

Appendix 6-4 Water Level Observation Record and Gates Operation Record

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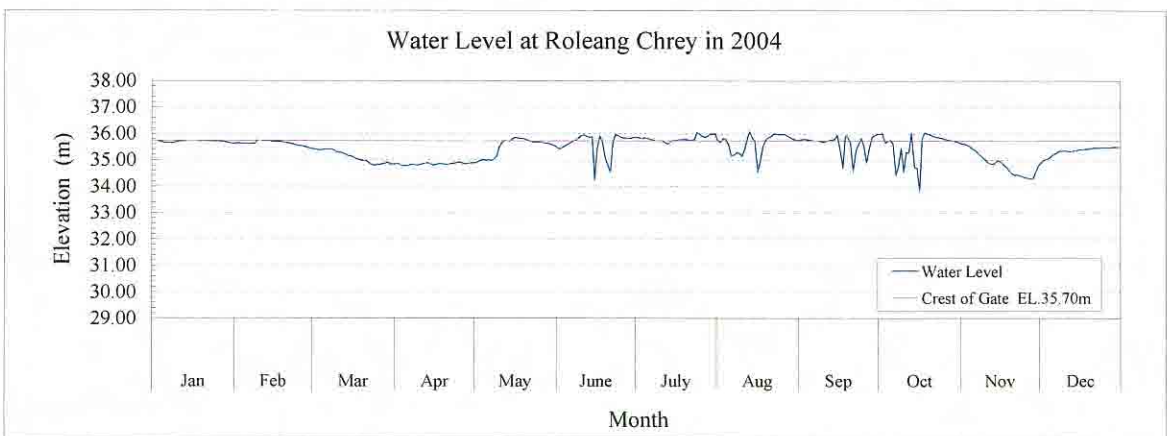
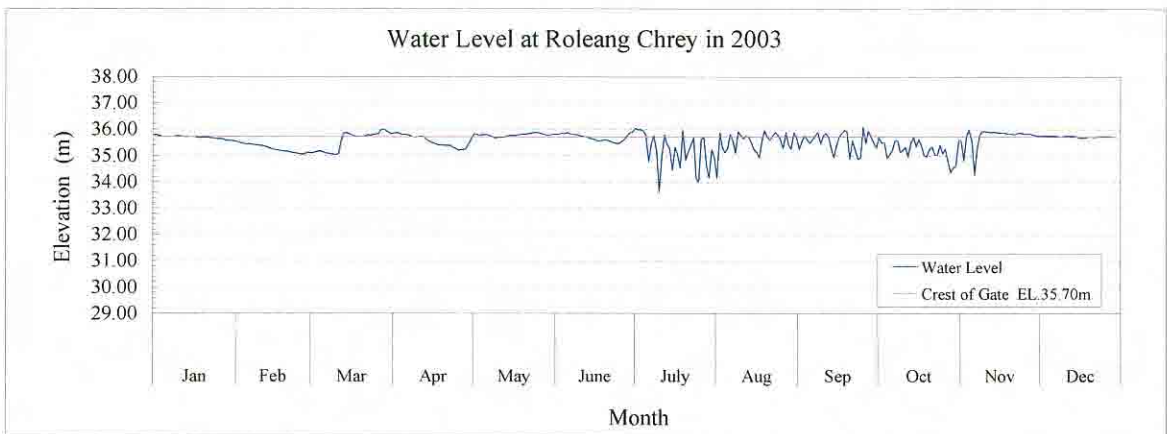
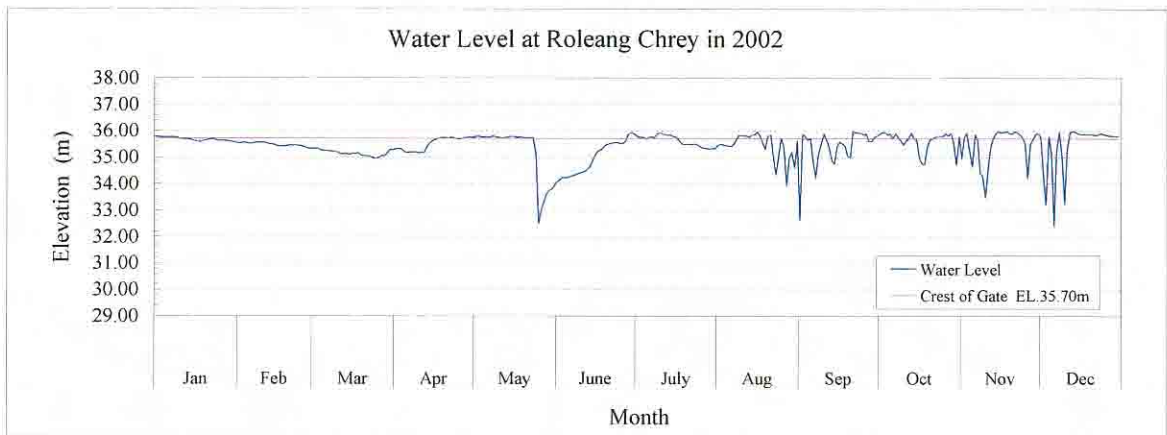
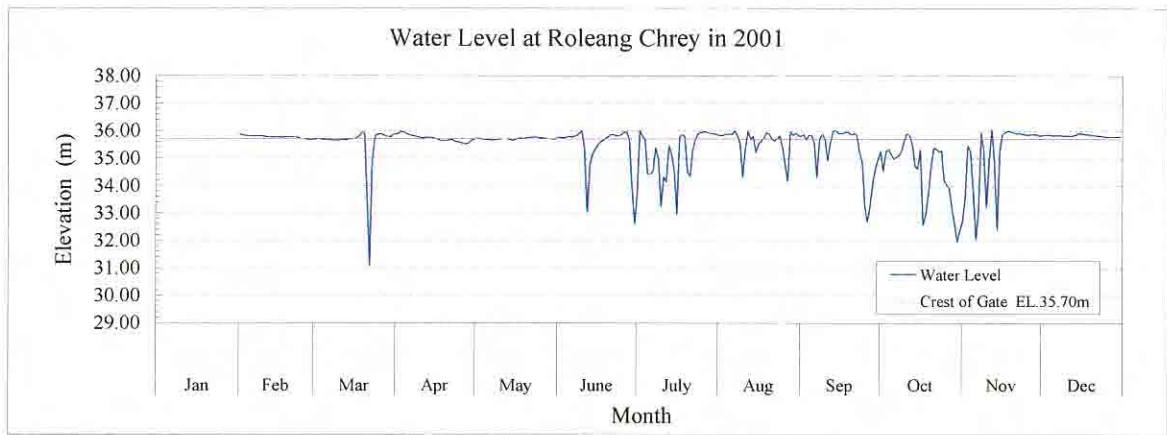


Figure 6.4.1 The Upstream Water Level Fluctuation (1/2)

