

APÊNDICES

APÊNDICE-A

Photograph Report

Appendix-A: Photograph Report

Period: 5/14/2007~5/19/2007



Setting out of the Building: Indoor Market Shed



Delivered Concrete Block materials



Foundation Work: Indoor Market Shed excavation



Delivered Cement materials



Foundation Work: Indoor Market Shed laid rubbles



General view (5/19)

Period: 5/21/2007~5/26/2007



Foundation work: Indoor Market Shed view



Foundation work: Indoor Market Shed corner stone



Survey by District Office for registry of land (5/23)



Foundation work: Public Toilet Shed excavation



Foundation work: Public Toilet Shed completed



General View (5/26)

Period: 5/28/2007~6/2/2007



Floor concrete : Indoor Market Shed rubbles preparation Sewage piping: Public Toilet Shed



Site inspection by JICA and Japan embassy (5/30) Septic Tank: excavation



Floor concrete : Indoor Market Shed casting General View (6/1)

Period: 6/4/2007~6/9/2007



General view1 (6/4) : from terrace side



General view 2 (6/4) : public toilet side



Construction registered plate



Septic Tank: excavation



Septic Tank: wall masonry



General View (6/9)

Period: 6/11/2007~6/15/2007



Parking Area: preparation borrow material for grading



Septic Tank: side wall completed



Masonry wall: Indoor Market Shed from patio side



Masonry wall: Public Toilet from patio side



General View 1 (6/14)



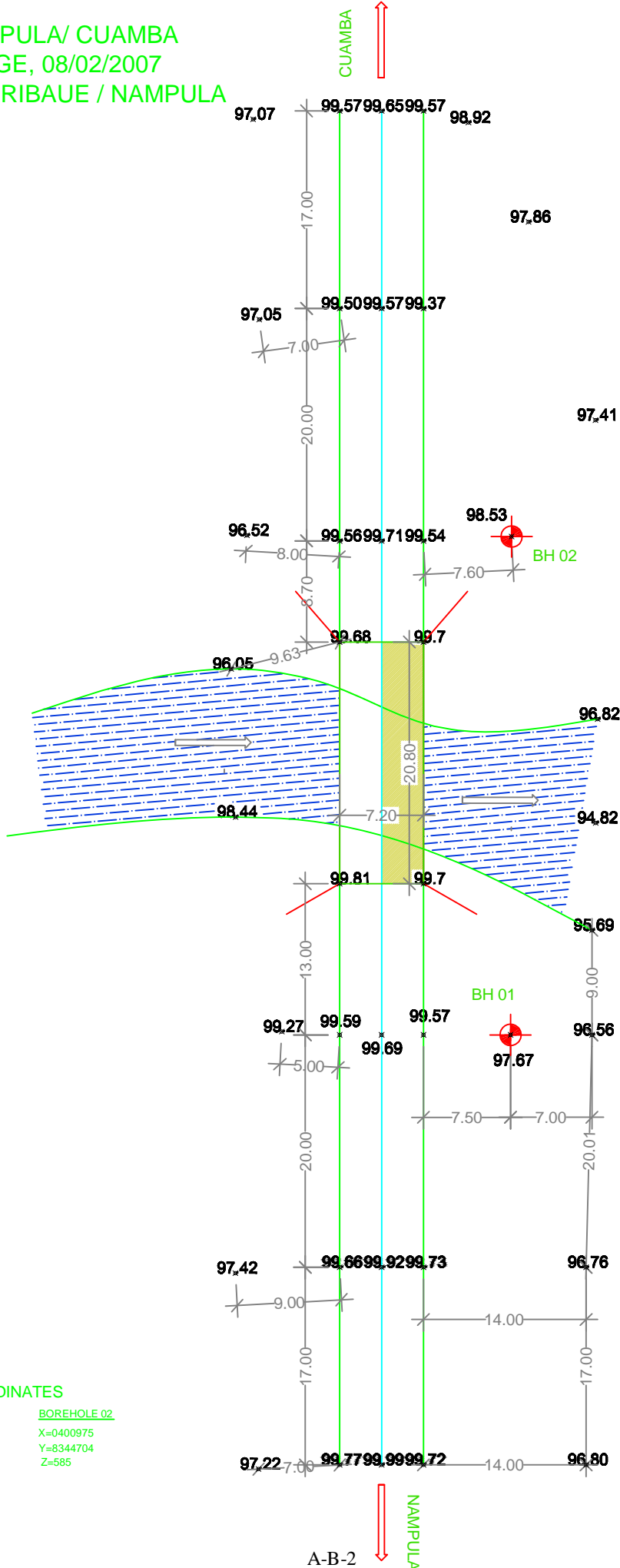
General View 2 (6/14)

APÊNDICE-B

Geotechnical Survey Result

Appendix B-1
BOREHOLE LOGS AND LOCATION MAP

ROAD N13 - NAMPULA/ CUAMBA
 MONAPO BRIDGE, 08/02/2007
 BOREHOLE 01 & 02 - RIBAUE / NAMPULA



COORDINATES

BOREHOLE 01	BOREHOLE 02
X=0401025	X=0400975
Y=8344703	Y=8344704
Z=582	Z=585

TÉCNICA - ENGENHEIROS CONSULTORES, LDA

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DRILLING RECORD

BORING NO: BH 01

DRILLING METHOD	% CORE RECOVERY	SAMPLING	TESTING	FRACTURE SPACING	FRACTURE ANGLE (DEG)				DEPTH (Meters)	PROFILE (Meters)	MATERIAL DESCRIPTION		
					15	30	45	60			Hardness/ Dimension	Color	Soil/Rock Quality
			SPT N=4						1		Medium grained	Brown	Sand
			SPT N=5						2				
			SPT N=3						3		Medium and fine grained	Brown and white	Sand
			SPT N=1						4				
			SPT N=4						5				
	100		SPT N=20						6				
			SPT N=33/15						7		Stiff (low moisture content)	Grey	Clay with some fine white sand
			SPT N>50						8				
			SPT N>50						9				
	78		SPT N>50						10		Very weak	Grey	Weathered granite rock
			SPT N>50						11				
			SPT N>50						12				
	100		SPT N>50						13		Weak	Grey	Slightly weathered granite rock
			SPT N>50						14				
			SPT N>50						15				
DRILLING RIG			D900 ATLAS COPCO				DATE STARTED			2007-02-06	DATE COMP.		2007-02-08
CONTRACTOR							LOGGED BY			S . Taula	DATE		2007-02-12
LOCATION			Monapa River, Iapala				DEPTH W/TABLE				AFTER		
PROJECT			N 13 - Nampula , Cuamba				ELEVATION			97.67	JOB NO		P 318
CLIENT			JICA / ANE				ORIENTATION			VERTICAL	SHEET NO		1 OF 2

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DRILLING RECORD

BORING NO: BH 02

DRILLING METHOD	% CORE RECOVERY	SAMPLING	TESTING	FRACTURE SPACING	FRACTURE ANGLE (DEG)				DEPTH (Meters)	PROFILE (Meters)	MATERIAL DESCRIPTION		
					15	30	45	60			Hardness/ Dimension	Color	Soil/Rock Quality
			SPT N=5						1		Medium grained	Brown	Sand
			SPT N=7						2				
			SPT N=13						3		Fine Grained sand	Brown	Clayey
			SPT N=9						4				
			SPT N>50						5				
			SPT N>50						6		Coarse grained	Brown	Sand mixed with high Clayey
			SPT N>50						7				
			SPT N>50						8				
			SPT N>50						9				
			SPT N>50						10				
	100		SPT N>50						11		Very weak	Grey	Severily weathered granite Rock
			SPT N>50						12				
			SPT N>50						13				
			SPT N>50						14				
			SPT N>50						15				
DRILLING RIG			D900 ATLAS COPCO					DATE STARTED		2007-01-31	DATE COMP.		2007-01-31
CONTRACTOR								LOGGED BY		S. Taula	DATE		2007-02-04
LOCATION			MONAPO RIVER					DEPTH W/TABLE			AFTER		
PROJECT			N 13 - Nampula , Cuamba					ELEVATION		98.53	JOB NO		P 318
CLIENT			JICA / ANE					ORIENTATION		Vertical	SHEET NO		1 OF 2

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DRILLING RECORD

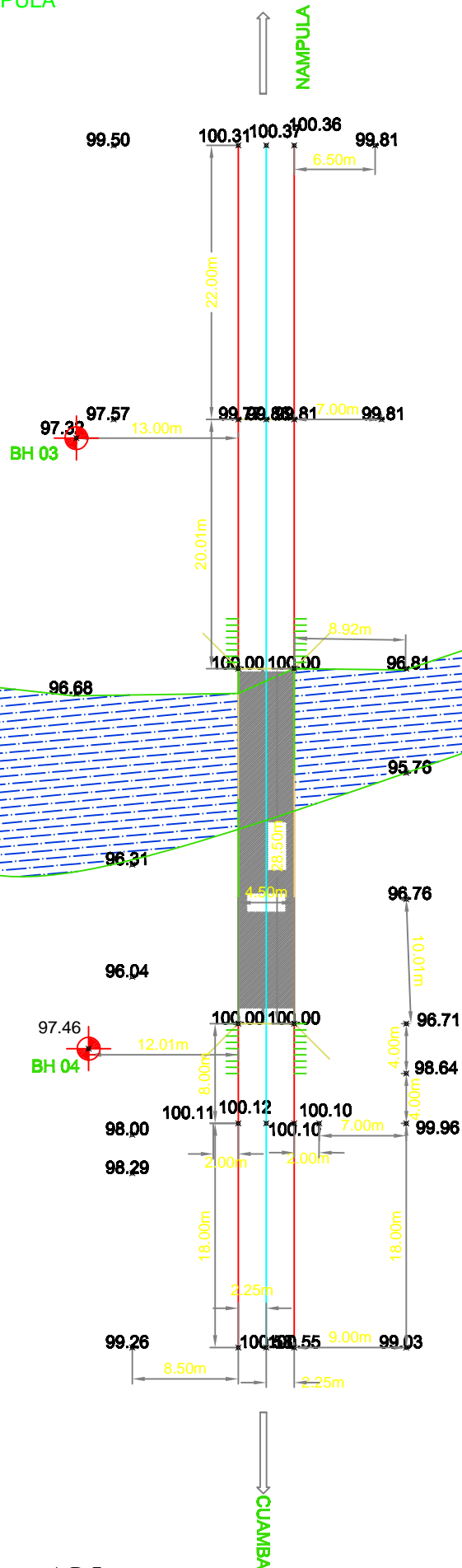
BORING NO: BH 02

DRILLING METHOD	% CORE RECOVERY	SAMPLING	TESTING	FRACTURE SPACING	FRACTURE ANGLE (DEG)				DEPTH (Meters)	PROFILE (Meters)	MATERIAL DESCRIPTION					
					15	30	45	60			Hardness/ Dimension	Color	Soil/Rock Quality			
			SPT N>50													
			SPT N>50					16								
			SPT N>50					17								
			SPT N>50					18								
			SPT N>50					19								
			SPT N>50					20								
	100		SPT N>50					21				Very weak	Grey		Severily weathered granite Rock	
			SPT N>50					22								
			SPT N>50					23								
			SPT N>50					24								
			SPT N>50					25								
			SPT N>50					26								
	100							27				Weak	Grey		Slightly weathered granite Rock	
								28								
	100							29				Hard	Grey		Unweathered granite rock	
								30								
											End of borehole at 68.99m					
DRILLING RIG			D900 ATLAS COPCO			DATE STARTED			2007-01-31		DATE COMP.		2007-01-31			
CONTRACTOR						LOGGED BY			S. Taula		DATE		2007-02-04			
LOCATION			MONAPO RIVER			DEPTH W/TABLE					AFTER					
PROJECT			N 13 - Nampula , Cuamba			ELEVATION			98.53		JOB NO		P 318			
CLIENT			JICA / ANE			ORIENTATION			Vertical		SHEET NO		2 OF 2			

ROAD N13 - NAMPULA/ CUAMBA
 LALAU BRIDGE, 27/01/2007
 BOREHOLE 03 & 04 - MALEMA / NAMPULA
 CH = 190.80 km

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COORDINATES

BOREHOLE 03
 X=0401025
 Y=8344703
 Z=582

BOREHOLE 04
 X=0400975
 Y=8344704
 Z=585

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DRILLING RECORD

BORING NO: BH 03

DRILLING METHOD	% CORE RECOVERY	SAMPLING	TESTING	FRACTURE SPACING	FRACTURE ANGLE (DEG)				DEPTH (Meters)	PROFILE (Meters)	MATERIAL DESCRIPTION								
					15	30	45	60			Hardness/ Dimension	Color	Soil/Rock Quality						
	100		SPT N=7					1			Fine grained	Brown	Sand						
	100		SPT N=6/12					2			Coarse grained	Brown	Sand mixed with some rock material						
	100							3											
								4											
								5			Hard	Grey	Unweathered granite rock						
								6											
								7											
								8			End of borehole at 89.56m								
								9											
								10											
								11											
								12											
								13											
								14											
								15											
DRILLING RIG	D900 ATLAS COPCO			DATE STARTED	2006-12-28	DATE COMP.	2006-01-29												
CONTRACTOR				LOGGED BY	S . Taula	DATE	2007-01-30												
LOCATION	Lalava River			DEPTH W/TABLE		AFTER													
PROJECT	N 13 - Nampula Cuamba			ELEVATION	97.32	JOB NO	P 318												
CLIENT	JICA / ANE			ORIENTATION	VERTICAL	SHEET NO	1 OF 1												

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DRILLING RECORD

BORING NO: BH 04

DRILLING METHOD	% CORE RECOVERY	SAMPLING	TESTING	FRACTURE SPACING	FRACTURE ANGLE (DEG)				DEPTH (Meters)	PROFILE (Meters)	MATERIAL DESCRIPTION		
					15	30	45	60			Hardness/ Dimension	Color	Soil/Rock Quality
			SPT N=1/37						1		Medium grained coarse grained	Brown	Sand
			SPT N=2						2				
	100		SPT N=5						3				
			SPT N=12/15						4		Coarse Grained	Brown	Sand with some clay content
			SPT N=4/3						5				
			SPT N>50						6				
	100								7		Very weak to weak	Brown	Weathered granite rock
									8				
	100								9		Hard	Grey	Unweathered granite rock
									10				
	100								11				
									12		Weak	Grey	Weathered granite rock
									13				
									14				
									15				
DRILLING RIG			D900 ATLAS COPCO			DATE STARTED			2006-12-28		DATE COMP.		
CONTRACTOR						LOGGED BY			S. Taula		DATE		
LOCATION			Lalava River			DEPTH W/TABLE					AFTER		
PROJECT			N 13 - Nampula Cuamba			ELEVATION			97.46		JOB NO		
CLIENT			JICA / ANE			ORIENTATION			Vertical		SHEET NO		
											1 OF 2		

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DRILLING RECORD

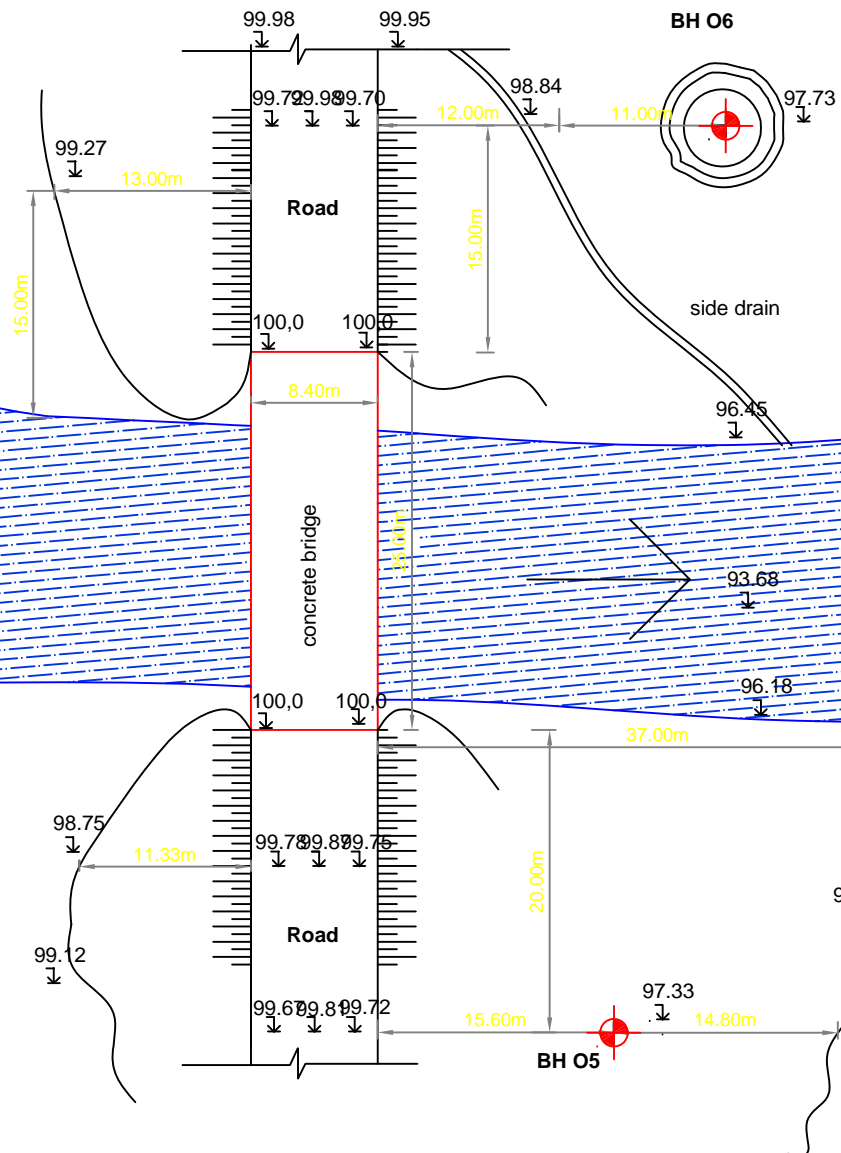
BORING NO: BH 04

DRILLING METHOD	% CORE RECOVERY	SAMPLING	TESTING	FRACTURE SPACING	FRACTURE ANGLE (DEG)				DEPTH (Meters)	PROFILE (Meters)	MATERIAL DESCRIPTION		
					15	30	45	60			Hardness/ Dimension	Color	Soil/Rock Quality
	100										Weak	Grey	Weathered granite rock
								16			End of borehole at 81.39m		
								17					
								18					
								19					
								20					
								21					
								22					
								23					
								24					
								25					
								26					
								27					
								28					
								29					
								30					
DRILLING RIG			D900 ATLAS COPCO			DATE STARTED			2006-12-28		DATE COMP.		
CONTRACTOR						LOGGED BY			S. Taula		DATE		2007-01-25
LOCATION			Lalava River			DEPTH W/TABLE					AFTER		2007-01-28
PROJECT			N 13 - Nampula Cuamba			ELEVATION			97.46 m		JOB NO		P 318
CLIENT			JICA / ANE			ORIENTATION			Vertical		SHEET NO		2 OF 2

ROAD N13 - NAMPULA/ CUAMBA
 NATALEIA BRIDGE, 09/01/2007
 BOREHOLE 05 & 06 - MALEMA / NAMPULA
 CH = 230 km

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COORDINATES

<u>BOREHOLE 05</u>	<u>BOREHOLE 06</u>
X=0338602	X=0338799
Y=8347016	Y=8347018
Z=599	Z=612

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DRILLING RECORD

BORING NO: BH 05

DRILLING METHOD	% CORE RECOVERY	SAMPLING	TESTING	FRACTURE SPACING	FRACTURE ANGLE (DEG)				DEPTH (Meters)	PROFILE (Meters)	MATERIAL DESCRIPTION		
					15	30	45	60			Hardness/ Dimension	Color	Soil/Rock Quality
	100		SPT N=20					1		Medium grained	Dark brown	Sand	
	100		SPT N>50					2		Stiff silty clay	Brown	Weathered fracture granite rock	
			SPT N>50					3					
	100	UCS S1V1						4		Hard	Grey	Unweathered granite	
		UCS S2V1						5					
							6						
							7						
							8						
							9		End of borehole at 88.83 m				
							10						
							11						
							12						
							13						
							14						
							15						
DRILLING RIG			D900 ATLAS COPCO			DATE STARTED			2007-01-09	DATE COMP.		2006-09-01	
CONTRACTOR						LOGGED BY			S.TAULA	DATE		2007-01-30	
LOCATION			NATALEIA RIVER, MALEMA			DEPTH W/TABLE				AFTER			
PROJECT			N13 - NAMPULA CUAMBA			ELEVATION			97.33 m	JOB NO		P318	
CLIENT			JICA / ANE			ORIENTATION			VERTICAL	SHEET NO		1 OF 1	

