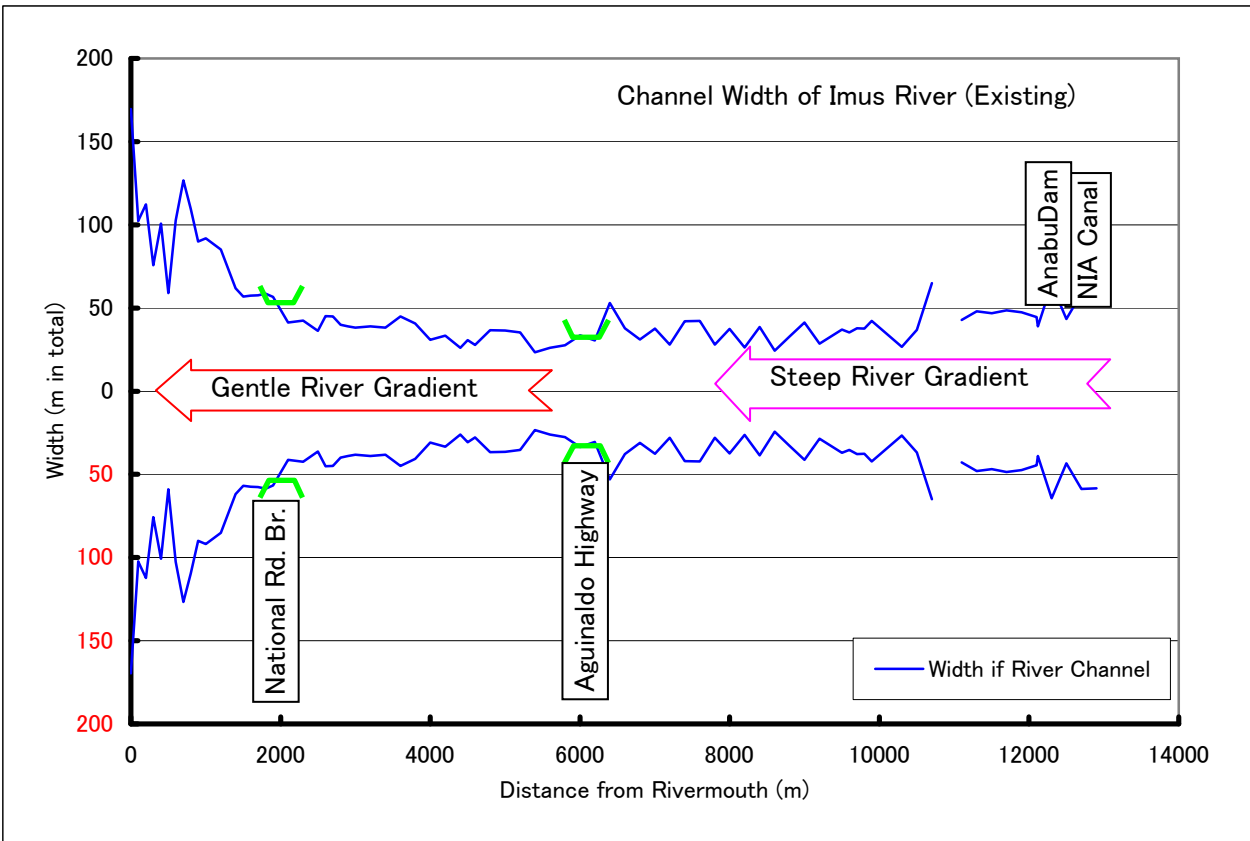
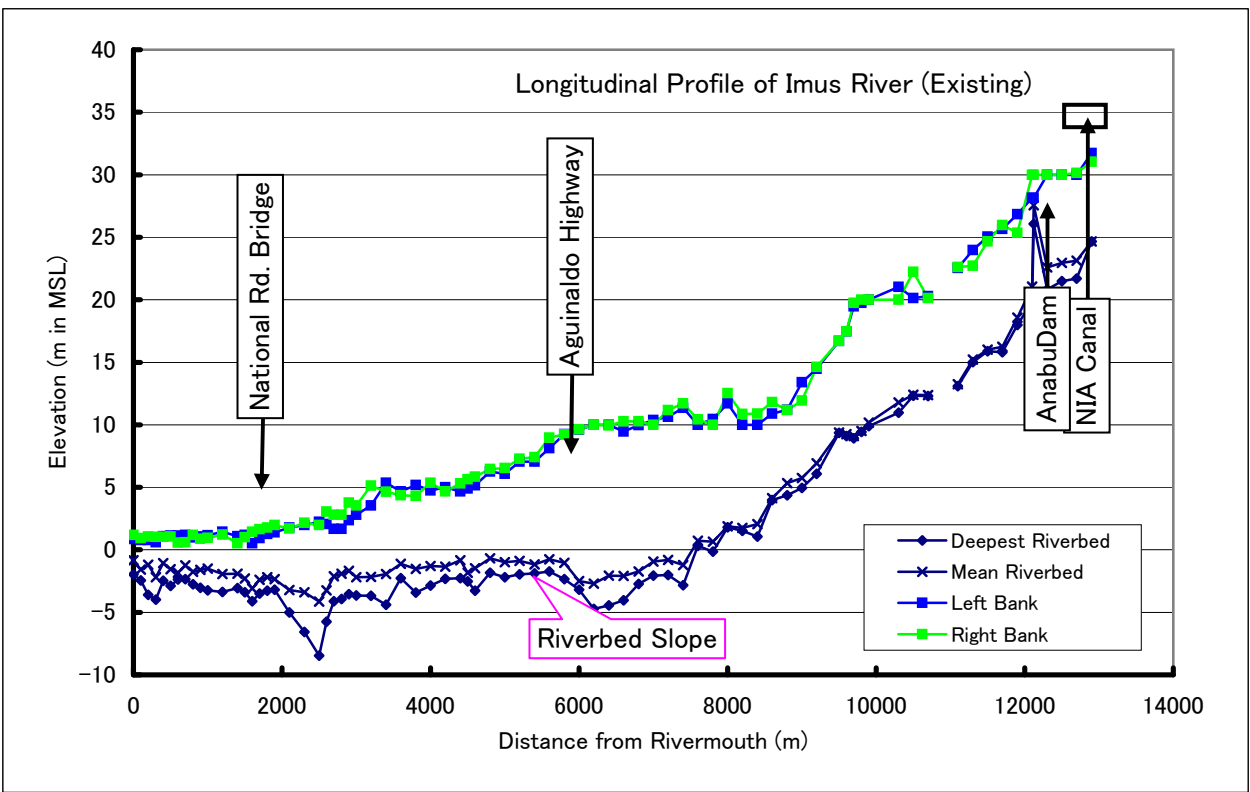


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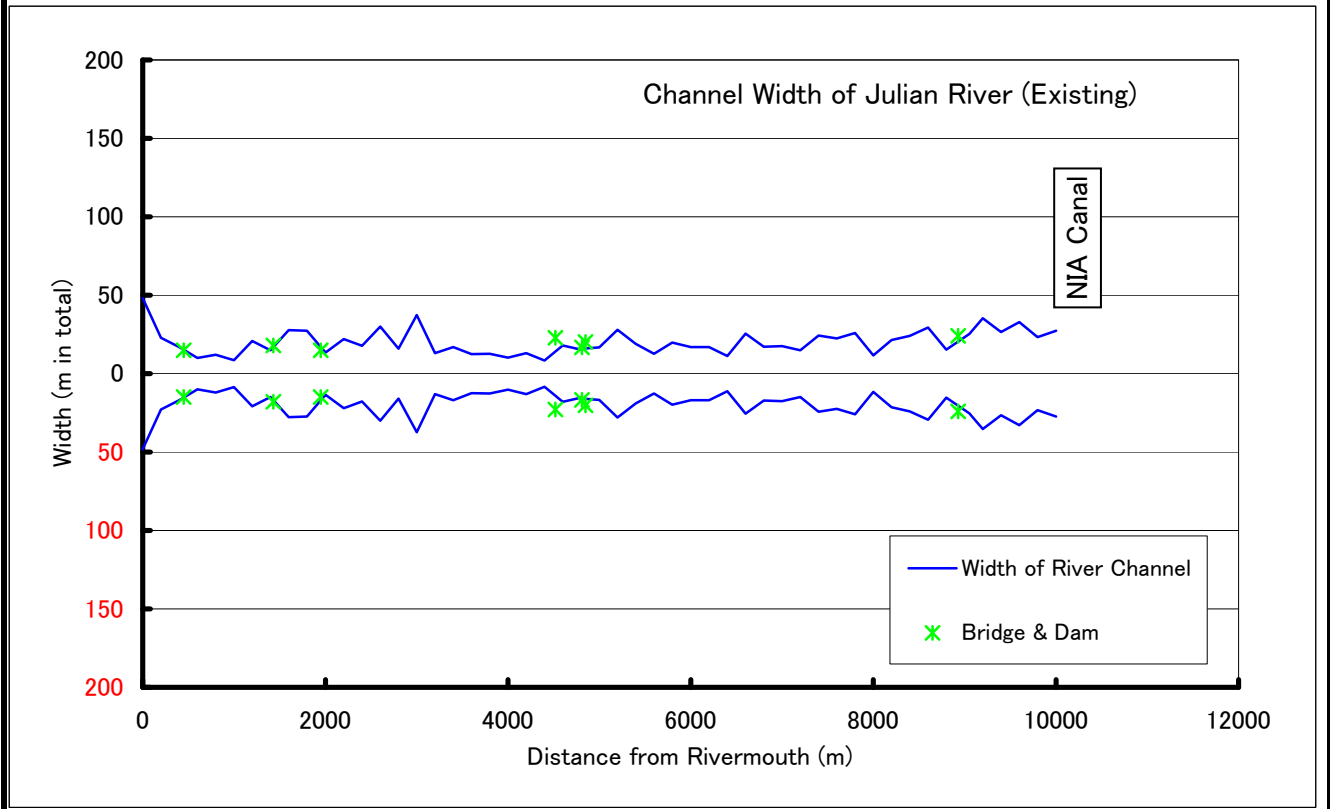
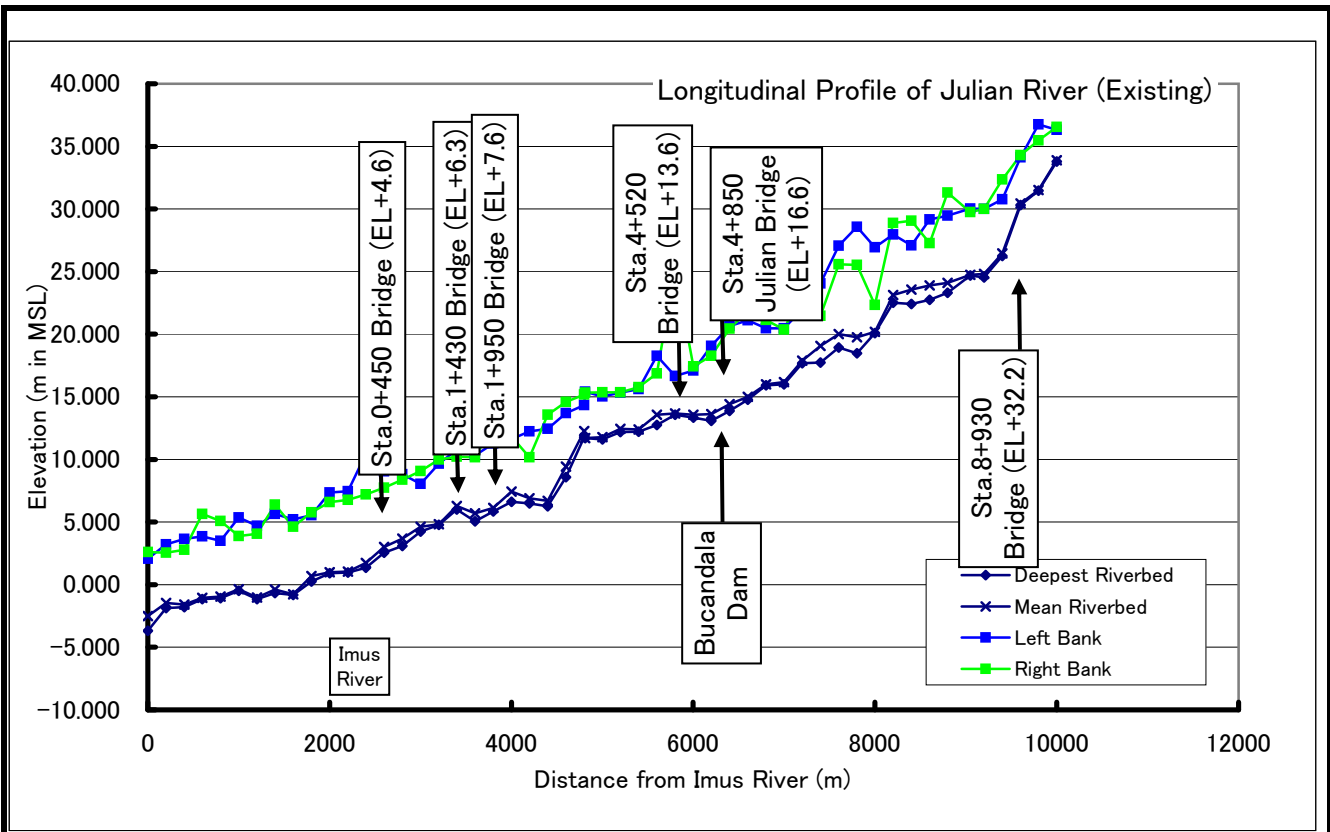
Fig. 6.1

Location of Transfer Station and Sanitary Landfill
Sites for New Solid Waste Management System in
Cavite Province



