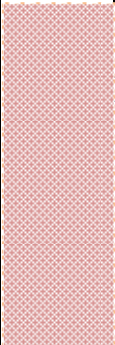
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				Borehole and Test Pit No: 4					
				Sheet No: 1 of 1					
Equipment : Backhoe, DCP & Hand Auger		Location: HANUABADA PS : 4							
CLIENT : njs - JICA (eda ranu)									
PROJECT : Port Moresby Sewerage System Upgrading									
		Ground Level: 3.269		Coordinates GPS 106943.456 N 105447.235 E					
				Program Duration Commence : 01/04/11 Completed: 01/04/11					
Description	USC	Graphic Log	Depth (m)	Samples / Tests			ABC (kPa)	Field Records	
				Strata Thickness (m)	Type	Samples Depth (m)			SPT N' VALUE
Topsoil: Fill material			0.1 0.2 0.3	0.300					
Brown silty CLAY, medium plasticity, mc ≤ PI (300 – 800mm thickness)			0.4 0.5 0.6 0.7	0.800	DCP	0.5	5/5/6 N = 11	240	0.0 – 0.5m depth Soft
Silty gravel of reddish/brown mixture with Dark Brown, silty clay (800 – 1.3m thickness)			0.8 0.9 1.0 1.1 1.2	1.3	DCP	1.0	6/7/7 N = 14	286	0.5 – 1.0m depth Soft
Black, CLAY with some fine to medium size grain, medium – high plasticity, mc ≤ PI (1.3 – 3.7m depth)			1.3 1.4 1.5 1.6 1.7 1.8 1.9 2.0 2.1 2.2 2.3 2.4 2.5 2.6 2.7 2.8 2.9 3.0 3.1 3.2 3.3 3.4 3.5 3.6		DCP	1.5	10/7/9 N = 16	316	1.0 – 1.5m depth Soft - Firm
					DCP	2.0	9/15/5 N = 20	372	1.5 – 2.0m depth Soft - Firm
					DCP	2.5	5/7/7 N = 14	286	2.0 – 2.5m depth Firm - stiff
					DCP	3.0	7/6/4 N = 10	223	2.5 – 3.0m depth Firm
					DCP	3.5	3/3/3 N = 6	190	3.0 – 3.7m depth Firm – stiff or hard
Terminated at 3.7m depth on bed rock			3.7 3.8 3.9 4.0 4.1 4.2 4.3 4.4 4.5 4.6 4.7		DCP	3.7	12/24 N = 36	564	
KEY: DCP Dynamic Cone Penetrometer SPT Standard Penetration Test VST Vane Shear Test CR Core Recovery (%) RQD Rock Quality Designation (%) ABC Allowable Bearing Capacity (kPa)		EGL Existing Ground Level USC Unified Soil Classification D Disturbed UD Undisturbed PP Pocket Penetrometer GWT Ground Water Table		Remarks: Water was encountered from the runoff from the hills and Eda Ranu sump.			Logged by: WG/PE Checked by: ME Drafted by : Scale:		


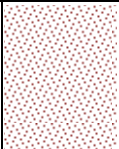

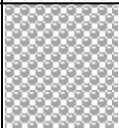
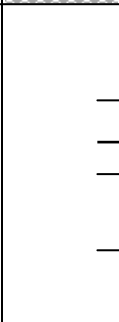
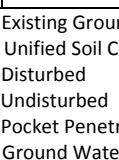

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				Borehole and Test Pit No: 5					
				Sheet No: 1 of 1					
Equipment : Backhoe, Drill Rig, DCP & Hand Auger		Location: DAVARA , ELA BEACH PS : 9							
CLIENT : njs - JICA (eda ranu)									
PROJECT : Port Moresby Sewerage System Upgrading									
		Ground Level: 1.355		Coordinates GPS 104632.233 N 105304.996 E					
				Program Duration Commence : 17/04/11 Completed: 03/05/11					
Description	USC	Graphical Log	Depth (m)	Samples / Tests				Field Records	
				Strata Thickness (m)	Type	Samples Depth (m)	SPT N' VALUE		ABC (kPa)
Bitumen (20mm thickness)			0.4			0.0 – 1.0		Excavation	
Road Base (20 – 250mm thickness)			0.8						
			1.2						
Sub-Base (250 – 500mm thickness)			1.6		DCP	1.5	2/4/3 N = 7	172	1.0 – 2.0m depth Very loose - loose
medium plasticity, red ridge Sandy Gravelly CLAY (FILL) 500 – 800mm thick			2.0		DCP	2.0	4/4/8 N = 12	255	2.0 – 2.5m depth Medium dense
WASHED OUT Drill core was completely washed out. This section is therefore interpreted as being occupied by fine grained beach sand and coral mixture.			2.4		DCP	2.7	12/11/15 N = 26	456	2.5 – 3.0m depth Dense
			3.2						
			3.6						
			4.0						
			4.4						
			4.8						
			5.2						
			5.6						
			6.0						
			6.4						
	6.8								
	7.2								
	7.6								
	8.0								
	8.4								
	8.8								
	9.2								
Bed rock			9.6						
			10.0						
			10.4						
Terminated at 10.5m depth			10.8						
			11.2						
			11.6						
			12.0						
			12.4						
			12.8						
			13.2						
			13.6						
			14.0						
			14.4						
			14.8						
			15.2						
			15.6						
			16.0						
			16.4						
			16.8						
			17.2						
			17.6						
			18.0						
			18.4						
			18.6						
.KEY: DCP Dynamic Cone Penetrometer SPT Standard Penetration Test VST Vane Shear Test CR Core Recovery (%) RQD Rock Quality Designation (%)		EGL Existing Ground Level USC Unified Soil Classification D Disturbed UD Undisturbed PP Pocket Penetrometer GWT  Ground Water Table		Remarks: No Ground Water Table was encountered on completion		Logged by: WG/PE Checked by: ME Drafted by : Scale:			

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				Borehole and Test Pit No: 7						
				Sheet No: 1 of 1						
Equipment : Backhoe, DCP & Hand Auger		Location: KILA POLICE PS : 7								
CLIENT : njs - JICA (eda ranu)										
PROJECT : Port Moresby Sewerage System Upgrading										
		Ground Level: 0.533		Coordinates GPS 102934.812 N 108384.989 E						
				Program Duration Commence : 23/04/11 Completed: 23/04/11						
Description	USC	Graphic Log	Depth (m)	Samples / Tests				Field Records		
				Strata Thickness (m)	Type	Samples Depth (m)	SPT N' VALUE		ABC (kPa)	
Gravelly Sandy CLAY High plasticity, soft o firm clay, brown			0.1					0.0 – 0.5m depth Soft - fir m		
					0.2					
					0.3					
					0.4					
					0.5		DCP	0.5	3/4/5 N = 9	207
					0.6					
					0.7					
					0.8					
					0.9		DCP	1.0	4/4/5 N = 9	207
					1.0					
					1.1					
					1.2		DCP	1.2	20/16/20 N = 36	564
			1.3							
			1.4							
Terminated at 1.4m depth on bed rock			1.5					1.0 - 1.5m depth Firm to stiff		
			1.6							
			1.7							
			1.8							
			1.9							
			2.0							
			2.1							
			2.2							
			2.3							
			2.4							
			2.5							
			2.6							
			2.7							
			2.8							
		2.9								
		3.0								
		3.1								
		3.2								
		3.3								
		3.4								
		3.5								
		3.6								
		3.7								
		3.8								
		3.9								
		4.0								
		4.1								
		4.2								
		4.3								
		4.4								
		4.5								
		4.6								
		4.7								
KEY: DCP Dynamic Cone Penetrometer SPT Standard Penetration Test VST Vane Shear Test CR Core Recovery (%) RQD Rock Quality Designation (%)		EGL Existing Ground Level USC Unified Soil Classification D Disturbed UD Undisturbed PP Pocket Penetrometer GWT Ground Water Table		Remarks:		Logged by: WG/PE Checked by: ME Drafted by : Scale:				

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				Borehole and Test Pit No: 8					
				Sheet No: 1 of 1					
Equipment : Backhoe DCP & Hand Auger		Location: KONEBADA PS : 8							
CLIENT : njs - JICA (eda ranu)									
PROJECT : Port Moresby Sewerage System Upgrading									
		Ground Level: 3.057		Coordinates GPS 103380.583 N 108120.595 E					
				Program Duration Commence : 23/04/11 Completed: 23/04/11					
Description	USC	Graphic Log	Depth (m)	Samples / Tests				Field Records	
				Strata Thickness (m)	Type	Samples Depth (m)	SPT N' VALUE		ABC (kPa)
Sandy Gravelly CLAY, low – medium plasticity, brown			0.1	0.600	DCP	0.5	2/4/3 N = 7	172	0.0 – 0.5m depth loose
			0.2						
			0.3						
			0.4						
			0.5						
			0.6						
Silty SAND with trace of gravel, non – plasticity, white (0.600 – 3.0m depth)			0.7	3.00	DCP	1.0	3/4/6 N = 10	223	0.5 – 1.0m depth Loose
			0.8						
			0.9						
			1.0						
			1.1						
			1.2						
			1.3						
			1.4						
			1.5						
			1.6						
			1.7						
			1.8						
			1.9						
			2.0						
			2.1	3.00	DCP	2.0	7/5/6 N = 11	240	1.5 – 2.0m depth Loose
			2.2						
			2.3						
			2.4						
			2.5						
			2.6						
Terminated at 3.5m depth on bed rock			2.7	3.00	DCP	2.5	9/14/18 N = 32	536	2.0 – 2.5m depth Medium dense – dense
			2.8						
			2.9						
			3.0						
			3.1						
			3.2						
			3.3	3.00	DCP	3.0	16/14/10 N = 24	425	2.5 – 3.0m depth Medium dense
			3.4						
			3.5						
			3.6						
			3.7						
			3.8						
			3.9	3.00	DCP	3.5	9/10/18 N = 28	472	3.0 – 3.5m depth Medium dense
			4.0						
			4.1						
			4.2						
			4.3						
			4.4						
4.5									
4.6									
4.7									
KEY: DCP Dynamic Cone Penetrometer SPT Standard Penetration Test VST Vane Shear Test CR Core Recovery (%) RQD Rock Quality Designation (%)	EGL Existing Ground Level USC Unified Soil Classification D Disturbed UD Undisturbed PP Pocket Penetrometer GWT Ground Water Table	Remarks: No Ground Water Table was encountered, dry on completion.				Logged by: WG/PE Checked by: ME Drafted by : Scale:			

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	Borehole and Test Pit No: 9
	Sheet No: 1 of 1

Equipment : Backhoe, DCP & Hand Auger	Location: GABUTU PS : 9
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CLIENT : njs - JICA (eda ranu)



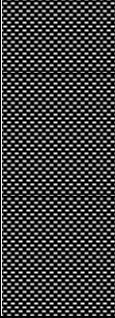

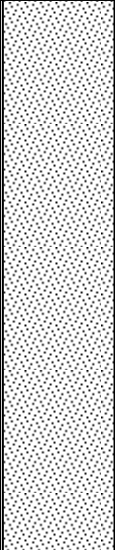


PROJECT : Port Moresby Sewerage System Upgrading

Ground Level: 1.888	GPS Coordinates 103870.850 N 108132.109 E	Program Duration	
		Commence : 23/04/11	Completed: 23/04/11





Description	USC	Graphic Log	Depth (m)	Samples / Tests				Field Records	
				Strata Thickness (m)	Type	Samples Depth (m)	SPT N' VALUE		ABC (kPa)
GRAVELLY SANDY CLAY High plasticity, soft to firm, moist, black clay			0.1	1.2	DCP	0.5	4/4/3 N = 7	172	0.0 – 0.5m depth soft - firm
			0.2						
			0.3						
			0.4						
			0.5						
			0.6						
			0.7						
			0.8						
			0.9						
			1.0						
..As above but becomes firm/hard, grey.....			1.1	3.0	DCP	1.0	3/6/6 N = 12	255	0.5 – 1.0m depth firm - stiff
			1.2						
			1.3						
			1.4						
			1.5						
			1.6						
			1.7						
			1.8						
			1.9						
			2.0						
Terminated at 3.0m depth on bed rock			2.1	3.0	DCP	1.5	8/9/9 N = 18	344	1.0 - 1.5m depth firm - stiff
			2.2						
			2.3						
			2.4						
			2.5						
			2.6						
			2.7						
			2.8						
			2.9						
			3.0						
			3.1	3.0	DCP	2.0	9/10/11 N = 21	385	1.5 – 2.0m depth firm = Stiff
			3.2						
			3.3						
			3.4						
			3.5						
			3.6						
			3.7						
			3.8						
			3.9	3.0	DCP	2.5	10/10/16 N = 26	456	2.0 – 2.5m depth firm or hard
			4.0						
			4.1						
			4.2						
			4.3						
			4.4						
			4.5						
			4.6						
			4.7						

KEY: DCP Dynamic Cone Penetrometer SPT Standard Penetration Test VST Vane Shear Test CR Core Recovery (%) RQD Rock Quality Designation (%)	EGL Existing Ground Level USC Unified Soil Classification D Disturbed UD Undisturbed PP Pocket Penetrometer GWT Ground Water Table	Remarks: Ground Water Table was encountered at 3.0 meters, run off from the hills and table drain.	Logged by: WG/PE Checked by: ME Drafted by : Scale:
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APPENDIX A

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		Borehole and Test Pit No: 10							
		Sheet No: 1 of 1							
Equipment : Backhoe DCP & Hand Auger	Location: KILAKILA (Horse Camp) PS : 10								
CLIENT : njs - JICA (eda ranu)									
PROJECT : Port Moresby Sewerage System Upgrading									
Ground Level: 1.534		GPS Coordinates 103047.593 N 109671.233 E	Program Duration Commenced : 23/03/11 Completed : 23/03/11						
Description	USC	Graphic Log	Depth (m)	Samples / Tests				Field Records	
				Strata Thickness (m)	Type	Samples Depth (m)	SPT N' VALUE		ABC (kPa)
Topsoil: root fibres (20mm thickness)			0.1					0.0 -0.2m	
Gravelly Sandy CLAY , medium to high plasticity, black. (0.1 – 1.4m depth)			0.2					penetration by weight of the DCP (Very Soft)	
			0.3						
			0.4						
			0.5		DCP	0.5	2/1/2 N = 3	92	0.3– 0.5m depth Very Soft
			0.6						
			0.7						
			0.8						
			0.9		0.9  GWT				
			1.0		DCP	1.0	1/2/1 N = 3	92	0.6 - 1.0m depth Very Soft
			1.1						
High plasticity, CLAY with some sand and trace of gravel, black (1.4 – 3.5m depth)			1.2					1.1 – 1.5m depth Very Soft	
			1.3						
			1.5		DCP	1.5	2/1/1 N = 2	68	1.6 – 2.0m depth Soft
			1.6						
			1.7						
			1.8						
			2.0		DCP	2.0	1/2/3 N = 5	134	2.1 – 2.5m depth Soft
			2.1						
			2.2						
			2.3						
Terminated at 3.5m depth			2.4						
			2.5		DCP	2.5	1/3/3 N = 6	153	2.6 – 3.0m depth Soft
			2.6						
			2.7						
			2.8						
			3.0		DCP	3.0	3/3/3 N = 6	153	3.1 – 3.5m depth Soft – Firm (Clay)
			3.1						
			3.2						
			3.3						
			3.4						
	3.5		DCP	3.5	4/7/8 N = 15	301	3.6 – 4.0m depth Firm – Stiff (Clay)		
	3.6								
	3.7								
	3.8								
	3.9								
	4.0		DCP	4.0	10/15/11 N = 26	456	4.1 – 4.5m depth Firm – Stiff (Clay)		
	4.1								
	4.2								
	4.3								
	4.4								
	4.5		DCP	4.5	14/12/12 N = 24	425			
	4.6								
	4.7								
KEY: DCP Dynamic Cone Penetrometer SPT Standard Penetration Test VST Vane Shear Test CR Core Recovery (%) RQD Rock Quality Designation (%) ABC Allowable Bearing Capacity (kPa)	EGL Existing Ground Level USC Unified Soil Classification D Disturbed UD Undisturbed PP Pocket Penetrometer GWT  Ground Water Table	Remarks: The soil profile was soft in depth so further investigation was carried out by drilling down to 10 meters depth.		Logged by: WG/PE Checked by: ME Drafted by : Scale:					


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	Borehole No: DCP/TP : 1								
	Sheet No: 1 of 1								
Equipment : Backhoe DCP & Hand Auger	Location: KILAKILA DCP/TP : 1								
CLIENT : njs - JICA (eda ranu)									
PROJECT : Port Moresby Sewerage System Upgrading									
	Ground Level: 15.354	Coordinates GPS 8949805.202 N 521093.746 E	Program Duration Commenced : 23/03/11 Completed : 23/03/11						
Description	USC	Graphic Log	Depth (m)	Samples / Tests				Field Records	
				Strata Thickness (m)	Type	Samples Depth (m)	SPT N' VALUE		ABC (kPa)
Sandy Gravelly CLAY Black silty CLAY, medium plasticity (0 - 500mm thickness)			0.1	0.7	DCP	0.5	2/4/4 N = 8	190	0.0 – 0.5m depth Firm
			0.2						
			0.3						
			0.4						
			0.5						
Sandy CLAY brown stiff CLAY high plasticity, mc ≤ PI 500 – 800mm thick			0.6	1.3	DCP	1.0	4/15 N = 19	358	0.5 – 1.0m depth Firm – Stiff or Hard
			0.7						
			0.8						
			0.9						
			1.0						
BED ROCK Hard weathered mudstone			1.1	1.6					Firm to Stiff or Hard weathered bed rock, very hard to excavate at 1.3m depth
			1.2						
			1.3						
			1.4						
			1.5						
Terminated at 1.6m depth on bed rock/hard weathered mudstone			1.6						
			1.7						
			1.8						
			1.9						
			2.0						
			2.1						
			2.2						
			2.3						
			2.4						
			2.5						
			2.6						
			2.7						
			2.8						
			2.9						
			3.0						
3.1									
3.2									
3.3									
3.4									
3.5									
3.6									
3.7									
3.8									
3.9									
4.0									
4.1									
4.2									
4.3									
4.4									
4.5									
4.6									
4.7									
KEY: DCP Dynamic Cone Penetrometer SPT Standard Penetration Test VST Vane Shear Test CR Core Recovery (%) RQD Rock Quality Designation (%) ABC Allowable Bearing Capacity	EGL Existing Ground Level USC Unified Soil Classification D Disturbed UD Undisturbed PP Pocket Penetrometer GWT ▽ Ground Water Table	Remarks: No Ground Water Table was encountered, dry on completion.	Logged by: WG/EPE Checked by: ME Drafted by : Scale:						

