

ANALYSIS STATUS REPORT

Job ..... C:\SGWIN\DATA\11006\OXDT\SSTRC  
 Units system ..... Kilonewtons, Metres

Nodes .....	49	( 2500)
Members .....	71	( 5000)
Restrained nodes .....	17	( 2500)
Nodes with spring restraints .....	0	( 2500)
Section properties .....	7	( 100)
Material properties .....	3	( 25)
Constrained nodes .....	0	( 2500)
Member offsets .....	0	( 5000)
Node loads .....	3	(10000)
Prescribed node displacements .....	0	(10000)
Member concentrated loads .....	1	(10000)
Member distributed forces .....	39	(10000)
Member distributed torsions .....	0	(10000)
Thermal/Prestress loads .....	0	(10000)
Self weight load cases .....	0	( 100)
Combination load cases .....	4	( 100)
Load cases with titles .....	8	( 100)
Lumped masses .....	0	(10000)
Spectral load cases .....	0	( 100)
Static analysis .....	Y	
Dynamic analysis .....	N	
Response analysis .....	N	
Buckling analysis .....	N	
Ill-conditioned .....	N	
Non-linear convergence .....	Y	
Frontwidth .....	85	
Total degrees of freedom .....	207	
Primary load cases .....	3	( 100)
Mass load cases .....	0	( 100)

SECTION PROPERTIES (m,m^2,m^4,deg)

Sect	Section Name	Mark	Angle	Type	Flipped	Source
1	700DP X 450WD	S1		Not applicable	No	Standard shape
2	400SQR COLUMN	S2		Not applicable	No	Standard shape
3	600DP X 400WD	S3		Not applicable	No	Standard shape
4	310 UB 40.4	S4		Not applicable	No	AUST300.LSS
5	200 PFC	S5		Not applicable	No	Standard shape
6	200 X 75 HWD					
7	600DP X 200WD	S7		Not applicable	No	Standard shape

Sect	Section	Area of	Torsion	Y-Axis	Z-Axis	Y-Axis	Z-Axis	Princ
		Constant	Mom of In	Mom of In	Mom of In	Shr Area	Shr Area	Angle
1	3.1500E-01	1.2774E-02	5.3156E-03	1.2862E-02	INFINITE	INFINITE	0.00	
2	1.6000E-01	3.6053E-03	2.1333E-03	2.1333E-03	INFINITE	INFINITE	0.00	
3	2.4000E-01	7.5125E-03	3.2000E-03	7.2000E-03	INFINITE	INFINITE	0.00	
4	5.2100E-03	1.5700E-07	7.6500E-06	8.6400E-05	INFINITE	INFINITE	0.00	
5	2.0000E-02	4.5800E-05	1.6700E-05	6.6700E-05	INFINITE	INFINITE	0.00	
6	1.5000E-02	2.1500E-05	7.0312E-06	5.0000E-05	INFINITE	INFINITE	0.00	
7	1.2000E-01	1.2643E-03	4.0000E-04	3.6000E-03	INFINITE	INFINITE	0.00	

Sect	Section	Shape	D	B/Bt	Bb/Hf	Tw	Tf
1	Rectangle		0.700	0.450			
2	Rectangle		0.400	0.400			
3	Rectangle		0.600	0.400			
5	Rectangle		0.200	0.100			
6	Rectangle		0.200	0.075			
7	Rectangle		0.600	0.200			

MATERIAL PROPERTIES (kPa,Kg/m^3)

Matl	Material Name	Young's Modulus	Poisson's Ratio	Mass Density	Coeff of Expansion	Concrete Strength
1	TIMBER	1.0500E+07	6.50	1.1000E+03	1.0000E-10	
2	CONCRETE-32	2.8600E+07	0.15	2.4500E+03	1.0000E-05	32000.00
3	TIMBER	1.0500E+07	6.50	1.1000E+03	1.0000E-10	

MEMBER CONCENTRATED LOADS (m,kN,kNm)

Load Case	Sub Axes	Load	X Force/	Y Force/	Z Force/
	Membr	Load Sys	Position	Moment	Moment
2	59	1 G	50.000%	0.000	-37.500
				0.000	0.000

MEMBER DISTRIBUTED FORCES (m,kN/m)

Load Case	Sub Axes	Start	Finish	X Start/	Y Start/	Z Start/
	Membr	Load Sys	Position	Finish	Finish	Finish
1	26	1 GI	0.000%	100.000%	0.000	-24.840
					0.000	-24.840
	26	2 GI	0.000%	100.000%	0.000	-27.840
					0.000	-27.840
	27	1 GI	0.000%	100.000%	0.000	-24.840
					0.000	-24.840
	27	2 GI	0.000%	100.000%	0.000	-27.840
					0.000	-27.840
	28	1 GI	0.000%	100.000%	0.000	-24.840
					0.000	-24.840
	28	2 GI	0.000%	100.000%	0.000	-27.840
					0.000	-27.840
	29	1 GI	0.000%	100.000%	0.000	-24.840
					0.000	-24.840
	29	2 GI	0.000%	100.000%	0.000	-27.840
					0.000	-27.840
	30	1 GI	0.000%	100.000%	0.000	-24.840
					0.000	-24.840
	30	2 GI	0.000%	100.000%	0.000	-27.840
					0.000	-27.840
	31	1 GI	0.000%	100.000%	0.000	-24.840
					0.000	-24.840
	31	2 GI	0.000%	100.000%	0.000	-27.840
					0.000	-27.840
	32	1 GI	0.000%	100.000%	0.000	-24.840
					0.000	-24.840

Load Case	Sub Axes	Start	Finish	X Start/	Y Start/	Z Start/
	Membr	Load Sys	Position	Finish	Finish	Finish
32	2 GI	0.000%	100.000%	0.000	-27.840	0.000
				0.000	-27.840	0.000
33	1 GI	0.000%	100.000%	0.000	-51.000	0.000
				0.000	-51.000	0.000
34	1 GI	0.000%	100.000%	0.000	-51.000	0.000
				0.000	-51.000	0.000
40	1 GI	0.000%	100.000%	0.000	-1.290	0.000
				0.000	-1.290	0.000
41	1 GI	0.000%	100.000%	0.000	-1.290	0.000
				0.000	-1.290	0.000
42	1 GI	0.000%	100.000%	0.000	-1.290	0.000
				0.000	-1.290	0.000
43	1 GI	0.000%	100.000%	0.000	-1.290	0.000
				0.000	-1.290	0.000
45	1 GI	0.000%	100.000%	0.000	-0.780	0.000
				0.000	-0.780	0.000
46	1 GI	0.000%	100.000%	0.000	-0.780	0.000
				0.000	-0.780	0.000
47	1 GI	0.000%	100.000%	0.000	-0.780	0.000
				0.000	-0.780	0.000
48	1 GI	0.000%	100.000%	0.000	-0.780	0.000
				0.000	-0.780	0.000
67	1 GI	0.000%	100.000%	0.000	-2.070	0.000
				0.000	-2.070	0.000
71	1 GI	0.000%	100.000%	0.000	-2.070	0.000
				0.000	-2.070	0.000

Sect	Section	Area of	Torsion	Y-Axis	Z-Axis	Y-Axis	Z-Axis	Princ
		Constant	Mom of In	Mom of In	Mom of In	Shr Area	Shr Area	Angle
26	1 GI	0.000%	100.000%	0.000	-0.540	0.000	-0.540	0.000
				0.000	-0.540	0.000	-0.540	0.000
27	1 GI	0.000%	100.000%	0.000	-0.540	0.000	-0.540	0.000
				0.000	-0.540	0.000	-0.540	0.000
28	1 GI	0.000%	100.000%	0.000	-0.540	0.000	-0.540	0.000
				0.000	-0.540	0.000	-0.540	0.000
29	1 GI	0.000%	100.000%	0.000	-0.540	0.000	-0.540	0.000
				0.000	-0.540	0.000	-0.540	0.000
30	1 GI	0.000%	100.000%	0.000	-0.540	0.000	-0.540	0.000
				0.000	-0.540	0.000	-0.540	0.000
31	1 GI	0.000%	100.000%	0.000	-0.540	0.000	-0.540	0.000
				0.000	-0.540	0.000	-0.540	0.000
32	1 GI	0.000%	100.000%	0.000	-0.540	0.000	-0.540	0.000
				0.000	-0.540	0.000	-0.540	0.000
67	1 GI	0.000%	100.000%	0.000	-0.863	0.000	-0.863	0.000
				0.000	-0.863	0.000	-0.863	0.000
71	1 GI	0.000%	100.000%	0.000	-0.863	0.000	-0.863	0.000
				0.000	-0.863	0.000	-0.863	0.000

COMBINATION LOAD CASES

Load case 5: 1.4DL + 1.7LL  
 1.400 \* Load case 1: DL  
 1.700 \* Load case 2: LL

Load case 6: DL + 1.3 LL + 1.3WL  
 1.000 \* Load case 1: DL  
 1.300 \* Load case 2: LL  
 1.300 \* Load case 3: WL

Load case 7: DL + LL + EQ  
 1.000 \* Load case 1: DL  
 1.300 \* Load case 2: LL  
 1.000 \* Load case 4: EQ

Load case 8: DL + LL  
 1.000 \* Load case 1: DL  
 1.000 \* Load case 2: LL

LOAD CASE TITLES

Load Case	Title
1	DL
2	LL
3	WL
4	EQ
5	1.4DL + 1.7LL
6	DL + 1.3 LL + 1.3WL
7	DL + LL + EQ
8	DL + LL

MEMBER FORCES AND MOMENTS (kN,kNm)

Load case 1 (Linear): DL

Membr	Node	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
1	3	0.000	0.005	0.000	-0.001	0.000	0.000
	34	0.000	0.005	0.000	-0.001	0.000	0.002
2	5	2.100	0.000	-0.378	-0.038	1.220	0.000
	4	2.100	0.000	-0.378	-0.038	-0.814	-0.001

Memb	Node	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment	Memb	Node	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
3	1	0.000	0.000	0.000	0.000	0.000	0.000	50	36	-3.802	7.187	-0.960	-2.003	4.178	-31.132
	3	0.000	0.000	0.000	0.000	0.000	0.000		41	-3.802	7.187	-0.960	-2.003	0.241	-1.667
4	3	-0.005	0.000	0.000	0.000	-0.001	0.000	51	37	-6.184	8.383	0.583	1.270	-2.498	-36.219
	5	-0.005	0.000	0.000	0.000	0.000	0.000		42	-6.184	8.383	0.583	1.270	-0.106	-1.850
5	11	-0.016	-0.257	0.001	0.003	0.000	0.000	52	35	2.767	-1.140	-1.290	-0.491	3.733	3.060
	35	-0.016	-0.257	0.001	0.003	0.000	-0.107		43	2.767	-1.140	-1.290	-0.491	-1.554	-1.616
6	5	0.000	0.000	0.000	0.000	0.000	0.000	53	39	9.584	-0.068	0.001	0.000	0.000	0.061
	6	0.000	0.000	0.000	0.000	0.000	0.000		22	9.584	-0.068	0.001	0.000	0.003	-0.156
7	12	3.727	0.001	-0.023	-0.372	-0.058	0.000	54	40	1.686	0.946	0.678	-0.228	-0.982	-1.292
	13	3.727	0.001	-0.023	-0.372	-0.182	0.006		38	1.686	0.946	0.678	-0.228	1.796	2.588
8	14	0.000	0.000	0.000	0.000	0.000	0.000	55	49	-2.696	0.152	-1.622	0.379	0.539	-0.337
	11	0.000	0.000	0.000	0.000	0.000	0.000		12	-2.696	0.152	-1.622	0.379	-3.678	0.057
9	11	0.257	0.000	-0.001	0.000	0.003	-0.000	57	21	-0.645	-9.562	-0.001	-0.003	0.000	-0.312
	53	0.257	0.000	-0.001	0.000	-0.001	-0.002		37	-0.645	-9.562	-0.001	-0.003	0.000	-4.297
10	12	0.000	0.000	0.000	0.000	0.000	0.000	58	43	0.155	1.300	0.027	0.000	-0.042	-2.117
	15	0.000	0.000	0.000	0.000	0.000	0.000		41	0.155	1.300	0.027	0.000	0.043	2.042
11	16	-0.439	-7.275	0.001	0.005	0.000	0.000	59	41	0.077	0.103	0.003	0.000	-0.008	-0.215
	36	-0.439	-7.275	0.001	0.005	0.000	-3.031		42	0.077	0.103	0.003	0.000	0.006	0.288
12	17	3.306	1.011	-0.230	-0.816	0.689	0.000	60	42	0.173	-0.830	-0.017	0.000	0.036	1.686
	18	3.306	1.011	-0.230	-0.816	-0.549	5.440		40	0.173	-0.830	-0.017	0.000	-0.035	-1.716
13	19	0.000	0.000	0.000	0.000	0.000	0.000	61	44	0.000	-6.120	0.851	0.000	-0.085	-0.068
	16	0.000	0.000	0.000	0.000	0.000	0.000		40	0.000	-6.120	0.851	0.000	0.085	-1.292
14	16	7.288	0.000	-0.001	0.000	0.005	0.000	62	45	0.105	0.040	0.206	-0.108	-0.288	-0.106
	17	7.288	0.000	-0.001	0.000	-0.005	0.002		5	0.105	0.040	0.206	-0.108	0.248	-0.002
15	17	0.000	0.000	0.000	0.000	0.000	0.000	63	37	11.085	-5.277	0.660	0.265	-1.495	27.397
	20	0.000	0.000	0.000	0.000	0.000	0.000		9	11.085	-5.277	0.660	0.265	2.056	-0.988
17	22	4.606	1.402	-0.090	0.435	0.198	0.000	67	49	0.047	6.003	0.000	0.000	0.000	0.000
	23	4.606	1.402	-0.090	0.435	-0.284	7.544		45	0.047	-6.003	0.000	0.000	0.000	0.000
18	24	0.000	0.000	0.000	0.000	0.000	0.000	68	49	0.000	-6.126	-1.445	0.000	0.145	-0.391
	21	0.000	0.000	0.000	0.000	0.000	0.000		43	0.000	-6.126	-1.445	0.000	-0.145	-1.616
19	21	9.584	-0.068	0.001	0.000	-0.003	0.312	70	50	-3.825	-8.383	-0.881	0.254	-0.115	0.009
	39	9.584	-0.068	0.001	0.000	0.000	0.061		41	-3.825	-8.383	-0.881	0.254	-0.291	-1.667
20	22	0.000	0.000	0.000	0.000	0.000	0.000	71	49	0.224	3.312	0.058	0.011	-0.091	0.000
	25	0.000	0.000	0.000	0.000	0.000	0.000		50	0.224	-3.312	0.058	0.011	0.094	0.000
21	26	-0.015	-0.250	-0.003	-0.009	0.001	0.000	72	51	-6.205	-9.315	0.486	-0.128	0.039	0.013
	38	-0.015	-0.250	-0.003	-0.009	0.000	-0.104		42	-6.205	-9.315	0.486	-0.128	0.137	-1.850
22	27	1.148	0.002	-0.014	1.031	-0.068	0.000	73	50	0.775	5.071	0.006	0.002	-0.017	0.000
	28	1.148	0.002	-0.014	1.031	-0.141	0.009		51	0.775	-5.071	0.006	0.002	0.013	0.000
23	29	0.000	0.000	0.000	0.000	0.000	0.000	74	51	0.385	4.244	-0.038	-0.011	0.079	0.000
	26	0.000	0.000	0.000	0.000	0.000	0.000		44	0.385	-4.244	-0.038	-0.011	-0.077	0.000
24	26	0.250	0.000	0.003	0.000	-0.009	0.000	75	51	-6.248	0.000	0.876	-0.128	-0.106	0.000
	27	0.250	0.000	0.003	0.000	0.009	0.000		22	-6.248	0.000	0.876	-0.128	2.172	0.000
25	27	0.000	0.000	0.000	0.000	0.000	0.000	76	50	-3.877	0.000	-1.432	0.254	0.226	0.000
	30	0.000	0.000	0.000	0.000	0.000	0.000		17	-3.877	0.000	-1.432	0.254	-3.497	0.000
26	2	0.000	63.621	0.000	0.000	0.000	-25.462	77	53	-0.045	-0.002	-0.001	0.000	0.000	0.003
	31	0.000	-73.347	0.000	0.000	0.000	-38.105		12	-0.045	-0.002	-0.001	0.000	-0.003	-0.002
27	7	0.000	84.288	0.000	0.000	0.000	-44.954	78	53	0.017	0.302	0.000	0.001	0.000	-0.005
	8	0.000	-84.288	0.000	0.000	0.000	-44.954		49	0.017	0.302	0.000	0.001	0.000	0.043
28	8	0.000	76.386	0.000	0.000	0.000	-36.920	Load case 2 (Linear): LL							
	32	0.000	-76.386	0.000	0.000	0.000	-36.920								
29	9	0.377	-15.300	2.831	0.324	-2.739	35.402	Memb	Node	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
	33	0.377	-94.320	2.831	0.324	1.508	-46.813								
30	31	0.000	81.078	0.000	0.000	0.000	-38.105	1	3	0.000	-0.004	0.000	-0.002	0.000	0.000
	7	0.000	-87.498	0.000	0.000	0.000	-48.378		34	0.000	-0.004	0.000	-0.002	0.000	-0.002
31	32	-0.283	101.145	-2.446	-0.664	2.417	-63.585	2	5	0.208	0.000	-0.304	-0.098	0.731	0.000
	9	-0.283	-4.215	-2.446	-0.664	-2.474	33.346		4	0.208	0.000	-0.304	-0.098	-0.901	-0.001
32	33	0.000	78.370	-0.870	0.324	1.508	-46.813	3	1	0.000	0.000	0.000	0.000	0.000	0.000
	10	0.000	-58.598	-0.870	0.324	-0.754	-21.108		3	0.000	0.000	0.000	0.000	0.000	0.000
33	2	0.000	175.950	0.000	0.000	0.000	-202.342	4	3	0.004	0.000	0.000	0.000	-0.002	0.000
	4	0.000	-175.950	0.000	0.000	0.000	-202.342		5	0.004	0.000	0.000	0.000	0.001	0.000
34	10	0.000	175.950	0.000	0.000	0.000	-202.342	5	11	-0.043	-0.705	0.002	0.007	0.000	0.000
	28	0.000	-175.950	0.000	0.000	0.000	-202.342		35	-0.043	-0.705	0.002	0.007	0.001	-0.294
35	34	3.393	0.008	-0.401	0.037	1.516	-0.029	6	5	0.000	0.000	0.000	0.000	0.000	0.000
	2	3.393	0.008	-0.401	0.037	-0.642	0.014		6	0.000	0.000	0.000	0.000	0.000	0.000
36	35	4.038	-0.167	0.356	-0.407	-1.203	0.605	7	12	-0.435	0.003	-0.345	-0.990	0.875	0.000
	7	4.038	-0.167	0.356	-0.407	0.714	-0.292		13	-0.435	0.003	-0.345	-0.990	-0.982	0.017
37	36	14.141	-5.972	-0.481	-0.590	1.795	25.016	8	14	0.000	0.000	0.000	0.000	0.000	0.000
	8	14.141	-5.972	-0.481	-0.590	-0.792	-7.108		11	0.000	0.000	0.000	0.000	0.000	0.000
39	38	2.889	-0.306	0.805	1.215	-2.841	1.107	9	11	0.706	-0.001	-0.002	0.000	0.007	0.000
	10	2.889	-0.306	0.805	1.215	1.490	-0.541		53	0.706	-0.001	-0.002	0.000	-0.002	-0.003
40	34	0.353	3.433	-0.089	0.039	0.154	-1.657	10	12	0.000	0.000	0.000	0.000	0.000	0.000
	35	0.353	-4.049	-0.089	0.039	-0.364	-3.441		15	0.000	0.000	0.000	0.000	0.000	0.000
41	35	1.287	1.146	2.768	3.811	-4.507	-1.747	11	16	-1.129	-18.703	0.003	0.011		

Memb	Node	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment	Memb	Node	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
22	27	-0.621	0.004	-0.318	2.476	0.838	0.000	73	50	1.846	2.113	0.012	0.003	-0.034	0.000
	28	-0.621	0.004	-0.318	2.476	-0.870	0.022		51	1.846	-2.113	0.012	0.003	0.024	0.000
23	29	0.000	0.000	0.000	0.000	0.000	0.000	74	51	0.919	1.768	-0.093	-0.026	0.195	0.000
	26	0.000	0.000	0.000	0.000	0.000	0.000		44	0.919	-1.768	-0.093	-0.026	-0.188	0.000
24	26	0.621	0.000	0.006	0.000	-0.023	0.000	75	51	-14.497	0.000	2.203	-2.266	-0.158	0.000
	27	0.621	0.000	0.006	0.000	0.022	0.001		22	-14.497	0.000	2.203	-2.266	5.569	0.000
25	27	0.000	0.000	0.000	0.000	0.000	0.000	76	50	-9.900	0.000	-3.585	2.449	0.700	0.000
	30	0.000	0.000	0.000	0.000	0.000	0.000		17	-9.900	0.000	-3.585	2.449	-8.621	0.000
26	2	0.000	0.652	0.000	0.000	0.000	-0.261	77	53	-0.077	-0.005	-0.003	0.000	0.000	0.009
	31	0.000	-0.752	0.000	0.000	0.000	-0.391		12	-0.077	-0.005	-0.003	0.000	-0.007	-0.004
27	7	0.000	0.864	0.000	0.000	0.000	-0.461	78	53	0.043	0.783	0.000	0.002	0.000	-0.012
	8	0.000	-0.864	0.000	0.000	0.000	-0.461		49	0.043	0.783	0.000	0.002	0.000	0.110
28	8	0.000	0.783	0.000	0.000	0.000	-0.378	Load case 3 (Linear): WL							
	32	0.000	-0.783	-0.000	0.000	0.000	-0.378	Memb	Node	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
29	9	1.199	-12.731	6.781	0.761	-6.558	12.867	1	3	0.000	-0.001	0.000	0.001	0.000	0.000
	33	1.199	-13.541	6.781	0.761	3.614	-6.837		34	0.000	-0.001	0.000	0.001	0.000	0.000
30	31	0.000	0.831	0.000	0.000	0.000	-0.391	2	5	-0.093	0.000	0.147	0.049	-0.350	0.000
	7	0.000	-0.897	0.000	0.000	0.000	-0.496		4	-0.093	0.000	0.147	0.049	0.441	0.001
31	32	-0.899	9.736	-5.876	-1.560	5.805	-9.590	3	1	0.000	0.000	0.000	0.000	0.000	0.000
	9	-0.899	8.656	-5.876	-1.560	-5.946	8.802		3	0.000	0.000	0.000	0.000	0.000	0.000
32	33	0.000	4.471	-2.085	0.761	3.614	-6.837	4	3	0.001	0.000	0.000	0.000	0.001	0.000
	10	0.000	3.067	-2.085	0.761	-1.807	2.962		5	0.001	0.000	0.000	0.000	-0.001	0.000
33	2	0.000	0.000	0.000	0.000	0.000	0.000	5	11	0.008	0.128	-0.001	-0.004	0.000	0.000
	4	0.000	0.000	0.000	0.000	0.000	0.000		35	0.008	0.128	-0.001	-0.004	0.000	0.053
34	10	0.000	0.000	0.000	0.000	0.000	0.000	6	5	0.000	0.000	0.000	0.000	0.000	0.000
	28	0.000	0.000	0.000	0.000	0.000	0.000		6	0.000	0.000	0.000	0.000	0.000	0.000
35	34	0.056	-0.011	0.262	0.050	-0.525	0.039	7	12	-0.037	-0.006	0.187	0.504	-0.491	0.000
	2	0.056	-0.011	0.262	0.050	0.892	-0.022		13	-0.037	-0.006	0.187	0.504	0.516	-0.031
36	35	-8.437	-0.507	0.762	-1.134	-2.315	1.836	8	14	0.000	0.000	0.000	0.000	0.000	0.000
	7	-8.437	-0.507	0.762	-1.134	1.782	-0.891		11	0.000	0.000	0.000	0.000	0.000	0.000
37	36	24.422	-15.450	-1.339	-1.523	5.211	64.579	9	11	-0.129	0.000	0.001	0.000	-0.004	0.000
	8	24.422	-15.450	-1.339	-1.523	-1.991	-18.527		53	-0.129	0.000	0.001	0.000	0.001	0.002
39	38	-4.547	-0.778	-0.014	2.920	0.419	2.810	10	12	0.000	0.000	0.000	0.000	0.000	0.000
	10	-4.547	-0.778	-0.014	2.920	0.343	-1.373		15	0.000	0.000	0.000	0.000	0.000	0.000
40	34	-0.454	0.079	-0.262	0.111	0.480	0.467	11	16	0.123	2.043	-0.002	-0.007	0.000	0.000
	35	-0.454	0.079	-0.262	0.111	-1.041	0.926		36	0.123	2.043	-0.002	-0.007	0.000	0.851
41	35	1.754	-5.688	6.960	9.769	-11.303	3.778	12	17	-0.048	-1.351	0.178	1.167	-0.465	0.000
	36	1.754	-5.688	6.960	9.769	10.969	-14.423		18	-0.048	-1.351	0.178	1.167	0.495	-7.267
42	36	5.379	0.319	0.509	1.339	-0.654	5.058	13	19	0.000	0.000	0.000	0.000	0.000	0.000
	37	5.379	0.319	0.509	1.339	1.838	6.620		16	0.000	0.000	0.000	0.000	0.000	0.000
43	37	1.718	2.345	-4.019	-9.142	9.019	-9.065	14	16	-2.047	0.000	0.002	0.000	-0.007	0.000
	38	1.718	2.345	-4.019	-9.142	-7.459	0.549		17	-2.047	0.000	0.002	0.000	0.007	-0.001
44	34	-0.255	-0.023	0.192	0.058	-0.428	0.070	15	17	0.000	0.000	0.000	0.000	0.000	0.000
	45	-0.255	-0.023	0.192	0.058	0.396	-0.027		20	0.000	0.000	0.000	0.000	0.000	0.000
45	5	0.846	0.220	-0.264	-0.003	0.556	-0.686	17	22	-0.224	-2.148	0.155	-0.605	-0.401	0.000
	12	0.846	0.220	-0.264	-0.003	-0.978	0.587		23	-0.224	-2.148	0.155	-0.605	0.432	-11.553
46	12	-2.706	-0.217	6.730	-0.114	-11.221	-0.119	18	24	0.000	0.000	0.000	0.000	0.000	0.000
	17	-2.706	-0.217	6.730	-0.114	10.316	-0.812		21	0.000	0.000	0.000	0.000	0.000	0.000
47	17	-5.778	0.164	0.505	-0.120	-0.364	0.137	19	21	-2.381	0.018	-0.001	0.000	0.004	-0.082
	22	-5.778	0.164	0.505	-0.120	2.113	0.940		39	-2.381	0.018	-0.001	0.000	-0.001	-0.017
48	22	-3.420	0.582	-4.048	0.228	8.767	-1.557	20	22	0.000	0.000	0.000	0.000	0.000	0.000
	27	-3.420	0.582	-4.048	0.228	-7.830	0.828		25	0.000	0.000	0.000	0.000	0.000	0.000
49	27	-4.664	-0.077	3.096	0.008	-5.332	0.227	21	26	0.005	0.091	0.004	0.014	-0.001	0.000
	44	-4.664	-0.077	3.096	0.008	2.718	0.028		38	0.005	0.091	0.004	0.014	0.001	0.038
50	36	-9.704	19.544	-2.282	-24.691	10.089	-80.802	22	27	0.136	-0.002	0.129	-1.585	-0.320	0.000
	41	-9.704	19.544	-2.282	-24.691	0.732	-0.673		28	0.136	-0.002	0.129	-1.585	0.376	-0.013
51	37	-14.344	20.874	1.560	22.906	-6.562	-86.330	23	29	0.000	0.000	0.000	0.000	0.000	0.000
	42	-14.344	20.874	1.560	22.906	-0.164	-0.747		26	0.000	0.000	0.000	0.000	0.000	0.000
52	35	7.024	-2.627	-2.968	-0.536	9.121	7.529	24	26	-0.091	0.000	-0.004	0.000	0.014	0.000
	43	7.024	-2.627	-2.968	-0.536	-3.046	-3.241		27	-0.091	0.000	-0.004	0.000	-0.014	0.000
53	39	22.523	-0.155	0.002	0.000	0.001	0.149	25	27	0.000	0.000	0.000	0.000	0.000	0.000
	22	22.523	-0.155	0.002	0.000	0.008	-0.348		30	0.000	0.000	0.000	0.000	0.000	0.000
54	40	4.176	2.165	1.726	0.129	-2.561	-2.802	26	2	0.000	0.000	0.000	0.000	0.000	0.000
	38	4.176	2.165	1.726	0.129	4.516	6.074		31	0.000	0.000	0.000	0.000	0.000	0.000
55	49	-6.914	0.000	-3.895	0.170	0.881	0.116	27	7	0.000	0.000	0.000	0.000	0.000	0.000
	12	-6.914	0.000	-3.895	0.170	-9.246	0.115		8	0.000	0.000	0.000	0.000	0.000	0.000
57	21	-1.512	-22.473	-0.002	-0.007	0.001	-0.724	28	8	0.000	0.000	0.000	0.000	0.000	0.000
	37	-1.512	-22.473	-0.002	-0.007	0.000	-10.088		32	0.000	0.000	0.000	0.000	0.000	0.000
58	43	0.310	2.857	0.067	0.000	-0.106	-4.918	29	9	-0.111	0.015	0.368	0.694	-0.315	-0.074
	41	0.310	2.857	0.067	0.000	0.109	4.223		33	-0.111	0.015	0.368	0.694	0.237	-0.052
59	41	0.167	18.907	0.005	0.000	-0.016	-22.916	30	31	0.000	0.000	0.000	0.000	0.000	0.000
	42	0.167	-18.593	0.005	0.000	0.011	-22.145		7	0.000	0.000	0.000	0.000	0.000	0.000
60	42	0.451	-1.600	-0.043	0.000	0.									