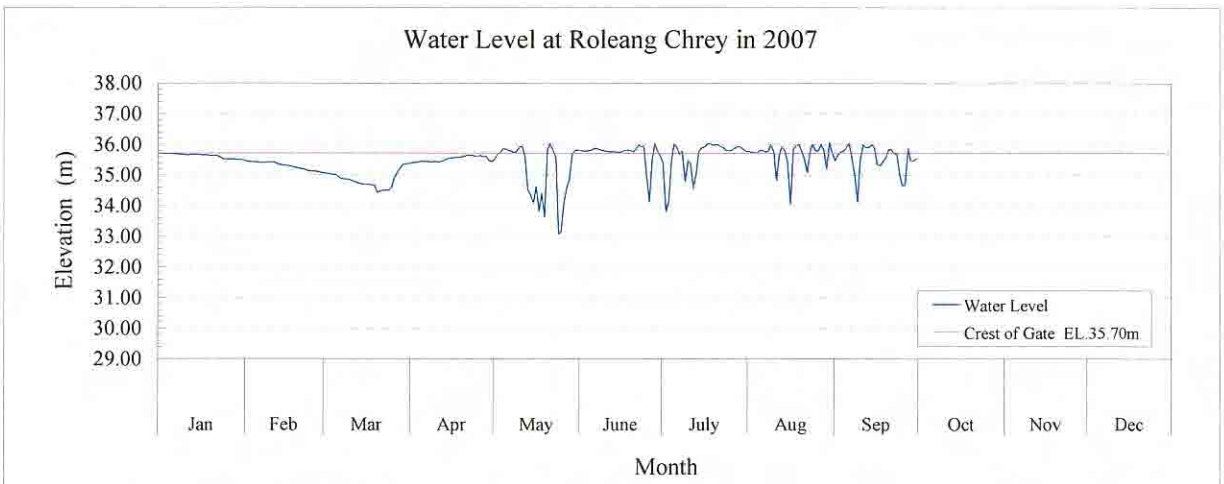
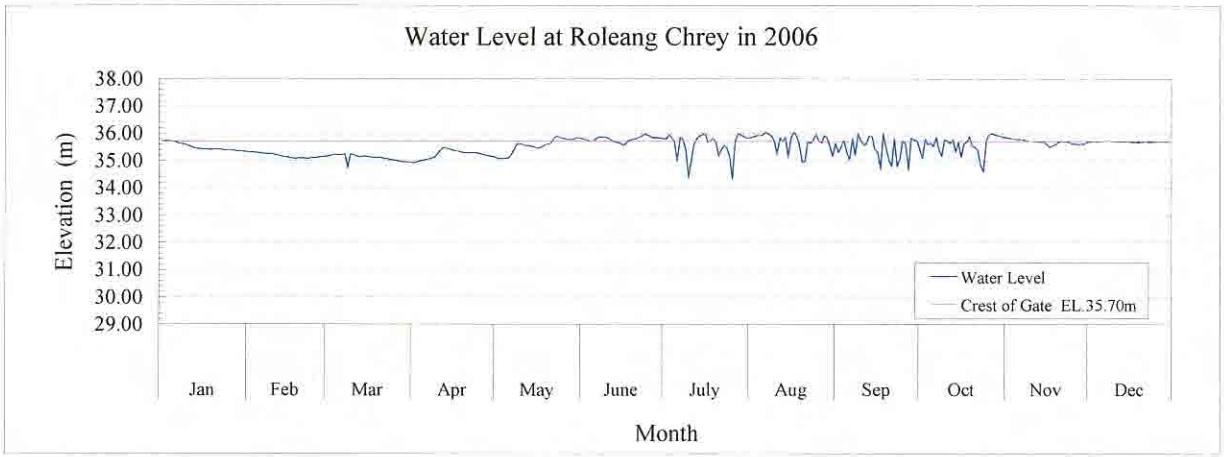
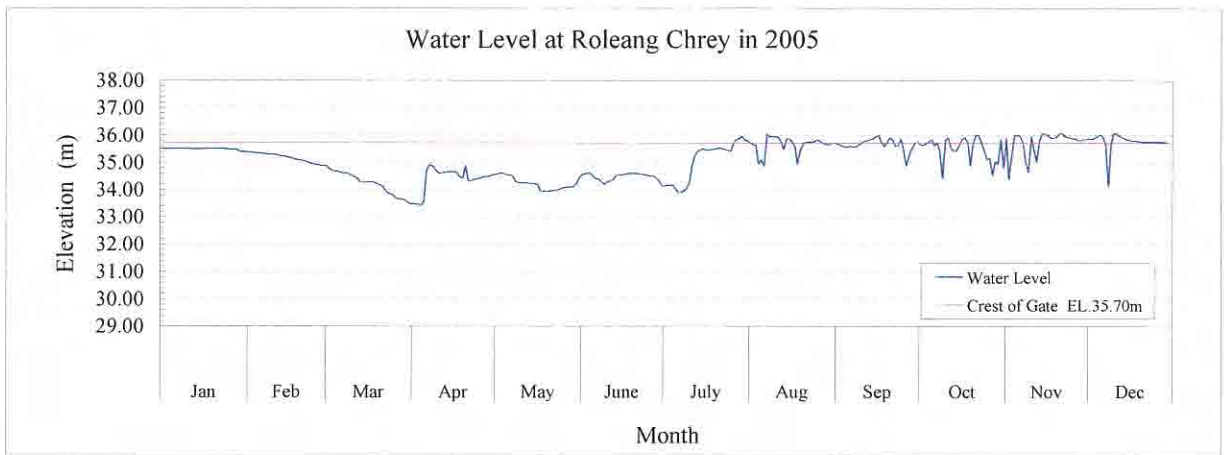


Figure 6.4.1 The Upstream Water Level Fluctuation (2/2)

Table 6.4.3 The Highest Upstream Water Level and Maximum Gate Opening Height

Year: February 2001 ~ September 2007
 Station: Roleang Chrey Regulator

Note: There is no reliable relation between the highest water level each month and the maximum gate opening height.

Year	January		February		March		April		
	Reading	Water level (m)	Maximum gate opening height	Reading	Water level (m)	Maximum gate opening height	Reading	Water level (m)	Maximum gate opening height
2001	-	-	-	7.15	35.85	-	7.31	36.01	1.0m×1no.
2002	7.06	35.76	-	6.85	35.55	-	6.62	35.32	-
2003	7.06	35.76	-	6.82	35.52	-	7.31	36.01	-
2004	7.00	35.70	-	7.02	35.72	-	6.69	35.39	-
2005	6.80	35.50	-	6.67	35.37	-	6.16	34.86	-
2006	7.03	35.73	-	6.61	35.31	-	6.53	35.23	-
2007	6.99	35.69	-	6.76	35.46	-	6.66	35.36	-
Highest water level		35.76			35.85			36.01	