APPENDIX A

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					Borehole No: DCP/TP 2				
					Sheet No: 1 of 1				
Equipment : Backhoe DCP & Hand Auger		Location: KILAKILA DCP/TP : 2							
CLIENT : njs - JICA (eda ranu)									
PROJECT : Port Moresby Sewerage System Upgrading									
		Ground Level: 12.739		Coordinates GPS 8949813.912 N 521131.161 E		Program Duration Commenced : 23/03/11 Completed : 23/03/11			
Description	USC	Graphic Log	Depth (m)	Samples / Tests				Field Records	
				Strata Thickness (m)	Type	Samples Depth (m)	SPT N' VALUE		ABC (kPa)
Sandy GRAVEL Brown, with trace of silty clay, medium plasticity (0 - 600mm depeth)			0.1		DCP	0.5	7/2/1 N = 3	92	0.0 – 0.5m depth Soft
			0.2						
			0.3						
			0.4						
			0.5						
Charted bedrock (600 – 1.00m thick)			0.6		DCP	1.0	3/12/20 N = 32	534	0.5 – 1.0m depth Firm – Stiff or Hard
			0.7						
			0.8						
			0.9						
			1.0						
Terminated at 1.0m depth and Charted bed rock			1.1						Firm to Stiff or Hard very hard to excavate at 0.6m depth
			1.2						
			1.3						
			1.4						
			1.5						
			1.6						
			1.7						
			1.8						
			1.9						
			2.0						
			2.1						
			2.2						
			2.3						
			2.4						
			2.5						
2.6									
2.7									
2.8									
2.9									
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3.2									
3.3									
3.4									
3.5									
3.6									
3.7									
3.8									
3.9									
4.0									
4.1									
4.2									
4.3									
4.4									
4.5									
4.6									
4.7									
KEY: DCP Dynamic Cone Penetrometer SPT Standard Penetration Test VST Vane Shear Test CR Core Recovery (%) RQD Rock Quality Designation (%) ABC Allowance Bearing Capacity		EGL Existing Ground Level USC Unified Soil Classification D Disturbed UD Undisturbed PP Pocket Penetrometer GWT  Ground Water Table		Remarks: No Ground Water Table was encountered, dry on completion.		Logged by: WG/PE Checked by: ME Drafted by : Scale:			

APPENDIX A





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	Borehole No: DCP/TP 3
	Sheet No: 1 of 1

Equipment : Backhoe DCP & Hand Auger	Location: KILAKILA TP: 3
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CLIENT : njs - JICA (eda ranu)








PROJECT : Port Moresby Sewerage System Upgrading

Ground Level: 14.099	GPS Coordinates 8949896.153 N 521062.589 E	Program Duration Commenced : 23/03/11 Completed : 23/03/11
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Description	USC	Graphic Log	Depth (m)	Samples / Tests				Field Records	
				Strata Thickness (m)	Type	Samples Depth (m)	SPT N' VALUE		ABC (kPa)
Sandy Gravelly CLAY, low - medium plasticity, brown. (0 - 400mm depth)			0.1 0.2 0.3 0.4	0.400	DCP	0.5	2/4/3 N = 7	172	0.0 – 0.5m depth Very Soft - Soft
Sandy Gravelly CLAY, medium - high plasticity, brown. (400 – 1.7m depth)			0.5 0.6 0.7 0.8 0.9 1.0 1.1 1.2 1.3 1.4 1.5	1.7	DCP	1.0	2/2/6 N = 8	190	0.5 – 1.0m depth Soft - Firm
			1.5			5/7/7 N = 14	286		1.0 - 1.5m depth Firm
Sandy CLAY with trace of gravel, non - plasticity, whitish, mc ≤ PI 1.7 – 2.1m depth			1.7 1.8 1.9 2.0	2.5	DCP	2.0	10/20 N = 30	521	1.5 – 2.0m depth Firm – Stiff or Hard
Highly weathered weak mudstone			2.1 2.2 2.3 2.4 2.5			2.0 – 2.5m depthAs Above.....			
Terminated at 2.5m depth on bed rock			2.5 2.6 2.7 2.8 2.9 3.0 3.1 3.2 3.3 3.4 3.5 3.6 3.7 3.8 3.9 4.0 4.1 4.2 4.3 4.4 4.5 4.6 4.7						Firm to Stiff or Hard on weathered bed rock, very hard to excavate at 2.1m depth

KEY: DCP Dynamic Cone Penetrometer SPT Standard Penetration Test VST Vane Shear Test CR Core Recovery (%) RQD Rock Quality Designation (%) ABC Allowable Bearing Capacity	EGL Existing Ground Level USC Unified Soil Classification D Disturbed UD Undisturbed PP Pocket Penetrometer GWT √Ground Water Table	Remarks: No Ground Water Table was encountered, dry on completion.	Logged by: WG/PE Checked by: ME Drafted by : Scale:
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				Borehole No: DCP/TP : 4				
				Sheet No: 1 of 1				
Equipment : Backhoe DCP & Hand Auger		Location: KILAKILA DCP/TP : 4						
CLIENT : njs - JICA (eda ranu)								
PROJECT : Port Moresby Sewerage System Upgrading								
Equipment : Backhoe DCP & Hand Auger		Ground Level: 6.099		Coordinates GPS 8949911.096 N 521108.675 E				
				Program Duration Commenced : 23/03/11 Completed : 23/03/11				
Description	USC	Graphic Log	Depth (m)	Samples / Tests			Field Records	
				Strata Thickness (m)	Type	Samples Depth (m)		SPT N' VALUE
Topsoil: root fibres and peat (0 – 0-20mm depth)			0.1	0.20				
brown Sandy Gravelly CLAY, medium - high plasticity, mc ≤ PI 200 – 700mm depth			0.2	0.70	DCP	0.5	2/3/5 N = 8	190
			0.3					
Sandy Gravelly CLAY, medium - high plasticity, brown. (0.700 – 1.3m depth)			0.4	1.3	DCP	1.0	5/5/11 N = 16	316
			0.5					
chart weathered mudstone			0.6	1.7				0.5 – 1.0m depth Soft - Firm
			0.7					
Terminated at 1.7m depth on bed rock/hard weathered mudstone			0.8					1.0 – 1.5m depth Firm – Stiff or Hard
			0.9					
			1.0					
			1.1					
			1.2					
			1.3					
			1.4					
			1.5					
			1.6					
			1.7					
			1.8					
			1.9					
			2.0					
			2.1					
2.2								
2.3								
2.4								
2.5								
2.6								
2.7								
2.8								
2.9								
3.0								
3.1								
3.2								
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3.4								
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3.7								
3.8								
3.9								
4.0								
4.1								
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4.3								
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4.6								
4.7								
KEY: DCP Dynamic Cone Penetrometer SPT Standard Penetration Test VST Vane Shear Test CR Core Recovery (%) RQD Rock Quality Designation (%) ABC Allowable Bearing Capacity (kPa)		EGL Existing Ground Level USC Unified Soil Classification D Disturbed UD Undisturbed PP Pocket Penetrometer GWT  Ground Water Table		Remarks: No Ground Water Table was encountered, dry on completion.		Logged by: WG/PE Checked by: ME Drafted by : Scale:		



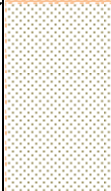




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	Borehole No: DCP/TP : 5										
	Sheet No: 1 of 1										
Equipment : Backhoe DCP & Hand Auger	Location: KILAKILA DCP/TP : 5										
CLIENT : njs - JICA (eda ranu)											
PROJECT : Port Moresby Sewerage System Upgrading											
	Ground Level: 19.626	GPS Coordinates 8949974.770 N 521042.324 E	Program Duration Commenced : 23/03/11 Completed : 23/03/11								
Description	USC	Graphic Log	Depth (m)	Samples / Tests				Field Records			
				Strata Thickness (m)	Type	Samples Depth (m)	SPT N' VALUE		ABC (kPa)		
Charted Bedrock, high weathered mudstone (0 – 1.00m depth)			0.1					0.0 – 0.5m depth Soft 0.5 – 1.0m depth Firm to Stiff or Hard			
	0.2										
	0.3										
	0.4										
	0.5					DCP	0.5		3/2/1 N = 3	92	
	0.6										
	0.7										
	0.8										
	0.9						DCP		1.0	2/10/20 N = 30	521
	1.0										
	Terminated at 1.0m depth on bedrock/high weathered				1.1						Firm to Stiff or Hard weathered bed rock, very hard to excavate at 1.0m depth
1.2											
1.3											
1.4											
1.5											
1.6											
1.7											
1.8											
1.9											
2.0											
2.1											
2.2											
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4.0											
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4.4											
4.5											
4.6											
4.7											
KEY: DCP Dynamic Cone Penetrometer SPT Standard Penetration Test VST Vane Shear Test CR Core Recovery (%) RQD Rock Quality Designation (%) ABC Allowable Bearing Capacity (kPa)	EGL Existing Ground Level USC Unified Soil Classification D Disturbed UD Undisturbed PP Pocket Penetrometer GWT Ground Water Table	Remarks: No Ground Water Table was encountered, dry on completion.	Logged by: WG/PE Checked by: ME Drafted by : Scale:								















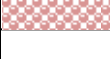







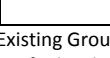
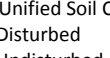
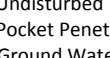
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				Borehole No: DCP/TP 6					
				Sheet No: 1 of 1					
Equipment : Backhoe DCP & Hand Auger		Location: KILAKILA DCP/TP: 6							
CLIENT : njs - JICA (eda ranu)									
PROJECT : Port Moresby Sewerage System Upgrading									
		Ground Level: 7.662		Coordinates GPS 8949985.557 N 521086.753 E					
				Program Duration Commenced : 23/03/11 Completed : 23/03/11					
Description	USC	Graphic Log	Depth (m)	Samples / Tests			Field Records		
				Strata Thickness (m)	Type	SPT N' VALUE		ABC (kPa)	
Topsoil: Black silty CLAY, medium plasticity (0 - 200mm thickness)			0.1 0.2	0.20					
Whitish silty CLAY, medium plasticity, mc ≤ PI 200 – 800mm thick			0.3 0.4 0.5 0.6 0.7	0.800	DCP	0.5	3/2/1 N = 3	92	0.0 – 0.5m depth Very Soft
			0.8						
Weathered mudstone			0.8 0.9 1.0 1.1		DCP	1.0	2/8/20 N = 28	472	0.5 – 1.0m depth Very Soft - Firm
Terminated at 1.1m depth on bed rock/hard weathered mudstone			1.1 1.2 1.3 1.4 1.5 1.6 1.7 1.8 1.9 2.0 2.1 2.2 2.3 2.4 2.5 2.6 2.7 2.8 2.9 3.0 3.1 3.2 3.3 3.4 3.5 3.6 3.7 3.8 3.9 4.0 4.1 4.2 4.3 4.4 4.5						1.0 - 1.5m depth Firm – Stiff or Hard Firm to Stiff or Hard on weathered bed rock, very hard to excavate at 0.8m depth
KEY: DCP Dynamic Cone Penetrometer SPT Standard Penetration Test VST Vane Shear Test CR Core Recovery (%) RQD Rock Quality Designation (%) ABC Allowable Bearing Pressure (kPa)		EGL Existing Ground Level USC Unified Soil Classification D Disturbed UD Undisturbed PP Pocket Penetrometer GWT  Ground Water Table		Remarks: No Ground Water Table was encountered, dry on completion.			Logged by: WG/PE Checked by: ME Drafted by : Scale:		







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				Borehole No: DCP/TP: 1				
				Sheet No: 1 of 1				
Equipment : Backhoe DCP & Hand Auger		Location: MORATA STP DCP/TP : 1						
CLIENT : njs - JICA (eda ranu)								
PROJECT : Port Moresby Sewerage System Upgrading								
		Ground Level: 17.708		Coordinates GPS 8962014.389 N 520227.242 E				
				Program Duration Commence : 24/03/11 Completed: 12/05/11				
Description	USC	Graphic Log	Depth (m)	Samples / Tests				Field Records
				Strata Thickness (m)	Type	Samples Depth (m)	SPT N' VALUE	
Topsoil: mixture with root fibres, red ridge silty CLAY low – medium plasticity, reddih brown (00 - 300mm depth)			0.1 0.2 0.3 0.4	0.300				
Silty Gravelly CLAY, medium – high plasticity, white, mc ≤ PI			0.5 0.6 0.7 0.8 0.9		DCP	0.5	2/4/2 N = 6	153
highly weathered mudstone ,medium plasticity, whitish brown (1.0 –1.3m depth)			1.0 1.1 1.2 1.3	1.3	DCP	1.0	2/6/12 N = 18	344
Bed rock, highly weathered (1.3 - 2.0m depth)			1.4 1.5 1.6 1.7 1.8 1.9	2.0	DCP	1.5	N = 20	372
Bed rock, highly weathered (2.0 – 2.8m depth)			2.0 2.1 2.2 2.3 2.4 2.5 2.6 2.7					
Terminated at 2.0m depth on bed rock			2.8 2.9 3.0 3.1 3.2 3.3 3.4 3.5 3.6 3.7 3.8 3.9 4.0 4.1 4.2 4.3 4.4 4.5 4.6 4.7					
KEY: DCP Dynamic Cone Penetrometer SPT Standard Penetration Test VST Vane Shear Test CR Core Recovery (%) RQD Rock Quality Designation (%) ABC Allowable Bearing Capacity (kPa)		EGL Existing Ground Level USC Unified Soil Classification D Disturbed UD Undisturbed PP Pocket Penetrometer GWT Ground Water Table		Remarks: No Ground water Table encountered, dry on completion.			Logged by: WG/PE Checked by: ME Drafted by : Scale:	



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				Borehole No: DCP/TP :2					
				Sheet No: 1 of 1					
Equipment : Backhoe DCP & Hand Auger		Location: MORATA STP DCP/TP : 2							
CLIENT : njs - JICA (eda ranu)									
PROJECT : Port Moresby Sewerage System Upgrading									
		Ground Level: 16.757		Coordinates GPS 520225.540 E 8962028.369 N					
				Program Duration Commence : 24/03/11 Completed: 12/05/11					
Description	USC	Graphic Log	Depth (m)	Samples / Tests			Field Records		
				Strata Thickness (m)	Type	Samples Depth (m)		SPT N' VALUE	ABC (kPa)
Topsoil: mixture with root fibres, red ridge silty CLAY low – medium plasticity, reddih brown (00 - 300mm depth)			0.1	0.300	DCP	0.5	1/2/1 N = 3	92	0.0 – 0.5m depth Soft
			0.2						
Silty Gravelly CLAY, medium – high plasticity, white, mc ≤ PI			0.3	0.8 ∇ GWT	DCP	1.0	1/1/2 N = 3	92	0.5 – 1.0m depth Soft
			0.4						
highly weathered mudstone ,medium plasticity, whitish brown (1.3 – 2.0m depth)			0.5	1.3	DCP	1.5	4/2/12 N = 14	286	1.0 - 1.5m depth Firm
			0.6						
Bed rock, highly weathered (2.0 – 3.0m depth)			0.7	2.0	DCP	2.0	13/15/23 N = 38	584	1.5 – 2.0m depth Firm – Stiff/Hard
			0.8						
Terminated at 3.0m depth on bed rock			0.9						
			1.0						
			1.1						
			1.2						
			1.3						
			1.4						
			1.5						
			1.6						
			1.7						
			1.8						
			1.9						
			2.0						
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			4.0						
			4.1						
			4.2						
			4.3						
			4.4						
			4.5						
			4.6						
			4.7						
			4.8						
KEY: DCP Dynamic Cone Penetrometer SPT Standard Penetration Test VST Vane Shear Test CR Core Recovery (%) RQD Rock Quality Designation (%)		EGL Existing Ground Level USC Unified Soil Classification D Disturbed UD Undisturbed PP Pocket Penetrometer GWT Ground Water Table		Remarks: Ground Water Table encountered at 0.800m depth , seepage from the existing sewerage pond.		Logged by: WG/PE Checked by: ME Drafted by : Scale:			

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				Borehole No: DCP/TP 4					
				Sheet No: 1 of 1					
Equipment : Backhoe DCP & Hand Auger		Location: MORATA STP DCP/TP : 4							
CLIENT : njs - JICA (eda ranu)									
PROJECT : Port Moresby Sewerage System Upgrading									
		Ground Level: 18.725		Coordinates GPS 8962043.714 N 520269.721 E					
				Program Duration Commence : 24/03/11 Completed: 12/05/11					
Description	USC	Graphic Log	Depth (m)	Samples / Tests				Field Records	
				Strata Thickness (m)	Type	Samples Depth (m)	SPT N' VALUE		ABC (kPa)
Topsoil: mixture with root fibres, red ridge silty CLAY low – medium plasticity, reddih brown (00 - 300mm depth)			0.1 0.2 0.3 0.4	0.300	DCP	0.5	2/1/6 N = 7	172	0.0 – 0.5m depth Soft
Silty Gravelly CLAY, medium – high plasticity, brown, mc ≤ PI			0.5 0.6 0.7 0.8 0.9 1.0 1.1 1.2	1.300					
highly weathered mudstone ,medium plasticity, brown (1.3 – 2.0m depth)			1.3 1.4 1.5 1.6 1.7 1.8 1.9	2.00	DCP	2.0	15/15/15 N = 30	521	1.0 – 1.5m depth soft
Bed rock, highly weathered (2.0 – 3.0m depth)			2.0 2.1 2.2 2.3 2.4 2.5 2.6 2.7 2.8 2.9						
Terminated at 3.0m depth on bed rock			3.0 3.1 3.2 3.3 3.4 3.5 3.6 3.7 3.8 3.9 4.0 4.1 4.2 4.3 4.4 4.5 4.6 4.7						
KEY: DCP Dynamic Cone Penetrometer SPT Standard Penetration Test VST Vane Shear Test CR Core Recovered by 25m depth on Bedrock RQD Rock Quality Designation (%)		EGL Existing Ground Level USC Unified Soil Classification D Disturbed UD Undisturbed PP Pocket Penetrometer GWT Ground Water Table		Remarks: No Ground Water Table was encountered, dry on completion			Logged by: WG/PE Checked by: ME Drafted by : Scale:		

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				Borehole No: DCP/TP: 5						
				Sheet No: 1 of 1						
Equipment : Backhoe DCP & Hand Auger		Location: MORATA STP DCP/TP : 5								
CLIENT : njs - JICA (eda ranu)										
PROJECT : Port Moresby Sewerage System Upgrading										
		Ground Level: 16.907		Coordinates 8962066.174 N 520258.222 E						
				Program Duration Commence : 24/03/11 Completed: 12/05/11						
Description	USC	Graphic Log	Depth (m)	Samples / Tests				Field Records		
				Strata Thickness (m)	Type	Samples Depth (m)	SPT N' VALUE		ABC (kPa)	
Topsoil: mixture with root fibres, red ridge silty CLAY low – medium plasticity, reddih brown (00 - 400mm depth)			0.1 0.2 0.3 0.4	0.400	DCP	0.5	2/3/4 N = 7	172	0.0 – 0.5m depth Soft	
Silty Gravelly CLAY, low - medium plasticity, white, mc ≤ PI	0.5 0.6 0.7 0.8 0.9 1.0		1.0	▽ GWT						DCP
highly weathered mudstone ,medium plasticity, whitish brown (1.0 – 1.7m depth)			1.1 1.2 1.3 1.4 1.5 1.6 1.7	1.7	DCP	1.5	10/11/19 N = 30	521	1.0 - 1.5m depth Firm – stiff/hard	
Bed rock, highly weathered (2.0 – 3.0m depth)	1.8 1.9 2.0 2.1 2.2 2.3 2.4 2.5		2.5							
Terminated at 2.5m depth on bed rock				2.6 2.7 2.8 2.9 3.0 3.1 3.2 3.3 3.4 3.5 3.6 3.7 3.8 3.9 4.0 4.1 4.2 4.3 4.4 4.5 4.6 4.7						
KEY: DCP Dynamic Cone Penetrometer SPT Standard Penetration Test VST Vane Shear Test CR Core Recovery (%) RQD Rock Quality Designation (%)			EGL Existing Ground Level USC Unified Soil Classification D Disturbed UD Undisturbed PP Pocket Penetrometer GWT Ground Water Table		Remarks: Ground Water Table encountered at 1.0m depth, seepage from existing sewerage pond			Logged by: WG/PE Checked by: ME Drafted by : Scale:		



DEPARTMENT OF WORKS

DATA SHEET

Sht 1 of 4

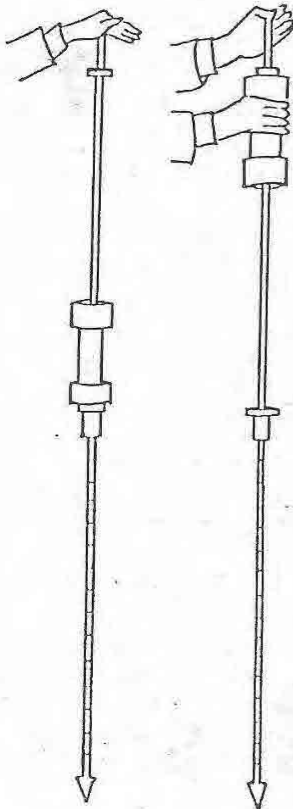
SCALA DYNAMIC CONE PENETROMETER

INTRODUCTION

The Scala Penetrometer Test is one of several simple and quick methods of obtaining a measure of subgrade strength for use in the design of road and airport pavements. This test, which was developed by A J Scala, was the first insitu cone penetration test to be widely used in this part of the world for the assessment of CBR strengths. Scala's paper outlining the design, use and interpretation of this dynamic cone penetrometer is titled "Simple Methods of Flexible Pavement Design Using Cone Penetrometers" and was published in February 1956 in New Zealand Engineering.

