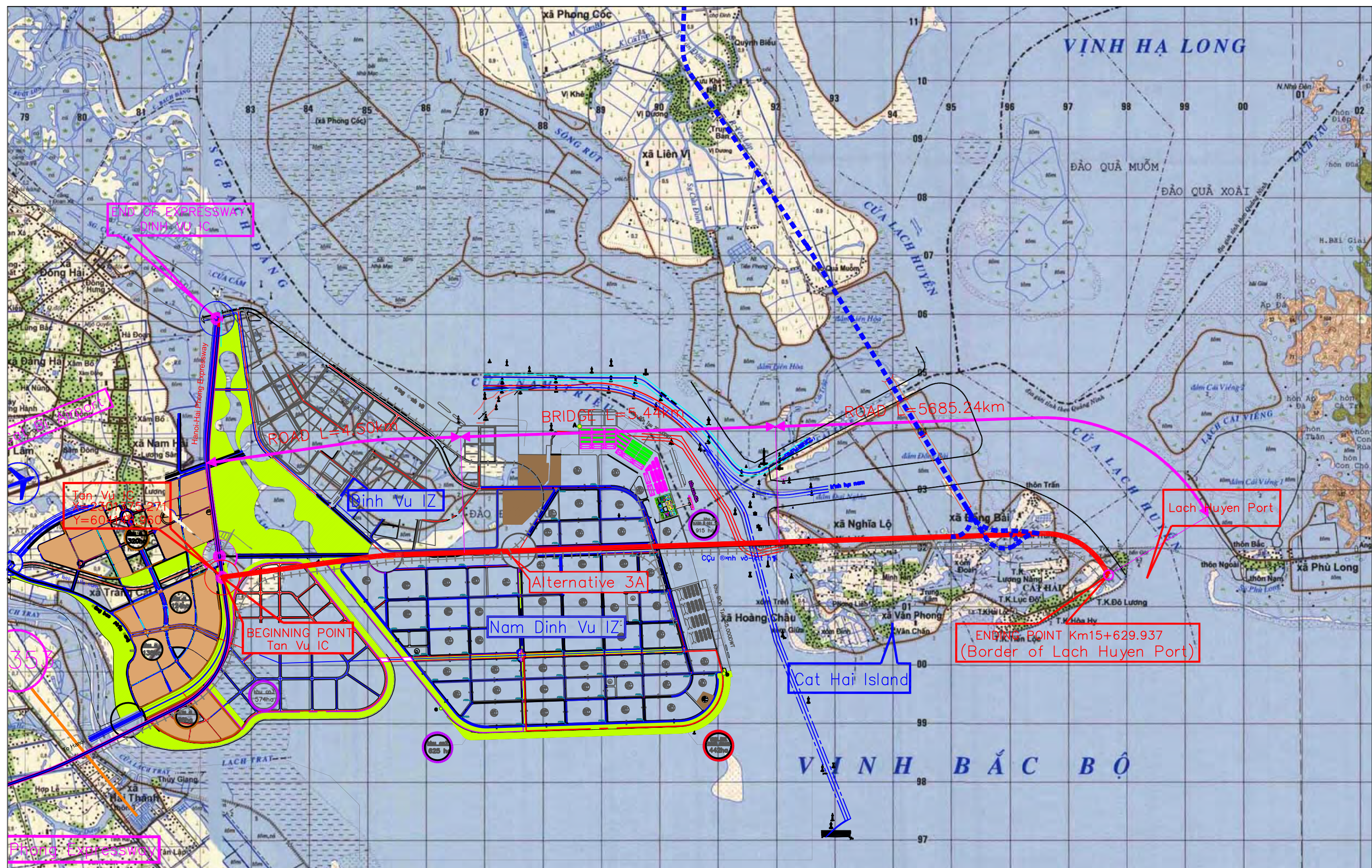


Appendix-1: Drawings

List of Drawings

No.	Title
I	General
G-01	Project Location Map
II	Road
R-01-01	Typical Cross Section(1) Project Highway
R-01-02	Typical Cross Section(2) Crossing Roads
R-02-01	Plan and Profile
R-02-02	Plan and Profile
R-02-03	Plan and Profile
R-02-04	Plan and Profile
R-02-05	Plan and Profile
R-02-06	Plan and Profile
R-02-07	Plan and Profile
R-02-08	Plan and Profile
R-02-09	Plan and Profile
R-02-10	Plan and Profile
R-02-11	Plan and Profile
R-02-12	Plan and Profile
R-03	Tan Vu Intersection (at grade)

No.	Title
III	Bridge
B-01	Typical Cross Sections (Stage Construction, Full 6-lane)
B-02	General View(1) Main Bridge
B-03	General View(2) Approach Bridge
IV	Construction
C-01	Construction Method of Abutment and Approach Bridge Pier (On Land)
C-02	Construction Method of Approach Bridge Pier (Off-shore)
C-03	Construction Method of Main Bridge Pier
C-04	Construction Method of Main Bridge Superstructure
C-05	Construction Method of Approach Bridge Superstructure
C-06	Construction Schedule



THE PREPARATORY SURVEY
OF
LACH HUYEN PORT CONSTRUCTION
(ROAD AND BRIDGE PORTION)

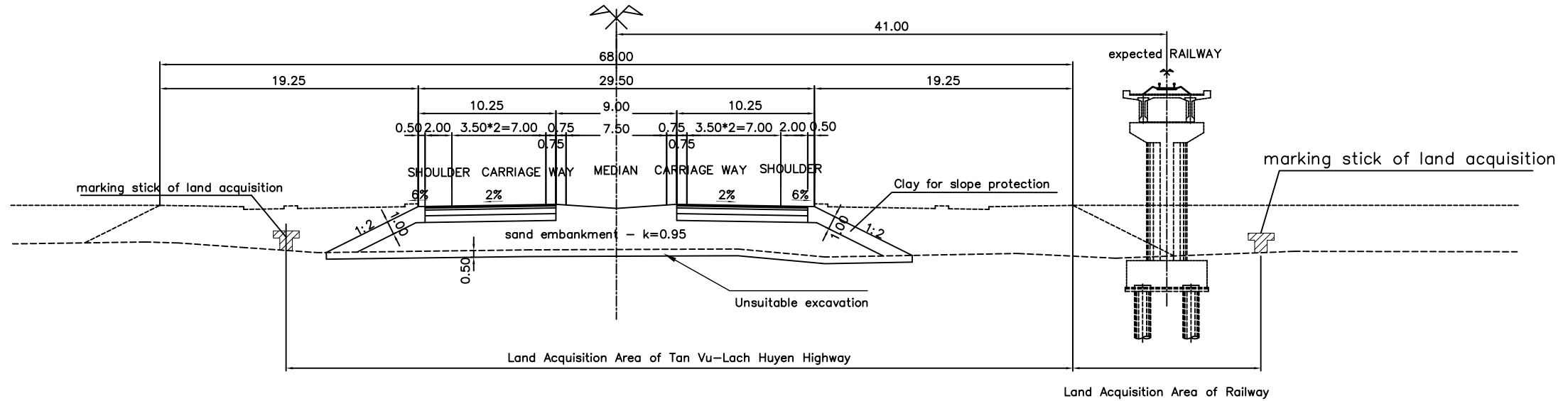


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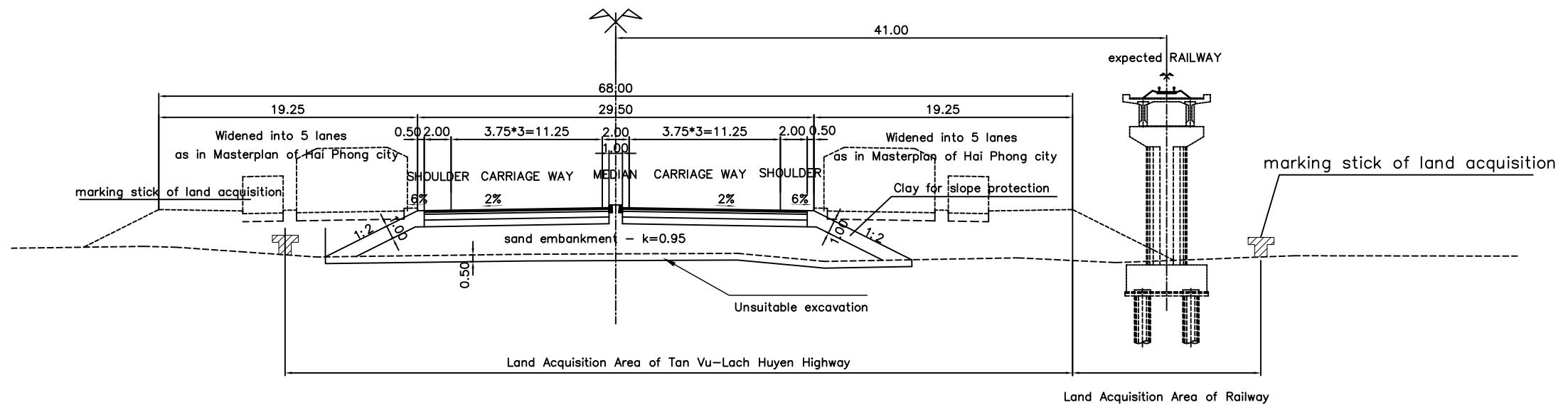
TITLE
Project Location Map

SCALE
1:60,000
NO.
G-01

Typical Cross Section for First Stage 4-Lane



Typical Cross Section for second stage 6-Lane



THE PREPARATORY SURVEY
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TITLE

Typical Cross Section(1)
Project Highway

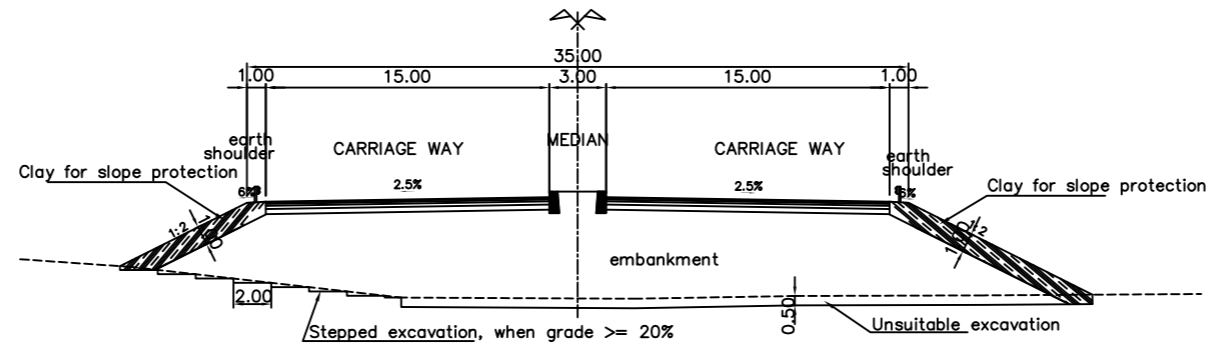
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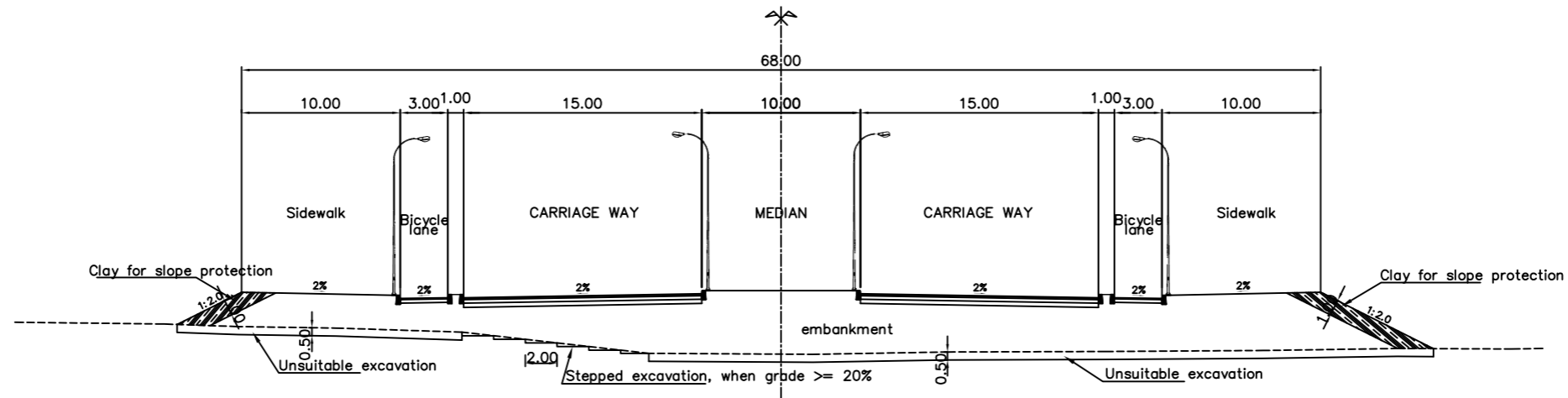
NO.

R-01-01

Ha noi – Hai Phong Expressway



Third Ring Road as in Masterplan of Hai Phong City



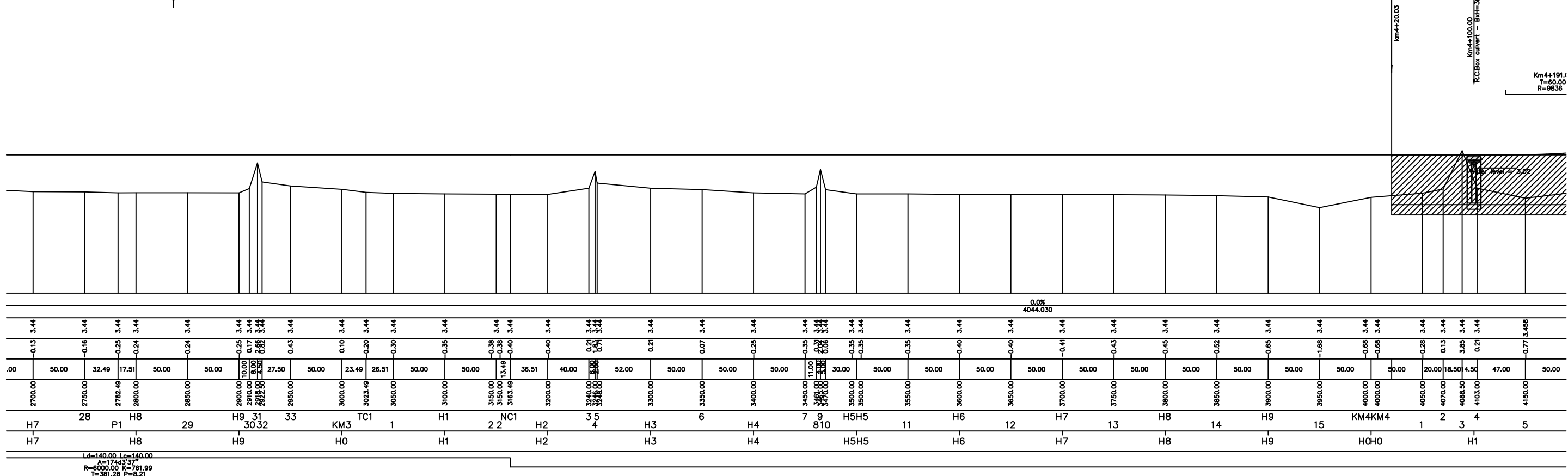
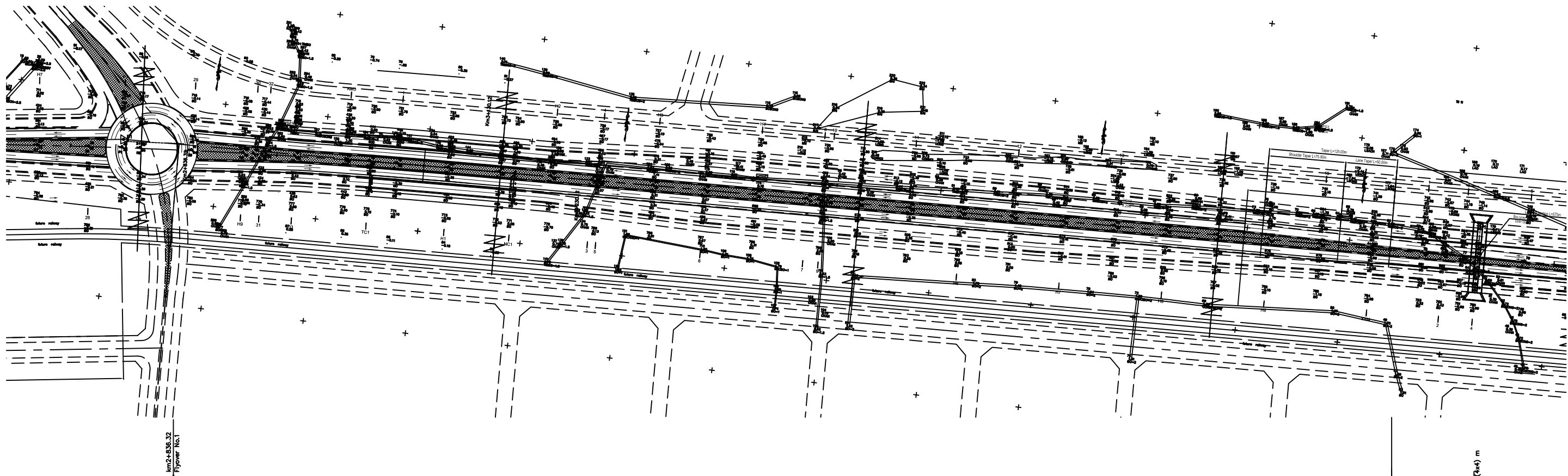
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TITLE
Typical Cross Section(2)
Crossing Roads

SCALE
1:40
NO.
R-01-02



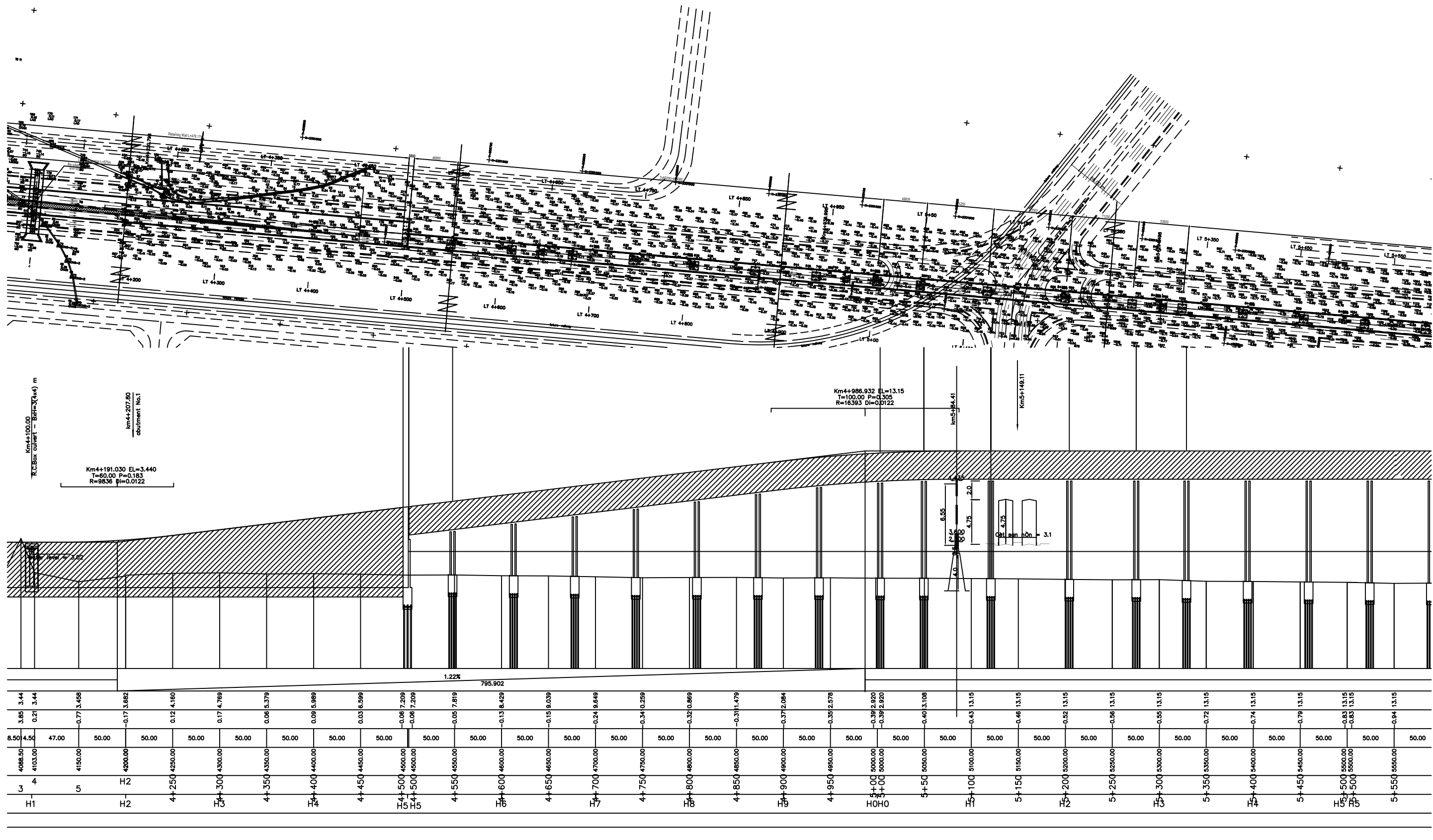
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TITLE
Plan and Profile

SCALE
1:4000
NO.
R-02-03



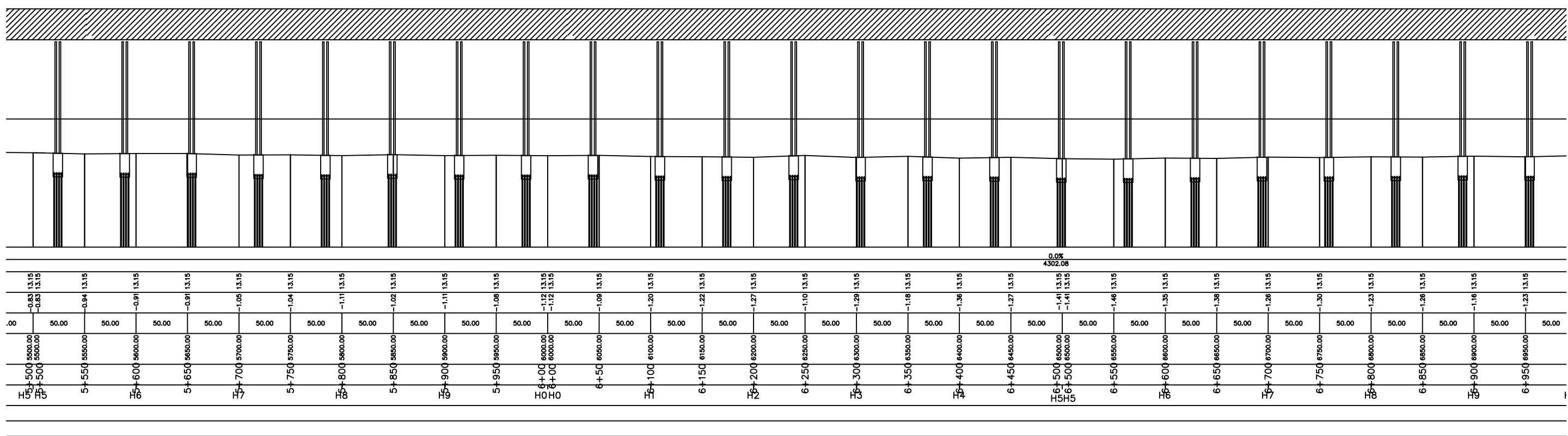
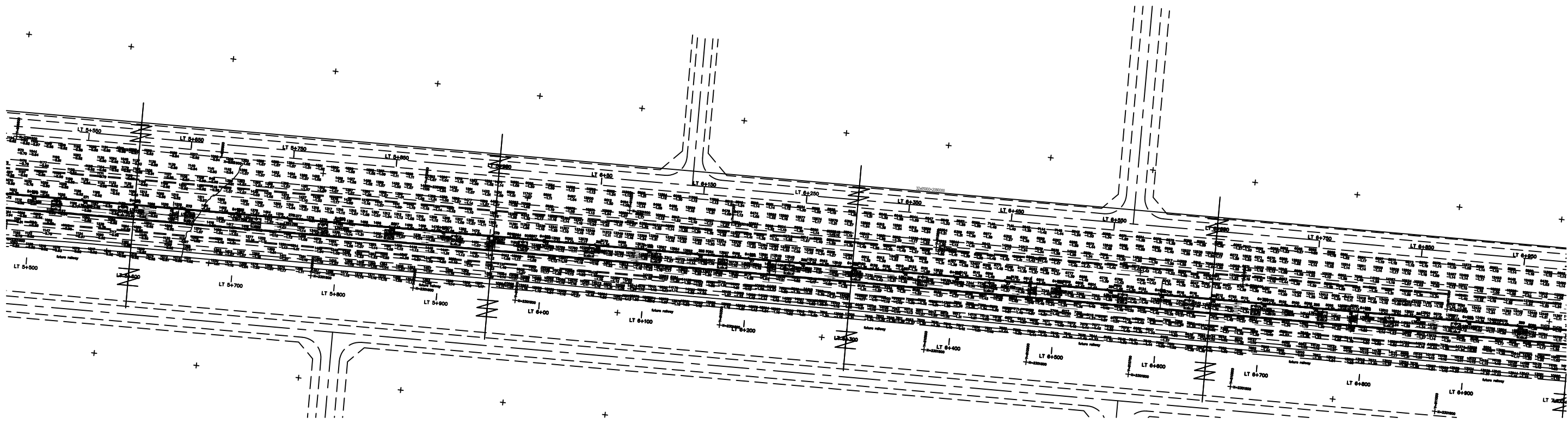
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 OF
 LACH HUYEN PORT CONSTRUCTION
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TITLE
 Plan and Profile

SCALE
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 NO.
 R-02-04



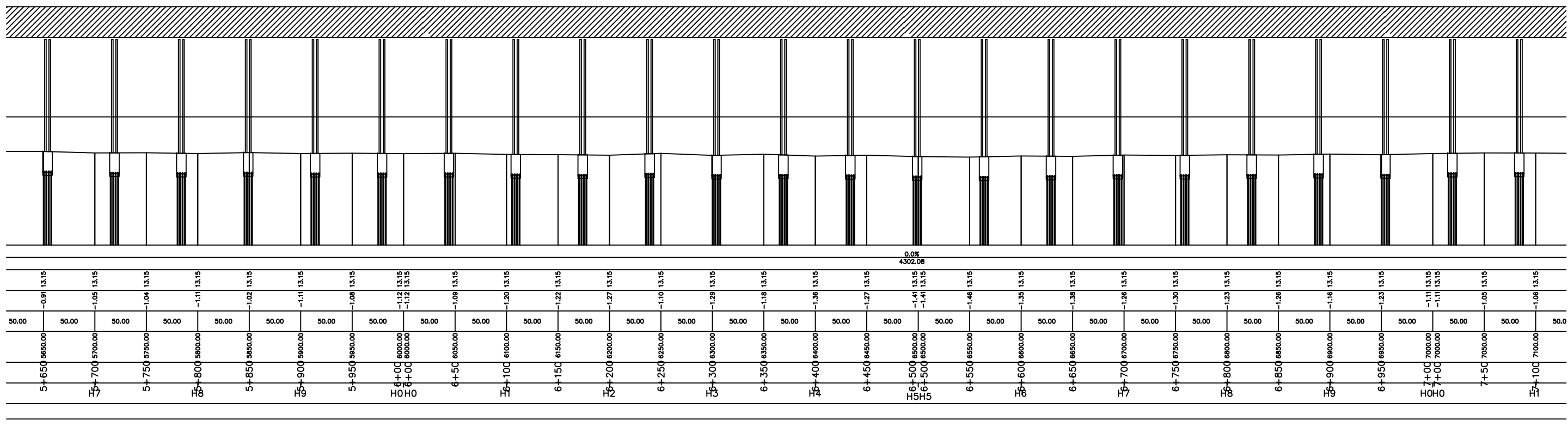
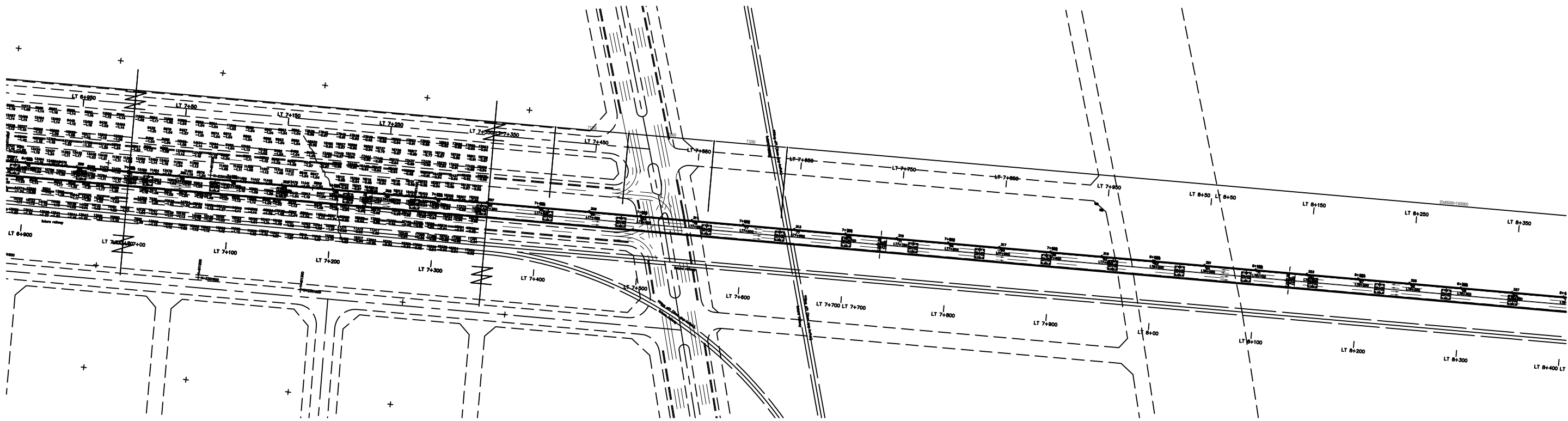
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LACH HUYEN PORT CONSTRUCTION
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TITLE
Plan and Profile

SCALE
1:4000
NO.
R-02-05



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TITLE

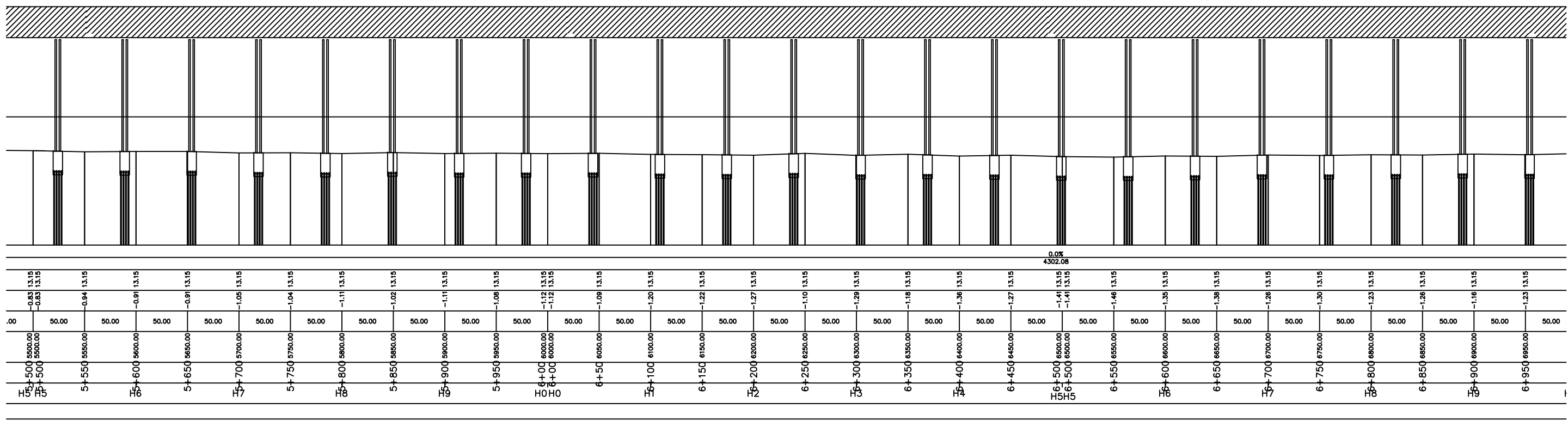
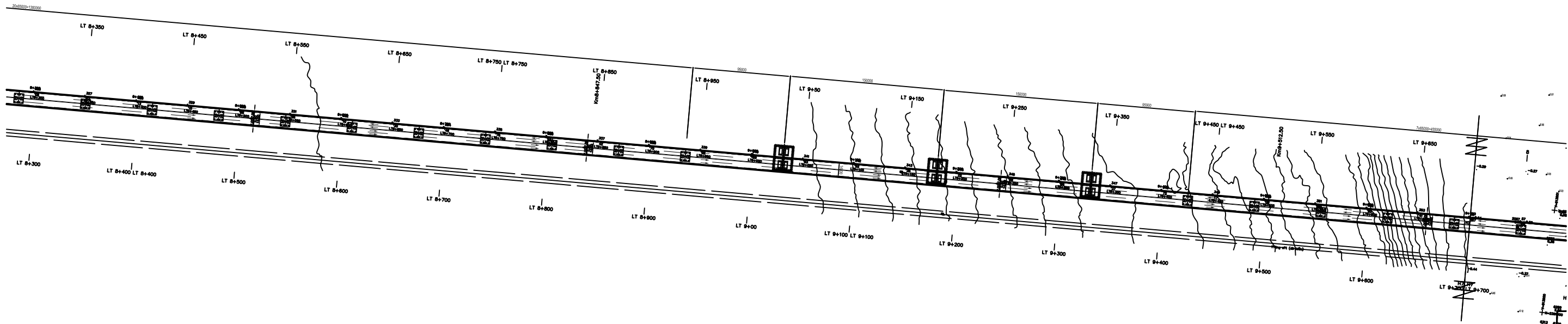
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NO.

R-02-06



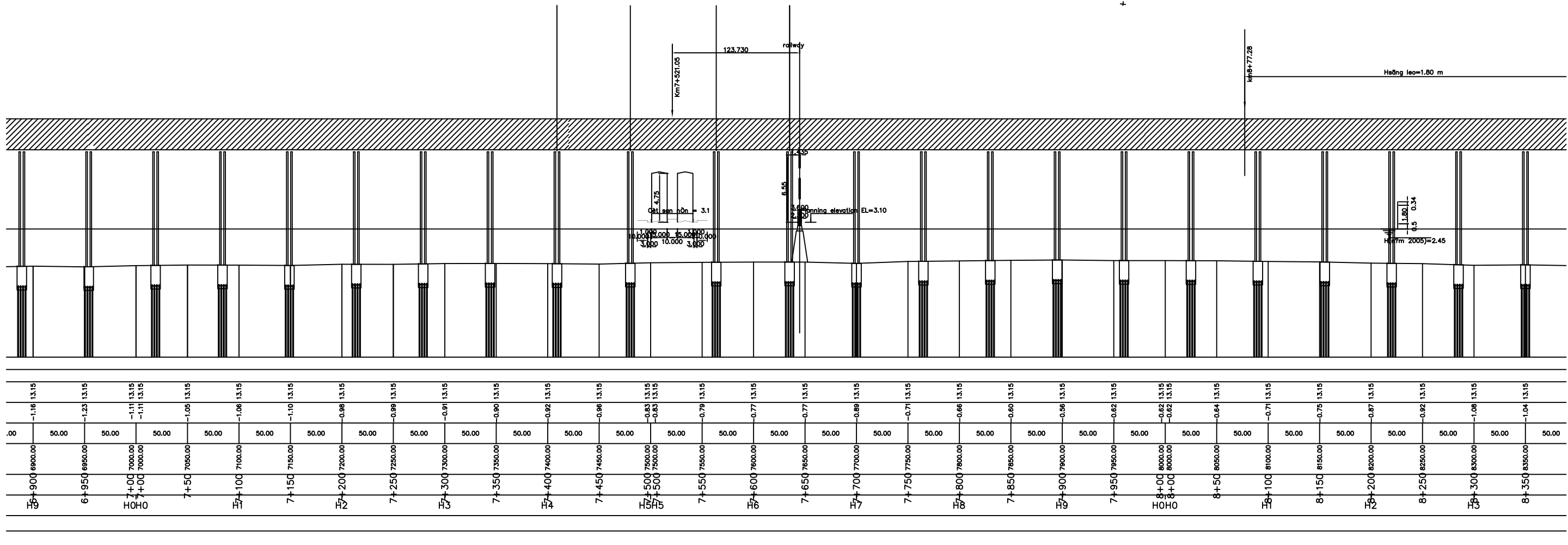
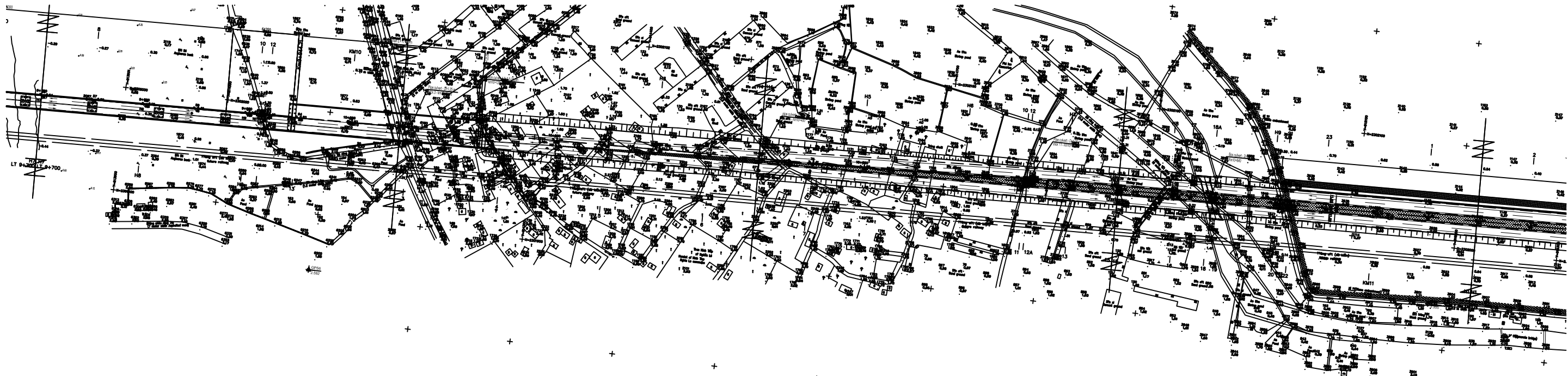
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LACH HUYEN PORT CONSTRUCTION
(ROAD AND BRIDGE PORTION)



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TITLE
Plan and Profile

SCALE
1:4000
NO.
R-02-07



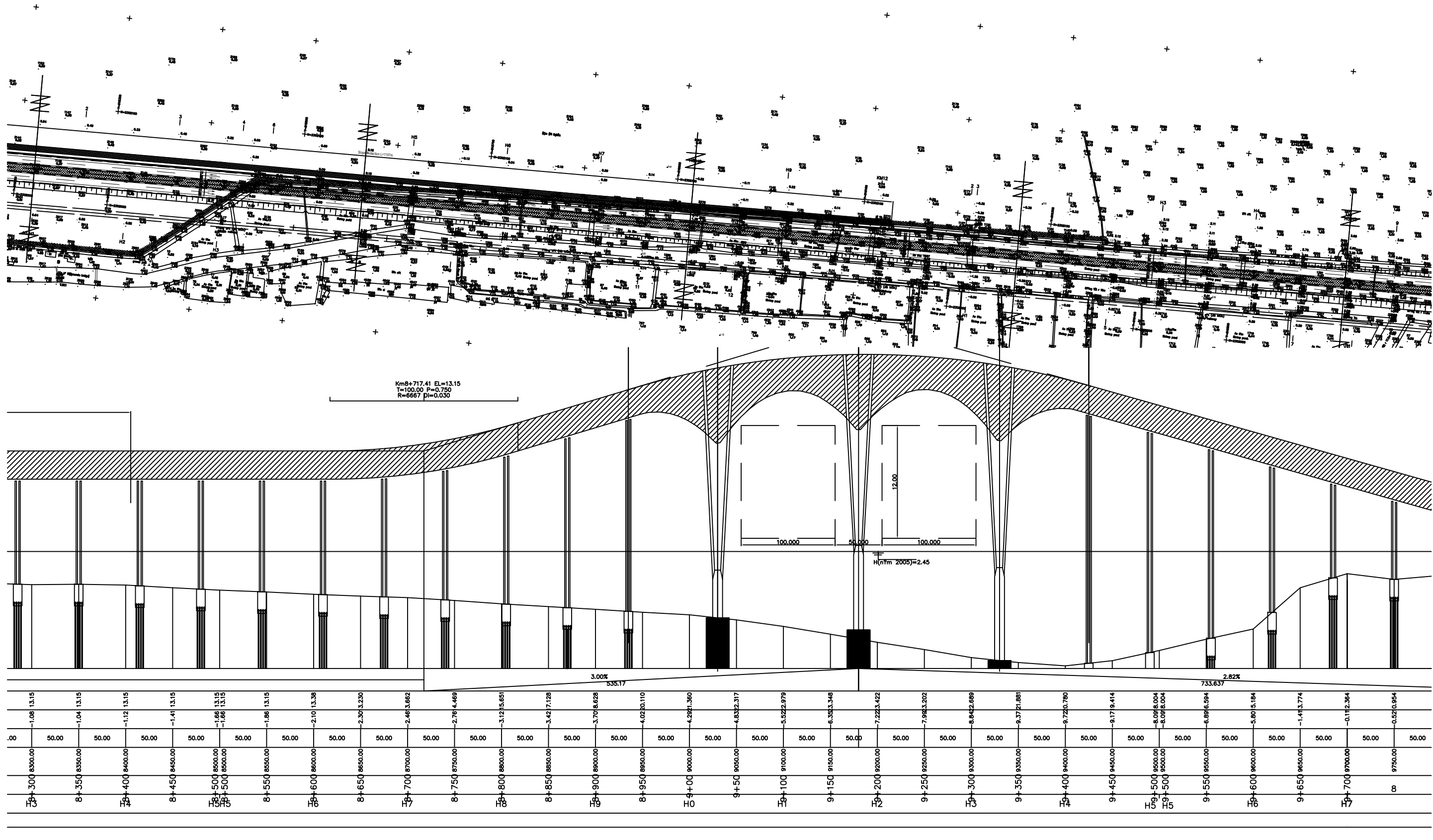
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OF
LACH HUYEN PORT CONSTRUCTION
(ROAD AND BRIDGE PORTION)



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TITLE
Plan and Profile

SCALE
1:4000
NO.
R-02-08



THE PREPARATORY SURVEY
OF
LACH HUYEN PORT CONSTRUCTION
(ROAD AND BRIDGE PORTION)



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TITLE

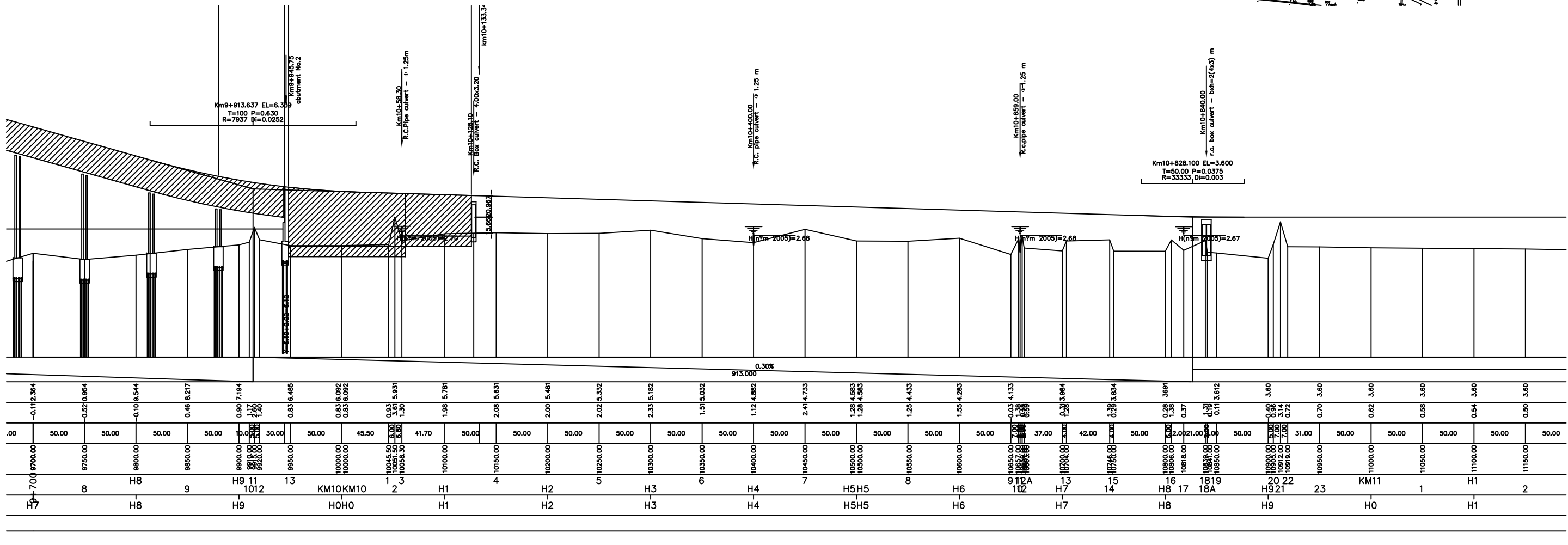
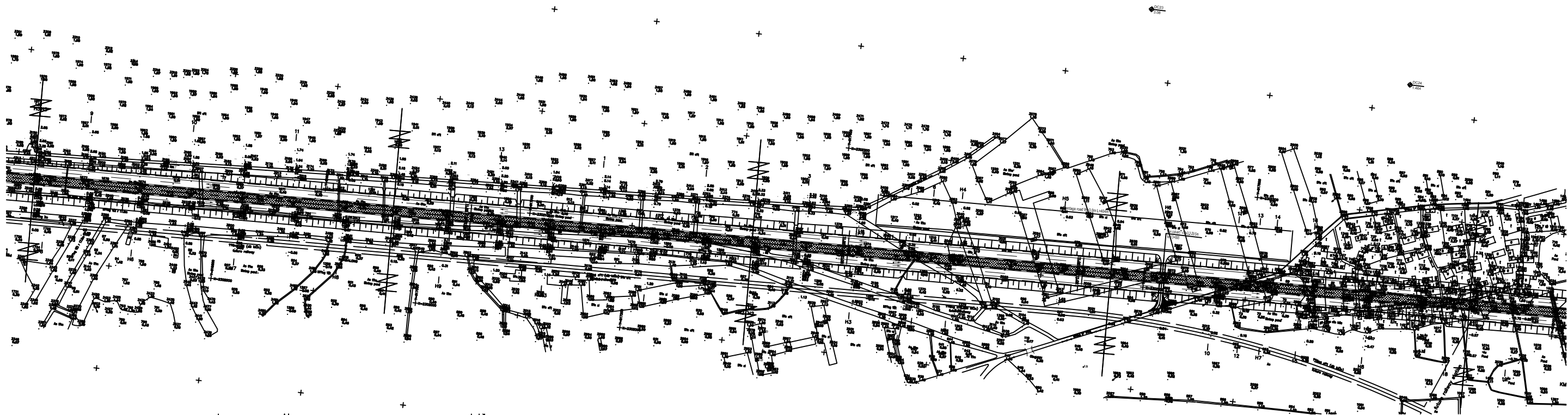
Plan and Profile

SCALE

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NO.

R-02-09



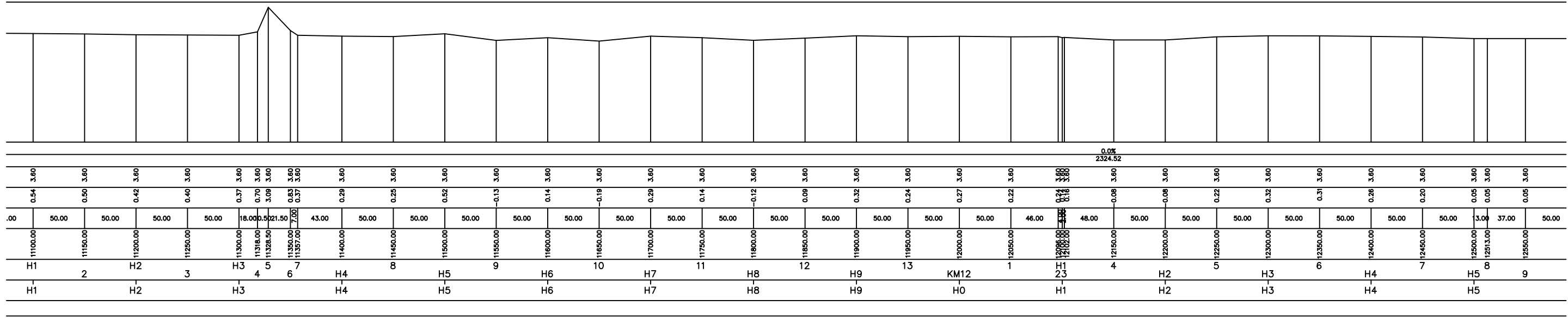
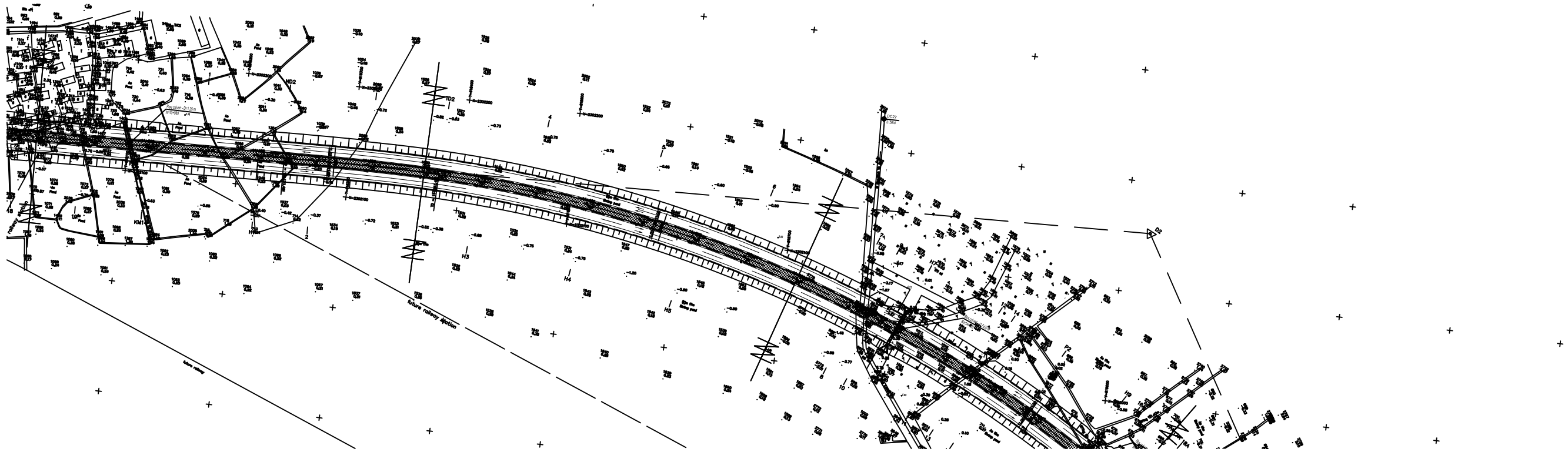
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LACH HUYEN PORT CONSTRUCTION
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TITLE
Plan and Profile

SCALE
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NO.
R-02-10



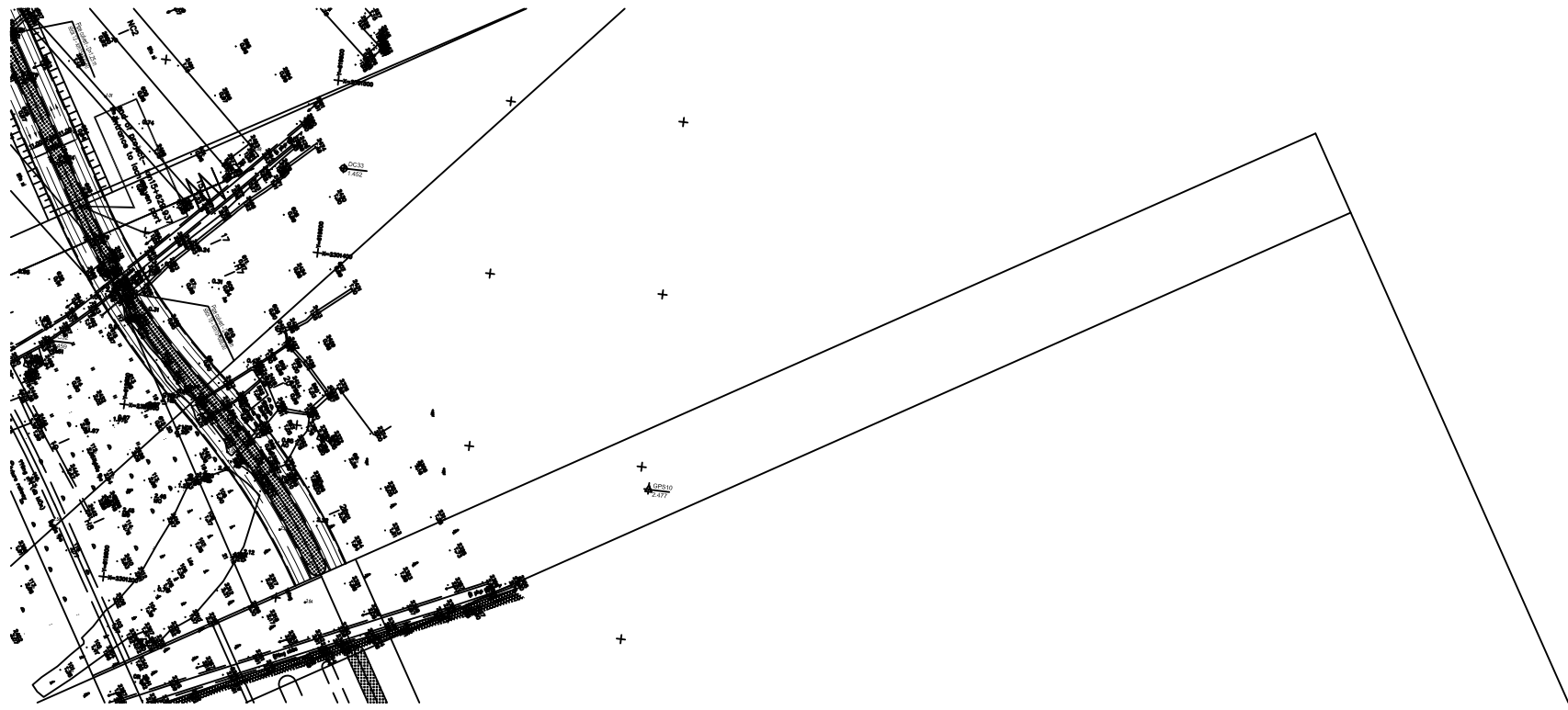
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TITLE
Plan and Profile

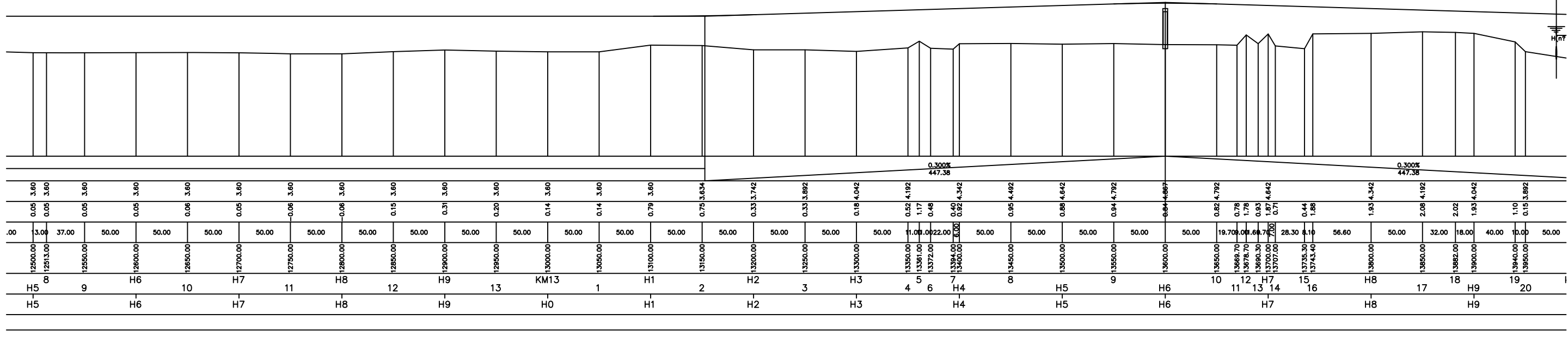
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NO.
R-02-11



