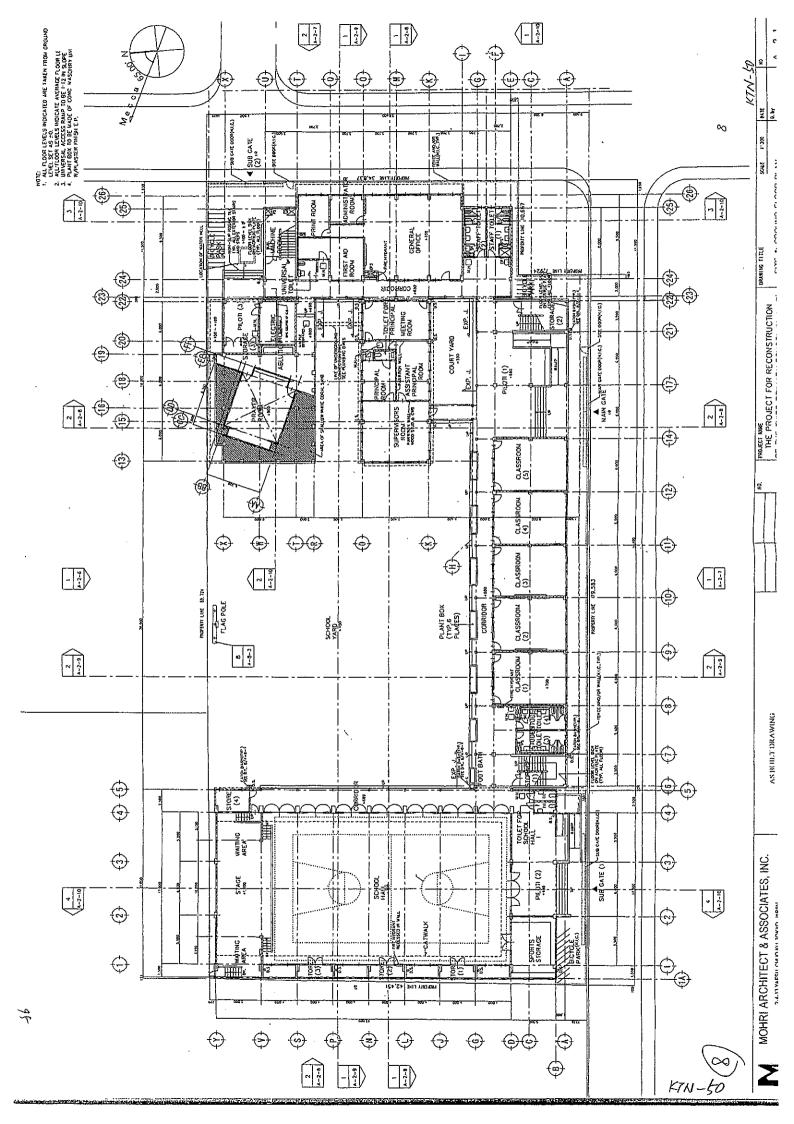
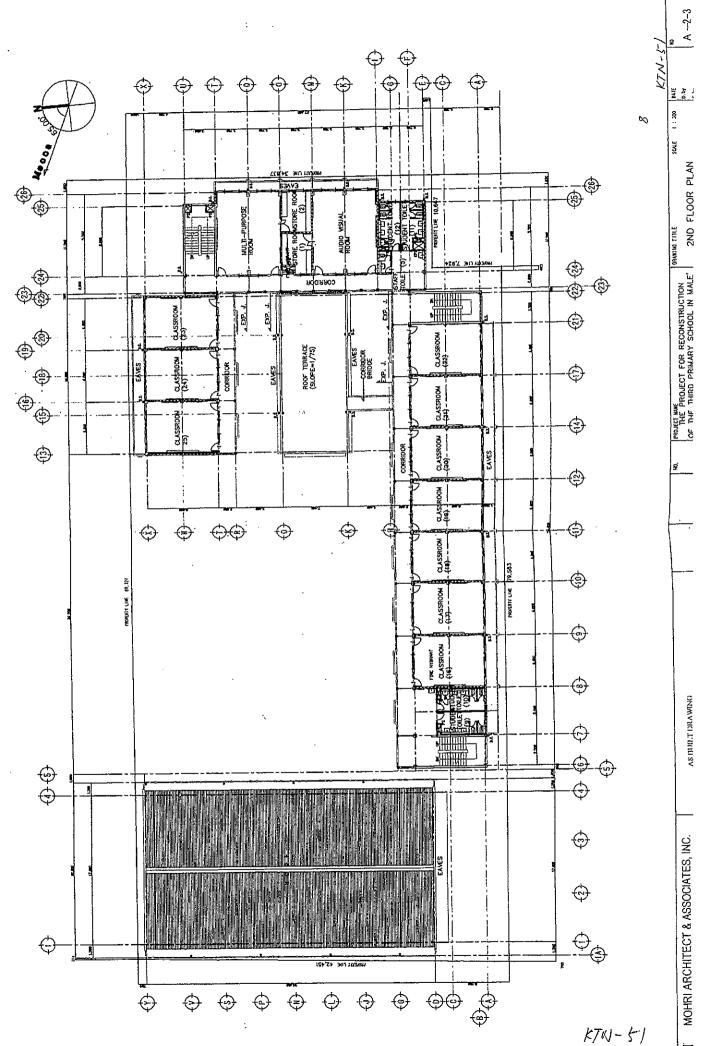
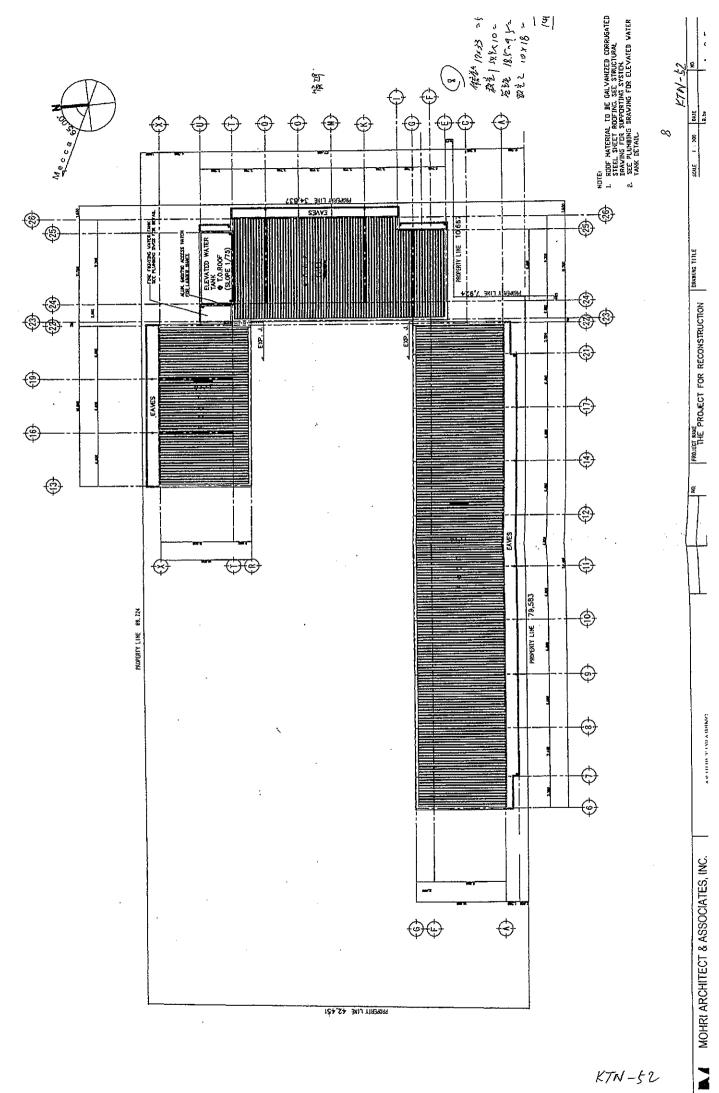
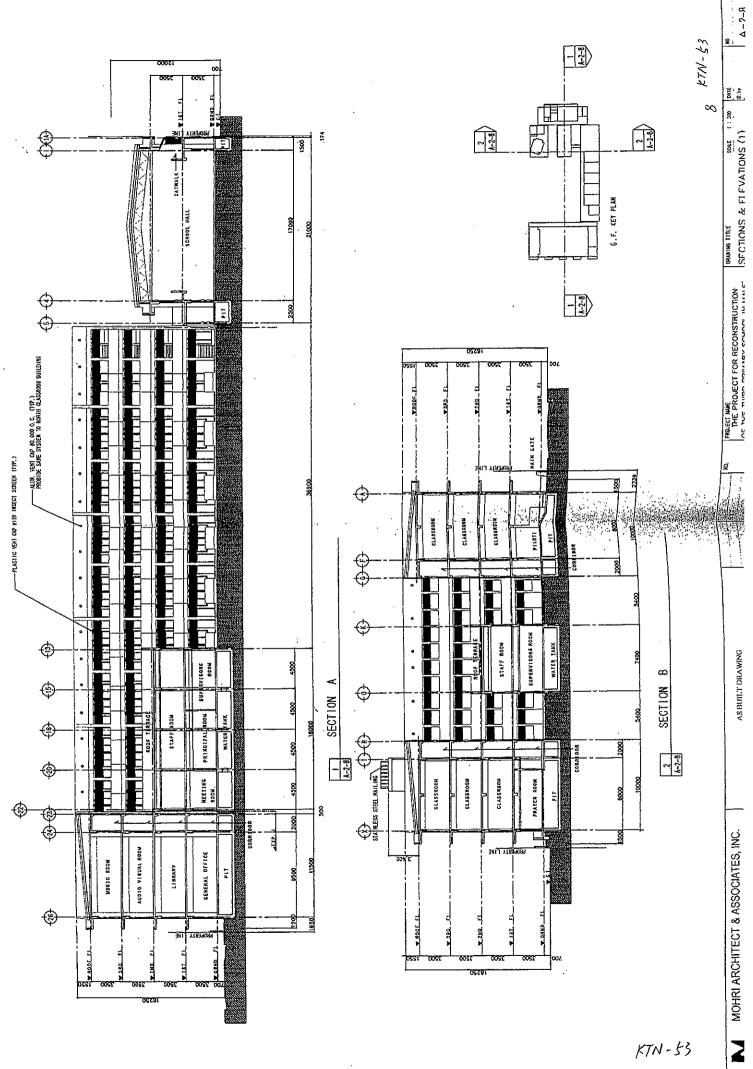
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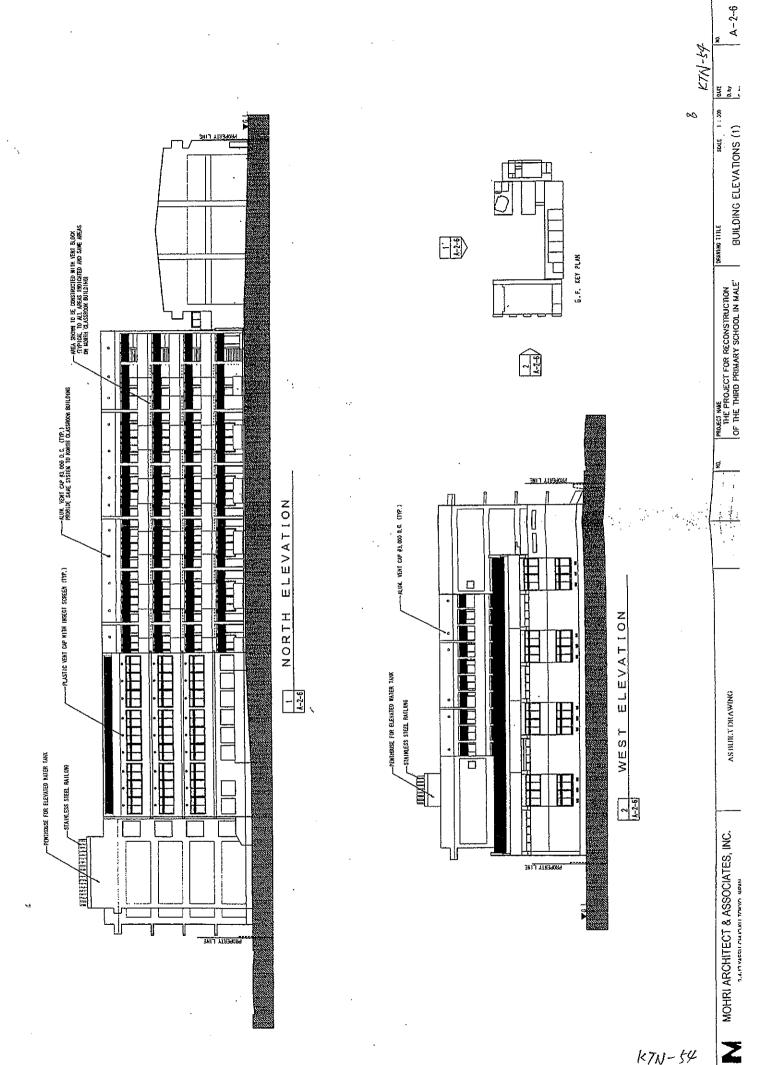
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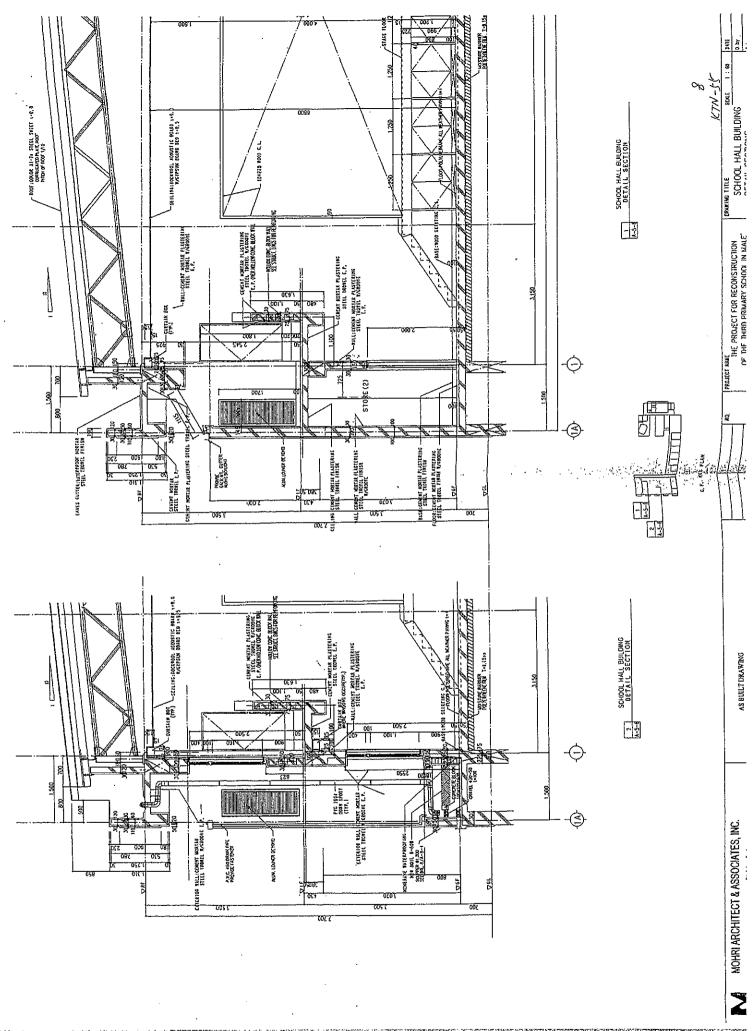


