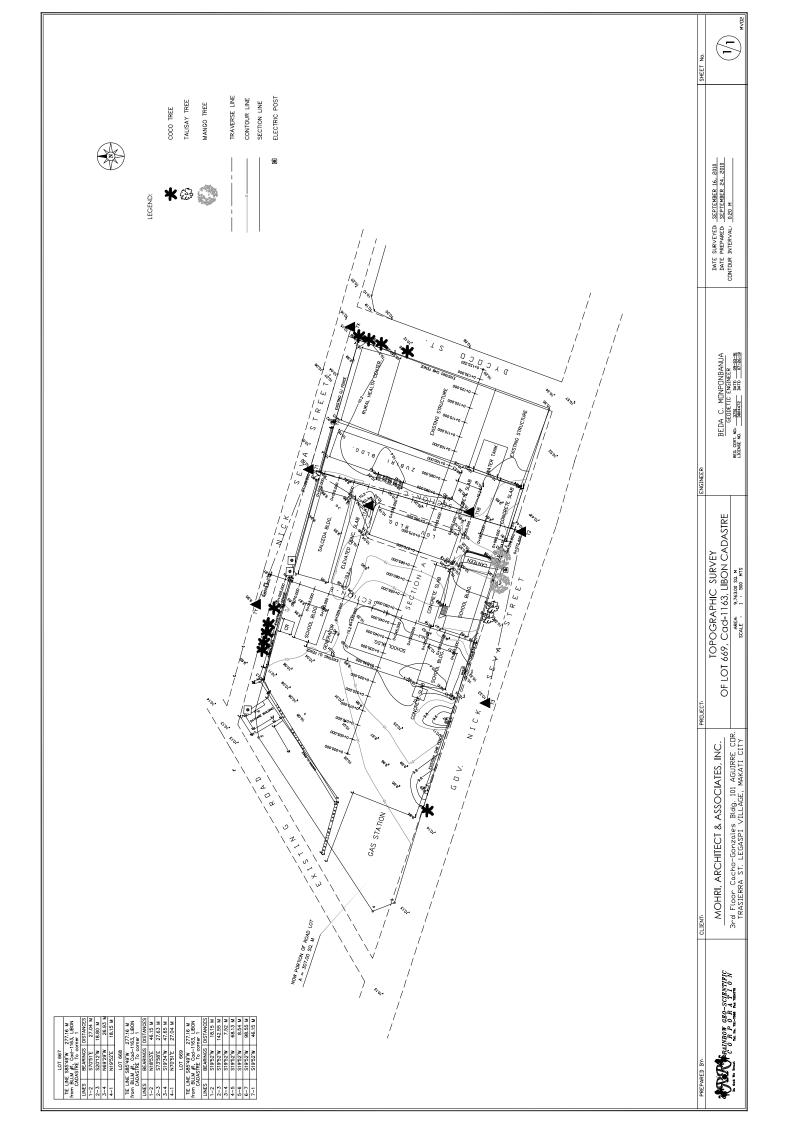
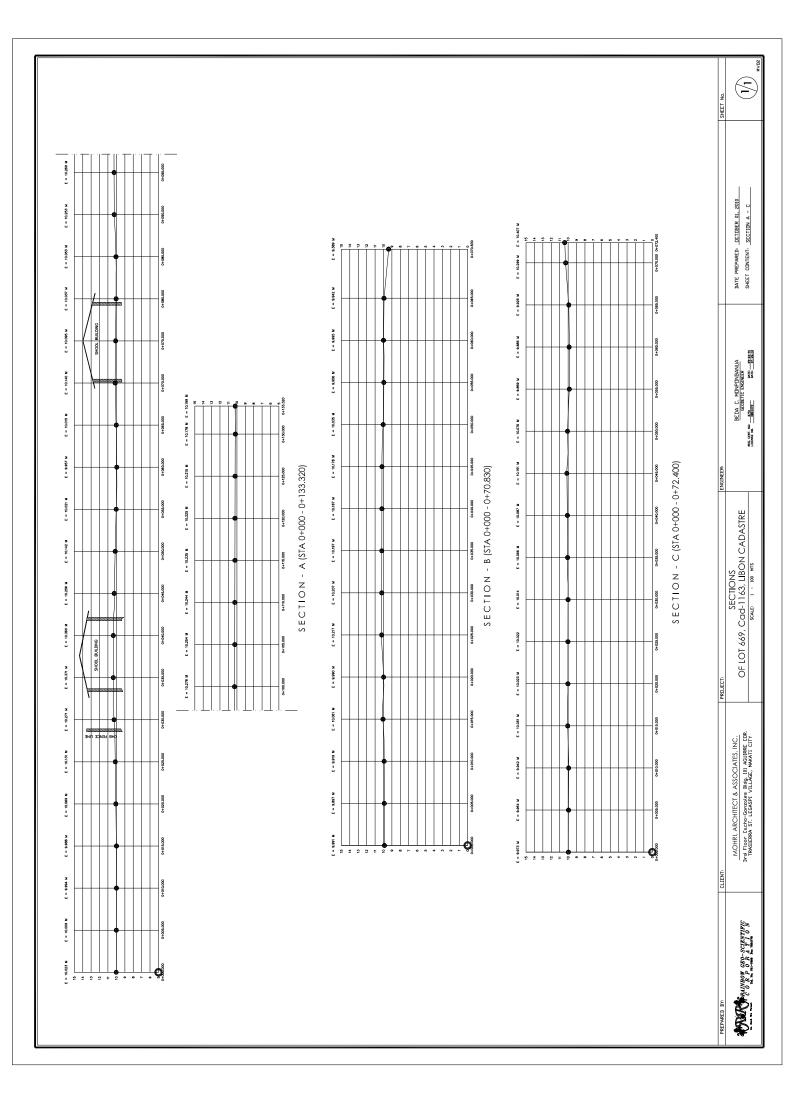