Km13+152.620 EL=3.600
T=50.00 P=0.0375
R=33333, Di=0.003

Km13+600.00 EL=4.942
T=50.00 P=0.075
R=16667, Di=0.006

Km13+880.00
r.c. pipe culvert - w=1.25 m



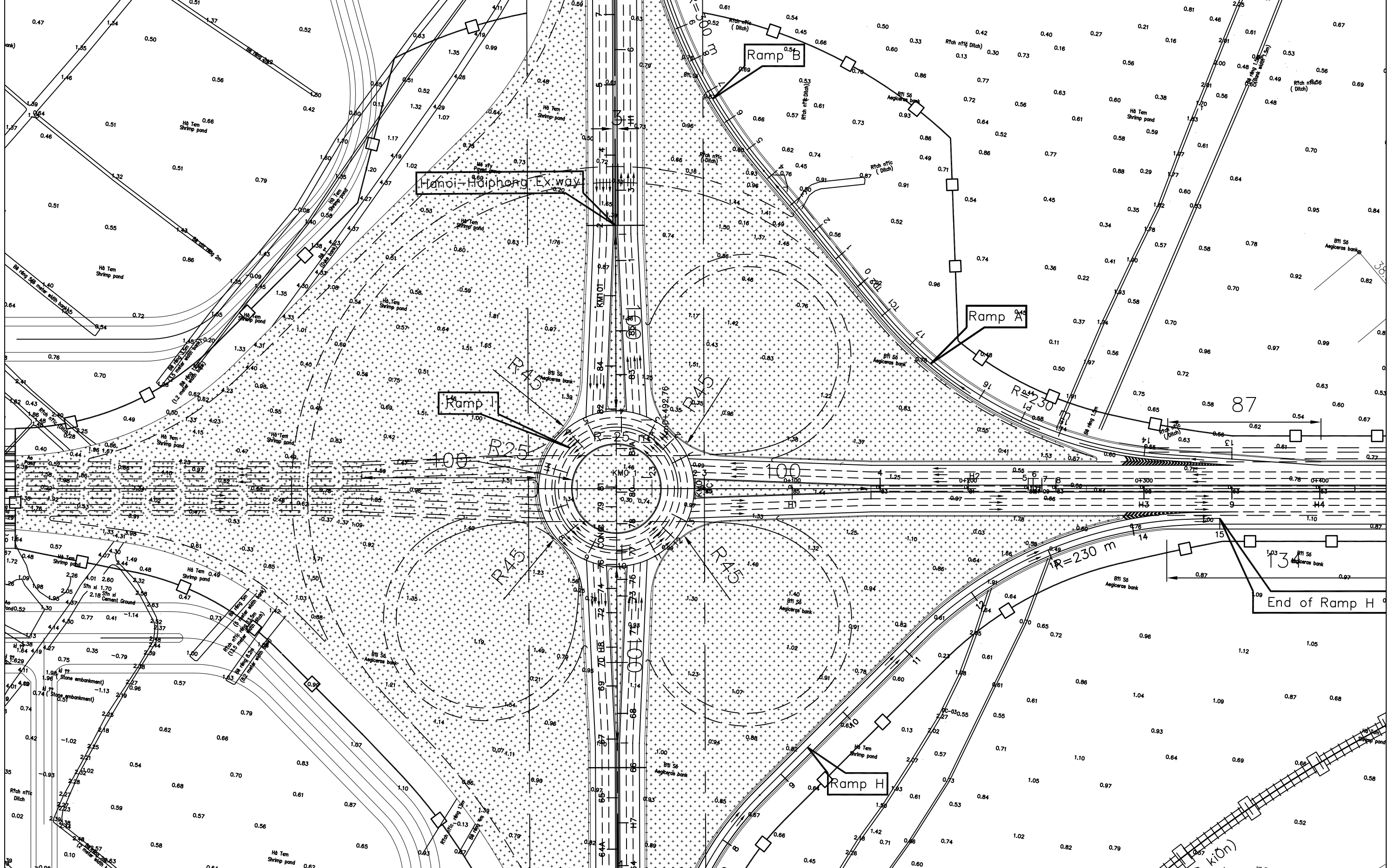
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OF
LACH HUYEN PORT CONSTRUCTION
(ROAD AND BRIDGE PORTION)



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TITLE
Plan and Profile

SCALE
1:4000
NO.
R-02-12



THE PREPARATORY SURVEY
OF
LACH HUYEN PORT CONSTRUCTION
(ROAD AND BRIDGE PORTION)



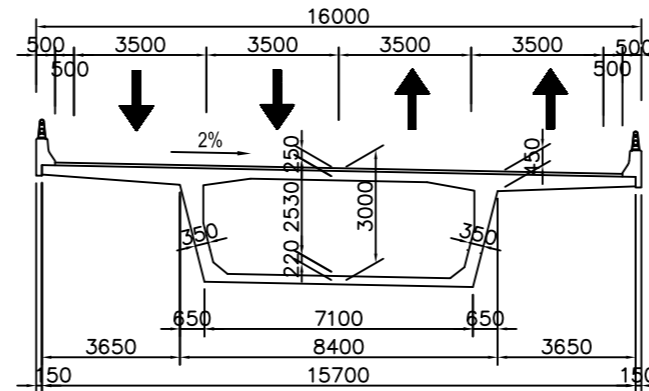
JAPAN INTERNATIONAL COOPERATION AGENCY (JICA)
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TITLE
Tan Vu Intersection
(at grade)

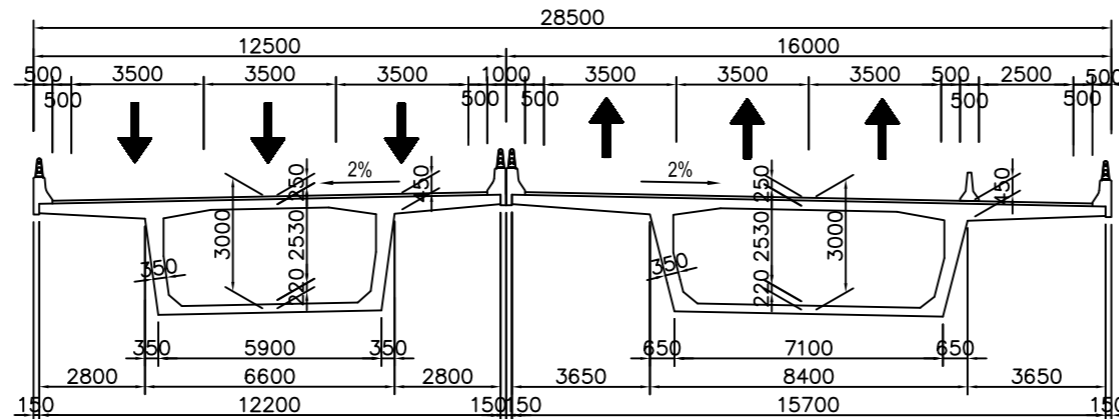
SCALE
1:2000
NO.
R-03

TYPICAL CROSS SECTIONS (Stage Construction, Full 6-lane)

1st STAGE CONSTRUCTION, $B_{\text{bridge}} = 16\text{m}$
 $B_{\text{lane}} = 3.5\text{m}$ (4 LANES)



FULL SCALE CONSTRUCTION, $B_{\text{bridge}} = 28.5\text{m}$
 $B_{\text{lane}} = 3.5\text{m}$ (6 LANES)



THE PREPARATORY SURVEY
 OF
 LACH HUYEN PORT CONSTRUCTION
 (ROAD AND BRIDGE PORTION)

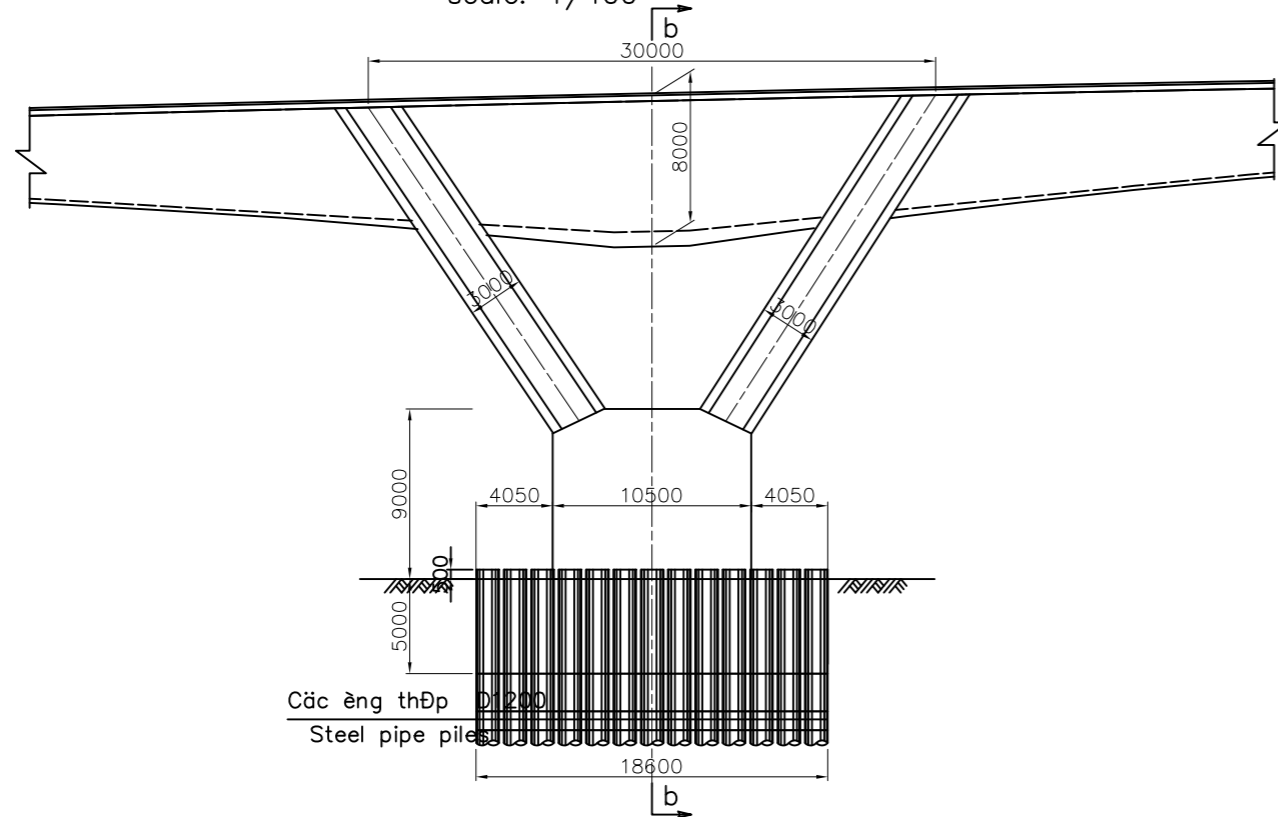


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TITLE
 TYPICAL CROSS SECTIONS
 (Stage Construction, Full 6-lane)

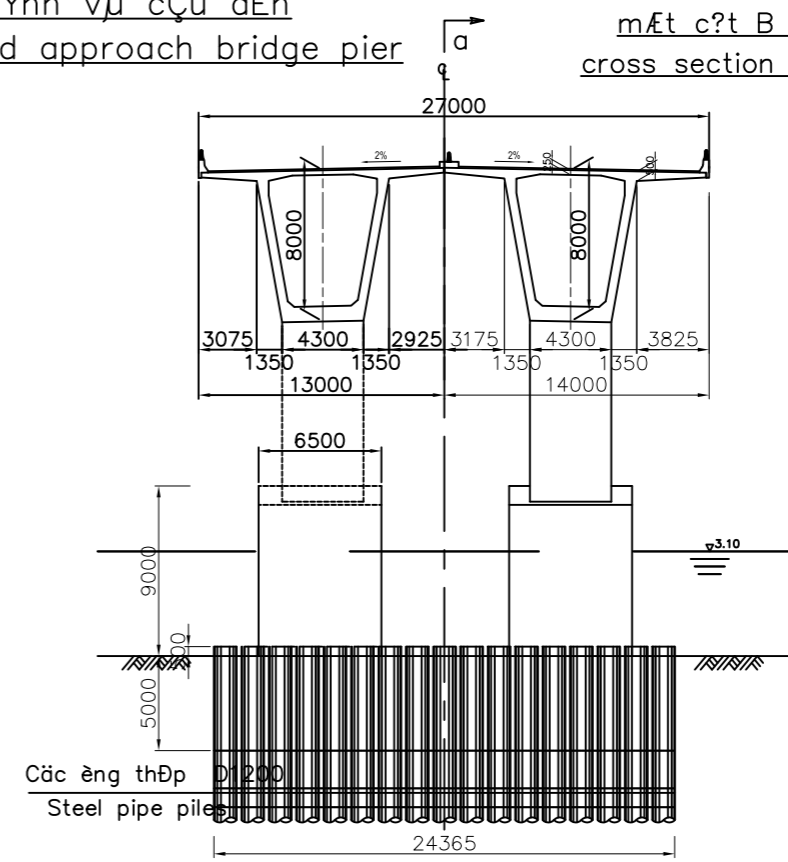
SCALE
 1:200
 NO.
 B-01

mÆt c?t A - A
cross section A - A
tû lÖ: 1/400
scale: 1/400

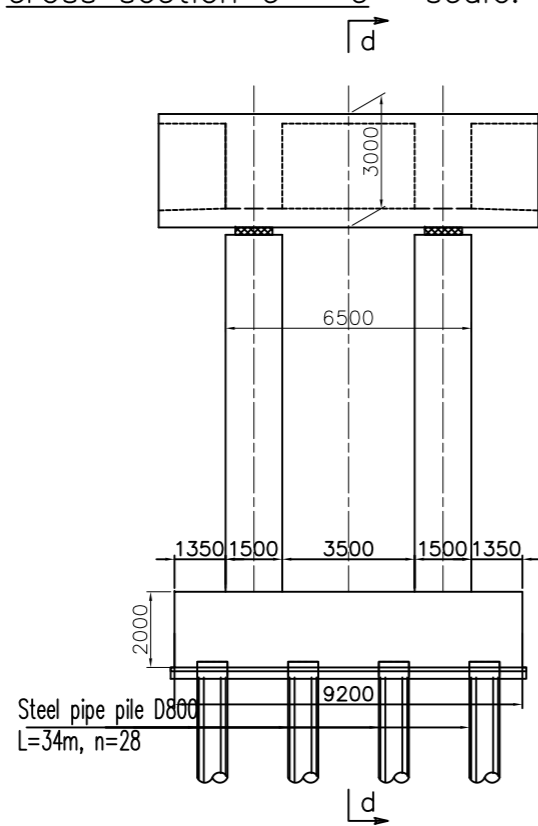


kÝch th?ic chung trÖ cÇu chÝnh vµ cÇu dãn
general view of main bridge pier and approach bridge pier

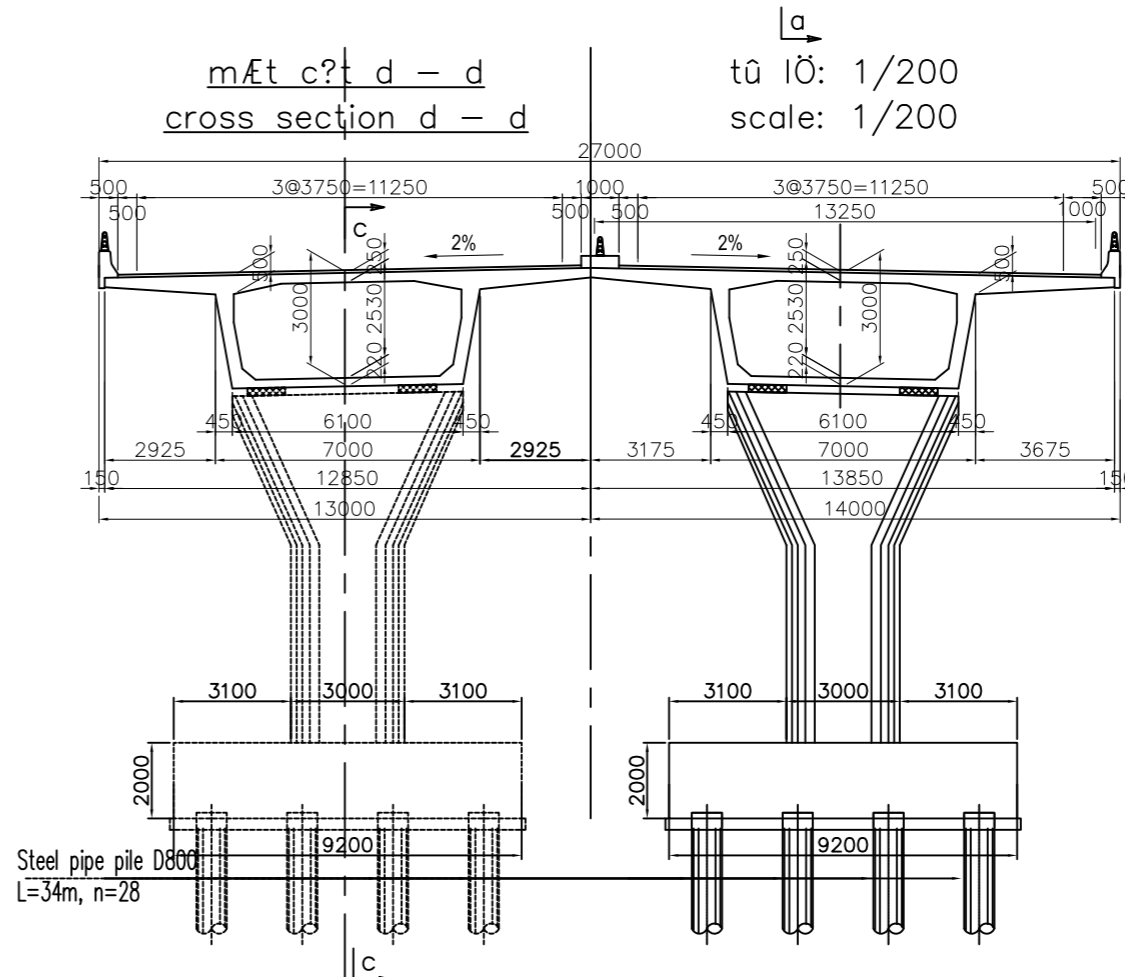
mÆt c?t B - B
cross section B - B
tû lÖ: 1/400
scale: 1/400



mÆt c?t c - c
cross section c - c
tû lÖ: 1/200
scale: 1/200



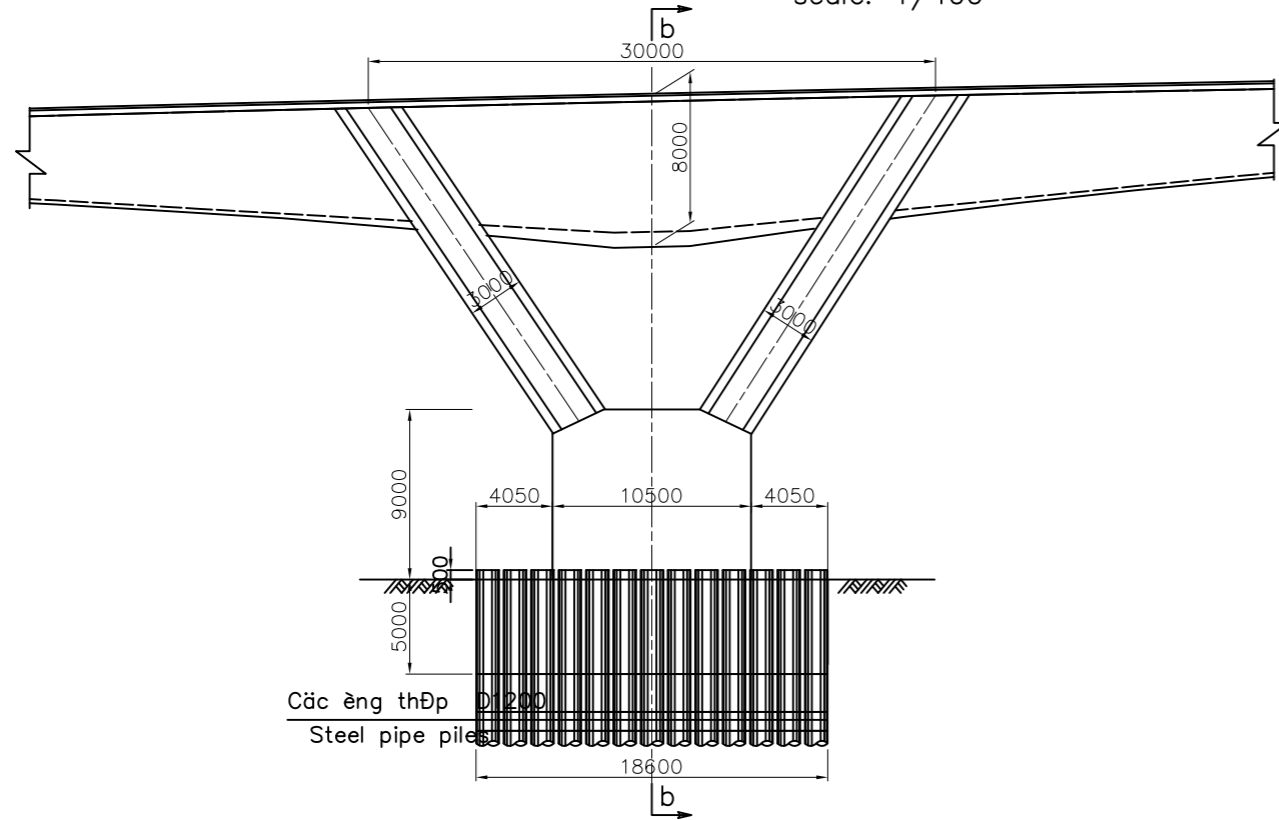
mÆt c?t d - d
cross section d - d
tû lÖ: 1/200
scale: 1/200



kÝch th?íc chung tr?c cÇu chÝnh vµ cÇu d?n
 general view of main bridge pier and approach bridge pier

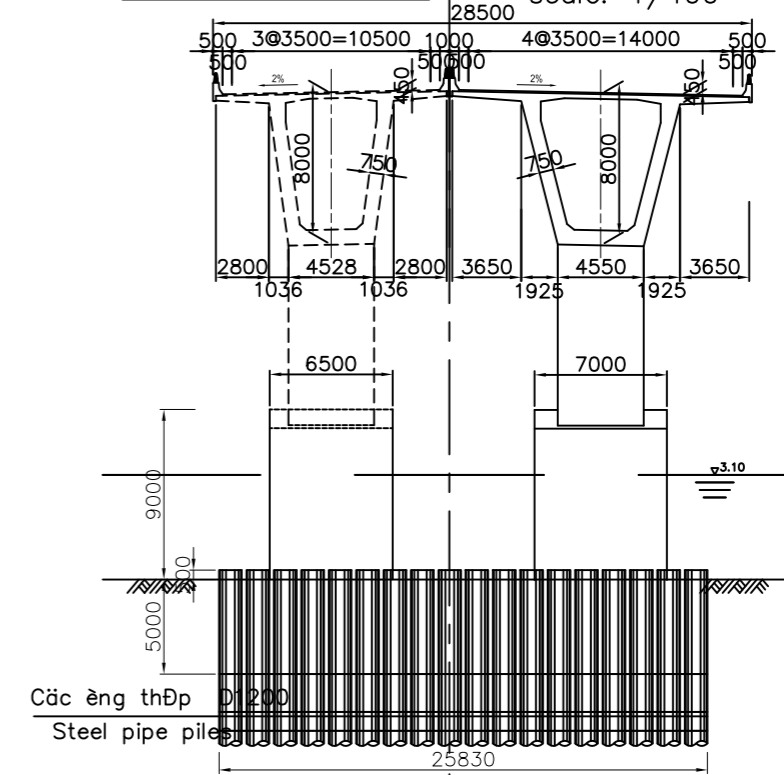
mÆt c?t A - A
 cross section A - A

tû l?i: 1/400
 scale: 1/400



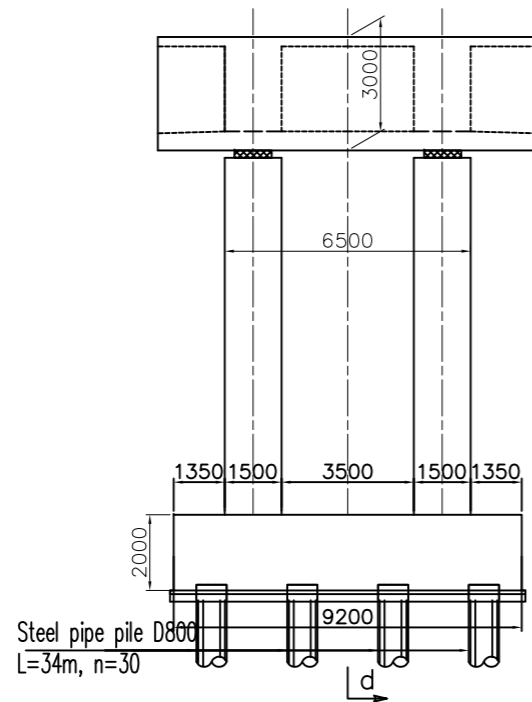
mÆt c?t B - B
 cross section B - B

tû l?i: 1/400
 scale: 1/400



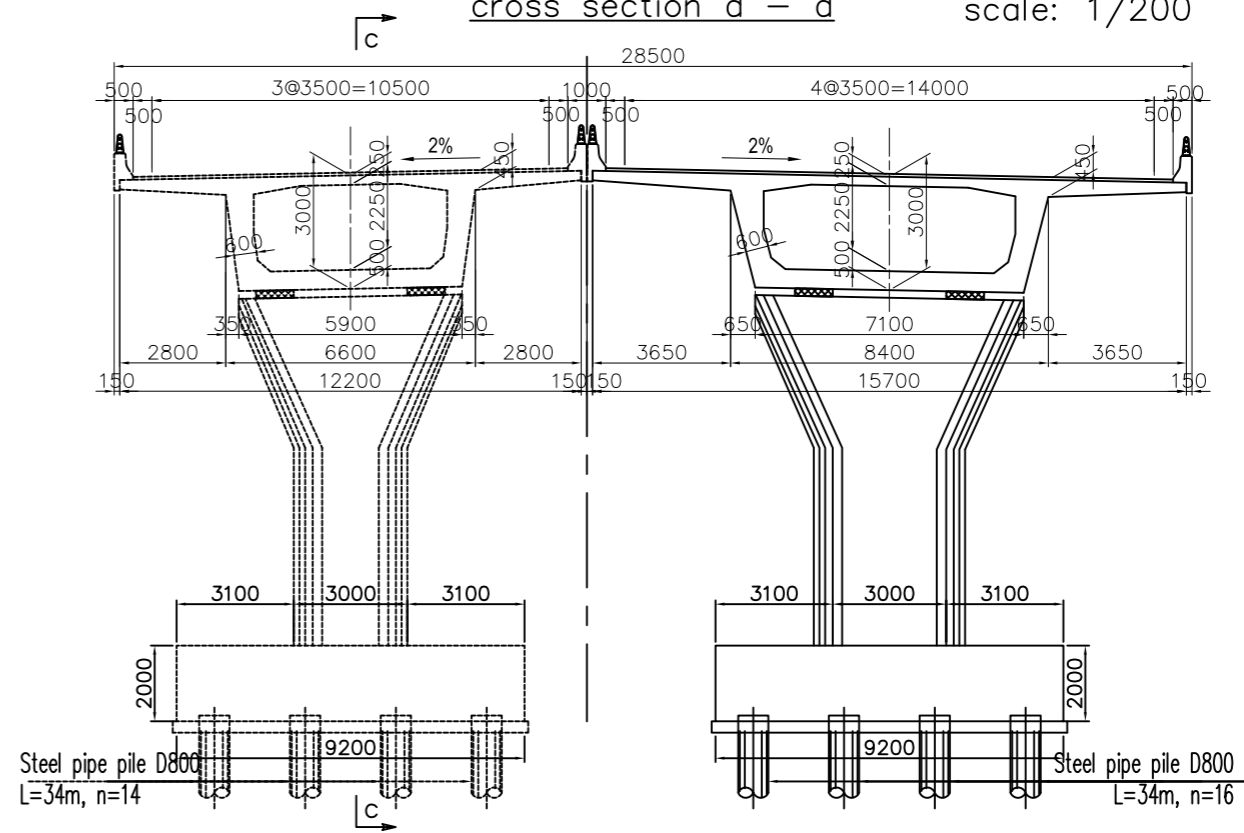
mÆt c?t c - c
 cross section c - c

tû l?i: 1/200
 scale: 1/200



mÆt c?t d - d
 cross section d - d

tû l?i: 1/200
 scale: 1/200



THE PREPARATORY SURVEY
 OF
 LACH HUYEN PORT CONSTRUCTION
 (ROAD AND BRIDGE PORTION)



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TITLE

General View(2)
 Approach Bridge

SCALE

NO.

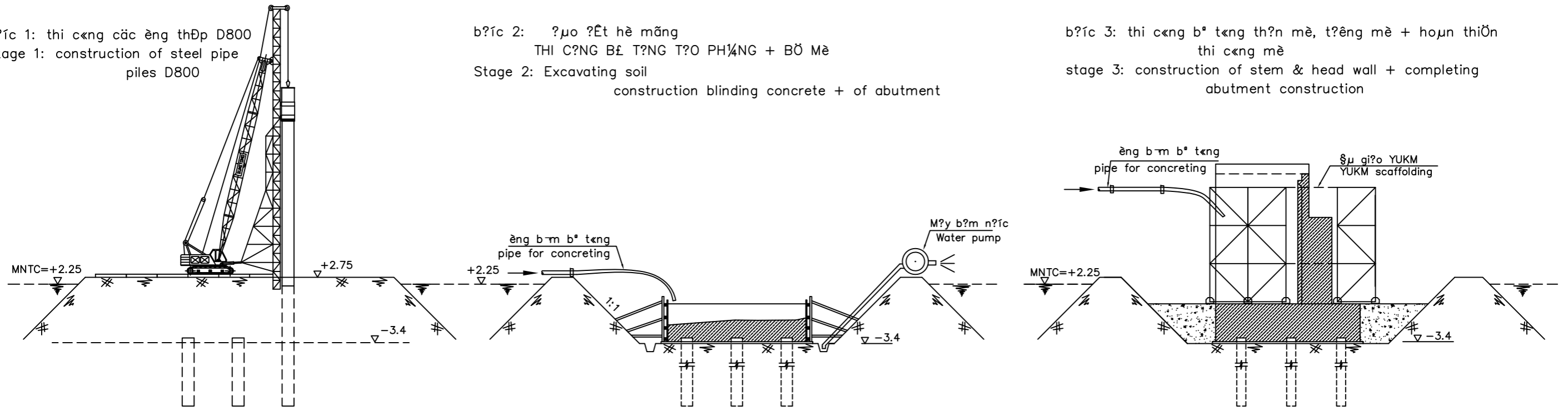
B-03

c?c b?íc thi c?ng mề – construction method of abutment

b?íc 1: thi c?ng các òng th?p D800
Stage 1: construction of steel pipe piles D800

b?íc 2: ?µo ?Èt h? m?ng
THI C?NG B? T?NG T?O PH?NG + B? Mề
Stage 2: Excavating soil
construction blinding concrete + of abutment

b?íc 3: thi c?ng b? t?ng th?n mề, t?ng mề + hoµn thi?n
thi c?ng mề
stage 3: construction of stem & head wall + completing
abutment construction

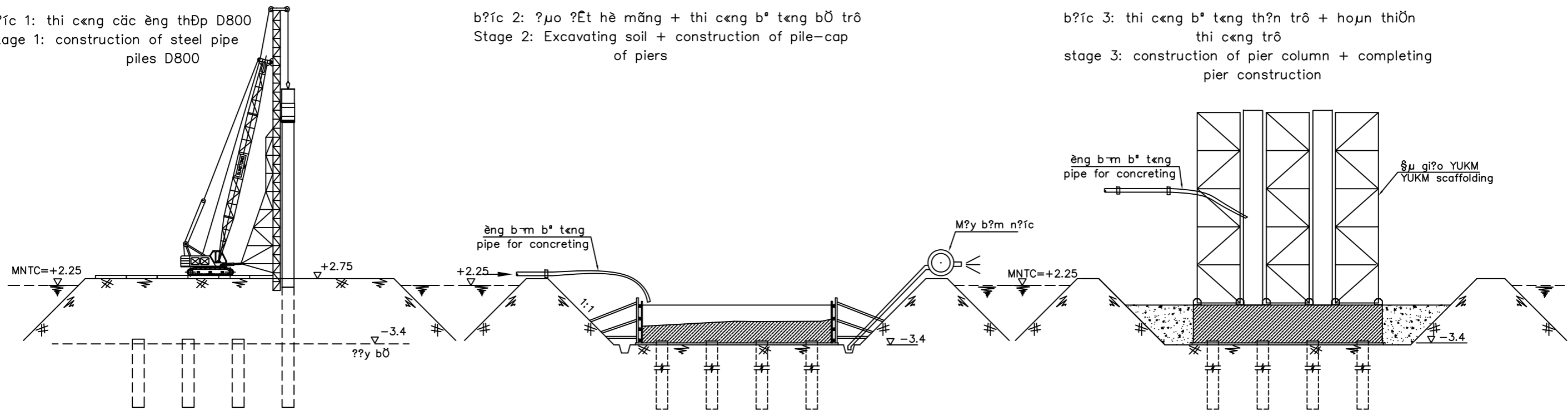


c?c b?íc thi c?ng tr? c?u d?n – construction method of approach bridge pier

b?íc 1: thi c?ng các òng th?p D800
Stage 1: construction of steel pipe piles D800

b?íc 2: ?µo ?Èt h? m?ng + thi c?ng b? t?ng b? tr?
Stage 2: Excavating soil + construction of pile-cap
of piers

b?íc 3: thi c?ng b? t?ng th?n tr?, hoµn thi?n
thi c?ng tr?
stage 3: construction of pier column + completing
pier construction



THE PREPARATORY SURVEY
OF
LACH HUYEN PORT CONSTRUCTION
(ROAD AND BRIDGE PORTION)



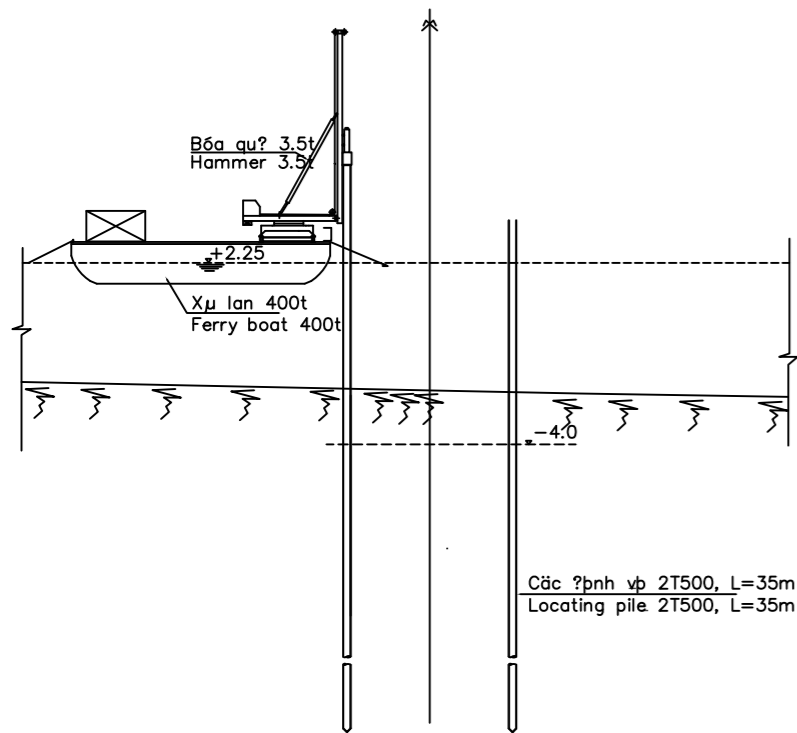
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TITLE
Construction Method
of Abutment and
Approach Bridge Pier(On Land)

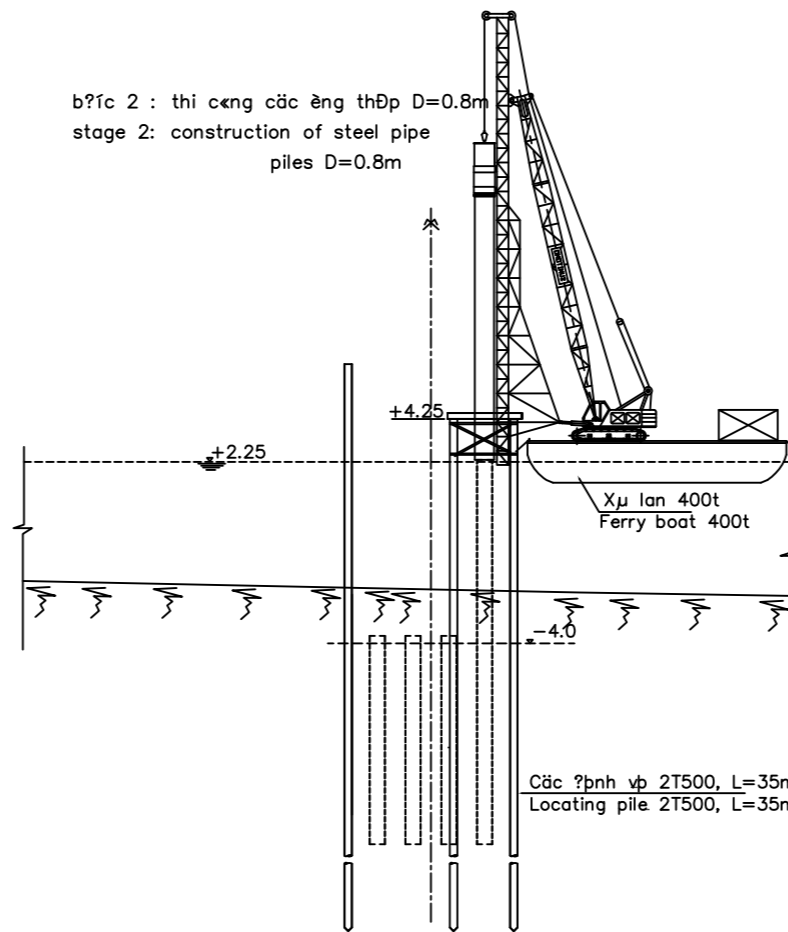
SCALE
1:1
NO.
C-01

thi c«ng trô cÇu dĒn – construction method of approach bridge pier

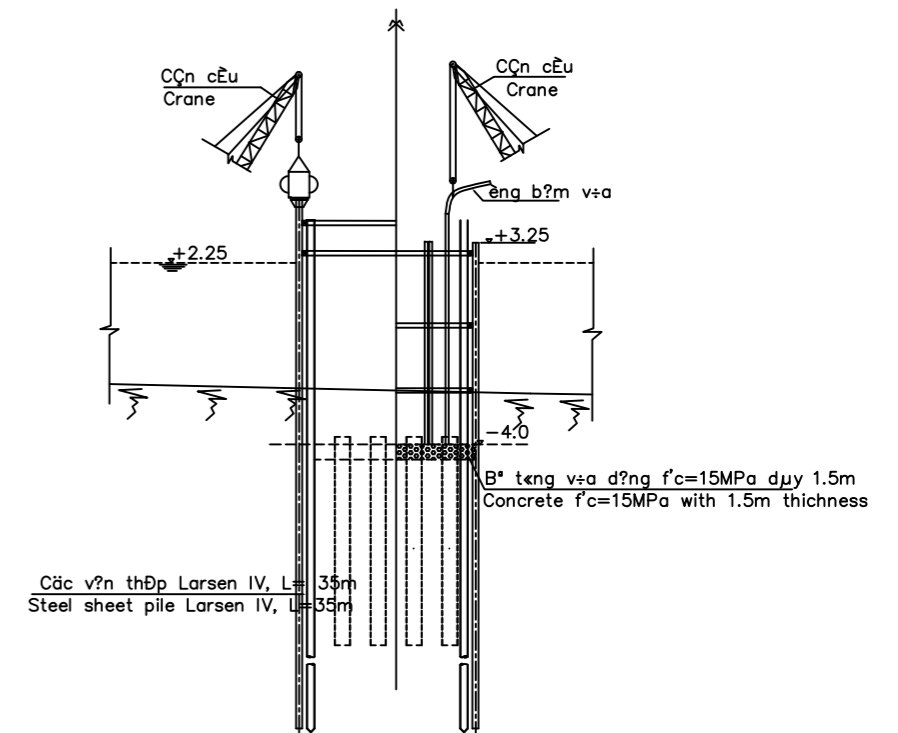
b?íc 1: thi c«ng c«c ?pnh v?p
stage 1: construction of locating piles



b?íc 2 : thi c«ng c«c ãng thĐp D=0.8m
stage 2: construction of steel pipe piles D=0.8m

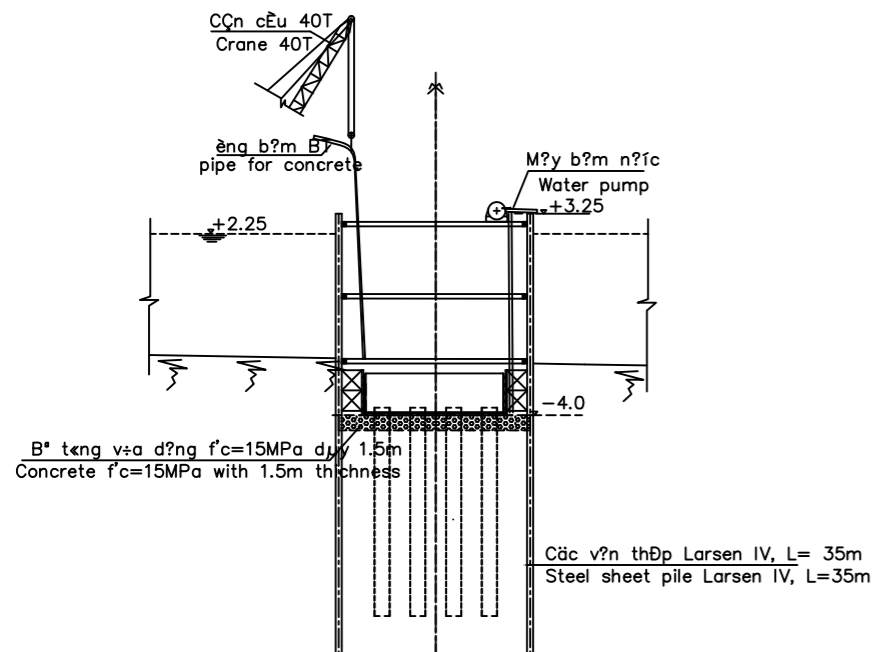


b?íc 3: thi c«ng c«c v?n thĐp
stage 3: construction of steel sheet pile

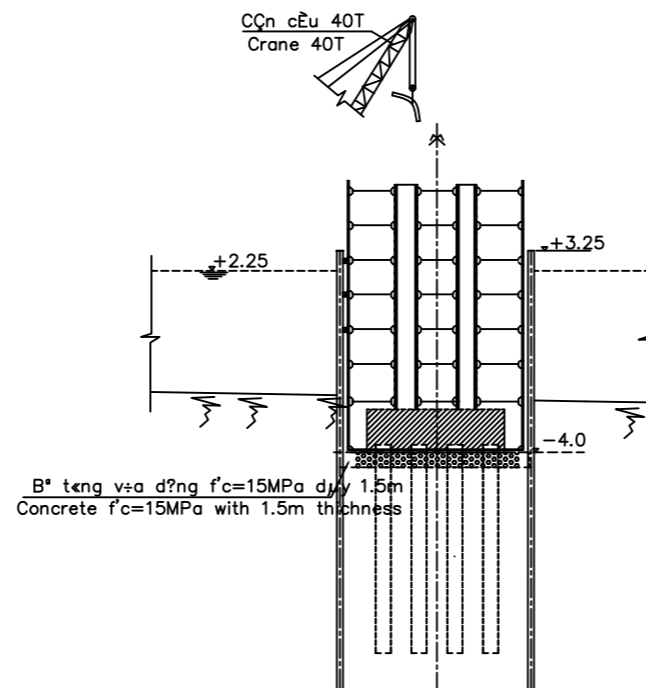


b?íc 4: ?µo ?Ēt trong v8ng v?y c«c v?n thĐp
thi c«ng b?c ??y hĒ mǎng
stage 4: Excavating soil inside sheet pile wall

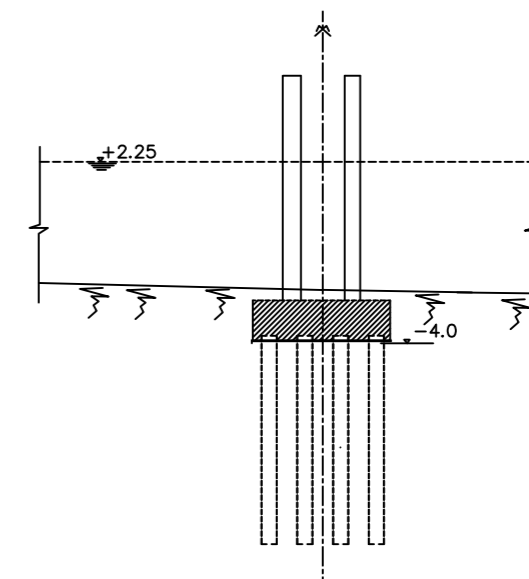
b?íc 5 : thi c«ng b* t«ng bŔ trô
stage 5: construction of pile-cap



b?íc 6 : thi c«ng b* t«ng th?n trô
stage 6: construction of pier column

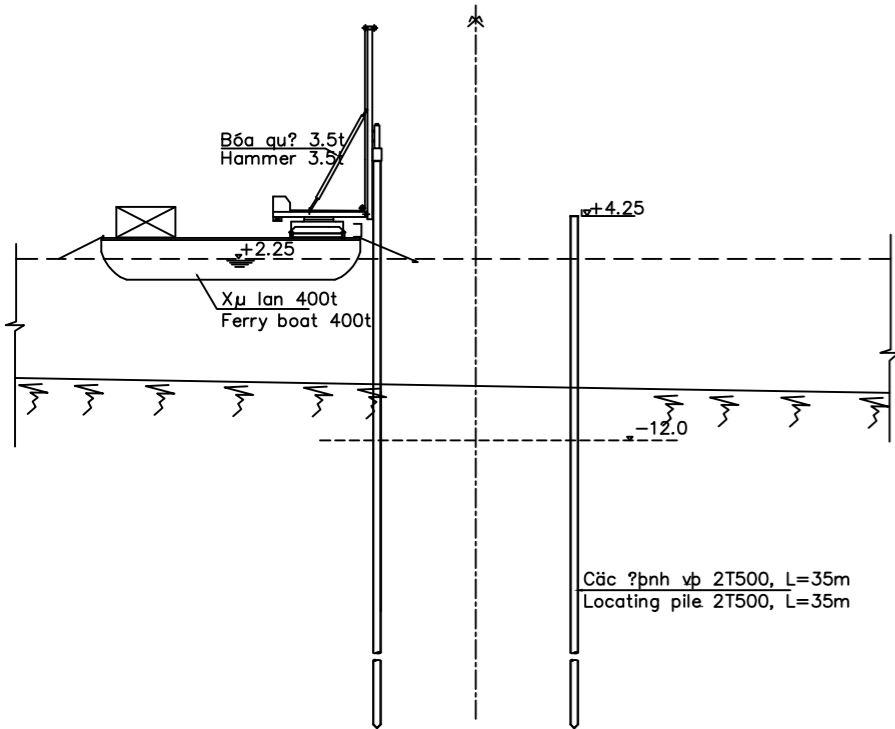


b?íc 7: HOµN THµNH C?NG T?C THI C?NG trô
stage 7: completing pier construction

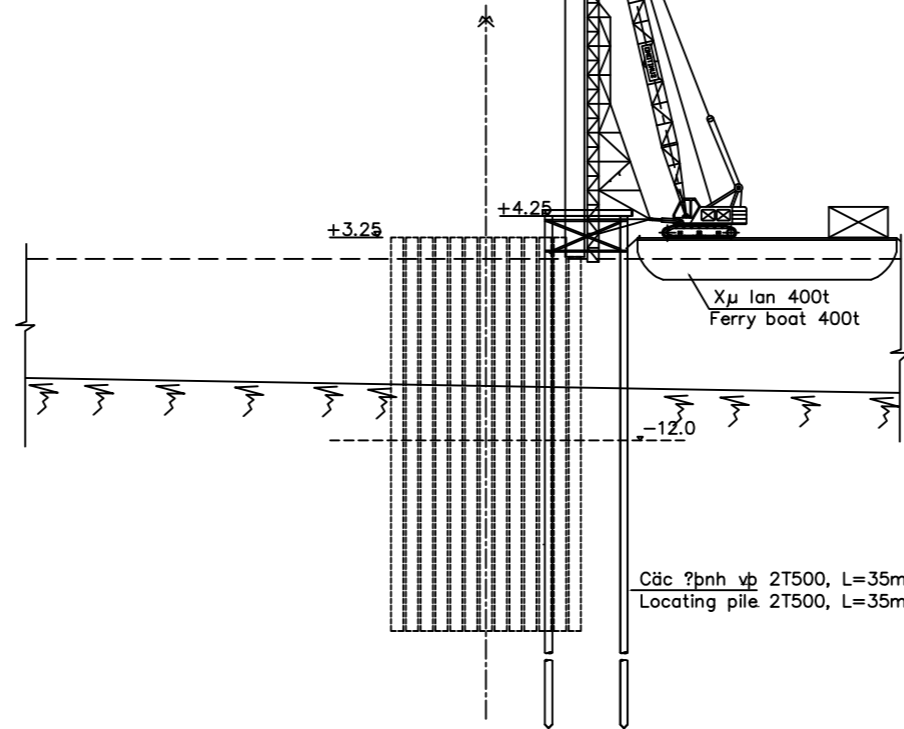


thi cng trô cÇu chÝnh – construction method of main bridge pier

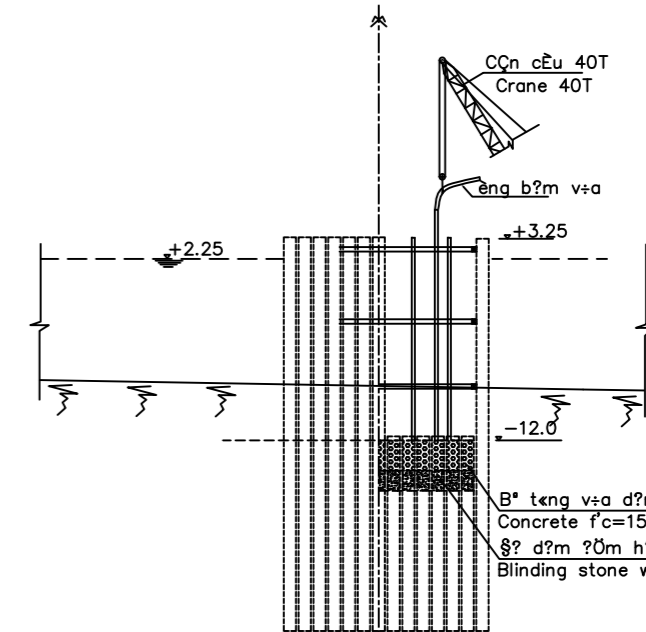
B?íc 1: thi cng c¸c ?pnh v¸p
stage 1: construction of locating piles
and putting in grid frame



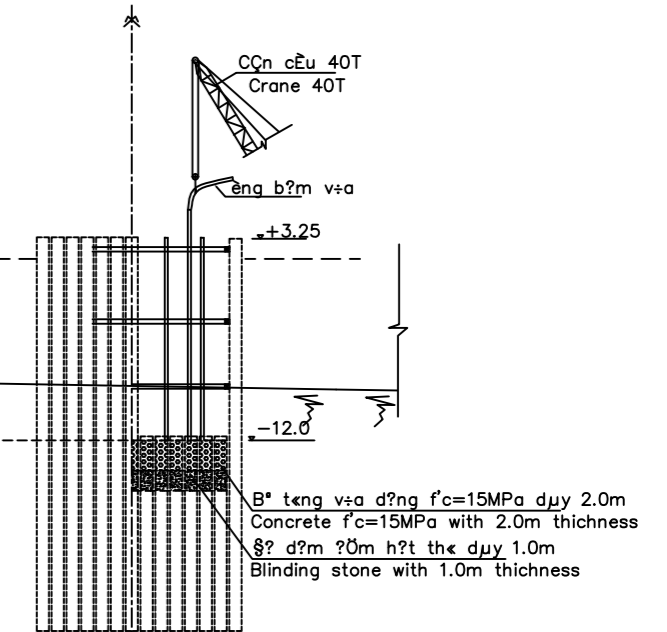
b?íc 2 : thi cng c¸c ãng th¸p D=1.2m
stage 2: construction of steel pipe
piles D=1.2m



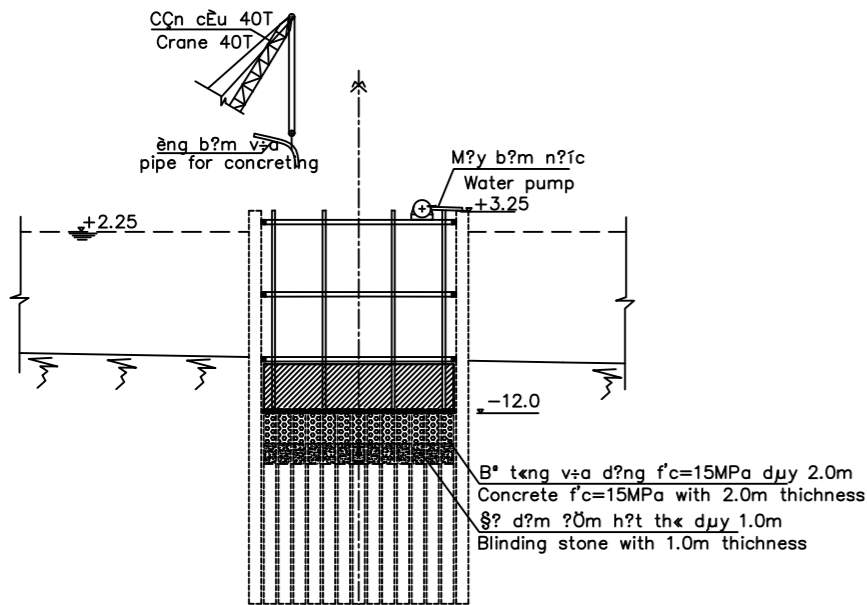
b?íc 3: thi cng c¸c ãng th¸p
stage 3: construction of steel
sheet pile



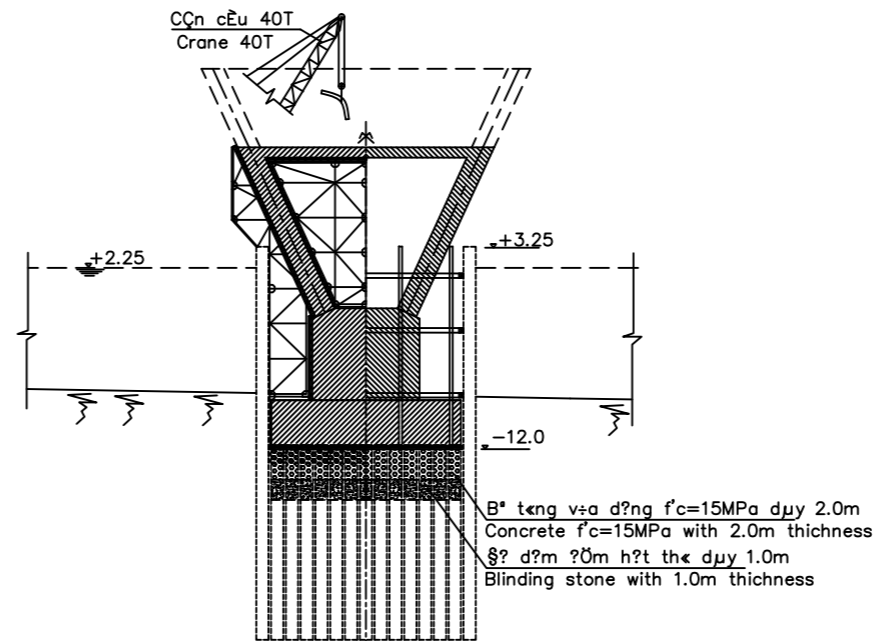
b?íc 4: ?µo ?¸t trong v¸ng v?y c¸c v?n th¸p
thi cng b¸t ?y m¸ng
stage 4: Excavating soil inside sheet pile wall
construction of seal concrete



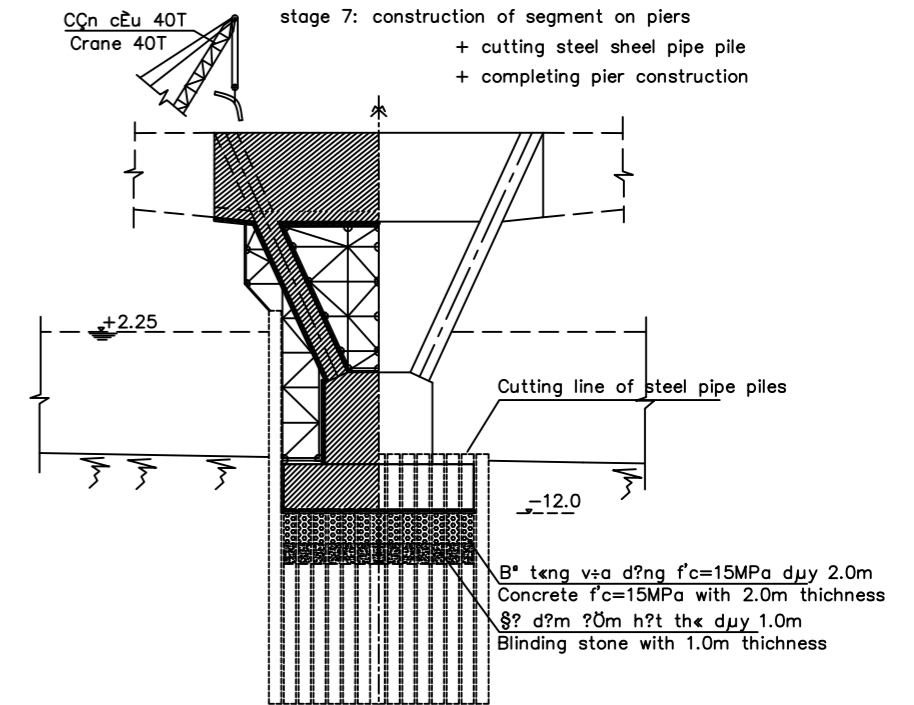
b?íc 5 : thi cng b* tng b¸ trô
stage 5: construction of pile-cap of piers