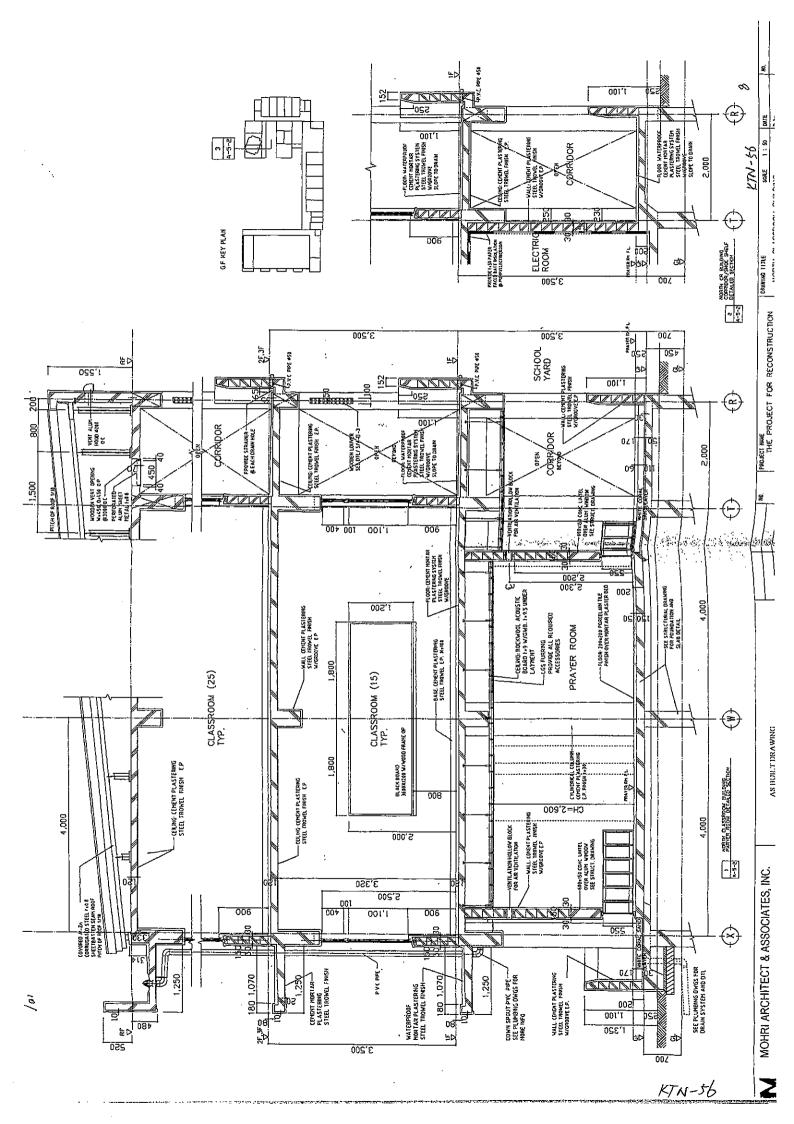


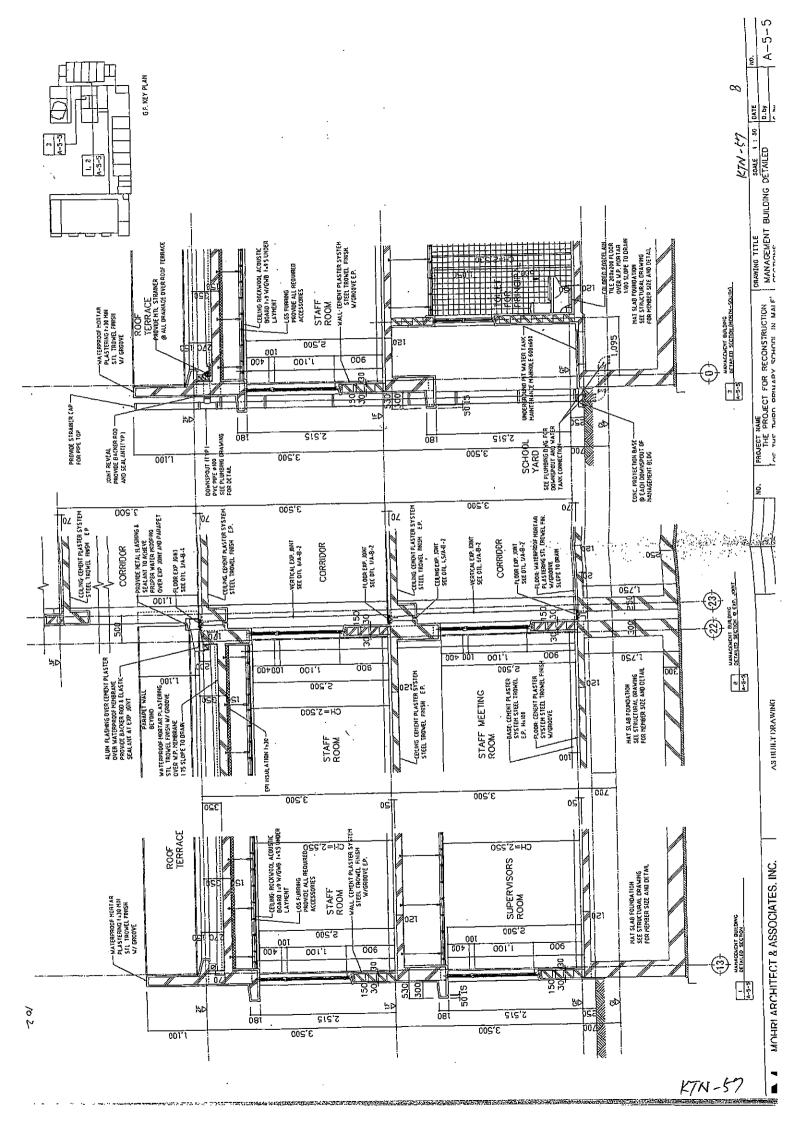


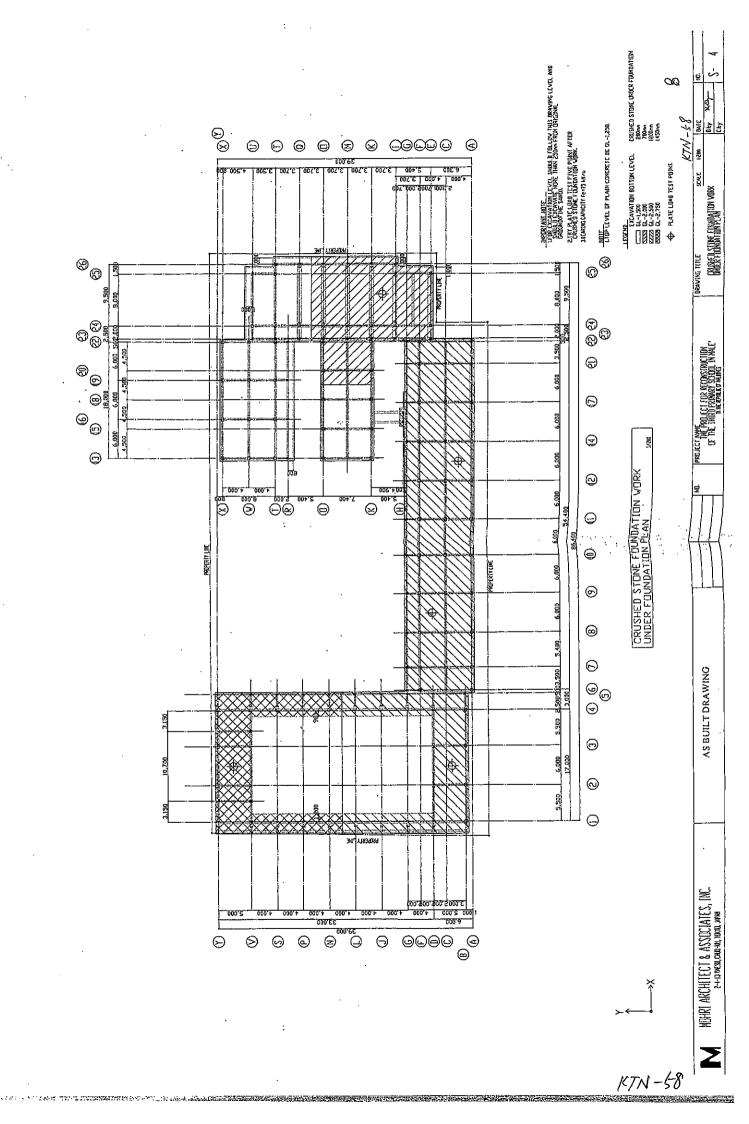




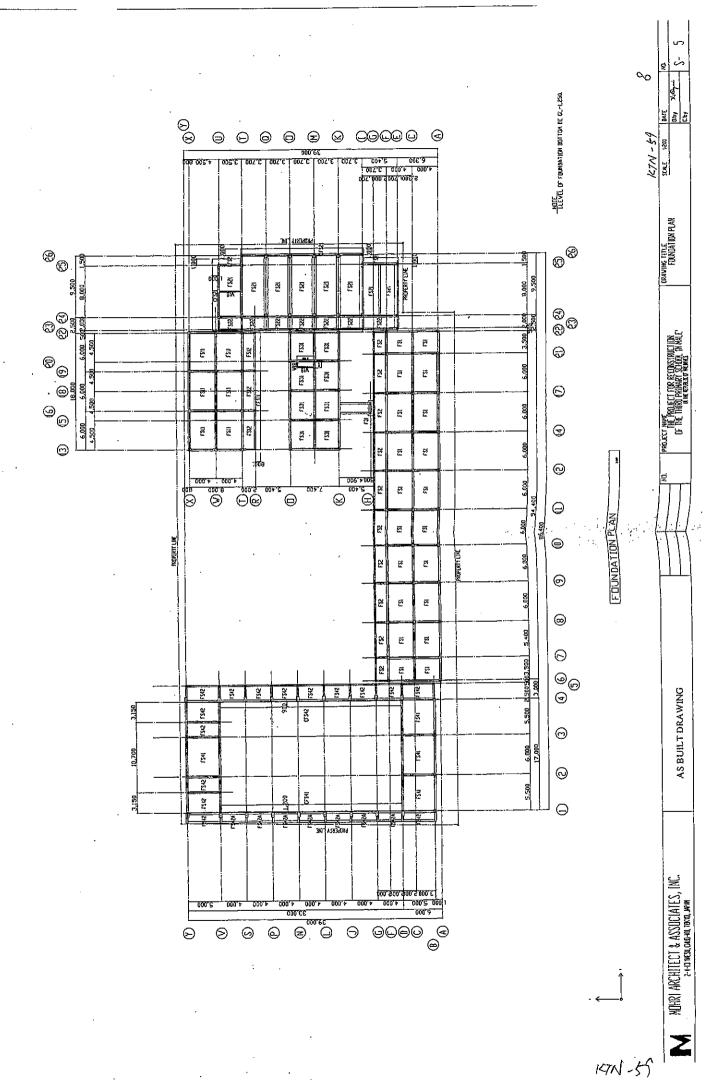




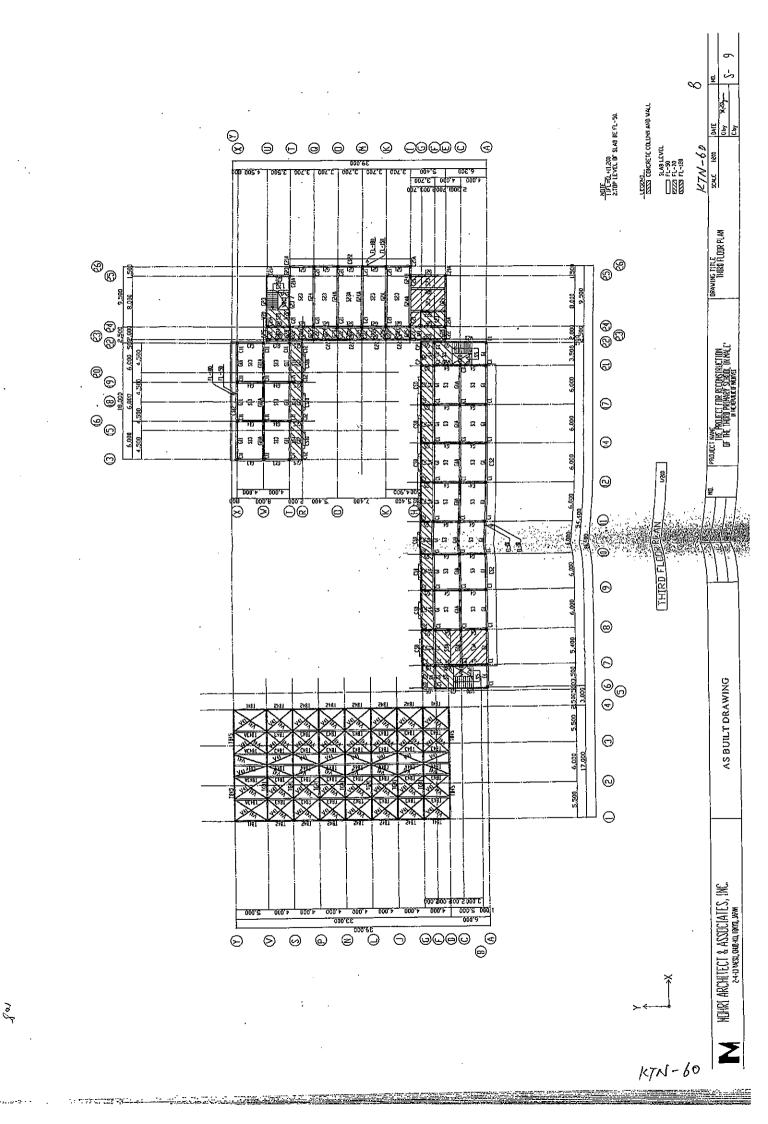


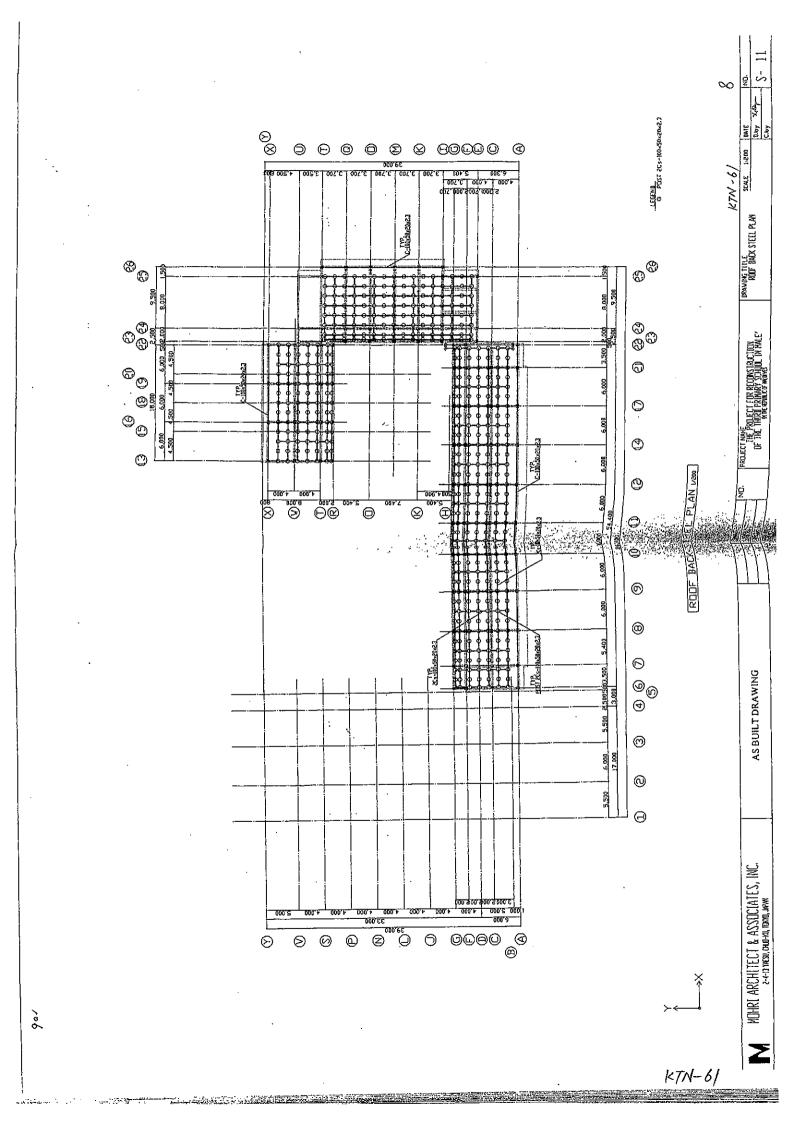


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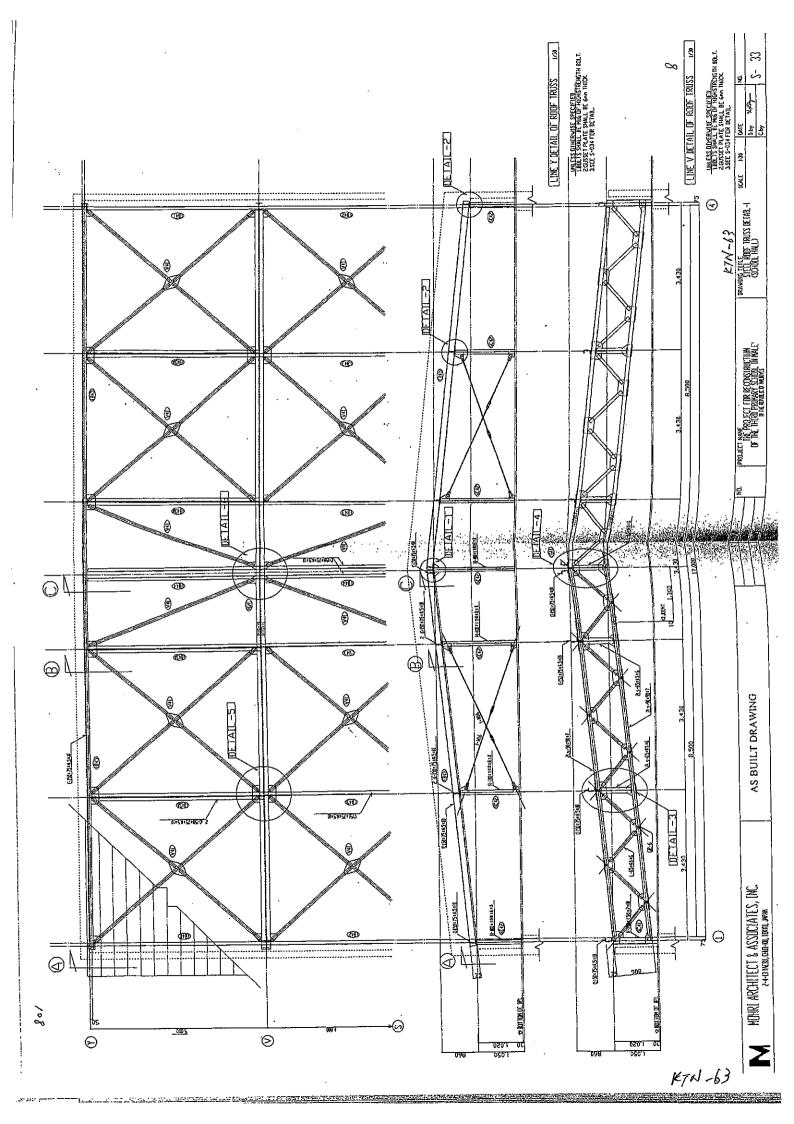
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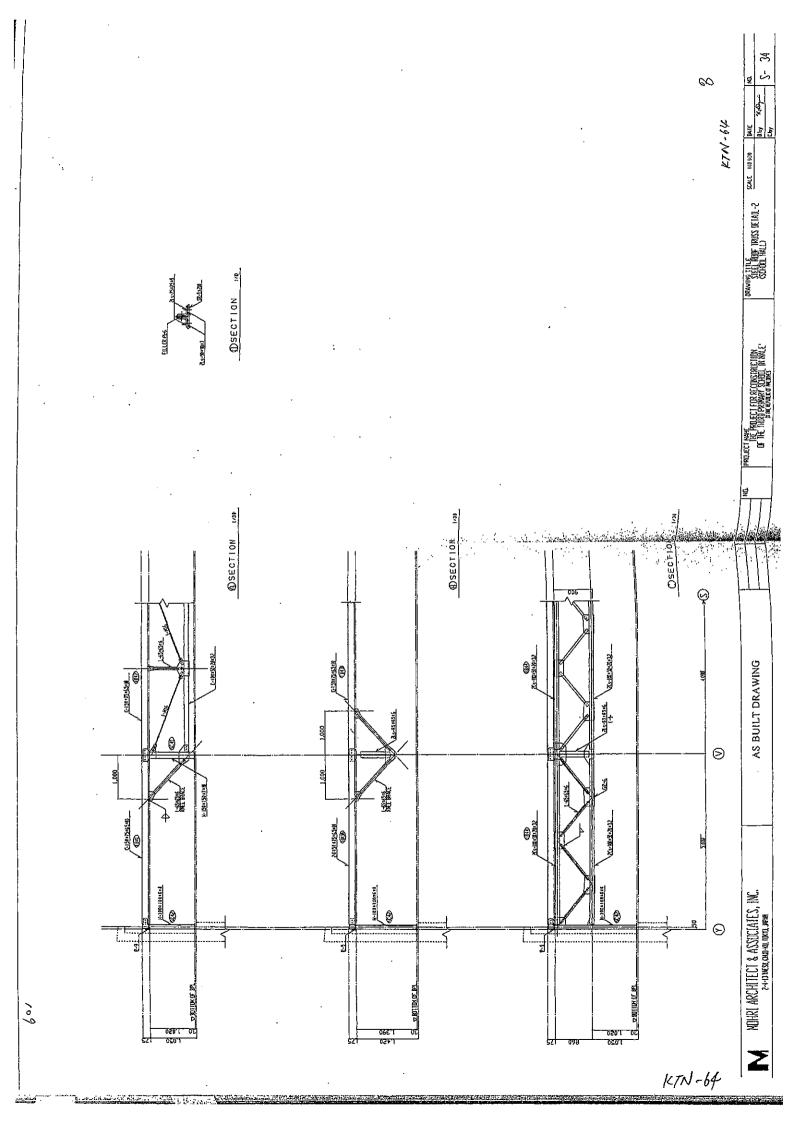




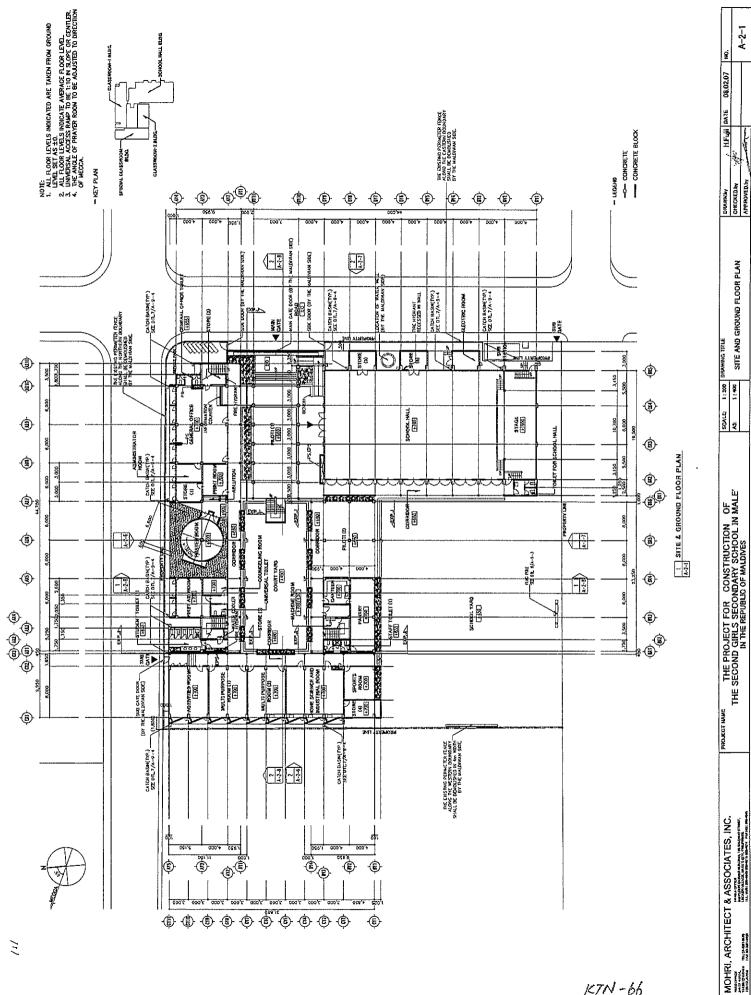
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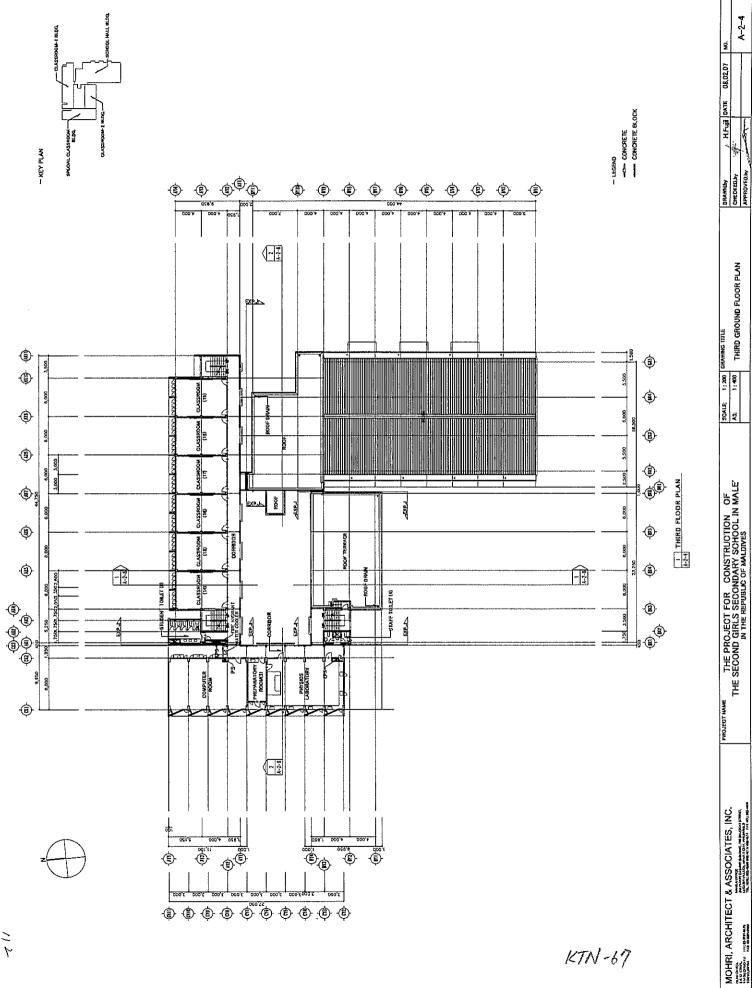
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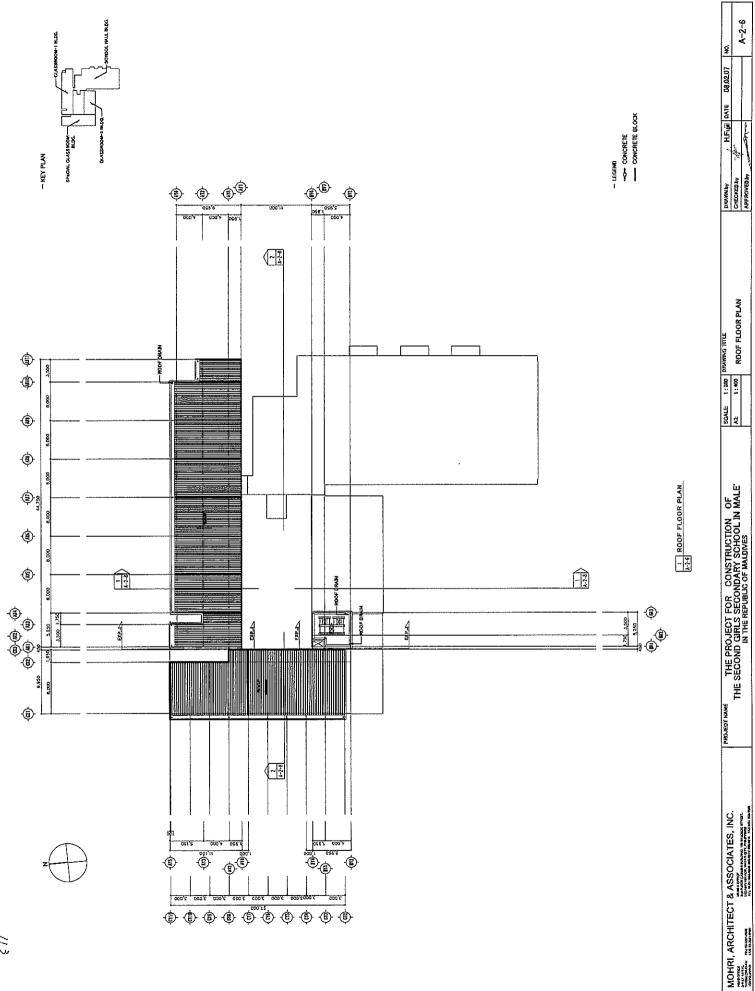


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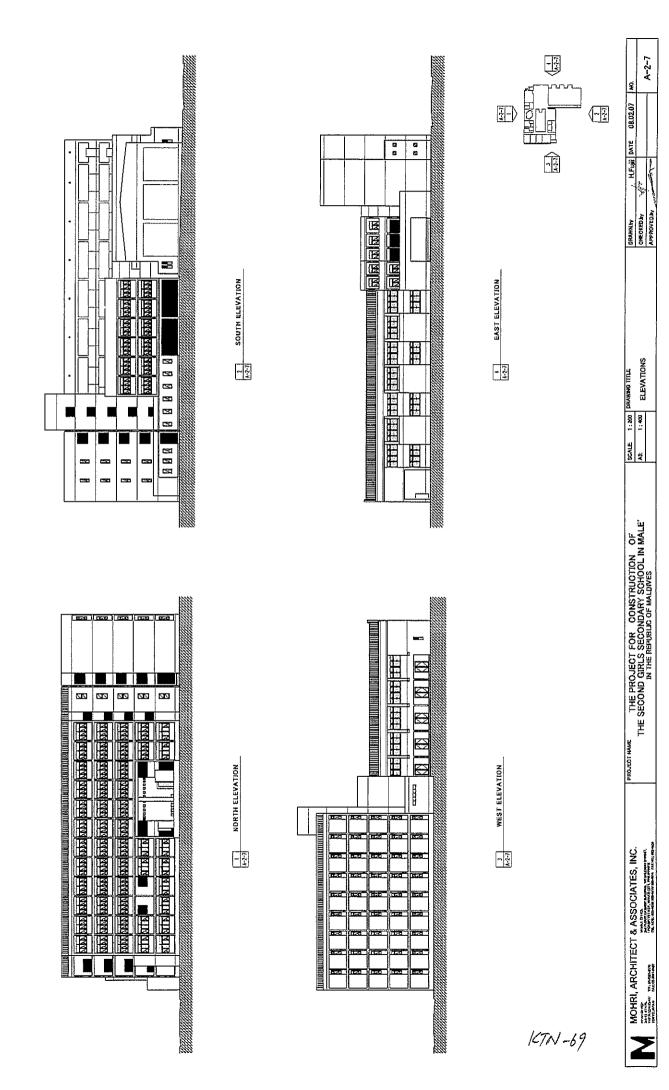


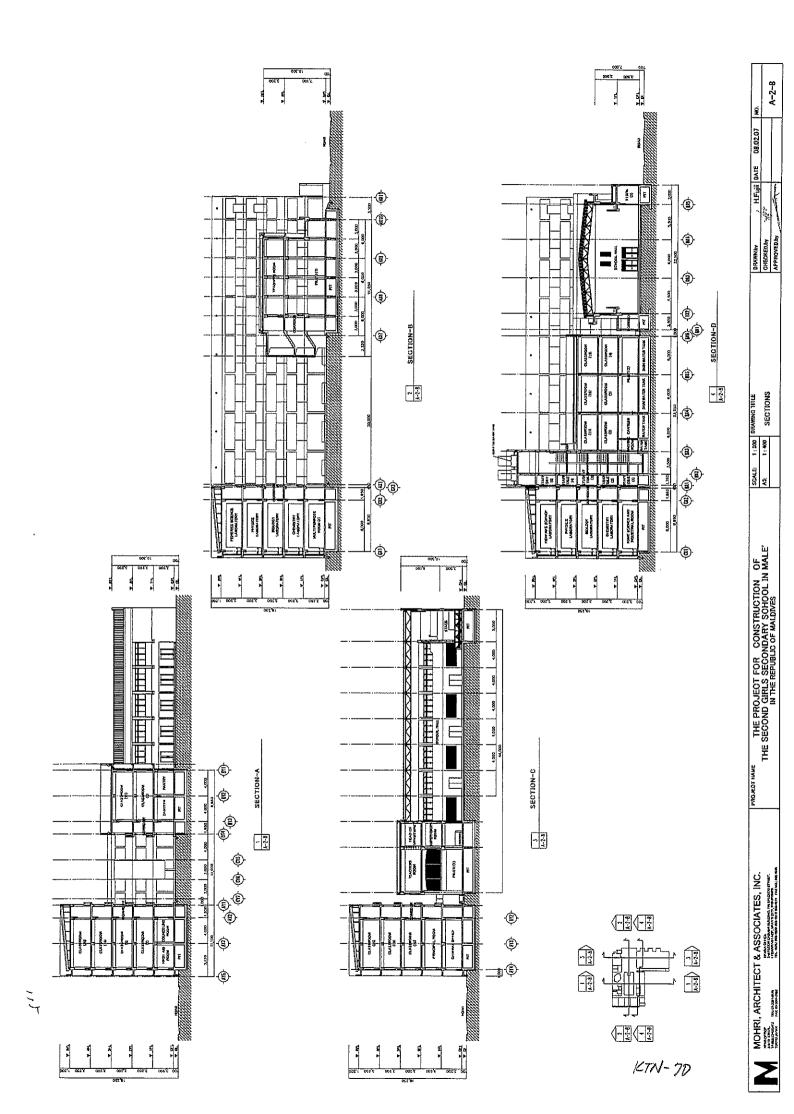
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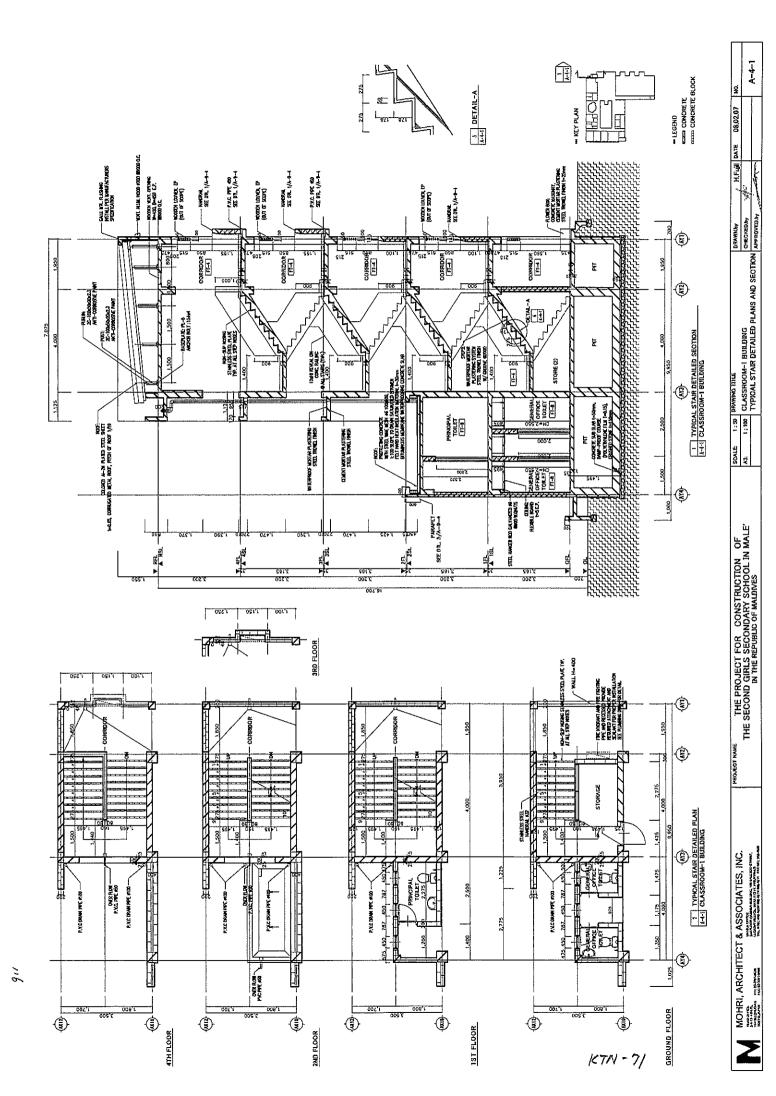


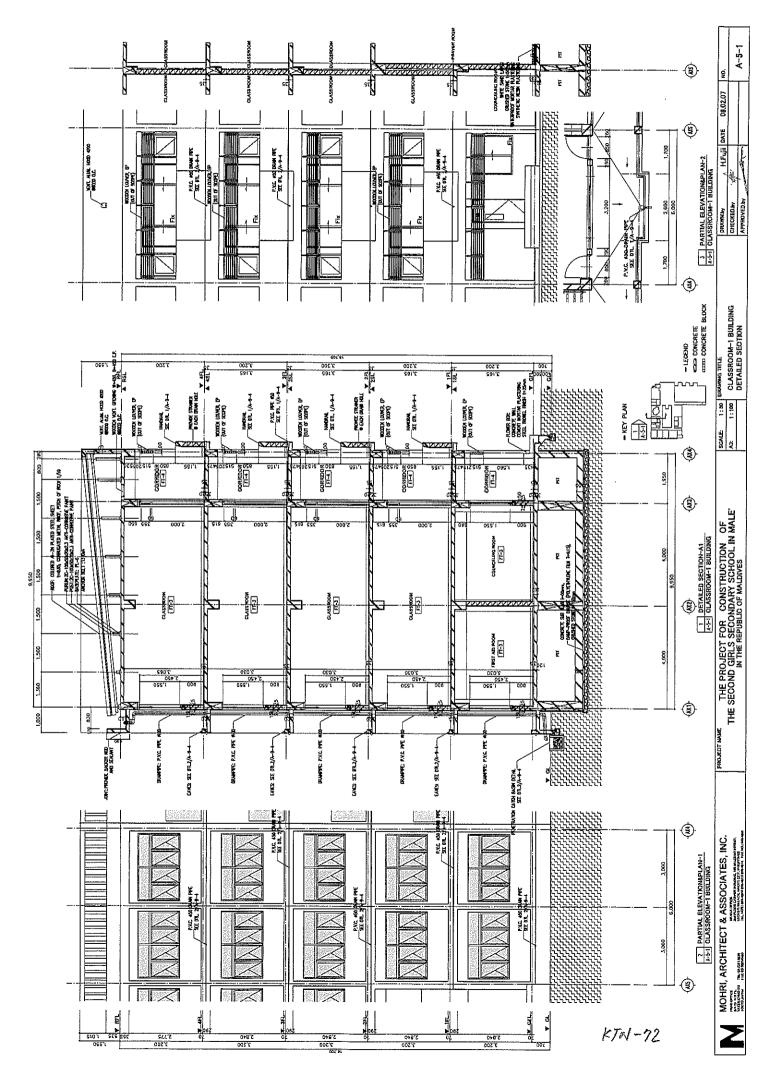
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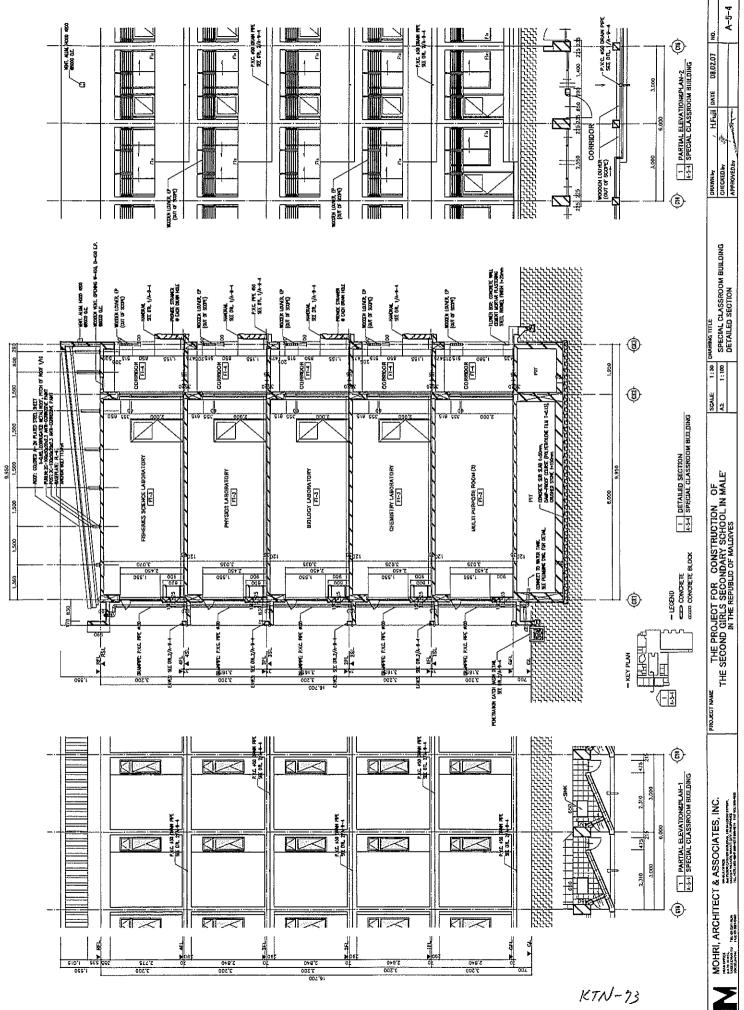
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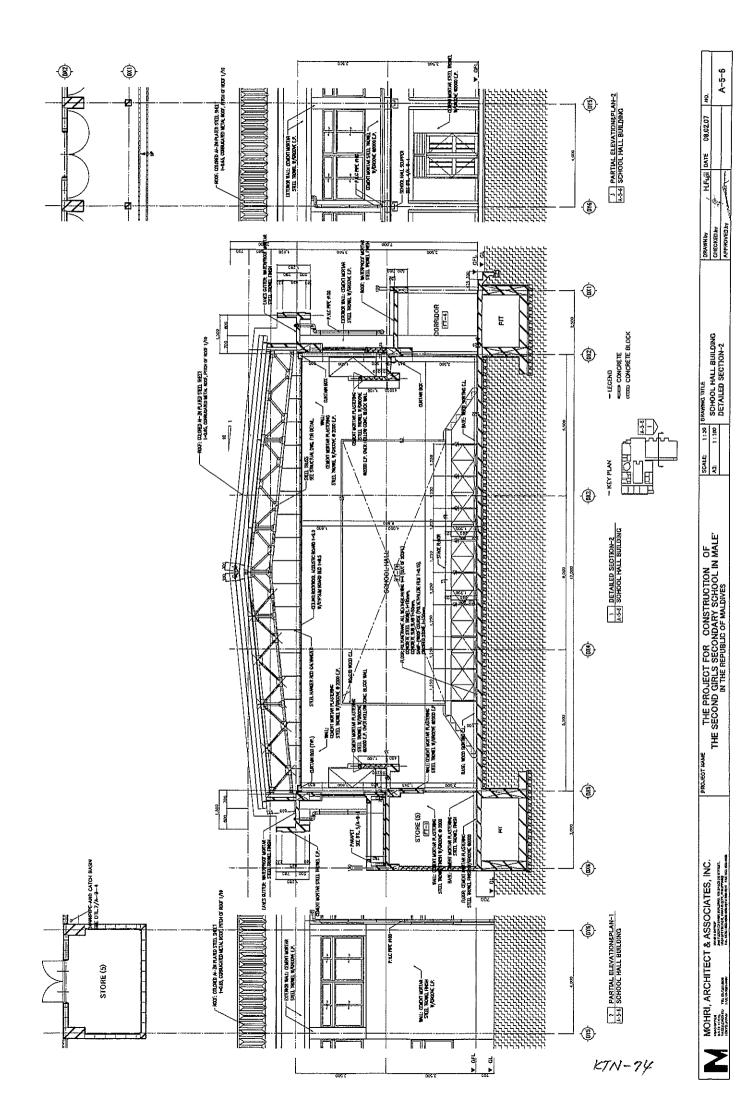