Year	May		June		July		August		
	Reading	Water level (m)	Maximum gate opening height	Reading	Water level (m)	Maximum gate opening height	Reading	Water level (m)	Maximum gate opening height
2001	7.06	35.76	-	7.31	36.01	7.32	36.02	7.36	36.06
2002	7.10	35.80	0.7m×1no.	7.24	35.94	7.22	35.92	7.24	35.94
2003	7.16	35.86	-	7.20	35.90	7.38	36.08	7.34	36.04
2004	7.13	35.83	-	7.28	35.98	7.32	36.02	7.36	36.06
2005	5.90	34.60	-	5.89	34.59	7.28	35.98	7.38	36.08
2006	7.19	35.89	-	7.26	35.96	7.33	36.03	7.35	36.05
2007	7.33	36.03	2.0m×1no.+1.7m×2nos.	7.38	36.08	7.36	36.06	7.45	36.15
Highest water level		36.03			36.08			36.15	

Year	September		October		November		December		
	Reading	Water level (m)	Maximum gate opening height	Reading	Water level (m)	Maximum gate opening height	Reading	Water level (m)	Maximum gate opening height
2001	7.32	36.02	5.0m×5nos.	7.22	35.92	7.28	35.98	7.22	35.92
2002	7.28	35.98	1.0m×1no.+0.5m×2nos.	7.23	35.93	7.32	36.02	7.28	35.98
2003	7.36	36.06	2.0m×1no.+1.0m×2nos.	7.14	35.84	7.32	36.02	7.05	35.75
2004	7.26	35.96	1.1m×1no.	7.35	36.05	6.90	35.60	6.78	35.48
2005	7.27	35.97	1.2m×1no.+0.5m×1no.	7.30	36.00	7.39	36.09	7.36	36.06
2006	7.37	36.07	(2.4m+0.5m)×2+2.3m	7.28	35.98	7.14	35.84	7.01	35.71
2007	7.40	36.10	1.8m×1no.+1.0m×2nos.	-	-	-	-	-	-
Highest water level		36.10			36.05		36.09		36.06

Appendix 6-5 Hydraulic Calculation Results

Table 6.5.1	Non-uniform Flow Calculation of Prek Thnot River.....	A6-48
Table 6.5.2	Overflow Water Depth and Level over the Existing End Sill.....	A6-50

Table 6.5.1 Non-uniform Flow Calculation of Prek Thnot River (1/2) (Flood Discharge 1,600 m³/s)

No.	Station No. of Cross Section	Distance L (m)	Accumulated Distance Lac (m)	Discharge Q (m ³ /s)	River Bed		River Bank		Water		Flow Area A (m ²)	Velocity V (m/s)	Velocity Head		Hydraulic Mean Depth R (m)	Roughness Coefficient n	Slope of Friction Sf	Friction Loss hl (m)	Friction Loss = Sf x L x cv	Energy Head EL+hv (m)	Floude Fr
					Elevation BL (m)	Elevation BH (m)	Depth d (m)	Level EL (m)	Head hv (m)	Head											
1	C-31	0.00	0.00	1600.00	25.581	34.106	7.000	32.581	378.335	4.229	0.912	4.877	0.035	0.003	0.000	0.000	33.493	0.602			
2	C-30	89.26	89.26	1600.00	25.388	34.324	7.331	32.719	356.507	4.488	1.028	4.830	0.035	0.003	0.253	0.003	33.747	0.640			
3	C-29	229.28	318.54	1600.00	26.702	34.515	6.943	33.645	444.900	3.596	0.660	5.012	0.035	0.002	0.558	0.002	34.305	0.504			
4	C-28	218.74	537.28	1600.00	25.394	35.350	8.504	33.898	394.950	4.051	0.837	5.472	0.035	0.002	0.430	0.002	34.736	0.530			
5	C-27	188.58	725.85	1600.00	26.845	35.198	7.885	34.730	641.047	2.496	0.318	3.927	0.035	0.001	0.313	0.001	35.048	0.397			
6	C-26	231.89	957.75	1600.00	27.338	34.943	7.595	34.933	593.225	2.697	0.371	5.254	0.035	0.001	0.256	0.001	35.304	0.371			
7	C-25	241.15	1198.90	1600.00	26.447	35.472	8.756	35.203	657.489	2.434	0.302	5.802	0.035	0.001	0.202	0.001	35.505	0.317			
8	C-24	209.61	1408.51	1600.00	26.517	35.951	8.770	35.287	590.179	2.711	0.375	6.148	0.035	0.001	0.157	0.001	35.662	0.342			
9	C-23	182.37	1590.87	1600.00	24.766	35.627	10.800	35.566	800.993	1.998	0.204	6.768	0.035	0.000	0.108	0.000	35.770	0.240			
10	C-22	172.08	1762.95	1600.00	27.546	35.622	8.018	35.564	659.124	2.427	0.301	5.651	0.035	0.001	0.095	0.001	35.864	0.320			
11	C-21	171.96	1934.92	1600.00	28.238	36.349	7.579	35.817	976.439	1.639	0.137	5.694	0.035	0.000	0.089	0.000	35.954	0.218			
12	C-20	27.67	1962.59	1600.00	28.058	36.579	7.767	35.825	973.114	1.644	0.138	5.605	0.035	0.000	0.009	0.000	35.963	0.220			
13	C-19	23.96	1986.55	1600.00	27.311	36.093	8.545	35.856	1067.198	1.499	0.115	5.191	0.035	0.000	0.008	0.000	35.971	0.209			
14	C-18	23.01	2009.56	1600.00	25.481	36.534	10.354	35.835	956.465	1.673	0.143	6.145	0.035	0.000	0.007	0.000	35.978	0.212			
15	C-17	25.30	2034.86	1600.00	26.183	36.663	9.544	35.727	707.103	2.263	0.261	6.254	0.035	0.001	0.011	0.001	35.988	0.282			
16	C-16	28.82	2063.68	1600.00	27.886	39.750	7.555	35.441	477.512	3.351	0.573	6.099	0.035	0.001	0.026	0.001	36.014	0.392			

Table 6.5.1 Non-uniform Flow Calculation of Prek Thnot River (2/2) (Flood Discharge 1,000 m³/s)

Downstream from the Regulator (Q=1000m³/s) Coefficient of velocity cv = 1.0 Unit: m