When Scala Penetrometer testing is carried out by the DOW Materials Laboratories it is generally done in association with a limited programme of laboratory or insitu CBR testing. The Scala Penetrometer Test has proved to be very useful in clay soils and reasonable correlation has been found with CBR measurements. Since a great number of tests can be carried out very quickly it can be used to confirm the uniformity of subgrade strength or to identify localised occurrences of soft subgrade ("soft spots") which might otherwise have remained unidentified.

INSTRUMENT DESCRIPTION



The apparatus consists of a 9.07 kg (20 lb) steel drop hammer which falls through a distance of 508 mm (20 in). The hammer slides on a 16 mm diameter steel rod and strikes an anvil at the lower end of that rod. A second length of 16 mm diameter steel rod with a hardened steel point having a cross sectional area of 322 mm² (0.5 sq in) is screwed into the lower extremity, the point itself having a 30 degree point. The lower section of the rod is marked at intervals of 50 mm (2 in).

The overall length of the apparatus when assembled is 1.83 meters (6ft) and the weight is 12.7 kg (28 lb). When dis-assembled it can be readily packed in a car.

TEST PROCEDURE AND PRESENTATION OF RESULTS

The penetrometer may be driven from the surface on undeveloped sites. Where the subgrade of an existing pavement is to be tested it is necessary to make a small hole through the pavement so that the penetrometer will not have to penetrate any hard or large material. The penetrometer may be driven adjacent to a road under certain circumstances but the moisture content and density of soils beside the road are unlikely to be truly representative of those beneath the pavement.



DEPARTMENT OF WORKS

DATA SHEET

Sht 2 of 4

SCALA DYNAMIC CONE PENETROMETER

The penetrometer is driven by blows of the drop hammer to a depth of 760 mm and a record is made of the depth of the point below the surface (or top of the pavement) after each blow. This is best done with the aid of a measuring stick or ruler 760 mm long which is held beside the instrument, then the decrease in height of the anvil from the ground can be measured after each blow. Plainly, one person must operate the penetrometer whilst another measures and records the amount of penetration.

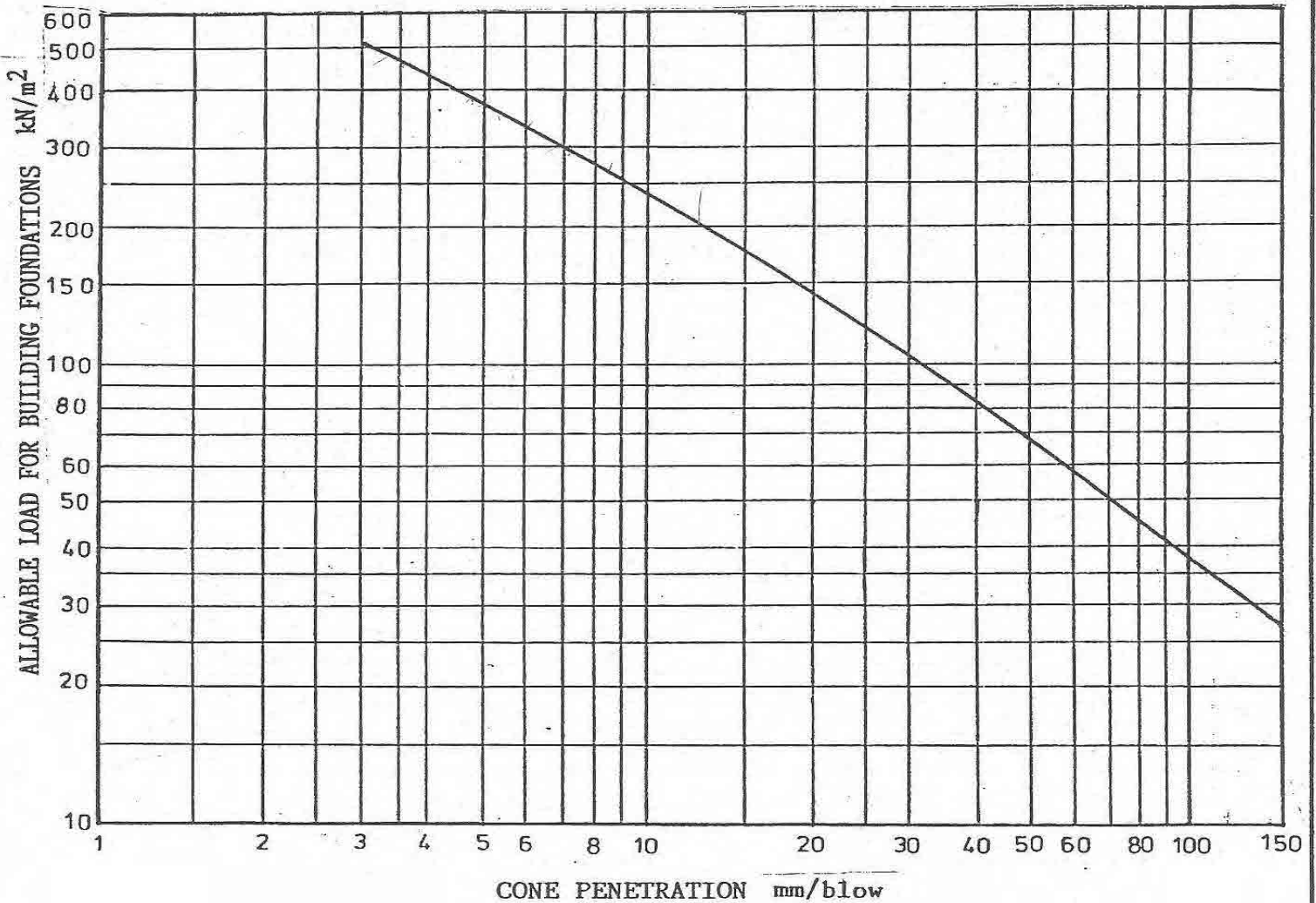
The results should be plotted graphically with the cumulative number of blows being plotted against depth of penetration. The slope of the curve at any depth is then a measure of the strength of the material over that depth range. A typical trial pit log and penetration record is shown on the example following this Data Sheet.

INTERPRETATION OF RESULTS

Scala (1956) reports the findings of a comprehensive investigation made to correlate the results of tests obtained using the cone penetrometer with actual field CBR values for the soils tested. The correlation with CBR at 2.5 mm (max CBR) is shown on Figure 1. This Figure should be used for the determination of CBR values from Scala Penetrometer results. Pavement design can then be determined in accordance with published CBR design charts.



SCALA DYNAMIC CONE PENETROMETER



After:
Stockwell MJ
New Zealand Engineering
(32,6) 15 June 1977

SCALA PENETROMETER
ALLOWABLE BEARING
PRESSURE CORRELATION

Figure 2

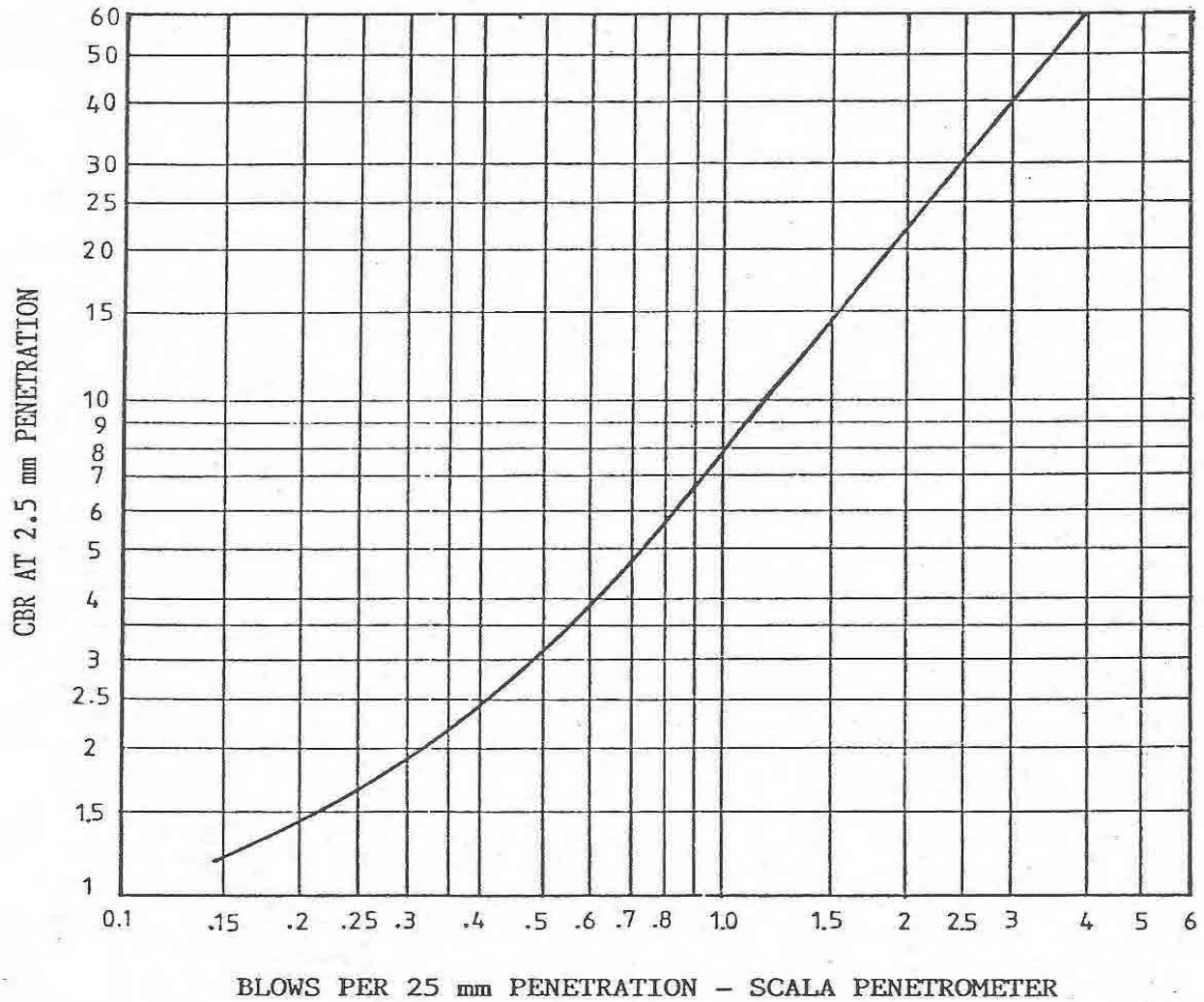


DEPARTMENT OF WORKS

DATA SHEET

Sht 3 of 4

SCALA DYNAMIC CONE PENETROMETER



SCALA PENETROMETER
CBR CORRELATION

Figure 1



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Project: Port Moresby Sewerage Upgrading

Location: New Pump Station - No. 1 Kanudi

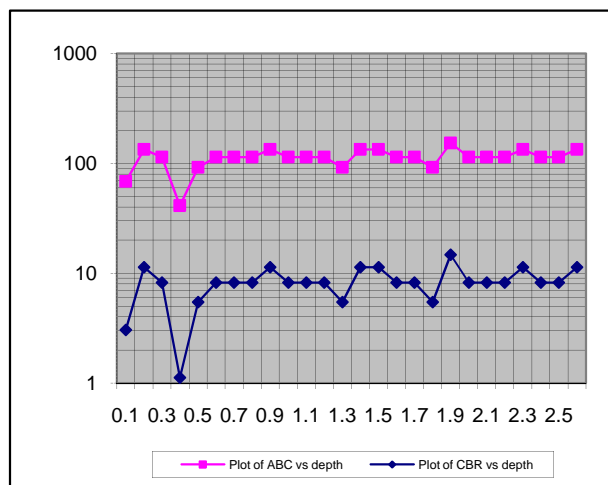
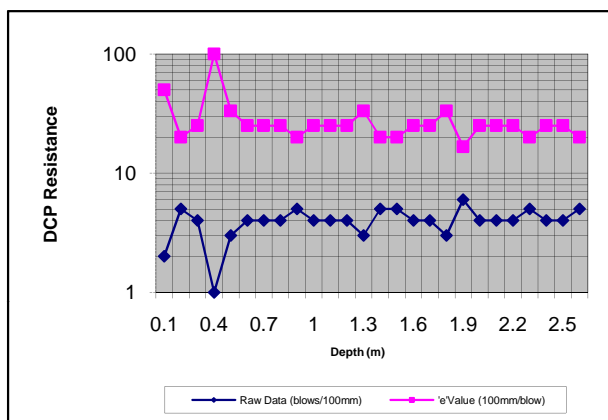
Client: njs - JICA

Job No: NGTS/01011

Date: March, 2011

DCP TEST 1 at Kanudi Pump Station

Depth (m)	Field Data (Blw/100mm)	e' Value (100mm/Blw)	ABC (kPa)	CBR (%)
0.1	2	50	68	3
0.2	5	20	134	11
0.3	4	25	114	8
0.4	1	100	41	1
0.5	3	33	92	5
0.6	4	25	114	8
0.7	4	25	114	8
0.8	4	25	114	8
0.9	5	20	134	11
1	4	25	114	8
1.1	4	25	114	8
1.2	4	25	114	8
1.3	3	33	92	5
1.4	5	20	134	11
1.5	5	20	134	11
1.6	4	25	114	8
1.7	4	25	114	8
1.8	3	33	92	5
1.9	6	17	153	15
2	4	25	114	8
2.1	4	25	114	8
2.2	4	25	114	8
2.3	5	20	134	11
2.4	4	25	114	8
2.5	4	25	114	8
2.6	5	20	134	11
2.7	6	17	153	15
2.8	5	20	134	11
2.9	4	25	114	8
3	5	20	134	11
3.1	7	14	172	18
3.2	7	14	172	18
3.3	9	11	207	27
3.4	9	11	207	27
3.5	8	13	190	22



APPENDIX B

DCP 'e' Values, California Bearing Ratio (CBR) & Allowable Bearing Capacity (ABC) vs Depth

Project: Port Moresby Sewerage Upgrading

Location: New Pump Station - No. 1 Kanudi

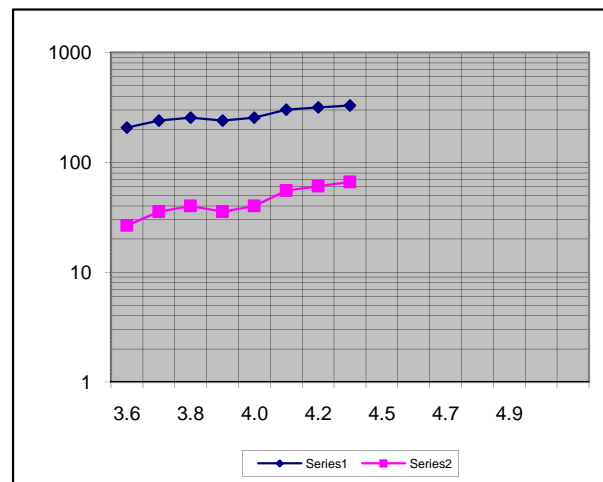
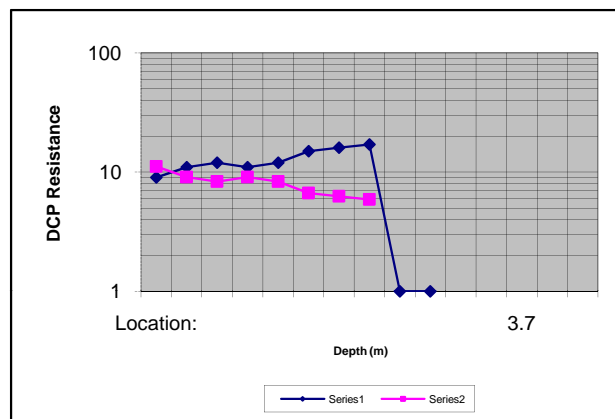
Client: njs - JICA

Job No.: NGTS/01011

Date: March, 2011

Continuation

Depth (m)	Field Data (Blw/100mm)	e' Value (100mm/Blw)	ABC (kPa)	CBR (%)
3.6	9	11	207	27
3.7	11	9	240	35
3.8	12	8	255	40
3.9	11	9	240	35
4.0	12	8	255	40
4.1	15	7	301	55
4.2	16	6	316	61
4.3	17	6	330	66
4.5	Refusal at			
4.6	4.3m depth			
4.7				
4.8				
4.9				
5.0				



APPENDIX B

DCP 'e' Values & Allowable Bearing Capacity (ABC) vs Depth



Niawang Geotech

Project: Port Moresby Sewerage Upgrading

Location: New Pump Station - No. 2 Idubada

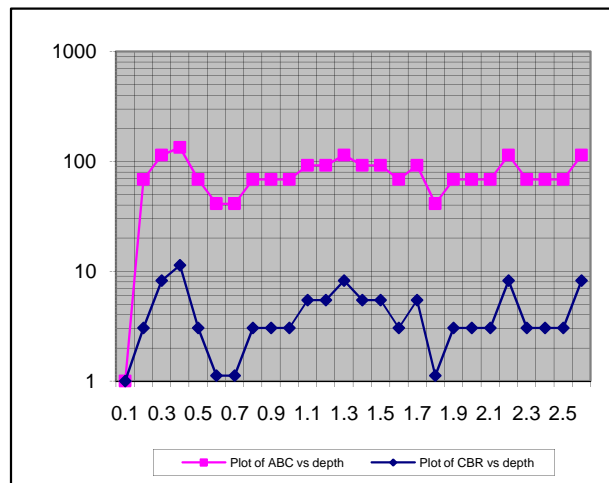
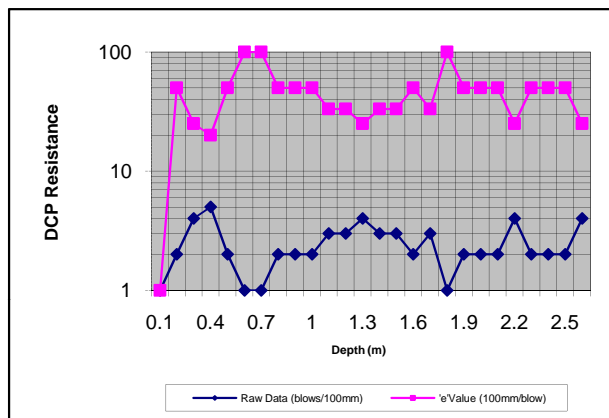
Client: njs - JICA

