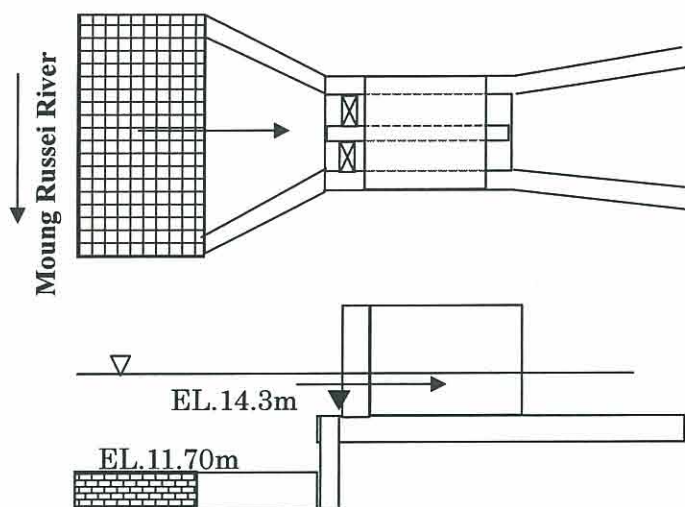


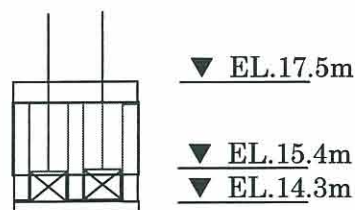
**Summary for Design of Ream Kon Intake**

Design Parameter	Condition	Remarks
Location	800 m upstream from the Diversion weir,	Right side of the Moung Russei River. <u>same location</u> as the existing intake
Design Intake Discharge rate:	Q: 2.66 m <sup>3</sup> /s	Ream Kon Sub-project A=1,890 ha
Irrigation Water Level WL1:	WL. 15.50m	Top of Gate
Elevation at Inlet (River bed):	EL. 11.70m	
Elevation at Inlet (Intake):	EL. 14.30m	>14.00 (within upper 0.4 of water depth of the river (referring to Japanese design criteria for headworks))
Gate Sill Elevation:	EL. 14.30m	
Width, Height and Length	Width: 3.5m Height: 3.5m Length: 7.0m (+ Apron length)	Breast wall is installed between EL. 15.4 and EL.17.8 to prevent The entrance of flood water.
Gate Type and Gate Size, nos.	Slide gate, B:1.0 x H:1.2 x 2 nos.	
Bridge	Total width 4.0 m	



Note:

- Gabion mattress is installed at both river side and downstream of intake structure.
- Foundation pile and sheet pile are to be installed under dike



Prepared by JICA Study Team

**Design Elevation of Intake Inlet:**

Elevation of an intake is to be designed, considering the design water depth of the both river and intake at inlet, in order to reduce and minimize the entrance of suspended particle (sediment). Water depth at inlet of intake is desirable to be within upper 0.4 of that of river (referring to Japanese design criteria for headworks):

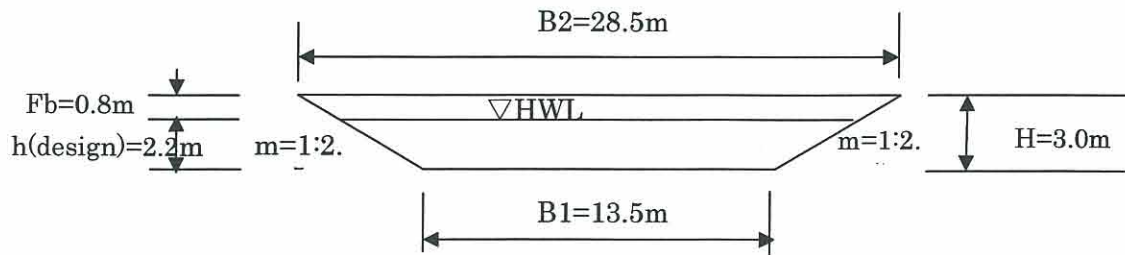
- Water depth at river, H: 3.8 m (= NWL .15.50m – EL.11.70 m)  
Elevation of intake inlet > NWL - 0.4 x H=15.50 –0.4 x 3.8 = 14.0
- Water depth at intake, h: 1.10 m (refer to “rehab. of irrigation and drainage facilities”)  
Elevation of intake inlet = NWL- head loss – h = 15.50 –0.1 –1.1 = 14.3 > 14.0 OK

Prepared by JICA Study Team

**Figure 5.2-7 Schematic Drawing of Ream Kon Intake**

**Summary for Design of Collector Drain-2 (CD-2)**

Design Parameter	Condition	Remarks
Design Discharge rate:	Q1: 30.0 m <sup>3</sup> /s	Maximum Diversion discharge
Design Temporary Discharge	Q2: 8.7 m <sup>3</sup> /s	Max. during Mid-Nov. – End. May
Canal bed Inclination	1/3,000	Refer to landscape
Canal type	Earth canal having trapezoidal cross section	Considering land use
Canal Base width, top width	B1:13.5 m, B2:28.5 m	Considering land use
Canal Side Slope, Maning's n coefficient Freeboard	m : 1:2.5 n : 0.035 Fb: 0.8 m	MOWRAM design criteria for Irr. main canal class with sandy soil applied.
Canal Height,	H: 3.0m,	Present condition
Water Depth	H1: 2.2m (Q1=30 m <sup>3</sup> /s) h2: 1.1m (Q2=8.7 m <sup>3</sup> /s) hmax:3.0 m(Qmax=54 m <sup>3</sup> /s)	hmax: Fb=0



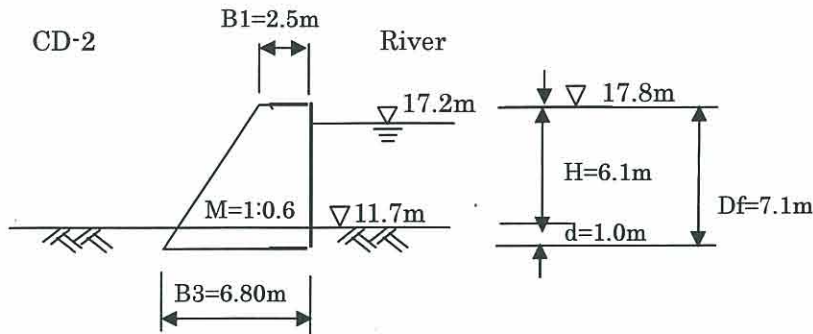
Prepared by JICA Study Team

**Figure 5.2-8 Schematic typical section of Collector Drain-2**

**Summary for Design of Inlet Structure for CD-2**

Design Parameter	Condition	Remarks
Location:	At the entrance of CD-2 from new Diversion weir	
Design Discharge rate:	Q1: 30.0 m <sup>3</sup> /s	Maximum Diversion discharge
Type of Structure	Concrete Dyke with gate	
Elevation of Dyke Non-overflow section Overflow section	EL1.: 17.8m EL2.: 15.7m	> HWL <sub>F1</sub> = 17.20 m (Design NWL+0.2m) > NWL.15.5 m
Elevation of River Bed	EL3.: 11.7m	30 m Upstream of Weir
Elevation of Dyke Bottom	EL4.: 10.7m	EL3. - 1.0 m
Height of Dyke Non-overflow section Overflow section	H1: 7.1 m (6.1 m) H2: 5.0 m (4.0 m)	Apparent height in ( )
Width Top of Non-overflow section Top of Overflow section Bottom of section	B1: 2.5 m B2: 3.77m B3: 6.8 m	Slope M=1:0.6
Length Top of Non-overflow section Top of Overflow section Bottom of section	L1: 38.5 m L2: 20.0 m L3: 38.5 m	Actual length 32.5 m + 3.0 x both side
Gate Type, Dimension, nos.	Slide Gate H:1.5 m x B:4 m x 5 nos.	Pier width 0.7 m
Elevation of Operation Deck	EL5.: 19.0 m	