No.	No. of Cross Section	Distance		Accumulated Distance	Discharge Q (m ³ /s)	Elevation		River Bed Elevation BL (m)	River Bank Elevation BH (m)	Water Depth d (m)	Water Level EL (m)	Flow Area A (m ²)	Velocity V (m/s)	Velocity Head hv (m)	Hydraulic		Slope of Friction Friction	Friction Loss hl (m)	Energy Head EL+hv (m)	Floude Fr
		L (m)	Lac (m)			River Bed	River Bank								Mean Depth R (m)	Roughness Coefficient n				
1	C-31	0.00	0.00	0.00	1000.00	25.581	34.106	5.000	30.581	5.000	30.581	236.807	4.223	0.910	3.493	0.035	0.004	0.000	31.491	0.713
2	C-30	89.26	89.26	89.26	1000.00	25.388	34.324	5.560	30.948	5.560	30.948	238.063	4.201	0.900	3.620	0.035	0.004	0.357	31.848	0.696
3	C-29	229.28	318.54	318.54	1000.00	26.702	34.515	5.302	32.004	5.302	32.004	308.447	3.242	0.536	3.831	0.035	0.002	0.692	32.540	0.522
4	C-28	218.74	537.28	537.28	1000.00	25.394	35.350	7.014	32.408	7.014	32.408	298.516	3.350	0.573	4.449	0.035	0.002	0.440	32.980	0.491
5	C-27	188.58	725.85	725.85	1000.00	26.845	35.198	6.136	32.981	6.136	32.981	441.460	2.265	0.262	4.272	0.035	0.001	0.263	33.243	0.345
6	C-26	231.89	957.75	957.75	1000.00	27.338	34.943	5.829	33.167	5.829	33.167	408.611	2.447	0.306	4.225	0.035	0.001	0.230	33.473	0.376
7	C-25	241.15	1198.90	1198.90	1000.00	26.447	35.472	7.011	33.458	7.011	33.458	482.810	2.071	0.219	4.984	0.035	0.001	0.204	33.676	0.292
8	C-24	209.61	1408.51	1408.51	1000.00	26.517	35.951	7.037	33.554	7.037	33.554	434.934	2.299	0.270	4.851	0.035	0.001	0.147	33.824	0.328
9	C-23	182.37	1590.87	1590.87	1000.00	24.766	35.627	9.021	33.787	9.021	33.787	603.036	1.658	0.140	5.381	0.035	0.000	0.104	33.928	0.225
10	C-22	172.08	1762.95	1762.95	1000.00	27.546	35.622	6.248	33.794	6.248	33.794	473.804	2.111	0.227	4.495	0.035	0.001	0.094	34.021	0.313
11	C-21	171.96	1934.92	1934.92	1000.00	28.238	36.349	5.770	34.008	5.770	34.008	675.602	1.480	0.112	4.118	0.035	0.000	0.098	34.120	0.232
12	C-20	27.67	1962.59	1962.59	1000.00	28.058	36.579	5.960	34.018	5.960	34.018	670.773	1.491	0.113	4.087	0.035	0.000	0.011	34.131	0.235
13	C-19	23.96	1986.55	1986.55	1000.00	27.311	36.093	6.738	34.049	6.738	34.049	751.127	1.331	0.090	4.484	0.035	0.000	0.009	34.140	0.200
14	C-18	23.01	2009.56	2009.56	1000.00	25.481	36.534	8.560	34.041	8.560	34.041	694.543	1.440	0.106	4.967	0.035	0.000	0.007	34.147	0.203
15	C-17	25.30	2034.86	2034.86	1000.00	26.183	36.663	7.795	33.978	7.795	33.978	536.764	1.863	0.177	5.777	0.035	0.000	0.009	34.155	0.241
16	C-16	28.82	2063.68	2063.68	1000.00	27.886	39.750	5.925	33.811	5.925	33.811	373.435	2.678	0.366	4.977	0.035	0.001	0.021	34.176	0.354

**Table 6.5.2 Overflow Water Depth and Level over the Existing End Sill (1/3)
(Flood Discharge 1,600 m³/s)**

Design flood discharge	Q = 1,600.0 m ³ /s	(50-year probable flood discharge)													
Width of downstream apron	B = 72.50 m		<table border="1"> <thead> <tr> <th colspan="2">Flow coefficient for rectangle weir</th> </tr> <tr> <th>h1/L</th> <th>C1</th> </tr> </thead> <tbody> <tr> <td>0 < h1/L ≤ 0.1</td> <td>1.642 (h1/L)^{0.022}</td> </tr> <tr> <td>0.1 ≤ h1/L ≤ 0.4</td> <td>1.552 + 0.083 (h1/L)</td> </tr> <tr> <td>0.4 ≤ h1/L ≤ (1.5 to 1.9)</td> <td>1.444 + 0.352 (h1/L)</td> </tr> <tr> <td>(1.5 to 1.9) ≤ h1/L</td> <td>1.785 + 0.237 (h1/W)</td> </tr> </tbody> </table> <p>(Hydraulic calculation formula, 1999, pp.244)</p>	Flow coefficient for rectangle weir		h1/L	C1	0 < h1/L ≤ 0.1	1.642 (h1/L) ^{0.022}	0.1 ≤ h1/L ≤ 0.4	1.552 + 0.083 (h1/L)	0.4 ≤ h1/L ≤ (1.5 to 1.9)	1.444 + 0.352 (h1/L)	(1.5 to 1.9) ≤ h1/L	1.785 + 0.237 (h1/W)
Flow coefficient for rectangle weir															
h1/L	C1														
0 < h1/L ≤ 0.1	1.642 (h1/L) ^{0.022}														
0.1 ≤ h1/L ≤ 0.4	1.552 + 0.083 (h1/L)														
0.4 ≤ h1/L ≤ (1.5 to 1.9)	1.444 + 0.352 (h1/L)														
(1.5 to 1.9) ≤ h1/L	1.785 + 0.237 (h1/W)														
Length of downstream apron	L = 10.00 m														
Height of end sill	W = 1.25 m														
Overflow depth (input)	h1 = 5.651 m														
Ratio of h1/L	h1/L = 0.57														
Flow coefficient	C1 = 1.643														
Overflow depth (output)	H1 = 5.651 m	= exp[2/3 ln { Q / (C1 B) }]													
	= (input) OK														
Top elevation of end sill	EL1 = 29.25 m														
Upstream water level	WL = 34.90 m														
(Reference)															
Critical depth at end sill	Hc = 3.68 m														
Water level at critical depth	WL = 32.93 m														

**Table 6.5.2 Overflow Water Depth and Level over the Existing End Sill (1/3)
(Flood Discharge 1,000 m³/s)**

Design flood discharge	Q = 1,000.0 m ³ /s	(5-year probable flood discharge)													
Width of downstream apron	B = 72.50 m		<table border="1"> <thead> <tr> <th colspan="2">Flow coefficient for rectangle weir</th> </tr> <tr> <th>h1/L</th> <th>C1</th> </tr> </thead> <tbody> <tr> <td>0 < h1/L ≤ 0.1</td> <td>1.642 (h1/L)^{0.022}</td> </tr> <tr> <td>0.1 ≤ h1/L ≤ 0.4</td> <td>1.552 + 0.083 (h1/L)</td> </tr> <tr> <td>0.4 ≤ h1/L ≤ (1.5 to 1.9)</td> <td>1.444 + 0.352 (h1/L)</td> </tr> <tr> <td>(1.5 to 1.9) ≤ h1/L</td> <td>1.785 + 0.237 (h1/W)</td> </tr> </tbody> </table> <p>(Hydraulic calculation formula, 1999, pp.244)</p>	Flow coefficient for rectangle weir		h1/L	C1	0 < h1/L ≤ 0.1	1.642 (h1/L) ^{0.022}	0.1 ≤ h1/L ≤ 0.4	1.552 + 0.083 (h1/L)	0.4 ≤ h1/L ≤ (1.5 to 1.9)	1.444 + 0.352 (h1/L)	(1.5 to 1.9) ≤ h1/L	1.785 + 0.237 (h1/W)
Flow coefficient for rectangle weir															
h1/L	C1														
0 < h1/L ≤ 0.1	1.642 (h1/L) ^{0.022}														
0.1 ≤ h1/L ≤ 0.4	1.552 + 0.083 (h1/L)														
0.4 ≤ h1/L ≤ (1.5 to 1.9)	1.444 + 0.352 (h1/L)														
(1.5 to 1.9) ≤ h1/L	1.785 + 0.237 (h1/W)														
Length of downstream apron	L = 10.00 m														
Height of end sill	W = 1.25 m														
Overflow depth (input)	h1 = 4.218 m														
Ratio of h1/L	h1/L = 0.42														
Flow coefficient	C1 = 1.592														
Overflow depth (output)	H1 = 4.218 m	= exp[2/3 ln { Q / (C1 B) }]													
	= (input) OK														
Top elevation of end sill	EL1 = 29.25 m														
Upstream water level	WL = 33.47 m														
(Reference)															
Critical depth at end sill	Hc = 2.69 m														
Water level at critical depth	WL = 31.94 m														