Job No: NGTS/01011

Date: March, 2011

DCP TEST # 1

Depth (m)	Field Data (Blw/100mm)	e' Value (100mm/Blw)	ABC (kPa)	CBR (%)
0.1	By weight	#VALUE!	#####	#####
0.2	2	50	68	3
0.3	4	25	114	8
0.4	5	20	134	11
0.5	2	50	68	3
0.6	1	100	41	1
0.7	1	100	41	1
0.8	2	50	68	3
0.9	2	50	68	3
1	2	50	68	3
1.1	3	33	92	5
1.2	3	33	92	5
1.3	4	25	114	8
1.4	3	33	92	5
1.5	3	33	92	5
1.6	2	50	68	3
1.7	3	33	92	5
1.8	1	100	41	1
1.9	2	50	68	3
2	2	50	68	3
2.1	2	50	68	3
2.2	4	25	114	8
2.3	2	50	68	3
2.4	2	50	68	3
2.5	2	50	68	3
2.6	4	25	114	8
2.7	6	17	153	15
2.8	6	17	153	15
2.9	9	11	207	27
3	9	11	207	27
3.1	11	9	240	35
3.2	12	8	255	40
3.3	13	8	271	45
3.4	12	8	255	40
3.5	11	9	240	35



APPENDIX B

DCP 'e' Values, California Bearing Ratio (CBR) & Allowable Bearing Capacity (ABC) vs Depth

Project: Port Moresby Sewerage Upgrading

Location: New Pump Station - No. 2 Idubada

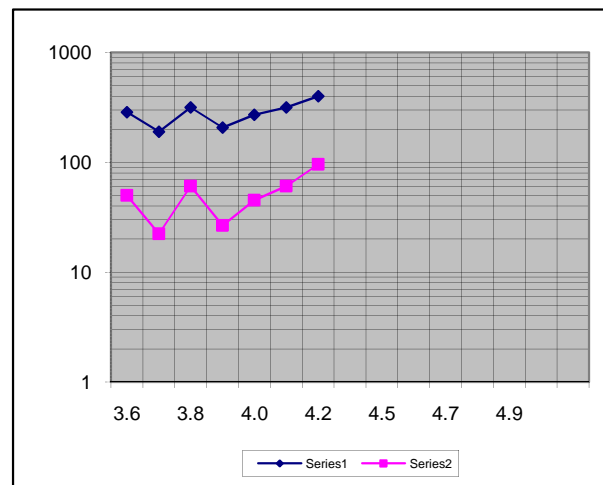
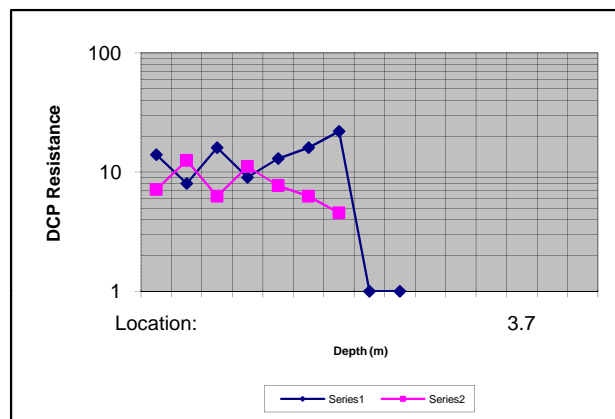
Client: njs - JICA

Job No.: NGTS/01011

Date: March, 2011

Continuation

Depth (m)	Field Data (Blw/100mm)	e' Value (100mm/Blw)	ABC (kPa)	CBR (%)
3.6	14	7	286	50
3.7	8	13	190	22
3.8	16	6	316	61
3.9	9	11	207	27
4.0	13	8	271	45
4.1	16	6	316	61
4.2	22	5	399	96
4.3	Refusal at 4.2m depth			
4.5				
4.6				
4.7				
4.8				
4.9				
5.0				



APPENDIX B

DCP 'e' Values & Allowable Bearing Capacity (ABC) vs Depth



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Project: Port Moresby Sewerage Upgrading

Location: New Pump Station - No. 3 Elavala

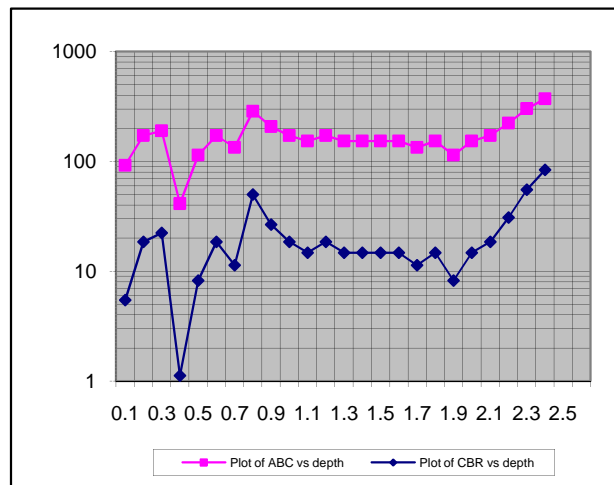
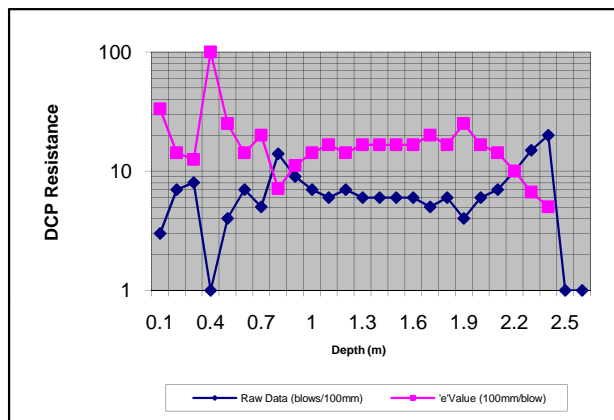
Client: njs - JICA

Job No: NGTS/01011

Date: March, 2011

DCP TEST # 1

Depth (m)	Field Data (Blw/100mm)	e' Value (100mm/Blw)	ABC (kPa)	CBR (%)
0.1	3	33	92	5
0.2	7	14	172	18
0.3	8	13	190	22
0.4	1	100	41	1
0.5	4	25	114	8
0.6	7	14	172	18
0.7	5	20	134	11
0.8	14	7	286	50
0.9	9	11	207	27
1	7	14	172	18
1.1	6	17	153	15
1.2	7	14	172	18
1.3	6	17	153	15
1.4	6	17	153	15
1.5	6	17	153	15
1.6	6	17	153	15
1.7	5	20	134	11
1.8	6	17	153	15
1.9	4	25	114	8
2	6	17	153	15
2.1	7	14	172	18
2.2	10	10	223	31
2.3	15	7	301	55
2.4	20	5	372	84
2.5	Refusal at 2.4m			
2.6				
2.7				
2.8				
2.9				
3				
3.1				
3.2				
3.3				
3.4				
3.5				



APPENDIX B

DCP 'e' Values, California Bearing Ratio (CBR) & Allowable Bearing Capacity (ABC) vs Depth

Project: Port Moresby Sewerage Upgrading

Location: New Pump Station - No. 1 Kanudi

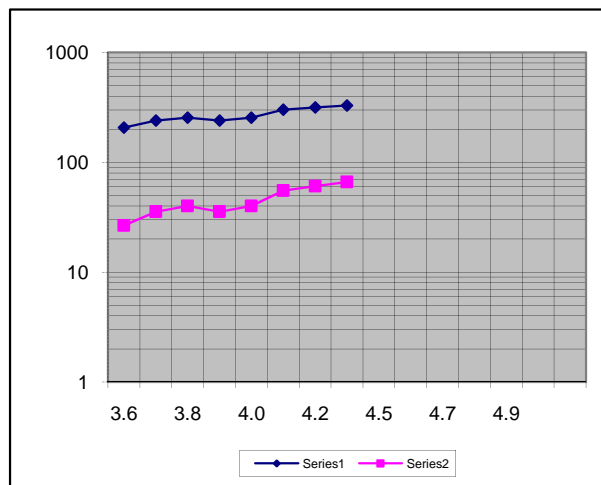
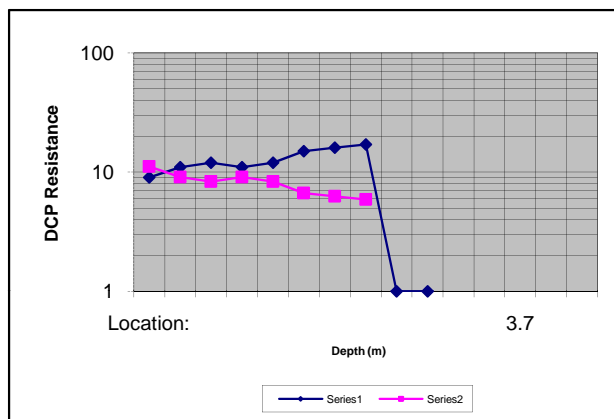
Client: njs - JICA

Job No.: NGTS/01011

Date: March, 2011

Continuation

Depth (m)	Field Data (Blw/100mm)	e' Value (100mm/Blw)	ABC (kPa)	CBR (%)
3.6	9	11	207	27
3.7	11	9	240	35
3.8	12	8	255	40
3.9	11	9	240	35
4.0	12	8	255	40
4.1	15	7	301	55
4.2	16	6	316	61
4.3	17	6	330	66
4.5	Refusal at			
4.6	4.5m depth			
4.7				
4.8				
4.9				
5.0				



APPENDIX A

DCP 'e' Values & Allowable Bearing Capacity (ABC) vs Depth



Niawang Geotech

Project: Port Moresby Sewerage Upgrading

Location: New Pump Station - No. 4 Hanuabada

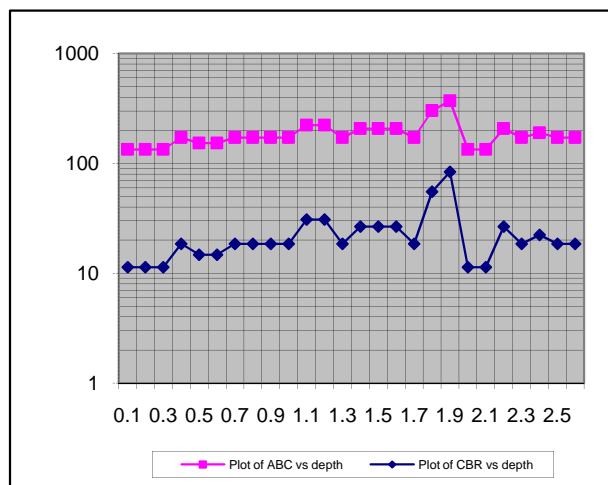
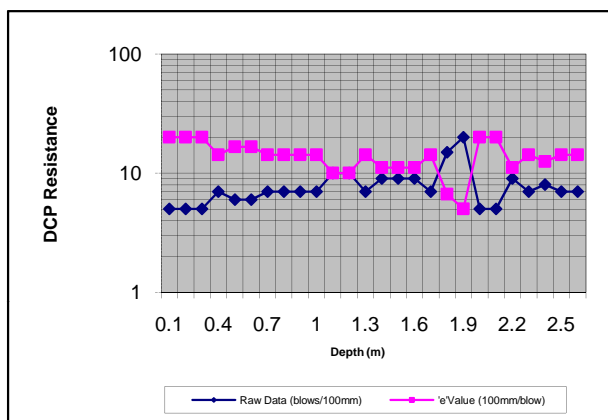
Client: njs - JICA

Job No: NGTS/01011

Date: March, 2011

DCP TEST # 1

Depth (m)	Field Data (Blw/100mm)	e' Value (100mm/Blw)	ABC (kPa)	CBR (%)
0.1	5	20	134	11
0.2	5	20	134	11
0.3	5	20	134	11
0.4	7	14	172	18
0.5	6	17	153	15
0.6	6	17	153	15
0.7	7	14	172	18
0.8	7	14	172	18
0.9	7	14	172	18
1	7	14	172	18
1.1	10	10	223	31
1.2	10	10	223	31
1.3	7	14	172	18
1.4	9	11	207	27
1.5	9	11	207	27
1.6	9	11	207	27
1.7	7	14	172	18
1.8	15	7	301	55
1.9	20	5	372	84
2	5	20	134	11
2.1	5	20	134	11
2.2	9	11	207	27
2.3	7	14	172	18
2.4	8	13	190	22
2.5	7	14	172	18
2.6	7	14	172	18
2.7	6	17	153	15
2.8	6	17	153	15
2.9	8	13	190	22
3	4	25	114	8
3.1	3	33	92	5
3.2	3	33	92	5
3.3	3	33	92	5
3.4	2	50	68	3
3.5	3	33	92	5



APPENDIX B

DCP 'e' Values, California Bearing Ratio (CBR) & Allowable Bearing Capacity (ABC) vs Depth

Project: Port Moresby Sewerage Upgrading

Location: New Pump Station - No. 4 Hanuabada

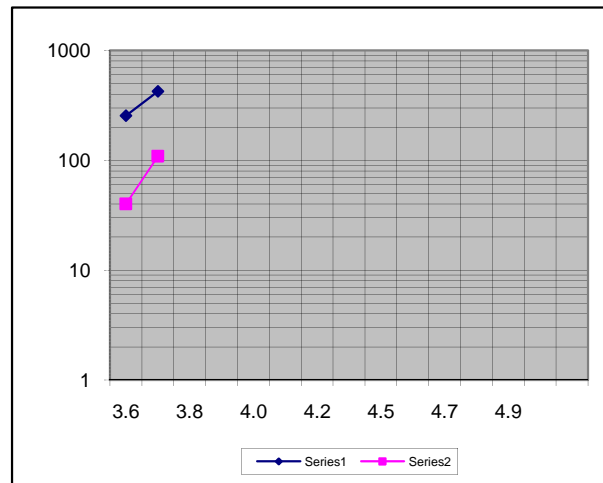
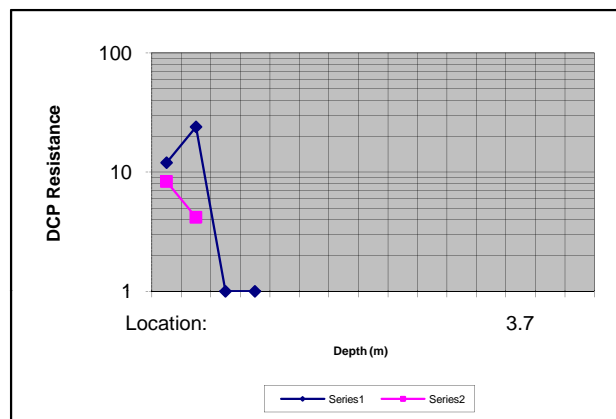
Client: njs - JICA

Job No.: NGTS/01011

Date: March, 2011

Continuation

Depth (m)	Field Data (Blw/100mm) (100mm/Blw)	e' Value (100mm/Blw)	ABC (kPa)	CBR (%)
3.6	12	8	255	40
3.7	24	4	425	109
3.8	Refusal at			
3.9	3.7m depth			
4.0				
4.1				
4.2				
4.3				
4.5				
4.6				
4.7				
4.8				
4.9				
5.0				



APPENDIX B

DCP 'e' Values & Allowable Bearing Capacity (ABC) vs Depth



Niwang Geotech

Project: Port Moresby Sewerage Upgrading

Location: Davara Pump Station - No. 05 Ela Beach

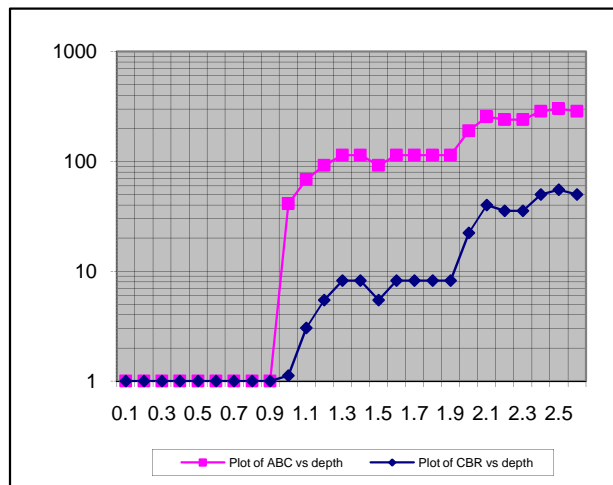
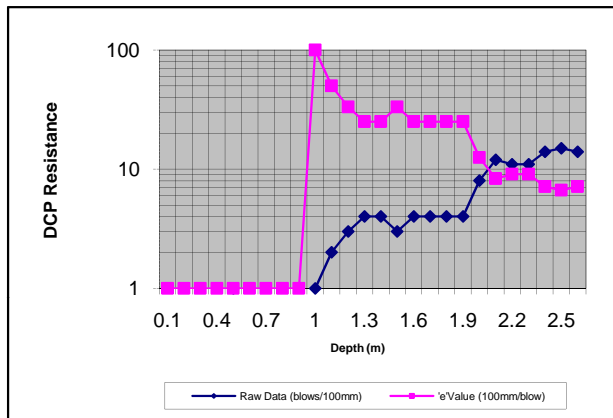
Client: njs - JICA

Job No: NGTS/01011

Date: March, 2011

DCP TEST # 05

Depth (m)	Field Data (Blw/100mm)	e' Value (100mm/Blw)	ABC (kPa)	CBR (%)
0.1		#DIV/0!	#DIV/0!	#DIV/0!
0.2		#DIV/0!	#DIV/0!	#DIV/0!
0.3		#DIV/0!	#DIV/0!	#DIV/0!
0.4		#DIV/0!	#DIV/0!	#DIV/0!
0.5	Excavation	#VALUE!	#####	#####
0.6		#DIV/0!	#DIV/0!	#DIV/0!
0.7		#DIV/0!	#DIV/0!	#DIV/0!
0.8		#DIV/0!	#DIV/0!	#DIV/0!
0.9		#DIV/0!	#DIV/0!	#DIV/0!
1	1	100	41	1
1.1	2	50	68	3
1.2	3	33	92	5
1.3	4	25	114	8
1.4	4	25	114	8
1.5	3	33	92	5
1.6	4	25	114	8
1.7	4	25	114	8
1.8	4	25	114	8
1.9	4	25	114	8
2	8	13	190	22
2.1	12	8	255	40
2.2	11	9	240	35
2.3	11	9	240	35
2.4	14	7	286	50
2.5	15	7	301	55
2.6	14	7	286	50
2.7	20	5	372	84
2.8	Terminated at			
2.9	2.7m			
3				
3.1				
3.2				
3.3				
3.4				
3.5				



APPENDIX B

DCP 'e' Values, California Bearing Ratio (CBR) & Allowable Bearing Capacity (ABC) vs Depth