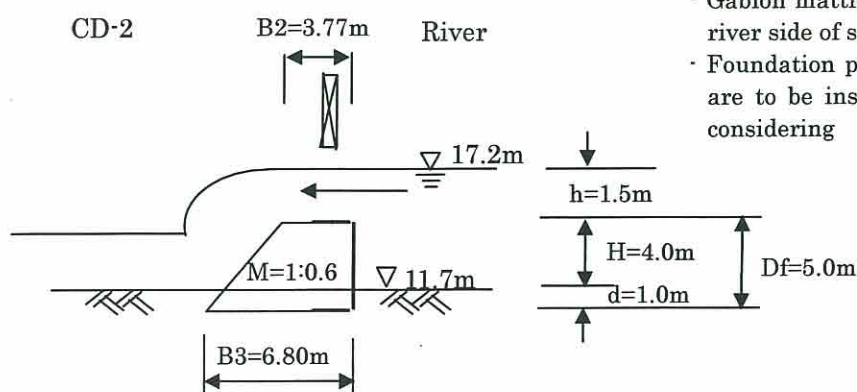
**Non-Overflow Section**



**Note:**

- Apron is installed at CD-2 side of inlet structure for protection.
- Gabion mattress is installed at river side of structure.
- Foundation pile and sheet pile are to be installed under dike considering geological

**Overflow Section**



Prepared by JICA Study Team

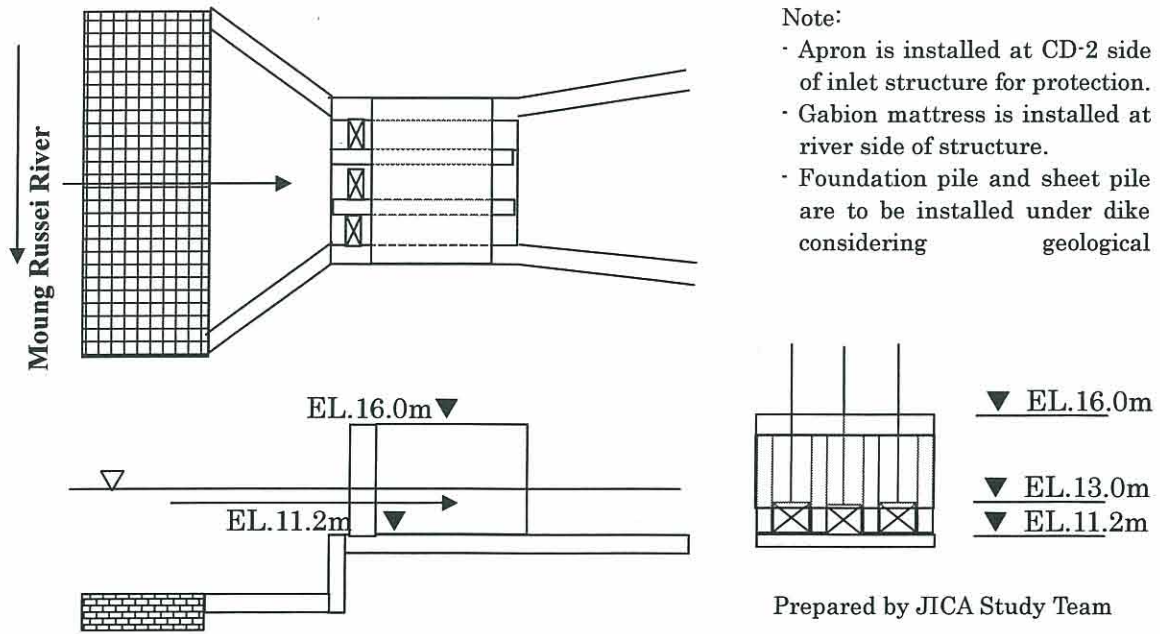
**Figure 5.2-9 Schematic Drawing of Inlet Structure of CD-2**

### Summary for Design of Drainage Gate for CD-2

Design Parameter	Condition	Remarks
Location:	At the end of CD-2	
Design Discharge rate:	Q1: 30.0 m <sup>3</sup> /s	Maximum Diversion discharge
Design Elevation of Drain outlet base	EL. 11.20m	
Water Level WLD1:	WL. 13.40m	Maximum
Gate Sill Elevation:	WL. 11.20m	
Design Width, Height and Length	Width: 7.6m Height: 4.8m Length: 7.0m (+ Apron length)	Breast wall is installed between EL. 13.0m and EL. 16.0m to prevent entrance of flood water.
Design Gate Type and Gate Size, nos.	Slide gate, B:1.8 x H:1.8 x 3 nos.	
Bridge	Total width 4.0 m	

**Note:**

The dimension of the above drainage gate is assuming free discharge condition. It is noted that the water level in the river may be risen under influence of Lake Tonle Sap, so that it is not insured that the design discharge of 30m<sup>3</sup>/s flows out under such influence. Therefore, permanent river diversion measure should be studied as needs of protection of the Moung Russei Town and National Road No.5 from floods arises in future.