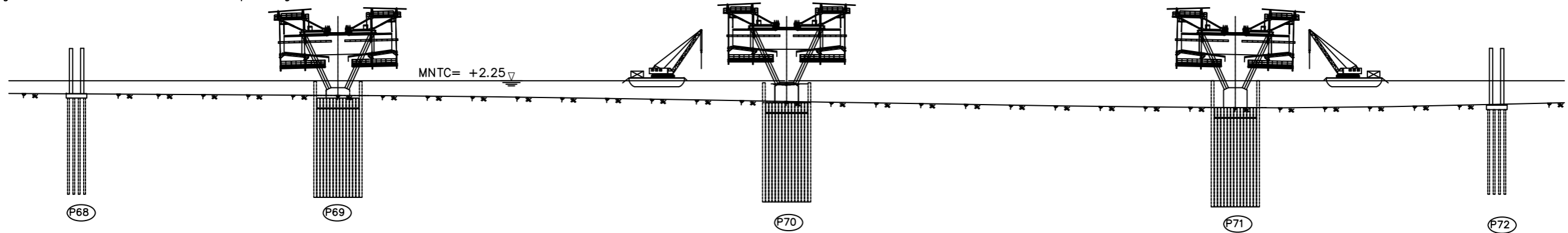
b?íc 6 : thi cng b* tng th?n trô
stage 6: construction of pier column



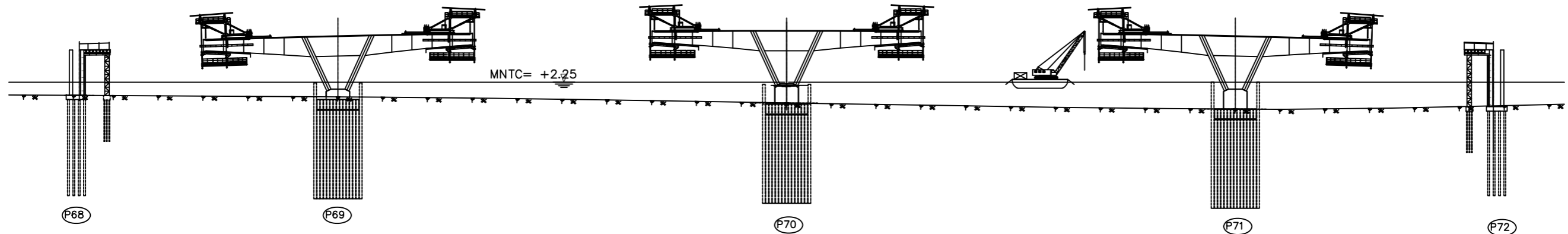
b?íc 7: thi cng kh¸i ?¸nh trô
+ c?t h¸ c¸c ãng th¸p
+ ho¸n thi¸n thi cng
stage 7: construction of segment on piers
+ cutting steel sheet pile
+ completing pier construction



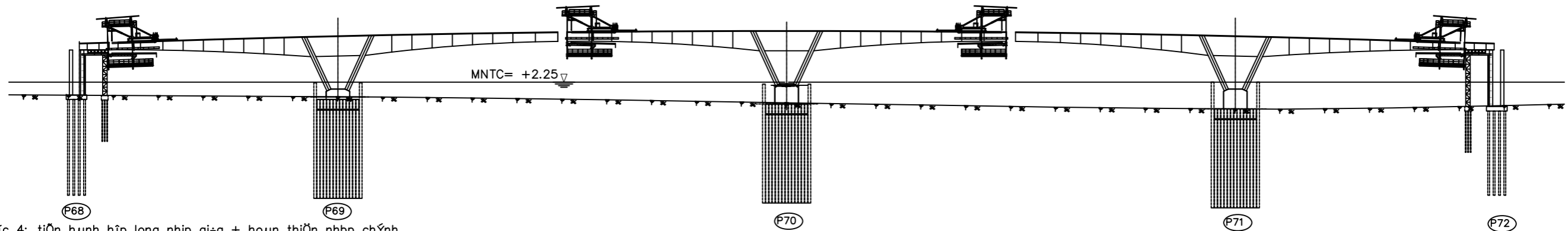
b?íc 1: Lắp dựng xe ?óc t?i kh?i tr?n ??nh tr?o
stage 1: Installation of traveller on the pier segments



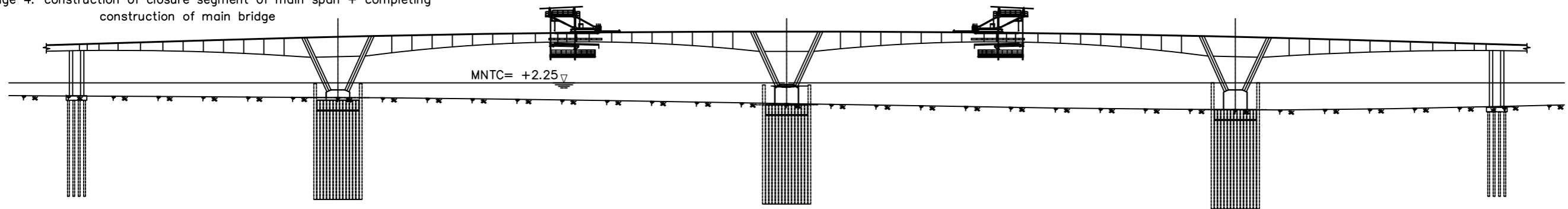
b?íc 2: thi công c?c kh?i theo ph??ng ph?p c?n b?ng ??i x?ng
stage 2: construction of segment by the balance cantilever method



b?íc 3: thi công kh?i h?p long c?a nhịp bi?n
stage 3: construction of closure segment of side span



b?íc 4: ti?n h?nh h?p long nhịp gi?a + ho?n thi?n nh?p ch?nh
stage 4: construction of closure segment of main span + completing construction of main bridge

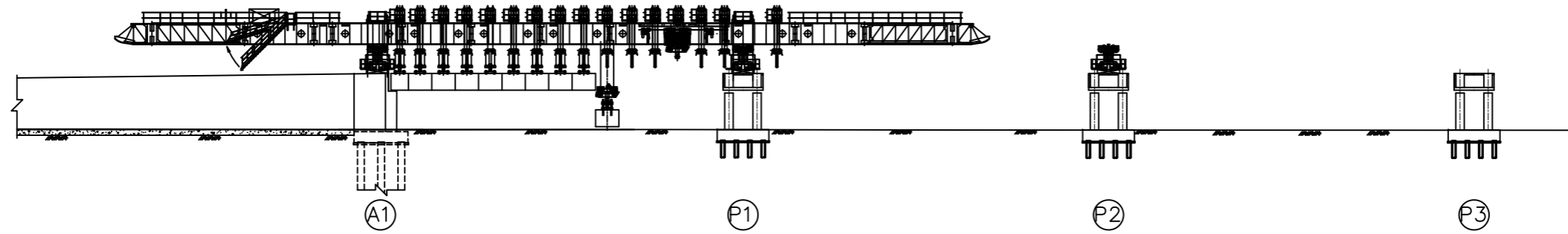


thi c«ng k«t c«u phÇn trªn cªa cÇu d«n – construction method of approach bridge superstructure

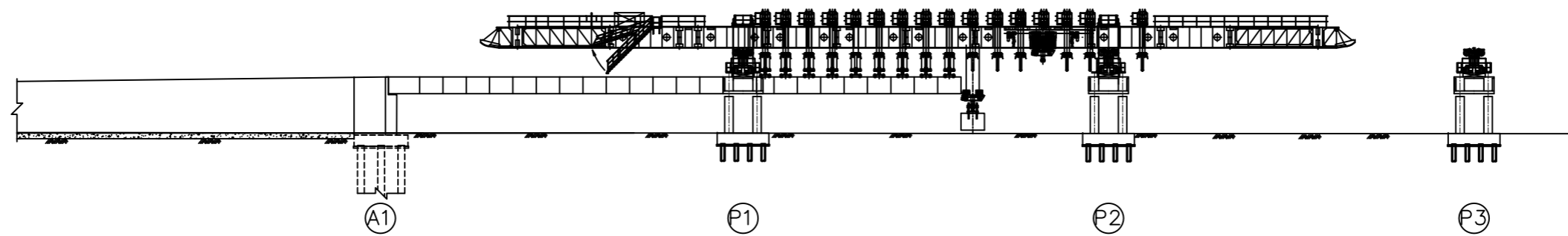
«i T?n v«
to Tan vu

«i l?ch huy«n
to lach huyen

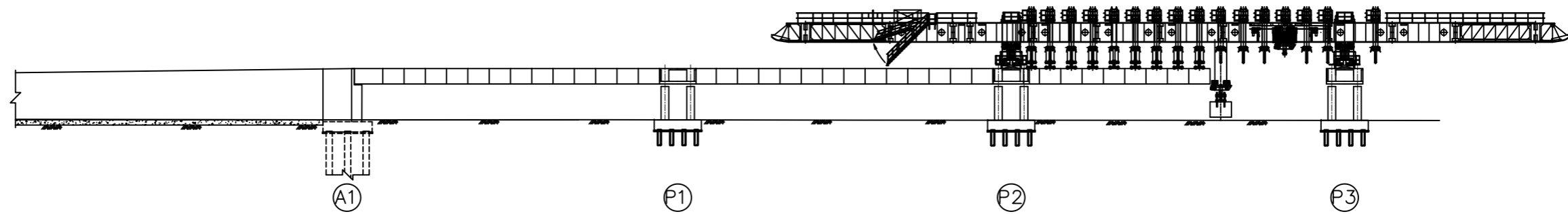
B?iC 1: THI C?NG nh?p 1
STAGE 1: CONSTRUCTION OF SPAN No.1



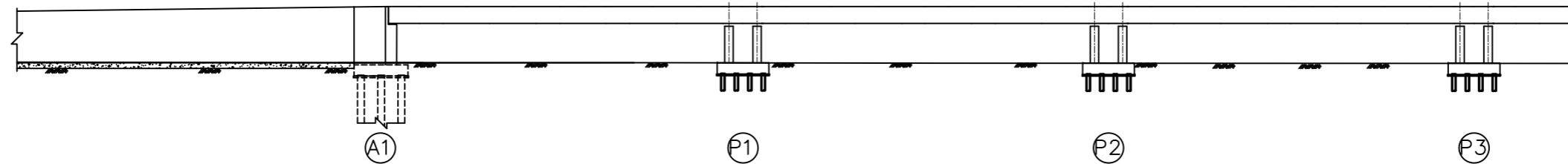
B?iC 2: THI C?NG nh?p 2
STAGE 2: CONSTRUCTION OF SPAN No.2



B?iC 3: THI C?NG nh?p 3
STAGE 3: CONSTRUCTION OF SPAN No.3



B?iC 4: – TI«P T«C THI C?NG C?C NH?p D«N KH?C t?ng t«
– h«m thi«n THI C?NG NH?p D«N



STAGE 4: – other spans are continue constructed as above stages
– CONSTRUCTION OF APPROACH SPANs IS COMPLETED

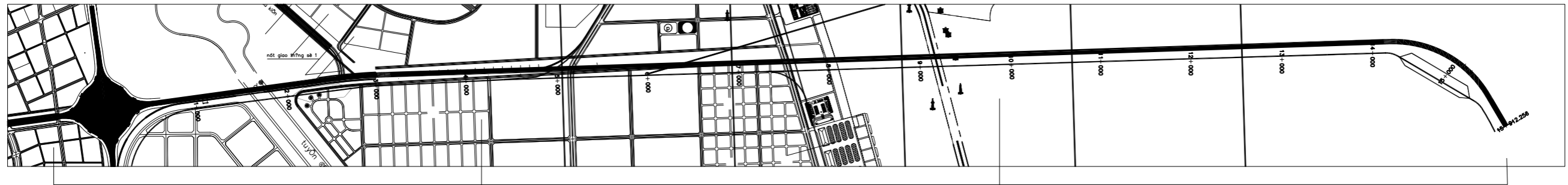
THE PREPARATORY SURVEY
OF
LACH HUYEN PORT CONSTRUCTION
(ROAD AND BRIDGE PORTION)



JAPAN INTERNATIONAL COOPERATION AGENCY (JICA)
Joint Venture of
Nippon Koei Co., Ltd. and
Japan Bridge & Structure Institute, Inc.

TITLE
Construction Method of
Approach Bridge
Superstructure

SCALE
1:1000
NO.
C-05



road at hai an side

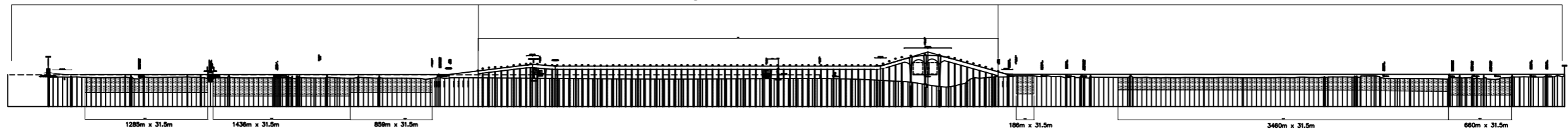
oversea bridge

road at cathai side

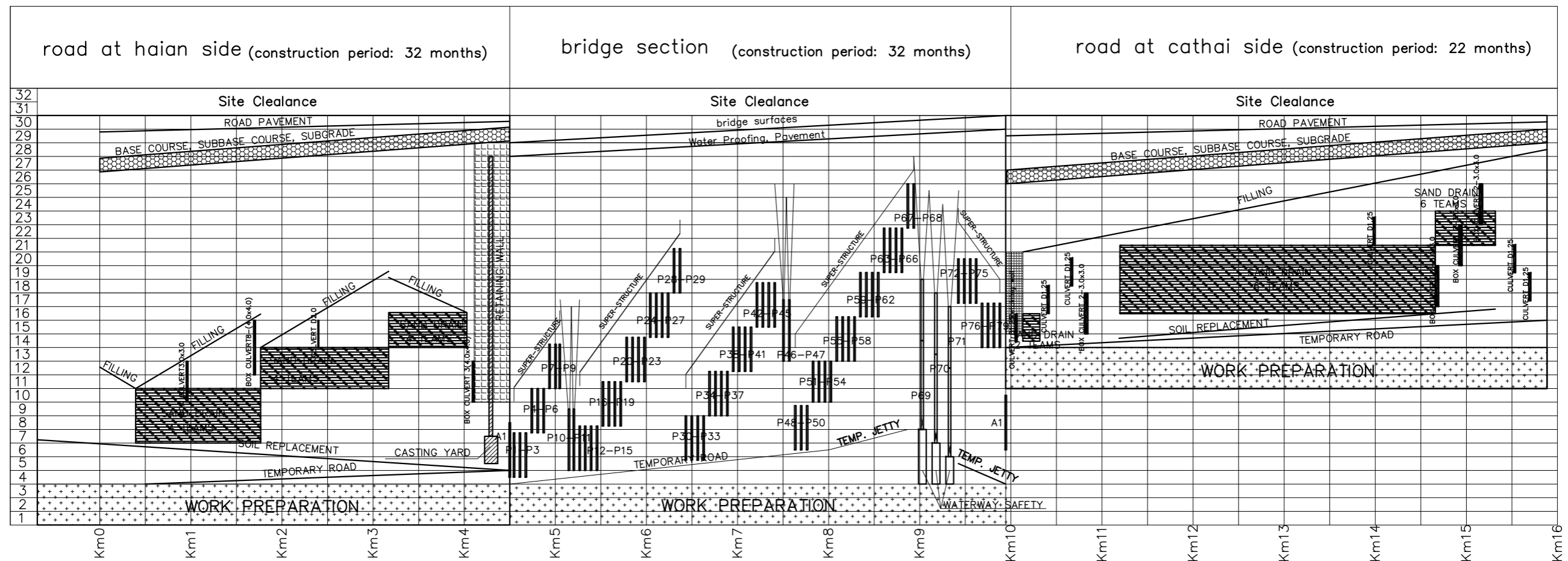
road at Hai An side

bridge section

road at Cat Hai side



construction schedule of alternative 3a



Appendix-2: Traffic Data

Appendix-2-1: Traffic Count Data and Converted PCU

1. Summary of Counted Vehicles Number and PCU Number

Summary of Counted Vehicles Number

Unit: Vehicle

Time	Ninh Tiép			Cat Hai			Ben Got		
	To Cat Ba Direction	To Dinh Vu Direction	Total	To Cat Ba Direction	To Dinh Vu Direction	Total	To Cat Ba Direction	To Dinh Vu Direction	Total
6:00-7:00	12	89	101	66	76	142	47	72	119
7:00-8:00	85	53	138	84	45	129	62	21	83
8:00-9:00	31	21	52	42	40	82	36	35	71
9:00-10:00	53	16	69	72	34	106	45	31	76
10:00-11:00	31	11	42	47	21	68	19	18	37
11:00-12:00	23	21	44	32	19	51	24	39	63
12:00-13:00	0	15	15	28	34	62	9	0	9
13:00-14:00	46	28	74	56	79	135	26	53	79
14:00-15:00	28	35	63	45	35	80	18	19	37
15:00-16:00	27	28	55	69	47	116	53	24	77
16:00-17:00	23	40	63	67	61	128	32	38	70
17:00-18:00	86	0	86	76	54	130	24	30	54
12hr Total	445	357	802	684	545	1229	395	380	775

Summary of PCU Number

Unit: pcu

Time	Ninh Tiép			Cat Hai			Ben Got		
	To Cat Ba Direction	To Dinh Vu Direction	Total	To Cat Ba Direction	To Dinh Vu Direction	Total	To Cat Ba Direction	To Dinh Vu Direction	Total
6:00-7:00	3	24	27	22	31	53	12	31	43
7:00-8:00	55	33	88	53	10	63	39	7	46
8:00-9:00	20	6	26	29	12	41	17	10	27
9:00-10:00	18	5	23	30	11	41	15	9	24
10:00-11:00	25	5	30	27	10	37	8	7	15
11:00-12:00	11	21	32	11	12	23	20	30	50
12:00-13:00	0	9	9	7	11	18	3	0	3
13:00-14:00	23	18	41	25	26	51	13	26	39
14:00-15:00	27	17	44	19	12	31	5	7	12
15:00-16:00	14	21	35	30	27	57	34	20	54
16:00-17:00	15	29	44	21	25	46	8	18	26
17:00-18:00	23	0	23	31	14	45	17	8	25
12hr Total	234	188	422	305	201	506	191	173	364
24hr	281	226	507	366	241	607	229	208	437
D value	55.5%	44.5%		60.3%	39.7%		52.5%	47.5%	

2. Manual Classified Counts Vehicle and PCU

2.1. Ninh Tiep (Dinh Vu side ferry terminal)

DATE	27/04/2010							Investigator name							Dương Văn Tắc		
LOCATION	Ninh Tiếp																
Time	To Cat Ba Direction							To Dinh Vu Direction							Total Movement		
	1	2	3	4	5	6	Total	1	2	3	4	5	6	Total			
6:15	0.2	0.3	1.0	2.0	2.5	4.0	0	44	12	1						57	57
6:30	2	4					6		20							20	26
6:45		4					4		8							8	12
7:00		2					2		4							4	6
Hourly Total	2	10	0	0	0	0	12	44	44	0	1	0	0	0	89	101	
pcu/hr	0	3	0	0	0	0	3	9	13	0	2	0	0	0	24	27	
7:15		3					3	4	12		8	1			25	28	
7:30	2	26	1	14			43	6	14						20	63	
7:45	1	2					3	1	3		1				5	8	
8:00	1	31	2	2			36	2	1						3	39	
Hourly Total	4	62	3	16	0	0	85	11	31	1	9	1	0	53	138		
pcu/hr	1	19	3	32	0	0	55	2	9	1	18	3	0	33			
8:15							0		4						4	4	
8:30							0	1	13						14	14	
8:45							0	1	1						2	2	
9:00	4	21		5	1		31		1						1	32	
Hourly Total	4	21	0	5	1	0	31	2	19	0	0	0	0	21	52		
pcu/hr	1	6	0	10	3	0	20	0	6	0	0	0	0	6			
9:15							0		5						5	5	
9:30							0		5						5	5	
9:45							0		5						5	5	
10:00	10	40	2	1			53		1						1	54	
Hourly Total	10	40	2	1	0	0	53	0	16	0	0	0	0	16	69		
pcu/hr	2	12	2	2	0	0	18	0	5	0	0	0	0	5			
10:15							0								0	0	
10:30							0	1	6	1					8	8	
10:45							0		1						1	1	
11:00	3	18	1	9			31		1		1				2	33	
Hourly Total	3	18	1	9	0	0	31	1	8	1	1	0	0	11	42		
pcu/hr	1	5	1	18	0	0	25	0	2	1	2	0	0	5			
11:15							0		1	1	4	1			7	7	
11:30							0		5	2					7	7	
11:45	5	14	2	2			23		2		1				3	26	
12:00							0		3		1				4	4	
Hourly Total	5	14	2	2	0	0	23	0	11	3	6	1	0	21	44		
pcu/hr	1	4	2	4	0	0	11	0	3	3	12	3	0	21			
12:15							0		6	1	2				9	9	
12:30							0		5						5	5	
12:45							0		1						1	1	
13:00							0								0	0	
Hourly Total	0	0	0	0	0	0	0	0	12	1	2	0	0	15	15		
pcu/hr	0	0	0	0	0	0	0	0	4	1	4	0	0	9			
13:15							0		5	1	2				8	8	
13:30	5	15		6			26	2	8		3				13	39	
13:45							0		3						3	3	
14:00	1	19					20	1	3						4	24	
Hourly Total	6	34	0	6	0	0	46	3	19	1	5	0	0	28	74		
pcu/hr	1	10	0	12	0	0	23	1	6	1	10	0	0	18			
14:15							0	1	18	2	1				22	22	
14:30							0	1	5						6	6	
14:45							0		4	1	1				6	6	
15:00	3	15		9	1		28				1				1	29	
Hourly Total	3	15	0	9	1	0	28	2	27	3	3	0	0	35	63		
pcu/hr	1	5	0	18	3	0	27	0	8	3	6	0	0	17			
15:15							0	1	7		2				10	10	
15:30	1	4		1	1		7		7		4	1			12	19	
15:45		19		1			20		6						6	26	
16:00							0								0	0	
Hourly Total	1	23	0	2	1	0	27	1	20	0	6	1	0	28	55		
pcu/hr	0	7	0	4	3	0	14	0	6	0	12	3	0	21			
16:15							0		3						3	3	
16:30							0	1	14		1				16	16	
16:45							0								0	0	
17:00	2	16		5			23		12		9				21	44	
Hourly Total	2	16	0	5	0	0	23	1	29	0	10	0	0	40	63		
pcu/hr	0	5	0	10	0	0	15	0	9	0	20	0	0	29			
17:15							0								0	0	
17:30							0								0	0	
17:45	41	44		1			86								0	86	
18:00							0								0	0	
Hourly Total	41	44	0	1	0	0	86	0	0	0	0	0	0	0	0	86	
pcu/hr	8	13	0	2	0	0	23	0	0	0	0	0	0	0			
Total	81	297	8	56	3	0	445	65	236	10	43	3	0	357	802		
pcu/12hr	16	89	8	112	9	0	234	12	71	10	86	9	0	188			

2.2. Cat Hai (Center of Cat Hai road)

DATE	27/04/2010						Investigator name						Nguyễn Minh Tài					
LOCATION	Cat Hai														Total Movement			
Time	To Cat Ba Direction						Total	To Dinh Vu Direction						Total				
	1	2	3	4	5	6		1	2	3	4	5	6					
	0.2	0.3	1.0	2.0	2.5	4.0		0.2	0.3	1.0	2.0	2.5	4.0					
6:15	15	9					24	11	8					19	43			
6:30	11	8		1			20	18	6					24	44			
6:45	5	5	1	1			12	8	9					17	29			
7:00	5	4		1			10	4	5		6	1		16	26			
Hourly Total	36	26	1	3	0	0	66	41	28	0	6	1	0	76	142			
pcu/hr	7	8	1	6	0	0	22	8	8	0	12	3	0	31	53			
7:15	21	8					29	16	4					20	49			
7:30		14		7			21	2	5					7	28			
7:45	5	6		7			18	4	3					7	25			
8:00	7	5		4			16	5	5					10	26			
Hourly Total	33	33	0	18	0	0	84	27	17	0	0	0	0	44	128			
pcu/hr	7	10	0	36	0	0	53	5	5	0	0	0	0	10	63			
8:15	1	4					5	3	1					4	9			
8:30	2	1		1			4	1	5					6	10			
8:45	6	7		3			16	6	13					19	35			
9:00	4	7		6			17	6	4		1			11	28			
Hourly Total	13	19	0	10	0	0	42	16	23	0	1	0	0	40	82			
pcu/hr	3	6	0	20	0	0	29	3	7	0	2	0	0	12	41			
9:15	5	7		1	1		14	4	7		1			12	26			
9:30	6	2			1		9	3	5					8	17			
9:45	15	4		2			21	3	4					7	28			
10:00	9	16	3				28	1	6					7	35			
Hourly Total	35	29	3	3	2	0	72	11	22	0	1	0	0	34	106			
pcu/hr	7	9	3	6	5	0	30	2	7	0	2	0	0	11	41			
10:15	6	6		1			13	1	3	1				5	18			
10:30	5	6					11	4	1	1				6	17			
10:45		8					8		5					5	13			
11:00		5	5	5			15		3	1	1			5	20			
Hourly Total	11	25	5	6	0	0	47	5	12	3	1	0	0	21	68			
pcu/hr	2	8	5	12	0	0	27	1	4	3	2	0	0	10	37			
11:15	6	8					14	5	6		2	1		14	28			
11:30	2	1					3	2	1		1			4	7			
11:45	2	5	2				9		1					1	10			
12:00	4	1	1				6							0	6			
Hourly Total	12	16	4	0	0	0	32	7	8	0	3	1	0	19	51			
pcu/hr	2	5	4	0	0	0	11	1	2	0	6	3	0	12	23			
12:15	1	3					4	1	3			1		5	9			
12:30	4	4					8	1						1	9			
12:45	1	2					3	1	4					5	8			
13:00	4	9					13	12	11					23	36			
Hourly Total	10	18	0	0	0	0	28	15	18	0	0	1	0	34	62			
pcu/hr	2	5	0	0	0	0	7	3	5	0	0	3	0	11	18			
13:15	10	10		2			22	13	13		3			29	51			
13:30	4	10		3			17	14	5					19	36			
13:45	2	4		1			7	17	5					22	29			
14:00	3	7					10	2	6		1			9	19			
Hourly Total	19	31	0	6	0	0	56	46	29	0	4	0	0	79	135			
pcu/hr	4	9	0	12	0	0	25	9	9	0	8	0	0	26	51			
14:15	5	3					8	5	2	2				9	17			
14:30	5	4		1			10	6	1		1			8	18			
14:45	6	9					15	4	3					7	22			
15:00		8	1	3			12	5	6					11	23			
Hourly Total	16	24	1	4	0	0	45	20	12	2	1	0	0	35	80			
pcu/hr	3	7	1	8	0	0	19	4	4	2	2	0	0	12	31			
15:15	4	9	1	4	1		19	2	4		6	1		13	32			
15:30	5	1		1			7	6	6					12	19			
15:45	18	4			1		23	7	11		1			19	42			
16:00	14	6					20	1	2					3	23			
Hourly Total	41	20	1	5	2	0	69	16	23	0	7	1	0	47	116			
pcu/hr	8	6	1	10	5	0	30	3	7	0	14	3	0	27	57			
16:15	10	4		1			15	6	6					12	27			
16:30	24	8		1			33	8	10		1			19	52			
16:45	2	12		1			15	6	10		1			17	32			
17:00	1	3					4	1	7	3	2			13	17			
Hourly Total	37	27	0	3	0	0	67	21	33	3	4	0	0	61	128			
pcu/hr	7	8	0	6	0	0	21	4	10	3	8	0	0	25	46			
17:15	9	5		2			16	9	5					14	30			
17:30	14	14	1	3			32	5	8					13	45			
17:45	8	7					15	7	7					14	29			
18:00	2	10		1			13	4	9					13	26			
Hourly Total	33	36	1	6	0	0	76	25	29	0	0	0	0	54	130			
pcu/hr	7	11	1	12	0	0	31	5	9	0	0	0	0	14	45			
Total	296	304	16	64	4	0	684	250	254	8	28	4	0	544	1228			
pcu/12hr	59	92	16	128	10	0	305	48	77	8	56	12	0	201	506			

2.3. Ben GOT (Cat Ba side ferry terminal)

DATE	27/04/2010						Investigator name						Pham Đức Hoàn					
LOCATION	Ben Got																	
Time	Counted Vehicle Number														Total Movement			
	To Cat Ba Direction							To Dinh Vu Direction										
	1	2	3	4	5	6	Total	1	2	3	4	5	6	Total				
	0.2	0.3	1.0	2.0	2.5	4.0		0.2	0.3	1.0	2.0	2.5	4.0					
6:15	13	4					17	28	9					37				
6:30	2	16					18	4						4				
6:45	1	6					7	2	2					4				
7:00	1	4					5	7	12	1	6	1		27				
Hourly Total	17	30	0	0	0	0	47	41	23	1	6	1	0	72				
	3	9	0	0	0	0	12	8	7	1	12	3	0	31				
7:15	5	10					15	3	11	1				15				
7:30		9					9	1	3					4				
7:45	2	12		10			24		2					2				
8:00		10	2	2			14							0				
Hourly Total	7	41	2	12	0	0	62	4	16	1	0	0	0	21				
	1	12	2	24	0	0	39	1	5	1	0	0	0	7				
8:15	1	19					20	3	21	1				25				
8:30		5					5	1	1					2				
8:45	3	2					5	2	4					6				
9:00		2		4			6		2					2				
Hourly Total	4	28	0	4	0	0	36	6	28	1	0	0	0	35				
	1	8	0	8	0	0	17	1	8	1	0	0	0	10				
9:15	1	3					4		2					2				
9:30	1	8			1		10	2	21					23				
9:45		1					1							0				
10:00	13	16	1				30	3	3					6				
Hourly Total	15	28	1	0	1	0	45	5	26	0	0	0	0	31				
	3	8	1	0	3	0	15	1	8	0	0	0	0	9				
10:15	4						4	1	8	2				11				
10:30	1	1					2		3					3				
10:45	1	2					3		1					1				
11:00	1	7		2			10		3					3				
Hourly Total	7	10	0	2	0	0	19	1	15	2	0	0	0	18				
	1	3	0	4	0	0	8	0	5	2	0	0	0	7				
11:15		1		5			6	3	16	2	2	5		28				
11:30		2					2	1						1				
11:45		4		1			5		9		1			10				
12:00		7	4				11							0				
Hourly Total	0	14	4	6	0	0	24	4	25	2	3	5	0	39				
	0	4	4	12	0	0	20	1	8	2	6	13	0	30				
12:15							0							0				
12:30	3	1					4							4				
12:45	1	4					5							0				
13:00							0							0				
Hourly Total	4	5	0	0	0	0	9	0	0	0	0	0	0	9				
	1	2	0	0	0	0	3	0	0	0	0	0	0	3				
13:15	1	2					3	14	9		6			29				
13:30	2	6					8	2	9	1				12				
13:45		2		2			4	1						1				
14:00	1	9		1			11	1	9	1				11				
Hourly Total	4	19	0	3	0	0	26	18	27	2	6	0	0	53				
	1	6	0	6	0	0	13	4	8	2	12	0	0	26				
14:15	1	4					5		1					1				
14:30	2						2	1	4	2				7				
14:45	3	1					4	2	2					4				
15:00	2	5					7	5	2					7				
Hourly Total	8	10	0	0	0	0	18	8	9	2	0	0	0	19				
	2	3	0	0	0	0	5	2	3	2	0	0	0	7				
15:15		5		6	2		13	2	15		6	1		24				
15:30	1	7					8							0				
15:45	1	14		1	1		17							0				
16:00	7	8					15							0				
Hourly Total	9	34	0	7	3	0	53	2	15	0	6	1	0	24				
	2	10	0	14	8	0	34	0	5	0	12	3	0	20				
16:15	2	2					4							0				
16:30	5	3					8		4					4				
16:45	5	1					6		1					1				
17:00	8	6					14	2	25	3	3			33				
Hourly Total	20	12	0	0	0	0	32	2	30	3	3	0	0	38				
	4	4	0	0	0	0	8	0	9	3	6	0	0	18				
17:15	1	6		3			12	1	1					2				
17:30				1			1							0				
17:45		3					3	5	23					28				
18:00	3	4		1			8							0				
Hourly Total	4	13	2	5	0	0	24	6	24	0	0	0	0	30				
pcu/hr	1	4	2	10	0	0	17	1	7	0	0	0	0	8				
Total	99	244	9	39	4	0	395	97	238	14	24	7	0	380				
pcu/12hr	20	73	9	78	11	0	191	19	73	14	48	19	0	173				

3. Speed Survey

Time	From Ninh Tiep to Cat Hai			From Cat Hai to Ninh Tiep		
	Departure time	Arrival time	Time required	Departure time	Arrival time	Time required
7:00	7h40'	8h01'	21'	7h11'	7h32'	21'
9:00	8h35'	8h54'	19'	8h05'	8h27'	22'
11:00	11h05'	11h25'	20'	11h40'	11h59'	19'
13:00	13h45'	14h06'	21'	14h20'	14h42'	22'
15:00	15h10'	15h30'	20'	16h15'	16h33'	18'
17:00	17h05'	17h26'	21'	17h35'	17h55'	20'

4. Ferry Track Record Survey

4.1. Summary of Dinh Vu-Cat Hai Ferry Track Record Survey

Year	Month	Type of vehicle						Total	pcu/month
		From Dinh Vu to Cat Hai and Cat Hai to Dinh Vu							
		1 0.2	2 0.3	3 1.0	4 2.0	5 2.5	6 4.0		
2002	Jan.								
	Feb.								
	Mar.								
	Apr.								
	May	1,349	5,601	328	628	317		8,223	4,327
	Jun	4,173	15,827	1,365	2,242	1,373		24,980	14,864
	Jul	4,262	16,510	1,356	2,290	1,088		25,506	14,461
	Aug	4,180	16,520	821	1,500	485		23,506	10,826
	Sep	3,962	14,782	498	1,020	252		20,514	8,395
	Oct	3,728	12,467	506	1,145	226		18,072	7,847
	Nov	3,738	11,168	324	1,077	199		16,506	7,074
	Dec	3,030	10,696	294	1,171	217		15,408	6,993
	Total	28,422	103,571	5,492	11,073	4,157	-	152,715	74,787
	Total pcu/year	5,684	31,071	5,492	22,146	10,393	-	74,786	
Average pcu/month	711	3,884	687	2,768	1,299	-	9,349		
pcu/day	24	129	23	92	43	-	311		
2003	Jan.	4,239	12,280	281	1,050	222		18,072	7,468
	Feb.	2,436	8,615	323	710	138		12,222	5,160
	Mar.	2,949	8,081	400	1,212	210		12,852	6,363
	Apr.	3,405	9,626	513	1,115	190		14,849	6,787
	May	2,728	10,288	661	1,281	322		15,280	7,660
	Jun	2,740	10,694	1,150	1,973	828		17,385	10,922
	Jul	2,098	9,858	1,049	1,966	698		15,669	10,103
	Aug	2,798	10,239	856	1,707	486		16,086	9,116
	Sep	2,689	9,744	422	966	192		14,013	6,295
	Oct	2,705	8,723	481	1,015	172		13,096	6,099
	Nov	3,049	9,796	381	821	192		14,239	6,052
	Dec	2,782	8,863	413	832	141		13,031	5,645
	Total	34,618	116,807	6,930	14,648	3,791	-	176,794	87,670
	Total pcu/year	6,924	35,042	6,930	29,296	9,478	-	87,670	
Average pcu/month	866	4,380	866	3,662	1,185	-	10,959		
pcu/day	29	146	29	122	40	-	366		
2004	Jan.	3,060	10,678	360	674	122		14,894	5,828
	Feb.	2,595	7,335	508	779	136		11,353	5,126
	Mar.	2,634	7,088	706	1,013	193		11,634	5,868
	Apr.	3,157	11,500	628	1,133	202		16,620	7,480
	May	2,760	14,156	987	1,487	357		19,747	9,652
	Jun	2,488	10,841	1,194	2,197	956		17,676	11,728
	Jul	2,362	11,498	1,365	2,675	1,004		18,904	13,147
	Aug	2,406	10,586	906	2,244	695		16,837	10,789
	Sep	2,276	8,714	565	1,038	296		12,889	6,450
	Oct	2,249	8,904	479	947	232		12,811	6,074
	Nov	2,279	7,524	331	838	313		11,285	5,503
	Dec	1,950	7,454	447	974	222		11,047	5,576
	Total	30,216	116,278	8,476	15,999	4,728	-	175,697	93,221
	Total pcu/year	6,043	34,883	8,476	31,998	11,820	-	93,220	
Average pcu/month	755	4,360	1,060	4,000	1,478	-	11,653		
pcu/day	25	145	35	133	49	-	387		

Year	Month	Type of vehicle							pcu/month
		From Dinh Vu to Cat Hai and Cat Hai to Dinh Vu							
		1	2	3	4	5	6	Total	
		0.2	0.3	1.0	2.0	2.5	4.0		
2005	Jan.	1,968	6,794	456	1,003	199		10,420	5,391
	Feb.	1,888	10,081	500	658	170		13,297	5,643
	Mar.	2,026	8,409	709	1,189	268		12,601	6,685
	Apr.	1,575	10,586	824	1,362	277		14,624	7,731
	May	1,591	11,449	1,074	1,698	559		16,371	9,620
	Jun.	1,401	11,002	1,599	2,584	1,433		18,019	13,930
	Jul.	1,851	10,996	1,678	3,009	1,455		18,989	15,003
	Aug.	2,362	12,204	707	1,548	629		17,450	9,509
	Sep.	1,990	10,082	520	1,156	304		14,052	7,015
	Oct.	1,986	9,212	398	1,053	279		12,928	6,362
	Nov.	1,767	8,598	319	999	227		11,910	5,817
	Dec.	1,797	7,645	372	950	188		10,952	5,395
	Total	22,202	117,058	9,156	17,209	5,988	-	171,613	98,101
	Total pcu/year	4,440	35,117	9,156	34,418	14,970	-	98,101	
	Average pcu/month	555	4,390	1,145	4,302	1,871	-	12,263	
pcu/day	19	146	38	143	62	-	408		
2006	Jan.	2,079	10,173	412	908	172		13,744	6,126
	Feb.	1,429	9,604	401	730	183		12,347	5,486
	Mar.	1,087	7,453	525	1,074	264		10,403	5,786
	Apr.	1,308	9,356	767	1,617	319		13,367	7,867
	May	1,654	9,778	870	1,768	565		14,635	9,083
	Jun.	1,701	8,847	1,128	2,963	1,809		16,448	14,571
	Jul.	1,645	9,413	1,303	3,423	1,562		17,346	15,207
	Aug.	1,355	6,531	607	1,503	631		10,627	7,421
	Sep.	1,508	7,547	606	1,266	418		11,345	6,749
	Oct.	1,484	6,834	477	1,137	335		10,267	5,936
	Nov.	1,305	7,224	390	1,205	256		10,380	5,868
	Dec.	1,621	8,273	540	1,301	282		12,017	6,653
	Total	18,176	101,033	8,026	18,895	6,796	-	152,926	96,753
	Total pcu/year	3,635	30,310	8,026	37,790	16,990	-	96,751	
	Average pcu/month	454	3,789	1,003	4,724	2,124	-	12,094	
pcu/day	15	126	33	157	71	-	402		
2007	Jan.	1,603	6,708	400	1,047	189		9,947	5,300
	Feb.	1,161	9,180	417	699	152		11,609	5,181
	Mar.	1,034	6,511	440	1,074	204		9,263	5,258
	Apr.	1,238	9,412	892	1,807	389		13,738	8,550
	May	1,101	8,032	655	1,639	539		11,966	7,910
	Jun.	1,138	8,920	1,345	3,348	1,880		16,631	15,645
	Jul.	1,814	8,929	1,458	3,972	1,741		17,914	16,796
	Aug.	1,282	7,439	875	2,114	763		12,473	9,499
	Sep.	1,167	6,984	610	1,577	362		10,700	6,998
	Oct.	1,075	6,211	382	1,224	221		9,113	5,461
	Nov.	1,042	6,731	443	1,253	216		9,685	5,717
	Dec.	1,121	7,443	405	1,236	224		10,429	5,894
	Total	14,776	92,500	8,322	20,990	6,880	-	143,468	98,209
	Total pcu/year	2,955	27,750	8,322	41,980	17,200	-	98,207	
	Average pcu/month	369	3,469	1,040	5,248	2,150	-	12,276	
pcu/day	12	116	35	175	72	-	410		

Year	Month	Type of vehicle						Total	pcu/month
		From Dinh Vu to Cat Hai and Cat Hai to Dinh Vu							
		1 0.2	2 0.3	3 1.0	4 2.0	5 2.5	6 4.0		
2008	Jan.	960	5,714	408	1,205	210		8,497	5,249
	Feb	814	9,073	380	950	177		11,394	5,607
	Mar	1,092	7,380	455	1,387	293		10,607	6,394
	Apr	766	8,553	594	1,786	323		12,022	7,693
	May	1,102	9,464	796	2,210	630		14,202	9,851
	Jun	1,255	8,631	1,052	3,330	1,458		15,726	14,197
	Jul	1,294	11,020	1,538	4,096	1,574		19,522	17,230
	Aug	1,196	8,619	718	2,049	638		13,220	9,236
	Sep	1,157	7,895	468	1,297	299		11,116	6,409
	Oct	1,571	7,284	300	1,286	244		10,685	5,981
	Nov	1,821	8,005	338	1,228	204		11,596	6,070
	Dec	1,750	7,932	347	1,179	196		11,404	5,925
	Total	14,778	99,570	7,394	22,003	6,246	-	149,991	99,842
	Total pcu/year	2,956	29,871	7,394	44,006	15,615	-	99,842	
Average pcu/month	370	3,734	924	5,501	1,952	-	12,481		
pcu/day	12	124	31	183	65	-	415		
2009	Jan.	996	10,604	415	978	184		13,177	6,211
	Feb	675	7,079	268	925	166		9,113	4,792
	Mar	772	7,940	490	1,406	242		10,850	6,443
	Apr	734	9,218	592	1,700	345		12,589	7,767
	May	859	9,934	774	2,108	705		14,380	9,905
	Jun	1,086	9,714	1,197	3,461	1,337		16,795	14,593
	Jul	838	10,545	1,366	3,912	1,201		17,862	15,524
	Aug	869	8,418	634	2,109	504		12,534	8,811
	Sep	1,238	8,282	379	1,288	300		11,487	6,437
	Oct	1,678	7,458	376	1,305	244		11,061	6,169
	Nov	2,007	7,198	400	1,208	215		11,028	5,914
	Dec	1,446	7,044	369	1,228	213		10,300	5,760
	Total	13,198	103,434	7,260	21,628	5,656	-	151,176	98,326
	Total pcu/year	2,640	31,030	7,260	43,256	14,140	-	98,326	
Average pcu/month	330	3,879	908	5,407	1,768	-	12,292		
pcu/day	11	129	30	180	59	-	409		

4.2. Summary of Cat Hai – Cat Ba Ferry Track Record Survey

Year	Month	Type of vehicle							pcu/month
		From Cat Hai to Cat Ba and Cat Ba Cat Hai							
		1	2	3	4	5	6	Total	
		0.2	0.3	1.0	2.0	2.5	4.0		
2002	Jan.								
	Feb.								
	Mar.								
	Apr.								
	May	936	4,058	355	610	341		6,300	3,832
	Jun	2,172	10,875	1,118	1,893	1,263		17,321	11,758
	Jul	2,537	10,415	1,022	1,861	1,013		16,848	10,908
	Aug	2,192	9,985	553	1,304	508		14,542	7,865
	Sep	1,661	8,228	270	844	231		11,234	5,336
	Oct	1,180	5,936	283	911	215		8,525	4,659
	Nov	1,142	4,941	208	780	209		7,280	4,001
	Dec	1,528	5,338	156	885	229		8,136	4,406
	Total	13,348	59,776	3,965	9,088	4,009	-	90,186	52,765
	Total pcu/year	2,670	17,933	3,965	18,176	10,023	-	52,767	
Average pcu/month	334	2,242	496	2,272	1,253	-	6,597		
pcu/day	11	75	17	76	42	-	221		
2003	Jan.	2,000	6,651	149	837	236		9,873	4,808
	Feb.	646	3,944	228	586	129		5,533	3,035
	Mar.	463	3,606	259	955	198		5,481	3,838
	Apr.	307	3,512	363	845	227		5,254	3,736
	May	133	2,700	549	957	320		4,659	4,100
	Jun	83	1,913	1,042	1,619	834		5,491	6,956
	Jul	37	1,279	843	1,489	616		4,264	5,752
	Aug	44	1,129	639	1,240	386		3,438	4,432
	Sep	35	621	268	686	121		1,731	2,136
	Oct	13	426	314	674	120		1,547	2,092
	Nov	16	417	209	522	187		1,351	1,849
	Dec	22	351	215	527	125		1,240	1,691
	Total	3,799	26,549	5,078	10,937	3,499	-	49,862	44,425
	Total pcu/year	760	7,965	5,078	21,874	8,748	-	44,425	
Average pcu/month	95	996	635	2,734	1,094	-	5,554		
pcu/day	3	33	21	91	36	-	184		
2004	Jan.	26	346	151	439	117		1,079	1,431
	Feb.	12	287	232	516	132		1,179	1,683
	Mar.	156	299	314	759	189		1,717	2,425
	Apr.	113	1,180	443	800	184		2,720	2,880
	May	40	2,892	810	1,149	367		5,258	4,901
	Jun	37	1,477	917	1,778	907		5,116	7,191
	Jul	1,074	7,671	1,144	2,221	942		13,052	10,457
	Aug	1,171	6,468	679	1,589	592		10,499	7,512
	Sep	966	5,463	375	597	275		7,676	4,089
	Oct	754	5,684	268	578	238		7,522	3,875
	Nov	780	4,480	201	481	206		6,148	3,178
	Dec	980	4,234	237	462	184		6,097	3,087
	Total	6,109	40,481	5,771	11,369	4,333	-	68,063	52,709
	Total pcu/year	1,222	12,144	5,771	22,738	10,833	-	52,708	
Average pcu/month	153	1,518	721	2,842	1,354	-	6,588		
pcu/day	5	51	24	95	45	-	220		

Year	Month	Type of vehicle							pcu/month
		From Cat Hai to Cat Ba and Cat Ba Cat Hai							
		1	2	3	4	5	6	Total	
	0.2	0.3	1.0	2.0	2.5	4.0			
2005	Jan.	908	3,789	219	482	153		5,551	2,884
	Feb	1,041	5,428	201	327	141		7,138	3,044
	Mar	967	5,404	343	633	196		7,543	3,914
	Apr	976	7,070	508	856	242		9,652	5,141
	May	943	7,734	790	1,174	469		11,110	6,819
	Jun	1,031	7,226	1,065	1,948	1,158		12,428	10,230
	Jul	751	8,920	1,261	2,532	1,272		14,736	12,331
	Aug	1,071	9,058	558	1,233	597		12,517	7,448
	Sep	993	7,653	339	846	277		10,108	5,218
	Oct	928	7,110	214	679	228		9,159	4,461
	Nov	1,041	6,849	172	610	207		8,879	4,172
	Dec	1,109	5,610	162	639	175		7,695	3,782
	Total	11,759	81,851	5,832	11,959	5,115	-	116,516	69,444
	Total pcu/year	2,352	24,555	5,832	23,918	12,788	-	69,445	
Average pcu/month	294	3,069	729	2,990	1,599	-	8,681		
pcu/day	10	102	24	100	53	-	289		
2006	Jan.	1,273	6,812	193	536	160		8,974	3,963
	Feb	941	6,990	185	484	171		8,771	3,866
	Mar	776	5,543	269	770	230		7,588	4,202
	Apr	596	6,413	468	1,158	294		8,929	5,562
	May	895	6,304	538	1,297	492		9,526	6,432
	Jun	893	5,559	851	2,407	1,669		11,379	11,684
	Jul	852	5,762	1,009	2,814	1,413		11,850	12,069
	Aug	639	3,516	346	1,085	569		6,155	5,121
	Sep	544	4,389	382	826	346		6,487	4,325
	Oct	336	4,546	260	644	292		6,078	3,709
	Nov	450	4,412	178	573	226		5,839	3,303
	Dec	480	4,733	167	580	224		6,184	3,403
	Total	8,675	64,979	4,846	13,174	6,086	-	97,760	67,639
	Total pcu/year	1,735	19,494	4,846	26,348	15,215	-	67,638	
Average pcu/month	217	2,437	606	3,294	1,902	-	8,456		
pcu/day	7	81	20	110	63	-	281		
2007	Jan.	207	3,615	156	490	97		4,565	2,504
	Feb	328	5,140	135	330	23		5,956	2,460
	Mar	324	3,857	164	507	83		4,935	2,607
	Apr	210	5,119	527	1,119	296		7,271	5,083
	May	281	4,234	347	897	410		6,169	4,492
	Jun	557	6,418	1,169	3,090	1,969		13,203	14,308
	Jul	363	5,577	1,256	3,697	1,868		12,761	15,066
	Aug	138	4,366	811	2,019	854		8,188	8,321
	Sep	120	3,778	390	1,039	189		5,516	4,098
	Oct	132	3,086	200	697	184		4,299	3,006
	Nov	198	3,599	237	775	178		4,987	3,351
	Dec	103	3,503	201	732	198		4,737	3,232
	Total	2,961	52,292	5,593	15,392	6,349	-	82,587	68,528
	Total pcu/year	592	15,688	5,593	30,784	15,873	-	68,530	
Average pcu/month	74	1,961	699	3,848	1,984	-	8,566		
pcu/day	2	65	23	128	66	-	284		

Year	Month	Type of vehicle							pcu/month
		From Cat Hai to Cat Ba and Cat Ba Cat Hai							
		1	2	3	4	5	6	Total	
	0.2	0.3	1.0	2.0	2.5	4.0			
2008	Jan.	152	2,682	171	559	179		3,743	2,572
	Feb.	44	4,610	155	454	166		5,429	2,870
	Mar.	172	4,344	331	919	240		6,006	4,107
	Apr.	87	4,768	463	1,324	349		6,991	5,431
	May	52	4,933	683	1,526	532		7,726	6,555
	Jun.	40	4,751	1,113	2,888	1,441		10,233	11,925
	Jul.	46	6,019	1,488	3,736	1,586		12,875	14,740
	Aug.	42	4,615	657	1,716	692		7,722	7,212
	Sep.	20	4,431	337	1,039	275		6,102	4,436
	Oct.	22	4,244	256	994	232		5,748	4,102
	Nov.	67	4,603	242	859	183		5,954	3,812
	Dec.	73	4,325	236	870	192		5,696	3,768
	Total	817	54,325	6,132	16,884	6,067	-	84,225	71,530
	Total pcu/year	163	16,298	6,132	33,768	15,168	-	71,529	
Average pcu/month	20	2,037	767	4,221	1,896	-	8,941		
pcu/day	1	68	26	141	63	-	299		
2009	Jan.	23	4,663	231	698	193		5,808	3,513
	Feb.	45	4,856	268	774	172		6,115	3,712
	Mar.	42	4,937	397	1,213	244		6,833	4,923
	Apr.	53	5,136	551	1,527	363		7,630	6,064
	May	47	6,163	796	1,991	742		9,739	8,491
	Jun.	36	5,909	1,144	3,256	1,438		11,783	13,031
	Jul.	62	6,238	1,325	3,562	1,290		12,477	13,558
	Aug.	19	5,138	611	1,825	567		8,160	7,224
	Sep.	23	4,236	325	1,038	303		5,925	4,434
	Oct.	5	3,885	276	932	235		5,333	3,894
	Nov.	1,231	4,128	223	883	189		6,654	3,946
	Dec.	1,184	4,098	249	888	164		6,583	3,901
	Total	2,770	59,387	6,396	18,587	5,900	-	93,040	76,691
	Total pcu/year	554	17,816	6,396	37,174	14,750	-	76,690	
Average pcu/month	69	2,227	800	4,647	1,844	-	9,587		
pcu/day	2	74	27	155	61	-	319		

4.4. Ferry Track Record Survey Data (Cat Hai – Cat Ba Ferry)

Year 2002																	
Order number	Type of vehicle	Unit	amount												Total	Total PCU	Average PCU/Mon.
			Month														
			Jan.	Feb	Mar	Apr	May	Jun	Jul	Aug	Sep	Oct	Nov	Dec			
1	Passenger						14,342	47,522	44,075	25,636	13,739	9,178	7,203	7,660	169,355		
2	Bicycle						935	2,168	2,535	2,192	1,661	1,180	1,142	1,528	13,341	2,668	334
3	Bicycle with goods																
4	Motorcycle						4,058	10,875	10,415	9,985	8,228	5,936	4,941	5,338	59,776	17,933	2,242
5	Cyclo						1	4	2						7	2	0
6	Car 4-6 seats						355	1,058	918	489	258	254	159	146	3,637	3,637	455
7	Bus						728	2,616	2,327	1,206	560	468	317	369	8,591	19,085	2,386
	< 9 seats						76	274	325	210	157	116	72	66	1,296	2,592	324
	9-24 seats						321	1,124	1,009	521	181	157	77	101	3,491	6,982	873
	24-32 seats						201	852	692	359	173	148	106	132	2,663	6,658	832
	< 32 seats						130	366	301	116	49	47	62	70	1,141	2,853	357
8	Truck						223	323	315	426	448	553	582	681	3,551	7,148	894
	< 1 ton																
	with goods						16	15	12	18	14	17	19	26	137	274	34
	non goods						16	26	16	20	26	20	25	33	182	364	46
	1 ton-3 ton																
	with goods						68	108	98	150	148	173	165	206	1,116	2,232	279
	non goods						64	74	70	105	116	151	141	184	905	1,810	226
	3 ton-5 ton																
	with goods						8	20	31	35	29	53	39	62	277	554	69
	non goods						11	23	26	26	31	38	34	57	246	492	62
	5 ton-10 ton																
	with goods						10	27	32	34	42	57	69	52	323	646	81
	non goods						20	21	24	36	35	39	55	44	274	548	69
	10 ton-13 ton																
	with goods						5	4	5	2	5	3	31	15	70	175	22
	non goods						5	5	1		2	2	4	2	21	53	7
9	Livestock																
10	Goods more 61 kg																
	Total						6,300	17,044	16,512	14,298	11,155	8,391	7,141	8,062	88,903	50,473	6,309

Year 2003																	
Order number	Type of vehicle	Unit	amount												Total	Total PCU	Average PCU/Mon.
			Month														
			Jan.	Feb	Mar	Apr	May	Jun	Jul	Aug	Sep	Oct	Nov	Dec			
1	Passenger		8,648	7,917	7,675	8,887	10,287	23,187	17,977	13,617	4,028	2,673	2,144	1,660	108,700		
2	Bicycle		1,998	646	463	307	133	83	37	44	35	13	16	22	3,797	759	63
3	Bicycle with goods																
4	Motorcycle		6,651	3,944	3,606	3,512	2,700	1,913	1,279	1,129	621	426	417	351	26,549	7,965	664
5	Cyclo		2												2	1	0
6	Car 4-6 seats		121	200	249	352	527	1,004	799	600	256	304	202	195	4,809	4,809	401
7	Bus		300	230	431	507	718	2,008	1,517	1,179	384	302	303	253	8,132	17,830	1,486
	< 9 seats		49	39	124	118	113	278	177	247	95	80	52	50	1,422	2,844	237
	9-24 seats		70	79	146	191	317	915	778	567	181	132	100	103	3,579	7,158	597
	24-32 seats		114	44	72	109	176	585	414	300	70	41	48	22	1,995	4,988	416
	< 32 seats		67	68	89	89	112	230	148	65	38	49	103	78	1,136	2,840	237
8	Truck		625	357	674	533	506	375	307	343	374	434	375	345	5,248	10,636	886
	< 1 ton																
	with goods		20	20	39	34	39	25	24	22	21	22	7	16	289	578	48
	non goods		21	18	52	43	43	46	23	39	15	30	11	18	359	718	60
	1 ton-3 ton																
	with goods		177	95	191	150	125	95	84	81	93	101	94	89	1,375	2,750	229
	non goods		149	78	135	142	113	112	65	52	108	108	94	70	1,226	2,452	204
	3 ton-5 ton																
	with goods		48	32	73	48	54	27	29	34	28	43	41	51	508	1,016	85
	non goods		42	24	61	37	39	18	16	29	31	37	47	41	422	844	70
	5 ton-10 ton																
	with goods		57	33	43	28	31	23	33	40	29	29	22	19	387	774	65
	non goods		56	42	43	22	30	21	32	37	39	38	24	20	404	808	67
	10 ton-13 ton																
	with goods		43	7	25	14	12	6	1	4	5	15	30	13	175	438	37
	non goods		12	8	12	15	20	2		5	5	11	5	8	103	258	22
9	Livestock																
10	Goods more 61 kg									1					1		
	Total		9,697	5,377	5,423	5,211	4,584	5,383	3,939	3,295	1,670	1,479	1,313	1,166	48,537	42,000	3,500