Niwang Geotech

Project: Port Moresby Sewerage Upgrading

Location: Lawes Road Pump Station - No. 6 Ela Beach

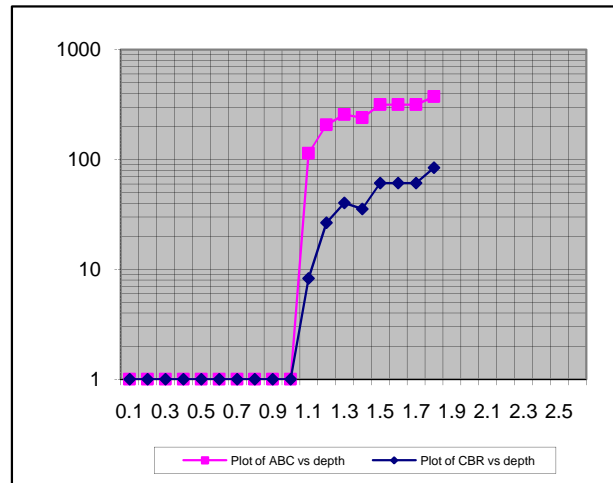
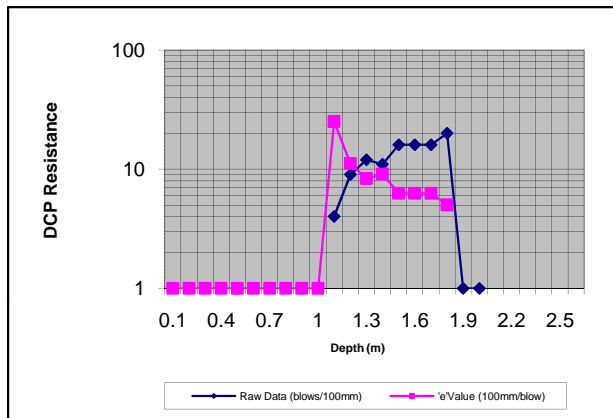
Client: njs - JICA

Job No: NGTS/01011

Date: March, 2011

DCP TEST # 6

Depth (m)	Field Data (Blw/100mm)	e' Value (100mm/Blw)	ABC (kPa)	CBR (%)
0.1		#DIV/0!	#DIV/0!	#DIV/0!
0.2		#DIV/0!	#DIV/0!	#DIV/0!
0.3		#DIV/0!	#DIV/0!	#DIV/0!
0.4		#DIV/0!	#DIV/0!	#DIV/0!
0.5	Excavation	#VALUE!	#####	#####
0.6		#DIV/0!	#DIV/0!	#DIV/0!
0.7		#DIV/0!	#DIV/0!	#DIV/0!
0.8		#DIV/0!	#DIV/0!	#DIV/0!
0.9	By weight	#VALUE!	#####	#####
1		#DIV/0!	#DIV/0!	#DIV/0!
1.1	4	25	114	8
1.2	9	11	207	27
1.3	12	8	255	40
1.4	11	9	240	35
1.5	16	6	316	61
1.6	16	6	316	61
1.7	16	6	316	61
1.8	20	5	372	84
1.9	Terminated at 1.8m			
2				
2.1				
2.2				
2.3				
2.4				
2.5				
2.6				
2.7				
2.8				
2.9				
3				
3.1				
3.2				
3.3				
3.4				
3.5				



APPENDIX B

DCP 'e' Values, California Bearing Ratio (CBR) & Allowable Bearing Capacity (ABC) vs Depth



Niawang Geotech

Project: Port Moresby Sewerage Upgrading

Location: New Pump Station - No. 7 Kila Rep Police

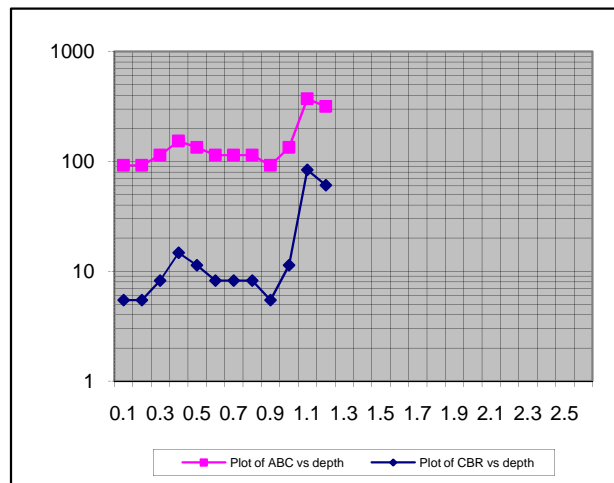
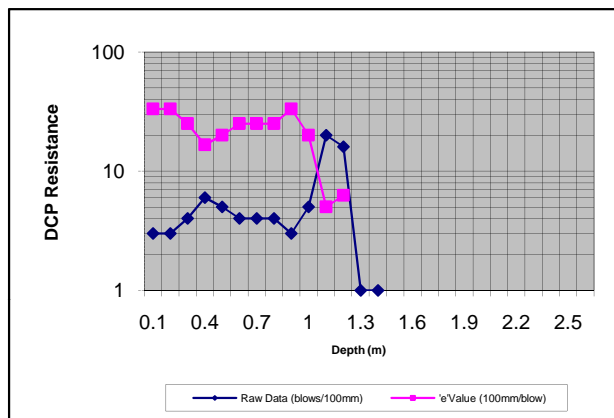
Client: njs - JICA

Job No: NGTS/01011

Date: March, 2011

DCP TEST # 7

Depth (m)	Field Data (Blw/100mm)	e' Value (100mm/Blw)	ABC (kPa)	CBR (%)
0.1	3	33	92	5
0.2	3	33	92	5
0.3	4	25	114	8
0.4	6	17	153	15
0.5	5	20	134	11
0.6	4	25	114	8
0.7	4	25	114	8
0.8	4	25	114	8
0.9	3	33	92	5
1	5	20	134	11
1.1	20	5	372	84
1.2	16	6	316	61
1.3	Refusal at			
1.4	1.2m depth			
1.5				
1.6				
1.7				
1.8				
1.9				
2				
2.1				
2.2				
2.3				
2.4				
2.5				
2.6				
2.7				
2.8				
2.9				
3				
3.1				
3.2				
3.3				
3.4				
3.5				



APPENDIX B

DCP 'e' Values, California Bearing Ratio (CBR) & Allowable Bearing Capacity (ABC) vs Depth



Niawang Geotech

Project: Port Moresby Sewerage Upgrading

Location: New Pump Station - No. 8 Konebada

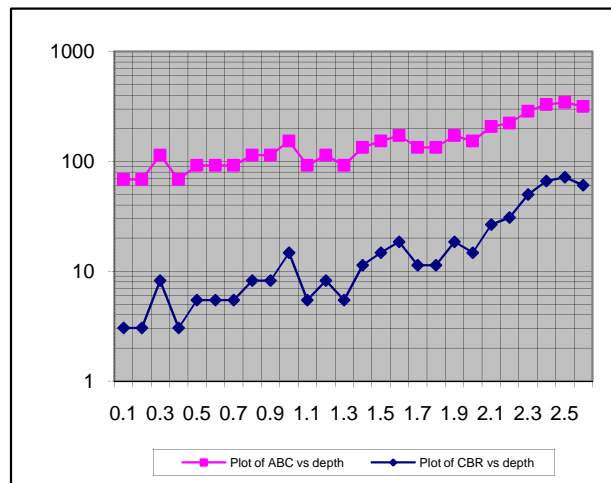
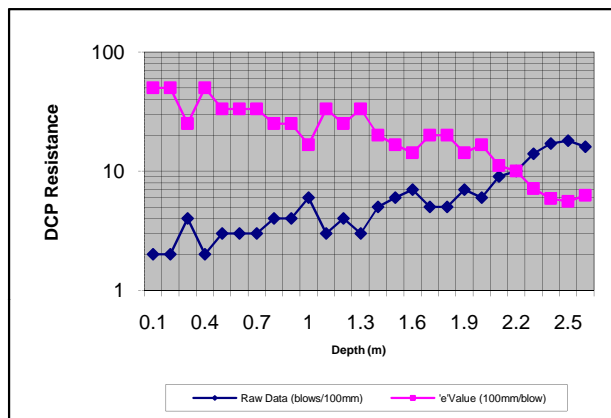
Client: njs - JICA

Job No: NGTS/01011

Date: March, 2011

DCP TEST # 8

Depth (m)	Field Data (Blw/100mm)	e' Value (100mm/Blw)	ABC (kPa)	CBR (%)
0.1	2	50	68	3
0.2	2	50	68	3
0.3	4	25	114	8
0.4	2	50	68	3
0.5	3	33	92	5
0.6	3	33	92	5
0.7	3	33	92	5
0.8	4	25	114	8
0.9	4	25	114	8
1	6	17	153	15
1.1	3	33	92	5
1.2	4	25	114	8
1.3	3	33	92	5
1.4	5	20	134	11
1.5	6	17	153	15
1.6	7	14	172	18
1.7	5	20	134	11
1.8	5	20	134	11
1.9	7	14	172	18
2	6	17	153	15
2.1	9	11	207	27
2.2	10	10	223	31
2.3	14	7	286	50
2.4	17	6	330	66
2.5	18	6	344	72
2.6	16	6	316	61
2.7	11	9	240	35
2.8	14	7	286	50
2.9	10	10	223	31
3	10	10	223	31
3.1	9	11	207	27
3.2	8	13	190	22
3.3	10	10	223	31
3.4	18	6	344	72
3.5	Refusal at 3.4m depth			



APPENDIX B

DCP 'e' Values, California Bearing Ratio (CBR) & Allowable Bearing Capacity (ABC) vs Depth



Niawang Geotech

Project: Port Moresby Sewerage Upgrading

Location: New Pump Station - No. 9 Gabutu

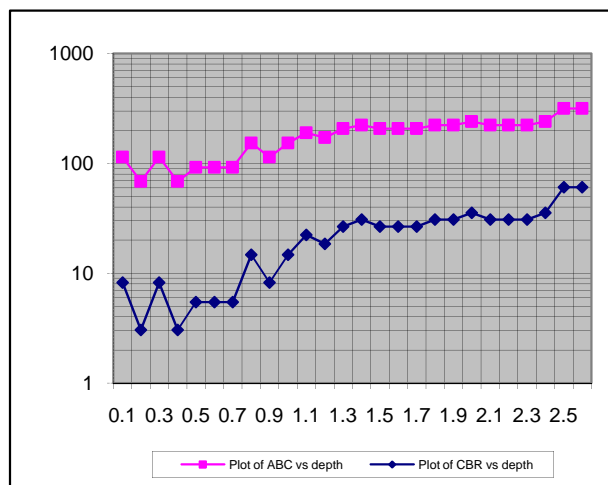
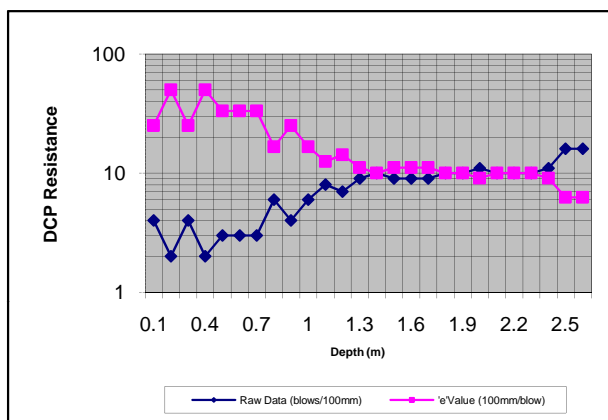
Client: njs - JICA

Job No: NGTS/01011

Date: March, 2011

DCP TEST # 9

Depth (m)	Field Data (Blw/100mm)	e' Value (100mm/Blw)	ABC (kPa)	CBR (%)
0.1	4	25	114	8
0.2	2	50	68	3
0.3	4	25	114	8
0.4	2	50	68	3
0.5	3	33	92	5
0.6	3	33	92	5
0.7	3	33	92	5
0.8	6	17	153	15
0.9	4	25	114	8
1	6	17	153	15
1.1	8	13	190	22
1.2	7	14	172	18
1.3	9	11	207	27
1.4	10	10	223	31
1.5	9	11	207	27
1.6	9	11	207	27
1.7	9	11	207	27
1.8	10	10	223	31
1.9	10	10	223	31
2	11	9	240	35
2.1	10	10	223	31
2.2	10	10	223	31
2.3	10	10	223	31
2.4	11	9	240	35
2.5	16	6	316	61
2.6	16	6	316	61
2.7	15	7	301	55
2.8	15	7	301	55
2.9	18	6	344	72
3	21	5	385	90
3.1	Terminate at			
3.2	3.00m			
3.3				
3.4				
3.5				



APPENDIX B

DCP 'e' Values, California Bearing Ratio (CBR) & Allowable Bearing Capacity (ABC) vs Depth



Niawang Geotech

Project: Port Moresby Sewerage Upgrading

Location: New Pump Station - No. 10 Kila Kila

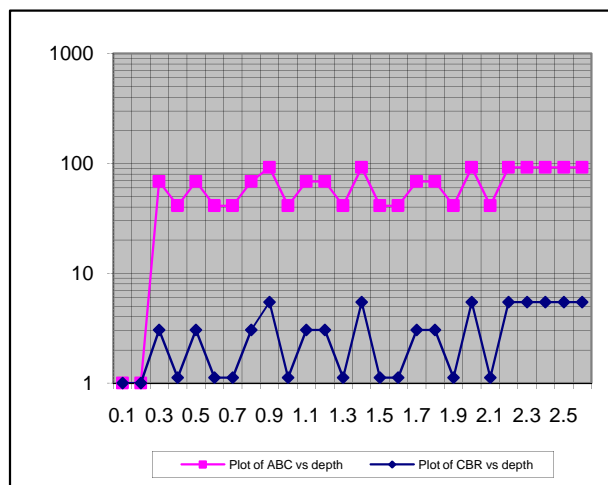
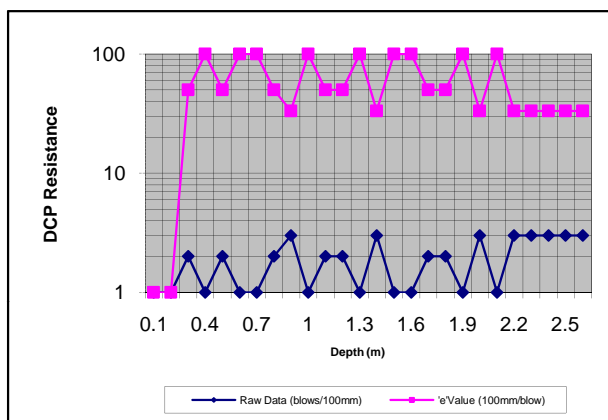
Client: njs - JICA

Job No: NGTS/01011

Date: March, 2011

DCP TEST # 10

Depth (m)	Field Data (Blw/100mm)	e' Value (100mm/Blw)	ABC (kPa)	CBR (%)
0.1] by] weight	#VALUE!	#####	#####
0.2		#VALUE!	#####	#####
0.3	2	50	68	3
0.4	1	100	41	1
0.5	2	50	68	3
0.6	1	100	41	1
0.7	1	100	41	1
0.8	2	50	68	3
0.9	3	33	92	5
1	1	100	41	1
1.1	2	50	68	3
1.2	2	50	68	3
1.3	1	100	41	1
1.4	3	33	92	5
1.5	1	100	41	1
1.6	1	100	41	1
1.7	2	50	68	3
1.8	2	50	68	3
1.9	1	100	41	1
2	3	33	92	5
2.1	1	100	41	1
2.2	3	33	92	5
2.3	3	33	92	5
2.4	3	33	92	5
2.5	3	33	92	5
2.6	3	33	92	5
2.7	3	33	92	5
2.8	3	33	92	5
2.9	4	25	114	8
3	3	33	92	5
3.1	4	25	114	8
3.2	5	20	134	11
3.3	7	14	172	18
3.4	7	14	172	18
3.5	8	13	190	22



APPENDIX B

DCP 'e' Values, California Bearing Ratio (CBR) & Allowable Bearing Capacity (ABC) vs Depth



Niwang Geotech

Project: Port Moresby Sewerage Upgrading

Location: New Pump Station - No. 10 Kila Kila

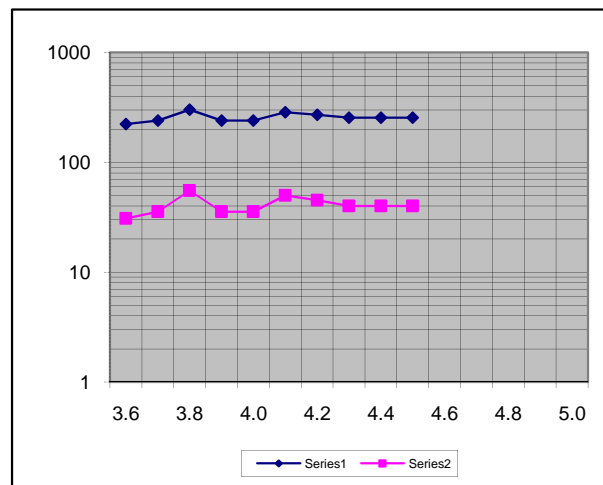
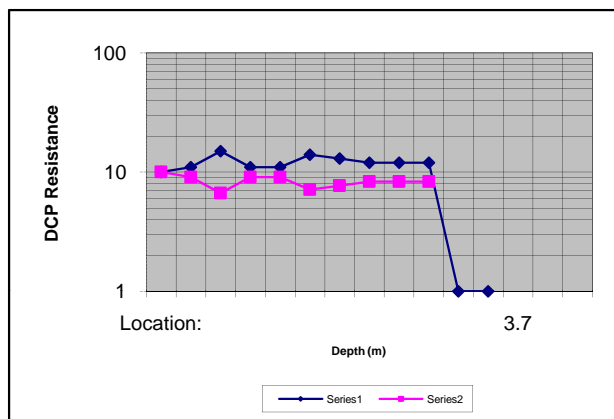
Client: njs - JICA

Job No.: NGTS/01011

Date: March, 2011

DCP 1 continuation

Depth (m)	Field Data (Blw/100mm)	e' Value (100mm/Blw)	ABC (kPa)	CBR (%)
3.6	10	10	223	31
3.7	11	9	240	35
3.8	15	7	301	55
3.9	11	9	240	35
4.0	11	9	240	35
4.1	14	7	286	50
4.2	13	8	271	45
4.3	12	8	255	40
4.4	12	8	255	40
4.5	12	8	255	40
4.6	Terminate at 4.5m depth			
4.7				
4.8				
4.9				
5.0				
1.6				
1.7				
1.8				
1.9				
2				
2.1				
2.2				
2.3				