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 MAPUTO-MOCAMBIQUE

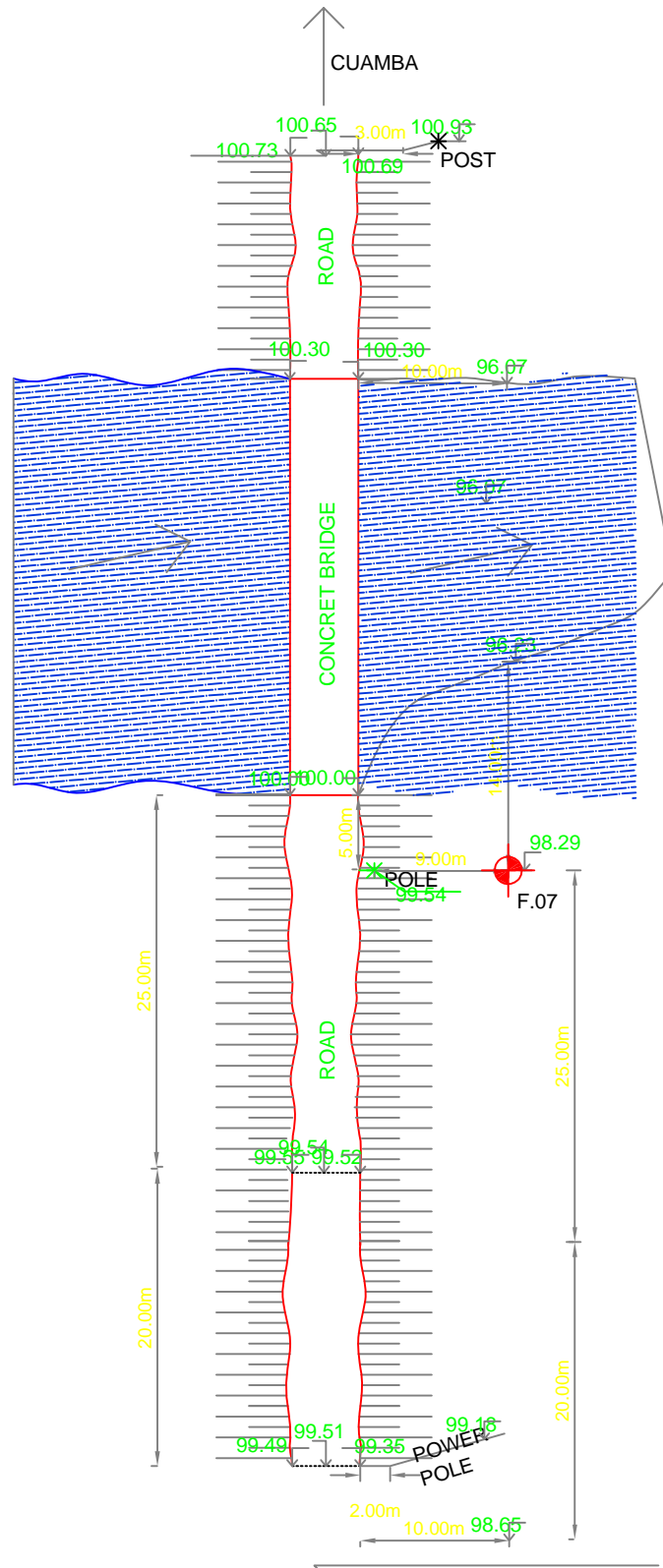
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DRILLING RECORD

BORING NO: BH 06

DRILLING METHOD	% CORE RECOVERY	SAMPLING	TESTING	FRACTURE SPACING	FRACTURE ANGLE (DEG)				DEPTH (Meters)	PROFILE (Meters)	MATERIAL DESCRIPTION		
					15	30	45	60			Hardness/ Dimension	Color	Soil/Rock Quality
	100										Medium grained	Brown	Sand
	100	UCS S1V1						1					
	100	UCS S2V1						2					
								3		Hard	Grey	Sound granite	
								4					
								5					
								6		End of borehole a 92.18 m			
								7					
								8					
								9					
								10					
								11					
								12					
								13					
								14					
								15					
DRILLING RIG	D900 ATLAS COPCO			DATE STARTED	2006-12-28	DATE COMP.	2006-12-28						
CONTRACTOR				LOGGED BY	S. Taula	DATE	2007-01-30						
LOCATION	Nataleia River ,Malema			DEPTH W/TABLE		AFTER							
PROJECT	N 13 - Nampula Cuamba			ELEVATION	97.93 m	JOB NO	P 318						
CLIENT	JICA / ANE			ORIENTATION	Vertical	SHEET NO	1 OF 1						

ROAD N13 - NAMPULA/ CUAMBA
 MUTIVASSE BRIDGE, 27/12/2006
 BOREHOLE 07 - MALEMA / NAMPULA
 CH = 239.80 Km



BOREHOLE COORDINATES:
 X = 0329917, Y = 8346484, Z=608
 COORDINATE SYSTEM: UTM

DRAWING NOT TO BE SCALED

NAMPULA

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DRILLING RECORD

BORING NO: BH 08

DRILLING METHOD	% CORE RECOVERY	SAMPLING	TESTING	FRACTURE SPACING	FRACTURE ANGLE (DEG)				DEPTH (Meters)	PROFILE (Meters)	MATERIAL DESCRIPTION			
					15	30	45	60			Hardness/ Dimension	Color	Soil/Rock Quality	
											Fine grained	Dark brown	Clayey Sand	
			SPT N=22					1						
			SPT N=3					2						
			SPT N=2					3						
			SPT N=4					4			Medium to coarse grained	Grey to brown	Sand	
			SPT N=10					5						
			SPT N>50					6						
	100		SPT N>50					7						
			SPT N>50					8						
			SPT N>50					9			Very weak to weak	Grey	Weathered granite rock	
			SPT N>50					10						
			SPT N>50					11						
	100	UCS S1V1						12						
								13			Hard to very hard	Grey	Sound granite rock	
								14						
								15			End of borehole at 83.30 m			
DRILLING RIG			D900 ATLAS COPCO			DATE STARTED			2006-12-23		DATE COMP.		2006-12-24	
CONTRACTOR						LOGGED BY			S.TAULA		DATE		2007-01-30	
LOCATION			NONOLA RIVER, NACATA			DEPTH W/TABLE					AFTER			
PROJECT			N13 - NAMPULA CUAMBA			ELEVATION			98.55 m		JOB NO		P318	
CLIENT			JICA / ANE			ORIENTATION			VERTICAL		SHEET NO		1 OF 1	

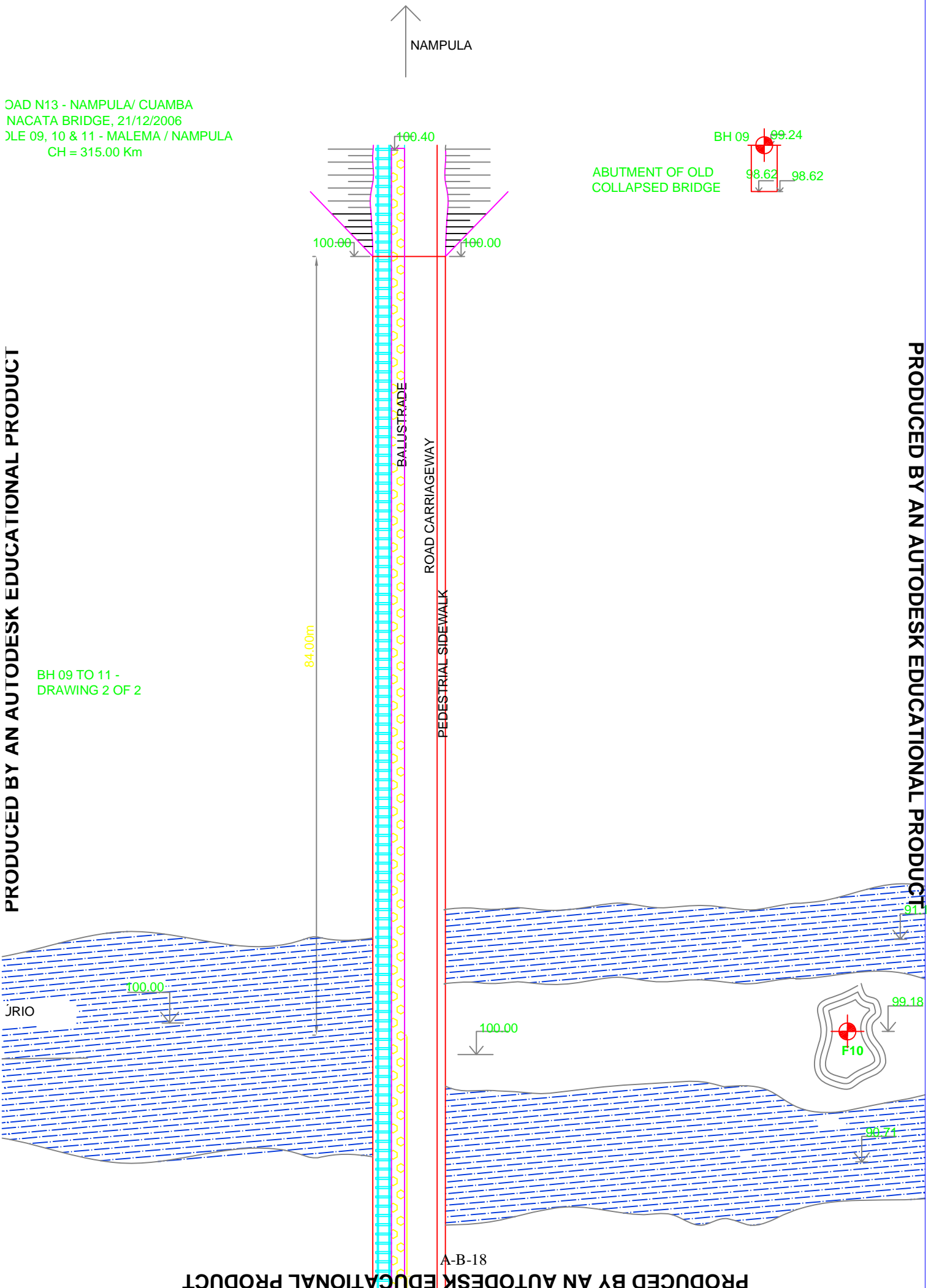
OAD N13 - NAMPULA/ CUAMBA
NACATA BRIDGE, 21/12/2006
JLE 09, 10 & 11 - MALEMA / NAMPULA
CH = 315.00 Km

NAMPULA
↑

BH 09 99.24
98.62 98.62
ABUTMENT OF OLD
COLLAPSED BRIDGE

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BH 09 TO 11 -
DRAWING 2 OF 2

84.00m

100.40

100.00

100.00

100.00

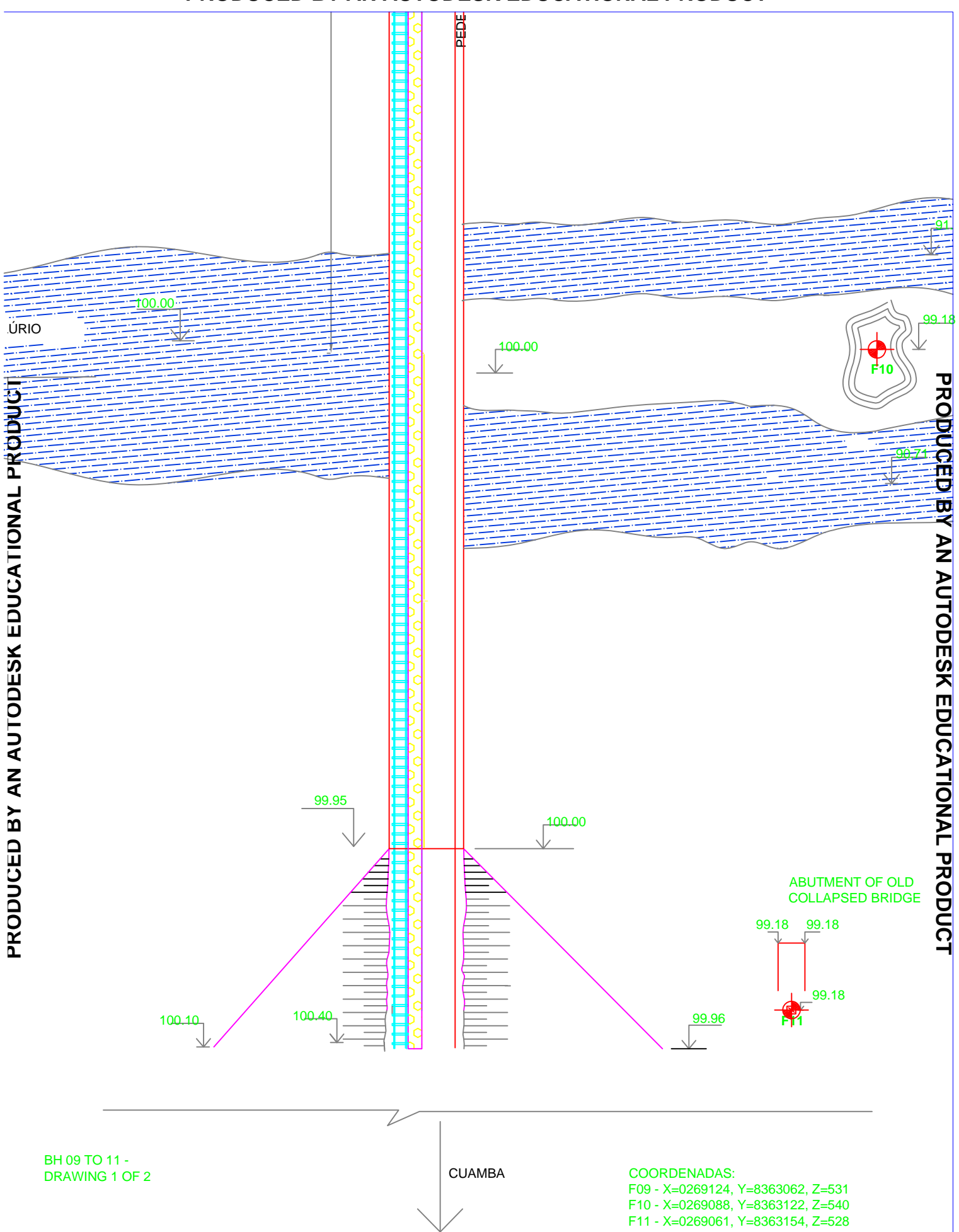
100.00

99.18

99.18

90.74

A-B-18



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BH 09 TO 11 -
DRAWING 1 OF 2

CUAMBA

COORDENADAS:
 F09 - X=0269124, Y=8363062, Z=531
 F10 - X=0269088, Y=8363122, Z=540
 F11 - X=0269061, Y=8363154, Z=528

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DRILLING RECORD

BORING NO: BH 09

DRILLING METHOD	% CORE RECOVERY	SAMPLING	TESTING	FRACTURE SPACING	FRACTURE ANGLE (DEG)				DEPTH (Meters)	PROFILE (Meters)	MATERIAL DESCRIPTION								
					15	30	45	60			Hardness/ Dimension	Color	Soil/Rock Quality						
			SPT N=24					1		Fine grained	Dark brown	Silty clay							
			SPT N=11					2											
			SPT N=13					3											
			SPT N=22					4											
			SPT N=16					5		Fine to medium grained	Grey to dark brown	Sand							
			SPT N=16					6											
			SPT N=16					7											
			SPT N=21					8		Coarse grained	Brown	Sand							
								9											
		UCS S1V1						10											
	100	UCS S2V1						11											
								12		Hard	Grey	Sound granite rock							
								13											
								14											
								15											
											End of borehole at 84.29 m								
DRILLING RIG			D900 ATLAS COPCO				DATE STARTED			2006-12-28		DATE COMP.		2006-12-28					
CONTRACTOR							LOGGED BY			S. Taula		DATE		2007-01-30					
LOCATION			Lurio River, Malema				DEPTH W/TABLE					AFTER							
PROJECT			N 13 - Nampula , Cuamba				ELEVATION			98.29		JOB NO		P 318					
CLIENT			JICA / ANE				ORIENTATION			Vertical		SHEET NO		1 OF 1					

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DRILLING RECORD

BORING NO: BH 10

DRILLING METHOD	% CORE RECOVERY	SAMPLING	TESTING	FRACTURE SPACING	FRACTURE ANGLE (DEG)				DEPTH (Meters)	PROFILE (Meters)	MATERIAL DESCRIPTION							
					15	30	45	60			Hardness/ Dimension	Color	Soil/Rock Quality					
			SPT * N=3/15						1		Fine to medium grained	Brown	Clayey sand					
			SPT N>50						2		Coarse grained		Sand mixed with gravel and boulders					
		UCS S1V1							3		Concrete from the foundation of an old bridge							
	100	UCS S2V1 S2V2 S2V3							4									
									5		Hard	Grey	Sound granite rock					
									6									
									7									
									8		End of borehole at 84.99 m NOTE: * SPT refusal when concrete was hit.							
									9									
									10									
									11									
									12									
									13									
									14									
									15									
DRILLING RIG			D900 ATLAS COPCO			DATE STARTED			2006-12-28	DATE COMP.		2006-12-28						
CONTRACTOR						LOGGED BY			S . Taula	DATE		2007-01-30						
LOCATION			Lurio river , Malema			DEPTH W/TABLE				AFTER								
PROJECT			N 13 - Nampula Cuamba			ELEVATION			92.49	JOB NO		P 318						
CLIENT			JICA / ANE			ORIENTATION			VERTICAL	SHEET NO		1 OF 1						

Appendix B-2
UCS TEST RESULTS ROCK CORES

TÉCNICA - ENGENHEIROS CONSULTORES, LDA

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BOREHOLE NO 05
SITE: NATALEMA RIVER - MALEMA / NAMPULA
UNCONFINED COMPRESSIVE STRENGTH (UCS)

DRY SAMPLE

Sample	Depth (m)	Weight (kg)	Height (mm)	Diameter (mm)	Area (mm ²)	Total Failure Load (N)	Failure Strength (Mpa)	Average Failure Strength (Mpa)
S1V1	3.75	1.300	87.5	87.5	5765.4	446875	77.5	77.5

SATURATED SAMPLE

Sample	Depth (m)	Weight (kg)	Height (mm)	Diameter (mm)	Area (mm ²)	Total Failure Load (N)	Failure Strength (Mpa)	Average Failure Strength (Mpa)
S1V1	3.75	1.250	85.7	85.7	5765.4	340000	59.0	58.6
S1V1	3.75	1.350	85.7	85.7	5765.4	340000	58.3	
S2V1	5.00	0.640	66.3	66.3	3450.6	300000	86.9	86.9

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BOREHOLE NO 06
SITE: NATALEMA RIVER - MALEMA / NAMPULA
UNCONFINED COMPRESSIVE STRENGTH (UCS)

DRY SAMPLE

Sample	Depth (m)	Weight (kg)	Height (mm)	Diameter (mm)	Area (mm ²)	Total Failure Load (N)	Failure Strength (Mpa)	Average Failure Strength (Mpa)
S1V1	2.00	0.640	66.3	66.3	3450.6	337500	97.8	98.7
S1V1	2.00	0.620	66.3	66.3	3450.6	343750	99.6	
S2V1	3.75	0.630	66.3	66.3	3450.6	322500	108.3	108.3

SATURATED SAMPLE

Sample	Depth (m)	Weight (kg)	Height (mm)	Diameter (mm)	Area (mm ²)	Total Failure Load (N)	Failure Strength (Mpa)	Average Failure Strength (Mpa)
S1V1	2.00	0.630	66.3	66.3	3450.6	375000	108.7	108.7
S1V1	2.00	0.620	66.3	66.3	3450.6	375000	108.7	
S2V1	3.75	0.620	66.3	66.3	3450.6	368750	106.9	107.8
S2V1	3.75	0.620	66.3	66.3	3450.6	365000	108.7	

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BOREHOLE NO 07
SITE: NATALEMA RIVER - MALEMA / NAMPULA
UNCONFINED COMPRESSIVE STRENGTH (UCS)

DRY SAMPLE

Sample	Depth (m)	Weight (kg)	Height (mm)	Diameter (mm)	Area (mm ²)	Total Failure Load (N)	Failure Strength (Mpa)	Average Failure Strength (Mpa)
S2V1	8.00	0.620	66.3	66.3	3450.6	336300	97.5	97.2
S2V1	8.00	0.600	66.3	66.3	3450.6	334500	96.9	
S2V1	8.00	0.610	66.3	66.3	3450.6	334600	97.0	
S2V1	8.00	0.620	66.3	66.3	3450.6	335600	97.3	97.1
S3V1	10.00	0.600	66.3	66.3	3450.6	261250	75.7	75.7
S3V1	10.00	0.610	66.3	66.3	3450.6	331250	96.0	94.1
S3V1	10.00	0.600	66.3	66.3	3450.6	331150	96.0	
S3V1	10.00	0.620	66.3	66.3	3450.6	336400	97.5	
S3V1	10.00	0.620	66.3	66.3	3450.6	300000	86.9	

SATURATED SAMPLE

Sample	Depth (m)	Weight (kg)	Height (mm)	Diameter (mm)	Area (mm ²)	Total Failure Load (N)	Failure Strength (Mpa)	Average Failure Strength (Mpa)
S1V1	6.50	1.300	85.7	85.7	57.65.4	237500	41.20	41.2
S1V1	6.50	1.300	85.7	85.7	5765.4	237500	41.20	
S2V1	8.00	0.640	66.3	66.3	3450.6	231250	67.00	67
S2V2	8.00	0.600	66.3	66.3	3450.6	187500	54.30	57.9
S2V2	8.00	0.63	66.3	66.3	3450.6	200000	58.00	
S2V2	8.00	0.6	66.3	66.3	3450.6	181250	52.50	
S2V3	8.00	0.61	66.3	66.3	3450.6	212500	61.60	61.3
S2V3	8.00	0.6	66.3	66.3	3450.6	250625	61.00	
S3V1	10.00	0.62	66.3	66.3	3450.6	177500	51.40	54.9
S3V1	10.00	0.61	66.3	66.3	3450.6	201250	58.30	
S3V1	10.00	0.63	66.3	66.3	3450.6	200000	58.00	57.4
S3V1	10.00	0.62	66.3	66.3	3450.6	181250	52.50	
S3V1	10.00	0.62	66.3	66.3	3450.6	212500	61.60	