Memb	Node	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment	Memb	Node	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
41	35	-1.103	0.267	-3.589	-1.190	5.867	-0.323	12	17	7.193	5.798	-1.200	-4.620	3.513	0.000
	36	-1.103	0.267	-3.589	-1.190	-5.617	0.531		18	7.193	5.798	-1.200	-4.620	-2.942	31.188
42	36	-2.270	0.097	-0.606	-0.106	1.150	-0.331	13	19	0.000	0.000	0.000	0.000	0.000	0.000
	37	-2.270	0.097	-0.606	-0.106	-1.821	0.144		16	0.000	0.000	0.000	0.000	0.000	0.000
43	37	-1.214	0.104	2.537	1.046	-5.715	-0.101	14	16	42.056	0.002	-0.008	0.001	0.025	0.000
	38	-1.214	0.104	2.537	1.046	4.687	0.325		17	42.056	0.002	-0.008	0.001	-0.028	0.014
44	34	0.130	0.002	-0.093	-0.017	0.210	-0.007	15	17	0.000	0.000	0.000	0.000	0.000	0.000
	45	0.130	0.002	-0.093	-0.017	-0.191	0.003		20	0.000	0.000	0.000	0.000	0.000	0.000
45	5	-0.416	-0.094	0.132	0.000	-0.275	0.325	17	22	9.730	7.781	-0.386	2.440	0.671	0.000
	12	-0.416	-0.094	0.132	0.000	0.490	-0.222		23	9.730	7.781	-0.386	2.440	-1.404	41.854
46	12	1.459	-0.147	-3.449	0.013	5.802	0.229	18	24	0.000	0.000	0.000	0.000	0.000	0.000
	17	1.459	-0.147	-3.449	0.013	-5.233	-0.241		21	0.000	0.000	0.000	0.000	0.000	0.000
47	17	3.377	-0.071	-0.598	0.014	0.968	0.173	19	21	51.707	-0.358	0.005	0.000	-0.016	1.667
	22	3.377	-0.071	-0.598	0.014	-1.961	-0.176		39	51.707	-0.358	0.005	0.000	0.002	0.339
48	22	2.046	-0.135	2.561	-0.026	-5.578	0.248	20	22	0.000	0.000	0.000	0.000	0.000	0.000
	27	2.046	-0.135	2.561	-0.026	4.921	-0.304		25	0.000	0.000	0.000	0.000	0.000	0.000
49	27	2.649	0.007	-1.912	-0.015	3.323	-0.025	21	26	-0.085	-1.404	-0.015	-0.051	0.003	0.000
	44	2.649	0.007	-1.912	-0.015	-1.649	-0.007		38	-0.085	-1.404	-0.015	-0.051	-0.003	-0.585
50	36	7.246	0.301	1.330	0.462	-5.891	-1.237	22	27	0.551	0.009	-0.559	5.653	1.329	0.000
	41	7.246	0.301	1.330	0.462	-0.438	-0.002		28	0.551	0.009	-0.559	5.653	-1.676	0.051
51	37	6.467	0.185	-0.860	-0.294	3.577	-0.759	23	29	0.000	0.000	0.000	0.000	0.000	0.000
	42	6.467	0.185	-0.860	-0.294	0.049	-0.002		26	0.000	0.000	0.000	0.000	0.000	0.000
52	35	-3.645	0.329	1.565	-0.016	-4.757	-0.928	24	26	1.406	0.000	0.015	0.000	-0.052	0.000
	43	-3.645	0.329	1.565	-0.016	1.661	0.421		27	1.406	0.000	0.015	0.000	0.049	0.001
53	39	-2.381	0.018	-0.001	0.000	-0.001	-0.017	25	27	0.000	0.000	0.000	0.000	0.000	0.000
	22	-2.381	0.018	-0.001	0.000	-0.005	0.039		30	0.000	0.000	0.000	0.000	0.000	0.000
54	40	-2.524	-0.257	-1.067	-0.015	1.563	0.338	26	2	0.000	90.178	0.000	0.000	0.000	-36.090
	38	-2.524	-0.257	-1.067	-0.015	-2.811	-0.715		31	0.000	-103.964	0.000	0.000	0.000	-54.011
55	49	12.819	-0.002	2.060	-0.040	-0.552	-0.008	27	7	0.000	119.472	0.000	0.000	0.000	-63.718
	12	12.819	-0.002	2.060	-0.040	4.805	-0.015		8	0.000	-119.472	0.000	0.000	0.000	-63.718
57	21	0.161	2.375	0.001	0.004	0.000	0.082	28	8	0.000	108.271	0.000	0.000	0.000	-52.331
	37	0.161	2.375	0.001	0.004	0.000	1.072		32	0.000	-108.271	0.000	0.000	0.000	-52.331
58	43	-0.184	-0.312	-0.035	0.000	0.056	0.507	29	9	2.566	-43.063	15.492	1.747	-14.982	71.437
	41	-0.184	-0.312	-0.035	0.000	-0.057	-0.493		33	2.566	-155.068	15.492	1.747	8.255	-77.161
59	41	-0.098	-0.011	-0.005	0.000	0.015	0.020	30	31	0.000	114.922	0.000	0.000	0.000	-54.011
	42	-0.098	-0.011	-0.005	0.000	-0.012	-0.034		7	0.000	-124.022	0.000	0.000	0.000	-68.572
60	42	-0.263	0.173	0.027	0.000	-0.055	-0.353	31	32	-1.925	158.155	-13.412	-3.582	13.252	-105.322
	40	-0.263	0.173	0.027	0.000	0.054	0.359		9	-1.925	8.815	-13.412	-3.582	-13.572	61.648
61	44	0.000	1.714	-1.330	0.000	0.133	-0.005	32	33	0.000	117.319	-4.762	1.747	8.255	-77.161
	40	0.000	1.714	-1.330	0.000	-0.133	0.338		10	0.000	-76.823	-4.762	1.747	-4.127	-24.516
62	45	-0.132	-0.001	-0.269	-0.024	0.376	0.003	33	2	0.000	246.330	0.000	0.000	0.000	-283.280
	5	-0.132	-0.001	-0.269	-0.024	-0.323	0.000		4	0.000	-246.330	0.000	0.000	0.000	-283.280
63	37	0.353	-0.948	-0.194	-0.313	0.539	2.983	34	10	0.000	246.330	0.000	0.000	0.000	-283.280
	9	0.353	-0.948	-0.194	-0.313	-0.505	-2.116		28	0.000	-246.330	0.000	0.000	0.000	-283.280
67	49	0.100	0.000	0.000	0.000	0.000	0.000	35	34	4.845	-0.008	-0.117	0.138	1.231	0.025
	45	0.100	0.000	0.000	0.000	0.000	0.000		2	4.845	-0.008	-0.117	0.138	0.601	-0.017
68	49	0.000	1.863	1.750	0.000	-0.175	0.048	36	35	-8.691	-1.095	1.794	-2.498	-5.620	3.967
	43	0.000	1.863	1.750	0.000	0.175	0.421		7	-8.691	-1.095	1.794	-2.498	4.030	-1.924
70	50	7.276	0.000	1.244	-0.051	0.261	-0.002	37	36	61.315	-34.625	-2.950	-3.415	11.371	144.807
	41	7.276	0.000	1.244	-0.051	0.510	-0.002		8	61.315	-34.625	-2.950	-3.415	-4.495	-41.447
71	49	-0.211	0.000	-0.077	-0.002	0.121	0.000	39	38	-3.685	-1.751	1.103	6.665	-3.266	6.326
	50	-0.211	0.000	-0.077	-0.002	-0.126	0.000		10	-3.685	-1.751	1.103	6.665	2.669	-3.092
72	51	6.499	0.000	-0.696	0.024	0.047	-0.002	40	34	-0.277	4.941	-0.571	0.243	1.031	-1.526
	42	6.499	0.000	-0.696	0.024	-0.092	-0.002		35	-0.277	-5.533	-0.571	0.243	-2.278	-3.243
73	50	-1.062	0.000	-0.012	0.000	0.032	0.000	41	35	4.784	-8.066	15.707	21.942	-25.525	3.977
	51	-1.062	0.000	-0.012	0.000	-0.026	0.000		36	4.784	-13.845	15.707	21.942	24.737	-31.079
74	51	-0.583	0.000	0.058	0.002	-0.121	0.000	42	36	12.966	6.718	1.242	3.293	-1.704	2.329
	44	-0.583	0.000	0.058	0.002	0.117	0.000		37	12.966	-2.131	1.242	3.293	4.380	13.566
75	51	6.569	0.000	-1.175	0.024	0.048	0.000	43	37	5.001	6.000	-9.114	-20.860	20.459	-12.784
	22	6.569	0.000	-1.175	0.024	-3.007	0.000		38	5.001	-1.405	-9.114	-20.860	-16.908	-3.364
76	50	7.341	0.000	2.095	-0.051	-0.419	0.000	44	34	-0.563	-0.096	0.393	0.295	-0.890	0.218
	17	7.341	0.000	2.095	-0.051	5.027	0.000		45	-0.563	-0.096	0.393	0.295	0.802	-0.194
77	53	0.256	0.003	0.001	0.000	0.000	-0.004	45	5	2.256	3.369	-0.589	-0.002	1.239	-3.025
	12	0.256	0.003	0.001	0.000	0.004	0.002		12	2.256	-2.965	-0.589	-0.002	-2.178	-1.852
78	53	-0.021	-0.384	0.000	-0.001	0.000	0.006	46	12	-6.022	1.726	15.142	-0.269	-25.264	-2.441
	49	-0.021	-0.384	0.000	-0.001	0.000	-0.054		17	-6.022	-1.769	15.142	-0.269	23.191	-2.510
Load case 5 (Linear): 1.4DL +1.7LL								47	17	-12.930	2.891	1.219	-0.283	-1.010	-1.506
									22	-12.930	-2.459	1.219	-0.283	4.965	-0.448
Memb	Node	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment	48	22	-7.568	3.796	-9.198	0.527	19.932	-5.149
									27	-7.568	-0.682	-9.198	0.527	-17.780	1.234
1	3	0.000	0.000	-0.001	-0.004	0.000	0.000	49	27	-10.592	-0.215	6.994	0.092	-12.077	0.526
	34	0.000	0.000	-0.001	-0.004	0.000	0.000		44	-10.592	-0.215	6.994	0.092	6.108	

Memb	Node	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment	Memb	Node	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
60	42	1.010	-3.881	-0.097	0.000	0.202	7.507	31	32	-1.344	114.280	-10.838	-4.542	10.655	-76.447
	40	1.010	-3.881	-0.097	0.000	-0.196	-8.407		9	-1.344	7.516	-10.838	-4.542	-11.021	45.349
61	44	0.000	-32.398	4.893	0.000	-0.489	-0.092	32	33	0.000	84.222	-3.758	2.215	6.514	-55.769
	40	0.000	-32.398	4.893	0.000	0.489	-6.572		10	0.000	-54.572	-3.758	2.215	-3.257	-17.224
62	45	0.589	0.076	1.210	-0.074	-1.689	-0.194	33	2	0.000	175.950	0.000	0.000	0.000	-202.342
	5	0.589	0.076	1.210	-0.074	1.456	0.003		4	0.000	-175.950	0.000	0.000	0.000	-202.342
63	37	51.878	-28.904	4.491	1.410	-14.368	150.150	34	10	0.000	175.950	0.000	0.000	0.000	-202.342
	9	51.878	-28.904	4.491	1.410	9.789	-5.329		28	0.000	-175.950	0.000	0.000	0.000	-202.342
67	49	-0.304	12.656	0.000	0.000	0.000	0.000	35	34	3.599	-0.004	-0.268	0.064	1.324	0.013
	45	-0.304	-12.656	0.000	0.000	0.000	0.000		2	3.599	-0.004	-0.268	0.064	-0.120	-0.008
68	49	0.000	-35.928	-7.596	0.000	0.760	-0.587	36	35	-6.276	-0.758	1.073	-1.130	-3.486	2.737
	43	0.000	-35.928	-7.596	0.000	-0.760	-7.772		7	-6.276	-0.758	1.073	-1.130	2.284	-1.342
70	50	-21.958	-17.675	-4.871	4.518	-0.889	0.056	37	36	46.221	-28.944	-2.436	-1.440	9.089	113.093
	41	-21.958	-17.675	-4.871	4.518	-1.863	-3.479		8	46.221	-28.944	-2.436	-1.440	-4.016	-42.602
71	49	0.993	6.983	0.330	0.064	-0.520	0.000	39	38	-2.816	-1.216	0.601	2.590	-1.854	4.379
	50	0.993	-6.983	0.330	0.064	0.536	0.000		10	-2.816	-1.216	0.601	2.590	1.378	-2.161
72	51	-33.152	-19.639	2.850	-4.031	0.034	0.067	40	34	0.092	3.667	-0.258	0.166	0.465	-1.518
	42	-33.152	-19.639	2.850	-4.031	0.604	-3.861		35	0.092	-3.667	-0.258	0.166	-1.030	-1.950
73	50	4.222	10.692	0.029	0.008	-0.081	0.000	41	35	2.134	-5.902	7.151	14.964	-11.574	2.745
	51	4.222	-10.692	0.029	0.008	0.059	0.000		36	2.134	-10.030	7.151	14.964	11.308	-22.746
74	51	2.102	8.947	-0.212	-0.059	0.442	0.000	42	36	6.771	4.952	0.142	2.330	0.222	1.666
	44	2.102	-8.947	-0.212	-0.059	-0.427	0.000		37	6.771	-1.369	0.142	2.330	0.919	10.445
75	51	-33.393	0.000	4.971	-4.031	-0.417	0.000	43	37	2.141	4.622	-3.556	-14.325	7.957	-10.040
	22	-33.393	0.000	4.971	-4.031	12.508	0.000		38	2.141	-0.667	-3.556	-14.325	-6.623	-1.933
76	50	-22.259	0.000	-8.099	4.518	1.506	0.000	44	34	-0.255	-0.068	0.176	0.194	-0.399	0.153
	17	-22.259	0.000	-8.099	4.518	-19.552	0.000		45	-0.255	-0.068	0.176	0.194	0.359	-0.138
77	53	-0.195	-0.011	-0.006	0.000	-0.001	0.019	45	5	1.143	2.303	-0.272	-0.001	0.575	-1.797
	12	-0.195	-0.011	-0.006	0.000	-0.015	-0.009		12	1.143	-2.221	-0.272	-0.001	-1.002	-1.561
78	53	0.097	1.753	0.001	0.004	-0.001	-0.028	46	12	-2.638	1.023	6.910	-0.185	-11.465	-1.456
	49	0.097	1.753	0.001	0.004	-0.001	0.248		17	-2.638	-1.473	6.910	-0.185	10.646	-2.175
Load case 6 (Linear): DL + 1.3 LL + 1.3WL															
Memb	Node	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment	Memb	Node	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
1	3	0.000	-0.001	0.000	-0.002	0.000	0.000	47	17	-5.341	1.986	0.137	-0.195	0.506	-0.839
	34	0.000	-0.001	0.000	-0.002	0.000	-0.001		22	-5.341	-1.836	0.137	-0.195	1.179	-0.469
2	5	2.249	0.000	-0.582	-0.102	1.716	0.000	48	22	-3.039	2.586	-3.588	0.363	7.737	-3.489
	4	2.249	0.000	-0.582	-0.102	-1.413	-0.001		27	-3.039	-0.612	-3.588	0.363	-6.973	0.558
3	1	0.000	0.000	0.000	0.000	0.000	0.000	49	27	-4.521	-0.151	2.775	0.047	-4.764	0.362
	3	0.000	0.000	0.000	0.000	0.000	0.000		44	-4.521	-0.151	2.775	0.047	2.452	-0.030
4	3	0.001	0.000	0.000	0.000	-0.002	0.000	50	36	-6.997	32.985	-2.198	-33.501	9.636	-137.783
	5	0.001	0.000	0.000	0.000	0.001	0.000		41	-6.997	32.985	-2.198	-33.501	0.623	-2.545
5	11	-0.062	-1.006	0.002	0.007	0.000	0.000	51	37	-16.424	35.759	1.493	30.665	-6.379	-149.436
	35	-0.062	-1.006	0.002	0.007	0.001	-0.419		42	-16.424	35.759	1.493	30.665	-2.256	-2.825
6	5	0.000	0.000	0.000	0.000	0.000	0.000	52	35	7.160	-4.128	-3.113	-1.208	9.407	11.641
	6	0.000	0.000	0.000	0.000	0.000	0.000		43	7.160	-4.128	-3.113	-1.208	-3.355	-5.283
7	12	3.112	-0.002	-0.229	-1.004	0.442	0.000	53	39	35.769	-0.246	0.002	0.000	0.001	0.232
	13	3.112	-0.002	-0.229	-1.004	-0.788	-0.012		22	35.769	-0.246	0.002	0.000	0.008	-0.558
8	14	0.000	0.000	0.000	0.000	0.000	0.000	54	40	3.834	3.427	1.534	-0.080	-2.278	-4.496
	11	0.000	0.000	0.000	0.000	0.000	0.000		38	3.834	3.427	1.534	-0.080	4.013	9.555
9	11	1.008	-0.001	-0.002	0.000	0.007	0.000	55	49	4.979	0.148	-4.007	0.548	0.967	-0.198
	53	1.008	-0.001	-0.002	0.000	-0.002	-0.004		12	4.979	0.148	-4.007	0.548	-9.451	0.188
10	12	0.000	0.000	0.000	0.000	0.000	0.000	56	41	0.166	24.668	0.003	0.000	-0.009	-29.979
	15	0.000	0.000	0.000	0.000	0.000	0.000		42	0.166	-24.082	0.003	0.000	0.004	-28.545
11	16	-1.746	-28.933	0.003	0.010	-0.001	0.000	60	42	0.419	-2.684	-0.038	0.000	0.080	5.163
	36	-1.746	-28.933	0.003	0.010	0.000	-12.056		40	0.419	-2.684	-0.038	0.000	-0.077	-5.842
12	17	5.205	2.606	-0.669	-1.959	2.033	0.000	61	44	0.000	-22.115	1.953	0.000	-0.195	-0.072
	18	5.205	2.606	-0.669	-1.959	-1.568	14.019		40	0.000	-22.115	1.953	0.000	0.195	-4.495
13	19	0.000	0.000	0.000	0.000	0.000	0.000	62	45	0.270	0.054	0.561	-0.081	-0.783	-0.138
	16	0.000	0.000	0.000	0.000	0.000	0.000		5	0.270	0.054	0.561	-0.081	0.676	0.002
14	16	28.986	0.001	-0.003	0.000	0.011	0.000	63	37	39.347	-22.963	3.135	0.652	-10.181	116.764
	17	28.986	0.001	-0.003	0.000	-0.012	0.009		9	39.347	-22.963	3.135	0.652	6.684	-6.757
15	17	0.000	0.000	0.000	0.000	0.000	0.000	67	49	-0.106	9.255	0.000	0.000	0.000	0.000
	20	0.000	0.000	0.000	0.000	0.000	0.000		45	-0.106	-9.255	0.000	0.000	0.000	0.000
17	22	6.824	3.059	-0.087	1.049	-0.022	0.000	68	49	0.000	-24.620	-3.432	0.000	0.343	-0.359
	23	6.824	3.059	-0.087	1.049	-0.492	16.455		43	0.000	-24.620	-3.432	0.000	-0.343	-5.283
18	24	0.000	0.000	0.000	0.000	0.000	0.000	70	50	-7.062	-12.925	-2.045	3.370	-0.332	0.040
	21	0.000	0.000	0.000	0.000	0.000	0.000		41	-7.062	-12.925	-2.045	3.370	-0.741	-2.545
19	21	35.769	-0.246	0.002	0.000	-0.007	1.146	71	49	0.469	5.106	0.148	0.045	-0.233	0.000
	39	35.769	-0.246	0.002	0.000	0.001	0.232		50	0.469	-5.106	0.148	0.045	0.240	0.000
20	22	0.000	0.000	0.000	0.000	0.000	0.000	72	51	-16.465	-14.361	1.241	-3.042	0.084	0.047
	25	0.000	0.000	0.000	0.000	0.000	0.000		42	-16.465	-14.361	1.241	-3.042	0.332	-2.825
21	26	-0.057	-0.938	-0.006	-0.020	0.001	0.000	73	50	1.794	7.819	0.006	0.006	-0.019	0.000
	38	-0.057	-0.938	-0.006	-0.020	-0.001	-0.391		51	1.794	-7.819	0.006	0.006	0.010	0.000
22	27	0.518	0.004	-0.258</											

Memb	Node	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment	Memb	Node	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
3	1	0.000	0.000	0.000	0.000	0.000	0.000	50	36	-16.417	32.593	-3.927	-34.101	17.294	-136.175
	3	0.000	0.000	0.000	0.000	0.000	0.000		41	-16.417	32.593	-3.927	-34.101	1.192	-2.542
4	3	0.000	0.000	0.001	0.000	-0.003	0.000	51	37	-24.831	35.519	2.612	31.048	-11.028	-148.448
	5	0.000	0.000	0.001	0.000	0.002	0.000		42	-24.831	35.519	2.612	31.048	-0.320	-2.822
5	11	-0.072	-1.173	0.004	0.012	0.000	0.000	52	35	11.898	-4.555	-5.148	-1.187	15.591	12.847
	35	-0.072	-1.173	0.004	0.012	0.001	-0.489		43	11.898	-4.555	-5.148	-1.187	-5.514	-5.830
6	5	0.000	0.000	0.000	0.000	0.000	0.000	53	39	38.864	-0.269	0.004	0.000	0.002	0.255
	6	0.000	0.000	0.000	0.000	0.000	0.000		22	38.864	-0.269	0.004	0.000	0.014	-0.609
7	12	3.161	0.005	-0.472	-1.659	1.080	0.000	54	40	7.116	3.761	2.921	-0.061	-4.311	-4.935
	13	3.161	0.005	-0.472	-1.659	-1.459	0.028		38	7.116	3.761	2.921	-0.061	7.666	10.484
8	14	0.000	0.000	0.000	0.000	0.000	0.000	55	49	-11.685	0.152	-6.685	0.600	1.684	-0.187
	11	0.000	0.000	0.000	0.000	0.000	0.000		12	-11.685	0.152	-6.685	0.600	-15.698	0.207
9	11	1.176	-0.001	-0.004	0.000	0.012	0.000	57	21	-2.612	-38.777	-0.004	-0.012	0.001	-1.253
	53	1.176	-0.001	-0.004	0.000	-0.004	-0.006		37	-2.612	-38.777	-0.004	-0.012	0.000	-17.411
10	12	0.000	0.000	0.000	0.000	0.000	0.000	58	43	0.559	5.013	0.114	0.000	-0.179	-8.511
	15	0.000	0.000	0.000	0.000	0.000	0.000		41	0.559	5.013	0.114	0.000	0.184	7.532
11	16	-1.906	-31.589	0.006	0.019	-0.002	0.000	59	41	0.294	24.682	0.010	0.000	-0.028	-30.006
	36	-1.906	-31.589	0.006	0.019	0.001	-13.163		42	0.294	-24.068	0.010	0.000	0.020	-28.500
12	17	5.267	4.362	-0.901	-3.476	2.638	0.000	60	42	0.760	-2.910	-0.073	0.000	0.152	5.622
	18	5.267	4.362	-0.901	-3.476	-2.211	23.466		40	0.760	-2.910	-0.073	0.000	-0.147	-6.308
13	19	0.000	0.000	0.000	0.000	0.000	0.000	61	44	0.000	-24.343	3.681	0.000	-0.368	-0.066
	16	0.000	0.000	0.000	0.000	0.000	0.000		40	0.000	-24.343	3.681	0.000	0.368	-4.934
14	16	31.646	0.001	-0.006	0.000	0.019	0.000	62	45	0.443	0.055	0.910	-0.049	-1.271	-0.141
	17	31.646	0.001	-0.006	0.000	-0.021	0.010		5	0.443	0.055	0.910	-0.049	1.096	0.002
15	17	0.000	0.000	0.000	0.000	0.000	0.000	63	37	38.889	-21.731	3.388	1.060	-10.882	112.886
	20	0.000	0.000	0.000	0.000	0.000	0.000		9	38.889	-21.731	3.388	1.060	7.341	-4.006
17	22	7.115	5.851	-0.289	1.835	0.499	0.000	67	49	-0.236	9.255	0.000	0.000	0.000	0.000
	23	7.115	5.851	-0.289	1.835	-1.054	31.474		45	-0.236	-9.255	0.000	0.000	0.000	0.000
18	24	0.000	0.000	0.000	0.000	0.000	0.000	68	49	0.000	-27.042	-5.706	0.000	0.571	-0.421
	21	0.000	0.000	0.000	0.000	0.000	0.000		43	0.000	-27.042	-5.706	0.000	-0.571	-5.829
19	21	38.864	-0.269	0.004	0.000	-0.012	1.253	70	50	-16.521	-12.925	-3.662	3.437	-0.672	0.042
	39	38.864	-0.269	0.004	0.000	0.002	0.255		41	-16.521	-12.925	-3.662	3.437	-1.404	-2.543
20	22	0.000	0.000	0.000	0.000	0.000	0.000	71	49	0.744	5.106	0.248	0.048	-0.391	0.000
	25	0.000	0.000	0.000	0.000	0.000	0.000		50	0.744	-5.106	0.248	0.048	0.403	0.000
21	26	-0.064	-1.056	-0.011	-0.039	0.002	0.000	72	51	-24.914	-14.361	2.145	-3.074	0.023	0.050
	38	-0.064	-1.056	-0.011	-0.039	-0.002	-0.440		42	-24.914	-14.361	2.145	-3.074	0.452	-2.822
22	27	0.340	0.007	-0.426	4.250	1.021	0.000	73	50	3.174	7.819	0.021	0.006	-0.061	0.000
	28	0.340	0.007	-0.426	4.250	-1.272	0.038		51	3.174	-7.819	0.021	0.006	0.044	0.000
23	29	0.000	0.000	0.000	0.000	0.000	0.000	74	51	1.580	6.542	-0.159	-0.045	0.332	0.000
	26	0.000	0.000	0.000	0.000	0.000	0.000		44	1.580	-6.542	-0.159	-0.045	-0.321	0.000
24	26	1.058	0.000	0.011	0.000	-0.039	0.000	75	51	-25.095	0.000	3.740	-3.074	-0.312	0.000
	27	1.058	0.000	0.011	0.000	0.037	0.001		22	-25.095	0.000	3.740	-3.074	9.411	0.000
25	27	0.000	0.000	0.000	0.000	0.000	0.000	76	50	-16.748	0.000	-6.093	3.437	1.136	0.000
	30	0.000	0.000	0.000	0.000	0.000	0.000		17	-16.748	0.000	-6.093	3.437	-14.705	0.000
26	2	0.000	64.469	0.000	0.000	0.000	-25.801	77	53	-0.146	-0.008	-0.004	0.000	0.000	0.015
	31	0.000	-74.324	0.000	0.000	0.000	-38.613		12	-0.146	-0.008	-0.004	0.000	-0.012	-0.007
27	7	0.000	85.411	0.000	0.000	0.000	-45.553	78	53	0.073	1.319	0.001	0.003	-0.001	-0.021
	8	0.000	-85.411	0.000	0.000	0.000	-45.553		49	0.073	1.319	0.001	0.003	0.000	0.186
28	8	0.000	77.404	0.000	0.000	0.000	-37.412	Load case 8 (Linear): DL + LL							
	32	0.000	-77.404	0.000	0.000	0.000	-37.412								
29	9	1.936	-31.850	11.647	1.313	-11.264	52.129								
	33	1.936	-111.923	11.647	1.313	6.206	-55.701								
30	31	0.000	82.158	0.000	0.000	0.000	-38.613								
	7	0.000	-88.664	0.000	0.000	0.000	-49.022		1	3	0.000	0.001	-0.001	-0.002	0.000
31	32	-1.452	113.802	-10.084	-2.692	9.963	-76.052		2	5	2.308	0.000	-0.682	-0.136	1.951
	9	-1.452	7.038	-10.084	-2.692	-10.204	44.789			4	2.308	0.000	-0.682	-0.136	-1.716
32	33	0.000	84.183	-3.581	1.313	6.206	-55.701		3	1	0.000	0.000	0.000	0.000	0.000
	10	0.000	-54.610	-3.581	1.313	-3.103	-17.257			3	0.000	0.000	0.000	0.000	0.000
33	2	0.000	175.950	0.000	0.000	0.000	-202.342		4	3	-0.001	0.000	0.001	0.000	-0.002
	4	0.000	-175.950	0.000	0.000	0.000	-202.342			5	-0.001	0.000	0.001	0.000	0.002
34	10	0.000	175.950	0.000	0.000	0.000	-202.342		5	11	-0.059	-0.962	0.003	0.010	0.000
	28	0.000	-175.950	0.000	0.000	0.000	-202.342			35	-0.059	-0.962	0.003	0.010	0.001
35	34	3.466	-0.007	-0.061	0.103	0.834	0.021		6	5	0.000	0.000	0.000	0.000	0.000
	2	3.466	-0.007	-0.061	0.103	0.505	-0.014			6	0.000	0.000	0.000	0.000	0.000
36	35	-6.931	-0.826	1.347	-1.881	-4.213	2.991		7	12	3.291	0.004	-0.368	-1.362	0.818
	7	-6.931	-0.826	1.347	-1.881	3.031	-1.451			13	3.291	0.004	-0.368	-1.362	-1.164
37	36	45.890	-26.057	-2.222	-2.570	8.569	108.969		8	14	0.000	0.000	0.000	0.000	0.000
	8	45.890	-26.057	-2.222	-2.570	-3.381	-31.193			11	0.000	0.000	0.000	0.000	0.000
39	38	-3.022	-1.317	0.787	5.011	-2.297	4.760		9	11	0.964	-0.001	-0.003	0.000	0.000
	10	-3.022	-1.317	0.787	5.011	1.936	-2.326			53	0.964	-0.001	-0.003	0.000	-0.013
40	34	-0.237	3.536	-0.430	0.183	0.778	-1.050		10	12	0.000	0.000	0.000	0.000	0.000
	35	-0.237	-3.946	-0.430	0.183	-1.717	-2.237			15	0.000	0.000	0.000	0.000	0.000
41	35	3.568	-6.249	11.816	16.511	-19.201	3.164		11	16	-1.568	-25.978	0.005	0.016	-0.001
	36	3.568	-10.377	11.816	16.511	18.610	-23.436								

