3.3 Risk Assessment of Road Slope Disasters

3.3.1 Outline of Risk Assessment

Road slope disaster risk is evaluated using two risk indicators in this report. One is the frequency of road closure disaster of a site (FRCDp) while the other is potential annual loss of the site (ALp).

Relation of FRCDp and ALp is shown in Figure 3.3.1 below.



RCD: Road closure disaster

Figure 3.3.1 Relation of Risk Indicators

In general, risk is considered as the product of frequency and magnitude. FRCDp is an index which only shows the frequency element of risk. ALp is the overall index which is the product of frequency and the magnitude of risk, evaluated as monetary loss.

(1) Potential Frequency of Road Closure Disaster of a site (FRCDp)

Estimated structure of FRCDp is shown in Figure 3.3.2.



Figure 3.3.2 Estimation Structure of FRCDp

Geometry, surface situation and disturbance are FRCDp factors in the absence of existing

measures (FRCDpom). On the other hand, effect of existing structural measures is defined as the coefficient of effectiveness of existing structural measures (CEM). FRCDp is estimated as the product of FRCDpom and CEM.

(2) Potential Annual Loss of a Site (ALp)

Estimation structure of FRCDp is shown in Figure 3.3.3.



Figure 3.3.3 Estimation Structure of Annual Losses

ALp is the product of FRCDp and potential loss of a site (Lp) as previously mentioned. Lp is composed of four elements consisting of reopening cost, human lives loss, vehicle loss, and suspension loss. Suspension loss is caused by impassable road site, which is composed of losses of waiting, detour, and cancellation.

3.3.2 Workflow

Figure 3.3.4 shows flow of assessment of risk and feasibility of structural measures on N-M Highway.

Data Collectio	n Data C	ollection	
Preparation of			
Preliminary Ha	zard Map	· · · · · · · · · · · · · · · · · · ·	
	Analysis of Road Disaster in 2003 Disaster	Photo Interpret on Slope E	tation Map Disaster
		*	
	- Classification of H	lazard Type (Draft)	
	- Slope Inventory F	ormat (Draft)	Lacand for Harner Man (Draft)
	OUTPUT Preliminary	◆ Hazard Map	
			Work in Japan
- · · ·	<u> </u>	⊢ · — · —	Field Survey
Formulation of	Hazard Man		
	18 Diald Summer (Summer for	★	
	1 Fleid Survey (Survey for		d map)
ſ	+	Tristribution N	fan an Slama
	Geological Map	Distribution M	ster
		¥	
	Reclassification	ı of Hazard Type —	•
		¥	Legend for Hazard Map
		rd Map 🖌	
			Lind of AUG 2007
Risk Assessme	ent of Sites	★	
	2 nd Field Survey (Risk Assessme Crossing	nt Survey for All Roa Streams)	d Slopes and
	crossing	▼	
	- Road Slope Asse	ssment Sheet 1 to (3
		T	
Feasibility Ass	essment of		
Structural Mea	sures		
	3rd Field Survey (Structural Meas	sure Planning Survey (of High Risk
	Si	tes) 1	
		v	
	Road Slope Ass	sessment Sheet 4	End of FEB 2008

Figure 3.3.4 Flow of Road Slope Assessment on N-M Highway

Hazard map is used as part of input information for risk assessment of sites (road slopes and road crossing streams).

3.3.3 Road Slope Assessment Sheet

(1) Outline

Assessment sheet for risk & feasibility of structural measures (excel spread sheets) are prepared. Figure 3.3.5 shows the flowchart of road slope assessment sheet. Table 3.3.1 shows items required for the assessment of risk and feasibility of structural measures.





Sheet No. Name	Description/data
(contents of work)	
	Risk Assessment of Sites
Sheet 1.	- Location of site (km post, right/left of road, expected hazard type)
General Information	- Photographs of site (slope/stream) situation
(Screening/	- FRCDa: Actual frequency of RCD of a site [RCD/ year]
identification of sites to	-FRCDabm: Actual frequency of RCD before structural measures of a site
be surveyed)	[RCD/year]
Sheet 2.	- Check sheet of hazardous factor items and their categories (item groups are
Potential Frequency of	geometry, surface situation, and disturbance), and existing structural measures.
RCD (FRCDp)	- Evaluation results of disaster frequency
(Disaster frequency	FRCDpom: FRCDp without existing structural measures [RCD/year]
assessment)	CEM: Coefficient of Effectiveness of structural Measures [ratio]
	FRCDp: Potential Frequency of RCD [RCD/year]
	= FRCDpom x CEM
Sheet 3.	- Sketch of hazard situation and risk object
Potential Disaster	- Evaluation of disaster magnitude
Magnitude and Annual	LRCpoF: potential Length of Road Closure section of Full width [m]
Loss	LRCpoP: potential Length of Road Closure section of Partial width [m]
(Disaster magnitude	- Evaluation of annual losses
identification and risk	RCp: potential reopening cost of a RCD
estimation)	HLLp: potential value of human lives loss of a RCD
	VLp: potential value of vehicles loss of a RCD
	LTSp: Potential Value of Losses of Traffic Suspension of a RCD
	Lp: potential loss of a RCD
	ALp: potential annual loss of a site
	Feasibility Assessment of Structural Measures of Sites
Sheet 4	- Plane layout of structural measures
Planning of Structural	- Section layout of structural measures
Measures	- Cost
(planning of structural	C: cost estimation with 20 years maintenance [Rs]
measure and feasibility	
assessment)	- Benefit /outcome
4-1 Alternative I	RRR: risk reduction ratio in RCD due to structural measures [Ratio]
High risk reduction	DAL: Decrease in annual loss due to structural measures [Rs/year]
	FRCDpwm: Potential frequency of road closure disaster with structural
4-2 Alternative II	measures [RCD/year]
Medium risk reduction	
	-Feasibility Indicators
4-3 Alternative III	BCR: Benefit/cost ratio at 12% discount rate [ratio]
Low risk reduction	ENPV: Economic net present value at 12% discount rate [Rs]
	EIRR: Economic internal rate of return [percent]
	Disaster Record
Sheet 5	- Disaster occurrence date
Disaster Record	- Disaster magnitude
(records of when	- Damage: road closure days, reopening cost, human loss if any, vehicle loss if
disasters occur after the	any
Inventory Survey)	- Existing countermeasures

RCD: Road closure disaster

From results of risk assessment survey, high risk sites (road slopes/crossing streams) were selected as priority sites for structural measures. Feasibility assessment of the planned structural measures were done (layout and cost estimate).

(2) Road Slope Assessment Sheet 1: General Information

First of all, the sites where the assessment should be done were specified. Road slope is divided into three slope types (mountainside slope, crossing stream, and riverside slope). Road slope types and their screening criteria for risk assessment are shown in Table 3.3.2.

Slope Type	Screening Criteria
Mountainside slope	Gradient of mountainside slope $> 10^{\circ}$
	And
	Distance from road to toe of mountainside slope < 10 m
Crossing stream	Wide of crossing stream $< 3 \text{ m}$
Riverside slope	Gradient of mountainside slope $> 10^{\circ}$
	And
	Distance from road to toe of mountainside slope $< 5 \text{ m}$

In 'Sheet 1 General information', following data were arranged.

- Location of site (km post, right/left of road, slope type, expected hazard type)

- Slope type
- Photographs of site situation
- FRCDa: Actual frequency of RCD of a site [RCD/year]

-FRCDabm: Actual frequency of RCD before structural measure of a site [RCD/year]

FRCDa is a current state value (if structural measure is constructed, FRCDa is value of after construction term).

Figure 3.3.6 shows example of road slope assessment sheet 1: general information.



Figure 3.3.6 Example of Assessment Sheet1 General Information

(3) Road Slope Assessment Sheet 2: Potential Frequency of RCD

(a) Calculation Method of Potential Frequency of RCD

'Sheet 2 Potential Frequency of RCD' was prepared on three different formats for (1) Mountainside Slope, (2) Crossing Stream and (3) Road and Riverside Slopes.

The road slope inventory survey can provide FRCDp as a risk level indicator as shown in Figure 3.3.7. FRCDp is calculated using the following formula.

FRCDp = FRCDpom x CEM

FRCDpom = \sum FS

Where:

FRCDp = Potential frequency of Road Closure Disaster of a site [RCD per year]

FRCDpom = FRCDp of a site without structural measures [RCD per year]

CEM = Coefficient of effectiveness of structural measures effectiveness for FRCDp [coefficient]

FS= Frequency score for FRCDp (FS is assigned to each factor category of each factor item for FRCDp) [RCD per year]

Factor items for FRCDp are set with all considerable factors; factor categories are set from 2 to 4 categories, by referring Japanese road slope inspection manual (Ministry of Construction Japan 1996).