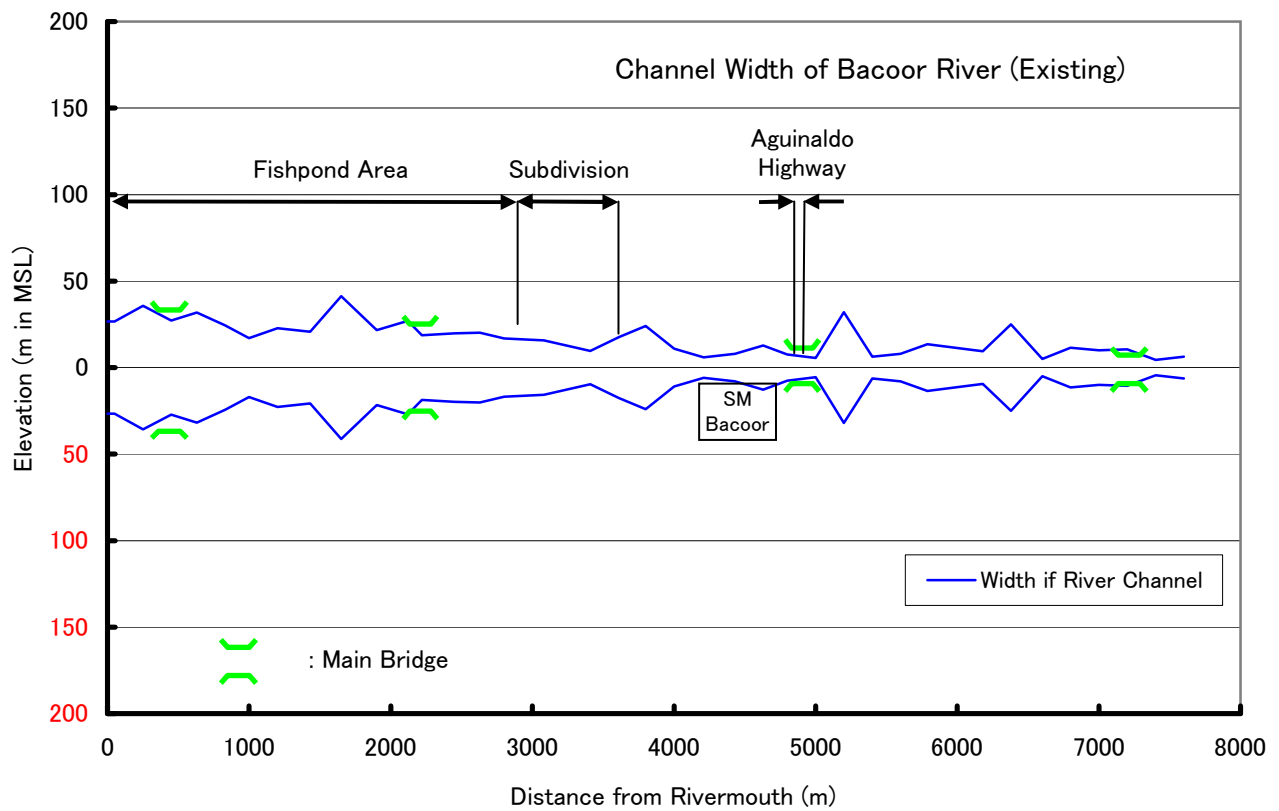
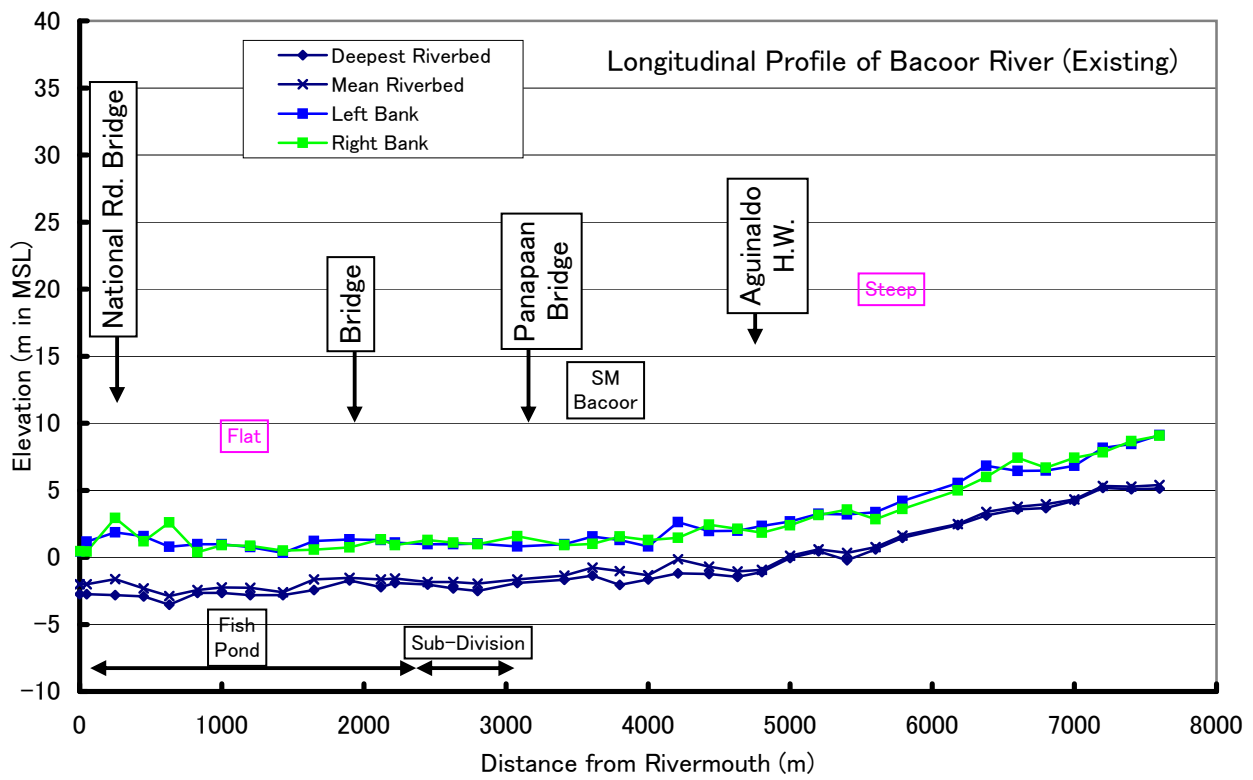
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Fig. 8.1
 Longitudinal Profile and Channel Width
 of Imus River (Present Condition)



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Fig. 8.2
 Longitudinal Profile and Channel Width
 of Julian River (Present Condition)

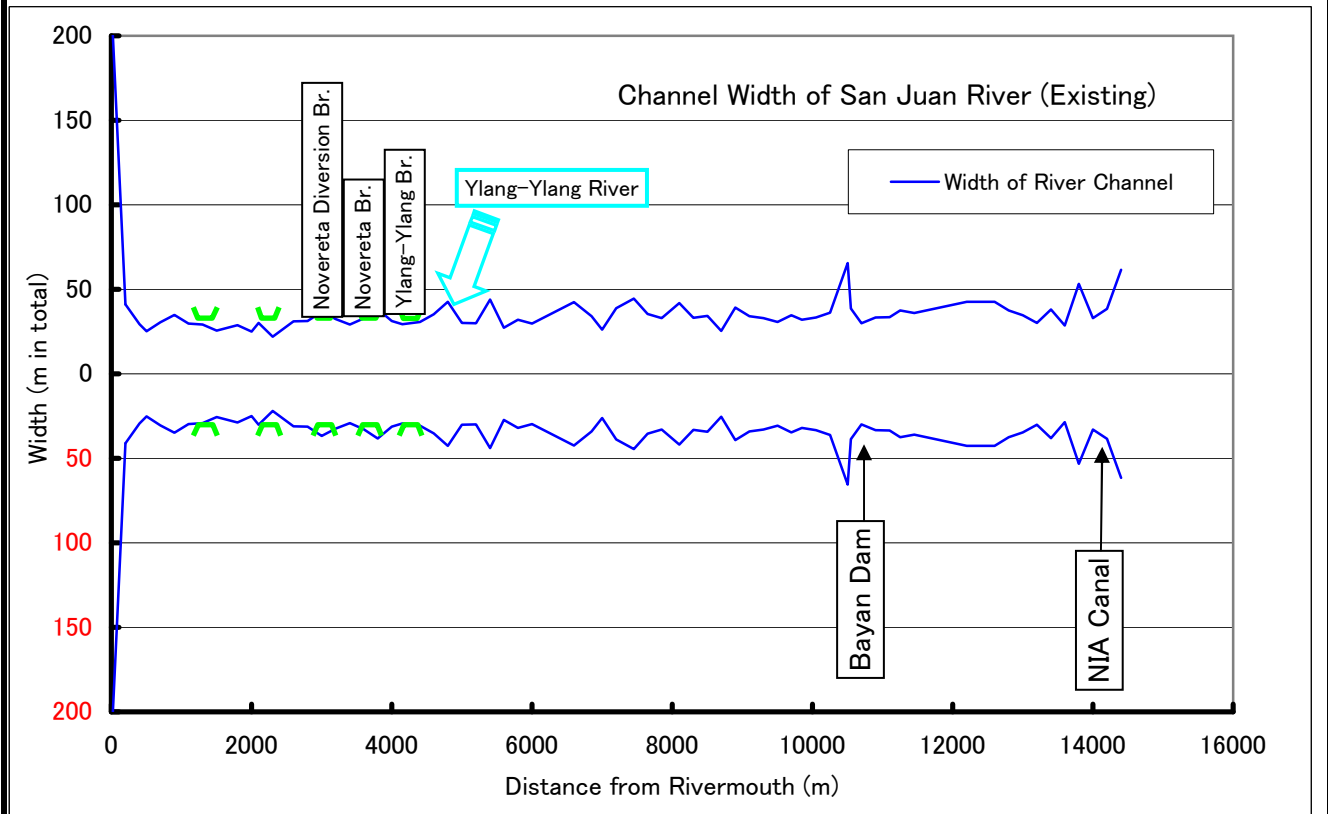
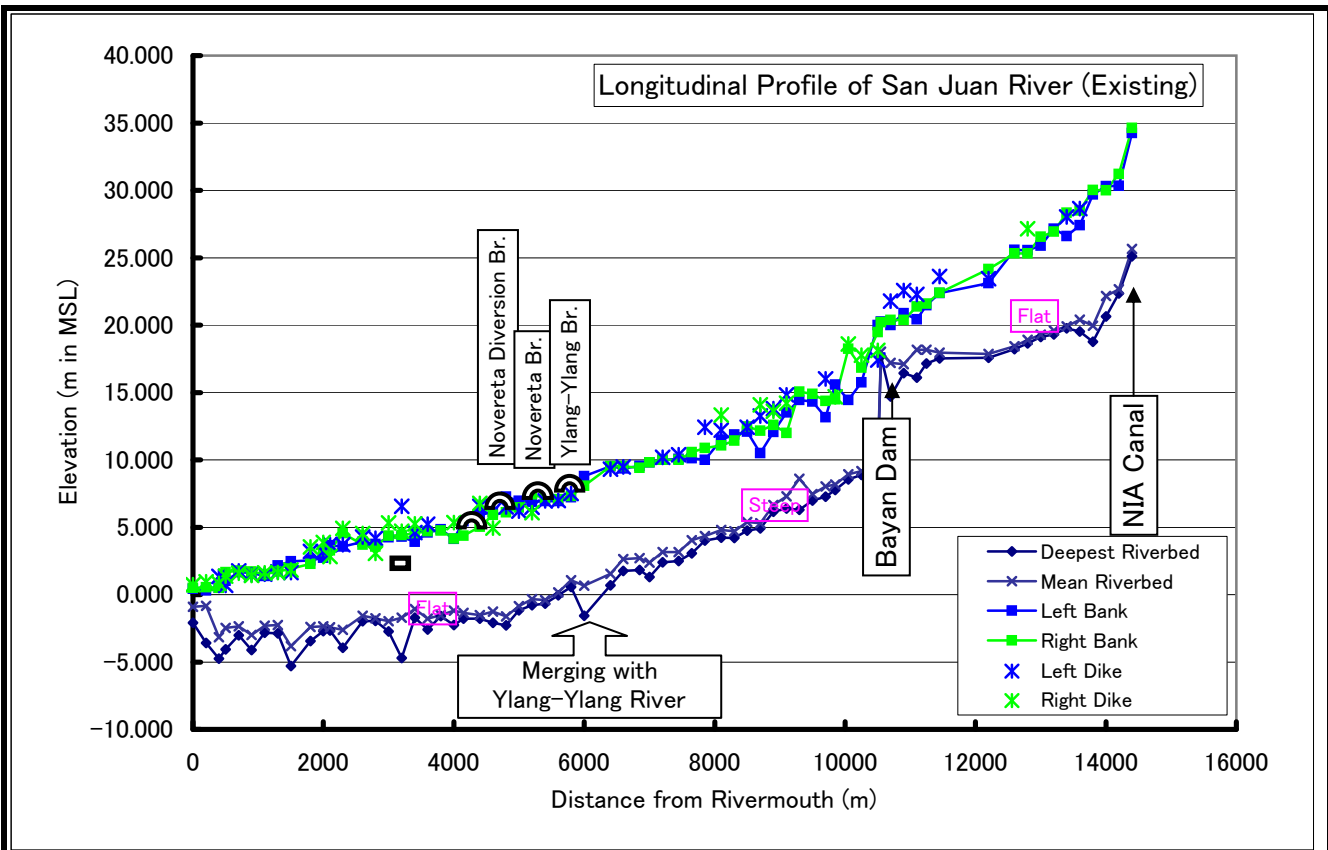


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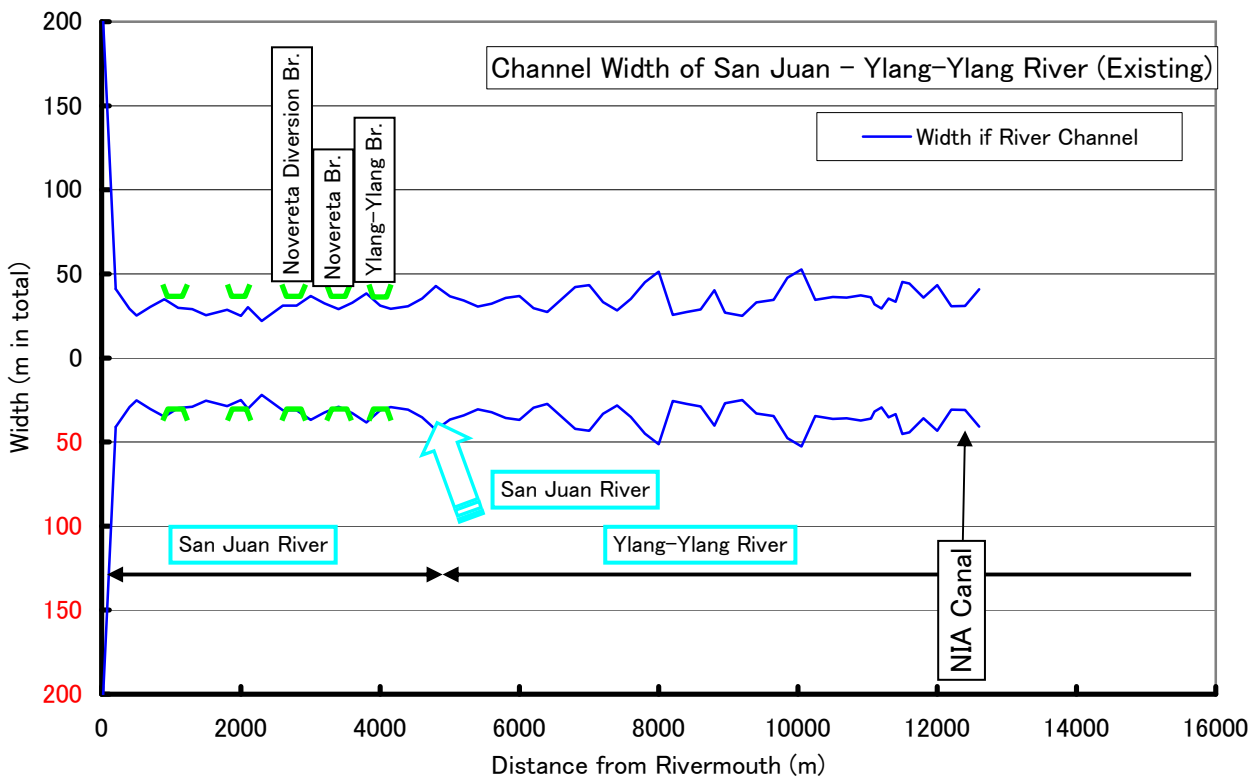
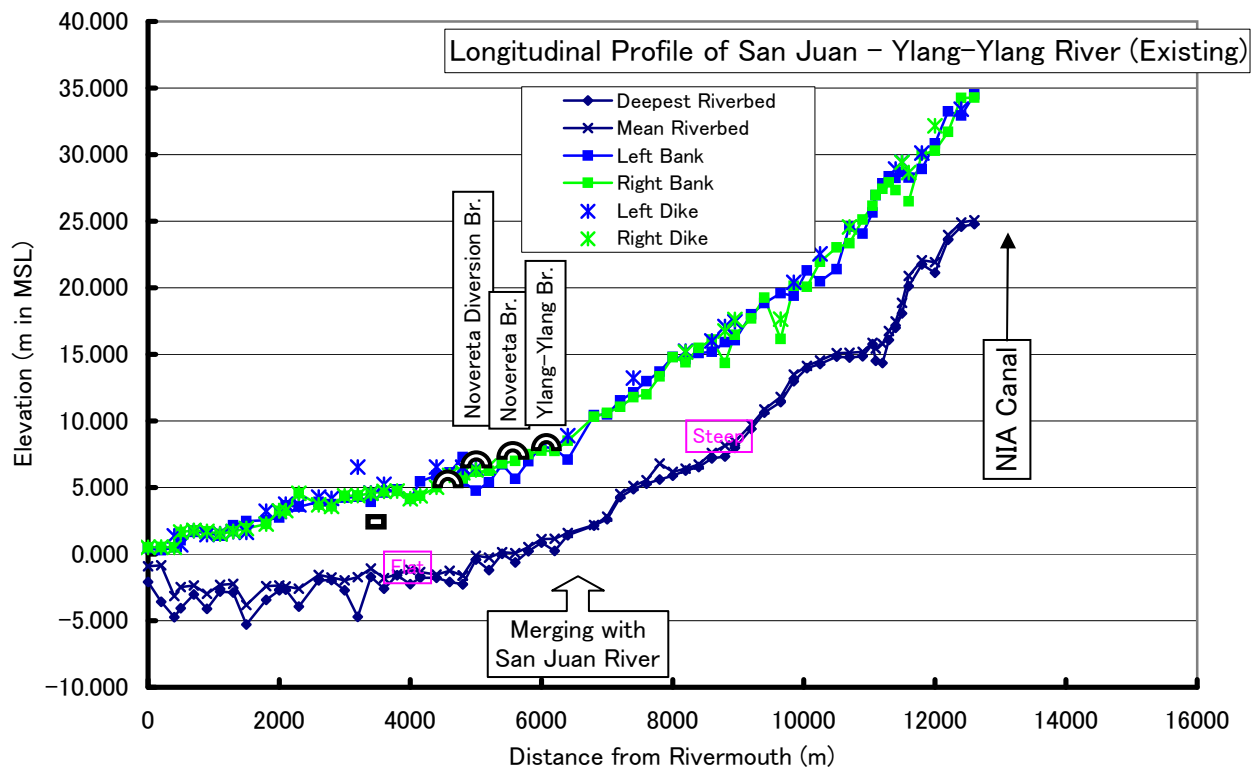
Fig. 8.3