4.5. The total number of vehicles priority (Dinh Vu-Cat Hai Ferry)

Year	Month	Type of vehicle															
		Total	Car 4 seats	Car <9 seats	Bus 9<24	Bus 24<32	Bus more 32 seats	Truck <1 ton with good	Truck <1 ton non good	Truck 1 -3 ton with good	Truck 1 -3 ton non good	Truck 3 -5 ton with good	Truck 3 -5 ton non good	Truck 5 -10 ton with good	Truck 5 -10 ton non good	Truck 10 -13 ton with good	Truck 10 -13 ton non good
2002	Jan.																
	Feb																
	Mar																
	Apr																
	May																
	Jun	467	266	36	129	32	2	0	0	2	0	0	0	0	0	0	0
	Jul	636	336	30	248	22	0	0	0	0	0	0	0	0	0	0	0
	Aug	444	296	28	104	16	0	0	0	0	0	0	0	0	0	0	0
	Sep	251	135	9	104	3	0	0	0	0	0	0	0	0	0	0	0
	Oct	401	203	46	142	10	0	0	0	0	0	0	0	0	0	0	0
	Nov	255	111	42	100	2	0	0	0	0	0	0	0	0	0	0	0
	Dec	235	63	46	122	4	0	0	0	0	0	0	0	0	0	0	0
	Total	2689	1410	237	949	89	2	0	0	2	0	0	0	0	0	0	0
Ave./Mon	384.1	201.4	33.9	135.6	12.7	0.3	-	-	0.3	-	-	-	-	-	-	-	
2003	Jan.	151	56	22	69	4	0	0	0	0	0	0	0	0	0	0	
	Feb	185	72	23	86	4	0	0	0	0	0	0	0	0	0	0	
	Mar	196	70	28	92	2	4	0	0	0	0	0	0	0	0	0	
	Apr	226	88	14	122	2	0	0	0	0	0	0	0	0	0	0	
	May	230	80	22	120	8	0	0	0	0	0	0	0	0	0	0	
	Jun	387	164	32	189	2	0	0	0	0	0	0	0	0	0	0	
	Jul	597	228	66	231	50	2	0	0	8	0	10	0	4	0	0	
	Aug	395	222	46	117	8	0	0	0	0	0	0	0	0	0	0	
	Sep	239	142	15	78	3	2	0	0	1	0	0	0	0	0	0	
	Oct	219	143	30	42	2	0	0	0	0	0	0	0	0	0	0	
	Nov	143	109	4	28	0	0	0	0	1	1	0	0	0	0	0	
	Dec	160	120	12	24	4	0	0	0	0	0	0	0	0	0	0	
	Total	3128	1494	314	1198	89	8	0	0	10	1	10	0	4	0	0	
Ave./Mon	260.7	124.5	26.2	99.8	7.4	0.7	-	-	0.8	0.1	0.8	-	0.3	-	-		
2004	Jan.	98	80	4	9	0	0	0	0	1	2	1	0	0	0	0	
	Feb	206	168	13	21	2	2	0	0	0	0	0	0	0	0	0	
	Mar	553	348	60	54	24	21	6	1	15	3	11	4	3	1	1	
	Apr	175	151	4	14	4	2	0	0	0	0	0	0	0	0	0	
	May	196	140	2	36	8	4	0	0	3	1	0	0	1	1	0	
	Jun	350	230	44	50	16	4	0	0	4	0	0	0	1	1	0	
	Jul	368	187	26	113	23	11	0	0	4	0	2	0	2	0	0	
	Aug	368	178	52	81	32	7	0	0	2	0	2	0	14	0	0	
	Sep	153	107	16	22	2	2	0	0	0	0	3	0	1	0	0	
	Oct	123	90	14	18	0	0	0	0	1	0	0	0	0	0	0	
	Nov	41	29	8	4	0	0	0	0	0	0	0	0	0	0	0	
	Dec	101	68	14	17	0	0	0	0	1	0	1	0	0	0	0	
	Total	2732	1776	257	439	111	53	6	1	30	5	21	5	22	3	1	
Ave./Mon	260.7	124.5	26.2	99.8	7.4	0.7	-	-	0.8	0.1	0.8	-	0.3	-	-		
2005	Jan.	150	97	19	28	0	0	0	2	0	1	0	3	0	0	0	
	Feb	178	141	11	17	2	3	0	0	0	0	0	4	0	0	0	
	Mar	238	146	29	37	9	2	0	0	2	2	2	2	4	3	0	
	Apr	195	128	30	27	1	0	0	0	1	0	2	2	1	3	0	
	May	220	95	28	67	9	6	0	0	1	0	5	4	3	2	0	
	Jun	420	251	45	77	14	21	0	0	2	0	3	1	3	3	0	
	Jul	423	204	61	87	48	16	0	0	3	0	0	0	4	0	0	
	Aug	155	98	29	26	0	0	0	0	0	0	0	0	2	0	0	
	Sep	135	89	19	22	2	1	0	0	1	1	0	0	0	0	0	
	Oct	129	85	20	15	0	0	0	0	3	2	0	0	2	2	0	
	Nov	81	33	19	20	0	0	0	0	0	0	1	1	5	2	0	
	Dec	110	68	12	16	0	1	0	0	2	0	5	2	2	2	0	
	Total	2434	1435	322	439	85	50	0	0	17	5	19	12	33	17	0	
Ave./Mon	202.8	119.6	26.8	36.6	7.1	4.2	-	-	1.4	0.4	1.6	1.0	2.8	1.4	-		
2006	Jan.	109	89	2	13	1	0	0	0	0	3	1	0	0	0	0	
	Feb	83	59	8	12	2	0	0	0	0	1	1	0	0	0	0	
	Mar	134	87	27	18	0	0	0	0	0	1	0	1	0	0	0	
	Apr	149	104	23	12	1	0	0	0	3	1	2	2	0	1	0	
	May	259	133	52	51	8	3	0	0	0	0	5	3	3	1	0	
	Jun	301	147	34	62	23	11	0	0	0	0	0	1	14	9	0	
	Jul	180	81	35	46	4	0	0	0	1	2	0	0	5	6	0	
	Aug	231	145	29	34	11	0	0	0	4	2	3	3	0	0	0	
	Sep	178	111	23	32	8	0	0	0	0	0	2	1	1	0	0	
	Oct	183	119	37	23	0	0	0	0	0	0	0	0	4	0	0	
	Nov	172	69	56	33	9	1	0	0	1	1	0	0	1	1	0	
	Dec	271	174	40	44	5	0	0	1	2	3	0	0	1	1	0	
	Total	2250	1318	366	380	72	15	0	1	11	9	17	12	30	19	0	
Ave./Mon	187.5	109.8	30.5	31.7	6.0	1.3	-	0.1	0.9	0.8	1.4	1.0	2.5	1.6	-		

Year	Month	Type of vehicle																
		Total	Car 4 seats	Car <9 seats	Bus 9-24	Bus 24<32	Bus more 32 seats	Truck <1 ton		Truck 1 -3 ton		Truck 3 -5 ton		Truck 5 -10 ton		Truck 10 -13 ton		
							with good	non goods	with good	non goods	with good	non goods	with good	non goods	with good	non goods		
2007	Jan.	150	91	32	17	2	0	3	3	0	0	0	0	0	1	1	0	0
	Feb	151	97	22	24	2	0	0	0	2	2	1	1	0	0	0	0	0
	Mar	161	93	28	24	4	3	0	0	3	2	0	0	2	2	0	0	0
	Apr	221	130	35	35	11	6	0	0	1	1	1	1	0	0	0	0	0
	May	185	107	34	29	5	0	1	0	4	3	1	1	0	0	0	0	0
	Jun	255	103	57	67	21	5	0	0	1	1	0	0	0	0	0	0	0
	Jul	287	106	68	78	14	11	0	0	4	5	0	0	1	0	0	0	0
	Aug	199	82	40	58	9	5	0	0	5	0	0	0	0	0	0	0	0
	Sep	273	94	85	60	5	0	0	0	22	0	2	0	5	0	0	0	0
	Oct	152	51	46	35	5	0	0	0	0	0	4	5	2	0	4	0	0
	Nov	137	48	40	36	5	0	0	0	0	0	4	0	4	0	0	0	0
	Dec	152	70	43	23		0	0	0	1	0	13	0	0	0	2	0	0
	Total	2323	1072	530	486	83	30	4	3	43	14	26	8	15	3	6	0	0
	Ave./Mon	193.6	89.3	44.2	40.5	7.5	2.5	0.3	0.3	3.6	1.2	2.2	0.7	1.3	0.3	0.5	-	-
2008	Jan.	158	66	32	45	8	0	0	2	0	5	0	0	0	0	0	0	
	Feb	154	63	34	41	9	0	0	0	0	7	0	0	0	0	0	0	
	Mar	178	61	39	50	7	4	0	5	0	8	0	4	0	0	0	0	
	Apr	148	65	25	40	8	0	0	6	0	1	0	3	0	0	0	0	
	May	146	56	29	42	9	9	0	0	0	1	0	0	0	0	0	0	
	Jun	143	54	23	49	14	2	0	1	0	0	0	0	0	0	0	0	
	Jul	181	58	41	43	18	13	0	5	0	3	0	0	0	0	0	0	
	Aug	154	63	34	41	9	0	0	0	0	7	0	0	0	0	0	0	
	Sep	179	61	39	50	8	4	0	5	0	8	0	4	0	0	0	0	
	Oct	100	22	29	27	13	0	0	5	0	0	0	2	2	0	0	0	
	Nov	135	32	37	40	10	2	0	2	0	4	0	4	4	0	0	0	
	Dec	94	28	14	36	11	0	2	0	0	0	2	1	0	0	0	0	
	Total	1770	629	376	504	124	34	2	31	0	44	2	18	6	0	0	0	
	Ave./Mon	147.5	52.4	31.3	42.0	10.3	2.8	0.2	2.6	-	3.7	0.2	1.5	0.5	-	-	-	
2009	Jan.	69	28	19	16	1	0	0	0	2	0	1	0	2	0	0	0	
	Feb	42	16	6	17	0	1	0	0	0	0	2	0	0	0	0	0	
	Mar	97	38	17	34	4	0	0	0	3	0	1	0	0	0	0		
	Apr	101	25	35	26	12	0	0	0	3	0	0	0	0	0	0		
	May	116	27	38	36	12	1	0	0	0	0	2	0	0	0	0		
	Jun	146	37	30	44	13	10	0	0	10	0	2	0	0	0	0		
	Jul	196	42	82	40	23	4	0	0	3	0	2	0	0	0	0		
	Aug	90	37	28	20	2	0	0	0	2	0	1	0	0	0	0		
	Sep	44	8	11	9	0	0	0	0	0	0	2	0	14	0	0		
	Oct	106	36	32	27	1	0	0	0	0	0	10	0	0	0	0		
	Nov	143	62	42	30		0	0	0	5	0	2	0	2	0	0		
	Dec	77	23	29	22		0	0	0	3	0	0	0	0	0	0		
	Total	1227	379	369	321	68	16	0	0	31	0	25	0	18	0	0		
	Ave./Mon	102.3	31.6	30.8	26.8	6.8	1.3	-	-	2.6	-	2.1	-	1.5	-	-		

Year	Month	Type of vehicle															
		Total	Car 4 seats	Car <9 seats	Bus 9<24	Bus 24<32	Bus more 32 seats	Truck <1 ton with good	Truck <1 ton non goods	Truck 1 - 3 ton with good	Truck 1 - 3 ton non goods	Truck 3 - 5 ton with good	Truck 3 - 5 ton non goods	Truck 5 - 10 ton with good	Truck 5 - 10 ton non goods	Truck 10 - 13 ton with good	Truck 10 - 13 ton non goods
2007	Jan.	86	53	12	9	0	0	0	0	6	0	6	0	0	0	0	0
	Feb	94	62	5	6	1	0	0	0	2	0	16	0	2	0	0	0
	Mar	111	82	9	10	4	0	0	0	0	2	0	4	0	0	0	0
	Apr	162	115	15	18	6	7	0	0	0	1	0	0	0	0	0	0
	May	178	127	18	21	2	0	0	0	2	0	8	0	0	0	0	0
	Jun	232	129	35	46	13	6	0	0	2	0	1	0	0	0	0	0
	Jul	374	170	66	91	22	16	0	0	7	0	2	0	0	0	0	0
	Aug	313	103	90	81	18	7	0	0	8	4	1	1	0	0	0	0
	Sep	89	45	19	17	3	0	0	0	0	5	0	0	0	0	0	0
	Oct	185	83	43	32	0	0	0	0	19	0	0	6	0	2	0	2
	Nov	179	80	57	28	1	0	0	0	8	0	4	0	1	0	0	0
	Dec	192	64	71	26	0	0	0	0	23	0	6	0	2	0	0	0
	Total	2195	1113	440	385	70	36	0	0	77	4	52	1	15	0	2	0
	Ave./Mon	182.9	92.8	36.7	32.1	5.8	3.0	-	-	6.4	0.6	4.3	0.1	1.3	-	0.3	-
2008	Jan.	124	48	53	13	0	0	0	0	1	0	3	0	6	0	0	
	Feb	91	32	47	6	0	0	0	0	0	3	3	0	0	0	0	
	Mar	183	69	85	14	5	0	0	0	0	6	0	4	0	0	0	
	Apr	239	103	65	61	4	0	0	0	1	0	2	0	3	0	0	0
	May	231	63	90	57	12	4	0	0	0	4	1	0	0	0	0	0
	Jun	359	76	143	83	42	7	0	0	3	4	1	0	0	0	0	0
	Jul	478	108	162	144	45	11	0	0	0	2	1	4	1	0	0	0
	Aug	183	69	85	14	5	0	0	0	0	6	0	4	0	0	0	0
	Sep	178	53	81	39	3	0	0	0	0	0	0	2	0	0	0	0
	Oct	147	30	74	38	2	0	0	0	1	0	2	0	0	0	0	0
	Nov	166	47	69	33	6	0	0	0	0	5	1	4	1	0	0	0
	Dec	173	63	62	32	2	0	0	0	0	10	1	2	1	0	0	0
	Total	2552	761	1016	534	126	22	0	0	6	4	44	7	29	3	0	0
	Ave./Mon	212.7	63.4	84.7	44.5	10.5	1.8	-	-	0.5	0.6	3.7	0.6	2.4	0.3	-	-
2009	Jan.	118	26	68	11	0	0	0	0	6	5	0	0	2	0	0	
	Feb	124	34	74	16	0	0	0	0	0	0	0	0	0	0	0	
	Mar	148	34	79	24	2	0	6	1	0	0	0	2	0	0	0	
	Apr	186	35	95	38	2	0	0	0	16	0	0	0	0	0	0	
	May	172	53	71	35	4	0	0	0	9	0	0	0	0	0	0	0
	Jun	167	49	67	49	2	0	0	0	0	0	0	0	0	0	0	0
	Jul	193	83	70	30	2	0	0	0	7	1	0	0	0	0	0	0
	Aug	99	40	53	6	0	0	0	0	0	0	0	0	0	0	0	0
	Sep	106	35	42	5	0	0	0	0	10	0	0	0	7	7	0	0
	Oct	98	38	47	9	0	0	0	0	4	0	0	0	0	0	0	0
	Nov	76	33	32	7	0	0	0	0	4	0	0	0	0	0	0	0
	Dec	82	19	45	12	0	0	0	0	1	5	0	0	0	0	0	0
	Total	1569	479	743	242	12	0	6	1	57	11	0	0	11	7	0	0
	Ave./Mon	130.8	39.9	61.9	20.2	1.0	-	-	0.5	0.2	4.8	1.7	-	0.9	0.6	-	-

APPENDIX 2 -2: TRAFFIC DEMAND FORECSAST CALCULATION

1. Summary of Peak Hour Trip Generation (pcu/hr)

Position	Year	AM Peak		PM Peak	
		Generation	Attraction	Generation	Attraction
		(outbound)	(inbound)	(outbound)	(inbound)
Dinh Vu	2015	349	394	200	198
	2020	654	706	353	379
	2030	2,138	2,618	1,141	1,770

Position	Year	AM Peak		PM Peak	
		Generation	Attraction	Generation	Attraction
		(outbound)	(inbound)	(outbound)	(inbound)
Cat Hai	2015	792	307	307	792
	2020	1,309	686	686	1,309
	2030	1,846	1,300	1,300	1,846

Position	Year	AM Peak		PM Peak	
		Generation	Attraction	Generation	Attraction
		(outbound)	(inbound)	(outbound)	(inbound)
Cat Ba	2015	135	43	43	135
	2020	185	59	59	185
	2030	156	50	50	156

2. Summary of Future Traffic Demands

Section	Peak Hour	Direction	Year			
			2015	2020	2025	2030
Tan Vu IC - Dinh Vu	AM	To Tan Vu Interchange	1,276	2,149	3,145	4,140
		From Tan Vu Interchange	745	1,451	2,709	3,967
	PM	To Tan Vu Interchange	550	1,098	1,794	2,490
		From Tan Vu Interchange	1,125	1,874	2,823	3,772
Dinh Vu - Cat Hai	AM	Cat Hai to Dinh Vu	927	1,494	1,748	2,002
		Dinh Vu to Cat Hai	351	745	1,047	1,350
	PM	Cat Hai to Dinh Vu	351	745	1,047	1,350
		Dinh Vu to Cat Hai	927	1,494	1,748	2,002

3. Forecasted Traffic Demands on Dinh Vu Industrial Zone

Dinh Vu Industrial Zone 2015 (Without Railway)

No.	Item	Amount		Trip Generation Rate				Trip Generation (pcu/hr)			
				AM		PM		AM		PM	
				Generation	Attraction	Generation	Attraction	Generation	Attraction	Generation	Attraction
I	Industrial Zone	(100m ²)	(100m ²)	(pcu/hr/100m ²)		(pcu/hr/100m ²)		(pcu/hr)		(pcu/hr)	
		16,375	4,913	0.110	0.150	0.060	0.040	238	324	130	86
		16375*30%						4913*0.11*0.44	4913*0.15*0.44	4913*0.06*0.44	4913*0.04*0.44
II	Dinh Vu Port Area	(tons /yr)	(tons /yr)	(pcu/ton)		(pcu/ton)		(pcu/ton)		(pcu/ton)	
		4,500,000	616	0.082	0.082	0.082	0.082	51	51	51	51
		4500000/365*5%						616*0.082	616*0.082	616*0.082	616*0.082
III	Appartment block for resident	(m2)	(units)	(pcu/hr/unit)		(pcu/hr/unit)		(pcu/hr)		(pcu/hr)	
		162,500	406	0.250	0.080	0.080	0.250	61	19	19	61
		162500/1000*50%*5						406*0.25*0.6	406*0.08*0.6	406*0.08*0.6	406*0.25*0.6
TOTAL							349	394	200	198	

Dinh Vu Industrial Zone 2020 (Without Railway)

No.	Item	Amount		Trip Generation Rate				Trip Generation (pcu/hr)			
				AM		PM		AM		PM	
				Generation	Attraction	Generation	Attraction	Generation	Attraction	Generation	Attraction
I	Industrial Zone	(100m ²)	(100m ²)	(pcu/hr/100m ²)		(pcu/hr/100m ²)		(pcu/hr)		(pcu/hr)	
		32,750	9,825	0.110	0.150	0.060	0.040	432	590	236	157
		32750*30%						9825*0.11*0.4	9825*0.15*0.4	9825*0.06*0.4	9825*0.04*0.4
II	Dinh Vu Port Area	(tons /yr)	(tons /yr)	(pcu/ton)		(pcu/ton)		(pcu/ton)		(pcu/ton)	
		6,000,000	822	0.082	0.082	0.082	0.082	67	67	67	67
		6000000/365*5%						822*0.082	822*0.082	822*0.082	822*0.082
III	Appartment block for resident	(m2)	(units)	(pcu/hr/unit)		(pcu/hr/unit)		(pcu/hr)		(pcu/hr)	
		325,000	813	0.250	0.080	0.080	0.250	154	49	49	154
		325000/1000*50%*5						813*0.25*0.76	813*0.08*0.76	813*0.08*0.76	813*0.25*0.76
TOTAL							654	706	353	379	

Dinh Vu Industrial Zone 2030 (With Railway)

No.	Item	Amount		Trip Generation Rate				Trip Generation (pcu/hr)			
				AM		PM		AM		PM	
				Generation	Attraction	Generation	Attraction	Generation	Attraction	Generation	Attraction
I	Industrial Zone	(100m ²)	(100m ²)	(pcu/hr/100m ²)		(pcu/hr/100m ²)		(pcu/hr)		(pcu/hr)	
		97,680	29,304	0.110	0.150	0.060	0.040	1,805	2,462	985	656
		97680*30%						29304*0.11*0.56	29304*0.15*0.56	29304*0.06*0.56	29304*0.04*0.56
II	Dinh Vu Port Area	(tons /yr)	(tons /yr)	(pcu/ton)		(pcu/ton)		(pcu/ton)		(pcu/ton)	
		10,000,000	890	0.082	0.082	0.082	0.082	73	73	73	73
		10000000/365*0.65*5%						890*0.082	890*0.082	890*0.082	890*0.082
III	Appartment block for resident	(m2)	(units)	(pcu/hr/unit)		(pcu/hr/unit)		(pcu/hr)		(pcu/hr)	
		650,000	1,625	0.250	0.080	0.080	0.250	260	83	83	1,040
		650000/1000*50%*5						1625*0.25*0.64	1625*0.08*0.64	1625*0.08*0.64	1625*0.25*0.64
TOTAL							2,138	2,618	1,141	1,770	

4. Forecasted Traffic Demands on Cat Hai Island

Cat Hai Island 2015 (Without Railway)

No.	Item	Amount		Trip Generation Rate				Trip Generation (pcu/hr)			
				AM		PM		AM		PM	
				Generation	Attraction	Generation	Attraction	Generation	Attraction	Generation	Attraction
I	Population	(100m ²)	(100m ²)	(pcu/hr/100m ²)		(pcu/hr/100m ²)		(pcu/hr)		(pcu/hr)	
		19,000	4,750	0.250	0.080	0.080	0.250	713	228	228	713
		19000*1/4						4750*0.25*0.6	4750*0.08*0.6	4750*0.08*0.6	4750*0.25*0.6
II	Lach Huyen Port Area	(tons /yr)	(tons /yr)	(pcu/ton)		(pcu/ton)		(pcu/ton)		(pcu/ton)	
		5,394,000	739	0.082	0.082	0.082	0.082	61	61	61	61
		5394000/365*0.05						739*0.082	739*0.082	739*0.082	739*0.082
III	Tourists	(m2)	(units)	(pcu/hr/unit)		(pcu/hr/unit)		(pcu/hr)		(pcu/hr)	
		500,000	62	0.400	0.400	0.400	0.400	19	19	19	19
		500000/365*0.76*0.06						62*0.4*0.76	62*0.4*0.76	62*0.4*0.76	62*0.4*0.76
TOTAL							792	307	307	792	

Cat Hai Island 2020 (Without Railway)

No.	Item	Amount		Trip Generation Rate				Trip Generation (pcu/hr)			
				AM		PM		AM		PM	
				Generation	Attraction	Generation	Attraction	Generation	Attraction	Generation	Attraction
I	Population	(100m ²)	(100m ²)	(pcu/hr/100m ²)		(pcu/hr/100m ²)		(pcu/hr)		(pcu/hr)	
		19,300	4,825	0.250	0.080	0.080	0.250	917	293	293	917
		19300*1/4						4825*0.25*0.76	4825*0.08*0.76	4825*0.08*0.76	4825*0.25*0.76
II	Lach Huyen Port Area	(tons /yr)	(tons /yr)	(pcu/ton)		(pcu/ton)		(pcu/ton)		(pcu/ton)	
		29,525,000	4,045	0.082	0.082	0.082	0.082	332	332	332	332
		29525000/365*0.05						4045*0.082	4045*0.082	4045*0.082	4045*0.082
III	Tourists	(m2)	(units)	(pcu/hr/unit)		(pcu/hr/unit)		(pcu/hr)		(pcu/hr)	
		1,600,000	200	0.400	4.000	0.400	4.000	61	61	61	61
		1600000/365*0.76*0.06						200*0.4*0.76	200*0.4*0.76	200*0.4*0.76	200*0.4*0.76
TOTAL							1,309	686	686	1,309	

Cat Hai Island Zone 2030 (With Railway)

No.	Item	Amount		Trip Generation Rate				Trip Generation (pcu/hr)			
				AM		PM		AM		PM	
				Generation	Attraction	Generation	Attraction	Generation	Attraction	Generation	Attraction
I	Population	(100m ²)	(100m ²)	(pcu/hr/100m ²)		(pcu/hr/100m ²)		(pcu/hr)		(pcu/hr)	
		20,100	5,025	0.250	0.080	0.080	0.250	804	257	257	804
		20100*1/4						5025*0.25*0.64	5025*0.08*0.64	5025*0.08*0.64	5025*0.25*0.64
II	Lach Huyen Port Area	(tons /yr)	(tons /yr)	(pcu/ton)		(pcu/ton)		(pcu/ton)		(pcu/ton)	
		120,000,000	11,507	0.082	0.082	0.082	0.082	944	944	944	944
		120000000/365*0.7*0.05						11507*0.082	11507*0.082	11507*0.082	11507*0.082
III	Tourists	(m2)	(units)	(pcu/hr/unit)		(pcu/hr/unit)		(pcu/hr)		(pcu/hr)	
		2,600,000	325	0.400	0.400	0.400	0.400	99	99	99	99
		2600000/365*0.76*0.06						325*0.4*0.76	325*0.4*0.76	325*0.4*0.76	325*0.4*0.76
TOTAL							1,846	1,300	1,300	1,846	

5. Forecasted Traffic Demands on Cat Ba Island

Cat Ba Island 2015 (Without Railway)

No.	Item	Amount		Trip Generation Rate				Trip Generation (pcu/hr)			
				AM		PM		AM		PM	
				Generation	Attraction	Generation	Attraction	Generation	Attraction	Generation	Attraction
I	Population	(100m ²)	(100m ²)	(pcu/hr/100m ²)		(pcu/hr/100m ²)		(pcu/hr)		(pcu/hr)	
		12,000	900	0.250	0.080	0.080	0.250	135	43	43	135
		12000/4*0.3						900*0.25*0.6	900*0.08*0.6	900*0.08*0.6	900*0.25*0.6
III	Tourists	(m2)	(units)	(pcu/hr/unit)		(pcu/hr/unit)		(pcu/hr)		(pcu/hr)	
		500,000	----	0.400	0.400	0.400	0.400	0	0	0	0
TOTAL								135	43	43	135

Cat Ba Island 2020 (Without Railway)

No.	Item	Amount		Trip Generation Rate				Trip Generation (pcu/hr)			
				AM		PM		AM		PM	
				Generation	Attraction	Generation	Attraction	Generation	Attraction	Generation	Attraction
I	Population	(100m ²)	(100m ²)	(pcu/hr/100m ²)		(pcu/hr/100m ²)		(pcu/hr)		(pcu/hr)	
		13,000	975	0.250	0.080	0.080	0.250	185	59	59	185
		13000/4*0.3						975*0.25*0.76	975*0.08*0.76	975*0.08*0.76	975*0.25*0.76
III	Tourists	(m2)	(units)	(pcu/hr/unit)		(pcu/hr/unit)		(pcu/hr)		(pcu/hr)	
		348,700	----	0.400	0.400	0.400	0.400	0	0	0	0
TOTAL								185	59	59	185

Cat Ba Island 2030 (With Railway)

No.	Item	Amount		Trip Generation Rate				Trip Generation (pcu/hr)			
				AM		PM		AM		PM	
				Generation	Attraction	Generation	Attraction	Generation	Attraction	Generation	Attraction
I	Population	(100m ²)	(100m ²)	(pcu/hr/100m ²)		(pcu/hr/100m ²)		(pcu/hr)		(pcu/hr)	
		13,000	975	0.250	0.080	0.080	0.250	156	50	50	156
		13000/4*0.3						975*0.25*0.64	975*0.08*0.64	975*0.08*0.64	975*0.25*0.64
III	Tourists	(m2)	(units)	(pcu/hr/unit)		(pcu/hr/unit)		(pcu/hr)		(pcu/hr)	
		2,600,000	----	0.400	0.400	0.400	0.400	0	0	0	0
TOTAL								156	50	50	156

8. Daily Traffic Volume Based on Traffic Survey (Dinh – Cat Hai)

Unit:pcu/day

Year	Dinh Vu-Cat Hai Ferry and Ninh Tiep Ferry terminal									
	1	2	3	4	5	6	Port	Total	To Tan Vu IC Direction	To Cat Ba Direction
2010	33	192	22	237	22	0		506	278	228
2011	36	207	24	256	24	0		547	301	246
2012	39	224	26	276	26	0		591	325	266
2013	42	242	28	298	28	0		638	351	287
2014	45	261	30	322	30	0		688	378	310
2015	49	282	32	348	32	0	3,170	3,913	2,152	1,761
2016	53	305	35	376	35	0	5,658	6,462	3,554	2,908
2017	57	329	38	406	38	0	8,158	9,026	4,964	4,062
2018	62	355	41	438	41	0	10,678	11,615	6,388	5,227
2019	67	383	44	473	44	0	13,205	14,216	7,819	6,397
2020	72	414	48	511	48	0	15,748	16,841	9,263	7,578
2021	77	443	51	547	51	0	21,863	23,032	12,668	10,364
2022	82	474	55	585	55	0	26,035	27,286	15,007	12,279
2023	88	507	59	626	59	0	30,513	31,852	17,519	14,333
2024	94	542	63	670	63	0	35,315	36,747	20,211	16,536
2025	101	580	67	717	67	0	40,433	41,965	23,081	18,884
2026	108	621	72	767	72	0	32,103	33,743	18,559	15,184
2027	116	664	77	821	77	0	36,130	37,885	20,837	17,048
2028	124	710	82	878	82	0	40,378	42,254	23,240	19,014
2029	133	760	88	939	88	0	44,843	46,851	25,768	21,083
2030	142	813	94	1,005	94	0	49,555	51,703	28,437	23,266
2031	151	862	100	1,065	100	0	53,685	55,963	30,780	25,183
2032	160	914	106	1,129	106	0	58,048	60,463	33,255	27,208
2033	170	969	112	1,197	112	0	62,410	64,970	35,734	29,237
2034	180	1,027	119	1,269	119	0	66,768	69,482	38,215	31,267
2035	191	1,089	126	1,345	126	0	71,130	74,007	40,704	33,303

Year	Cat Hai Road									
	1	2	3	4	5	6	Port	Total	To Tan Vu IC Direction	To Cat Ba Direction
2010	129	202	29	221	26	0		607	334	273
2011	139	218	31	239	28	0		655	360	295
2012	150	235	33	258	30	0		706	388	318
2013	162	254	36	279	32	0		763	420	343
2014	175	274	39	301	35	0		824	453	371
2015	189	296	42	325	38	0	3,170	4,060	2,233	1,827
2016	204	320	45	351	41	0	5,658	6,619	3,640	2,979
2017	220	346	49	379	44	0	8,158	9,196	5,058	4,138
2018	238	374	53	409	48	0	10,678	11,800	6,490	5,310
2019	257	404	57	442	52	0	13,205	14,417	7,929	6,488
2020	278	436	62	477	56	0	15,748	17,057	9,381	7,676
2021	297	467	66	510	60	0	21,863	23,263	12,795	10,468
2022	318	500	71	546	64	0	26,035	27,534	15,144	12,390
2023	340	535	76	584	68	0	30,513	32,116	17,664	14,452
2024	364	572	81	625	73	0	35,315	37,030	20,367	16,664
2025	389	612	87	669	78	0	40,433	42,268	23,247	19,021
2026	416	655	93	716	83	0	32,103	34,066	18,736	15,330
2027	445	701	100	766	89	0	36,130	38,231	21,027	17,204
2028	476	750	107	820	95	0	40,378	42,626	23,444	19,182
2029	509	803	114	877	102	0	44,843	47,248	25,986	21,262
2030	545	859	122	938	109	0	49,555	52,128	28,670	23,458
2031	578	911	129	994	116	0	53,685	56,413	31,027	25,386
2032	613	966	137	1,054	123	0	58,048	60,941	33,518	27,423
2033	650	1,024	145	1,117	130	0	62,410	65,476	36,012	29,464
2034	689	1,085	154	1,184	138	0	66,768	70,018	38,510	31,508
2035	730	1,150	163	1,255	146	0	71,130	74,574	41,016	33,558

Year	Ben Got Ferry Terminal and Cat Hai-Cat Ba Ferry								
	1	2	3	4	5	6	Total	To Tan Vu IC Direction	To Cat Ba Direction
2010	47	176	28	152	36		439	241	198
2011	51	190	30	164	39	0	474	261	213
2012	55	205	32	177	42	0	511	281	230
2013	59	221	35	191	45	0	551	303	248
2014	64	239	38	206	49	0	596	328	268
2015	69	258	41	222	53	0	643	354	289
2016	75	279	44	240	57	0	695	382	313
2017	81	301	48	259	62	0	751	413	338
2018	87	325	52	280	67	0	811	446	365
2019	94	351	56	302	72	0	875	481	394
2020	102	379	60	326	78	0	945	520	425
2021	109	406	64	349	83	0	1,011	556	455
2022	117	434	68	373	89	0	1,081	595	486
2023	125	464	73	399	95	0	1,156	636	520
2024	134	496	78	427	102	0	1,237	680	557
2025	143	531	83	457	109	0	1,323	728	595
2026	153	568	89	489	117	0	1,416	779	637
2027	164	608	95	523	125	0	1,515	833	682
2028	175	651	102	560	134	0	1,622	892	730
2029	187	697	109	599	143	0	1,735	954	781
2030	200	746	117	641	153	0	1,857	1,021	836
2031	212	791	124	679	162	0	1,968	1,082	886
2032	225	838	131	720	172	0	2,086	1,147	939
2033	239	888	139	763	182	0	2,211	1,216	995
2034	253	941	147	809	193	0	2,343	1,289	1,054
2035	268	997	156	858	205	0	2,484	1,366	1,118

9. Daily Traffic Volume Based on Traffic Survey (Dinh – Cat Hai) Unit: Vehicles/day

Unit: Vehicles/day

Year	Dinh Vu-Cat Hai Ferry and Ninh Tiep Ferry terminal										
	1	2	3	4	5	6	Port	Total	To Tan Vu IC Direction	To Cat Ba Direction	
2010	165	640	22	119	9	0		954	525	429	
2011	178	691	24	128	10	0		1,031	567	464	
2012	192	746	26	138	11	0		1,113	612	501	
2013	207	806	28	149	12	0		1,202	661	541	
2014	224	870	30	161	13	0		1,298	714	584	
2015	242	940	32	174	14	0	1,268	2,670	1,469	1,202	
2016	261	1,015	35	188	15	0	2,263	3,777	2,077	1,700	
2017	282	1,096	38	203	16	0	3,263	4,898	2,694	2,204	
2018	305	1,184	41	219	17	0	4,271	6,037	3,320	2,717	
2019	329	1,279	44	237	18	0	5,282	7,189	3,954	3,235	
2020	355	1,381	48	256	19	0	6,299	8,358	4,597	3,761	
2021	380	1,478	51	274	20	0	8,745	10,948	6,021	4,927	
2022	407	1,581	55	293	21	0	10,414	12,771	7,024	5,747	
2023	435	1,692	59	314	22	0	12,205	14,727	8,100	6,627	
2024	465	1,810	63	336	24	0	14,126	16,824	9,253	7,571	
2025	498	1,937	67	360	26	0	16,173	19,061	10,484	8,577	
2026	533	2,073	72	385	28	0	12,841	15,932	8,763	7,169	
2027	570	2,218	77	412	30	0	14,452	17,759	9,767	7,992	
2028	610	2,373	82	441	32	0	16,151	19,689	10,829	8,860	
2029	653	2,539	88	472	34	0	17,937	21,723	11,948	9,775	
2030	699	2,717	94	505	36	0	19,822	23,873	13,130	10,743	

Unit: Vehicles/day

Year	Cat Hai Road										
	1	2	3	4	5	6	Port	Total	To Tan Vu IC Direction	To Cat Ba Direction	
2010	645	673	291	111	10	0		1,730	952	779	
2011	697	727	314	119	11	0		1,868	1,027	841	
2012	753	785	339	129	12	0		2,018	1,110	908	
2013	813	848	366	139	13	0		2,179	1,198	981	
2014	878	916	395	150	14	0		2,353	1,294	1,059	
2015	948	989	427	162	15	0	1,268	3,809	2,095	1,714	
2016	1,024	1,068	461	175	16	0	2,263	5,007	2,754	2,253	
2017	1,106	1,153	498	189	17	0	3,263	6,226	3,424	2,802	
2018	1,194	1,245	538	204	18	0	4,271	7,470	4,109	3,362	
2019	1,290	1,345	581	220	19	0	5,282	8,737	4,805	3,932	
2020	1,393	1,453	627	238	21	0	6,299	10,031	5,517	4,514	
2021	1,491	1,555	671	255	22	0	8,745	12,739	7,006	5,733	
2022	1,595	1,664	718	273	24	0	10,414	14,688	8,078	6,610	
2023	1,707	1,780	768	292	26	0	12,205	16,778	9,228	7,550	
2024	1,826	1,905	822	312	28	0	14,126	19,019	10,460	8,559	
2025	1,954	2,038	880	334	30	0	16,173	21,409	11,775	9,634	
2026	2,091	2,181	942	357	32	0	12,841	18,444	10,144	8,300	
2027	2,237	2,334	1,008	382	34	0	14,452	20,447	11,246	9,201	
2028	2,394	2,497	1,079	409	36	0	16,151	22,566	12,411	10,155	
2029	2,562	2,672	1,155	438	39	0	17,937	24,803	13,642	11,161	
2030	2,741	2,859	1,236	469	42	0	19,822	27,169	14,943	12,226	

Unit: Vehicles/day

Year	Ben Got Ferry Terminal and Cat Hai-Cat Ba Ferry								
	1	2	3	4	5	6	Total	To Tan Vu IC Direction	To Cat Ba Direction
2010	235	587	28	76	14		940	517	423
2011	254	634	30	82	16	0	1,016	559	457
2012	274	685	32	89	17	0	1,097	603	494
2013	296	740	35	96	18	0	1,185	652	533
2014	320	799	38	104	19	0	1,280	704	576
2015	346	863	41	112	21	0	1,383	761	622
2016	374	932	44	121	23	0	1,494	822	672
2017	404	1,007	48	131	25	0	1,615	888	727
2018	436	1,088	52	141	27	0	1,744	959	785
2019	471	1,175	56	152	29	0	1,883	1,036	847
2020	509	1,269	60	164	31	0	2,033	1,118	915
2021	545	1,358	64	175	33	0	2,175	1,196	979
2022	583	1,453	68	187	35	0	2,326	1,279	1,047
2023	624	1,555	73	200	37	0	2,489	1,369	1,120
2024	668	1,664	78	214	40	0	2,664	1,465	1,199
2025	715	1,780	83	229	43	0	2,850	1,568	1,283
2026	765	1,905	89	245	46	0	3,050	1,678	1,373
2027	819	2,038	95	262	49	0	3,263	1,795	1,468
2028	876	2,181	102	280	52	0	3,491	1,920	1,571
2029	937	2,334	109	300	56	0	3,736	2,055	1,681
2030	1,003	2,497	117	321	60	0	3,998	2,199	1,799

Unit: Vehicles/day

Year	Ben Got Ferry Terminal and Cat Hai-Cat Ba Ferry								
	1	2	3	4	5	6	Total	To Tan Vu IC Direction	To Cat Ba Direction
2010	235	587	28	76	14		940	517	423
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2020	509	1,269	60	164	31	0	2,033	1,118	915
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2023	624	1,555	73	200	37	0	2,489	1,369	1,120
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2029	937	2,334	109	300	56	0	3,736	2,055	1,681
2030	1,003	2,497	117	321	60	0	3,998	2,199	1,799

Appendix-3: Standard and Criteria for Bridge Design

1. Design Standards

Basically, the bridges and structures in this project shall be designed with the Vietnamese Design Standard (22 TCN 272-05) and AASHTO-LRFD (Load and Resistance Factor Design, 3rd Edition 2004). However, the some items shall be considered in accordance with the other international standards.

The adopted items for this project are summarized in Table 1.

Table 1 Adopted Items for this Project

Item	Specification	Standard
Design Method	Limit State Design	Vietnamese
Design Life	100 years	Vietnamese
Design Lane Width	3600 mm or 3750 mm	Vietnamese
Load Combination		Vietnamese
Live Load	HL-93	Vietnamese
Dynamic Load Allowance, IM	0.25 for main part of bridge	Vietnamese
Wind Load	Depend on the site	Vietnamese
Vessel Collision Force	Depend on the site	Vietnamese
Earthquake	Depend on the site	Vietnamese/Japanese
Seismic Earth Pressure	Depend on the site	Vietnamese/Japanese
Stress Loss in Tendons		Vietnamese/Japanese
Creep & Shrinkage		Vietnamese/Japanese /CEB-FIP
Pile Foundation Analysis	Displacement Method	Vietnamese/Japanese
Train Load	T-26	Vietnamese

The items not fit for these standards, shall be determined referring to AASHTO (Allowable stress design method, 17th Edition 2002) or Japanese Standard of Highway Bridge (JSHB-96).

The highway cum railway bridge is planned in this project. The clause 1.1 of 22 TCN 272-05 described that “ It is envisaged that a supplement on the design of railway bridges will be produced in the future.” Accordingly, the design concept of the highway cum railway bridge will be established in accordance with AASHTO. The train load shall be taken from Vietnamese standards, which the Railway Projects Management Unit (RPMU) has applied for Hanoi - Ho Chi Minh City Railway Bridge Rehabilitation Project. Since the load combination is not specified in these standards, the Consultant will establish the load combination and load factors.

1.1. Limit State Design

The bridge and structures shall be designed for specified limit states in the Vietnamese Standard (22 TCN 272-05) to achieve the objectives of constructability, safety, and serviceability with due regard to issues of inspectibility, economy, and aesthetics in considering

with the design life of bridge and structures shall be 100 years.

1.1.1. Limit States

The bridge and structures shall be verified under the following limit states. And all limit states shall be considered of equal importance.

- Strength Limit State
- Extreme Event Limit State
- Service Limit State
- Fatigue Limit State

Each component and connection shall satisfy the following equation for service limit, fatigue and fracture limit, strength limit and extreme event limit states.

$$Q = \sum \eta_i \gamma_i Q_i \leq \phi R_n = R_r$$

where:

- Q = factored load
- Q_i = force effect
- R_n = nominal resistance
- R_r = factored resistance
- γ_i = load factor
- ϕ = resistance factor
- η_i = load modifier

(1) Load Modifier (η_i)

The load modifier for strength limit state is calculated by the following equation. Besides the load modifier for the other limit state should be 1.0.

$$\eta_i = \eta_D * \eta_R * \eta_I$$

where:

- η_D = a factor relating to ductility
- η_R = a factor relating to redundancy
- η_I = a factor relating to operational importance

Table 2 Load Modifier

Factor	Category	Strength Limit State
η_D	For nonductile components and connections	≥ 1.05
	For conventional designs and details complying with TVCN 22 TCN-272-05	1.00
	For components and connections for which additional ductile-enhancing measures have been specified beyond those required by TVCN 22 TCN-272-05	≥ 0.95
η_R	For nonredundant members	≥ 1.05
	For conventional levels of redundancy	1.00
	For exceptional levels of redundancy	≥ 0.95
η_I	For important bridges	≥ 1.05
	For typical bridges	1.00
	For relatively less important bridges	≥ 0.95

(2) **Limit States for the bridge and structures**

The limit states for the bridge and structures are shown in Table 3.

Table 3 Limit States for the bridge and structures

Limit State	Outline of Limit State
Strength-I	Basic load combination relating to the normal vehicular use of the bridge without wind.
Strength-II	Load combination relating to the bridge exposed to wind velocity exceeding 25 m/s without live load.
Strength-III	Load combination relating to normal vehicular use of the bridge with wind of 25 m/s velocity.
Extrem Event	Load combination relating to earthquake, collision by vessels and vehicles, and certain hydraulic events with a reduced live load other than that which is part of the vehicular collision load, CT.
Service	Load combination relating to the normal use of the bridge with a 25 m/s wind and all loads taken at their normal values, to control deflections, crack width in RC and PC structure, yielding of steel structures and slip of slip critical connections due to vehicular live load, and to investigate slope stability.
Fatigue	Fatigue and fracture load combination relating to repetitive gravitational vehicular live load and dynamic responses under a single design truck.

1.2. Load Factor and Combination

1.2.1. Loads

The following permanent and transient loads shall be considered.

Table 4 Permanent and Transient Loads

Permanent Loads	DD = Downdrag DC = Dead load of structural components and nonstructural attachment DW = Dead load of wearing surfaces and utilities EH = Horizontal earth pressure load EL = Accumulated locked-in force effects resulting from the construction process, including the secondary forces from post-tensioning ES = Earth surcharge load EV = Vertical pressure from dead load of earth fill
Transient Loads	BR = Vehicular braking force CE = Vehicular centrifugal force CR = Creep CT = Vehicular collision force CV = Vessel collision force EQ = Earthquake FR = Friction IM = Vehicular dynamic load allowance LL = Vehicular live load LS = Live load surcharge PL = Pedestrian live load SE = Settlement SH = Shrinkage TG = Temperature gradient TL = Train Load TU = Uniform temperature WA = Water load and stream pressure WL = Wind on live load WS = Wind load on structure

1.2.2. Load Factor and Combination

The total factored force effect shall be taken as:

$$Q = \sum \eta_i * \gamma_i * Q_i$$

where:

Q_i = force effects from loads

γ_i = load factors specified in Tables 5 to 7

Table 5 Load Combinations and Factors

Load Combination	DC DD DW EH EV ES	LL IM CE BR PL LS EL	TL	WA	WS	WL	FR	TU CR SH	TG	SE	Use One of These At a Time		
											EQ	CT	CV
Strength-I	γ_p	1.75	1.75	1.00	-	-	1.00	0.50/1.20	γ_{TG}	γ_{SE}	-	-	-
Strength-II	γ_p	-	-	1.00	1.40		1.00	0.50/1.20	γ_{TG}	γ_{SE}	-	-	-
Strength-III	γ_p	1.35	1.35	1.00	0.40	1.00	1.00	0.50/1.20	γ_{TG}	γ_{SE}	-	-	-
Extreme	γ_p	0.50	0.50	1.00	-	-	1.00	-	-	-	1.00	1.00	1.00
Service	1.00	1.00	1.00	1.00	0.30	1.00	1.00	1.00/1.20	γ_{TG}	γ_{SE}	-	-	-
Fatigue-LL, IM&CE only	-	0.75	0.75	-	-	-	-	-	-	-	-	-	-

Note: For checking crack widths in prestressed concrete structures at the service limit state, the load factor for live load may be reduced to 0.80

Table 6 Load Factors for Permanent Loads, γ_p

Type of Load			Load Factor	
			Maximum	Minimum
DC	:	Component and Attachments	1.25	0.90
DD	:	Downdrag	1.80	0.45
DW	:	Wearing Surfaces and Utilities	1.50	0.65
EH	:	Horizontal Earth Pressure		
		Active	1.50	0.90
		At Rest	1.35	0.90
EL	:	Locked-in Erection Stress	1.00	1.00
EV	:	Vertical Earth Pressure		
		Overall Stability	1.00	N/A
		Retaining Structures	1.35	1.00
		Rigid Buried Structures	1.30	0.90
		Rigid Frames	1.35	0.90
		Flexible Buried Structures other than Metal Box Culverts	1.95	0.90
ES	:	Flexible Metal Box Culverts	1.50	0.90
		Earth Surcharge	1.50	0.75

Table 7 Load Factor for Temperature Gradient, γ_{TG}

γ_{TG}	Conditions
0.00	at the strength and extreme event limit states
1.00	at the service limit state when live load is not considered
0.50	at the service limit state when live load is considered

1.3. Design Load

1.3.1. Dead Load: DC, DW and EV

Dead loads shall include the weight of all components of the structure, appurtenances and utilities attached thereto, earth cover, wearing surface and future overlays.

The following densities specified in Table 8 for each material is used for dead loads. And the weight of utilities shall be decided due to the site investigations.

Table 8 Densities

Material	Density (kg/m ³)	
Aluminum Alloys	2800	
Bituminous Wearing Surfaces	2250	
Cinder Filling	960	
Compacted Sand, Silt or Clay	Due to soil investigation	
Concrete	Low-density	1775
	Sand-low-density	1925
	Normal	2400
Loose Sand, Silt or Gravel, Soft Clay	Due to soil investigation	
Rolled Gravel, Macadam or Ballast	2250	
Steel	7850	
Stone Masonry	2725	
Water	Fresh	1000
	Salt	1025

1.3.2. Live Loads

(1) Vehicular Live Load: LL

1) Number of Design Lanes

The number of design lanes should be determined by taking the integer part of the ratio $w/3600$, where w is the clear roadway width in mm between curbs and/or barriers.

2) Multiple Presence Factor

The extreme live load force effect shall be determined by considering each possible combination of number of loaded lanes multiplied by a corresponding multiple presence factor to account for the probability of simultaneous lane occupation by the HL-93 design live load. The multiple presence factors are shown in Table 9.

For the purpose of determining the number of lanes when the loading condition includes the pedestrian loads combined one or more lanes of the vehicular live load, pedestrian loads may be taken to be one loaded lane.

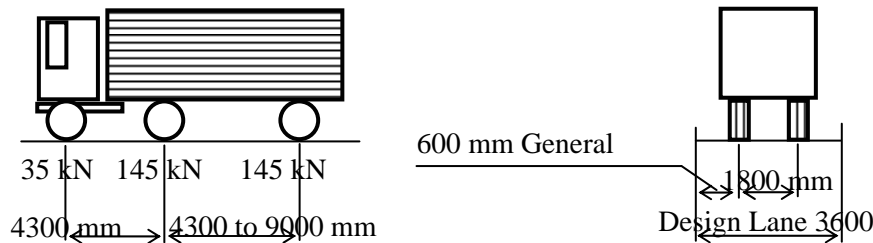
Table 9 Multiple Presence factors “m”

Number of Loaded Lanes	1	2	3	> 3
Multiple Presence Factors “m”	1.20	1.00	0.85	0.65

3) Design Vehicular Live Load

Vehicular live loading (HL-93) shall consist of a combination of the followings:

- Design Truck
- Design Tandem
- Design Lane Load



Note: For fatigue load, the distance between 145 kN axles shall be constant of 9000 mm

Figure 1 Design Truck

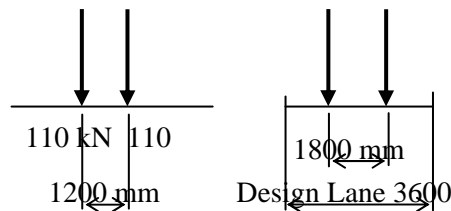


Figure 2 Design Tandem

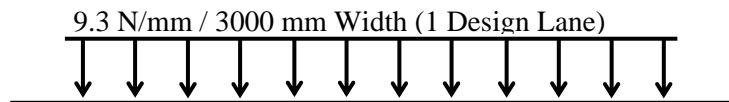


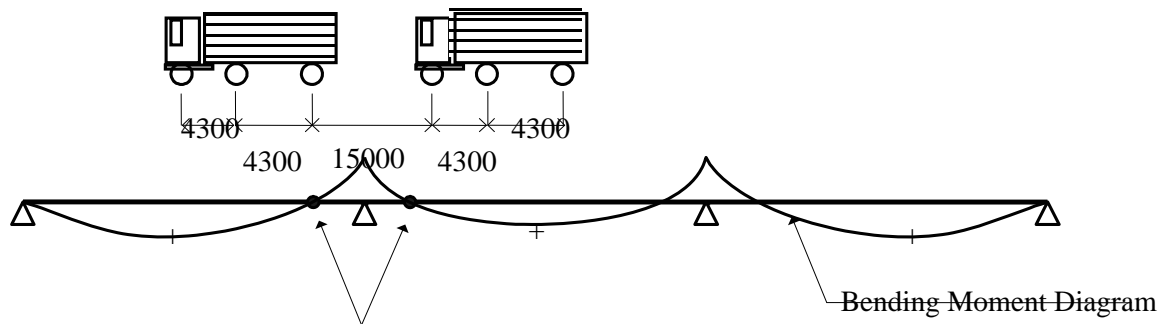
Figure 3 Design Lane Load

The extreme force effect shall be taken as the larger of the followings:

- The effects of Design Tandem and Design Lane Load
- The effects of Design Truck with variable axle spacing and Design Lane Load
- For both negative moment between points of contra flexure under a uniform load on all spans, and reaction at interior pier, 90% of the effect of two design trucks spaced a minimum 15000 mm between the lead axle of one truck and the rear axle of the other truck, combined with 90 % of the effect of the design lane load. The distance between the 145000 N axles of each truck shall be 4300 mm

And the extreme force effect shall be considered as follows:

- Longitudinally, the axles that do not contribute to the extreme force effect under consideration shall be neglected
- Transverse, both the design lanes and the 3000 mm loaded width in each lane shall be positioned to produce extreme force effects



Contra flexure Points under uniform load on all spans

Figure 4 Two Design Trucks Loadings for Negative Moment and Reaction at Interior Pier

(2) **Pedestrian Loads: PL**

A pedestrian load of 3×10^{-3} MPa shall be applied to all sidewalks wider than 600mm and considered simultaneously with the vehicular design live load.

(3) **Dynamic Load Allowance: IM**

In case of the both of the design truck and tandem, the static effects shall be increased by the percentage specified in Table 10 for dynamic load allowance.

Table 10 Dynamic Load Allowance, IM

Component	Deck Joints - All Limit States	All Other Components	
		Fatigue and Fracture Limit State	All Other Limit States
IM	75 %	15 %	25 %

Dynamic load allowance need not be applied to:

- Retaining walls not subject to vertical reactions from the superstructure
- Foundation components that are entirely below ground level

For buried structures such as culverts, IM shall be taken as:

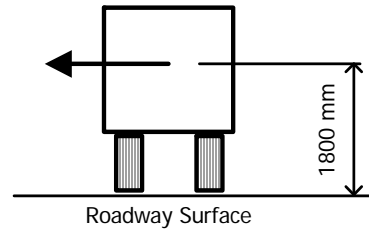
$$IM = 33 * (1.0 - 4.1 * 10^{-4} * DE) \geq 0 \%$$

Where;

DE = Minimum depth of earth cover above the structure (mm).

(4) **Centrifugal Forces: CE**

Centrifugal forces, which is to be applied horizontally at a distance 1800 mm above the roadway surface, shall be taken as the product of the axle weights of the design truck or tandem and the factor C, taken as:



Centrifugal Force

$$C = \frac{4 v^2}{3 g R}$$

where:

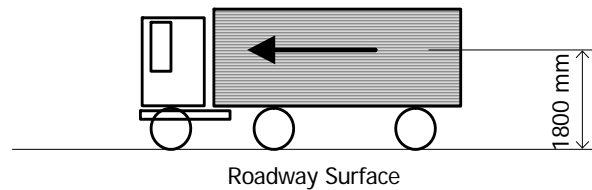
- v = highway design speed (m/s)
- g = gravitational acceleration: 9.807 (m/s²)
- R = radius of curvature of traffic lane (m)

The multiple presence factors shall apply.

(5) **Braking Force: BR**

The braking forces shall be taken as 25% of the axle weights of the design truck or tandem per lane placed in all design lanes which are carrying traffic headed in the same direction. Besides all design lanes shall be simultaneously loaded for bridges likely to become one-directional in the future.

These forces shall be assumed to act horizontally at a distance of 1800 mm above the roadway surface in either longitudinal direction to cause extreme force effects.



Braking Force

The multiple presence factors shall apply.

(6) **Vehicular Collision Force: CT**

Unless protected as followings, abutments and piers located within a distance of 9000 mm to the edge of roadway, shall be designed for an equivalent static force of 1800 kN, which is assumed to act in any direction in a horizontal plane, at a distance of 1200 mm above ground.

- An embankment;
- A structurally independent, crashworthy ground mounted 1370 mm high barrier, located within 3000 mm from the component being protected
- A 1070 mm high barrier, located at more than 3000 mm from the component being protected

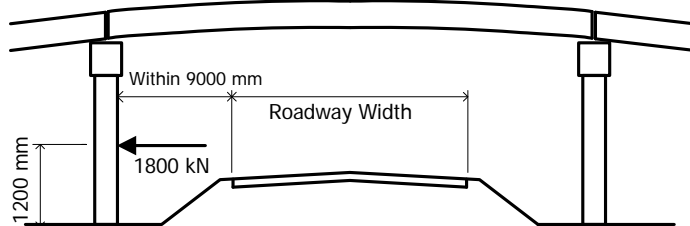


Figure 5 Vehicular Collision Force

(7) **Train Live Load: TL**

Train Load

A train load of T-26 shall be applied and considered simultaneously with the vehicular design live load. The train load is shown in Figure 6.

Train Load (T-26)

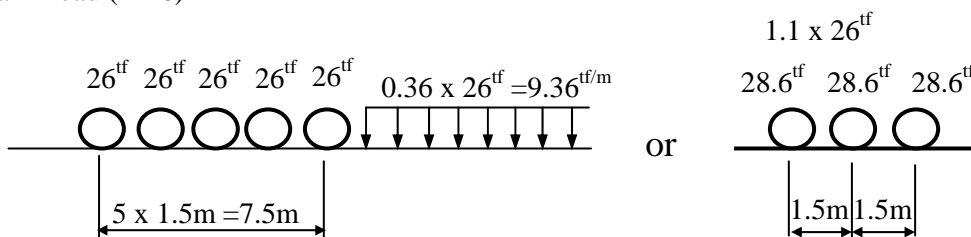


Figure 6 Train Load of T-26

Centrifugal Load

The Centrifugal Load shall act at right angles and horizontally to the truck at the center of gravity of the train load.

Centrifugal Load = Train Load x α (tf)

$$\alpha = \frac{v^2}{127R}$$

where:

- α = Factor of train load
- v = Maximum speed (km/h) of the train traveling on the curved track (m/s) : **to be specified.**
- R = Radius of the curve (m)

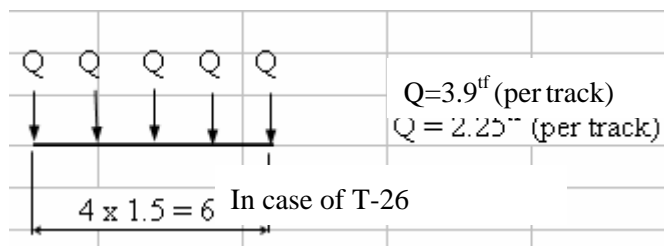
Long Rail Longitudinal Load

Long Rail Longitudinal Load = 1.0tf/m/truck x the overall length of the member (tf/truck)

This value shall not exceed 200tf/truck.

Rolling Stock Lateral Load

The rolling stock lateral load shall act at right angle and horizontally to the rail surface height.



Breaking Load and Starting Load

The breaking load and starting load shall act on the track at the center of gravity height of the train load.

Breaking Load	25% of the characteristic value of train load
Starting Load	25% of the drive wheel axle weight constituting the characteristic value of train load

Note: Loading length of train load shall be within the range of maximum effect on the member

1.3.3. Water Loads: WA

For bridges over water way, the water loads as static pressure, buoyancy and stream pressure shall be adopted with following considerations;

Strength and Service Limit State

The consequences of changes in foundation conditions resulting from the design flood for scour shall be considered.