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BOREHOLE NO 08
SITE: NATALEMA RIVER - MALEMA / NAMPULA
UNCONFINED COMPRESSIVE STRENGTH (UCS)

DRY SAMPLE

Sample	Depth (m)	Weight (kg)	Height (mm)	Diameter (mm)	Area (mm ²)	Total Failure Load (N)	Failure Strength (Mpa)	Average Failure Strength (Mpa)
S1V1	11.25	0.680	66.3	66.3	368750.0	337500	106.9	103.5
S1V1	11.25	0.650	66.3	66.3	367850.0	343750	106.9	
S1V1	11.25	0.610	66.3	66.3	335000.0	322500	97.1	

SATURATED SAMPLE

Sample	Depth (m)	Weight (kg)	Height (mm)	Diameter (mm)	Area (mm ²)	Total Failure Load (N)	Failure Strength (Mpa)	Average Failure Strength (Mpa)
S1V1	11.25	0.640	66.3	66.3	3450.6	375000	108.7	108.7
S1V1	11.25	0.620	66.3	66.3	3450.6	375000	108.7	

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BOREHOLE NO 09
SITE: NATALEMA RIVER - MALEMA / NAMPULA
UNCONFINED COMPRESSIVE STRENGTH (UCS)

DRY SAMPLE

Sample	Depth (m)	Weight (kg)	Height (mm)	Diameter (mm)	Area (mm ²)	Total Failure Load (N)	Failure Strength (Mpa)	Average Failure Strength (Mpa)
S1V1	9.75	1.240	85.7	85.7	5765.4	496875	86.2	86.2
S1V2	9.75	1.200	85.7	85.7	5765.4	750000	130.1	130.1
S1V2	9.75	1.210	85.7	85.7	5765.4	750000	130.1	
S2V1	11.50	1.260	85.7	85.7	5765.4	668125	115.9	118.1
S2V1	11.50	1.270	85.7	85.7	5765.4	693125	120.2	

SATURATED SAMPLE

Sample	Depth (m)	Weight (kg)	Height (mm)	Diameter (mm)	Area (mm ²)	Total Failure Load (N)	Failure Strength (Mpa)	Average Failure Strength (Mpa)
S1V1	9.75	1.220	85.7	85.7	5765.4	756000	130.10	130.1
S1V1	9.75	1.210	85.7	85.7	5765.4	750000	130.10	
S1V2	9.75	1.230	85.7	85.7	5765.4	750000	130.10	130.1
S1V2	9.75	1.220	85.7	85.7	5765.4	750000	130.10	
S2V1	11.50	0.63	66.3	85.7	5765.4	593750	103.00	101.9
S2V1	11.50	0.6	66.3	85.7	5765.4	581250	100.80	

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BOREHOLE NO 10
SITE: NATALEMA RIVER - MALEMA / NAMPULA
UNCONFINED COMPRESSIVE STRENGTH (UCS)

DRY SAMPLE

Sample	Depth (m)	Weight (kg)	Height (mm)	Diameter (mm)	Area (mm ²)	Total Failure Load (N)	Failure Strength (Mpa)	Average Failure Strength (Mpa)
S1V1	2.50	1.330	85.7	85.7	5765.4	276875	48.0	48.0
S2V1	3.00	1.240	85.7	85.7	5765.4	284375	49.3	49.3
S2V2	3.00	1.360	85.7	85.7	5765.4	450000	78.1	78.1
S2V3	3.00	1.240	85.7	85.7	5765.4	493125	75.5	75.5

SATURATED SAMPLE

Sample	Depth (m)	Weight (kg)	Height (mm)	Diameter (mm)	Area (mm ²)	Total Failure Load (N)	Failure Strength (Mpa)	Average Failure Strength (Mpa)
S1V1	2.50	1.300	85.7	85.7	5765.4	212500	36.90	36.9
S2V1	3.00	1.250	85.7	85.7	5765.4	718750	124.70	124.7
S2V2	3.00	1.400	85.7	85.7	5765.4	337500	58.50	59.1
S2V2	3.00	1.400	85.7	85.7	5765.4	343750	59.60	
S2V3	3.00	1.3	85.7	85.7	5765.4	493750	85.60	84.6
S2V3	3.00	1.3	85.7	85.7	5765.4	481250	83.50	

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BOREHOLE NO 11
SITE: NATALEMA RIVER - MALEMA / NAMPULA
UNCONFINED COMPRESSIVE STRENGTH (UCS)

DRY SAMPLE

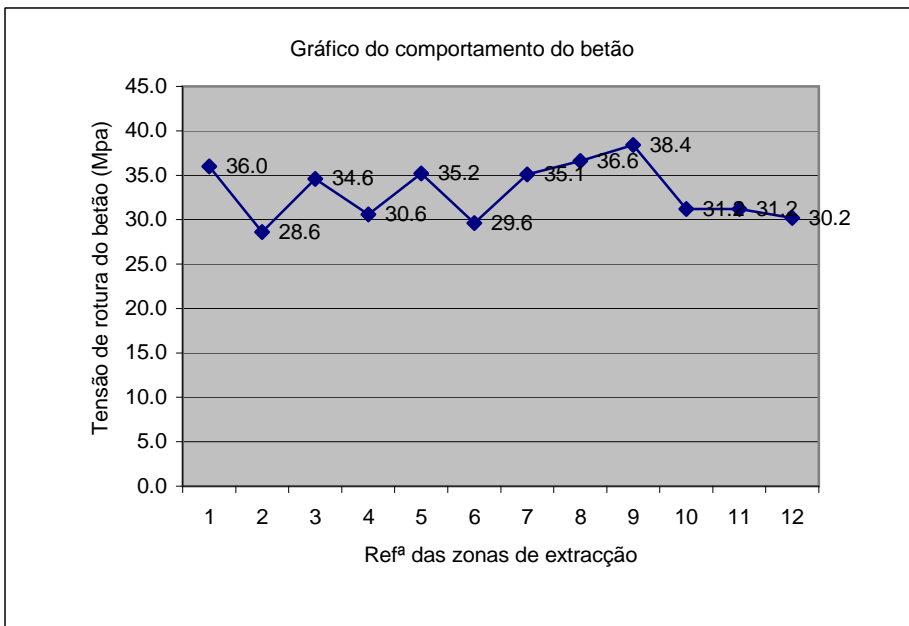
Sample	Depth (m)	Weight (kg)	Height (mm)	Diameter (mm)	Area (mm ²)	Total Failure Load (N)	Failure Strength (Mpa)	Average Failure Strength (Mpa)
S1V1	4.25	1.140	85.7	85.7	5765.4	287500	49.9	52.0
S1V1	4.25	1.140	85.7	85.7	5765.4	312500	54.2	
S1V2	4.25	1.140	85.7	85.7	5765.4	385000	66.8	66.8
S2V1	8.00	1.140	85.7	85.7	5765.4	371875	64.5	64.5

SATURATED SAMPLE

Sample	Depth (m)	Weight (kg)	Height (mm)	Diameter (mm)	Area (mm ²)	Total Failure Load (N)	Failure Strength (Mpa)	Average Failure Strength (Mpa)
S1V1	4.25	1.170	85.7	85.7	5765.4	296875	51.50	55.8
S1V1	4.25	1.160	85.7	85.7	5765.4	346875	60.20	
S1V2	4.25	1.150	85.7	85.7	5765.4	353125	61.20	61.20
S2V1	8.00	1.140	85.7	85.7	5765.4	343125	59.50	59.50

Laboratório de Engenharia de Moçambique
Departamento de Materiais de Construção e Estruturas
Tratamento Estatístico dos resultados
Registo nº.24.026

Nº de Ordem	Refª. dos Pilares	Tensão média Individual (Mpa)
1	PED-1	36.0
2	PLEFX-1	28.6
3	PPDN-1	34.6
4	PLEINT-1	30.6
5	PPEINT-1	35.2
6	PPDNEXT-1	29.6
7	PAE-1	35.1
8	PG-1	36.6
9	PES-1	38.4
10	PPN-1	31.2
11	PCE-2	31.2
12	PAE-2	30.2
Média		33.1
Desvio Padrão		3.2

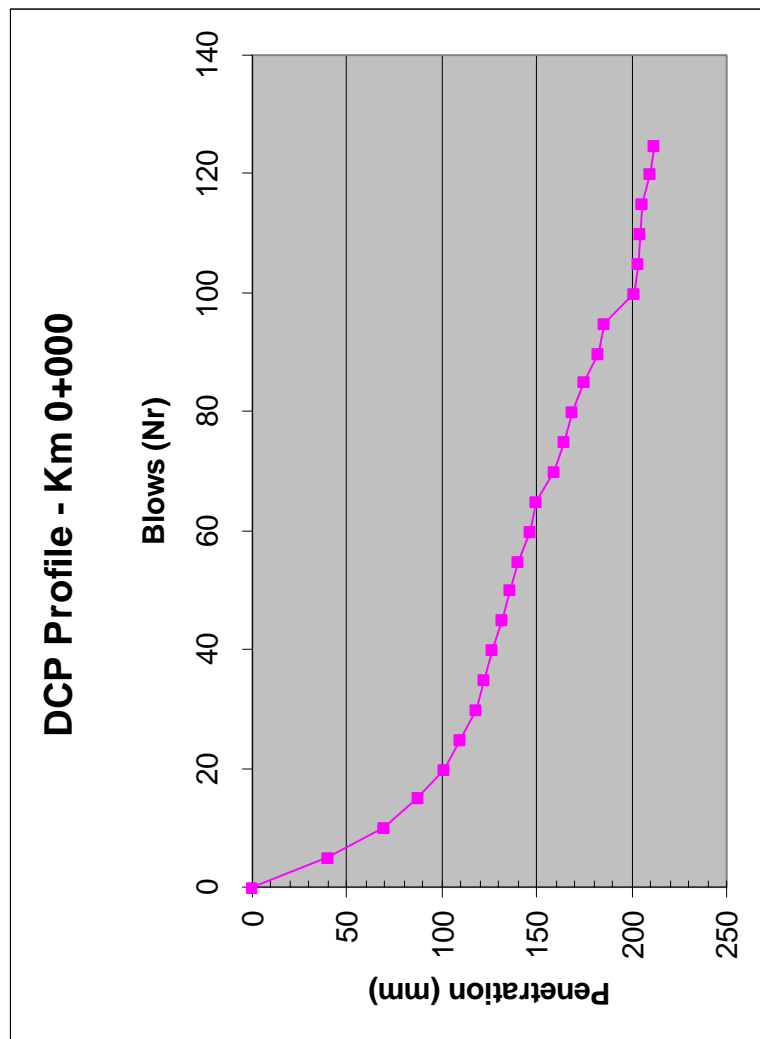


Appendix B-3
DCP TEST RESULTS

DCP Tests
Section 0+000

UTM Coordinates: X=0524455, Y=8329373

Blows (Nr)	Readings (mm)	Penetration (mm)	DN (mm/blow)	CBR in-situ (%)
0	1000.1	0		
5	960	40.1	8.02	29.1
10	930	70.1	6	42.1
15	913	87.1	3.4	86.7
20	899	101.1	2.8	110.9
25	890	110.1	1.8	185.3
30	882	118.1	1.6	206.0
35	878	122.1	0.8	>210
40	873	127.1	1	>210
45	868	132.1	1	>210
50	864	136.1	0.8	>210
55	860	140.1	0.8	>210
60	853	147.1	1.4	>210
65	850	150.1	0.6	>210
70	841	159.1	1.8	185.3
75	836	164.1	1	>210
80	831	169.1	1	>210
85	825	175.1	1.2	>210
90	818	182.1	1.4	>210
95	814	186.1	0.8	>210
100	799	201.1	3	101.6
105	797	203.1	0.4	>210
110	795	205.1	0.4	>210
115	794	206.1	0.2	>210
120	790	210.1	0.8	>210
125	788	212.1	0.4	>210

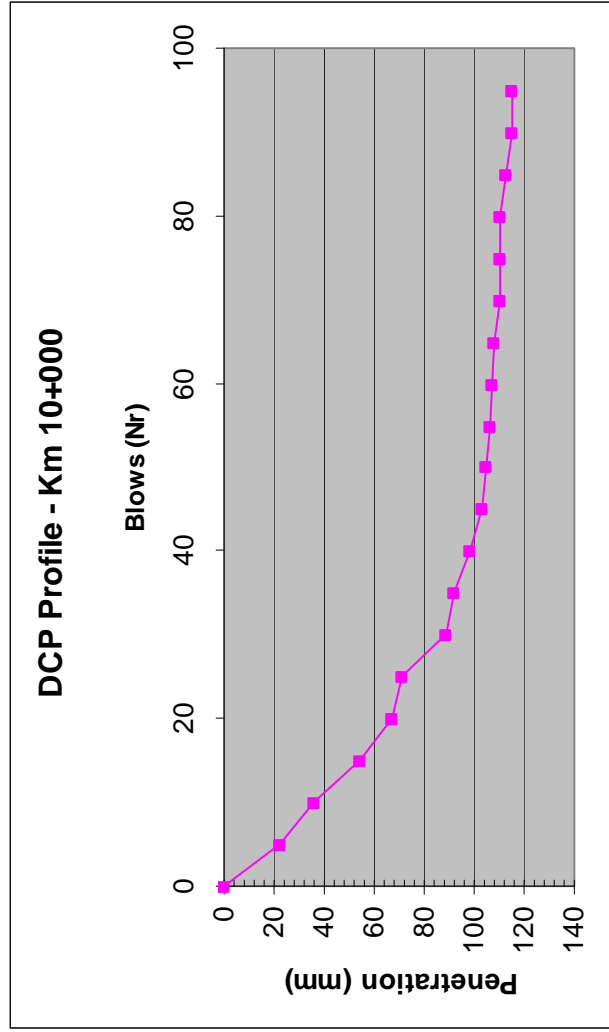


DCP Tests

Section 10+000

UTM Coordinates: X=0521848, Y=8332402

Blows	Readings	Penetration	DN (mm/blow)	CBR in- situ (%)
0	1000.1	0		
5	978	22.1	4.42	62.1
10	964	36.1	2.8	110.9
15	946	54.1	3.6	80.6
20	933	67.1	2.6	121.8
25	929	71.1	0.8	>210
30	911	89.1	3.6	80.6
35	908	92.1	0.6	>210
40	902	98.1	1.2	>210
45	897	103.1	1	>210
50	895	105.1	0.4	>210
55	894	106.1	0.2	>210
60	893	107.1	0.2	>210
65	892	108.1	0.2	>210
70	890	110.1	0.4	>210
75	890	110.1	0	>210
80	890	110.1	0	>210
85	887	113.1	0.6	>210
90	885	115.1	0.4	>210
95	885	115.1	0	>210

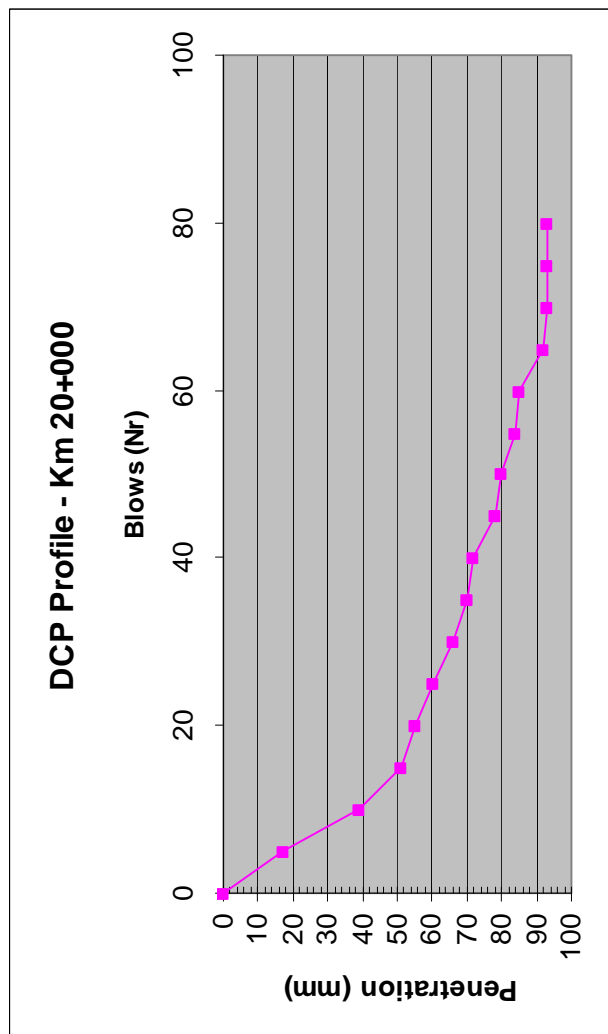


DCP Tests

Section 20+000

UTM Coordinates: X=0515002, Y=8336253

Blows (Nr)	Readings (mm)	Penetration (mm)	DN (mm/blow)	CBR in-situ (%)
0	1000.12	0		
5	983	17.12	3.424	85.9
10	961	39.12	4.4	62.5
15	949	51.12	2.4	134.9
20	945	55.12	0.8	>210
25	940	60.12	1	>210
30	934	66.12	1.2	>210
35	930	70.12	0.8	>210
40	928	72.12	0.4	>210
45	922	78.12	1.2	>210
50	920	80.12	0.4	>210
55	916	84.12	0.8	>210
60	915	85.12	0.2	>210
65	908	92.12	1.4	>210
70	907	93.12	0.2	>210
75	907	93.12	0	>210
80	907	93.12	0	>210

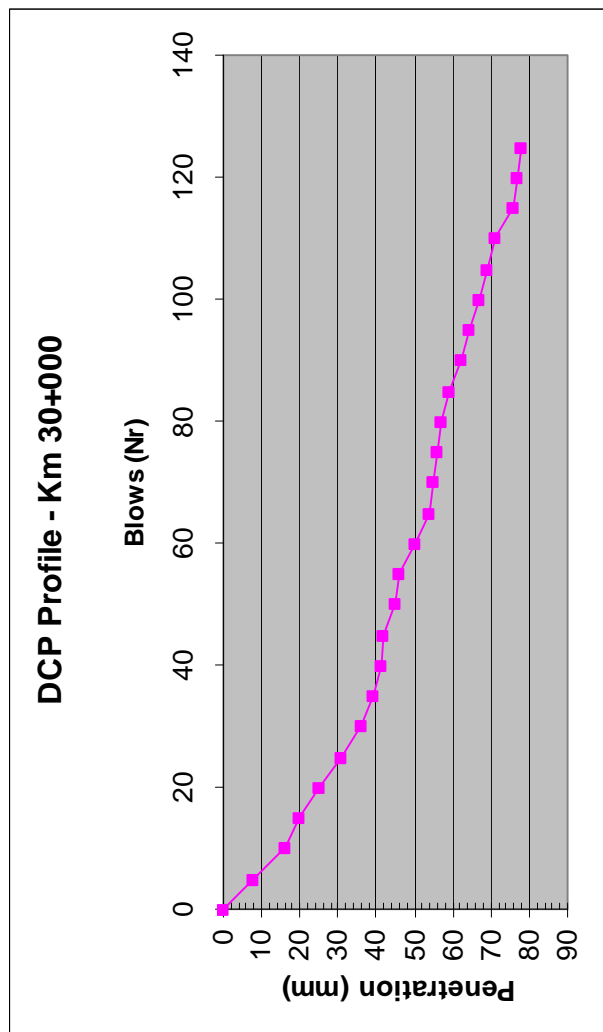


DCP Tests

Section 30+000

UTM Coordinates: X=0508883, Y=8340263

Blows (Nr)	Readings (mm)	Penetration (mm)	DN (mm/blow)	CBR in-situ (%)
0	1000.1	0		
5	992	8.1	1.62	203.7
10	984	16.1	1.6	206.0
15	980	20.1	0.8	>210
20	975	25.1	1	>210
25	969	31.1	1.2	>210
30	964	36.1	1	>210
35	961	39.1	0.6	>210
40	959	41.1	0.4	>210
45	958	42.1	0.2	>210
50	955	45.1	0.6	>210
55	954	46.1	0.2	>210
60	950	50.1	0.8	>210
65	946	54.1	0.8	>210
70	945	55.1	0.2	>210
75	944	56.1	0.2	>210
80	943	57.1	0.2	>210
85	941	59.1	0.4	>210
90	938	62.1	0.6	>210
95	936	64.1	0.4	>210
100	933	67.1	0.6	>210
105	931	69.1	0.4	>210
110	929	71.1	0.4	>210
115	924	76.1	1	>210
120	923	77.1	0.2	>210
125	922	78.1	0.2	>210