**Table 6.5.2 Overflow Water Depth and Level over the Existing End Sill (3/3)
(Flood Discharge 1,200 m³/s)**

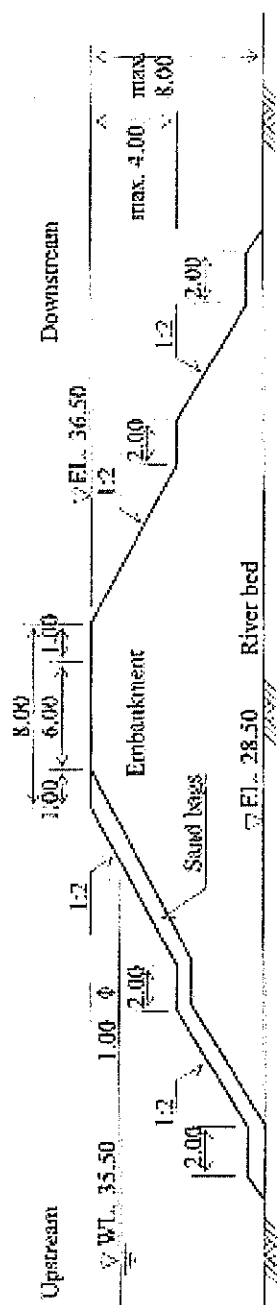
Design flood discharge	Q =	1,200.0 m ³ /s	(10-year probable flood discharge)
Width of downstream apron	B =	72.50 m	
Length of downstream apron	L =	10.00 m	
Height of end sill	W =	1.25 m	
Overflow depth (input)	h1 =	4.727 m	
Ratio of h1/L	h1/L =	0.47	
Flow coefficient	C1 =	1.610	
Overflow depth (output)	H1 =	4.727 m	= exp[2/3 ln{ Q / (C1 B) }]
		= (input) OK	
Top elevation of end sill	EL1 =	29.25 m	
Upstream water level	WL =	33.98 m	
(Reference)			
Critical depth at end sill	Hc =	3.03 m	
Water level at critical depth	WL =	32.28 m	

Flow coefficient for rectangle weir	
h1/L	C1
0 < h1/L ≤ 0.1	1.642 (h1/L) ^{0.022}
0.1 ≤ h1/L ≤ 0.4	1.552 + 0.083 (h1/L)
0.4 ≤ h1/L ≤ (1.5 to 1.9)	1.444 + 0.352 (h1/L)
(1.5 to 1.9) ≤ h1/L	1.785 + 0.237 (h1/W)

(Hydraulic calculation formula, 1999, pp.244)

Appendix 6-6 Design of Temporary Cofferd Dam

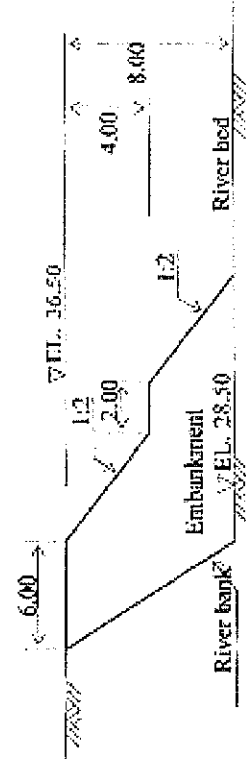
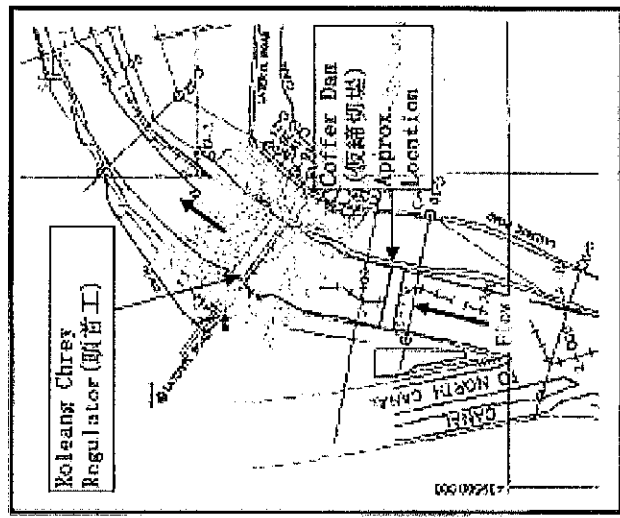
Figure 6.6.1	Typical Cross Section of Temporary Cofferd Dam and Approach Road to River Inside Area	A6-53
Figure 6.6.2	Slope Stability Analysis Result (Normal Case)	A6-54
Figure 6.6.3	Slope Stability Analysis Result (Under Earthquake Condition) ..	A6-55



Typical Cross Section of Cofferdam

Top length of cofferdam L = 100 m

PLAN



Typical Cross Section of Access Road

Slope of road I = 8.0%
Length of road L = 100 m

Figure 6.6.1 Typical Cross Section of Temporary Cofferdam and Approach Road to River Inside Area

ローレンチエリ一環道工 崖切取

縮尺 : 1/334

最小安全率 F.S.M. = 2.434
 円弧の中心 X = 45.00 (m)
 Y = 19.00 (m)
 半径 R = 19.00 (m)
 抵抗モーメント M_B = 13796.4 (kN · m)
 起動力モーメント M_S = 7810.3 (kN · m)

項目名	基本重量 (kN/m ²)	土体重量 (kN/m ²)	水重量 (kN/m ²)	土中圧力 (kN/m ²)	起動力 (kN/m)	抵抗モーメント (kN · m)	起動力 (kN · m)
1	18.00	18.00	9.80	20.00	18.00	2.000	0.000
2	18.00	18.00	9.80	20.00	18.00	2.000	0.000

次の項目を参照 - 11.01 (4.10.1)