Longitudinal Profile and Channel Width
 of Bacoor River (Present Condition)



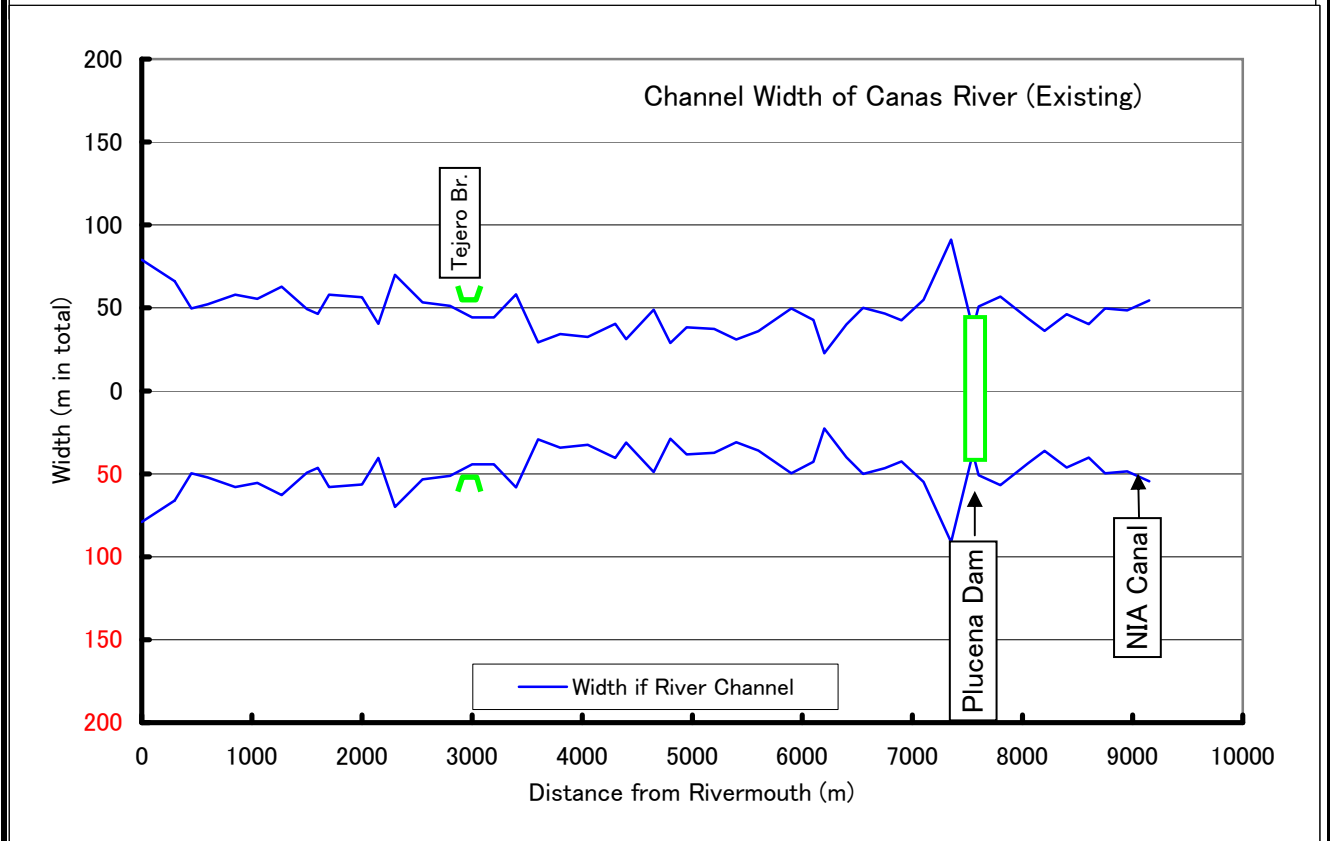
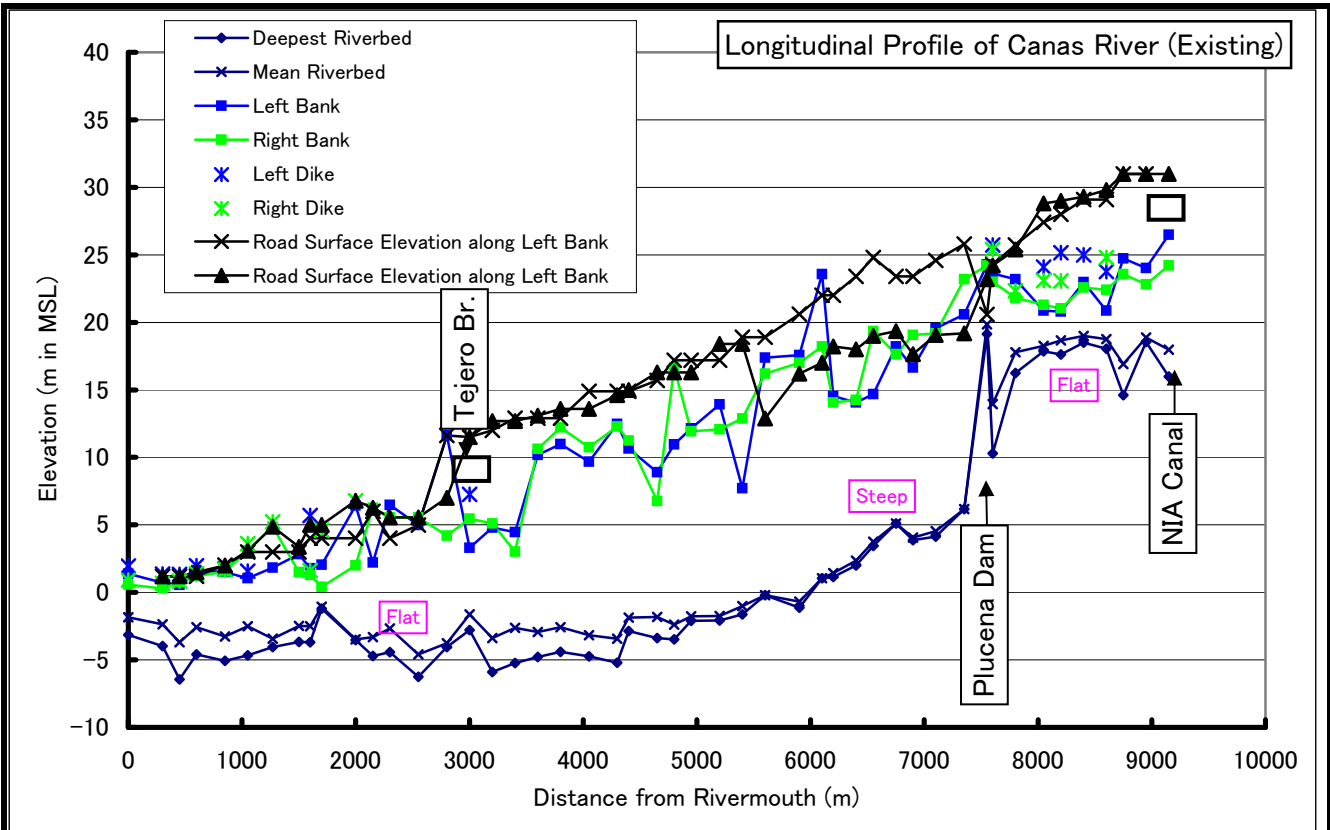
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Fig. 8.4
 Longitudinal Profile and Channel Width
 of San Juan River (Present Condition)



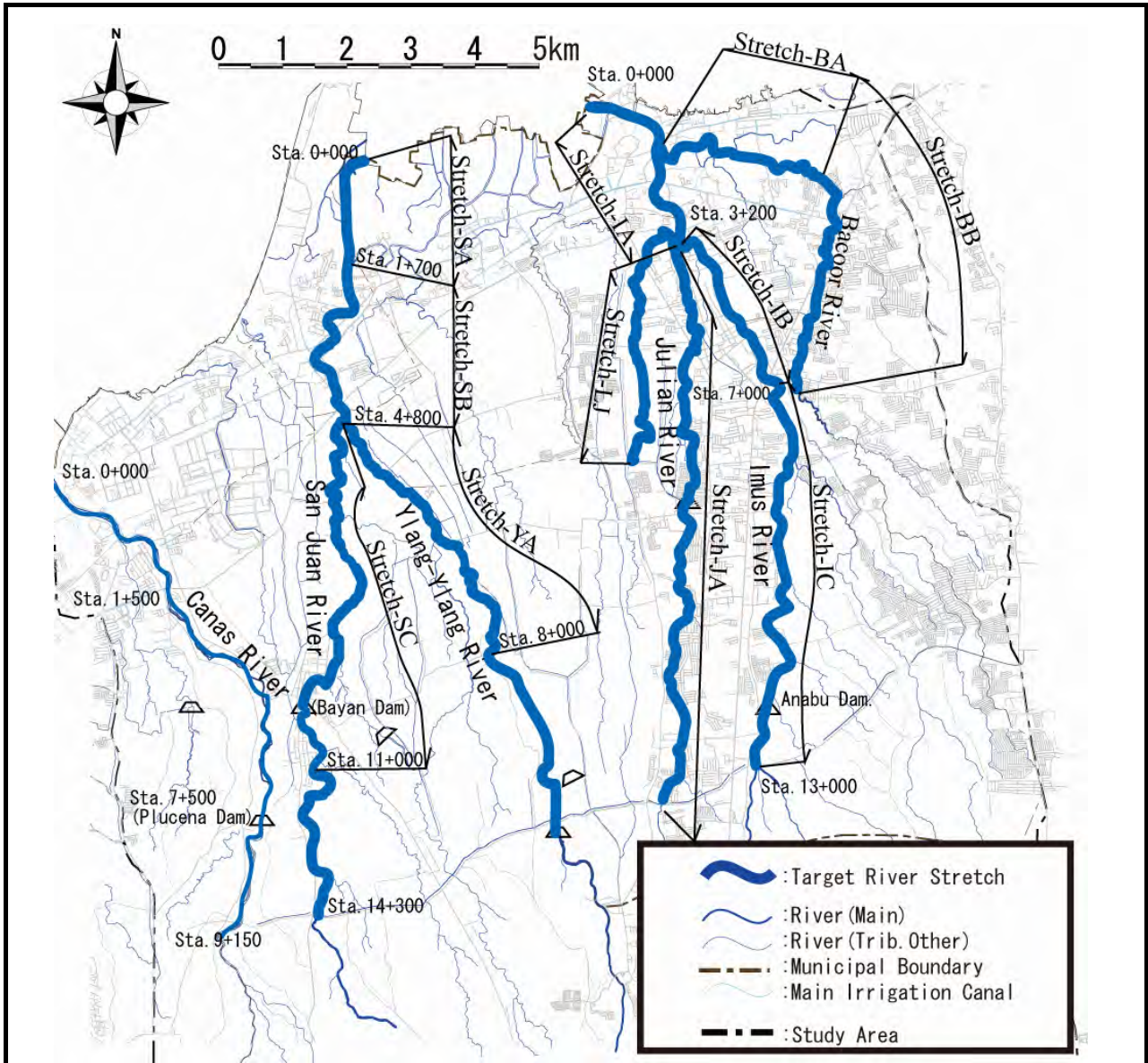
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Fig. 8.5
 Longitudinal Profile and Channel Width
 of Ylang-Ylang River (Present Condition)



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Fig. 8.6
 Longitudinal Profile and Channel Width
 of Canas River (Present Condition)