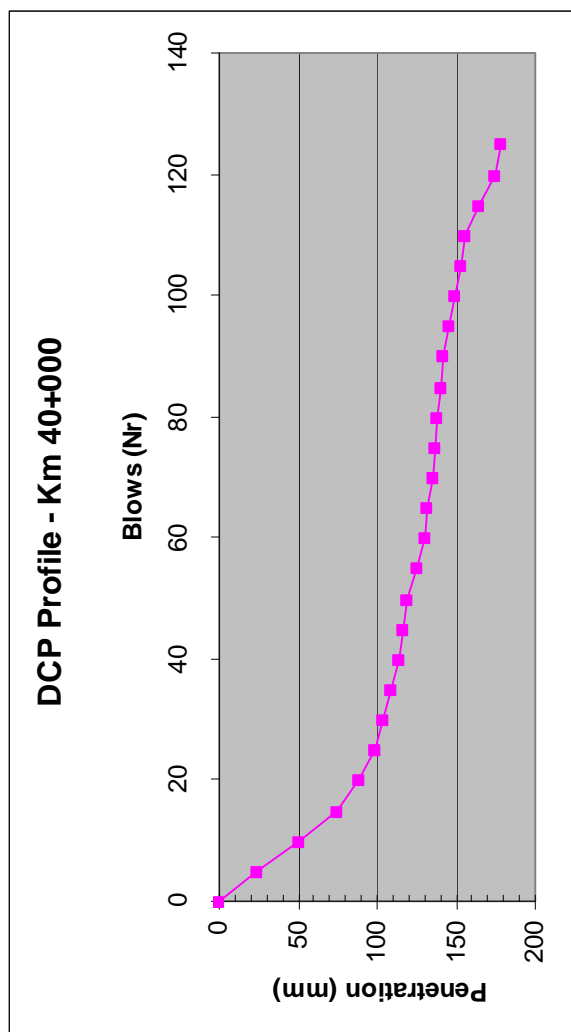


DCP Tests

Section 40+000

UTM Coordinates: X=0499651, Y=8341554

Blows (Nr)	Readings (mm)	Penetration (mm)	DN (mm/blow)	CBR in-situ (%)
0	1000	0		
5	976	24	4.8	56
10	950	50	5.2	51
15	926	74	4.8	56
20	911	89	3.0	102
25	901	99	2.0	170
30	896	104	1.0	>210
35	891	109	1.0	>210
40	886	114	1.0	>210
45	884	116	0.4	>210
50	881	119	0.6	>210
55	875	125	1.2	>210
60	870	130	1.0	>210
65	868	132	0.4	>210
70	865	135	0.6	>210
75	864	136	0.2	>210
80	862	138	0.4	>210
85	860	140	0.4	>210
90	858	142	0.4	>210
95	854	146	0.8	>210
100	851	149	0.6	>210
105	847	153	0.8	>210
110	845	155	0.4	>210
115	835	165	2.0	170
120	825	175	2.0	170
125	821	179	0.8	>210

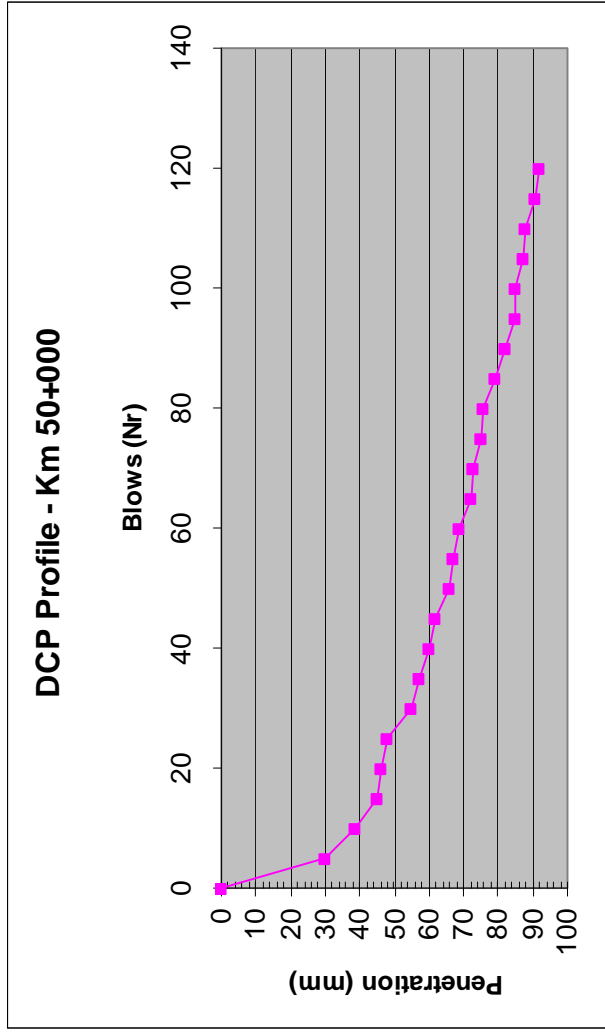


DCP Tests

Section 50+000

UTM Coordinates: X=0490613, Y=8343436

Blows (Nr)	Readings (mm)	Penetration (mm)	DN (mm/blow)	CBR in-situ (%)
0	1000	0		
5	970	30	6.0	42
10	961	39	1.8	185
15	955	45	1.2	>210
20	954	46	0.2	>210
25	952	48	0.4	>210
30	945	55	1.4	>210
35	943	57	0.4	>210
40	940	60	0.6	>210
45	938	62	0.4	>210
50	934	66	0.8	>210
55	933	67	0.2	>210
60	931	69	0.4	>210
65	928	72	0.6	>210
70	927	73	0.2	>210
75	925	75	0.4	>210
80	924	76	0.2	>210
85	921	79	0.6	>210
90	918	82	0.6	>210
95	915	85	0.6	>210
100	915	85	0.0	>210
105	913	87	0.4	>210
110	912	88	0.2	>210
115	909	91	0.6	>210
120	908	92	0.2	>210

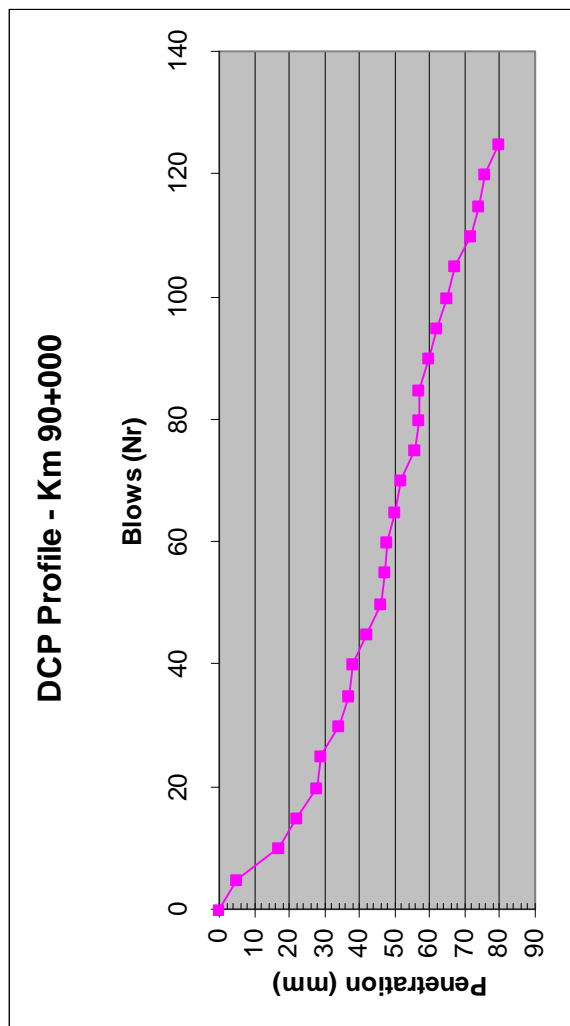


DCP Tests

Section 90+000

UTM Coordinates: X=0459081, Y=8346001

Blows (Nr)	Readings (mm)	Penetration (mm)	DN (mm/blow)	CBR in-situ (%)
0	1000	0		
5	995	5	1.0	>210
10	983	17	2.4	135
15	978	22	1.0	>210
20	972	28	1.2	>210
25	971	29	0.2	>210
30	966	34	1.0	>210
35	963	37	0.6	>210
40	962	38	0.2	>210
45	958	42	0.8	>210
50	954	46	0.8	>210
55	953	47	0.2	>210
60	952	48	0.2	>210
65	950	50	0.4	>210
70	948	52	0.4	>210
75	944	56	0.8	>210
80	943	57	0.2	>210
85	943	57	0.0	>210
90	940	60	0.6	>210
95	938	62	0.4	>210
100	935	65	0.6	>210
105	933	67	0.4	>210
110	928	72	1.0	>210
115	926	74	0.4	>210
120	924	76	0.4	>210
125	920	80	0.8	>210

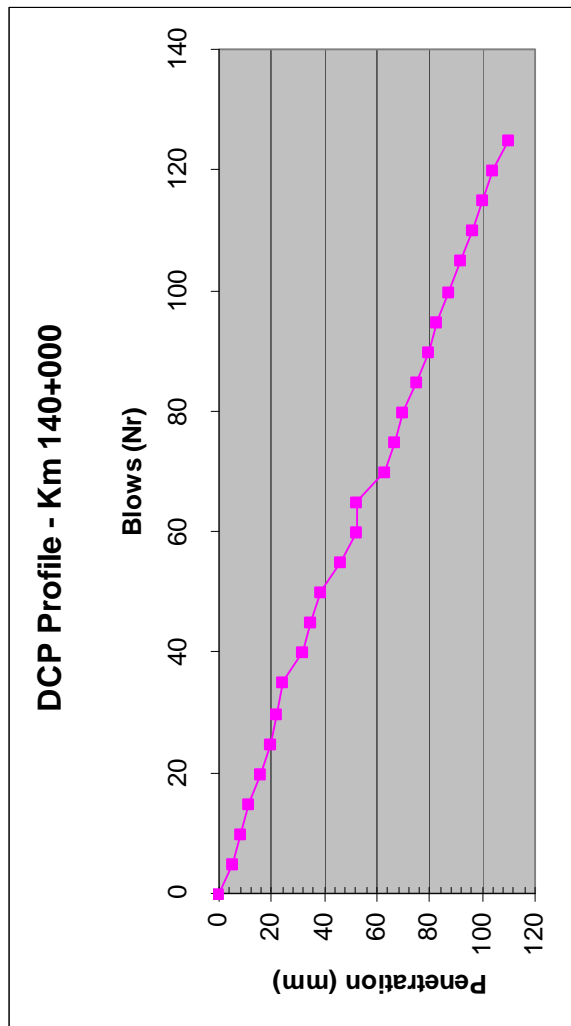


DCP Tests

Section 140+000

UTM Coordinates: X=0426920, Y=8348122

Blows (Nr)	Readings (mm)	Penetration (mm)	DN (mm/blow)	CBR in-situ (%)
0	0	1000		
5.09	5	995	1.0	>210
8.09	10	992	0.6	>210
11.09	15	989	0.6	>210
16.09	20	984	1.0	>210
20.09	25	980	0.8	>210
22.09	30	978	0.4	>210
24.09	35	976	0.4	>210
32.09	40	968	1.6	206
35.09	45	965	0.6	>210
39.09	50	961	0.8	>210
46.09	55	954	1.4	>210
52.09	60	948	1.2	>210
52.09	65	948	0.0	>210
63.09	70	937	2.2	151
67.09	75	933	0.8	>210
70.09	80	930	0.6	>210
75.09	85	925	1.0	>210
80.09	90	920	1.0	>210
83.09	95	917	0.6	>210
87.09	100	913	0.8	>210
92.09	105	908	1.0	>210
96.09	110	904	0.8	>210
100.09	115	900	0.8	>210
104.09	120	896	0.8	>210
110.09	125	890	1.2	>210

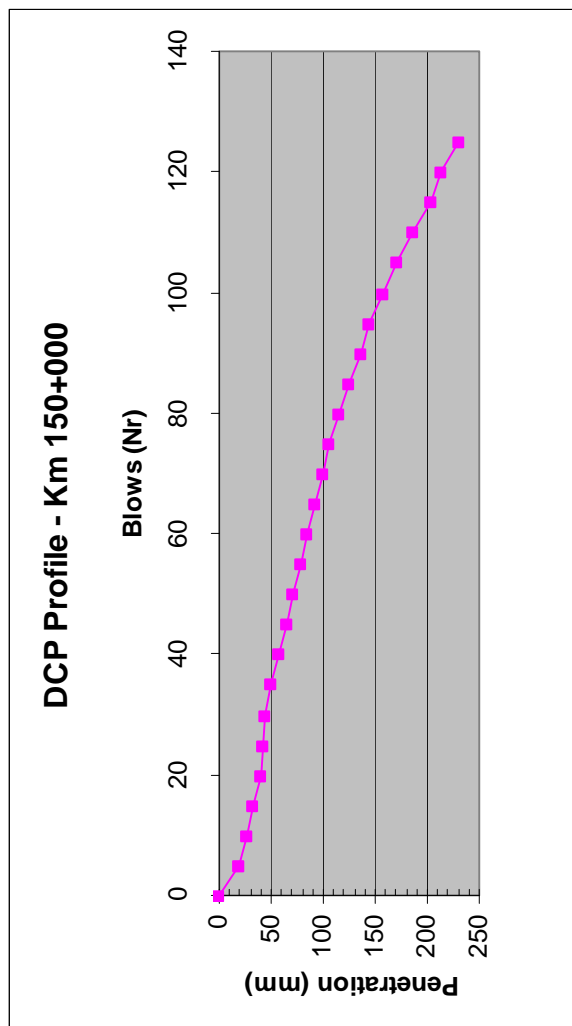


DCP Tests

Section 150+000

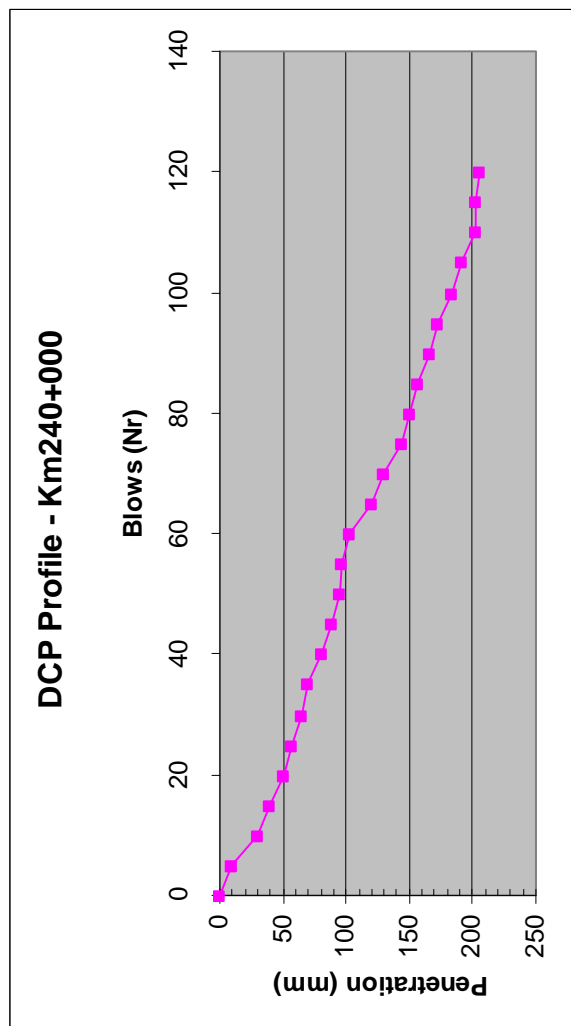
UTM Coordinates: X=0417824, Y=8344814

Blows (Nr)	Readings (mm)	Penetration (mm)	DN (mm/blow)	CBR in-situ (%)
0	1000	0		
5	981	19	3.8	75
10	973	27	1.6	206
15	967	33	1.2	>210
20	959	41	1.6	206
25	958	42	0.2	>210
30	955	45	0.6	>210
35	950	50	1.0	>210
40	942	58	1.6	206
45	934	66	1.6	206
50	929	71	1.0	>210
55	922	78	1.4	>210
60	916	84	1.2	>210
65	908	92	1.6	206
70	901	99	1.4	>210
75	895	105	1.2	>210
80	885	115	2.0	170
85	876	124	1.8	185
90	864	136	2.4	135
95	856	144	1.6	206
100	843	157	2.6	122
105	829	171	2.8	111
110	814	186	3.0	102
115	797	203	3.4	87
120	787	213	2.1	160
125	770	230	3.4	87



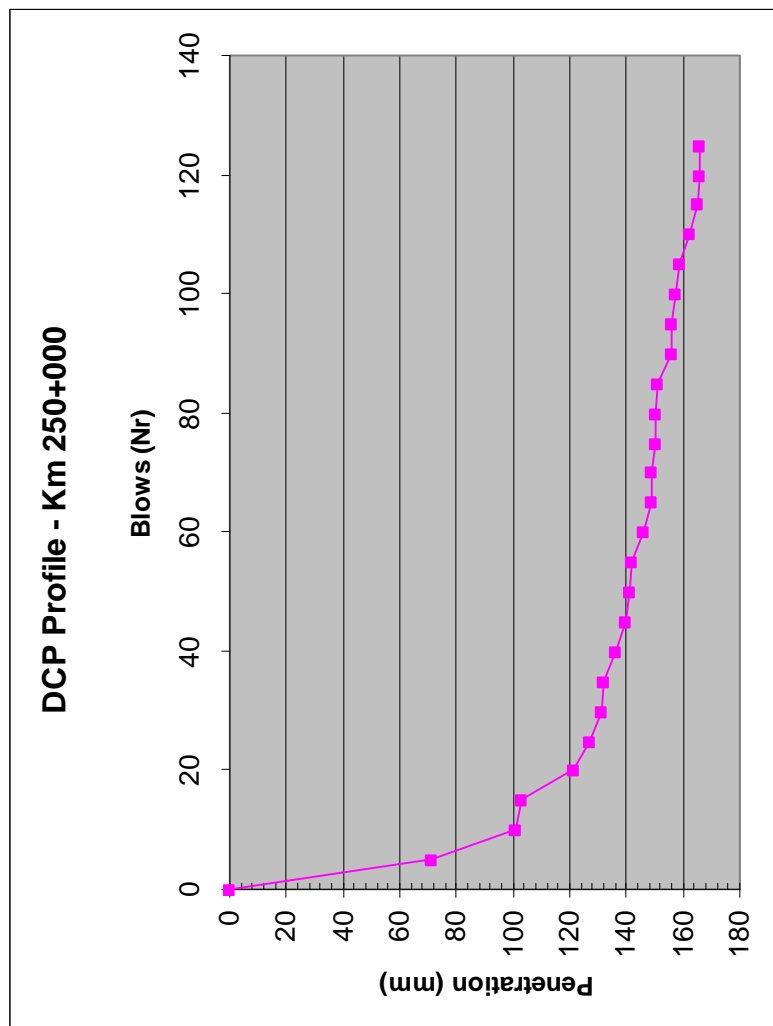
DCP Tests
Section 240+000
UTM Coordinates: X=0829914, Y=8346467

Blows (Nr)	Readings (mm)	Penetration (mm)	DN (mm/blow)	CBR in-situ (%)
0	1000	0		
5	990	10	2.0	168
10	970	30	4.0	70
15	960	40	2.0	170
20	950	50	2.0	170
25	943	57	1.4	>210
30	935	65	1.6	206
35	930	70	1.0	>210
40	920	80	2.0	170
45	911	89	1.8	185
50	905	95	1.2	>210
55	903	97	0.4	>210
60	897	103	1.2	>210
65	880	120	3.4	87
70	870	130	2.0	170
75	856	144	2.8	111
80	850	150	1.2	>210
85	843	157	1.4	>210
90	834	166	1.8	185
95	828	172	1.2	>210
100	816	184	2.4	135
105	809	191	1.4	>210
110	798	202	2.2	151
115	798	202	0.0	>210
120	795	205	0.6	>210



DCP Tests
Section 250+000
UTM Coordinates: X=0321870, Y=8344650

Blows (Nr)	Readings (mm)	Penetration (mm)	DN (mm/blow)	CBR in-situ (%)
0	1000	0		
5	929	71	14.2	14
10	899	101	6.0	42
15	897	103	0.4	>210
20	879	121	3.6	81
25	873	127	1.2	>210
30	869	131	0.8	>210
35	868	132	0.2	>210
40	864	136	0.8	>210
45	860	140	0.8	>210
50	859	141	0.2	>210
55	858	142	0.2	>210
60	854	146	0.8	>210
65	851	149	0.6	>210
70	851	149	0.0	>210
75	850	150	0.2	>210
80	850	150	0.0	>210
85	849	151	0.2	>210
90	844	156	1.0	>210
95	844	156	0.0	>210
100	843	157	0.2	>210
105	841	159	0.4	>210
110	838	162	0.6	>210
115	835	165	0.6	>210
120	834	166	0.2	>210
125	834	166	0.0	>210