APPENDIX B

DCP 'e' Values & Allowable Bearing Capacity (ABC) vs Depth



Niwang Geotech

Project: Port Moresby Sewerage Upgrading

Location: KilaKila STP

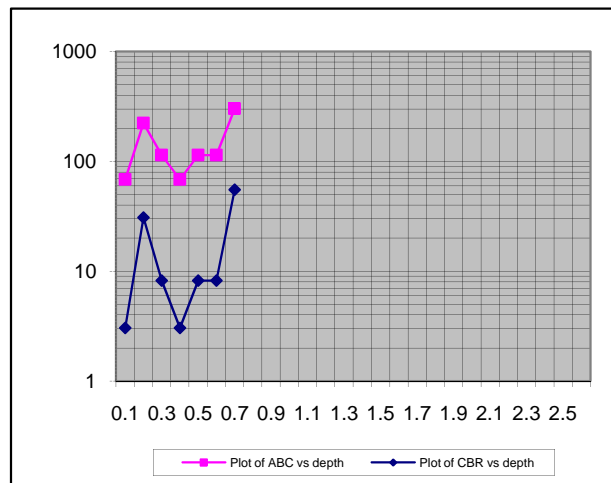
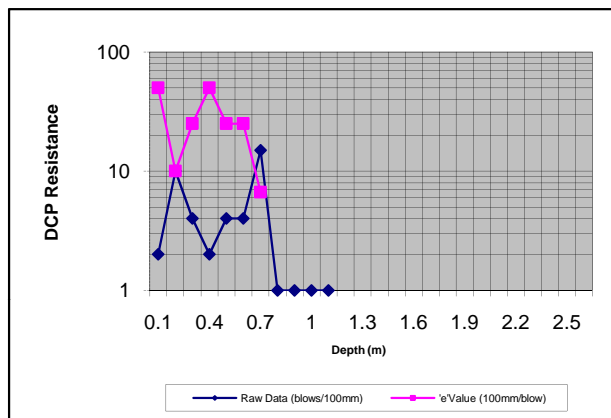
Client: njs - JICA

Job No: NGTS/01011

Date: March, 2011

DCP TEST # 1

Depth (m)	Field Data (Blw/100mm)	e' Value (100mm/Blw)	ABC (kPa)	CBR (%)
0.1	2	50	68	3
0.2	10	10	223	31
0.3	4	25	114	8
0.4	2	50	68	3
0.5	4	25	114	8
0.6	4	25	114	8
0.7	15	7	301	55
0.8	Refusal or			
0.9	Rebounce on			
1	bedrock at			
1.1	0.700mm depth			
1.2				
1.3				
1.4				
1.5				
1.6				
1.7				
1.8				
1.9				
2				
2.1				
2.2				
2.3				
2.4				
2.5				
2.6				
2.7				
2.8				
2.9				
3				
3.1				
3.2				
3.3				
3.4				
3.5				



APPENDIX B

DCP 'e' Values, California Bearing Ratio (CBR) & Allowable Bearing Capacity (ABC) vs Depth



Niawang Geotech

Project: Port Moresby Sewerage Upgrading

Location: KilaKila STP

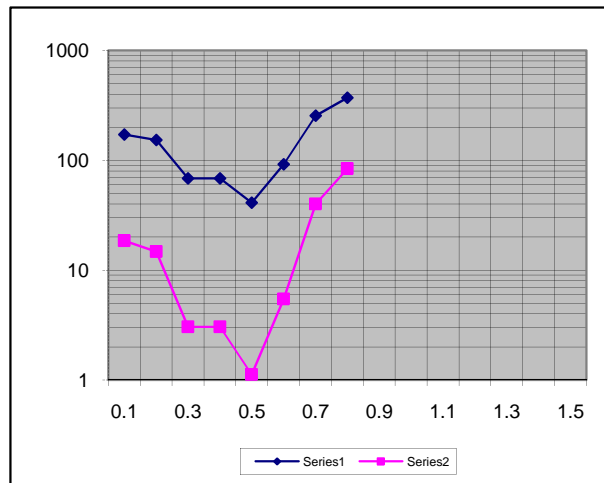
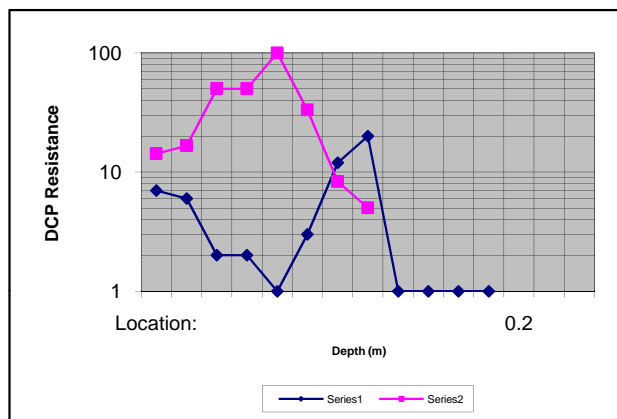
Client: njs - JICA

Job No.: NGTS/01011

Date: March, 2011

DCP 2

Depth (m)	Field Data (Blw/100mm)	e' Value (100mm/Blw)	ABC (kPa)	CBR (%)
0.1	7	14	172	18
0.2	6	17	153	15
0.3	2	50	68	3
0.4	2	50	68	3
0.5	1	100	41	1
0.6	3	33	92	5
0.7	12	8	255	40
0.8	20	5	372	84
0.9	Refusal or			
1	Rebounce on			
1.1	bedrock at			
1.2	0.800mm			
1.3				
1.4				
1.5				
1.6				
1.7				
1.8				
1.9				
2				
2.1				
2.2				
2.3				



APPENDIX B

DCP 'e' Values & Allowable Bearing Capacity (ABC) vs Depth



Niawang Geotech

Project: Port Moresby Sewerage Upgrading

Location: KilaKila STP

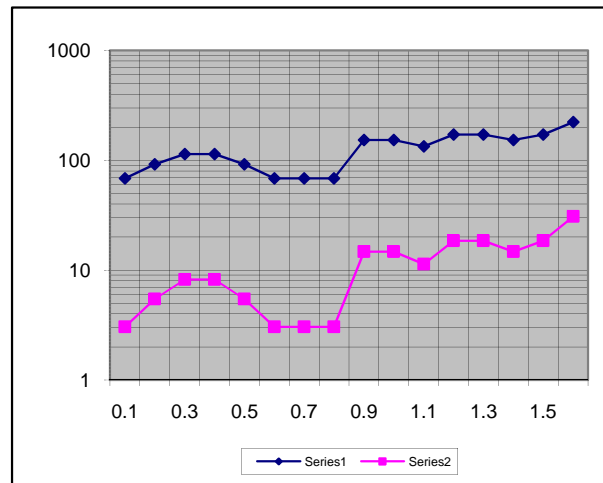
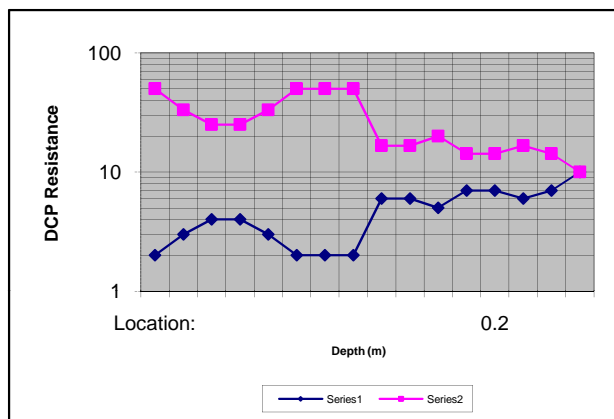
Client: njs - JICA

Job No.: NGTS/01011

Date: March, 2011

DCP 3

Depth (m)	Field Data (Blw/100mm)	e' Value (100mm/Blw)	ABC (kPa)	CBR (%)
0.1	2	50	68	3
0.2	3	33	92	5
0.3	4	25	114	8
0.4	4	25	114	8
0.5	3	33	92	5
0.6	2	50	68	3
0.7	2	50	68	3
0.8	2	50	68	3
0.9	6	17	153	15
1	6	17	153	15
1.1	5	20	134	11
1.2	7	14	172	18
1.3	7	14	172	18
1.4	6	17	153	15
1.5	7	14	172	18
1.6	10	10	223	31
1.7	20	5	372	84
1.8	Refusal or			
1.9	Rebounce on			
2.0	bedrock at			
2.1	1.7m depth			
2.2				
2.3				
2.4				
2.5				
2.6				
2.7				
2.8				
2.9				
3.0				



APPENDIX B

DCP 'e' Values & Allowable Bearing Capacity (ABC) vs Depth



Niwang Geotech

Project: Port Moresby Sewerage Upgrading

Location: KilaKila STP

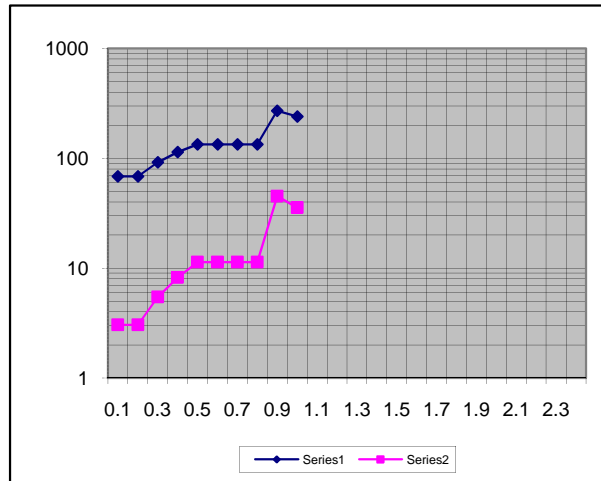
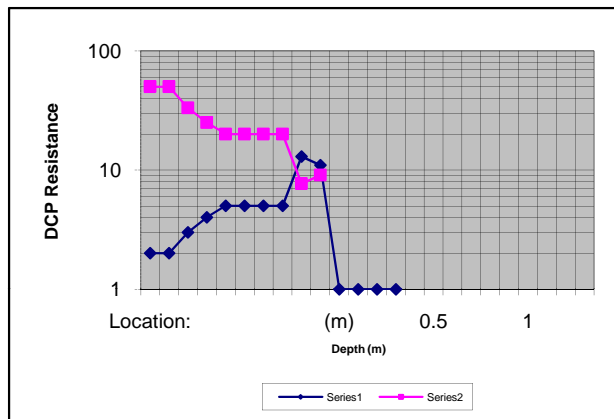
Client: njs - JICA

Job No.: NGTS/01011

Date: March, 2011

DCP 4

Depth (m)	Field Data (Blw/100mm)	e' Value (100mm/Blw)	ABC (kPa)	CBR (%)
0.1	2	50	68	3
0.2	2	50	68	3
0.3	3	33	92	5
0.4	4	25	114	8
0.5	5	20	134	11
0.6	5	20	134	11
0.7	5	20	134	11
0.8	5	20	134	11
0.9	13	8	271	45
1	11	9	240	35
1.1	Refusal or			
1.2	Rebounce on			
1.3	bedrock at			
1.4	1.0m depth			
1.5				
1.6				
1.7				
1.8				
1.9				
2				
2.1				
2.2				
2.3				
2.4				



APPENDIX B

DCP 'e' Values & Allowable Bearing Capacity (ABC) vs Depth



Niwang Geotech

Project: Port Moresby Sewerage Upgrading

Location: KilaKila STP

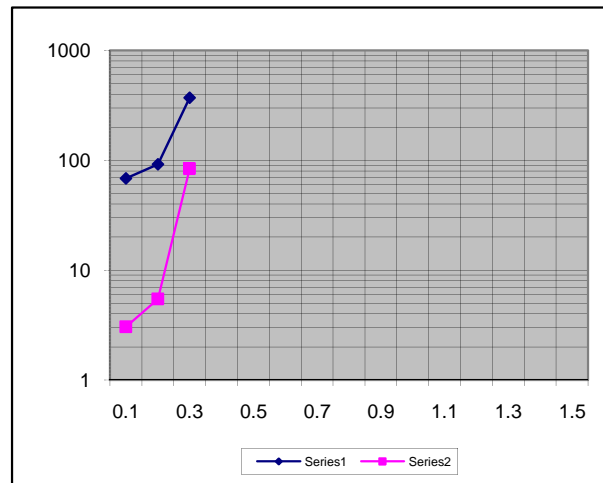
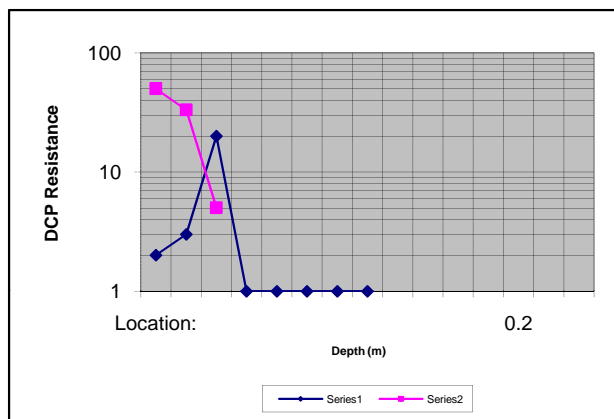
Client: njs - JICA

Job No.: NGTS/01011

Date: March, 2011

DCP 5

Depth (m)	Field Data (Blw/100mm)	e' Value (100mm/Blw)	ABC (kPa)	CBR (%)
0.1	2	50	68	3
0.2	3	33	92	5
0.3	20	5	372	84
0.4	Refusal or			
0.5	Rebounce on			
0.6	bedrock at			
0.7	0.3m depth			
0.8	Rocky area			
0.9				
1				
1.1				
1.2				
1.3				
1.4				
1.5				
1.6				
1.7				
1.8				
1.9				
2.0				
2.1				
2.2				
2.3				
2.4				
2.5				
2.6				
2.7				
2.8				
2.9				
3.0				
3.1				
3.2				
3.3				
3.4				
3.5				



APPENDIX B

DCP 'e' Values & Allowable Bearing Capacity (ABC) vs Depth



Niawang Geotech

Project: Port Moresby Sewerage System Upgrading

Location: Kila Kila STP

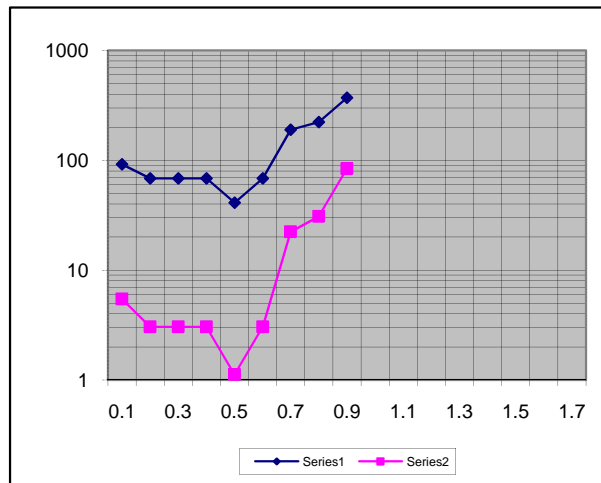
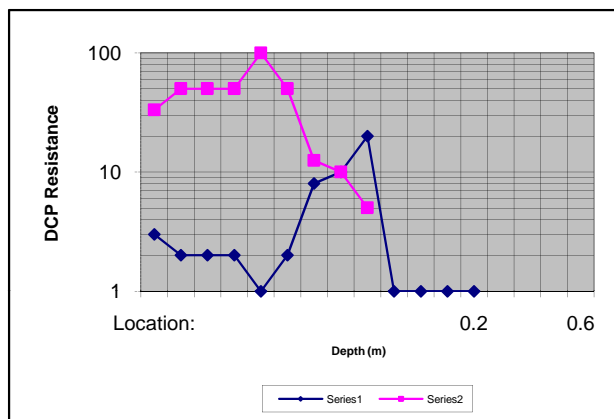
Client: njs - JICA

Job No.: NGTS/01011

Date: March, 2011

DCP 6

Depth (m)	Field Data (Blw/100mm)	e' Value (100mm/Blw)	ABC (kPa)	CBR (%)
0.1	3	33	92	5
0.2	2	50	68	3
0.3	2	50	68	3
0.4	2	50	68	3
0.5	1	100	41	1
0.6	2	50	68	3
0.7	8	13	190	22
0.8	10	10	223	31
0.9	20	5	372	84
1	Refusal or			
1.1	Rebounce on			
1.2	bedrock at			
1.3	0.900m			
1.4				
1.5				
1.6				
1.7				



APPENDIX B

DCP 'e' Values & Allowable Bearing Capacity (ABC) vs Depth



Niwang Geotech

Project: Port Moresby Sewerage Upgrading

Location: Morata STP

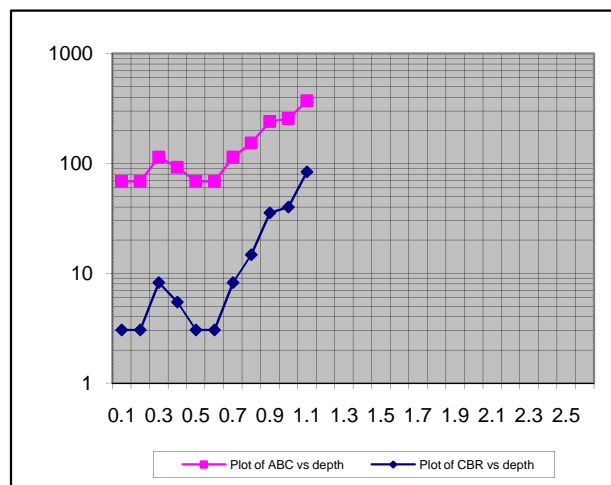
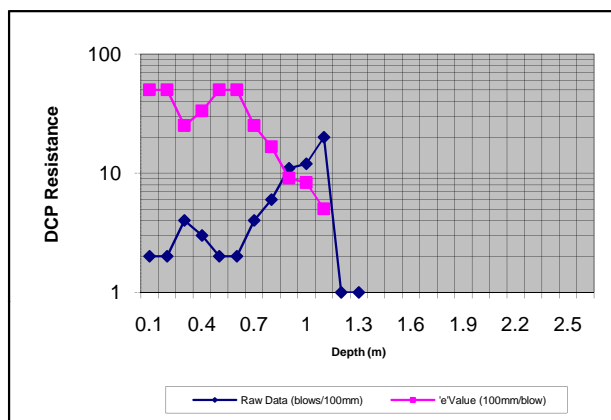
Client: njs - JICA

Job No: NGTS/01011

Date: March, 2011

DCP TEST # 1

Depth (m)	Field Data (Blw/100mm)	e' Value (100mm/Blw)	ABC (kPa)	CBR (%)
0.1	2	50	68	3
0.2	2	50	68	3
0.3	4	25	114	8
0.4	3	33	92	5
0.5	2	50	68	3
0.6	2	50	68	3
0.7	4	25	114	8
0.8	6	17	153	15
0.9	11	9	240	35
1	12	8	255	40
1.1	20	5	372	84
1.2	Refusal at			
1.3	1.1m depth			
1.4				
1.5				
1.6				
1.7				
1.8				
1.9				
2				
2.1				
2.2				
2.3				
2.4				
2.5				
2.6				
2.7				
2.8				
2.9				
3				
3.1				



APPENDIX B

DCP 'e' Values, California Bearing Ratio (CBR) & Allowable Bearing Capacity (ABC) vs Depth