Extreme Event Limit State (with EQ, CT and CV)

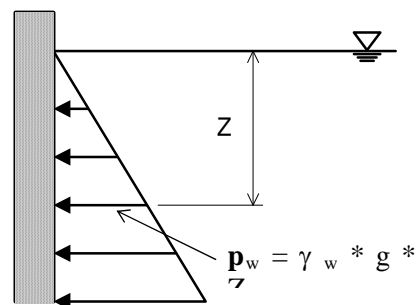
The water loads and scour depths may be based on the mean annual discharge.

Extreme Event Limit State (without EQ, CT and CV)

The structure shall be checked for the consequences of changes in foundation conditions resulting from the check flood for scour.

(1) **Static Pressure**

Static pressure of water shall be assumed to act perpendicular to the surface that is retaining water. Pressure shall be calculated as the product of height (Z) of water above the point of consideration, the density of water (γ_w), and g (the acceleration of gravity).



Static Water Pressure

(2) **Buoyancy**

Buoyancy shall be considered to be an uplift force, taken as the sum of the vertical components of static pressures acting on all components below the design water level.

(3) **Stream Pressure**

1) **Longitudinal**

The pressure of flowing water acting in the longitudinal direction of substructure shall be taken as:

$$p = 5.14 * 10^{-4} C_D V^2$$

where:

p = pressure of flowing water (MPa)

C_D = drag coefficient for piers as specified in Table 11

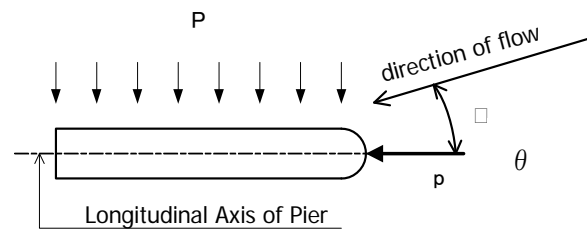
V = velocity of water for the design flood for scour in strength and service limit states and for the check flood for scour in the extreme event limit state (m/s)

Table 11 Drag Coefficient

Type	C _D
Semicircular-nosed Pier	0.70
Square-ended Pier	1.40
Debris lodges against the Pier	1.40
Wedge-nosed Pier with Nose Angle 90o or less	0.80

2) Transverse

The lateral, uniformly distributed pressure on a substructure due to water flowing at an angle, θ, to the longitudinal axis of the pier shall be taken as:



Transverse Water Pressure

$$p = 5.14 * 10^{-4} C_L V^2$$

where:

p = lateral pressure (MPa)

C_L = lateral drag coefficient specified in Table 12

Table 12 Lateral Drag Coefficient

Angle, θ, between direction of flow and longitudinal axis of the pier	C _L
0°	0.0
5°	0.5
10°	0.7
20°	0.9
>= 30°	1.0

(4) Wave load

Wave load on the bridge structures shall be considered for exposed where the development of significant wave forces may occur.

1.3.4. Wind Load

(1) Horizontal Wind Load

1) General

This Article provides design wind loads for conventional bridge structures. For long span, specific wind climate studies should be carried out to determine the wind effects.

The design wind velocity, V, shall be determined from:

$V = V_B * S$ where:

V_B = basic 3 second gust wind velocity with 100 years return period appropriate to the Wind Zone in which the bridge is located, as specified in Table 13

S = correction factor for upwind terrain and deck height, as specified in Table 14

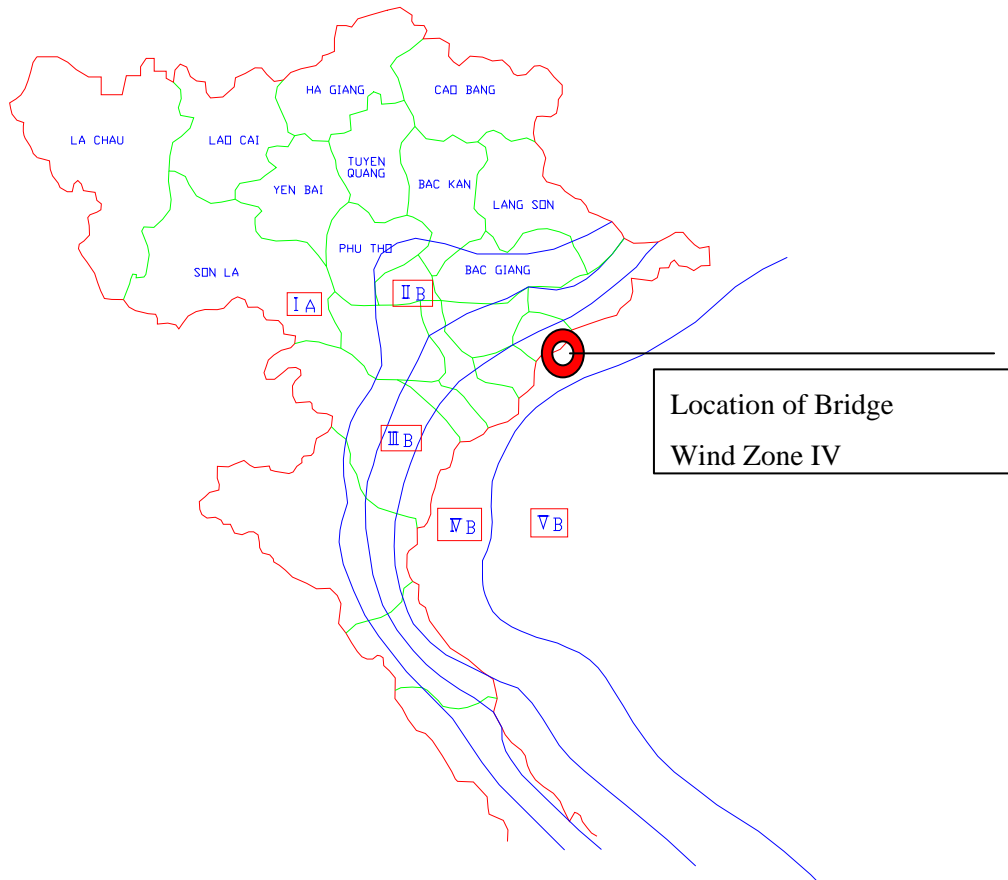


Figure 7 Wind Zone Map in Vietnam

Table 13 Values of V_B for Wind Zones in Vietnam

Wind zone (TCVN2737-1995)	V_B (m/s)
I	38
II	45
III	53
IV	59

Table 14 Values of S

Height of bridge deck above surrounding ground or water level (m)	Open country or open water	Wooded country or built-up areas, with trees or buildings up to a maximum height of about 10 m	Built-up areas with buildings predominantly over 10 m high
10	1.09	1.00	0.81
20	1.14	1.06	0.89
30	1.17	1.10	0.94
40	1.20	1.13	0.98
50	1.21	1.16	1.01

2) Wind Load on Structures: WS

Transverse Wind Load

The transverse wind load, P_D , shall be taken as acting horizontally at the centroids of the appropriate areas, and shall be calculated as:

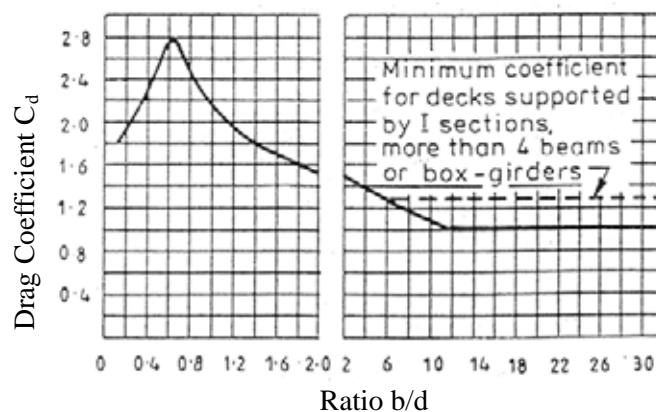
$$P_D = 0.0006V^2 A_t C_d \geq 1.8A_t \quad \text{where:}$$

(kN)

V = design wind velocity (m/s)

A_t = solid area of the structure or element for calculation of transverse wind load (m^2)

C_d = drag coefficient specified in Figure 8



b =overall width of bridge between outer faces of parapets(mm)

d =depth of superstructure including solid parapets if applicable(mm)

Figure 8 Drag Coefficient C_d for Superstructures with Solid Elevation

The area of the structure or element under consideration A_t , shall be the solid area in normal projected elevation, without live load, subjected the following provisions;

For superstructures with solid parapets: The area of superstructure shall include the area of the solid windward parapet, but the effect of the leeward parapet need not be considered.

For superstructures with open parapets: The area of superstructure shall include both windward and leeward parapets separately considered. Where there are more than two parapets, only those two having the greatest unshielded effect shall be considered.

For truss girder superstructures: The wind force shall be calculated for each component separately, both windward and leeward, without considering shielding.

For piers: Shielding shall not be considered

Longitudinal Wind Load

The longitudinal wind load shall be considered as follows;

For piers, abutments, truss girder superstructures and other superstructure forms which represent a significant surface area to wind loads parallel to longitudinal centerline similar way to those for transverse wind loads

For superstructure with solid elevation: 0.25 times the transverse wind load

Longitudinal and transverse wind loads shall be applied as separate load cases and, where appropriate, the structure shall be checked for the effect of intermediate angles of wind by resolution of forces.

3) Wind Load on Vehicles: WL

When considering STRENGTH III load combination, the design wind load shall be applied to both structure and vehicles. And longitudinal and transverse wind loads shall be applied as separate load cases and, where appropriate, the structure shall be checked for the effect of intermediate angles of wind by resolution of forces.

Transverse

The transverse wind load on vehicles shall be represented by a line load of 1.50 kN/m acting horizontally, transverse to the longitudinal centerline of the structure and 1800 mm above the roadway.

Longitudinal

The longitudinal wind load on vehicles shall be represented by a line load of 0.75 kN/m acting horizontally, parallel to the longitudinal centerline of the structure and 1800 mm above the roadway.

(2) Vertical Wind Load

In case the angle of inclination of the wind to the structure less than 5 degrees, a vertical wind load, P_v , shall be taken as acting at the centroid of the appropriate area, and shall be calculated as:

$$P_v = 0.00045V^2 A_v \quad (\text{kN})$$

where:

V = design wind velocity (m/s)

A_v = plan area of the bridge deck or element for calculation of vertical wind load (m^2)

This load shall be applied only for limit states that do not involve wind on live load, and only when the direction of wind is taken to be perpendicular to the longitudinal axis of the bridge.

1.3.5. Earthquake Effects: EQ

Earthquake loads shall be taken to be horizontal force effects for rigid-frame superstructures,

substructures, foundations and connections between superstructures and substructures.

Seismic effects for box culverts and buried structures need not be considered, except where they cross active fault.

These loads are determined based on the following items.

- Acceleration Coefficient (AC) at each bridge
- Importance Categories (IC) for each bridge
- Seismic Zone based on AC for each bridge
- Site Effects (S) based on soil profile type
- Period of Vibration of the m-th mode (Tm) for the structure
- Response Modification Factor (R) for the substructures and connections

(1) Analysis for Earthquake Loads

The minimum analysis requirements for seismic effects shall be as specified in Table 15 depend on structural type, seismic zone, importance category, and part of the structure.

The connections between the superstructure and substructure shall be designed for the minimum force requirements.

Also the minimum seat width requirement shall be satisfied.

- UL = uniform load elastic method
- SM = single-mode elastic method
- MM = multimode elastic method
- TH = time history method

Table 15 Minimum Analysis Requirements for Seismic Effects

Seismic Zone	Single- Span Bridges	Multi-span Bridges					
		Other Bridges		Essential Bridges		Critical Bridges	
		Regular	Irregular	Regular	Irregular	Regular	Irregular
1	No need	No need	No need	No need	No need	No need	No need
2	No need	SM/UL	SM	SM/UL	MM	MM	MM
3	No need	SM/UL	MM	MM	MM	MM	TH

(2) Acceleration Coefficients (AC) and Seismic Zone

The seismic zone of each bridge shall be determined base on the acceleration coefficient using following table:

Table 16 Seismic Zones

Acceleration coefficients	Seismic zone	MSK – 64 class
$A \leq 0.09$	1	Class ≤ 6.5
$0.09 < A \leq 0.19$	2	$6.5 < \text{Class} \leq 7.5$
$0.19 < A < 0.29$	3	$7.5 < \text{Class} \leq 8$

According to the Vietnamese Design Code TCXDVN 375:2006 “Design of Structures with Seismic Isolation”, project site is located on the seismic zone 7, acceleration coefficient $A=0.1291$.

(3) Importance Categories

The Owner shall classify the bridge one of three Importance Categories as follows;

- Critical Bridges
- Essential Bridges
- Other Bridges

(4) Site Effects

Based on the soil profile at each bridge site, site effects shall be included in the determination of seismic loads for bridges. The site coefficients are shown in Table 17.

Table 17 Site Coefficients

Site Coefficient	Soil Profile Type			
	I	II	III	IV
S	1.00	1.20	1.50	2.00

Where the soil profiles are follows;

Soil Profile Type I

Rock of any description, either shale - like or crystalline in nature, or Stiff soils where the soil depth is less than 60 m, and the soil types overlying rock are stable deposits of sands, gravels, or stiff clays

Soil Profile Type II

Stiff cohesive or deep cohesionless soils where the soil depth exceeds 60 m and the soil types overlaying the rock are stable deposits of sands, gravels, or stiff clays

Soil Profile Type III

Soft to medium-stiff clays and sands, characterized by 9 m or more of soft to medium-stiff clays

Soil Profile Type IV

Soft clays or silts greater than 12 m in depth

(5) Elastic Seismic Response Coefficient

The elastic seismic response coefficient, C_{sm} for the m^{th} mode of vibration shall be taken as :

$$C_{sm} = 1.2 AS / T_m^{2/3} \quad \text{where :}$$

$$\leq 2.5A$$

T_m = period of vibration of the m^{th} mode (s); based on the nominal unfactored
 Mass of the component or structure.
 A = acceleration coefficient
 S =site coefficient

For soil profiles III and IV, and for modes other than the fundamental mode that have periods less 0.30s, C_{sm} shall be taken as:

➤ $C_{sm} = A(0.8+4.0 T_m)$

If the period of vibration for any mode exceeds 4.0s, the value of C_{sm} for that mode shall be taken as;

➤ $C_{sm} = 3.0 AS / T_m^{4/3}$

(6) Response Modification Factors

Seismic design force effects for substructures and the connections between parts of structure, shall be determined by dividing the force effects resulting from elastic analysis by the appropriate response modification factor, R.

Table 18 Response Modification Factors-Substructures

Substructure	Importance category		
	Critical	Essential	Other
Wall-type piers larger dimension	1.5	1.5	2.0
Reinforced concrete pile bents			
- Vertical piles only	1.5	2.0	3.0
- With batter piles	1.5	1.5	2.0
Single columns	1.5	2.0	3.0
Steel or composite steel and concrete pile bents			
- Vertical piles only	1.5	3.5	5.0
- With batter piles	1.5	2.0	3.0
Multiple column bents	1.5	3.5	5.0

Table 19 Response Modification Factors-Connections

Connection	All importance categories
Superstructure to abutment	0.8
Expansion joints within a span of the superstructure	0.8
Columns, piers, or pile bents to cap beam or superstructure	1.0
Columns or piers to foundations	1.0

(2) **At-Rest Lateral Earth Pressure Coefficient, K_0**

$K_0 = 1 - \sin\phi_f$: for normally consolidated soils
 $K_0 = (1 - \sin\phi_f) (OCR)^{\sin\phi_f}$: for overconsolidated soils

where:
 ϕ_f = effective friction angle of soil
 K_0 = coefficient of earth pressure at rest
 OCR = overconsolidation ratio (refer to Table 20)

Table 20 Typical Coefficient of Lateral Earth Pressure At-Rest

Soil Type	Coefficient of Lateral Earth Pressure, K_0			
	OCR=1	OCR=2	OCR=5	OCR=10
Loose Sand	0.45	0.65	1.10	1.60
Medium Sand	0.40	0.60	1.05	1.55
Dense Sand	0.35	0.55	1.00	1.50
Silt (ML)	0.50	0.70	1.10	1.60
Lean Clay (CL)	0.60	0.80	1.20	1.65
Highly Plastic Clay (CH)	0.65	0.80	1.10	1.40

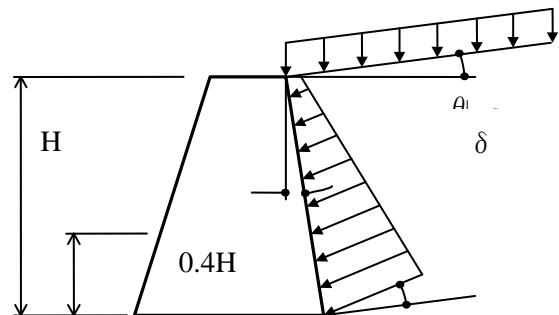
(3) **Active Lateral Earth Pressure Coefficient, K_a**

$$K_a = \frac{\cos^2(\phi - \theta)}{\Gamma_1 \left[\frac{\cos^2 \theta \sin(\phi + \delta) \sin(\phi - \alpha)}{\cos(\theta + \delta) \cos(\theta - \alpha)} \right]^2}$$

$$\Gamma_1 = \frac{1}{\sqrt{\cos(\theta + \delta) \cos(\theta - \alpha)}}$$

where:

δ = friction angle between fill and wall (deg)
 α = angle of fill to the horizontal (deg)
 θ = angle of back face of wall to the vertical (deg)
 ϕ = effective angle of internal friction (deg)



Earth Pressure

(4) **Passive Lateral Earth Pressure Coefficient, K_p**

For noncohesive soils, values of the coefficient of passive lateral earth pressure K_p may be taken from Figure 1 of Specification 22 TCN 272-05 for the case of a sloping or vertical wall with a horizontal backfill or from 3.11.5.4-2 of Specification 22 TCN 272-05 for the case of a vertical wall and sloping backfill.

For conditions that deviate from those described in Figures 1 and 2, the passive pressure may be calculated by using a trial procedure based on wedge theory. When wedge theory is used, the limiting value of the wall friction angle should not be taken larger than one-half the angle of internal friction, ϕ .

For cohesive soils, passive pressures may be estimated by:

$$pp = K_p * \gamma_s * g * z * 10^{-9} + 2c\sqrt{Kp} \quad \text{where:}$$

pp = lateral earth pressure (MPa)

γ_s = density of soil (kg/m³)

z = depth below the surface of soil (mm)

c = unit cohesion (MPa)

K_p = coefficient of passive lateral earth pressure

(5) **Seismic Active Earth Pressure Coefficient K_{ae}:**

Seismic active earth pressure P_{ae} shall be taken as:

$$P_{ae} = \frac{1}{2} g \gamma H^2 (1 - k_v) K_{ae} \times 10^{-9}$$

for which:

$$K_{ae} = \frac{\cos^2(\phi - \theta_o - \theta)}{\Gamma_2 \cos \theta_o \cos^2 \theta \cos(\delta + \theta + \theta_o)}$$

$$\Gamma_2 = \left[1 + \sqrt{\frac{\sin(\phi + \delta) \sin(\phi - \theta_o - \alpha)}{\cos(\delta + \theta + \theta_o) \cos(\alpha - \theta)}} \right]^2$$

where:

$$\theta_o = \arctan(kh / (1 - kv)) \quad (\text{deg})$$

kh = horizontal acceleration coefficient

kv = vertical acceleration coefficient

(6) **Seismic Passive Earth Pressure Coefficient K_{pe}:**

Seismic active earth pressure P_{ae} shall be taken as:

$$P_{pe} = \frac{1}{2} g \gamma H^2 (1 - k_v) K_{pe} \times 10^{-9}$$

for which:

$$K_{pe} = \frac{\cos^2(\phi - \theta_o + \theta)}{\Gamma_3 \cos \theta_o \cos^2 \theta \cos(\delta - \theta + \theta_o)}$$

$$\Gamma_3 = \left[1 - \sqrt{\frac{\sin(\phi + \delta) \sin(\phi - \theta_o + \alpha)}{\cos(\delta - \theta + \theta_o) \cos(\alpha - \theta)}} \right]^2$$

where:

$$\theta_o = \arctan(kh / (1 - kv)) \quad (\text{deg})$$

kh = horizontal acceleration coefficient

kv = vertical acceleration coefficient

1.3.7. Force Effects due to Superimposed Deformations: TU, TG, SH, CR, SE

(1) Uniform Temperature: TU

The maximum and minimum average bridge temperature specified in TVCN are shown in Table 21. The difference between the maximum and minimum average bridge temperature and the base construction temperature assumed in the design shall be used to calculate thermal deformation effects. These are based on shade air temperature ranges of 0 °C to +45 °C north of latitude 16° N (Hai Van Pass) and +5 °C to +45 °C south of latitude 16° N.

The setting temperature of the bridge shall be taken as the actual air temperature averaged over the 24-hour period immediately preceding the setting event.

These temperatures should be reviewed in considering with the meteorological data of the site.

Table 21 Bridge Temperature Ranges

Climate Zone	Concrete superstructure	Concrete deck on steel girders or box	Steel deck on steel girders or box
North of Latitude 16 Deg. N (Hai Van Pass)*	+5 °C to +47 °C	+1 °C to +55 °C	-3 °C to +63 °C
South of Latitude 16 Deg. N (Hai Van Pass)	+10 °C to +47 °C	+6 °C to +55 °C	+2 °C to +63 °C

*: For sites north of latitude 16° N and at an elevation above sea level greater than 700 m, the minimum temperature in the Figure 9 shall be reduced by 5 °C.

(2) Temperature Gradient: TG

The effect of vertical differential temperature gradients through a bridge superstructure shall be derived for both positive temperature differential conditions (top surface hotter) and negative temperature differentials (top surface cooler). Dimension “A” in Figure 9 shall be taken as;

For concrete superstructures that are 400 mm or more in depth ----- A = 300 mm:

For concrete sections shallower than 400 mm ----- A = 100 mm less than actual depth

For steel/concrete composite superstructures ----- t = depth of concrete deck

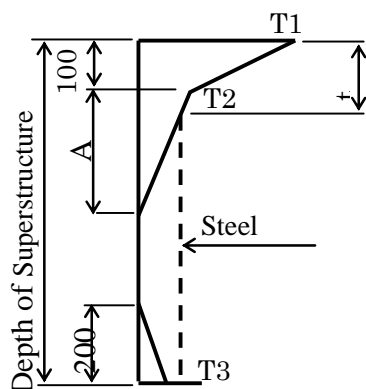


Figure 9 Vertical Temperature Gradient

The temperature gradients given in Table 22 apply to bridge decks with 100 mm thickness of surfacing. Where a different surfacing thickness is used, the values should be adjusted accordingly.

Table 22 Temperature Gradient (°C)

	T1	T2	T3
Positive	+23	+6	+3
Negative	-7	-1	0

(3) Creep and Drying Shrinkage

Influences of creep and drying shrinkage are varied from condition of the land, quality of the materials, dimension of the members, the ages, and the erection methods. To evaluate the differences of the ages varied from the erection methods accurately have to consider not only the amount of the final creep drying shrinkage but also the erection order and progress of the creep drying shrinkage. In this project, the regulations for creep drying shrinkage of CEB-FIP model code (MC78) mentioned in AASHTO 5.4.2.3.1 is applied.

1) Creep Coefficient

According to MC78, the total strain related the stress at t when the fixed continue stress is acted by t0 is shown in the following equation as the sum of elastic strain occurred at t0 and creep strain occurred from between t0 and t.

$$\Sigma \varepsilon (t, t_0) = (\sigma_0)/E_c(t_0) + (\sigma_0)/E_{c,28} * \phi (t, t_0)$$

- Where, $E_c(t_0)$: Elastic modulus for concrete of the age t0
 $E_{c,28}$: Elastic modulus for concrete of the age for 28days.
 $\phi (t, t_0)$: Creep strain progress between from t0 to t given in MC78.

According to the equation, elastic modulus for concrete decided the creep strain by MC78 have to use the elastic modulus for concrete of the age for 28days all the time.

The creep coefficient is decided by the following equation:

$$\phi_i = \phi_{d_0} * \beta_d (t'_i - t'_{i-1}) + \phi_{f_0} \{ \beta_f (t'_i) - \beta_f (t'_{i-1}) \}$$

- where, $\Delta\phi_i$: Creep coefficient progress between ti-1 and ti.
 ϕ_{d_0} : Basic creep coefficient 0.4 against late elastic strain.
 ϕ_{f_0} : Using the number of Table 23 according to the environmental condition by basic creep coefficient against the flow.
 $\beta_d(t'_i - t'_{i-1})$: Using the number of the Table 24 by the coefficient for the changes with times of late elastic strain.
 $\beta_f (t'_i)$: Using the number of Table 24 according to the theoretical thickness of materials by the coefficient for the changes with times of flow.
 t i : The ages of concrete after loading.
 t' i : Effective age fixed by the kinds of cement and surround average temperature during concrete hardening.
 $=\alpha/30\Sigma t_i (T(t_i) + 10)*t_i$
 α : Using the coefficient related the hardening speed of concrete.
 normal concrete: 1.0, high speed concrete: 2.0
 T(t i) : Average temperature of t i days.
 hth : $\lambda * A_0 / u$ (theoretical thickness)
 λ : Using the number of Table 23 by coefficient related the environmental condition.

A_o : Sectional area of the materials.
 u : Circumference length(m) of the connected section to the air of material section.

Table 23 $\phi f_o, \epsilon s_o$ and λ

Environment Condition		ϕf_o	$\epsilon s_o (\times 10^{-6})$	λ
Relative Humidity (%)	100 (Underwater)	0.8	-100	60
	90	1.3	100	10
	70	2.0	250	3
	40	3.0	400	2

Table 24 Coefficient $\beta d(t_i)$ and Coefficient $\beta f(t''i)$

Effective Age (Day)	$\beta d(t''i-t'-1)$	$\beta f(t''i)$ against hth (cm)					
		≤ 5	10	20	40	80	≥ 160
1	0.280	-	-	-	-	-	-
2	0.300	-	-	-	-	-	-
3	-	0.240	0.210	0.190	0.170	0.155	0.140
5	0.350	0.345	0.310	0.270	0.235	0.210	0.185
10	0.400	0.505	0.440	0.380	0.328	0.280	0.235
20	0.465	0.685	0.575	0.500	0.420	0.350	0.280
30	0.580	—	—	—	—	—	—
50	—	0.964	0.810	0.690	0.562	0.443	0.330
100	0.700	1.195	1.025	0.850	0.680	0.52	0.375
200	0.830	1.395	1.215	1.020	0.800	0.603	0.435
500	0.945	1.600	1.413	1.208	0.980	0.750	0.566
1000	0.985	1.698	1.514	1.320	1.107	0.884	0.703
2000	1.000	1.762	1.589	1.416	1.217	1.010	0.842
5000	1.000	1.820	1.660	1.510	1.330	1.148	1.000
10000	1.000	1.846	1.695	1.545	1.383	1.225	1.085
20000	1.000	1.850	1.700	1.500	1.400	1.250	1.120
∞	1.000	1.850	1.700	1.550	1.400	1.250	1.120

2) Drying Shrinkage

Drying shrinkage is decided by the following equation,

$$\epsilon_{cs,i} = \epsilon_{so} (\beta_s (t''i) - \beta_s) \text{ where,}$$

$$(t''i - 1)$$

$\Delta \epsilon_{cs,i}$: Drying shrinkage which progress during $t_i - 1$ to t_i
 ϵ_{so} :Using the number of Table 23 according to the basic drying shrinkage strain and environmental condition.

$\beta_s(t''i)$:Using the number of Table 25 according to the theoretical thickness of materials by the coefficient for the changes with times of drying shrinkage.

$t''i$:Effective age fixed by the surrounding average temperature during concrete hardening.

$$= 1/30 \sum t_i (T(t_i) + 10) * t_i$$

Table 25 Coefficient $\beta_s(t''i)$

Effective Age (Day)	Coefficient $\beta_s(t''i)$ against hth (cm)					
	≤ 5	10	20	40	80	≥ 160
1	0.110	0.040	0.010	0.0	0.0	0.0
2	0.170	0.080	0.020	0.0	0.0	0.0
5	0.290	0.160	0.055	0.005	0.005	0.0
10	0.420	0.240	0.100	0.005	0.020	0.0
20	0.560	0.340	0.160	0.060	0.030	0.0
50	0.760	0.510	0.270	0.120	0.055	0.010
100	0.900	0.650	0.375	0.185	0.085	0.020
200	1.020	0.780	0.490	0.260	0.120	0.045
500	1.100	0.910	0.660	0.410	0.210	0.090
1000	1.160	0.980	0.770	0.550	0.340	0.175
2000	1.190	1.040	0.840	0.660	0.500	0.310
5000	1.200	1.050	0.885	0.750	0.660	0.510
10000	1.200	1.050	0.895	0.790	0.725	0.640
20000	1.200	1.050	0.900	0.800	0.750	0.700
∞	1.200	1.050	0.900	0.800	0.750	0.700

(4) Friction Forces: FR

Forces due to friction on the sliding or rotating surface shall be considered. The value of friction coefficients are depend on the specification of each product. Based on the former projects in Vietnam, the friction coefficients of elastmetric bearing shall be as 0.15.

(5) Vessel Collision: CV

All bridges crossing navigable waterways shall be designed for vessel collision with the substructure and, where appropriate, the superstructure.

The Owner shall establish and/or approve the design vessel(s), design velocity, and any specific requirements for the bridge in consideration with the Vietnam Inland Waterways Bureau or the Vietnam Marine Authority, as appropriate.

1) Design Vessel

The design vessels and their dimensions are given for various classes of navigable waterway shown in Table 26 and Table 27.

Table 26 Design Vessels for Classes of Navigable Waterway

Class of navigable waterway	Design vessel tonnage (dwt)	
	Self-propelled vessel	Towed barge
I	2000	500
II	1000	500
III	300	400
IV	200	400
V	100	100
VI	40	100

Table 27 Dimensions of Design Vessels

	Self-propelled vessel						Towed barge		
	2000	1000	300	200	100	40	500	400	100
Maximum Length (LOA) (m)	90	75	38	34	15	8	40	41	27
Maximum Breadth (m)	12.0	10.5	7.0	6.6	5.0	3.0	10.0	11.2	6.4
Laden draught (m)	3.5	2.8	2.2	1.7	1.0	0.8	1.7	1.3	1.0

(6) Design Collision Velocity

The recommended design impact velocity, V, to be used with each design vessel shall be as given in Table 28, where:

Vs = mean annual stream velocity adjacent to the bridge element under consideration (m/s)

Table 28 Design Impact Velocity for Design Vessels

Design vessel	Design impact velocity, V (m/s)
Self-propelled Vessel >= 1000 DWT	3.3 + Vs
Self-propelled Vessel < 1000 DWT	2.5 + Vs
Towed Barge	1.6 + Vs

(7) Vessel Collision Energy

The kinetic energy of a moving vessel to be absorbed during a non-eccentric collision with a bridge pier shall be taken as:

$$KE = 500CHMV^2$$

where:

KE = Vessel collision energy (joule)

M = Vessel displacement tonnage (Mg)

The vessel mass, M, shall be based on the loading condition of the vessel and shall include the empty mass of the vessel, plus consideration of the mass of cargo, for loaded vessels, or the mass of water ballast for vessels transiting in an empty or lightly loaded conditions.

CH = Hydrodynamic mass coefficient

= 1.05 Underkeel clearance >= 0.5*draft

= interpolate

= 1.25 Underkeel clearance <= 0.1*draft

V = Vessel impact velocity (m/s)

(8) Ship Collision Force on Pier

The head-on ship collision impact force on a pier shall be taken as:

$$PS = 1.2 * 10^5 V (DWT)^{0.5}$$

where;

PS = Equivalent static vessel impact force

DWT = Deadweight tonnage of vessel

V = Vessel impact velocity

(9) Barge Collision Force on Pier

The collision impact force, N, on a pier for a standard hopper barge shall be taken as:

$PB = 6.0 \cdot 10^4 aB$ $= 6.0 \cdot 10^6 + 1600 aB$	$aB < 100 \text{ mm}$ $aB \geq 100 \text{ mm}$	where; PB = Equivalent static barge impact force (N) aB = Barge bow damage length specified in the following equation (mm) $aB = 3100 [(1 + 1.3 \cdot 10^{-7} KE)^{0.5} - 1]$
---	---	--

The impact force for design barges larger than the standard hopper barge shall be determined by increasing the standard hopper barge impact force by the ratio of the larger barge's width to the width of the standard hopper barge.

(10) Application of Impact Forces

1) Substructure Design

For substructure design, equivalent static force, parallel and normal to the centerline of the navigable channel, shall be applied separately as follows;

- * Parallel to the alignment of the centerline of the channel: 100% of the design impact force
- * Normal to the alignment of the centerline of the channel: 50% of the design impact force

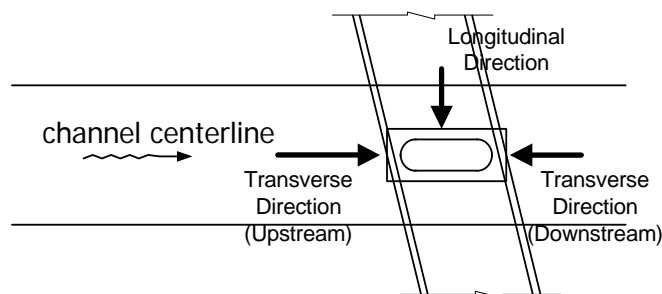


Figure 10 Application of Vessel Collision Force on Pier

The impact force shall be applied to a substructure in accordance with the following criteria:

- * For overall stability, design impact force is applied as a concentrated force on the substructure at the mean annual high water level of the waterway, as shown in Figure 11.
- * For local collision forces, design impact force is applied as a vertical line load equally distributed on the depth of the head block (HL), as shown in Figure 12.

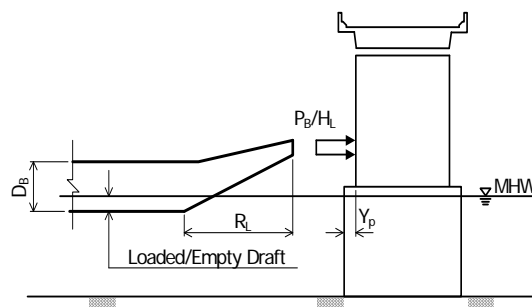
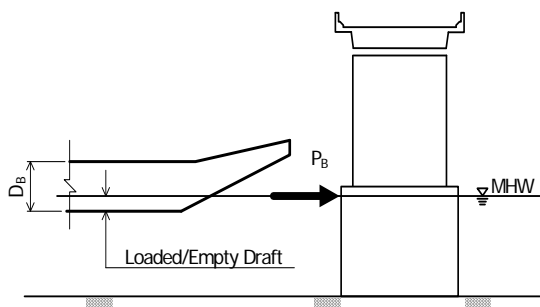


Figure 11 Load Application for Overall Stability Figure 12 Load Application for Local Collision

2) Superstructure

For superstructure design, design impact force may be applied as an equivalent static force transverse to the superstructure component in a direction parallel to the centerline of navigable channel, if necessary.

1.3.8. Prestress Force

The Prestressing force shall be computed using the equation as bellow.

$$P(x) = P_i - [\Delta P_i(x) + \Delta P_t(x)]$$

where,

$P(x)$:Prestressing force of cross section under consideration

P_i :Prestressing force at prestressing work at the tensioning end of tendon

$\Delta P_i(x)$:Loss of prestressing force immediately after prestressing to be computed considering the following effects

Elastic deformation of concrete

Friction between tendon and duct

Anchorage seating loss, or set loss

Others

$\Delta P_t(x)$:Loss of prestress force over time to be computed considering the following effects

Relaxation of prestressing steel

Creep of concrete

Shrinkage of concrete

To calculate the indeterminate forces at the serviceability or the fatigue limit state, the prestress force may be taken to be the characteristic value of the prestressing force.

[Commentary] (1) The following effects shall be considered when calculating the prestress losses, namely $\Delta P_i(x)$ and $\Delta P_t(x)$.

(1) Elastic deformation of concrete

The prestress loss due to elastic deformation of concrete shall always be considered for the pre-tensioning system. When post-tensioning tendons are tensioned one by one, the prestress loss due to elastic deformation of concrete shall be calculated, and the average prestress loss may be computed.

for pre-tensioning system $\Delta pEs = np \cdot f'_{cpg}$

for post-tensioning system $\Delta \sigma p = 1/2 np \cdot f'_{cpg} (N-1)/N$

where, $\Delta \sigma p$: Prestress loss in prestressing tendon

np : Young' modulus ratio of prestressing tendon to concrete = E_p / E_c

f'_{cpg} : Concrete compressive stress due to prestressing at the centroid of prestressing tendons

N : Number of tensioning times (number of groups of the tendon)

(2) Friction between prestressing tendon and duct

The prestress loss in prestressing tendon due to friction varies considerably on condition of the inner surface of the duct and type, degrees of rusting, and alignments of the prestressing tendon.

Loss of prestressing tendon force due to friction can generally be separated into two terms – one related to the angular change of the centroid line of the prestressing tendons, and the other related to length of the prestressing tendon. tension in the prestressing tendon at cross section under consideration can be expressed by following equation.

$$P_x = P_i \cdot e^{(\mu\alpha + \lambda x)}$$

where,

P_x : Tension force of tendon at considered cross section

P_i : Tension force at the tensioning end of tendon

μ : Friction coefficient for angular change of 1 radian

α : Angular change of the tendon in radian

λ : Friction coefficient per unit length

x : Length from the end of the tendon to the considered cross section

Though, μ and λ should be determined by site measurement, but values shown in next table may generally be used for calculation for prestressing force in a tendon encased in a sheath.

Table 29 Friction Coefficient

	μ	λ
Prestressing wire, Prestressing wire strand	0.3	0.004
Prestressing steel bar	0.3	0.003

Because external tendons are arranged outside of the concrete and are free from friction except at anchorages or deviators, friction loss at a section may not be considered. Depending on the material of the duct, the value of the coefficient of friction at anchorages or deviators given in following table may be used.

Table 30 Friction coefficient between prestressing wire strand and duct

	μ	λ
Steel	0.30	0.004
Polyethylene	0.15	0.004
(for prestressing wire and prestressing wire strand)		

The wobble coefficient may be determined on the basis of prior experimental or other data in case a special sheath or spacer is used in order to reduce the friction, or the prestressing steel is specially processed, or the friction is reduced by giving impact to the prestressing bar.

(3) Anchorage seating, or set

If during the anchoring of the tendon ‘set’ occurs, the ensuing loss in prestress shall be taken into account. Especially in the case of a wedge-type anchorage system, since the amount of set is relatively large, the loss of prestress and the affected length, shall be determined prior to tensioning on the basis of previous experience or available data. The “set” refers to the pulling in of a prestressing tendon at the anchoring device during anchoring. As the actual amount of set varies depending on the anchoring device used, the actual amount corresponding for each device shall be determined (See “Guidelines for Design and Construction of Prestressed Concrete Structures”, 1991, JSCE).

When there is no friction between the prestressing tendon and duct, loss of the prestressing tendon force due to set may be calculated using the following equation;

$$\Delta P = (\Delta \iota) / \iota A P E P$$

where, ΔP :Loss of tension force due to set of tendon
 $\Delta \iota$:Setting length
 ι :Length of tendon
 AP :Area of tendon
 EP :Young’s modulus of tendon

1.4. Pile Foundation Design

The bridge is located in the soft ground area, which bearing stratum is about -50 m. Therefore, the pile foundation shall be adopted for foundation type.

Pile foundation design shall be made for service limit states, strength limit states and extreme event limit states respectively. Each limit state include the followings:

Table 31 Verification Items for Limit States

	Verification Items	Remark
Service Limit States	Adequate Bearing resistance	Allowable Bearing Resistance
	Structural Resistance	Control of Cracking
	Tolerable Settlement	Considered Bridge Performance
	Tolerable Horizontal Displacement	
Strength Limit States	Adequate Bearing Resistance	Considered punching failure
	Structural Resistance	
	Horizontal Displacement	P-Y curve
Extreme Event Limit States	Bearing Resistance	
	Structural Resistance	
Service Limit state	Overall stability	Considered Lateral Flow

1.4.1. Transverse Spring Coefficients of Pile

Transverse Spring Coefficients of Pile for the analysis are shown in Table 32.

Table 32 Transverse Spring Coefficients of Pile

	$h \neq 0$	$h = 0$	$h \neq 0$	$h = 0$
K1	$\frac{12EI\beta^3}{(1 + \beta h)^3 + 2}$	$4EI\beta^3$	$\frac{3EI\beta^3}{(1 + \beta h)^3 + 0.5}$	$2EI\beta^3$
K2, K3	$K_1 \frac{\lambda}{2}$	$2EI\beta^2$	0	0
K4	$\frac{4EI\beta (1 + \beta h)^3 + 0.5}{1 + \beta h (1 + \beta h)^3 + 2}$	$2EI\beta$	0	0
$K_v =$	$\frac{A E_p}{L}$ (kN/m)	$\beta = \sqrt[4]{\frac{k_H D}{4EI}}$ (1/m) = characteristic value of a pile		
$\gamma =$		$h + \frac{1}{\beta}$		
kH =		coefficient of lateral ground spring (kN/m ³)		
D =		pile diameter (m)		
EI =		flexure rigidity of a pile (kN.m ²)		
h =		axial length of a pile above the ground level (m); if $h < 0$, $h = 0$		
a =		0.014(L/D) + 0.72 for driven pile by percussion 0.017(L/D) - 0.014 for driven pile by vibro hammer 0.031(L/D) - 0.15 for cast-in-place concrete pile		
Ap =		net area of a pile (mm ²)		
Ep =		modulus of elasticity of a pile (kN/mm ²)		
L =		pile length (m)		
D =		pile diameter (m)		

1.5. Materials

In this section, the specified values for the concrete, reinforcing bar and prestressing tendon on the TVCN and AASHTO are described. In the detail design, these values should be reviewed and modified if necessary.

1.5.1. Concrete

Although concrete strength for each structural element shall basically follow the Vietnamese Standard considering local conditions, they may be modified based on the AASHTO LRFD and Japanese Specifications for reasons of required properties. The followings are concrete strengths for each structural element to be used in this Project.

Table 33 Concrete Strength by Structural Member

Compressive Strength at 28 days (MPa) (Cylinder Specimen)	Structural Member	Remarks
50	Pretensioned Slab/Girder	During design period, these may be modified due to requirements.
45	Free Cantilever PC Girder	
40	Post-tensioned PC I-Girder Cast-in-situ PC Slab/Girder	
35	Cast-in-situ PC Slab Cast-in-situ PC Crossbeam	
28	RC Girder Diaphragm (Crossbeam) RC Deck Slab Substructure (Pier, Abutment, Pile Caps, Wingwall) Retaining Wall, Box Culvert Precast Reinforced Concrete Plate Precast Pile Precast Parapet	
21	Approach Slab Pipe Culvert Precast Concrete Curb	
30	Cast-in-situ Bored Pile	
18	Non-reinforced Concrete Structure Lean Concrete	

In this project, only normal density concrete shall be used. The properties of concrete are as shown below.

Table 34 Concrete Properties

Modulus of Elasticity (MPa)	Poisson's Ratio	Modulus of Rupture (MPa)
$E_c = 0.043\gamma_c^{1.5}\sqrt{f'_c} \quad (1440 \leq \gamma_c \leq 2500)$	0.20	$f_{r} = 0.63\sqrt{f'_c}$
γ_c = density of concrete (kg/m ³)		flexure tensile stress
f'_c = specified strength of concrete (MPa)		

Stress limits for concrete in Service Limit State in PC are shown in Tables 35 and 36. For RC, as the width of flexure cracks is controlled by distributing the reinforcement over the region of maximum concrete tension, stress limit for concrete is not described.

Table 35 Temporary Tensile Stress Limits in PC before Losses

Bridge Type	Location		Stress Limit (MPa)
Other Than Segmentally Constructed Bridges	*	In precompressed tensile zone without bonded reinforcement	Not Applicable
	*	In areas other than the precompressed tensile zone and without bonded reinforcement	$0.25\sqrt{f'_{ci}} \leq 1.38$
	*	In areas with bonded reinforcement (reinforcing bars or prestressing steel) sufficient to resist the tensile force in the concrete computed assuming an uncracked section, where reinforcement is proportioned using a stress of 0.5 fy, not to exceed 210 MPa.	$0.63\sqrt{f'_{ci}}$
	*	For handling stresses in prestressed piles	$0.415\sqrt{f'_{ci}}$

Table 36 Compressive Stress Limits in PC at Service Limit State after Losses

Location		Stress Limit (MPa)
*	In other than segmentally constructed bridges due to the sum of effective prestress and permanent loads	0.45 f'c
*	In other than segmentally constructed bridges due to live load and one-half the sum of effective prestress and permanent loads	0.40 f'c
*	Due to the sum of effective prestress, permanent loads, and transient loads and during shipping and handling	0.60 φw f'c

Table 37 Tensile Stress Limits in PC at Service Limit State after Losses

Bridge Type	Location		Stress Limit (MPa)
Other Than Segmentally Constructed Bridges	Tension in the Precompressed Tensile Zone Bridges, assuming uncracked sections		
	*	For components with bonded prestressing tendons or reinforcement that are subjected to not worse than moderate corrosion conditions	$0.50\sqrt{f'_{ci}}$
	*	For components with bonded prestressing tendons or reinforcement that are subjected to severe corrosive conditions	$0.25\sqrt{f'_{ci}}$
	*	For components with unbonded prestressing tendons	No tension

1.5.2. Reinforcing Bar

Two types of Grade 300 and Grade 420 shall be used. The properties and strength are as shown below.

Table 38 Properties and Stress Limit of Reinforcing Bars

Type	Yield Strength f _y (MPa)	Tensile Strength f _u (MPa)	Modulus of Elasticity (MPa)
Grade 300	300	500	200,000
Grade 420	420	620	200,000

1.5.3. Prestressing Steel

Uncoated, stress-relieved or low-relaxation, seven-wire strand, or uncoated plain or deformed, high-strength bars, shall have the following properties and strength as shown in Table 39.

Table 39 Properties of Prestressing Strand and Bar

Material	Grade or Type	Diameter (mm)	Tensile Strength fpu (MPa)	Modulus of Elasticity Ep (MPa)	Yield Strength fpy (MPa)
Strand	1725 MPa (Grade 250)	6.35 – 15.24	1725	197,000	0.85 f pu for stress-relieved 0.90 f pu for low-relaxation
	1860 MPa (Grade 270)	9.53 – 15.24	1860		0.90 f pu
Bar	Type 1, Plain	19 – 35	1035	207,000	0.85 f pu
	Type 2, Deformed	16 – 35	1035		0.80 f pu

Stress limits for each tendon type are as shown in Table 40.

Table 40 Stress Limits for Prestressing Tendons

	Tendon Type		
	Stress-relieved Strand / Plain high-strength bars	Low Relaxation Strand	Deformed High-strength Bars
Pretensioning			
* Immediately prior to transfer (f pt + Δf pES)	0.70 f pu	0.75 f pu	-
* At service limit state after all losses (f pe)	0.80 f py	0.80 f py	0.80 f py
Post-tensioning			
* Prior to seating—short-term fs may be allowed	0.90 f py	0.90 f py	0.90 f py
* At anchorages and couplers immediately after anchor set (f pt + Δf pES + Δ fpA)	0.70 f pu	0.70 f pu	0.70 f pu
* At end of the seating loss zone immediately after anchor set (f pt + Δf pES + Δ fpA)	0.70 f pu	0.74 f pu	0.70 f pu
* At service limit state after all losses (f pe)	0.80 f py	0.80 f py	0.80 f py

1.5.4. Durability of Concrete

(I) Environmental condition

It is considered extremely severe environment for a concrete structure that a region where saline moisture, seawater etc., are splashed or sprayed constantly, such as a location for pier at the coast.

It is because the increment of the density of salinity occurs in such a region where supply and dry of salt water being repeated.

It seems appropriate to take some counter measures, considering the following conditions;

The bridge crosses the sea.

It is an especially important structure.

The design service life shall be 100 years.

(2) **Proposal of counter measure concerning durability of bridge structures**

We would propose to provide following counter measures as recommendable.

- 1) The cover of the reinforcing bar of main girder is to be 45mm that is 10mm more than the designed cover.
- 2) The cover of the reinforcing bar of pier & abutment is to be 40mm when the structure is under water, and is to be 60mm when the structure is at elevations of tidal water.
- 3) Painted reinforcing bars, performance of which is equal to or greater than that of epoxy painted reinforcing bars, shall be used for the reinforcing bars at the most outside perimeter of structure, excluding that of upper slab structure.

Regarding 1)

The standard cover is 35mm according to the Specifications for Highway Bridges of the Japan Road Association.

Here, 45mm cover would be proposed with additional thickness of 10mm, in consideration of a margin for the construction error etc.

Regarding 2)

The standard cover is 40mm and 60mm according to the Vietnamese standard TCXDVN 327:2004.

Regarding 3)

It was anticipated from the beginning stage that the structure would be affected by saline splash. As for the Dinh Vu-Cat Hai Bridge, it would be proposed to adopt the painted reinforcing bars for the most outside perimeter bars so as to secure the durability which is equal to or greater than that of above-mentioned bridge.

The bars painted with "Magne line", which is categorized as polyacrylic ester paint, may be adopted as the said painted reinforcing bar.

Regarding "Magne line"

This material is widely adopted as coating material on the surface of concrete and rust prevention material for steel material, and it retains the following features;

Certain rust prevention action

The salt water atomization tests have been performed by official body for the steel material painted with the said material.

The result is obtained as "Rust will not be generated after 4000 hours of the salt water atomization".

This result excels the required standard of the epoxy painting reinforcing bars to the same tests, which is "Rust shall not be generated by 1000 hours or more and within 1100 hours under the salt water atomization "

Therefore, it could be expected that the performance of reinforcing bars painted with the said material would be more than that of the epoxy painted reinforcing bars for the rust prevention of the steel material when the said material is applied and spread on to steel surface.

Excellent adhesion strength

The adhesive pulling out test also have been performed by the official body, for the reinforcing bars painted with Mange line and for the reinforcing bars without paint.

Result shows, the reinforcing bars painted with Magne Line has approximately 1.4 times larger adhesive strength comparing to bare reinforcing bars.

Therefore, it could be expected that the reinforcing bar painted with this material has more than or the equal adhesion strength to bare reinforcing bar.

Mechanism of rust prevention

The epoxy painted reinforcing bar secures the effect of rust prevention by intercepting salinity. Therefore, there is no effect of rust prevention, once salinity invades from the pinhole etc. In addition, once rust would be generated, it is considered that behavior to pull the epoxy painting apart from the inside of the paint will occur.

On the other hand, as for the mechanism of the rust prevention of reinforcing bar painted with Magne-Line, the stable rust formed on the surface of the reinforcing bar secures rust prevention effect. Therefore, if salinity invades from the pinhole and rust is generated, it is less likely to extend its surrounding part. Thus, it can be said that reinforcing bar painted with Magne-Line has grater performance against the unanticipated scratch comparing than the epoxy painted reinforcing bar.

It is therefore considered that the durability of a main girder can be secured by applying the counter measures of 1), 2) & 3) as stated above

(3) Regarding execution management

It is necessary to provide the construction management standard, since the said material shall be applied at the site. For this purpose, the quality standard of the epoxy painting reinforcing bars

specified by Japan Society of Civil Engineers shall be adopted.

And that can be defined by "The number of pinholes in 1 m shall be no more than 5 for D19 or smaller diameter, and no more than 8 for D22 or bigger diameter." The standard for D19 refers to D20.

< Reference literature >

"Concrete and reinforced concrete structures – Requirements of Protection from Corrosion in Marine Environment" TCXDVN 327:2004.

"Preliminarily design & construction guideline for the reinforced concrete which utilize epoxy resin painting " Japan Society of Civil Engineers: November, 2003

1.5.5. Steel Pipe

Two types of steel pipe Grade SKK400 and Grade SKK 490 based on the Japanese Standard JIS 5525 or equivalent international standard shall be used. The properties and strength are as shown below.

Table 41 Properties and Stress Limit of Steel Pipe

Type	Yield Strength f_y (MPa)	Tensile Strength f_u (MPa)	Modulus of Elasticity (MPa)
Grade SKK 400	235	400	200,000
Grade SKK 490	315	490	200,000

Appendix 4: List of Construction Equipment

For reference, list of construction equipment for the smooth implementation of the construction works are as follows but not limited to. Number, capacity and specifications of each equipment will be optimized when the scale of contract package is finalized.

1. General

- Air Compressor
- Bar Bending Machine
- Cargo Truck
- Center Hole Jack
- Forklift
- Four Wheel Drive Car (Jeep)
- Fuel Truck
- Generator
- High Pressure Pump
- Lighting Tower
- Material Testing Laboratory
- Submersible Pump
- Truck Crane
- Vehicle
- Water Tanker
- Welding Generator
- Welding Machine

2. Road Works

- Backhoe
- Bulldozer
- Dump Truck
- Motor Grader
- Wheel Loader
- Tired Roller

3. Bridge Works (On Land)

Clamshell
Concrete Batching Plant
Concrete Pump
Concrete Pump Truck
Concrete Vibrator
Crawler Crane
Diesel Hammer
Engine Welder
Erection Girder
Form & Fabrication Facility for PC-BOX Segment
Gantry Crane
PC Grout Mixer
PC Grout Pump
Piling Machine
Post Tensioning Stressing Jacks & Pump
Steel Launching Girder or Overhead Gantry Crane with Legs
Strand Pushing Machine for PC Cables
Trailer Segment Transport
Vibration Hammer

4. Offshore Work

Barge
Crawler Crane
Diesel Hammer
Passenger Boat
Speed Boats
Tug Boat
Vibration Hammer

5. Softground Works

Boring Machine
Crawler Crane

6. Pavement Works

Asphalt Plant

Asphalt Paver / Finisher

Asphalt Distributer

Pneumatic Tired Roller

Tamper (Plate Compactor)

Three Wheel Roller

Traffic Lane Marker

Vibrating Roller

Wheel Loader

Appendix-5: Annual Fund Requirement

The disbursement schedule has been prepared as shown in Table 1, in accordance with the implementation program of the project, described in Section 2.9.

Table 1 Annual Fund Requirement

(1) Eligible Portion

(F/C&Total+ Million JPY, L/C: Billion VND)

Item	Yearly Cost during Construction Period																					Total (Million JPY)					
	2011			2012			2013			2014			2015			2016			2017			F/C	L/C	Total			
	F/C	L/C	Total	F/C	L/C	Total	F/C	L/C	Total	F/C	L/C	Total	F/C	L/C	Total	F/C	L/C	Total	F/C	L/C	Total						
(1) Construction Cost	0	0	0	1,064	759	5,104	3,192	2,278	15,311	1,064	759	5,104	0	0	0	0	0	0	0	0	0	0	0	0	5,320	3,797	25,518
(2) Price Contingency = (1) x {Yearly Price index}	0	0	0	39	164	914	175	779	4,319	79	365	2,018	0	0	0	0	0	0	0	0	0	0	0	0	293	1,308	7,251
(3) Physical contingency = {(1)+(2)} x 5%	0	0	0	55	46	301	168	153	981	57	56	356	0	0	0	0	0	0	0	0	0	0	0	0	281	255	1,638
sub-total = (1)+(2)+(3)	0	0	0	1,158	970	6,318	3,536	3,210	20,611	1,200	1,180	7,478	0	0	0	0	0	0	0	0	0	0	0	0	5,893	5,360	34,407
(4) Consulting Services for Construction Supervision	0	0	0	156	18	249	467	53	747	156	18	249	0	0	0	0	0	0	0	0	0	0	0	0	779	88	1,245
(5) Interest during Consturction	0	0	0	12	10	64	29	26	166	8	7	47	0	0	0	0	0	0	0	0	0	0	0	0	48	43	277
(6) Commitment Charge	36	0	36	36	0	36	36	0	36	36	0	36	36	0	36	36	0	36	36	0	36	36	0	36	251	0	251
Total = (1)+(2)+(3)+(4)+(5)+(6)	36	0	36	1,361	997	6,666	4,067	3,288	21,560	1,399	1,205	7,810	36	0	36	36	0	36	36	0	36	36	0	36	6,972	5,490	36,180

(2) Non Eligible Portion (For Reference Only)

Item	Yearly Cost during Construction Period																					Total (Million JPY)					
	2011			2012			2013			2014			2015			2016			2017			F/C	L/C	Total			
	F/C	L/C	Total	F/C	L/C	Total	F/C	L/C	Total	F/C	L/C	Total	F/C	L/C	Total	F/C	L/C	Total	F/C	L/C	Total						
(7) Environmental Management and Monitoring Cost	0	140	745	0	146	777	0	22	117	0	6	32	0	0	0	0	0	0	0	0	0	0	0	0	0	314	1,671
(8) Administration Cost	0	21	112	0	49	261	0	210	1,120	0	70	373	0	0	0	0	0	0	0	0	0	0	0	0	0	351	1,866
(9) Value Added Tax (VAT)	0	0	0	0	123	657	0	401	2,136	0	145	773	0	0	0	0	0	0	0	0	0	0	0	0	0	670	3,565
(10) Import Tax	0	0	0	0	20	106	0	60	319	0	20	106	0	0	0	0	0	0	0	0	0	0	0	0	0	100	532
Total = (7)+(8)+(9)+(10)	0	161	857	0	339	1,801	0	694	3,692	0	241	1,284	0	0	0	0	0	0	0	0	0	0	0	0	0	1,435	7,634
Grand Total Annual Requirement = sum{(1):(10)}	36	161	893	1,361	1,336	8,468	4,067	3,982	25,251	1,399	1,446	9,094	36	0	36	36	0	36	36	0	36	36	0	36	6,972	6,925	43,814

Note:

1) Exchange rate: USD1=VND 17,002 = JPY 90.50
VND1=JPY 0.00532

2) Price escalation Rate: Yearly Price Index (Index2010=100)

	2010		2011		2012		2013		2014	
	F/C	L/C	F/C	L/C	F/C	L/C	F/C	L/C	F/C	L/C
*Detailed information is to be referred to Sectase Year:	1.8%	per year								
F/C portion:	1.8%	per year								
L/C portion:	10.3%	per year	100	100	101.8	110.3	103.63	121.66	105.50	134.19
Price Index										

3) Physical Contingency Rate: 5.0%

4) Interest during Construction
Construction Cost: 0.2% per year
Consulting Services 0.01% per year

5) Commitment Charge
Loan Amount x number of years of the disbursement period x 0.1%

6) Environmental Management and Monitoring Cost includes land acquisition and resettlement, livelihood restoration plan, HIV prevention program, environmental management program, environmental monitoring program.