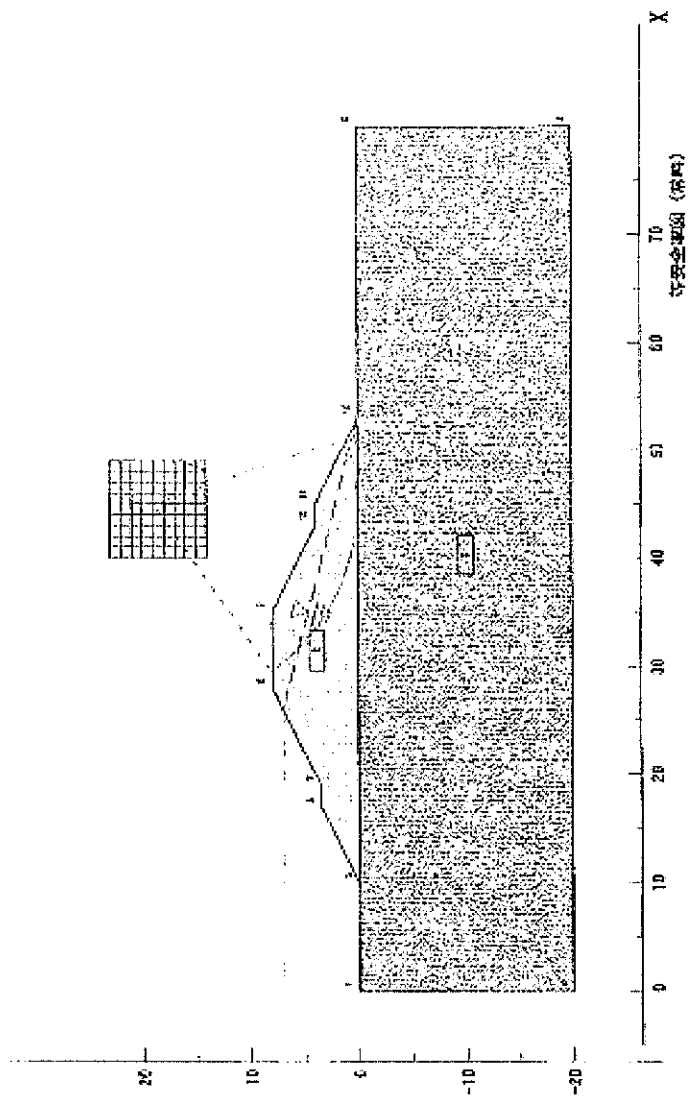


Figure 6.6.2 Slope Stability Analysis Result (Normal Case)

図尺 : 1/ 614

最小安全率 $F_s = 1.761$
 円錐の中心 $X = 43.00$ (m)
 $Y = 21.05$ (m)
 半径 $R = 21.05$ (m)
 基底モーメント $M_3 = 22733.9$ (kN·m)
 回転モーメント $M_0 = 32914.9$ (kN·m)

項目	計算式	計算値	単位	計算式	計算値	単位
1	$\frac{1}{2} \pi R^2$	346.6	m ²	$\frac{1}{2} \pi R^2$	346.6	m ²
2	πR^2	693.2	m ²	πR^2	693.2	m ²
3	$\frac{1}{2} \pi R^2$	346.6	m ²	$\frac{1}{2} \pi R^2$	346.6	m ²
4	$\frac{1}{2} \pi R^2$	346.6	m ²	$\frac{1}{2} \pi R^2$	346.6	m ²

等価土質係数 $\mu = 0.50$ (0.07/m)

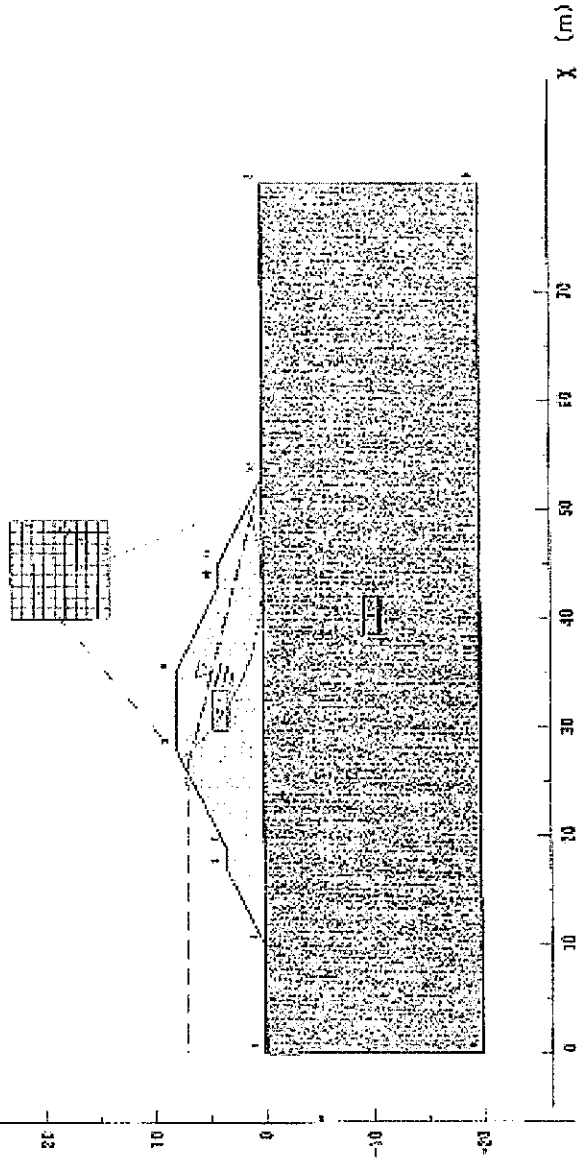


Figure 6.6.3 Slope Stability Analysis Result (Under Earthquake Condition)

谷波平岡 (地震時)

Appendix 6-7 Basic Design of Gate and Mechanical Works

6-7.1	Gates and Mechanical Works for Roleang Chrey Regulator	A6-57
6-7.2	Gate and Mechanical Works for River Outlet Works	A6-60
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6-7 Basic Design of Gate and Mechanical Works

6-7.1 Gates and Mechanical Works for Roleang Chrey Regulator

(1) Basic Design Policy

- 1) All the existing bushings and pins should be replaced with new ones.
- 2) Since five (5) sets of gate leaves and wheels are still in good condition, they should be re-used after rust removal and new painting.
- 3) Since the hoist system with control panels, such as, hoists, motors, wire-ropes, etc. have deteriorated for the past 33 years; they should be totally replaced with new ones.