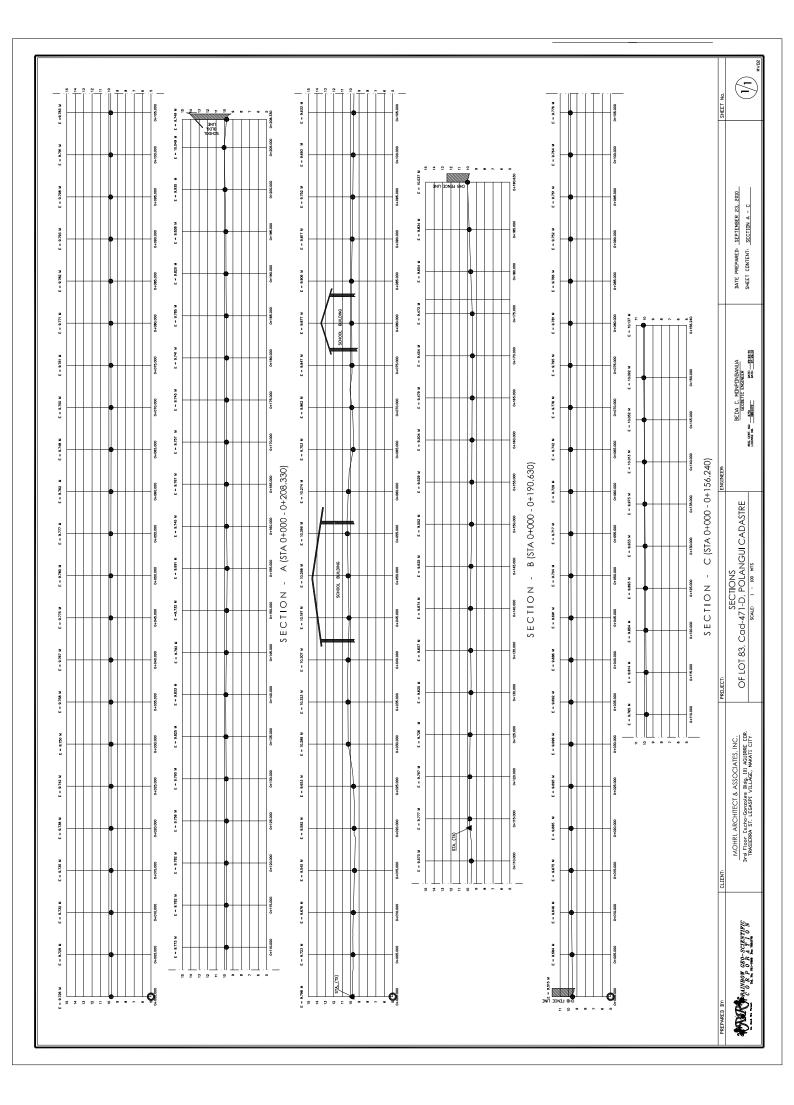
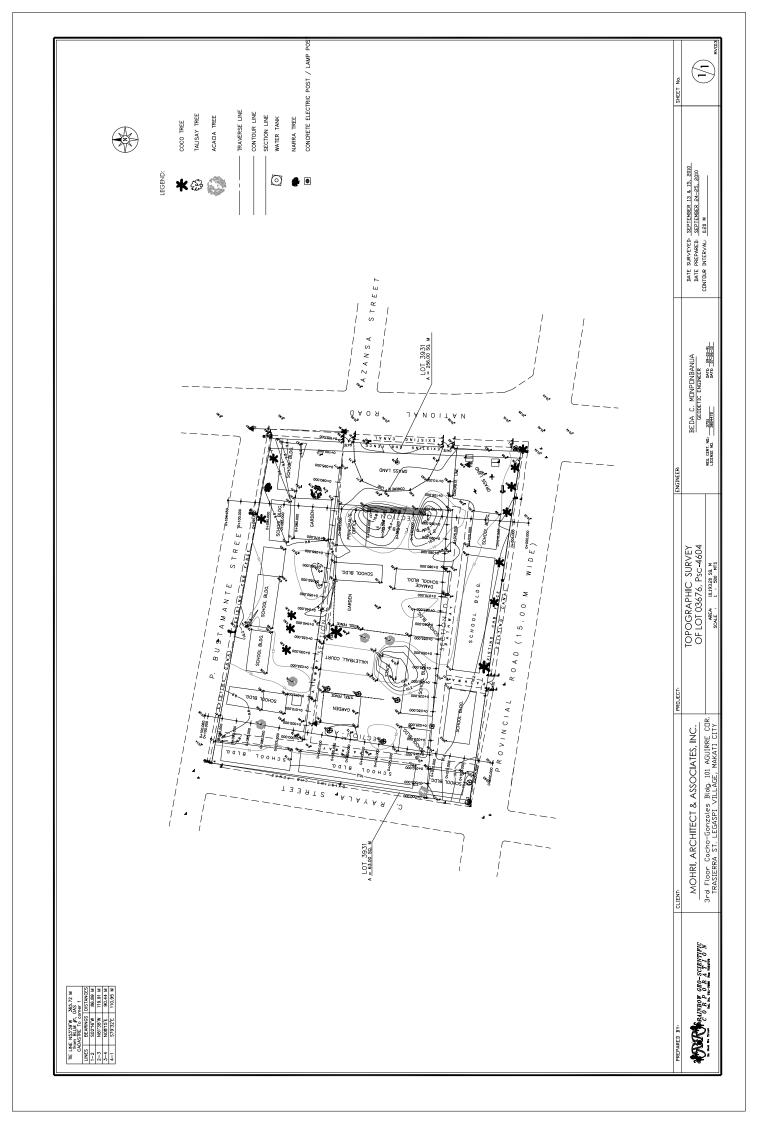
Ⅱ. 地形測量図/地質調査報告書