*Detailed information is to be referred to Section 3.4.3

7) Administration Cost
5% of the sum of construction cost, price contingency, physical contingency, consulting services and environmental management and monitoring cost

8) Value Added Tax
10% of amount of JICA loan eligible portion was estimated as value added tax (VAT) in this Study.

9) Import Tax
10% in average (assumed average rate; the rates of import tax are different item by item.)

Appendix-6: Cost Data

(Reference Only)

The construction cost estimated in this study, as approximately JPY 25.5 Billion, is tentative and subject to update.

1. BREAKDOWN OF CONSTRUCTION COST

The breakdown of the construction cost is shown in the tables hereafter.

Table 1 Breakdown of Construction Cost (Total)

Section	Construction Works	Construction Cost (in VND)	Other construction Cost(2%)	Total Amount (VND)	Construction Cost (in JPY)	
Temporary works	Other Cost(Temporary houses for management)	103,455,267,757	—		550,382,024	
	Temporary road(Embankment)	273,200,373,348	5,464,007,467		1,453,425,986	
	Temporary Jetty	97,857,000,000	1,957,140,000		520,599,240	
	Temporary Road for Hai An side Road Work	78,303,298,368	1,566,065,967		416,573,547	
	Temporary works Subtotal=	552,815,939,473	8,987,213,434	561,803,152,907	2,940,980,798	
Road Hai An Side	Embankment	74,714,931,346	1,494,298,627		397,483,435	
	Approach Road (Soft Soil Treatment)	415,169,708,824	8,303,394,176		2,208,702,851	
	Pavement	56,495,133,094	1,129,902,662		300,554,108	
	Traffic Safety	11,427,717,516	228,554,350		60,795,457	
	Culvert	19,071,956,260	381,439,125		101,462,807	
	Cam box culvert	22,171,111,326	443,422,227		117,950,312	
	Tan Vu Interchange	268,618,947,329	5,372,378,947		1,429,052,800	
		Hai An side road works TOTAL=	867,669,505,695	17,353,390,114	885,022,895,809	4,616,001,770
Bridge Hai An Side	Approach Bridge(1)(2)(3)	928,244,566,180	18,564,891,324		4,938,261,092	
	Flyover Bridge(1)(2)	140,910,008,951	2,818,200,179		749,641,248	
		776,661,624,819	15,533,232,496		4,131,839,844	
		105,492,692,744	2,109,853,855		561,221,125	
		Approach Bridge+Flyover Bridge TOTAL=	1,951,308,892,695	39,026,177,854	1,990,335,070,549	10,380,963,309
	Approach Road to the end of behind abutment	16,423,668,452	328,473,369		87,373,916	
	Retaining Wall	212,289,351,224	4,245,787,024		1,129,379,349	
		Retaining Wall+App.Road TOTAL=	228,713,019,676	4,574,260,394	233,287,280,069	1,216,753,265
	Bridge+Retaining wall=	2,180,021,912,370	43,600,438,247	2,223,622,350,618	11,597,716,574	
Main Bridge	Superstructure(PC-BOX)	175,550,168,179	3,511,003,364		933,926,895	
	Substructure	484,533,313,527	9,690,666,271		2,577,717,228	
		Subtotal(3)=	660,083,481,706	13,201,669,634	673,285,151,340	3,511,644,123
Bridge Cat Hai Side	Approach Bridge	120,899,219,631	2,417,984,393		643,183,848	
		122,443,344,242	2,448,866,885		651,398,591	
		Sub Total(4)=	243,342,563,873	4,866,851,277	248,209,415,150	1,294,582,440
	Approach Road to the end of behind abutment	6,625,654,160	132,513,083		35,248,480	
	Retaining Wall	84,302,380,353	1,686,047,607		448,488,663	
		Approach Bridge Total=	90,928,034,513	1,818,560,690	92,746,595,203	483,737,144
	Cat Hai Side Total=	334,270,598,386	6,685,411,968	340,956,010,354	1,778,319,583	
	Bridge Total=	3,174,375,992,463	63,487,519,849	3,237,863,512,312	16,887,680,280	
Road Cat Hai Side	Embankment	141,290,225,418	2,825,804,508		751,663,999	
	Approach Road (Soft Soil Treatment)	356,657,461,383	7,133,149,228		1,897,417,695	
	Pavement	116,207,888,267	2,324,157,765		618,225,966	
	Traffic Safety	17,768,417,889	355,368,358		94,527,983	
	Culvert	49,433,224,998	988,664,500		262,984,757	
		TOTAL=	681,357,217,955	13,627,144,359	694,984,362,314	3,624,820,400
	GRAND TOTAL=	5,276,218,655,586	103,455,267,757	5,276,218,655,586	28,069,483,248	
	Exclude VAT=	4,796,562,414,169			25,517,712,043	

Table 2 Breakdown of Construction Cost (Temporary Works)

BILL ITEM	No. of UNIT Price	Code of Norm	Item	UNIT	UNIT PRICE	Quantities	Amount (VND)
TEMPORARY WORKS							
			Other Cost	LS	Calculated in the TOTL sheet		
			Fill of Stone	m ³	299,236	248,688.0	74,416,402,368
			Crushed stone 2*4	m ³	404,885	9,600.0	3,886,896,000
			Temporary road for Road Hai An Side				78,303,298,368
			Temporary Jetty	m ²	4,500,000	21,746.0	97,857,000,000
			Embankment	m ³	299,236	912,993.0	273,200,373,348

Note: VAT is included

Table 3 Breakdown of Construction Cost (Road Portion in Hai An Side)

BILL ITEM	No. of UNIT Price	Code of Norm	Item	UNIT	UNIT PRICE	Quantities	Amount (VND)
HAI AN side ROAD							599,050,558,367
A	1.1 EMBANKMENT						74,714,931,346
	15-HA		Excavation of soil	m ³	59,987	2,375.2	142,481,122
	17-HA		Excavation of organic soil	m ³	42,867	88,406.2	3,789,708,575
	20-HA		Embankment of sand, K=0.95	m ³	136,637	357,207.2	48,807,720,186
	22-HA		Embankment of sand, K=0.98	m ³	140,617	63,716.1	8,959,566,834
	23-HA		Embankment of Clay (Slope Protection)	m ³	263,825	46,708.3	12,322,817,248
	51-HA		Sodding (Slope Protection)	m ³	14,829	46,708.3	692,637,381
A	1.2 APPROACH ROAD (Soft Soil Treatment)						415,169,708,824
	28-HA		Geotextile Filter Fabric (non-woven)	m ²	18,536	259,178.3	4,804,128,969
	30-HA		Geotextile Filter Fabric (woven)	m ²	57,870	93,390.8	5,404,525,596
	21-HA		Sand Blanket (medium sand)	m ³	383,737	266,363.9	102,213,683,894
	27-HA		Sand Drain (D400)	m	141,774	1,798,841.4	255,028,940,644
	20-HA		Embankment of sand for compensation	m ³	136,637	251,710.5	34,392,967,589
	20-HA		Embankment of sand for compensation	m ³	136,637	179,569.8	24,535,878,763
	18-HA		Removal of surcharge	m ³	8,481	179,569.8	1,522,931,474
	31-HA		Settlement Plate by steel 0.8*0.8*	each	2,355,676	105.0	247,345,980
	32-HA		Wooden Pile 10*10*170cm	each	25,162	280.0	7,045,360
			Piezometer, observation well, inc	set	85,000,000	6.0	510,000,000
			Reusable sand (surcharge)	m ³	83,519	-161,612.8	-13,497,739,443
A	1.3 PAVEMENT						56,495,133,094
	33-HA		Fine Asphalt Concrete -5cm	m ²	156,560	72,421.8	11,338,362,325
	39-HA		Tack Coat 0.5kg/m ²	m ²	10,351	72,421.8	749,638,403
	35-HA		Medium Asphalt Concrete-7cm	m ²	205,245	72,421.8	14,864,219,312
	38-HA		Prime Coat 1.0kg/m ²	m ²	18,340	72,421.8	1,328,216,435
	36-HA		Aggregate Base-15cm	m ³	451,362	15,151.4	6,838,766,207
	37-HA		Aggregate Subbase-43cm	m ³	403,549	43,434.0	17,527,747,266
	28-HA		Geotextile Filter Fabric (non-woven, 25kn/m)	m ²	36,642	105,021.1	3,848,183,146
A	1.4 TRAFFIC SAFETY						11,427,717,516
	45-HA		Guide Posts	each	118,088	823.0	97,186,424
	46-HA		Kilometer Posts	each	394,478	3.0	1,183,434
	43-HA		Regulatory Signs	each	1,563,626	6.0	9,381,756
	41-HA		Information and Guidance signs	each	5,493,688	6.0	32,962,128
	40-HA		Area Reflection Pavement Marking	each	260,842	5,738.4	1,496,815,733
	3-HA		Guardrail	m	1,270,022	748.8	950,992,474
	49-HA		Reflectorized Pavement Stud	each	68,702	1,099.0	75,503,498
	2-HA		Concrete curb	m	175,009	8,208.1	1,436,491,373
	50-HA		Planting	each	278,300	1,369.0	380,992,700
			Lighting Pole-Single Arms	pole	30,000,000	216.0	6,480,000,000
	51-HA		Sodding	m ²	14,829	2,626.6	38,949,851
	23-HA		Embankment of clay	m ³	263,825	1,179.9	311,287,118
	19-HA		Organic soil	m ³	100,347	1,155.7	115,971,028
A	1.5 Culvert						19,071,956,260
	5-HA		RC Pipe Culvert-D2.0m	m	12,488,327	43.0	536,998,061
			RC Box Culvert-3m*3m(Km0+950)	m	144,690,939	46.6	6,742,597,757
			RC Box Culvert-3*4m*4m(Km4+100)	m	405,235,754	29.1	11,792,360,441
A	1.6 Cam Box Culvert BTCT 8*(4*4)m (Km1+697.6)						22,171,111,326
	102-HA		Concrete of box culvert, wall 28MPa	m ³	2,652,255	1,912.0	5,071,111,560
	100-HA		Reinforcement of box culvert, wall	ton	19,769,693	315.8	6,244,059,837
	106-HA		Lean Concrete	m ³	1,723,811	144.0	248,228,784
	63-HA		Billing Stone	m ³	672,724	119.0	80,054,156
	65-HA		Masonry	m ³	1,036,880	44.0	45,622,720
	72-HA		Concrete of approach slab, 28MPa	m ³	2,232,178	38.0	84,822,764
	73-HA		Reinforcement of approach slab	ton	20,878,827	4.7	98,339,275
	34-HA		Pavement(Fine,asphalt concrete-7cm)	m ²	218,252	1,360.0	296,822,720
	97-HA		Water proofing layer	m ²	222,000	1,360.0	301,920,000
	21-HA		Embankment of drainage material	m ³	383,737	2,120.0	813,522,440
	107-HA		Excavation of soil for foundation pit	m ³	68,819	1,770.0	121,809,630
	164-HA		Drive test pile 35*35cm (2piles)	m	1,439,008	80.0	115,120,640
	164-HA		Drive test pile 35*35cm	m	1,351,512	6,400.0	8,649,676,800

Note: VAT is included

Table 4 Breakdown of Construction Cost (Tan Vu Interchange)

BILL ITEM	No. of UNIT Price	Code of Norm	Item	UNIT	UNIT PRICE	Quantites	Amount (VND)
TAN VU INTERCHANGE							268,618,947,329
B	2.1 EMBANKMENT						43,242,017,435
	17-HA		Excavation of organic soil	m ³	42,867	38,797.7	1,663,141,006
	20-HA		Embankment of sand, K=0.95	m ³	136,637	194,954.9	26,638,052,671
	22-HA		Embankment of sand, K=0.98	m ³	140,617	21,467.9	3,018,751,694
	23-HA		Embankment of Clay (Slope Protection)	m ³	263,825	42,784.5	11,287,620,713
	51-HA		Sodding (Slope Protection)	m ³	14,829	42,784.5	634,451,351
B	2.2 Soft Soil Treatment						191,356,707,790
	28-HA		Geotextile Filter Fabric (non-woven 12kN/m)	m ²	18,536	190,356.2	3,528,442,523
	21-HA		Sand Blanket (medium sand)	m ³	383,737	145,959.0	56,009,868,783
	27-HA		Sand Drain (D400)	m	141,774	758,248.4	107,499,908,662
	20-HA		Embankment of sand for compensation	m ³	136,637	132,795.8	18,144,819,725
	20-HA		Embankment of sand for compensation	m ³	136,637	76,882.3	10,504,966,825
	18-HA		Removal of surcharge	m ³	8,481	76,882.3	652,038,786
	31-HA		Settlement Plate by steel 0.8*0.8*	each	2,355,676	153.0	360,418,428
	32-HA		Wooden Pile 10*10*170cm	each	25,162	408.0	10,266,096
			Piezometer, observation well, inc	set	85,000,000	5.0	425,000,000
			Reusable sand (surcharge)	m ³	83,519	-69,194.1	-5,779,022,038
B	1.3 PAVEMENT						28,800,925,874
	1-HA		Pavement areas	m ²	631,726	42,935.7	27,123,598,018
	28-HA		Geotextile Filter Fabric (non-woven, 25kN/m)	m ²	36,642	45,776.1	1,677,327,856
B	1.4 TRAFFIC SAFETY						5,219,296,230
	45-HA		Guide Posts	each	118,088	220.0	25,979,360
	43-HA		Regulatory Signs	each	1,563,626	17.0	26,581,642
	41-HA		Information and Guidance signs	each	5,493,688	6.0	32,962,128
	40-HA		Area Reflection Pavement Marking	each	260,842	2,293.3	598,188,959
	3-HA		Guardrail	m	1,270,022	946.9	1,202,583,832
	49-HA		Reflectorized Pavement Stud	each	68,702	524.0	35,999,848
	2-HA		Concrete curb	m	175,009	1,700.3	297,567,803
	50-HA		Planting	each	278,300	611.0	170,041,300
			Lighting Pole-Single Arms	pole	30,000,000	69.0	2,070,000,000
	51-HA		Sodding	m ²	14,829	5,497.7	81,525,393
	23-HA		Embankment of clay	m ³	263,825	1,649.3	435,126,573
	19-HA		Organic soil	m ³	100,347	2,419.0	242,739,393

Note: VAT is included

Table 5 Breakdown of Construction Cost (Approache Road in Hai An Side)

BILL ITEM	No. of UNIT Price	Code of Norm	Item	UNIT	UNIT PRICE	Quantites	Amount (VND)
APPROACH ROAD & RETAINING WALL HAI AN SIDE							228,713,019,676
	2.1 EMBANKMENT						16,423,668,452
	20-HA		Embankment of sand, K=0.95	m ³	136,637	52,024.0	7,108,403,288
	22-HA		Embankment of sand, K=0.98	m ³	140,617	6,296.0	885,324,632
	23-HA		Geotextile Filter Fabric (non-woven, 25kN/m)	m ²	36,642	12,970.0	475,246,740
			Pavement Structure	m ²	631,726	12,592.0	7,954,693,792
	2.2 RETAINING WALL						212,289,351,224
			Concrete of Retainingwall, 28MPa	m ³	2,240,947	22,012.0	49,327,725,364
			Reinforcement of retaining wall	ton	18,021,434	1,761.0	31,735,006,395
			Lean Concrete	m ³	1,723,811	1,269.0	2,187,516,159
			RC Piles 35*35cm	m	1,351,512	81,198.0	109,740,071,376
			Auxiliary	%	192,990,319,294	0.1	19,299,031,929

Note: VAT is included

Table 12 Breakdown of Construction Cost (Approache Bridge in Cat Hai Side)

BILL ITEM	No. of UNIT Price	Code of Norm	Item	UNIT	Quantities	UNIT PRICE (VND)	UNIT PRICE (JP¥)	Amount (VND)	Amount (JP¥)	Amount (in VND)	Amount (in JP¥)
Cat Hai Side APPROACH BRIDGE L=519.2m (off shore)											
								(VND)	(JP¥)	0.00532	1,294,582,440
A	Super Structure										
			Box girder 45MPa for Box Girder bridge	m ³	4,731.0	16,382,949	13,402	77,507,731,719	63,404,862	89,425,938,862	475,745,995
			High Strength cable, transverse	ton	25.0	46,998,431		1,174,960,775	0	1,174,960,775	6,250,791
			Transverse Anchor	set	692.0	1,146,867		793,631,964	0	793,631,964	4,222,122
	72-HA		Concrete of deck, curb 28MPa	m ³	373.0	2,232,178		832,602,394	0	832,602,394	4,429,445
			Reinforcement of deck, curb	ton	37.0	22,546,530		834,221,610	0	834,221,610	4,438,059
	34-HA		Asphalt concrete of bridge deck	m ²	6,490.0	228,603		1,483,633,470	0	1,483,633,470	7,892,930
	96-HA		Metal Railing	m	1,038.0	2,058,567		2,136,792,546	0	2,136,792,546	11,367,736
			Bearing	each	32.0	5,516,875		176,540,000	0	176,540,000	939,193
			Water proofing layer	m ²	6,490.0	436,000		2,829,640,000	0	2,829,640,000	15,053,685
			Bridge name sign	each	1.0	1,366,116		1,366,116	0	1,366,116	7,268
			Expansion Joint	m	14.0	15,862,554		222,075,756	0	222,075,756	1,181,443
			Lighting Pole -Single Arms	each	26.0	30,000,000		780,000,000	0	780,000,000	4,149,600
	98-HA		Cast iron drain pipe D150	set	130.0	445,740		57,946,200	0	57,946,200	308,274
			Auxiliary (20%)	%	0.2			17,766,228,510	12,680,972	20,149,869,939	107,197,308
										0	0
A	Substructure										
								56,603,387,738	350,268,569	122,443,344,242	651,398,591
	102-HA		Concrete of Abutment, pier, 28MPa (Under W	m ³	2,741.0	3,564,274		9,769,675,034		9,769,675,034	51,974,671
	59-CH		Reinforcement of abutment, pier	ton	244.0	21,966,562		5,359,841,128		5,359,841,128	28,514,355
	62-CH		Lean Concrete	m ³	81.0	1,741,595		141,069,195		141,069,195	750,488
	81-CH		Blinding stone	m ³	161.0	737,139		118,679,379		118,679,379	631,374
			Steel Pipe Pile	ton	1,357.0	1,785,438	224,452	2,422,839,366	304,581,364	59,674,975,456	317,470,869
	63-CH		Foundation Excavation	m ³	3,161.0	318,747		1,007,559,267		1,007,559,267	5,360,215
	80-CH		Embankment of drainage material	m ³	1,374.0	389,875		535,688,250		535,688,250	2,849,861
			Auxiliary (15%)	%	0.15			2,903,302,743	45,687,205	11,491,123,156	61,132,775
	86-CH		Sheet Pile (=11.12%)	ton	903.0	24,798,638		22,393,170,114		22,393,170,114	119,131,665
	87-CH		Driving steel sheet pile	m	8,603.0	416,961		3,587,115,483		3,587,115,483	19,083,454
	88-CH		Pulling sheet pile	m	8,603.0	205,276		1,765,989,428		1,765,989,428	9,395,064
	84-CH		Face timpering (=19.0%)	ton	181.0	22,206,791		4,019,429,171		4,019,429,171	21,383,363
	84-CH		Manufacture of Face Timperring	ton	181.0	7,034,461		1,273,237,441		1,273,237,441	6,773,623
	85-CH		Installation and Removal of Face timpering	ton	181.0	7,214,319		1,305,791,739		1,305,791,739	6,946,812

Note: VAT is included

Table 13 Breakdown of Construction Cost (Approache Road in Cat Hai Side)

BILL ITEM	No. of UNIT Price	Code of Norm	Item	UNIT	UNIT PRICE	Quantities	Amount (VND)
APPROACH ROAD & RETAINING WALL CAT HAI SIDE							90,928,034,513
2.1EMBANKMENT							6,625,654,160
			Embankment of sand, K=0.95	m ³	190,170	15,117.0	2,874,799,890
			Embankment of sand, K=0.98	m ³	194,243	2,500.0	485,607,500
			Geotextile Filter Fabric (non-woven, 25kN/m ²)	m ²	36,659	5,149.0	188,757,191
			Pavement Structure	m ²	615,421	4,999.0	3,076,489,579
2.2 RETAINING WALL							84,302,380,353
			Concrete of Retainingwall, 28MPa	m ³	2,248,743	8,492.0	19,096,325,556
			Reinforcement of retaining wall	ton	18,025,372	679.3	12,245,031,758
			Lean Concrete	m ³	1,741,595	504.0	877,763,880
			RC Piles 35*35cm	m	1,353,921	32,234.0	43,642,289,514
			Metal Railing	m	2,061,318	377.0	777,116,886
			Auxiliary	%	76,638,527,594	0.1	7,663,852,759

Note: VAT is included

Table 14 Breakdown of Construction Cost (Road work in Cat Hai Side)

BILL ITEM	No. of UNIT Price	Code of Norm	Item	UNIT	UNIT PRICE	Quantities	Amount (VND)
CAT HAI Side ROAD WORKS							681,357,217,955
A	1.1 EMBANKMENT						141,290,225,418
			Excavation of soil	m ³	60,030	2,659.5	159,649,785
			Excavation of organic soil	m ³	42,794	134,312.5	5,747,769,125
			Embankment of sand, K=0.95	m ³	190,170	509,403.3	96,873,225,561
			Embankment of sand, K=0.98	m ³	194,243	94,735.6	18,401,727,151
			Embankment of Clay (Slope Protection)	m ³	229,903	82,125.5	18,880,898,827
			Sodding (Slope Protection)	m ³	14,940	82,125.5	1,226,954,970
A	1.2 APPROACH ROAD (Soft Soil Treatment)						356,657,461,383
			Excavation of unsuitable soil	m ³	52,395	107,107.7	5,611,907,942
			Embankment of sand, K=0.95	m ³	190,170	107,107.7	20,368,671,309
			Geotextile Filter Fabric (non-woven)	m ²	18,553	378,043.9	7,013,848,477
			Geotextile Filter Fabric (woven)	m ²	57,887	221,355.8	12,813,623,195
			Sand Blanket (medium sand)	m ³	391,215	188,786.2	73,855,993,233
			Sand Drain (D400)	m	142,919	1,313,630.9	187,742,814,597
			Embankment of sand for compensation by Sa	m ³	190,170	173,682.3	33,029,162,991
			Embankment of sand for compensation by Pa	m ³	190,170	171,021.8	32,523,215,706
			Removal of surcharge	m ³	8,478	171,021.8	1,449,922,820
			Settlement Plate by steel 0.8*0.8*	each	2,359,416	180.0	424,694,880
			Wooden Pile 10*10*170cm	each	25,203	480.0	12,097,440
			Piezometer, observation well, inc	set	85,000,000	1.0	85,000,000
			Reusable sand (surcharge)	m ³	118,721	-153,919.6	-18,273,491,206
A	1.3 PAVEMENT						116,207,888,267
			Fine Asphalt Concrete -5cm	m ²	159,415	135,847.3	21,656,093,419
			Tack Coat 0.5kg/m2	m ²	10,352	135,847.3	1,406,290,996
			Medium Asphalt Concrete -7cm	m ²	209,184	135,847.3	28,417,076,472
			Prime Coat 1.0kg/m2	m ²	18,340	135,847.3	2,491,439,032
			Aggregate Base -15cm	m ³	553,398	28,420.7	15,727,958,539
			Aggregate Subbase -43cm	m ³	482,571	81,472.6	39,316,314,055
			Geotextile Filter Fabric (non-woven, 25kn/m)	m ²	36,659	196,206.0	7,192,715,754
A	1.4 TRAFFIC SAFETY						17,768,417,889
			Guide Posts	each	119,005	1,380.0	164,226,900
			Kilometer Posts	each	400,726	6.0	2,404,356
			Regulatory Signs	each	1,567,972	10.0	15,679,720
			Information and Guidance signs	each	5,499,512	6.0	32,997,072
			Area Reflection Pavement Marking	each	261,074	8,598.6	2,244,870,896
			Guardrail	m	1,278,433	480.0	613,647,840
			ReflectORIZED Pavement Stud	each	68,780	1,842.0	126,692,760
			Concrete curb	m	176,176	13,779.7	2,427,652,427
			Planting	each	278,416	2,298.0	639,799,968
			Lighting Pole -Single Arms	pole	30,000,000	361.0	10,830,000,000
			Sodding	m ²	14,940	4,409.5	65,877,930
			Embankment of clay	m ³	229,903	1,980.8	455,391,862
			Organic soil	m ³	76,887	1,940.2	149,176,157
A	1.5 Culvert						49,433,224,998
			RC Pipe Culvert -D1.25m	m	8,514,271	258.0	2,196,681,918
			RC Box Culvert -2*4m*3m(Km10+818)	m	246,545,728	31.5	7,765,450,795
			RC Box Culvert -2*4m*3.2m(Km10+128.1)	m	262,982,110	29.5	7,757,972,245
			RC Box Culvert -4m*3.2m(Km13+600)	m	262,982,110	29.5	7,757,972,245
			RC Box Culvert -1.5*3m(Km14+669)	m	142,359,293	31.6	4,494,282,880
			RC Box Culvert -3m*3m(Km 14+926)	m	161,191,286	31.8	5,125,399,321
			RC Box Culvert -3*4m*4m (Km 15+150)	m	449,754,207	31.9	14,335,465,594

Note: VAT is included

Appendix-7: Prediction of Impact on Ambient Air Quality and Impact of Noise during Operation Phase

1. Impact on Ambient Air Quality during Operation Phase

1.1. Pollutant emission source intensity of automobile tail gas

a) Pollutant emission source intensity of automobile tail gas

Sources of air pollution during operation phase are mainly from emission of vehicle engines, emission of friction between vehicle tires and road pavement.

The following formula is used to calculate gaseous pollutant (SO₂, NO_x, CO, and TSP) discharge source intensity of the planned highway¹.

$$Q_t = V_w \times \frac{1}{3600} \times \frac{1}{1000} \times \sum_{i=1}^2 (N_{it} \times E_i)$$

In which, Q_t : discharge intensity of gaseous pollutant (ml/m³·s (or mg/m³·s))
 E_i : air emission coefficient of i type vehicle (g/km·vehicle)
 N_{it} : hour traffic volume by i type vehicle (vehicle/h)
 V_w : conversion coefficient (ml/g (or mg/g))

Regarding the air emission coefficient E_i , the following formulation is used¹.

$$E_i = a/x + bx + cx^2 + d$$

In which, x is vehicle average hour speed (km/h), and a, b, c, d are regression parameters.

Table 1 shows the results of calculation of E_i in cases of x= 50km/h, 60km/h and 80km/h for small car and big car.

¹ Referred to “道路環境影響評価の技術手法 (Technical Handbook for Environmental Impact Assessment of Roads)”, 2007 Revision, Japan Highway Environment Research Institute (HERI).

Table 1 Air emission coefficients of vehicles

(Unit: g/km per vehicle unit)

Pollutants	Vehicle size	a	b	c	d	Average speed of vehicle		
						50km/h	60km/h	80km/h
NO_x	Small car	-0.902	-0.00578	4.39E-05	0.261	0.0637	0.0572	0.0683
	Big car	-7.12	-0.0895	0.000735	3.93	1.1501	1.0873	1.3850
SPM	Small car	-0.0687	-0.000385	2.87E-06	0.017	0.0036	0.0031	0.0037
	Big car	0.0318	-0.0031	2.27E-05	0.158	0.0604	0.0543	0.0557
CO	Small car	-12.5	-0.0559	0.000448	2.2	0.2750	0.2505	0.4390
	Big car	10.9	-0.0168	0.000115	1.19	0.8555	0.7777	0.7183
SO₂	Small car	0.0783	-0.000162	1.31E-06	0.0112	0.0079	0.0075	0.0076
	Big car	0.0411	-0.000699	5.51E-05	0.0424	0.1460	0.1995	0.3396

Source of data: Technical Handbook for Environmental Impact Assessment of Roads, 2007 Revision, HERI

Table 2 Daily traffic volume - TanVu-DinhVu Section

(Unit: vehicle /day)

Year	2015	2020	2030
(a) Bicycle	42,400	64,800	40,500
(b) Motorcycle	65,800	91,200	108,067
(c) Car	3,960	13,540	48,000
(d) Trucks of 2 axles and mini bus with less than 25 seats 4	1,243	2,571	8,107
(e) Truck of more than 3 axles and large bus	1,246	2,920	13,851
(f) Trailer and bus with trailer	71	129	436
Total	114,720	175,160	218,961
Small car [(b) + (c)]	69,760	104,740	156,067
Big car [(d)+(e)+(f)]	2,560	5,620	22,394

Source of data: JICA Preparatory Survey Team, May 2010

Table 3 Daily traffic volume – Dinh Vu-Cat Hai Section

(Unit: vehicle /day)

Year	2015	2020	2030
(a) Bicycle	26,900	40,300	16,800
(b) Motorcycle	41,533	56,733	44,667
(c) Car	2,500	8,420	19,860
(d) Trucks of 2 axles and mini bus with less than 25 seats 4	214	536	1,350
(e) Truck of more than 3 axles and large bus	789	1,817	5,731
(f) Trailer and bus with trailer	46	79	179
Total	71,982	107,885	88,587
Small car [(b) + (c)]	44,033	65,153	64,527
Big car [d)+(e)+(f)]	1,049	2,432	7,260

Source of data: JICA Preparatory Survey Team, May 2010

Table 4 Tan Vu - Dinh Vu Section, forecasted air pollutant emission

(Unit: g/km ·day)

Year	2015	2020	2030
SO ₂	927.7	897.8	8,791.5
NO _x	7,388.7	12,102.6	41,672.7
CO	21,374.1	30,604.4	84,590.1
TSP	402.3	628.2	1,825.7

Source of data: JICA Preparatory Survey Team, May 2010

Table 5 Dinh Vu – Cat Hai Section, forecasted air pollutant emission

(Unit: g/km ·day)

Year	2015	2020	2030
SO ₂	502.8	537.2	2,956.1
NO _x	4,011.8	6,371.6	14,461.3
CO	13,006.5	18,209.9	33,538.6
TSP	219.7	333.1	643.6

Source of data: JICA Preparatory Survey Team, May 2010

1.2. Ambient air pollutant diffusion model

To predict air pollutants emitted by moving vehicles during operation phase (with wind velocity > 1.0m/s), the following Plume Model is used (Source: Technical Handbook for Environmental Impact Assessment of Roads, 2007 Revision, HERI).

$$C(x,y,z) = \frac{Q}{2\pi \cdot u \cdot \sigma_y \cdot \sigma_z} \exp\left(-\frac{y^2}{2\sigma_y^2}\right) \left[\exp\left\{-\frac{(Z-H)^2}{2\sigma_z^2}\right\} + \exp\left\{-\frac{(Z+H)^2}{2\sigma_z^2}\right\} \right]$$

In which,

- C(x,y,z) : air pollutant concentration at survey site (x,y,z) (ppm or mg/m³)
- Q : air pollutant emission rate (ml/s or mg/s)
- u : vehicle average speed (m/s)
- H : height of source of emission (m)
- σ_y, σ_z : diffusion coefficient toward y dimension and z dimension (m)
- x : distance from emission point source to survey site along the wind direction (m)
- y : horizontal distance from survey point to x axis (m)
- z : vertical distance from survey point to x axis (m)

σ_y, σ_z are calculated by the following formulations:

$$\sigma_y = W/2 + 0.46 L^{0.81} \text{ (in case of } x < W/2 : \sigma_y = W/2) \text{ (m)}$$

$$\sigma_z = \sigma_{z0} + 0.31 L^{0.83} \text{ (in case of } x < W/2 : \sigma_z = \sigma_{z0}) \text{ (m)}$$

In which,

- L : distance from the survey point to the road side = x-W/2 (m)
- x : distance from emission point source to survey site along the wind direction (m)
- W : road width (m)
- σ_{z0} : initial vertical diffusion coefficient (m)
 - in case of no existing of noise barrier : $\sigma_{z0} = 1.5$
 - in case of existing of noise barrier : $\sigma_{z0} = 4.0$

Based on result of the EIA study relating to meteorological conditions of the Project area, input data using to predict ambient air quality for the Project are setting as following.

Table 6 Input data for prediction of ambient air quality

Z (m)	h (m)	Summer				Winter			
		Wind direction	Wind velocity (m/s)	Temperature (°C)	Atmosphere stability	Wind direction	Wind velocity (m/s)	Temperature (°C)	Atmosphere stability
1	Note*)	SE	2.5	28.2	C	NE	1.7	16.7	B

Data source: EIA Report of Tan Vu – Lach Huyen Highway Construction Project, Hanoi May 2010, Section 2.1.4

Since the wind velocity in winter (1.7m/s) is lower than the one in summer (2.5m/s), the wind

velocity in winter is selected for obtaining more conservative prediction results. In this case, predicted air pollutant concentrations (with lower wind speed) may present higher values than it is in case of summer.

And, to convert the concentration of NO_x to NO₂, the following formulation is applied (Source: Technical Handbook for Environmental Impact Assessment of Roads, 2007 Revision, HERI).

$$[NO_2]_R = 0.0683[NO_x]_R^{0.499} \times \{1 - \{[NO_x]_{BG}/([NO_x]_R + [NO_x]_{BG})\}\}^{0.507}$$

In which,

[NO₂]_R : NO₂ contributed by the road (ppm)

[NO_x]_R : NO_x contributed by the road (ppm)

[NO_x]_{BG} : background NO_x (ppm)

The following data described in the report of the study “Integrated Action Plan to Reduce Vehicle Emissions in Viet Nam” (Prepared by Multi-sectoral Action Plan Group, and chaired by Viet Nam Register, March 2002) are referred to for [NO_x]_{BG}.

Hanoi City (Nga Tu Vong Road Intersection), [NO_x] = 0.13 mg/m³ (in 1999).

1.3. Prediction results

Results of prediction of pollutants in ambient air in one hour in a winter day of the years 2015, 2020 and 2030 are described in the following tables.

In addition, the following background air pollutant concentrations are included to the predicted air pollutants emitted by vehicles.

Table 7 Background air pollution concentration (unit: µg/m³)

Survey site	Survey day	SO2	NO2	SPM	CO
A1	10-Aug-08	50	42	130	3,448
	12-Aug-08	52	42	131	3,497
	Average	51	42	131	3,473
A2	10-Aug-08	46	51	93	4,019
	12-Aug-08	46	49	91	4,035
	Average	46	50	92	4,027
A3	10-Aug-08	47	35	92	3,786
	12-Aug-08	48	36	88	3,899
	Average	48	36	90	3,843
A4	10-Aug-08	62	45	119	4,128
	12-Aug-08	61	44	120	4,227
	Average	62	45	120	4,178

Data source: EIA Report of Tan Vu – Lach Huyen Highway Construction Project, Hanoi May 2010.

**Table 8 Predicted air pollutant concentrations
 in winter at A1 (K1) survey point (h =3.3m, with background concentration added)**

(unit: $\mu\text{g}/\text{m}^3$)

Year	2015				2020				2030			
Distance (m)	SO2	NO2	CO	TSP	SO2	NO2	CO	TSP	SO2	NO2	CO	TSP
10	52.0	44.1	3,496	130.9	53.1	45.4	3,506	131.2	60.7	52.7	3,566	132.5
20	51.8	43.7	3,491	130.8	52.6	44.7	3,499	131.0	58.6	50.6	3,546	132.1
30	51.7	43.4	3,488	130.8	52.4	44.2	3,494	130.9	57.3	49.2	3,533	131.8
40	51.6	43.2	3,486	130.7	52.2	43.9	3,491	130.9	56.4	48.3	3,524	131.6
50	51.5	43.0	3,484	130.7	52.0	43.7	3,489	130.8	55.7	47.5	3,518	131.5
70	51.4	42.8	3,482	130.7	51.8	43.3	3,485	130.8	54.7	46.4	3,508	131.3
100	51.3	42.6	3,479	130.6	51.6	43.0	3,482	130.7	53.7	45.3	3,499	131.1
TCVN 5937-2005	350	200	30,000	300	350	200	30,000	300	350	200	30,000	300

**Table 9 Predicted air pollutant concentrations
 in winter at A2 (K3) survey point (h =2.3m, with background concentration added)**

(unit: $\mu\text{g}/\text{m}^3$)

Year	2015				2020				2030			
Distance (m)	SO2	NO2	CO	TSP	SO2	NO2	CO	TSP	SO2	NO2	CO	TSP
10	46.6	51.3	4,043	92.3	47.2	52.0	4,049	92.4	49.6	54.5	4,068	92.8
20	46.5	51.0	4,039	92.2	46.9	51.5	4,044	92.3	48.7	53.4	4,058	92.6
30	46.4	50.8	4,037	92.2	46.7	51.3	4,041	92.2	48.2	52.8	4,052	92.5
40	46.3	50.7	4,035	92.1	46.6	51.1	4,039	92.2	47.9	52.4	4,048	92.4
50	46.3	50.6	4,034	92.1	46.5	50.9	4,037	92.2	47.6	52.1	4,045	92.4
70	46.2	50.5	4,033	92.1	46.4	50.7	4,035	92.1	47.3	51.6	4,042	92.3
100	46.2	50.3	4,031	92.1	46.3	50.5	4,033	92.1	46.9	51.2	4,038	92.2
TCVN 5937-2005	350	200	30,000	300	350	200	30,000	300	350	200	30,000	300

**Table 10 Predicted air pollutant concentrations
 in winter at A3 (K4) survey point (h =3.4 m, with background concentration added)**

(unit: $\mu\text{g}/\text{m}^3$)

Year	2015				2020				2030			
Distance (m)	SO2	NO2	CO	TSP	SO2	NO2	CO	TSP	SO2	NO2	CO	TSP
10	48.0	36.6	3,857	90.2	48.6	37.3	3,862	90.4	50.7	39.5	3,879	90.7
20	47.9	36.4	3,854	90.2	48.3	36.9	3,858	90.3	50.0	38.7	3,871	90.6
30	47.9	36.3	3,852	90.2	48.2	36.7	3,855	90.2	49.6	38.2	3,866	90.5
40	47.8	36.1	3,850	90.1	48.1	36.5	3,854	90.2	49.3	37.8	3,863	90.4
50	47.8	36.1	3,849	90.1	48.0	36.4	3,852	90.2	49.1	37.5	3,860	90.3
70	47.7	35.9	3,848	90.1	47.9	36.2	3,850	90.1	48.7	37.1	3,857	90.3
100	47.7	35.8	3,846	90.1	47.8	36.0	3,848	90.1	48.4	36.7	3,853	90.2
TCVN 5937-2005	350	200	30,000	300	350	200	30,000	300	350	200	30,000	300

**Table 11 Predicted air pollutant concentrations
 in winter at A4 (K5) survey point (h =4.2 m, with background concentration added)**

(unit: $\mu\text{g}/\text{m}^3$)

Year	2015				2020				2030			
Distance (m)	SO2	NO2	CO	TSP	SO2	NO2	CO	TSP	SO2	NO2	CO	TSP
10	62.0	45.6	4,191	119.7	62.5	46.2	4,196	119.8	64.5	48.2	4,211	120.1
20	61.9	45.4	4,188	119.7	62.3	45.9	4,192	119.8	63.9	47.5	4,205	120.0
30	61.8	45.2	4,186	119.7	62.2	45.6	4,190	119.7	63.5	47.1	4,200	119.9
40	61.8	45.1	4,185	119.6	62.1	45.5	4,188	119.7	63.2	46.7	4,197	119.9
50	61.8	45.0	4,184	119.6	62.0	45.4	4,187	119.7	63.0	46.4	4,195	119.8
70	61.7	44.9	4,183	119.6	61.9	45.2	4,185	119.6	62.7	46.1	4,191	119.8
100	61.7	44.8	4,181	119.6	61.8	45.0	4,183	119.6	62.4	45.6	4,188	119.7
TCVN 5937-2005	350	200	30,000	300	350	200	30,000	300	350	200	30,000	300

2. Impact of Noise during Operation Phase

2.1. Prediction model

Road traffic noise prediction model “ASJ RTN-Model 2003” developed by the Acoustical Society of Japan is used to predict impact of noise caused by the Project during operation phase.

The mathematical calculation equation is as follow:

$$LA_{eq} = 10 \log_{10} \left(\sum_{i=1}^k 10^{L_{Ai}/10} * \Delta t * N/t \right)$$

where

- L_{Ai} = $L_w - 8 - 20 \log_{10}(r) + \Delta L_d + \Delta L_g + \Delta L_m$
- L_w : the A-weighted sound power level of a single running vehicle at the i^{th} source position (dB)
- r : the direct distance from the i^{th} source position to the prediction point (m)
- ΔL_d : correction for diffraction (dB),
- ΔL_g : correction for the ground effect (dB)
- ΔL_m : correction for atmospheric absorption (dB)
- N : predicted traffic volume (unit/hr)
- Δt : passing time = $\Delta D/V$
- ΔD : distance between noise source (m)
- V : average vehicle speed (m/s)

The A-weighted sound power level of a road vehicle is given by:

$$L_w = r_1 + r_2 * \log_{10}(V) + C$$

where L_w is the sound power level [dB], V is the vehicle speed [km/h], r_1 and r_2 are regression coefficients, and C is the correction term from a reference value (the power level when the vehicle runs on a dense asphalt pavement constructed within the last several years).

For the Project, to simplify the calculation, C is intentionally omitted, and L_w is calculated as follow:

$$L_w (\text{big car}) = 53.2 + 30 \log_{10}(v)$$

$$L_w (\text{small car}) = 46.7 + 30 \log_{10}(v)$$

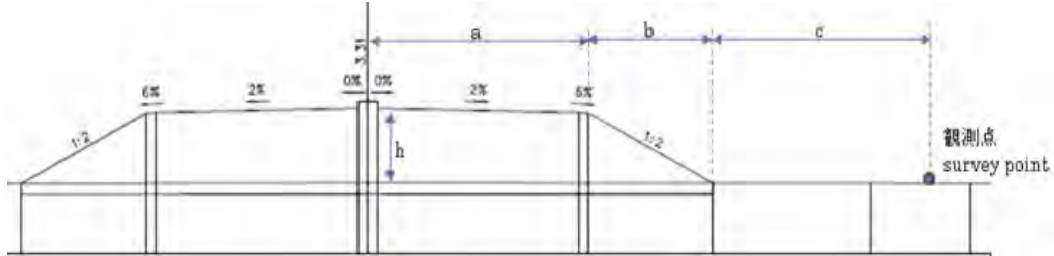
2.2. Input data

Table 12 shows the location, height of road surface, distance from the center of the road to the road side of the road section at the survey sites (A1~A4).

Table 12 Dimension of road cross-sections

(unit=m)

No.	Section	h	a	b	a+b
A1	0+600	3.3	14.75	6.2	21.35
A2	10+500	2.3	14.75	4.6	19.35
A3	12+000	3.4	14.75	6.8	21.55
A4	14+000	4.2	14.75	8.4	23.15



The distances (c) from the survey point are set as 10m, 20m, 30, 40m, 50, 70m, and 100m from the road embankment side (at the distance a+b from the road center), at 1.2 m high from the land surface.

Table 13 Hour Traffic Volume at Section of Tan Vu – Dinh Vu

(Unit: vehicles/hr)

Year	At day time (6am~18pm)			At night time (18pm~22pm)			At midnight (22pm~6am)		
	2015	2020	2030	2015	2020	2030	2015	2020	2030
Motorcycle	5,483	7,600	9,006	2,303	3,192	3,782	494	684	811
Small car	330	1,128	4,000	139	474	1,680	30	102	360
Big car	214	468	1,866	90	197	784	19	42	168

Table 14 Hour Traffic Volume at Section of Dinh Vu – Cat Hai

(Unit: vehicles/hr)

Year	At day time (6am~18pm)			At night time (18pm~22pm)			At midnight (22pm~6am)		
	2015	2020	2030	2015	2020	2030	2015	2020	2030
Motorcycle	3,461	4,728	3,722	1,454	1,986	1,563	311	425	335
Small car	208	702	1,655	88	295	695	19	63	149
Big car	88	203	606	38	86	254	8	19	54

2.3. Prediction results

Table 15 Predicted Noise Level at Survey Site A1 (K1)

(Unit: dBA)

Distance c (m)	In day-time (6am~18pm)			In night-time (18pm~22pm)			In midnight (22pm~6am)		
	2015	2020	2030	2015	2020	2030	2015	2020	2030
10	61.9	65.5	71.0	58.1	61.8	67.2	51.5	55.1	60.5
20	61.7	65.3	70.8	58.0	61.6	67.0	51.3	54.9	60.3
30	61.3	65.0	70.4	57.6	61.2	66.7	50.9	54.5	60.0
40	60.9	64.6	70.0	57.2	60.8	66.2	50.5	54.1	59.6
50	60.6	64.2	69.6	56.8	60.4	65.9	50.1	53.7	59.2
70	59.8	63.4	68.9	56.1	59.7	65.1	49.4	53.0	58.4
100	58.9	62.5	68.0	55.1	58.8	64.2	48.4	52.1	57.5
TCVN5949-1998*	75 dBA			70 dBA			50 dBA		

*Note: Allowable maximum noise level at business-service-shopping-industrial mixed residential area

Table 16 Predicted Noise Level at Survey Site A2 (K3)

(Unit: dBA)

Distance c (m)	In day-time (6am~18pm)			In night-time (18pm~22pm)			In midnight (22pm~6am)		
	2015	2020	2030	2015	2020	2030	2015	2020	2030
10	61.4	64.9	68.5	57.6	61.1	64.7	50.9	54.5	58.0
20	60.8	64.3	67.9	57.1	60.6	64.1	50.4	53.9	57.4
30	60.2	63.7	67.3	56.5	60.0	63.5	49.8	53.3	56.8
40	59.7	63.2	66.7	55.9	59.4	63.0	49.2	52.7	56.3
50	59.1	62.7	66.2	55.4	58.9	62.4	48.7	52.2	55.7
70	58.3	61.8	65.3	54.5	58.0	61.6	47.8	51.3	54.9
100	57.2	60.7	64.3	53.4	56.9	60.5	46.7	50.3	53.8
TCVN5949-1998	60 dBA			55 dBA			50 dBA		

Table 17 Predicted Noise Level at Survey Site A3 (K4)

(Unit: dBA)

Distance c (m)	In day-time (6am~18pm)			In night-time (18pm~22pm)			In midnight (22pm~6am)		
	2015	2020	2030	2015	2020	2030	2015	2020	2030
10	59.5	63.2	66.6	55.8	59.3	62.8	49.1	52.6	56.1
20	59.4	62.9	66.5	55.6	59.1	62.7	48.9	52.5	56.0
30	59.0	62.5	66.1	55.3	58.8	62.3	48.6	52.1	55.6
40	58.6	62.2	65.7	54.9	58.4	61.9	48.2	51.7	55.2
50	58.3	61.8	65.3	54.5	58.0	61.6	47.8	51.3	54.9
70	57.5	61.1	64.6	53.8	57.3	60.8	47.1	50.6	54.1
100	56.6	60.2	63.7	52.9	56.4	59.9	46.2	49.7	53.2
TCVN5949-1998	60 dBA			55 dBA			50 dBA		

Table 18 Predicted Noise Level at Survey Site A4 (K5)

(Unit: dBA)

Distance c (m)	In day-time (6am~18pm)			In night-time (18pm~22pm)			In midnight (22pm~6am)		
	2015	2020	2030	2015	2020	2030	2015	2020	2030
10	58.5	62.0	65.6	54.7	58.3	61.8	48.0	51.6	55.1
20	58.5	62.0	65.6	54.8	58.3	61.8	48.1	51.6	55.1
30	58.3	61.8	65.4	54.5	58.1	61.6	47.8	51.4	54.9
40	58.0	61.5	65.1	54.3	57.8	61.3	47.6	51.1	54.6
50	57.7	61.2	64.8	53.9	57.4	61.0	47.2	50.8	54.3
70	57.1	60.6	64.2	53.3	56.8	60.4	46.6	50.2	53.7
100	56.3	59.8	63.3	52.5	56.0	59.6	45.8	49.4	52.9
TCVN5949-1998	60 dBA			55 dBA			50 dBA		

Appendix-8: Environmental Checklist for Tan Vu - Lach Huyen Highway Construction Project

Category	Environmental Item	Main Check Items	Confirmation of Environmental Considerations
1 Permits and Explanation	(1) EIA and Environmental Permits	1) Have EIA reports been officially completed? 2) Have EIA reports been approved by authorities of the host country's government? 3) Have EIA reports been unconditionally approved? If conditions are imposed on the approval of EIA reports, are the conditions satisfied? 4) In addition to the above approvals, have other required environmental permits been obtained from the appropriate regulatory authorities of the host country's government?	1) In 2009, the EIA report was submitted by VIDIFI (the former Project proponent) to Hai Phong City People's Committee for approval. However, in December 2009, The Prime Minister decided to transfer the Project Proponent title from VIDIFI to MoT. Therefore, under the VN regulations, it is MoT who is authorized entity to approve the EIA report. The PMU 2 under MoT revised the EIA report due to changing the Project Proponent and to be accordance with JBIC Guidelines. 2) The revised EIA report was approved by MoT on May 27, 2010 (Decision 1420/QD-BGTVT). 3) The EIA report was approved with a list of requirements. 4) Only the EIA report approval is required. No other environmental permit is required.
	(2) Explanation to the Public	1) Are contents of the project and the potential impacts adequately explained to the public based on appropriate procedures, including information disclosure? Is understanding obtained from the public? 2) Are proper responses made to comments from the public and regulatory authorities?	1) Contents of the project, and potential impacts and proposed mitigation measures have been explained to the public, especially to five communes in the project site, under the procedure specified in the Vietnamese regulations. In addition, a consultation meeting was organized on April 28, 2010. Approximately 80 local residents and representatives of local authorities of Cat Hai District, Cat Hai Townlet, Nghia Lo Commune and Dong Bai Commune have participated the meeting. Participants

			<p>had raised many comments and recommendations during the meeting. It seems that residents have been informed about outlines of the project through many formal and informal channels including hearing surveys carried out during F/S study.</p> <p>2) Comments raised by local residents in the public consultation meeting had been recorded and carefully considered during the revision of EIA Report and RAP Report.</p>																																																																											
<p>2 Mitigation Measures</p>	<p>(1) Air Quality</p>	<p>1) Is there a possibility that air pollutants emitted from various sources, such as vehicle traffic will affect ambient air quality? Does ambient air quality comply with the country's ambient air quality standards?</p> <p>2) Where industrial areas already exist near the route, is there a possibility that the project will make air pollution worse?</p>	<p>1) Yes. The following table shows predicted data on ambient air quality.</p> <p>Predicted ambient quality (unit: $\mu\text{g}/\text{m}^3$) at the survey point 10m from the road embankment side.</p> <table border="1" data-bbox="1267 707 1995 1190"> <thead> <tr> <th></th> <th></th> <th>A1</th> <th>A2</th> <th>A3</th> <th>A4</th> <th>Standard^{*)}</th> </tr> </thead> <tbody> <tr> <td rowspan="3">TSP</td> <td>2015</td> <td>19</td> <td>45</td> <td>10</td> <td>2</td> <td rowspan="3">300$\mu\text{g}/\text{m}^3$ (1 hour average)</td> </tr> <tr> <td>2022</td> <td>31</td> <td>71</td> <td>16</td> <td>3</td> </tr> <tr> <td>2032</td> <td>72</td> <td>108</td> <td>24</td> <td>5</td> </tr> <tr> <td rowspan="3">SO2</td> <td>2015</td> <td>112</td> <td>254</td> <td>56</td> <td>11</td> <td rowspan="3">350$\mu\text{g}/\text{m}^3$ (1 hour average)</td> </tr> <tr> <td>2022</td> <td>207</td> <td>460</td> <td>102</td> <td>20</td> </tr> <tr> <td>2032</td> <td>535</td> <td>783</td> <td>173</td> <td>34</td> </tr> <tr> <td rowspan="3">NO2</td> <td>2015</td> <td>154</td> <td>354</td> <td>78</td> <td>15</td> <td rowspan="3">200$\mu\text{g}/\text{m}^3$ (1 hour average)</td> </tr> <tr> <td>2022</td> <td>275</td> <td>620</td> <td>137</td> <td>27</td> </tr> <tr> <td>2032</td> <td>758</td> <td>1134</td> <td>250</td> <td>49</td> </tr> <tr> <td rowspan="3">CO</td> <td>2015</td> <td>2864</td> <td>6622</td> <td>1460</td> <td>284</td> <td rowspan="3">30,000 $\mu\text{g}/\text{m}^3$ (1 hour)</td> </tr> <tr> <td>2022</td> <td>4205</td> <td>9468</td> <td>2088</td> <td>405</td> </tr> <tr> <td>2032</td> <td>6205</td> <td>8818</td> <td>1944</td> <td>378</td> </tr> </tbody> </table> <p>*) TCVN 5937:2005 Ambient air quality standard</p> <p>Proposed mitigation measures described in the EIA Report:</p> <ul style="list-style-type: none"> - Planting trees at the road sides near the populous residential areas; - Strictly control exhaust gas and dust emission during construction 			A1	A2	A3	A4	Standard ^{*)}	TSP	2015	19	45	10	2	300 $\mu\text{g}/\text{m}^3$ (1 hour average)	2022	31	71	16	3	2032	72	108	24	5	SO2	2015	112	254	56	11	350 $\mu\text{g}/\text{m}^3$ (1 hour average)	2022	207	460	102	20	2032	535	783	173	34	NO2	2015	154	354	78	15	200 $\mu\text{g}/\text{m}^3$ (1 hour average)	2022	275	620	137	27	2032	758	1134	250	49	CO	2015	2864	6622	1460	284	30,000 $\mu\text{g}/\text{m}^3$ (1 hour)	2022	4205	9468	2088	405	2032	6205	8818	1944	378
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			<ul style="list-style-type: none"> - Forbid trucks with over exhaust gas to use the road during operation phase. - Carry out regular maintenance of road and bridge pavement. Spray water regularly on road surface at least 10 days/time in dry season; - Take care of trees and landscape along the road. - Carry out monitoring of ambient air quality; <p>2) At present, industrial areas are not yet developed along the road. However, in the near future, in the west side (Dinh Vu peninsula) the Dinh Vu Industrial Zone will be expanded and in the east side (Cat Hai Island) the Lach Huyen International Port will be developed. There is a possibility that the project will make air pollution worse.</p>
	(2) Water Quality	<p>1) Is there a possibility that soil runoff from the bare lands resulting from earthmoving activities, such as cutting and filling will cause water quality degradation in downstream water areas?</p> <p>2) Is there a possibility that surface runoff from roads will contaminate water sources, such as surface water, seawater, and groundwater?</p> <p>3) Do effluents from various facilities, such as toll gate and parking areas/service areas comply with the country's effluent standards and ambient water quality standards? Is there a possibility that the effluents will cause areas that do not comply with the country's ambient water quality standards?</p>	<p>1) Yes.</p> <p>Mitigation measures:</p> <ul style="list-style-type: none"> - At the start of site establishment, perimeter cut-off drains to direct off-site water around the site shall be constructed and internal temporary drainage works and erosion and sediment control facilities implemented. - The Contractor shall plan his works to minimize surface excavation works during the rainy season where practicable. - Channels, earth bunds, netting, tarpaulin and or sand bag barriers shall be used on site to manage surface water runoff and minimize erosion - All exposed earth areas shall be completed and re-vegetated as soon as possible after earthworks have been completed.

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| | <p>4) Is there a possibility that oceanographic changes, such as alteration of ocean currents, and reduction in seawater exchange rates (deterioration of seawater circulation) due to modification of water areas, such as shoreline modifications, reduction in water areas, and creation of new water areas will cause changes in water temperature and water quality?</p> <p>5) In the case of the projects including land reclamation, are adequate measures taken to prevent contamination of surface water, seawater, and groundwater by leachates from the reclamation areas?</p> | <p>2) Yes. However the following mitigation measures will be included in the bidding documents and contracts, and would be carried out by contractors under supervision of General Consultant.</p> <ul style="list-style-type: none"> - Drainage system and retention ponds will be constructed to collect and treat surface runoff from road prior to discharge to the local surface water bodies. A reservoir (200 ha) will be planned near the Tan Vu Interchange to collect and regulate runoff water from road on the Dinh Vu side, and other two retention ponds will be planned on the Cat Hai side (one near Ninh Tiep Hamlet, and one near Trung Hamlet) with similar functions . <p>3) Once the proposed mitigation measures to be strictly applied and well controlled, there is not a possibility.
Sewerage from parking areas/ service areas shall be collected and treated by specified processes prior to discharge.</p> <p>4) No.</p> <p>5) For the land reclamation and other earthworks, the following measures are proposed.</p> <ul style="list-style-type: none"> - Material stockpile sites, earthwork sites, and other construction sites where exposed land surface is vulnerable to runoff, etc. should be consolidated and/or covered; - The material stockpile site should be far away from surface water body and the area prone to surface run-off. The loose materials should be bagged and covered. Open ditch should be built around the stockpile site to intercept wastewater; |
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			<ul style="list-style-type: none"> - Construction wastes should be collected and re-used wherever possible, otherwise should be disposed in the small deposit area invulnerable to surface run-off, along with soil erosion prevention measures; - Prevent the oil leak from the operation of the machinery by the regular check; - Utilize excavated soil through recycling within the project; - Contract out treatment / dumping / recycling fo residual soil depending on soil quality.
(3) Wastes		<p>1) Is offshore dumping of dredged materials and soils properly performed in accordance with the country's standards to prevent impacts on the surrounding waters?</p> <p>2) Are adequate measures taken to prevent discharge or dumping of hazardous materials to the surrounding water areas?</p>	<p>1) The sites and methods to dump dredged materials and soils will be examined carefully in the D/D stage.</p> <p>2) The following measures are proposed.</p> <ul style="list-style-type: none"> - Carry out analysis of toxic components of soil to be excavated; - Prohibit dumping of hazardous soils and wastes. - Obligate contractors to segregate construction wastes on-site to facilitate re-use, recycling and proper disposal; - Obligate contractors to contract out treatment/ dumping/ recycling of construction wastes to competent companies. - Waste oils, chemicals, paints and other materials used for machinery maintenance and construction shall be collected and stored in bunded areas on-site for resale/re-use or managed disposal without resulting in damage or pollution of the environment. Waste storage sites shall be located away from water areas. Designated waste storage areas shall be well maintained and cleaned regularly.