(2) Scale and dimension of Rehabilitation Work (Mechanical Work)

The scale and dimension of rehabilitation work for Mechanical Work are as follows:

- 1) Replacement of the existing wheel bushings and pins with new ones : refer to Drawings No.2-12, 13
Scope of works: Disassemble, design, repair and installation of main wheels
Quantity : 40 Units (=8 units/gate×5 gate leaf)
Bushing : The bearing bush shall be replaced by oilless bearing, Oilless Industries, #500SP or equivalent.
Pin : Pin shall be replaced by stainless steel or hard chromium plated forged steel pin.
Pin shall be of eccentric adjustable type. It shall be adjusted at site so that the tread of four wheels lies on common plane.
Grease nipple shall be equipped to the pin.
Main wheel and pin shall be assembled.
Pins and bushings shall be purchased in Japan as a rule and transported to Cambodia from Japan by air cargo.
- 2) Rust removal and re-painting of five (5) gate leaves : refer to Drawing No.2-12
Scope of work:
Gate leaf with clear span 12.5m ×effective height 6.7m × 5gates shall be sand blasted and painted. Approximate Area for painting = 522.1m²/gate leaf × 5 sets
Cleaning : Sand blasting, surface grade ISO Sa 2 1/2
Painting : Nonbleed type tar epoxy paint, minimum dry film thickness 300 μm
Final color will be directed later by MOWRAM.
- 3) Replacement of rubber seals fixed on the gate leaves with new ones : refer to Drawing No.2-12
Type : Three edges with rubber seal, clear span:12.5m, effective height: 6.7m x 5 gate leaf
Disassembling and disposing of existing seal rubber (Sides: music note type, Bottom: flat type). They should be disassembled before Rust removal.
Renewal of rubber seal:
Seal rubber clamp and fixing bolts shall be replaced with stainless steel ones.
Type of Seal rubber shall be same as present ones, and shall be installed after repainting of gate leaf.
Seal rubber shall be purchased in Japan as a rule and transported to Cambodia from Japan.
- 4) Replacement of all the existing hoist systems with new ones : refer to Drawing No.2-14
Scope of work: Designing, manufacturing, transportation, installation, testing, and commissioning of the new hoist on existing operation bridge.
Type : Wirerope winch type hoist with counter weight, one motor two drums
Quantity : Five (5) sets

Capacity :Self weight 40tons + friction force of wheel + friction forth of rubber seal + weight of overflow water of 50cm depth – Existing counter-weight(approx. 14tons/gate leaf)

Hoist type: Electric and manual operation

Control system: Local control and remote control from control house of Roleang Chrey

Speed : 0.3 m/min at electric drive

Accessories : Upper and lower limit switches, safety equipment, overload relay, position indicator and so on required for gate operation.

Removal of hoist:

Scope of work : Removing, checking, demolishing, transportation and disposing of hoist including dram, wire rope, speed reducer, brake, electric motor, control panel etc. Note: Existing counterweights and their wire ropes will be reused.

- 5) Replacement of existing control panels with new ones on the operation deck : refer to Drawing No.2-14

Type Self standing, outdoor, whether proof control panel

Quantity : Five (5) sets

Details : Following equipment shall be installed but not be confined to; Three push button switches, volt meter, am meter, molded case circuit breaker (MCCB), magnetic contactor, relay, indication lights, 200 V convenience outlet, space heater and other necessary equipment, and connection cable from panel to hoist equipment.

Quantity : Five (5) sets

- 6) Installation of new control panel at the existing O&M office : refer to Drawing No.2-14

Type : Self standing, indoor control panel

Quantity : One (1) complete set

Purpose : To control the gates of Roleang Chrey Regulator

Details Following equipment shall be installed but not be confined to; Three push button switches, volt meter, am meter, molded case circuit breaker (MCCB), magnetic contactor, gate position indicators, indication lights, 200 V convenience outlet, space heater and other necessary equipment

Remarks : One (1) complete set of remote panels for 5 gates installed at the existing control house of Roleang Chrey regulator, including new main distribution panels and new distribution panels for lighting of the control house. The Main distribution panels include all connection cables between the existing engine generators remote control panels, local control panels and new hoist systems. New distribution panels for lighting also includes connection cables between panels and lighting equipments.

- 7) Installation of lightening arrestors (3 sets) on the regulator including earthing networks:

- 8) Spare part (rubber seal for one gate, magnetic contactor 40 units for control panels)

(2) Hoisting Load

The approximate hoisting load of hoist system are as follows:

Approximate Hoisting Load:

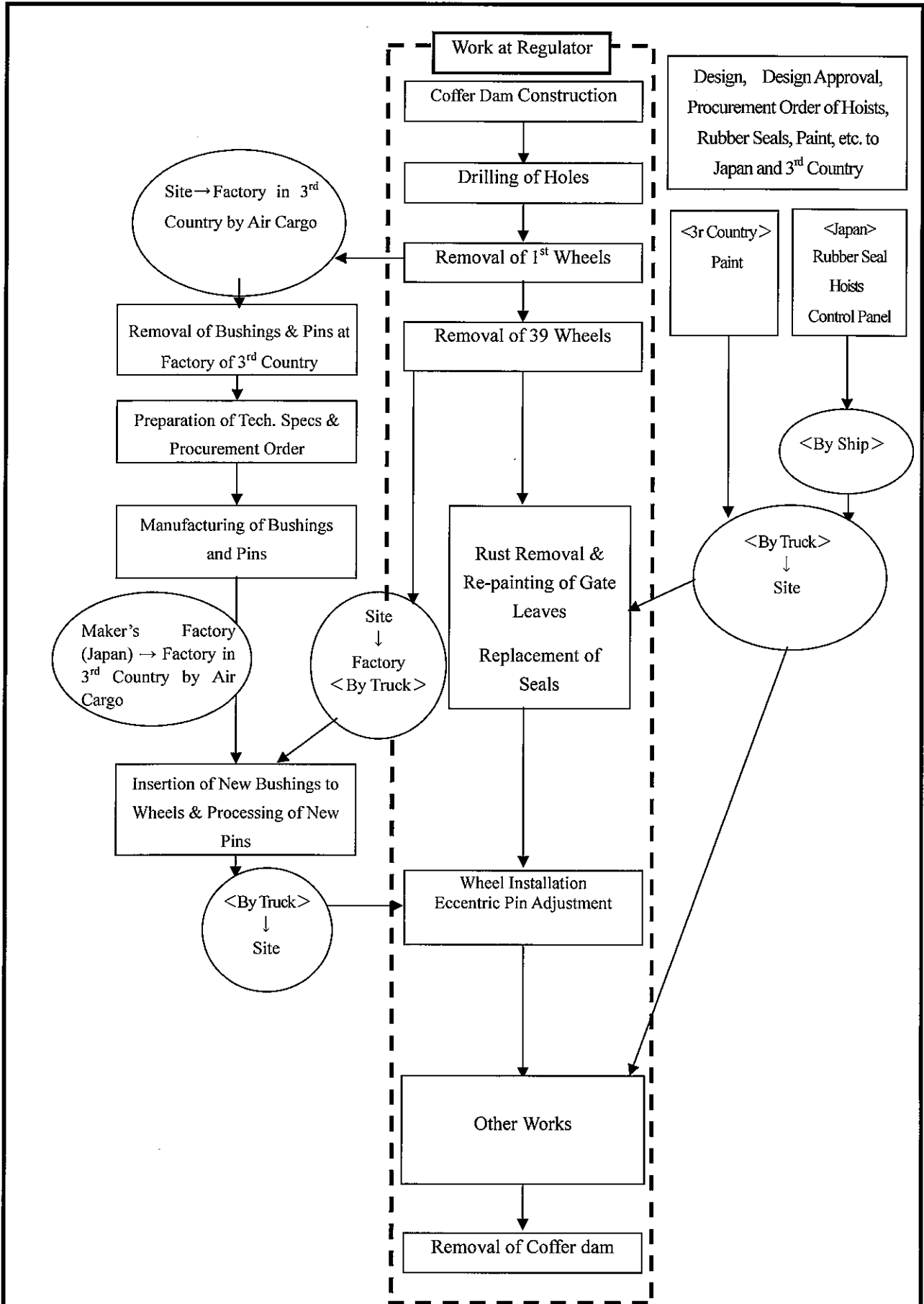
Gate Leaf dead load : 40 t (downward)