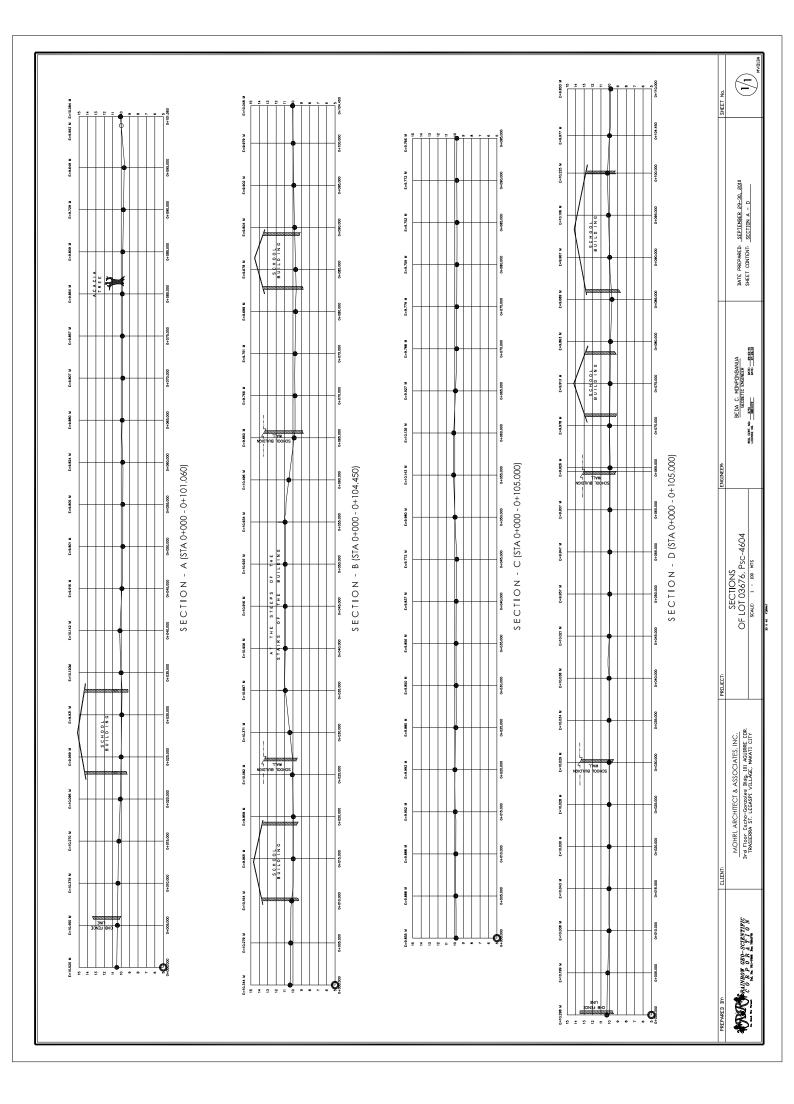


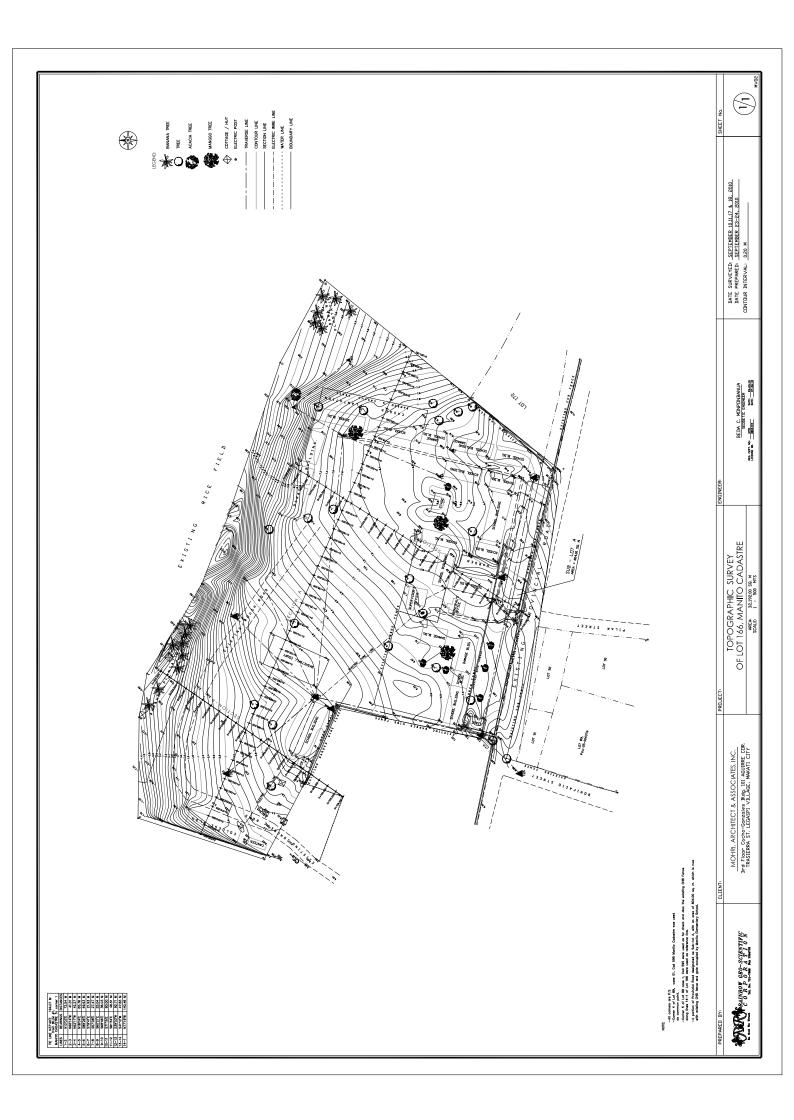


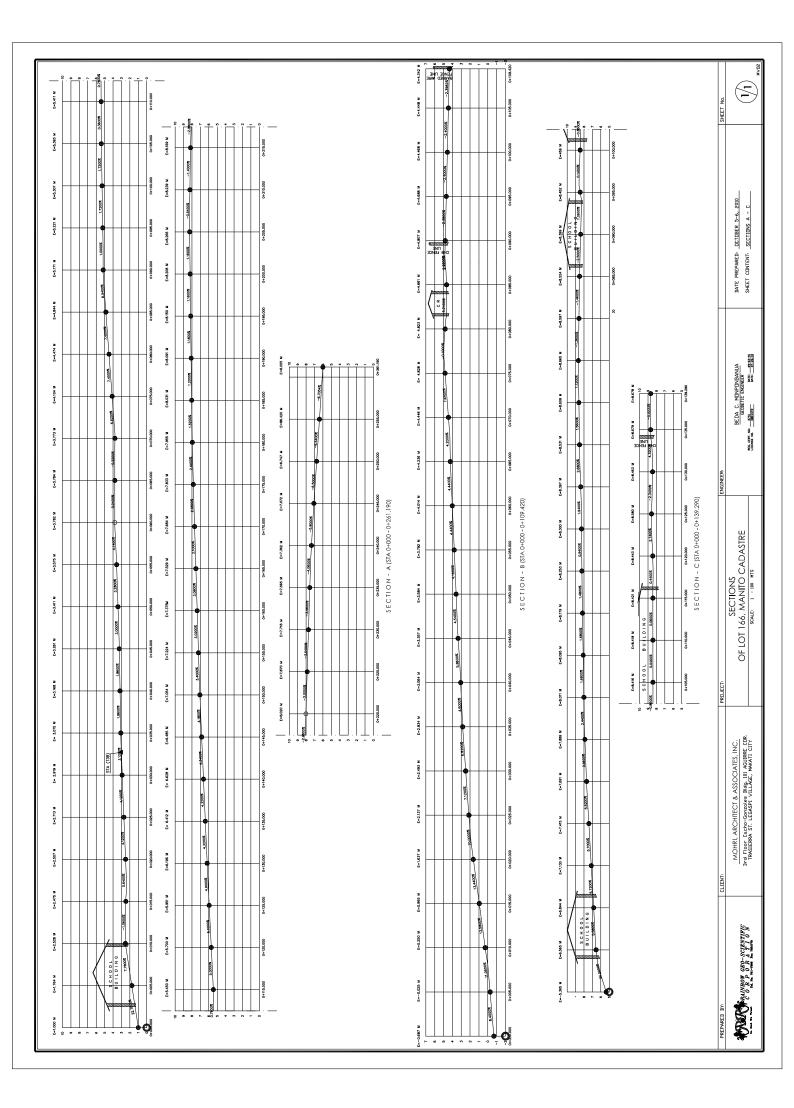


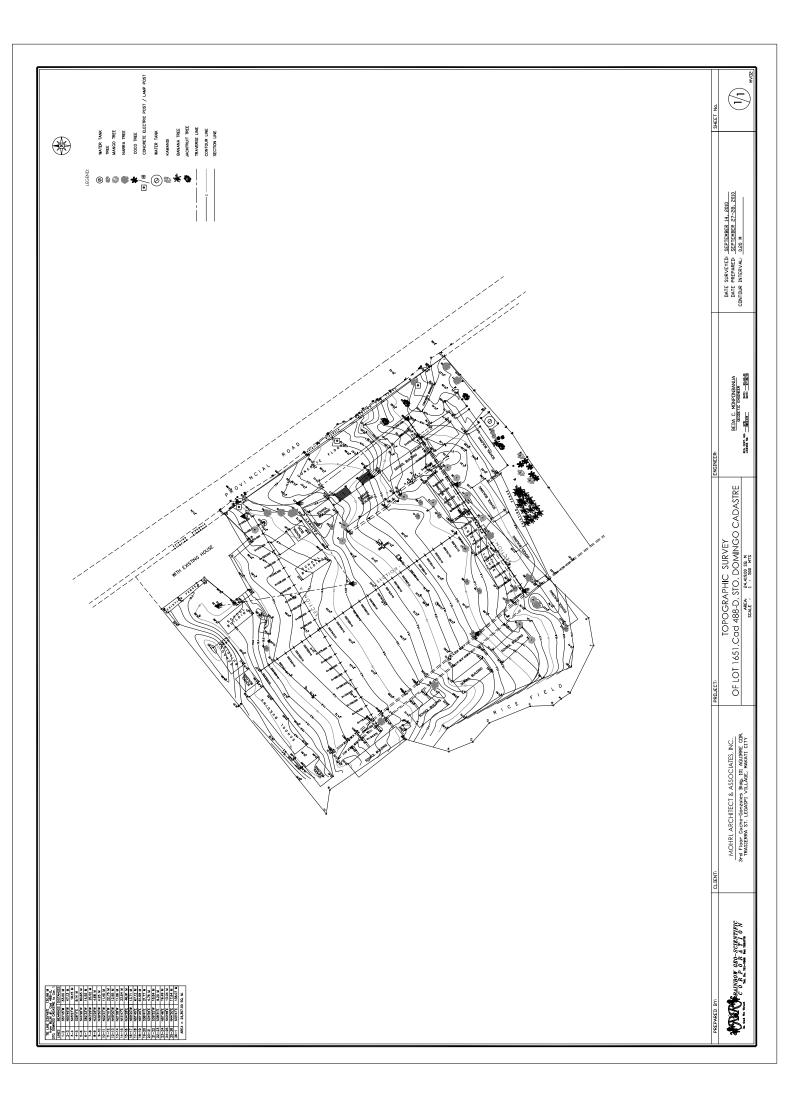


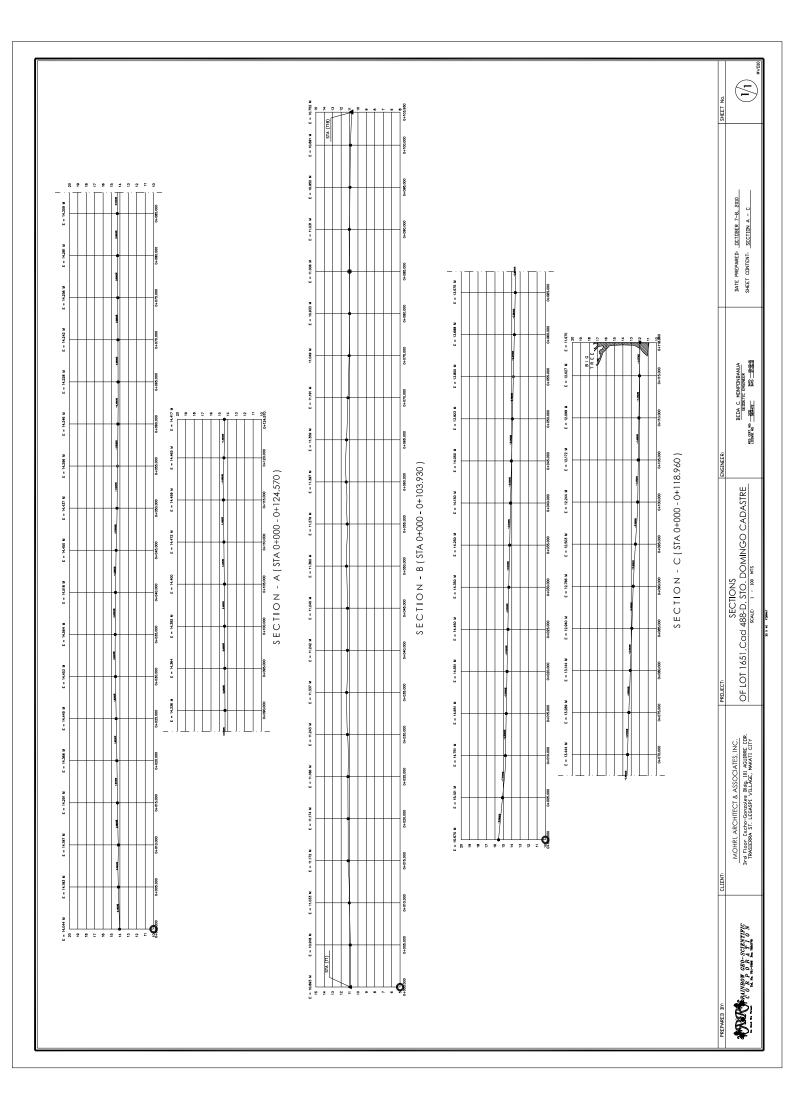


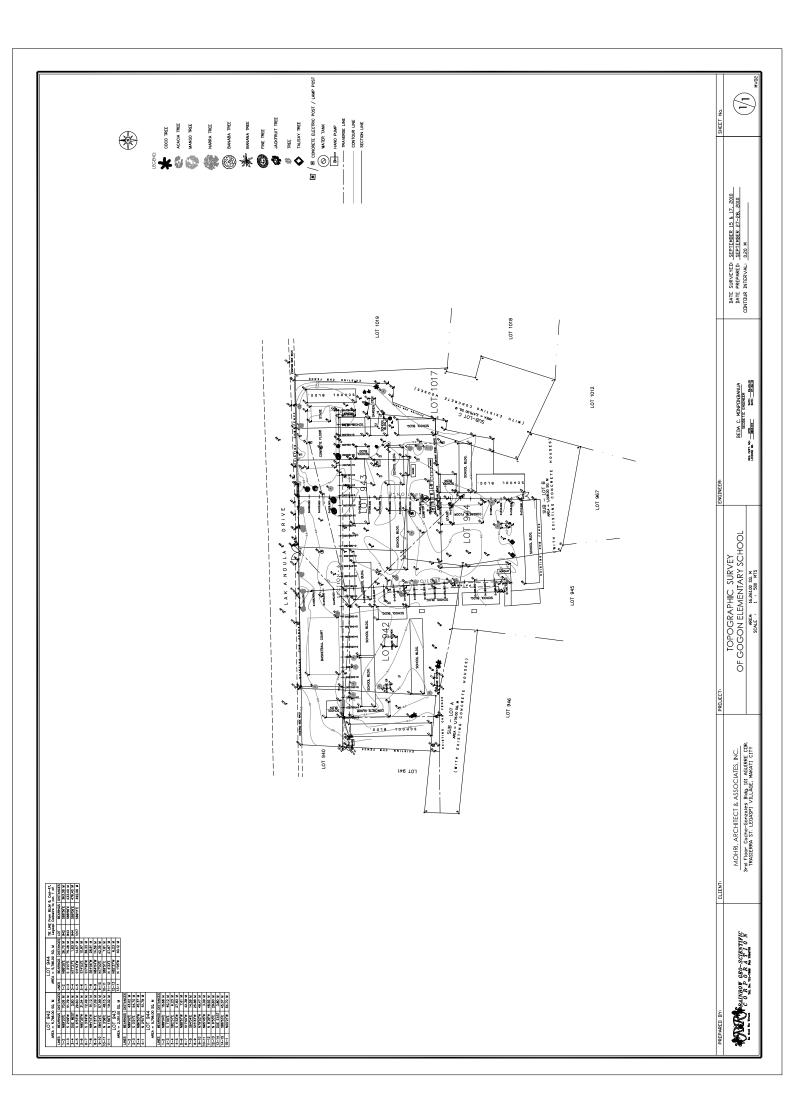


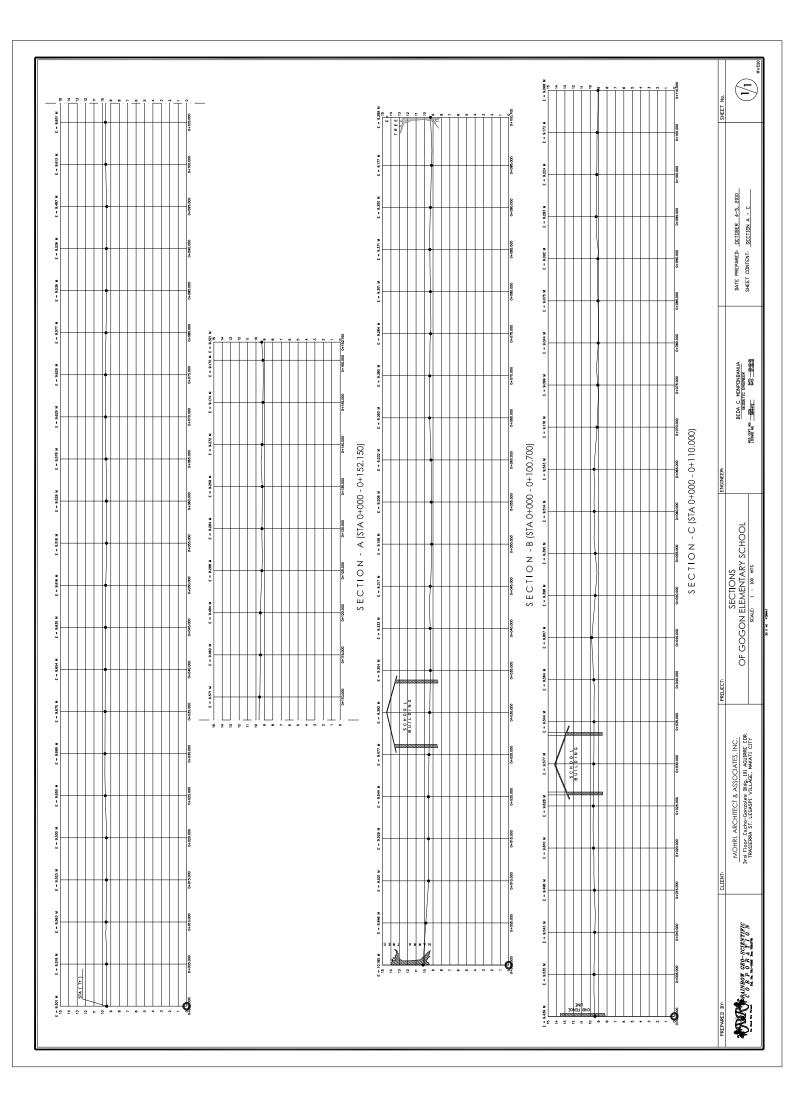












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				PREPARED BY:

## **FINAL REPORT**

# SUBSURFACE INVESTIGATION PROPOSED MAYON EVACUATION CENTER (3-STOREY) LIBON COMMUNITY COLLEGE BRGY. ZONE 4 LIBON, PROVINCE OF ALBAY

MOHRI, ARCHITECT & ASSOCIATES, INC.

OCTOBER 2010 JOB NO. 2209-10.R1