**Figure 5.2-10 Schematic Drawing of Drainage Gate for CD-2**



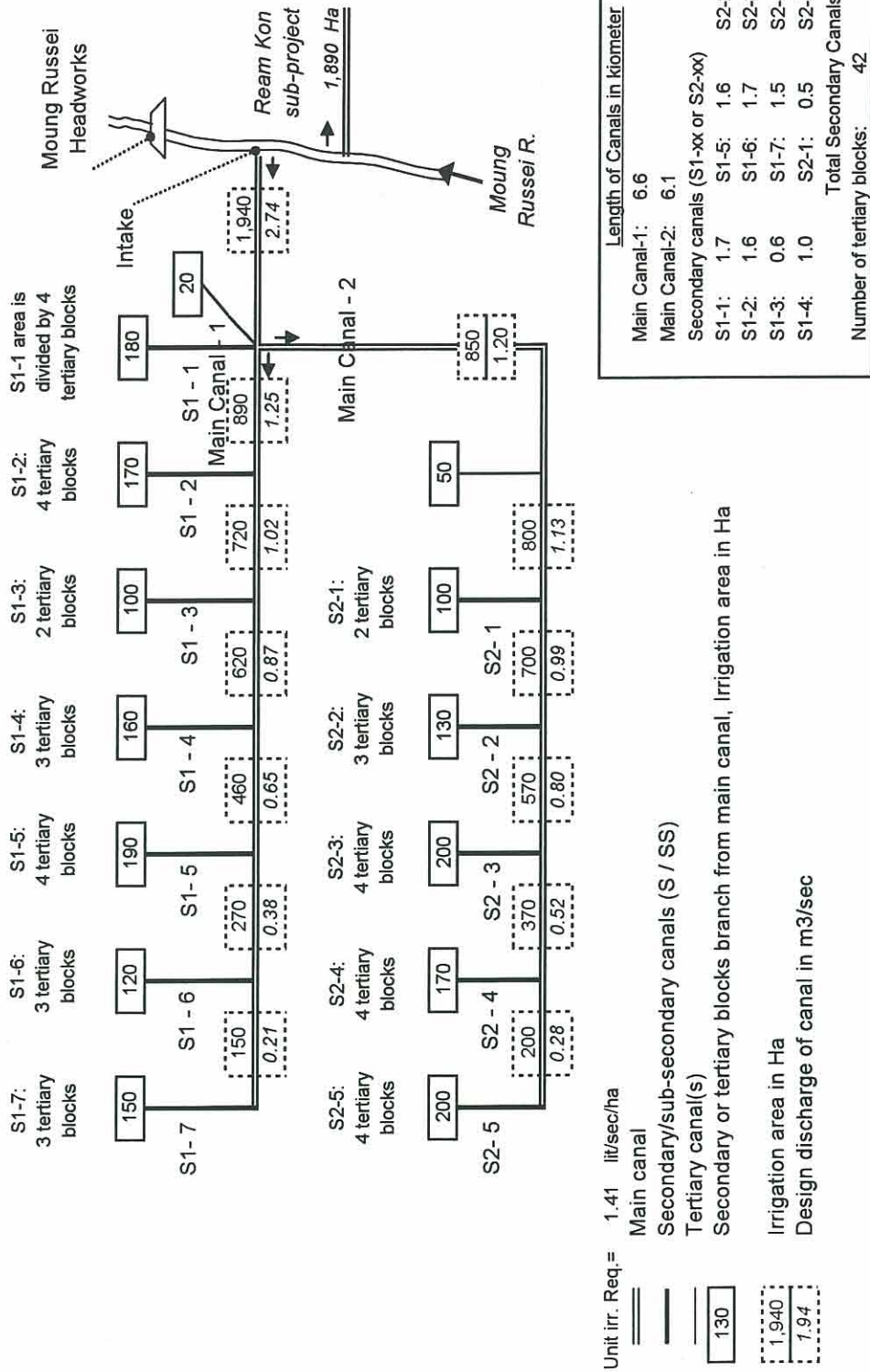


Figure 5.3-1 Irrigation Area Diagram of Por Canal Rehabilitation Sub-project

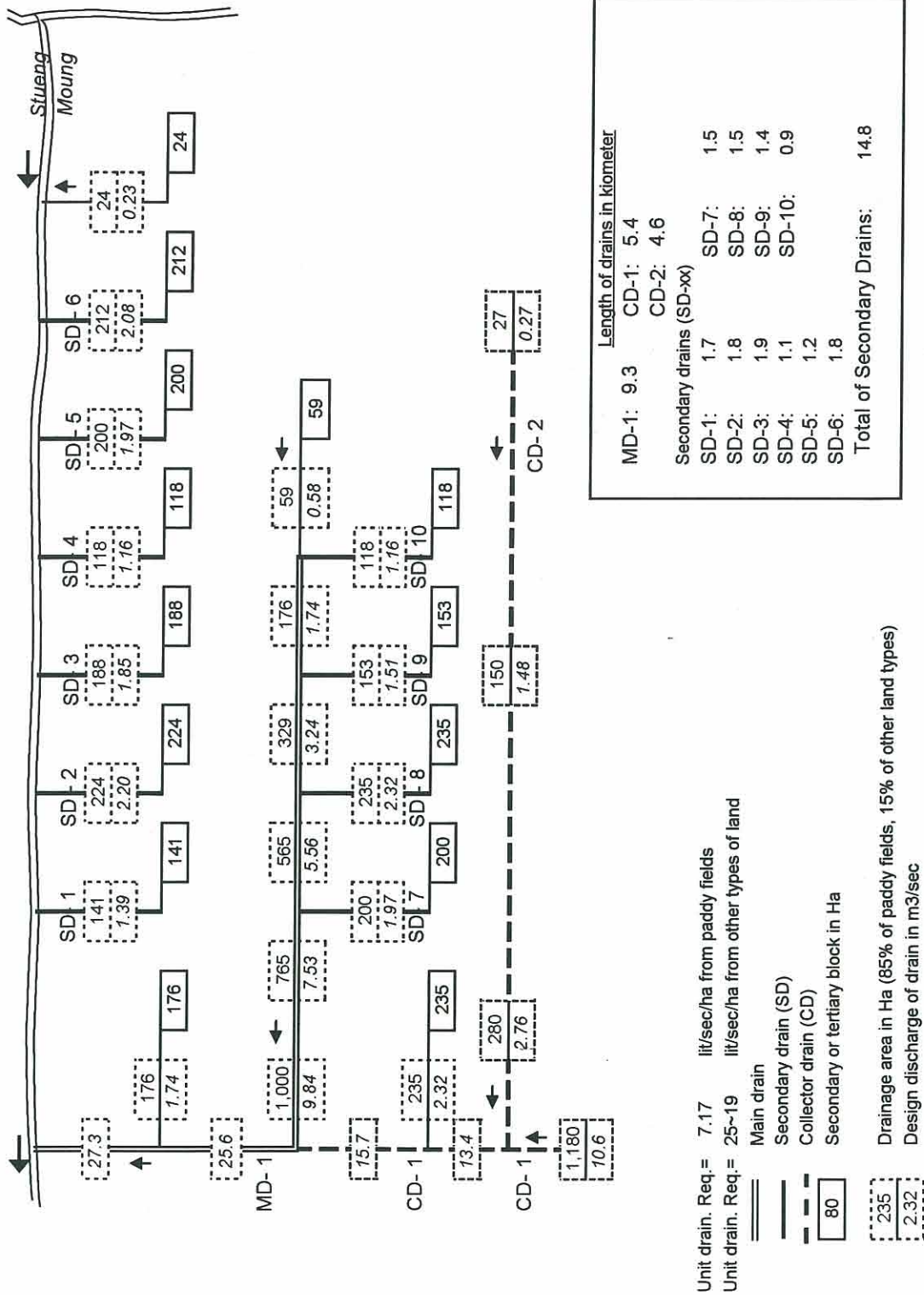
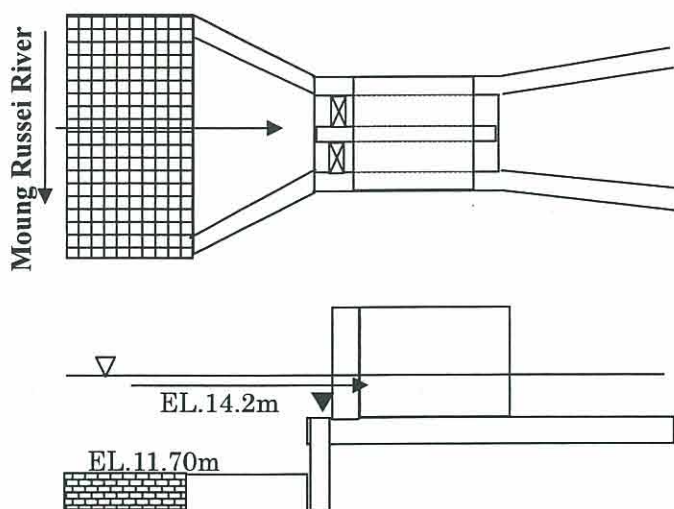


Figure 5.3-2 Drainage Area Diagram of Por Canal Rehabilitation Sub-project

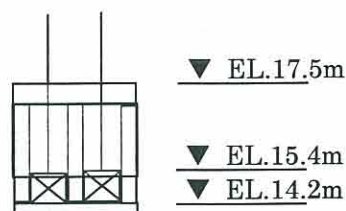
### Summary for Design of Por Canal Intake