Memb	Node	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment	Memb	Node	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
22	27	0.527	0.006	-0.331	3.508	0.770	0.000	73	50	2.620	7.185	0.018	0.005	-0.051	0.000
	28	0.527	0.006	-0.331	3.508	-1.011	0.031		51	2.620	-7.185	0.018	0.005	0.037	0.000
23	29	0.000	0.000	0.000	0.000	0.000	0.000	74	51	1.304	6.012	-0.131	-0.037	0.274	0.000
	26	0.000	0.000	0.000	0.000	0.000	0.000		44	1.304	-6.012	-0.131	-0.037	-0.265	0.000
24	26	0.871	0.000	0.009	0.000	-0.032	0.000	75	51	-20.746	0.000	3.079	-2.394	-0.264	0.000
	27	0.871	0.000	0.009	0.000	0.030	0.001		22	-20.746	0.000	3.079	-2.394	7.741	0.000
25	27	0.000	0.000	0.000	0.000	0.000	0.000	76	50	-13.778	0.000	-5.017	2.702	0.926	0.000
	30	0.000	0.000	0.000	0.000	0.000	0.000		17	-13.778	0.000	-5.017	2.702	-12.118	0.000
26	2	0.000	64.273	0.000	0.000	0.000	-25.723	77	53	-0.123	-0.007	-0.004	0.000	0.000	0.012
	31	0.000	-74.099	0.000	0.000	0.000	-38.496		12	-0.123	-0.007	-0.004	0.000	-0.010	-0.006
27	7	0.000	85.152	0.000	0.000	0.000	-45.414	78	53	0.060	1.085	0.001	0.003	-0.001	-0.017
	8	0.000	-85.152	0.000	0.000	0.000	-45.414		49	0.060	1.085	0.001	0.003	0.000	0.153
28	8	0.000	77.169	0.000	0.000	0.000	-37.298	NODE REACTIONS (kN,kNm) Load case 1 (Linear): DL							
	32	0.000	-77.169	0.000	0.000	0.000	-37.298								
29	9	1.576	-28.031	9.612	1.085	-9.297	48.269	Node	X-Axis Force	Y-Axis Force	Z-Axis Force	X-Axis Moment	Y-Axis Moment	Z-Axis Moment	
	33	1.576	-107.861	9.612	1.085	5.122	-53.650	2	-0.008	242.964	0.401	-24.820	0.037	202.356	
30	31	0.000	81.909	0.000	0.000	0.000	-38.496	4	0.000	178.050	0.378	0.814	-0.038	-202.343	
	7	0.000	-88.395	0.000	0.000	0.000	-48.874	7	0.167	175.824	-0.356	2.710	-0.407	-0.292	
31	32	-1.182	110.882	-8.321	-2.224	8.222	-73.175	8	5.972	174.815	0.481	8.826	-0.590	-7.108	
	9	-1.182	4.442	-8.321	-2.224	-8.420	42.148	10	-0.564	237.437	-0.805	19.618	1.969	201.478	
32	33	0.000	82.841	-2.955	1.085	5.122	-53.650	13	-0.001	9.727	0.023	0.182	-0.372	0.006	
	10	0.000	-55.531	-2.955	1.085	-2.561	-18.146	13	-0.001	3.306	0.230	0.549	-0.816	5.440	
33	2	0.000	175.950	0.000	0.000	0.000	-202.342	18	-1.011	4.606	0.090	0.284	0.435	7.544	
	4	0.000	-175.950	0.000	0.000	0.000	-202.342	20	-0.002	177.098	0.014	0.141	1.031	-202.333	
34	10	0.000	175.950	0.000	0.000	0.000	-202.342	31	0.000	154.425	0.000	0.000	0.000	0.000	
	28	0.000	-175.950	0.000	0.000	0.000	-202.342	32	2.446	177.531	-0.283	-26.665	2.417	-0.664	
35	34	3.449	-0.003	-0.140	0.088	0.992	0.010	33	3.701	172.690	-0.377	0.000	0.000	0.000	
	2	3.449	-0.003	-0.140	0.088	0.240	-0.008	40	-1.704	7.896	0.000	1.944	-1.032	0.000	
36	35	-4.400	-0.674	1.118	-1.541	-3.518	2.440	43	-2.794	8.566	0.000	-1.626	1.657	0.000	
	7	-4.400	-0.674	1.118	-1.541	2.496	-1.183	44	-1.940	-1.816	0.000	0.056	-1.071	0.000	
37	36	38.563	-21.422	-1.820	-2.113	7.006	89.595	45	0.197	6.084	0.205	-0.249	-0.380	0.000	
	8	38.563	-21.422	-1.820	-2.113	-2.784	-25.635	49	-3.056	3.358	0.000	0.380	0.593	0.000	
39	38	-1.658	-1.084	0.791	4.135	-2.422	3.917	Load	0.000	-1726.560	0.000	0.000	0.000	0.000	
	10	-1.658	-1.084	0.791	4.135	1.833	-1.914	Reac	0.000	1726.560	0.000	0.000	0.000	0.000	
40	34	-0.101	3.513	-0.351	0.150	0.634	-1.190	Load case 2 (Linear): LL							
	35	-0.101	-3.969	-0.351	0.150	-1.404	-2.515								
41	35	3.041	-4.542	9.728	13.580	-15.810	2.031	Node	X-Axis Force	Y-Axis Force	Z-Axis Force	X-Axis Moment	Y-Axis Moment	Z-Axis Moment	
	36	3.041	-8.670	9.728	13.580	15.319	-19.109	2	0.011	0.708	-0.262	-1.143	0.050	-0.022	
42	36	8.109	4.730	0.778	2.065	-1.077	0.580	4	0.000	0.208	0.304	0.901	-0.098	-0.001	
	37	8.109	-1.591	0.778	2.065	8.272	8.272	7	0.507	-6.676	-0.762	-1.747	-1.134	-0.891	
43	37	3.204	3.783	-5.649	-12.941	12.681	-7.189	8	15.450	26.069	1.339	2.074	-1.523	-18.527	
	38	3.204	-1.506	-5.649	-12.941	-10.479	-2.520	10	-1.307	-7.614	0.014	-3.305	4.727	-2.135	
44	34	-0.347	-0.064	0.240	0.199	-0.544	0.141	13	-0.003	-0.435	0.435	0.982	-0.990	0.017	
	45	-0.347	-0.064	0.240	0.199	0.488	-0.133	18	-2.578	1.509	0.516	1.278	-2.046	13.866	
45	5	1.430	2.359	-0.364	-0.001	0.766	-2.014	23	-3.422	1.930	0.153	0.592	1.077	18.407	
	12	1.430	-2.165	-0.364	-0.001	-1.346	-1.449	28	-0.004	-0.621	0.318	0.876	0.022	0.022	
46	12	-3.722	1.279	9.374	-0.168	-15.641	-1.718	31	0.000	1.583	0.000	0.000	0.000	0.000	
	17	-3.722	-1.217	9.374	-0.168	14.354	-1.619	32	5.876	10.519	-0.899	-9.212	5.805	-1.560	
47	17	-7.997	2.030	0.763	-0.176	-0.643	-1.105	33	8.866	18.012	-1.199	0.000	0.000	0.000	
	22	-7.997	-1.792	0.763	-0.176	3.094	-0.521	40	-4.219	17.782	0.000	3.403	-2.692	0.000	
48	22	-4.673	2.586	-5.702	0.328	-12.358	-3.344	43	-7.091	21.573	0.000	-4.383	3.268	0.000	
	27	-4.673	-0.612	-5.702	0.328	-11.022	0.704	44	-4.758	-12.173	0.000	0.008	-2.748	0.000	
49	27	-6.566	-0.137	4.332	0.064	-7.484	0.327	45	0.515	2.536	0.132	-0.012	-1.152	0.000	
	44	-6.566	-0.137	4.332	0.064	3.780	-0.029	49	-7.843	-12.165	0.000	0.170	0.980	0.000	
50	36	-13.506	26.730	-3.243	-26.694	14.267	-111.935	Load	0.000	-62.745	0.000	0.000	0.000	0.000	
	41	-13.506	26.730	-3.243	-26.694	0.972	-2.340	Reac	0.000	62.745	0.000	0.000	0.000	0.000	
51	37	-20.528	29.257	2.144	24.176	-9.060	-122.549	Load case 3 (Linear): WL							
	42	-20.528	29.257	2.144	24.176	-0.271	-2.597								
52	35	9.791	-3.767	-4.257	-1.027	12.855	10.589	Node	X-Axis Force	Y-Axis Force	Z-Axis Force	X-Axis Moment	Y-Axis Moment	Z-Axis Moment	
	43	9.791	-3.767	-4.257	-1.027	-4.600	-4.857	2	-0.002	0.102	0.159	0.481	-0.030	0.004	
53	39	32.107	-0.223	0.003	0.000	0.002	0.210	4	0.000	0.093	-0.147	0.441	0.049	0.001	
	22	32.107	-0.223	0.003	0.000	0.011	-0.504	7	-0.052	0.504	0.211	0.575	0.578	0.084	
54	40	5.863	3.111	2.403	-0.100	-3.543	-4.094	8	-2.221	0.255	0.165	0.869	-8.776	0.567	
	38	5.863	3.111	2.403	-0.100	6.312	8.662	10	-0.215	0.128	0.143	0.404	-1.744	-0.567	
55	49	-9.611	0.152	-5.517	0.549	1.420	-0.221	13	0.006	-0.037	-0.187	-0.516	0.504	-0.031	
	12	-9.611	0.152	-5.517	0.549	-12.924	0.173	18	1.351	-0.048	-0.178	-0.495	1.167	-7.267	
57	21	-2.158	-32.035	-0.003	-0.010	0.001	-1.036	23	2.148	-0.224	-0.155	-0.432	-0.605	-11.553	
	37	-2.158	-32.035	-0.003	-0.010	0.000	-14.384	28	0.002	0.136	-0.129	-0.376	-1.585	-0.013	
58	43	0.466	4.156	0.093	0.000	-0.147	-7.035	31	0.000	0.000	0.000	0.000	0.000	0.000	
	41	0.466	4.156	0.093	0.000	0.152	6.265	32	0.580	0.367	0.083	-0.303	0.532	-1.422	
59	41	0.244	19.010	0.008	0.000	-0.023	-23.131	33	0.505	0.015	0.111	0.000	0.000	0.000	
	42	0.244	-18.490	0.008	0.000	0.017	-21.857	40	2.551	-2.144	0.000	-0.344	1.643	0.000	
60	40	0.625	-2.430	-0.060	0.000	0.125	4.713	43	3.680	-2.504	0.000	0.522	-1.780	0.000	
	42	0.625	-2.430	-0.060	0.000	-0.121	-5.248	44	2.708	1.707	0.000	-0.015	1.665	0.000	
61	44	0.000	-20.138	3.028	0.000	-0.303	-0.066	45	-0.262	-0.003	-0.076	-0.008	0.566	0.000	
	40	0.000	-20.138	3.028	0.000	0.303	-4.094	49	13.280	1.839	0.000	-0.040	-0.606	0.000	
62</															

Node	X-Axis Force	Y-Axis Force	Z-Axis Force	X-Axis Moment	Y-Axis Moment	Z-Axis Moment	Memb 6, Case 1 (Linear): DL						
							Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
13	0.002	3.112	0.229	0.788	-1.004	-0.012							
18	-2.606	5.205	0.669	1.568	-1.959	14.019							
23	-3.059	6.824	0.087	0.492	1.049	16.455	0.000	0.000	0.000	0.000	0.000	0.000#	0.000#
28	-0.004	176.468	0.258	0.783	2.190	-202.321	0.067	0.000	0.000	0.000	0.000	0.000	0.000
31	0.000	156.482	0.000	0.000	0.000	0.000	0.134	0.000	0.000	0.000	0.000	0.000	0.000
32	10.838	191.684	-1.344	-39.035	10.655	-4.542	0.000	0.000	0.000	0.000	0.000	0.000	0.000
33	15.883	196.126	-1.792	0.000	0.000	0.000	0.267	0.000	0.000	0.000	0.000	0.000	0.000
40	-3.873	28.226	0.000	5.922	-2.397	0.000	0.334	0.000	0.000	0.000	0.000	0.000	0.000
43	-7.228	33.356	0.000	-6.645	3.591	0.000	0.401	0.000	0.000	0.000	0.000	0.000*	0.000*
44	-4.605	-15.422	0.000	0.047	-2.479	0.000							
45	0.526	9.376	0.279	-0.275	-1.142	0.000							
49	4.012	-10.066	0.000	0.548	1.079	0.000							
Load	-37.050	-1808.129	0.000				Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
Reac	37.050	1808.129	0.000				0.000	3.727	0.001	-0.023	-0.372	-0.058*	0.000#

Node	X-Axis Force	Y-Axis Force	Z-Axis Force	X-Axis Moment	Y-Axis Moment	Z-Axis Moment	Memb 7, Case 1 (Linear): DL						
							Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
2	0.007	243.885	0.061	-26.306	0.103	202.328	0.000	0.000	0.001	-0.023	-0.372	-0.058*	0.000#
4	0.000	178.320	0.773	1.986	-0.166	-202.345	0.897	3.727	0.001	-0.023	-0.372	-0.078	0.001
7	0.826	167.145	-1.347	0.439	-1.881	-1.451	1.793	3.727	0.001	-0.023	-0.372	-0.099	0.002
8	26.057	208.705	2.222	11.522	-2.570	-31.193	2.690	3.727	0.001	-0.023	-0.372	-0.120	0.003
10	-2.263	227.538	-0.787	15.321	8.114	198.703	3.586	3.727	0.001	-0.023	-0.372	-0.141	0.004
13	-0.005	3.161	0.472	1.459	-1.659	0.028	4.483	3.727	0.001	-0.023	-0.372	-0.161	0.005
18	-4.362	5.267	0.901	2.211	-3.476	23.466	5.379	3.727	0.001	-0.023	-0.372	-0.182#	0.006*
23	-5.851	7.115	0.289	1.054	1.835	31.474	0.000	0.000	0.000	0.000	0.000	0.000	0.000#
28	-0.007	176.290	0.426	1.272	4.250	-202.304	0.067	0.000	0.000	0.000	0.000	0.000	0.000
31	0.000	156.482	0.000	0.000	0.000	0.000	0.134	0.000	0.000	0.000	0.000	0.000	0.000
32	10.084	191.206	-1.452	-38.640	9.963	-2.692	0.200	0.000	0.000	0.000	0.000	0.000	0.000
33	15.227	196.106	-1.936	0.000	0.000	0.000	0.267	0.000	0.000	0.000	0.000	0.000	0.000
40	-7.189	31.013	0.000	6.369	-4.532	0.000	0.334	0.000	0.000	0.000	0.000	0.000	0.000
43	-12.012	36.611	0.000	-7.324	5.906	0.000	0.401	0.000	0.000	0.000	0.000	0.000	0.000*
44	-8.125	-17.641	0.000	0.067	-4.643	0.000							
45	0.866	9.380	0.377	-0.265	-1.878	0.000							
49	-13.252	-12.457	0.000	0.600	1.867	0.000							
Load	0.000	-1808.129	0.000				Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
Reac	0.000	1808.129	0.000				0.000	0.257	0.000	-0.001	0.000	0.003*	0.000*

Node	X-Axis Force	Y-Axis Force	Z-Axis Force	X-Axis Moment	Y-Axis Moment	Z-Axis Moment	Memb 8, Case 1 (Linear): DL						
							Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
2	0.003	243.672	0.140	-25.963	0.088	202.335	0.000	0.257	0.000	-0.001	0.000	0.001	0.000*
4	0.000	178.258	0.682	1.716	-0.136	-202.344	0.718	0.257	0.000	-0.001	0.000	0.002	0.000
7	0.674	169.148	-1.118	0.963	-1.541	-1.183	1.436	0.257	0.000	-0.001	0.000	0.002	-0.001
8	21.422	200.884	1.820	10.900	-2.113	-25.635	2.154	0.257	0.000	-0.001	0.000	0.001	-0.001
10	-1.871	229.823	-0.791	16.313	6.696	199.343	2.872	0.257	0.000	-0.001	0.000	0.000	-0.001
13	-0.004	3.291	0.368	1.164	-1.362	0.023	3.590	0.257	0.000	-0.001	0.000	0.000	-0.002
18	-3.589	4.815	0.746	1.827	-2.862	19.306	4.308	0.257	0.000	-0.001	0.000	0.000	-0.002#
23	-4.824	6.536	0.243	0.876	1.512	25.951	0.000	0.000	0.000	0.000	0.000	0.000#	0.000#
28	-0.006	176.477	0.331	1.011	3.508	-202.311	0.067	0.000	0.000	0.000	0.000	0.000	0.000
31	0.000	156.008	0.000	0.000	0.000	0.000	0.134	0.000	0.000	0.000	0.000	0.000	0.000
32	8.321	188.051	-1.182	-35.877	8.222	-2.224	0.200	0.000	0.000	0.000	0.000	0.000	0.000
33	12.567	190.703	-1.576	0.000	0.000	0.000	0.267	0.000	0.000	0.000	0.000	0.000	0.000
40	-5.923	25.679	0.000	5.348	-3.724	0.000	0.334	0.000	0.000	0.000	0.000	0.000	0.000
43	-9.884	30.139	0.000	-6.009	4.925	0.000	0.401	0.000	0.000	0.000	0.000	0.000	0.000*
44	-6.698	-13.989	0.000	0.064	-3.819	0.000							
45	0.712	8.620	0.337	-0.261	-1.532	0.000							
49	-10.900	-8.808	0.000	0.549	1.573	0.000							
Load	0.000	-1789.305	0.000				Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
Reac	0.000	1789.305	0.000				0.000	-0.439	-7.275	0.001	0.005	0.000#	0.000*

Node	X-Axis Force	Y-Axis Force	Z-Axis Force	X-Axis Moment	Y-Axis Moment	Z-Axis Moment	Memb 9, Case 1 (Linear): DL						
							Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
2	0.003	243.672	0.140	-25.963	0.088	202.335	0.000	0.257	0.000	-0.001	0.000	0.001	0.000*
4	0.000	178.258	0.682	1.716	-0.136	-202.344	0.718	0.257	0.000	-0.001	0.000	0.002	0.000
7	0.674	169.148	-1.118	0.963	-1.541	-1.183	1.436	0.257	0.000	-0.001	0.000	0.002	-0.001
8	21.422	200.884	1.820	10.900	-2.113	-25.635	2.154	0.257	0.000	-0.001	0.000	0.001	-0.001
10	-1.871	229.823	-0.791	16.313	6.696	199.343	2.872	0.257	0.000	-0.001	0.000	0.000	-0.001
13	-0.004	3.291	0.368	1.164	-1.362	0.023	3.590	0.257	0.000	-0.001	0.000	0.000	-0.002
18	-3.589	4.815	0.746	1.827	-2.862	19.306	4.308	0.257	0.000	-0.001	0.000	0.000	-0.002#
23	-4.824	6.536	0.243	0.876	1.512	25.951	0.000	0.000	0.000	0.000	0.000	0.000#	0.000#
28	-0.006	176.477	0.331	1.011	3.508	-202.311	0.067	0.000	0.000	0.000	0.000	0.000	0.000
31	0.000	156.008	0.000	0.000	0.000	0.000	0.134	0.000	0.000	0.000	0.000	0.000	0.000
32	8.321	188.051	-1.182	-35.877	8.222	-2.224	0.200	0.000	0.000	0.000	0.000	0.000	0.000
33	12.567	190.703	-1.576	0.000	0.000	0.000	0.267	0.000	0.000	0.000	0.000	0.000	0.000
40	-5.923	25.679	0.000	5.348	-3.724	0.000	0.334	0.000	0.000	0.000	0.000	0.000	0.000
43	-9.884	30.139	0.000	-6.009	4.925	0.000	0.401	0.000	0.000	0.000	0.000	0.000	0.000*
44	-6.698	-13.989	0.000	0.064	-3.819	0.000							
45	0.712	8.620	0.337	-0.261	-1.532	0.000							
49	-10.900	-8.808	0.000	0.549	1.573	0.000							
Load	0.000	-1789.305	0.000				Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
Reac	0.000	1789.305	0.000				0.000	-0.439	-7.275	0.001	0.005	0.000#	0.000*

Node	X-Axis Force	Y-Axis Force	Z-Axis Force	X-Axis Moment	Y-Axis Moment	Z-Axis Moment	Memb 10, Case 1 (Linear): DL						
							Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
2	0.003	243.672	0.140	-25.963	0.088	202.335	0.000	0.257	0.000	-0.001	0.000	0.001	0.000*
4	0.000	178.258	0.682	1.716	-0.136	-202.344	0.718	0.257	0.000	-0.001	0.000	0.002	0.000
7	0.674	169.148	-1.118	0.963	-1.541	-1.183	1.436	0.257	0.000	-0.001	0.000	0.002	-0.001
8	21.422	200.884	1.820	10.900	-2.113	-25.635	2.154	0.257	0.000	-0.001	0.000	0.001	-0.001
10	-1.871	229.823	-0.791	16.313	6.696	199.343	2.872	0.257	0.000	-0.001	0.000	0.000	-0.001
13	-0.004	3.291	0.368	1.164	-1.362	0.023	3.590	0.257	0.000	-0.001	0.000	0.000	-0.002
18	-3.589	4.815	0.746	1.827	-2.862	19.306	4.308	0.257	0.000	-0.001	0.000	0.000	-0.002#
23	-4.824	6.536	0.243	0.876	1.512	25.951	0.000	0.000	0.000	0.000	0.000	0.000#	0.000#
28	-0.006	176.477	0.331	1.011	3.508	-202.311	0.067	0.000	0.000	0.000	0.000	0.000	0.000
31	0.000	156.008	0.000	0.000	0.000	0.000	0.134	0.000	0.000	0.000	0.000	0.000	0.000
32	8.321	188.051	-1.182	-35.877	8.222	-2.224	0.200	0.000	0.00				



Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment	Memb 28, Case 1 (Linear): DL						
Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment	Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
3.586	4.606	1.402	-0.090	0.435	-0.123	5.029	0.000	0.000	76.386*	0.000	0.000	0.000	-36.920#
4.483	4.606	1.402	-0.090	0.435	-0.203	6.287	0.483	0.000	50.924	0.000	0.000	0.000	-6.153
5.379	4.606	1.402	-0.090	0.435	-0.284#	7.544*	0.967	0.000	25.462	0.000	0.000	0.000	12.307
Memb 18, Case 1 (Linear): DL							1.450	0.000	0.000	0.000	0.000	0.000	18.460*
0.000	0.000	0.000	0.000	0.000	0.000	0.000#	1.933	0.000	-25.462	0.000	0.000	0.000	12.307
0.067	0.000	0.000	0.000	0.000	0.000	0.000	2.417	0.000	-50.924	0.000	0.000	0.000	-6.153
0.134	0.000	0.000	0.000	0.000	0.000	0.000	2.900	0.000	-76.386#	0.000	0.000	0.000	-36.920
0.200	0.000	0.000	0.000	0.000	0.000	0.000	Memb 29, Case 1 (Linear): DL						
0.267	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.377	-15.300*	2.831	0.324	-2.739#	35.402*
0.334	0.000	0.000	0.000	0.000	0.000	0.000	0.250	0.377	-28.470	2.831	0.324	-2.031	29.931
0.401	0.000	0.000	0.000	0.000	0.000	0.000*	0.500	0.377	-41.640	2.831	0.324	-1.323	21.167
Memb 19, Case 1 (Linear): DL							0.750	0.377	-54.810	2.831	0.324	-0.616	9.111
0.000	9.584	-0.068	0.001	0.000	-0.003#	0.312*	1.000	0.377	-67.980	2.831	0.324	0.092	-6.238
0.618	9.584	-0.068	0.001	0.000	-0.002	0.270	1.250	0.377	-81.150	2.831	0.324	0.800	-24.879
1.236	9.584	-0.068	0.001	0.000	-0.002	0.228	1.500	0.377	-94.320#	2.831	0.324	1.508*	-46.813#
1.853	9.584	-0.068	0.001	0.000	-0.001	0.187	Memb 30, Case 1 (Linear): DL						
2.471	9.584	-0.068	0.001	0.000	-0.001	0.145	0.000	0.000	81.078*	0.000	0.000	0.000	-38.105
3.089	9.584	-0.068	0.001	0.000	0.000	0.103	0.533	0.000	52.982	0.000	0.000	0.000	-2.356
3.707	9.584	-0.068	0.001	0.000	0.000*	0.061#	1.067	0.000	24.886	0.000	0.000	0.000	18.409
Memb 20, Case 1 (Linear): DL							1.600	0.000	-3.210	0.000	0.000	0.000	24.189*
0.000	0.000	0.000	0.000	0.000	0.000#	0.000#	2.133	0.000	-31.306	0.000	0.000	0.000	14.985
0.067	0.000	0.000	0.000	0.000	0.000	0.000	2.667	0.000	-59.402	0.000	0.000	0.000	-9.204
0.134	0.000	0.000	0.000	0.000	0.000	0.000	3.200	0.000	-87.498#	0.000	0.000	0.000	-48.378#
0.200	0.000	0.000	0.000	0.000	0.000	0.000	Memb 31, Case 1 (Linear): DL						
0.267	0.000	0.000	0.000	0.000	0.000	0.000	0.000	-0.283	101.145*	-2.446	-0.664	2.417*	-63.585#
0.334	0.000	0.000	0.000	0.000	0.000	0.000	0.333	-0.283	83.585	-2.446	-0.664	1.602	-32.797
0.401	0.000	0.000	0.000	0.000	0.000*	0.000*	0.667	-0.283	66.025	-2.446	-0.664	0.787	-7.862
Memb 21, Case 1 (Linear): DL							1.000	-0.283	48.465	-2.446	-0.664	-0.028	11.220
0.000	-0.015	-0.250	-0.003	-0.009	0.001*	0.000*	1.333	-0.283	30.905	-2.446	-0.664	-0.844	24.449
0.069	-0.015	-0.250	-0.003	-0.009	0.000	-0.017	1.667	-0.283	13.345	-2.446	-0.664	-1.659	31.824
0.139	-0.015	-0.250	-0.003	-0.009	0.000	-0.035	2.000	-0.283	-4.215#	-2.446	-0.664	-2.474#	33.346*
0.208	-0.015	-0.250	-0.003	-0.009	0.000	-0.052	Memb 32, Case 1 (Linear): DL						
0.278	-0.015	-0.250	-0.003	-0.009	0.000	-0.069	0.000	0.000	78.370*	-0.870	0.324	1.508*	-46.813#
0.347	-0.015	-0.250	-0.003	-0.009	0.000	-0.087	0.433	0.000	55.542	-0.870	0.324	1.131	-17.798
0.417	-0.015	-0.250	-0.003	-0.009	0.000#	-0.104#	0.867	0.000	32.714	-0.870	0.324	0.754	1.324
Memb 22, Case 1 (Linear): DL							1.300	0.000	9.886	-0.870	0.324	0.377	10.554*
0.000	1.148	0.002	-0.014	1.031	-0.068*	0.000#	1.733	0.000	-12.942	-0.870	0.324	0.000	9.892
0.897	1.148	0.002	-0.014	1.031	-0.080	0.002	2.167	0.000	-35.770	-0.870	0.324	-0.377	-0.662
1.793	1.148	0.002	-0.014	1.031	-0.093	0.003	2.600	0.000	-58.598#	-0.870	0.324	-0.754#	-21.108
2.690	1.148	0.002	-0.014	1.031	-0.105	0.005	Memb 33, Case 1 (Linear): DL						
3.586	1.148	0.002	-0.014	1.031	-0.117	0.006	0.000	0.000	175.950*	0.000	0.000	0.000	-202.342#
4.483	1.148	0.002	-0.014	1.031	-0.129	0.008	1.150	0.000	117.300	0.000	0.000	0.000	-33.724
5.379	1.148	0.002	-0.014	1.031	-0.141#	0.009*	2.300	0.000	58.650	0.000	0.000	0.000	67.447
Memb 23, Case 1 (Linear): DL							3.450	0.000	0.000	0.000	0.000	0.000	101.171*
0.000	0.000	0.000	0.000	0.000	0.000#	0.000#	4.600	0.000	-58.650	0.000	0.000	0.000	67.447
0.067	0.000	0.000	0.000	0.000	0.000	0.000	5.750	0.000	-117.300	0.000	0.000	0.000	-33.724
0.134	0.000	0.000	0.000	0.000	0.000	0.000	6.900	0.000	-175.950#	0.000	0.000	0.000	-202.342#
0.200	0.000	0.000	0.000	0.000	0.000	0.000	Memb 34, Case 1 (Linear): DL						
0.267	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	175.950*	0.000	0.000	0.000	-202.342#
0.334	0.000	0.000	0.000	0.000	0.000	0.000	1.150	0.000	117.300	0.000	0.000	0.000	-33.724
0.401	0.000	0.000	0.000	0.000	0.000*	0.000*	2.300	0.000	58.650	0.000	0.000	0.000	67.447
Memb 24, Case 1 (Linear): DL							3.450	0.000	0.000	0.000	0.000	0.000	101.171*
0.000	0.250	0.000	0.003	0.000	-0.009#	0.000*	4.600	0.000	-58.650	0.000	0.000	0.000	67.447
1.152	0.250	0.000	0.003	0.000	-0.006	0.000	5.750	0.000	-117.300	0.000	0.000	0.000	-33.724
2.304	0.250	0.000	0.003	0.000	-0.003	0.000	6.900	0.000	-175.950#	0.000	0.000	0.000	-202.342#
3.456	0.250	0.000	0.003	0.000	0.000	0.000	Memb 35, Case 1 (Linear): DL						
4.608	0.250	0.000	0.003	0.000	0.003	0.000	0.000	3.393	0.008	-0.401	0.037	1.516*	-0.029#
5.760	0.250	0.000	0.003	0.000	0.006	0.000	0.897	3.393	0.008	-0.401	0.037	1.157	-0.022
6.913	0.250	0.000	0.003	0.000	0.009*	0.000#	1.793	3.393	0.008	-0.401	0.037	0.797	-0.015
Memb 25, Case 1 (Linear): DL							2.690	3.393	0.008	-0.401	0.037	0.437	-0.008
0.000	0.000	0.000	0.000	0.000	0.000	0.000#	3.586	3.393	0.008	-0.401	0.037	0.078	0.000
0.067	0.000	0.000	0.000	0.000	0.000	0.000	4.483	3.393	0.008	-0.401	0.037	-0.282	0.007
0.134	0.000	0.000	0.000	0.000	0.000	0.000	5.379	3.393	0.008	-0.401	0.037	-0.642#	0.014*
0.200	0.000	0.000	0.000	0.000	0.000	0.000	Memb 36, Case 1 (Linear): DL						
0.267	0.000	0.000	0.000	0.000	0.000	0.000	0.000	4.038	-0.167	0.356	-0.407	-1.203#	0.605*
0.334	0.000	0.000	0.000	0.000	0.000	0.000	0.897	4.038	-0.167	0.356	-0.407	-0.884	0.455
0.401	0.000	0.000	0.000	0.000	0.000*	0.000*	1.793	4.038	-0.167	0.356	-0.407	-0.564	0.306
Memb 26, Case 1 (Linear): DL							2.690	4.038	-0.167	0.356	-0.407	-0.244	0.156
0.000	0.000	63.621*	0.000	0.000	0.000	-25.462	3.586	4.038	-0.167	0.356	-0.407	0.075	0.007
0.433	0.000	40.793	0.000	0.000	0.000	-2.839	4.483	4.038	-0.167	0.356	-0.407	0.395	-0.142
0.867	0.000	17.965	0.000	0.000	0.000	9.892	5.379	4.038	-0.167	0.356	-0.407	0.714*	-0.292#
1.300	0.000	-4.863	0.000	0.000	0.000	12.731*	Memb 37, Case 1 (Linear): DL						
1.733	0.000	-27.691	0.000	0.000	0.000	5.678	0.000	14.141	-5.972	-0.481	-0.590	1.795*	25.016*
2.167	0.000	-50.519	0.000	0.000	0.000	-11.268	0.897	14.141	-5.972	-0.481	-0.590	1.364	19.662
2.600	0.000	-73.347#	0.000	0.000	0.000	-38.105#	1.793	14.141	-5.972	-0.481	-0.590	0.933	14.308
Memb 27, Case 1 (Linear): DL							2.690	14.141	-5.972	-0.481	-0.590	0.501	8.954
0.000	0.000	84.288*	0.000	0.000	0.000	-44.954#	3.586	14.141	-5.972	-0.481	-0.590	0.070	3.600
0.533	0.000	56.192	0.000	0.000	0.000	-7.492	4.483	14.141	-5.972	-0.481	-0.590	-0.361	-1.754
1.067	0.000	28.096	0.000	0.000	0.000	14.985	5.379	14.141	-5.972	-0.481	-0.590	-0.792#	-7.108#
1.600	0.000	0.000	0.000	0.000	0.000	22.477*	Memb 39, Case 1 (Linear): DL						
2.133	0.000	-28.096	0.000	0.000	0.000	14.985	0.000	2.889	-0.306	0.805	1.215	-2.841#	1.107*
2.667	0.000	-56.192	0.000	0.000	0.000	-7.492	0.897	2.889	-0.306	0.805	1.215	-2.119	0.832
3.200	0.000	-84.288#	0.000	0.000	0.000	-44.954#	1.793	2.889	-0.306	0.805	1.215	-1.398	0.558
Memb 28, Case 1 (Linear): DL							2.690	2.889	-0.306	0.805	1.215	-0.676	0.283

















Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment	Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
2.733	-1.214	0.104	2.537	1.046	1.220	0.183	Memb 54, Case 3 (Linear): WL						
3.417	-1.214	0.104	2.537	1.046	2.954	0.254	0.000	-2.524	-0.257	-1.067	-0.015	1.563*	0.338*
4.100	-1.214	0.104	2.537	1.046	4.687*	0.325*	0.683	-2.524	-0.257	-1.067	-0.015	0.834	0.162
Memb 44, Case 3 (Linear): WL							1.367	-2.524	-0.257	-1.067	-0.015	0.105	-0.013
0.000	0.130	0.002	-0.093	-0.017	0.210*	-0.007#	2.050	-2.524	-0.257	-1.067	-0.015	-0.624	-0.189
0.717	0.130	0.002	-0.093	-0.017	0.143	-0.005	2.733	-2.524	-0.257	-1.067	-0.015	-1.353	-0.364
1.433	0.130	0.002	-0.093	-0.017	0.076	-0.004	3.417	-2.524	-0.257	-1.067	-0.015	-2.082	-0.539
2.150	0.130	0.002	-0.093	-0.017	0.010	-0.002	4.100	-2.524	-0.257	-1.067	-0.015	-2.811#	-0.715#
2.867	0.130	0.002	-0.093	-0.017	-0.057	0.000	Memb 55, Case 3 (Linear): WL						
3.583	0.130	0.002	-0.093	-0.017	-0.124	0.001	0.000	12.819	-0.002	2.060	-0.040	-0.552#	-0.008*
4.300	0.130	0.002	-0.093	-0.017	-0.191#	0.003*	0.433	12.819	-0.002	2.060	-0.040	0.341	-0.010
Memb 45, Case 3 (Linear): WL							0.867	12.819	-0.002	2.060	-0.040	1.234	-0.011
0.000	-0.416	-0.094	0.132	0.000	-0.275#	0.325*	1.300	12.819	-0.002	2.060	-0.040	2.127	-0.012
0.967	-0.416	-0.094	0.132	0.000	-0.147	0.234	1.733	12.819	-0.002	2.060	-0.040	3.020	-0.013
1.933	-0.416	-0.094	0.132	0.000	-0.020	0.143	2.167	12.819	-0.002	2.060	-0.040	3.912	-0.014
2.900	-0.416	-0.094	0.132	0.000	0.108	0.052	2.600	12.819	-0.002	2.060	-0.040	4.805*	-0.015#
3.867	-0.416	-0.094	0.132	0.000	0.235	-0.039	Memb 57, Case 3 (Linear): WL						
4.833	-0.416	-0.094	0.132	0.000	0.362	-0.131	0.000	0.161	2.375	0.001	0.004	0.000#	0.082#
5.800	-0.416	-0.094	0.132	0.000	0.490*	-0.222#	0.069	0.161	2.375	0.001	0.004	0.000	0.247
Memb 46, Case 3 (Linear): WL							0.139	0.161	2.375	0.001	0.004	0.000	0.412
0.000	1.459	-0.147	-3.449	0.013	5.802*	0.229*	0.208	0.161	2.375	0.001	0.004	0.000	0.577
0.533	1.459	-0.147	-3.449	0.013	3.963	0.151	0.278	0.161	2.375	0.001	0.004	0.000	0.742
1.067	1.459	-0.147	-3.449	0.013	2.124	0.072	0.347	0.161	2.375	0.001	0.004	0.000	0.907
1.600	1.459	-0.147	-3.449	0.013	0.285	-0.006	0.417	0.161	2.375	0.001	0.004	0.000*	1.072*
2.133	1.459	-0.147	-3.449	0.013	-1.555	-0.084	Memb 58, Case 3 (Linear): WL						
2.667	1.459	-0.147	-3.449	0.013	-3.394	-0.163	0.000	-0.184	-0.312	-0.035	0.000	0.056*	0.507*
3.200	1.459	-0.147	-3.449	0.013	-5.233#	-0.241#	0.533	-0.184	-0.312	-0.035	0.000	0.037	0.340
Memb 47, Case 3 (Linear): WL							1.067	-0.184	-0.312	-0.035	0.000	0.018	0.173
0.000	3.377	-0.071	-0.598	0.014	0.968*	0.173*	1.600	-0.184	-0.312	-0.035	0.000	-0.001	0.007
0.817	3.377	-0.071	-0.598	0.014	0.480	0.115	2.133	-0.184	-0.312	-0.035	0.000	-0.020	-0.160
1.633	3.377	-0.071	-0.598	0.014	-0.008	0.057	2.667	-0.184	-0.312	-0.035	0.000	-0.039	-0.326
2.450	3.377	-0.071	-0.598	0.014	-0.496	-0.002	3.200	-0.184	-0.312	-0.035	0.000	-0.057#	-0.493#
3.267	3.377	-0.071	-0.598	0.014	-0.985	-0.060	Memb 59, Case 3 (Linear): WL						
4.083	3.377	-0.071	-0.598	0.014	-1.473	-0.118	0.000	-0.098	-0.011	-0.005	0.000	0.015*	0.020*
4.900	3.377	-0.071	-0.598	0.014	-1.961#	-0.176#	0.817	-0.098	-0.011	-0.005	0.000	0.010	0.011
Memb 48, Case 3 (Linear): WL							1.633	-0.098	-0.011	-0.005	0.000	0.006	0.002
0.000	2.046	-0.135	2.561	-0.026	-5.578#	0.248*	2.450	-0.098	-0.011	-0.005	0.000	0.001	-0.007
0.683	2.046	-0.135	2.561	-0.026	-3.828	0.156	3.267	-0.098	-0.011	-0.005	0.000	0.001	-0.007
1.367	2.046	-0.135	2.561	-0.026	-2.078	0.064	4.083	-0.098	-0.011	-0.005	0.000	-0.008	-0.025
2.050	2.046	-0.135	2.561	-0.026	-0.328	-0.028	4.900	-0.098	-0.011	-0.005	0.000	-0.012#	-0.034#
2.733	2.046	-0.135	2.561	-0.026	1.422	-0.120	Memb 60, Case 3 (Linear): WL						
3.417	2.046	-0.135	2.561	-0.026	3.172	-0.212	0.000	-0.263	0.173	0.027	0.000	-0.055#	-0.353#
4.100	2.046	-0.135	2.561	-0.026	4.921*	-0.304#	0.683	-0.263	0.173	0.027	0.000	-0.037	-0.234
Memb 49, Case 3 (Linear): WL							1.367	-0.263	0.173	0.027	0.000	-0.019	-0.116
0.000	2.649	0.007	-1.912	-0.015	3.323*	-0.025#	2.050	-0.263	0.173	0.027	0.000	-0.001	0.003
0.433	2.649	0.007	-1.912	-0.015	2.494	-0.022	2.733	-0.263	0.173	0.027	0.000	0.017	0.122
0.867	2.649	0.007	-1.912	-0.015	1.665	-0.019	3.417	-0.263	0.173	0.027	0.000	0.036	0.240
1.300	2.649	0.007	-1.912	-0.015	0.837	-0.016	4.100	-0.263	0.173	0.027	0.000	0.054*	0.359*
1.733	2.649	0.007	-1.912	-0.015	0.008	-0.013	Memb 61, Case 3 (Linear): WL						
2.167	2.649	0.007	-1.912	-0.015	-0.820	-0.010	0.000	0.000	1.714	-1.330	0.000	0.133*	-0.005#
2.600	2.649	0.007	-1.912	-0.015	-1.649#	-0.007*	0.033	0.000	1.714	-1.330	0.000	0.089	0.052
Memb 50, Case 3 (Linear): WL							0.067	0.000	1.714	-1.330	0.000	0.044	0.109
0.000	7.246	0.301	1.330	0.462	-5.891#	-1.237#	0.100	0.000	1.714	-1.330	0.000	0.000	0.166
0.683	7.246	0.301	1.330	0.462	-4.982	-1.031	0.133	0.000	1.714	-1.330	0.000	-0.044	0.223
1.367	7.246	0.301	1.330	0.462	-4.073	-0.825	0.167	0.000	1.714	-1.330	0.000	-0.089	0.281
2.050	7.246	0.301	1.330	0.462	-3.164	-0.620	0.200	0.000	1.714	-1.330	0.000	-0.133#	0.338*
2.733	7.246	0.301	1.330	0.462	-2.255	-0.414	Memb 62, Case 3 (Linear): WL						
3.417	7.246	0.301	1.330	0.462	-1.346	-0.208	0.000	-0.132	-0.001	-0.269	-0.024	0.376*	0.003*
4.100	7.246	0.301	1.330	0.462	-0.438*	-0.002*	0.433	-0.132	-0.001	-0.269	-0.024	0.259	0.002
Memb 51, Case 3 (Linear): WL							0.867	-0.132	-0.001	-0.269	-0.024	0.143	0.002
0.000	6.467	0.185	-0.860	-0.294	3.577*	-0.759#	1.300	-0.132	-0.001	-0.269	-0.024	0.026	0.001
0.683	6.467	0.185	-0.860	-0.294	2.989	-0.633	1.733	-0.132	-0.001	-0.269	-0.024	-0.090	0.001
1.367	6.467	0.185	-0.860	-0.294	2.401	-0.507	2.167	-0.132	-0.001	-0.269	-0.024	-0.207	0.000
2.050	6.467	0.185	-0.860	-0.294	1.813	-0.381	2.600	-0.132	-0.001	-0.269	-0.024	-0.323#	0.000#
2.733	6.467	0.185	-0.860	-0.294	1.225	-0.255	Memb 63, Case 3 (Linear): WL						
3.417	6.467	0.185	-0.860	-0.294	0.637	-0.129	0.000	0.353	-0.948	-0.194	-0.313	0.539*	2.983*
4.100	6.467	0.185	-0.860	-0.294	0.049#	-0.002*	0.897	0.353	-0.948	-0.194	-0.313	0.365	2.133
Memb 52, Case 3 (Linear): WL							1.793	0.353	-0.948	-0.194	-0.313	0.191	1.283
0.000	-3.645	0.329	1.565	-0.016	-4.757#	-0.928#	2.690	0.353	-0.948	-0.194	-0.313	0.017	0.433
0.683	-3.645	0.329	1.565	-0.016	-3.687	-0.703	3.586	0.353	-0.948	-0.194	-0.313	-0.157	-0.417
1.367	-3.645	0.329	1.565	-0.016	-2.618	-0.478	4.483	0.353	-0.948	-0.194	-0.313	-0.331	-1.266
2.050	-3.645	0.329	1.565	-0.016	-1.548	-0.254	5.379	0.353	-0.948	-0.194	-0.313	-0.505#	-2.116#
2.733	-3.645	0.329	1.565	-0.016	-0.478	-0.029	Memb 67, Case 3 (Linear): WL						
3.417	-3.645	0.329	1.565	-0.016	0.591	0.196	0.000	0.100	0.000	0.000	0.000	0.000	0.000
4.100	-3.645	0.329	1.565	-0.016	1.661*	0.421*	0.967	0.100	0.000	0.000	0.000	0.000	0.000
Memb 53, Case 3 (Linear): WL							1.933	0.100	0.000	0.000	0.000	0.000	0.000
0.000	-2.381	0.018	-0.001	0.000	-0.001*	-0.017#	2.900	0.100	0.000	0.000	0.000	0.000	0.000
0.534	-2.381	0.018	-0.001	0.000	-0.001	-0.008	3.867	0.100	0.000	0.000	0.000	0.000	0.000
1.069	-2.381	0.018	-0.001	0.000	-0.002	0.002	4.833	0.100	0.000	0.000	0.000	0.000	0.000
1.603	-2.381	0.018	-0.001	0.000	-0.003	0.011	5.800	0.100	0.000	0.000	0.000	0.000	0.000
2.137	-2.381	0.018	-0.001	0.000	-0.003	0.020							
2.672	-2.381	0.018	-0.001	0.000	-0.004	0.030							
3.206	-2.381	0.018	-0.001	0.000	-0.005#	0.039*							

















Memb 73, Case 6 (Linear): DL +1.3 LL + 1.3WL

Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment	Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
0.000	1.794	7.819*	0.006	0.006	-0.019#	0.000	0.278	-0.072	-1.173	0.004	0.012	0.001	-0.326
0.817	1.794	5.212	0.006	0.006	-0.014	5.321	0.347	-0.072	-1.173	0.004	0.012	0.001	-0.407
1.633	1.794	2.606	0.006	0.006	-0.010	8.514	0.417	-0.072	-1.173	0.004	0.012	0.001*	-0.489#
2.450	1.794	0.000	0.006	0.006	-0.005	9.578*	Memb 6, Case 7 (Linear): DL + LL + EQL						
3.267	1.794	-2.606	0.006	0.006	0.000	8.514	Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
4.083	1.794	-5.212	0.006	0.006	0.005	5.321	0.000	0.000	0.000	0.000	0.000	0.000*	0.000#
4.900	1.794	-7.819#	0.006	0.006	0.010*	0.000#	0.067	0.000	0.000	0.000	0.000	0.000	0.000

Memb 74, Case 6 (Linear): DL +1.3 LL + 1.3WL

Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment	Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
0.000	0.822	6.542*	-0.084	-0.042	0.175*	0.000	0.000	0.000	0.000	0.000	0.000	0.000*	0.000#
0.683	0.822	4.361	-0.084	-0.042	0.118	3.725	0.067	0.000	0.000	0.000	0.000	0.000	0.000
1.367	0.822	2.181	-0.084	-0.042	0.061	5.961	0.134	0.000	0.000	0.000	0.000	0.000	0.000
2.050	0.822	0.000	-0.084	-0.042	0.003	6.706*	0.200	0.000	0.000	0.000	0.000	0.000	0.000
2.733	0.822	-2.181	-0.084	-0.042	-0.054	5.961	0.267	0.000	0.000	0.000	0.000	0.000	0.000
3.417	0.822	-4.361	-0.084	-0.042	-0.111	3.725	0.334	0.000	0.000	0.000	0.000	0.000	0.000
4.100	0.822	-6.542#	-0.084	-0.042	-0.168#	0.000#	0.401	0.000	0.000	0.000	0.000	0.000	0.000*

Memb 75, Case 6 (Linear): DL +1.3 LL + 1.3WL

Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment	Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
0.000	-16.555	0.000	2.212	-3.042	-0.249#	0.000	0.000	3.161	0.005	-0.472	-1.659	1.080*	0.000#
0.433	-16.555	0.000	2.212	-3.042	0.709	0.000	0.897	3.161	0.005	-0.472	-1.659	0.657	0.000
0.867	-16.555	0.000	2.212	-3.042	1.658	0.000	1.793	3.161	0.005	-0.472	-1.659	0.234	0.009
1.300	-16.555	0.000	2.212	-3.042	2.626	0.000	2.690	3.161	0.005	-0.472	-1.659	-0.189	0.014
1.733	-16.555	0.000	2.212	-3.042	3.585	0.000	3.586	3.161	0.005	-0.472	-1.659	-0.613	0.019
2.167	-16.555	0.000	2.212	-3.042	4.543	0.000	4.483	3.161	0.005	-0.472	-1.659	-1.036	0.023
2.600	-16.555	0.000	2.212	-3.042	5.502*	0.000	5.379	3.161	0.005	-0.472	-1.659	-1.459#	0.028*

Memb 76, Case 6 (Linear): DL +1.3 LL + 1.3WL

Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment	Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
0.000	-7.204	0.000	-3.370	3.370	0.591*	0.000	0.000	0.000	0.000	0.000	0.000	0.000*	0.000*
0.433	-7.204	0.000	-3.370	3.370	-0.869	0.000	0.067	0.000	0.000	0.000	0.000	0.000	0.000
0.867	-7.204	0.000	-3.370	3.370	-2.329	0.000	0.134	0.000	0.000	0.000	0.000	0.000	0.000
1.300	-7.204	0.000	-3.370	3.370	-3.789	0.000	0.200	0.000	0.000	0.000	0.000	0.000	0.000
1.733	-7.204	0.000	-3.370	3.370	-5.249	0.000	0.267	0.000	0.000	0.000	0.000	0.000	0.000
2.167	-7.204	0.000	-3.370	3.370	-6.710	0.000	0.334	0.000	0.000	0.000	0.000	0.000	0.000
2.600	-7.204	0.000	-3.370	3.370	-8.170#	0.000	0.401	0.000	0.000	0.000	0.000	0.000#	0.000#

Memb 77, Case 6 (Linear): DL +1.3 LL + 1.3WL

Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment	Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
0.000	0.188	-0.005	-0.003	0.000	0.000*	0.009*	0.000	1.176	-0.001	-0.004	0.000	0.012*	0.000*
0.434	0.188	-0.005	-0.003	0.000	-0.001	0.007	0.718	1.176	-0.001	-0.004	0.000	0.009	-0.001
0.868	0.188	-0.005	-0.003	0.000	-0.003	0.004	1.436	1.176	-0.001	-0.004	0.000	0.007	-0.002
1.302	0.188	-0.005	-0.003	0.000	-0.004	0.002	2.154	1.176	-0.001	-0.004	0.000	0.004	-0.003
1.736	0.188	-0.005	-0.003	0.000	-0.005	0.000	2.872	1.176	-0.001	-0.004	0.000	0.001	-0.004
2.171	0.188	-0.005	-0.003	0.000	-0.006	-0.002	3.590	1.176	-0.001	-0.004	0.000	-0.001	-0.005
2.605	0.188	-0.005	-0.003	0.000	-0.007#	-0.004#	4.308	1.176	-0.001	-0.004	0.000	-0.004#	-0.006#

Memb 78, Case 6 (Linear): DL +1.3 LL + 1.3WL

Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment	Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
0.000	0.045	0.819	0.000	0.002	0.000#	-0.013#	0.000	0.000	0.000	0.000	0.000	0.000#	0.000*
0.026	0.045	0.819	0.000	0.002	0.000	0.009	0.067	0.000	0.000	0.000	0.000	0.000	0.000
0.052	0.045	0.819	0.000	0.002	0.000	0.030	0.134	0.000	0.000	0.000	0.000	0.000	0.000
0.079	0.045	0.819	0.000	0.002	0.000	0.052	0.200	0.000	0.000	0.000	0.000	0.000	0.000
0.105	0.045	0.819	0.000	0.002	0.000	0.073	0.267	0.000	0.000	0.000	0.000	0.000	0.000
0.131	0.045	0.819	0.000	0.002	0.000	0.095	0.334	0.000	0.000	0.000	0.000	0.000	0.000
0.157	0.045	0.819	0.000	0.002	0.000*	0.116*	0.401	0.000	0.000	0.000	0.000	0.000*	0.000#

Memb 1, Case 7 (Linear): DL + LL + EQL

Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment	Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
0.000	0.000	0.000	-0.001	-0.003	0.000*	0.000*	0.000	-1.906	-31.589	0.006	0.019	-0.002#	0.000*
0.069	0.000	0.000	-0.001	-0.003	0.000	0.000	0.069	-1.906	-31.589	0.006	0.019	-0.001	-2.194
0.139	0.000	0.000	-0.001	-0.003	0.000	0.000	0.139	-1.906	-31.589	0.006	0.019	-0.001	-4.388
0.208	0.000	0.000	-0.001	-0.003	0.000	0.000	0.208	-1.906	-31.589	0.006	0.019	0.000	-6.581
0.278	0.000	0.000	-0.001	-0.003	0.000	0.000	0.278	-1.906	-31.589	0.006	0.019	0.000	-8.775
0.347	0.000	0.000	-0.001	-0.003	0.000	0.000	0.347	-1.906	-31.589	0.006	0.019	0.000	-10.969
0.417	0.000	0.000	-0.001	-0.003	0.000#	0.000#	0.417	-1.906	-31.589	0.006	0.019	0.001*	-13.163#

Memb 2, Case 7 (Linear): DL + LL + EQL

Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment	Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
0.000	2.370	0.000	-0.773	-0.166	2.171*	0.000*	0.000	5.267	4.362	-0.901	-3.476	2.638*	0.000#
0.897	2.370	0.000	-0.773	-0.166	1.478	0.000	0.897	5.267	4.362	-0.901	-3.476	1.830	3.911
1.793	2.370	0.000	-0.773	-0.166	0.785	-0.001	1.793	5.267	4.362	-0.901	-3.476	1.021	7.822
2.690	2.370	0.000	-0.773	-0.166	0.092	-0.001	2.690	5.267	4.362	-0.901	-3.476	0.213	11.733
3.586	2.370	0.000	-0.773	-0.166	-0.601	-0.001	3.586	5.267	4.362	-0.901	-3.476	-0.595	15.644
4.483	2.370	0.000	-0.773	-0.166	-1.293	-0.002	4.483	5.267	4.362	-0.901	-3.476	-1.403	19.555
5.379	2.370	0.000	-0.773	-0.166	-1.986#	-0.002#	5.379	5.267	4.362	-0.901	-3.476	-2.211#	23.466*

Memb 3, Case 7 (Linear): DL + LL + EQL

Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment	Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
0.000	0.000	0.000	0.000	0.000	0.000*	0.000*	0.000	0.000	0.000	0.000	0.000	0.000	0.000#
0.067	0.000	0.000	0.000	0.000	0.000	0.000	0.067	0.000	0.000	0.000	0.000	0.000	0.000
0.134	0.000	0.000	0.000	0.000	0.000	0.000	0.134	0.000	0.000	0.000	0.000	0.000	0.000
0.200	0.000	0.000	0.000	0.000	0.000	0.000	0.200	0.000	0.000	0.000	0.000	0.000	0.000
0.267	0.000	0.000	0.000	0.000	0.000	0.000	0.267	0.000	0.000	0.000	0.000	0.000	0.000
0.334	0.000	0.000	0.000	0.000	0.000	0.000	0.334	0.000	0.000	0.000	0.000	0.000	0.000
0.401													



Memb 39, Case 7 (Linear): DL + LL + EQL

Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment	Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
							1.733	-7.966	-0.160	5.261	0.067	0.036	0.118
							2.167	-7.966	-0.160	5.261	0.067	2.316	0.048
0.000	-3.022	-1.317	0.787	5.011	-2.297#	4.760*	2.600	-7.966	-0.160	5.261	0.067	4.596*	-0.021#
0.897	-3.022	-1.317	0.787	5.011	-1.591	3.579							
1.793	-3.022	-1.317	0.787	5.011	-0.886	2.398							
2.690	-3.022	-1.317	0.787	5.011	-0.180	1.217							
3.586	-3.022	-1.317	0.787	5.011	0.525	0.036							
4.483	-3.022	-1.317	0.787	5.011	1.230	-1.145							
5.379	-3.022	-1.317	0.787	5.011	1.936*	-2.326#							

Memb 50, Case 7 (Linear): DL + LL + EQL

Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
0.000	-16.417	32.593	-3.927	-34.101	17.294*	-136.175#
0.683	-16.417	32.593	-3.927	-34.101	14.610	-113.903
1.367	-16.417	32.593	-3.927	-34.101	11.926	-91.631
2.050	-16.417	32.593	-3.927	-34.101	9.243	-69.359
2.733	-16.417	32.593	-3.927	-34.101	6.559	-47.087
3.417	-16.417	32.593	-3.927	-34.101	3.875	-24.815
4.100	-16.417	32.593	-3.927	-34.101	1.192#	-2.542*

Memb 40, Case 7 (Linear): DL + LL + EQL

Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
0.000	-0.237	3.536*	-0.430	0.183	0.778*	-1.050
0.967	-0.237	2.289	-0.430	0.183	0.362	1.766
1.933	-0.237	1.042	-0.430	0.183	-0.054	3.376
2.900	-0.237	-0.205	-0.430	0.183	-0.470	3.781*
3.867	-0.237	-1.452	-0.430	0.183	-0.885	2.981
4.833	-0.237	-2.699	-0.430	0.183	-1.301	0.375
5.800	-0.237	-3.946#	-0.430	0.183	-1.717#	-2.237#

Memb 51, Case 7 (Linear): DL + LL + EQL

Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
0.000	-24.831	35.519	2.612	31.048	-11.028#	-148.448#
0.683	-24.831	35.519	2.612	31.048	-9.244	-124.177
1.367	-24.831	35.519	2.612	31.048	-7.459	-99.906
2.050	-24.831	35.519	2.612	31.048	-5.674	-75.335
2.733	-24.831	35.519	2.612	31.048	-3.890	-51.364
3.417	-24.831	35.519	2.612	31.048	-2.105	-27.093
4.100	-24.831	35.519	2.612	31.048	-0.320*	-2.822*

Memb 41, Case 7 (Linear): DL + LL + EQL

Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
0.000	3.568	-6.249*	11.816	16.511	-19.201#	3.164*
0.533	3.568	-6.937	11.816	16.511	-12.899	-0.352
1.067	3.568	-7.625	11.816	16.511	-6.597	-4.235
1.600	3.568	-8.313	11.816	16.511	-0.295	-8.485
2.133	3.568	-9.001	11.816	16.511	6.006	-13.101
2.667	3.568	-9.689	11.816	16.511	12.308	-18.085
3.200	3.568	-10.377#	11.816	16.511	18.610*	-23.436#

Memb 52, Case 7 (Linear): DL + LL + EQL

Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
0.000	11.898	-4.555	-5.148	-1.187	15.591*	12.847*
0.683	11.898	-4.555	-5.148	-1.187	12.073	9.735
1.367	11.898	-4.555	-5.148	-1.187	8.556	6.622
2.050	11.898	-4.555	-5.148	-1.187	5.038	3.509
2.733	11.898	-4.555	-5.148	-1.187	1.521	0.396
3.417	11.898	-4.555	-5.148	-1.187	-1.997	-2.717
4.100	11.898	-4.555	-5.148	-1.187	-5.514#	-5.830#

Memb 42, Case 7 (Linear): DL + LL + EQL

Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
0.000	9.722	4.826*	0.930	2.467	-1.273#	2.097#
0.817	9.722	3.772	0.930	2.467	-0.513	5.608
1.633	9.722	2.719	0.930	2.467	0.247	8.259
2.450	9.722	1.665	0.930	2.467	1.007	10.049
3.267	9.722	0.612	0.930	2.467	1.767	10.979
4.083	9.722	-0.442	0.930	2.467	2.526	11.049*
4.900	9.722	-1.495#	0.930	2.467	3.286*	10.258

Memb 53, Case 7 (Linear): DL + LL + EQL

Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
0.000	38.864	-0.269	0.004	0.000	0.002#	0.255*
0.534	38.864	-0.269	0.004	0.000	0.004	0.111
1.069	38.864	-0.269	0.004	0.000	0.006	-0.033
1.603	38.864	-0.269	0.004	0.000	0.008	-0.177
2.137	38.864	-0.269	0.004	0.000	0.010	-0.321
2.672	38.864	-0.269	0.004	0.000	0.012	-0.465
3.206	38.864	-0.269	0.004	0.000	0.014*	-0.609#

Memb 43, Case 7 (Linear): DL + LL + EQL

Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
0.000	3.719	4.487*	-6.854	-15.684	15.386*	-9.909#
0.683	3.719	3.605	-6.854	-15.684	10.703	-7.144
1.367	3.719	2.724	-6.854	-15.684	6.019	-4.981
2.050	3.719	1.842	-6.854	-15.684	1.335	-3.421
2.733	3.719	0.961	-6.854	-15.684	-3.349	-2.464
3.417	3.719	0.079	-6.854	-15.684	-8.032	-2.108*
4.100	3.719	-0.802#	-6.854	-15.684	-12.716#	-2.355

Memb 54, Case 7 (Linear): DL + LL + EQL

Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
0.000	7.116	3.761	2.921	-0.061	-4.311#	-4.935#
0.683	7.116	3.761	2.921	-0.061	-2.315	-2.365
1.367	7.116	3.761	2.921	-0.061	-0.319	0.205
2.050	7.116	3.761	2.921	-0.061	1.678	2.775
2.733	7.116	3.761	2.921	-0.061	3.674	5.345
3.417	7.116	3.761	2.921	-0.061	5.670	7.914
4.100	7.116	3.761	2.921	-0.061	7.666*	10.484*

Memb 44, Case 7 (Linear): DL + LL + EQL

Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
0.000	-0.424	-0.070	0.297	0.216	-0.672#	0.162*
0.717	-0.424	-0.070	0.297	0.216	-0.459	0.111
1.433	-0.424	-0.070	0.297	0.216	-0.246	0.061
2.150	-0.424	-0.070	0.297	0.216	-0.033	0.010
2.867	-0.424	-0.070	0.297	0.216	0.180	-0.040
3.583	-0.424	-0.070	0.297	0.216	0.393	-0.091
4.300	-0.424	-0.070	0.297	0.216	0.607*	-0.141#

Memb 55, Case 7 (Linear): DL + LL + EQL

Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
0.000	-11.685	0.152	-6.685	0.600	1.684*	-0.187#
0.433	-11.685	0.152	-6.685	0.600	-1.213	-0.121
0.867	-11.685	0.152	-6.685	0.600	-4.110	-0.055
1.300	-11.685	0.152	-6.685	0.600	-7.007	0.010
1.733	-11.685	0.152	-6.685	0.600	-9.904	0.076
2.167	-11.685	0.152	-6.685	0.600	-12.801	0.142
2.600	-11.685	0.152	-6.685	0.600	-15.698#	0.207*

Memb 45, Case 7 (Linear): DL + LL + EQL

Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
0.000	1.684	2.425*	-0.443	-0.002	0.932*	-2.219#
0.967	1.684	1.671	-0.443	-0.002	0.504	-0.239
1.933	1.684	0.917	-0.443	-0.002	0.075	1.012
2.900	1.684	0.163	-0.443	-0.002	-0.353	1.534*
3.867	1.684	-0.591	-0.443	-0.002	-0.782	1.327
4.833	1.684	-1.345	-0.443	-0.002	-1.211	0.392
5.800	1.684	-2.099#	-0.443	-0.002	-1.639#	-1.273

Memb 57, Case 7 (Linear): DL + LL + EQL

Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
0.000	-2.612	-38.777	-0.004	-0.012	0.001*	-1.253*
0.069	-2.612	-38.777	-0.004	-0.012	0.001	-3.946
0.139	-2.612	-38.777	-0.004	-0.012	0.001	-6.639
0.208	-2.612	-38.777	-0.004	-0.012	0.000	-9.332
0.278	-2.612	-38.777	-0.004	-0.012	0.000	-12.025
0.347	-2.612	-38.777	-0.004	-0.012	0.000	-14.718
0.417	-2.612	-38.777	-0.004	-0.012	0.000#	-17.411#

Memb 46, Case 7 (Linear): DL + LL + EQL

Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
0.000	-4.534	1.214*	11.393	-0.202	-19.008#	-1.754
0.533	-4.534	0.798	11.393	-0.202	-12.932	-1.217
1.067	-4.534	0.382	11.393	-0.202	-6.855	-0.902
1.600	-4.534	-0.034	11.393	-0.202	-0.779	-0.809*
2.133	-4.534	-0.450	11.393	-0.202	5.297	-0.939
2.667	-4.534	-0.866	11.393	-0.202	11.373	-1.289
3.2						