Wheel Friction Load : 9 t (downward)

Rubber Seal Friction Load : 3 t (downward)

Counter-weight dead load : 14 t (upward)

Total : 38 t (downward)



Standard Work Flow of Gate Rehabilitation

6-7.2 Gate and Mechanical Works for River Outlet Works

(1) Basic Design Policy

- 1) The river outlet structure should be constructed on the right bank of the regulator for stable irrigation water supply to Kandal Steung irrigation area of 1,950 ha (completed in August 2007 under Japan's grand aid scheme and located at about 40 km downstream of the regulator) without using the regulator's gate.
- 2) Since five (5) sets of gate leaves and wheels are still in good condition, they should be re-used after rust removal and new painting.
- 3) Since the hoist system with control panels, such as, hoists, motors, wire-ropes, etc. have deteriorated for the past 33 years; they should be totally replaced with new ones.

(2) Design Features

- 1) The river outlet structure will consist of inlet, culvert and outlet. The inlet of the river outlet structure will be constructed at the upstream of the regulator, while the outlet of the river outlet structure will be constructed at the downstream of the regulator.
- 2) At the inlet, two (2) sets of slide gates for discharge regulation and another two (2) sets of slide gates for repair and maintenance will be installed..
- 3) All the gates will be of clear span 1 m x effective height 1 m, four-edge seal, and the gates will be operated by hands. The river outlet structure will be constructed on the right bank of the regulator since O&M office is located on the right bank..
- 4) The inlet base elevation will be higher than that of the concrete apron of the regulator in order to minimize the floating sand entrance, and in front of the inlet gates, trash rack will be installed in order to prevent entrance of foreign materials and floating logs. At the outlet, an energy dissipater will be constructed, and grid-bars will be installed for safety of residents.

(3) Scale and dimension of Construction Work (Mechanical Work)

The scale and dimension of construction work for mechanical work of river outlet works are as follows:

- 1) Bypass gate for river outlet works : refer to Drawing No.2-15

Scope of work:

Designing, manufacturing, supplying, transporting, installation, testing and commissioning of following gate.

Type	: Steel slide gate
Quantity	: Discharge regulating gate : 2 sets Maintenance gate : 2 sets
Clear span	: 1.0 m
Height	: 1.0 m
Lift	: 1.0 m
Seal	: Four edges with rubber seal
Design head	: 6.0 m
Head at operation	: 6.0 m
Hoist type:	: Manual operation with Rack-pinion/ screw spindle type

- 2) Screen at Inlet : refer to Drawing No.2-10

Scope of work:

Designing, manufacturing, supplying, transporting, installation of following Screen.

Type	: Slant type fixed trashrack
Quantity	: Upper deck: 2 sets, Lower deck: 2 sets
Clear span	: 1.0 m
Effective Height:	Vertical height 3.0 m (= 3.030m actual length)
Angle	: 16.7 degree to y axis

Bar Spacing : 75mm (at bar center)
Design head : 0.4 m
Maximum allowable torsion at center: 1.7 mm ($= \delta = 1/600 \times \text{clear span } 1,000 \text{ mm}$)
Allowable thickness of corrosion: 2 mm
Minimum thickness of plates: 4.5 mm
Removal operation: Manual Rake
Cleaning (Factory): Sand blasting, surface grade ISO Sa 2 1/2
Painting : In-organic Zinc rich type + tar epoxy paint, minimum dry film thickness
300 μm

3) Screen at Outlet : refer to Drawing No.2-10

Scope of work:

Designing, manufacturing, supplying, transporting, installation of following
Screen.

Type :fixed type grid bars

Quantity : Two (2) sets

Clear span : 1.4 m

Effective Height: Vertical height 1.4 m

Bar Spacing : 150 mm (at bar center, vertical bars), 350 mm (max., at bar center, horizontal)

6-7.3 Gate and Mechanical Works for Andong Sla Intake

(1) Basic Design Policy

- 1) The intake gates should be replaced with new ones in order to recover the original functions and prevent leakage.
- 2) Since design discharge of the intake gates is 10.4 m³/sec, four (4) sets of the existing gates should be dismantled and two (2) sets of new radial gates should be installed at the center of the intake. The gates should be operated by hands.
- 3) Since the location of trunnion pin of the existing gates is lower than design water level, it should be located above the design water level at EL 36.0 m.

(2) Design Features

The base elevation of the new gates will be EL 32.0 m, which is the same as that of the existing gate. The gate will have such design figures as gate radius of 5 m, design water head of 4.0 m and hoisting height of 5.0 m.

(3) Scale and dimension of Construction Work (Mechanical Work)

The scale of replacement of the intake gates is as follows:

- 1) Renewal of Andong Sla Intake Gate : refer to Drawing No.3-6

Scope of work:

Designing, manufacturing, supplying, transporting, installation, testing and commissioning of following gate.

Type	: Steel radial gate
Quantity	: Two 2 sets
Clear span	: 4.0 m
Height	: 2.7 m
Radius of skin plate	: 5.0 m
Hoisting height	: 3.0 m
Seal system	: Four edges rubber seal (Front seal)
Design head	: 4.5 m
Head at operation	: 4.5 m
Height of trunnion pin	: 4.0 m from sill level
Control system	: Manual operation
Hoist	: Dual swing type Rack pinion/ Dual swing type Screw spindle hoist
Accessories	: Position indicator and so on required for gate operation.

- 2) Removal of existing Andong Sla Intake Gate : refer to Drawing No.3-6

Scope of work:

Removing, checking, demolishing, transporting and disposing of following gates

Type	: Steel radial gate
Quantity	: Four (4) sets
Clear span	: 4.0 m
Height	: 2.7 m
Hoist	: Manual hoist

- 3) Hoist Deck: refer to Drawing No.3-6

Scope of work:

Designing, manufacturing, supplying, transporting, installation, and commissioning of following hoist deck.

Type	: Steel girder construction with cover plate
Quantity	: 2 sets
Minimum width	: 1.0 m

Clear span of hoist deck: 4.0 m

The minimum thickness of checkered plates: 4.5 mm

Design load:

Normal load condition

Dead weight of hoist deck and hoist

Operating load of hoist

Crowded load of 500 kgf/m² on the deck

Thermal load due to temperature variation of 40 degree Celsius

Overload condition

All loads imposed due to the gate jammed condition during raising operation

Seismic load (Seismic intensity of 0.1)

Wind load of 300 kgf/m²