Design Parameter	Condition	Remarks
Location:	600m upstream from the Diversion weir	Left side of the Moung Russei River. <u>same location</u> of the existing intake
Design Intake Discharge rate:	Q: 2.74 m <sup>3</sup> /s	Por Canal Sub-project A=1,940 ha
Irrigation Water Level WL1:	WL. 15.50m	Top of Gate
Irrigation Water Level WL1':	WL. 15.70m	WL1+max. Overflow depth=0.2m
Elevation at Inlet (River bed):	EL. 11.70m	
Elevation at Inlet (Intake):	EL. 14.20m	>14.00 OK, (within upper 0.4 of water depth of the river (referring to Japanese design criteria for headworks))
Gate Sill Elevation:	EL. 14.20m	
Width, Height and Length	Width: 3.5m Height: 3.6m Length: 8.0m (+ Apron length)	Breast wall is installed between EL. 15.4 and EL.17.8 to prevent The entrance of flood water.
Gate Type and Gate Size, nos.	Slide gate, B:1.0 x H:1.2 x 2 nos.	
Bridge	Total width 4.0 m	



Note:

- Gabion mattress is installed at both river side and downstream of intake structure.
- Foundation pile and sheet pile are to be installed under dike



#### Design Elevation of Intake Inlet:

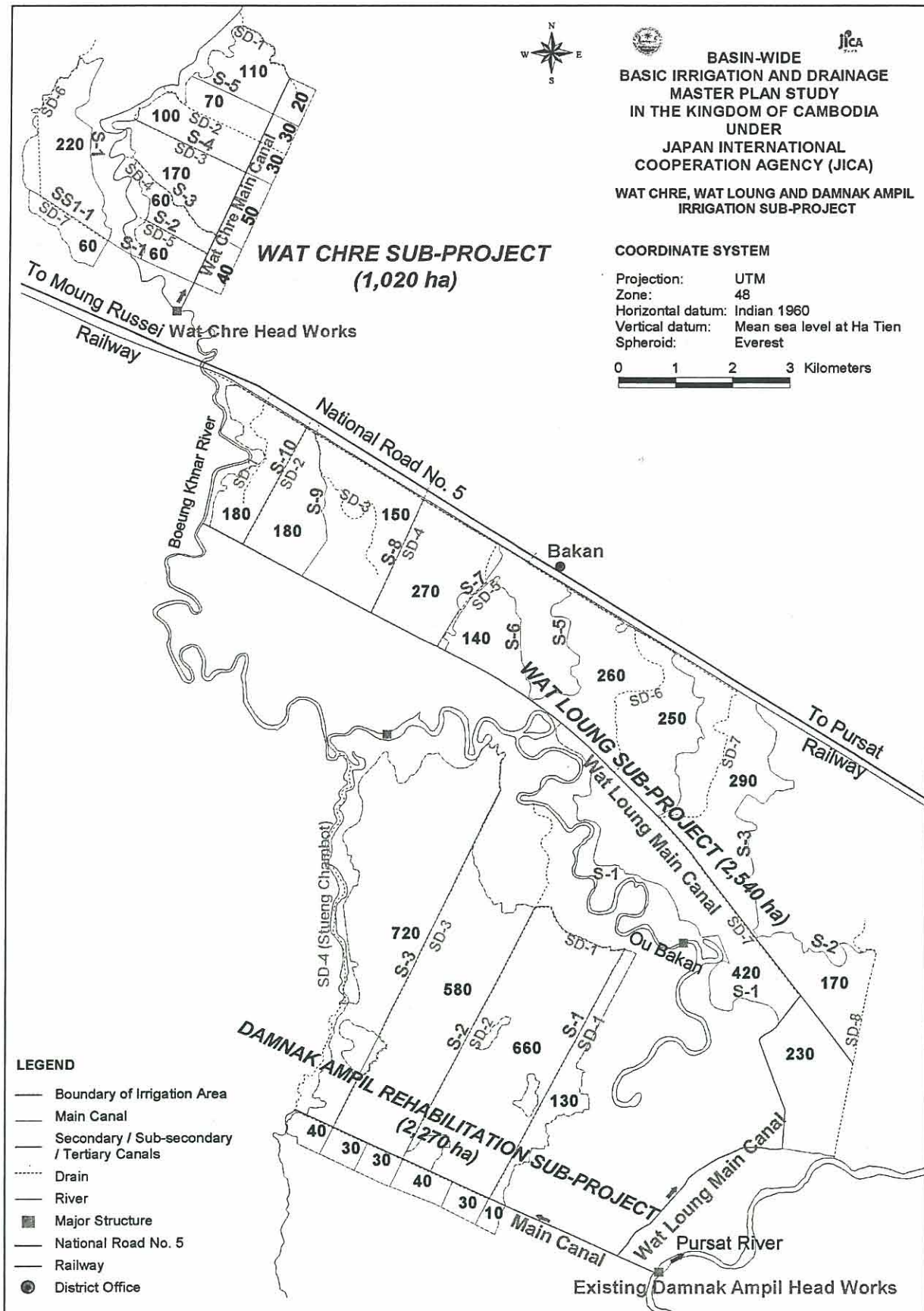
Elevation of an intake is to be designed, considering the design water depth of the both river and intake at inlet, in order to reduce and minimize the entrance of suspended particle (sediment). Water depth at inlet of intake is desirable to be within upper 0.4 of that of river (referring to Japanese design criteria for headworks):

- Water depth at river, H: 3.8 m (= NWL .15.50m – EL.11.70 m)  
Elevation of intake inlet > NWL - 0.4 x H=15.50 –0.4 x 3.8 = 14.0
- Water depth at intake, h: 1.20 m (refer to “rehab. of irrigation and drainage facilities”)  
Elevation of intake inlet = NWL- head loss – h = 15.50 –0.1 –1.2 = 14.2 > 14.0 OK

Prepared by JICA Study Team

Figure 5.3-3 Schematic Drawing of Por Canal Intake





**Figure 5.4-1**  
**Irrigation and Drainage Canal Layout of**  
**Damnak Ampil, Wat Loung, and Wat Chre**  
**Rehabilitation Sub-projects**