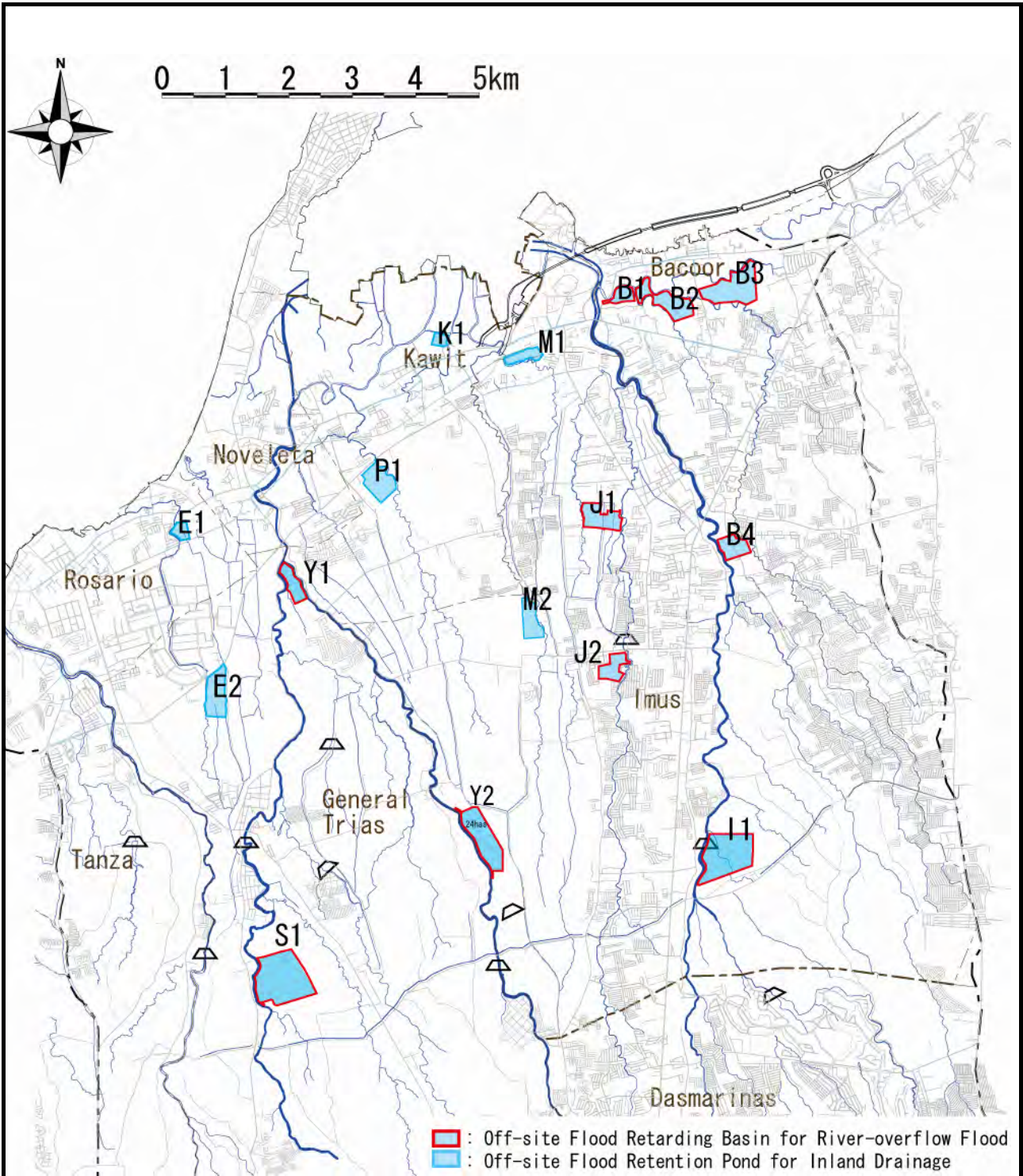
River Basin	River	Code of Stretch*	Extent	Distance (km)
Imus	Imus	IA	From river mouth to Coastal National Road Bridge (Sta. 2+000)	2.0km
	Imus	IB	From Coastal National Road Bridge (Sta. 2+000) to confluence point with Julian River (Sta. 3+400)	1.4km
	Imus	IC	From confluence point with Julian River (Sta. 3+400) to NIA Cala Canal (Sta. 13+000)	9.6km
	Bacoor	BA	From confluence with Imus River to upstream end of fishpond (Sta. 3+000)	3.0km
	Bacoor	BB	From upstream end of fishpond (Sta. 3+000) to Sta. 7+000	4.0km
	Julian	JA	Whole river stretch	10.0km
	Left Tributary	LJ	Whole river stretch	4.5km
San Juan	San Juan	SA	From river mouth to Sta. 1+700 upstream	1.7km
	San Juan	SB	From Sta. 1+700 to merging point of San Juan and Ylang-Ylang (Sta. 4+800)	3.1km
	San Juan	SC	From Sta. 4+800 to upstream of Bayan Dam (Sta. 11+000)	6.2km
	Ylang-Ylang	YA	From Sta. 4+800 to Sta. 8+000	3.2km

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Fig. 8.7

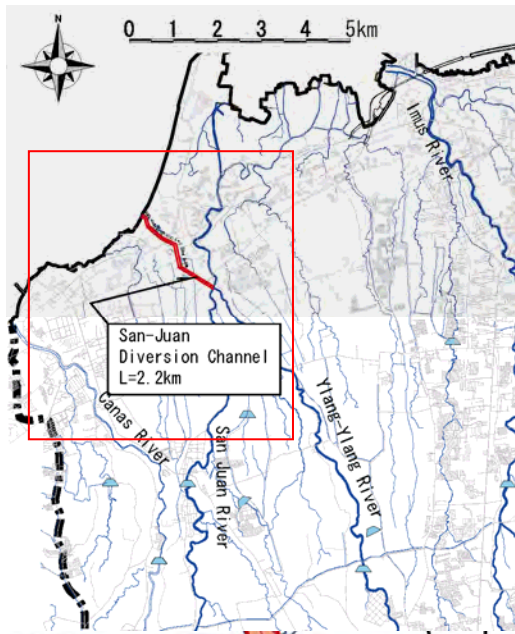
Targeted River Stretch for River Channel
Improvement for 20-year Return Period Flood



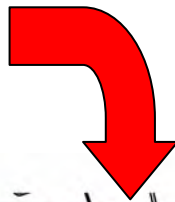
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Fig. 8.8
 Proposed Sites for Off-site Flood Retarding Basins and On-site Retention Ponds



	Design Discharge (m ³ /s)	
	w/ on-site	w/o on-site
2-year	0	60
5-year	200	270
10-year	430	480
20-year	670	700

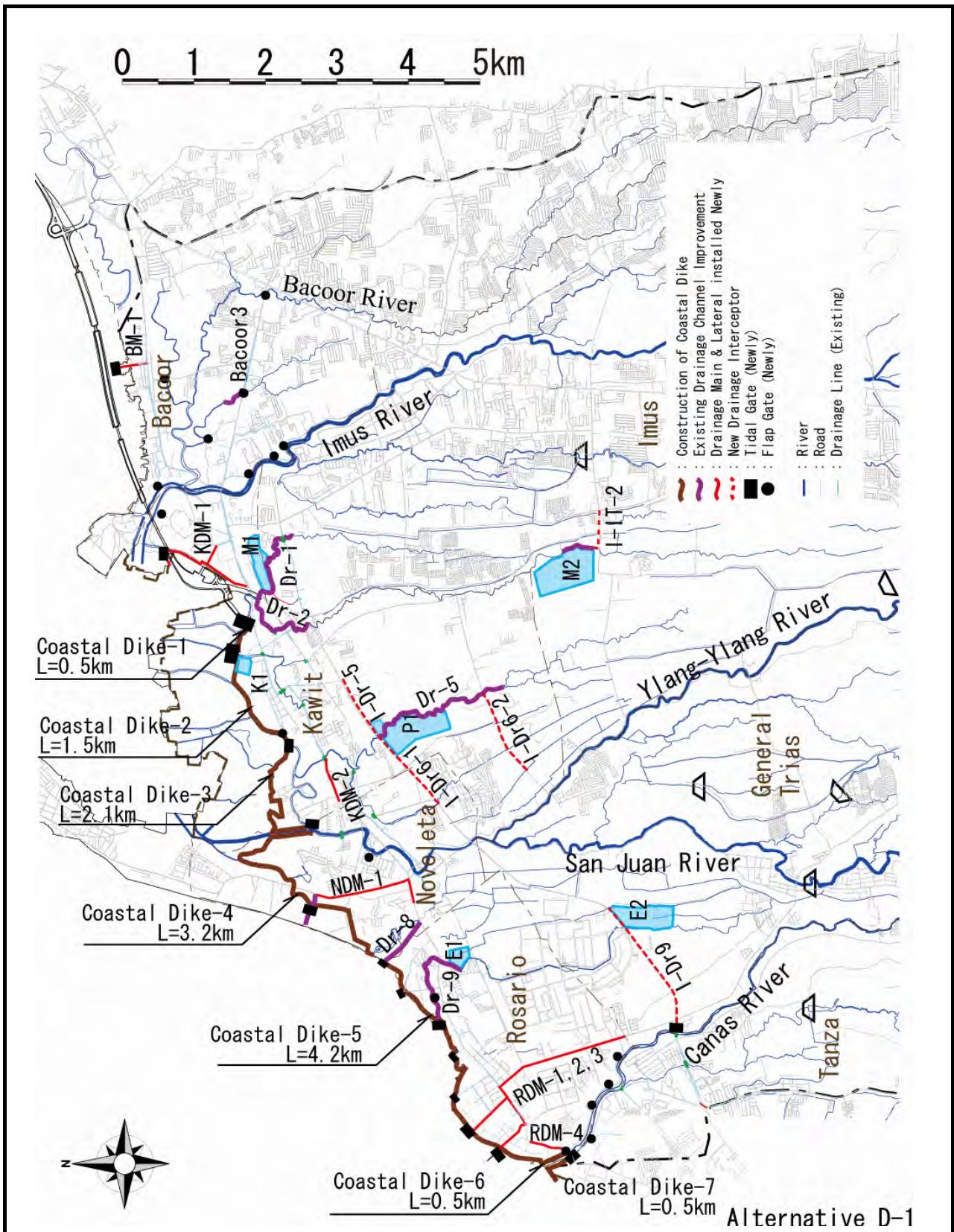


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Fig. 8.9

Proposed Alignment of San Juan Diversion Channel

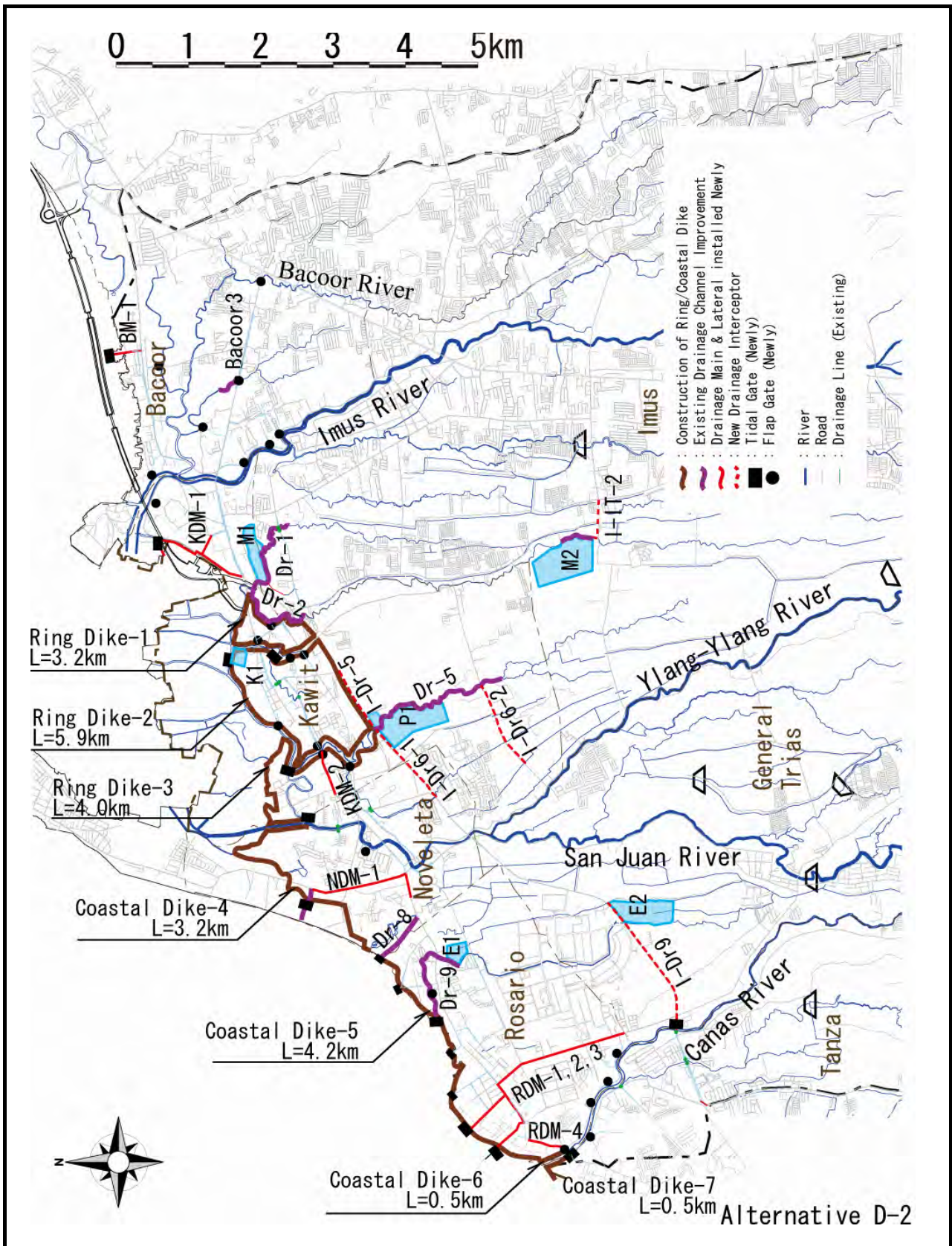


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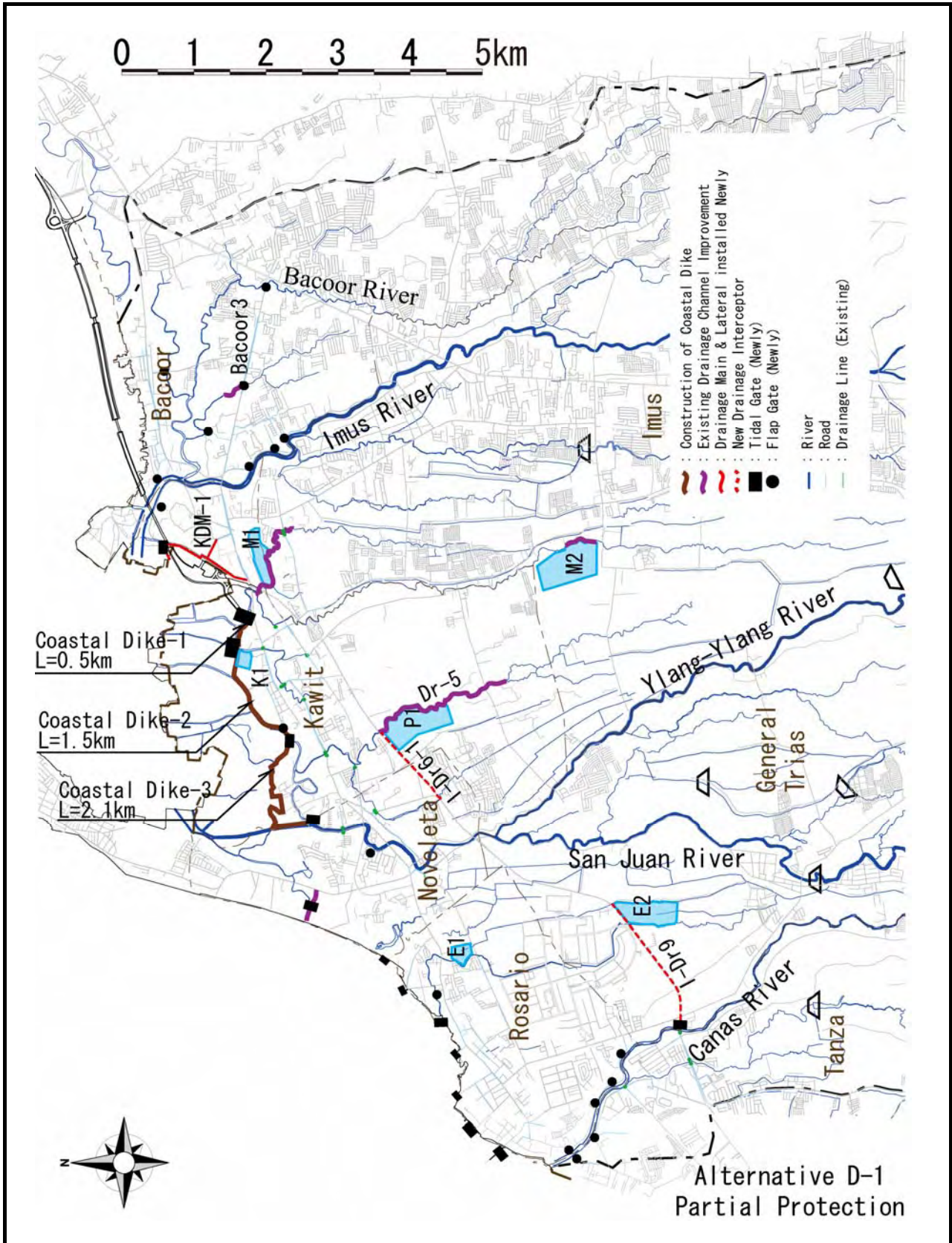
Fig. 8.10