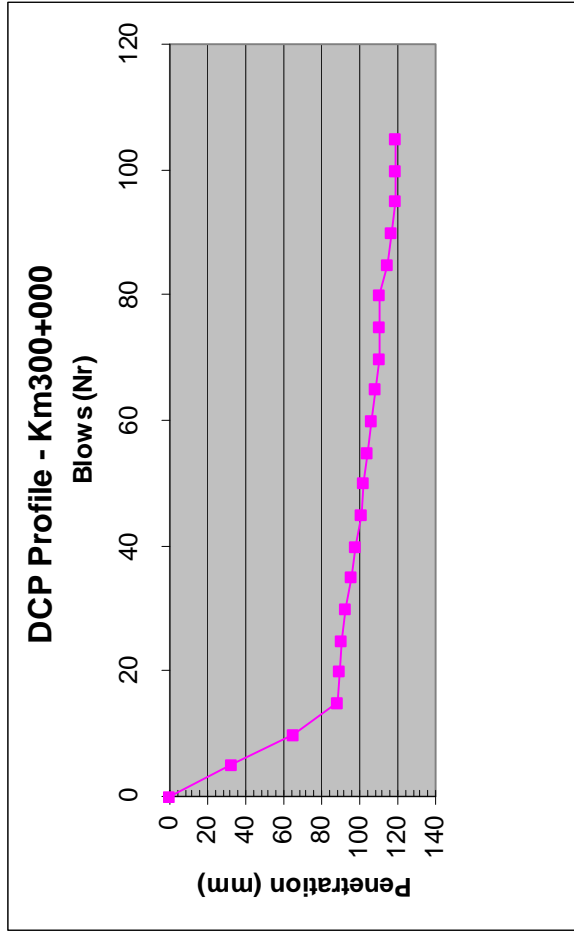


DCP Tests

Section 300+000

UTM Coordinates: X=0280865, Y=8363820

Blows (Nr)	Readings (mm)	Penetration (mm)	DN (mm/blow)	CBR in-situ (%)
0	1000	0		
5	967	33	6.6	37
10	935	65	6.4	39
15	912	88	4.6	59
20	911	89	0.2	>210
25	909	91	0.4	>210
30	907	93	0.4	>210
35	904	96	0.6	>210
40	902	98	0.4	>210
45	899	101	0.6	>210
50	898	102	0.2	>210
55	896	104	0.4	>210
60	894	106	0.4	>210
65	892	108	0.4	>210
70	890	110	0.4	>210
75	889	111	0.2	>210
80	889	111	0.0	>210
85	885	115	0.8	>210
90	883	117	0.4	>210
95	881	119	0.4	>210
100	881	119	0.0	>210
105	881	119	0.0	>210



Appendix B-4
CBR TEST FOR SUBGRADE

CONSULTOR / EMPREITEIRO : TÉC. ENGS. CONSULTORES	DATA: <u>13-11-06</u>	ENSAIO DE COMPACTAÇÃO	
PROJECTO : N13 NAMPULA/CUAMBA	VERIF. POR: _____	Barid. Seca Máx. <u>2.086</u>	gr/cm3
CAMADA ENSAIADA : BASE - NATURAL Km: 15+00 L/E	EFFECT. POR : _____	Teor Ópt. Água <u>9,8</u>	%
AMOSTRA Nº : <u>NAM/0027/06</u>	RAFAEL		
DESCRIÇÃO DO SOLO : <u>AREIA ARGILOSA (SAIBRO)</u>			

DETERMINAÇÃO DO TEOR DE HUMIDADE

Água a Adicionar

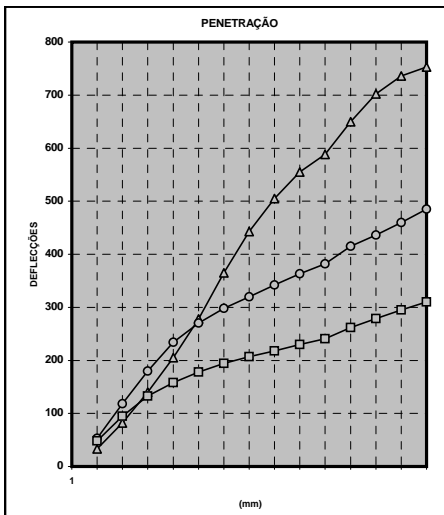
Nº da Cápsula/Molde	Nº	Hygroscópico			Depois da Mistura			Ótimo % (a)	9.8
		6	7	8	6	7	8		
Peso do Solo Húmido + Cápsula (P0)	gr	303.60	299.80	294.1	Hygroscópico % (b)	1.7			
Peso do Solo Seco + Cápsula (P1)	gr	283.40	279.10	274.8	Volume de Água =	8.1			
Peso da Cápsula (P2)	gr	84.00	84.60	85	Solo seco x(100+a) - Solo seco =	100			
Peso da Água (P3)=(P0)-(P1)	gr	20.20	20.70	19.30	=	1434			
Peso do Solo Seco (P4)=(P1)-(P2)	gr	199.40	194.50	189.80					
Teor de Humidade W=(P3)/(P4)x100	%	1.7	10.1	10.6					

Moldagem

Expansão

Compacção	Nº	Nº	55x10x5	25x10x5	55x5,5x3
			4	5	6
Peso do Molde + Solo Húmido (M0)	gr	11052	10825	10534	
Peso do Molde (M1)	gr	6221	6243	6181	
Peso do Solo Húmido (M2)=(M0)-(M1)	gr	4831	4582	4353	
Volume do Molde V	cc	2104	7104	2104	
Baridade Húmida do Solo Dh=(M2)/V	gr/cc	2.296	7.178	2.069	
Teor Humidade da Moldagem W	%	10.3	10.3	11.3	
Baridade Seca do Solo Ds=Dh x 100 / (100+W)	gr/cc	2.082	1.971	1.876	
% de Compacção Ds/Ds Máx. x 100	%	99.8	94.7	89.9	
Peso Molde+Solo Após Embebição (M3)	gr	11088	10901	10665	
Peso Solo Húmido. Final (M4)=(M3)-(M1)	gr	4867	4658	4484	
Teor Humidade Final Wf=[M4-(Ds x V)]/(Ds x V)	%	11.1	12.1	13.6	

Nº do Molde	1	2	3
Leitura Inicial			
Leitura Final			
% Expansão			



Nº Molde	4 △		5 ○		6 □	
	100.0		95.0		90	
% Mod AASHTO						
Penetração	Leitura	Corrig.	Leitura	Corrig.	Leitura	Corrig.
0.0						
0.5	33		53		48	
1.0	82		118		95	
1.5	140		180		133	
2.0	205		234		158	
2.5*	278	278	270	270	178	178
3.0	365		298		194	
3.5	443		320		207	
4.0	505		342		218	
4.5	555		363		230	
5.0*	588	588	382	382	241	241
6.0	650		415		262	
7.0	702		436		279	
8.0	736		460		295	
9.0	753		485		310	
Factor do Anel			0.012 KN			
CBR	2.5mm=13.344KN		5.0mm=20.016KN		7.5mm=25.354KN	
2,5mm *	25.0		24.3		16.0	
5,0mm **	35.3		22.9		14.4	
% Compacção						
CBR 2,5mm						
CBR 5,0mm						

CONSULTOR / EMPREITEIRO : TÉC. ENGS. CONSULTORES	DATA: 24-11-06	ENSAIO DE COMPACTAÇÃO	
PROJECTO : N13 NAMPULA/CUAMBA		Barid. Seca Máx. 1.852	gr/cm ³
CAMADA ENSAIADA : BASE - NATURAL Km: 35+00 L/E	VERIF. POR:	Teor Ópt. Água 15.5	%
AMOSTRA Nº : NAM/00 /06	EFFECT. POR : RAFAEL		
DESCRIÇÃO DO SOLO : AREIA ARGILOSA CASTANHO CLARO			

DETERMINAÇÃO DO TEOR DE HUMIDADE

Água a Adicionar

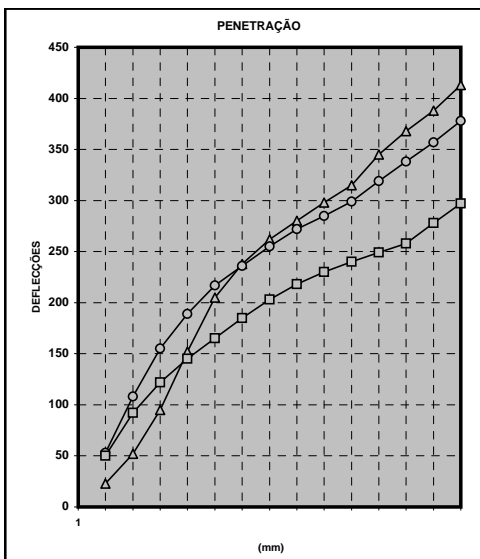
Nº da Cápsula/Molde	Nº	Hygroscópico		Depois da Mistura			Ótimo % (a)	15.5
		15	16	17	15	16		
Peso do Solo Húmido + Cápsula (P0)	gr		289.90	316.10	319.5		Hygroscópico % (b)	1.6
Peso do Solo Seco + Cápsula (P1)	gr		262.60	286.00	288		Volume de Água =	13.9
Peso da Cápsula (P2)	gr		84.40	84.50	84.3		Solo seco x(100+a) - Solo seco =	
Peso da Água (P3)=(P0)-(P1)	gr		27.30	30.10	31.50		100	
Peso do Solo Seco (P4)=(P1)-(P2)	gr		178.20	201.50	208.70		=	2463
Teor de Humidade W=(P3)/(P4)x100	%	1.6	15.3	14.9	15.5			

Moldagem

Expansão

Compactação		Moldagem		
Nº do Molde	Nº	55x10x5	25x10x5	55x5,5x3
Peso do Molde + Solo Húmido (M0)	gr	10702	10481	10259
Peso do Molde (M1)	gr	6201	6200	6196
Peso do Solo Húmido (M2)=(M0)-(M1)	gr	4501	4276	4063
Volume do Molde V	cc		2104	
Baridade Húmida do Solo Dh=(M2)/V	gr/cc	2.139	2.032	1.931
Teor Humidade da Moldagem W	%	15.2	15.2	15.2
Baridade Seca do Solo Ds=Dh x100/100+W	gr/cc	1.857	1.764	1.676
% de Compactação Ds/Ds Máx.x100	%	100.3	95.2	90.5
Peso Molde+Solo Após Embebição (M3)	gr	10733	10550	10340
Peso Solo Húmido Final (M4)=(M3)-(M1)	gr	4532	4345	4144
Teor Humidade Final Wf=[M4-(Ds x V)]/(Ds x V)	%	16.0	17.1	17.5

Nº do Molde	1	2	3
Leitura Inicial			
Leitura Final			
% Expansão			



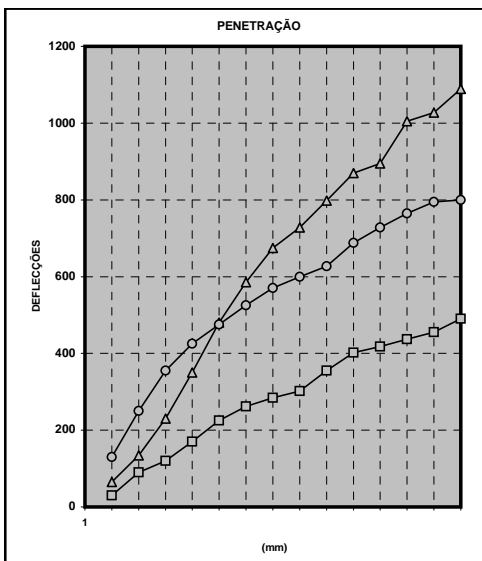
Nº Molde	1 Δ		2 \circ		3 \square	
	% Mod AASHTO 100.0		95.0		90	
Penetração	Leitura	Corrig.	Leitura	Corrig.	Leitura	Corrig.
0.0						
0.5	23		53		50	
1.0	52		108		92	
1.5	95		155		122	
2.0	152		189		145	
2.5*	205	205	217	217	165	165
3.0	238		236		185	
3.5	262		255		203	
4.0	280		272		218	
4.5	298		285		230	
5.0*	315	315	299	299	240	240
6.0	345		319		249	
7.0	368		338		258	
8.0	388		357		278	
9.0	413		378		297	
Factor do Anel			0.012 KN			
CBR	2.5mm=13.344KN		5.0mm=20.016KN		7.5mm=25.354KN	
2,5mm *	18.4		19.5		14.8	
5,0mm **	18.9		17.9		14.4	
% Compactação						
CBR 2,5mm						
CBR 5,0mm						

CONSULTOR / EMPREITEIRO : TÉC. ENGS. CONSULTORES	DATA: 24-11-06	ENSAIO DE COMPACTAÇÃO	
PROJECTO : N13 NAMPULA/CUAMBA		Barid. Seca Máx. 1.958	gr/cm ³
CAMADA ENSAIADA : BASE - NATURAL Km: 148+10 L/E	VERIF. POR:	Teor Ópt. Água 12.9	%
AMOSTRA Nº : NAM/00 /06	EFECT. POR : RAFAEL		
DESCRIÇÃO DO SOLO : AREIA ARGILOSA CASTANHO CLARO COM CASCALHO			

DETERMINAÇÃO DO TEOR DE HUMIDADE				Água a Adicionar		
Nº da Cápsula/Molde	Nº	Hygroscópico		Depois da Mistura		
		18	19	20	Ótimo % (a)	
Peso do Solo Húmido + Cápsula (P0)	gr		328.40	271.90	279.84	12.9
Peso do Solo Seco + Cápsula (P1)	gr		300.60	250.10	217.1	2.7
Peso da Cápsula (P2)	gr		84.20	84.30	84.8	10.2
Peso da Água (P3)=(P0)-(P1)	gr		27.80	21.80	22.74	Solo seco x(100+a) = Solo seco =
Peso do Solo Seco (P4)=(P1)-(P2)	gr		216.40	165.80	172.30	100 =
Teor de Humidade W=(P3)/(P4)x100	%	2.7	12.8	13.1	13.2	1787

Compactação		Moldagem			
Nº do Molde	Nº	55x10x5	25x10x5	55x5,5x3	
		4	5	6	
Peso do Molde + Solo Húmido (M0)	gr	10872	10665	10367	
Peso do Molde (M1)	gr	6221	6243	6181	
Peso do Solo Húmido (M2)=(M0)-(M1)	gr	4651	4422	4186	
Volume do Molde V	cc		2104		
Baridade Húmida do Solo Dh=(M2)/V	gr/cc	2.211	2.102	1.990	
Teor Humidade da Moldagem W	%	13.0	13.0	13.0	
Baridade Seca do Solo Ds=Dh x100/100+W	gr/cc	1.957	1.860	1.761	
% de Compactação Ds/Ds Máx.x100	%	99.9	95.0	90.0	
Peso Molde+Solo Após Embebição (M3)	gr	10936	10788	10544	
Peso Solo Húmido Final (M4)=(M3)-(M1)	gr	4715	454.5	4363	
Teor Humidade Final Wf=[M4-(Ds x V)]/(Ds x V)	%	14.5	16.2	17.8	

Expansão			
Nº do Molde	1	2	3
Leitura Inicial			
Leitura Final			
% Expansão			



Nº Molde	4 △		5 ○		6 □	
	% Mod AASHTO 100.0		95.0		90	
Penetração	Leitura	Corrig.	Leitura	Corrig.	Leitura	Corrig.
0.0						
0.5	65		130		30	
1.0	134		250		90	
1.5	230		355		120	
2.0	350		425		170	
2.5*	480	480	475	475	225	215
3.0	585		525		262	
3.5	675		570		284	
4.0	728		600		302	
4.5	798		627		355	
5.0*	870	870	688	688	402	402
6.0	895		728		418	
7.0	1005		765		437	
8.0	1027		795		455	
9.0	1089		800		490	
Factor do Anel			0.012 KN			
CBR	2.5mm=13.344KN		5.0mm=20.016KN		7.5mm=25.354KN	
2,5mm *	43.2		42.7		19.3	
5,0mm **	52.2		41.2		24.1	
% Compactação						
CBR 2,5mm						
CBR 5,0mm						

CONSULTOR / EMPREITEIRO : TÉC. ENGS. CONSULTORES	DATA: <u>25-11-06</u>	ENSAIO DE COMPACTAÇÃO	
PROJECTO : N13 NAMPULA/CUAMBA		Barid. Seca Máx. <u>2.160</u>	gr/cm ³
CAMADA ENSAIADA : BASE - NATURAL Km: 199+30 L/E	VERIF. POR:	Teor Ópt. Água <u>7.9</u>	%
AMOSTRA Nº : <u>NAM/00 /06</u>	EFFECT. POR : <u>RAFAEL</u>		
DESCRIÇÃO DO SOLO : AREIA ARGILOSA CASTANHO ESCURO COM CASCALHO			

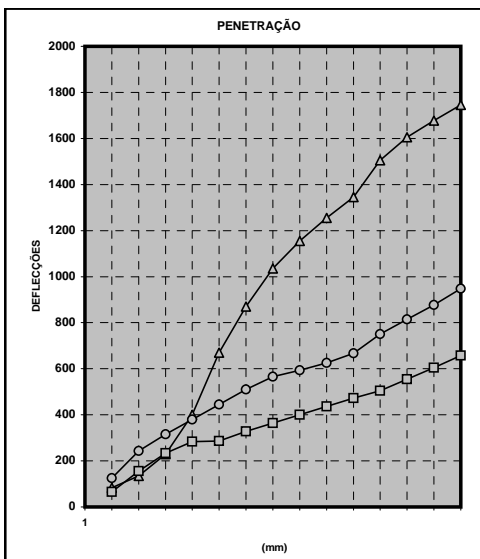
DETERMINAÇÃO DO TEOR DE HUMIDADE

Nº da Cápsula/Molde	Nº	Hygroscópico	Depois da Mistura			Ótimo % (a)	7.9
			1	2	3		
Peso do Solo Húmido + Cápsula (P0)	gr		287.10	299.10	180.6	Hygroscópico % (b)	2.0
Peso do Solo Seco + Cápsula (P1)	gr		273.10	283.30	266.1	Volume de Água =	5.9
Peso da Cápsula (P2)	gr		84.30	84.10	84.5	Solo seco x(100+a) - Solo seco =	
Peso da Água (P3)=(P0)-(P1)	gr		14.00	15.80	14.50	100	
Peso do Solo Seco (P4)=(P1)-(P2)	gr		188.80	199.20	181.60	=	1041
Teor de Humidade W=(P3)/(P4)x100	%	2	7.4	7.9	8.0		

Água a Adicionar

Moldagem					
Compacção	Nº	55x10x5	25x10x5	55x5,5x3	
Nº do Molde	Nº	18	20	24	
Peso do Molde + Solo Húmido (M0)	gr	12151	11662	11705	
Peso do Molde (M1)	gr	7158	6908	7193	
Peso do Solo Húmido (M2)=(M0)-(M1)	gr	4993	4754	4512	
Volume do Molde V	cc		2151		
Baridade Húmida do Solo Dh=(M2)/V	gr/cc	2.321	2.210	2.098	
Teor Humidade da Moldagem W	%	7.8	7.8	7.8	
Baridade Seca do Solo Ds=Dh x 100 / (100+W)	gr/cc	2.153	2.050	1.946	
% de Compacção Ds/Ds Máx. x 100	%	99.7	94.9	90.1	
Peso Molde+Solo Após Embebição (M3)	gr	12228	11836	11915	
Peso Solo Húmido Final (M4)=(M3)-(M1)	gr	5070	4928	4722	
Teor Humidade Final Wf=[M4-(Ds x V)]/(Ds x V)	%	9.5	11.7	12.8	

Expansão			
Nº do Molde	1	2	3
Leitura Inicial			
Leitura Final			
% Expansão			



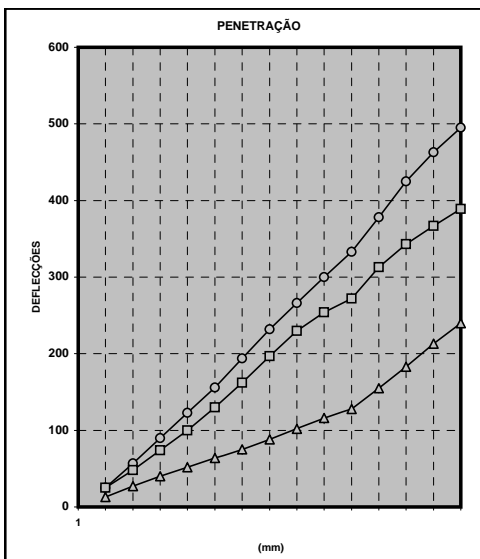
Nº Molde	18 △		20 ○		24 □	
	% Mod AASHTO 100.0		95.0		90	
Penetração	Leitura	Corrig.	Leitura	Corrig.	Leitura	Corrig.
0.0						
0.5	83		125		65	
1.0	135		243		155	
1.5	230		315		233	
2.0	400		380		283	
2.5*	670	825	445	445	287	287
3.0	870		510		328	
3.5	1035		565		364	
4.0	1155		593		400	
4.5	1255		625		437	
5.0*	1345	1400	667	667	472	472
6.0	1505		750		504	
7.0	1605		815		555	
8.0	1678		877		604	
9.0	1745		948		658	
Factor do Anel			0.012 KN			
CBR	2.5mm=13.344KN		5.0mm=20.016KN		7.5mm=25.354KN	
2,5mm *	74.2		40.0		25.8	
5,0mm **	83.9		40.0		28.3	
% Compacção						
CBR 2,5mm						
CBR 5,0mm						

CONSULTOR / EMPREITEIRO : TÉC. ENGS. CONSULTORES	DATA: <u>25-11-06</u>	ENSAIO DE COMPACTAÇÃO	
PROJECTO : N13 NAMPULA/CUAMBA		Barid. Seca Máx. <u>2.080</u>	gr/cm ³
CAMADA ENSAIADA : BASE - NATURAL Km: 351+00 L/E	VERIF. POR:	Teor Ópt. Água <u>9.6</u>	%
AMOSTRA Nº : <u>NAM/00 /06</u>	EFFECT. POR : <u>RAFAEL</u>		
DESCRIÇÃO DO SOLO : AREIA ARGILOSA CIZENTA COM CASCALHO			