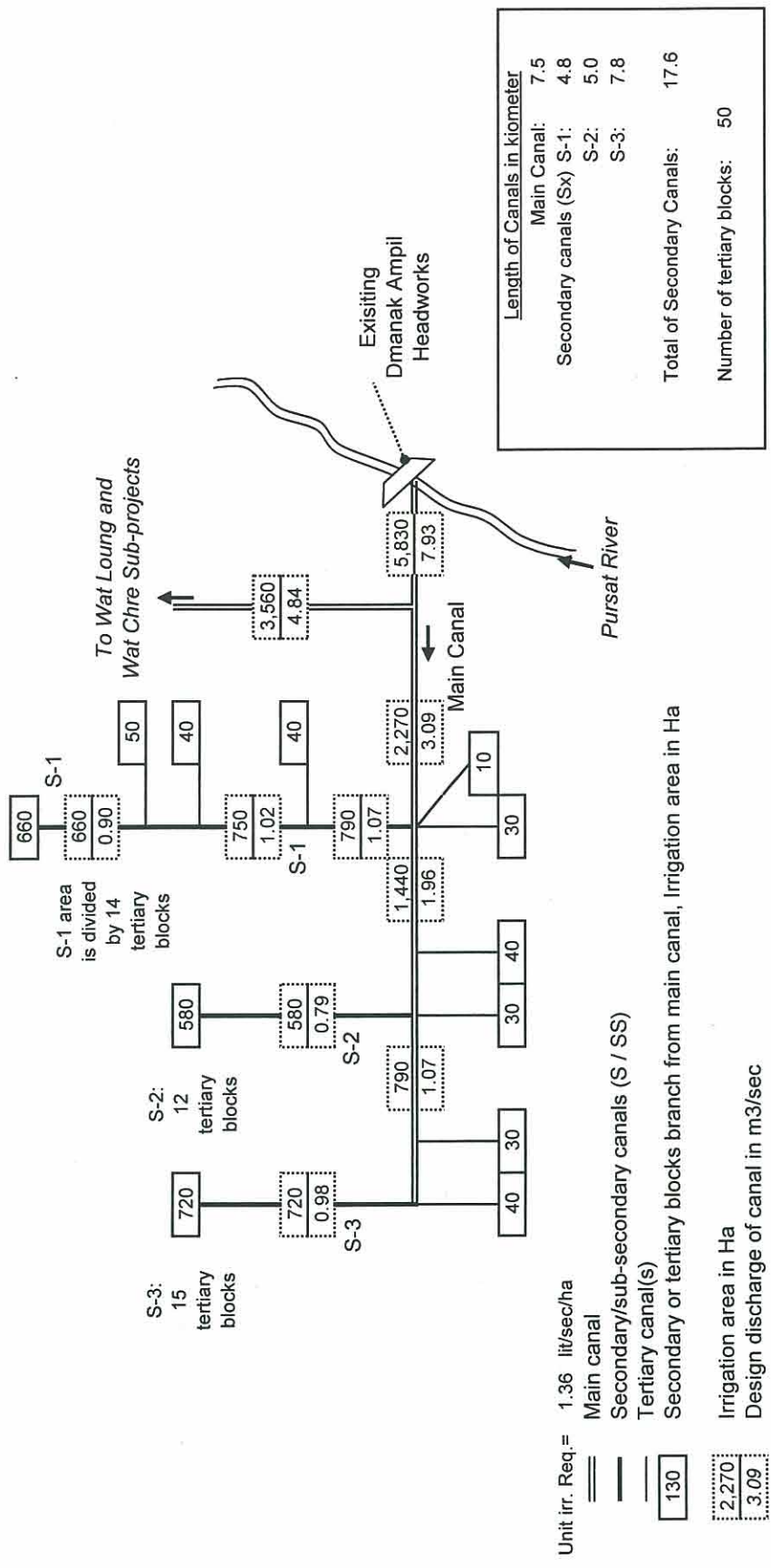
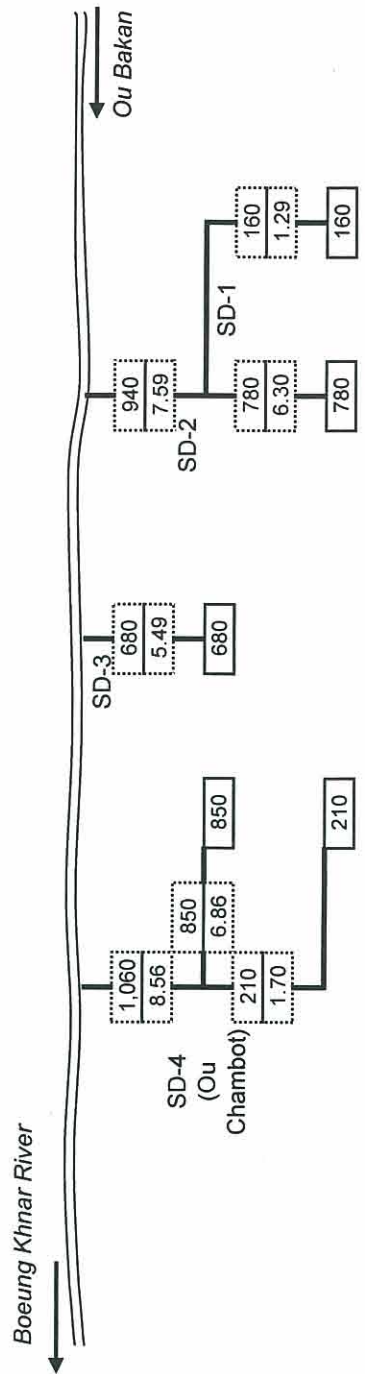