Niawang Geotech

Project: Port Moresby Sewerage Upgrading

Location: Morata STP

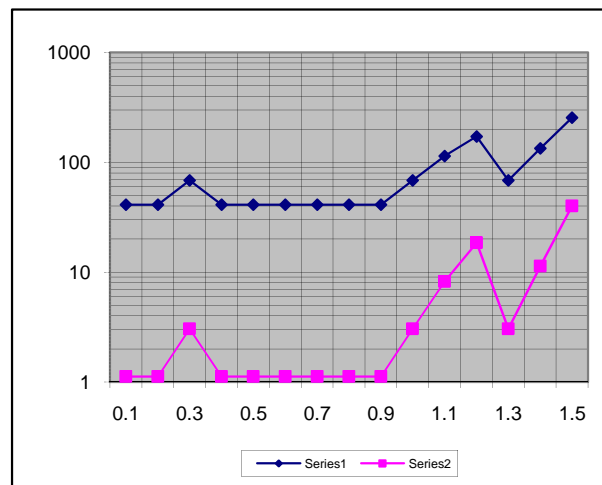
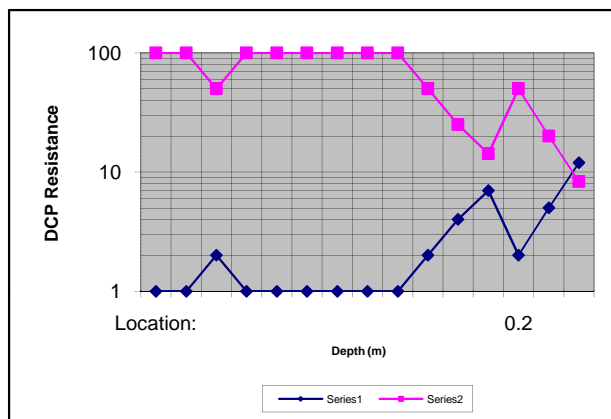
Client: njs - JICA

Job No.: NGTS/01011

Date: March, 2011

DCP 2

Depth (m)	Field Data (Blw/100mm)	e' Value (100mm/Blw)	ABC (kPa)	CBR (%)
0.1	1	100	41	1
0.2	1	100	41	1
0.3	2	50	68	3
0.4	1	100	41	1
0.5	1	100	41	1
0.6	1	100	41	1
0.7	1	100	41	1
0.8	1	100	41	1
0.9	1	100	41	1
1	2	50	68	3
1.1	4	25	114	8
1.2	7	14	172	18
1.3	2	50	68	3
1.4	5	20	134	11
1.5	12	8	255	40
1.6	13	8	271	45
1.7	11	9	240	35
1.8	15	7	301	55
1.9	18	6	344	72
2	23	4	412	103
2.1	Refusal at			
2.2	2.0m depth			
2.3				



APPENDIX B

DCP 'e' Values & Allowable Bearing Capacity (ABC) vs Depth



Niawang Geotech

Project: Port Moresby Sewerage Upgrading

Location: Morata STP

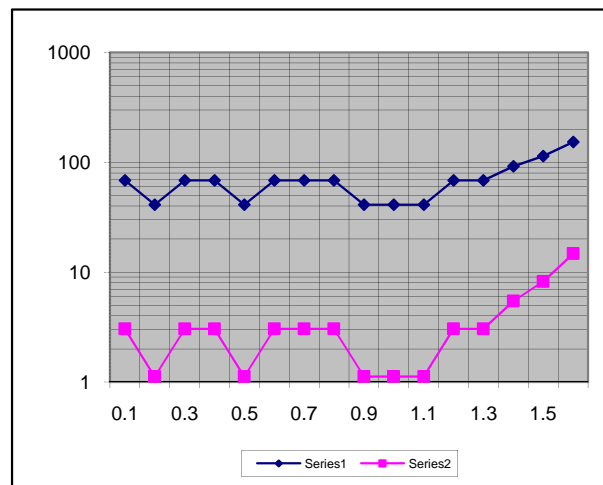
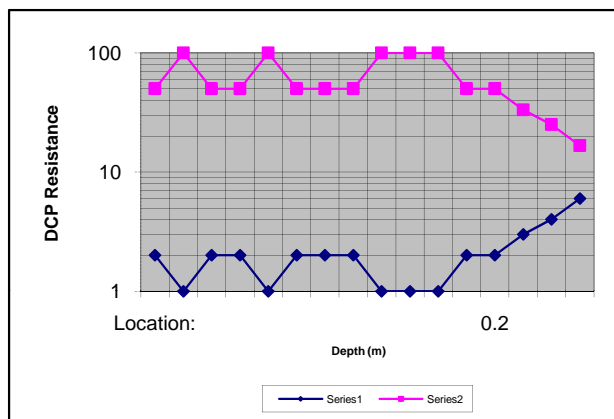
Client: njs - JICA

Job No.: NGTS/01011

Date: March, 2011

DCP 3

Depth (m)	Field Data (Blw/100mm)	e' Value (100mm/Blw)	ABC (kPa)	CBR (%)
0.1	2	50	68	3
0.2	1	100	41	1
0.3	2	50	68	3
0.4	2	50	68	3
0.5	1	100	41	1
0.6	2	50	68	3
0.7	2	50	68	3
0.8	2	50	68	3
0.9	1	100	41	1
1	1	100	41	1
1.1	1	100	41	1
1.2	2	50	68	3
1.3	2	50	68	3
1.4	3	33	92	5
1.5	4	25	114	8
1.6	6	17	153	15
1.7	9	11	207	27
1.8	9	11	207	27
1.9	14	7	286	50
2.0	14	7	286	50
2.1	16	6	316	61
2.2	19	5	358	78
2.3	20	5	372	84
2.4	24	4	425	109
2.5	Refusal at			
2.6	2.4m depth			
2.7				
2.8				
2.9				
3.0				



APPENDIX B

DCP 'e' Values & Allowable Bearing Capacity (ABC) vs Depth



Niawang Geotech

Project: Port Moresby Sewerage Upgrading

Location: Morata STP

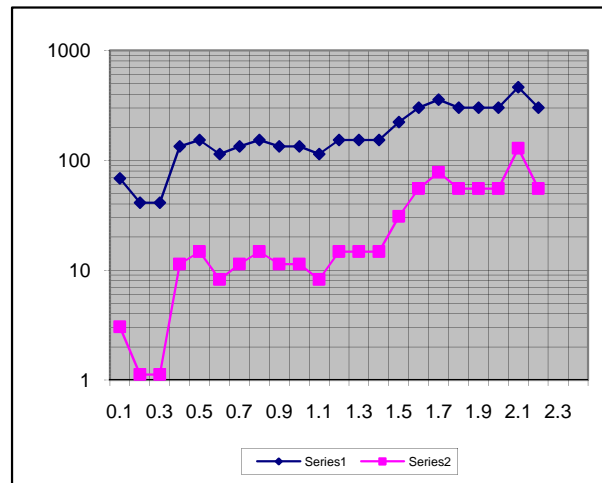
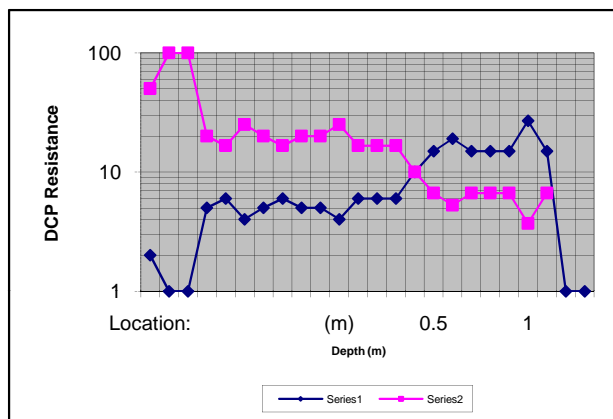
Client: njs - JICA

Job No.: NGTS/01011

Date: March, 2011

DCP 4

Depth (m)	Field Data (Blw/100mm)	e' Value (100mm/Blw)	ABC (kPa)	CBR (%)
0.1	2	50	68	3
0.2	1	100	41	1
0.3	1	100	41	1
0.4	5	20	134	11
0.5	6	17	153	15
0.6	4	25	114	8
0.7	5	20	134	11
0.8	6	17	153	15
0.9	5	20	134	11
1	5	20	134	11
1.1	4	25	114	8
1.2	6	17	153	15
1.3	6	17	153	15
1.4	6	17	153	15
1.5	10	10	223	31
1.6	15	7	301	55
1.7	19	5	358	78
1.8	15	7	301	55
1.9	15	7	301	55
2	15	7	301	55
2.1	27	4	464	129
2.2	15	7	301	55
2.3	Refusal at 2.2m depth			
2.4	Refusal at 2.2m depth			



APPENDIX B

DCP 'e' Values & Allowable Bearing Capacity (ABC) vs Depth



Niawang Geotech

Project: Port Moresby Sewerage Upgrading

Location: Morata STP

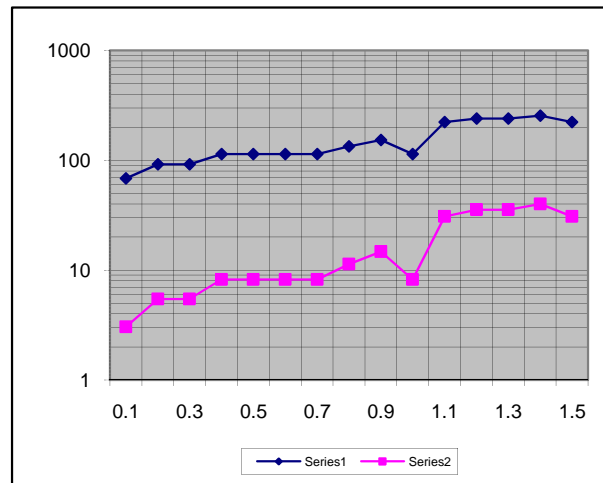
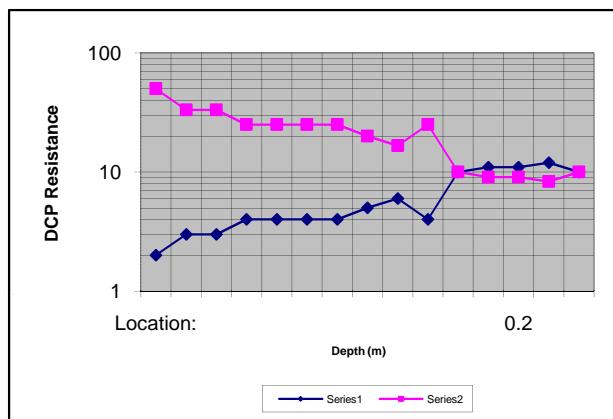
Client: njs - JICA

Job No.: NGTS/01011

Date: March, 2011

DCP 5

Depth (m)	Field Data (Blw/100mm) (100mm/Blw)	e' Value (100mm/Blw)	ABC (kPa)	CBR (%)
0.1	2	50	68	3
0.2	3	33	92	5
0.3	3	33	92	5
0.4	4	25	114	8
0.5	4	25	114	8
0.6	4	25	114	8
0.7	4	25	114	8
0.8	5	20	134	11
0.9	6	17	153	15
1	4	25	114	8
1.1	10	10	223	31
1.2	11	9	240	35
1.3	11	9	240	35
1.4	12	8	255	40
1.5	10	10	223	31
1.6	19	5	358	78
1.7	Refusal at			
1.8	1.6m depth			
1.9				
2.0				
2.1				
2.2				
2.3				
2.4				
2.5				
2.6				
2.7				
2.8				
2.9				
3.0				
3.1				
3.2				
3.3				
3.4				
3.5				




APPENDIX B

DCP 'e' Values & Allowable Bearing Capacity (ABC) vs Depth



DEPARTMENT OF WORKS
MATERIALS TESTING LABORATORY
TEST SUMMARY
(Particle Size Distribution)




Report To: njs - JICA		Sheet (1) of (1)			
Project: Port Moresby Sewerage System Upgrading – PORT MORESBY, NCD					
Other Details: Idubada New Pump Station # 2					
Sample No.	2011 - 1	2011 - 2			
Location:	TP # 02	TP # 02			
Chainage:					
Offset/					
Depth: (m)	0.0 – 0.800	0.800 – 2.00			
Classification:					
A. S Sieves Percent Passing (AS 1289.3.6.1-1995)					
75.0mm					
53.0mm					
37.5mm	100				
26.5mm	96	100			
19.0mm	88	96			
13.2mm	81	89			
9.50mm	79	86			
6.70mm	74	83			
4.75mm	70	80			
2.36mm	64	74			
1.18mm	58	69			
600um	54	63			
425um	53	61			
300um	52	59			
150um	50	51			
75um	49	47			
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DEPARTMENT OF WORKS
MATERIALS TESTING LABORATORY
TEST SUMMARY
(Particle Size Distribution)




Report To: njs - JICA		Sheet (1) of (1)			
Project: Port Moresby Sewerage System Upgrading – PORT MORESBY, NCD					
Other Details: Hanuabada New Pump Station # 4					
Sample No.	2011 - 1	2011 - 2	2011 - 3		
Location:	TP # 4	TP # 4	TP # (4)		
Chainage:					
Offset/					
Depth: (m)	0.300 – 0.800	1.3 – 2.4	2.4 – 3.00		
Classification:					
A. S Sieves Percent Passing (AS 1289.3.6.1-1995)					
75.0mm					
53.0mm					
37.5mm					
26.5mm	100	100	100		
19.0mm	94	93	94		
13.2mm	92	86	86		
9.50mm	89	84	81		
6.70mm	85	80	77		
4.75mm	80	77	71		
2.36mm	67	73	63		
1.18mm	52	69	56		
600um	40	66	49		
425um	34	65	46		
300um	30	64	42		
150um	23	60	31		
75um	20	57	29		
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DEPARTMENT OF WORKS
MATERIALS TESTING LABORATORY
TEST SUMMARY
(Particle Size Distribution)




Report To: njs - JICA		Sheet (1) of (1)			
Project: Port Moresby Sewerage System Upgrading – PORT MORESBY, NCD					
Other Details: Lawes Road Pump Station # 6					
Sample No.	2011 - 1	2011 - 2	2011 - 3		
Location:	TP # 6	TP # 6	TP # 6		
Chainage:					
Offset/					
Depth: (m)	0.0 – 0.700	0.700 – 1.4	1.4 – 3.5		
Classification:	Silty Sandy GRAVEL	Silty SAND	SAND		
A. S Sieves Percent Passing (AS 1289.3.6.1-1995)					
75.0mm					
53.0mm					
37.5mm					
26.5mm	100				
19.0mm	87				
13.2mm	75				
9.50mm	67				
6.70mm	60				
4.75mm	53	100	100		
2.36mm	42	99	99		
1.18mm	33	87	92		
600um	22	46	62		
425um	16	32	42		
300um	11	22	28		
150um	2	8	4		
75um	0	6	2		
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MATERIALS TESTING LABORATORY
TEST SUMMARY
(Particle Size Distribution)




Report To: njs - JICA		Sheet (1) of (1)			
Project: Port Moresby Sewerage System Upgrading – PORT MORESBY, NCD					
Other Details: Konebada New Pump Station # 8					
Sample No.	2011 - 1	2011 - 2			
Location:	TP # 8	TP # 8			
Chainage:					
Offset/					
Depth: (m)	0.0 – 0.600	0.600 – 2.2			
Classification:	Sandy Gravelly SILT	Silty SAND			
A. S Sieves Percent Passing (AS 1289.3.6.1-1995)					
75.0mm					
53.0mm					
37.5mm					
26.5mm	100				
19.0mm	97				
13.2mm	94				
9.50mm	90				
6.70mm	87				
4.75mm	83	100			
2.36mm	77	98			
1.18mm	73	96			
600um	69	83			
425um	66	58			
300um	63	42			
150um	58	20			
75um	56	16			
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DEPARTMENT OF WORKS
MATERIALS TESTING LABORATORY
TEST SUMMARY
(Particle Size Distribution)



Report To: njs - JICA		Sheet (1) of (2)			
Project: Port Moresby Sewerage System Upgrading – PORT MORESBY, NCD					
Other Details: Kila Kila STP					
Sample No.	2011 - 1	2011 - 2	2011 - 3	2011 - 4	2011 - 5
Location:	BH/TP # 1	BH/TP # 1	BH/TP # 2	BH/TP # 3	BH/TP # 3
Chainage:					
Offset/					
Depth: (m)	0.0 – 300	700 – 1.3	0.0 – 300	0.0 – 400	400 - 1.7
Classification:	Sandy Gravelly CLAY	Gravelly Sandy CLAY	Sandy GRAVEL	Sandy Gravelly CLAY	Sandy Gravelly CLAY
A. S Sieves Percent Passing (AS 1289.3.6.1-1995)					
75.0mm					
53.0mm			100		
37.5mm			51	100	100
26.5mm	100		38	80	97
19.0mm	95		31	80	91
13.2mm	93		27	79	88
9.50mm	89		22	77	84
6.70mm	85		18	73	80
4.75mm	82	100	16	71	76
2.36mm	76	97	12	67	69
1.18mm	71	95	9	64	63
600um	68	93	8	61	59
425um	67	92	7	60	58
300um	65	92	6	59	57
150um	63	90	3	56	53
75um	61	88	2	55	49
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TEST SUMMARY
(Particle Size Distribution)




Report To: njs - JICA		Sheet (2) of (2)			
Project: Port Moresby Sewerage System Upgrading – PORT MORESBY, NCD					
Other Details: Kila Kila STP					
Sample No.	2011 - 6	2011 - 7	2011 - 8	2011 - 9	
Location:	<i>BH/TP # 3</i>	BH/TP # 4	BH/TP # 6	BH/TP # 3	
Chainage:					
Offset/					
Depth: (m)	1.7 – 2.1	200 – 700	200 – 550	550 - 900	
Classification:	Sandy Gravelly CLAY	Sandy Gravelly CLAY	Sandy CLAY	Sandy Gravelly CLAY	
A. S Sieves Percent Passing (AS 1289.3.6.1-1995)					
75.0mm					
53.0mm					
37.5mm				100	
26.5mm		100		93	
19.0mm		96		90	
13.2mm		92	100	89	
9.50mm		86	99	87	
6.70mm		82	98	84	
4.75mm	100	79	97	82	
2.36mm	97	73	95	79	
1.18mm	95	69	92	77	
600um	95	66	91	76	
425um	94	65	90	75	
300um	93	64	89	75	
150um	90	61	85	73	
75um	90	59	82	70	
		<div style="display: flex; align-items: center;"> <p style="font-size: small;"> PNGLAS Accredited Laboratory Number: 10 This laboratory is accredited by the Papua New Guinea Laboratory Accreditation Scheme. The tests reported herein have been performed in accordance with its scope of accreditation and PNGLAS requirements which include the requirements of ISO/IEC 17025. This document shall not be reproduced, except in full. </p> </div>			
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TEST SUMMARY
(Particle Size Distribution)



Report To: njs - JICA		Sheet (2) of (2)			
Project: Port Moresby Sewerage System Upgrading – PORT MORESBY, NCD					
Other Details: Morata STP Site					
Sample No.	2011 - 1				
Location:	TP # 2				
Chainage:					
Offset/					
Depth: (m)	1.200 – 2.500				
Classification:	Sandy GRAVEL				
A. S Sieves Percent Passing (AS 1289.3.6.1-1995)					
75.0mm					
53.0mm					
37.5mm	100				
26.5mm	73				
19.0mm	72				
13.2mm	70				
9.50mm	70				
6.70mm	68				
4.75mm	65				
2.36mm	56				
1.18mm	47				
600um	40				
425um	37				
300um	35				
150um	30				
75um	26				
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TEL: (675) 324 1372 FAX: (675) 324 1374



Client: njs - JICA **Job No:** J11/P/LTS01
Project: Port Moresby Sewerage System Upgrading **Lab Test No:** _____
Location: Kanudi New Pump station # 1 - NCD **Sampled By:** NGTS
Test Methods: AS.1289.2.1.1,3.6.1,3.1,3.4.1 **Sampling Date:** April 2011

LABORATORY TEST RESULTS

Sample No.	Location Depth (m)	Material Description	Particle Size Distribution			Atterberg					Compaction		CBR		
			CI/Si %	Sa %	Gr %	Nat %	LL %	PL %	PI %	LS %	MDD t/m3	Opt w%	CBR %	DD t/m3	w %
11 - 01	DCP/TP 1 0.300 - 0.500	GP: Sandy Clayey GRAVEL, low plasticity, Grey.	33	29	38	25.8	47	20	27	5.5					
11 - 02	DCP/TP 1 0.500 - 0.700	GW: Sandy Silty GRAVEL, non - plasticity, Grey.	6	9	85	21.1		Non - Plasticity							
11 - 03	DCP/TP 1 0.700 - 1.500	GW: Sandy Silty GRAVEL, non - plasticity, Grey.	18	17	65	29.8		Non - Plasticity							

Remarks:

.....Date .../.../....