DETERMINAÇÃO DO TEOR DE HUMIDADE			Água a Adicionar				
Nº da Cápsula/Molde	Nº	Hygroscópico	Depois da Mistura			Ótimo % (a)	9.6
			23	14	11		
Peso do Solo Húmido + Cápsula (P0)	gr		285.50	283.87	251.2	Volume de Água =	7.0
Peso do Solo Seco + Cápsula (P1)	gr		268.60	266.10	236.6	Solo seco x(100+a) - Solo seco =	
Peso da Cápsula (P2)	gr		83.80	84.80	84.5	100	
Peso da Água (P3)=(P0)-(P1)	gr		16.90	17.77	14.60	=	1228
Peso do Solo Seco (P4)=(P1)-(P2)	gr		184.80	181.30	152.10		
Teor de Humidade W=(P3)/(P4)x100	%	2.6	9.1	9.8	9.6		

Compactação		Moldagem		
Nº do Molde	Nº	55x10x5	25x10x5	55x5,5x3
		X-10	X-11	11
Peso do Molde + Solo Húmido (M0)	gr	11594	11368	12074
Peso do Molde (M1)	gr	6267	6295	7661
Peso do Solo Húmido (M2)=(M0)-(M1)	gr	5327	5073	4413
Volume do Molde V	cc	2340	2240	2115
Baridade Húmida do Solo Dh=(M2)/V	gr/cc	2.276	2.168	2.052
Teor Humidade da Moldagem W	%	9.5	9.5	9.5
Baridade Seca do Solo Ds=Dh x100/100+W	gr/cc	2.079	1.980	1.874
% de Compactação Ds/Ds Máx.x100	%	100.0	95.2	90.1
Peso Molde+Solo Após Embebição (M3)	gr	11650	11420	12126
Peso Solo Húmido Final (M4)=(M3)-(M1)	gr	5383	5125	4465
Teor Humidade Final Wf=[M4-(Ds x V)]/(Ds x V)	%	10.6	10.6	10.8

Expansão			
Nº do Molde	1	2	3
Leitura Inicial			
Leitura Final			
% Expansão			



Nº Molde	X-10 △		X-11 ○		11 □	
	% Mod AASHTO 100.0		95.0		90	
Penetração	Leitura	Corrig.	Leitura	Corrig.	Leitura	Corrig.
0.0						
0.5	13		25		25	
1.0	27		57		48	
1.5	40		90		74	
2.0	52		123		100	
2.5*	64	64	156	156	130	180
3.0	75		194		162	
3.5	88		232		197	
4.0	102		266		230	
4.5	116		300		254	
5.0*	128	128	333	333	272	272
6.0	155		378		313	
7.0	183		425		343	
8.0	213		463		367	
9.0	240		495		389	
Factor do Anel			0.012 KN			
CBR	2.5mm=13.344KN		5.0mm=20.016KN		7.5mm=25.354KN	
2,5mm *	5.8		14.0		11.7	
5,0mm **	7.7		20.0		16.3	
% Compactação						
CBR 2,5mm						
CBR 5,0mm						

CONSULTOR/EMPREENHEIRO : TÉC. ENGS. CONSULTORES
 PROJECTO : N13 NAMPULA/CUAMBA
 CAMADA ENSAIADA : BASE - NATURAL Km: 15+00 L/E
 AMOSTRA Nº : NAM/0022/06 EFECTUADO POR : RAFAEL

BARIDADE SECA MÁX. 2.086 gr/cm³
 TEOR HUMIDADE ÓPTIMO 9.8 %

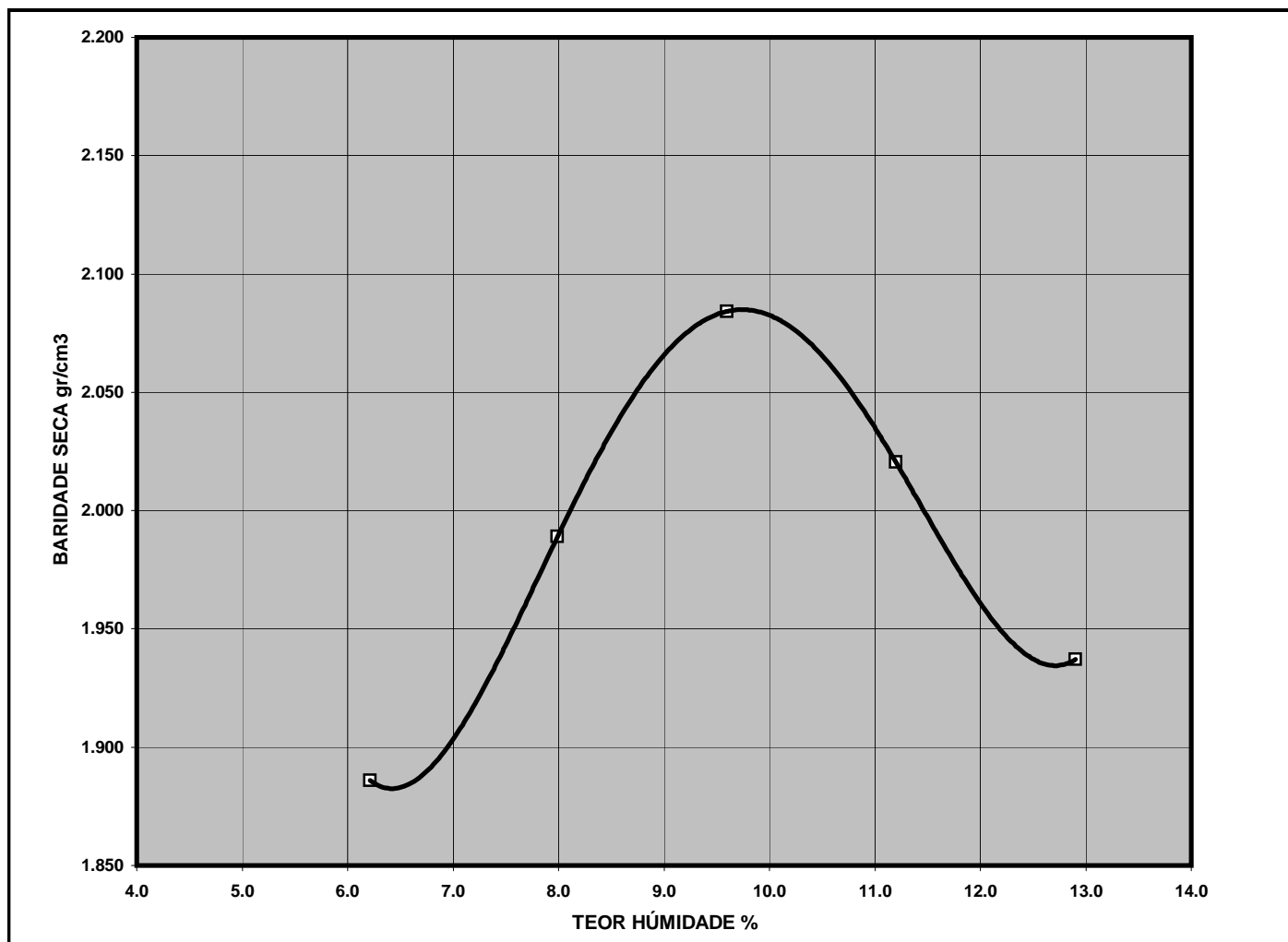
VERIFICADO POR: RAFAEL DATA : #

1. DETERMINAÇÃO DA BARIDADE HÚMIDA

NÚMERO DO MOLDE	Nº			1		
% DE ÁGUA ADICIONADA + (7.0% B.E.)	%	4	6	8	10	12
QUANTIDADE DE ÁGUA ADICIONADA	cc	240	360	480	600	720
PESO DO MOLDE + SOLO HÚMIDO (P1)	gr	9547	9852	10139	10066	9934
PESO DO MOLDE (P2)	gr			5330		
PESO DO SOLO HÚMIDO (Ph)= (P1) - (P2)	gr	4217	4522	4809	4730	4604
VOLUME DO MOLDE (V)	cm ³	0.000475	0.000475	0.000475	0.000475	0.000475
BARIDADE HÚMIDA DO SOLO (Dh) = (Ph)/V	gr/cm ³	2.003	2.148	2.284	2.247	2.187

2. DETERMINAÇÃO DO TEOR DE HÚMIDADE

NÚMERO DA CÁPSULA	Nº	1	2	3	4	5
PESO DA CÁPSULA + SOLO HÚMIDO (M1)	gr	354.20	315.20	292.19	324.00	313.00
PESO DA CÁPSULA + SOLO SECO (M2)	gr	338.50	298.10	274.00	300.50	286.80
PESO DA CÁPSULA (M3)	gr	84.30	84.10	84.50	84.40	83.70
PESO DA ÁGUA (Ma) = (M1) - (M2)	gr	15.80	17.10	18.19	24.20	26.20
PESO DO SOLO SECO (Ms) = (M2) - (M3)	gr	254.20	214.00	189.50	216.10	203.10
TEOR DE HÚMIDADE W = (Ma)/(Ms) x 100	%	6.2	8.0	9.6	11.2	12.9
BARIDADE SECA SOLO (Ds) = (Dh)x100/100+W	gr/cm ³	1.886	1.989	2.084	2.020	1.937



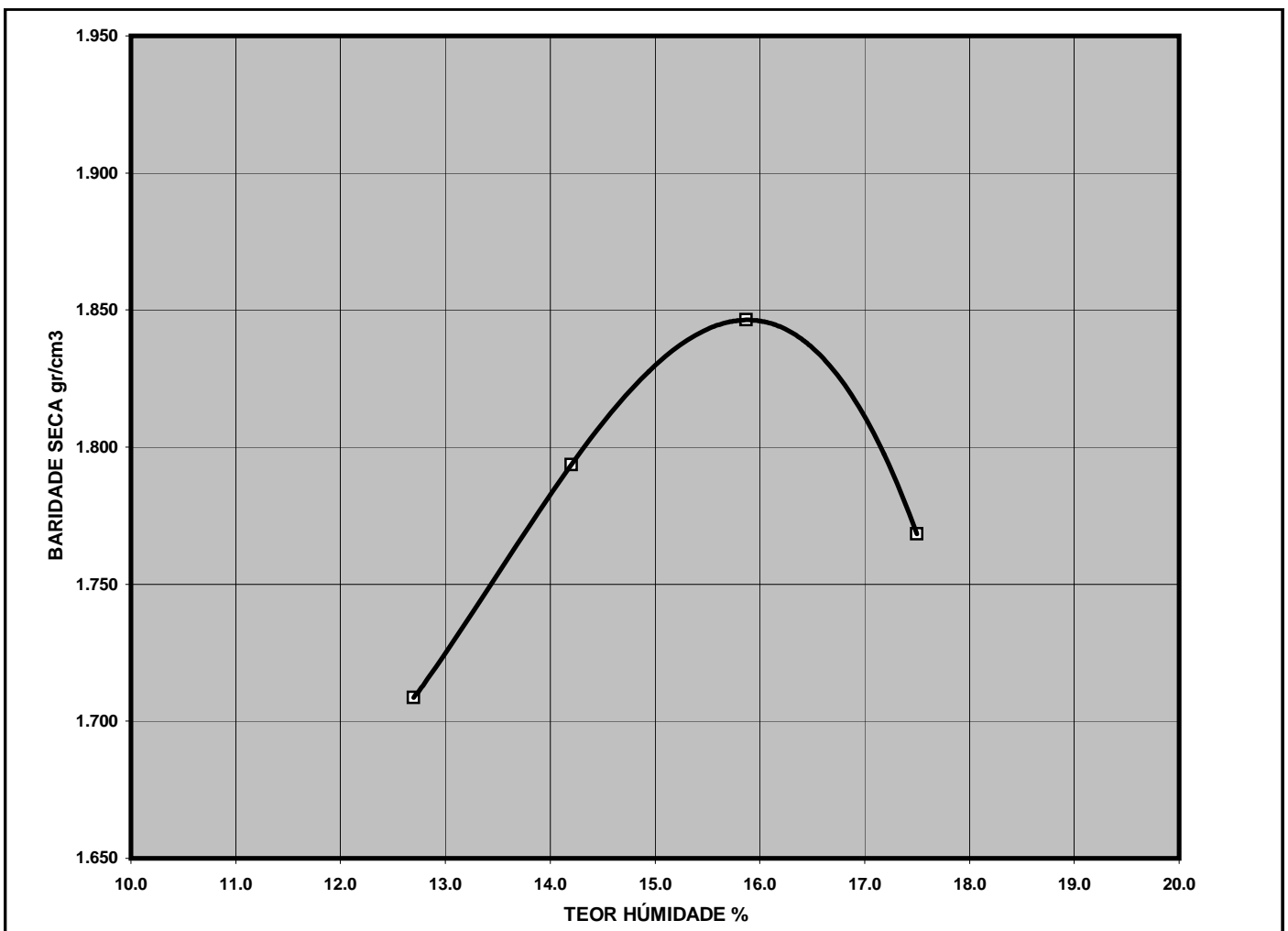
CONSULTOR/EMPREENHEIRO : TÉC. ENGS. CONSULTORESBARIDADE SECA MÁX. 1.851 gr/cm³PROJECTO : N13 NAMPULA/CUAMBATEOR HUMIDADE ÓPTIMO 15.5 %CAMADA ENSAIADA : BASE - NATURAL Km: 35+10 L/EAMOSTRA Nº : NAM/0022/06EFECTUADO POR : RAFAELVERIFICADO POR: RAFAELDATA : #

1. DETERMINAÇÃO DA BARIDADE HÚMIDA

NÚMERO DO MOLDE	Nº			1		
% DE ÁGUA ADICIONADA + (7.0% B.E.)	%	6	8	10	12	
QUANTIDADE DE ÁGUA ADICIONADA	cc	360	480	600	720	
PESO DO MOLDE + SOLO HÚMIDO (P1)	gr	9384	9642	9834	9704	
PESO DO MOLDE (P2)	gr	5330	5330	5330	5330	
PESO DO SOLO HÚMIDO (Ph)= (P1) - (P2)	gr	4054	4312	4504	4374	
VOLUME DO MOLDE (V)	cm ³	0.000475	0.000475	0.000475	0.000475	
BARIDADE HÚMIDA DO SOLO (Dh) = (Ph)/V	gr/cm ³	1.926	2.048	2.139	2.078	

2. DETERMINAÇÃO DO TEOR DE HÚMIDADE

NÚMERO DA CÁPSULA	Nº	7	8	10	11	
PESO DA CÁPSULA + SOLO HÚMIDO (M1)	gr	369.05	348.80	366.30	379.54	
PESO DA CÁPSULA + SOLO SECO (M2)	gr	337.00	316.00	327.70	335.60	
PESO DA CÁPSULA (M3)	gr	84.60	85.00	84.50	84.50	
PESO DA ÁGUA (Ma) = (M1) - (M2)	gr	32.05	32.80	38.60	43.94	
PESO DO SOLO SECO (Ms) = (M2) - (M3)	gr	252.40	231.00	243.20	251.10	
TEOR DE HÚMIDADE W = (Ma)/(Ms) x 100	%	12.7	14.2	15.9	17.5	
BARIDADE SECA SOLO (Ds) = (Dh)x100/100+W	gr/cm ³	1.709	1.794	1.846	1.768	



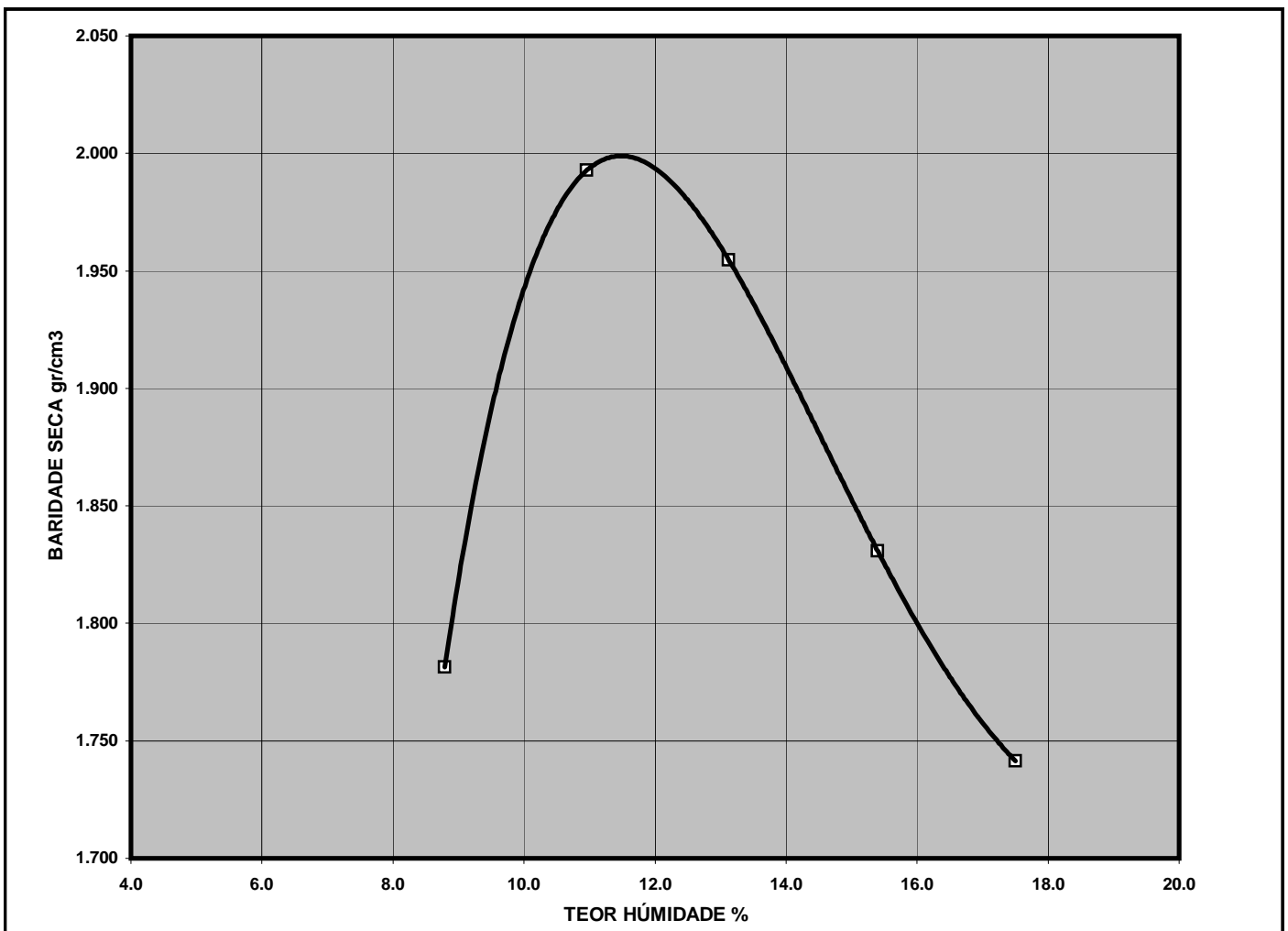
CONSULTOR/EMPREENHEIRO : TÉC. ENGS. CONSULTORESBARIDADE SECA MÁX. 1.958 gr/cm³PROJECTO : N13 NAMPULA/CUAMBATEOR HUMIDADE ÓPTIMO 12.9 %CAMADA ENSAIADA : BASE - NATURAL Km: 148+00 L/EAMOSTRA Nº : NAM/0024/06EFECTUADO POR : RAFAELVERIFICADO POR: RAFAELDATA : 29-04-99

1. DETERMINAÇÃO DA BARIDADE HÚMIDA

NÚMERO DO MOLDE	Nº			1			
% DE ÁGUA ADICIONADA + (7.0% B.E.)	%	5	7	9	11	13	
QUANTIDADE DE ÁGUA ADICIONADA	cc	300	420	540	660	780	
PESO DO MOLDE + SOLO HÚMIDO (P1)	gr	9410	9728	9985	9778	9638	
PESO DO MOLDE (P2)	gr	5330	5330	5330	5330	5330	
PESO DO SOLO HÚMIDO (Ph)= (P1) - (P2)	gr	4080	4655	4655	4448	4308	
VOLUME DO MOLDE (V)	cm ³	0.000475	0.000475	0.000475	0.000475	0.000475	
BARIDADE HÚMIDA DO SOLO (Dh) = (Ph)/V	gr/cm ³	1.938	2.211	2.211	2.113	2.046	

2. DETERMINAÇÃO DO TEOR DE HÚMIDADE

NÚMERO DA CÁPSULA	Nº	12	13	14	15	16	
PESO DA CÁPSULA + SOLO HÚMIDO (M1)	gr	329.40	373.70	336.10	326.62	334.89	
PESO DA CÁPSULA + SOLO SECO (M2)	gr	309.60	345.10	306.80	294.30	297.60	
PESO DA CÁPSULA (M3)	gr	84.40	84.10	84.80	84.40	84.50	
PESO DA ÁGUA (Ma) = (M1) - (M2)	gr	19.80	28.60	29.80	32.32	37.29	
PESO DO SOLO SECO (Ms) = (M2) - (M3)	gr	225.20	261.00	227.00	209.90	213.10	
TEOR DE HÚMIDADE W = (Ma)/(Ms) x 100	%	8.8	11.0	13.1	15.4	17.5	
BARIDADE SECA SOLO (Ds) = (Dh)x100/100+W	gr/cm ³	1.781	1.993	1.955	1.831	1.742	