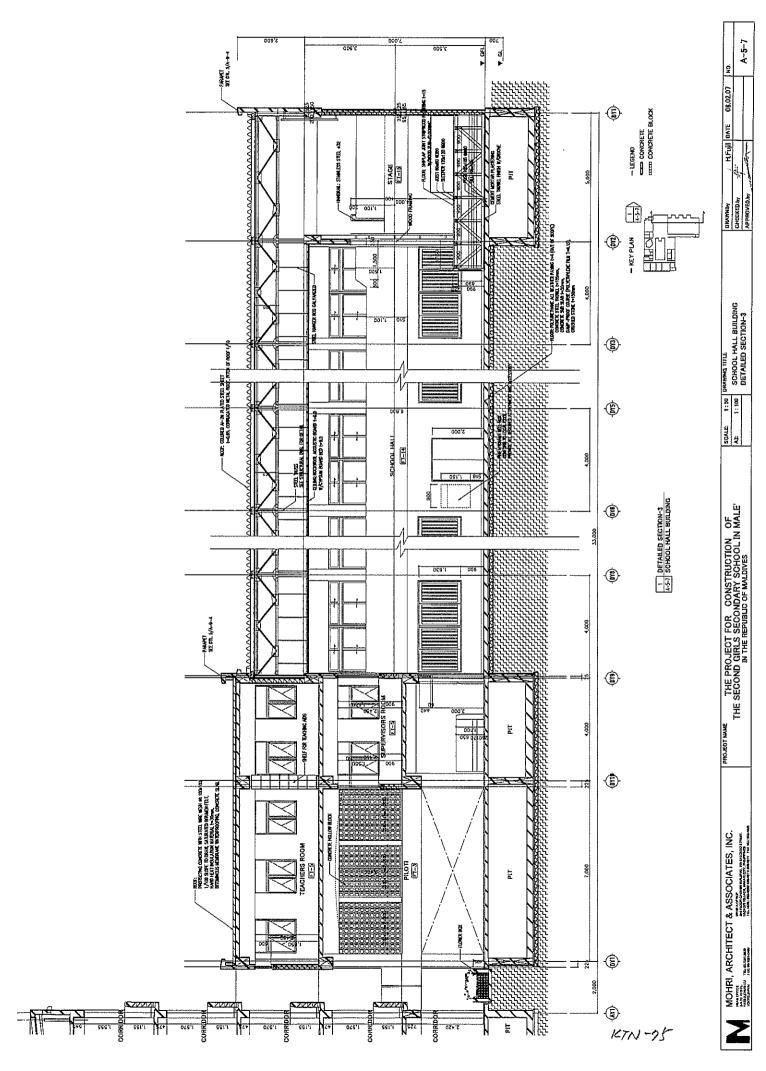


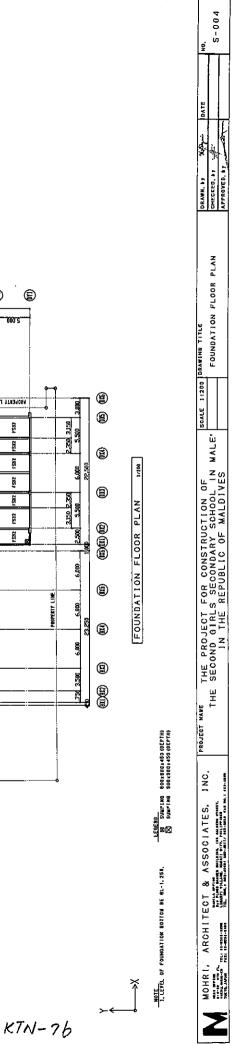


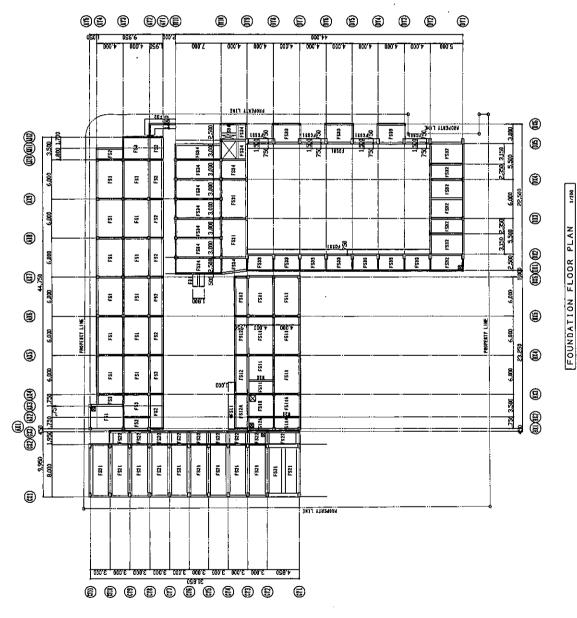


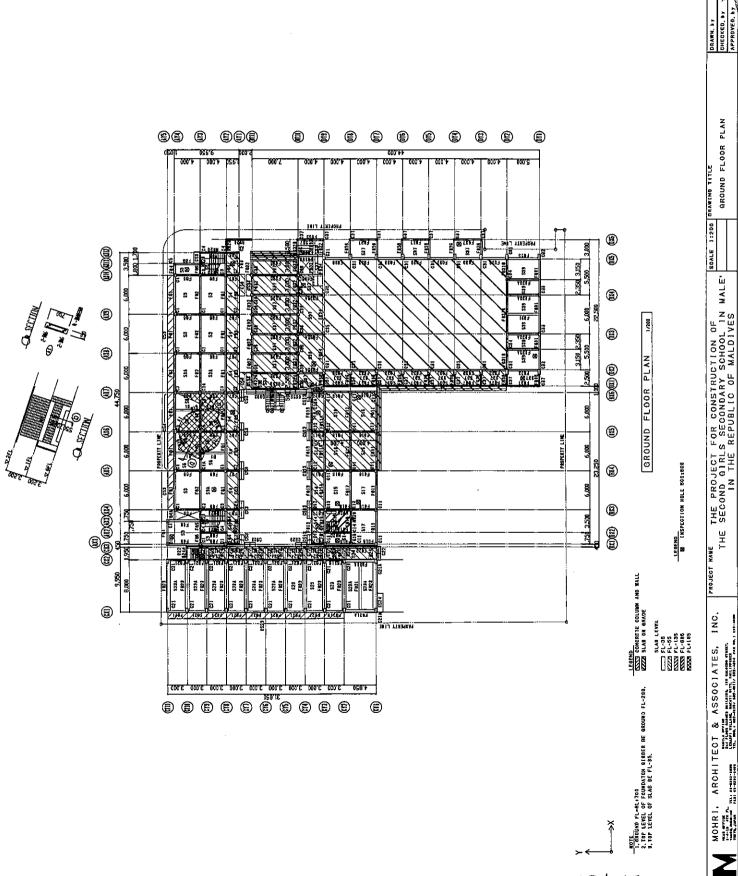
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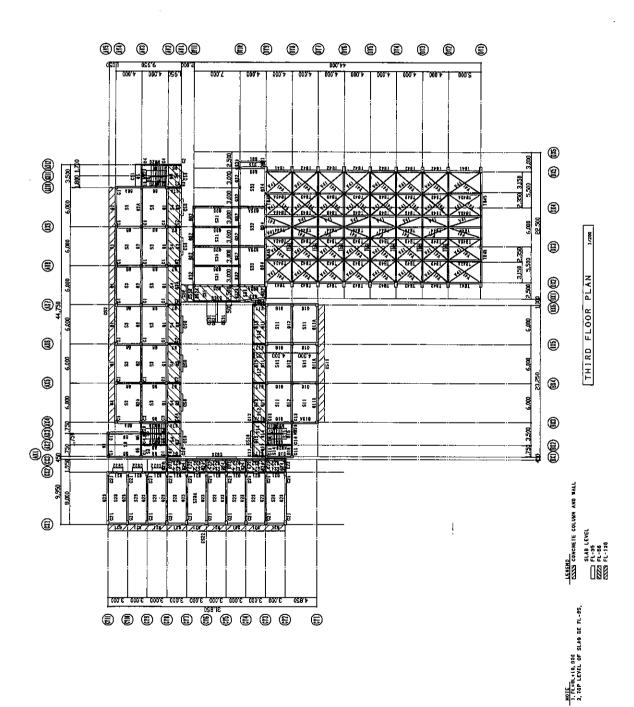
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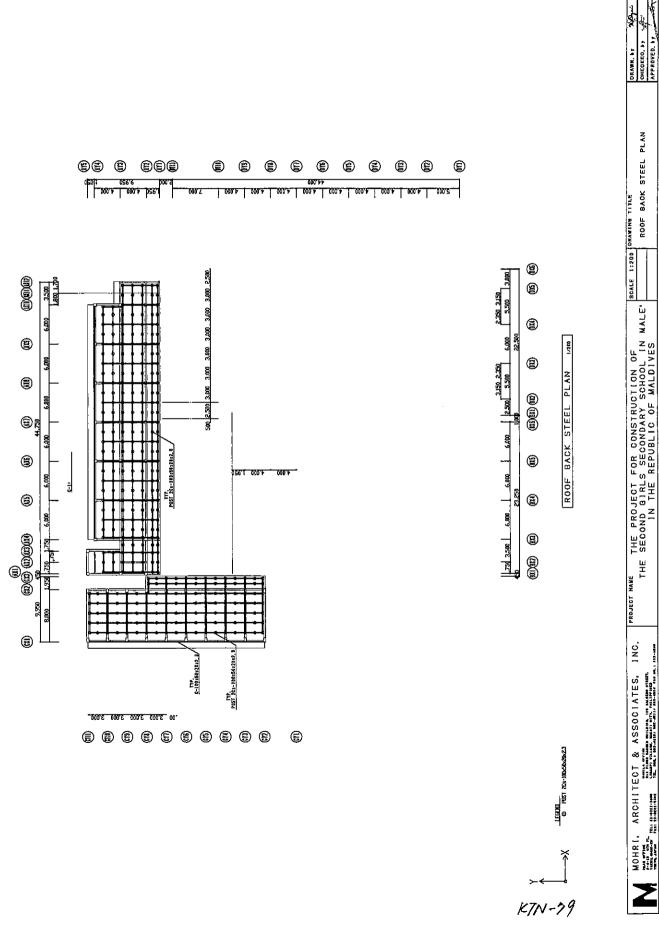
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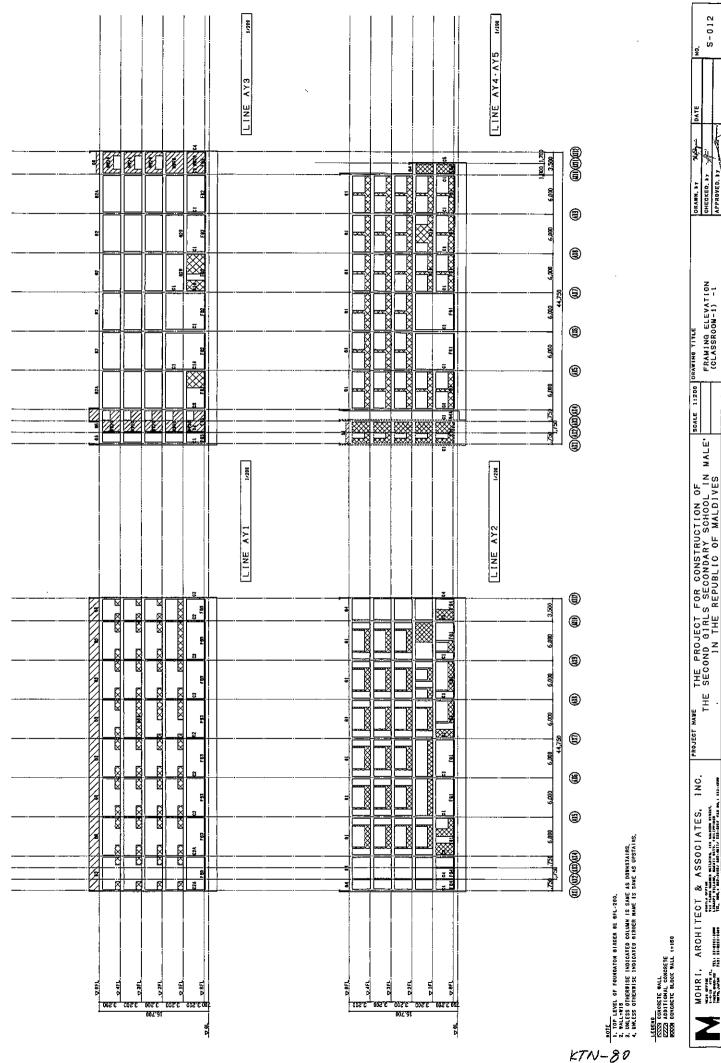


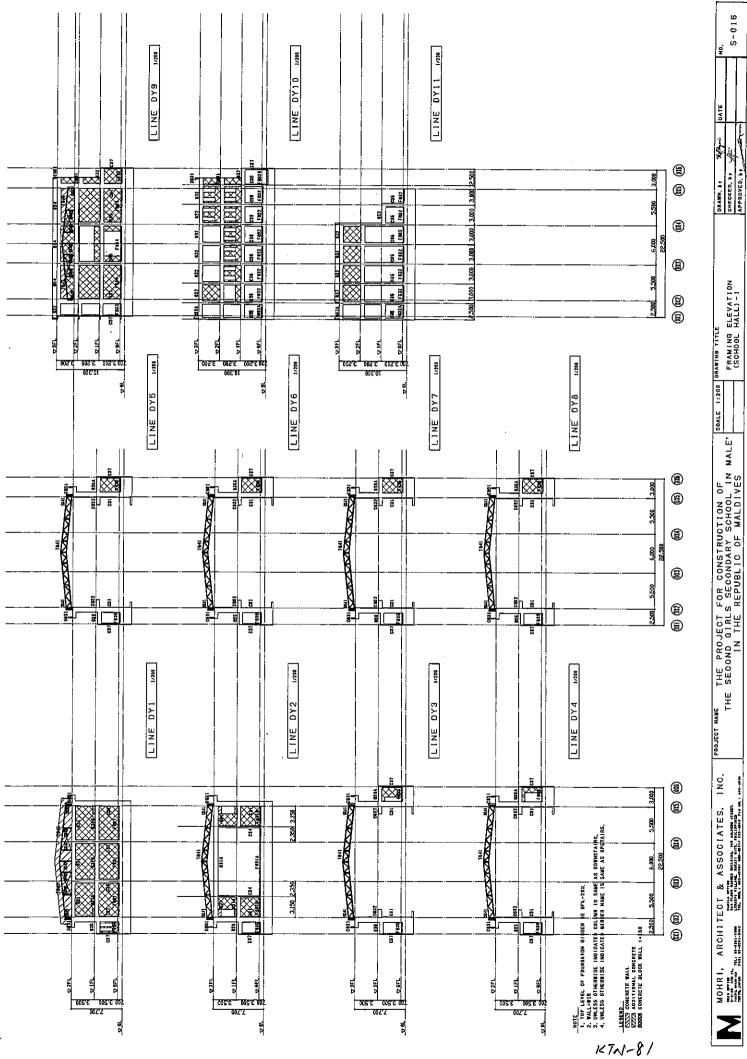
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THE

MOHRI, ARCHITEOT & ASSOCIATES, INC.

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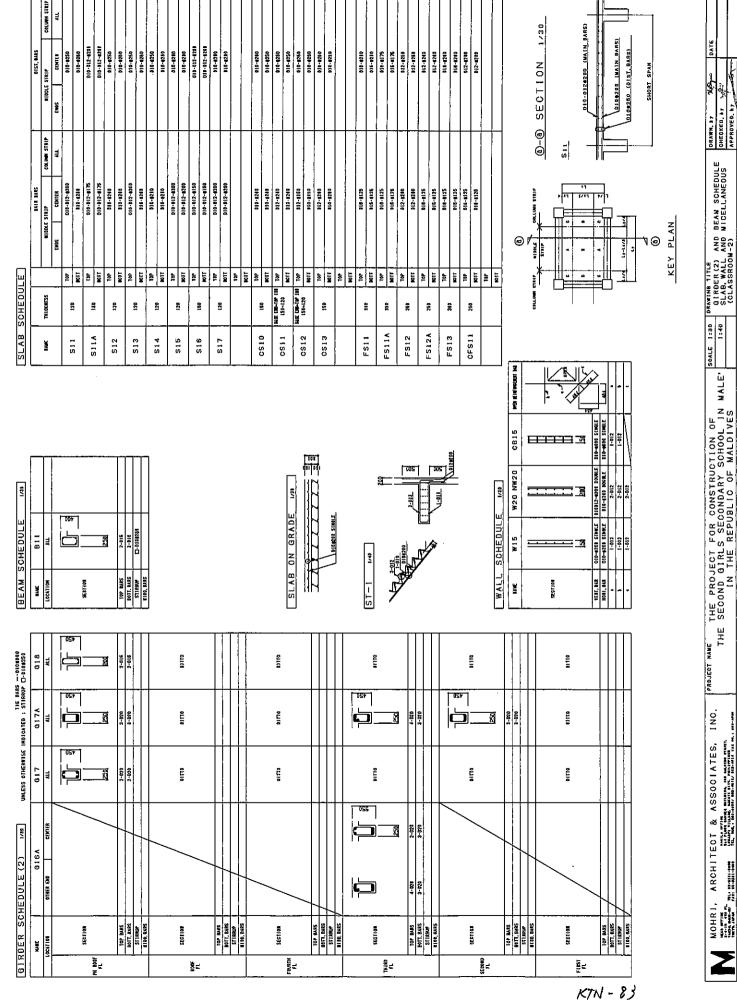
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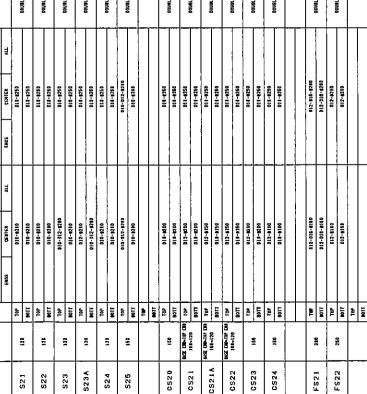
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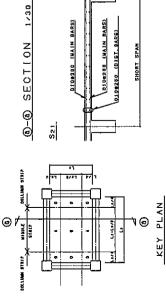
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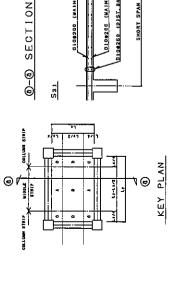


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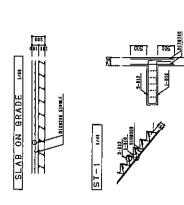
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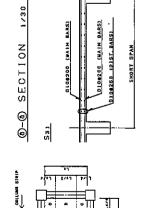
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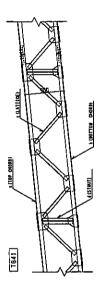
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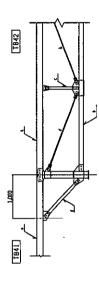
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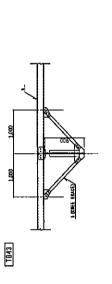
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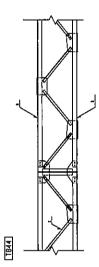
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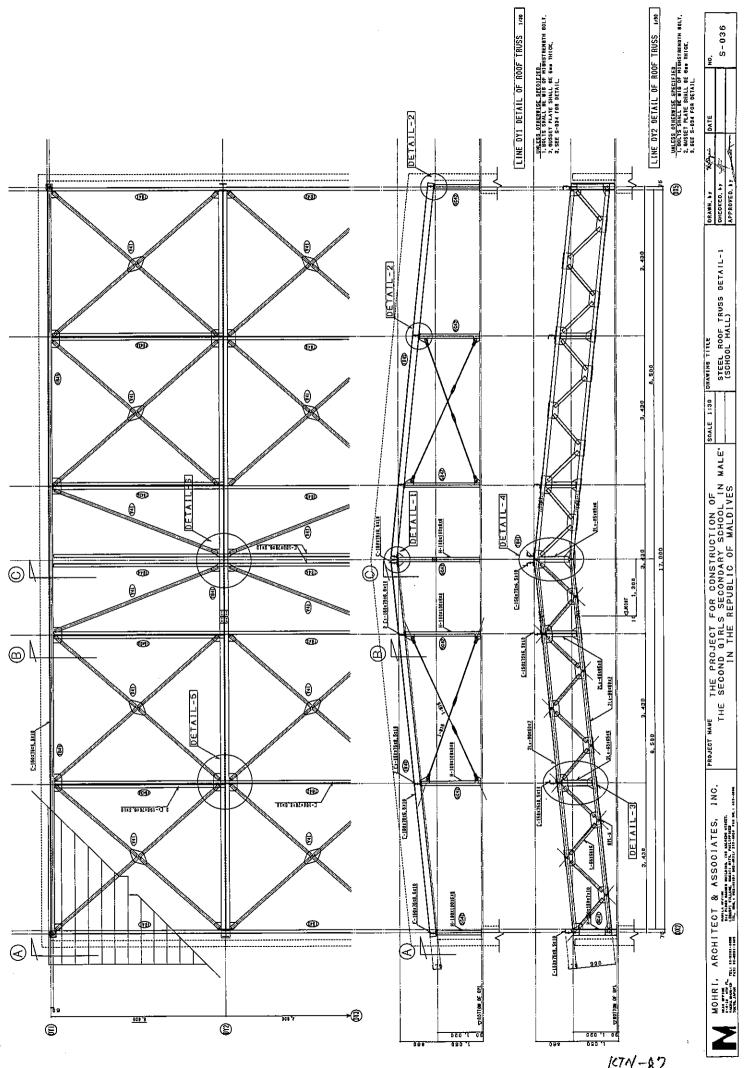






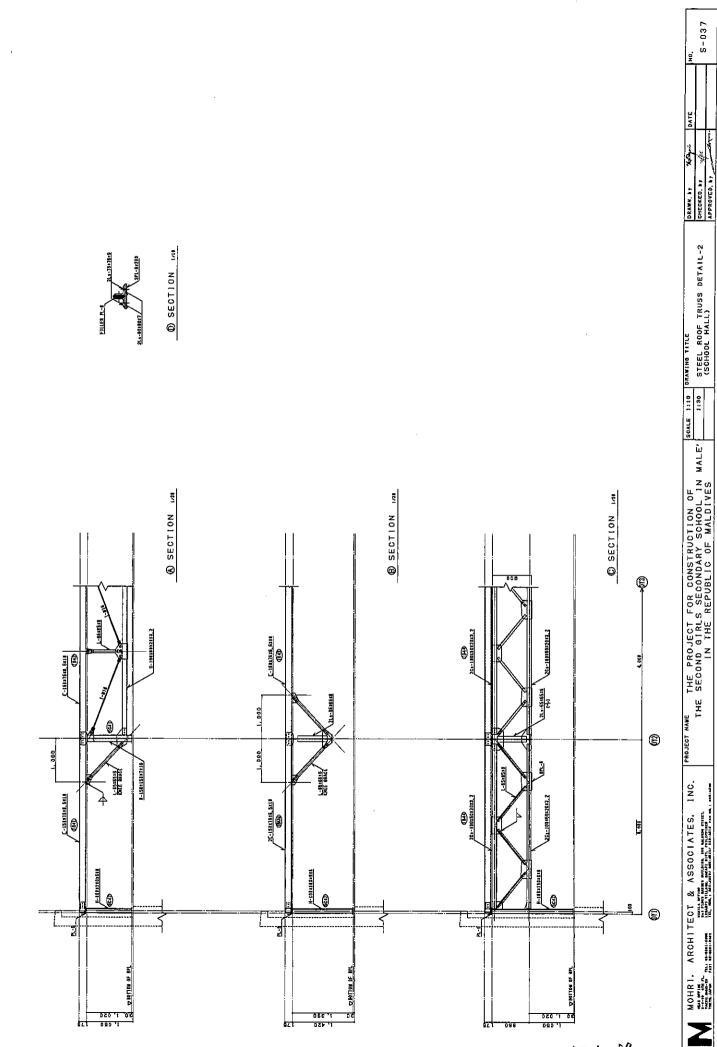
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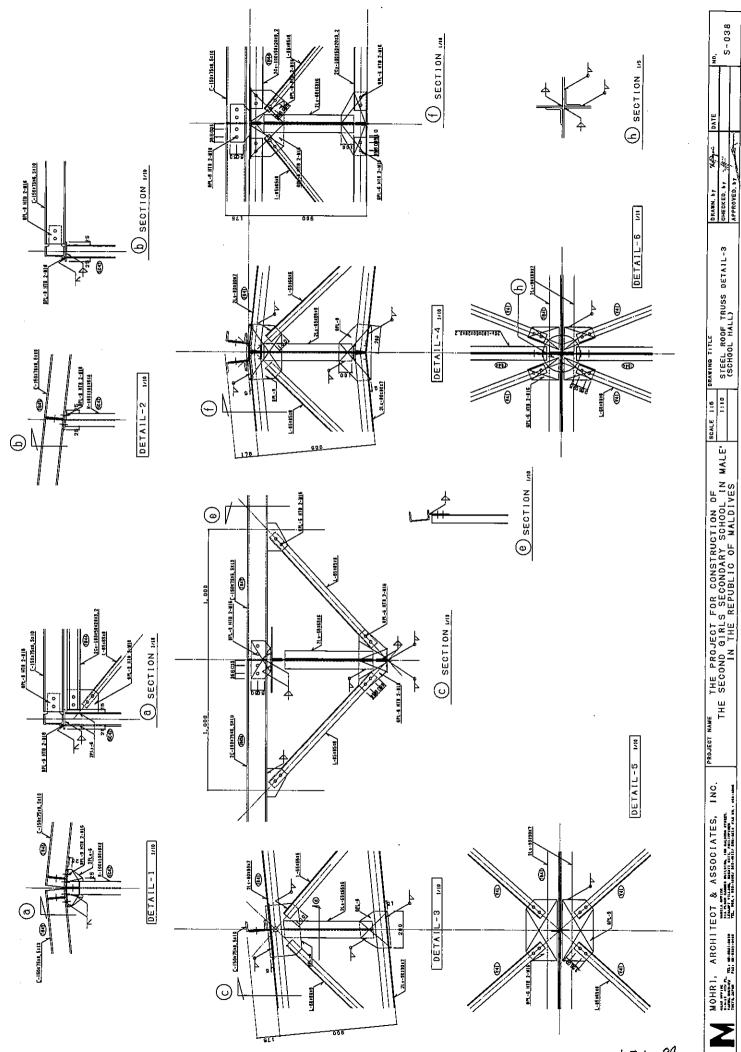


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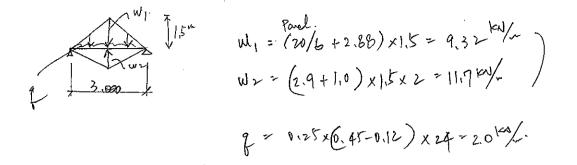


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KTN-90



$$M_{c} = \frac{1}{2 \times 21,02 \times 3,0} + \frac{1}{8 \times 2.0 \times 3.0} = 18.1 \text{ prim}$$

$$Q = \frac{14 \times 21,02 \times 3,0}{4 \times 2.0 \times 3.0} = 18.8 \text{ prim}$$

$$(411)$$

$$250 \times 450 \quad d = 400 \quad f = 350$$

$$250$$

$$250 \quad d = \frac{18.1 \times 10}{2.15 \times 350} = 241^{\text{mm}} \rightarrow \frac{2-919}{573^{\text{mm}}} \quad 0 \text{ cc}$$

$$T = \frac{18.8 \times 10^{3}}{250 \times 350} = 0.272 \times 0.7 \quad 0 \text{ cc}$$

$$\begin{array}{c} (68) \\ 250 \times 650 \quad d=600 \quad d=575 \\ \hline 270 \\ 650 \quad f=\frac{25 \times 10^{\frac{10}{2}}}{215 \times 575} = 311^{nn^{-1}} \\ 2-922 \quad (9114^{m2}) \\ 0 \\ \hline 0 \\ \hline \end{array} \\ \end{array}$$

KTN-91

Auxial Force.