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KEY:

- | | |
|-----------------------|--------------------------------------|
| CI/Si = Clay / Silt | LS = Linear Shrinkages |
| Sa = Sand | PD = Soil Particle Density |
| LL = Liquid Limits | OPT w (%) = Optimum Moisture Content |
| PL = Plastic Limits | CBR = California Bearing Ratio |
| Gr = Gravel | DD = Dry Density |
| PI = Plasticity Index | w = Moisture |



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Client: njs - JICA **Job No:** J11/P/LTS01
Project: Port Moresby Sewerage System Upgrading **Lab Test No:** _____
Location: Idubada New Pump station # 2 - NCD **Sampled By:** NGTS
Test Methods: AS.1289.2.1.1,3.6.1,3.1,3.4.1 **Sampling Date:** April 2011

LABORATORY TEST RESULTS

Sample No.	Location Depth (m)	Material Description	Particle Size Distribution			Atterberg					Compaction		CBR		
			CI/Si %	Sa %	Gr %	Nat %	LL %	PL %	PI %	LS %	MDD t/m3	Opt w%	CBR %	DD t/m3	w %
11 - 01	DGP/TP 2 0.0 - 800	Sandy Gravelly CLAY, high plasticity, black.	49	15	36	23.5	48	23	25	13					
11 - 02	DGP/TP 2 0.800 - 2.00	Gravelly Sandy CLAY, high plasticity, Grey.	46	28	26	35.2	52	22	30	11					

Remarks:

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| PI = Plasticity Index | w = Moisture |



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Client: njs - JICA **Job No:** J11/P/LTS01
Project: Port Moresby Sewerage System Upgrading **Lab Test No:** _____
Location: Elevala New Pump station # 03 - NCD **Sampled By:** NGTS
Test Methods: AS.1289.2.1.1,3.6.1,3.1,3.4.1 **Sampling Date:** April 2011

LABORATORY TEST RESULTS

Sample No.	Location Depth (m)	Material Description	Particle Size Distribution			Atterberg					Compaction		CBR				
			CI/Si %	Sa %	Gr %	Nat %	LL %	PL %	PI %	LS %	MDD t/m3	Opt w%	CBR %	DD t/m3	w %		
11 - 01	DCP/TP 03 0.400 - 0.800	Silty Gravelly SAND, low plasticity, brown.	28	37	35	12.7											
11 - 02	DCP/TP 03 0.800 - 1.500	Sandy Silty GRAVEL gravel, non - plasticity, white.	31	21	48	23.5											
11 - 03	DCP/TP 03 1.500 - 2.100	CLAY of high plasticity, Black				28.5	75	31	44	13.5							

Remarks:

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KEY:

- | | |
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Client: njs - JICA **Job No:** J11/P/LTS01
Project: Port Moresby Sewerage System Upgrading **Lab Test No:**
Location: Hanubada New Pump station # 4 - NCD **Sampled By:** NGTS
Test Methods: AS.1289.2.1.1,3.6.1,3.1,3.4.1 **Sampling Date:** April 2011

LABORATORY TEST RESULTS

Sample No.	Location Depth (m)	Material Description	Particle Size Distribution			Atterberg					Compaction		CBR		
			Cl/Si %	Sa %	Gr %	Nat %	LL %	PL %	PI %	LS %	MDD t/m3	Opt w%	CBR %	DD t/m3	w %
11 - 01	DCP/TP 4 0.300 - 800	Silty Gravelly SAND, medium plasticity, brown.	20	47	33	15.5	37	18	19	8.5					
11 - 02	DCP/TP 4 1.3 - 2.4	Sandy Gravelly CLAY, high plasticity, brown	57	16	27	24.1	59	23	36	14					
11 - 03	DCP/TP 4 2.4 - 3.0	Silty Sandy GRAVEL, medium plasticity, black	29	34	37	26.1	45	20	25	6.5					

Remarks:

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KEY:

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 Sa = Sand
 LL = Liquid Limits
 PL = Plastic Limits
 Gr = Gravel
 PI = Plasticity Index

LS = Linear Shrinkages
 PD = Soil Particle Density
 OPT w (%) = Optimum Moisture Content
 CBR = California Bearing Ratio
 DD = Dry Density
 w = Moisture



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Client: njs - JICA **Job No:** J11/P/LTS01
Project: Port Moresby Sewerage System Upgrading **Lab Test No:**
Location: Davara Pump station # 05 - NCD **Sampled By:** NGTS
Test Methods: AS.1289.2.1.1,3.6.1,3.1,3.4.1 **Sampling Date:** April 2011

LABORATORY TEST RESULTS

Sample No.	Location Depth (m)	Material Description	Particle Size Distribution			Atterberg					Compaction		CBR				
			CI/Si %	Sa %	Gr %	Nat %	LL %	PL %	PI %	LS %	MDD t/m3	Opt w%	CBR %	DD t/m3	w %		
11 - 01	DCP/TP 05 0.500 - 0.800	Sandy Gravelly SILT, non plasticity, brown.	45	22	33	24.7											
							Non - Plastic										
11 - 02	DCP/TP 05 0.800 - 1.500	Silty SAND with trace of gravel, non - plasticity, white.	10	87	3	10.5											
							Non - Plastic										
11 - 03	DCP/TP 05 1.500 - 2.500	SAND with trace of silt and gravel, non - plasticity, white.	2	97	1	18.5											
							Non - Plastic										

Remarks:

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KEY:

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PL = Plastic Limits	CBR = California Bearing Ratio
Gr = Gravel	DD = Dry Density
PI = Plasticity Index	w = Moisture



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Page No. 1 of 1

Client: njs - JICA **Job No:** J11/P/LTS01
Project: Port Moresby Sewerage System Upgrading **Lab Test No:**
Location: Lawes Road Pump station # 6 - NCD **Sampled By:** NGTS
Test Methods: AS.1289.2.1.1,3.6.1,3.1,3.4.1 **Sampling Date:** April 2011

LABORATORY TEST RESULTS

Sample No.	Location Depth (m)	Material Description	Particle Size Distribution			Atterberg					Compaction		CBR				
			Cl/Si %	Sa %	Gr %	Nat %	LL %	PL %	PI %	LS %	MDD t/m3	Opt w%	CBR %	DD t/m3	w %		
11 - 01	DCP/TP 6 0.0 - 700	Silty Sandy GRAVEL, non plasticity, grey.	10	32	58	10.3											
							Non - Plastic										
11 - 02	DCP/TP 6 0.700 - 1.4	Silty SAND with trace of gravel, non - plasticity, white	16	82	2	7.5											
							Non - Plastic										
11 - 03	DCP/TP 6 1.4 - 3.5	SAND with trace of silt and gravel, non - plasticity, white	2	97	1	5.7											
							Non - Plastic										

Remarks:

.....Date .../.../....

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 PD = Soil Particle Density
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Client: njs - JICA **Job No:** J11/P/LTS01
Project: Port Moresby Sewerage System Upgrading **Lab Test No:** _____
Location: Kila Police Rep Pump station # 7- NCD **Sampled By:** NGTS
Test Methods: AS.1289.2.1.1,3.6.1,3.1,3.4.1 **Sampling Date:** April 2011

LABORATORY TEST RESULTS

Sample No.	Location Depth (m)	Material Description	Particle Size Distribution			Atterberg					Compaction		CBR		
			CI/Si %	Sa %	Gr %	Nat %	LL %	PL %	PI %	LS %	MDD t/m3	Opt w%	CBR %	DD t/m3	w %
11 - 01	DCP/TP 7 0.0 - 1.4	Gravelly Sandy CLAY, high plasticity, brown.	40	32	28	19.6	79	29	50	17.1					

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Page No. 1 of 1

Client: njs - JICA **Job No:** J11/P/LTS01
Project: Port Moresby Sewerage System Upgrading **Lab Test No:** _____
Location: Konebada New Pump station # 8 - NCD **Sampled By:** NGTS
Test Methods: AS.1289.2.1.1,3.6.1,3.1,3.4.1 **Sampling Date:** April 2011

LABORATORY TEST RESULTS

Sample No.	Location Depth (m)	Material Description	Particle Size Distribution			Atterberg					Compaction		CBR		
			Cl/Si %	Sa %	Gr %	Nat %	LL %	PL %	PI %	LS %	MDD t/m3	Opt w%	CBR %	DD t/m3	w %
11 - 01	DCP/TP 8 0.0 - 600	Sandy Gravelly SILT, high plasticity, brown.	56	21	23	24.1	54	23	31	12					
11 - 02	DCP/TP 8 0.600 - 2.200	Silty SAND with trace of gravel, non - plasticity, white.	16	82	2	9.7	Non - Plastic								

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Client: njs - JICA **Job No:** J11/P/LTS01
Project: Port Moresby Sewerage System Upgrading **Lab Test No:** _____
Location: Gabutu New Pump station # 9 - NCD **Sampled By:** NGTS
Test Methods: AS.1289.2.1.1,3.6.1,3.1,3.4.1 **Sampling Date:** April 2011

LABORATORY TEST RESULTS

Sample No.	Location Depth (m)	Material Description	Particle Size Distribution			Atterberg					Compaction		CBR		
			CI/Si %	Sa %	Gr %	Nat %	LL %	PL %	PI %	LS %	MDD t/m3	Opt w%	CBR %	DD t/m3	w %
11 - 01	DCP/TP 15 0.0 - 1.2	CI: Gravelly Sandy CLAY, medium plasticity, brown.	67	21	46	33.9	67	21	46	10					
11 - 02	DCP/TP 15 1.2 - 2.1	CL: Gravelly Sandy CLAY low - medium plasticity, grey.	57	32	11	32.2	33	19	14	10					

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Client: njs - JICA **Job No:** J11/P/LTS01
Project: Port Moresby Sewerage System Upgrading **Lab Test No:** _____
Location: KilaKila - NCD **Sampled By:** NGTS
Test Methods: AS.1289.2.1.1,3.6.1,3.1,3.4.1 **Sampling Date:** March 2011

LABORATORY TEST RESULTS

Sample No.	Location Depth (m)	Material Description	Particle Size Distribution			Atterberg					Compaction		CBR		
			CI/Si %	Sa %	Gr %	Nat %	LL %	PL %	PI %	LS %	MDD t/m3	Opt w%	CBR %	DD t/m3	w %
11 - 01	DCP/TP 1 0.0 - 300	Sandy Gravelly CLAY, high plasticity, brown.	61	15	24	28.8	70	33	37	13.5					
11 - 02	DCP/TP 1 0.700 - 1.3	Sandy CLAY with trace of gravel, high plasticity, brown.	88	9	3	34.1									
11 - 03	DCP/TP 2 0.0 - 0.300	Sandy GRAVEL with trace of clay, high plasticity, brown.	2	10	88	28.8	56	30	26	13.5					
11 - 04	DCP/TP 3 0.0 - 400	Sandy Gravelly CLAY, high plasticity, brown.	55	12	33	27.4	66	34	31	11					
11 - 05	DCP/TP 3 400 - 1.7	Sandy Gravelly CLAY, intermediate to high plasticity, brown.	49	20	31	24.9	49	24	25	8.5					
11 - 06	DCP/TP 3 1.7 - 2.1	Sandy CLAY with trace of gravel, high plasticity, white.	90	7	3	35.1									
11 - 07	DCP/TP 4 200 - 700	Sandy Gravelly CLAY, high plasticity, brown.	59	14	27	27.3	61	26	35	9.5					

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Client: njs - JICA **Job No:** J11/P/LTS01
Project: Port Moresby Sewerage System Upgrading **Lab Test No:** _____
Location: KilaKila - NCD **Sampled By:** NGTS
Test Methods: AS.1289.2.1.1,3.6.1,3.1,3.4.1 **Sampling Date:** March 2011

LABORATORY TEST RESULTS

Sample No.	Location Depth (m)	Material Description	Particle Size Distribution			Atterberg					Compaction		CBR		
			Cl/Si %	Sa %	Gr %	Nat %	LL %	PL %	PI %	LS %	MDD t/m3	Opt w%	CBR %	DD t/m3	w %
11 - 08	DCP/TP 6 200 - 550	Sandy CLAY with trace of gravel, high plasticity, white.	82	13	5	34.8									
11 - 09	DCP/TP 6 550 - 900	Sandy Gravelly CLAY, intermediate to high plasticity, brown.	70	9	21	23.6	49	21	28	15					

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Client: njs - JICA **Job No:** J11/P/LTS01
Project: Port Moresby Sewerage System Upgrading **Lab Test No:** _____
Location: Morata - NCD **Sampled By:** NGTS
Test Methods: AS.1289.2.1.1,3.6.1,3.1,3.4.1 **Sampling Date:** March 2011

LABORATORY TEST RESULTS

Sample No.	Location Depth (m)	Material Description	Particle Size Distribution			Atterberg					Compaction		CBR		
			CI/Si %	Sa %	Gr %	Nat %	LL %	PL %	PI %	LS %	MDD t/m3	Opt w%	CBR %	DD t/m3	w %
11 - 01	DCP/TP 3 0.0 - 200	Gravelly Sandy CLAY, high plasticity, black.	56	34	10	24.5	60	23	37	15					
11 - 02	DCP/TP 3 0.200 - 0.400	Gravelly Clayey SAND gravel, intermediate to high plas black.	41	51	8	23.5	49	21	28	15					
11 - 03	DCP/TP 3 0.400 - 0.800	Sandy CLAY with some gravel very high plasticity, grey.	79	14	7	34.3	72	24	48	16.5					
11 - 04	DCP/TP 3 0.800 - 1.200	Gravelly Sandy CLAY, high plasticity, grey.	50	38	12	38.9	55	24	31	15.5					
11 - 05	DCP/TP 3 1.200 - 2.200	Sandy CLAY with trace gravel high plasticity, grey.	81	18	1	34.4	69	23	46	12.5					
11 - 06	DCP/TP 3 2.200 - 3.5	Gravelly Sandy CLAY, high plasticity, grey.	52	31	17	22.0	44	17	27	12.5					
11 - 07	DCP/TP 2 1.200 - 2.5	Gravelly Sandy CLAY, high plasticity, grey.	26	30	44	10.7	38	19	19	8					

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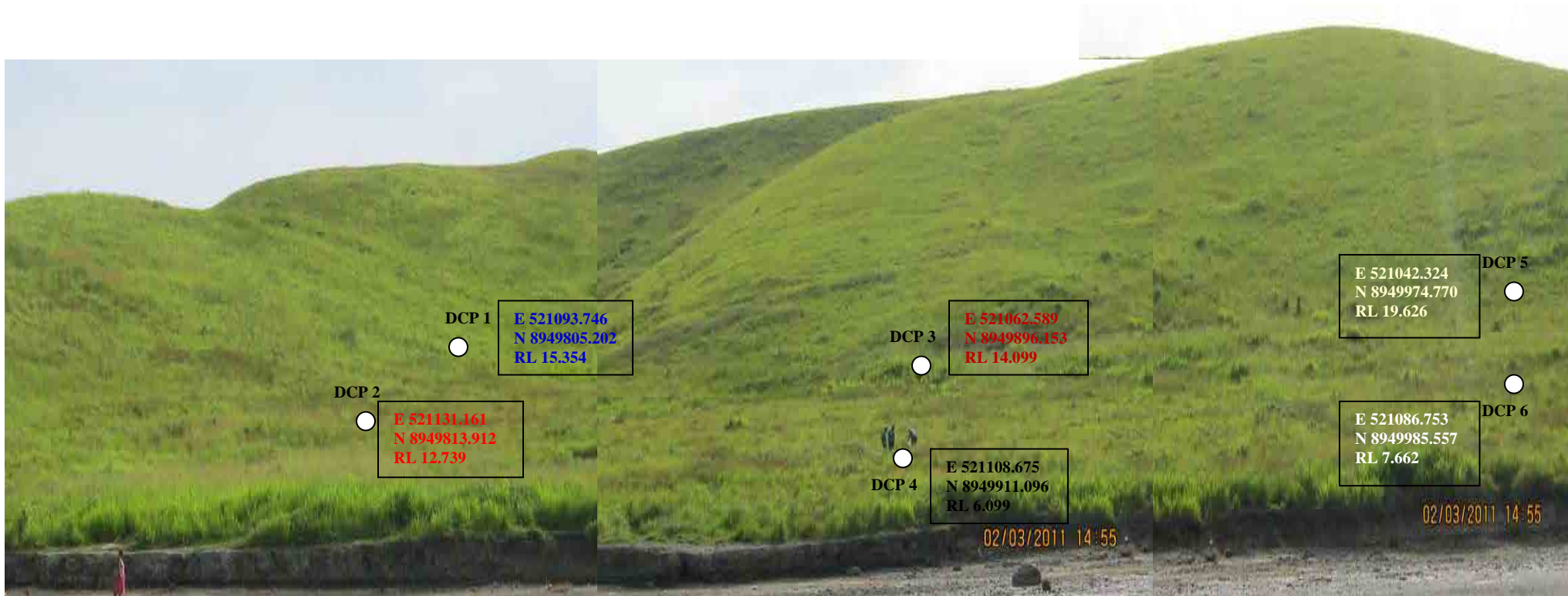
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Photograph 1 : General Landscape of the KilaKila STP Project Site and the Test Location.



Photograph 2 : General Landscape of the Morata STP Project Site and the Test Location

Site Photos of Pumping Station No: 1 - Kanudi



Existing Surface view



Bore Hole



Backhoe Excavation



Drilling Rig Setup before Land dispute

Site Photos of Pumping Station No: 2 - Idubada



Site Photos of Pumping station No: 3 - Eevala



Surface view of the PS Location



Inter Oil pipeline lying 3metres away from the proposed location



DCP Test done on Site



Excavation done using a backhoe

Site Photo of Pumping Station No: 4 - Hanuabada



Excavation at PS Number 4

Site Photos of Pump Station No : 5 - Davara



Existing pumping station



Excavation at the car park



Sewerage outlet to the sea



Excavation of car park fill

Site Photos of Pumping Station No: 6 – Lawes Road



Existing Surface View



Excavation



DCP Testing



Wash Boring

Site Photos of Pumping Station No: 7 – Kila Police Barracks



Existing Surface



Existing Pumping Station outlet



Site Photos of Pumping Station No. 8 - Konebada



DCP Testing



Site Photo of Pumping Station No. 9 - Gabutu



Excavation at PS No: 9

Site Photos of Pumping station No: 10 – Horse Camp



Access Road to the Pump Station



PS surrounded by residents



Stand Pipe for Piezometer Measurement



Wash Boring at 8.0 meters

Site Photos of Kila Kila STP



Calcurious mudstone at shoreline during low tide



Photos taken during Low Tide



Site Photos for Morata STP



Over View of the Morata STP Site



Bore Hole



Drilling Rig



Stand Pipe for Piezometer Measurement