Figure 5.4-2 Irrigation Area Diagram of Damnak Ampil Rehabilitation Sub-project



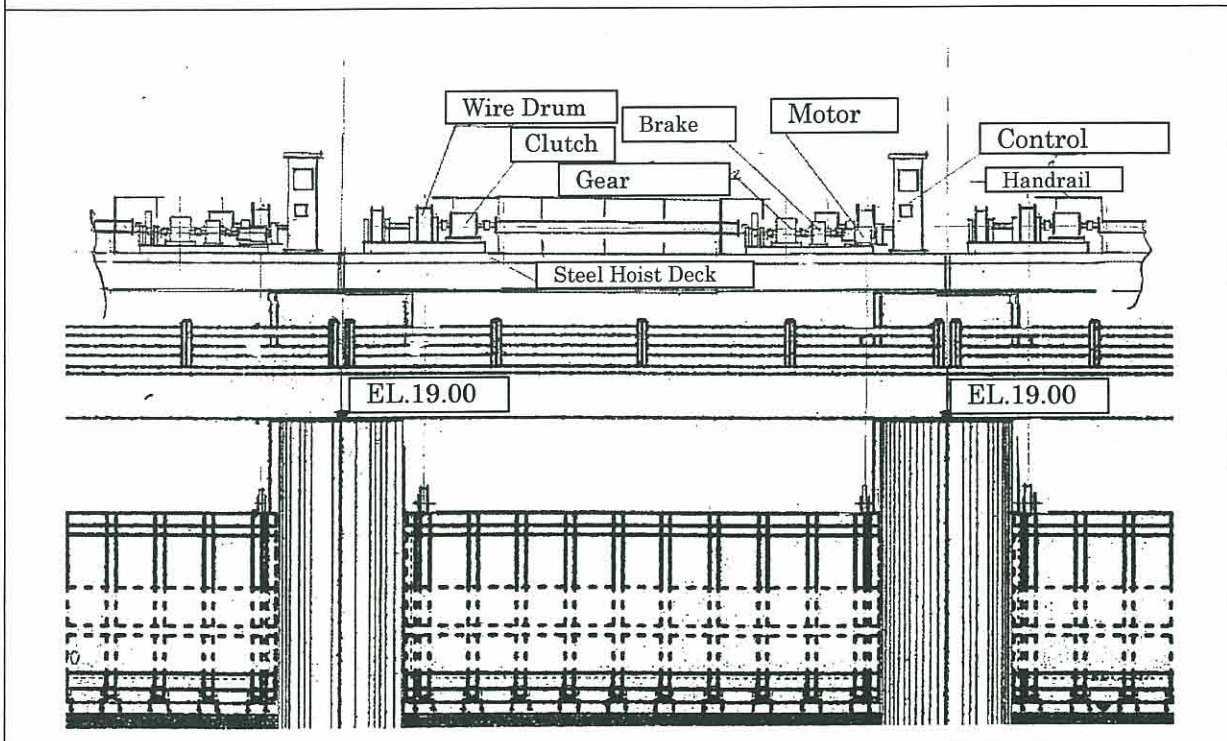
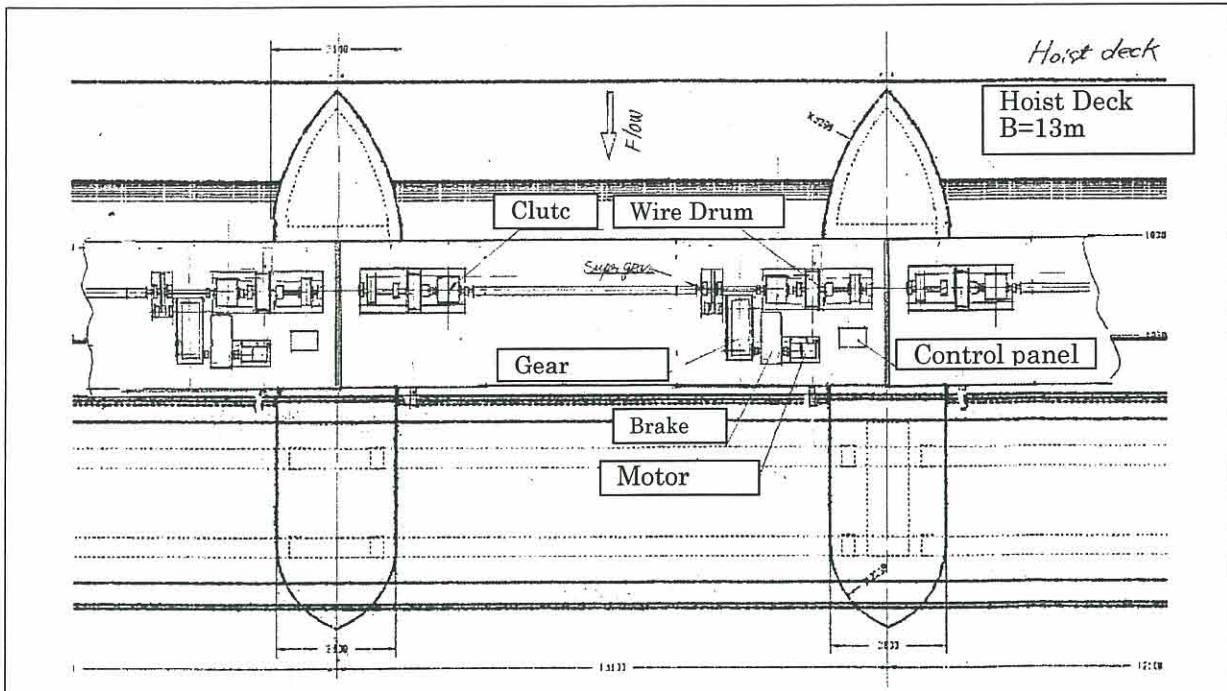
Unit drain. Req. = 6.32 lit/sec/ha from paddy fields  
 Unit drain. Req. = 25-18 lit/sec/ha from other types of land

Main drain  
 Secondary drain  
 Collector drain  
 Secondary or tertiary block in Ha

Drainage area in Ha (85% of paddy fields, 15% of other land types)  
 Design discharge of drain in m<sup>3</sup>/sec

Length of Drains in kilometer	
Secondary Drains (SD-xx)	
SD-1:	6.4
SD-2:	6.8
SD-3:	7.8
SD-4:	7.2
<b>Total of Secondary Drains:</b>	<b>28.2</b>

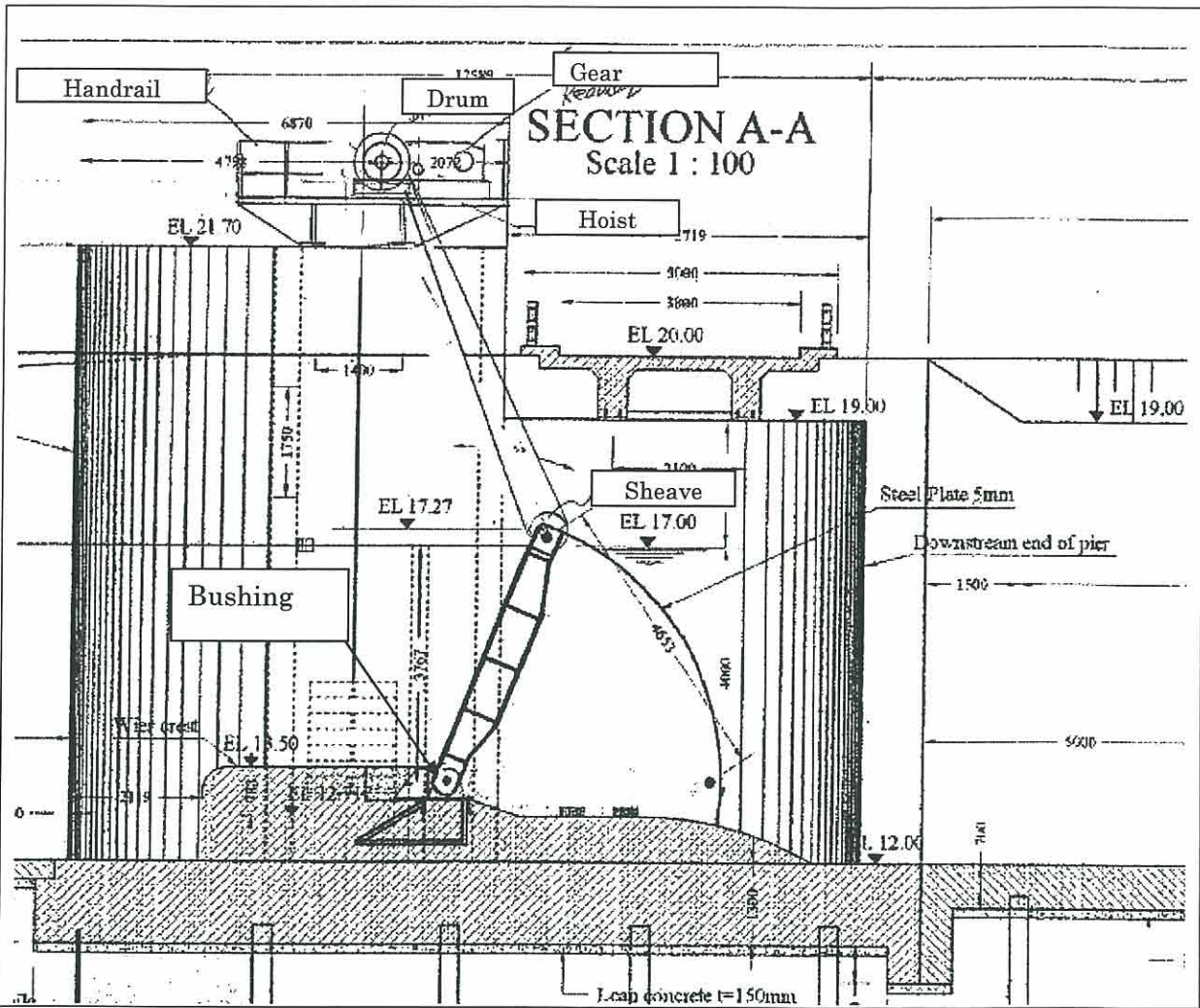
Figure 5.4-3 Drainage Area Diagram of Damnak Ampil Rehabilitation Sub-project