Conceived Structural Measures against Inland
Flood based on Coastal Dike with Tidal Gates
(Alternative D-1 : Full Protection)



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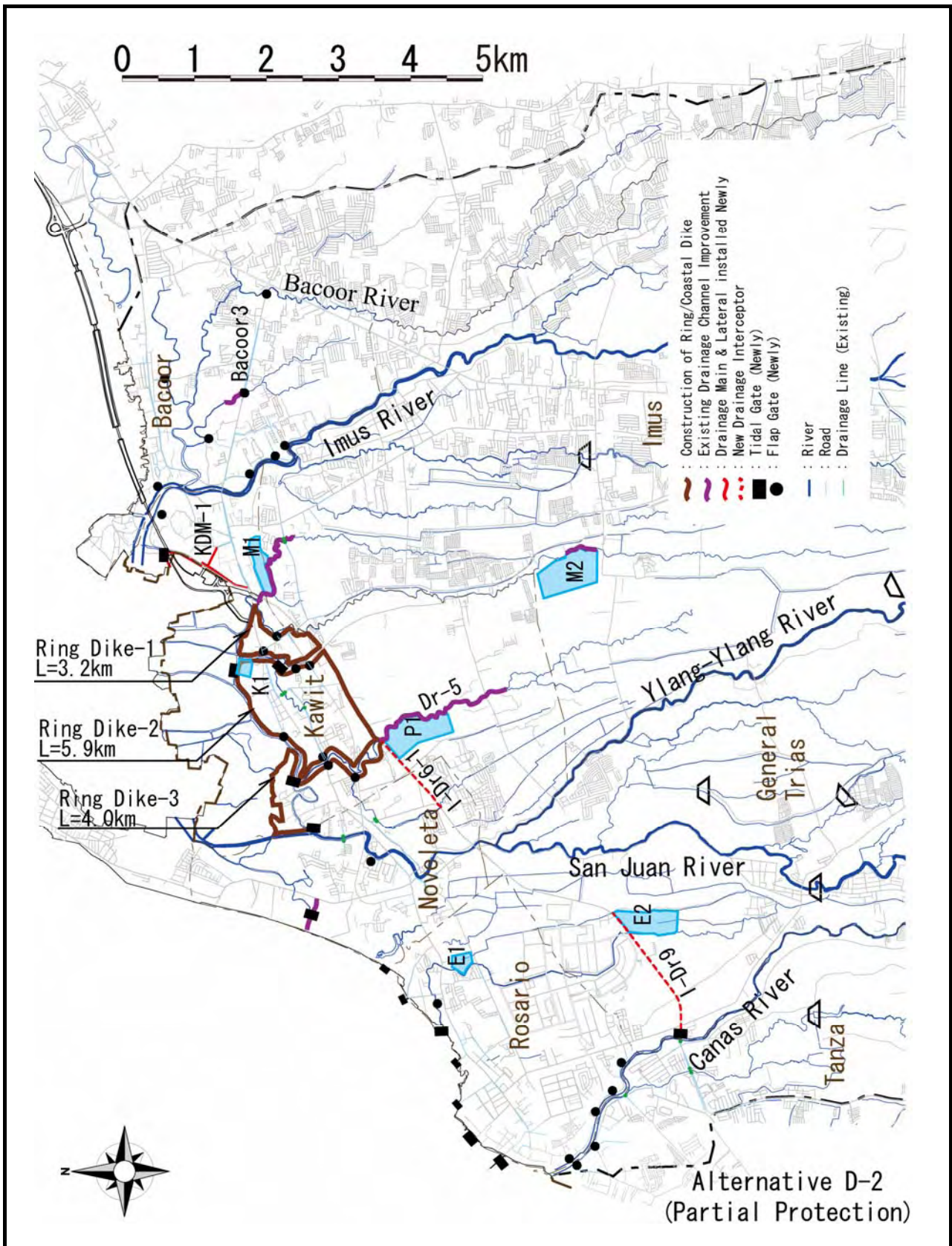
Fig. 8.11
Conceived Structural Measures against Inland
Flood based on Ring Dike System
(Alternative D-2 : Full Protection)



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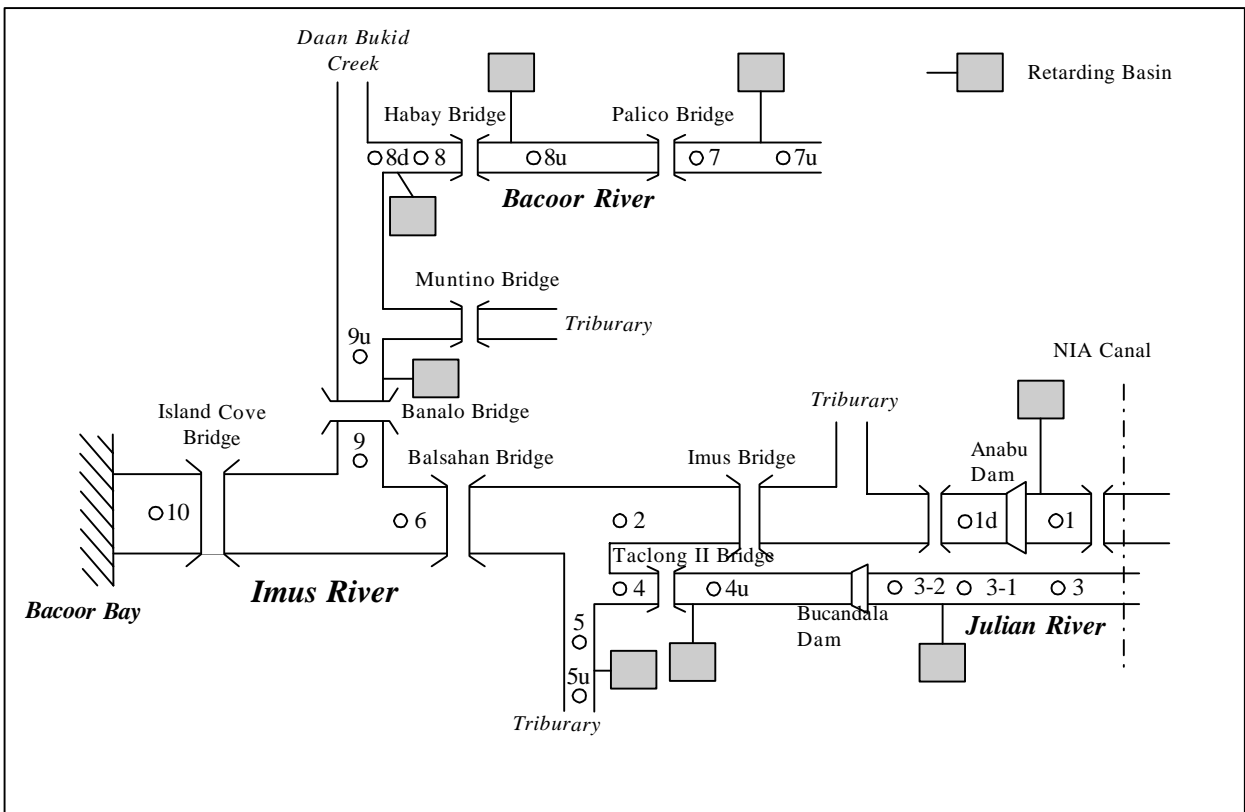
Fig. 8.12
Conceived Structural Measures against Inland
Flood based on Coastal Dike with Tidal Gates
(Alternative D-1 : Partial Protection)



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Fig. 8.13
Conceived Structural Measures against Inland
Flood based on Ring Dike System
(Alternative D-2 : Partial Protection)

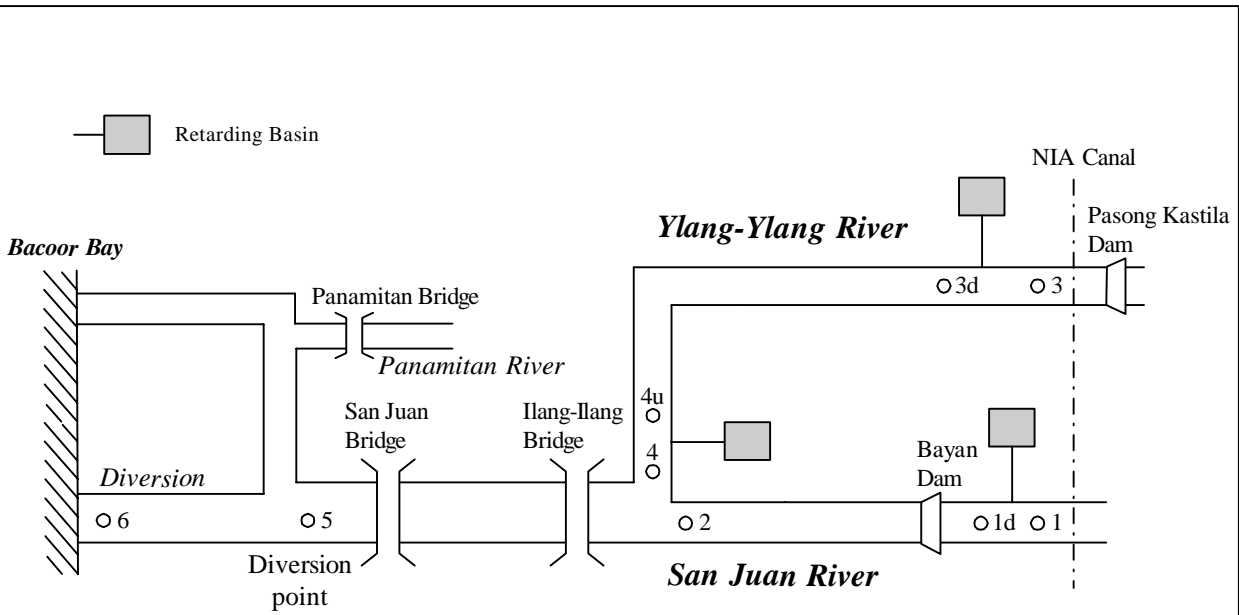


Point No.	Peak Discharge for Each Return Period (m ³ /s)							
	without On-Site				with On-Site			
	F_I.2				F_I.3			
	2-year	5-year	10-year	20-year	2-year	5-year	10-year	20-year
1	310	430	500	600	250	350	430	550
1d	190	250	280	350	160	210	250	300
2	280	380	440	520	230	320	390	470
3	130	160	170	190	90	120	140	160
3-1	170	180	180	180	150	180	180	180
3-2	100	130	140	140	70	110	130	140
4u	110	130	140	150	70	110	130	140
4	85	120	130	130	55	95	120	130
5u	30	35	35	35	25	35	35	35
5	15	15	15	15	15	15	15	15
6	350	450	520	540	290	400	470	530
7u	80	100	110	130	70	90	100	120
7	25	45	55	60	20	40	50	60
8u	75	75	75	75	70	75	75	75
8	50	50	50	50	50	50	50	50
8d	35	45	55	60	35	45	50	55
9u	65	65	65	75	65	65	65	70
9	50	75	95	110	50	70	80	100
10	370	450	520	560	320	400	470	550

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Fig. 8.14
 Design Discharge Distribution of Imus River Basin



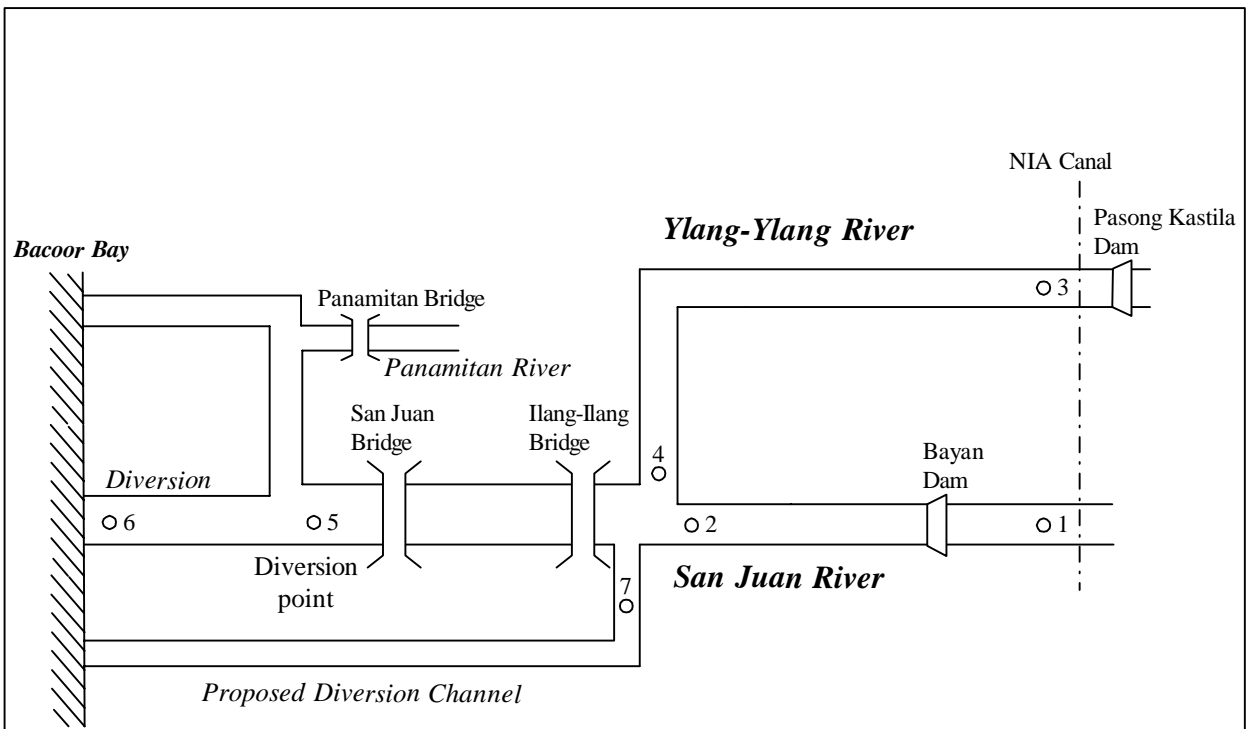
Point No.	Peak Discharge for Each Return Period (m ³ /s)							
	without On-Site				with On-Site			
	F_S.2				F_S.5R			
	2-year	5-year	10-year	20-year	2-year	5-year	10-year	20-year
1	160	230	350	470	130	220	340	460
1d	120	130	140	160	120	130	140	160
2	180	240	270	300	170	230	260	290
3	270	370	490	620	220	300	430	580
3d	200	220	250	320	190	210	240	300
4u	220	260	290	350	200	240	280	330
4	170	210	260	350	170	200	240	330
5	330	420	485	610	310	410	460	580
6	330	420	485	610	310	410	460	580