The Study on Disaster Risk Management for Narayangharh-Mugling Highway

Main Report

Potential frequency of PCD	(FPCDn)	ĺ	FS: I item	Frequency sc	ore of the factor		
	(FKCDp)	·			······		
Factor items for FRCDp	Factor categorie	es for FRCDp	Fitted	category		Frequen score fo	r r
						ctor Frequenc score for FRCDp RCD/ye FS1 A FS2 FS3 FS4 FS5 FS6 FS6 FS6 FS7 FS7 FS7 FS8 FS8 FS8 FS8 FS8 FS8 FS8 FS8	ear]
		Geometry					
Wide of stream: W Frequency score for FRCDp [RCD/year]	3≥W 0.06	5≥W	0.00	10≥W>5 0.00	W>10 0.00	FS1	▶ 0.00
	0	0		1	0		
Area of drainage basin: A Frequency score for FRCDp [RCD/year]	A≥0.5km ² 0.00	0.5km ²	>A≥0.1 -0.05	5km ²	-0.07	FS2 >	-0.05
	0		1		0		
Area of drainage basin: A Frequency score for FRCDp [RCD/year]						FS3 FS4 FS5	0.04 0.05 0.03
	Su	rface situation	1				
Dominant vegetation of drainage area Frequency score for FRCDp	Bare 0.20	Grasses 0.09		Trees 0.09	Unknown 0.07		0.09
[RCD/year]	0	0		1	0		
Dominant materials of stream	Gravel	U Sand		1 ht:::Charg	Bedrock		
Frequency score for FRCDp [RCD/year]	0.13	0.01	51	0.01	0.01	FS7 >	0.13
	1	0		0	0		
		Disturbance					
Slope failure situation in drainage	Newly-for	Newly-fo		Newly-for	Newly-for		
area	med	rmed	1	med	med		
	are	are		are	are existing		
Frequency score for FRCDp [Nos. of	existing in	existing		existing	only in		
RCD/year]	main	only in	(only in	branch	FS8	0.05
	valley and	main	1	branch	valleys		
	branch	valley		valleys			
	valleys	0.07			0.07	······	···· >
	0.06	0.06		0.05	0		
Trace of debris on or baside the read	U Trace of dobri	0		1	0		
Frequency score for FRCDp [Nos. of RCD/year]	the r	s on or beside oad		If there ar categories, Faitem is 0.00	e no fitted S of a factor	F 	0.00
		0	····.				
		FRCDpom: F	RCDp	without exis	ting countermeas	ure [RCD	/ year]
				F	$racDpom = \sum (FS)$	1:FS9)	0.34
Existing structure	l measure-type (I	Description)			CEM: Coe	efficient o of struct	of ural
Small about dam (5 m haight v 2 nos.)	Center Cem	is input by o lering predictiv	engineer /e haza	ing judgmen rd magnitude	mea	sure	
Sman check dam (5 m height x 2 hos.)	, Causeway and en	xisting structure'	's situatio	on	CEM		0. 04
				FRCDp	of survey site [nos	s. of RCD	/ year]
				FRO	CDp = FRCDpom	x CEM	0.01

Figure 3.3.7 Calculation Procedure for FRCDp in Road Slope Assessment Sheet 2

(b) Calculation Method

The most suitable frequency scores (FSs) were analyzed by multivariate statistical analysis : minimizing the residual sum of squares between actual value (FRCDabm) and the predicted value (FRCDpom), as shown in figure 3.3.8 and Appendix 3.

Where:

FRCDabm= Actual frequency of RCD of a slope before structural measure is installed [no. of RCD per year]

FRCDpom= potential frequency of RCD of a slope without structural measures [RCD /year]



FRCDabm (Actual Frequency of Road Closure Disaster before structural [RCD/year]

Figure 3.3.8 Illustration for Searching Most Suitable Frequency Scores

The analyzed frequency scores are shown in Figure 3.3.9, 3.3.10, 3.3.11.

The bigger factor scores are relatively dangerous factor categories in specific factor item.

Score range is difference between maximum and minimum frequency score of factor categories in specific factor item. A bigger score range shows that a factor item has relatively big affection on RCD occurrence.

Table 3.3.3 summarized highly affecting factor items for RCD of each slope types (mountainside slope, riverside slope, and crossing streams).

	8,	8	
Order of top three highly	Mountain side slope	Crossing stream	River side slope
affecting factor items			
1	- Distance from road	- Height from stream	- Distance from
	to mountainside slope	bottom to road	road to toe of
			mountain side slope
2	- Gradient of slope	- Dominant vegetation of	- Gradient of slope
		drainage area	
3	- Road section length	- Dominant materials of	- Road section
	of survey slope	stream sediment at road	length of survey
		crossing	slope

 Table 3.3.3 Highly Affecting Factor Items for RCD

	Factor Items		Frequence	y Score (1	RCD/year)		Scor	e Range (RCD)	/year)	
				-0.05	0.00	0,05		0.00	0.05	0,10
	Geometry: Factor Item Gro	ups l	Score for yes							
	Boad section length of summer	$L \ge 300 \text{ m}$	0.07							
	slope: L	$200 \text{m} \ge L \ge 200 \text{ m}$	-0.02				0.09			
	anope. E	100m > L	-0.02							
		$H \ge 90 \ m$	0.05							
	Height of mountain side slope: H	$90m \ge H \ge 60 m$	0.04				0.03			
	· · ·	$60m > H \ge 30m$	0.03							
		$G \ge 60^{\circ}$	0.02							
	Cradient of slopes C	$60^\circ > G \ge 40^\circ$	0.00		Accord 1146 cold and a cold 1146		0.10			
	Gradient of slope: G	$40^\circ > G\!\!\geq 20^\circ$	-0.05				0.10			
		$20^\circ > G$	-0.05							
ice	Distance from road to toe of	IM > D 3m > D > 1m	0.07							
cho	mountainside slope : D	$5m \ge D > 3m$ $5m \ge D > 3m$	-0.04				0.11			
â	-	D > 5 m	-0.04							
049	Surface situation: Factor Ite	em Groups II	Score for yes							
cat		Valley type Straight type	0.02							
ne	Slope shape	Ridge type	0.05				0.03			
0		Combined type	-0.01							
		Bare	0.07							
	Dominant vegetation	Grasses	0.03				0.07			
	Dominant vegetation	Surface protection	0.03				0.01			
		bv	0.00							
		Silt, Clay	0.02							
		Sand	0.02							
	Dominant materials of slope	Cobbles, or	-0.03				0.07			
	surface	Fractured rock	0.03				0.07			
		Weathered rock	0.03							
		Soft fresh rock	0.02							
	a	Yes	0.04				0.02			
	Spring is present	No	0.00				0.03			
	Surface water is present	Yes	0.02				0.02			
		No	0.00							
	Erosion is present	No	0.02		NCCC200111188		0.02			
	Slide configuration is lapping	Yes	0.02				0.02			
	over the road	No	0.00				0.02			
	Disturbance: Factor Item G	Vac	Score for yes			<u>.</u>				
	Collapse/ Fall	No	0.00				0.01			
	Continuous Cracks (more than 5	Yes	0.01				0.01			
ice	meter), Crevices on Slope	No	0.00				0.01			
1040	Fallen/ Inclined trees	Yes	0.07				0.07			
10	0 h	Yes	0.00				0.01			
2	Open cracks below an over hang	No	0.00				0.01			
ese	Open cracks by toppling	Yes	0.02				0.02			
<u>م</u> ا	Cross open cracks to cause	Yes	0.00							
	wedge shape slide	No	0.00				0.01			
	Sliding direction open cracks	Yes	0.01				0.01			
	Vertical Crakes on Retaining	No	0.00	+ + + +						
	Wall	No	0.07				0.07			
	Continuous Cracks (more than 5	Yes	0.03				0.02			
	meter), Crevices on Road	No	0.00				0.03			
	Continuous Cracks retaining	Yes	0.02				0.02			
	wall and Road	No	0.00						· · · · · · · · · · · · · · · · · · ·	
	Upheaval on Road	No	0.02				0.02			