$$P_{1} = 12 \frac{100}{10^{2}} \times 2.25^{m} \times 5.75^{m} + 6.1\frac{100}{10^{2}} \times 2.9^{m} = 181.8$$

$$P_{2} = 12 \frac{100}{10^{2}} \times 1.5^{m} \times 5.95^{m} + 11 + 3.5\times17\times10 \times 1/4 = 279.$$

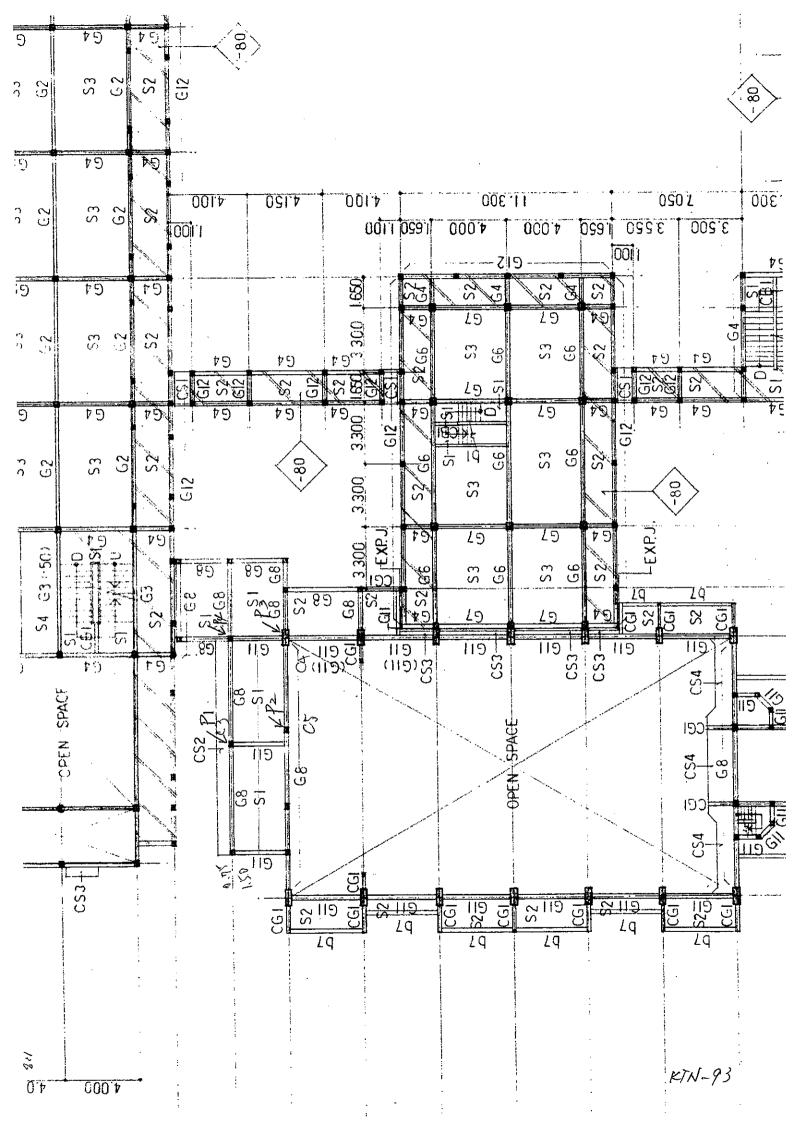
$$P_{3} = 12 \frac{100}{12} \times (1.5^{m} \times 5.0^{m} + 2.0\times2.5) + 6.1\times5.95\times1/2\times1.5$$

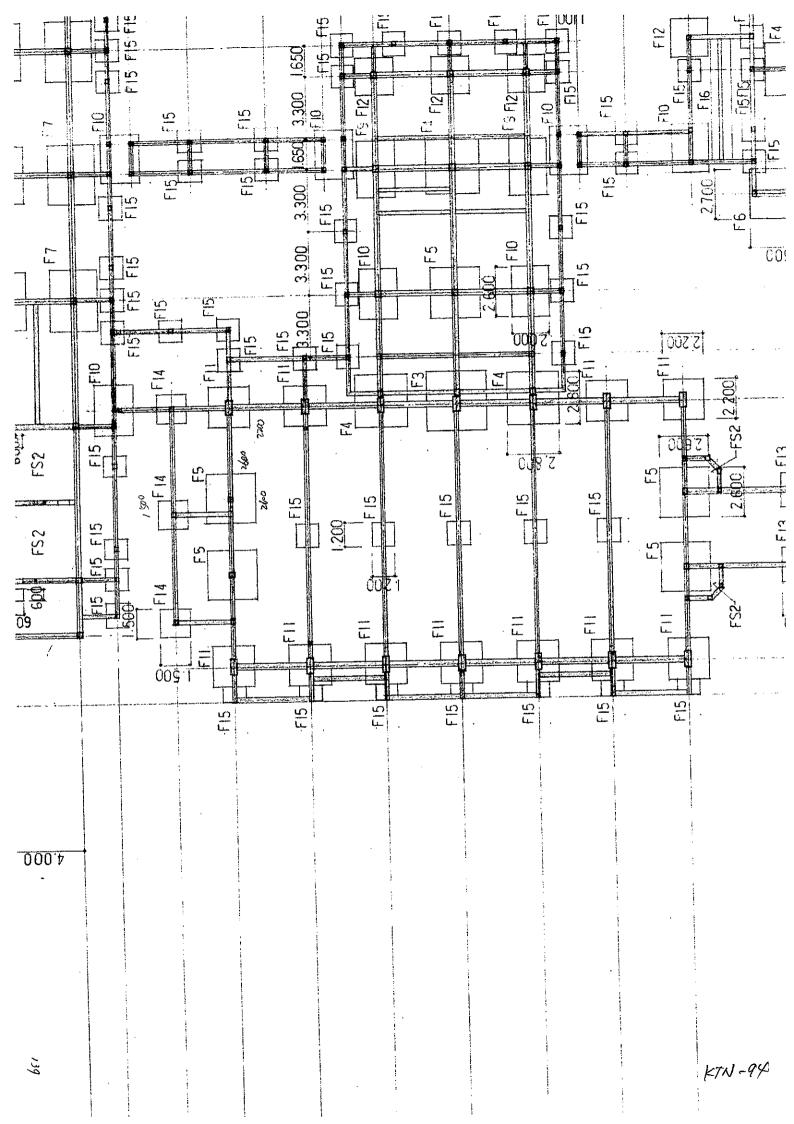
$$+ 3.5\times19\times10\times1/4 = 3.25.1$$

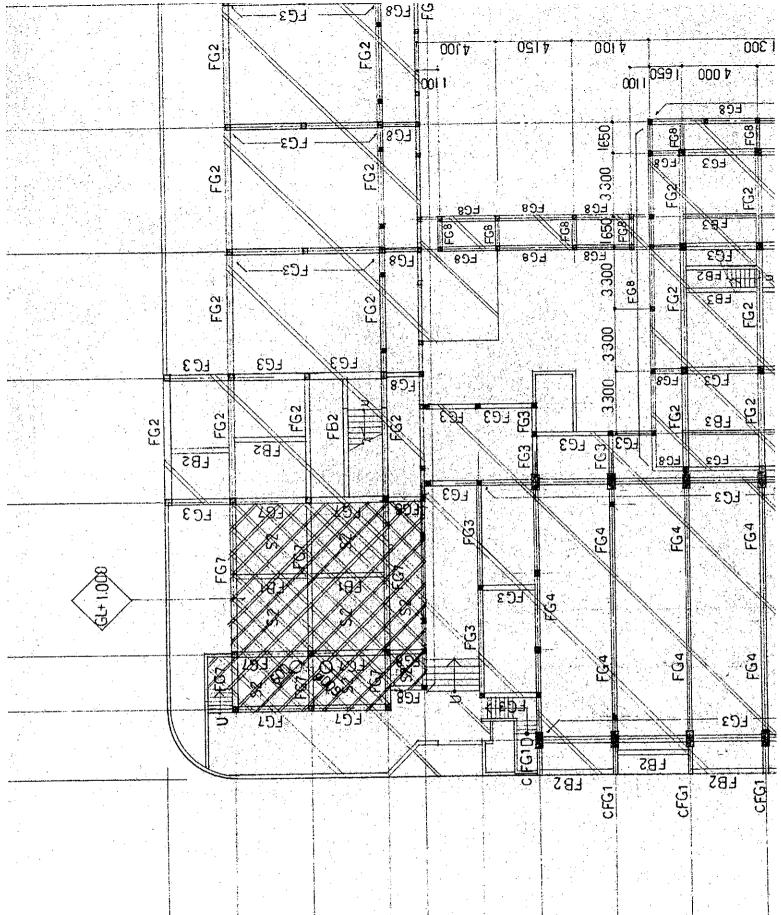
 $P = 12 \times 2.0 \times 5.75 \times 1/2 + 12 \times 2.5 \times 5.55 \times 1/2 + 6.1 \times 5.75 \times 1/2 \times 1.5$ = 173.0

 $P \to F / 4$ $18 / 8 / (1.5 \times 1.5) = 81^{100} / 1 - 7 F - 7 F - 7 f - 7$

325. 1/ (2.2×2.2) = 68 1/1-

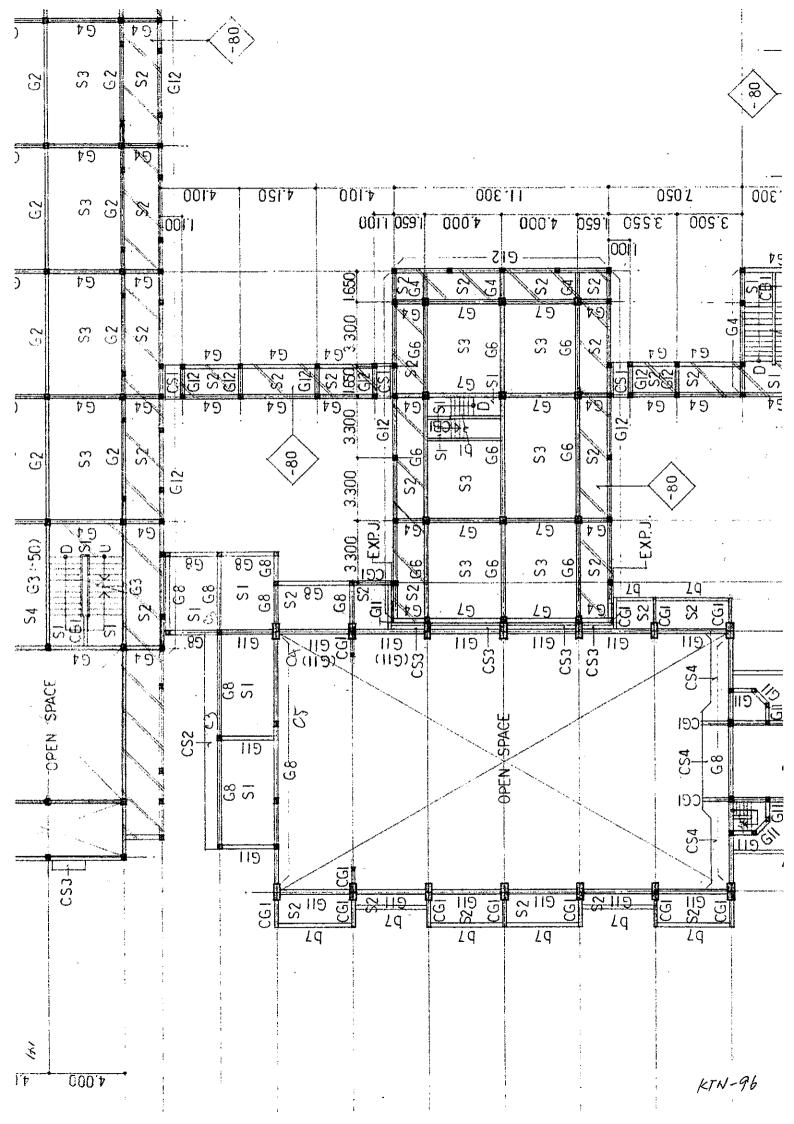


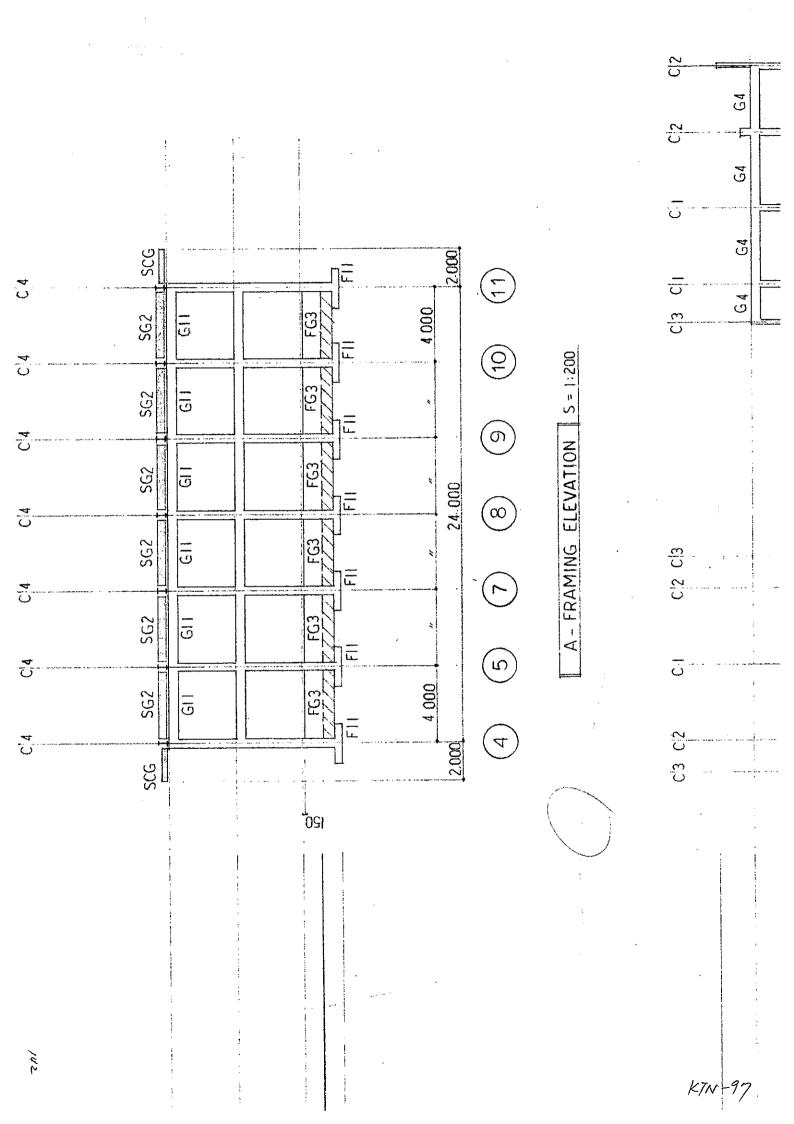




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KTN-95

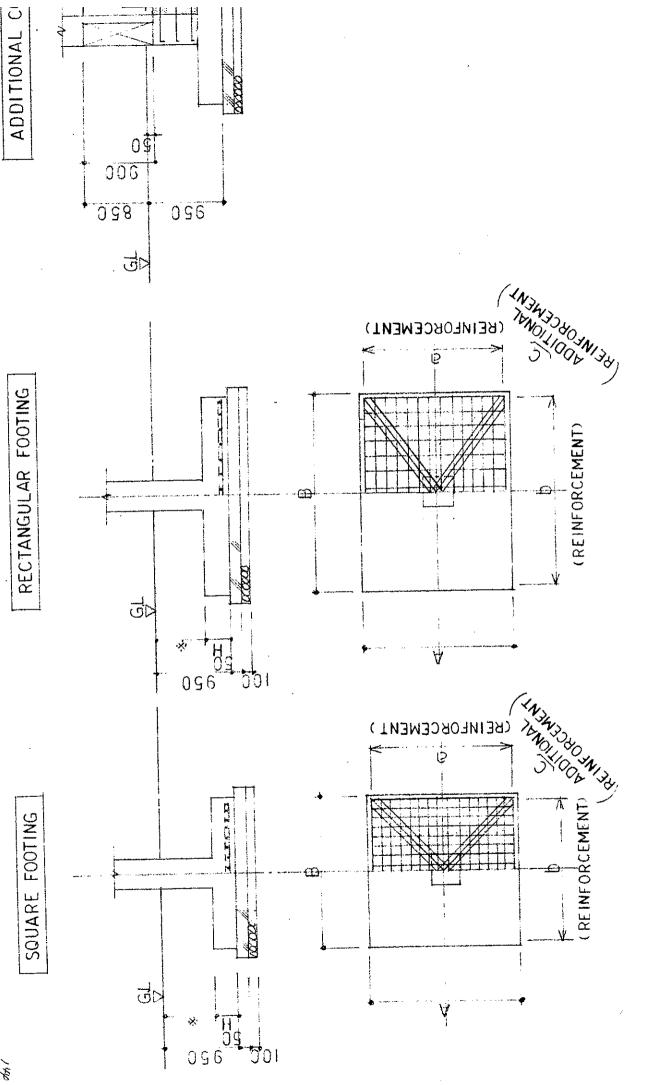




	WULTUR 1	I	2	-	Ð	Ģ
	 L	3,800	3,800	200	20-D19	20 - D19
	F2	3,300	3,300	500	1 7 – D 19	17 - D19
		3.100	3,100	500	16-019	16 – D19
	4	2,800	2,800	4 0 0	4 - D 9	4 - D 9
	(F5)	2.600	2.600	400	13 - D16	13 - D16
~	F6	2.700	4.500	500	16- D22	22-D16
	4	2500	3,200	400	12 - D19	16 – D16
	2	2.000	3.500	400	12 - D19	18 - D16
	C L	2,000	3,000	350	16 – D16	16 – D16
	FIO	2,000	2.600	350	4 – D 6	14 - D16
Ō	(EII)	2.200	2.200	300	12 – DI6	12 - DI6
1	F12	2.000	2,000	300	12-DIE	12- DIE
	<u>연</u> (1,800	1,800	300	- I I - DI6	D 6
\mathcal{O}	(E14)	1,500	1.500	250	8 – DI3	8 – DI3
	<u> </u>	1,200	1,200	250	7 — DI3	7 - DI3
	9 1-	1 O C	6,600	250	10-016	D13 @ 200
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		rankara a sangana - ► - Ayr sangangananan sa	·····			
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KTN-98

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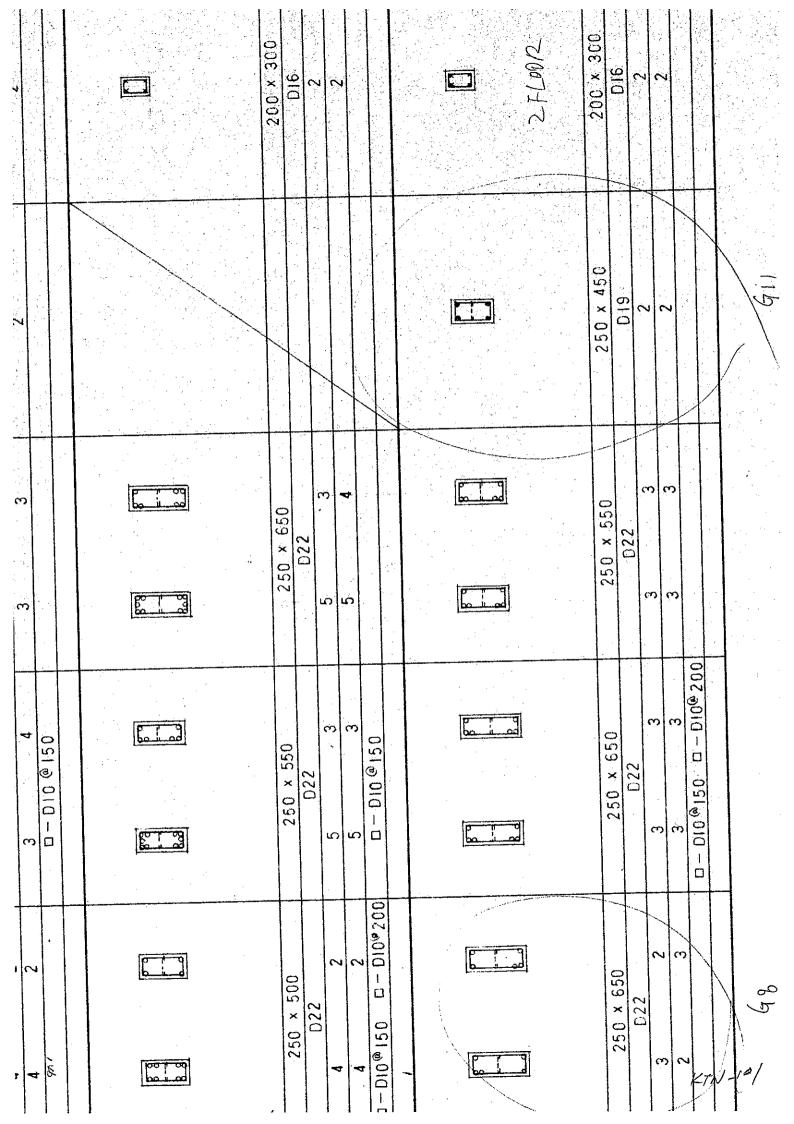


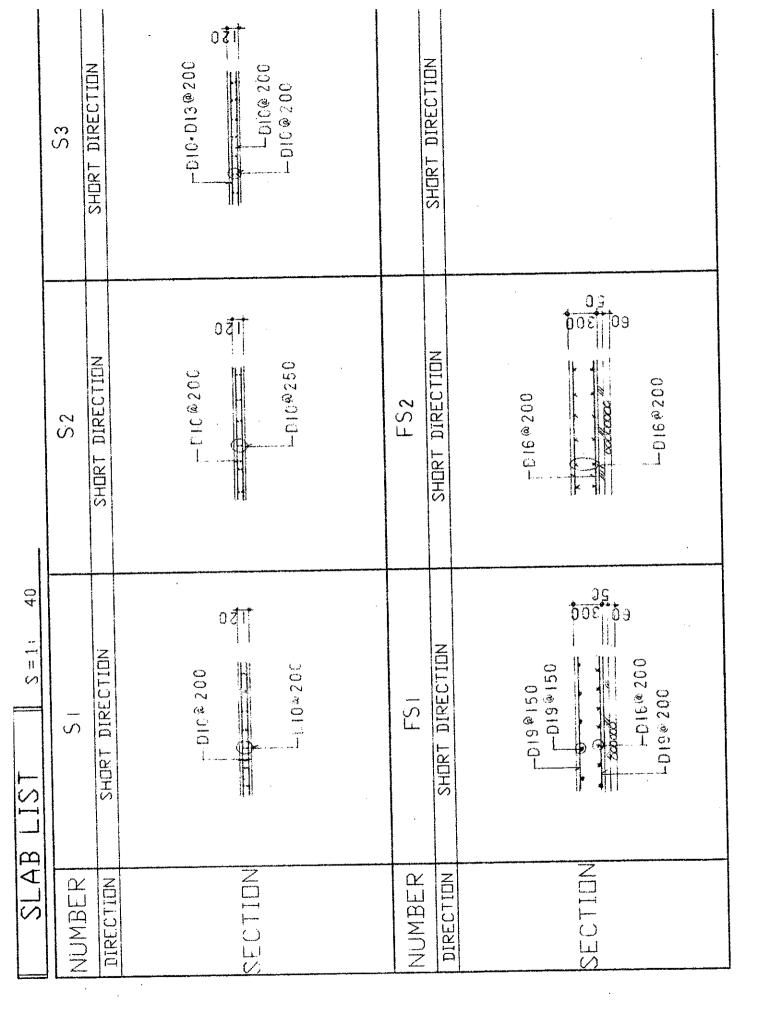
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dr l

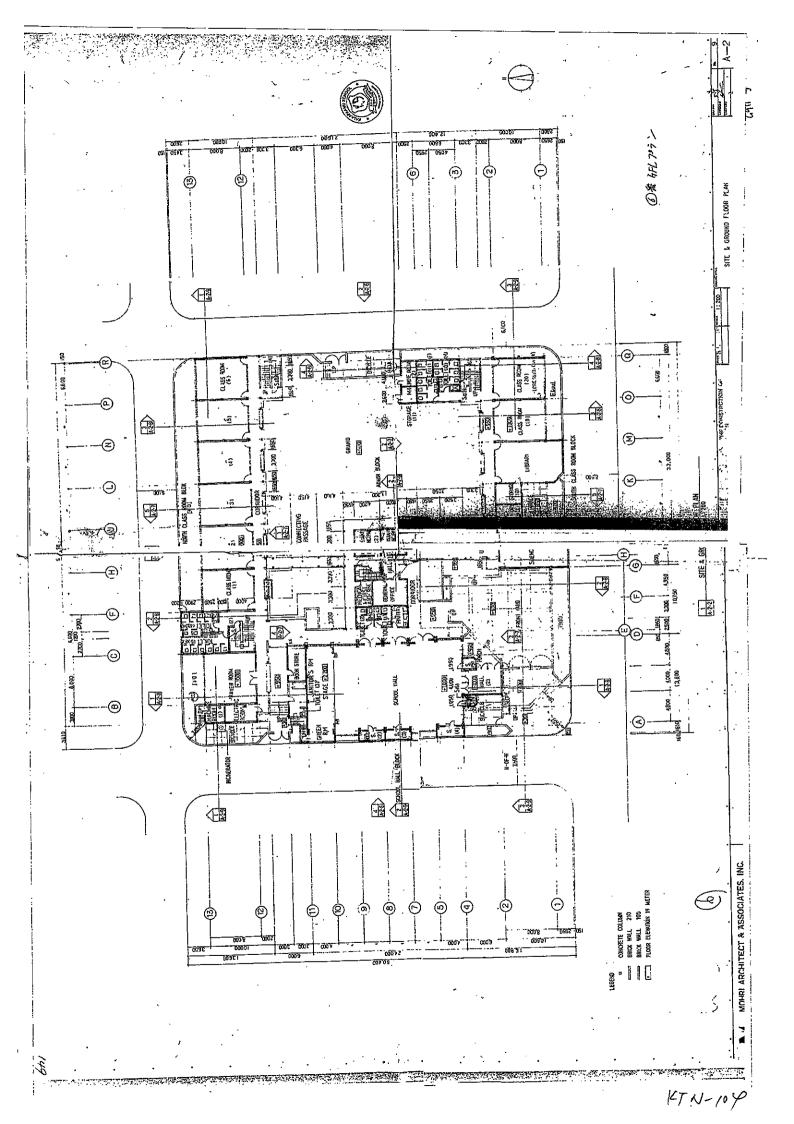
	Marine Andrews (1997) - 11 - 12 - 12 - 12 - 12 - 12 - 12 - 1	FG4			beed					400 X 300							
SPECIAL MENTION MATTER STIRRUP . DIO © 200 WEBREINFORCEMENT 4 - DIC		FG3 · FB2	E.C		Ľ.				250 × 900	: 6	06.6	2					···
SPECI Stirrup WEBREIT	S = 1 1 40 - 5PACINGBAR	FG2	E.C		B B) 		300 × 900	$1 \propto$	3	m					
	UNDERGROUND GIRDER LIST	FG I	OE C.IE	GL+850					30C × 900	D25.D22	6 - 025 4 - 625	3 - D22 3 - D22	E - D 3 @ 200				
	UNDERGRO	NUMBER	POSITION			SECTION			BXD	4IN REINFORCEMENT	TUP BAR	BDTTOM BAR	STIRRUP	SPACING BAR	147	TXI -	100

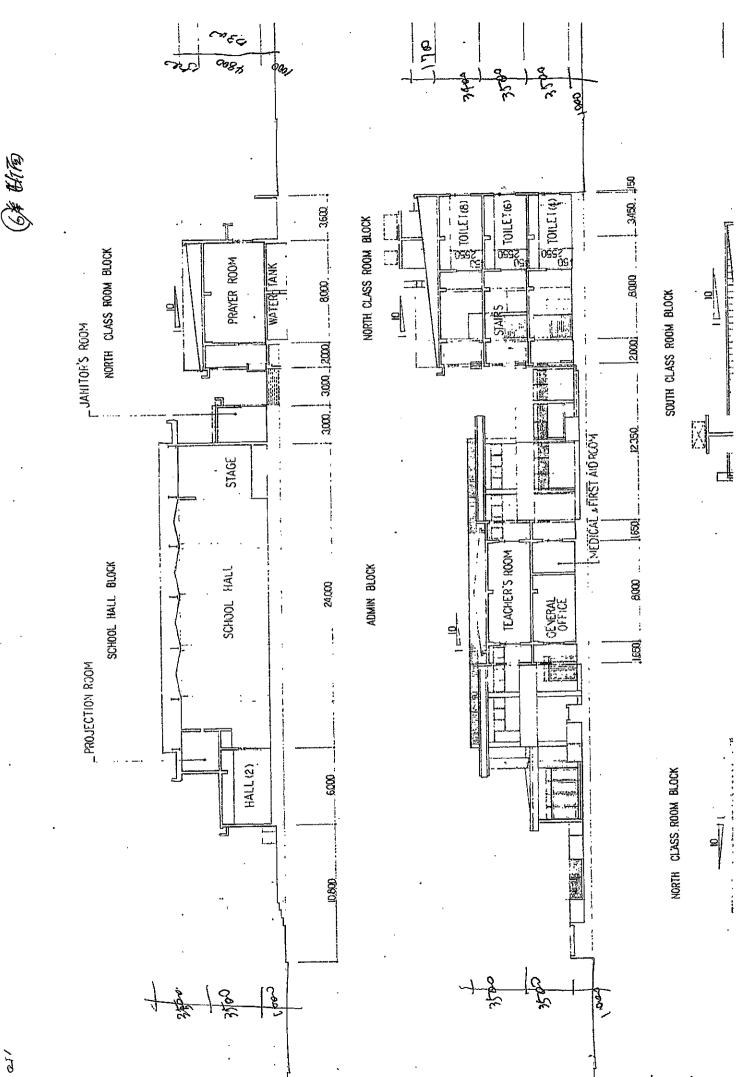
SPECIAL MENTION MA





	000.8	0 2'000 8'000	300 1000 1000	°	
		 400 × 400 4 - D22 • 4 - D19 ♦ - D6@600 	,	400 × 400 •4−D22 •4−D15 ◆−D6@600	5
		460 × 860 14 −D22 \$\$\$ −D10 @ 200		400×800 14-D22 曲-D10 ^毫 200	() C4
200 × 200 4 - D13 □ - D6 @ 100		200 × 200 4 - D13 □ - D6@ 100		200 × 200 4 - D16 E - D6@150	C.S.
3C0 × 30C °4-D22 *4-D16 □-D€ [@] 150 ◇-DE [®] 60C		300 × 500 -4 - 52 -4 - 519 -5 - 5 - 50 -5 - 5		300 × 300 300 × 300 •4 - €22 •4 - €19 □ - €6@150 ◇ - DE@ €C6	
00 4M - D13 - 50 - 600		00 - L 6 60C 60C	1 F. Lew	CC 50 50	KTN-103