## FINAL REPORT

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#### **FINAL REPORT**

## SUB-SURFACE INVESTIGATION FOR THE **PROPOSED MAYON EVACUATION CENTER (3-STOREY)** LOCATED AT BRGY. ZONE 4 LIBON, PROVINCE OF ALBAY

#### **1. INTRODUCTION**

Geotechnics Philippines, Incorporated (GPI) completed the subsurface soil investigation for the proposed Mayon Evacuation Center. The proposed site explored is located at Brgy. Zone 4 Libon, Province of Albay.

Two (2) boreholes were drilled at the proposed site on October 16, 2010. Borings were undertaken down to 10m for both BH-1 and BH-2 below existing natural ground line. Borehole locations are as indicated on the accompanying Boring Plan and Soil Profile Sheets.

The subsurface soil exploration was undertaken upon the request of Mohri & PA Associates, Inc. in order to gain information on the subsurface conditions and bearing characteristics of the underlying soils at site.

The undersigned was tasked to evaluate the results of the completed subsurface soil exploration and to recommend a suitable foundation solution for the proposed structure.

This report embodies the undersigned's engineering analysis and recommendations based mainly on the results of the geotechnical soil borings and pertinent laboratory tests performed on extracted samples.

The results of geotechnical soil borings and laboratory tests can be referred to in the Attachments accompanying this report.

#### 2. FIELD AND LABORATORY TEST PROCEDURES

**Drilling Procedure** 

The boreholes were advanced by wash boring to the maximum boring depths. Standard Penetration Tests were conducted at every 1.5m interval or at change in soil formations. It consisted of driving a standard split spoon sampler of 5.08cm (2" 0.D.) diameter in three (3) successive 15cm (6") intervals using a drop hammer of 64kg (140 lbs) weight from a height of 76cm (30"). The number of blows required to penetrate 15cm are recorded successively until the third interval is penetrated. The first interval blow count is called as the seating drive and is discarded. The last two blow counts are added to give the Nvalue, a measure of the density or consistency of the soil layer. SPT procedures are conducted in accordance with ASTM D-1586. Undisturbed soil samples were taken in soft to stiff soil deposits for Natural Moisture Content (NMC) testing and particle size analysis of soil.