Figure 3.3.9 Frequency Scores of Mountainside Slope

	Factor Items		Frequenc	y S	Sc	ore	(Re	CD/y	/ea	ar)						Sco	re l	R٤	ing	ge (R	CD)/y	ear)			
						-0,	20	-0, 1	0	(),0()	().10			0.0	0	0.	05	0.	10	0.	15	0.2	20	0.2	25
	Geometry: Factor Ite	em Groups I	Score for yes																									
		$3 m \geq W$	0.06																									
	Width of stream: W	$5 \text{ m} \ge W \ge 3 \text{ m}$	0.00													0.06												
		$5 \text{ m} \ge W \ge 3 \text{ m}$	0.00																									
		W > 10 m	0.00						1			.											++				!	
	Area of drainage basin:	$A \ge 0.5 \text{ km}^2$	0.00													0.07												
	Α	$0.5 \text{ km}^2 > \text{A} \ge 0.15 \text{ km}^2$	-0.05													0.07												
		0.5 km > A	-0.07						+					+							++	$\left \right $	+		+			
	Gradient of stream at	$0 \ge 20$ $20^\circ > C > 15^\circ$	0.07									1																
	road crossing: G	$15^{\circ} > G \ge 10^{\circ}$	0.05													0.03												
	roud crossing. o	$10^\circ > G$	0.04									Ş.																
		$G \ge 40^{\circ}$	0.00				1	1	1	- 69		1											$^{++}$					
	Steepest gradient of	$40^\circ > G \ge 30^\circ$	-0.03													0.04												
	stream : G	$30^\circ > G \ge 15^\circ$	-0.03													10.00												
		$15^{\circ} > G$	-0.06																									
		$I m \ge H$	0.02								101105																	
ice	Height from stream	$2 m \ge H > 1 m$	0.02							-	1980BE					0 30							į.					
o_{i}	bottom to road : H	$5 m \ge H \ge 2 m$	-0.01				Ш.,									0.00												
r_{c}		H > 5 m	-0.28	Ш											Ш.													
102	Surface situation: Fa	ctor Item Groups II	Score for yes				1				SERT	aene	1003	1000	1636166								+				ĺ	
tte	D 1 <i>i i i i i</i>	Bare	0.20									1	11															
50	Dominant vegetation of	Grasses	0.09													0.13												
)ne	uramage area	Hinknown	0.09																									
<u> </u>		Boulders Cobbles	0.07			+++		1	+				1	841									+		+			
	Dominant materials of	Sands	0.01								350) 																	
	stream sediment at road	Silt. Clay	0.01													0,13												
	crossing	Bedrock	0.00							#*											Ì						I.	
	Disturbance: Factor	Item Groups III	Score for yes	1		T	1	1	1	1		1					1						T		1		1	
		Newly-formed collapse								l.																		
		are existing in main	0.06							1		ł																
		valley and branch valleys										÷.																
		Newly-formed collapses																										
		are existing only in main	0.06							1		111																
	Slope failure situation	valley										t i				0.07												
	lm drainage area	Newly-formed collapses														10.07												
		are existing only in	0.05									10001																
		branch valleys																										
		Marily formed collenses																										
		newly-formed conapses	-0.01																									
		are not recognized																										
ce.				T					Π						Π			Π	T		T		T		T			
loi		Yes	0.01							1000																		
2	Trace of debits on or															0.07												
N_{c}	beside the road									8						0.01												
2		No	0.00																									
'es		NU	0.00																									
\sim								11																				

Figure 3.3.10 Frequency Scores of Crossing Stream

	Factor Items		Frequenc	y Score (RCD/yea	r)	Score Range (RCI	D/year)	
				-0.10	-0.05	0.00 0.05	0.00	0.05	0.10
	Geometry: Factor Item Grou	os I	Score for yes						
	Road section length of survey slope: L	$L \ge 300 \text{ m}$ $300\text{m} > L \ge 200 \text{ m}$ $200\text{m} > L \ge 100 \text{ m}$ 100m > L	0.01 0.01 0.00				0.01		
	Height of Valley side slope: H	$H \ge 90 \text{ m}$ $90\text{m} > H \ge 60 \text{ m}$ $60\text{m} > H \ge 30 \text{ m}$ $30\text{m} \ge H$	0.05 0.05 0.04 0.03				0.02		
	Gradient of valley side slope	$ \begin{array}{l} \mathbf{G} \geq \mathbf{60^\circ} \\ \mathbf{G} \geq \mathbf{60^\circ} \\ \mathbf{60^\circ} \geq \mathbf{G} \geq \mathbf{40^\circ} \\ \mathbf{40^\circ} \geq \mathbf{G} \geq \mathbf{20^\circ} \\ \mathbf{20^\circ} \geq \mathbf{G} \\ \end{array} $	0.02 0.02 0.00 0.00				0.02		
	Distance from road to head of river side slope	$\begin{array}{l} 1 \ m \geq D \\ 3m \ \geq \ D \geq \ 1m \\ 5m \ \geq \ D \geq \ 3m \\ D \geq \ 5 \ m \end{array}$	0.00 -0.01 -0.06 -0.10				0.10		
9	Distance from low water of river to road: D	$\begin{array}{l} 0.5 \ m \geq D \\ 1.0m \ \geq D > \ 0.5m \\ 2.0m \ \geq D > \ 1.0m \\ D > \ 1.0m \\ D > \ 2.0m \end{array}$	0.00 0.00 0.00 0.00				0.00		
gory choic	Height from high water of road to road surface or head of revetment: H	$0.0m \ge H$ $1.0m \ge H \ge 0.0m$ $2.0m \ge H \ge 1.0m$ $H \ge 2m$	0.03 0.03 0.03 0.00				0.03		
One cate	Slope shape	Valley type Straight type Ridge type Combined type	0.03 0.03 0.03 0.04				0.01		
	Surface situation: Factor Iten	n Groups II	Score for yes						
		Grasses	-0.01						
	Dominant vegetation	Trees Surface protection by concrete/stone/block	-0.07 -0.07				0.06		
	Sløpe typer	Embankment slope Combined or unknown	0.10				0.08		
		Natural slope	0.02						
	Dominant materials of slope surface	Silt, Clay Sand Gravels Cobbles, or Boulders Fractured rock Weathered rock Soft fresh rock Hard fresh rock Surface protection by concretestone-block	-0.01 -0.01 -0.04 -0.06 -0.06 -0.06 -0.06 -0.06 -0.06				0.05		
\vdash	Dip slope structure (bedding	Yes	0.05				0,05		
	Soil covering impervious bedrock	Yes	0.05				0.05		
	The rocks are hard at upper part and soft at foot part	Yes	0.00				0.00		
	The rocks are soft at upper part and hard at foot part	Yes	0.03				0.03		
	Spring is present	Yes No	0.07				0.07		
	Surface water is present	Yes No	0.00				0.00		
	Rainwater flow from road to valley side is present	Yes No	0.02				0.02		
ice	Slide configuration is lapping over	Yes	0.05				0.05		
Cho	Disturbance: Factor Item Gro		0.00 Score for ves					***	
No	Erosion is present	Yes	0.01				0.01		
ss or	Piping hole is present	Yes	0.00				0.00		
ľ	Fall Channels and the l	No Yes	0.00				0.05		
	Fail, Slump in riverside slope	No Yes	0.00				0.05		
	Depression on road	No	0.00				0.05		
	Cracks/Crevices on road	No	0.00			per l'And Edd (1991) BERNER	0.05		

Figure 3.3.11 Frequency Scores of Riverside Slope

(c) Coefficient of Effectiveness of Structural Measures (CEM)

The assessment surveyor evaluated the effect of the measures on FRCDp as CEM. CEM is coefficient for calculation of FRCDp as shown in the expression given below.

where

FRCDp = FRCDpom x CEM

FRCDp = Potential frequency of RCD [RCD/year]

FRCD = Potential frequency of RCD without countermeasures [RCD/year]

CEM = Coefficient of effectiveness of structural measures

Table 3.3.4 shows average CEM of Philippines national highway as a reference.

CEM is not set by measure-type. It is differs with strength, scale of structural measures and magnitude of hazard. CEM was set by engineering judgment of assessment surveyor. CEM for multiple measure-types, it is also evaluated by engineering judgment considering compound effect.

Structural measure type	CEM	Structural measure type	CEM
Mountainside slope		Crossing stream	
Catch wall	0.2	Small sabo dam (less than 10 m height	0.2
Retaining wall	0.1	Riverside slope	
Slope protection by vegetation	0.4	Road drainage	0.05
(countries)		Retaining wall/Revetment	0.05

 Table 3.3.4 Example of Average CEM (Philippines National Highway)

(d) Format of Road Slope Assessment Sheet 2: Potential Frequency of RCD

Three different formats of road slope assessment sheet 2: were used for assessment of potential frequency of RCD by slope types (mountainside slope, crossing stream and riverside slope) and are shown in Figure 3.3.9, 3.3.10, and 3.3.11.