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KTN-105

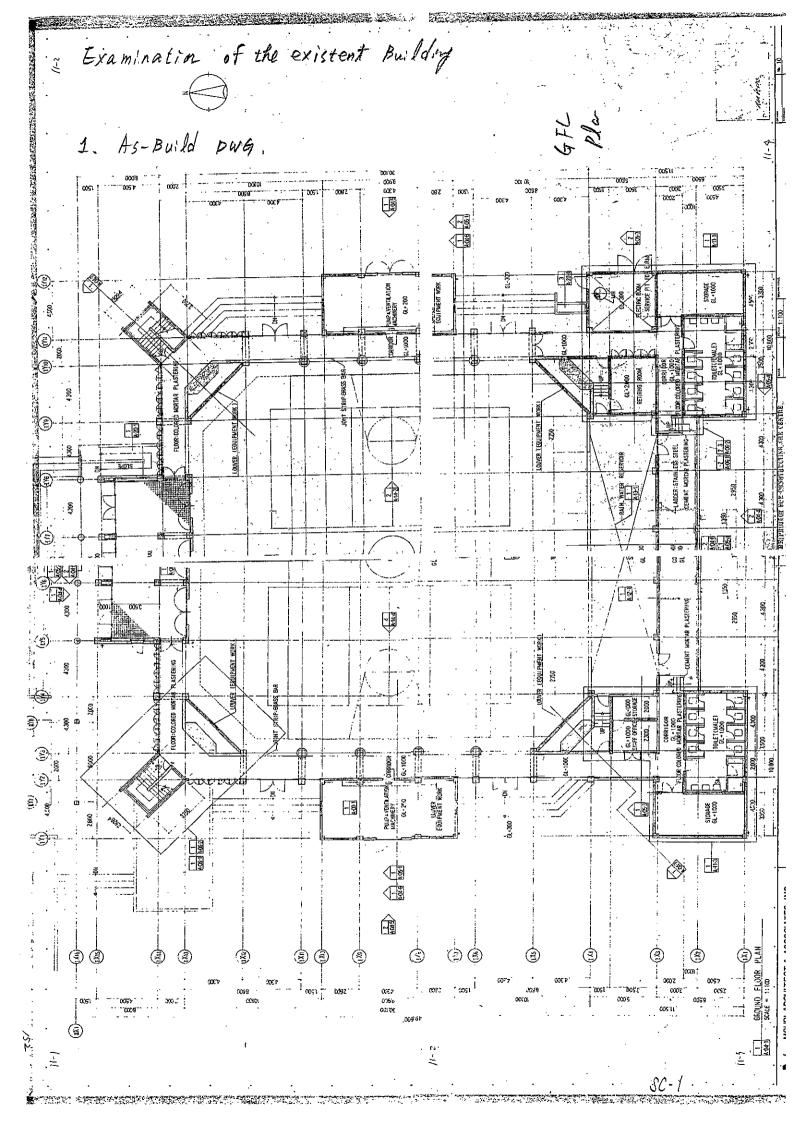
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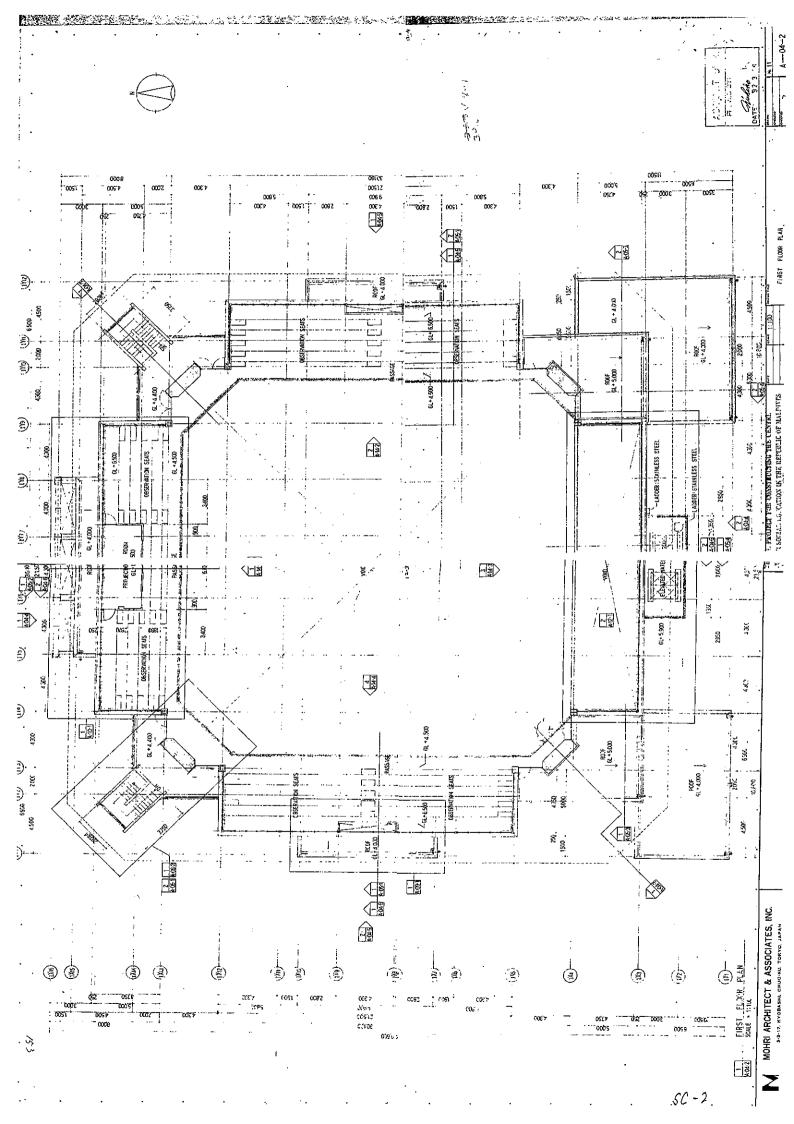
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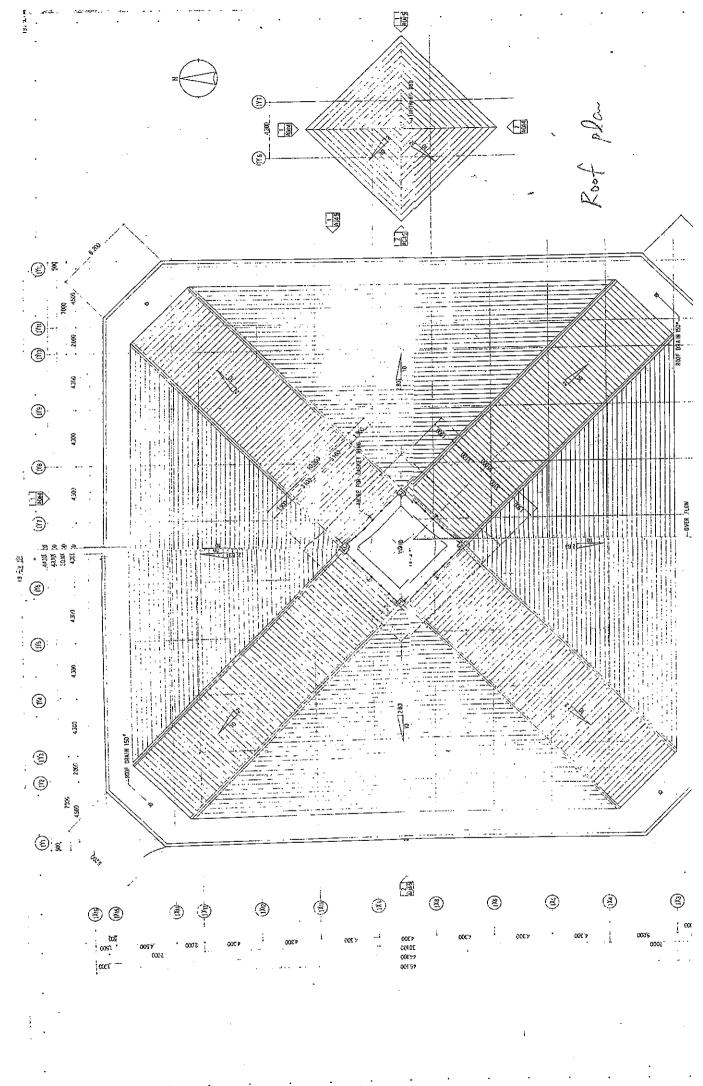
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- + PV panel and support frame : Weight is 0.5kN/m^2
- Sub beam and Truss are confirmed with the existen member.
- There is no problem in the safety because additional weight is 0.5kN/m^2 with the main structure member.

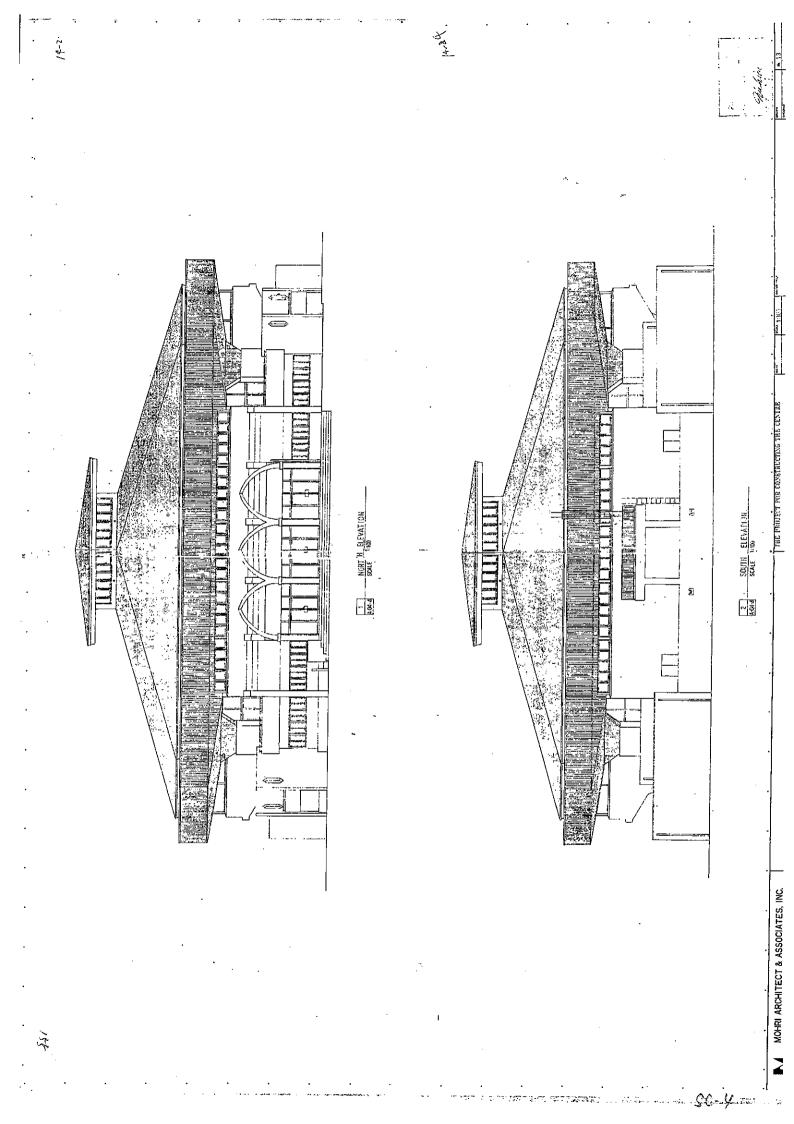


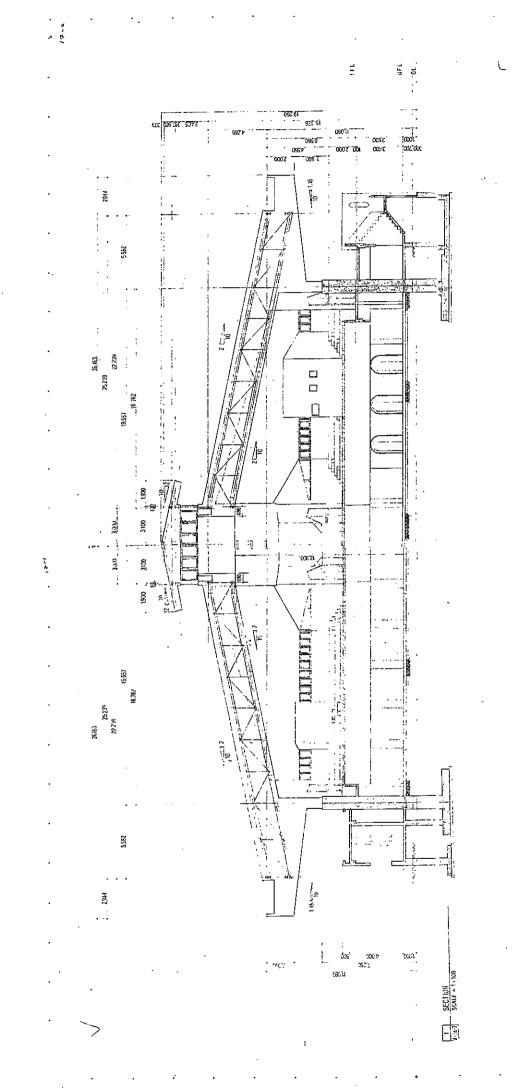




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50-3

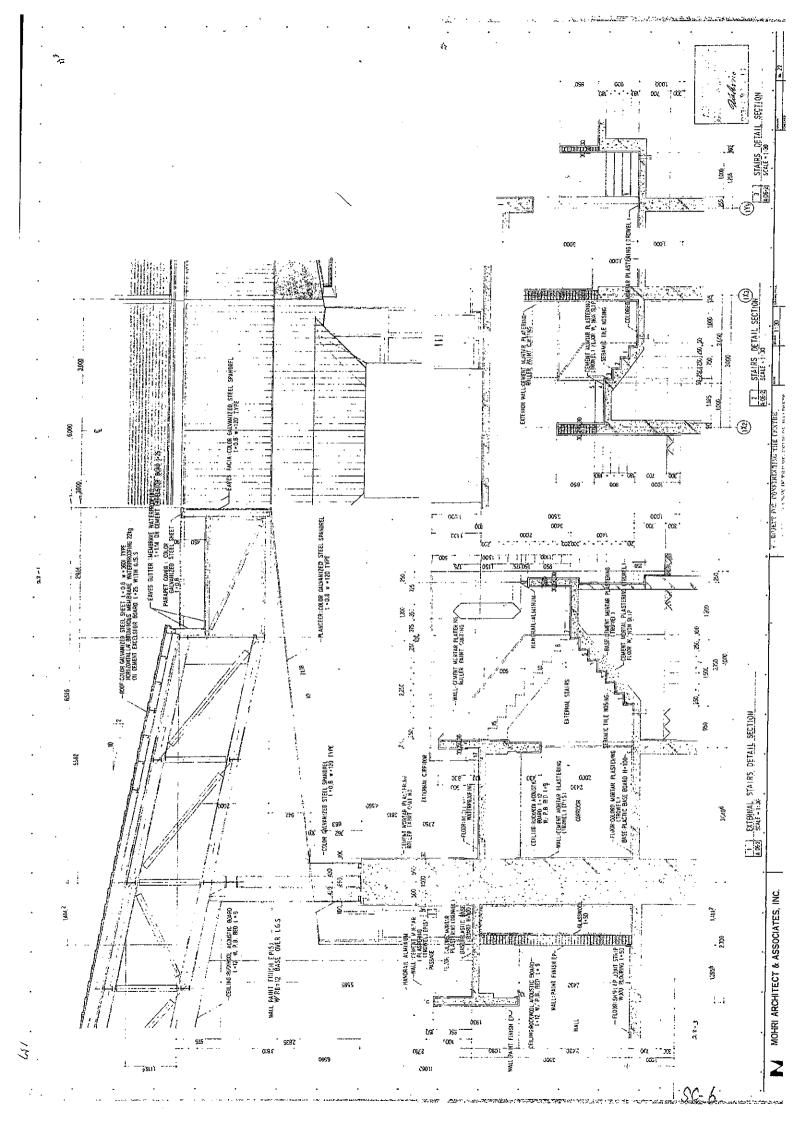


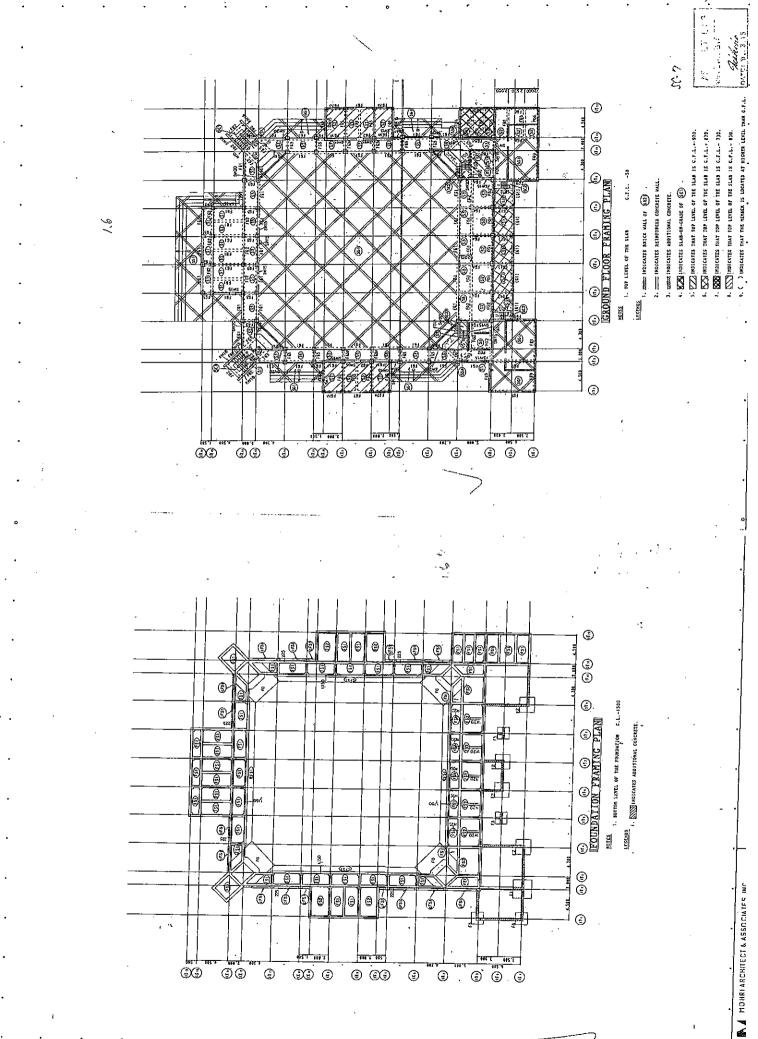


15-6

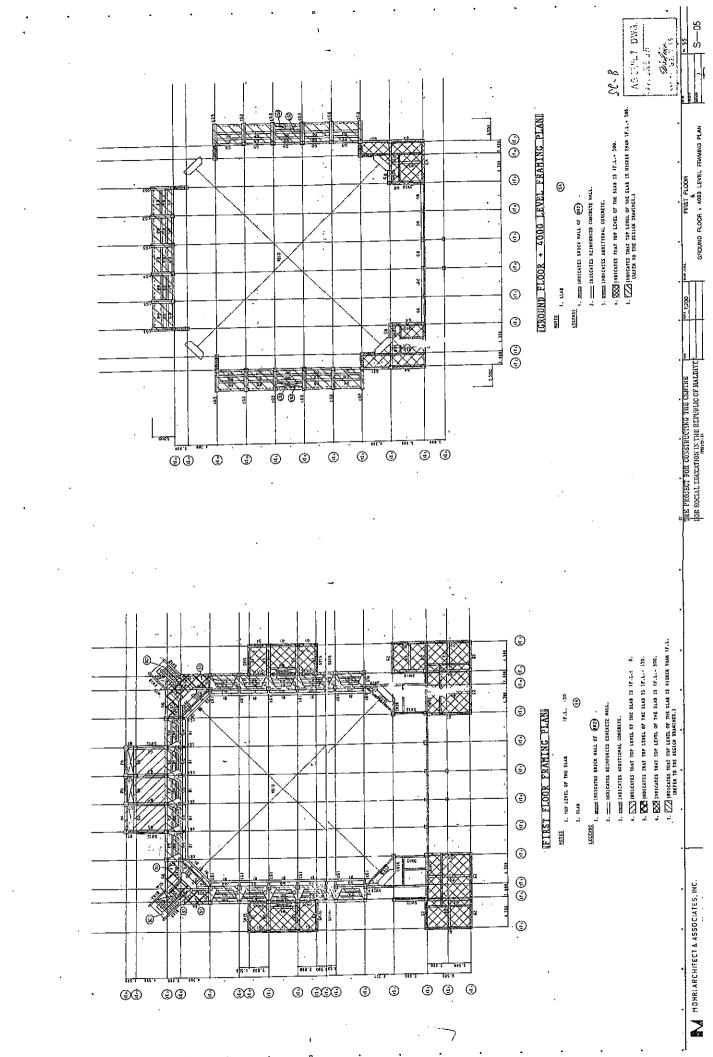
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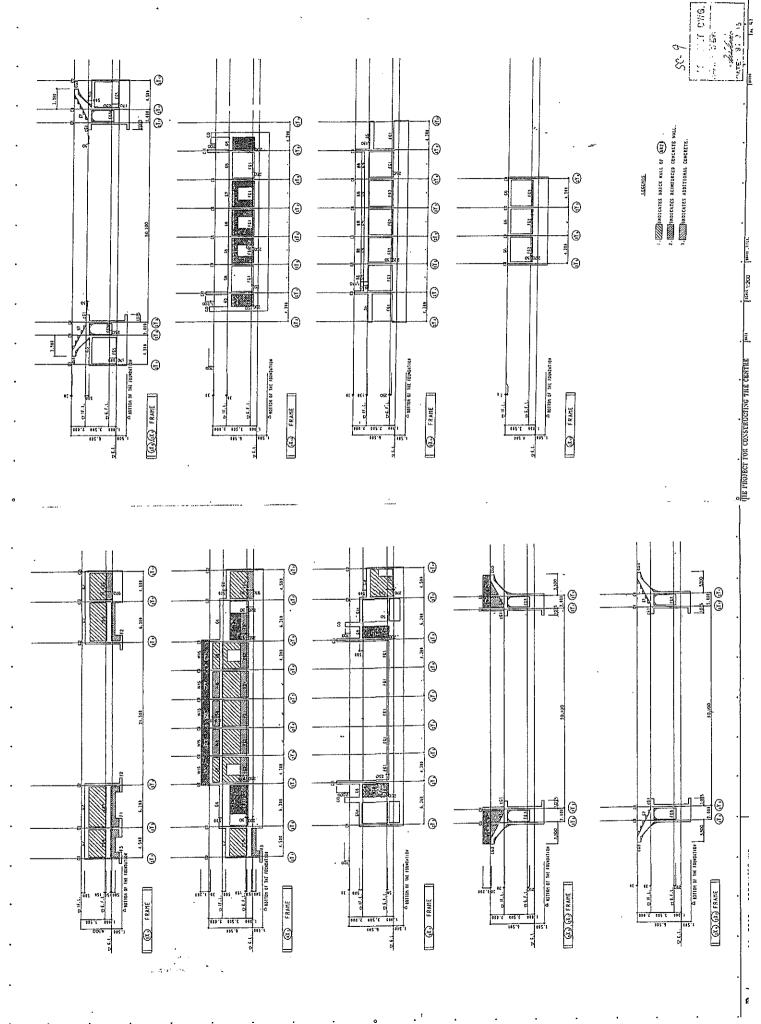


SC-7

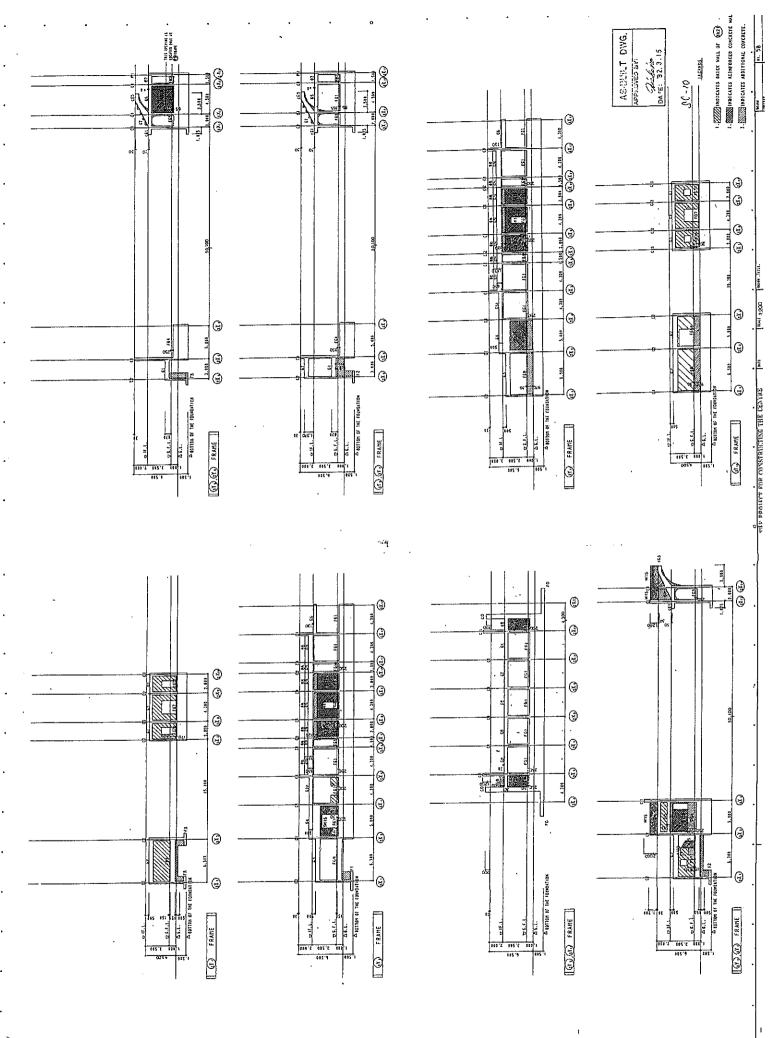


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SC-8



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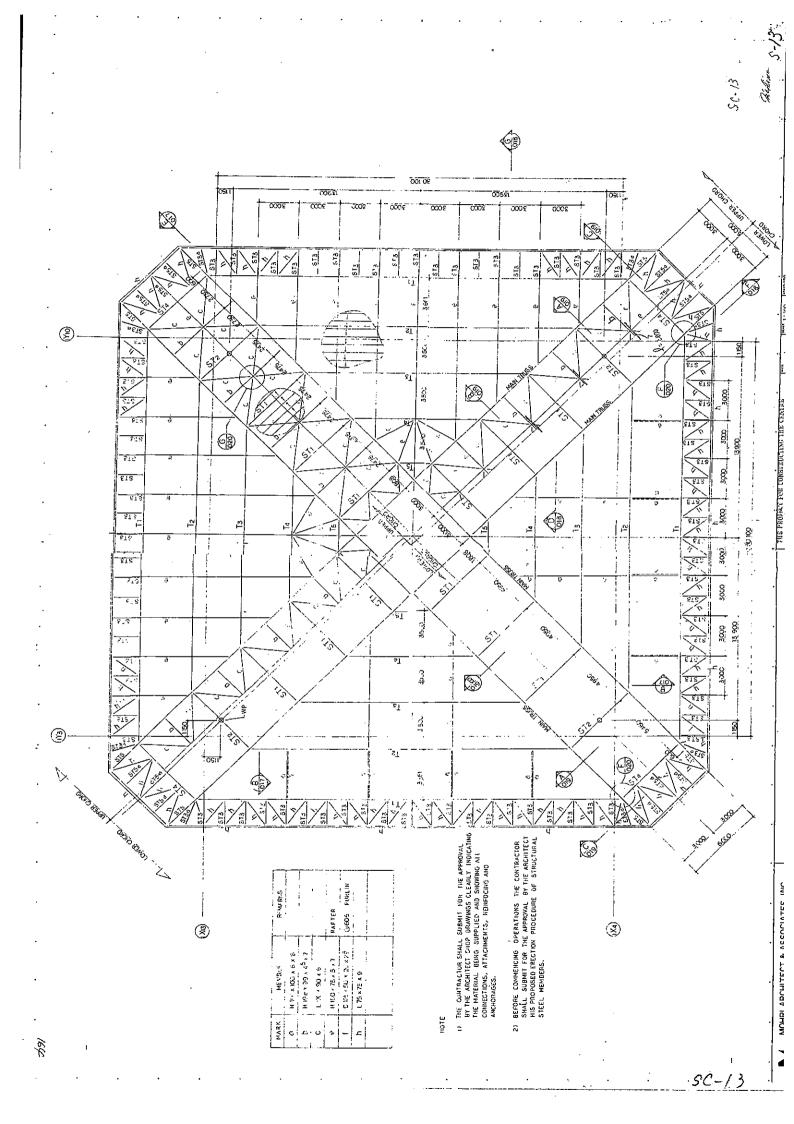