Memb 61, Case 7 (Linear): DL + LL + EQL								Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment	
Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment									
0.000	0.000	-24.343	3.681	0.000	-0.368#	-0.066*	1.733	-25.095	0.000	3.740	-3.074	6.170	0.000		
0.033	0.000	-24.343	3.681	0.000	-0.245	-0.877	2.167	-25.095	0.000	3.740	-3.074	7.791	0.000		
0.067	0.000	-24.343	3.681	0.000	-0.123	-1.689	2.600	-25.095	0.000	3.740	-3.074	9.411*	0.000		
0.100	0.000	-24.343	3.681	0.000	0.000	-2.500									
0.133	0.000	-24.343	3.681	0.000	0.123	-3.311									
0.167	0.000	-24.343	3.681	0.000	0.245	-4.123									
0.200	0.000	-24.343	3.681	0.000	0.368*	-4.934#									
Memb 62, Case 7 (Linear): DL + LL + EQL								Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment	
0.000	0.443	0.055	0.910	-0.049	-1.271#	-0.141#	0.000	-16.748	0.000	-6.093	3.437	1.136*	0.000		
0.433	0.443	0.055	0.910	-0.049	-0.877	-0.117	0.433	-16.748	0.000	-6.093	3.437	-1.504	0.000		
0.867	0.443	0.055	0.910	-0.049	-0.482	-0.093	0.867	-16.748	0.000	-6.093	3.437	-4.144	0.000		
1.300	0.443	0.055	0.910	-0.049	-0.088	-0.069	1.300	-16.748	0.000	-6.093	3.437	-6.785	0.000		
1.733	0.443	0.055	0.910	-0.049	0.307	-0.046	1.733	-16.748	0.000	-6.093	3.437	-9.425	0.000		
2.167	0.443	0.055	0.910	-0.049	0.702	-0.022	2.167	-16.748	0.000	-6.093	3.437	-12.065	0.000		
2.600	0.443	0.055	0.910	-0.049	1.096*	0.002*	2.600	-16.748	0.000	-6.093	3.437	-14.705#	0.000		
Memb 63, Case 7 (Linear): DL + LL + EQL								Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment	
0.000	38.889	-21.731	3.388	1.060	-10.882#	112.886*	0.000	-0.146	-0.008	-0.004	0.000	0.000*	0.015*		
0.897	38.889	-21.731	3.388	1.060	-7.845	93.404	0.434	-0.146	-0.008	-0.004	0.000	-0.002	0.011		
1.793	38.889	-21.731	3.388	1.060	-4.808	73.922	0.868	-0.146	-0.008	-0.004	0.000	-0.004	0.007		
2.690	38.889	-21.731	3.388	1.060	-1.771	54.440	1.302	-0.146	-0.008	-0.004	0.000	-0.006	0.004		
3.586	38.889	-21.731	3.388	1.060	1.266	34.958	1.736	-0.146	-0.008	-0.004	0.000	-0.008	0.000		
4.483	38.889	-21.731	3.388	1.060	4.304	15.476	2.171	-0.146	-0.008	-0.004	0.000	-0.010	-0.003		
5.379	38.889	-21.731	3.388	1.060	7.341*	-4.006#	2.605	-0.146	-0.008	-0.004	0.000	-0.012#	-0.007#		
Memb 67, Case 7 (Linear): DL + LL + EQL								Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment	
0.000	-0.236	9.255*	0.000	0.000	0.000	0.000#	0.000	0.073	1.319	0.001	0.003	-0.001#	-0.021#		
0.967	-0.236	6.170	0.000	0.000	0.000	7.455	0.026	0.073	1.319	0.001	0.003	-0.001	0.014		
1.933	-0.236	3.085	0.000	0.000	0.000	11.928	0.052	0.073	1.319	0.001	0.003	-0.001	0.048		
2.900	-0.236	0.000	0.000	0.000	0.000	13.419*	0.079	0.073	1.319	0.001	0.003	-0.001	0.083		
3.867	-0.236	-3.085	0.000	0.000	0.000	11.928	0.105	0.073	1.319	0.001	0.003	-0.001	0.117		
4.833	-0.236	-6.170	0.000	0.000	0.000	7.455	0.131	0.073	1.319	0.001	0.003	-0.001	0.152		
5.800	-0.236	-9.255#	0.000	0.000	0.000	0.000	0.157	0.073	1.319	0.001	0.003	0.000*	0.186*		
Memb 68, Case 7 (Linear): DL + LL + EQL								Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment	
0.000	0.000	-27.042	-5.706	0.000	0.571*	-0.421*	0.000	0.000	0.001	-0.001	-0.002	0.000*	0.000#		
0.033	0.000	-27.042	-5.706	0.000	0.380	-1.322	0.069	0.000	0.001	-0.001	-0.002	0.000	0.000		
0.067	0.000	-27.042	-5.706	0.000	0.190	-2.224	0.139	0.000	0.001	-0.001	-0.002	0.000	0.000		
0.100	0.000	-27.042	-5.706	0.000	0.000	-3.125	0.208	0.000	0.001	-0.001	-0.002	0.000	0.000		
0.133	0.000	-27.042	-5.706	0.000	-0.190	-4.027	0.278	0.000	0.001	-0.001	-0.002	0.000	0.000		
0.167	0.000	-27.042	-5.706	0.000	-0.380	-4.928	0.347	0.000	0.001	-0.001	-0.002	0.000	0.000		
0.200	0.000	-27.042	-5.706	0.000	-0.571#	-5.829#	0.417	0.000	0.001	-0.001	-0.002	0.000*	0.000*		
Memb 70, Case 7 (Linear): DL + LL + EQL								Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment	
0.000	-16.521	-12.925	-3.662	3.437	-0.672*	0.042*	0.000	2.308	0.000	-0.682	-0.136	1.951*	0.000*		
0.033	-16.521	-12.925	-3.662	3.437	-0.798	-0.388	0.897	2.308	0.000	-0.682	-0.136	1.340	0.000		
0.067	-16.521	-12.925	-3.662	3.437	-0.916	-0.819	1.793	2.308	0.000	-0.682	-0.136	0.729	-0.001		
0.100	-16.521	-12.925	-3.662	3.437	-1.038	-1.250	2.690	2.308	0.000	-0.682	-0.136	0.118	-0.001		
0.133	-16.521	-12.925	-3.662	3.437	-1.160	-1.681	3.586	2.308	0.000	-0.682	-0.136	-0.493	-0.001		
0.167	-16.521	-12.925	-3.662	3.437	-1.282	-2.112	4.483	2.308	0.000	-0.682	-0.136	-1.105	-0.001		
0.200	-16.521	-12.925	-3.662	3.437	-1.404#	-2.543#	5.379	2.308	0.000	-0.682	-0.136	-1.716#	-0.002#		
Memb 71, Case 7 (Linear): DL + LL + EQL								Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment	
0.000	0.744	5.106*	0.248	0.048	-0.391#	0.000	0.000	0.000	0.000	0.000	0.000	0.000#	0.000*		
0.533	0.744	3.404	0.248	0.048	-0.259	2.269	0.067	0.000	0.000	0.000	0.000	0.000	0.000		
1.067	0.744	1.702	0.248	0.048	-0.126	3.631	0.134	0.000	0.000	0.000	0.000	0.000	0.000		
1.600	0.744	0.000	0.248	0.048	0.006	4.085*	0.200	0.000	0.000	0.000	0.000	0.000	0.000		
2.133	0.744	-1.702	0.248	0.048	0.138	3.631	0.267	0.000	0.000	0.000	0.000	0.000	0.000		
2.667	0.744	-3.404	0.248	0.048	0.271	2.269	0.334	0.000	0.000	0.000	0.000	0.000	0.000		
3.200	0.744	-5.106#	0.248	0.048	0.403*	0.000#	0.401	0.000	0.000	0.000	0.000	0.000*	0.000#		
Memb 72, Case 7 (Linear): DL + LL + EQL								Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment	
0.000	-24.914	-14.361	2.145	-3.074	0.023#	0.050*	0.000	-0.001	0.000	0.001	0.000	-0.002#	0.000*		
0.033	-24.914	-14.361	2.145	-3.074	0.095	-0.428	1.152	-0.001	0.000	0.001	0.000	-0.002	0.000		
0.067	-24.914	-14.361	2.145	-3.074	0.166	-0.907	2.304	-0.001	0.000	0.001	0.000	-0.001	0.000		
0.100	-24.914	-14.361	2.145	-3.074	0.238	-1.386	3.456	-0.001	0.000	0.001	0.000	0.000	0.000		
0.133	-24.914	-14.361	2.145	-3.074	0.309	-1.864	4.608	-0.001	0.000	0.001	0.000	0.000	0.000		
0.167	-24.914	-14.361	2.145	-3.074	0.381	-2.343	5.760	-0.001	0.000	0.001	0.000	0.001	0.000		
0.200	-24.914	-14.361	2.145	-3.074	0.452*	-2.822#	6.913	-0.001	0.000	0.001	0.000	0.002*	0.000#		
Memb 73, Case 7 (Linear): DL + LL + EQL								Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment	
0.000	3.174	7.819*	0.021	0.006	-0.061#	0.000	0.000	-0.059	-0.962	0.003	0.010	0.000#	0.000*		
0.817	3.174	5.212	0.021	0.006	-0.044	5.321	0.069	-0.059	-0.962	0.003	0.010	0.000	-0.067		
1.633	3.174	2.606	0.021	0.006	-0.026	8.514	0.139	-0.059	-0.962	0.003	0.010	0.000	-0.134		
2.450	3.174	0.000	0.021	0.006	-0.009	9.578*	0.208	-0.059	-0.962	0.003	0.010	0.000	-0.200		
3.267	3.174	-2.606	0.021	0.006	0.009	8.514	0.278	-0.059	-0.962	0.003	0.010	0.000	-0.267		
4.083	3.174	-5.212	0.021	0.006	0.026	5.321	0.347	-0.059	-0.962	0.003	0.010	0.001	-0.334		
4.900	3.174	-7.819#	0.021	0.006	0.044*	0.000#	0.417	-0.059	-0.962	0.003	0.010	0.001*	-0.401#		
Memb 74, Case 7 (Linear): DL + LL + EQL								Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment	
0.000	1.580	6.542*	-0.159	-0.045	0.332*	0.000	0.000	0.000	0.000	0.000	0.000	0.000#	0.000*		
0.683	1.580	4.361	-0.159	-0.045	0.223	3.725	0.067	0.000	0.000	0.000	0.000	0.000	0.000		
1.367	1.580	2.181	-0.159	-0.045	0.115	5.961	0.134	0.000	0.000	0.000	0.000	0.000	0.000		
2.050	1.580	0.000	-0.159	-0.045	0.006	6.706*	0.200	0.000	0.000	0.000	0.000	0.000	0.000		
2.733	1.580	-2.181	-0.159	-0.045	-0.103	5.961	0.267	0.000	0.000	0.000	0.000	0.000	0.000		
3.417	1.580	-4.361	-0.159	-0.045	-0.212	3.725	0.334	0.000	0.000	0.000	0.000	0.000	0.000		
4.100	1.580	-6.542#	-0.159	-0.045	-0.321#	0.000#	0.401	0.000	0.000	0.000	0.000	0.000*	0.000#		
Memb 75, Case 7 (Linear): DL + LL + EQL								Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment	
0.000	-25.095	0.000	3.740	-3.074	-0.312#	0.000	0.000	3.291	0.004	-0.368	-1.362	0.818*	0.000#		
0.433	-25.095	0.000	3.740	-3.074	1.309	0.000	0.897	3.291	0.004	-0.368	-1.362	0.487	0.004		
0.867	-25.095	0.000	3.												



Memb 30, Case 8 (Linear): DL + LL

Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment	Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
							2.133	3.041	-7.294	9.728	13.580	4.943	-10.595
							2.667	3.041	-7.982	9.728	13.580	10.131	-14.668
0.000	0.000	81.909*	0.000	0.000	0.000	-38.496	3.200	3.041	-8.670#	9.728	13.580	15.319*	-19.109#
0.533	0.000	53.525	0.000	0.000	0.000	-2.380							
1.067	0.000	25.141	0.000	0.000	0.000	18.597							
1.600	0.000	-3.243	0.000	0.000	0.000	24.437*							
2.133	0.000	-31.627	0.000	0.000	0.000	15.138							
2.667	0.000	-60.011	0.000	0.000	0.000	-9.299							
3.200	0.000	-88.395#	0.000	0.000	0.000	-48.874#							

Memb 42, Case 8 (Linear): DL + LL

Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
0.000	8.109	4.730*	0.778	2.065	-1.077#	0.580#
0.817	8.109	3.677	0.778	2.065	-0.441	4.013
1.633	8.109	2.623	0.778	2.065	0.194	6.585
2.450	8.109	1.570	0.778	2.065	0.829	8.297
3.267	8.109	0.516	0.778	2.065	1.464	9.149*
4.083	8.109	-0.537	0.778	2.065	2.100	9.141
4.900	8.109	-1.591#	0.778	2.065	2.735*	8.272

Memb 31, Case 8 (Linear): DL + LL

Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
0.000	-1.182	110.882*	-8.321	-2.224	8.222*	-73.175#
0.333	-1.182	93.142	-8.321	-2.224	5.448	-39.171
0.667	-1.182	75.402	-8.321	-2.224	2.675	-11.081
1.000	-1.182	57.662	-8.321	-2.224	-0.099	11.096
1.333	-1.182	39.922	-8.321	-2.224	-2.873	27.360
1.667	-1.182	22.182	-8.321	-2.224	-5.647	37.711
2.000	-1.182	4.442#	-8.321	-2.224	-8.420#	42.148*

Memb 43, Case 8 (Linear): DL + LL

Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
0.000	3.204	3.783*	-5.649	-12.941	12.681*	-7.189#
0.683	3.204	2.902	-5.649	-12.941	8.821	-4.905
1.367	3.204	2.020	-5.649	-12.941	4.961	-3.223
2.050	3.204	1.139	-5.649	-12.941	1.101	-2.144
2.733	3.204	0.257	-5.649	-12.941	-2.759	-1.667*
3.417	3.204	-0.624	-5.649	-12.941	-6.619	-1.792
4.100	3.204	-1.506#	-5.649	-12.941	-10.479#	-2.520

Memb 32, Case 8 (Linear): DL + LL

Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
0.000	0.000	82.841*	-2.955	1.085	5.122*	-53.650#
0.433	0.000	59.779	-2.955	1.085	3.841	-22.749
0.867	0.000	36.717	-2.955	1.085	2.561	-1.841
1.300	0.000	13.655	-2.955	1.085	1.280	9.073
1.733	0.000	-9.407	-2.955	1.085	0.000	9.994*
2.167	0.000	-32.469	-2.955	1.085	-1.280	0.921
2.600	0.000	-55.531#	-2.955	1.085	-2.561#	-18.146

Memb 44, Case 8 (Linear): DL + LL

Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
0.000	-0.347	-0.064	0.240	0.199	-0.544#	0.141*
0.717	-0.347	-0.064	0.240	0.199	-0.372	0.095
1.433	-0.347	-0.064	0.240	0.199	-0.200	0.049
2.150	-0.347	-0.064	0.240	0.199	-0.028	0.004
2.867	-0.347	-0.064	0.240	0.199	0.144	-0.042
3.583	-0.347	-0.064	0.240	0.199	0.316	-0.087
4.300	-0.347	-0.064	0.240	0.199	0.488*	-0.133#

Memb 33, Case 8 (Linear): DL + LL

Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
0.000	0.000	175.950*	0.000	0.000	0.000	-202.342#
1.150	0.000	117.300	0.000	0.000	0.000	-33.724
2.300	0.000	58.650	0.000	0.000	0.000	67.447
3.450	0.000	0.000	0.000	0.000	0.000	101.171*
4.600	0.000	-58.650	0.000	0.000	0.000	67.447
5.750	0.000	-117.300	0.000	0.000	0.000	-33.724
6.900	0.000	-175.950#	0.000	0.000	0.000	-202.342#

Memb 45, Case 8 (Linear): DL + LL

Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
0.000	1.430	2.359*	-0.364	-0.001	0.766*	-2.014#
0.967	1.430	1.605	-0.364	-0.001	0.414	-0.097
1.933	1.430	0.851	-0.364	-0.001	0.062	1.090
2.900	1.430	0.097	-0.364	-0.001	-0.290	1.549*
3.867	1.430	-0.657	-0.364	-0.001	-0.642	1.278
4.833	1.430	-1.411	-0.364	-0.001	-0.994	0.279
5.800	1.430	-2.165#	-0.364	-0.001	-1.346#	-1.449

Memb 34, Case 8 (Linear): DL + LL

Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
0.000	0.000	175.950*	0.000	0.000	0.000	-202.342#
1.150	0.000	117.300	0.000	0.000	0.000	-33.724
2.300	0.000	58.650	0.000	0.000	0.000	67.447
3.450	0.000	0.000	0.000	0.000	0.000	101.171*
4.600	0.000	-58.650	0.000	0.000	0.000	67.447
5.750	0.000	-117.300	0.000	0.000	0.000	-33.724
6.900	0.000	-175.950#	0.000	0.000	0.000	-202.342#

Memb 46, Case 8 (Linear): DL + LL

Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
0.000	-3.722	1.279*	9.374	-0.168	-15.641#	-1.718#
0.533	-3.722	0.863	9.374	-0.168	-10.642	-1.147
1.067	-3.722	0.447	9.374	-0.168	-5.643	-0.797
1.600	-3.722	0.031	9.374	-0.168	-0.643	-0.670*
2.133	-3.722	-0.385	9.374	-0.168	4.356	-0.764
2.667	-3.722	-0.801	9.374	-0.168	9.355	-1.081
3.200	-3.722	-1.217#	9.374	-0.168	14.354*	-1.619

Memb 35, Case 8 (Linear): DL + LL

Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
0.000	0.000	175.950*	0.000	0.000	0.000	-202.342#
1.150	0.000	117.300	0.000	0.000	0.000	-33.724
2.300	0.000	58.650	0.000	0.000	0.000	67.447
3.450	0.000	0.000	0.000	0.000	0.000	101.171*
4.600	0.000	-58.650	0.000	0.000	0.000	67.447
5.750	0.000	-117.300	0.000	0.000	0.000	-33.724
6.900	0.000	-175.950#	0.000	0.000	0.000	-202.342#

Memb 47, Case 8 (Linear): DL + LL

Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
0.000	-7.997	2.030*	0.763	-0.176	-0.643#	-1.105#
0.817	-7.997	1.393	0.763	-0.176	-0.021	0.293
1.633	-7.997	0.756	0.763	-0.176	0.602	1.170
2.450	-7.997	0.119	0.763	-0.176	1.225	1.528*
3.267	-7.997	-0.518	0.763	-0.176	1.848	1.365
4.083	-7.997	-1.155	0.763	-0.176	2.471	0.682
4.900	-7.997	-1.792#	0.763	-0.176	3.094*	-0.521

Memb 36, Case 8 (Linear): DL + LL

Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
0.000	-4.400	-0.674	1.118	-1.541	-3.518#	2.440*
0.897	-4.400	-0.674	1.118	-1.541	-2.516	1.836
1.793	-4.400	-0.674	1.118	-1.541	-1.513	1.233
2.690	-4.400	-0.674	1.118	-1.541	-0.511	0.629
3.586	-4.400	-0.674	1.118	-1.541	0.491	0.025
4.483	-4.400	-0.674	1.118	-1.541	1.494	-0.579
5.379	-4.400	-0.674	1.118	-1.541	2.496*	-1.183#

Memb 48, Case 8 (Linear): DL + LL

Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
0.000	-4.673	2.586*	-5.702	0.328	12.358*	-3.344#
0.683	-4.673	2.053	-5.702	0.328	8.461	-1.759
1.367	-4.673	1.520	-5.702	0.328	4.565	-0.538
2.050	-4.673	0.987	-5.702	0.328	0.668	0.319
2.733	-4.673	0.454	-5.702	0.328	-3.229	0.811
3.417	-4.673	-0.079	-5.702	0.328	-7.125	0.940*
4.100	-4.673	-0.612#	-5.702	0.328	-11.022#	0.704

Memb 37, Case 8 (Linear): DL + LL

Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
0.000	38.563	-21.422	-1.820	-2.113	7.006*	89.595*
0.897	38.563	-21.422	-1.820	-2.113	5.374	70.390
1.793	38.563	-21.422	-1.820	-2.113	3.743	51.185
2.690	38.563	-21.422	-1.820	-2.113	2.111	31.980
3.586	38.563	-21.422	-1.820	-2.113	0.479	12.775
4.483	38.563	-21.422	-1.820	-2.113	-1.152	-6.430
5.379	38.563	-21.422	-1.820	-2.113		

Memb 52, Case 8 (Linear): DL + LL

Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
0.000	9.791	-3.767	-4.257	-1.027	12.855*	10.589*
0.683	9.791	-3.767	-4.257	-1.027	9.945	8.014
1.367	9.791	-3.767	-4.257	-1.027	7.036	5.440
2.050	9.791	-3.767	-4.257	-1.027	4.127	2.866
2.733	9.791	-3.767	-4.257	-1.027	1.218	0.291
3.417	9.791	-3.767	-4.257	-1.027	-1.691	-2.283
4.100	9.791	-3.767	-4.257	-1.027	-4.600#	-4.857#

Memb 63, Case 8 (Linear): DL + LL

Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
0.000	32.473	-17.934	2.758	0.876	-8.716#	93.158*
0.683	32.473	-17.934	2.758	0.876	-6.243	77.080
1.367	32.473	-17.934	2.758	0.876	-3.770	61.002
2.050	32.473	-17.934	2.758	0.876	-1.297	44.924
2.733	32.473	-17.934	2.758	0.876	1.176	28.847
3.417	32.473	-17.934	2.758	0.876	3.648	12.769
4.100	32.473	-17.934	2.758	0.876	6.121*	-3.309#

Memb 53, Case 8 (Linear): DL + LL

Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
0.000	32.107	-0.223	0.003	0.000	0.002#	0.210*
0.534	32.107	-0.223	0.003	0.000	0.003	0.091
1.069	32.107	-0.223	0.003	0.000	0.005	-0.028
1.603	32.107	-0.223	0.003	0.000	0.007	-0.147
2.137	32.107	-0.223	0.003	0.000	0.008	-0.266
2.672	32.107	-0.223	0.003	0.000	0.010	-0.385
3.206	32.107	-0.223	0.003	0.000	0.011*	-0.504#

Memb 67, Case 8 (Linear): DL + LL

Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
0.000	-0.171	8.504*	0.000	0.000	0.000	0.000#
0.967	-0.171	5.670	0.000	0.000	0.000	6.851
1.933	-0.171	2.835	0.000	0.000	0.000	10.961
2.900	-0.171	0.000	0.000	0.000	0.000	12.331*
3.867	-0.171	-2.835	0.000	0.000	0.000	10.961
4.833	-0.171	-5.669	0.000	0.000	0.000	6.851
5.800	-0.171	-8.504#	0.000	0.000	0.000	0.000

Memb 54, Case 8 (Linear): DL + LL

Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
0.000	5.863	3.111	2.403	-0.100	-3.543#	-4.094#
0.683	5.863	3.111	2.403	-0.100	-1.900	-1.968
1.367	5.863	3.111	2.403	-0.100	-0.258	0.159
2.050	5.863	3.111	2.403	-0.100	1.384	2.284
2.733	5.863	3.111	2.403	-0.100	3.027	4.410
3.417	5.863	3.111	2.403	-0.100	4.669	6.536
4.100	5.863	3.111	2.403	-0.100	6.312*	8.662*

Memb 68, Case 8 (Linear): DL + LL

Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
0.000	0.000	-22.215	-4.723	0.000	0.472*	-0.414*
0.033	0.000	-22.215	-4.723	0.000	0.315	-1.155
0.067	0.000	-22.215	-4.723	0.000	0.157	-1.895
0.100	0.000	-22.215	-4.723	0.000	0.000	-2.636
0.133	0.000	-22.215	-4.723	0.000	0.000	-3.376
0.167	0.000	-22.215	-4.723	0.000	-0.315	-4.117
0.200	0.000	-22.215	-4.723	0.000	-0.472#	-4.857#

Memb 55, Case 8 (Linear): DL + LL

Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
0.000	-9.611	0.152	-5.517	0.549	1.420*	-0.223#
0.433	-9.611	0.152	-5.517	0.549	-0.971	-0.156
0.867	-9.611	0.152	-5.517	0.549	-3.361	-0.090
1.300	-9.611	0.152	-5.517	0.549	-5.752	-0.024
1.733	-9.611	0.152	-5.517	0.549	-8.143	0.041
2.167	-9.611	0.152	-5.517	0.549	-10.533	0.107
2.600	-9.611	0.152	-5.517	0.549	-12.924#	0.173*

Memb 70, Case 8 (Linear): DL + LL

Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
0.000	-13.591	-11.877	-3.021	2.702	-0.543*	0.035*
0.033	-13.591	-11.877	-3.021	2.702	-0.644	-0.361
0.067	-13.591	-11.877	-3.021	2.702	-0.745	-0.757
0.100	-13.591	-11.877	-3.021	2.702	-0.845	-1.153
0.133	-13.591	-11.877	-3.021	2.702	-0.946	-1.549
0.167	-13.591	-11.877	-3.021	2.702	-1.047	-1.945
0.200	-13.591	-11.877	-3.021	2.702	-1.147#	-2.341#

Memb 57, Case 8 (Linear): DL + LL

Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
0.000	-2.158	-32.035	-0.003	-0.010	0.001*	-1.036*
0.069	-2.158	-32.035	-0.003	-0.010	0.001	-3.261
0.139	-2.158	-32.035	-0.003	-0.010	0.000	-5.485
0.208	-2.158	-32.035	-0.003	-0.010	0.000	-7.710
0.278	-2.158	-32.035	-0.003	-0.010	0.000	-9.935
0.347	-2.158	-32.035	-0.003	-0.010	0.000	-12.160
0.417	-2.158	-32.035	-0.003	-0.010	0.000#	-14.384#

Memb 71, Case 8 (Linear): DL + LL

Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
0.000	0.624	4.692*	0.204	0.040	-0.322#	0.000
0.533	0.624	3.128	0.204	0.040	-0.213	2.085
1.067	0.624	1.564	0.204	0.040	-0.104	3.337
1.600	0.624	0.000	0.204	0.040	0.005	3.754*
2.133	0.624	-1.564	0.204	0.040	0.114	3.337
2.667	0.624	-3.128	0.204	0.040	0.223	2.085
3.200	0.624	-4.692#	0.204	0.040	0.332*	0.000#

Memb 58, Case 8 (Linear): DL + LL

Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
0.000	0.466	4.156	0.093	0.000	-0.147#	-7.035#
0.533	0.466	4.156	0.093	0.000	-0.098	-4.818
1.067	0.466	4.156	0.093	0.000	-0.048	-2.602
1.600	0.466	4.156	0.093	0.000	0.002	-0.385
2.133	0.466	4.156	0.093	0.000	0.052	1.832
2.667	0.466	4.156	0.093	0.000	0.102	4.049
3.200	0.466	4.156	0.093	0.000	0.152*	6.265*

Memb 72, Case 8 (Linear): DL + LL

Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
0.000	-20.596	-13.196	1.763	-2.394	0.027#	0.042*
0.033	-20.596	-13.196	1.763	-2.394	0.086	-0.398
0.067	-20.596	-13.196	1.763	-2.394	0.144	-0.838
0.100	-20.596	-13.196	1.763	-2.394	0.203	-1.278
0.133	-20.596	-13.196	1.763	-2.394	0.262	-1.718
0.167	-20.596	-13.196	1.763	-2.394	0.321	-2.158
0.200	-20.596	-13.196	1.763	-2.394	0.379*	-2.598#

Memb 59, Case 8 (Linear): DL + LL

Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
0.000	0.244	19.010*	0.008	0.000	-0.023#	-23.131#
0.817	0.244	19.010*	0.008	0.000	-0.017	-7.606
1.633	0.244	19.010*	0.008	0.000	-0.010	7.919
2.450	0.244	19.010*	0.008	0.000	-0.003	23.444*
2.450	0.244	-18.490#	0.008	0.000	-0.003	23.444*
3.267	0.244	-18.490#	0.008	0.000	0.003	8.344
4.083	0.244	-18.490#	0.008	0.000	0.010	-6.756
4.900	0.244	-18.490#	0.008	0.000	0.017*	-21.857

Memb 73, Case 8 (Linear): DL + LL

Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
0.000	2.620	7.185*	0.018	0.005	-0.051#	0.000
0.817	2.620	4.790	0.018	0.005	-0.036	4.890
1.633	2.620	2.395	0.018	0.005	-0.022	7.823
2.450	2.620	0.000	0.018	0.005	-0.007	8.801*
3.267	2.620	-2.395	0.018	0.005	0.008	7.823
4.083	2.620	-4.790	0.018	0.005	0.022	4.890
4.900	2.620	-7.185#	0.018	0.005	0.037*	0.000#

Memb 60, Case 8 (Linear): DL + LL

Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
0.000	0.625	-2.430	-0.060	0.000	0.125*	4.713*
0.683	0.625	-2.430	-0.060	0.000	0.084	3.053
1.367	0.625	-2.430	-0.060	0.000	0.043	1.393
2.050	0.625	-2.430	-0.060	0.000	0.002	-0.267
2.733	0.625	-2.430	-0.060	0.000	-0.039	-1.928
3.417	0.625	-2.430	-0.060	0.000	-0.080	-3.588
4.100	0.625	-2.430	-0.060	0.000	-0.121#	-5.248#

Memb 74, Case 8 (Linear): DL + LL

Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
0.000	1.304	6.012*	-0.131	-0.037	0.274*	0.000#
0.683	1.304	4.008	-0.131	-0.037	0.184	3.423
1.367	1.304	2.004	-0.131	-0.037	0.094	5.477
2.050	1.304	0.000	-0.131	-0.037	0.005	6.162*
2.733	1.304	-2.004	-0.131	-0.037	-0.085	5.477
3.417	1.304	-4.008	-0.131	-0.037	-0.175	3.423
4.100	1.304	-6.012#	-0.131	-0.037	-0.265#	0.000

Memb 61, Case 8 (Linear): DL + LL

Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
0.000	0.000	-20.138	3.028	0.000	-0.303#	-0.066*
0.033	0.000	-20.138	3.028	0.000	-0.202	-0.737
0.067	0.000	-20.138	3.028	0.000	-0.101	-1.409
0.100	0.000	-20.138	3.028	0.000	0.000	-2.080
0.133	0.000	-20.138	3.028	0.000	0.101	-2.751
0.167	0.000	-20.138	3.028	0.000	0.202	-3.423
0.200	0.000	-20.138	3.028	0.000	0.303*	-4.094#

Memb 75, Case 8 (Linear): DL + LL

Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
1.736	-0.123	-0.007	-0.004	0.000	-0.006	0.000
2.171	-0.123	-0.007	-0.004	0.000	-0.008	-0.003
2.605	-0.123	-0.007	-0.004	0.000	-0.010#	-0.006#

Memb 78, Case 8 (Linear): DL + LL

Station Location	Axial Force	Y-Axis Shear	Z-Axis Shear	X-Axis Torsion	Y-Axis Moment	Z-Axis Moment
0.000	0.060	1.085	0.001	0.003	-0.001#	-0.017#
0.026	0.060	1.085	0.001	0.003	0.000	0.011
0.052	0.060	1.085	0.001	0.003	0.000	0.040
0.079	0.060	1.085	0.001	0.003	0.000	0.068
0.105	0.060	1.085	0.001	0.003	0.000	0.096
0.131	0.060	1.085	0.001	0.003	0.000	0.125
0.157	0.060	1.085	0.001	0.003	0.000*	0.153*



### **3 DESIGN OF MEMBERS**

## SUSPENDED SLAB

LOCAL CONTROL ROOM & BLOWER ROOM

Designed as a Continuous Beam :

REF.

AS 3600

### 1. Loads

DL	:	Wdl	=	1.00 kN/m	(plus super)
Self wt	:	Wsw	=	6.88 kN/m	
Blockwall	:	Wb	=	0.00 kN/m	
TOTAL	:		=	7.88 kN/m	
LL	:	Wll	=	2 kN/m	
LL factor - long term				0.4	
- short term				0.7	

### 2. Beam Section Properties

bw	1000	mm
D	275	mm
tff	0	mm
Cover	65	mm
D = Overall depth of section	275	mm
Actual length, L	3250	mm
Effective Length, L <sub>ef</sub>	2850	mm
Area, A	2.75E+05	mm <sup>2</sup>
Neutral Axis, y <sub>c</sub>	137.50	mm
Moment of Inertia, I <sub>x-x</sub>	1.73E+09	mm <sup>4</sup>
Neutral Axis, x <sub>c</sub>	785.00	mm
Moment of Inertia, I <sub>y-y</sub>	2.29E+10	mm <sup>4</sup>
Effective width, b <sub>eff</sub>	1455.00	mm
Gross Area, A <sub>g</sub>	2.75E+05	mm <sup>2</sup>

### 3. Design Parameters

F' <sub>c</sub> =	40	MPa
F <sub>sy</sub> =	410	MPa
m	2400	kg/m <sup>3</sup>
D =	275	mm
E <sub>c</sub> =	31975.3505	MPa
E <sub>s</sub> =	200000	MPa
Gamma =	0.766	

### 4. INITIAL CHOICE OF SECTION

Effective width =	1570 mm	clause 8.8.2
Is the section RECT or T, L (ans: 1 or 2)	=	1
k <sub>1</sub> =	0.045	(RECT sections)
=	0.03183953	(T and L sections)
what type of span?	6	interior span =
		end span =
		6
k <sub>2</sub> =	0.0054054	
k <sub>cs</sub> =	1.64	(because there will be comp steel at midspan=say .5A <sub>st</sub> )
F <sub>def</sub> =	15.627	
defln/L <sub>ef</sub> =	0.004	k <sub>1</sub> /k <sub>2</sub> = 8.32500833
d =	60 mm	
take d =	190 mm	Distance from d to extreme fibre of beam (d <sub>o</sub> ) =
		85 mm

**5. Forces from an Elastic Analysis by Space Gass :**

Mmax -ve	=	72.00 kNm	(left support)
Mmax +ve	=	7.40 kNm	(mid-span)
M -ve	=	12.00 kNm	(right support)
Vmax	=	49.00 kN	
V (other end)	=	6.00 kN	

**6. FLECTURAL STRENGTH STEEL**

Determine -ve steel required: (At arbitrary left support)

Number of reinforcement layers	1 or 2 :	1	
take do =	85 mm	d =	190
Estimate Area of main steel	Ast =	1208.18546 mm <sup>2</sup>	Say
5 Y 16	Ast =	1005.30965 mm <sup>2</sup>	176.4729903
314 CTS	p =	0.0052911	
4 Y 12	Asc =	452.389342 mm <sup>2</sup>	
	pc =	0.002381	< pd = 0.025409
	p-pc =	0.00291011	pd = 0.025409
	Astmin =	1044.05 mm <sup>2</sup>	Governs
	Spacing =	302.348767 mm	
	Y16 at	300 CTS	

**Check M\* <= phi Mu**

ku =	0.045813	dsc =	83 mm
------	----------	-------	-------

$$\phi \text{ Mu} = \phi [ F_{sy} \times A_{sc} (d - d_{sc}) + 0.85 \times F'c \times b \times \Gamma \times k_u \times d \times (d - 0.5 \times 0.85 \times k_u \times d) ]$$

=	79 kNm	>	M* =	72.00 kNm	OK
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Determine -ve steel at right support:

M -ve	=	12.00 kNm	
Number of reinforcement layers	1 or 2 :	1	
take do =	93 mm	dst =	182 mm
Estimate Area of main steel	Ast =	210.215418 mm <sup>2</sup>	
5 Y 16	Ast =	1005.30965 mm <sup>2</sup>	
314 CTS	p =	0.00552368	
5 Y 12	Asc =	565.486678 mm <sup>2</sup>	
314	pc =	0.00310707	< pd = 0.025409
	p-pc =	0.00241661	
	Astmin =	1044.05 mm <sup>2</sup>	
	Spacing =	302.348767 mm	
	Y16 at	300 CTS	

**Check M\* <= phi Mu**

ku =	0.038044	dsc =	83 mm
------	----------	-------	-------

$$\phi \text{ Mu} = \phi [ F_{sy} \times A_{sc} (d - d_{sc}) + 0.85 \times F'c \times b \times \Gamma \times k_u \times d \times (d - 0.5 \times 0.85 \times k_u \times d) ]$$

=	67 kNm	>	M* =	12.00 kNm	OK
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FOR DUCTILITY CHECK AT BOTH SUPPORTS USE EMPHI

**Tensile Steel at Midspan:**

M*	=	7.4 kNm	
Steel in one layer then	code = 1	take dist between centroid of	
" " two " "	code = 2	bar/s and soffit =	93 mm
what is the code?	1	d =	190 mm
Ast =	M*/(Phi x 0.85 x d x Fsy)	=	124.174616 mm <sup>2</sup>
Bottom	3 Y 16	Ast =	603.185789 mm <sup>2</sup>
Top	3 Y 16	Asc =	603.185789 mm <sup>2</sup>
Assume	N - A in flange		
Effective width :	b <sub>eff</sub> =	1570 mm	
ku =	1/(0.85 x 0.85) x [Ast/(bw x d)] x (Fsy/F'c)		
=	0.03183274		

hence  $d_n = 6 <$  flange thickness  
hence N-A is in flange

Ast min Governs - Use Y1 6at 300 CTS

$$\Phi \mu = 0.9 [F_{sy} \times A_{st} \times d (1 - 0.5 \times 0.85 \times k_u)]$$

=	42 kNm	>	M*+ve =	7.40 kNm	OK
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Summary of reinforcement requirement :

left support :	A <sub>st</sub>	=	Y16		300 CTS
	A <sub>sc</sub>	=	Y16		300 CTS
Right support :	A <sub>st</sub>	=	Y16		300 CTS
	A <sub>sc</sub>	=	Y16		300 CTS
midspan	A <sub>st</sub>	=	Y16		300 CTS
	A <sub>sc</sub>	=	Y16		300 CTS

### 7. STEEL CURTAILMENT REQUIREMENT CHECK

The following requirements regarding curtailment are subject to stress development length requirements.