Narayangharh-Mugling Highway

Road Name

Left side of road	Km	2	m		
Dn)					
Factor categories for	r FRCDp			Frequency sc. [RCI	ore for FRCD /year]
	Geometry				
L ≥ 300 m 0.07	300 m > L ≥ 200 m 0.02 0	200 m > L ≥ 100 m -0.02 0	100 m > L. -0.02	FSI	(0.02
$H \ge 90 \text{ m}$ 0.05	$90 \text{ m} > \text{H} \ge 60 \text{ m}$ 0.04	60 m ≥ H ≥ 30 m 0.03	30 m > H 0.02	FS2	0.00
$G \ge 60^{\circ}$ 0.05	$60^{\circ} \ge G \ge 40^{\circ}$ -0.05	40° > G≥ 20° -0.05	20° > G -0.05	FS3	0.00
1 m > D 0.07	3 m ≥ D> 1m 0,00	5 m ≥ D > 3 m -0.04	D > 5 m -0.04	FS4	0.00
Valley type 0.02	0 Straight type 0.03 0	Ridge type 0.00	Combined type -0.01	FS5	0.00
-	Surface situa	tion			
Bare 0.07	Grasses 0.03	Trees 0.03	Surface protection by concrete/stone/block 0.00	FS6	0.00
Silt, Clay	Sand	Gravels	Cobbles, or Boulders		-
0.02	0.02	0.02	-0.03	ES7	0.00
Fractured rock	Weathered rock 0.03	Soft fresh rock 0.02	Hard fresh rock 0.04	ulders -0.03 0 FS7 ock 0.04 0 soft at hard rt 0.03 0 ration er the 0.02	0.00
Dip slope structure (bedding plane) is present 0.05	Soil covering impervious bedrock 0.05	The rocks are hard at upper part and soft at foot part 0.00	The rocks are soft at upper part and hard at foot part 0.03	FS8	0.00
0 Spring is Present 0.03	Surface Water is Present 0.02	Erosion is Present	0 Slide Configuration is lapping over the 0.02	FS9	0.00
0	0	0	0		
Collapse/ Fall 0.01	Continuous Cracks (more than 5 meter), Crevices on Slope 0.01	e Fallen/ Ino 0.	clined trees		
0 Open cracks below an over hang 0.01	Open cracks by toppling 0.02	Cross open cracks to cause wedge shape slide 0.01	0 Sliding direction open cracks 0.01	FS10	0.00
0 Vertical Crakes on Retaining Wall	Continuous Cracks (more than 5 m), Crevices on Road	Continuous Cracks retaining wall and Road	0 Depression/ Upheaval on Road		
0.07	0.03	0.02	0.02		
	FRCDpo	m: FRCDp withou	it existing structur	al measure	[RCD/year]
			FRCDpom = Σ	(FSI:FSI0)	0.00
(Description)			CEM. Coe	struct CEM	ural measure
			FRCDp = FRCDp	FRCDp:	[RCD/year]
] 1 should be input to] 1 should be input w Numerical value or	selected category's ce hen corresponding to term is automatically	ll. situation. input.			
	Left side of road Total State of Pactor Categories for Eactor categories for $L \ge 300 \text{ m}$ 0.07 0.07 0 $H \ge 90 \text{ m}$ 0.05 0 $G \ge 60^{\circ}$ 0.07 0 1 m > D 0.07 0 0.07 0 0.07 0 0.07 0 0.07 0 0.02 0.03 0.03 0.03 0.03 0.03 0.03 0.05 0.03 0.03 0.05 0.05 0.03 0.05 0.01	Left side of road Geometry Geometry L \geq 300 m 300 m > L \geq 200 m 0 0 0.02 0 90 m > H \geq 60 m 0.04 0 0 0 0 H \geq 90 m 90 m > H \geq 60 m 0.04 0 0 0 0 G \geq 60° 60° > G \geq 40° 0.05 -0.05 0 0 3 m \geq D > 1 m 0.07 0.00 0 0 3 m \geq D > 1 m 0.07 0.00 0 0 3 m \geq D > 1 m 0.07 0.00 0 0 0 0 0 0 Valley type 0.02 0.03 0 0 0 0.07 0.03 0 0 0 Stilt. Clay Sand 0.02 0.03 0 0 0 Fractured rock Weathered rock 0.03 0.03 0 0 0 0 0 0 0 0 0 0 0 0 0 <td>Left side of road Geometry Geometry L $\geq 300 \text{ m}$ 300 m > L $\geq 200 \text{ m}$ 200 m > L $\geq 100 \text{ m}$ 0.07 0.02 0.02 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0</td> <td>Tert side of road</td> <td>Tech sade of road Frequency see [RCT Frequency see [RCT Commetry L $\geq 300 m$ $300 m > L \ge 200$ $200 m > L \ge 100 m$ $100 m > L$ -0.02 [FS1] H $\ge 90 m$ $90 m > H \ge 20 m$ $60 m > H \ge 30 m$ $30 m > L$ -0.02 [FS1] H $\ge 90 m$ $90 m > H \ge 00 m$ $60 m > H \ge 30 m$ $30 m > L$ -0.02 [FS3] G $\ge 20^{0}$ $60^{0} \Rightarrow -6 \ 2 \ 40^{0}$ $40^{0} \Rightarrow (22 \ 20^{0} > 6$ $20^{0} \Rightarrow 6$ 00^{0} $00^{0} = 0^{0} 0^{0} = 0^{0} 0^{0}$ $00^{0} = 0^{0} 0^{0} = 0^{0}$</td>	Left side of road Geometry Geometry L $\geq 300 \text{ m}$ 300 m > L $\geq 200 \text{ m}$ 200 m > L $\geq 100 \text{ m}$ 0.07 0.02 0.02 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Tert side of road	Tech sade of road Frequency see [RCT Frequency see [RCT Commetry L $\geq 300 m$ $300 m > L \ge 200$ $200 m > L \ge 100 m$ $100 m > L$ -0.02 [FS1] H $\ge 90 m$ $90 m > H \ge 20 m$ $60 m > H \ge 30 m$ $30 m > L$ -0.02 [FS1] H $\ge 90 m$ $90 m > H \ge 00 m$ $60 m > H \ge 30 m$ $30 m > L$ -0.02 [FS3] G $\ge 20^{0}$ $60^{0} \Rightarrow -6 \ 2 \ 40^{0}$ $40^{0} \Rightarrow (22 \ 20^{0} > 6$ $20^{0} \Rightarrow 6$ 00^{0} $00^{0} = 0^{0} 0^{0} = 0^{0} 0^{0}$ $00^{0} = 0^{0} 0^{0} = 0^{0} $

Road Slope Assessment Sheet 2-1: Potential Frequency of RCD (Mountainside Slope)

Figure 3.3.12 Road Slope Assessment Sheet 2-1: Potential Frequency of RCD (Mountainside Slope)

Road name	Narayangharh-Mug	gling Highway		luis.		
Side of the site	Left side of road					
Potential frequency of RCD (FRC	Dn)					
Factor items for FRCDp	tor items for FRCDp Factor categories for FRCDp				FS: Frequency score for FRCDp	
		Geometry			IN	conjean
Width of stream: W	$3 m \ge W$	$5 \text{ m} \ge \text{W} > 3 \text{ m}$	$10 \text{ m} \ge \text{W} > 5 \text{ m}$	W > 10 m		Jan
Frequency score for FRCDp [RCD year]	0.06	0.00	0.00	0.00	FS1	0.06
Area of drainage basin : A Frequency score for FRCDp [RCD year]	$A \ge 0.5 \text{ km}^2$ 0.00	0.5 km ² > A -0	$\begin{array}{c c} 0 & 0 \\ \hline 0 & 0 \\ 0 & 0.15 \text{ km}^2 \\ 0.5 & -0.07 \\ 0 & 0 \end{array}$		FS2	0.00
Gradient of stream at road crossing: G Frequency score for FRCDp [RCD year]	G ≥ 20 ° 0.07	$20^{\circ} > G \ge 15^{\circ}$ 0.06	$15^{\circ} > G \ge 10^{\circ}$ 0.05	10° > G 0.04	FS3	0.00
Steepest gradient of stream: G Frequency score for FRCDp [RCD year]	G ≥ 40 ° 0.00 0	40° > G ≥ 30 ° -0.03	30° > G ≥ 15 ° -0.03 0	15°>G -0.06	FS4	0.00
Height from stream bottom to road: H Frequency score for FRCDp [RCD year]	1 m ≥ H 0,02	$2 \text{ m} \ge \text{H} > 1 \text{ m}$ 0.02	$5 \text{ m} \ge \text{H} > 2 \text{ m}$ -0.01	H > 5 m -0.28	FS5	0.00
	0	Surface situati	on	0	-	
Dominant vegetation of drainage area Frequency score for FRCDp [RCD/year]	Bare 0.20	Grasses 0.09	Trees 0.09	Unknown 0.07	FS6	0.00
Dominant materials of stream sediment at road crossing	Cobbles, Boulders, Gravel	Sand	Silt, Clay	Bedrock		
Frequency score for FRCDp [RCD year]	0,13	0.01	0.01	0,00	FS7	0.00
	0	0	0	0	_	
	No. 1. Consul	Disturbance				
Slope failure situation in drainage area	collapses are existing in main valley and branch valleys	Newly-formed collapses are existing only in main valley	Newly-formed collapses are existing only in branch valleys	Newly-formed collapses are not recognized	FS8	0.00
Frequency score for FRCDp [RCD/year]	0.06	0,06	0.05	-0.01	0	
Trace of debris on or beside the road	Trace of debris	on or beside the	-			
Frequency score for FRCDp [RCD year]	rc 0.	0 0 0 0			FS9	0.00
						1
				FRCDpom = Σ (F	S1:FS9)	0.06
Existing structural measure-type (Description)			CEM: Coefficie structu	ent of eff iral mea	fectiveness of sure
					CEM	IDCD/www.l
			FR	CDn = FRCDnom	x CEM	[KCD/year]
			TR	ebp - ricebpoin	ACLIVI	0.00
Note	1 should be input 1 should be input Numerical value Numerical value	to selected category when corresponding or term is automatica should be input (by e	's cell. g to situation. ally input. engineering judgmen	0.		

Road Slope Assessment Sheet 2-2: Potential Frequency of RCD (Crossing Stream)

Disturbance: deformation and collapses that do not close the road is not included in RCD and are called 'disturbance'.