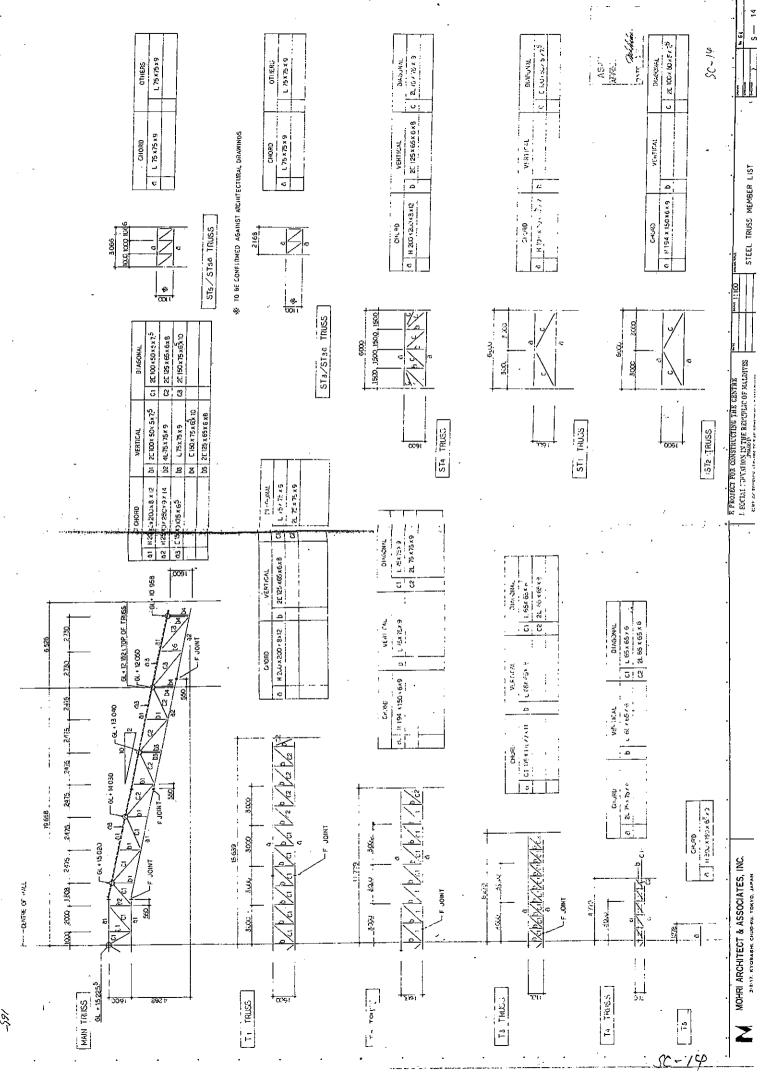
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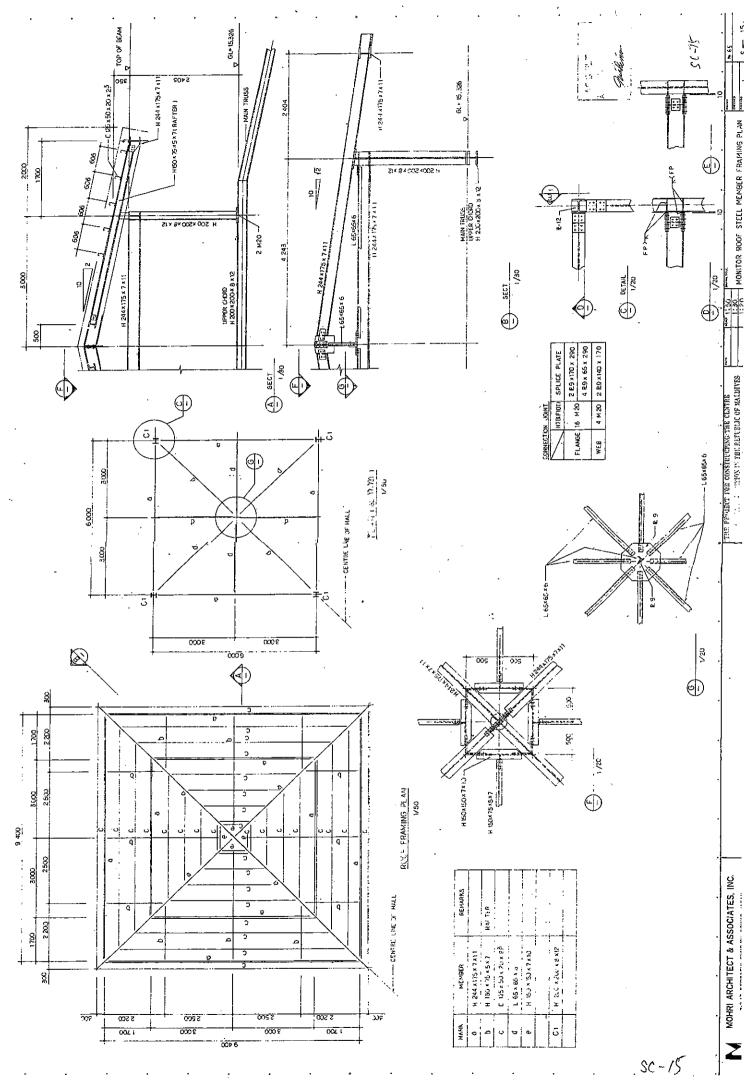
ĺ		- - -	•	•	•	•	•	•	•	•	•
BEAM SCHEDULE	CHEDULE S=1/50	NOTE :	NOTE : UNLESS OTHERWISE NOTED, 1.STIRRUPS O-Y106200 2 VEH /	2 MEH 6165 2-110 3 50575							
MARKS	10	33			f (81		E e		
SECTION					420 	1000 EEN.	50 F		12 Parts		
		ł	(951) 00	100		S	400				
10P BARS -	2-120	2-120	2-129	5-126 2-120 3-126 4-120	3-126	3-120	3-128		7:18 2:18		İ
VEB BARS				D-1124240		0-1124200 2-120(NAIN BARS)	-112+200				
HARKS		F92	FB3					·····			
POST TEON											· · ·
TOP BARS BOT.BARS STLRRUPS WEB BARS NOTE	2-120 3-120 3-120 3-120 3-114-100 8-112 8-112	2-120 2-120 2-120 2-120 8-112	3-120 End 3-120 1-1200 5-120 6-120								
SLAB SCHEDULE	CHEONLE S=1/50				WALL SCHEDUF C-1/60		NOTE : UNLESS DIERVISE NOTED	IOTED			İT.
HARKS	THICKNESS FOSITION	SHDRT SPAN	NF45 9001		5	SINS	SV20	IEK			
5	10P 8485 901.94RS	110,1129200 110,1129200	110,112,820		 						•
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ä		:	F10, F12+220		N101200						
cs i		1124299			119+200		2-12				•
CS2	120 TOP BARS 901 BARS TOP BARS	Y10,112 6200	10,1126,000			SALAR VALL	TILS SHEAR WALL	Π			
	SALA PARS				HARKS KV18			- <u>-</u> [•
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	[12, F154280	12,116920	•							•
123	900 001, BARS	112,4164204 1164204	T12,T16#200 T12,T16#200								
	EUL AASS TOP BARS BOL BARS	ac74011	112,7159286		H07, BARS 115.712 e200 V. BARS 2-712 H. BARS 2-712						
cf51	300 TOP BARS 801, BARS	T12,T16+205 T12,T16+209	· 112, 1164200				HO REINFORCEMENT NO RENTORCEMENT	ERENT			
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Z	MOHRI ARCHITECT & ASSUCIATES, INC. 3-5-17, KT06ASH1, CH00-KU, T0KT6, JAPAK	S, INC. IPAK			ROBJ THE CONTRACT OF STREET	THE PROJECT FOR CONSTRUCTING THE CENTRE		150 39m Juit.	BEAN, SLAB & WALL SCHEDULE	2001.000	E

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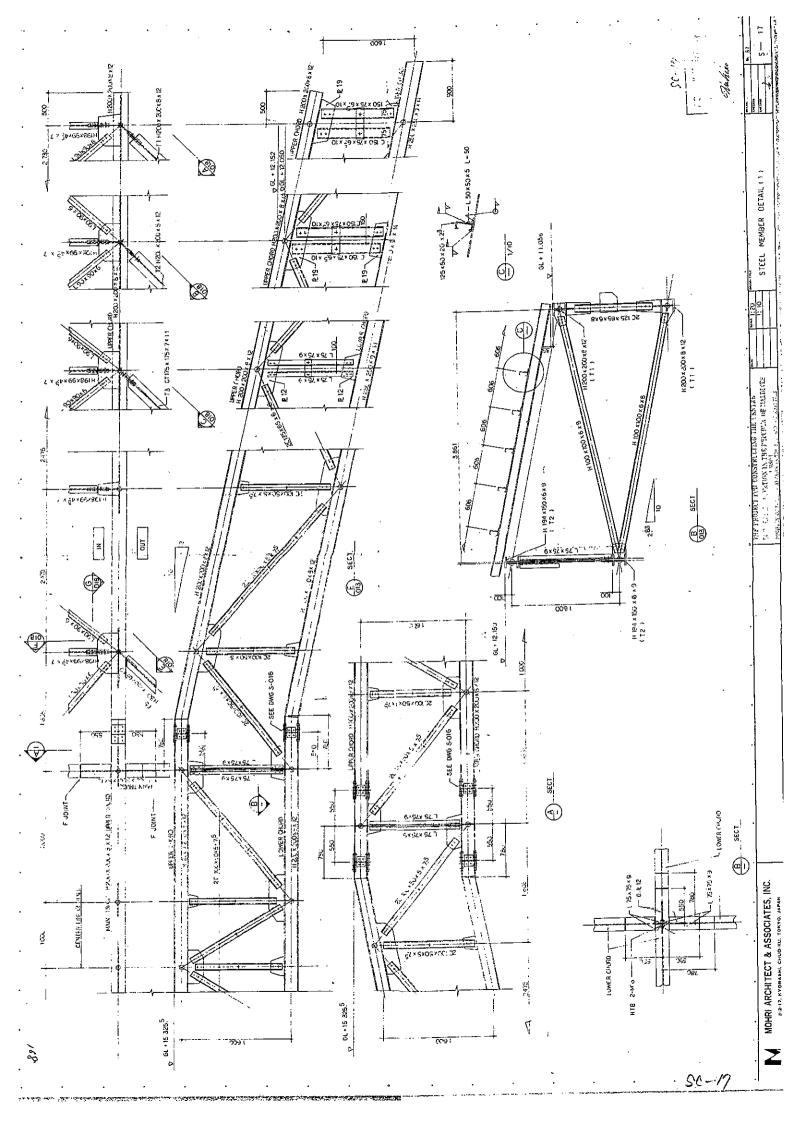
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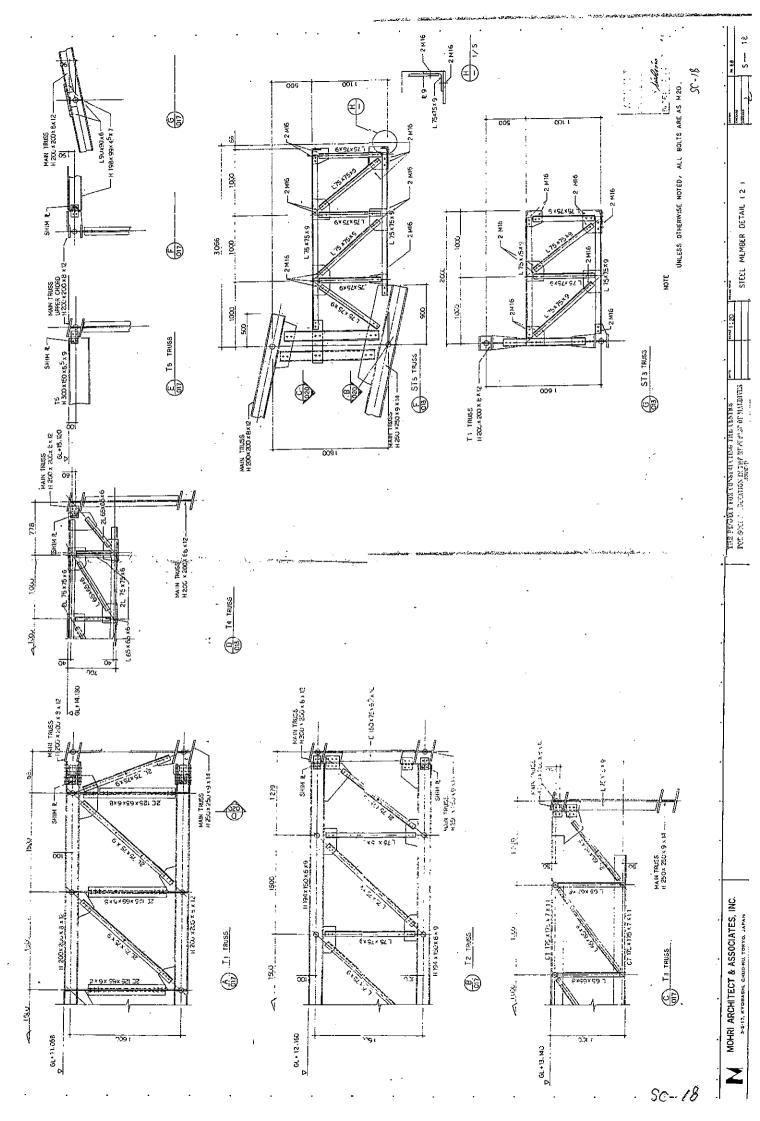


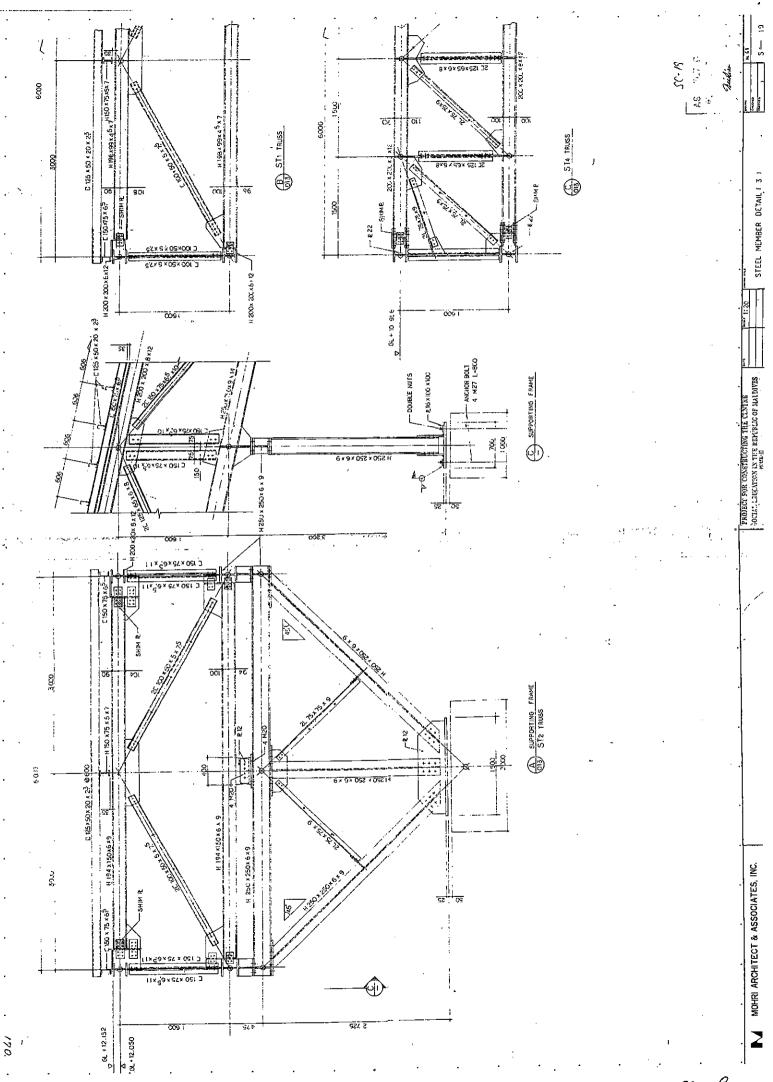
	DESCRIPTION	DESIGN STANE	STRUCTURAL SHAPES JIS 6 3192 - 1977 * 2 JIS 6 3350 - 1977 * 2	GRADE 55 41 JIS 6 3101 0R EDUIVALENT	 +1 AIJ : AKCHTECTURAL INCITITE OF JAPAN * 2 JIS : JAPANESE INVISTINAL STANDAOP 		21 BOLT	DESCRIPTION SPECIFICATION	HTB FVT JIS	80LT HOLE d + 1.5 mm	BOLT SPACE M20	MIN EDGE DISTANCE M20 - 40	1		DEIAL BOLT HOLE		*2 1	3 WELDING		DESCRIPTION SPECIFICATION	1A(FILLET-WELD SIZE 0.7 × 1 (PLATE THICKWESS)	*1 AWS : AMERICAN WELDING SCCIETY	4) ALL STRUCTURAL STEEL MEMBER SHALL BE PAIN TO WITH 2 COATS OF RED	e E	5) TIE-PLATE	21 ZC		1	61 EDGE CLEARANCE			1) GAUGE GAUGEL GAUGAUGEL GAUGEL GAUGEL GAUGEL GAUGEL GAUGEL GAUGEL GAUGEL GAUGEL GAUG	[++]→ ↓ [- 75x75x9] . 40]
GUSSET PLATE REMARKS	-+					¢ 4	MAX L1 \$ 1100		1 MAY 11 < 050			MAX L1 \$ 700			- 1			MAX LI \$ 600		MAX L1 5 600									SEE DWG S-O20 DETAIL	TO BE WELDED	 1			
Ţ	MEB	2. Ex 140 x 210	2R 9x140x210				P12	6 2		6		E12	65 84/	C10		1 0		;;;;;;;;;;;;;;;;;;;;;;;;;;;;;;;;;;;;;;	6. 	612	50	051 ¥ 141 × 14	28 3×140×170	2 E 9x140 x170	-	62		ת שיייייייייייייייייייייייייייייייייייי	P 12	-				
+	PECANGE	0	0 2 F 9 × 245 × 290 4 F 9 × 95 × 290						L						· .		,	, , , ,	:	., `		:				0			· · ·		: ; ;	:		-
-	r Lange	16-M20 4-M20	16-M20					D2M-2	L 2-M2Ú	2-M20	•	2-M20	••••••	- 2-M20	2-M20	0-M-2			24-2		2-M20	4-W20	: : 	4-M20		: : :	2 M2D		2 M20	<u>.</u>		•		
SHAPE MEMBER		H200×200×8×12	H 250×250×9×14			: : : : : : : : : : : : : : : : : : :	20 150×75×65×10		2L 125×65×6×8	L 125×65×6×8		2C 100×50×5× 7°		4L75×75×9	2L75×75×9	L75 × 75 × 3		L 21.65×65×4		121.65×65> 5	+ L65×35×6	H 200×200×15		1 H 198 × 90 × 45		H 150 ×150 × 7×10	H 100×100×6×8		L 30 × 50 × 6				TES	
		40 4040 40	· ††		H11					• •	() — нтв	<u>20140 40</u>	1	•		3 .0 ()		Ì.	· · · · · · · · · · · · · · · · · · ·		6	۲- ۱۱۲۴		SHM 2- 2 5F.E.	10°		,						NOTE LI DISTANCE SETWEEN TIE-PLATES	

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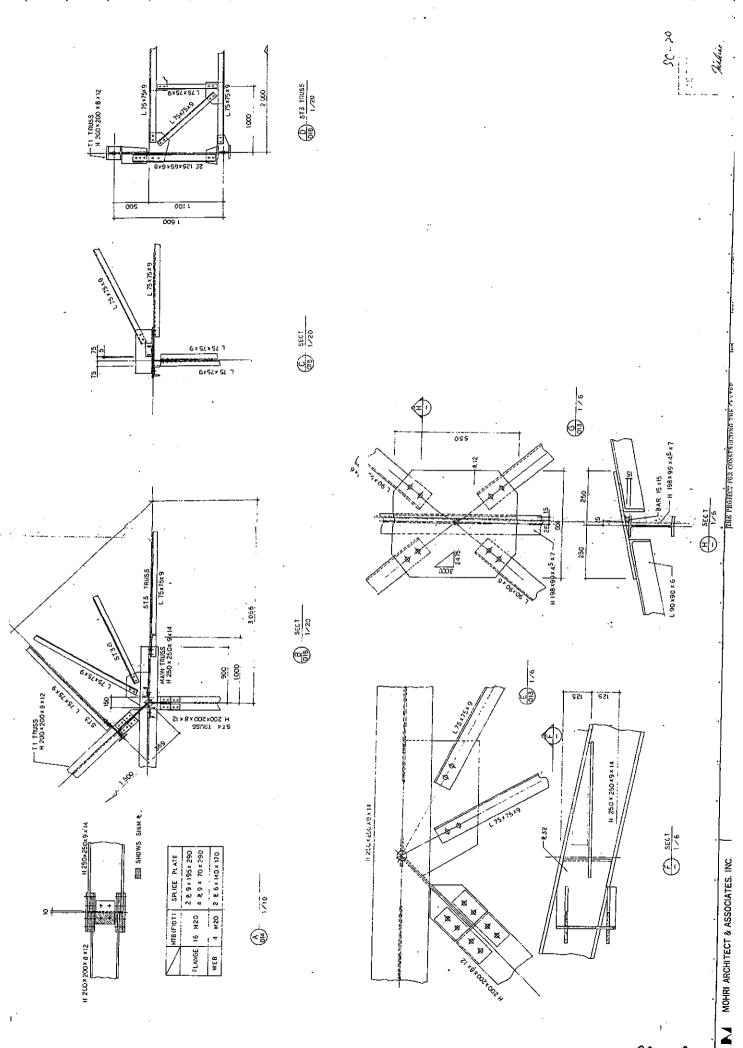
SC-16



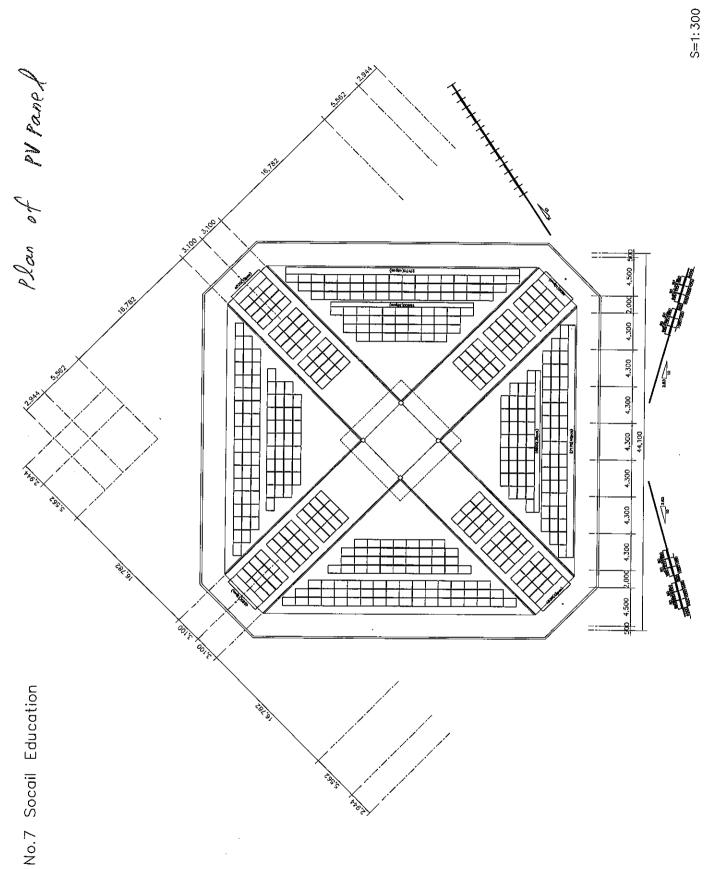




50-19



. 50 - 20



Structural Calculan (Dead load)	2、BBtz E) The e	B村の稼ず. xamination	of the	existent	member
Steel roof (+ board)	260.				
Purlin	150				
Ceiling PV parel + bear.	200 600 7 500 }	1. 1 100 m			
SUB BEAM See dwg S-13 [C]					

 $W = 1.1^{KN/m^{-1}} \times 3.0^{M} + 0.5^{12N/m} \times 3.8^{16N/m} \quad (-5.2) < 1.5,$ $M = \frac{1}{8} \times 3.8 \times 2.75^{2-2} = 3.6^{KN/m} \qquad \text{Wind} \quad (3000^{K/m^{-2}})$ 2.750 M= 1/8×3.8×2.75 = 3.6 KN. Q= 1/2×3.8×2.75 = 5.3 KN.

$$\frac{H-150\times 75\times 5\times 7}{86.8} = 1 = 1 = 166 = 2 = 88.8 = 157.$$

$$\frac{5}{6} = \frac{3.6\times 10^3}{88.8} = 41 = 56/f_0 = 0.27 < 1.0$$

$$\frac{5}{7.84} \times 3.8 \times 2.75 \times 10^{5}$$

$$= 2.1^{m^2} = \frac{1}{1309}$$

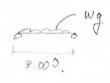
	(b) Spe dwg.	5-13
	En le	~ w when h 1 lon/2 × 0.5" + 0.5 100/ - 1.05 KN/m
	5.000	P= 5.3 KN x 2 = 10.6 KN
		M= 1/8 × 105 × 6.0" + 1/4 × 10.0 × 6.0 = 20.7 100.00
		Q= 1/2 x 1.05 x 6.0 + 1/2 x 10.6 = 8.5KM
r4ms	·	H-198×99×45×7 I×-1540 Ex=156 lb= 3000 fb=159
12	105	66 = 20.7×103 = 183 1/1 6h/fb. 0.85<1.0
	, [`	June - 5 x 1.05 x 6.0 x 10 + 10600 x 6.0 x 5.62 + 15. 11 = 20.73 = 1.62 + 15. 11 = 20.73 = 1/189
~		l=2.0m

193

50-22



2/10



3. 屋根鉄骨部材の算定 母屋^(purlin)

部材 C -	125 x 50	x 20	x 2.3	
スパン L(m), ピッチ(m)		3.00	0.606	
荷重 Wg (KN/m²) (LOAD)		$3.00 \\ 1.10$		
曲げモーメント xMg (KN・m) (Bond Homoni) yMg (KN・m)	$\omega \cdot \text{SIN } \theta \cdot \text{L}^2/8 =$	0.15		
yMg (KN·m)	$\omega \cdot \cos \theta \cdot c^{2}/8 =$	0.74		
せん断力 yQg (KN) (Shea)	$\omega \cdot \cos \theta \cdot L/2 =$	1		
断面性能 Iy (cm ⁴)	137			
Ix (cm^4)	20.6			
$Zy (cm^3)$	21.9			
$Zx (cm^3)$	6.22			
Alaucha				
許容曲げ応力度 fb (KN/cm ²)	15.7			
曲げ応力度 xσb = xM/Zx(KN/cm ²)	2.42			
曲げ応力度 y σ b = yM/Zy (KN/cm ²)	3.38			
応力度比 (x σ b+y σ b)/fb ≦ 1	0.37	\leq	1	OK
撓み δg (cm)	0.49			
$\delta/L \ge 300$	612	\geq	300	OK

(e) $M = \frac{1}{8} \times \frac{3.0 + 0.5}{8.02} = \frac{1.7 \times 10^{-1}}{1.7 \times 10^{-1}}

SC-22a

6

$$\begin{split} sub TRuss \\ (ST1) & see PMD 5-12 \\ & f$$

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135

Тор

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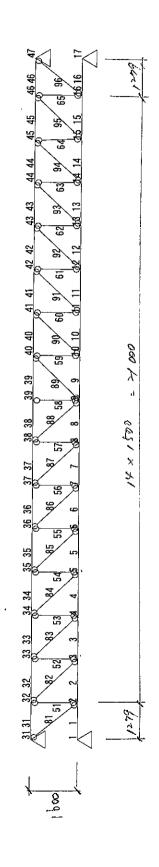
$$Trisss
[T2] J + 7Z
Main Nomber
 $\pm 783,77$
 $T35,77$
 $N=203 \text{ km}. \text{ sc}^{-37}$
 $\frac{11.194\times150\times6\times9}{735,77}$
 $\frac{11.194\times150\times6\times9}{100} = 100\%$
 $6:-\frac{2030}{33.11} = 54$
 $6:-\frac{2030}{33.11} = 54$
 $6:-\frac{2030}{33.11} = 54$
 $6:-\frac{50}{74} = 5\% \text{ sc}^{-2} = 10\%$
 $1\times = 81 \text{ km}$
 $\frac{5C-14,18}{10}$
 $1\times = 81 \text{ km}$
 $\frac{1.-25\times92\times9}{100} = 54$
 $6:-\frac{810}{1269} = 64$
 $5C-14,18$
 $5C-14,18$
 $1\times = 58.$
 $6:-\frac{810}{1269} = 64$
 $5C-14,18$
 $5C-14,18$
 $1\times = 103 \text{ km}$
 $5C-14,18$
 $5C-14,18$
 $1\times = 103 \text{ km}$
 $1\times = 103 \text{ km}$
 $1\times = 103 \text{ km}$
 $5C-14,18$
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 $1\times = 103 \text{ km}$
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 $1\times = 103 \text{ km}$
 $1\times = 103 \text{ km}$
 $5C-14,18$
 $1\times = 103 \text{ km}$
 $5C-14,18$
 $1\times = 103 \text{ km}$
 $1\times = 103 \text$$$

56-14.18

** UNION SYSTEM ** 3.37 2009/06/12 20:00 PAGE-

*** Super Build/FA1 *** 250780 [N07T2] 架構図

Frame

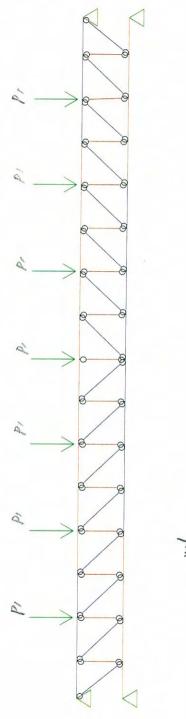


441

25-25

** UNION SYSTEM ** 3.37 2009/06/12 20:06 PAGE- 1

1040



Pr = 23,4 KN.