- For -ve steel
- i) 1/4 A<sub>st</sub> to continue over length of beam = 2 bars
  - ii) Remainder to be curtailed at 0.3L<sub>n</sub> from face of support.
    - = 0.3 x l<sub>e</sub> = 855 mm ?
- For -ve steel
- i) 1/4 A<sub>st</sub> to continue over length of beam = 2 bars
  - ii) Remainder to be curtailed at 0.3L<sub>n</sub> from face of support.
    - = 0.3 x l<sub>e</sub> = 855 mm ?
- For +ve steel :
- i) 1/2 A<sub>st</sub> must be carried into outer support = 2 bars
  - ii) 1/4 A<sub>st</sub> into interior support = 2 bars
  - iii) Remainder curtailed at 0.1L<sub>n</sub> from supports
    - = 0.1 x l<sub>e</sub> = 285 mm

### 8. STRESS DEVELOPMENT

For Top Steel:

cover : c = 65 mm

d<sub>b</sub> = 16 mm # of lines of bars = 2

a<sub>b</sub> = 423 mm distance between bars

If a<sub>b</sub> > 2c C = 2c + d<sub>b</sub> = 146 mm  
C is the outside diameter of a concrete annulus

if a<sub>b</sub> <= 2c C = a<sub>b</sub> + d<sub>b</sub> = 439 mm  
coaxial with and surrounding a bar

k<sub>1</sub> = 1.25 A<sub>b</sub> = 201.06193 mm<sup>2</sup>

k<sub>2</sub> = 2.2 a<sub>b</sub> = 423 mm (dist between bars)

factor = 35843.9945

L<sub>syt</sub> = k<sub>1</sub> x k<sub>2</sub> x f<sub>sy</sub> x A<sub>b</sub>/C x sqrt(f'<sub>c</sub>) = factor/C

= 245.5068 mm

take fs/F<sub>sy</sub> = M\*/Phi Mu = 0.908715771

L <sub>sy</sub> =	223 mm	≥	25k1db	500
say =	250 mm			

For Bottom Steel:

Cover, c = 65 mm d<sub>b</sub> = 16 mm

# of lines of bars = 2

If a<sub>b</sub> > 2c C = 2c + d<sub>b</sub> = 146 mm

if a<sub>b</sub> < 2c C = a<sub>b</sub> + d<sub>b</sub> = 593 mm

k<sub>1</sub> = 1 A<sub>b</sub> = 201.06193 mm<sup>2</sup>

k<sub>2</sub> = 2.2 a<sub>b</sub> = 577 mm (dist between bars)

factor = 28675.1956

L<sub>syt</sub> = 196.4054 mm take fs/F<sub>sy</sub> = M\*/Phi Mu = 0.177144668

L <sub>sy</sub> =	35 mm	≥	25k1db	400
say =	50 mm			



## 10. SERVICEABILITY CHECK

From an Elastic Linear Analysis :

Mr	=	72.00 kNm	Es	=	200000 MPa
MI	=	12.00 kNm	Ec	=	31975.35051 MPa
Mo	=	49.4 kNm			
Ms	=	7.40 kNm			
Ig	=	1.73E+09 mm <sup>4</sup>			

Calculate Icr, Mcr, and Ief at midspan :

Eff. Flange width	b <sub>eff</sub> =	1570 mm	A <sub>sc</sub> =	603.1857895 mm <sup>2</sup>
webwidth	b <sub>w</sub> =	1000 mm	A <sub>st</sub> =	603.1857895 mm <sup>2</sup>
Flange thickness	t <sub>ff</sub> =	0 mm	d <sub>sc</sub> =	93 mm
			d <sub>st</sub> =	190 mm
			d =	190 mm
			D =	275 mm

$$n = E_s/E_c = 6.25481807$$

$$1/2 \times b_w \times d_n^2 + (b_{eff} - b_w) \times t \times (d_n - t/2) + (n-1) A_{sc} (d_n - d_{sc}) = n A_{st} (d - d_n)$$

$$500 d_n^2 + 6942.44896 d_n - 1011611.04 = 0$$

$$d_n = 38.5704028 \text{ mm}$$

$$d = 190 \text{ mm}$$

$$I_{cr} = b d_n^3/3 + 1/12 (b_{eff} - b_w) \times t^3 + (b_{eff} - b_w) \times t \times (d_n - t/2)^2 + n x A_{st} (d - d_n)^2$$

$$= 1.06E+08 \text{ mm}^4$$

$$M_{cr} = 0.6 \sqrt{F'_c} \times b D^2/6 = 47.8294496 \text{ kNm}$$

$$I_m = [I_{cr} + (I_g - I_{cr}) (M_{cr} / M_s)^3]$$

$$= 4.40E+11 \text{ mm}^4$$

Calculate Icr, Mcr, Ilef and Iref at end supports:

b <sub>w</sub> =	1000 mm	A <sub>st</sub> =	1005.30965 mm <sup>2</sup>
d <sub>sc</sub> =	83 mm	A <sub>sc</sub> =	565.486678 mm <sup>2</sup>
d =	190 mm		
Ig =	1.73E+09 mm <sup>4</sup>		

At right end (arbitrary)

$$1/2 \times b d_n^2 + (n-1) A_{sc} (d_n - d_{sc}) = n A_{st} (d - d_n)$$

$$500 d_n^2 + 9259.55856 d_n - 1441362.46 = 0$$

$$d_n = 45.22 \text{ mm}$$

$$d = 182 \text{ mm}$$

$$I_{cr} = b d_n^3/3 + n \times A_{sc} (d - d_n)^2 = 1.48E+08 \text{ mm}^4$$

$$M_{cr} = 0.6 \sqrt{F'_c} \times b D^2/6 = 47.8294496 \text{ kNm}$$

$$I_{ref} = [I_{cr} + (I_g - I_{cr}) (M_{cr} / M_r)^3] = 6.13E+08 \text{ mm}^4$$

At left end (arbitrary)

b <sub>w</sub> =	1000 mm		
d <sub>sc</sub> =	83 mm		
A <sub>st</sub> =	1005.3096 mm <sup>2</sup>		
A <sub>sc</sub> =	452.38934 mm <sup>2</sup>		
d =	190 mm		

$$1/2 \times b d_n^2 + (n-1) A_{sc} (d_n - d_{sc}) = n A_{st} (d - d_n)$$

$$500 d_n^2 + 8665.25264 d_n - 1392035.07 = 0$$

$$d_n = 44.8058306 \text{ mm}$$

$$d = 190 \text{ mm}$$

$$I_{cr} = b d_n^3/3 + n \times A_{sc} (d - d_n)^2 = 1.63E+08 \text{ mm}^4$$

$$M_{cr} = 0.6 \sqrt{F'_c} \times b D^2/6 = 47.8294496 \text{ kNm}$$

$$I_{lef} = [I_{cr} + (I_g - I_{cr}) (M_{cr} / M_l)^3] = 9.96E+10 \text{ mm}^4$$

$$I_{lav} = [I_m + (I_l + I_r)/2]/2 = 2.45E+11 \text{ mm}^4$$

Midspan Deflection :

$$L_{ef} = 2850 \text{ mm}$$

$$\text{Defln,s} = L_{ef}^2/E_c \times I_{lav} [5/48 \cdot M_o - 1/16 M_L - 1/16 M_r]$$

$$= -0.00010808 \text{ mm}$$

**Total Deflection :**

$$\begin{aligned} \text{Long term factor} &= 0.2 \\ \text{Short term factor} &= 0.5 \\ w - \text{long term} &= 8.275 \text{ kN/m} \\ w - \text{short term} &= 8.875 \text{ kN/m} \\ \text{Defln}_{s,sus} &= -0.000100774 \text{ mm} \\ \text{At midspan :} & \quad \text{Ast} = 603.185789 \text{ mm}^2 \\ & \quad \text{Asc} = 603.185789 \text{ mm}^2 \\ \text{Asc/Ast} &= 1 \\ \text{kcs} &= 2 - (1.2 \times \text{Asc/Ast}) \quad \geq 0.8 \\ &= 0.8 \end{aligned}$$

therefore

Total Defln	=	Dfln,s + ( kcs x Defln,s,sus )
	=	-0.00019 mm

FROM A MORE ACCURATE ANALYSIS Warner,Rangan,Hall HAVE SHOWN THAT THIS CAN BE REDUCED BY UP TO 10%.

ALLOWABLE DEFLN	=	Lef / 500 =	5.7 mm
			> 0.9 x Total Defln
ALLOWABLE DEFLN	=	Lef / 250 =	11.4 mm
			> 0.9 x Total Defln

OK



## Beam Design

Blower Control Room Beam :

REF.

AS 3600

### 1. Loads

DL	:	Wdl =	7.20 kN/m	(plus super)
Self wt	:	Wsw =	7.88 kN/m	
Blockwall	:	Wb =	24.00 kN/m	
TOTAL	:	=	39.08 kN/m	
LL	:	Wll =	0.25 kN/m	
LL factor	- long term		0.4	
	- short term		0.7	

### 2. Beam Section Properties

bw	450	mm
D	700	mm
tff	0	mm
Cover	65	mm
D = Overall depth of section	700	mm
Actual length, L	6900	mm
Effective Length, L <sub>ef</sub>	6500	mm
Area, A	3.15E+05	mm <sup>2</sup>
Neutral Axis, y <sub>c</sub>	350.00	mm
Moment of Inertia, I <sub>x-x</sub>	1.29E+10	mm <sup>4</sup>
Neutral Axis, x <sub>c</sub>	875.00	mm
Moment of Inertia, I <sub>y-y</sub>	5.32E+09	mm <sup>4</sup>
Effective width, b <sub>eff</sub>	1750.00	mm
Gross Area, A <sub>g</sub>	3.15E+05	mm <sup>2</sup>

### 3. Design Parameters

F' <sub>c</sub>	=	40	MPa
F <sub>sy</sub>	=	410	MPa
m		2400	kg/m <sup>3</sup>
D	=	700	mm
E <sub>c</sub>	=	31975.3505	MPa
E <sub>s</sub>	=	200000	MPa
Gamma	=	0.766	

### 4. INITIAL CHOICE OF SECTION

Effective width	=	1750 mm	clause 8.8.2	
Is the section RECT or T, L (ans : 1 or 2)	=			1
k <sub>1</sub>	=	0.045	(RECT sections)	1
	=	0.02112103	(T and L sections)	2
what type of span?		6	interior span =	5
			end span =	6
k <sub>2</sub>	=	0.0054054		
k <sub>cs</sub>	=	1.64	(because there will be comp steel at midspan=say .5A <sub>st</sub> )	
F <sub>def</sub>	=	64.422		
defln/L <sub>ef</sub>	=	0.004	k <sub>1</sub> /k <sub>2</sub> =	8.32500833
d	=	212 mm		
take d =		599 mm	Distance from d to extreme fibre of beam (d <sub>o</sub> ) =	101 mm

**5. Forces from an Elastic Analysis by Space Gass :**

Mmax -ve	=	283.00 kNm	(left support)
Mmax +ve	=	141.00 kNm	(mid-span)
M -ve	=	283.00 kNm	(right support)
Vmax	=	246.00 kN	
V (other end)	=	246.00 kN	
N*	=	0.00 kN	Axial Load

**6. FLECTURAL STRENGTH STEEL**

Determine -ve steel required: (At arbitrary left support)

Number of reinforcement layers 1 or 2 :					
take do =	101 mm	d =	599		
Estimate Area of main steel	Ast =	1506.30987 mm <sup>2</sup>			
4 Y 24	Ast =	1809.55737 mm <sup>2</sup>			
	p =	0.00671325			
3 Y 20	Asc =	942.477796 mm <sup>2</sup>			
	pc =	0.00349649	<	pd =	0.025409
	p-pc =	0.00321677			

Check  $M^* \leq \phi \mu$

ku =	0.05064	dsc =	87 mm
------	---------	-------	-------

$$\phi \mu = \phi [F_{sy} \times A_{sc} (d - d_{sc}) + 0.85 \times F'_c \times b \times \Gamma \times k_u \times d \times (d - 0.5 \times 0.85 \times k_u \times d)]$$

=	366 kNm	>	M* =	283.00 kNm	OK
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Determine -ve steel at right support:

M -ve	=	283.00 kNm			
Number of reinforcement layers 1 or 2 :					
take do =	113 mm	dst =	599 mm		
Estimate Area of main steel	Ast =	1506.30987 mm <sup>2</sup>			
4 Y 24	Ast =	1809.55737 mm <sup>2</sup>			
	p =	0.00671325			
3 Y 20	Asc =	942.477796 mm <sup>2</sup>			
	pc =	0.00349649	<	pd =	0.025409
	p-pc =	0.00321677			

Check  $M^* \leq \phi \mu$

ku =	0.05064	dsc =	87 mm
------	---------	-------	-------

$$\phi \mu = \phi [F_{sy} \times A_{sc} (d - d_{sc}) + 0.85 \times F'_c \times b \times \Gamma \times k_u \times d \times (d - 0.5 \times 0.85 \times k_u \times d)]$$

=	366 kNm	>	M* =	283.00 kNm	OK
---	---------	---	------	------------	----

FOR DUCTILITY CHECK AT BOTH SUPPORTS USE EMPHI

**Tensile Steel at Midspan:**

M*	=	141 kNm			
Steel in one layer then	code = 1	take dist between centroid of			
" " two " "	code = 2	bar/s and soffit =	97 mm		
what is the code?	1	d =	599 mm		
Ast =	M*/(Phi x 0.85 x d x Fsy)	=	750.493609 mm <sup>2</sup>		
Bottom	3 Y 20	Ast =	942.477796 mm <sup>2</sup>		
Top	3 Y 16	Asc =	603.185789 mm <sup>2</sup>		
Assume	N - A in flange				
Effective width :	b <sub>eff</sub> =	1750 mm			
ku =	1/(0.85 x 0.85) x [Ast/(bw x d)] x (Fsy/F'c)				
=	0.01415411				
hence	dn =	8	<	flange thickness	
				hence N-A is in flange	

$$\Phi \mu = 0.9 [F_{sy} \times A_{st} \times d (1 - 0.5 \times 0.85 \times k_u)]$$

$=$	207 kNm	$>$	$M^*_{+ve} =$	141.00 kNm	OK
-----	---------	-----	---------------	------------	----

Summary of reinforcement requirement :

left support :	Ast =	4	Y 24
	Asc =	3	Y 20
Right support :	Ast =	4	Y 24
	Asc =	3	Y 20
midspan	Ast =	3	Y 20
	Asc =	3	Y 16

### 7. STEEL CURTAILMENT REQUIREMENT CHECK

The following requirements regarding curtailment are subject to stress development length requirements.

- For -ve steel
- i)  $1/4 A_{st}$  to continue over length of beam  
= 2 bars
  - ii) Remainder to be curtailed at  $0.3L_n$  from face of support.  
=  $0.3 \times l_e =$  1950 mm ?
- For -ve steel
- i)  $1/4 A_{st}$  to continue over length of beam  
= 2 bars
  - ii) Remainder to be curtailed at  $0.3L_n$  from face of support.  
=  $0.3 \times l_e =$  1950 mm ?
- For +ve steel :
- i)  $1/2 A_{st}$  must be carried into outer support  
= 2 bars
  - ii)  $1/4 A_{st}$  into interior support  
= 2 bars
  - iii) Remainder curtailed at  $0.1L_n$  from supports  
=  $0.1 \times l_e =$  650 mm

### 8. STRESS DEVELOPMENT

For Top Steel :

cover :  $c =$  65 mm

$d_b =$  16 mm      # of lines of bars = 2

$a_b =$  148 mm      distance between bars

If  $a_b > 2c$        $C = 2c + d_b =$  146 mm  
C is the outside diameter of a concrete annulus

if  $a_b \leq 2c$        $C = a_b + d_b =$  164 mm  
coaxial with and surrounding a bar

$k_1 =$  [redacted]       $A_b =$  201.06193 mm<sup>2</sup>

$k_2 =$  [redacted]       $a_b =$  148 mm (dist between bars)

factor = 35843.9945

$L_{syt} = k_1 \times k_2 \times f_{sy} \times A_b / C \times \sqrt{f'_c} =$  factor/C  
= 245.5068 mm

take  $f_s / F_{sy} = M^* / \Phi \mu =$  0.774097193

therefore 

$L_{sy} =$	190 mm
$say =$	200 mm

 $\geq$  25k1db      500

For Bottom Steel :

Cover,  $c =$  65 mm       $d_b =$  20 mm

# of lines of bars = 2

If  $a_b > 2c$        $C = 2c + d_b =$  150 mm

if  $a_b < 2c$        $C = a_b + d_b =$  322 mm

$k_1 =$  1       $A_b =$  314.159265 mm<sup>2</sup>

$k_2 =$  2.2       $a_b =$  302 mm (dist between bars)

factor = 44804.9931

$L_{syt} =$  298.7 mm      take  $f_s / F_{sy} = M^* / \Phi \mu =$  0.680542922

therefore 

$L_{sy} =$	203 mm
$say =$	200 mm

 $\geq$  25k1db      400

**9. SHEAR CHECK**

$V^* = 246.00 \text{ kN}$  Dist to centreline of bottom steel from soffit  
 critical location :  $d = 599 \text{ mm}$   $d_q = 93 \text{ mm}$   
 $d_o = 607 \text{ mm}$   
 $V^* = 222 \text{ kN}$   $\beta_1 = 1.0965$  (but not less than 1.1)  
 hence  $\beta_2 = 1.1$   
 $\beta_3 = 1$   
 $\beta_3 = 1$  Unity  
 $\phi V_{uc} = \phi \times \beta_1 \times \beta_2 \times \beta_3 \times b_w \times d_o [A_{st} \times F'_c / b_w \times d_o]^{0.333}$   
 $= 135.15 \text{ kN}$   
 $\phi V_{umax} = 1529.64 \text{ kN} > V^*$  No web crushing  
 $\phi V_{umin} = 250 \text{ kN} > V^*$  OK

If shear reinforcement required then proceed as follows :

choose stirrup spacing  $s = 300 \text{ mm}$   $F_{syf} = 410 \text{ MPa}$   
 Determine theta :

$b_v \times s / F_{syf} = 329.268293$   
 $A_{sv.min} = 115.243902 \text{ mm}^2$   
 $A_{sv.max} = 2401 \text{ mm}^2$   
 $A_{sv} = 2 \times \text{area of stirrups} = 220 \text{ mm}^2$  2 Ties  
 $\theta = 30.68733 \text{ degrees}$   
 Determine  $\phi V_u$  :  
 $V_u = V_{uc} + V_{us}$   
 $= V_{uc} + A_{sv} / s \times F_{syf} \times d_o \cot \theta$   
 $= 500.60 \text{ kN}$   
 $\phi V_u = 350 \text{ kN} > V^* = 222.444325 \text{ kN}$   
 hence

**PROVIDE :** 1 Y12 LIGS @ 300 centres

Other Support :

$V^* = 246.00 \text{ kN}$  Dist to centreline of bottom steel from soffit  
 critical location :  $d = 599 \text{ mm}$   $d_q = 93 \text{ mm}$   
 $d_o = 607 \text{ mm}$   
 $V^* = 222.44 \text{ kN}$   $\beta_1 = 1.0965$  (but not less than 1.1)  
 hence  $\beta_2 = 1.1$   
 $\beta_3 = 1$   
 $\beta_3 = 1$   
 $\phi V_{uc} = \phi \times \beta_1 \times \beta_2 \times \beta_3 \times b_w \times d_o [A_{st} \times F'_c / b_w \times d_o]^{0.333}$   
 $= 135.1537 \text{ kN}$   
 $\phi V_{umax} = 1529.64 \text{ kN} > V^*$  No web crushing  
 $\phi V_{umin} = 250 \text{ kN} < V^*$  OK

If shear reinforcement required then proceed as follows :

choose stirrup spacing  $s = 300 \text{ mm}$   $F_{syf} = 410 \text{ MPa}$   
 Determine theta :

$b_v \times s / F_{syf} = 329.268293$   
 $A_{sv.min} = 115.243902 \text{ mm}^2$   
 $A_{sv.max} = 2401.4023 \text{ mm}^2$   
 $A_{sv} = 2 \times \text{area of stirrups} = 330 \text{ mm}^2$  2 Ties  
 $\theta = 31.40906 \text{ degrees}$   
 Determine  $\phi V_u$  :  
 $V_u = V_{uc} + V_{us}$   
 $= V_{uc} + A_{sv} / s \times F_{syf} \times d_o \cot \theta$   
 $= 641.4032 \text{ kN}$   
 $\phi V_u = 448.9822 \text{ kN} > V^* = 222.44 \text{ kN}$   
 hence

**PROVIDE :** 1 Y12 LIGS @ 300 centres

### 10. SERVICEABILITY CHECK

From an Elastic Linear Analysis :

Mr	=	283.00 kNm	Es	=	200000 MPa
MI	=	283.00 kNm	Ec	=	31975.35051 MPa
Mo	=	424 kNm			
Ms	=	141.00 kNm			
Ig	=	1.29E+10 mm <sup>4</sup>			

Calculate Icr, Mcr, and Ief at midspan :

Eff. Flange width	b <sub>eff</sub> =	1750 mm	A <sub>sc</sub> =	603.1857895 mm <sup>2</sup>
webwidth	b <sub>w</sub> =	450 mm	A <sub>st</sub> =	942.4777961 mm <sup>2</sup>
Flange thickness	t <sub>ff</sub> =	0 mm	d <sub>sc</sub> =	97 mm
			d <sub>st</sub> =	599 mm
			d =	599 mm
			D =	700 mm

$$n = E_s/E_c = 6.25481807$$

$$1/2 \times b_w \times d_n^2 + (b_{eff} - b_w) \times t \times (d_n - t/2) + (n-1) A_{sc} (d_n - d_{sc}) = n A_{st} (d - d_n)$$

$$225 d_n^2 + 9064.65873 d_n - 3838575.52 = 0$$

$$d_n = 112.015702 \text{ mm}$$

$$d = 599 \text{ mm}$$

$$I_{cr} = b d_n^3/3 + 1/12 (b_{eff} - b_w) \times t^3 + (b_{eff} - b_w) \times t \times (d_n - t/2)^2 + n x A_{st} (d - d_n)^2$$

$$= 1.61E+09 \text{ mm}^4$$

$$M_{cr} = 0.6 \sqrt{F'_c} \times b D^2/6 = 139.456445 \text{ kNm}$$

Im	=	[Icr + (Ig - Icr) (Mcr / Ms) ^3]
	=	1.25E+10 mm <sup>4</sup>

Calculate Icr, Mcr, Ilef and Iref at end supports:

b <sub>w</sub> =	450 mm	A <sub>st</sub> =	1809.55737 mm <sup>2</sup>
d <sub>sc</sub> =	87 mm	A <sub>sc</sub> =	942.477796 mm <sup>2</sup>
d =	599 mm		
Ig =	1.29E+10 mm <sup>4</sup>		

At right end (arbitrary)

$$1/2 \times b d_n^2 + (n-1) A_{sc} (d_n - d_{sc}) = n A_{st} (d - d_n)$$

$$225 d_n^2 + 16271.0015 d_n - 7210624.61 = 0$$

$$d_n = 146.47 \text{ mm}$$

$$d = 599 \text{ mm}$$

$$I_{cr} = b d_n^3/3 + n \times A_{sc} (d - d_n)^2 = 2.79E+09 \text{ mm}^4$$

$$M_{cr} = 0.6 \sqrt{F'_c} \times b D^2/6 = 139.456445 \text{ kNm}$$

$$I_{ref} = [I_{cr} + (I_g - I_{cr}) (M_{cr} / M_r)^3] = 3.99E+09 \text{ mm}^4$$

At left end (arbitrary)

b <sub>w</sub> =	450 mm		
d <sub>sc</sub> =	87 mm		
A <sub>st</sub> =	1809.5574 mm <sup>2</sup>		
A <sub>sc</sub> =	942.4778 mm <sup>2</sup>		
d =	599 mm		
1/2 x b d <sub>n</sub> <sup>2</sup> + (n-1) A <sub>sc</sub> (d <sub>n</sub> - d <sub>sc</sub> ) = n A <sub>st</sub> (d - d <sub>n</sub> )			
225 d <sub>n</sub> <sup>2</sup> + 16271.0015 d <sub>n</sub> - 7210624.61	=	0	
d <sub>n</sub> =	146.474652 mm		
d =	599 mm		

$$I_{cr} = b d_n^3/3 + n \times A_{sc} (d - d_n)^2 = 2.79E+09 \text{ mm}^4$$

$$M_{cr} = 0.6 \sqrt{F'_c} \times b D^2/6 = 139.456445 \text{ kNm}$$

$$I_{lef} = [I_{cr} + (I_g - I_{cr}) (M_{cr} / M_l)^3] = 3.99E+09 \text{ mm}^4$$

I <sub>av</sub>	=	[Im + (I <sub>l</sub> + I <sub>r</sub> )/2]/2	=	8.25E+09 mm <sup>4</sup>
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Midspan Deflection :

$$L_{ef} = 6500 \text{ mm}$$

$$Defl_{n,s} = L_{ef}^2/E_c \times I_{av} [5/48 \cdot M_o - 1/16 ML - 1/16 Mr]$$

	=	1.4088092 mm
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**Total Deflection :**

Long term factor	=	0.2	
Short term factor	=	0.5	
w - long term	=	39.125 kN/m	
w - short term	=	39.2 kN/m	
Defln,s,sus	=	1.40611377 mm	
At midspan :			
		Ast	= 942.477796 mm <sup>2</sup>
		Asc	= 603.185789 mm <sup>2</sup>
	Asc/Ast =	0.64	
	kcs =	2 - (1.2 x Asc/Ast)	>= 0.8
	=	1.232	

therefore

Total Defln	=	Dfln,s + ( kcs x Defln,s,sus )
	=	3.14114 mm

FROM A MORE ACCURATE ANALYSIS Warner,Rangan,Hall HAVE SHOWN THAT THIS CAN BE REDUCED BY UP TO 10%.

ALLOWABLE DEFLN	=	Lef / 500 =	13 mm
0.9 x Total Defln =	2.827027225	<	13 OK
ALLOWABLE DEFLN	=	Lef / 250 =	26 mm
0.9 x Total Defln =	2.827027225	<	26 OK

<b>Main Wall Design</b>	
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Design of a Beam :

REF. AS 3600

**1. Loads**

DL	:	Wdl =	3.83 kN/m	(plus super)
Self wt	:	Wsw =	41.28 kN/m	
Blockwall	:	Wb =	0.00 kN/m	
TOTAL	:	=	45.11 kN/m	
LL	:	Wll =	0.25 kN/m	
LL factor - long term			0.4	
- short term			0.7	

**2. Beam Section Properties**

bw		2064	mm
D		800	mm
tff		0	mm
Cover		75	mm
D = Overall depth of section		800	mm
Actual length, L		7600	mm
Effective Length, L <sub>ef</sub>		6400	mm
Area, A		1.65E+06	mm <sup>2</sup>
Neutral Axis, y <sub>c</sub>		400.00	mm
Moment of Inertia, I <sub>x-x</sub>		8.81E+10	mm <sup>4</sup>
Neutral Axis, x <sub>c</sub>		1032.00	mm
Moment of Inertia, I <sub>y-y</sub>		5.86E+11	mm <sup>4</sup>
Effective width, b <sub>eff</sub>		2064.00	mm
Gross Area, A <sub>g</sub>		1.65E+06	mm <sup>2</sup>

**3. Design Parameters**

F' <sub>c</sub> =	40	MPa
F <sub>sy</sub> =	410	MPa
m	2400	kg/m <sup>3</sup>
D =	800	mm
E <sub>c</sub> =	31975.3505	MPa
E <sub>s</sub> =	200000	MPa
Gamma =	0.766	

**4. INITIAL CHOICE OF SECTION**

Effective width =	2064 mm	clause 8.8.2	
Is the section RECT or T, L (ans: 1 or 2)	=		1
k <sub>1</sub> =	0.045	(RECT sections)	1
=	0.045	(T and L sections)	2
what type of span?	6	interior span =	5
		end span =	6
k <sub>2</sub> =	0.0054054		
k <sub>cs</sub> =	1.64	(because there will be comp steel at midspan=say .5A <sub>st</sub> )	
F <sub>def</sub> =	74.3194		
defln/L <sub>ef</sub> =	0.004	k <sub>1</sub> /k <sub>2</sub> =	8.32500833
d =	208 mm		
take d =	697 mm	Distance from d to extreme fibre of beam (d <sub>o</sub> ) =	103 mm

**5. Forces from an Elastic Analysis by Space Gass :**

Mmax -ve	=	2300.00 kNm	(left support)
Mmax +ve	=	174.00 kNm	(mid-span)
M -ve	=	7.59 kNm	(right support)
Vmax	=	760.00 kN	
V (other end)	=	0.00 kN	
N*	=	138.00 kN	Axial Load

**6. FLECTURAL STRENGTH STEEL**

Determine -ve steel required: (At arbitrary left support)

Number of reinforcement layers	1 or 2 :		1
take do =	103 mm	d =	697
Estimate Area of main steel	Ast =	10520.8243 mm <sup>2</sup>	
13 Y 32	Ast =	10455.2204 mm <sup>2</sup>	
159 CTS	p =	0.0072676	
3 Y 32	Asc =	2412.74316 mm <sup>2</sup>	
688 CTS	pc =	0.00167714	< pd = 0.025409
Astmin .0035*	p-pc =	0.00559046	$p d = 0.4 \times 0.85 \mu^* f'c / f_{sy}$

Check  $M^* \leq \phi \mu$

ku =	0.088008	dsc =	103 mm
------	----------	-------	--------

$$\phi \mu = \phi [F_{sy} \times Asc (d - d_{sc}) + 0.85 \times F'c \times b \times \Gamma \times k_u \times d \times (d - 0.5 \times 0.85 \times k_u \times d)]$$

=	2520 kNm	>	M* =	2300.00 kNm	OK
---	----------	---	------	-------------	----

Determine -ve steel at right support:

M -ve	=	7.59 kNm	
Number of reinforcement layers	1 or 2 :		1
take do =	119 mm	dst =	681 mm
Estimate Area of main steel	Ast =	35.5344315 mm <sup>2</sup>	
3 Y 32	Ast =	2412.74316 mm <sup>2</sup>	
688 CTS	p =	0.00171654	
3 Y 16	Asc =	603.185789 mm <sup>2</sup>	
688 CTS	pc =	0.00042914	< pd = 0.025409
	p-pc =	0.00128741	

Check  $M^* \leq \phi \mu$

ku =	0.020267	dsc =	95 mm
------	----------	-------	-------

$$\phi \mu = \phi [F_{sy} \times Asc (d - d_{sc}) + 0.85 \times F'c \times b \times \Gamma \times k_u \times d \times (d - 0.5 \times 0.85 \times k_u \times d)]$$

=	581 kNm	>	M* =	7.59 kNm	OK
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FOR DUCTILITY CHECK AT BOTH SUPPORTS USE EMPHI

**Tensile Steel at Midspan:**

M*	=	174 kNm	
Steel in one layer then	code = 1	take dist between centroid of	
" " two " "	code = 2	bar/s and soffit =	119 mm
what is the code?	1	d =	697 mm
Ast =	M*/(Phi x 0.85 x d x Fsy)	=	795.923226 mm <sup>2</sup>
Bottom	3 Y 32	Ast =	2412.74316 mm <sup>2</sup>
Top	3 Y 16	Asc =	603.185789 mm <sup>2</sup>
Assume	N - A in flange		
Effective width :	b <sub>eff</sub> =	2064 mm	
ku =	1/(0.85 x 0.85) x [Ast/(b <sub>w</sub> x d)] x (F <sub>sy</sub> /F'c)		
=	0.02640248		
hence	d <sub>n</sub> =	18	< flange thickness
			hence N-A is in flange



$$\Phi \mu = 0.9 [F_{sy} \times A_{st} \times d (1 - 0.5 \times 0.85 \times k_u)]$$

= 614 kNm	>	M*+ve = 174.00 kNm	OK
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Summary of reinforcement requirement :

left support :	A <sub>st</sub> =	3	Y 32
	Asc =	3	Y 16
Right support :	A <sub>st</sub> =	3	Y 32
	Asc =	3	Y 32
midspan	A <sub>st</sub> =	3	Y 32
	Asc =	3	Y 16

### 7. STEEL CURTAILMENT REQUIREMENT CHECK

The following requirements regarding curtailment are subject to stress development length requirements.