Figure 3.3.13Road Slope Assessment Sheet 2-2: Potential Frequency of RCD (Crossing Stream)

Road Name	Narayangharh-Mugling	g Highway				
Station from Side of Survey	Left side of road	km	.2	m		_
Potential frequency of BCD (EDC	Do)					
Factor items for FRCDp	Factor categories for	FRCDp			Frequen FF	cy score for CDp D/yearl
		Geometry		1		27 Jean
Road section length of survey slope: L Frequency score for FRCDp [RCD year]	L. ≥300 m 0.01 0	300m > L ≥ 200 m 0.01 0	200m > L ≥100 m 0.00 0	100m > L 0.00	FS1	0.00
Height of Valley side slope: H Frequency score for FRCDp [RCD year]	H≥90 m 0.05	90m > H ≥ 60 m 0.05 0	60m > H ≥ 30 m 0.04 0	30m > H 0.03	FS2	0.00
Gradient of river side slope Frequency score for FRCDp [RCD year]	$G \ge 60^{\circ}$ 0.02	$60^{\circ} > G \ge 40^{\circ}$ 0.02	$40^{\circ} > G \ge 20^{\circ}$ 0.00	20° > G 0.00	F\$3	0,00
Distance from road to head of river side slone Frequency score for FRCDp [RCD year]	1 m ≥ D 0.00	3m ≥ D > 1m -0.01	5m ≥ D > 3m -0.06	D > 5m -0.10	FS4	0.00
Distance from low water of river to road: D Frequency score for FRCDp [RCD year]	0.5 m ≥ D 0.00	1.0m ≥ D > 0.5m 0.00	2,0m ≥ D > 1.0m 0,00	D > 2.0m	FS5	0.00
Height from high water of road to road surface or head of revetment: H Frequency score for FRCDp [RCD year]	0.0m ≥ H 0.03	0 1.0m ≥ H > 0.0m 0.03	0 2.0m >H≥ 1.0m 0.03	H> 2m	FS6	0.00
Slope shape Frequency score for FRCDp [RCD year]	Valley type 0.03 0	Straight type 0.03 0	Ridge type 0.03 0	Combined type 0.04 0	FS7	0.00
		Surface situation	on			
Dominant vegetation Frequency score for FRCDp [RCD year]	Bare -0.01	Grasses -0.05	Trees -0.07	Surface protection by concrete/stone/block -0.07	FS8	0.00
Slope type Frequency score for FRCDp [RCD year]	Embankment slope 0.10	Combined or unknown 0.02	Natural slope 0.02		FS9	0,00
Dominant materials of slope surface Frequency score for FRCDp [RCD year]	Silt, Clay -0.01 0 Eractured rock	Sand -0.01 0 Soft fresh rock	Gravels, Cobbles, or Boulders -0.04 0 Hard fresh rock	Weathered rock -0.06 0 Surface protection by	FS10	0.00
Frequency score for FRCDp [RCD year]	-0.06 0	-0,06	-0.06	concrete/stone/block -0.06 0		_
Spring/ Surface water /Rainwater flows/ Frequency score for FRCDp [RCD year]	Spring is present 0,07	Surface water is present 0.00	Rainwater flow from road to valley side is present 0,02	Slide configuration is lapping over the road 0.05	FS11	0.00
		Disturbance				
Erosion in valley side slope Frequency score for FRCDp [RCD year]	Erosion is present 0,01 0	Piping hole is present 0.00 0			FS12	0.00
Deformation/ Collapse on the slope Frequency score for FRCDp [RCD year]	Fall, Slump in 0.	river side slope 05 0	Depression on road 0,05 0	Cracks/Crevices on road 0,05 0	FS13	0.00
		FRCDpom	: FRCDp without	existing structural	measure	RCD/year
				FRCDpom = Σ (F	S1:FS13)	0.00
Existing structural measure-type ((Description)			structur	al measur CEM	e
				ERCDa = ERCD	RCDp:	RCD/year]
Note	I should be input to s I should be input wh Numerical value or to Numerical value shou Torme chould be in	selected category's cell. en corresponding to situ rrm is automatically inp uld be input (by engined	uation. ut. rring judgment).		III A CEIVI	0,00
Disturbance: deformation and collapses t	Terms should be input hat do not close the road	it. I is not included in RCI	D and are called			

Road Slope Assessment Sheet 2-3: Potential Frequency of RCD (Riverside Slope)

Figure 3.3.14 Road Slope Assessment Sheet 2-3: Potential Frequency of RCD (Riverside Slope)

(4) Road slope Assessment Sheet 3: Potential Disaster Magnitude and Annual Loss(a) Calculation Procedure of Disaster Magnitude and Annual Loss of a Site

Risk is defined as a multiplication of disaster frequency and magnitude. In the assessment sheet 3, risk is assessed as annual loss, which is multiplication of FRCDp (disaster frequency) and Lp (disaster magnitude).

Where

FRCDp = potential frequency of RCD of a site [RCD/year]

Lp = potential loss of a RCD [Rs/RCD]

The calculation procedure of annual loss is shown in Figure 3.3.15 and example of the assessment sheet is presented in Figure 3.3.16.



Figure 3.3.15 Calculation Procedure of Potential Annual Loss



Road Slope Assessment Sheet 3: Potential Disaster Magnitude and Annual Loss



(b) Potential Disaster Magnitude

1) Potential Length of Road Closure Section of a RCD

As shown in Figure 3.3.17, at first, the disaster magnitude is evaluated by potential length of road closure section. The definition of length of road closure section of a RCD, which is estimated by a site investigator based on the hazard situation and disaster records in analogous slopes is shown in table 3.3.5.

Item	Symbol	Unit	Definition
Potential length of road closure section of full width of a RCD	LRCpoF	m	No traffic lane is secured
Potential length of road closure section of partial width of a RCD	LRCpoP	m	At least one traffic lane is secured

Table 3	3.3.5	Definition	of length	of road	closure	section of	a RCD
---------	-------	------------	-----------	---------	---------	------------	-------

Assessment surveyors evaluated the length of road closure section by observing hazard condition and referring to past disaster magnitude of similar slopes.

2) Potential Loss of a RCD (Lp)

a) General

Potential loss of a RCD (Lp) is automatically calculated when the length of road closure section is input in the road slope assessment sheet 3. Lp is a function of nos. of RCD and length of road closure section. And Lp is evaluated at 2007 value.

The unit values or formulas for potential loss calculation is determined using statistical data, existing studies reports of roads in Nepal, and passengers interview surveys including origin-destination surveys conducted in September 2007 under this study. These analyses detail are shown in Appendix 4. The analyzed results are summarized and presented.

Potential loss (Lp) is calculated by summing up four components as shown in Table 3.3.6.

Lp=Potential Loss of a	RCp =Potential Reopening Cost of a RCD
RCD	[Rs/RCD]
[Rs/ RCD]	HLLp=Potential value of Human Life Loss of a RCD
	[Rs/RCD]
	VLp=Potential value of Vehicle Loss of a RCD
I n = PCn + HI I n + VI n	[Rs/RCD]
Lp = KCp + HLLp + VLp	LTSp=Potential Losses of Traffic Suspension
+LTSp	(detour/waiting/cancellation) of a RCD
	[Rs/RCD]

b) RCp: Potential Reopening Cost per RCD [Rs/RCD]

Formulas and unit cost for potential reopening cost estimation is shown in Table 3.3.7.

Unit cost is derived from the past costs data of N-M Highway.

The fixed cost (FCR) comprises the loader operator's salary and allowances, depreciation of loader and overhead costs for operation of site office.

Variable reopening cost of a RCD are unit reopening cost per one meter length of full width road closure (URCpMoF) or Unit reopening cost per one meter length of partial width road closure (URCpMoP). The variable cost comprises the costs of fuel and oil consumptions and cost of labours, and is calculated by assuming the typical road closure disaster debris accumulation volumes of full width or partial width road closure.

Formula for Loss Estimate	Unit	Unit cost at 2007
RCp=FCR + LRCpoF x URCpMoF + LRCpoP x URCpMoP		
Where		
RCp: potential reopening cost	[Rs/RCD]	
FCR: Fixed cost for reopening per RCD	[Rs/RCD]	31,412
LRCpoF: Potential length of road closure section of full width URCpMoF: Unit reopening cost per one mater length of full width road closure	[m] [Rs/m]	870
LRCpoP: Potential length of road closure section of partial width URCpMoP: Unit reopening cost per one meter length of partial width road closure	[m] [Rs/m]	218

Table 3.3.7 Formula and Unit Cost for Potential Reopening Cost

c) HLLp: Potential Values of Human Lives Lost per RCD [Rs/RCD]

Formulas and unit value for potential values of human lives lost is shown in Table 3.3.8.

ANHD: Average Number of Human Deaths per RCD [persons/RCD]

ANHD in past 10 years (1997 to 2006) is 1/308 (one dead person divided by 308 RCD) [persons/RCD]

Number of RCD is evaluated by the road slope assessment surveys conducted in 2007 under this study, which was based on interviews of DOR staffs and inhabitants along the road.

There were no human lives lost in 10 years, however, in 2003 a truck was buried in the debris at 21 km and the truck driver was badly injured but could escape. The badly injured

driver is considered as a dead person in the calculation.

ANHD should be increased promotionally by traffic volume. Traffic volume at 2007 is evaluated about 1.5 times than that of past 10 years, based on the traffic increase of 80% in 10 years (6% per year) of DOR estimation in 2006. Hence, ANHD at 2007 is evaluated as 1.5/308 [persons/RCD].