*** Super Build/FA1 *** 250780 [N07T2] 応力図 [DL+LL]

** UNION SYSTEM ** 3.37 2009/06/12 20:18 PAGE-

0 12 1/13. 17 18. 57 45. 07 50. 07 76. 77 82. 47 102. 37 1/13. 1 113 91 102 51 82 61 76 81 50 21 45 27 18 61 wes

応力図 [DL+LL]

chor of

्युट्रे के 131 जिले हा होले व डरेले वह होले 113 व्रेस 126 होकी 255 हा कि 175 हा के 25 हा के 47. हा के 4. हह के हा हह के 191 हा के 25 ह 102.51 172.51 46. 2T 71.7C 84.5C 93.0C 93.0C 83.9C 71.3C 40.5C 6.3C 40. 8C 6. 5C 46. 0T 172. 61 102. 51

応力図 [DL+LL]

7209

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【基本事項・計算条件】

工 事 名:モレディブ No7 T2 略 称:N07 T2 日 付:2009/06/04 担当者:	
・せん断による変形の考慮 ・ 関域の考慮 ・ 伊縮しない材(A を1000倍) ・ 節点同一変位の指定 ・ 部材毎の増減率の考慮 ・ バネ材の使用 ・ 結合状態の共通指定 ・ 応力着目点の追加 ・ 接合部パネル変形の考慮 ・ 剛減は・水ネルの軸変形の考慮 ・ 剛減は・水ネルの軸変形の考慮 ・ 剛減は、水、地の面変形の考慮 ・ 剛減は、水、地の面変形の考慮 ・ 剛減は、水、地の面変形の考慮 ・ 剛減は、水、地の面変形の考慮 ・ 剛減は、水、地の面変形の考慮 ・ 剛減は、水、地の面変形の考慮 ・ 剛減は、水、地の面変形の考慮 ・ 剛減は、水、地の面変形の考慮 ・ 剛減は、水、地の面変形の考慮	: すする すする : すす有無無な材ないい : むしなないい : ししなないい : ししなないい : : : : : : : : : : : : : : : : : : :
・出力単位	:SI単位

【節点座標】	[m]
--------	-----

No	X座標	Y座標	No	X座標	Y座標	No	X座標	Y座標	No	X座標	Y座標
1 2 3 4 5	0.000 1.279 2.779 4.279 5.779	0. 000 0. 000 0. 000 0. 000 0. 000 0. 000	11 12 13 14 15	14. 779 16. 279 17. 779 19. 279 20. 779	0.000 0.000 0.000 0.000 0.000 0.000	34 35 36 37 38	4. 279 5. 779 7. 279 8. 779 10. 279	1.600 1.600 1.600 1.600 1.600 1.600	44 45 46 47	19. 279 20. 779 22. 279 23. 558	1.600 1.600 1.600 1.600 1.600
6 7 8 9 10	7. 279 8. 779 10. 279 11. 729 13. 279	0.000 0.000 0.000 0.000 0.000 0.000	16 17 31 32 33	22. 279 23. 558 0. 000 1. 279 2. 779	0.000 0.000 1.600 1.600 1.600	39 40 41 42 43	11.729 13.279 14.779 16.279 17.779	1.600 1.600 1.600 1.600 1.600			

【支点データ】 [kN/cm] [kNm/rad] (Oは自由、1は拘束を表します。) No ^{/----} 節点No. ----/ (1) 〈2〉 〈3〉 X方向パネ Y方向パネ 回転パネ 1 1 31 1.0 1.0 0.0 2 17 47 1.0 1.0 0.0

【材質】 [kN/mm2]

No E G

1 205.00 79.00

【断面性能】

No	A [cm2]	I [cm4]	κ	断面名
1	38.11	2625	3.61	H−194≭150≭6≭9≭8
2	12.69	64	3.00	1_75*75≭9
3	25.38	129	3.00	2L−75*75≭9

【部材配置】(断面No.が負値の材は、伸縮しない材を表します。)

No	/節点 i端	No/ j端	断面No	材質No	/結合 i端	No// j端	——剛域 ;端	[cm]/ j 端
1 2 3 4 5	1 2 3 4 5	2 3 4 5 6	1 1 1 1	1 1 1 1	0 0 0 0 0	0 0 0 0	0.0 0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0 0.0 0.0

*** Super Build∕FA1 *** 250780 [N07T2]

Super	Build∠	'FA1 *	** 250780	[N07	T2]			
No	/節点 i端	āNo/ 〕端	断面No お	f質No	/結合N i端	o//- j 端	剛域 i 端	[cm]/ j 端
6 7 8 9 10	6 7 8 9 10	7 8 9 10 11	1 1 1 1	1 1 1 1	0 0 0 0	0 0 0 0 0	0.0 0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0 0.0
11 12 13 14 15	11 12 13 14 15	12 13 14 15 16	1 1 1 1	1 1 1 1	0 0 0 0	0 0 0 0 0	0.0 0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0 0.0 0.0
16 31 32 33 34	16 31 32 33 34	17 32 33 34 35	1 1 1 1	1 1 1 1	0 0 0 0 0	0 0 0 0	0.0 0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0 0.0 0.0
35 36 37 38 39	35 36 37 38 39	36 37 38 39 40	1 1 1 1	1 1 1 1	0 0 0 0 0	0 0 0 0	0.0 0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0 0.0
40 41 42 43 44	40 41 42 43 44	41 42 43 44 45	1 1 1 1	1 1 1 1	0 0 0 0 0	0 0 0 0	0.0 0.0 0.0 0.0 0.0	0. 0 0. 0 0. 0 0. 0 0. 0
45 46 51 52 53	45 46 2 3 4	46 47 32 33 34	1 2 2 2	1 1 1 1	0 0 1 1 1	0 0 1 1 1	0.0 0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0 0.0
54 55 56 57 58	5 6 7 8 9	35 36 37 38 39	2 2 2 2 2	1 1 1 1	1 1 1 1	1 1 1 1	0.0 0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0 0.0
59 60 61 62 63	10 11 12 13 14	40 41 42 43 44	2 2 2 2 2	1 1 1 1	1 1 1 1	1 1 1 1	0.0 0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0 0.0
64 65 81 82 83	15 16 31 32 33	45 46 2 3 4	2 2 3 2 2	1 1 1 1	1 1 1 1	1 1 1 1	0.0 0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0 0.0
84 85 86 87 88	34 35 36 37 38	5 6 7 8 9	222222	1 1 1 1	1 1 1	1 1 1 1 1	0.0 0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0 0.0
89 90 91 92 93	9 10 11 12 13	40 41 42 43 44	22222	1 1 1 1	1 1 1 1 1	1 1 1 1	0. 0 0. 0 0. 0 0. 0 0. 0	0-0 0.0 0.0 0.0 0.0
94 95 96	14 15 16	45 46 47	2 2 3	1 1 1	1 1 1	1 1 1	0.0 0.0 0.0	0. 0 0. 0 0. 0

** UNION SYSTEM ** 3.37 2009/06/12 20:16 PAGE- 2 [任意形平面フレーム応力解析]

*** Super Build/FA1 *** 250780 [N07T2]

** UNION SYSTEM ** 3.37 2009/06/12 20:16 PAGE- 3 [任意形平面フレーム応力解析]

【荷】

【荷重ケ-	ース 1 】 DL+	+LL						
No	/節点, 部材 〈1〉 〈2〉	No.~/ 〈3〉TYPE 方	向 P1	P 2	РЗ	Ρ4	P 5	Ρé
1 2 3	33 35 39 41 45	37 0 43 0 0	0. 000kN 0. 000kN 0. 000kN	-23. 400kN ~23. 400kN ~23. 400kN	0. 000kNm 0. 000kNm 0. 000kNm			
【支点反ス	ל 1							
※※ 荷	重ケース 1	≫≫ DL+LL						
節点No	Rx [kN]	Ry [kN]	Rm [kNm]					
1 17 31 47	202.2 -202.0 -243.7 243.5	-4.3 -4.3 86.2 86.1	0.0 0.0 0.0 0.0					
合計	0. 0	163. 8	0. 0					
【節点変位	z]							
※※ 荷	重ケース 1	₩₩ DL+LL						
節点No	δx [cm]	δy [cm]	θ [rad]	節点No	δx [cm]	δy [cm]	θ [rad]	
1 2 3 4 5	0.000000 -0.033102 -0.058273 -0.069981 -0.070848	-0.083853 -0.332915 -0.587852	-0.00048999 -0.00114086 -0.00178422 -0.00167229 -0.00150841	34 35 36 37 38	0.056786 0.055536 0.047694 0.033920 0.017698	-0. 623990 -0. 867646 -1. 057427 -1. 218206 -1. 301808	-0. 00163620 -0. 00145877 -0. 00118001 -0. 00081811 -0. 00043081	
6 7 8 9 10	-0.061624 -0.045808 -0.024060 -0.000670 0.024218	-1. 197286 -1. 294109	-0.00122649 -0.00086937 -0.00045933 -0.00045933 -0.00001143 0.00046302	39 40 41 42 43	0.000438 -0.018011 -0.034122 -0.047803 -0.055577	-1. 344993 -1. 300270 -1. 216570 -1. 055905 -0. 866376	-0. 00001293 0. 00043252 0. 00081795 0. 00117873 0. 00145677	
11 12 13 14	0.045874 0.061622 0.070802 0.069914 0.058208	-1. 195719 -1. 034050 -0. 831264 -0. 586997 -0. 332435	0.00086874 0.00122525 0.00150641 0.00166991 0.00178162	44 45 46 47	-0. 056783 -0. 047921 -0. 028240 0. 000000	-0. 623055 -0. 381329 -0. 133119 0. 000000	0.00163380 0.00168781 0.00134551 0.00093681	

10	0.024218	-1. 292635	0.00046302
11	0.045874	-1. 195719	0. 00086874
12	0.061622	-1. 034050	0. 00122525
13	0.070802	-0. 831264	0. 00150641
14	0.069914	-0. 586997	0. 00166991
15	0.058208	-0. 332435	0. 00178162
16	0. 033062	~0.083729	0.00113922
17	0. 000000	0.000000	0.00048926
31	0. 000000	0.000000	-0.00093827
32	0. 028262	-0.133322	-0.00134747
33	0. 047945	-0.381889	-0.00169035

【部材応力】

※※ 荷	重ケース	ح 1	XX DL+LL						
部材No	/─ 節点	No -/	/	M [kNm]	/	/ Q [kN]	/	/ N [kN]	/
	ⅰ端	j端	i 端	中央	j 端	i 端	j 端	i 端	j 端
1	1	2	0.0	-2.7	-5.5	-4. 3	4.3	202.2	-202.2
2	2	3	5.5	-2.3	0.9	4. 2	-4.2	131.1	-131.1
3	3	4	0.9	0.4	-0.1	-0. 6	0.6	61.0	-61.0
4	4	5	0.1	0.6	1.2	0. 9	-0.9	4.5	-4.5
5	5	6	1.2	1.0	0.8	-0. 3	0.3	-48.0	48.0
6	6	7	-0.8	1.3	1.8	0.7	-0.7	-82.4	82.4
7	7	8	-1.8	1.5	1.2	-0.4	0.4	-113.3	113.3
8	8	9	-1.2	1.7	2.2	0.7	-0.7	-126.0	126.0
9	9	10	-2.2	1.6	1.1	-0.7	0.7	-125.4	125.4
10	10	11	-1.1	1.5	1.8	0.4	-0.4	-112.8	112.8
11	11	12	-1.8	1.3	0.8	-0.7	0.7	-82.0	82. 0
12	12	13	-0.8	1.0	1.2	0.3	-0.3	-47.8	47. 8
13	13	14	-1.2	0.6	-0.1	-0.9	0.9	4.6	-4. 6
14	14	15	0.1	0.4	0.9	0.6	-0.6	61.0	-61. 0
15	15	16	-0.9	-2.3	-5.5	-4.2	4.2	131.0	-131. 0

*** Super Build/FA1 *** 250780 [N07T2]

** UNION SYSTEM ** 3.37 2009/06/12 20:16 PAGE- 4 「任意形平面フレーム応力解析]

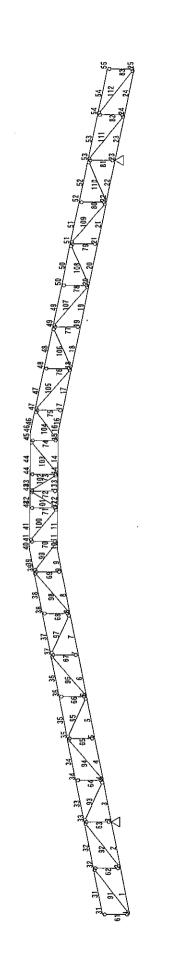
部材No	/- 節点 i端	īNo/ j端	/ i 端	M [kNm] 中央	/ j 端	/ Q [kN] i端	/ j 端	/ N [kN] i端	[任意飛 ────/ 」端	『平面フレーム応力解析]
16 31 32 33 34	16 31 32 33 34	17 32 33 34 35	5.5 0.0 3.4 -1.0 0.6	-2.7 -1.7 -1.2 0.2 0.6	0.0 -3.4 1.0 -0.6 1.9	4.3 -2.7 3.0 -1.1 1.6	-4.3 2.7 -3.0 1.1 -1.6	202.0 ~172.6 ~102.5	-202.0 172.6 102.5 46.0 -6.5	
35	35	36	-1.9	1.0	0. 1	-1.2	1.2	40.8	-40.8	
36	36	37	-0.1	1.3	2. 5	1.6	-1.6	71.7	-71.7	
37	37	38	-2.5	1.4	0. 3	~1.4	1.4	84.5	-84.5	
38	38	39	-0.3	1.6	2. 8	1.7	-1.7	93.0	-93.0	
39	39	40	-2.8	1.5	0. 3	-1.6	1.6	93.0	-93.0	
40	40	41	~0.3	1.4	2.5	1.4	-1.4	83. 9	-83.9	
41	41	42	-2.5	1.3	0.1	-1.6	1.6	71. 3	-71.3	
42	42	43	-0.1	1.0	1.9	1.2	-1.2	40. 5	-40.5	
43	43	44	-1.9	0.6	-0.6	-1.6	1.6	6. 3	-6.3	
44	44	45	0.6	0.2	1.0	1.1	-1.1	-46. 2	46.2	
45	45	46	-1.0	-1.2	-3.4	2.9	2.9	-102.5	102.5	
46	46	47	3.4	-1.7	0.0	2.7	-2.7	-172.5	172.5	
51	2	32	0.0	0.0	0.0	0.0	0.0	80.4	~80.4	
52	3	33	0.0	0.0	0.0	0.0	0.0	79.6	~79.6	
53	4	34	0.0	0.0	0.0	0.0	0.0	58.8	~58.8	
54	5	35	0.0	0.0	0.0	0.0	0.0	57.2	-57.2	
55	6	36	0.0	0.0	0.0	0.0	0.0	35.7	-35.7	
56	7	37	0.0	0.0	0.0	0.0	0.0	34.0	-34.0	
57	8	38	0.0	0.0	0.0	0.0	0.0	12.5	-12.5	
58	9	39	0.0	0.0	0.0	0.0	0.0	20.1	-20.1	
59	10	40	0.0	0.0	0. 0	0.0	0.0	12.4	-12.4	
60	11	41	0.0	0.0	0. 0	0.0	0.0	33.9	-33.9	
61	12	42	0.0	0.0	0. 0	0.0	0.0	35.5	-35.5	
62	13	43	0.0	0.0	0. 0	0.0	0.0	57.1	-57.1	
63	14	44	0.0	0.0	0. 0	0.0	0.0	58.6	-58.6	
64 65 81 82 83	15 16 31 32 33	45 46 2 3 4	0.0 0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0 0.0	79.5 80.3 -113.9 -102.5 -82.6	-79.5 -80.3 113.9 102.5 82.6	
84	34	5	0.0	0.0	0.0	0.0	0.0	-76.8	76.8	
85	35	6	0.0	0.0	0.0	0.0	0.0	-50.2	50.2	
86	36	7	0.0	0.0	0.0	0.0	0.0	-45.2	45.2	
87	37	8	0.0	0.0	0.0	0.0	0.0	-18.6	18.6	
88	38	9	0.0	0.0	0.0	0.0	0.0	-12.7	12.7	
89 90 91 92 93	9 10 11 12 13	40 41 42 43 44	0.0 0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0 0.0 0.0	0. 0 0. 0 0. 0 0. 0 0. 0	-13.1 -18.5 45.0 50.0 76.7	13. 1 18. 5 45. 0 50. 0 76. 7	
94	14	45	0. 0	0.0	0. 0	0. 0	0. 0	82.4	82.4	
95	15	46	0. 0	.0.0	0. 0	0. 0	0. 0	102.3	102.3	
96	16	47	0. 0	0.0	0. 0	0. 0	0. 0	113.7	113.7	

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*** Super Build/FA1 *** 250780 [N07MAINE] 架構図

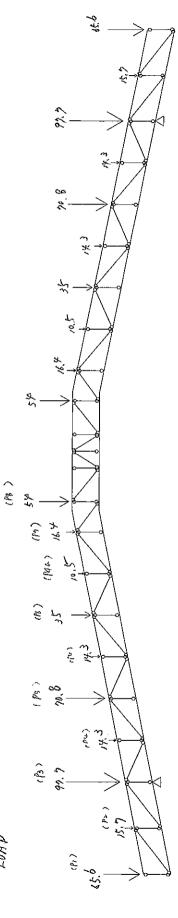
185

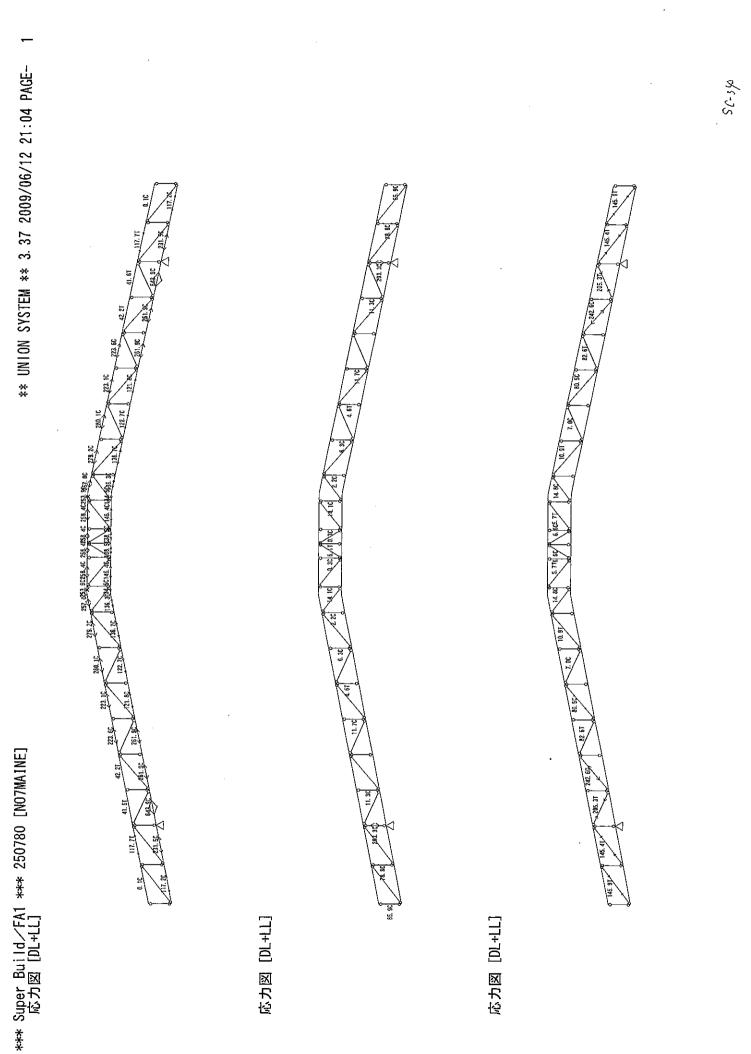
Franc



荷重図 [DL+LL]

d WO7





【基本事項・計算条件】

略日	事名: モルデ 称: N07M 付: 2009 当者: yec	ィプ メイントラン AINE /05/21	र								
・剛均・肥料	い或のになった。 いずのでは、 いずのでは、 してので、 たった、 たった、 たった、 たった、 たった、 たった、 たった、 たっ		すす有有有無し部ししししし無無 ?????????????????????????????	, D							
	〕単位			: S I 単位							
【節点座	標】 [m]										
No	X 座標	Y座標	No	X座標	Y座標	No	X 座標	Y座標	No	X座標	Y座標
1 2 3 4 5	0.000 2.730 .5.460 7.935 10.410	0.000 0.544 1.090 1.585 2.080	16 17 18 19 20	28. 613 29. 916 32. 391 34. 866 37. 341	4.320 4.060 3.565 3.070 2.575	36 37 38 39 40	12. 885 15. 360 17. 835 20. 310 21. 613	4, 175 4, 670 5, 165 5, 660 5, 920	51 52 53 54 55	39.816 42.291 44.766 47.496 50.226	3.680 3.185 2.690 2.144 1.600
6 7 8 9 10	12.885 15.360 17.835 20.310 21.613	2.575 3.070 3.565 4.060 4.320	21 22 23 24 25	39, 816 42, 291 44, 766 47, 496 50, 226	2.080 1.585 1.090 0.544 0.000	41 42 43 44 45	22. 113 24. 113 25. 113 26. 113 28. 113 28. 113	5.920 5.920 5.920 5.920 5.920 5.920			
11 12 13 14 15	22. 113 24. 113 25. 113 26. 113 28. 113	4. 320 4. 320 4. 320 4. 320 4. 320 4. 320	31 32 33 34 35	0. 000 2. 730 5. 460 7. 935 10. 410	1. 600 2. 144 2. 690 3. 185 3. 680	46 47 48 49 50	28. 613 29. 916 32. 391 34. 866 37. 341	5.920 5.660 5.165 4.670 4.175			
【支点デー	ータ】 [kN.	∕cm] [kNm∕r	ad] (0は自由、	1は拘束を表	します。)					
No	/ 節点No. <1> <2>	/ <3> ×方向/	i ∤ Y方	向バネ	回転バネ						
1 2	3 23	1 1	. 0 . 0	t. 0 1. 0	0. 0 0. 0						
【節点同- 	−変位】 入力値な	L									
【材質】	[kN∕mm2]										
No 1	E 205.00 79.	G 00									
【断面性俞	ŧ]										
No	A [cm2]	I [cm4]		断面名							
1 2 3 4 5	91. 43 63. 53 47. 42 34. 22 23. 84	1074 471 172 84 37	5 4.51 2 3.00 3 3.00	H−200*2 2 [−150* 2 [−125*	50*9*14*13 00*8*12*13 75*6. 5*10 65*6*8 50*5*7. 5						

6

25.38

129 3.00 2L-75*75*9

【結合状態】 [kNm/rad] (登録No.Oは剛接)

No 回転バネ係数

1 0.0

即材配	道」(断面NO.	か貝値	の材は、	伸縮しな	い材を着	受します。	,)
No	/節」 i 端	点No/ j端	断面No	材質No	/結合i ;端	No// j端	──剛域 [i 端	[cm]/ j 端
1 2 3 4 5	1 2 3 4 5	2 3 4 5 6	1 1 1 1	1 1 1 1	0 0 0 0	0 0 0 0 0	0.0 0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0 0.0
6 7 8 9 10	6 7 8 9 10	7 8 9 10 11	2 2 2 2 2	1 1 1 1	0 0 0 0	0 0 0 0	0. 0 0. 0 0. 0 0. 0 0. 0	0.0 0.0 0.0 0.0 0.0
11 12 13 14 15	11 12 13 14 15	12 13 14 15 16	2 2 2 2 2 2 2	1 1 1 1	0 0 0 0	0 0 0 0	0.0 0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0 0.0
76 17 18 19 20	16 17 18 19 20	17 18 19 20 21	2 2 2 2 2 1	1 1 1 1	0 0 0 0 0	0 0 0 0	0.0 0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0 0.0 0.0
21 22 23 24 31	21 22 23 24 31	22 23 24 25 32	1 1 1 2	1 1 1 1	0 0 0 0	0 0 0 0 0	0. 0 0. 0 0. 0 0. 0 0. 0	0.0 0.0 0.0 0.0 0.0
32 33 34 35 36	32 33 34 35 36	33 34 35 36 37	2 2 2 2 2	1 1 1 1	0 0 0 0	0 0 0 0	0. 0 0. 0 0. 0 0. 0 0. 0	0.0 0.0 0.0 0.0 0.0
37 38 39 40 41	37 38 39 40 41	38 39 40 41 42	2 2 2 2 2	1 1 1 1	0 0 0 0	0 0 0 0	0. 0 0. 0 0. 0 0. 0 0. 0	0.0 0.0 0.0 0.0 0.0
42 43 44 45 46	42 43 44 45 46	43 44 45 46 47	2 2 2 2 2	1 1 1	0 0 0 0 0	0 0 0 0	0.0 0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0 0.0
47 48 49 50 51	47 48 49 50 51	48 49 50 51 52	22222	1 1 1 1	0 0 0 0 0	0 0 0 0	0.0 0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0 0.0
52 53 54 61 62	52 53 54 1 2	53 54 55 31 32	2 2 3 4	1 1 1 1	0 0 1 1	0 0 1 1	0.0 0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0 0.0
63 64 65 66 67	3 4 5 6 7	33 34 35 36 37	35655	1 1 1 1	1 1 1 1	1 1 1 1 1	0.0 0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0 0.0

【部材配置】(断面No. が負値の材は、伸縮しない材を表します。)

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*** Super Build/FA1 *** 250780 [NO7MAINE]

***	Super	Build⁄	'FA1 *:	** 25078	0 [NO7	WAINE]			
	No	/節点 i端	iNo/ j端	断面No	材質No	/──結合No i端 j	//· 端	剛域 i 端	[cm]/ j 端
	68 69 70 71 72	8 9 11 12 13	38 39 41 42 43	55555	1 1 1 1	1 1 1 1	1 1 1 1	0.0 0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0 0.0
	73 74 75 76 77	14 15 17 18 19	44 45 47 48 49	5 5 5 5 5 5	1 1 1 1	1 1 1 1	1 1 1 1	0.0 0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0 0.0
	78 79 80 81 82	20 21 22 23 24	50 51 52 53 54	5 6 5 3 4	1 1 1 1 1	1 1 1 1	1 1 1 1 1	0.0 0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0 0.0
	83 91 92 93 94	25 1 2 33 4	55 32 33 4 35	3 3 4 4	1 1 1 1	1 1 1 1 1	1 1 1 7 1	0.0 0.0 0.0 0.0 0.0	0. 0 0. 0 0. 0 0. 0 0. 0
	95 96 97 98 99	35 6 37 8 39	6 37 8 39 11	4 4 5 5 5 5	1 1 1 1	1 1 1 1	1 1 1 1 1	0.0 0.0 0.0 0.0 0.0	0. 0 0. 0 0. 0 0. 0 0. 0
	100 101 102 103 104	41 12 43 14 15	12 43 14 45 47	5 5 5 5 5 5 5 5	1 1 1 1	1 1 1 1	1 1 1 1	0.0 0.0 0.0 0.0 0.0	0. 0 0. 0 0. 0 0. 0 0. 0
	105 106 107 108 109	47 18 49 20 51	18 49 20 51 22	5 5 4 4 4	1 1 1	1 1 1 1	1 1 1 1	0.0 0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0 0.0
	110 111 112	22 53 54	53 24 25	4 3 3	1 1 1	1 1 1	1 1 1	0. 0 0. 0 0. 0	0.0 0.0 0.0

【荷重ケース 1】DL+LL

No	/節点 〈1〉	i. 部材N 〈2〉	lo/ 〈3〉TYPE 方向	P 1	P 2	РЗ	P 4	P 5	P 6
1 2 3 4 5	31 32 33 34 35	55 54 53 52 51	0 0 0 0 0	0. 000kN 0. 000kN 0. 000kN 0. 000kN 0. 000kN	-65. 600kN -15. 700kN -97. 700kN -14. 300kN -70. 800kN	0. 000kNm 0. 000kNm 0. 000kNm 0. 000kNm 0. 000kNm 0. 000kNm			
6 7 8 9 10	36 37 38 39 41	50 49 48 47 45	0 0 0 0 0	0. 000kN 0. 000kN 0. 000kN 0. 000kN 0. 000kN 0. 000kN	-14. 300kN -35. 000kN -10. 500kN -16. 400kN -54. 000kN	0. 000kNm 0. 000kNm 0. 000kNm 0. 000kNm 0. 000kNm			

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【支点反力】

※※ 荷重	ケース 1	₩₩ DL+LL	
節点No	Rx [kN]	Ry [kN]	Rm [kNm]
23 23	400.4 -400.4	394.3 394.3	0. 0 0. 0
合計	0. 0	788. 6	0. 0

θ [rad]

.