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Fig. 8.15

Design Discharge Distribution of San Juan River
Basin



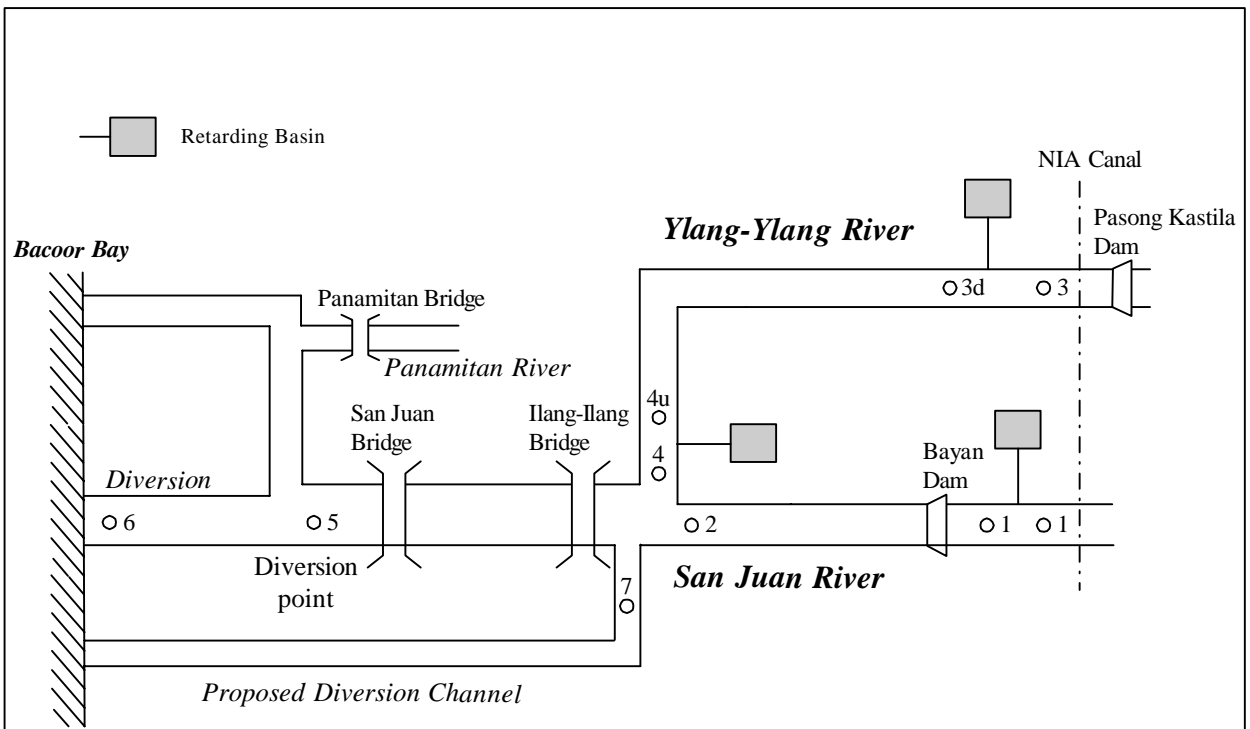
Point No.	Peak Discharge for Each Return Period (m ³ /s)							
	without On-Site				with On-Site			
	F_S.3				F_S.5D			
	2-year	5-year	10-year	20-year	2-year	5-year	10-year	20-year
1	160	230	350	470	130	220	340	460
2	220	340	460	600	190	320	450	580
3	270	370	490	620	220	300	430	580
4	280	390	510	640	220	330	460	600
5	350	400	400	400	340	400	400	400
6	350	400	400	400	340	400	400	400
7	60	270	480	700	0	200	430	670

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Fig. 8.16

Design Discharge Distribution of San Juan River
Basin



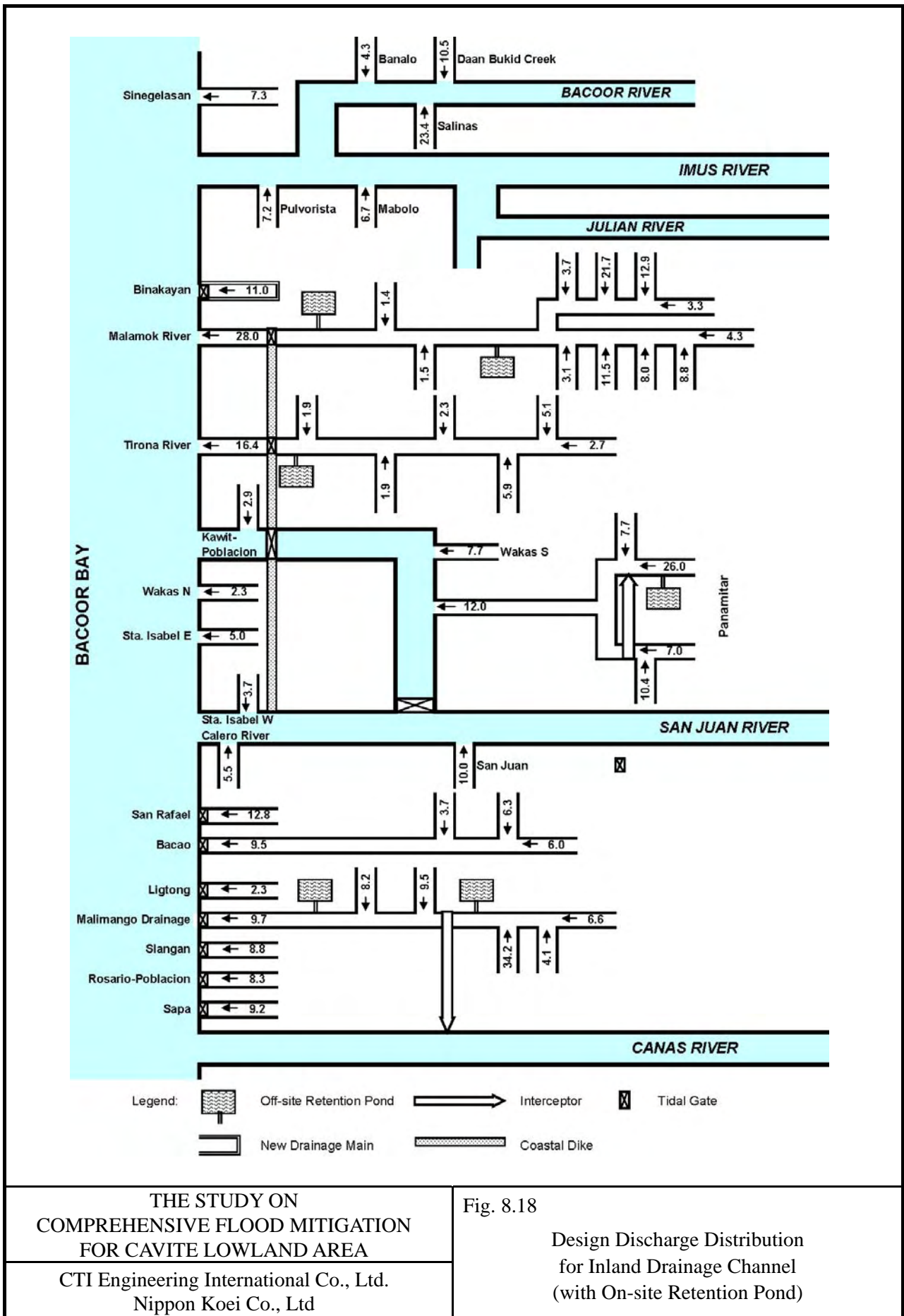
Point No.	Peak Discharge for Each Return Period (m ³ /s)							
	without On-Site				with On-Site			
	F_S.4wo				F_S.4with			
	2-year	5-year	10-year	20-year	2-year	5-year	10-year	20-year
1	160	230	350	470	130	220	340	460
1d	150	210	250	270	120	190	240	260
2	170	320	370	390	170	290	370	390
3	270	370	490	620	220	300	430	580
3d	260	350	410	450	190	290	380	440
4u	270	370	440	480	200	320	400	470
4	260	330	390	420	190	280	360	410
5	350	400	400	400	320	400	400	400
6	350	400	400	400	320	400	400	400
7	40	200	250	300	0	150	250	300

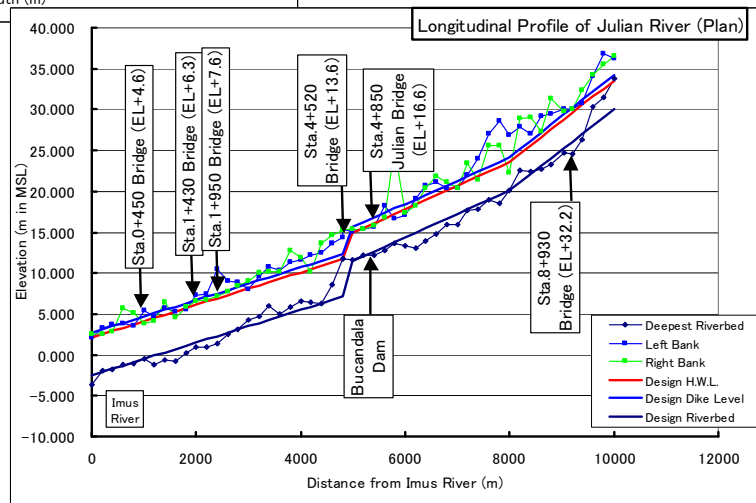
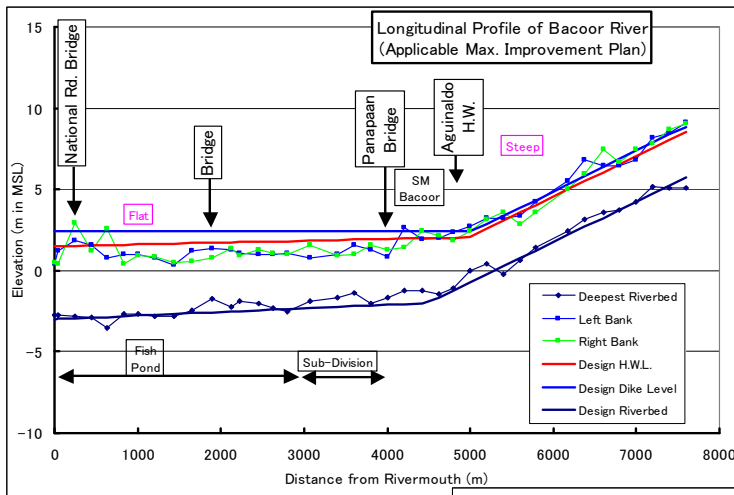
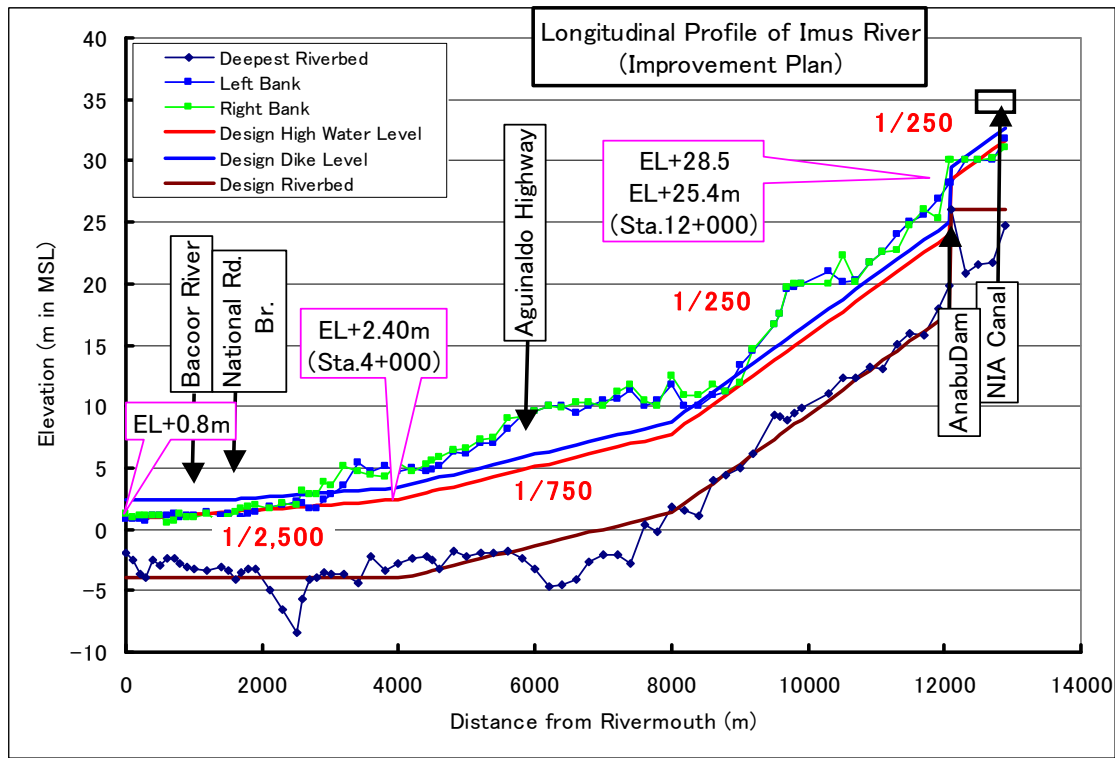
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Fig. 8.17