UHL: Unit Value of Human Life Lost [Rs/person]

The two main components in determining UHL consists of:

- The number of years/days of work lost due to death, and
- The average annual income of dead person

Table 5.5.6 Formula and Onte value for Fotential value of Human Lives Lost					
Formula for Loss Estimate	Unit	Unit value at 2007			
HLLp=ANHD x UHL	[Rs/RCD]	3,282			
Where					
HLLp: Potential value of human lives lost					
ANHD: Average number of human deaths per PCD	[persons/PCD]	1 5/308			
ANHD. Average number of numan deaths per KCD	[persons/ KCD]	1.3/308			
UHL: Unit Value of Human Life Lost	[Rs/person]	674,000			

Table 3.3.8 Formula and Unit Value for Potential value of Human Lives Lost

d) VLp: Potential Value of Vehicle Loss of a RCD [Rs/RCD]

Formulas and unit value for potential values of vehicle loss is shown in Table 3.3.9.

ANVL: Average Number of Vehicle Lost per RCD [vehicles/RCD]

ANVL in past 10 years (1997 to 2006) is 1/308 (one vehicle lost divided by 308 RCD) [vehicles/RCD].

Numbers of RCD is evaluated by the road slope assessment survey conducted in 2007 under this study, based on interviews of DOR staffs and inhabitants along the road.

One vehicle lost case is in 2003, a truck was buried in the debris at 21 km.

ANVL should be increased promotionally by traffic volume. Traffic volume at 2007 is evaluated about 1.5 times than that of past 10 years, based on the traffic increase of 80% in 10 years (6% per year) of DOR estimation in 2006. Hence, ANVL at 2007 is evaluated as 1.5/308 [vehicles/RCD].

UVL: Unit Value of Vehicle Loss [Rs/vehicle]

The net vehicle damage cost incurred in road accidents is estimated by using following relationship:

Net vehicle damage cost = Average vehicle repair cost

- Custom duties and VAT on spare parts and any salvage

+ Insurance excess (insured vehicles only)

+ Survey fees of accident (insured vehicles only – 10% vehicles)

+ Lost business (commercial vehicles only)

Formula for Loss Estimate	Unit	Unit value at 2007
VLp=ANVL x UVL	[Rs/RCD]	
Where		
VLp: Potential value of vehicle lost		
ANVL: Average Number of Vehicle Loss per RCD	[vehicles/RCD]	1.5/308
UVL: Unit Value of Vehicle Lost	[Rs/vehicle]	147,669

e) Potential Loss of Traffic Suspension of a RCD (LTSp) [Rs/RCD]

Formulas and unit value for potential values of vehicle loss of a RCD is shown in Table 3.3.10.

Formula for Loss Estimate	Unit	Unit value at 2007
$LTSp = AADT \times NCDp \times ASLoV$	[Rs/RCD]	
Where		
LTSp: Potential losses of traffic suspension of a RCD		
AADT: Annual average daily traffic	[vehicles/day]	3,225
NCDp: Nos. of predicted closure days of the whole width of the road on the survey site per RCD	[days]	Parameter
NCDp= 1+ LRCpoP/0.86/24 LRCpoF: Potential length of road closure section of full width [m]		
ASLpV : Average suspension loss per vehicles	[Rs/vehicle]	
If NCDp < 0.1 ,		
ASLpV = 1,580 x NCDp;		
If $0.1 \leq \text{NCDp} < 5.6$,		
ASLpV = 693 x Ln(NCDp) + 1,810;		
If $5.6 \le \text{NCDp}$, ASLoV = 3,030		

AADT: Annual Average Daily Traffic [vehicles/day]

The value of AADT is presented in the report of DOR Feasibility Study Report of N-M Highway prepared in March 2007. The AADT of 2 ways of N-M highway at 2006 is 3041 vehicles/day. In the report, the traffic volume increase is predicted as 6% per year. Therefore, AADT in 2007 is estimated 3,225 vehicles per day.

NCDp: Nos. of predicted closure days of the whole width of the road on the survey site per RCD [day]

NCDp is determined by using following past data on N-M highway.

ARToDV: average reopening time per debris volume = 0.0258 hr/m³

Meanwhile,

TVDoF: Typical volume of accumulated debris of one meter length of full width = $33.47 \text{ m}^3/\text{m}$.

Actual reopening hour per one meter length full width road closure is

ARToDV x TVDoF = 0.0258 hr/m3 x 33.47 m3/m ≈ 0.86 hr/m

Being the full width road closure is a significant disaster; one day should be added for preparation work and security assurance.

Therefore,

NCDp= 1+ LRCpoP/0.86/24

Where

LRCpoF: Potential length of road closure section of full width [m].

ASLoV : Average Suspension Loss per Vehicle [Rs/vehicle]

Average suspension loss per vehicle (ASLoV) is calculated by summing up three components as shown in Table 3.3.11.

ASLpV=Average	AWLpV = Average waiting loss per vehicle
Suspension Loss per	[Rs/vehicle]
vehicle	ADLpV = Average detour loss per vehicle
[Rs/vehicle]	[Rs/vehicle]
ASLpV=AWLpV+	ACLpV= Average cancellation loss per vehicle
	[Rs/vehicle]
ADLpv + ACLpv	

 Table 3.3.11 Composition of Average Suspension Loss per Vehicle

The road users select the option of detour, waiting, or cancellation according to the road closure days. The option is changed by suspension days. In this study the option proposition is determined by passenger interview surveys conducted in September and shown in Figure 3.3.17.



CpoWTP300	Cancellation percentage willing to pay up to Rs 300 instead of waiting (5.0% of non waiting)
CpoWTP150	Cancellation percentage, willingness to pay up to Rs 150 instead of
	waiting (17.0% of non waiting)
CpoWTP<150	Cancellation percentage willingness to pay under Rs 150 instead of
	waiting (42.9% of non waiting)
DPdPB	Detour percentage divert to Pokhara-Butawal (10.8% of no waiting)
DPdNH	Detour percentage divert to Naubise-Hetauda (24.3% of no waiting)
WP	Waiting percentage

Figure 3.3.17 Option Selection Proportion to N-M Highway Road Closure of Full Width

Formulas and unit value for average waiting loss per vehicle (AWLpV) is shown in Table 3.3.12.

Formula for Loss Estimate	Unit	Unit value at 2007
$AWLpV = \int NCDp/2 x 24 x WP x AVTT$		Variable by NCDp
Where		
AWLpV: Average waiting loss per vehicle	[Rs/vehicle]	
NCDp: Nos. of predicted closure days of the whole width of the road on the survey site per RCD	[days]	Parameter
WP: Waiting percentage	[%]	Variable by NCDp as shown in Figure 3.3.17
UVTT: Unit value of traffic time of a vehicle of N-M highway	[Rs/vehicle/ho ur]	130
(waited by vehicle-type proportion of N-M Highway	w.]	
based on unit value of traffic time of each vehicle-type		
of study on North-South Fast track linking Katmandu to		
Terai 2007 by DOR)		

Formulas and unit value for average detour loss per vehicle (ADLpV) is shown in Table 3.3.13.

Formula for Loss Estimate	Unit	Unit value at 200	7
ADLpV = DPdNH x UDLdNH +DPdPB x UDLdPB		Variable by NCD	р
Where			
ADLpV: Average detour loss per vehicle	[Rs/vehicle]		
DPdNH: Detour parentage of divert to Naubise or Hetauda (24.3% of no waiting)	[%]	Variable by NCD as shown in	р
DPdPB: Detour parentage of divert to Pokhara-Butawal (10.8% of no waiting)	[%]	Figure 3.3.17	
UDLdNH: Unit detour loss of a vehicle when divert to Naubise or Hetauda	[Rs/vehicle]	2,4	00
UDLdPB: Unit detour loss of a vehicle when divert to Pokhara or Butawal	[Rs/vehicle]	5,10	00
 Difference of vehicle operation cost and M-H highway and detour and; 			
- Multiplication of travel time increase and UVTT			
NCDp: Nos. of predicted closure days of the whole width of the road on the survey site per RCD	[days]	Parameter	
UVTT: Unit value of traffic time of a vehicle of N-M highway (waited by vehicle-type proportion of N-M highway based on unit value of traffic time of each vehicle-type of study on North-South Fast track linking Katmandu to	[Rs/vehicle/ho ur]	1.	30
Terai 2007 by DOR)			

Table 3.3.13 Formula and Unit Value for Average Detour Loss per Vehicle

Formulas and unit value for average cancellation loss per vehicle (ACLpV) is shown in Table 3.3.14.

Formula for Loss Estimate	Unit	Unit value at 2007
ACLpV = CPoWTP<150 x 75 + CPoWTP150 x 150 + CPoWTP300 x 300		Variable by NCDp
Where ACLpV: Average cancellation loss per vehicle	[Rs/vehicle]	
CPoWTP<150 : Cancellation percentage, willingness to pay under Rs 150 instead of canceling (42.9% of non waiting)	[%]	Variable by NCDp as shown in
CPoWTP150 : Cancellation percentage, willingness to pay up to Rs 150 instead of canceling (17.0% of non waiting)	[%]	Figure 3.3.17
CPoWTP300 : Cancellation percentage, willingness to pay up to Rs 300 instead of canceling (15.0% of non waiting)	[%]	
NCDp: Nos. of predicted closure days of the whole width of the road on the survey site per RCD	[days]	Parameter

Table 3.3.14 Formula and Unit	Value for Average	Cancellation Los	s ner Vehicle
Table 5.5.14 Pormula and Onic	value for Average	Cancenation Los	s per vennere

As above mentioned, calculation of average traffic suspension loss per vehicle (ASLpV) is done by given formula.