,

δу [cm]

 -0. 520649
 0. 00095163

 -0. 250847
 0. 00106958

 -0. 048274
 -0. 00018560

 -0. 312190
 -0. 00150639

 -0. 687226
 -0. 00146050

 $\begin{array}{cccccc} -1. & 007882 & -0. & 00096381 \\ -1. & 205662 & -0. & 00078889 \\ -1. & 402128 & -0. & 00041095 \\ -1. & 415296 & 0. & 00016561 \\ -1. & 426596 & -0. & 00073952 \end{array}$

 $\begin{array}{cccc} -1. \ 490084 & -0. \ 00089556 \\ -1. \ 556115 & -0. \ 00001387 \\ -1. \ 552363 & 0. \ 00000000 \\ -1. \ 556115 & 0. \ 00001387 \\ -1. \ 490084 & 0. \ 00089556 \end{array}$

 $\begin{array}{ccccc} -1. \ 426596 & 0. \ 00073952 \\ -1. \ 415296 & -0. \ 00016561 \\ -1. \ 402128 & 0. \ 00041095 \\ -1. \ 205662 & 0. \ 00078889 \\ -1. \ 007882 & 0. \ 00096381 \end{array}$

 -0. 687226
 0. 00146050

 -0. 312190
 0. 00150639

 -0. 048274
 0. 00018560

 -0. 250847
 --0. 00106958

 -0. 520649
 --0. 00095163

【節点変位】

※※ 荷	重ケース 1	XX DL+LL			
節点No	δx [cm]	δy [cm]	θ [rad]	節点No	δx [cm]
1	0. 154574	-0.509803	0.00095865	31	0. 071204
2	0. 081641	-0.232885	0.00118792	32	0. 017428
3	0. 000000	0.000000	-0.00016793	33	0. 002578
4	-0. 026723	-0.308493	-0.00156620	34	0. 063591
5	0. 013054	-0.687217	-0.00140175	35	0. 146944
6	0.040451	-1.004040	-0. 00106771	36	0. 166890
7	0.057010	-1.207184	-0. 00080179	37	0. 162360
8	0.071337	-1.400066	-0. 00040184	38	0. 146301
9	0.047220	-1.414586	0. 00001618	39	0. 093745
10	0.038283	-1.440861	-0. 00059538	40	0. 069269
11	0.033118	-1. 485455	-0.00069212	41	0. 059521
12	0.010631	-1. 556004	-0.00007553	42	0. 019840
13	0.000000	-1. 554373	0.0000000	43	0. 000000
14	-0.010631	-1. 556004	0.00007553	44	~0. 019840
15	-0.033118	-1. 485455	0.00069212	45	~0. 059521
16	-0.038283	-1. 440861	0. 00059538	46	-0.069269
17	-0.047220	-1. 414586	-0. 00001618	47	-0.093745
18	-0.071337	-1. 400066	0. 00040184	48	-0.146301
19	-0.057010	-1. 207184	0. 00080179	49	-0.162360
20	-0.040451	-1. 004040	0. 00106771	50	-0.166890
21	-0.013054	-0. 687217	0.00140175	51	-0. 146944
22	0.026723	-0. 308493	0.00156620	52	-0. 063591
23	0.000000	0. 000000	0.00016793	53	-0. 002578
24	-0.081641	-0. 232885	-0.00118792	54	-0. 017428
25	-0.154574	-0. 509803	-0.00095865	55	-0. 071204

【部材応力】

※※ 荷重ケース 1 ※※ DL+LL

部材No	/- 節点	iNo/	/	M [kNm]	/	/ Q [kN]	/	/ N [kN]	/
	i端	j端	i 端	中央	j 端	〕端	」端	;端	j端
1	1	2	0.0	1.8	3.6	1.3	-1.3	117.2	-117.2
2	2	3	-3.6	-10.7	-25.1	-10.3	10.3	231.5	-231.5
3	3	4	25.1	-12.2	0.7	10.2	-10.2	643.9	-643.9
4	4	5	-0.7	1.4	2.2	0.6	-0.6	261.9	-261.9
5	5	6	-2.2	2.9	3.6	0.6	-0.6	261.9	-261.9
6	6	7	-3.6	1.0	1.6	-2. 1	2. 1	121.8	-121.8
7	7	8	1.6	1.5	4.7	2. 5	-2. 5	122.7	-122.7
8	8	9	-4.7	1.6	1.5	-2. 4	2. 4	136.7	-136.7
9	9	10	1.5	-4.4	7.4	-4. 5	4. 5	136.3	-136.3
10	10	11	7.4	-1.9	3.7	22. 3	-22. 3	134.5	-134.5
11	11	12	-3.7	3.0	2. 3	-0.7	0.7	146.4	-146.4
12	12	13	-2.3	0.7	-0. 8	-3.1	3.1	138.5	-138.5
13	13	14	0.8	0.7	2. 3	3.1	-3.1	138.5	-138.5
14	14	15	-2.3	3.0	3. 7	0.7	-0.7	146.4	-146.4
15	15	16	-3.7	-1.9	-7. 4	-22.3	22.3	134.5	-134.5
16	16	17	7.4	-4.4	1.5	4.5	-4.5	136. 3	-136.3
17	17	18	1.5	1.6	4.7	2.4	-2.4	136. 7	-136.7
18	18	19	-4.7	1.5	1.6	-2.5	2.5	122. 7	-122.7
19	19	20	1.6	1.0	3.6	2.1	-2.1	121. 8	-121.8
20	20	21	-3.6	2.9	2.2	-0.6	0.6	261. 9	-261.9
21	21	22	-2.2	1.4	0.7	-0.6	0.6	261.9	-261.9
22	22	23	-0.7	-12.2	-25.1	-10.2	10.2	643.9	-643.9
23	23	24	25.1	-10.7	3.6	10.3	-10.3	231.5	-231.5
24	24	25	-3.6	1.8	0.0	-1.3	1.3	117.2	-117.2
31	31	32	0.0	0.4	0.8	0.3	-0.3	0.1	-0.1
32	32	33	-0.8	-4.4	-9.5	-3.7	3.7	~117.7	117.7
33	33	34	9.5	-5.1	-0.6	3.5	-3.5	-41.6	41.6
34	34	35	0.6	0.2	0.9	0.6	-0.6	-42.2	42.2
35	35	36	-0.9	1.9	2.9	0.8	-0.8	223.6	-223.6
36	36	37	-2.9	0.7	-1.5	-1.7	1.7	223.1	-223.1

*** Super Build/FA1 *** 250780 [NO7MAINE]

** UNION SYSTEM ** 3.37 2009/06/12 21:05 PAGE- 5 「任意形平面フレーム応力解析]

部材No	/- 節点 ;端	īNo -/ j端	/ i 端	M [kNm] 中央	/ j端	/ Q [kN] i	/ j 端	/ N [kN i 端]/	形平面フレーム応力解析
37 38 39 40 41	37 38 39 40 41	38 39 40 41 42	1.5 -4.4 0.0 13.2 -7.1	1. 4 2. 2 -6. 6 -3. 0 4. 3	4. 4 0. 0 -13. 2 7. 1 1. 4	2. 4 -1. 8 -9. 9 40. 6 -2. 9	-2.4 1.8 9.9 -40.6 2.9	280. 1 279. 2 257. 0 253. 9 258. 4	j 端 -280.1 -279.2 -257.0 -253.9 -258.4	
42 43 44 45 46	42 43 44 45 46	43 44 45 46 47	-1.4 1.1 -1.4 -7.1 13.2	0. 1 0. 1 4. 3 -3. 0 -6. 6	-1. 1 1. 4 7. 1 -13. 2 0. 0	-2.5 2.5 2.9 -40.6 9.9	2.5 -2.5 -2.9 40.6 -9.9	258.4 258.4 258.4 253.9 257.0	-258.4 -258.4 -258.4 -253.9 -257.0	
47 48 49 50 51	47 48 49 50 51	48 49 50 51 52	0.0 -4.4 1.5 -2.9 -0.9	2.2 1.4 0.7 1.9 0.2	4.4 -1.5 2.9 0.9 -0.6	1.8 2.4 1.7 -0.8 -0.6	1.8 2.4 -1.7 0.8 0.6	279.2 280.1 223.1 223.6 ~42.2	-279 2 -280 1 -223 1 -223 6 42 2	
52 53 54 61 62	52 53 54 1 2	53 54 55 31 32	0.6 9.5 -0.8 0.0 0.0	5. 1 4. 4 0. 4 0. 0 0. 0	-9,5 0,8 0,0 0,0 0,0	-3.5 3.7 -0.3 0.0 0.0	3 5 -3 7 0 3 0 0 0 0	41.6 117.7 0.1 65.9 78.8	41.6 117.7 -0.1 -65.9 -78.8	
63 64 65 66 67	3 4 5 6 7	33 34 35 36 37	0.0 0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0 0.0	0. 0 0. 0 0. 0 0. 0 0. 0	0.0 0.0 0.0 0.0 0.0	293. 3 11. 3 0. 0 11. 7 -4. 6	-293.3 -11.3 0.0 -11.7 4.6	
68 69 70 71 72	8 9 11 12 13	38 39 41 42 43	0.0 0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0 0.0	0. 0 0. 0 0. 0 0. 0 0. 0	0.0 0.0 0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0 0.0	6.3 2.2 14.1 0.3 -6.1	-6.3 -2.2 -14.1 -0.3 6.1	
73 74 75 76 77	14 15 17 18 19	44 45 47 48 49	0.0 0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0 0.0 0.0	0. 0 0. 0 0. 0 0. 0 0. 0	0.3 14.1 2.2 6.3 -4.6	-0.3 -14.1 -2.2 -6.3 4.6	
78 79 80 81 82	20 21 22 23 24	50 51 52 53 54	0.0 0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0 0.0 0.0	0. 0 0. 0 0. 0 0. 0 0. 0	11.7 0.0 11.3 293.3 78.8	-11.7 0.0 -11.3 -293.3 -78.8	
83 91 92 93 94	25 1 2 33 4	55 32 33 4 35	0. 0 0. 0 0. 0 0. 0 0. 0	0.0 0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0 0.0	0. 0 0. 0 0. 0 0. 0 0. 0	65.9 -145.9 -145.4 -205.3 242.6	-65.9 145.9 145.4 205.3 ~242.6	
95 96 97 98 99	35 6 37 8 39	6 37 8 39 11	0.0 0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0 0.0	-82.6 80.5 7.0 -10.9 14.8	82.6 80.5 7.0 10.9 14.8	
100 101 102 103 104	41 12 43 14 15	12 43 14 45 47	0.0 0.0 0.0 0.0 0.0	0. 0 0. 0 0. 0 0. 0 0. 0	0.0 0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0 0.0	-5.7 6.6 6.6 -5.7 14.8	5.7 -6.6 -6.6 5.7 -14.8	
105 106 107 108 109	47 18 49 20 51	18 49 20 51 22	0. 0 0. 0 0. 0 0. 0 0. 0	0.0 0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0 0.0	-10.9 7.0 80.5 -82.6 242.6	10.9 7.0 80.5 82.6 242.6	
110 111 112	22 53 54	53 24 25	0. 0 0. 0 0. 0	0. 0 0. 0 0. 0	0. 0 0. 0 0. 0	0. 0 0. 0 0. 0	0. 0 0. 0 0. 0	-205.3 -145.4 -145.9	205. 3 145. 4 145. 9	

45 V Roof load. finish 600 7 Roof frome 1200 pv 500 2.300 1/1.2 .**r**~ 45 45,45 x 23 = 4658 EN 2 Floor load. 8.0 M/2 × 2 = 16 Kal/2 5 m x 5 m Fo Foundati. To tal axial force = 4658/4/25 + 16 = 626 1012 < 100 km/ me 2.

50-40

No. 22. President Office.

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193

125

• PV panel and support frame : Weight is 0.5kN/m²

- Purlin and Rafter are confirmed with the existent member.
- There is no problem in the safety because additional weight is 0.5kN/m^2 with the main structure member.

The examination of the existent member

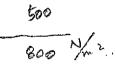
Roof ry Roof ry 10 Rafteg

1、既存招打 a挨計

Pregident office.

Weight-Reofing + bord. 150 W/2 Pullin 50 Raftes 100 - 300

PJ pavel + Gupport



for purla $\frac{1}{910} \qquad w = 0.7 \frac{1}{9} \frac{1}{10} \times 0.83 = 0.6 \frac{1}{8} \frac{1}{9.0} \times 0.83 = 0.6 \frac{1}{8} \frac{1}{10} = \frac{1}{10} \times 0.6 \times 0.91 = 0.062 \frac{1}{8} \frac{1}{10} = \frac{1}{10} \times 0.6 \times 0.91 = 0.28 \frac{1}{8} \frac{1}{10} = \frac{1}{10} \times 0.6 \times 0.91 = 0.28 \frac{1}{8} \frac{1}{10} = \frac{1}{10} \times 0.630 \frac{1}{10} = \frac{5000}{10} \frac{1}{2} \times 0.630 \frac{1}{10} = \frac{5000}{10} \frac{1}{2} \times 0.630 \frac{1}{10} = \frac{1}{10} \times 0.630 \frac{1}{10} = \frac{$ -3. 屋根鉄骨部材の算定

195-

308 TYPE

母屋 R aftes	Timber		300	Type
部材 □ -	150 x 50	Span.	Pite.	h
スパン L(m), ピッチ(m) しゅん 荷重 Wg (KN/m ²)		2.80 2.00 0.80	0.91	
Bending Monat 曲げモーメント xMg (KN·m) の yMg (KN·m)	$\omega \cdot \text{SIN} \theta \cdot \text{L}^2/8 = \\ \omega \cdot \text{COS} \theta \cdot \text{L}^2/8 =$	0.18 0.62		
yMg (KN·m) Shear force せん断力 yQg (KN)	$\omega \cdot \cos \theta \cdot L/2 =$	0.90		
断面性能 Iy (cm ⁴) Ix cm ⁴) Zy cm ³) Zx cm ³)	1400 155 185 62			
許容曲げ応力度 fb (KN/cm ²)	0.77			
曲げ応力度 x σ b = xM/Zx (KN/cm ²) 曲げ応力度 y σ b = yM/Zy (KN/cm ²) 応力度比 (x σ b+y σ b)/fb ≦ 1	0.3 0.34 0.83		1	0 K
撓み δg (cm) δ/L ≧ 300	0.67 418	\geq	300	0 K

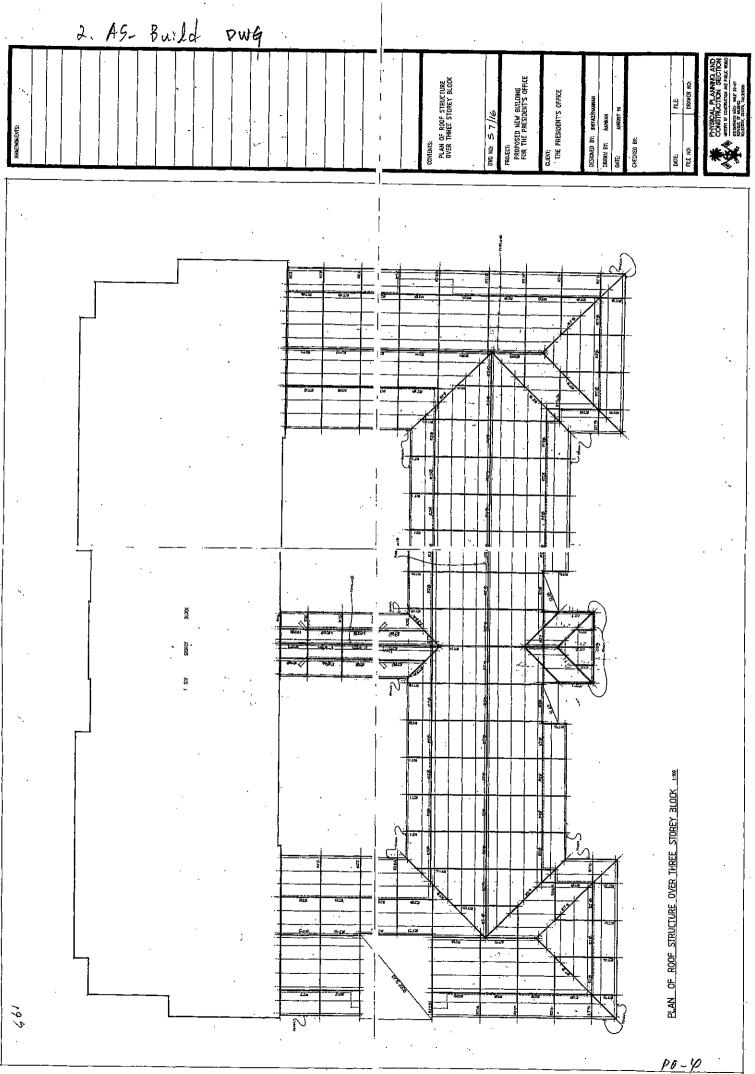
母屋

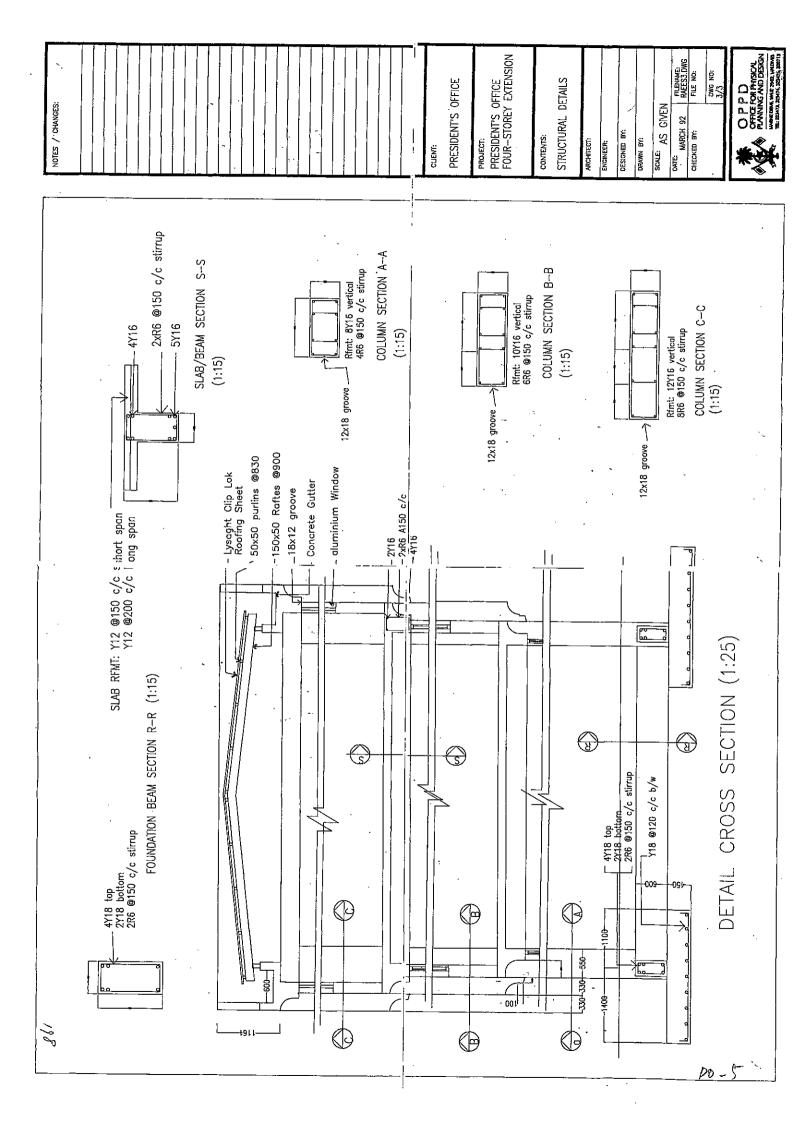
15° Type

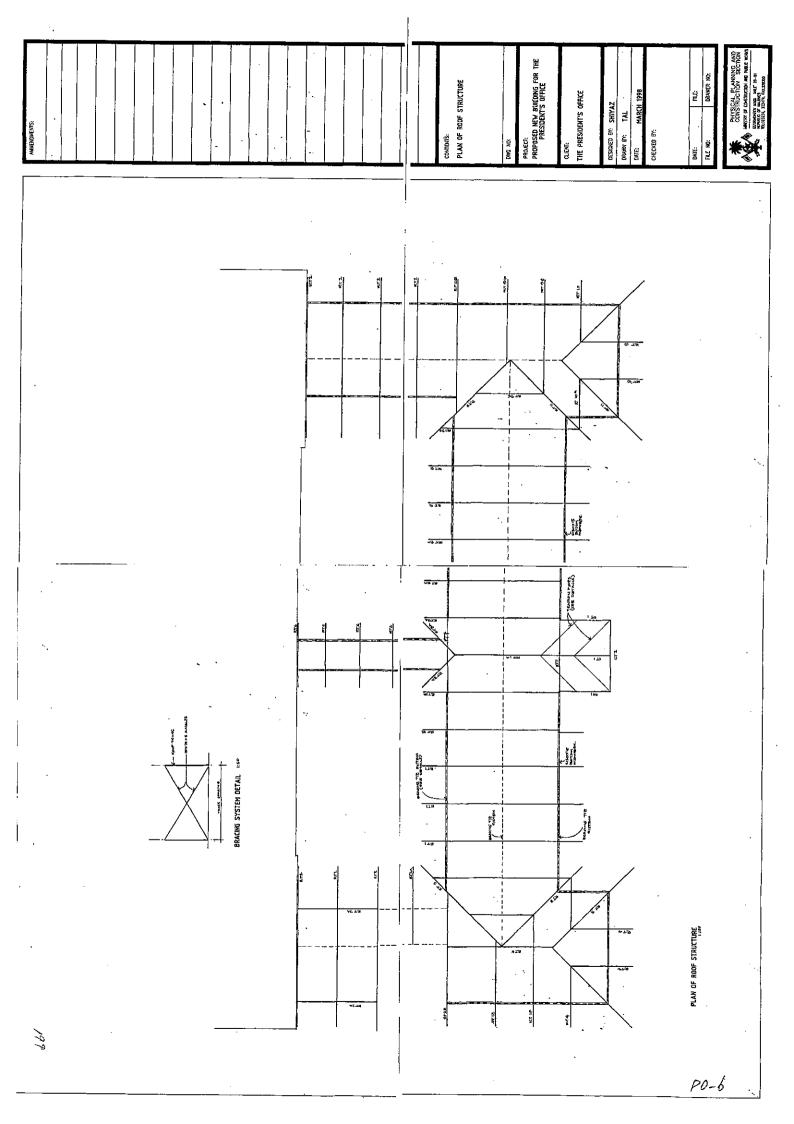
部材 🗆 - 150 x 50

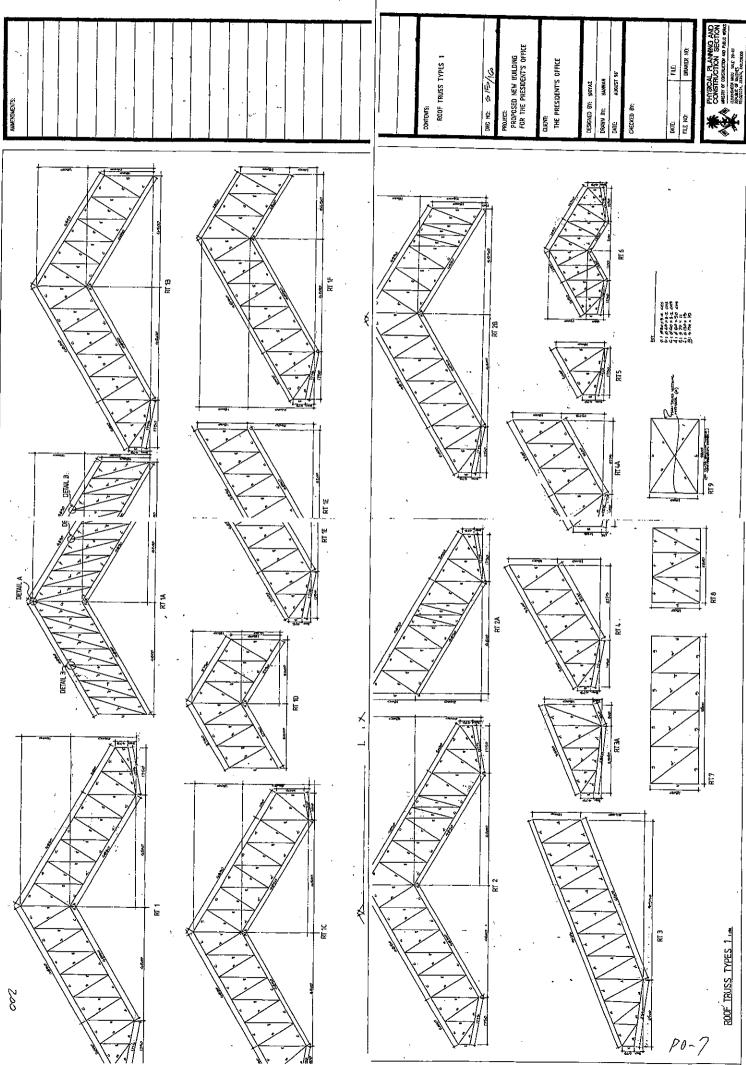
スパン L(m), ピッチ(m) 荷重 Wg (KN/m ²)		2.80 2.00 0.80	0.91	
曲げモーメント xMg (KN・m) yMg (KN・m)		0.09 0.69		
せん断力 yQg (KN)	$\omega \cdot \cos \theta \cdot L/2 =$	1.00		
断面性能 Iy (cm ⁴) Ix (cm ⁴) Zy (cm ³) Zx (cm ³)	$1400 \\ 155 \\ 185 \\ 62$			
許容曲げ応力度 fb (KN/cm ²)	0.77			
曲げ応力度 x σ b = xM/Zx (KN/cm ²) 曲げ応力度 y σ b = yM/Zy (KN/cm ²) 応力度比 (x σ b+y σ b)/fb ≦ 1	0.15 0.38 0.69		1	0 K
携み δg (cm) δ/L ≧ 300	$\begin{array}{c} 0.85\\ 329 \end{array}$	\geq	300	0 K

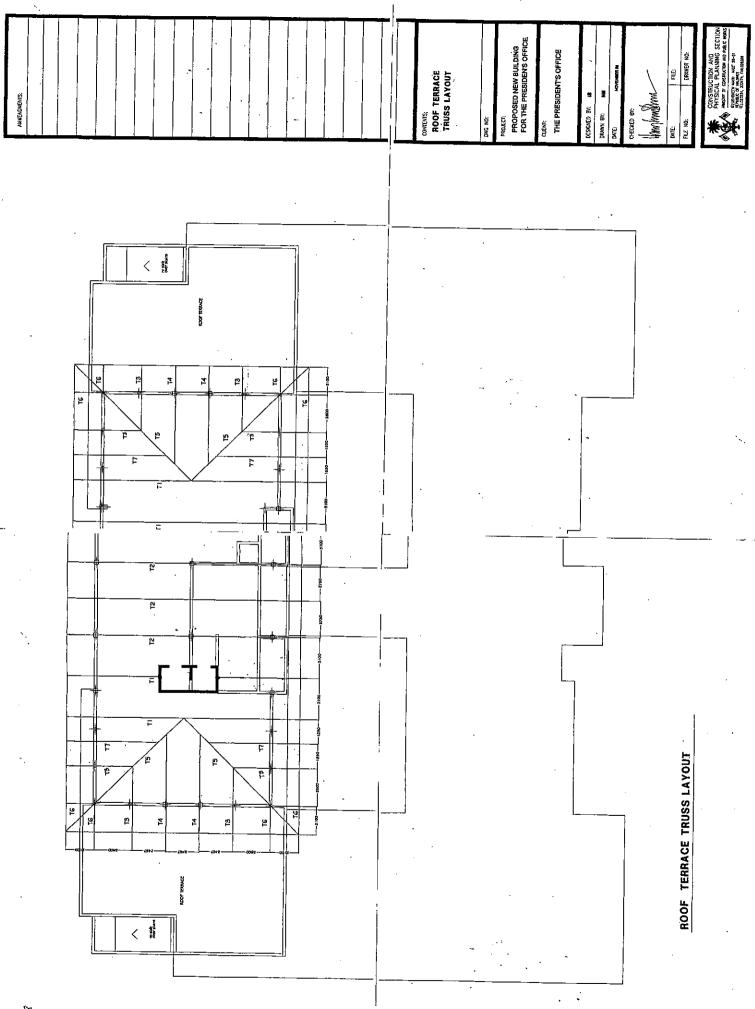
7 Po-3











po-8.