Design Discharge Distribution of San Juan River
Basin

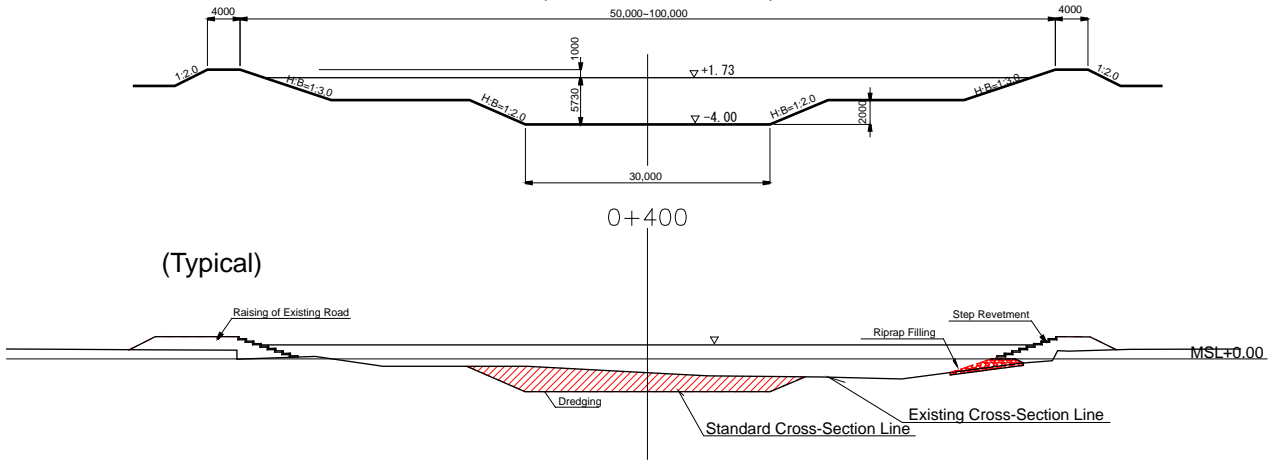




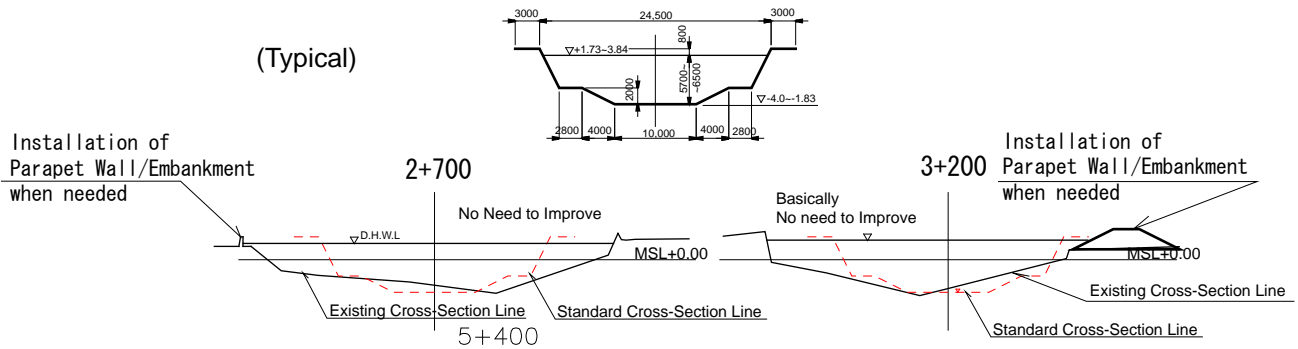
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Fig. 8.19
 Design Longitudinal Profile
 of Imus and Bacoor/Julian River

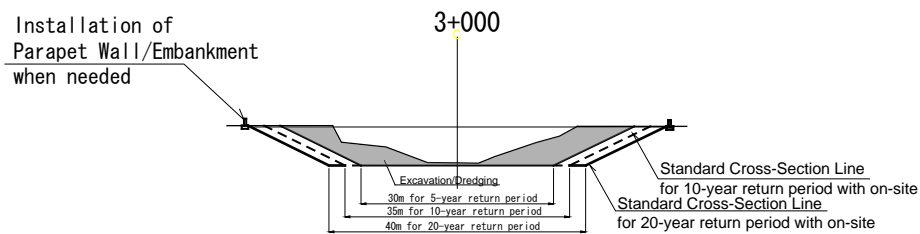
Standard Cross Section (Sta.0+000~Sta.2+000:Rivermouth to Binakayan Bridge):400m³/s
(For All Alternatives)



Standard Cross Section (Sta.2+000~Sta.3+200:Binakayan Bridge to Merging with Julian R.):400m³/s
(For Partial Improvement: F_I.2 and F.I.3)



Typical Cross Section for Full-Scaled Improvement : 660, 810 and 970m³/s
(For Partial Improvement: F_I.2 and F.I.3)



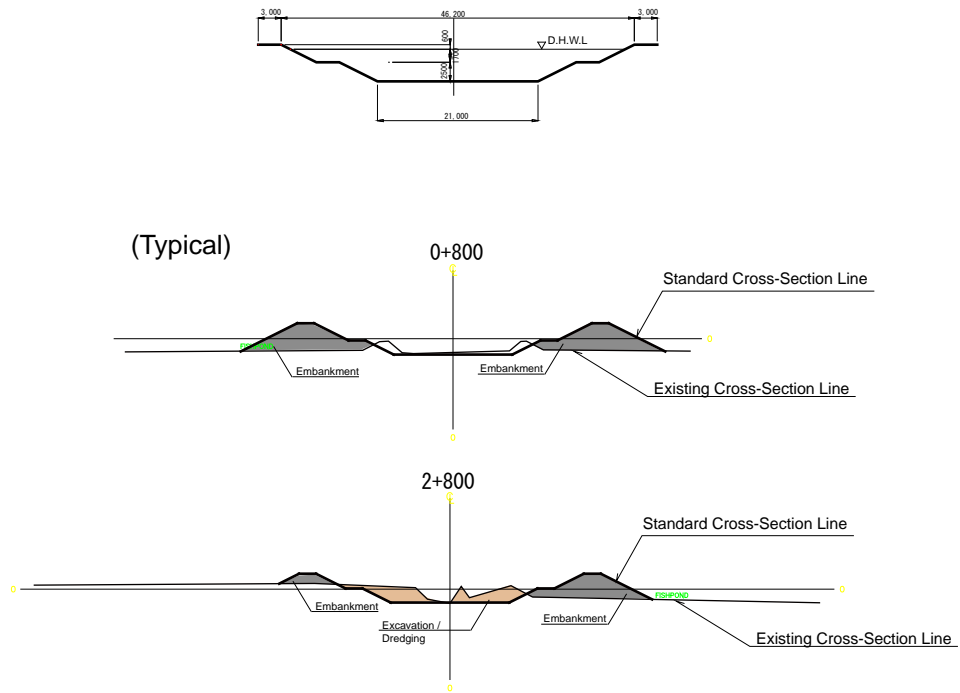
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Fig. 8.20

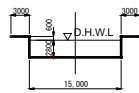
Typical River Channel Improvement
Plan of Imus River

Standard Cross Section (Sta.0+000~Sta.3+000:Rivermouth to Fishpond Area: 100~135m³/s)



Standard Cross Section (Sta.4+000~Sta.5+000:SM Bacoor to Aguinaldo H.W.)

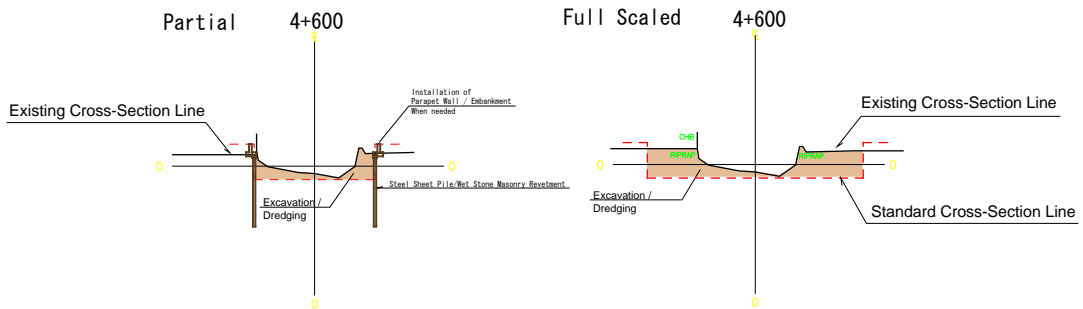
For Partial Improvement
Q=65m³/s



For Full Scaled Improvement
Q=125m³/s



(Typical)



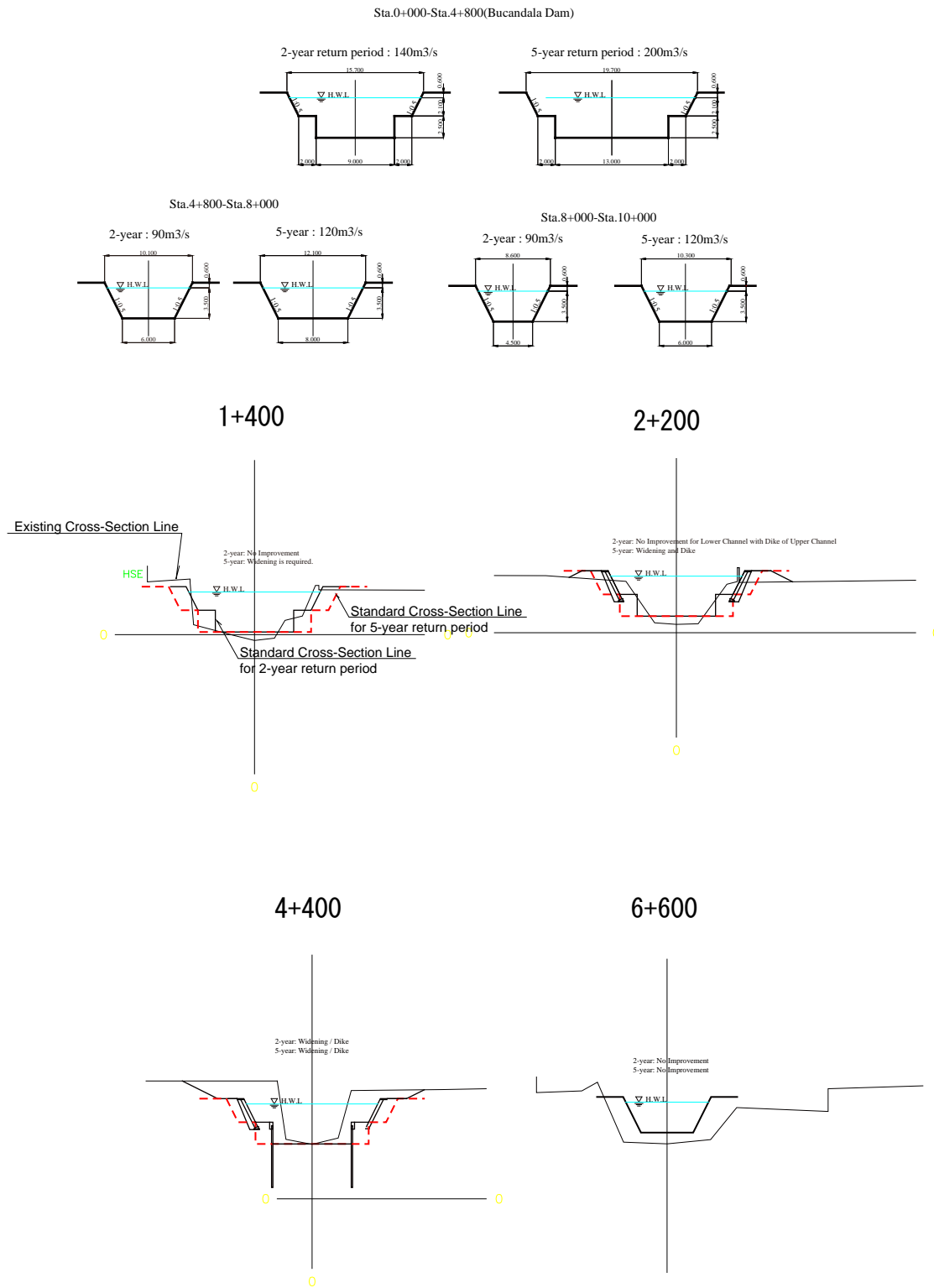
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Fig. 8.21

Typical River Channel Improvement
Plan of Bacoor River

Standard Cross Section of Julian River
 : 2-year (140m³/s) (Partial) and 5-year return period (200m³/s) (Full Scaled)

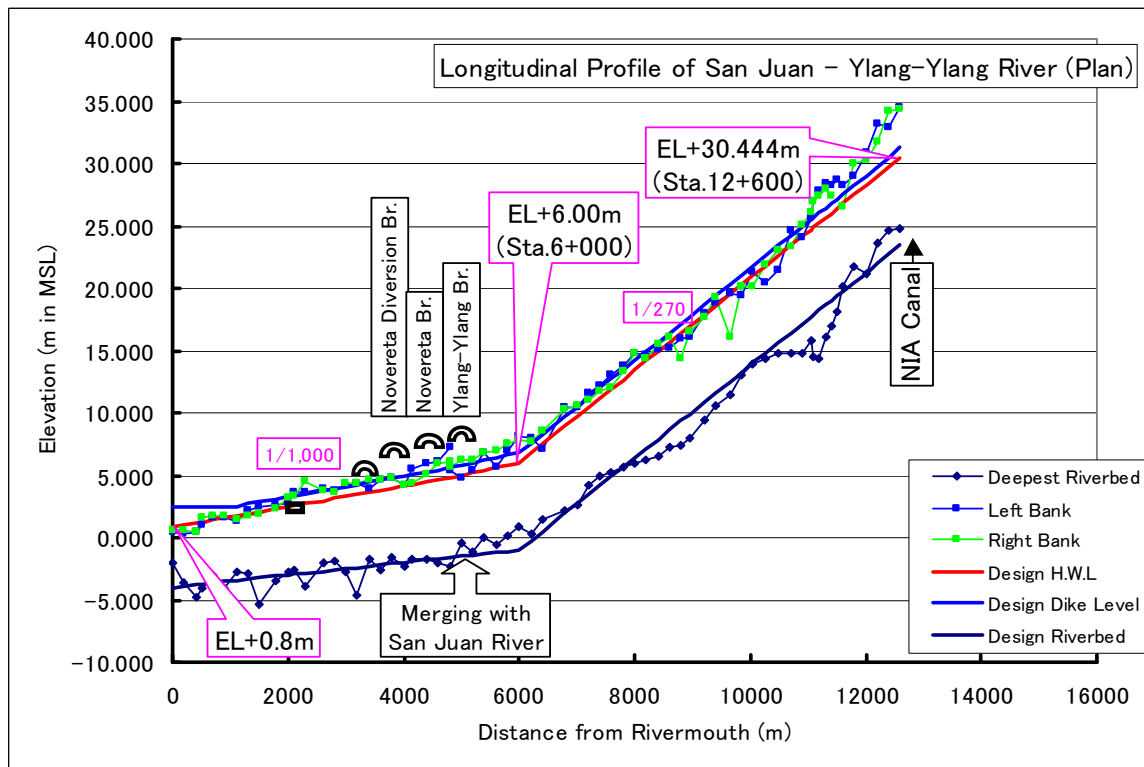
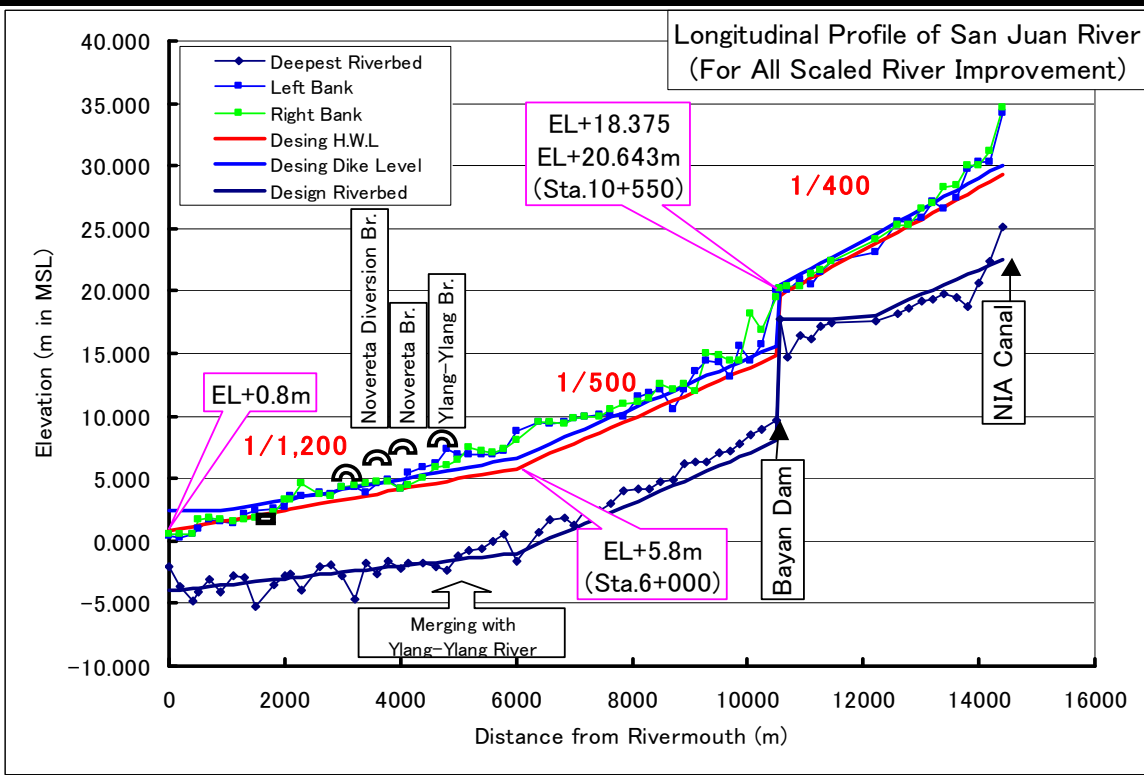