If NCDp < 0.1,

 $ASLpV = 1,580 \times NCDp;$

If $0.1 \le \text{NCDp} < 5.6$,

 $ASLpV = 693 \times Ln(NCDp) + 1,810;$

If $5.6 \leq$ NCDp, ASLoV = 3,030

Where

ASLpV: Average traffic suspension loss per vehicle [Rs/vehicle]

NCDp: Nos. of predicted closure days of the whole width of the road on the survey site per RCD [day]

The proportion of three components of ASLpV is shown in Figure 3.3.18.

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Figure 3.3.18 Proportion of Three components of Average Traffic suspension Loss per vehicle [Rs/vehicle]

(5) Road slope Assessment Sheet 4: Structural Measures Feasibility

The sheet 4: structural measures feasibility is described in Chapter 8.

(6) Road slope Assessment Sheet 5: Disasters Record

The sheet 5: disasters record is prepared for outcome evaluation and precision risk assessment for next stage which is shown in Figure 3.3.19.

tation from	23 km	960	m	Side of	survey Left s	side of	road		
ame of inspector for disaster record, sheet 6							Date	Month	Year
ame of surveyor	1			1	Survey date (d	im/y)			
	Hour	Date	Month	Vear	Disaster Ty	pe			
isaster Occurrence Date, Hour		Long	Monan	144	(Select from dra	p down			
meth of road closure site	-	Road clo	ore hore i	Tall width/	At least one lone is	mound	-		
angui of road closure she	Hour	Parts Close	size type (Full WHERE.	At reast one take is	sectero)	-		_
eopen Date, Hour at least one lane is secured	Hour	Linte	Monus	Year	Number of days of full width road closure (days)		(deve)		
constant stations	from	Im	-	m	until	km	-		-
atal reapening cost (Rs)	Estimated	-			Actual	Killi	-	101	
conening method (Select from drop down list)	Soll Rock remo	wal by ma	muni labo	ur	Treater				
imber of dead persons due to disaster (persons)	Construction of the local of th		Number	of injured a	ersons by the disa	ster (pers	(eno		
amber of broken vehicles due to disaster (vehicles)									
Countermeasure type					Station	_	_	_	_
	from	km		m	until	km		in	
	from	km		m	until	km		m	
	from	km		m	imtil	km		m	
12	from	km		m	linu	km		m	
	from	km		m	until	km	-	m	
And the second sec	from	km		m	untit	km		m	
chabilitation plan (planned, not yet planned)		Outline o	d the plan	1		-	-	-	-

Road Slope Assessment Sheet 5: Disaster Record

Figure 3.3.19 Road Slope Assessment Sheet 5: Disasters Record [Rs/vehicle]

3.4 Results of Risk Assessment

3.4.1 Narayangharh-Mugling Highway

(1) Risk Level of Sites

The Team carried out road slope disaster risk assessment survey along N-M highway in August 2007 with the local staff of DWIDP and DOR. The risk of 305 sites (134 mountainside slopes, 78 crossing streams, and 93 riverside slopes) were assessed using the road slope disaster assessment sheets, mentioned in section 3.3. Risk level indicators for the assessment are as follows:

- Potential frequency of RCD of a site (FRCDp) [RCD/year]

- Potential annual loss of a site (ALp) [Rs/year]

Generally, risk is indicated as multiplication of frequency and magnitude of disaster. FRCDp indicates only the frequency component of risk. The ALp is multiplication result of frequency and monetary magnitude of RCD; therefore it can be considered as a comprehensive risk level indicator of RCD.

Figure 3.4.1 Shows categorized FRCDp levels of sites on hazard map.

Figure 3.4.2 Shows categorized ALp levels of sites on hazard map.

Potential disaster sites are distributed along chainage 10 km to 36 km (26 km length section) of the N-M highway. High risk level sites (FRCDp is over 0.1 RCD/year, or ALp is over 1.0 million Rs/year) are scattered entirely along the 26 km stretch of the highway. Among the three slope-types (mountainside slope, crossing stream slope, and riverside slopes), the mountainside slope has the most risky.



Figure 3.4.1 Potential Frequency of RCD of a site (FRCDp) with Hazard



Figure 3.4.2 Potential Annual Loss of a site (ALp) with Hazard

(2) Total Risk of Narayangharh-Mugling Highway

The highway section which has risk of RCD is between CH 10 km and 36 km of N-M Highway.

Figure 3.4.3 shows summary of potential annual loss (ALp) of the N-M Highway. A total of 106.1 million Rs/year of annual loss is predicted based on 2007 market value. Proportions of the ALp by slope-type are: 65.9% of mountainside slope, 32.1% of crossing stream and 2.1% of riverside slope.



Figure 3.4.3 Potential Annual Loss (ALp) on N-M Highway by Slope Type at 2007 Value

Proportion of the ALp by loss component which is described in section 3.3.3 is shown in Figure 3.4.4. Potential annual loss caused by traffic suspension (ASLp) has big proportion with 99% of the total ALp.



Figure 3.4.4 Potential Annual Loss (ALp) on N-M Highway by Loss Component at 2007 Value

(3) Effectiveness of Existing Structural Measures

Road slope disaster risk estimate results show both potential annual loss (ALp) in 2007 and potential annual loss without structural measure (ALpom). Relation of ALp and ALpom is as shown below, and is described in section 3.3 in detail.

ALp = CEM x ALpom;

Where

ALp: potential annual loss [Rs/year]

CEM: coefficient of effectiveness of structural measures

ALpom: potential annual loss without structural measure [Rs/year]

Table 3.4.1 is the summary of ALp and ALpom of the N-M highway at 2007 value, and Figure 3.4.4 is the diagram of it.

Almost all the structural measures were installed after 2003 disaster. Therefore, ALpom indicates the risk time point in 2003 by 2007 value.

Existing structural measures have decreased 45 % potential annual loss i.e. from ALpom: 194 million Rs/year to ALp: 106 million Rs/year. They decreased high risk sites where annual loss are over 1.0 million Rs/year from 58 sites to 12 sites. Still there are 12 sites having high risk of slope disaster, they are: six (6) mountainside slopes and six (6) riverside slopes.



ALp or	ALpom of a site	Numbers of site					
(RS/year)		ALpom: Potential annual	ALp: Potential annual				
		loss without structural measure	loss				
	Bigger than 1.0 million	58	12				
	0.1 – 1.0 million	70	91				
	Smaller than 0.1 million	177	202				

Figure 3.4.5 Risk Reduction by Existing Structural Measure

	M	lountainside S	lope	(-1)	Crossing Stream			Riverside Slope			Total		
Categories of Potential Annual Loss (ALp) Rs/year	Nos. of Sites	Sub Total of ALp Rs/year	Percent age to the Total	Nos. of Sites	Sub Total of ALp Rs/year	Percenta ge to the Total	Nos. of Sites	Sub Total of ALp Rs/year	Percenta ge to the Total	Nos. of Sites	Sub Total of ALp Rs/year	Percenta ge to the Total	
ALp <100,000	61	449,453	1%	54	414,240	1%	87	41,900	2%	202	905,594	1%	
100,000 ≤ ALp < 1,000,000	67	41,616,513	60%	18	11,131,404	33%	6	2,083,521	98%	91	54,831,438	52%	
1000,000 ≤ ALp	6	27,871,818	40%	6	22,495,336	66%	0	0	0%	12	50,367,154	47%	
Total	134	69,937,784	100%	78	34,040,981	100%	93	2,125,422	100%	305	106,104,186	100%	
Categories of	Mountainside Slope			Crossing Stream			Riverside Slope			Total			
Categories of Potential Annual Loss without structural measure (ALpom) Rs/year	Nos. of Sites	Sub Total of ALpom Rs/year	Percent age to the Total	Nos. of Sites	Sub Total of ALpom Rs/year	Percenta ge to the Total	Nos. of Sites	Sub Total of ALpom Rs/year	Percenta ge to the Total	Nos. of Sites	Sub Total of ALpom Rs/year	Percenta ge to the Total	
ALpom <100,000	60	462,732	0%	51	209,036	0%	66	115,677	0%	177	787,445	0%	
100,000 ≤ ALpom < 1000,000	45	25,988,458	24%	12	6,786,916	13%	13	6,983,301	22%	70	39,758,675	21%	
1000,000 ≤ ALpom	29	81,421,819	75%	15	47,228,078	87%	14	24,355,134	77%	58	153,005,031	79%	
Total	134	107,873,009	100%	78	54,224,030	100%	93	31,454,111	100%	305	193,551,151	100%	

Table 3.4.1 Summary of Potential annual Loss (ALp) and ALp without Structural Measures (ALpom)

(4) High Risk 12 Sites

The risk level of slopes and stream are classified into three categories, using ALp ranking as indicator Table 3.4.2 shows 12 high risk sites where ALp is over 1.0 million per year. Figure 3.4.6 shows location of six (6) high risk mountainside slopes. Figure 3.4.7 shows location of six (6) high risk crossing streams. Table 3.4.2 List of 12 high risk sites where ALp is over 1.0 million.