Prepared by JICA Study Team based on Original Source  
 "Project Proposal for Rehabilitation of Damnak Ampil Irrigation Project in Pursat Province", MOWRAM, 2004

**Figure 5.4-4 (1/2) Schematic Drawing of New Hoisting System for Damnak Ampil Diversion Weir (Flood gate)**





Prepared by JICA Study Team based on Original Source  
 "Project Proposal for Rehabilitation of Damnak Ampil Irrigation Project in Pursat Province", MOWRAM, 2004

Figure 5.4-4 (2/2) Schematic Drawing of New Hoisting System for Damnak Ampil Diversion Weir (Flood gate)

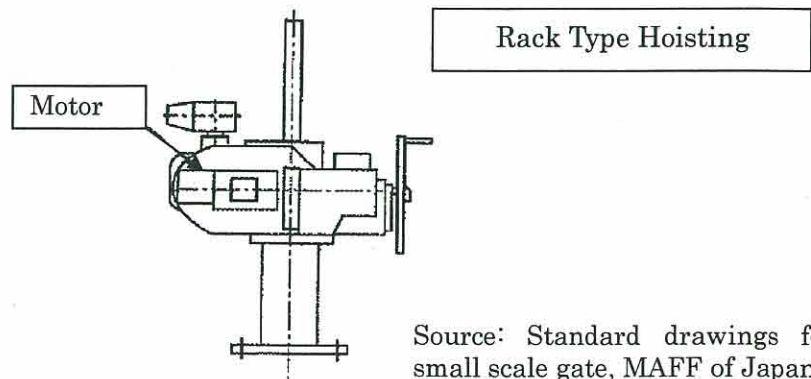
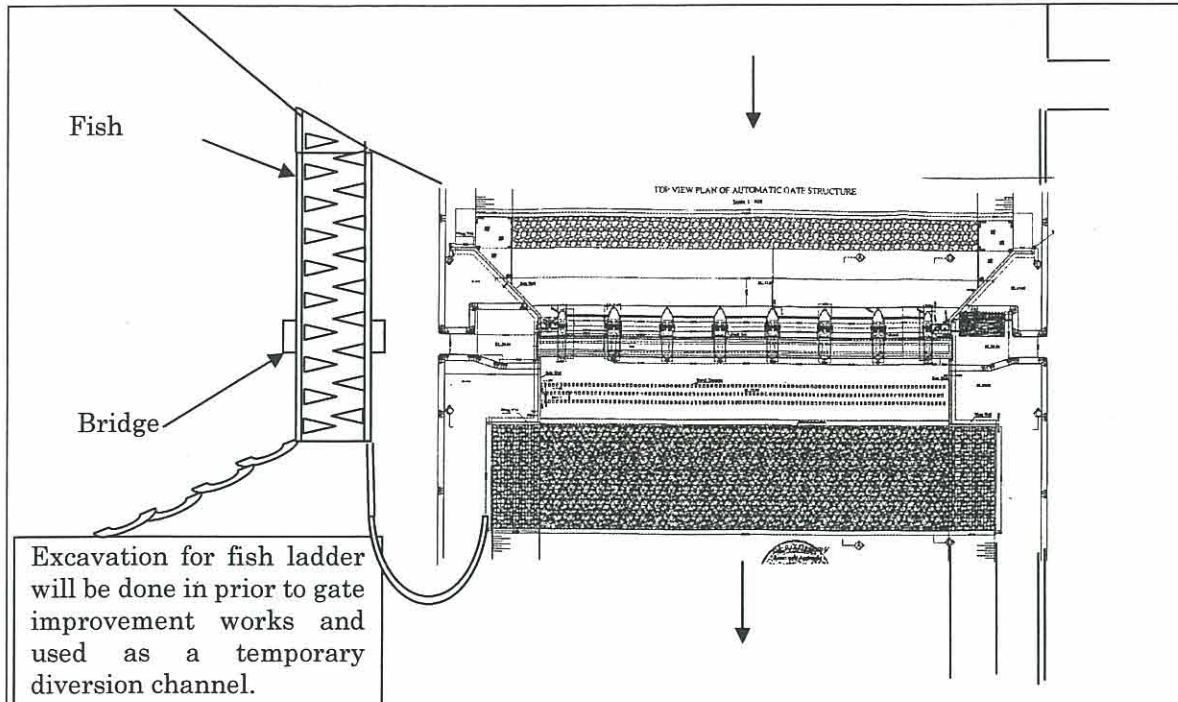


Figure 5.4-5 Schematic Drawing of New Hoisting System for Scouring sluice gate





Prepared by JICA Study Team based on Original Source "Project Proposal for Rehabilitation of Damnak Ampil Irrigation Project in Pursat Province", MOWRAM, 2004

**Figure 5.4-6 Schematic Drawing of Installation of Fish ladder**

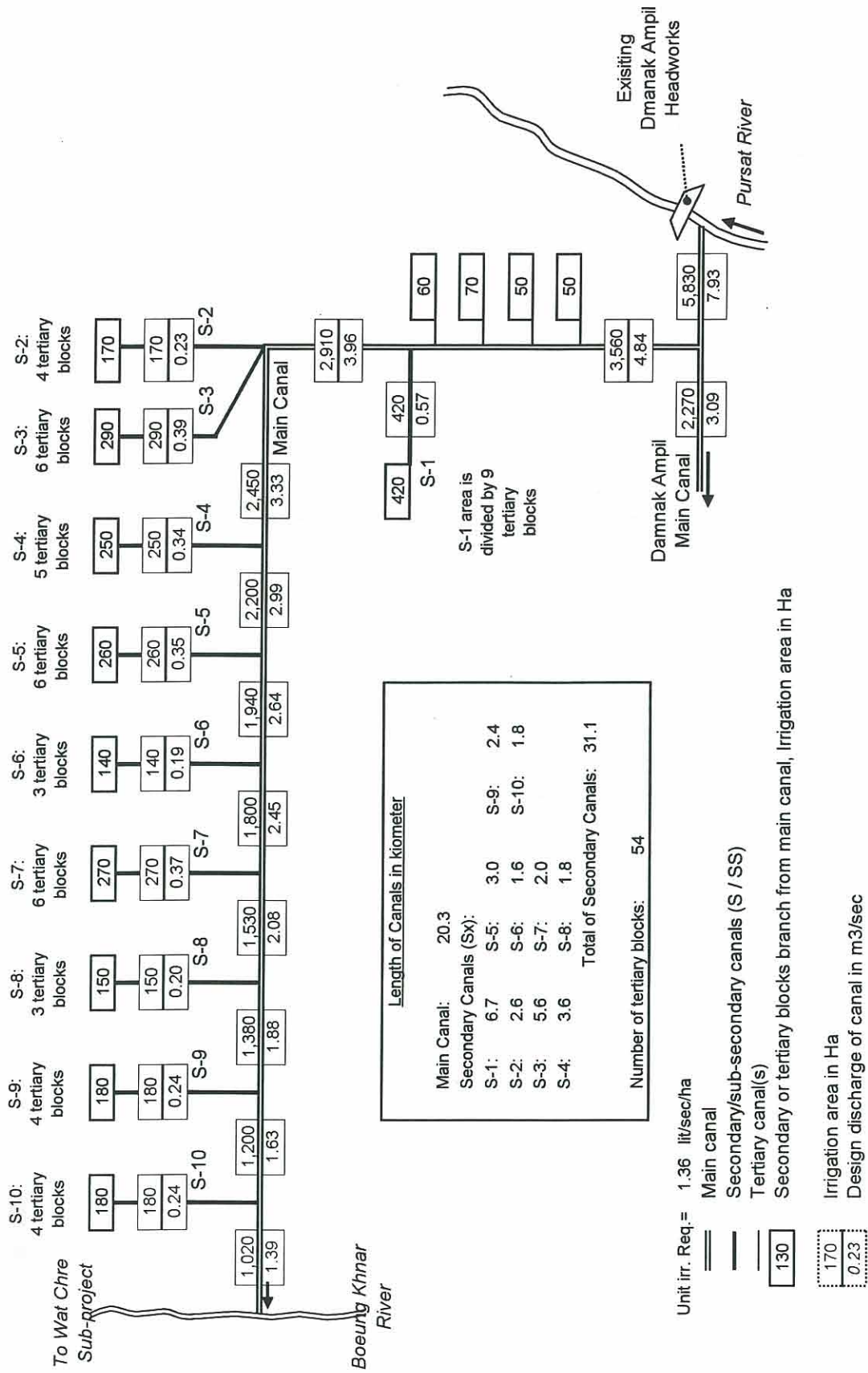
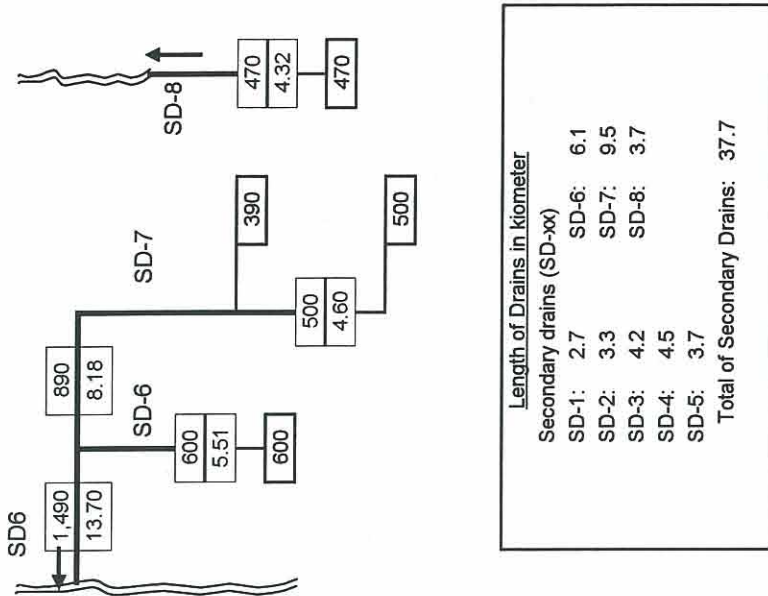