(4) Noise and
Vibration

1) Do noise and vibrations from vehicle comply with the country's standards?

1) The predicted noise level do not comply with the Vietnam country's standards.

Following table shows predicted data on noise level (at the survey site located 10m from the road embankment side):

		A1	A2	A3	A4	Standard ^{*)}
6am ~ 18pm	2015	95	92	89	87	60 dBA
	2020	104	100	98	96	
	2030	115	109	106	104	
18pm ~22pm	2015	78	75	72	70	55 dBA
	2020	87	83	81	79	
	2030	98	92	89	87	
22pm ~6am	2015	70	67	65	63	50 dBA
	2020	79	76	73	71	
	2030	91	85	82	80	

*) TCVN-5949-1998

Proposed mitigation measures described in the EIA Report:

- Plant trees along sections of road near the populous residential areas in Thon Hamlet and Ninh Tiep Hamlet, to mitigate impacts of noise, exhaust gas and dust to local residents;
- Take care of trees planted along the road, and grasses planted at the road slope surfaces;
- Install warning signs on road for honking bans and speed control at the road sections close to residential areas of Trung Hamlet and Ninh Tiep Hamlet;
- Respond to monitoring results which show higher noise than projected by the EIA;
- Regular maintenance on road to keep good road surface condition.

			- Carry out monitoring of noise.
3 Natural Environment	(1) Protected Areas	1) Is the project site located in protected areas designated by the country's laws or international treaties and conventions? Is there a possibility that the project will affect the protected areas?	1) No.
	(2) Ecosystem	<p>1) Does the project site encompass primeval forests, tropical rain forests, ecologically valuable habitats (e.g., coral reefs, mangroves, or tidal flats)?</p> <p>2) Does the project site encompass the protected habitats of endangered species designated by the country's laws or international treaties and conventions?</p> <p>3) If significant ecological impacts are anticipated, are adequate protection measures taken to reduce the impacts on the ecosystem?</p> <p>4) Is there a possibility that the project will adversely affect aquatic organisms? If significant impacts are anticipated, are adequate protection measures taken to reduce the impacts on aquatic organisms?</p> <p>5) Is there a possibility that the project will adversely affect vegetation and wildlife of coastal zones? If significant impacts are anticipated, are adequate measures taken to reduce the impacts on vegetation and wildlife?</p>	<p>1) The project area does not encompass primeval forests, tropical rain forests, ecologically valuable habitats. It will encompass tidal flats. However, the impact is not significant.</p> <p>2) No.</p> <p>3) It is anticipated that the ecological impacts are not significant</p> <p>4) It is anticipated that impacts to aquatic organisms are not significant</p> <p>5) It is anticipated that impacts to vegetation and wildlife of coastal zones are not significant</p> <p>6) There is not any valuable forest, wetland, fauna species, flora species in the project area.</p> <p>7) A part of the planned highway is located in the under-developing Dinh Vu Industrial Zone. However, there is not any possibility that the highway will result in extensive loss of natural environments.</p>

		<p>6) Is there a possibility that installation of roads will cause impacts, such as destruction of forest, poaching, desertification, reduction in wetland areas, and disturbance of ecosystems due to introduction of exotic (non-native invasive) species and pests? Are adequate measures for preventing such impacts considered?</p> <p>7) In cases where the project site is located at undeveloped areas, is there a possibility that the new development will result in extensive loss of natural environments?</p>	
	(3) Hydrology	<p>1) Is there a possibility that alteration of topographic features and installation of structures will adversely affect surface water and groundwater flows on the land?</p> <p>2) Is there a possibility that alteration of topographic features and installation of structures, such as piers and guide posts will cause oceanographic changes and adversely affect oceanographic conditions, such as induced currents, waves, and tidal currents?</p>	<p>1) Impact caused by the planned highway to the rivers, canals, and other surface water system and groundwater flows in the project area is anticipated not significant.</p> <p>2) Impact caused by the planned bridge piers to the river flows and current oceanographic conditions is anticipated not significant.</p>
	(4) Topography and Geology	<p>1) Is there a soft ground on the route that may cause slope failures or landslides? Are adequate measures considered to prevent slope failures or landslides, where needed?</p> <p>2) Is there a possibility that civil works, such as cutting and filling will cause slope failures or landslides? Are adequate measures considered to prevent slope failures or landslides?</p>	<p>1) The project area is generally flat. Slope failures or landslides are not likely to be induced. However, the highway is planned on soft ground of Cat Hai Island and Dinh Vu Peninsula. There would be possibility of land subsidence if proper measures to treat soft ground are not carried out appropriately.</p> <p>2) The highway is planned in the plain coastal areas, and therefore, occurrence of large-scale slope failures or landslides are not</p>

	<p>3) Is there a possibility that soil runoff will result from cut and fill areas, waste soil disposal sites, and borrow sites? Are adequate measures taken to prevent soil runoff?</p> <p>4) Is there a possibility that installation of structures, such as piers and guide posts will cause a large-scale alteration of topographic and geologic features in the surrounding areas or elimination of natural beaches?</p>	<p>anticipated.</p> <p>3) The following measures are proposed to prevent soil runoff from earthwork sites, waste soil disposal sites, and borrow sites:</p> <ul style="list-style-type: none"> - Contractors will be obligated to minimize exposition of soil surface caused by excavation works during the rainy season where practicable. - The material stockpile sites, the earthwork sites where exposed land surface is vulnerable to runoff, etc. should be consolidated and/or covered; - The material stockpile sites should be far away from surface water bodies and areas prone to surface run-off. Loose materials should be bagged and covered. Open ditch should be built around the stockpile sites to intercept wastewater. Channels, earth bunds, netting, tarpaulin and/or sand bag barriers shall be used on site to manage surface water runoff and minimise erosion; - All exposed earth areas shall be completed and re-vegetated as soon as possible after completion of earthworks. <p>4) No.</p>
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<p>4 Social Environment</p>	<p>(1) Resettlement</p>	<p>1) Is involuntary resettlement caused by project implementation? If involuntary resettlement is caused, are efforts made to minimize the impacts caused by the resettlement?</p> <p>2) Is adequate explanation on relocation and compensation given to affected persons prior to resettlement?</p> <p>3) Is the resettlement plan, including proper compensation, restoration of livelihoods and living standards developed based on socioeconomic studies on resettlement?</p> <p>4) Does the resettlement plan pay particular attention to vulnerable groups or persons, including women, children, the elderly, people below the poverty line, ethnic minorities, and indigenous peoples?</p> <p>5) Are agreements with the affected persons obtained prior to resettlement?</p> <p>6) Is the organizational framework established to properly implement resettlement? Are the capacity and budget secured to implement the plan?</p> <p>7) Is a plan developed to monitor the impacts of resettlement?</p>	<p>1) The Project would acquire some lots of residential land, especially in Cat Hai Island. This will cause the need of involuntary resettlement. In the D/D stage, efforts should be made to minimize scale of land acquisition and involuntary resettlement, through the careful examination of route alignment, particularly at the section near the Trung Hamlet in Cat Hai Island.</p> <p>2) At present time, the Project have not been formally approved, and impacts caused by the land acquisition for the Project have not been identified clearly yet. Therefore, PMU2 and local authorities can provide residents living in the affected areas with only limited explanations on resettlement and compensation.</p> <p>3) The detailed socio-economic survey has not been carried out yet. A Pre-RAP had been prepared as a part of the F/S report prepared in July 2009 by VIDIFI. This Pre-RAP includes legal and policy framework for compensation and resettlement. Policy on restoration of livelihoods and living standards of PAP is described briefly in this Pre-RAP.</p> <p>4) A section in the Pre-RAP describes the compensation policy which includes several statements on particular considerations to the poors. However, a detailed socio-economic survey should be carried out in the D/D stage, to identify characteristics and living conditions of the poors. the elderly, the children, as well as other vulnerable groups in the Project area. And a detailed RAP should be prepared in the D/D stage, which should take into considerations particular measures to assist vulnerable residents in restoring their livelihoods and living</p>
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			<p>standards.</p> <p>5) At this present time, PAP have not been formally identified yet, and negotiations on compensation and relocation have not been carried out yet.</p> <p>6) According to Vietnam regulations on compensation and resettlement in case of development project, a formal declaration on land acquisition will be done after the approval of the F/S and investment license. Organizational framework for implementation of compensation and resettlement will be established after such formal declaration on land acquisition.</p> <p>7) The Pre-RAP prepared by VIDIFI in July 2009 includes a section describing outline of a system proposed for monitoring the implementation of the RAP. However, it is recommended that a RAP Monitoring Plan should be prepared in detail in the D/D stage.</p>
	(2) Living and Livelihood	<p>1) Where roads are newly installed, is there a possibility that the project will affect the existing means of transportation and the associated workers? Is there a possibility that the project will cause significant impacts, such as extensive alteration of existing land uses, changes in sources of livelihood, or unemployment? Are adequate measures considered for preventing these impacts?</p> <p>2) Is there a possibility that changes in water uses (including fisheries and recreational uses) in the surrounding areas due to project will adversely affect</p>	<p>1) At present time, the main mean of transportation of residents in Cat Hai Island is motorbike and ferry (Dinh Vu Ferry). The Project may contribute to the improvement of residents' accessibility to other cities the main land. However, it may cause the termination of Dinh Vu Ferry. Workers of the Ferry as well as shopkeepers, peddlers, etc. who have means of livelihood in relation with the Ferry should lose income, and should be supported to change occupation.</p> <p>In addition, aquaculture and salt production which are main means of livelihood of local residents, would be significantly affected by the Project. Therefore, it is recommended that a proper Livelihood Restoration Program should be prepared and implemented in the D/D</p>

the livelihoods of inhabitants? Are adequate measures considered to reduce the impacts, if necessary?

- 3) Is there a possibility that diseases, including communicable diseases, such as HIV will be introduced due to immigration of workers associated with the project? Are adequate considerations given to public health, if necessary?
- 4) Is there a possibility that the project will adversely affect the existing water traffic and road traffic in the surrounding areas (e.g., by causing increases in traffic congestion and traffic accidents)?
- 5) Is there a possibility that roads will cause impede the movement of inhabitants?
- 6) Is there a possibility that structures associated with roads (such as bridges) will cause a sun shading and radio interference?

stage. It also recommends to construct a parking area/ service zone in the area near the Cat Hai side- terminal of Got Ferry in order to facilitate the implementation of Livelihood Restoration Program for PAP.

There is a plan to relocate all residents in the southern part of the Cat Hai Island to develop an industrial zone in connecting with the international port. However, detailed information about this plan are unknown.

- 2) As mentioned above, aquaculture and salt production would be significantly affected by the Project.
- 3) It is estimated that about 400 immigrant construction workers may come to work in and around the Project area. The EIA report identifies organic waste which affects human health, water-transmitted diseases and social evil activities as the negative impacts on health condition.
In addition, possibility of outbreaks of infectious diseases from such workers such as malaria, dengue and HIV cannot be neglected. It is recommended to prepare a proper HIV/AIDS Prevention Program in the D/D stage, and duly carry out this Program during construction phase.
- 4) During construction phase, traffic congestions and accidents may occurred frequently on the roads near Cat Hai City, due to the concentration of construction vehicles using these roads to access to construction sites. Several mitigation measures are recommended in the EIA Report such as the followings.

		<ul style="list-style-type: none"> - Carefully prepare the construction plan in order to minimize the area and period of road occupation / closure, and avoid concentration of construction vehicles; - Prior notice local residents on the road occupation / closure through sign boards and mass media; - Allocate personnel at place vulnerable to traffic congestion to instruct detour. <p>5) The highway will cause split of the Ninh Tiep Hamlet and Trung Hamlet, and would impede residents' movement in these hamlets. Therefore, it is recommended to construct several underpass routes at road sections near these hamlets to mitigate such impedance..</p> <p>6) For the road section in Cat Hai Island, the heights of the road embankment are planned about 1~4 meters from the existing land level. A few number of houses in Ninh Tiep Hamlet may be affected by sun shading. However, impact of radio interference is unlikely anticipated.</p>
(3) Heritage	1) Is there a possibility that the project will damage the local archeological, historical, cultural, and religious heritage sites? Are adequate measures considered to protect these sites in accordance with the country's laws?	1) No.
(4) Landscape	1) Is there a possibility that the project will adversely affect the local landscape? Are necessary measures taken?	1) No.

	(5) Ethnic Minorities and Indigenous Peoples	<p>1) Where ethnic minorities and indigenous peoples are living in the rights-of-way, are considerations given to reduce the impacts on culture and lifestyle of ethnic minorities and indigenous peoples?</p> <p>2) Does the project comply with the country's laws for rights of ethnic minorities and indigenous peoples?</p>	<p>1) The Project area is far away from the living habitats of ethnic minorities. There are no indigenous people in the Project area.</p> <p>2) Not applicable</p>
5 Others	(1) Impacts during Construction	<p>1) Are adequate measures considered to reduce impacts during construction (e.g., noise, vibrations, turbid water, dust, exhaust gases, and wastes)?</p> <p>2) If construction activities adversely affect the natural environment (ecosystem), are adequate measures considered to reduce impacts?</p> <p>3) If construction activities adversely affect the social environment, are adequate measures considered to reduce impacts?</p> <p>4) If necessary, is health and safety education (e.g., traffic safety, public health) provided for project personnel, including workers?</p>	<p>1) Measures described in the following annex are recommended to mitigate impacts during construction. However, an EMP (Environmental Management Program) should be prepared and implemented duly to ensure the implementation of these measures.</p> <p>2) Similar to Paragraph 1)</p> <p>3) Similar to Paragraph 1)</p> <p>4) The health and safety education (e.g., traffic safety, public health) will be provided for project personnel, including workers.</p>
	(2) Monitoring	<p>1) Does the proponent develop and implement monitoring program for the environmental items that are considered to have potential impacts?</p> <p>2) Are the items, methods and frequencies included in the monitoring program judged to be appropriate?</p> <p>3) Does the proponent establish an adequate monitoring</p>	<p>1) An environmental monitoring plan is described briefly in the EIA Report.</p> <p>2) Items, methods and frequencies included in the environmental monitoring plan described in the EIA Report are judged not appropriate. In the D/D stage, a detailed Environmental Management Program (EMP) which includes a concrete environmental monitoring</p>

		<p>framework (organization, personnel, equipment, and adequate budget to sustain the monitoring framework)?</p> <p>4) Are any regulatory requirements pertaining to the monitoring report system identified, such as the format and frequency of reports from the proponent to the regulatory authorities?</p>	<p>plan should be prepared.</p> <p>3) Establishment of an adequate monitoring framework should be examined and described in the EMP to be prepared in the D/D stage.</p> <p>4) The EIA report does not show any regulatory requirements on reporting monitoring results. However, the project proponent will have a responsibility for reporting periodically to the relevant governmental agencies as well as public.</p> <p>Besides, the Pre-RAP describes briefly about the need of monitoring of the RAP implementation, but does not go into detail about the organizational framework for the monitoring, and the systematical reporting process. It is recommended to examine this issue in more detail in the RAP to be prepared in the D/D stage.</p>
<p>6 Note</p>	<p>Note on Using Environmental Checklist</p>	<p>1) Where necessary, impacts on groundwater hydrology (groundwater level drawdown and salinization) that may be caused by alteration of topography, such as land reclamation and canal excavation should be considered, and impacts, such as land subsidence that may be caused by groundwater uses should be considered. If significant impacts are anticipated, adequate mitigation measures should be taken.</p> <p>2) If necessary, the impacts on transboundary or global issues should be confirmed, if necessary (e.g., the project includes factors that may cause problems, such as transboundary waste treatment, acid rain, destruction of the ozone layer, or global warming).</p>	<p>1) Not applicable.</p> <p>2) Not applicable.</p>

- 1) Regarding the term “Country’s Standards” mentioned in the above table, in the event that environmental standards in the country where the project is located diverge significantly from international standards, appropriate environmental considerations are made, if necessary.

In cases where local environmental regulations are yet to be established in some areas, considerations should be made based on comparisons with appropriate standards of other countries (including Japan' experience).

- 2) Environmental checklist provides general environmental items to be checked. It may be necessary to add or delete an item taking into account the characteristics of the project and the particular circumstances of the country and locality in which it is located.

Annex- Proposed mitigation measures in construction phase

A. AIR QUALITY CONTROL

- 1) Construction materials are supplied only from the quarries that have exploitation license and operated under a good environmental management
- 2) The Contractor shall not burn debris, construction wastes, vegetation or other materials on the site.
- 3) Specific mitigation measures to control air quality impacts arising from implementing the earthworks are as follows:
 - to minimize dust emissions, the amount of spoil exposed shall be kept as low as possible and
 - the dust generation potential shall be kept as low as possible, this can be accomplished by surface compaction, temporary fabric covers, minimizing the extent of exposed soil and the prompt re-vegetation of completed earthworks;
- 4) In transportation of earth and construction materials:
 - Materials having the potential to create dust shall not be loaded to a level higher than the side and tail boards, and shall be covered by a clean tarpaulin in good condition.
 - The Contractor shall be responsible for ensuring that no earth, rock or debris is deposited on public or private right of way as a result of his operations, including any deposits arising from the movement of Construction Plant or vehicles.

- Wheel washing facilities shall be provided at the exit of all construction sites to prevent dusty material from being carried off-site on vehicles and deposited on public roads. Wash-water shall have sand and silt settled out and removed at least on a weekly basis to ensure the continued efficiency of wheel wash operations.
 - The Contractor shall spray all roads within the construction sites and roads leading to the sites to control dust.
 - The Contractor shall require that all vehicles, while parked on the site have their engines turned off.
 - On site vehicle speeds shall be restricted to a maximum of 15km/hour to reduce dust resuspension and dispersion by traffic within sites;
 - Areas within the Site where there is a regular movement of vehicles shall have a hard surface and be kept clear of loose surface material
 - During breaking/crushing or demolition works watering shall be implemented to control dust. Water sprays shall be used during the handling of excavated material and at active cuts, excavation and fill sites. Excessive watering should be avoided.
- 5) Heights from which excavated materials are dropped shall be controlled to the minimum practical to limit the fugitive dust generation from unloading.
- 6) Specific mitigation measures to control air quality impacts arising from concrete batching plant operation are as follows:
- Cement and other such fine-grained materials delivered in bulk shall be stored in closed silos fitted with a high-level alarm indicator. All air vents on cement silos shall be fitted with suitable fabric filters provided with either shaking or pulse-air cleaning mechanisms.
 - The Contractor shall frequently clean and water the concrete batching plant and crushing plant sites and ancillary areas to minimize any dust emissions.
 - Where dusty materials are being discharged to vehicles from a conveying system at a fixed transfer point, a three-sided roofed enclosure with a flexible curtain across the entry shall be provided. Exhaust fans shall be provided for this enclosure and vented to a suitable fabric filter system;
 - All stockpiles of sand and aggregate within the batching plant site which are greater than 50m³ shall be enclosed on three sides with walls extending above the stockpile and 2000mm beyond the front of the stockpile.

B. NOISE AND VIBRATION CONTROL

- 1) Construction works within 100 meters of residential areas and hospitals, shall be restricted to daytime hours 0600 to 1800, to minimise noise disturbance at night.
- 2) The Contractor shall select equipment with considerations for using equipment with lowest noise levels;
- 3) Positioning air compressors for various construction plant on rubber sheets;
- 4) Sitting of mobile plant as far away from NSRs as possible. Orientation of plant known to emit noise strongly in one direction such that the noise is directed away from nearby SRs.

C. EROSION CONTROL

- 1) At the start of site establishment, perimeter cut-off drains to direct off-site water around the site shall be constructed and internal temporary drainage works and erosion and sediment control facilities implemented.
- 2) The Contractor shall plan his works to minimize surface excavation works during the rainy season (April to September) where practicable.
- 3) Precautions to be taken at any time of year when rainstorms are likely, actions to be taken when a rainstorm is imminent or forecast, and actions to be taken during or after rainstorms shall be developed by the Contractor. Particular attention shall be paid to the control of surface runoff during storm events, especially for sites located near steep slopes.

If excavation of soil cannot be avoided during the rainy season, or at any time of year when rainstorms are likely, exposed slope surfaces shall be protected by temporary drainage measures as detailed above.

- 4) Channels, earth bunds, netting, tarpaulin and or sand bag barriers shall be used on site to manage surface water runoff and minimize erosion. Failure to provide temporary drainage measures can result in considerable storm damage during the wet season to the site works as well as considerable water quality impacts.
- 5) All exposed earth areas shall be completed and revegetated as soon as possible after earthworks have been completed.
- 6) The overall slope of the works areas and construction yards shall be kept to a minimum to reduce the erosive potential of surface water flows

D. WATER POLLUTION CONTROL

1) The Contractor shall ensure that all temporary construction facilities are located at least 50 meters away from any waterbody.

The Contractor shall ensure that no tools or machinery are washed in any water source or areas that drain into an existing watercourse or to the marine environment.

2) The Contractor shall ensure that rain run-off from the construction sites is not deposited directly into any watercourse or the marine environment.

Drainage from vehicle maintenance areas, plant servicing areas and vehicle wash bays shall be passed via a petrol interceptor prior to discharge.

Wastewater shall be collected and disposed of off-site after oil/grease removal and settlement of suspended solids.

Sediment tanks of sufficient capacity, constructed from pre-formed individual cells of approximately 6-8m³ capacity shall be used at all sites for settling waste-waters prior to disposal. Wastewater arising from excavation works shall be discharged to storm drains via sediment tanks.

3) All drainage facilities and erosion and sediment control structures shall be regularly inspected and maintained to ensure proper and efficient operation at all times and particularly following rainstorms.

4) The Contractor shall weekly check all equipment for prevention of oil and or lubrication leaks and ensure that all equipment oil and lubrication replacements are performed only in bunded maintenance and repair areas.

5) The Contractor shall at all times ensure that all existing stream courses and drains within, and adjacent to, the Site are kept safe and free from any debris and any excavated materials arising from the Works.

E. WASTE MANAGEMENT

1) Raw material requirements shall be planned at the outset of each construction activity to avoid excess material storage and wastage on-site.

Fill required for site formation of construction yards shall be sourced from cuttings required within the works areas only.

2) No burning of debris, construction wastes or vegetation shall be allowed on-site.

3) The Contractor shall segregate construction waste materials on-site to facilitate re-use, recycling and waste disposal practice in accordance with the best available technology, as follows:

- For construction waste deemed by the Engineer to be suitable for reclamation or land formation: the Contractor shall liaise with the Municipal Environmental Company of Hai Phong City to determine the appropriate location for reuse. Reuse shall not have a detrimental impact on the environment.
- For construction waste deemed by the Engineer to be unsuitable for reclamation or land formation: the Contractor shall classify wastes on-site with dedicated areas for each waste stream including but not limited to: wood/timber, metals and plastics.

4) Wastes shall be stored and handled in dedicated areas with bunded sides such a way as to avoid loss or leakage and subsequent pollution. Waste storage sites shall be located away from sensitive areas such as: residential, surface/groundwater. Designated waste storage areas shall be well maintained and cleaned regularly.

5) The Contractor shall enter into a contract with the Municipal Environment Company of Hai Phong City for the collection of domestic refuse. To facilitate waste collection the Contractor is required to designate locations on-site shielded from wind and rain.

The Contractor shall enter into a contract with the Municipal Environment Company of Hai Phong City or similar approved company for the collection of asbestos waste arising from demolition works. The Contractor shall employ within his team a specialist in the identification of asbestos containing material (ACM). On removal ACM shall not be broken, shall be kept dampened and shall be stored in a dedicated enclosed area on-site.

6) Waste oils, chemicals, paints and other such materials used for machinery maintenance and construction shall be collected and stored in bunded areas on-site for resale/re-use or managed disposal without resulting in damage or pollution of the environment.

4) In locations remote from the site offices the Contractor shall provide latrine pits in suitable locations for the convenience of the construction workforce. If any office, site canteen or toilet facilities are erected foul water effluent shall be directed to a sewage treatment facility either directly or indirectly by pumping.

7) All water and liquid waste products arising on the site shall be collected and removed from Site via a suitable and properly designed temporary drainage system and disposed of at a location and in a manner that will cause neither pollution nor nuisance.

Sewage from site toilets, kitchens and similar, shall be discharged to a septic tank and soak-away system. Grease traps shall be installed where canteen waste is collected and shall be capable of providing at least 20 minutes retention during peak flow, prior to discharge.

F. ECOLOGY AND LANDSCAPE

- 1) No vegetation of any type shall be removed from lands outside the works boundary.
- 2) The Contractor shall preserve all trees within the works boundaries if they are outside the permanent works areas and do not interfere with construction or operation of the project. During site clearance the Contractor shall minimize loss of mature trees, particularly those on the downward slope of the road;
- 3) Site fencing shall be erected on the border of all construction sites, storage areas etc. to avoid unnecessary off-site damage to vegetation, trees and the landscape generally. Construction personnel, equipment, and vehicles shall be confined to the works areas as defined by site fences/hoardings erected at the works boundary.
- 4) Exposed slopes created during the works shall be stabilized by planting grass, trees to minimize erosion.

All cut slopes, embankments and cleared areas shall be planting grass immediately after works to provide a greening effect to mitigate visual appearance of cuttings and shall be subsequently planted with trees; Native species shall be used in replanting schemes to increase potential ecological value of these restored areas; Cut and fill areas subject to erosion are to be covered with organic, biodegradable, erosion-control mats after planting.

G. CONSTRUCTION WORKFORCE

- 1) In order to minimize impacts of an influx of new people into the local community, wherever possible, suitable local companies and organizations should be involved. For unskilled manual labour, men and women from Dinh Vu and Cat Hai should be employed.
- 2) Should there be a large influx of new people, then they are likely to be housed on the site. Adequate living standards with suitable services (water supply, sanitation and power supplies) should be provided for these temporary housing areas. Effluents discharged from these premises should employ mitigation measures discussed under wastes and water quality above.
- 3) Opportunities to use local services (e.g. for food supplies etc) should also be encouraged so that the local population benefits from an influx of new people.

Appendix-9: Comparison Study on Investment Timing for Tan Vu - Lach Huyen Highway Project

The JICA Follow-up Mission was carried out from 7 June to 18 June 2010. During said period, a discussion paper was prepared by the Study Team as guide for discussion between JICA and MOT. The discussion results were summarized in the Minutes of Discussion (M/D) dated 18 June 2010.

This Appendix-9 contains the discussion paper submitted to JICA on 11 June 2010.

1. Alternatives

1.1. Parameters

The following parameters were considered in the comparison study on cross section elements for optimum investment.

- Number of lanes (4, 6)
- Width of lane (3.0 m, 3.5 m, 3.75 m)
- Timing of investment (2011, 2024¹)

1.2. Alternatives

The following four alternatives are considered for the comparison study.

Table 1 List of Alternatives

No.	Alternative Code	Number of Lanes	Lane Width (m)	Investment Timing	
				1st	2nd
1	4-3.0	4	3.0	2011	2024
2	4-3.5	4	3.5	2011	2024
3	6-3.5	6	3.5	2011	-
4	6-3.75	6	3.75	2011	-

1.3. Road Classification

In the F/S, the project road was classified as “Expressway”, while in this Study, it is classified as “Highway”.

Standard lane width for “Expressway” is different from that of “Highway” which is 3.75 m and 3.50 m, respectively.

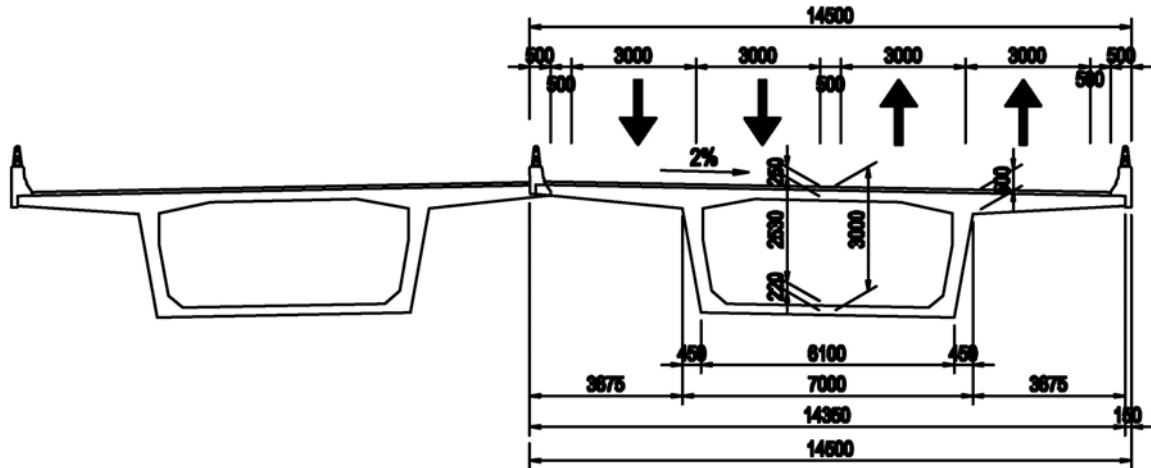
¹ Investment in 2024: Assumed that 2nd stage would open in 2027. Start investment in 2024.

Table 2 Road Classification (as per F/S)

Geometric Items	UNIT	TCVN 4054-98	TCVN 4054-2005	TCVN5729-97
Road classification		Technical class 80	Design category III	Class B-grade 80
		Highway	Highway	Expressway
Minimum of Design Volume PCU	PCU/day	≥ 3000	≥ 3000	$10,000 \geq V \geq 5,000$
Design speed	km/h	80	80	80
Cross Section				
Carriageway	m	2 x 3.50	2 x 3.50	4 x 3.75
Shoulder	m	2 x 3.00	2 x 2.50	2 x 3.25
Paved portion	m	2 x 2.50	2 x 2.00	2 x 2.50
Minimum width of Road bed	m	13.00	12.00	23.00
Median Separator(arranged for the 4 lane highway upward)				
Separator	m	——	——	0.50
Safety part	m	——	——	2 x 0.50
Minimum Width of Median	m	——	——	1.50

Typical cross sections of the alternatives are shown in Figures 1-4.

LIMITED 4-LANE, $B_{bridge} = 14.5m$
 $B_{lane} = 3.00m$ (DESIGN SPEED: 60km/h)
STAGE CONSTRUCTION: 1st stage - $B_{lane} = 3.0m$
2nd stage - $B_{lane} = 3.75m$



FULL SCALE 6-LANE, $B_{bridge} = 27.0m$
 $B_{lane} = 3.75m$ (DESIGN SPEED: 100km/h)
STAGE CONSTRUCTION: 1st stage - $B_{lane} = 3.0m$
2nd stage - $B_{lane} = 3.75m$

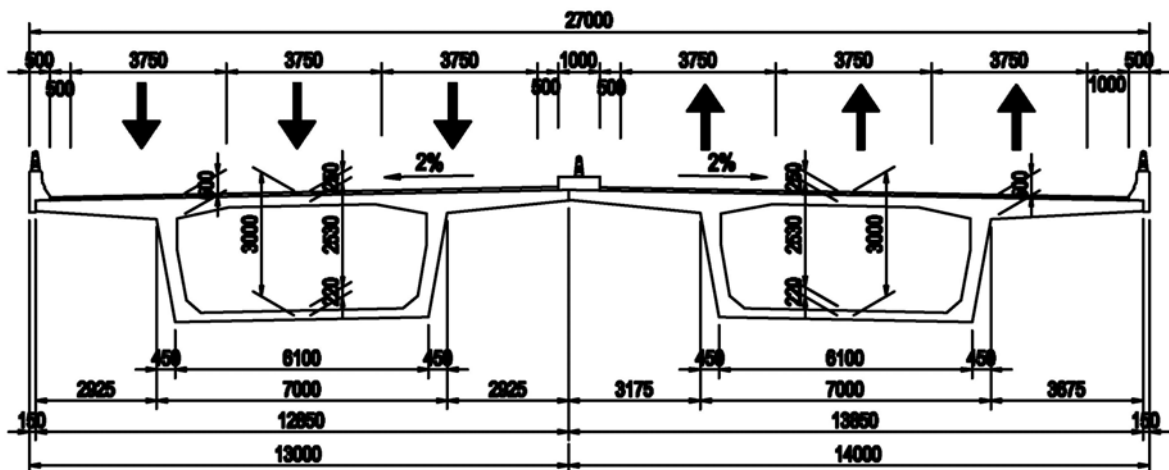


Figure 1 Typical Cross Sections of Alternative (1), Case 4-3.0

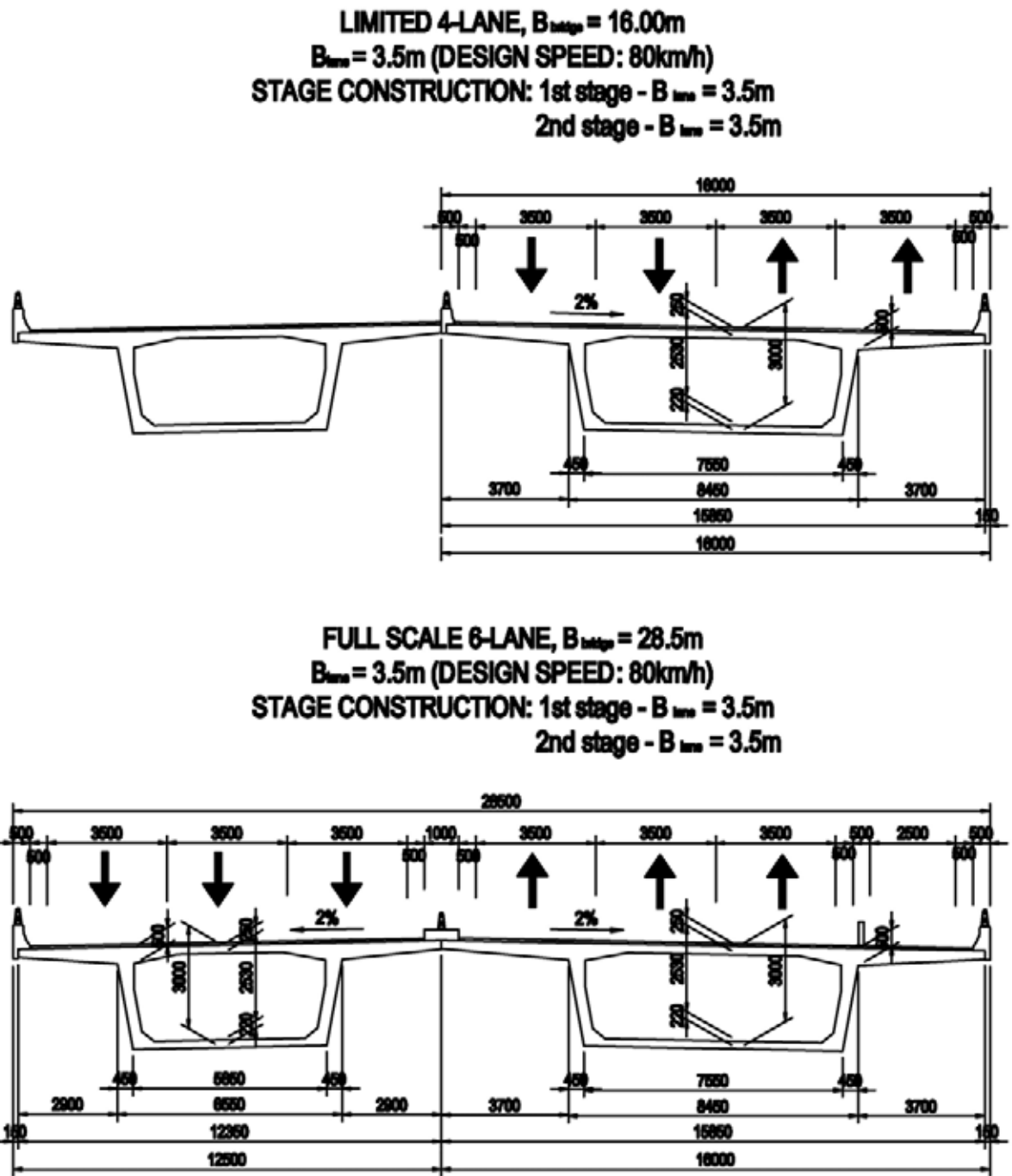


Figure 2 Typical Cross Sections of Alternative (2), Case 4-3.5

**FULL SCALE 6-LANE, $B_{total} = 25.0m$
 $B_{lane} = 3.5m$ (DESIGN SPEED: 80km/h)
 FULL CONSTRUCTION**

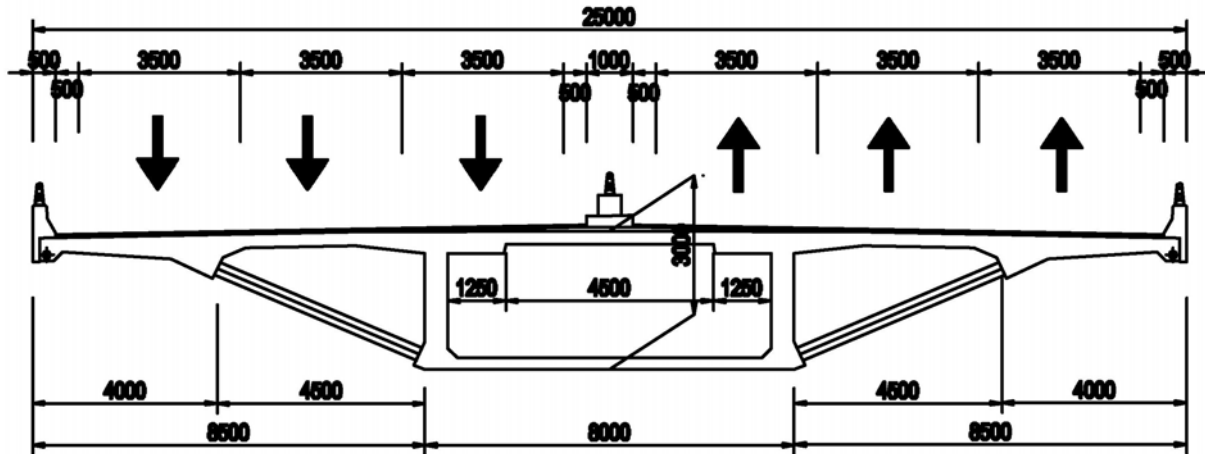


Figure 3 Typical Cross Sections of Alternative (3), Case 6-3.5

**FULL SCALE 6-LANE, $B_{total} = 26.5m$
 $B_{lane} = 3.75m$ (DESIGN SPEED: 100km/h)
 F/S STAGE - FULL CONSTRUCTION**

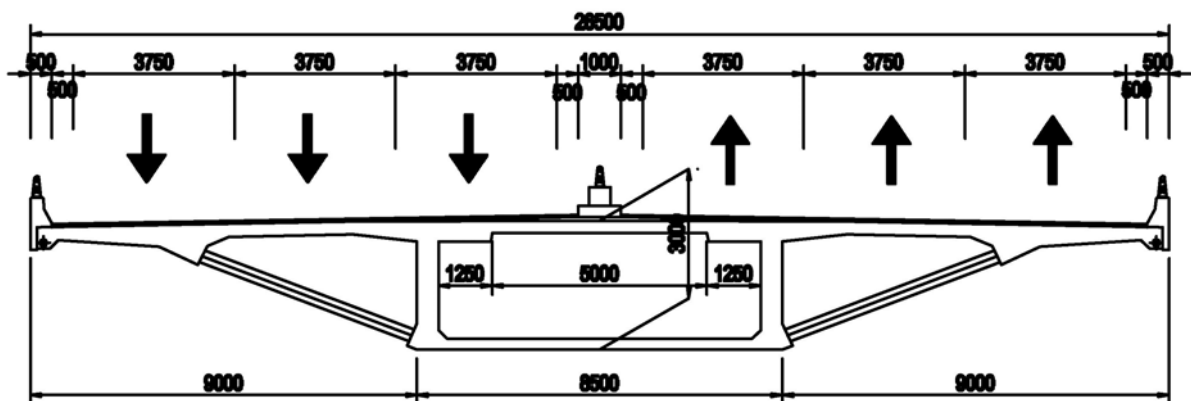


Figure 4 Typical Cross Sections of Alternative (4), Case 6-3.75

2. Future Lane Requirement

2.1. Update Base Data from the F/S

Table 3 shows the comparison between the F/S and this Study.

Table 3 Comparison of Input Data between the F/S and this Study

ITEM	FS Study	This Study
Traffic forecast method	➤ Use generation rate	➤ Use generation rate ➤ Prediction using a GDP rate of growth was carried out based on the traffic census as the abovementioned verification.
Analysis fiscal year	➤ 2015-2032	➤ 2015-2020 (First target) ➤ 2020-2030 (Second target)
Assumptions		
Conversion ratio of generating traffic	➤ Dinh Vu IZ:100%	➤ Dinh Vu IZ:50% ➤ Nam Dinh Vu IZ:80%
Development process	➤ Dinh Vu IZ: 2015(50% of 2020) 2020(100%) 2030(add 20%)	➤ Dinh Vu IZ: same as FS ➤ Nam Dinh Vu IZ: 2015(0%) 2020(0%) 2030(50%)
Population	➤ Cat Hai : 2015(31,000) 2020(33,000) 2030(38,500) ➤ Cat Ba : 2015(12,000) 2020(14,500) 2030(16,500) Based on statistical yearbook 2006	➤ Cat Hai : 2015(19,000) 2020(19,300) 2030(20,100) ➤ Cat Ba : 2015(12,000) 2020(13,000) 2030(14,600) Based on statistical yearbook 2008
Lach Huyen Port	Based on MOT Decision No.501	Based on result of Lach Huyen Port Preparatory Study

It is observed that:

- **Conversion ratio was too high in the F/S** considering the number of people from the industrial zone willing to use the project road. In the F/S, said ratio was 100%, which seems unrealistic. People working in the northern area of Dinh Vu Industrial Zone (IZ) will take TL356 and move to the city of Hai Phong. In this Study, the conversion rate was revised to 50% for Dinh Vu IZ and 80% for Nam Dinh Vu IZ.
- **Population in Cat Hai Island was counted twice.** In this Study, all socio-economic data was updated based on Statistical Yearbook 2008. It seems that population in Cat Hai Island indicated in the F/S was counted twice.

2.2. Future Lane Requirement

In this Study, future lane requirement was calculated in accordance with Section 4.2.2 of TCVN4054-2005.

Table 4 Future Lane Requirement (as per this Study)

Section	Peak Hour	Direction	Year							
			2015		2020		2025		2030	
Tan Vu IC - Dinh Vu	AM	To Tan Vu Interchange	1,276	2	2,149	3	3,145	4	4,140	5
		From Tan Vu Interchange	745	1	1,451	2	2,709	3	3,967	5
	PM	To Tan Vu Interchange	550	1	1,098	2	1,794	2	2,490	3
		From Tan Vu Interchange	1,125	2	1,874	2	2,823	3	3,772	4
Dinh Vu - Cat Hai	AM	Cat Hai to Dinh Vu	927	1	1,494	2	1,748	2	2,002	3
		Dinh Vu to Cat Hai	351	1	745	1	1,047	2	1,350	2
	PM	Cat Hai to Dinh Vu	351	1	745	1	1,047	2	1,350	2
		Dinh Vu to Cat Hai	927	1	1,494	2	1,748	2	2,002	3

Table 5 shows the future lane requirement in the F/S while the text box in the next page presents the capacity assumptions in the F/S.

Table 5 Future Lane Requirement (as per the F/S)

Section	Peak Hour	Direction	Year					
			2015		2022		2032	
			Future Traffic Demands	Lane Requirement	Future Traffic Demands	Lane Requirement	Future Traffic Demands	Lane Requirement
Tan Vu IC - Dinh Vu	AM	To Tan Vu Interchange	2,272	2	4,242	3	5,195	3
		From Tan Vu Interchange	1,304	2	2,751	3	3,949	3
Dinh Vu - Cat Hai	AM	Cat Hai to Dinh Vu	1,680	2	3,143	3	3,459	3
		Dinh Vu to Cat Hai	583	2	1,450	3	1,826	3

<BOX: Extraction from the F/S>

Capacity Assumptions

For a two-lane highway (i.e., two lanes in one direction), the current Vietnamese code of practice recommends a daily “Design Capacity” of 25,000 pcu/hr. If a peak hour factor of 12% is used, the “Design Capacity” will be about 3,000 pcu/hr. However, it is also understood that a capacity of 3,600 pcu/hr has been regarded and used as the “Practical Capacity” of a two-lane highway in the F/S. As a matter of fact, much higher capacity and traffic flows (i.e., about 4,800 pcu/hr) were commonly observed on many expressways and freeways around the world. For this analysis, **a capacity of 1800 pcu/hr per lane for expressway (3,600 for 2 lanes)** was assumed. For directional ramps, the practical capacity would be around 800-1000 pcu/hr/lane depending on the ramp design standards.

It is observed that

- **Too much traffic capacity was applied.** In Vietnamese expressway design standards, it is stipulated that the maximum traffic capacity is 2,000 pcu/hr/lane and Z-value, factor of using traffic capacity, is defined as $Z = 0.55$ for plain and hilly terrains (Section 4.5.3 of TCVN5729-2007). This results to $2,000 * 0.55 = 1,100$ pcu/hr/lane. It seems that traffic volume forecasted was too high. Therefore, **the traffic capacity was increased from 1,100 to 1,800.**

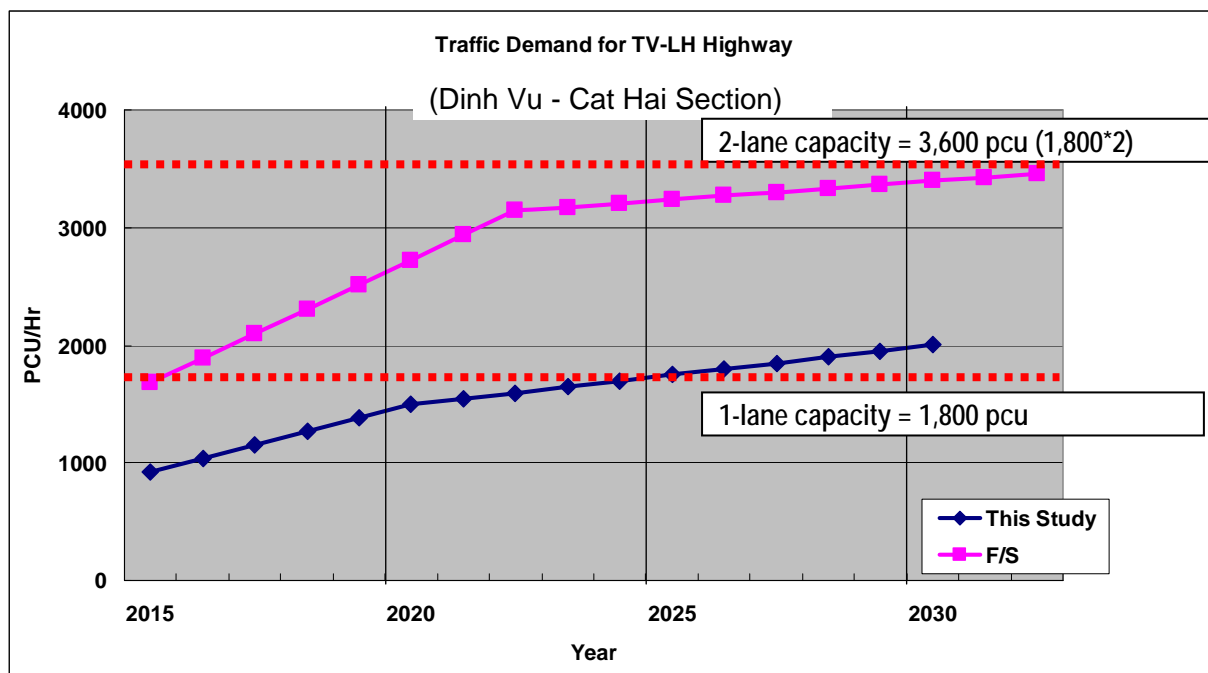


Figure 5 Future Lane Requirement (as per the F/S)

- In case 1,800 pcu/hr/lane is adopted to determine the required number of lanes as shown in Figure 5, **two-lane carriageway for one direction will still be sufficient in 2032 for Dinh Vu - Cat Hai Section, where a bridge structure is required.**
- In case 1,800 pcu/hr/lane is adopted as a result of traffic demand forecast in the F/S, the correct lane requirement should be as shown in Table 6. Hence, **three-lane expressway will not be necessary in 2032.**

Table 6 Future Lane Requirement (as per the F/S)

Section	Peak Hour	Direction	Year					
			2015		2022		2032	
			Future Traffic Demands	Lane Requirement	Future Traffic Demands	Lane Requirement	Future Traffic Demands	Lane Requirement
Tan Vu IC - Dinh Vu	AM	To Tan Vu Interchange	2,272	2	4,242	3	5,195	3
		From Tan Vu Interchange	1,304	1	2,751	2	3,949	3
Dinh Vu - Cat Hai	AM	Cat Hai to Dinh Vu	1,680	1	3,143	2	3,459	2
		Dinh Vu to Cat Hai	583	1	1,450	2	1,826	2

- In case TCVN4054 (990 pcu/hr/lane) is adopted as traffic volume in the F/S, **four-lane carriageway in one direction is required in 2022,** based on the number of lanes determined from Figure 5.

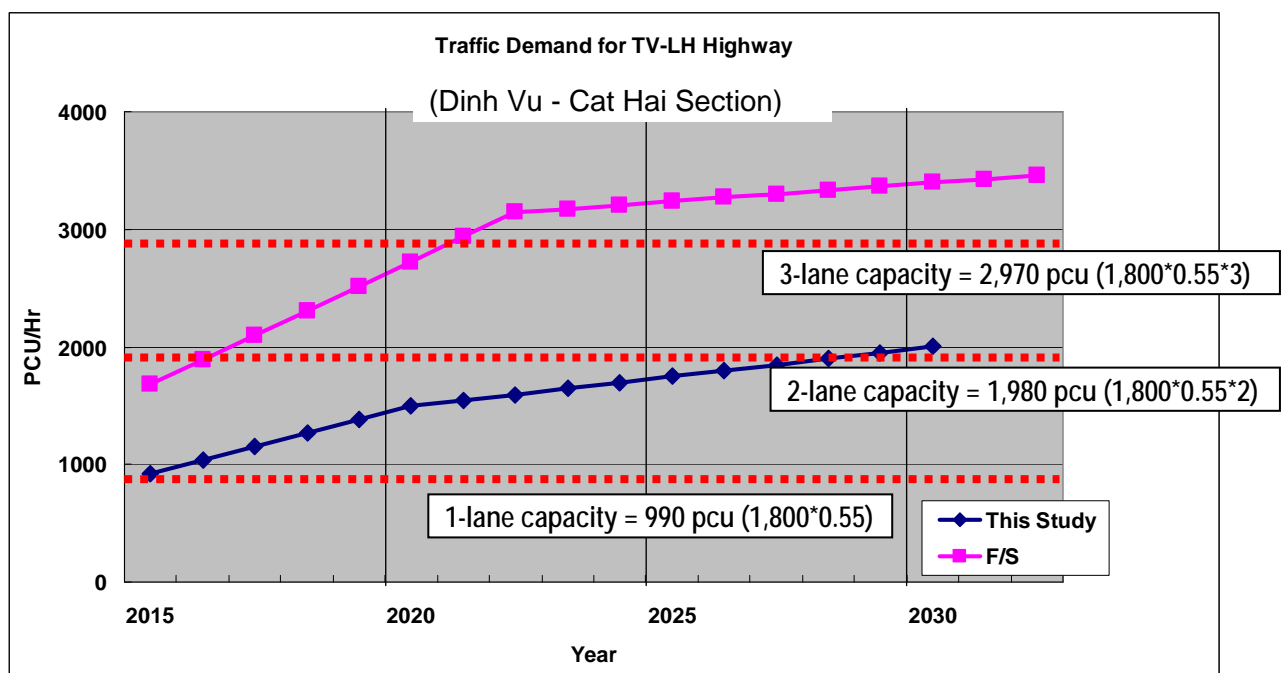


Figure 6 Future Lane Requirement (TCVN4054-2005)

2.3. Conclusion of Review of the Traffic Forecast in the F/S

In conclusion of the review of the F/S, the following are observed:

- Input data in the F/S was not correctly selected, and hence, the traffic volume forecasted is excessive.
- Traffic capacity in the F/S was set at 1,800 pcu/hr/lane, although this capacity does not meet the technical standards (TCVN5729-2007).
- In case this criterion is adopted, Dinh Vu - Cat Hai Section will not require three-lane per direction even in 2032.

2.4. Recommendation on Traffic Demand Capacity and Lane Requirement

- Traffic demand forecast was reviewed and updated in this Study.
- In this Study, TCVN4054 was applied to determine the required number of lanes.
- In this Study, it is concluded that **two-lane per direction could sufficiently accommodate traffic capacity until 2027.**
- This updated and technically correct determination of required number of lanes should be subject to further discussion.

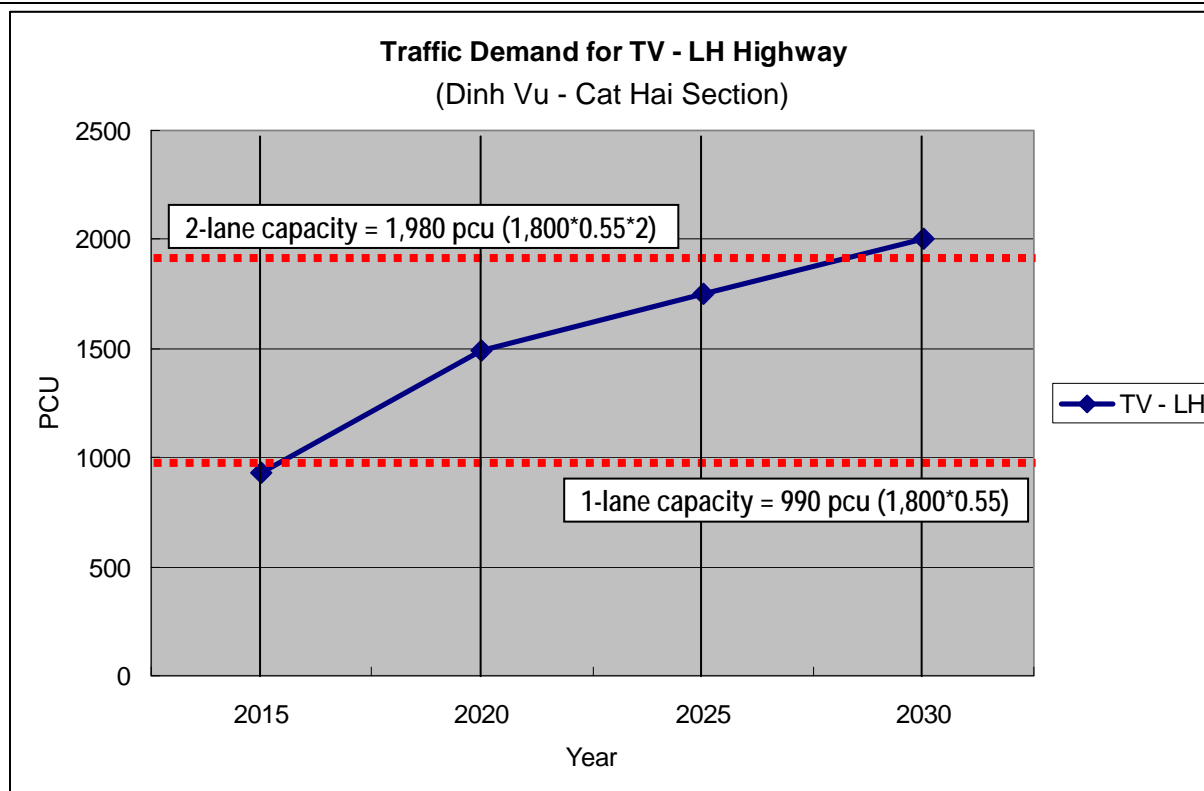


Figure 7 Future Lane Requirement

3. Project Cost

3.1. Premise and Conditions for Rough Cost Estimates

In order to compare the project benefit of each alternative, the following premises and conditions are considered.

Road Works

Table 7 Premises and Conditions for Cost Estimates of Road Works

No.	Alternative	Number of Lanes	Premise and Conditions
1	4-3.0	4	<1st Stage>
2	4-3.5	4	<ul style="list-style-type: none"> Complete 6-lane embankment. Complete 4-lane, 3.5 m wide pavement <2nd Stage> <ul style="list-style-type: none"> Complete 6-lane embankment. Complete 6-lane, 3.75 m wide pavement
3	6-3.5	6	<1st Stage>
4	6-3.75	6	<ul style="list-style-type: none"> Complete 6-lane embankment. Complete 6-lane, 3.75 m wide pavement

Bridge Works

Table 8 Premises and Conditions for Cost Estimates of Bridge Works

No.	Alternative	Number of Lanes	Premise and Conditions
1	4-3.0	4	<ul style="list-style-type: none"> Estimated cost based on quantities
2	4-3.5	4	<ul style="list-style-type: none"> Above estimated cost of (Alt. Code 4-3.0) multiply bridge area ratio $[(4-3.5)/(4-3.0)]$
3	6-3.5	6	<ul style="list-style-type: none"> Below estimated cost of (Alt. Code 6-3.75) multiply bridge area ratio $[(6-3.5)/(6-3.75)]$
4	6-3.75	6	<ul style="list-style-type: none"> Estimated cost based on quantities

3.2. Project Cost

In accordance with the above premises and conditions, the project cost is updated for each alternative as estimated below:

Table 9 Project Cost

No.	Code	Investment from 2011				Investment from 2024				Total				Rate (No.1 = 100)
		Project Cost (by Currency)		Project Cost (Total)		Project Cost (by Currency)		Project Cost (Total)		Project Cost (by Currency)		Project Cost (Total)		
		F/C (Mil.JPY)	L/C (Bil.VND)	F/C (Mil.JPY)	L/C (Bil.VND)	F/C (Mil.JPY)	L/C (Bil.VND)	F/C (Mil.JPY)	L/C (Bil.VND)	F/C (Mil.JPY)	L/C (Bil.VND)	F/C (Mil.JPY)	L/C (Bil.VND)	
1	4-3.0	6,734	6,838	43,110	8,103	3,981	3,714	23,739	4,462	10,715	10,551	66,849	12,566	100
2	4-3.5	7,295	7,125	45,202	8,497	3,851	3,616	23,088	4,340	11,146	10,741	68,289	12,836	102
3	6-3.5	5,248	10,002	58,460	10,989					5,248	10,002	58,460	10,989	87
4	6-3.75	5,483	10,352	60,555	11,382					5,483	10,352	60,555	11,382	91

4. Assumed Annual Disbursement

The following annual disbursement is assumed for comparison study. Commencement of works starting with land acquisition is assumed to be in 2011.