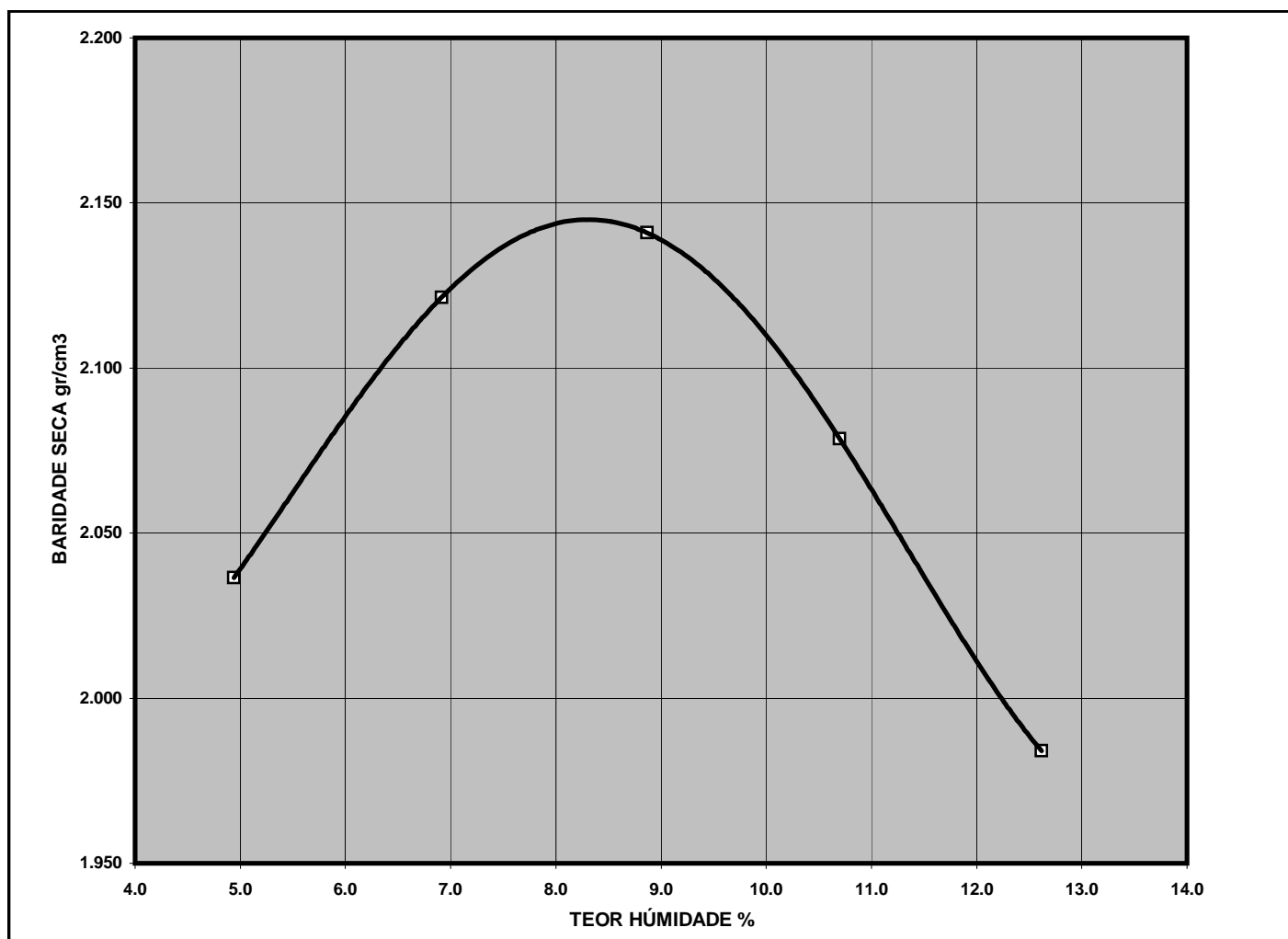
CONSULTOR/EMPREENHEIRO : TÉC. ENGS. CONSULTORESBARIDADE SECA MÁX. 2160.000 gr/cm³PROJECTO : N13 NAMPULA/CUAMBATEOR HUMIDADE ÓPTIMO 7.9 %CAMADA ENSAIADA : BASE - NATURAL Km: 199+30 L/EAMOSTRA Nº : NAM/00 /06EFECTUADO POR : RAFAELVERIFICADO POR: RAFAELDATA : 29-04-99

1. DETERMINAÇÃO DA BARIDADE HÚMIDA

NÚMERO DO MOLDE	Nº			1			
% DE ÁGUA ADICIONADA + (7.0% B.E.)	%	3	5	7	9	11	
QUANTIDADE DE ÁGUA ADICIONADA	cc	180	300	40	540	660	
PESO DO MOLDE + SOLO HÚMIDO (P1)	gr	9829	10105	10237	10174	10034	
PESO DO MOLDE (P2)	gr	5330	5330	5330	5330	5330	
PESO DO SOLO HÚMIDO (Ph)= (P1) - (P2)	gr	4499	4775	4907	4844	4704	
VOLUME DO MOLDE (V)	cm ³	0.000475	0.000475	0.000475	0.000475	0.000475	
BARIDADE HÚMIDA DO SOLO (Dh) = (Ph)/V	gr/cm ³	2.137	2.268	2.331	2.301	2.234	

2. DETERMINAÇÃO DO TEOR DE HÚMIDADE

NÚMERO DA CÁPSULA	Nº	6	7	8	10	11	
PESO DA CÁPSULA + SOLO HÚMIDO (M1)	gr	358.10	403.10	312.10	380.40	416.50	
PESO DA CÁPSULA + SOLO SECO (M2)	gr	345.20	382.50	293.60	352.80	379.30	
PESO DA CÁPSULA (M3)	gr	84.00	84.60	850.00	84.50	84.50	
PESO DA ÁGUA (Ma) = (M1) - (M2)	gr	12.90	20.60	18.50	28.60	37.20	
PESO DO SOLO SECO (Ms) = (M2) - (M3)	gr	261.20	297.90	208.60	267.30	294.80	
TEOR DE HÚMIDADE W = (Ma)/(Ms) x 100	%	4.9	6.9	8.9	10.7	12.6	
BARIDADE SECA SOLO (Ds) = (Dh)x100/100+W	gr/cm ³	2.036	2.121	2.141	2.079	1.984	



CONSULTOR/EMPREENHEIRO : TÉC. ENGS. CONSULTORES
 PROJECTO : N13 NAMPULA/CUAMBA
 CAMADA ENSAIADA : BASE - NATURAL Km: 351+00 L/E
 AMOSTRA Nº : NAM/0026/06 EFETUADO POR : RAFAEL

BARIDADE SECA MÁX. 2.080 gr/cm³
 TEOR HUMIDADE ÓPTIMO 9.6 %

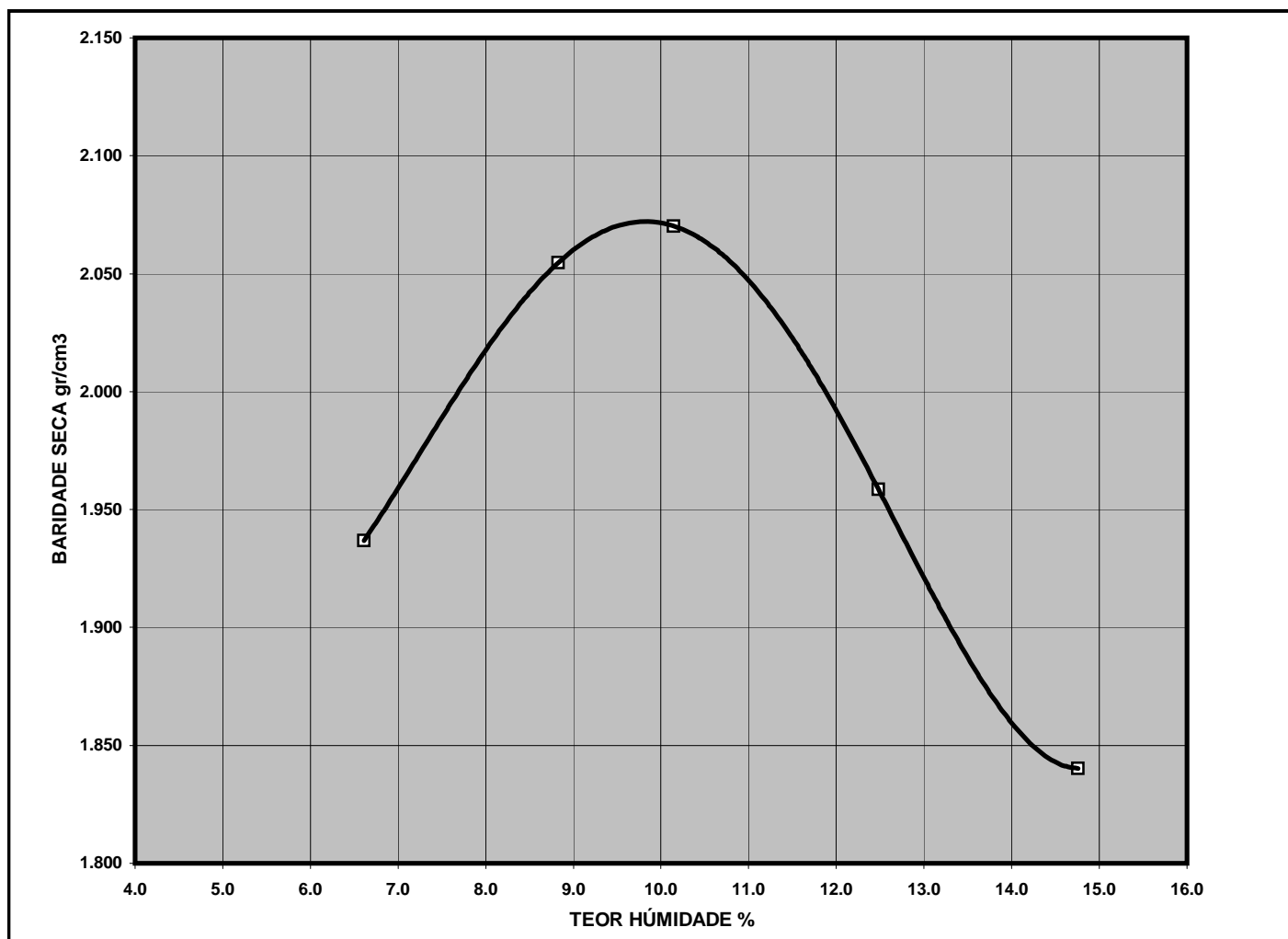
VERIFICADO POR: RAFAEL DATA : 29-04-99

1. DETERMINAÇÃO DA BARIDADE HÚMIDA

NÚMERO DO MOLDE	Nº			1			
% DE ÁGUA ADICIONADA + (7.0% B.E.)	%	4	6	8	10	12	
QUANTIDADE DE ÁGUA ADICIONADA	cc	240	360	480	600	720	
PESO DO MOLDE + SOLO HÚMIDO (P1)	gr	10218	10612	10714	10536	10326	
PESO DO MOLDE (P2)	gr	5460	5460	5460	5460	5460	
PESO DO SOLO HÚMIDO (Ph)= (P1) - (P2)	gr	4758	5152	5254	5076	4866	
VOLUME DO MOLDE (V)	cm ³	0.000434	0.000434	0.000434	0.000434	0.000434	
BARIDADE HÚMIDA DO SOLO (Dh) = (Ph)/V	gr/cm ³	2.065	2.236	2.280	2.203	2.112	

2. DETERMINAÇÃO DO TEOR DE HÚMIDADE

NÚMERO DA CÁPSULA	Nº	1	2	3	7	8	
PESO DA CÁPSULA + SOLO HÚMIDO (M1)	gr	366.50	319.60	341.80	363.10	347.10	
PESO DA CÁPSULA + SOLO SECO (M2)	gr	349.00	300.50	318.10	332.20	313.40	
PESO DA CÁPSULA (M3)	gr	84.30	84.10	84.50	84.60	85.00	
PESO DA ÁGUA (Ma) = (M1) - (M2)	gr	17.50	19.10	23.70	30.90	33.70	
PESO DO SOLO SECO (Ms) = (M2) - (M3)	gr	264.70	216.40	233.60	247.60	228.40	
TEOR DE HÚMIDADE W = (Ma)/(Ms) x 100	%	6.6	8.8	10.1	12.5	14.8	
BARIDADE SECA SOLO (Ds) = (Dh)x100/100+W	gr/cm ³	1.937	2.055	2.070	1.959	1.840	

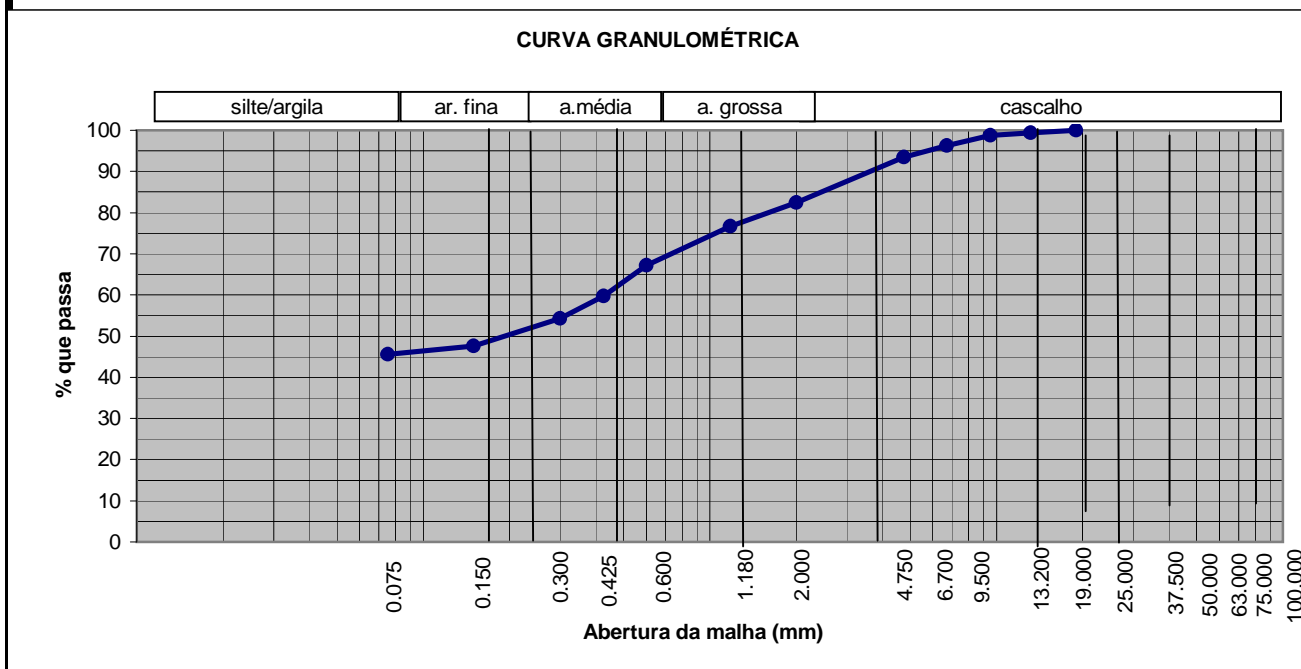


Appendix B-5
LATERITE TEST

Solos
ANÁLISE GRANULOMÉTRICA POR PENEIRAÇÃO HÚMIDA

PENEIROS		P. RETIDO (g)	% RETIDA	% TOTAL QUE PASSA	Amostra			
ASTM	(mm)				REFª DO LEM: - NAM/0017/06	REFª DO REQUISITANTE: KM: 79+00 L/D	O REQUISITANTE: TÊC. ENG. CONSULTORES	PROVENIÊNCIA: NAMPULA
3"	75.000				PROFUNDIDADE: ---			
2" 1/2	63.000				Análise granulométrica da fracção grossa			
2"	50.790				Peso total da amostra (g)	2000		
1" 1/2	37.500				Peso da fracção grossa (g)			
1"	25.000				Peso da fracção fina (g)			
3/4"	19.000			100	Peso da fracção grossa seca (g)			
1/2"	13.200	11.2	0.6	99.4	Percentagem total da fracção fina			
3/8"	9.500	13.00	0.7	98.7	Análise granulométrica da fracção fina			
1/4"	6.700	47.50	2.4	96.3	Peso total da amostra (g)	2000.00		
3/16"	4.750	58.80	2.9	93.4	Peso do material lavado seco (g)	1089.00		
14	2.000	218.20	10.9	82.5	Peso do material fino lavado (g)	911.00		
18	1.180	115.00	5.8	76.7	LIMITES			
36	0.600	190.04	9.5	67.2				
40	0.425	150.02	7.5	59.7	47.8	27.6	20.02	
52	0.300	108.00	5.4	54.3				
100	0.150	134.50	6.7	47.6				
200	0.075	39.40	2.0	45.6				

Análise Granulométrica por peneiração húmida LNEC E195/E239

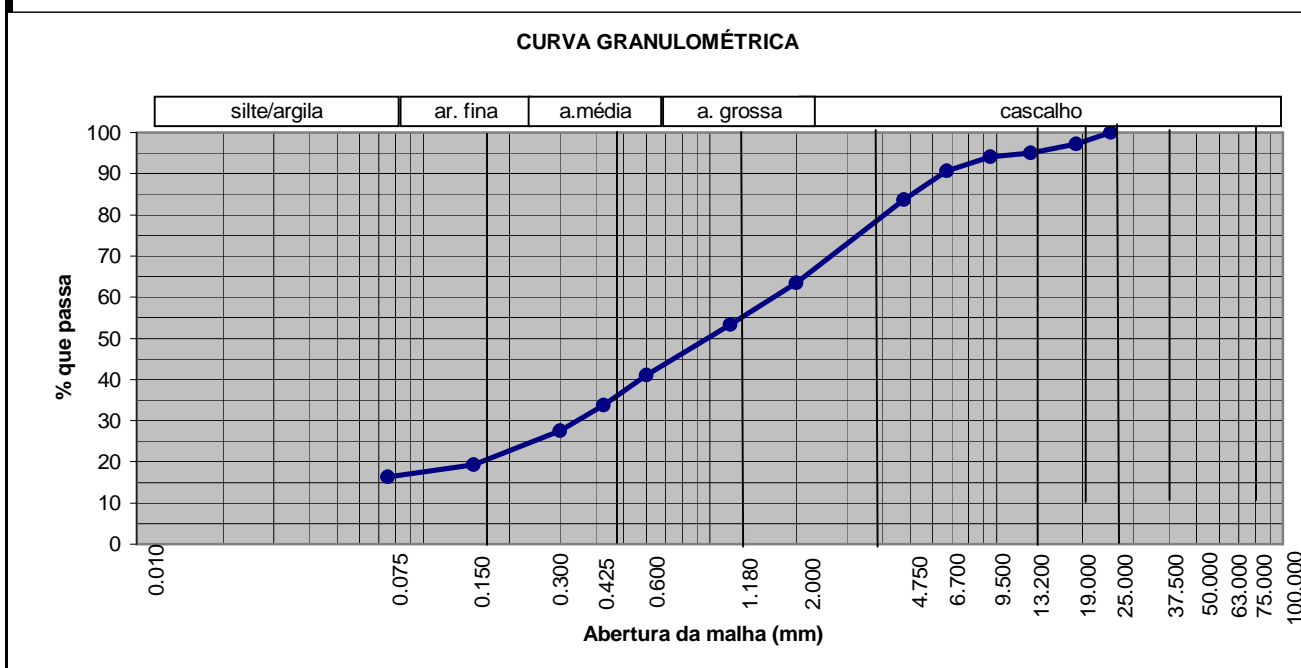


Albino Caetano

Solos
ANÁLISE GRANULOMÉTRICA POR PENEIRAÇÃO HÚMIDA

PENEIROS		P. RETIDO (g)	% RETIDA	% TOTAL QUE PASSA	Amostra			
ASTM	(mm)				REFª DO LEM: - NAM/0018/06 REFª DO REQUISITANTE: KM:124+00 L/D O REQUISITANTE: TÊC. ENG. CONSULTORES PROVENIÊNCIA: NAMPULA OBRA/DESTINO: N13 NPL/CUAMBA DATA DO ENSAIO: 10/11/2006 PROFUNDIDADE: ---			
3"	75.000				Análise granulométrica da fracção grossa			
2" 1/2	63.000				Peso total da amostra (g) 2000			
2"	50.790				Peso da fracção grossa (g)			
1" 1/2	37.500				Peso da fracção fina (g)			
1"	25.000			100	Peso da fracção grossa seca (g)			
3/4"	19.000	56.5	2.8	97.2	Percentagem total da fracção fina			
1/2"	13.200	42.4	2.1	95.1	Análise granulométrica da fracção fina			
3/8"	9.500	20.04	1.0	94.1	Peso total da amostra (g) 2000.00			
1/4"	6.700	68.50	3.4	90.7	Peso do material lavado seco (g) 1683.00			
3/16"	4.750	140.05	7.0	83.7	Peso do material fino lavado (g) 317.00			
14	2.000	405.60	20.3	63.4	LIMITES	LL (%)	LP (%)	IP (%)
18	1.180	199.40	10.0	53.4				
36	0.600	246.00	12.3	41.1				
40	0.425	148.40	7.4	33.7				
52	0.300	122.70	6.1	27.6				
100	0.150	166.20	8.3	19.3				
200	0.075	57.00	2.9	16.4				