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Fig. 8.22

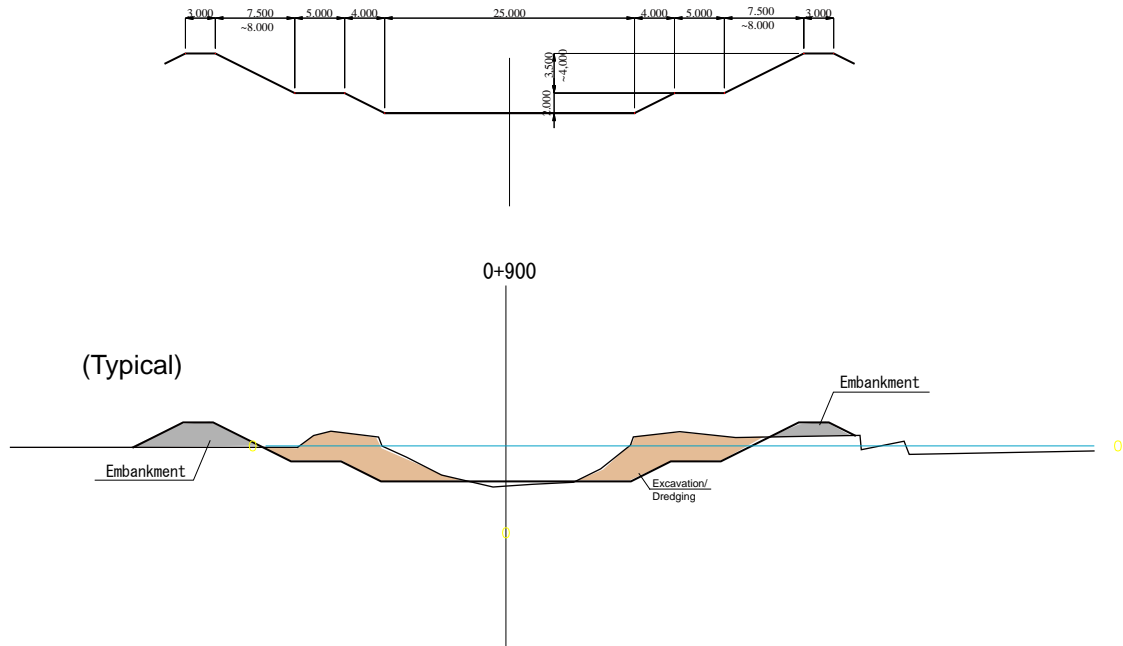
Typical River Channel Improvement
 Plan of Julian River



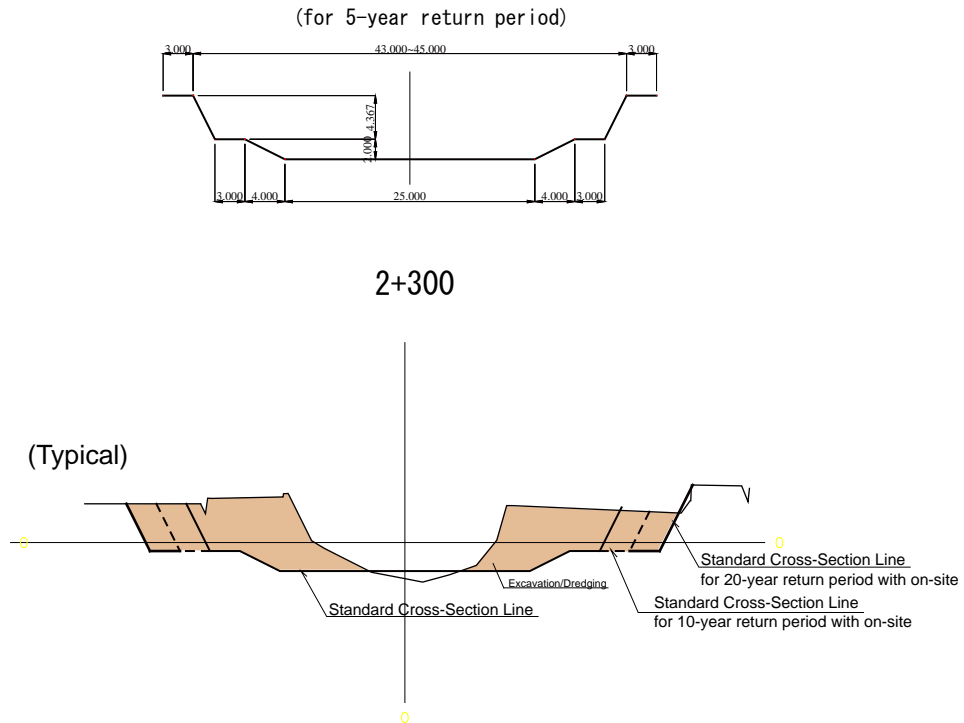
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Fig. 8.23
 Design Longitudinal Profile
 of San Juan and Ylang-Ylang River

Standard Cross Section (Sta.0+000~Sta.1+700:Rivermouth Area : 400m³/s)
(For All Alternatives)



Standard Cross Section (Sta.1+700~Sta.4+800:Brgy. Sta. Isabel~Merging P.) : 600, 810 and 1050m³/s
(For : Full Scaled Improvement)

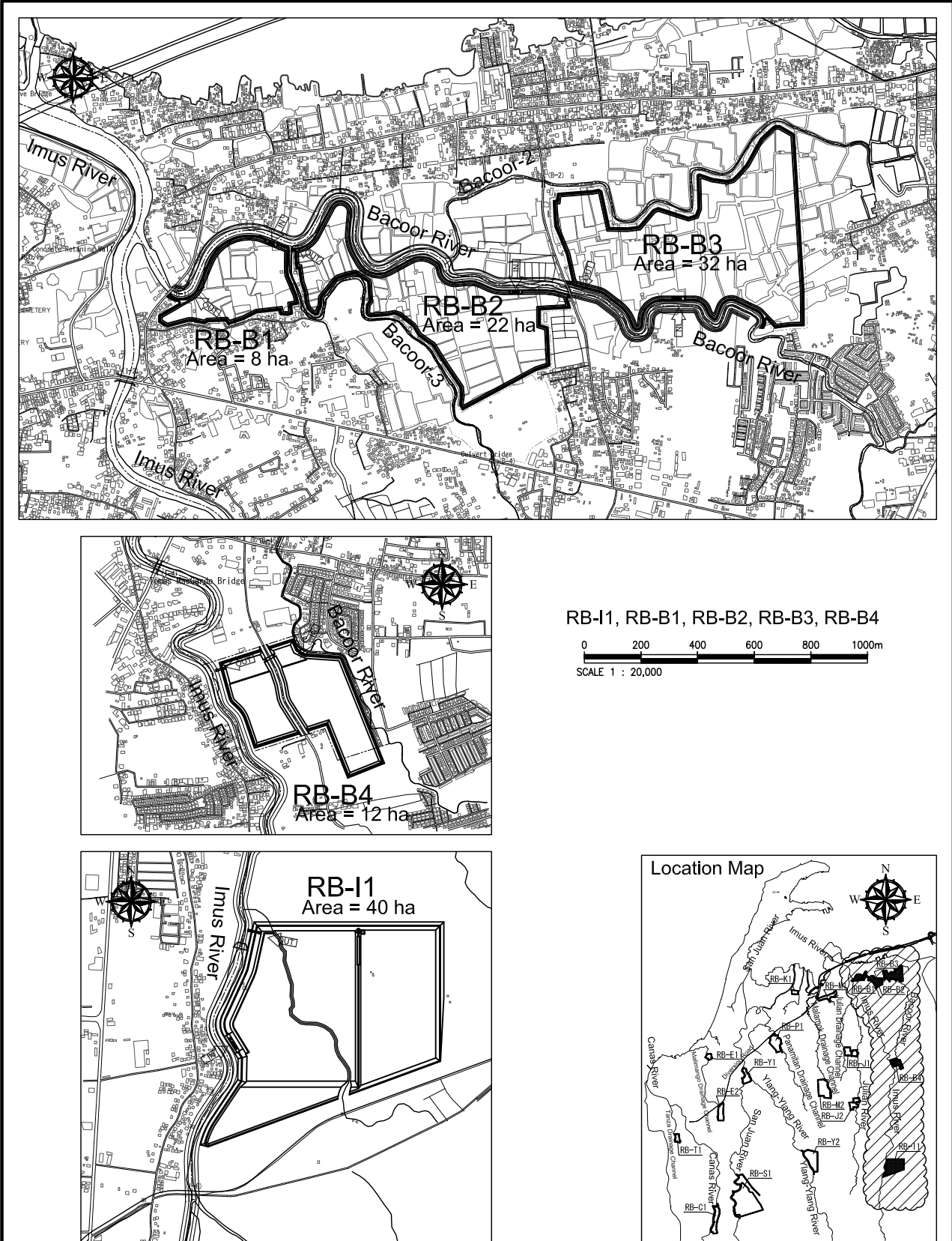


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Fig. 8.24

Typical River Channel Improvement
Plan of San Juan River

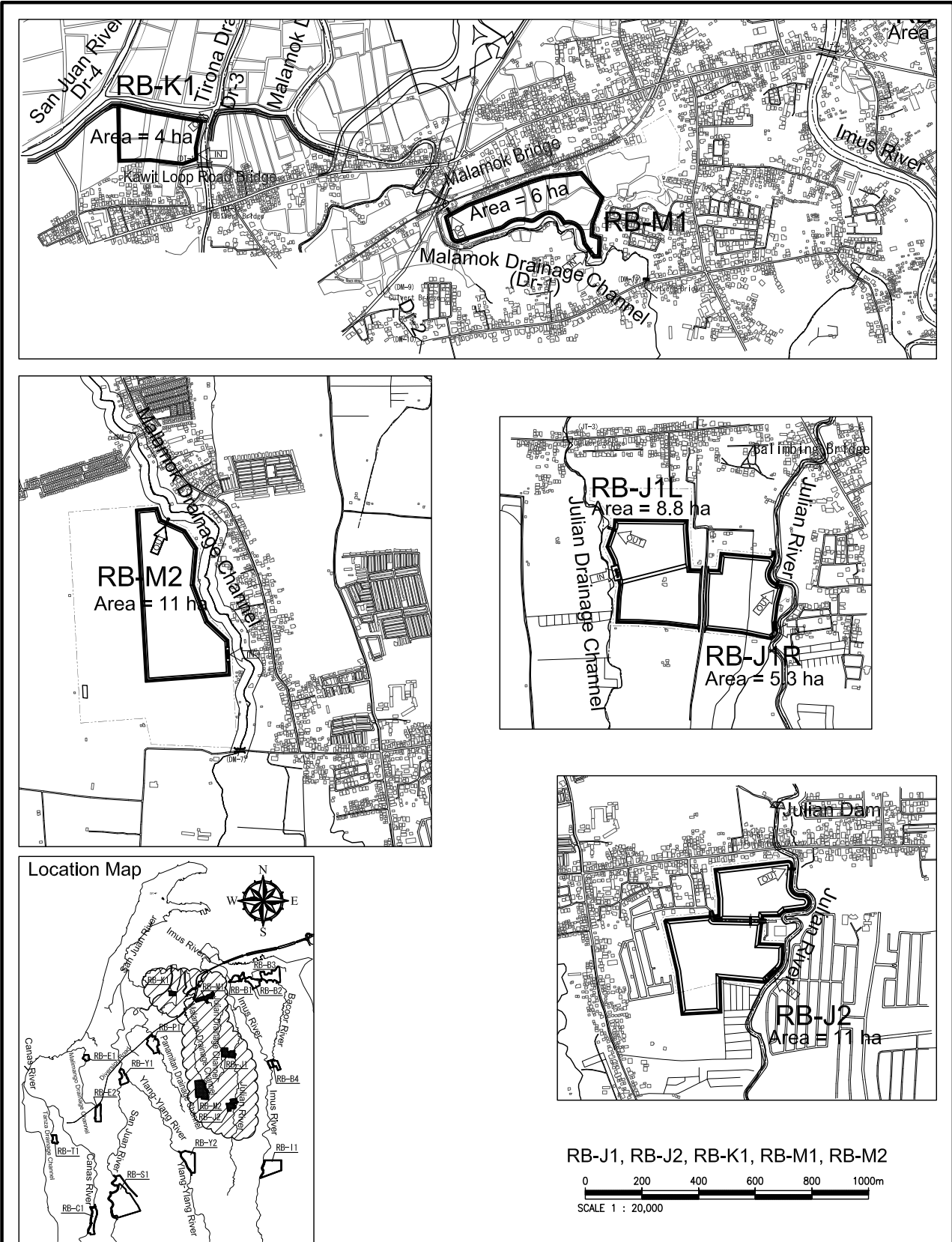


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Fig. 8.25

Layout Plan of Retarding Basin and Retention Pond
 (1/4) - in Imus River Basin
 (In Case of 10-year return period for River
 2-year return period for Inland with On-site)



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Fig. 8.26

Layout Plan of Retarding Basin and Retention Pond
 (2/4) - in/around Imus River Basin
 (In Case of 2-year return period with On-site)