- For -ve steel
- i) 1/4 A<sub>st</sub> to continue over length of beam = 2 bars
  - ii) Remainder to be curtailed at 0.3L<sub>n</sub> from face of support.  
= 0.3 x l<sub>e</sub> = 1920 mm ?
- For -ve steel
- i) 1/4 A<sub>st</sub> to continue over length of beam = 2 bars
  - ii) Remainder to be curtailed at 0.3L<sub>n</sub> from face of support.  
= 0.3 x l<sub>e</sub> = 1920 mm ?
- For +ve steel :
- i) 1/2 A<sub>st</sub> must be carried into outer support = 2 bars
  - ii) 1/4 A<sub>st</sub> into interior support = 2 bars
  - iii) Remainder curtailed at 0.1L<sub>n</sub> from supports  
= 0.1 x l<sub>e</sub> = 640 mm

### 8. STRESS DEVELOPMENT

For Top Steel:

cover : c = 75 mm

d<sub>b</sub> = 16 mm # of lines of bars = 2

a<sub>b</sub> = 945 mm distance between bars

If a<sub>b</sub> > 2c C = 2c + d<sub>b</sub> = 166 mm  
C is the outside diameter of a concrete annulus

if a<sub>b</sub> <= 2c C = a<sub>b</sub> + d<sub>b</sub> = 961 mm  
coaxial with and surrounding a bar

k<sub>1</sub> = 1 A<sub>b</sub> = 201.06193 mm<sup>2</sup>

k<sub>2</sub> = 2.2 a<sub>b</sub> = 945 mm (dist between bars)

factor = 35843.9945

L<sub>sy</sub> = k<sub>1</sub> x k<sub>2</sub> x f<sub>sy</sub> x A<sub>b</sub>/C x sqrt(f'c) = factor/C = 215.9277 mm

take fs/Fsy = M\*/Phi Mu = 0.912720019

L <sub>sy</sub> = 197 mm	≥	25k1db	500
say = 500 mm			

For Bottom Steel:

Cover, c = 75 mm d<sub>b</sub> = 32 mm

# of lines of bars = 2

If a<sub>b</sub> > 2c C = 2c + d<sub>b</sub> = 182 mm

if a<sub>b</sub> < 2c C = a<sub>b</sub> + d<sub>b</sub> = 1151 mm

k<sub>1</sub> = 1 A<sub>b</sub> = 804.247719 mm<sup>2</sup>

k<sub>2</sub> = 2.2 a<sub>b</sub> = 1119 mm (dist between bars)

factor = 114700.782

L<sub>sy</sub> = 630.2241 mm take fs/Fsy = M\*/Phi Mu = 0.283265066

L <sub>sy</sub> = 179 mm	≥	25k1db	400
say = 400 mm			

### 9. SHEAR CHECK

$V^* = 760.00 \text{ kN}$  Dist to centreline of bottom steel from soffit  
critical location :  $d = 697 \text{ mm}$   $d_q = 93 \text{ mm}$   
 $do = 707 \text{ mm}$   
 $V^* = 728 \text{ kN}$   $\beta_1 = 1.0465$  (but not less than 1.1)  
hence  $= 1.1$   
 $\beta_2 = 1$   
 $\beta_3 = 1$  Unity  
 $\phi V_{uc} = \phi \times \beta_1 \times \beta_2 \times \beta_3 \times b_w \times d_o [A_{st} \times F_c / b_w \times d_o]^{0.333}$   
 $= 741.12 \text{ kN}$   
 $\phi V_{u\max} = 8171.7888 \text{ kN} > V^*$  No web crushing  
 $\phi V_{u\min} = 1354 \text{ kN} > V^*$  OK

If shear reinforcement required then proceed as follows :

choose stirrup spacing  $s = 350 \text{ mm}$   $F_{syf} = 410 \text{ MPa}$   
Determine  $\theta$  :

$b_v \times s / F_{syf} = 1761.95122$   
 $A_{sv\min} = 616.682927 \text{ mm}^2$   
 $A_{sv\max} = 12817 \text{ mm}^2$   
 $A_{sv} = 2 \times \text{area of stirrups} = 220 \text{ mm}^2$  2 Ties  
 $\theta = 29.5123 \text{ degrees}$   
Determine  $\Phi V_u$  :  
 $V_u = V_{uc} + V_{us}$   
 $= V_{uc} + A_{sv} / s \times F_{syf} \times d_o \cot \theta$   
 $= 1380.63 \text{ kN}$   
 $\Phi V_u = 966 \text{ kN} > V^* = 728.38408 \text{ kN}$   
hence

PROVIDE : 1 Y12 LIGS @ 350 centres

Other Support :

$V^* = 0.00 \text{ kN}$  Dist to centreline of bottom steel from soffit  
critical location :  $d = 697 \text{ mm}$   $d_q = 93 \text{ mm}$   
 $do = 707 \text{ mm}$   
 $V^* = -31.62 \text{ kN}$   $\beta_1 = 1.0465$  (but not less than 1.1)  
hence  $= 1.1$   
 $\beta_2 = 1$   
 $\beta_3 = 1$   
 $\phi V_{uc} = \phi \times \beta_1 \times \beta_2 \times \beta_3 \times b_w \times d_o [A_{st} \times F_c / b_w \times d_o]^{0.333}$   
 $= 454.8069 \text{ kN}$   
 $\phi V_{u\max} = 8171.7888 \text{ kN} > V^*$  No web crushing  
 $\phi V_{u\min} = 1068 \text{ kN} < V^*$  OK

If shear reinforcement required then proceed as follows :

choose stirrup spacing  $s = 350 \text{ mm}$   $F_{syf} = 410 \text{ MPa}$   
Determine  $\theta$  :

$b_v \times s / F_{syf} = 1761.95122$   
 $A_{sv\min} = 616.682927 \text{ mm}^2$   
 $A_{sv\max} = 13311.1083 \text{ mm}^2$   
 $A_{sv} = 2 \times \text{area of stirrups} = 330 \text{ mm}^2$  2 Ties  
 $\theta = 29.66125 \text{ degrees}$   
Determine  $\Phi V_u$  :  
 $V_u = V_{uc} + V_{us}$   
 $= V_{uc} + A_{sv} / s \times F_{syf} \times d_o \cot \theta$   
 $= 1129.634 \text{ kN}$   
 $\Phi V_u = 790.7441 \text{ kN} > V^* = -31.62 \text{ kN}$   
hence

PROVIDE : 1 Y12 LIGS @ 350 centres

### 10. SERVICEABILITY CHECK

From an Elastic Linear Analysis :

Mr	=	2300.00 kNm	Es	=	200000 MPa
MI	=	7.59 kNm	Ec	=	31975.35051 MPa
Mo	=	1327.795 kNm			
Ms	=	174.00 kNm			
Ig	=	8.81E+10 mm <sup>4</sup>			

Calculate Icr, Mcr, and Ief at midspan :

Eff. Flange width	b <sub>eff</sub> =	2064 mm	A <sub>sc</sub> =	603.1857895 mm <sup>2</sup>
webwidth	b <sub>w</sub> =	2064 mm	A <sub>st</sub> =	2412.743158 mm <sup>2</sup>
Flange thickness	t <sub>ff</sub> =	0 mm	d <sub>sc</sub> =	119 mm
			d <sub>st</sub> =	697 mm
			d =	697 mm
			D =	800 mm

$$n = E_s/E_c = 6.25481807$$

$$1/2 \times b_w \times d_n^2 + (b_{eff} - b_w) \times t \times (d_n - t/2) + (n-1) A_{sc} (d_n - d_{sc}) = n A_{st} (d - d_n)$$

$$1032 d_n^2 + 18260.9011 d_n - 10895801 = 0$$

$$d_n = 94.2847254 \text{ mm}$$

$$d = 697 \text{ mm}$$

$$I_{cr} = b d_n^3/3 + 1/12 (b_{eff} - b_w) \times t^3 + (b_{eff} - b_w) \times t \times (d_n - t/2)^2 + n x A_{st} (d - d_n)^2$$

$$= 6.06E+09 \text{ mm}^4$$

$$M_{cr} = 0.6 \sqrt{F'_c} \times b D^2/6 = 835.44846 \text{ kNm}$$

$$I_m = [I_{cr} + (I_g - I_{cr}) (M_{cr} / M_s)^3]$$

$$= 9.08E+12 \text{ mm}^4$$

Calculate Icr, Mcr, Ilef and Iref at end supports:

$$b_w = 2064 \text{ mm} \quad A_{st} = 2412.74316 \text{ mm}^2$$

$$d_{sc} = 95 \text{ mm} \quad A_{sc} = 603.185789 \text{ mm}^2$$

$$d = 697 \text{ mm}$$

$$I_g = 8.81E+10 \text{ mm}^4$$

At right end (arbitrary)

$$1/2 \times b d_n^2 + (n-1) A_{sc} (d_n - d_{sc}) = n A_{st} (d - d_n)$$

$$1032 d_n^2 + 18260.9011 d_n - 10819729.8 = 0$$

$$d_n = 93.93 \text{ mm}$$

$$d = 681 \text{ mm}$$

$$I_{cr} = b d_n^3/3 + n \times A_{sc} (d - d_n)^2 = 5.77E+09 \text{ mm}^4$$

$$M_{cr} = 0.6 \sqrt{F'_c} \times b D^2/6 = 835.44846 \text{ kNm}$$

$$I_{ref} = [I_{cr} + (I_g - I_{cr}) (M_{cr} / M_r)^3] = 9.72E+09 \text{ mm}^4$$

At left end (arbitrary)

$$b_w = 2064 \text{ mm}$$

$$d_{sc} = 103 \text{ mm}$$

$$A_{st} = 10455.22 \text{ mm}^2$$

$$A_{sc} = 2412.7432 \text{ mm}^2$$

$$d = 697 \text{ mm}$$

$$1/2 \times b d_n^2 + (n-1) A_{sc} (d_n - d_{sc}) = n A_{st} (d - d_n)$$

$$1032 d_n^2 + 78074.0275 d_n - 46886552.5 = 0$$

$$d_n = 178.653354 \text{ mm}$$

$$d = 697 \text{ mm}$$

$$I_{cr} = b d_n^3/3 + n \times A_{sc} (d - d_n)^2 = 2.15E+10 \text{ mm}^4$$

$$M_{cr} = 0.6 \sqrt{F'_c} \times b D^2/6 = 835.44846 \text{ kNm}$$

$$I_{lef} = [I_{cr} + (I_g - I_{cr}) (M_{cr} / M_l)^3] = 8.88E+16 \text{ mm}^4$$

$$I_{av} = [I_m + (I_l + I_r)/2]/2 = 2.22E+16 \text{ mm}^4$$

Midspan Deflection :

$$L_{ef} = 6400 \text{ mm}$$

$$Defl_{n,s} = L_{ef}^2/E_c \times I_{av} [5/48 \cdot M_o - 1/16 \cdot M_L - 1/16 \cdot M_r]$$

$$= -3.4117E-07 \text{ mm}$$

**Total Deflection :**

Long term factor = 0.2  
Short term factor = 0.5  
w - long term = 45.16 kN/m  
w - short term = 45.235 kN/m  
Defln,s,sus = -3.406E-07 mm  
At midspan : Ast = 2412.74316 mm<sup>2</sup>  
Asc = 603.185789 mm<sup>2</sup>  
Asc/Ast = 0.25  
kcs = 2 - (1.2 x Asc/Ast) >= 0.8  
= 1.7

therefore

Total Defln	=	Dfln,s + ( kcs x Defln,s,sus )
	=	0.00000 mm

FROM A MORE ACCURATE ANALYSIS Warner,Rangan,Hall HAVE SHOWN THAT THIS CAN BE REDUCED BY UP TO 10%.

ALLOWABLE DEFLN	=	Lef / 500 =	12.8 mm
			> 0.9 x Total Defln
ALLOWABLE DEFLN	=	Lef / 250 =	25.6 mm
			> 0.9 x Total Defln

## Roof Beam Design

Design of a Continuous Beam :

REF. AS 3600

### 1. Loads

DL	:	Wdl =	0.60 kN/m	(plus super)
Self wt	:	Wsw =	2.19 kN/m	
Blockwall	:	Wb =	0.00 kN/m	
TOTAL	:	=	2.79 kN/m	
LL	:	Wll =	0.25 kN/m	
LL factor - long term			0.4	
- short term			0.7	

### 2. Beam Section Properties

bw	250	mm
D	350	mm
tff	0	mm
Cover	65	mm
D = Overall depth of section	350	mm
Actual length, L	6900	mm
Effective Length, L <sub>ef</sub>	6500	mm
Area, A	8.75E+04	mm <sup>2</sup>
Neutral Axis, y <sub>c</sub>	175.00	mm
Moment of Inertia, I <sub>x-x</sub>	8.93E+08	mm <sup>4</sup>
Neutral Axis, x <sub>c</sub>	775.00	mm
Moment of Inertia, I <sub>y-y</sub>	4.56E+08	mm <sup>4</sup>
Effective width, b <sub>eff</sub>	1550.00	mm
Gross Area, A <sub>g</sub>	8.75E+04	mm <sup>2</sup>

### 3. Design Parameters

F' <sub>c</sub> =	40	MPa
F <sub>sy</sub> =	410	MPa
m	2400	kg/m <sup>3</sup>
D =	350	mm
E <sub>c</sub> =	31975.3505	MPa
E <sub>s</sub> =	200000	MPa
Gamma =	0.766	

### 4. INITIAL CHOICE OF SECTION

Effective width =	1550 mm	clause 8.8.2	
Is the section RECT or T, L (ans : 1 or 2)	=		1
k <sub>1</sub> =	0.045	(RECT sections)	1
	= 0.01886216	(T and L sections)	2
what type of span?	5	interior span =	5
		end span =	6
k <sub>2</sub> =	0.0026042		
k <sub>cs</sub> =	1.64	(because there will be comp steel at midspan=say .5A <sub>st</sub> )	
F <sub>def</sub> =	4.9105		
defln/L <sub>ef</sub> =	0.004	k <sub>1</sub> /k <sub>2</sub> =	17.2797788
d =	74 mm		
take d =	253 mm	Distance from d to extreme fibre of beam (d <sub>o</sub> ) =	97 mm

**5. Forces from an Elastic Analysis by Space Gass :**

Mmax -ve	=	21.10 kNm	(left support)
Mmax +ve	=	21.10 kNm	(mid-span)
M -ve	=	21.10 kNm	(right support)
Vmax	=	13.00 kN	
V (other end)	=	13.00 kN	
N*	=	0.00 kN	Axial Load

**6. FLECTURAL STRENGTH STEEL**

Determine -ve steel required: (At arbitrary left support)

Number of reinforcement layers	1 or 2 :	1	
take do =	97 mm	d =	253
Estimate Area of main steel	Ast =	265.898962 mm <sup>2</sup>	
3 Y 20	Ast =	942.477796 mm <sup>2</sup>	
	p =	0.01490083	
3 Y 16	Asc =	603.185789 mm <sup>2</sup>	
	pc =	0.00953653	< pd = 0.025409
	p-pc =	0.0053643	pd = 0.4 * 0.85 * Mu * f'c / fsy

Check M\* <= phiMu

ku =	0.084448	dsc =	85 mm
------	----------	-------	-------

$$\phi Mu = \phi [ Fsy \times Asc (d - dsc) + 0.85 \times F'c \times b \times \text{Gamma} \times ku \times d \times (d - 0.5 \times 0.85 \times ku \times d) ]$$

=	68 kNm	>	M* =	21.10 kNm	OK
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Determine -ve steel at right support:

M -ve	=	21.10 kNm	
Number of reinforcement layers	1 or 2 :	1	
take do =	97 mm	dst =	253 mm
Estimate Area of main steel	Ast =	265.898962 mm <sup>2</sup>	
3 Y 20	Ast =	942.477796 mm <sup>2</sup>	
	p =	0.01490083	
3 Y 16	Asc =	603.185789 mm <sup>2</sup>	
	pc =	0.00953653	< pd = 0.025409
	p-pc =	0.0053643	

Check M\* <= phiMu

ku =	0.084448	dsc =	85 mm
------	----------	-------	-------

$$\phi Mu = \phi [ Fsy \times Asc (d - dsc) + 0.85 \times F'c \times b \times \text{Gamma} \times ku \times d \times (d - 0.5 \times 0.85 \times ku \times d) ]$$

=	68 kNm	>	M* =	21.10 kNm	OK
---	--------	---	------	-----------	----

FOR DUCTILITY CHECK AT BOTH SUPPORTS USE EMPHI

**Tensile Steel at Midspan:**

M*	=	21.1 kNm	
Steel in one layer then	code = 1	take dist between centroid of	
" " two " "	code = 2	bar/s and soffit =	97 mm
what is the code?	1	d =	253 mm
Ast =	M*/(Phi x 0.85 x d x Fsy)	=	265.898962 mm <sup>2</sup>
Bottom	3 Y 20	Ast =	942.477796 mm <sup>2</sup>
Top	3 Y 16	Asc =	603.185789 mm <sup>2</sup>
Assume	N - A in flange		
Effective width :	b <sub>eff</sub> =	1550 mm	
ku =	1/(0.85 x 0.85) x [Ast/(bw x d)] x (Fsy/F'c)		
	=	0.03783512	
hence	dn =	10	< flange thickness
			hence N-A is in flange

$$\Phi \mu = 0.9 [F_{sy} \times A_{st} \times d (1 - 0.5 \times 0.85 \times k_u)]$$

= 87 kNm	>	M*+ve = 21.10 kNm	OK
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Summary of reinforcement requirement :

left support :	A <sub>st</sub> =	3	Y 20
	A <sub>sc</sub> =	3	Y 16
Right support :	A <sub>st</sub> =	3	Y 20
	A <sub>sc</sub> =	3	Y 16
midspan	A <sub>st</sub> =	3	Y 20
	A <sub>sc</sub> =	3	Y 16

### 7. STEEL CURTAILMENT REQUIREMENT CHECK

The following requirements regarding curtailment are subject to stress development length requirements.

- For -ve steel
- i) 1/4 A<sub>st</sub> to continue over length of beam = 2 bars
  - ii) Remainder to be curtailed at 0.3L<sub>n</sub> from face of support.
    - = 0.3 x l<sub>e</sub> = 1950 mm ?
- For -ve steel
- i) 1/4 A<sub>st</sub> to continue over length of beam = 2 bars
  - ii) Remainder to be curtailed at 0.3L<sub>n</sub> from face of support.
    - = 0.3 x l<sub>e</sub> = 1950 mm ?
- For +ve steel :
- i) 1/2 A<sub>st</sub> must be carried into outer support = 2 bars
  - ii) 1/4 A<sub>st</sub> into interior support = 2 bars
  - iii) Remainder curtailed at 0.1L<sub>n</sub> from supports
    - = 0.1 x l<sub>e</sub> = 650 mm

### 8. STRESS DEVELOPMENT

For Top Steel:

cover : c = 65 mm

d<sub>b</sub> = 16 mm # of lines of bars = 2

a<sub>b</sub> = 48 mm distance between bars

If a<sub>b</sub> > 2c C = 2c + d<sub>b</sub> = 146 mm  
C is the outside diameter of a concrete annulus

if a<sub>b</sub> <= 2c C = a<sub>b</sub> + d<sub>b</sub> = 64 mm  
coaxial with and surrounding a bar

k<sub>1</sub> = 1.25 A<sub>b</sub> = 201.06193 mm<sup>2</sup>

k<sub>2</sub> = 2.2 a<sub>b</sub> = 48 mm (dist between bars)

factor = 35843.9945

L<sub>syt</sub> = k<sub>1</sub> x k<sub>2</sub> x f<sub>sy</sub> x A<sub>b</sub>/C x sqrt(f'<sub>c</sub>) = factor/C

= 560.0624 mm

take f<sub>s</sub>/F<sub>sy</sub> = M\*/Phi Mu = 0.310608659

L <sub>syt</sub> =	174 mm	≥	25k1db	500
say =	500 mm			

For Bottom Steel:

Cover, c = 65 mm d<sub>b</sub> = 20 mm

# of lines of bars = 2

If a<sub>b</sub> > 2c C = 2c + d<sub>b</sub> = 150 mm

if a<sub>b</sub> < 2c C = a<sub>b</sub> + d<sub>b</sub> = 222 mm

k<sub>1</sub> = 1 A<sub>b</sub> = 314.159265 mm<sup>2</sup>

k<sub>2</sub> = 2.2 a<sub>b</sub> = 202 mm (dist between bars)

factor = 44804.9931

L<sub>syt</sub> = 298.7 mm take f<sub>s</sub>/F<sub>sy</sub> = M\*/Phi Mu = 0.243334552

L <sub>syt</sub> =	73 mm	≥	25k1db	400
say =	400 mm			

**9. SHEAR CHECK**

$V^* = 13.00 \text{ kN}$  Dist to centreline of bottom steel from soffit  
critical location :  $d = 253 \text{ mm}$   $d_q = 93 \text{ mm}$   
 $do = 257 \text{ mm}$   
 $V^* = 12 \text{ kN}$  hence  $beta 1 = 1.2715$  (but not less than 1.1)  
 $beta 2 = 1$   
 $beta 3 = 1$  Unity  
 $\phi Vuc = \phi \times beta 1 \times beta 2 \times beta 3 \times bw \times do [Ast \times F'c / bw \times do]^{0.333}$   
 $= 47.88 \text{ kN}$   
 $\phi Vumax = 359.8 \text{ kN} > V^*$  No web crushing  
 $\phi Vumin = 75 \text{ kN} > V^*$  OK

If shear reinforcement required then proceed as follows :  
 choose stirrup spacing  $s = 350 \text{ mm}$   $Fsyf = 410 \text{ MPa}$   
 Determine theta :

$bv \times s / Fsyf = 213.414634$   
 $Asv.min = 74.695122 \text{ mm}^2$   
 $Asv.max = 1480 \text{ mm}^2$   
 $Asv = 2 \times \text{area of stirrups} = 220 \text{ mm}^2$  2 Ties  
 $theta = 31.55085 \text{ degrees}$   
 Determine  $\Phi Vu$  :  
 $Vu = Vuc + Vus$   
 $= Vuc + Asv / s \times Fsyf \times do \cot theta$   
 $= 176.27 \text{ kN}$   
 $\Phi Vu = 123 \text{ kN} > V^* = 12.2315125 \text{ kN}$   
 hence

<b>PROVIDE :</b>	<b>1 Y12 LIGS</b>	<b>@</b>	<b>350</b>	<b>centres</b>
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Other Support :

$V^* = 13.00 \text{ kN}$  Dist to centreline of bottom steel from soffit  
critical location :  $d = 253 \text{ mm}$   $d_q = 93 \text{ mm}$   
 $do = 257 \text{ mm}$   
 $V^* = 12.23 \text{ kN}$  hence  $beta 1 = 1.2715$  (but not less than 1.1)  
 $beta 2 = 1$   
 $beta 3 = 1$   
 $\phi Vuc = \phi \times beta 1 \times beta 2 \times beta 3 \times bw \times do [Ast \times F'c / bw \times do]^{0.333}$   
 $= 47.8833 \text{ kN}$   
 $\phi Vumax = 359.8 \text{ kN} > V^*$  No web crushing  
 $\phi Vumin = 75 \text{ kN} < V^*$  OK

If shear reinforcement required then proceed as follows :  
 choose stirrup spacing  $s = 350 \text{ mm}$   $Fsyf = 410 \text{ MPa}$   
 Determine theta :

$bv \times s / Fsyf = 213.414634$   
 $Asv.min = 74.695122 \text{ mm}^2$   
 $Asv.max = 1480.10204 \text{ mm}^2$   
 $Asv = 2 \times \text{area of stirrups} = 330 \text{ mm}^2$  2 Ties  
 $theta = 32.72489 \text{ degrees}$   
 Determine  $\Phi Vu$  :  
 $Vu = Vuc + Vus$   
 $= Vuc + Asv / s \times Fsyf \times do \cot theta$   
 $= 223.0087 \text{ kN}$   
 $\Phi Vu = 156.1061 \text{ kN} > V^* = 12.23 \text{ kN}$   
 hence

<b>PROVIDE :</b>	<b>1 Y12 LIGS</b>	<b>@</b>	<b>350</b>	<b>centres</b>
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### 10. SERVICEABILITY CHECK

From an Elastic Linear Analysis :

Mr	=	21.10 kNm	Es	=	200000 MPa
MI	=	21.10 kNm	Ec	=	31975.35051 MPa
Mo	=	42.2 kNm			
Ms	=	21.10 kNm			
Ig	=	8.93E+08 mm <sup>4</sup>			

Calculate I<sub>cr</sub>, M<sub>cr</sub>, and I<sub>ef</sub> at midspan :

Eff. Flange width	b <sub>eff</sub> =	1550 mm	A <sub>sc</sub> =	603.1857895 mm <sup>2</sup>
webwidth	b <sub>w</sub> =	250 mm	A <sub>st</sub> =	942.4777961 mm <sup>2</sup>
Flange thickness	t <sub>ff</sub> =	0 mm	d <sub>sc</sub> =	97 mm
			d <sub>st</sub> =	253 mm
			d =	253 mm
			D =	350 mm

$$n = E_s/E_c = 6.25481807$$

$$1/2 \times b_w \times d_n^2 + (b_{eff} - b_w) \times t \times (d_n - t/2) + (n - 1) A_{sc} (d_n - d_{sc}) = n A_{st} (d - d_n)$$

$$125 d_n^2 + 9064.65873 d_n - 1798896.13 = 0$$

$$d_n = 89.0643784 \text{ mm}$$

$$d = 253 \text{ mm}$$

$$I_{cr} = b d_n^3/3 + 1/12(b_{eff} - b_w) \times t^3 + (b_{eff} - b_w) \times t \times (d_n - t/2)^2 + n x A_{st} (d - d_n)^2$$

$$= 2.17E+08 \text{ mm}^4$$

$$M_{cr} = 0.6 \sqrt{f'_c} \times b D^2/6 = 19.3689507 \text{ kNm}$$

Im	=	[I <sub>cr</sub> + (I <sub>g</sub> - I <sub>cr</sub> ) (M <sub>cr</sub> / Ms) <sup>3</sup> ]
	=	7.40E+08 mm <sup>4</sup>

Calculate I<sub>cr</sub>, M<sub>cr</sub>, I<sub>lef</sub> and I<sub>ref</sub> at end supports:

b <sub>w</sub> =	250 mm	A <sub>st</sub> =	942.477796 mm <sup>2</sup>
d <sub>sc</sub> =	85 mm	A <sub>sc</sub> =	603.185789 mm <sup>2</sup>
d =	253 mm		
Ig =	8.93E+08 mm <sup>4</sup>		

At right end (arbitrary)

$$1/2 \times b d_n^2 + (n - 1) A_{sc} (d_n - d_{sc}) = n A_{st} (d - d_n)$$

$$125 d_n^2 + 9064.65873 d_n - 1760860.55 = 0$$

$$d_n = 87.84 \text{ mm}$$

$$d = 253 \text{ mm}$$

$$I_{cr} = b d_n^3/3 + n \times A_{sc} (d - d_n)^2 = 2.17E+08 \text{ mm}^4$$

$$M_{cr} = 0.6 \sqrt{f'_c} \times b D^2/6 = 19.3689507 \text{ kNm}$$

$$I_{ref} = [I_{cr} + (I_g - I_{cr}) (M_{cr} / Mr)^3] = 7.40E+08 \text{ mm}^4$$

At left end (arbitrary)

b <sub>w</sub> =	250 mm		
d <sub>sc</sub> =	85 mm		
A <sub>st</sub> =	942.4778 mm <sup>2</sup>		
A <sub>sc</sub> =	603.18579 mm <sup>2</sup>		
d =	253 mm		
1/2 x b d <sub>n</sub> <sup>2</sup> + (n - 1) A <sub>sc</sub> (d <sub>n</sub> - d <sub>sc</sub> ) = n A <sub>st</sub> (d - d <sub>n</sub> )			
125 d <sub>n</sub> <sup>2</sup> + 9064.65873 d <sub>n</sub> - 1760860.55	=	0	
d <sub>n</sub> =	87.8444393 mm		
d =	253 mm		

$$I_{cr} = b d_n^3/3 + n \times A_{sc} (d - d_n)^2 = 2.17E+08 \text{ mm}^4$$

$$M_{cr} = 0.6 \sqrt{f'_c} \times b D^2/6 = 19.3689507 \text{ kNm}$$

$$I_{lef} = [I_{cr} + (I_g - I_{cr}) (M_{cr} / Ml)^3] = 7.40E+08 \text{ mm}^4$$

I <sub>av</sub>	=	[Im + (I <sub>l</sub> + I <sub>r</sub> )/2]/2	=	7.40E+08 mm <sup>4</sup>
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Midspan Deflection :

$$L_{ef} = 6500 \text{ mm}$$

$$Defl_{n,s} = L_{ef}^2/E_c \times I_{av} [5/48 \cdot M_o - 1/16 ML - 1/16 Mr]$$

	=	3.13904315 mm
--	---	---------------

**Total Deflection :**

Long term factor = 0.2  
Short term factor = 0.5  
w - long term = 2.8375 kN/m  
w - short term = 2.9125 kN/m  
Defln,s,sus = 3.058209418 mm  
At midspan : Ast = 942.477796 mm<sup>2</sup>  
Asc = 603.185789 mm<sup>2</sup>  
Asc/Ast = 0.64  
kcs = 2 - (1.2 x Asc/Ast) >= 0.8  
= 1.232

therefore

Total Defln	=	Dfln,s + ( kcs x Defln,s,sus )
	=	6.90676 mm

FROM A MORE ACCURATE ANALYSIS Warner,Rangan,Hall HAVE SHOWN THAT THIS CAN BE REDUCED BY UP TO 10%.