> Page 11 Proposed Mayon Evaluation Center Libon, province of Albay

#### **2.2 LABORATORY TEST PROCEDURES**

The following laboratory tests were performed on the soil samples taken from the site;

a. Classification of Soils (USCS) for Engineering Purposes	ASTM D 2487	
b. Particle Size Analysis of Soil	ASTM D 422	
c. Determination of Moisture Content of Soils	ASTM D 2216	
d. Liquid Limit of Soils	ASTM D 4318	
e. Plastic Limit of Soils	ASTM D 4318	

#### 3. SITE SOILS AND OBSERVATIONS

The soil profile indicating the completed two (2) boreholes is attached in this report. Standard Penetration Tests (SPT) indicate shallow layers of very loose poorly graded sand with silt (SP-SM) at depth 2.0m from the existing natural ground line. Drill intersections indicate deposition of fat clay (CH) with intervening layers and pockets of silty sand (SM) with traces of gravel down to a depth of 10m. A layer of silty clay (CH) may also be found within this depth. Trend of N-values generally shows increasing consistency and density with increasing depth. However, it is important to note the existence of relatively looser soils in deeper layers especially in BH-2.

Groundwater table (GWT) levels can be found at a depth of 0.75m for BH-1 and 1.57m for BH-2 reckoned from the existing natural ground line at the time of borings. Thus, excavation may possibly be in wet condition unless otherwise controlled as the detected GWT is fairly within the depths of excavation of proposed foundation elements.

#### 4. ENGINEERING ANALYSIS AND CONSIDERATIONS

SPT N-values of 2 to 4 at a depth from 0 to 2.00 show very loose silty sand, thus shallow foundations to rest on the topmost loose formation (above 2-m) is not permitted. However, directly beneath is a layer of medium dense clayey and silty sand (SC-SM) with thickness ranging from 2m to 4m. Shallow foundations are possible to rest at 2.0m depth to bypass the relatively loose layers of poorly graded sand. This loosee sand have to be removed and replaced with selected granular fill preferably well graded. An engineered fill has to be compacted at 95% of the soil's maximum dry density (95% MDD) in 300mm lift.

These shallow foundations shall be in the form of a isolated footing stiffened by a structural tie beam cast in place between column. The system will essentially function as an integrated foundation. The rigidity will assist in bridging across localized settlements and assuring uniform settlement of the structure.

The spread footing shall be proportioned and designed based on a net Allowable Bearing Capacity of 96 kPa (2000 psf). The effect of overburden shall be added to obtain the allowable gross bearing capacity. Where necessary, depending on the final design on the reactions of the building, and due to the low bearing capacity, a mat foundation may also be adopted as an alternate solution to spread footing to support the building foundation. The mat foundation should be made to maintain nearly uniform pressure to avoid differential settlements. For settlement analysis, a compression index of 0.54, modulus

P a g ∈ | 2 Proposed Mayon Evaluation Center Libon, province of Albay of elasticity of 6 Mpa, and a Poisson's ratio of 0.35 may be used. Soil unit weight of 12 kN/m<sup>3</sup> and an angle of internal friction of 28 degrees may be used for shallow foundation calculations.

The floor slabs should not be connected to more rigid elements of the structure such as walls and columns, and should be allowed to settle independently of the building.

This solution, however, is not without possible problems. The relatively poor deposits underlying the site are within the zone of influence of shallow foundations. These poor soil deposits together with the intervening pockets of loose to very loose sands pose a potentially liquefiable and therefore a risk during a significant seismic event. The nature of the soil formations, however, requires considerable earthquake magnitude and epicentral distance to cause soil liquefaction. Hence, settlements can be expected as well as the inherent danger of liquefaction.

#### **5. CONSTRUCTION CONSIDERATIONS**

The shallow water table poses a problem during foundation construction. Adequate number of dewatering equipment should be provided in order to allow excavation in almost dry condition. Likewise, concreting shall also be done in the dry condition by dewatering the foundations continuously.

Engineered fill shall be compacted using a vibratory plate compactor of adequate size As previously noted, floor slabs should not be connected to more rigid elements of the structure such as walls and columns, and should be allowed to settle independently of the building.

#### 6. CONCLUSIONS AND RECOMMENDATIONS

The spread footing or mat solution may be adopted, the foundation are subject to settlements and possibility of liquefaction due to poor soil deposits underneath. Although the mat will minimize the effect if the cited recommendations are followed by the engineer on record of the structure. The economics for using shallow foundations is considered in the design of the proposed 3-storey structure.

The recommendations submitted in this report are based in part upon the data obtained from a limited number of soil samples. The nature and extent of variations between explorations may not become evident until construction or further investigation. If the variations are of considerable magnitude, it will be necessary to reevaluate the recommendations in this report.

This report has been done by the undersigned in accordance with generally accepted Engineering Principles and Practices.

If you require additional comments or clarifications pertaining to the recommendations, the undersigned will be pleased to comply.

DIOSDADO A. URENA CE Reg. No. 053884 PTR No. 3228274 Issued on January 8, 2010 Issued at Quezon City