Chainage of	Slope type	Disaster Type	FRCDp	ALp
starting side	Stope-type	Disaster Type	(RCD/year)	(mil. Rs/year)
11 km 280 m	Crossing stream	Debris flow	0.25	5.8
(Kahale Kola)				
11 km+500 m	Crossing stream	Slope failure & debris flow	0.26	2.1
12 km+600 m	Crossing stream	Debris flow	0.39	7.7
(Das Kola)				
21 km+200 m	Crossing stream	Slope failure & debris flow	0.34	2.7
21 km+560 m	Crossing stream	Debris flow	0.13	1.3
21 km+610 m	Mountainside slope	Slide	0.15	2.9
23 km+510 m	Mountainside slope	Slide & slope failure	0.24	3.5
23 km+930 m	Crossing stream	Debris flow	0.23	2.3
23 km+960 m	Mountainside slope	Slide	0.24	13.7
24 km+235 m	Mountainside slope	Slide & slope failure	0.19	1.5
30 km+690 m	Mountainside slope	Slope failure	0.24	1.9
34 km+200 m	Mountainside slope	Rock fall	0.55	4.3
Total of 12 sites		3.21	50.4	
Percentage divid	ed by total of all 305 sit	tes	15%	47%
Total of all 305 s	ites		22.02	106.1







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(5) Risk Level of Road Section for Every Kilometer

(a) General

For determining high risk road sections, risk level indicators for a site was processed for every kilometer of the road section as shown in Figure 3.4.8



Figure 3.4.8 Processing of Risk Level Indicators of a Road Section

(b) Potential Intensity of Road Closure Disaster of a Road Section (IRCDp)

The IRCD of a road section is calculated by the following formula.

$$IRCDp = \sum FRCDp/LS$$

Where:

IRCDp: Potential intensity of RCD of a road section [RCD/ year/km]

 \sum FRCDp: Total FRCDp of a road section [RCD/year]

LS : Length of a road section [km]

(c) Potential Intensity of Annual Loss of a Road Section (IALp) The IALp is calculated by the following formula.

 $IALp = \sum IALp/LS$

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Where:

IALp: Potential intensity of annual loss of a road section [Rs/year/km]

 \sum IALp: Total ALp of a road section [Rs/year]

LS : Length of a road section [km]

the risk of a one (1) km road section is assessed, based on the results of field survey.

(d) Risk Revel of One km Road Sections

Risk level indicators of road section at each kilometer are shown in Table 3.4.3, Figure 3.4.9 and 3.4.10.

High disaster frequency road sections, where IRCDp exceeds 1.0 [Nos. of RCD /year/ km] are: km 11-12, km 13-15, km 17-18, km 27-28, and km 34-35. High annual loss road sections, where IALp is higher than 10 million Rs/ km /year are: km 11-13, and km 23-25.

km Section					IRCDp: Potential Intensity of RCD of a section	IALp: Potential Intensity of Annual Loss of a section	Top 12 ALp Sites
					RCD/km	Rs/km/year	
km	10	-	km	11	0.20	636,349	
km	11	-	km	12	1.28	12,553,071	2 sites
km	12	-	km	13	0.90	11,596,374	1 site
km	13	-	km	14	1.28	5,532,572	
km	14	-	km	15	1.26	3,907,911	
km	15	-	km	16	0.73	2,629,073	
km	16	-	km	17	0.93	2,509,015	
km	17	-	km	18	1.76	6,125,221	
km	18	-	km	19	0.49	2,560,350	
km	19	-	km	20	0.62	3,139,973	
km	20	-	km	21	0.84	2,878,446	
km	21	-	km	22	0.84	8,443,483	3 sites
km	22	-	km	23	0.43	768,427	
km	23	-	km	24	0.94	11,427,335	3 sites
km	24	-	km	25	0.93	14,584,536	1 site
km	25	-	km	26	0.55	1,578,079	
km	26	-	km	27	0.55	2,228,505	
km	27	-	km	28	1.38	1,295,980	
km	28	-	km	29	0.75	488,570	
km	29	-	km	30	0.50	18,536	
km	30	-	km	31	0.63	2,787,817	1site
km	31	-	km	32	0.91	3,025,898	
km	32	-	km	33	0.85	304,143	
km	33	-	km	34	0.73	28,870	
km	34	-	km	35	1.07	4,369,938	1site
km	35	-	km	36	0.66	685,373	
km	36	-	km	36.1	0.09	3,417	
Average					0.84	4,065,294	

Table 3.4.3 Outline of Risk along N-M Highway







Figure 3.4.10 Intensity of Annual Loss (IALP)

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3.4.2 Ruwa Khola/ Marsyangdi Hydro Power Plant

(1) Policy and Flow of Risk Assessment

Hazard with same magnitude has occurred on 31st July 2003, the only one scenario which can be utilized to estimate values of frequency and magnitude of disaster. Consequently, risk is estimated based on this scenario in this study. Flow of risk assessment is shown in Figure 3.4.11.

Risk estimation without existing structural measure

The potential frequency of the disaster is evaluated as the same value of return period of rainfall amount of the disaster on 31^{st} July 2003.

The potential disaster magnitude/potential loss without existing structural measure is estimated as 2007 price of the same damage level in 31st July 2003 disaster, because existing structural measures are installed after 2006 disaster.

Risk is multiplication of the potential frequency and magnitude of disaster.

Risk estimation with existing structural measure

Risk with existing structural measure is evaluated by multiplication of the 'risk without existing measure' and 'coefficient of effectiveness of structural measure for disaster frequency (CEM)'.

Risk without existing structural measure

= Potential disaster frequency x Potential disaster magnitude [Rs/year]

Potential disaster frequency: same as return period of rainfall amount (24 hour rainfall amount) [disasters/year]

Potential disaster magnitude: same as loss by 31st July disaster in 2007 price [Rs]

Risk with existing structural measure

= Risk without existing measure x CEM [Rs/year]

CEM: Coefficient of effectiveness of structural measure for disaster frequency [no unit]

Figure 3.4.11 Flow of Risk Estimation for Ruwa Khola/ Marsyangdi Hydro Power Plant

(2) Disaster Frequency without Existing Structural Measure

The frequency of the disaster is evaluated as the same value of the return period which 16 years of 24 hour rainfall amount, hence, frequency is 1/16 [disasters/year] as per Devghat rainfall gauge station. If the other calculation method of rainfall amount is selected such as the "modified rainfall amount of 6, 12, 24 hour half value", a smaller return period (refer chapter 3) will be obtained. The 24-hour rainfall amount is therefore adapted as most conservative for the return period.

(3) Disaster Magnitude without Existing Structural Measure

Magnitude of monetary loss during the disaster that occurred on 31st July 2003 is estimated based on results of questionnaire survey on Mugling – Pokhara road conducted on February 2008 (national statistical data, and data form DOR and NER).

Results of monetary loss estimation are tentatively summarized in Table 3.4.4. The monetary losses are finalized and shown in the draft final report prepared on June 2008.

Items	Loss (Rs)
Reopening cost (tentative detour road)	40,000
Loss of road and power plant	58,350,000
Loss of vehicle	40,000
Loss of power plant electricity	87,700,000
Loss by traffic Suspension (waiting, detour, and cancellation)	9310,000
Total	155,440,000

 Table 3.4.4 Loss by disaster on 31st July in 2007 Price (Tentative)

(4) Risk without Existing Structural Measure

Risk: Potential annual loss without existing structural measure is estimated by multiplying the disaster frequency with the disaster magnitude as follows:

Potential annual loss without existing structural measure

= Potential disaster frequency x Potential disaster magnitude [Rs/year]

where;

Potential disaster frequency: same as return period of rainfall amount (24 hour rainfall amount) [disasters/year] = 1/16

Potential disaster magnitude: same as loss by 31st July disaster in 2007 price [Rs]

= 155,440,000 [Rs/disaster] x 1/16 [disasters/year]

As a result, potential annual loss without existing structural measure is estimated as 9,715 Rs/year.

(5) Risk with Existing Structural Measure

Risk causing Potential annual loss with existing structural measure is estimated by multiplying of "potential annual loss without existing structural measure" and "coefficient of effectiveness of structural measure for disaster frequency (CEM)" as follows:

Potential annual loss with existing structural measure

= Risk without existing measure x CEM [Rs/year]

Where;

CEM: coefficient of effectiveness of structural measure for disaster frequency [no unit]

Existing sabo dams have effectiveness of retaining the same scale debris flow of the hazard on 31st July 2003. But, in this site, the volume of debris generated due to slope failure is more than debris control volume of sabo-dams, while a part of existing flow sections is less than the flow section based on the rainfall intensity in 2003. Apart from this, the effectiveness of retaining debris in the sabo dams is reduced by the considerable volume of unstable debris deposited above these dams. Taking these things into consideration, CEM is set as "0.5".

= 9,715,000 [Rs/year] x 0.5 = 4,875,000 [Rs/year]

Potential annual loss with existing structural measure is estimated

4,875,000 [Rs/year].