ALLOWABLE DEFLN	=	Lef / 500 =	13 mm
			> 0.9 x Total Defln
ALLOWABLE DEFLN	=	Lef / 250 =	26 mm
			> 0.9 x Total Defln

## RC COLUMN DESIGN WITH MRCSECT PACKAGE

INTERACTION DIAGRAM FOR RECTANGULAR COLUMN SECTION  
(NON-PRESTRESSED REINFORCEMENT ONLY)

### INPUT

400 x 400 RC COLUMN DESIGN

Using RECTANGULAR STRESS BLOCK

Width (mm).....= 400 mm  
 Depth (mm).....= 400 mm  
 No of lines of reinforcement.= 3  
 Concrete strength = 32 MPa  
 Yield stress of steel..= 410 Mpa

### STEEL DETAILS

Line no	Depth to steel	Bar diameter	No of bars
1	87	20	3
2	200	16	2
3	317	20	3

### OUTPUT: MRCSECT ANALYSIS RESULTS

Depth to Plastic Centroid (Ref Axis for Moment) = 200.00 mm

### SERVICE ENVELOPE

Eccentricity (mm)	Moment (KNm)	Force (KN)	Top strain	Bottom strain	N.A.factor Ku
0.00	0.00	5042.36	0.003	0.003	1000.000
37.35	157.12	4206.89	0.003	0.000	1.000
52.15	199.15	3818.58	0.003	-0.0003	0.909
69.44	234.91	3382.86	0.003	-0.0007	0.811
90.85	261.44	2877.57	0.003	-0.0013	0.892
132.69	278.06	2095.48	0.003	-0.0024	0.710
147.89	280.26	1602.46	0.003	-0.0033	0.620
209.93	267.32	1273.40	0.003	-0.0042	0.566
313.89	232.82	741.72	0.003	-0.0620	0.475
409.53	216.90	529.81	0.003	-0.0072	0.439
834.42	181.74	217.80	0.003	-0.0097	0.352
1667.05	167.34	100.38	0.003	-0.0111	0.322
	154.84	-0.56	0.003	-0.0123	0.312

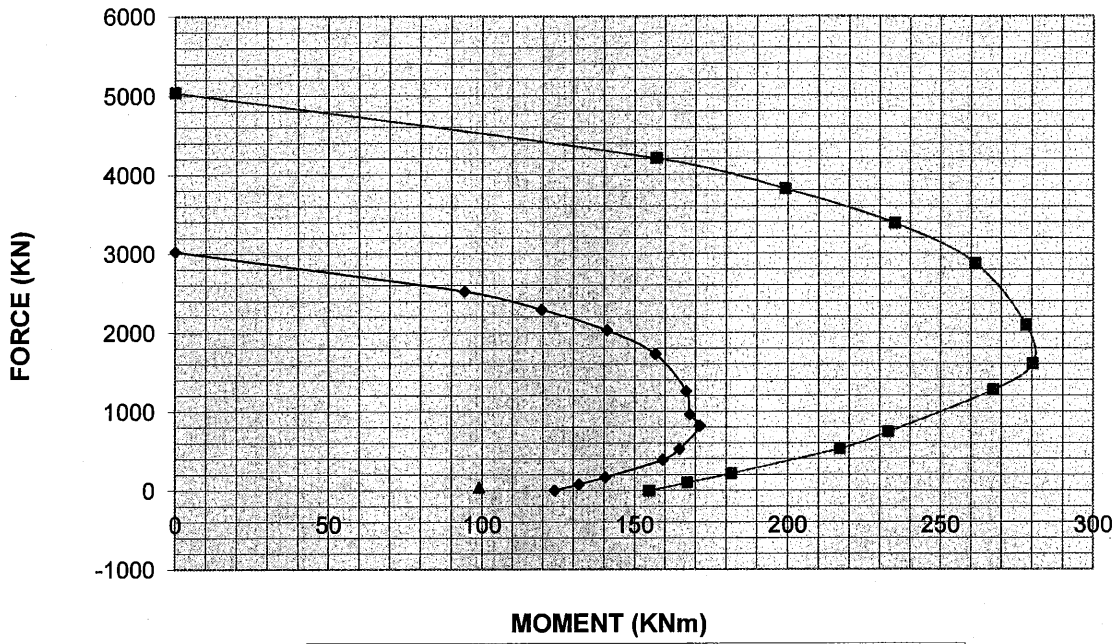
### DESIGN ENVELOPE

PHI	ØMOMENT (KNm)	ØFORCE (KN)
0.6	0.000	3027.218
0.6	94.272	2524.134
0.6	119.489	2291.146
0.6	140.945	2029.718
0.6	156.8622	1726.544
0.6	166.833	1257.291
0.6	168.16	961.478
0.64107	171.37	816.344
0.707428	164.70	524.712
0.73388	159.23	388.813
0.77282	140.45	168.322
0.78747	131.77	79.046
0.8	123.87	-0.447

### SERVICE FORCES

M* =	99.00 KNm
N* =	41.00 KNm

RC COLUMN  
INTERACTION DIAGRAM FOR A 400 SQR BLOCK PIER - 8-Y20



## STEEL BEAM DESIGN

PROJECT Blower Control Room  
 JOB NO. 11006  
 MEMBER DESIGNED ROOF BEAM DESIGN  
 DESIGNED BY TT  
 DATE 10/08/2011  
 CHECKED BY  
 DATE

	TRIAL SIZE =	310 UB 40.4	
d1=	284 mm	Es	2.00E+05 Mpa
tf=	10.2 mm	G	8.00E+04 Mpa
tw =	6.1 mm	J	1.57E+05 mm4
Ag =	5210 mm2	Iw	1.65E+11 mm6
Ix =	8.64E+07 mm4	Stee Grade	300 Mpa
Iy =	7.65E+06 mm4	L	4.9 m
Zex =	5.69E+05 mm3	Lef =	4.5 m
ry =	38.3 mm		

Unfactored Loads	Load Factors	
WDI	0.4 DI	1.4
WLL	0 LL	1.7
PDL	0	
PLL	37.5	

Long Term Loads	Short Term Loads
WG = 0.56	WG = 0
WQ = 0	WQ = 0
PG = 0	PG = 0
PQ = 0	PQ = 63.75

M\* = 59 KNm  
 V\* = 34 KN

Section Slenderness = Non-compact about both x- and y- axis

fyf = 320 MPa  
 fyw = 320 MPa

kt = twist restraint factor.

Using AS4100 Table 5.6.3(1) for restraint arrangement

FP

The equation is to change depending of restraint

$$k_t = 1 + (2d_1/l)(0.5t_f/t_w)^3 = 1.03 \quad F_u = 1.03$$

le = effective length (AS4100 Clause 5.6.3)

$$le = k_t k_l k_r l_s$$

$k_t = \text{see Note 1 in Table 5.2.1} = 1.03$   
 $k_l = \text{loads acting on top flange, acting downwards} = 1$   
 $k_r = \text{see Section 5.4.2 also} = 1$   
 $l_s = \text{distance between lateral restraints (LRs) = distance between support intermediate restraint the supports} = 4.9 \text{ m}$

$$le = k_t k_l k_r l_s$$

$le = 5 \text{ m}$   
 $5065.974 \text{ mm} \quad \text{Say} \quad 5000 \text{ mm}$

Mo = reference bucklin given by AS4100 Equation 5.6.1.1(3):  
Equation is split into 3 part A, B and C in these calculations:

$$\text{Let A} = \frac{\pi^2 E I_y}{le^2} = 6.04E+05 \text{ N}$$

$$\text{Let B} = \frac{\pi^2 E I_w}{le^2} = 1.30E+10 \text{ Nmm}^2$$

$$\text{Let C} = GJ = 1.26E+10 \text{ Nmm}^2$$

Equations are recombined int:

$$M_o = \sqrt{A(C+B)} = 1.24E+08 \text{ Nmm} = 124 \text{ KNm}$$

Max = Ms = nominal section moment capacity

$$= f_y Z_{ex} \quad \text{As 4100 Clause 5.2.1}$$

$$M_s = 1.82E+08 \text{ Nmm} = 182.08 \text{ KNm}$$

$$\phi M_{sx} =$$

$$\phi = 0.9 \text{ AS 4100 Table 3.4}$$

$$\phi M_{sx} = 163.872 \text{ KNm} > 59 \text{ OK}$$

as = slenderness reduction factor AS4100 Equation 5.6.1.1(2)

$$a_s = 0.6 \left( \sqrt{\frac{M_s^2}{M_o^2} + 3} \right) - M_s/M_o$$

$$a_s = 0.482202$$

$\phi M_b$  = Member Moment Capacity AS 4100 Equation 5.6.1.1(1)

$$M_b = \phi \alpha_m \alpha_s M_s \leq M_a$$
$$1.7 M_m^* / ((M_2^*)^2 + (M_3^*)^2 + (M_4^*)^2)^{0.5}$$

$M_m^*$ =	59 KNm	Max Design BMin the Segment
$M_2^*$ =	3 KNm	Design BM at 1/4 Points of Segment
$M_3^*$ =	17 KNm	Design BM at 1/2 Point of Segment
$M_4^*$ =	38 KNm	Design BM at 1/4 Points of Segment

$$\alpha_m = 2.403128$$

$$\phi M_b = 189.8938 \text{ KNm}$$

$M^* \leq \phi M_b$  must be satisfied

$$= 59 \leq 189.893814 \text{ OK}$$

$$\text{EFFICIENCY} = 31 \%$$

### Service ability design actions/loads for deflection

$$\text{Permissible Deflection } \Delta = l/250 = 20 \text{ mm}$$

Actual Deflection  $\delta \leq \Delta = l/250 =$  Must be Satisfied:

$$\Delta = 14 \leq 20 \text{ OK}$$

Space Gas Value =

## STEEL BEAM DESIGN

PROJECT	Blower Control Room
JOB NO.	11006
MEMBER DESIGNED	HOIST BEAM
DESIGNED BY	TT
DATE	4/11/2011
CHECKED BY	
DATE	

	TRIAL SIZE =	310 UB 40.4	
d1=	284 mm	Es	2.00E+05 Mpa
tf=	10.2 mm	G	8.00E+04 Mpa
tw =	6.1 mm	J	1.57E+05 mm <sup>4</sup>
Ag =	5210 mm <sup>2</sup>	Iw	1.65E+11 mm <sup>6</sup>
Ix =	8.64E+07 mm <sup>4</sup>	Stee Grade	300 Mpa
Iy =	7.65E+06 mm <sup>4</sup>	L	4.9 m
Zex =	5.69E+05 mm <sup>3</sup>	Lef =	4.5 m
ry =	38.3 mm		

Unfactored Loads	Load Factors
WDI            0.4 DI	1.4
WLL            0 LL	1.7
PDL            0	
PLL            37.5	

Long Term Loads	Short Term Loads
WG =            0.56 WG =	0
WQ =            0 WQ =	0
PG =            0 PG =	0
PQ =            0 PQ =	63.75

M\* =            71 KNm  
V\* =            35 KN

Section Slenderness = Non-compact about both x- and y- axis

fyf =            320 MPa  
fyw =            320 MPa

kt =            twist restraint factor.  
Using AS4100 Table 5.6.3(1) for restraint arrangement            FP  
The equation is to change depending of restraint



$$k_t = 1 + (2d_1/l)(0.5t_f/t_w)^3 \quad 1.03 \quad F_u \quad 1.03$$

$$K_t = 1.033872$$

le = effective length (AS4100 Clause 5.6.3)

$$le = k_t k_l k_r l_s$$

$$k_t = \text{see Note 1 in Table 5.2.1} \quad k_t = 1.03$$

$$k_l = \text{loads acting on top flange, acting downwards} \quad 1$$

$$k_r = \text{see Section 5.4.2 also} \quad 1$$

$$l_s = \text{distance between lateral restraints (LRs)} = 4.9 \text{ m}$$

s distance between support intermediate restraint  
the supports

$$le = k_t k_l k_r l_s$$

$$le = 5 \text{ m} \quad \text{Say} \quad 5000 \text{ mm}$$

$$5065.974 \text{ mm}$$

Mo = reference bucklin given by AS4100 Equation 5.6.1.1(3):  
Equation is split into 3 part A, B and C in these calculations:

$$\text{Let A} = \frac{\pi^2 E I_y}{le^2} = 6.04E+05 \text{ N}$$

$$\text{Let B} = \frac{\pi^2 E I_w}{le^2} = 1.30E+10 \text{ Nmm}^2$$

$$\text{Let C} = GJ = 1.26E+10 \text{ Nmm}^2$$

Equations are recombined int:

$$M_o = \sqrt{A(C+B)} = 1.24E+08 \text{ Nmm}$$

$$124 \text{ KNm}$$

Max = Ms = nominal section moment capacity

$$= f_y Z_{ex} \quad \text{As 4100 Clause 5.2.1}$$

$$M_s = 1.82E+08 \text{ Nmm}$$

$$182.08 \text{ KNm}$$

$$\phi M_{sx} =$$

$$\phi = 0.9 \text{ AS 4100 Table 3.4}$$

$$\phi M_{sx} = 163.872 \text{ KNm} > 71 \text{ OK}$$

as = slenderness reduction factor AS4100 Equation 5.6.1.1(2)

$$a_s = 0.6 \left( \sqrt{\frac{M_s^2}{M_o^2} + 3} \right) - M_s/M_o$$

$$a_s = 0.482202$$

$\phi M_b$  = Member Moment Capacity AS 4100 Equation 5.6.1.1(1)

$$M_b = \phi a m a s M_s \leq M_a$$
$$1.7 M m^* / ((M_2^*)^2 + (M_3^*)^2 + (M_4^*)^2)^{0.5}$$

$M m^*$ =	71 KNm	Max Design BMin the Segment
$M_2^*$ =	3 KNm	Design BM at 1/4 Points of Segment
$M_3^*$ =	17 KNm	Design BM at 1/2 Point of Segment
$M_4^*$ =	38 KNm	Design BM at 1/4 Points of Segment

$$a m = 2.891899$$

$$\phi M_b = 228.5163 \text{ KNm}$$

$M^* \leq \phi M_b$  must be satisfied

$$= 71 \leq 228.516285 \text{ OK}$$

$$\text{EFFICIENCY} = 31 \%$$

### Service ability design actions/loads for deflection

$$\text{Permissible Deflection } \Delta = l/250 = 20 \text{ mm}$$

Actual Deflection  $\delta \leq \Delta = l/250 =$  Must be Satisfied:

$$\Delta = 13 \leq 20 \text{ OK}$$

Space Gas Value =

## RC BEAM DESIGN

**Base Slab - Designed as a Continuous Beam :**

REF. AS 3600

### 1. Loads

DL	:	Wdl	=	0.50 kN/m	(plus super)
Self wt	:	Wsw	=	19.20 kN/m	
Blockwall	:	Wb	=	0.00 kN/m	
TOTAL	:		=	19.70 kN/m	
LL	:	Wll	=	2 kN/m	
LL factor - long term				0.4	
				- short term	0.7

### 2. Beam Section Properties

bw	1000	mm
D	850	mm
tff	800	mm
Cover	75	mm
D = Overall depth of section	1650	mm
Actual length, L	2850	mm
Effective Length, L <sub>ef</sub>	2550	mm
Area, A	1.26E+06	mm <sup>2</sup>
Neutral Axis, y <sub>c</sub>	433.11	mm
Moment of Inertia, I <sub>x-x</sub>	7.41E+10	mm <sup>4</sup>
Neutral Axis, x <sub>c</sub>	755.00	mm
Moment of Inertia, I <sub>y-y</sub>	2.34E+11	mm <sup>4</sup>
Effective width, b <sub>eff</sub>	1399.00	mm
Gross Area, A <sub>g</sub>	1.97E+06	mm <sup>2</sup>

### 3. Design Parameters

F' <sub>c</sub> =	40	MPa
F <sub>sy</sub> =	410	MPa
m	2400	kg/m <sup>3</sup>
D =	850	mm
E <sub>c</sub> =	31975.3505	MPa
E <sub>s</sub> =	200000	MPa
Gamma =	0.766	

### 4. INITIAL CHOICE OF SECTION

Effective width =	1510 mm	clause 8.8.2	
Is the section RECT or T, L (ans: 1 or 2)	=		1
k <sub>1</sub> =	0.045	(RECT sections)	1
	0.03266038	(T and L sections)	2
what type of span?	6	interior span =	5
		end span =	6
k <sub>2</sub> =	0.0054054		
k <sub>cs</sub> =	1.64	(because there will be comp steel at midspan=say .5A <sub>st</sub> )	
F <sub>def</sub> =	35.02		
defln/L <sub>ef</sub> =	0.004	k <sub>1</sub> /k <sub>2</sub> =	8.32500833
d =	71 mm		
take d =	759 mm	Distance from d to extreme fibre of beam (d <sub>o</sub> ) =	103 mm

**5. Forces from an Elastic Analysis by Space Gass :**

Mmax -ve = 2300.00 kNm (left support)  
 Mmax +ve = 773.00 kNm (mid-span)  
 M -ve = 711.00 kNm (right support)  
 Vmax = 445.00 kN  
 V (other end) = 160.00 kN

759

**6. FLECTURAL STRENGTH STEEL**

Determine -ve steel required: (At arbitrary left support)

Number of reinforcement layers 1 or 2 : 1  
 take do = 103 mm d = 1547  
 Estimate Area of main steel Ast = 4740.15159 mm<sup>2</sup> Say  
 6 Y 32 Ast = 4825.48632 mm<sup>2</sup>  
 at 252 CTS p = 0.00635769  
 4 Y 28 Asc = 2463.00864 mm<sup>2</sup>  
 at 377.5 pc = 0.00159212 < pd = 0.025409  
 p-pc = 0.00476557  
 Astmin = 4011.315 mm<sup>2</sup>  
 Spacing = 42.537173 mm  
 Y32 at 170 CTS

**Check M\* <= phiMu**

ku = 0.075022 dsc = 101 mm

$$\phi Mu = \phi [ Fsy \times Asc (d - dsc) + 0.85 \times F'c \times b \times \Gamma \times ku \times d \times (d - 0.5 \times 0.85 \times ku \times d) ]$$

=	6649	kNm	>	M*	=	2300.00	kNm	OK
---	------	-----	---	----	---	---------	-----	----

Determine -ve steel at right support:

M -ve = 711.00 kNm  
 Number of reinforcement layers 1 or 2 : 1  
 take do = 119 mm dst = 1531 mm  
 Estimate Area of main steel Ast = 1480.63877 mm<sup>2</sup>  
 6 Y 32 Ast = 4825.48632 mm<sup>2</sup>  
 p = 0.00315185  
 4 Y 32 Asc = 3216.99088 mm<sup>2</sup>  
 pc = 0.00210124 < pd = 0.025409  
 p-pc = 0.00105062  
 Astmin = 4011.315 mm<sup>2</sup>  
 Spacing = 42.537173 mm  
 Y12 at 170 CTS

**Check M\* <= phiMu**

ku = 0.016539 dsc = 103 mm

$$\phi Mu = \phi [ Fsy \times Asc (d - dsc) + 0.85 \times F'c \times b \times \Gamma \times ku \times d \times (d - 0.5 \times 0.85 \times ku \times d) ]$$

=	3016	kNm	>	M*	=	711.00	kNm	OK
---	------	-----	---	----	---	--------	-----	----

FOR DUCTILITY CHECK AT BOTH SUPPORTS USE EMPHI

**Tensile Steel at Midspan:**

M\* = 773 kNm  
 Steel in one layer then code = 1 take dist between centroid of bar/s and soffit = 119 mm  
 " " two " " code = 2  
 what is the code? 1 d = 759 mm  
 Ast = M\*/(Phi x 0.85 x d x Fsy) = 3247.07579 mm<sup>2</sup>  
 Bottom 6 Y 32 Ast = 4825.48632 mm<sup>2</sup>  
 Top 4 Y 28 Asc = 2463.00864 mm<sup>2</sup>  
 Assume N - A in flange  
 Effective width : beff = 1510 mm  
 ku = 1/(0.85 x 0.85) x [Ast/(bw x d)] x (Fsy/F'c)  
 = 0.06628245

hence dn = 50 < flange thickness  
hence N-A is in flange

Ast Governs - Use Y28 at 251.6666667 CTS Astmin 4011.315

$$\Phi \mu = 0.9 [F_{sy} \times A_{st} \times d (1 - 0.5 \times 0.85 \times k_u)]$$

=	1317 kNm	>	M*+ve =		773.00 kNm	OK
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Summary of reinforcement requirement :

left support :	Ast =	Y32	250 CTS
	Asc =	Y32	250 CTS
Right support :	Ast =	Y32	250 CTS
	Asc =	Y32	250 CTS
midspan	Ast =	Y32	250 CTS
	Asc =	Y32	250 CTS

### 7. STEEL CURTAILMENT REQUIREMENT CHECK

The following requirements regarding curtailment are subject to stress development length requirements.

- i)  $1/4 A_{st}$  to continue over length of beam
- For -ve steel = 2 bars
- ii) Remainder to be curtailed at  $0.3L_n$  from face of support.
- =  $0.3 \times l_e = 765 \text{ mm}$  ?
- i)  $1/4 A_{st}$  to continue over length of beam
- For -ve steel = 2 bars
- ii) Remainder to be curtailed at  $0.3L_n$  from face of support.
- =  $0.3 \times l_e = 765 \text{ mm}$  ?
- For +ve steel :
- i)  $1/2 A_{st}$  must be carried into outer support
- = 2 bars
- ii)  $1/4 A_{st}$  into interior support
- = 2 bars
- iii) Remainder curtailed at  $0.1L_n$  from supports
- =  $0.1 \times l_e = 255 \text{ mm}$

### 8. STRESS DEVELOPMENT

For Top Steel :

cover :  $c = 75 \text{ mm}$

$d_b = 28 \text{ mm}$  # of lines of bars = 2

$a_b = 413 \text{ mm}$  distance between bars

If  $a_b > 2c$   $C = 2c + d_b = 178 \text{ mm}$   
C is the outside diameter of a concrete annulus

if  $a_b \leq 2c$   $C = a_b + d_b = 441 \text{ mm}$   
coaxial with and surrounding a bar

$k_1 =$  [redacted]  $A_b = 615.75216 \text{ mm}^2$

$k_2 =$  [redacted]  $a_b = 413 \text{ mm}$  (dist between bars)

factor = 109772.233

$L_{sy} = k_1 \times k_2 \times f_{sy} \times A_b / C \times \sqrt{f'_c} = \text{factor} / C$

= 616.6979 mm

take  $f_s / F_{sy} = M^* / \Phi \mu = 0.345915812$

therefore 

$L_{sy} =$	213 mm	$\geq$	25k1db	875
say =	200 mm			

For Bottom Steel :

Cover,  $c = 75 \text{ mm}$   $d_b = 32 \text{ mm}$

# of lines of bars = 2

If  $a_b > 2c$   $C = 2c + d_b = 182 \text{ mm}$

if  $a_b < 2c$   $C = a_b + d_b = 619 \text{ mm}$

$k_1 = 1$   $A_b = 804.247719 \text{ mm}^2$

$k_2 = 2.2$   $a_b = 587 \text{ mm}$  (dist between bars)

factor = 114700.782

$L_{sy} = 630.2241 \text{ mm}$  take  $f_s / F_{sy} = M^* / \Phi \mu = 0.586864307$

therefore 

$L_{sy} =$	370 mm	$\geq$	25k1db	700
say =	400 mm			

### 9. SHEAR CHECK

$V^* = 445.00 \text{ kN}$

Dist to centreline of bottom steel from soffit

critical location :  $d = 759 \text{ mm}$

$d_q = 93 \text{ mm}$   
 $d_o = 757 \text{ mm}$

$V^* = 429 \text{ kN}$

hence  $\beta_1 = 1.0215$  (but not less than 1.1)  
 $\beta_2 = 1.1$   
 $\beta_3 = 1$   
 1 Unity

$\phi V_{uc} = \phi \times \beta_1 \times \beta_2 \times \beta_3 \times b_w \times d_o [A_{st} \times F'_c / b_w \times d_o]^{0.333}$   
 $= 369.79 \text{ kN}$

$\phi V_{umax} = 4239.2 \text{ kN} > V^*$  No web crushing  
 $\phi V_{umin} = 688 \text{ kN} > V^*$  OK

If shear reinforcement required then proceed as follows :

choose stirrup spacing  $s = 300 \text{ mm}$

$F_{syf} = 410 \text{ MPa}$

Determine theta :

$b_v \times s / F_{syf} = 731.707317$   
 $A_{sv.min} = 256.097561 \text{ mm}^2$   
 $A_{sv.max} = 5343 \text{ mm}^2$

$A_{sv} = 2 \times \text{area of stirrups} = 220 \text{ mm}^2$

$\theta = 29.89356 \text{ degrees}$

Determine  $\phi V_u$  :

$V_u = V_{uc} + V_{us}$   
 $= V_{uc} + A_{sv} / s \times F_{syf} \times d_o \cot \theta$   
 $= 924.19 \text{ kN}$

$\phi V_u = 647 \text{ kN} > V^* = 428.5297 \text{ kN}$

hence

**PROVIDE : 2 Y12 LIGS @ 300 centres**

Other Support :

$V^* = 160.00 \text{ kN}$

Dist to centreline of bottom steel from soffit

critical location :  $d = 759 \text{ mm}$

$d_q = 93 \text{ mm}$   
 $d_o = 757 \text{ mm}$

$V^* = 143.53 \text{ kN}$

hence  $\beta_1 = 1.0215$  (but not less than 1.1)  
 $\beta_2 = 1.1$   
 $\beta_3 = 1$

$\phi V_{uc} = \phi \times \beta_1 \times \beta_2 \times \beta_3 \times b_w \times d_o [A_{st} \times F'_c / b_w \times d_o]^{0.333}$   
 $= 369.788 \text{ kN}$

$\phi V_{umax} = 4239.2 \text{ kN} > V^*$  No web crushing  
 $\phi V_{umin} = 688 \text{ kN} < V^*$  OK

If shear reinforcement required then proceed as follows :

choose stirrup spacing  $s = 300 \text{ mm}$

$F_{syf} = 410 \text{ MPa}$

Determine theta :

$b_v \times s / F_{syf} = 731.707317$   
 $A_{sv.min} = 256.097561 \text{ mm}^2$   
 $A_{sv.max} = 5343.04031 \text{ mm}^2$

$A_{sv} = 2 \times \text{area of stirrups} = 330 \text{ mm}^2$

$\theta = 30.21792 \text{ degrees}$

Determine  $\phi V_u$  :

$V_u = V_{uc} + V_{us}$   
 $= V_{uc} + A_{sv} / s \times F_{syf} \times d_o \cot \theta$   
 $= 1114.443 \text{ kN}$

$\phi V_u = 780.11 \text{ kN} > V^* = 143.53 \text{ kN}$

hence

**PROVIDE : 2 Y12 LIGS @ 300 centres**

## 10. SERVICEABILITY CHECK

From an Elastic Linear Analysis :

Mr	=	2300.00 kNm	Es	=	200000 MPa
MI	=	711.00 kNm	Ec	=	31975.35051 MPa
Mo	=	2278.5 kNm			
Ms	=	773.00 kNm			
Ig	=	7.41E+10 mm <sup>4</sup>			

Calculate Icr, Mcr, and Ief at midspan :

Eff. Flange width	b <sub>eff</sub> =	1510 mm	A <sub>sc</sub> =	2463.00864 mm <sup>2</sup>
webwidth	b <sub>w</sub> =	1000 mm	A <sub>st</sub> =	4825.486316 mm <sup>2</sup>
Flange thickness	t <sub>ff</sub> =	800 mm	d <sub>sc</sub> =	119 mm
			d <sub>st</sub> =	759 mm
			d =	759 mm
			D =	1650 mm

$$n = E_s/E_c = 6.25481807$$

$$1/2 \times b_w \times d_n^2 + (b_{eff} - b_w) \times t (d_n - t/2) + (n - 1) A_{sc} (d_n - d_{sc}) = n A_{st} (d - d_n)$$

$$500 d_n^2 + 45125.201 d_n - 187648724 = 0$$

$$d_n = 309.671352 \text{ mm}$$

$$d = 759 \text{ mm}$$

$$I_{cr} = b d_n^3/3 + 1/12 (b_{eff} - b_w) \times t^3 + (b_{eff} - b_w) \times t (d_n - t/2)^2 + n x A_{st} (d - d_n)^2 = 4.11E+10 \text{ mm}^4$$

$$M_{cr} = 0.6 \sqrt{F'_c} \times b D^2/6 = 1721.86019 \text{ kNm}$$

$$I_m = [I_{cr} + (I_g - I_{cr}) (M_{cr} / M_s)^3] = 4.06E+11 \text{ mm}^4$$

Calculate Icr, Mcr, Ilef and Iref at end supports:

b <sub>w</sub> =	1000 mm	A <sub>st</sub> =	4825.48632 mm <sup>2</sup>
d <sub>sc</sub> =	103 mm	A <sub>sc</sub> =	3216.99088 mm <sup>2</sup>
d =	759 mm		
Ig =	7.41E+10 mm <sup>4</sup>		

At right end (arbitrary)

$$1/2 \times b d_n^2 + (n - 1) A_{sc} (d_n - d_{sc}) = n A_{st} (d - d_n)$$

$$500 d_n^2 + 47087.2408 d_n - 24649731.4 = 0$$

$$d_n = 179.89 \text{ mm}$$

$$d = 1531 \text{ mm}$$

$$I_{cr} = b d_n^3/3 + n x A_{sc} (d - d_n)^2 = 5.70E+10 \text{ mm}^4$$

$$M_{cr} = 0.6 \sqrt{F'_c} \times b D^2/6 = 456.949122 \text{ kNm}$$

$$I_{ref} = [I_{cr} + (I_g - I_{cr}) (M_{cr} / M_r)^3] = 5.72E+10 \text{ mm}^4$$

At left end (arbitrary)

b <sub>w</sub> =	1000 mm
d <sub>sc</sub> =	101 mm
A <sub>st</sub> =	4825.4863 mm <sup>2</sup>
A <sub>sc</sub> =	2463.0086 mm <sup>2</sup>
d =	759 mm

$$1/2 \times b d_n^2 + (n - 1) A_{sc} (d_n - d_{sc}) = n A_{st} (d - d_n)$$

$$500 d_n^2 + 43125.2013 d_n - 24215756 = 0$$

$$d_n = 181.132006 \text{ mm}$$

$$d = 1547 \text{ mm}$$

$$I_{cr} = b d_n^3/3 + n x A_{sc} (d - d_n)^2 = 5.83E+10 \text{ mm}^4$$

$$M_{cr} = 0.6 \sqrt{F'_c} \times b D^2/6 = 456.949122 \text{ kNm}$$

$$I_{lef} = [I_{cr} + (I_g - I_{cr}) (M_{cr} / M_l)^3] = 6.25E+10 \text{ mm}^4$$

$$I_{av} = [I_m + (I_l + I_r)/2]/2 = 2.33E+11 \text{ mm}^4$$

Midspan Deflection :

$$L_{ef} = 2550 \text{ mm}$$

$$Defl_{ms} = L_{ef}^2/E_c \times I_{av} [5/48 \cdot M_o - 1/16 ML - 1/16 Mr]$$

$$= 0.04292895 \text{ mm}$$



**Total Deflection :**

$$\begin{aligned} \text{Long term factor} &= 0.2 \\ \text{Short term factor} &= 0.5 \\ w - \text{long term} &= 20.1 \text{ kN/m} \\ w - \text{short term} &= 20.7 \text{ kN/m} \\ \text{Defln,s,sus} &= 0.041684631 \text{ mm} \\ \text{At midspan :} & \quad \text{Ast} = 4825.48632 \text{ mm}^2 \\ & \quad \text{Asc} = 2463.00864 \text{ mm}^2 \\ \text{Asc/Ast} &= 0.510416667 \\ \text{kcs} &= 2 - (1.2 \times \text{Asc/Ast}) \quad \geq 0.8 \\ &= 1.3875 \end{aligned}$$

therefore

Total Defln	=	Dfln,s + ( kcs x Defln,s,sus )
	=	0.10077 mm

FROM A MORE ACCURATE ANALYSIS Warner,Rangan,Hall HAVE SHOWN THAT THIS CAN BE REDUCED BY UP TO 10%.

ALLOWABLE DEFLN	=	$L_{ef} / 500 =$	5.1 mm
			> 0.9 x Total Defln
ALLOWABLE DEFLN	=	$L_{ef} / 250 =$	10.2 mm
			> 0.9 x Total Defln

OK