Figure 5.5-1 Irrigation Area Diagram of Wat Loung Rehabilitation Sub-project



Boeung Khnar River

Length of Drains in Kilometer	
Secondary drains (SD-xx)	
SD-1:	2.7
SD-2:	3.3
SD-3:	4.2
SD-4:	4.5
SD-5:	3.7
SD-6:	6.1
SD-7:	9.5
SD-8:	3.7
<b>Total of Secondary Drains:</b>	<b>37.7</b>

Unit drain. Req. = 6.32 lit/sec/ha from paddy fields  
 Unit drain. Req. = 25-18 lit/sec/ha from other types of land

Main drain  
 Secondary drain  
 Secondary or tertiary block in Ha

Drainage area in Ha (85% of paddy fields, 15% of other land types)  
 Design discharge of drain in m3/sec

Figure 5.5-2 Drainage Area Diagram of Wat Loung Rehabilitation Sub-project

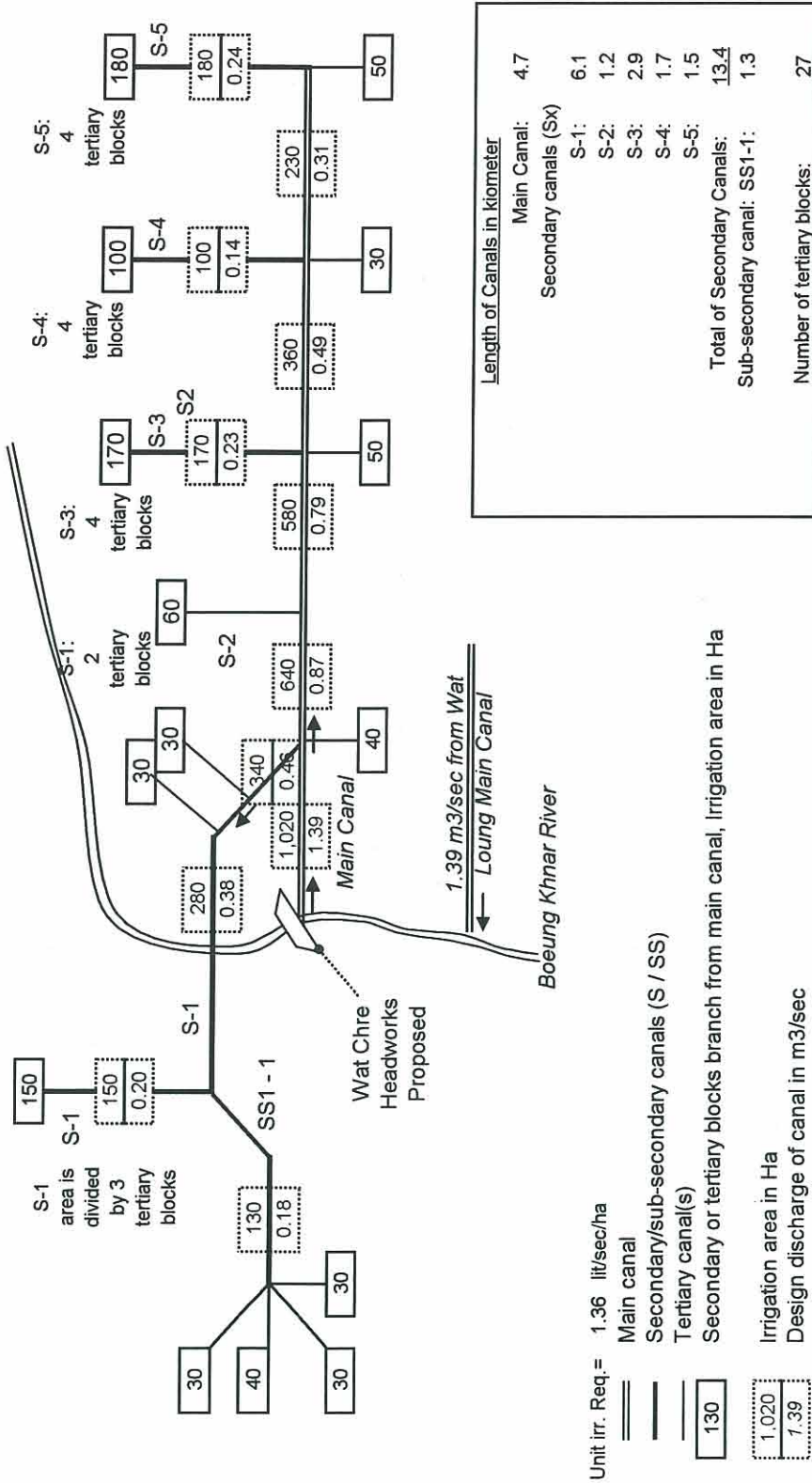


Figure 5.6-1 Irrigation Area Diagram of Wat Chre Rehabilitation Sub-project