Table 10 Assumed Annual Disbursement

No.	Alternative	Construction Period	2011	2012	2013	2014	2015
1	4-3.0	30 months	2%	18%	60%	20%	-
2	4-3.5	30 months	2%	18%	60%	20%	-
3	6-3.5	36 months	2%	18%	40%	30%	10%
4	6-3.75	36 months	2%	18%	40%	30%	10%

2024	2025	2026
30%	40%	30%
30%	40%	30%
-	-	-
-	-	-

5. Economic Evaluation

5.1. Method of Evaluation

The evaluation is undertaken based on the following conditions:

- Project costs are converted into economic prices by applying a Standard Conversion Factor (SCF) of 0.85 to non-traded goods and services, and eliminating transfer payment such as tax and duty.
- Discount rate of 12% is applied.
- Evaluation period of 2011-2030 is assumed.
- For Alternative 4-3.0 and 4-3.5, the project costs are allocated to 1st stage during 2011-2014, and 2nd stage during 2024-2026, according to the above disbursement schedule.
- For Alternative 6-3.5 and 6-3.75, the project costs are allocated immediately during 2011-2015, according to the above disbursement schedule.

5.2. EIRR

Based on the above project costs and annual disbursement, EIRR and NPV for each alternative are calculated as follows:

Table 11 Comparison of EIRR and NPV among Alternatives

No.	Alternative	EIRR	NPV (Bil. VND)
1	4-3.0	39.1%	21,911
2	4-3.5	38.2%	21,738
3	6-3.5	30.9%	17,083
4	6-3.75	30.3%	16,914

5.3. Selection of Optimum Investment in terms of Economic Evaluation

As indicated above, Alternative 4-3.0 presents the highest EIRR and NPV, followed by Alternative 4-3.5. On the other hand, relatively greater amount of investment from year 2011 (Alternative 6-3.5 and 6-3.75) would result in much lower EIRRs as compared with stage construction scenarios. Although the overall project costs of four-lane alternatives are higher than those of six-lane alternatives, the present value of investment in the 2nd Stage from year 2024 would be discounted to a minimum. This results in less present value of investment costs and higher EIRRs.

Therefore, Alternative 4-3.0 (4 lanes with 3.0 m width, stage construction) is the most efficient and economically reasonable investment alternative among others.

6. Conclusion and Recommendation

- a) It was confirmed that six-lane highway is not required until 2027 based on traffic demand forecast.
- b) Four alternatives are developed for the comparison of number of lanes, lane width and investment timings in order to evaluate investment efficiency.
- c) Based on economic aspects, Alternative 1, four 3.0 m-wide lanes exhibits the best rating among the alternatives.

Table 12 Results of Economic Evaluation in EIRR and NPV

No.	Alternative	EIRR	NPV (Bil. VND)
1	4-3.0	39.1%	21,911
2	4-3.5	38.2%	21,738
3	6-3.5	30.9%	17,083
4	6-3.75	30.3%	16,914

Appendix-10:

Updates in Accordance with Minutes of Discussion dated 18 June 2010

The JICA Follow-up Mission was carried out from 7 June to 18 June 2010. During said period, a discussion paper was prepared by the Study Team as guide for discussion between JICA and MOT. The discussion results were summarized in the M/D dated 18 June 2010.

This Appendix-10 contains updates of the Study in accordance with the M/D.

1. Items Updated during the Discussion between JICA and MOT

The following items were updated during the discussion between JICA and MOT.

Table 1 List of Items Updated in the JICA Mission in June 2010

No.	Items	Updated in JICA Mission	Study Team Proposed
1	Target Year of Traffic Forecast	2035	2030
2	Typical Cross Section 1st Stage of Bridge Section	4-lanes, 3.5m/lane	4-lanes, 3.0m/lane
3	Construction Period	Aug 2012 – Mar 2015 (32 Months)	Jun 2012 – Dec 2014 (30 Months)
4	Implementation Program	Aug 2012 – Mar 2015 (32 Months)	Jun 2012 – Dec 2014 (30 Months)

2. Traffic Demand Forecast

In accordance with the above updates realized during the discussion between JICA and MOT, the following were also updated concerning the target year of the traffic demand forecast.

2.1. Future Traffic Demands

The traffic in 2035 was calculated by extrapolation from traffic volume of 2020-2030 with 85% growth rate, which is 6%.

Table 2 Summary of Future Traffic Demands (Updated after JICA Mission)

Section	Peak Hour	Direction	Year			
			2015	2020	2030	2035
Tan Vu IC - Dinh Vu	AM	To Tan Vu IC	1,276	2,149	4,140	5,337
		From Tan Vu IC	745	1,451	3,967	6,101
	PM	To Tan Vu IC	550	1,098	2,490	3,534
		From Tan Vu IC	1,125	1,874	3,772	5,086
Dinh Vu - Cat Hai	AM	Cat Hai to Dinh Vu	927	1,494	2,002	2,267
		Dinh Vu to Cat Hai	351	745	1,350	1,740
	PM	Cat Hai to Dinh Vu	351	745	1,350	1,740
		Dinh Vu to Cat Hai	927	1,494	2,002	2,267

2.2. Forecast of Transport Growth

It was assumed that the growth rate of 2031-2035 is 6% which is 85% of the previous period.

Table 3 Forecast of Transport Growth Rate (Updated after JICA Mission)

Stage	2010-2015	2016-2020	2021-2025	2026-2030	2031-2035
Nationwide	7.0%	6.5%	6.5%	6.0%	---
Hanoi-Haiphong Expressway	7.67%	7.67%	6.67%	6.67%	---
Preparatory survey on Lach Huyen Port (Road and Bridge portion)	8.00%	8.00%	7.00%	7.00%	6.00%

2.3. Traffic Demand Forecast for Comparison of Updated Traffic

It was assumed that the growth rate of 2031-2035 is 6% which is 85% of the previous period.

Table 4 Traffic Demand Forecast Based on Socio-economic Data
(Updated after JICA Mission)

Year	Dinh Vu-Cat Hai Ferry and Ninh Tiep Ferry terminal			Cat Hai Roa			Ben Got Ferry Terminal and Cat Hai-Cat Ba Ferry		
	Total (pcu/day-night)	To Tan Vu IC Direction (pcu/peak hr)	To Cat Ba Direction (pcu/peak hr)	Total (pcu/day-night)	To Tan Vu IC Direction (pcu/peak hr)	To Cat Ba Direction (pcu/peak hr)	Total (pcu/day-night)	To Tan Vu IC Direction (pcu/peak hr)	To Cat Ba Direction (pcu/peak hr)
2010	506	28	23	607	33	27	439	24	20
2011	547	30	25	655	36	29	474	26	21
2012	591	33	27	706	39	32	511	28	23
2013	638	35	29	763	42	34	551	30	25
2014	688	38	31	824	45	37	596	33	27
2015	3,913	215	176	4,060	223	183	643	35	29
2016	6,462	355	291	6,619	364	298	695	38	31
2017	9,026	496	406	9,196	506	414	751	41	34
2018	11,615	639	523	11,800	649	531	811	45	36
2019	14,216	782	640	14,417	793	649	875	48	39
2020	16,841	926	758	17,057	938	768	945	52	43
2021	23,032	1,267	1,036	23,263	1,279	1,047	1,011	56	45
2022	27,286	1,501	1,228	27,534	1,514	1,239	1,081	59	49
2023	31,852	1,752	1,433	32,116	1,766	1,445	1,156	64	52
2024	36,747	2,021	1,654	37,030	2,037	1,666	1,237	68	56
2025	41,965	2,308	1,888	42,268	2,325	1,902	1,323	73	60
2026*	33,743	1,856	1,518	34,066	1,874	1,533	1,416	78	64
2027*	37,885	2,084	1,705	38,231	2,103	1,720	1,515	83	68
2028*	42,254	2,324	1,901	42,626	2,344	1,918	1,622	89	73
2029*	46,851	2,577	2,108	47,248	2,599	2,126	1,735	95	78
2030*	51,703	2,844	2,327	52,128	2,867	2,346	1,857	102	84
2031	55,963	3,078	2,518	56,413	3,103	2,539	1,968	108	89
2032	60,463	3,325	2,721	60,941	3,352	2,742	2,086	115	94
2033	64,970	3,573	2,924	65,476	3,601	2,946	2,211	122	99
2034	69,482	3,822	3,127	70,018	3,851	3,151	2,343	129	105
2035	74,007	4,070	3,330	74,574	4,102	3,356	2,484	137	112

Source: Study Team

2026*-2030*: Railway transportation is taken into consideration.

The traffic in 2031-2035 was calculated based on 85% of the growth rate for 2020-2030, which is 6%.

Table 5 Estimated Cargo Volume and Container Vehicles (Updated after JICA Mission)

	Cargo	TEU	Truck of more than 3 axles			
	1000ton/Year	1000TEU	Vehicle/Year	Vehicle/day	pcu/day	pcu/peak hr
2015	5,394	463	463,000	1,268	3,170	317
2016	9,607	826	826,000	2,263	5,658	566
2017	14,962	1,191	1,191,000	3,263	8,158	816
2018	19,816	1,559	1,559,000	4,271	10,678	1,068
2019	24,671	1,928	1,928,000	5,282	13,205	1,321
2020	29,525	2,299	2,299,000	6,299	15,748	1,575
2021	37,061	3,192	3,192,000	8,745	21,863	2,186
2022	44,126	3,801	3,801,000	10,414	26,035	2,604
2023	51,726	4,455	4,455,000	12,205	30,513	3,051
2024	59,863	5,156	5,156,000	14,126	35,315	3,532
2025	68,536	5,903	5,903,000	16,173	40,433	4,043
2026	54,421	4,687	4,687,000	12,841	32,103	3,210
2027	61,243	5,275	5,275,000	14,452	36,130	3,613
2028	68,439	5,895	5,895,000	16,151	40,378	4,038
2029	76,011	6,547	6,547,000	17,937	44,843	4,484
2030	84,000	7,235	7,235,000	19,822	49,555	4,956
2031	91,001	7,838	7,838,000	21,474	53,685	5,369
2032	98,393	8,475	8,475,000	23,219	58,048	5,805
2033	105,786	9,112	9,112,000	24,964	62,410	6,241
2034	113,178	9,748	9,748,000	26,707	66,768	6,677
2035	120,571	10,385	10,385,000	28,452	71,130	7,113

Source: Study Team

2026*-2030*: Railway transportation is taken into consideration.

The traffic of 2031-2035 was calculated based on 85% of the growth rate of 2020-2030, which is 6%.

**Table 6 Traffic Forecast between Revised F/S and Traffic Survey Basis
(Updated after JICA Mission)**

Forecast Method	Peak Hour	Direction	Year				
			2015	2020	2025	2030	2035
Revised FS Traffic Forecast	AM	Cat Hai to Dinh Vu	927	1,494	1,748	2,002*	2,267
		Dinh Vu to Cat Hai	351	745	1,047	1,350*	1,740
	PM	Cat Hai to Dinh Vu	351	745	1,047	1,350*	1,740
		Dinh Vu to Cat Hai	927	1,494	1,748	2,002*	2,267
Based on Traffic Survey in Cat Hai	AM	Cat Hai to Dinh Vu	215	926	2,308	2,844*	4,070
		Dinh Vu to Cat Hai	176	758	1,888	2,327*	3,330
	PM	Cat Hai to Dinh Vu	176	758	1,888	2,327*	3,330
		Dinh Vu to Cat Hai	215	926	2,308	2,844*	4,070

Source: Study Team

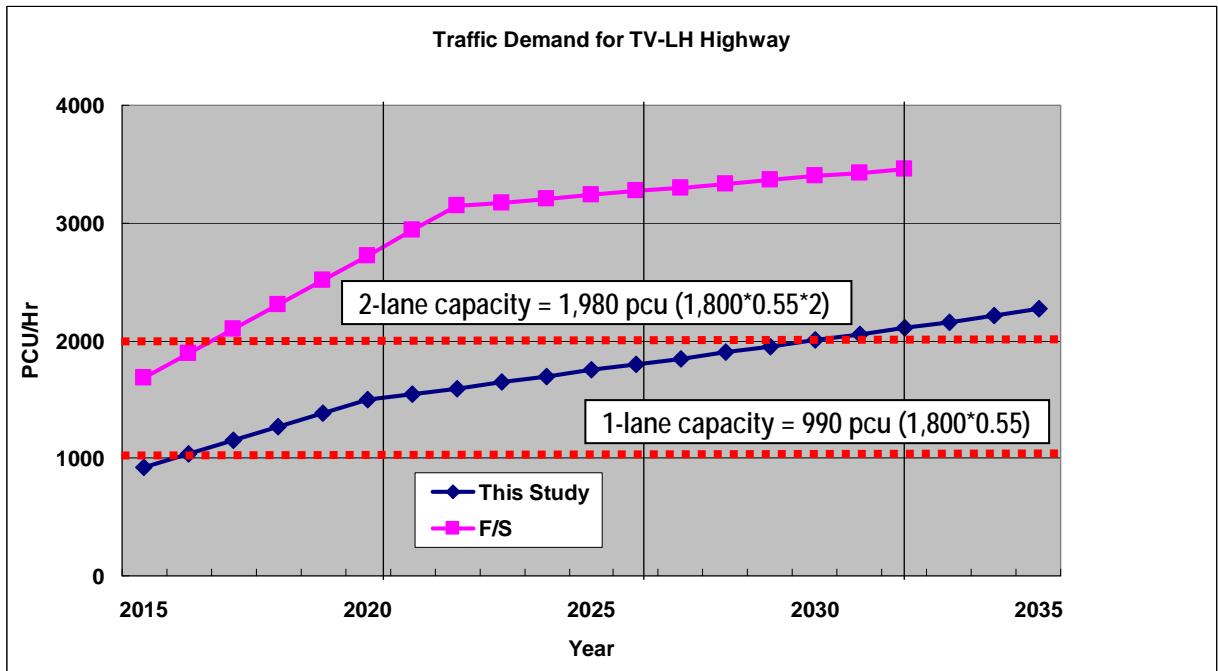
2030: Railway transportation is taken into consideration.

3. Typical Cross Section of Bridge Section

In accordance with the above updates, the typical cross section for the bridge section was updated during the discussion between JICA and MOT, as follows:

3.1. Updated Traffic Demand Forecast

In accordance with the updated traffic forecast, the required traffic lane is revised as follows:



Source: Study Team

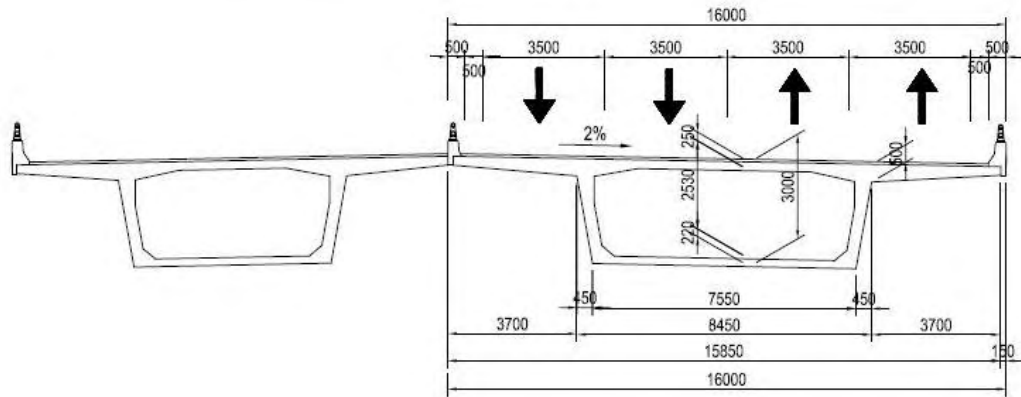
Figure 1 Updated Traffic Demand and Future Lane Requirement

- Traffic demand forecast is updated by extending the target year to 2035, which is 20 years after the highway is opened to the public.
- In this Study, it is concluded that two lanes per direction could sufficiently accommodate traffic capacity until 2027.

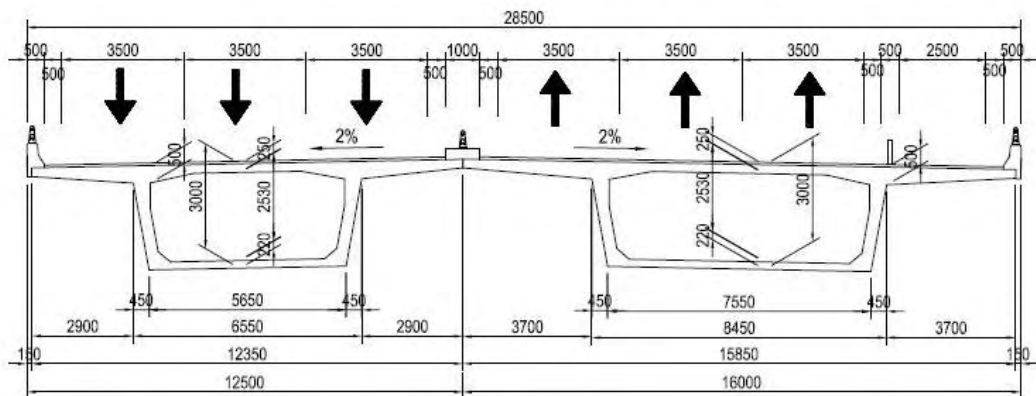
3.2. Updated Typical Cross Section of the Bridge

Typical cross section of the bridge during the first stage was updated as shown in figure below.

LIMITED 4-LANE, $B_{bridge} = 16.00m$
 $B_{lane} = 3.5m$ (DESIGN SPEED: 80km/h)
 STAGE CONSTRUCTION: 1st stage - $B_{lane} = 3.5m$
 2nd stage - $B_{lane} = 3.5m$



FULL SCALE 6-LANE, $B_{bridge} = 28.5m$
 $B_{lane} = 3.5m$ (DESIGN SPEED: 80km/h)
 STAGE CONSTRUCTION: 1st stage - $B_{lane} = 3.5m$
 2nd stage - $B_{lane} = 3.5m$



Source: Study Team

Figure 2 Updated Typical Cross Section of Bridge Section

4. Updated Project Cost Estimate

4.1. Project Cost

The following tables were updated considering 1) revision of the typical cross section of the bridge and 2) construction period of 32 months.

Table 7 Summary of Estimated Project Cost

Cost Items	Project Cost (by Currency)		Project Cost (Currency Exchange)	
	F/C (Mil.JPY)	L/C (Mil.VND)	F/C (Mil.JPY)	L/C (Mil.VND)
Total Project Cost	7,773	7,384,236	47,057	8,845,326
I JICA Loan Eligible Portion (STEP Scheme)	7,773	5,925,775	39,298	7,386,865
1 Construction Cost	5,390	3,954,100	26,426	4,967,258
2 Price Contingency	337	1,602,431	8,862	1,665,777
3 Physical Contingency	286	277,827	1,764	331,586
4 Consulting Services	1,115	91,417	1,601	301,003
5 Interest during Construction	333	0	333	62,594
6 Commitment Charge	312	0	312	58,647
II State Budget Portion	0	1,458,461	7,759	1,458,461
7 Environmental Management and Monitoring Cost	0	304,424	1,620	304,424
8 Administration Cost	0	378,506	2,014	378,506
9 Value Added Tax (VAT)	0	741,622	3,945	741,622
10 Import Tax	0	33,909	180	33,909

Source: Study Team

4.2. Procurement Ratio from Japan

Table 8 shows the procurement ratio from Japan.

Table 8 Procurement ratio from Japan

Unit: Yen

Construction Cost	26,426,000,000	
Goods procured from Japan		
1 Erection Girder	662,445,651	2.5%
2 Steel Pipe Pile	2,488,599,940	9.4%
3 Steel Pipe Sheet Pile	1,834,987,272	6.9%
4 PC Strand	678,996,366	2.6%
5 Reinforcement Steel	1,168,310,556	4.4%
6 Cement	368,364,482	1.4%
7 Steel Sheet Pile for Cofferdam	1,717,440,239	6.5%
8 H-shaped Steel for Jetty	429,413,302	1.6%
9 Japanese Engineer	315,061,980	1.2%
10 Japanese Skilled Labor	187,174,300	0.7%
11 Administration Overhead	1,770,202,690	6.7%
Total	11,620,996,780	44.0%

Source: Study Team

The contents and cost of major goods expected to be procured from Japan are shown in the Table 9.

Table 9 Goods expected to be Procured from Japan

Supplier	Item	Unit	Qty	Unit Price (JPY)	Amount(JPY)
Japanese Firm	Erection Girder	m ³	45,310	14,620	662,445,651
	Steel Pipe Pile	ton	11,087	224,452	2,488,599,940
	Steel pipe Sheet Pile	ton	6,961	263,606	1,834,987,272
	PC Strand	ton	2,716	250,032	678,996,366
Japanese Firm in Vietnam	Reinforcement Bar	ton	15,313	76,251	1,168,310,556
	Cement	ton	67,290	5,474	368,364,482
	Steel Sheet Pile	ton	11,404	131,929	1,504,533,710
	Equipment for Steel Sheet Pile	m	90,489	2,353	212,906,530
	H-shaped Steel	ton	3,367	99,102	333,711,630
	Equipment for Driving/Extracting H-shaped Steel	m	52,273	1,831	95,701,672

Source: Study Team

5. Project Evaluation

5.1. Estimated Project Benefit

The project benefit is re-estimated with the design speed of 80km/hr under the condition where the road width is set at 3.5m per lane. Assumed transportation routes and conditions as well as comparison with “With Project Case” are presented in Figure 3. The general conditions for benefit calculation are shown in Table 10.

Table10 General Conditions for Benefit Calculation

General Conditions for Passengers Traffic - Without Project Case

Without Project	Tan Vu IC-Dinh Vu	Dinh Vu-Ninh Tiep (Ferry)	Ninh Tiep-Ben Got	Total
Distance (km)	15	--	7.7	22.7
Travel Time (min)	45	90	20	155.0
Ave. Speed (km/hr)	20.0	--	23.1	

General Conditions for All Traffic - With Project Case

With Project	Tan Vu IC-Dinh Vu	Dinh Vu - Ben Got	Total
Distance (km)	4.50	11.37	15.9
Travel Time at 50km/hr speed (min)	5.4	13.6	19.0
Travel Time at 80km/hr speed (min)	3.4	8.5	11.9

Source: Study Team

Based on the above conditions, travel times by type of vehicle for “With” and “Without” project cases were estimated in Table 11. Daily values for VOC and TTC per vehicle were also computed in Table 11 in which detailed calculations for each unit by road sections are noted under each table for explanation. For the “With Project Case”, according to the four numbers of lanes and the road width of 3.5 m per lane, design speed of 80 km per hour is applied for VOC and TTC calculation. The same VOC and TTC units and formula presented in the main text Section 3.2.2 (3) and (4) are applied for the calculation.

Table 11 Travel Time by Type of Vehicle – With and Without Project Cases

Travel Time by Type of Vehicles and of Container Transportation - With and Without Project Cases

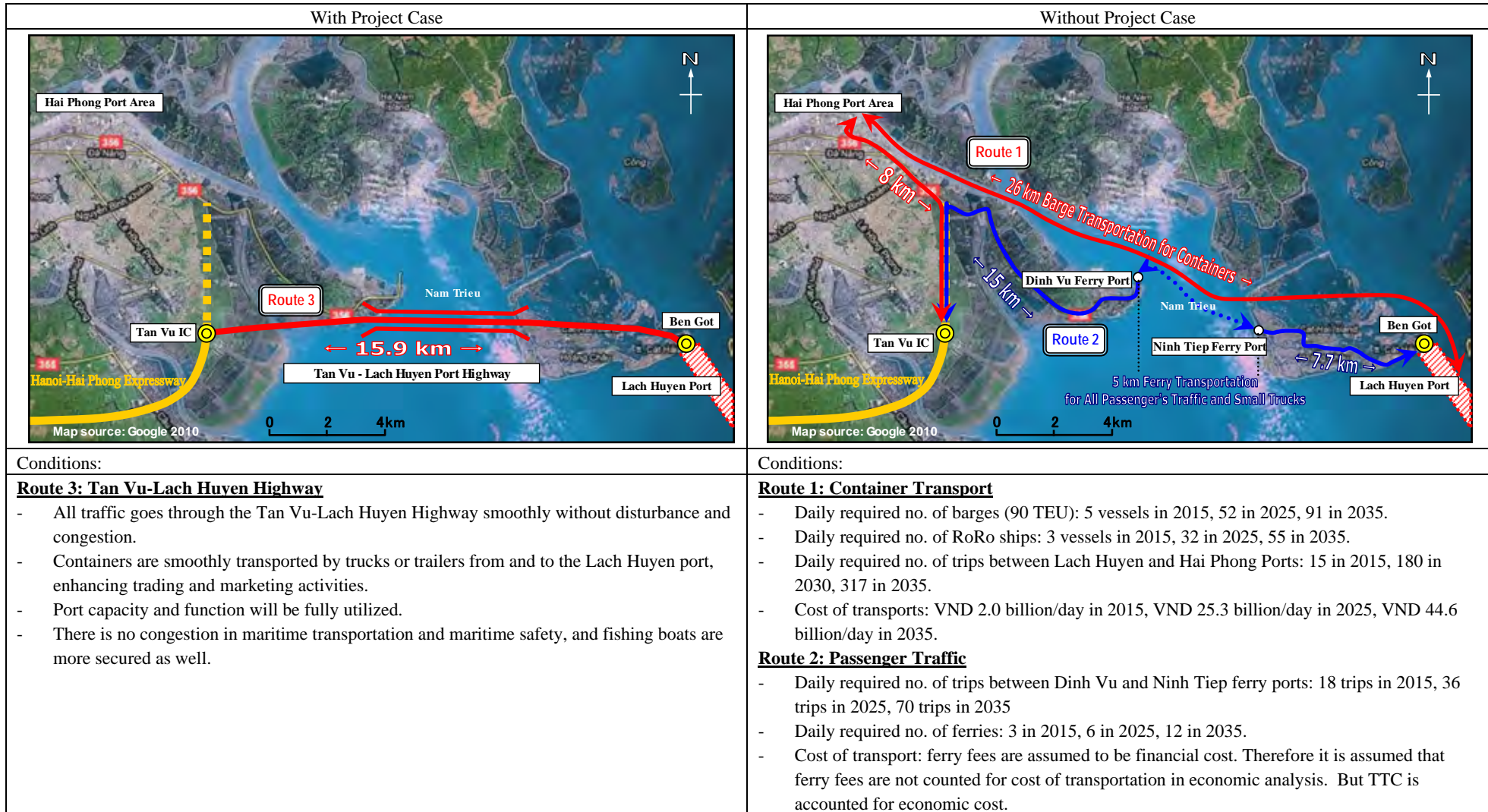
Route	Without Project	Tan Vu-Dinh Vu (15km)	Dinh Vu-Ninh Tiep (Ferry)	Ninh Tiep-Ben Got (7.7km)	Total
Route 2	Bicycle (min.)	90	90	40	220
	Motorcycle, Car, Bus (min.)	45	90	20	155

Route	Without Project	Port handling (transshipping)	Transport (Lach Huyen - Hai Phong)	Transport (Hai Phong - Tan Vu IC: 8km)	Total
Route 1	Container transport (Maritime)	24 hrs / 90TEU barge	120		1,560
	Container transport (Ground Transport by Trailers) at average 40km/hr speed			12	12

Route	With Project	Tan Vu IC - Dinh Vu (4.5km)	Dinh Vu - Ben Got (11.4km)	Tan Vu - Ben Got (15.9km)	Remarks
Route 3	Bicycle (min.)	27	68	95	Average 10km/hr speed
	Motorcycle, Car, Bus, Trailer* (min.)	5	14	19	Design speed: 50km/hr
		3	9	12	Design speed: 80km/hr

*For benefit calculation, the design speed at 80 km / hr is adapted.

Source: Study Team



Source: Study Team

Figure 3 Comparison of the “With” and “Without” Project Cases

Table 12 Values of VOC and TTC per Vehicle per Day – With and Without Project Cases

Unit VOC for Passengers' Traffic (per vehicle per day) - **With and Without Project Cases** (Unit: 1000VND/vehicle/day)

Route	Condition	Motorcycle	Car	Mini Bus	Large Bus	Motorcycle	Car	Mini Bus	Large Bus	Motorcycle	Car	Mini Bus	Large Bus
Route 2	Without Project*	Tan Vu IC - Dinh Vu (15km)				Ninh Tiep - Ben Got (7.7km)				Tan Vu IC - Ben Got			
		15.2	41.2	56.6	56.6	7.2	19.8	27.2	27.2	22.3	61.0	83.8	83.8
Route 3	With Project**	Tan Vu IC - Dinh Vu (4.5km)				Dinh Vu - Ben Got (11.4km)				Tan Vu IC - Ben Got			
		2.0	6.4	9.0	9.0	5.1	16.2	22.6	22.6	7.1	22.6	31.6	31.6
Unit VOC Saving =										15.2	38.4	52.2	52.2

*Ex.): Motorcycle VOC for Tan Vu IC - Dinh Vu Section under Without Project Case = 5909 x 20km/hr^{-0.589} x 15km = 15,181 VND/vehicle/day

**Ex): Motorcycle VOC for Tan Vu IC - Dinh Vu Section under With Project Case = 5909 x 80km/h^{-0.589} x 4.5km = 26,548 VND/vehicle/day

Unit TTC for Passengers' Traffic (per vehicle per day) - **With and Without Project Cases** (Unit: 1000VND/vehicle/day)

Route	Condition	Bicycle	Motorcycle	Car	Mini bus	Large bus	Bicycle	Motorcycle	Car	Mini bus	Large bus	Bicycle	Motorcycle	Car	Mini bus	Large bus
Route 2	Without Project*	Tan Vu IC - Dinh Vu including 90 min. ferry (15km)				Ninh Tiep-Ben Got (7.7km)				Tan Vu IC - Ben Got						
		43.8	32.9	155.7	328.5	887.0	9.7	4.9	23.1	48.7	131.4	53.5	37.7	178.8	377.2	1,018.4
Route 3	With Project**	Tan Vu IC - Dinh Vu (4.5km)				Dinh Vu - Ben Got (11.4km)				Tan Vu IC - Ben Got						
		6.6	0.8	3.9	8.3	22.3	16.5	2.1	9.8	20.7	55.8	23.1	2.9	13.7	29.0	78.2
Unit TTC Saving =												30.4	34.8	165.0	348.2	940.2

*Ex.): Bicycle TTC for Tan Vu IC - Dinh Vu Section under Without Project Case = (90min. + 90min.)/60 x 14,600 = 43,800 VND/vehicle/day

Car TTC for Tan Vu IC - Dinh Vu Section under Without Project Case = (45min. + 90min.)/60 x 69,200 = 155,700 VND/vehicle/day

**Ex): Bicycle TTC for Tan Vu IC - Dinh Vu Section under With Project Case = 27min./60 x 14,600 = 6,570 VND/vehicle/day

Car TTC for Tan Vu IC - Dinh Vu Section under With Project Case = 3.4min./60 x 69,200 = 3,921 VND/vehicle/day

VOC and TTC for Container Transportation by Trailers - With and Without Project Cases

Route	Conditions	VOC (1000VND / vehicle / day)	TTC (1000VND / vehicle / day)	Remarks
Route 1	Without Project	38.5	5.8	Hai Phong-Tan Vu IC: 8km section only
Route 3	With Project	53.1	5.8	Tan Vu IC-Ben Got: 15.9km whole section
Unit Saving =		-14.6	0.0	

*Ex): VOC under Without Project Case = 33065 x 40km/hr^{-0.5227} x 8km = 38,465 VND/vehicle/day

TTC under Without Project Case = 29,200 x 12min./60 = 5,840 VND/vehicle/day

**Ex): VOC under With Project Case = 33065 x 80km/hr^{-0.5227} x 15.9km = 53,113 VND/vehicle/day

TTC under With Project Case = 29,200 x 12min./60 = 5,791 VND/vehicle/day

Note: VOC and TTC savings associated with container transport can not be simply compared due to difference in the travel distances between With and Without Project Cases.

Source: Study Team

The following table presents the traffic demand forecast used for VOC and TTC calculations. It is basically the same as presented in Table 3.2-7 of the main text.

Table 13 Traffic Demand Forecast Used for Benefit Calculation

Daily Traffic Volume - Without Project Case (Unit: Vehicle/day)

Year	Tan Vu IC - Dinh Vu						Ninh Tiep - Ben Got					
	Bicycle	Motorcycle	Car	Minibus	Large Bus	Trailer	Bicycle	Motorcycle	Car	Minibus	Large Bus	Trailer
2010	165	640	22	119	9	0	645	673	291	111	10	0
2011	178	691	24	128	10	0	697	727	314	119	11	0
2012	192	746	26	138	11	0	753	785	339	129	12	0
2013	207	806	28	149	12	0	813	848	366	139	13	0
2014	224	870	30	161	13	0	878	916	395	150	14	0
2015	242	940	32	174	14	1,268	948	989	427	162	15	0
2016	261	1,015	35	188	15	2,263	1,024	1,068	461	175	16	0
2017	282	1,096	38	203	16	3,263	1,106	1,153	498	189	17	0
2018	305	1,184	41	219	17	4,271	1,194	1,245	538	204	18	0
2019	329	1,279	44	237	18	5,282	1,290	1,345	581	220	19	0
2020	355	1,381	48	256	19	6,299	1,393	1,453	627	238	21	0
2021	380	1,478	51	274	20	8,745	1,491	1,555	671	255	22	0
2022	407	1,581	55	293	21	10,414	1,595	1,664	718	273	24	0
2023	435	1,692	59	314	22	12,205	1,707	1,780	768	292	26	0
2024	465	1,810	63	336	24	14,126	1,826	1,905	822	312	28	0
2025	498	1,937	67	360	26	16,173	1,954	2,038	880	334	30	0
2026	533	2,073	72	385	28	18,841	2,091	2,181	942	357	32	0
2027	570	2,218	77	412	30	21,452	2,237	2,334	1,008	382	34	0
2028	610	2,373	82	441	32	24,151	2,394	2,497	1,079	409	36	0
2029	653	2,539	88	472	34	27,937	2,562	2,672	1,155	438	39	0
2030	699	2,717	94	505	36	31,822	2,741	2,859	1,236	469	42	0
2031	741	2,880	100	535	38	36,474	2,905	3,031	1,310	497	45	0
2032	785	3,053	106	567	40	41,219	3,079	3,213	1,389	527	48	0
2033	832	3,236	112	601	42	46,964	3,264	3,406	1,472	559	51	0
2034	882	3,430	119	637	45	52,707	3,460	3,610	1,560	593	54	0
2035	935	3,636	126	675	48	58,452	3,668	3,827	1,654	629	57	0

Daily Traffic Volume - With Project Case (Unit: Vehicle/day)

Year	Tan Vu IC - Dinh Vu						Dinh Vu - Ben Got					
	Bicycle	Motorcycle	Car	Minibus	Large bus	Trailer	Bicycle	Motorcycle	Car	Minibus	Large bus	Trailer
2015	42,400	65,800	3,960	534	234	1,791	26,900	41,533	2,500	338	148	1,135
2016	47,700	72,600	5,420	652	277	2,180	30,000	45,667	3,420	411	174	1,367
2017	52,600	78,667	7,120	784	319	2,586	32,900	49,333	4,460	490	201	1,624
2018	57,000	83,667	9,040	911	360	3,000	35,600	52,267	5,640	568	225	1,872
2019	61,100	88,067	11,180	1,058	402	3,459	38,100	54,867	6,960	662	251	2,160
2020	64,800	91,200	13,540	1,221	447	3,952	40,300	56,733	8,420	760	278	2,458
2021	67,600	97,733	16,120	1,458	538	4,829	39,300	56,733	9,340	849	312	2,803
2022	69,300	103,267	18,920	1,718	628	5,755	37,900	56,467	10,340	941	343	3,150
2023	69,800	107,600	21,780	2,002	722	6,800	36,300	55,933	11,320	1,039	375	3,531
2024	69,200	110,933	24,960	2,307	823	7,909	34,300	55,133	12,400	1,144	409	3,926
2025	67,400	113,200	28,340	2,632	924	9,122	32,200	54,067	13,540	1,260	441	4,362
2026	64,300	114,333	31,900	2,994	1,020	10,364	29,600	52,667	14,700	1,378	470	4,775
2027	60,100	114,400	35,660	3,380	1,119	11,750	26,900	51,133	15,940	1,512	500	5,251
2028	54,700	113,400	39,480	3,790	1,214	13,180	23,700	49,200	17,160	1,646	527	5,723
2029	48,200	111,267	43,640	4,231	1,313	14,686	20,400	47,067	18,480	1,788	555	6,210
2030	40,500	108,067	48,000	4,702	1,413	16,279	16,800	44,667	19,860	1,943	585	6,732
2031	38,600	112,333	52,800	5,379	1,490	17,888	15,300	44,600	20,960	2,135	591	7,100
2032	35,900	115,800	57,700	6,109	1,559	19,533	13,700	44,333	22,080	2,338	597	7,481
2033	32,300	118,600	62,740	6,891	1,619	21,213	12,000	43,933	23,260	2,556	600	7,862
2034	28,100	120,600	67,900	7,733	1,672	22,924	10,100	43,400	24,440	2,781	601	8,247
2035	22,900	122,000	73,200	8,645	1,736	24,634	8,000	42,733	25,640	3,026	608	8,627

Source: Study Team

The major project benefit from barge transportation cost under “Without” Project Case is calculated based on Table 14 below. These comprise port handling charges, economic costs of ship hiring, and fuel costs based on the container demand forecast at Lach Huyen Port.

Table 14 Benefit Calculation for Barge Transportation of Containers under “Without” Project Case

Port Handling Charge for Transshipment			
	Port	US\$/TEU	VND/TEU
(I)	Lach Huyen	40	680,080
(II)	Hai Phong	40	680,080

Source: Data from existing Hai Phong - Cai Lan maritime transportation

Economic Prices Estimate for Lach Huyen-Hai Phong Barge Transport

	Item	Value	Remarks
(a)	Ship hiring cost (RoRo ship) (VND/day/ship)	13,812,500	414 mil.VND per month hiring basis
(b)	Barge hiring cost (90TEU equivalent) (VND/day/ship)	9,668,800	290 mil.VND per month hiring basis
(c)	Diesel gasoline requirement* (liter/hr)	540	120 liter/hr x 90TEU/20TEU
(d)	Fuel cost (VND/ship/trip)	13,500,000	(c) x (g) x (i)
(e)	Distance (km) (Lach Huyen-Hai Phong)	26.0	
(f)	Speed at full container (km/hr)	14.8	8 knot x 1.852
(g)	Time required per trip (hr)	2.0	(e) / (f)
(h)	Marine Diesel Oil Price** (\$US/MT)	587	
(i)	Liter equivalent of Marine Diesel Oil Price (VND/liter)	12,500	(h) / 1,000kg x 0.8 x VND17,002

*Based on the data of barge transportation Hai Phong-Cai Lan (48km) at 20 million VND/month for 20TEU barge, 120 liter/hr of diesel fuel requirement, 8 knot ave. speed.

**As of May 25 at Singapore MDO price (Source: <http://www.bunkerworld.com/markets/prices/sg/sin/>)

Cost Estimation for the Barge Transportation of Containers under Without Project Case

	(1)	(2)	(3)	(4)	(5)	(6)
Year	Container Demand TEU/day	No. Trips/day	No. Barge/day	No. RoRo Ship/day	Costs of Daily Transports* (Mil.VND/day)	Costs of Annual Transports** (Mil.VND/year)
2015	1,268	15	5	3	2,017	736,192
2016	2,263	26	8	5	3,575	1,305,041
2017	3,263	37	11	7	5,141	1,876,372
2018	4,271	48	14	9	6,717	2,451,675
2019	5,282	59	17	11	8,297	3,028,468
2020	6,299	70	20	12	9,872	3,603,197
2021	8,745	98	28	17	13,723	5,008,945
2022	10,414	116	34	21	16,350	5,967,570
2023	12,205	136	39	24	19,145	6,988,048
2024	14,126	157	45	27	22,141	8,081,521
2025	16,173	180	52	32	25,373	9,261,016
2026	12,841	143	41	25	20,138	7,350,387
2027	14,452	161	46	28	22,662	8,271,647
2028	16,151	180	52	32	25,343	9,250,093
2029	17,937	200	58	35	28,141	10,271,618
2030	19,822	221	64	39	31,102	11,352,260
2031	21,474	239	69	42	33,682	12,293,875
2032	23,219	258	74	45	36,402	13,286,587
2033	24,964	278	80	48	39,145	14,287,756
2034	26,707	297	85	51	41,862	15,279,476
2035	28,452	317	91	55	44,618	16,285,687

*Costs of Daily Transportation (5) = (1)x(I) + (II) + (4)x(a) + (3)x(b) + (2)x(d)

**Costs of Annual Transportation (6) = (5) x 365

Source: Study Team

5.2. Economic Evaluation

(1) General Conditions for the Evaluation

The following conditions are assumed for the purpose of the economic evaluation:

- Price level is adapted to 2010 constant prices
- Economic project life is set for 2012-2035.
- Standard Conversion Factor (SCF) at 0.85 is applied for non-traded goods and services in the project costs, benefits and O&M costs
- Major rehabilitation works are assumed every seven years after the opening
- Procurement costs for O&M equipment and materials are assumed at 5% of the construction costs
- Opportunity cost of capital (discount rate) is set at 12%.
- Construction period is assumed at 32 months during 2012-2015

(2) Economic Project Cost and Investment Schedule

All the costs and benefits, estimated from market prices, need to be converted into economic terms in the economic analysis by excluding price escalation and transfer items such as taxes and subsidies. In this study, the Standard Conversion Factor (SCF) is at 0.85, which is generally used in the recent studies in Vietnam's transport sector, is applied to the construction costs in order to obtain the economic prices. The obtained economic project cost is presented in Table 16.

Table 16 Economic Project Cost

Item	Local Currency (in VND)	Foreign Currency (in JPY)	Economic Project Cost in VND
I Construction Expenses	3,360,985,000,000	5,390,000,000	4,374,142,894,737
II Price Escalation (I×10.3%(L), I×1.8%(F))	-	-	-
III Physical Contingency ((I+II)×5%)	168,049,250,000	269,500,000	218,707,144,737
IV Consulting Service	77,704,450,000	1,115,000,000	287,290,916,165
V Land Acquisition, HIV/AIDS prevention	258,760,400,000	-	258,760,400,000
VI Administration Cost ((I+II+III+IV+V)×5%)	193,274,955,000	338,725,000	256,945,067,782
VII VAT ((I+II+III+IV)×10%)	-	-	-
VIII Import Tax (10%)	-	-	-
IX Interest during Construction (Temporary)	-	-	-
X Commitment Charge	-	-	-
Total Economic Cost	4,058,774,055,000	7,113,225,000	5,395,846,423,421

Source: Study Team

The economic project costs are allocated according to the implementation schedule as shown in Table 17 below.

Table 17 Disbursement Schedule of Economic Project Cost

(Unit: Mil. VND)

Year	Costs	
	Mil.VND	%
2012	809,377	15%
2013	2,428,131	45%
2014	1,888,546	35%
2015	269,792	5%
Total	5,395,846	100%

Source: Study Team

(3) Evaluation Indicators and Cost-Benefit Streams

The following three kinds of evaluation indicators are calculated:

- Economic Internal Rate of Return (EIRR)
- Net Present Value (NPV)
- Benefit/Cost Ratio (B/C)

The cost and benefit streams and the results of evaluation are presented in Table 18 below.

Table 18 Results of Economic Evaluation

(Unit: Million VND)

Year	Initial Investment Cost	Routine/Repair Works*	Major Replacement	Annual Total Cost	Annual Incremental Benefit**	Annual Net Benefit
2012	809,377			809,377	0	-809,377
2013	2,428,131			2,428,131	0	-2,428,131
2014	1,888,546			1,888,546	0	-1,888,546
2015	269,792	218,707		488,499	249,042	-239,458
2016		15,522		15,522	852,061	836,539
2017		15,522		15,522	1,375,950	1,360,428
2018		15,522		15,522	1,906,845	1,891,323
2019		15,522		15,522	2,439,524	2,424,002
2020		15,522		15,522	2,973,288	2,957,766
2021		15,522	63,776	79,298	4,386,202	4,306,904
2022		15,522		15,522	5,344,939	5,329,417
2023		15,522		15,522	6,372,393	6,356,871
2024		15,522		15,522	7,479,653	7,464,131
2025		15,522		15,522	8,677,732	8,662,210
2026		15,522		15,522	6,706,991	6,691,469
2027		15,522		15,522	7,649,543	7,634,021
2028		15,522	63,776	79,298	8,659,396	8,580,098
2029		15,522		15,522	9,716,015	9,700,493
2030		15,522		15,522	10,837,866	10,822,344
2031		15,522		15,522	11,811,453	11,795,931
2032		15,522		15,522	12,809,410	12,793,888
2033		15,522		15,522	13,818,653	13,803,131
2034		15,522		15,522	14,822,492	14,806,970
2035		15,522	63,776	79,298	15,844,779	15,765,481
					EIRR =	32.1%
					NPV =	19,858,215
					B/C =	5.5
					Discount rate =	12%

Note: * Procurement cost for O&M equipment is included as 5% of construction cost before opening.
 ** Benefit in the first year (2015) is adjusted by deduction of the first quarter benefit (original x 3/4).

Source: Study Team

The above results indicate that implementation of the project is economically feasible with values of EIRR sufficiently higher than the opportunity cost of capital (>12%), B/C ratio higher

than unity (>1), and positive NPV (>0).

(4) Economic Evaluation under the Different Condition regarding Container Transportation

In the above analysis, all containers are assumed to be transported between Tan Vu IC and Lach Huyen port. In this alternative case, it is assumed that 50% of the projected containers be transported between Hai Phong port and Lach Huyen port, while holding all other conditions at the same as the above analysis in order to see the impact on how EIRR would change. This implies that, under “Without Project Case” 50% of the containers would not transferred to Tan Vu IC, while under “With Project Case” 50% of container traffic would need to go beyond Tan Vu IC toward Hai Phong port, which would require extending the travel distance by 8km (refer to Figure 3).

With such condition, the cost and benefit streams and the results of evaluation are presented in Table 19 below.

Table 19 Results of Economic Evaluation under the Different Condition

(Unit: Million VND)

Year	Initial Investment Cost	Routine/Repair Works*	Major Replacement	Annual Total Cost	Annual Incremental Benefit**	Annual Net Benefit
2012	809,377			809,377	0	-809,377
2013	2,428,131			2,428,131	0	-2,428,131
2014	1,888,546			1,888,546	0	-1,888,546
2015	269,792	218,707		488,499	230,490	-258,010
2016		15,522		15,522	816,137	800,615
2017		15,522		15,522	1,328,660	1,313,138
2018		15,522		15,522	1,848,056	1,832,534
2019		15,522		15,522	2,368,846	2,353,324
2020		15,522		15,522	2,890,403	2,874,881
2021		15,522	63,776	79,298	4,276,448	4,197,150
2022		15,522		15,522	5,214,201	5,198,679
2023		15,522		15,522	6,218,726	6,203,204
2024		15,522		15,522	7,301,484	7,285,962
2025		15,522		15,522	8,473,203	8,457,681
2026		15,522		15,522	6,519,368	6,503,846
2027		15,522		15,522	7,437,684	7,422,162
2028		15,522	63,776	79,298	8,422,239	8,342,941
2029		15,522		15,522	9,452,236	9,436,714
2030		15,522		15,522	10,545,966	10,530,444
2031		15,522		15,522	11,493,183	11,477,661
2032		15,522		15,522	12,463,732	12,448,210
2033		15,522		15,522	13,445,285	13,429,763
2034		15,522		15,522	14,421,196	14,405,674
2035		15,522	63,776	79,298	15,415,550	15,336,252
						EIRR = 31.6%
						NPV = 19,196,701
						B/C = 5.3
						Discount rate = 12%

Note: * Procurement cost for O&M equipment is included as 5% of construction cost before opening.
 ** Benefit in the first year (2015) is adjusted by deduction of the first quarter benefit (original x 3/4).

Source: Study Team

The result shows that the annual incremental benefits have slightly decreased and resulted in 0.5% reduction in EIRR from 32.1% to 31.6%.

Table 20 Construction Schedule (Bridge Section, 32 months)

YEAR		2012				2013												2014												2015		Remarks		
Month		7	8	9	10	1	2	3	4	5	6	7	8	9	10	11	12	1	2	3	4	5	6	7	8	9	10	11	12	1	2			
		1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30		31	32
Preparation work		[Gantt bar]																																
Approach Bridge(1)	Temporary Road	[Gantt bar]																																
	Sub Structure	[Gantt bar]																																A1 ABUTMENT
	Super Structure	[Gantt bar]																																Erection Girder-1
Flyover Bridge (1)	Temporary Road	[Gantt bar]																																
	Sub Structure	[Gantt bar]																																
	Super Structure	[Gantt bar]																																
Approach Bridge(2)	Temporary Road	[Gantt bar]																																
	Sub Structure	[Gantt bar]																																Erection Girder-2
	Super Structure	[Gantt bar]																																
Flyover Bridge (2)	Temporary Road	[Gantt bar]																																
	Sub Structure	[Gantt bar]																																
	Super Structure	[Gantt bar]																																Erection Girder-3
Approach Bridge (3)	Temporary Road	[Gantt bar]																																
	Temporary Jetty	[Gantt bar]																																
	Super Structure	[Gantt bar]																																Erection Girder-4
Main Bridge	Water-Way Safety	[Gantt bar]																																
	Sub Structure	[Gantt bar]																																
	Super Structure	[Gantt bar]																																
Approach Bridge(4)	Temporary Jetty	[Gantt bar]																																
	Sub Structure	[Gantt bar]																																A2 ABUTMENT
	Super Structure	[Gantt bar]																																Erection Girder-1
Pavement		[Gantt bar]																																
Bridge Surface		[Gantt bar]																																
Site Cleance		[Gantt bar]																																

Source: Study Team

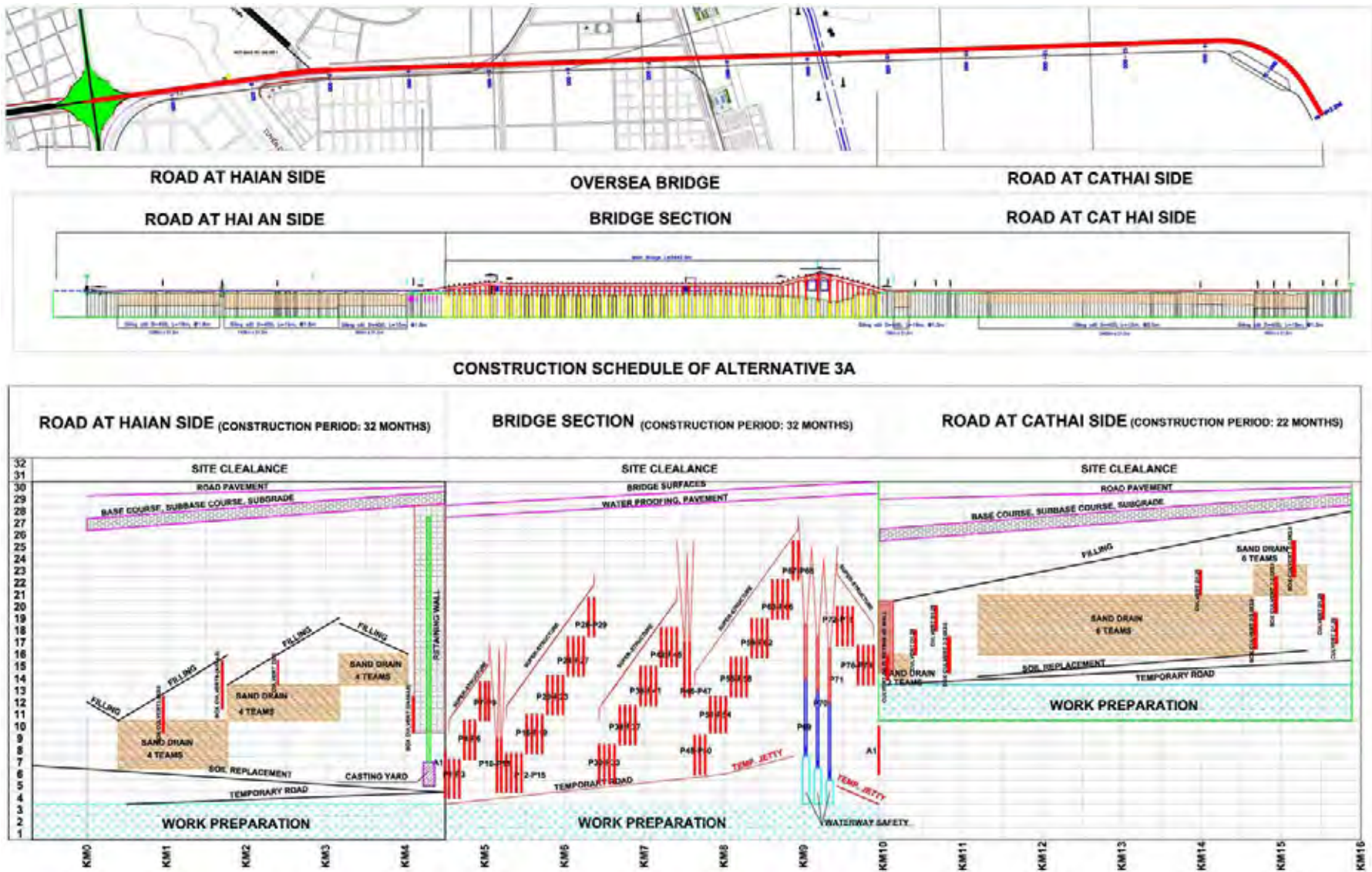


Figure 4 Construction Schedule (32 Months)

7. Implementation Agency and Implementation Program

The implementation program (I/P) in this Study is established based on following assumptions,

- STEP scheme of Japanese ODA Loan is applied,
- Consulting services of Detail Design and Tender Assistance are supported by Japanese Grant,
- Loan Agreement is signed in September 2010, and,
- Construction period is 32 months.

The implementation program (I/P) is as follows and shown in Table 20, assuming the common practice.

Table 21 Implementation Milestones (After JICA Follow-up Mission in June 2010)

Event/ Milestone	Time/ Period
Preparatory Stage	
SAPROF Study	: April 2010 to July 2010
JICA Follow-up Mission	: Jun 2010
Pledge by Japanese Government	: August 2010
Exchange Note & Loan Agreement	: September 2010
Procurement of D/D consultant	: August 2010 to October 2010
Detail Design	: October 2010 to August 2011
Procurement of C/S Consultant	: November 2010 to August 2011
P/Q Period	: May 2011 to July 2011
Bidding Time	: August 2011 to July 2012
Land Acquisition	: January 2011 to July 2012
Construction	: August 2012 to March 2015
Defect Liability Period	: March 2015 to February 2017

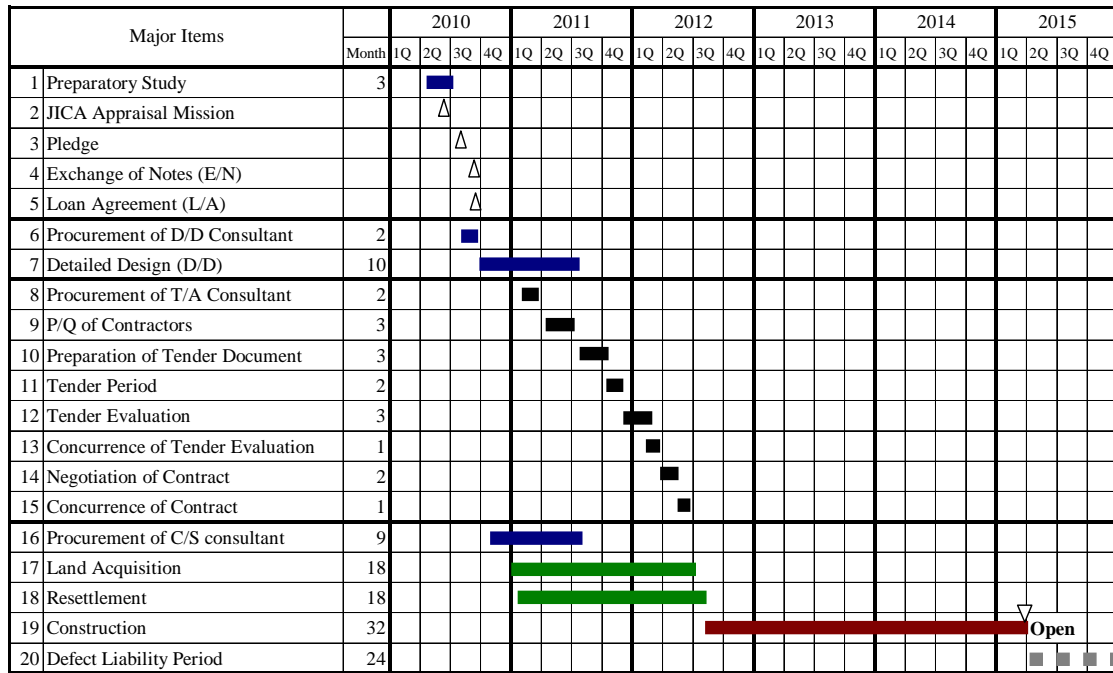


Figure 5 Proposed Implementation Program