Análise Granulométrica por peneiração húmida LNEC E195/E239



Albino Caetano

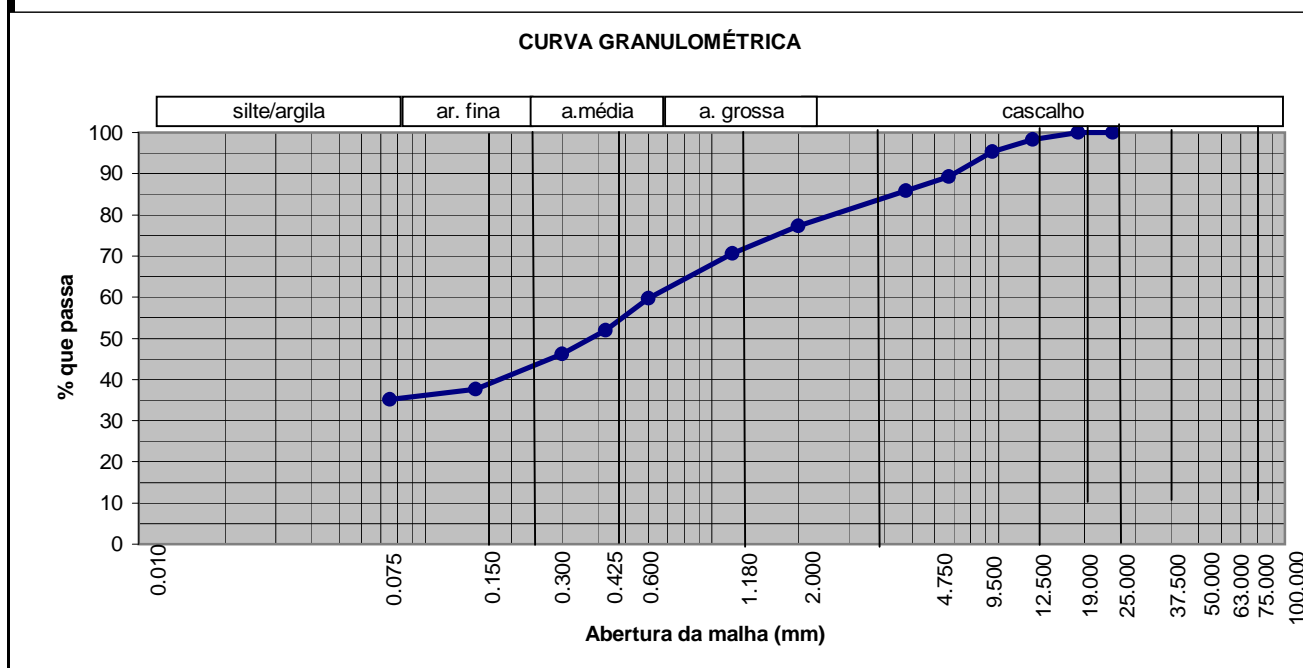
LABORATÓRIO DE ENGENHARIA DE MOÇAMBIQUE

DEPARTAMENTO DE VIAS DE COMUNICAÇÃO

Solos
ANÁLISE GRANULOMÉTRICA POR PENEIRAÇÃO HÚMIDA

PENEIROS		P. RETIDO (g)	% RETIDA	% TOTAL QUE PASSA	Amostra			
ASTM	(mm)				REFª DO LEM: - REFª DO REQUISITANTE: KM:204+00 L/E O REQUISITANTE: TÊC. ENG. CONSULTORES PROVENIÊNCIA: NAMPULA OBRA/DESTINO: N13 NPL/CUAMBA DATA DO ENSAIO: 10/11/2006 PROFUNDIDADE: ---			
3"	75.000				Análise granulométrica da fracção grossa			
2" 1/2	63.000				Peso total da amostra (g) 2000			
2"	50.790				Peso da fracção grossa (g)			
1"1/2	37.500				Peso da fracção fina (g)			
1"	25.000			100	Peso da fracção grossa seca (g)			
3/4"	19.000			100	Percentagem total da fracção fina			
1/2"	13.200	33.5	1.7	98.3	Análise granulométrica da fracção fina			
3/8"	9.500	58.60	2.9	95.4	Peso total da amostra (g) 2000.00			
1/4"	6.700	122.90	6.1	89.3	Peso do material lavado seco (g) 1308.00			
3/16"	4.750	68.30	3.4	85.9	Peso do material fino lavado (g) 692.00			
14	2.000	172.90	8.6	77.3	LIMITES	LL (%)	LP (%)	IP (%)
18	1.180	133.20	6.7	70.6				
36	0.600	218.80	10.9	59.7				
40	0.425	153.90	7.7	52.0				
52	0.300	115.60	5.8	46.2				
100	0.150	170.08	8.5	37.7				
200	0.075	52.80	2.6	35.1				

Análise Granulométrica por peneiração húmida LNEC E195/E239

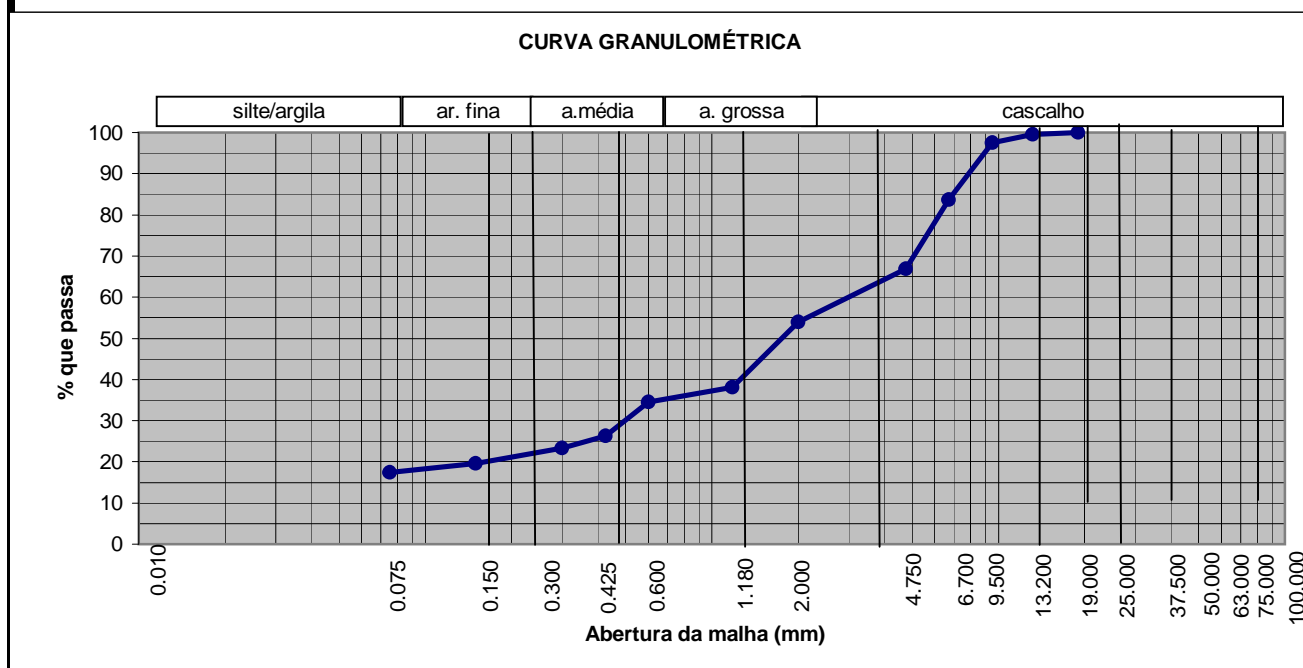


Albino Caetano

Solos
ANÁLISE GRANULOMÉTRICA POR PENEIRAÇÃO HÚMIDA

PENEIROS		P. RETIDO (g)	% RETIDA	% TOTAL QUE PASSA	Amostra			
ASTM	(mm)				REFª DO LEM: REFª DO REQUISITANTE: KM:256+50 L/D O REQUISITANTE: TÊC. ENG. CONSULTORE PROVENIÊNCIA: NAMPULA OBRA/DESTINO: N13 NPL/CUAMBA DATA DO ENSAIO: 16/11/2006 PROFUNDIDADE: ---			
3"	75.000				Análise granulométrica da fracção grossa			
2" 1/2	63.000				Peso total da amostra (g) 2000			
2"	50.790				Peso da fracção grossa (g)			
1" 1/2	37.500				Peso da fracção fina (g)			
1"	25.000				Peso da fracção grossa seca (g)			
3/4"	19.000			100	Percentagem total da fracção fina			
1/2"	13.200	8.3	0.4	99.6	Análise granulométrica da fracção fina			
3/8"	9.500	41.00	2.1	97.5	Peso total da amostra (g) 2000.00			
1/4"	6.700	277.70	13.9	83.6	Peso do material lavado seco (g) 1661.00			
3/16"	4.750	334.00	16.7	66.9	Peso do material fino lavado (g) 339.00			
14	2.000	260.00	13.0	53.9	LIMITES	LL (%)	LP (%)	IP (%)
18	1.180	316.20	15.8	38.1				
36	0.600	72.00	3.6	34.5				
40	0.425	164.20	8.2	26.3				
52	0.300	57.00	2.9	23.4				
100	0.150	76.10	3.8	19.6				
200	0.075	44.00	2.2	17.4				

Análise Granulométrica por peneiração húmida LNEC E195/E239

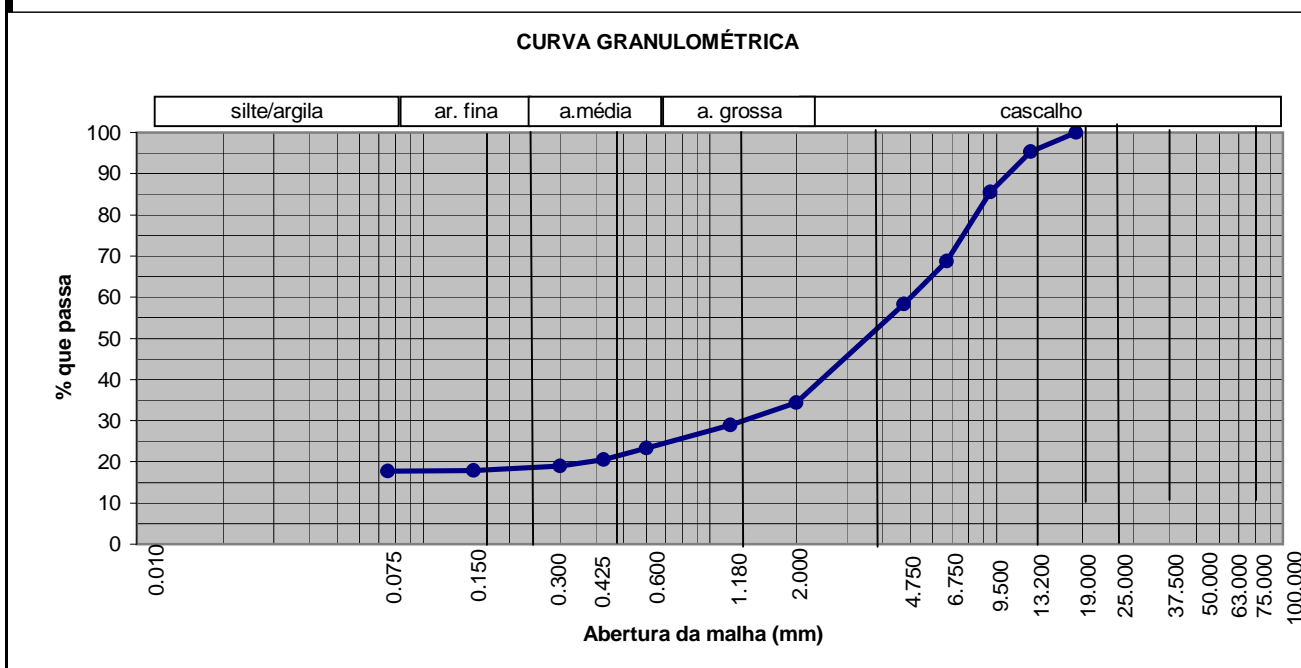


Albino Caetano

Solos
ANÁLISE GRANULOMÉTRICA POR PENEIRAÇÃO HÚMIDA

PENEIROS		P. RETIDO (g)	% RETIDA	% TOTAL QUE PASSA	Amostra			
ASTM	(mm)				REFª DO LEM: REFª DO REQUISITANTE: KM:332+10 L/D O REQUISITANTE: TÊC. ENG. CONSULTORE PROVENIÊNCIA: NAMPULA OBRA/DESTINO: N13 NPL/CUAMBA DATA DO ENSAIO: 13/11/2006 PROFUNDIDADE: ---			
3"	75.000				Análise granulométrica da fracção grossa			
2" 1/2	63.000				Peso total da amostra (g) 2000			
2"	50.790				Peso da fracção grossa (g)			
1" 1/2	37.500				Peso da fracção fina (g)			
1"	25.000				Peso da fracção grossa seca (g)			
3/4"	19.000			100	Percentagem total da fracção fina			
1/2"	13.200	92	4.6	95.4	Análise granulométrica da fracção fina			
3/8"	9.500	197.70	9.9	85.5	Peso total da amostra (g) 2000.00			
1/4"	6.700	335.40	16.8	68.7	Peso do material lavado seco (g) 1646.00			
3/16"	4.750	207.00	10.4	58.3	Peso do material fino lavado (g) 354.00			
14	2.000	480.00	24.0	34.3	LIMITES	LL (%)	LP (%)	IP (%)
18	1.180	108.60	5.4	28.9				
36	0.600	112.20	5.6	23.3				
40	0.425	35.70	2.8	20.5				
52	0.300	32.50	1.6	18.9				
100	0.150	20.02	1.0	17.9				
200	0.075	4.50	0.2	17.7				

Análise Granulométrica por peneiração húmida LNEC E195/E239



Albino Caetano

CONSULTOR / EMPREITEIRO : D.N.E.P.

PROJECTO : Téc. Engs. Consultores

CAMADA ENSAIADA : C.E do km: 79+00 L/E

AMOSTRA Nº : NAM/0017/06 EFECTUADO POR : Rafael VERIFICADO POR: Rafael DATA: 2006/6/11

DESCRIÇÃO DO SOLO : Areia argilosa Castanho Claro com Cascalho, "Laterite"

1. DETERMINAÇÃO DO LIMITE DE LIQUIDEZ

Nº da Cápsula	Nº	12	13	14	15				
Peso Solo Húmido + Cápsula (P0)	gr	24.40	25.30	23.00	24.14				
Peso Solo Seco + Cápsula (P1)	gr	21.60	22.30	20.80	21.5				
Peso da Cápsula (P2)	gr	16.04	16.13	16.14	15.8				
Peso da Água P3 = (P0) - (P1)	gr	2.80	3.00	2.20	2.64				
Peso do Solo Seco P4 = (P1) - (P2)	gr	5.56	6.17	4.66	5.7				
Teor de Humidade W = P3 / P4 x 100	gr	50.4	48.6	47.2	46.3				
Nº de Pancadas	Nº	12	20	30	40				

Teor de Humidade %



2. DETERMINAÇÃO DO LIMITE DE PLASTICIDADE

Nº da Cápsula	Nº	18	19	20				
Peso do Solo Húmido + Cápsula (P0)	gr	24.90	24.70	25.6				
Peso Solo Seco + Cápsula (P1)	gr	23.00	22.80	23.5				
Peso da Cápsula (P2)	gr	16.03	16.00	15.9				
Peso da Água P3 = (P0) - (P1)	gr	1.90	1.90	2.1				
Peso do Solo Seco P4 = (P1) - (P2)	gr	6.97	6.80	7.6				
Teor de Humidade W = P3 / P4 x 100	gr	27.3	27.9	27.6				

3. DETERMINAÇÃO DO LIMITE DE RETRAÇÃO

Nº do Moulde	Nº	1			
Comprimento do Moulde (L1)	mm	15.20			
Distância Retraída (L2)	mm	1.50			
Retração Linear L.S. = (L2) / (L1) x 100	%	9.9			

4. RESUMO RESULTADOS

Limite de Liquidez	<u>48</u>	%
Limite Plasticidade	<u>28</u>	%
Indice Plasticidade	<u>20</u>	%
Limite Retração	<u>9.9</u>	%

CONSULTOR / EMPREITEIRO : Téc. Engs. Consultores

PROJECTO : N13 Nampula/Cuamba

CAMADA ENSAIADA : C.E do km: 124+00 L/D "Namiganha"

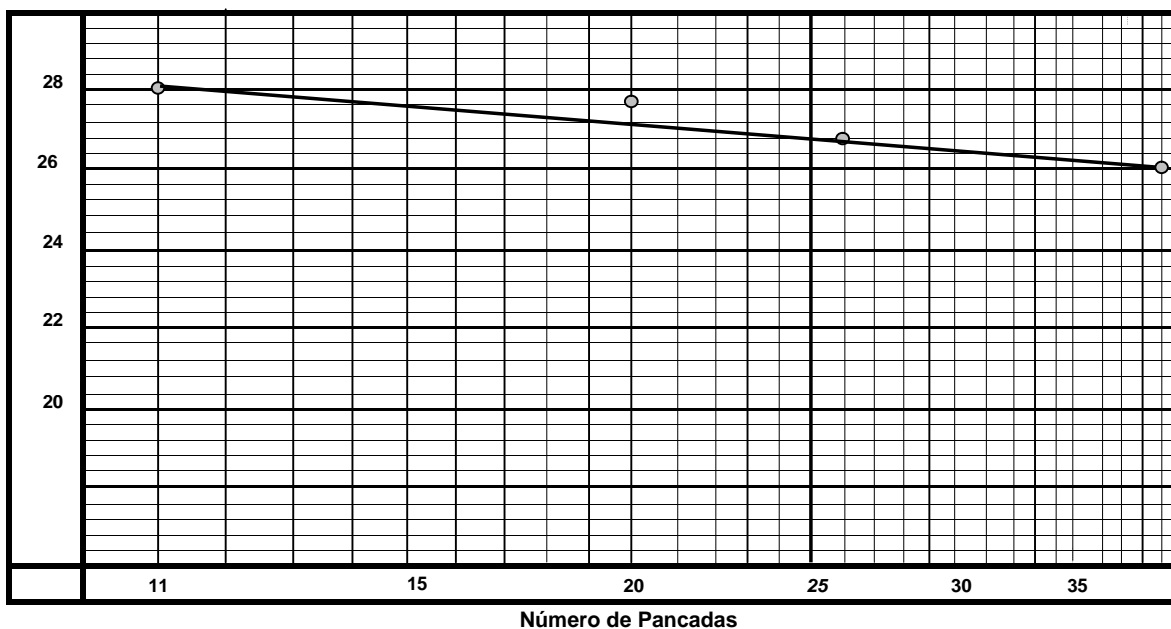
AMOSTRA Nº : NAM/0018/06 EFECTUADO POR : Rafael VERIFICADO POR: Rafael DATA: 2006/7/11

DESCRIÇÃO DO SOLO : Areia argilosa Castanho Claro com Cascalho, "Laterite"

1. DETERMINAÇÃO DO LIMITE DE LIQUIDEZ

Nº da Cápsula	Nº	1	2	3	4				
Peso Solo Húmido + Cápsula (P0)	gr	26.30	27.70	26.80	24.11				
Peso Solo Seco + Cápsula (P1)	gr	24.00	25.10	24.50	22.4				
Peso da Cápsula (P2)	gr	15.80	15.74	15.84	15.81				
Peso da Água P3 = (P0) - (P1)	gr	2.30	2.60	2.30	1.71				
Peso do Solo Seco P4 = (P1) - (P2)	gr	8.10	9.36	8.66	6.59				
Teor de Humidade W = P3 / P4 x 100	gr	28.0	27.8	26.6	25.9				
Nº de Pancadas	Nº	11	20	26	39				

Teor de Humidade %



2. DETERMINAÇÃO DO LIMITE DE PLASTICIDADE

Nº da Cápsula	Nº	5	624.71	7				
Peso do Solo Húmido + Cápsula (P0)	gr	25.60	24.71	24.44				
Peso Solo Seco + Cápsula (P1)	gr	24.30	23.60	23.3				
Peso da Cápsula (P2)	gr	15.93	15.95	15.81				
Peso da Água P3 = (P0) - (P1)	gr	1.30	1.11	1.14				
Peso do Solo Seco P4 = (P1) - (P2)	gr	8.37	7.65	7.49				
Teor de Humidade W = P3 / P4 x 100	gr	15.5	14.5	15.2				

3. DETERMINAÇÃO DO LIMITE DE RETRAÇÃO

Nº do Moulde	Nº	2			
Comprimento do Moulde (L1)	mm	15.20			
Distância Retraída (L2)	mm	0.50			
Retração Linear L.S. = (L2) / (L1) x 100	%	5.3			

4. RESUMO RESULTADOS

Limite de Liquidez	27	%
Limite Plasticidade	15	%
Índice Plasticidade	12	%
Limite Retração	5.3	%

CONSULTOR / EMPREITEIRO : Téc. Engs. Consultores

PROJECTO : N13 Nampula/Cuamba

CAMADA ENSAIADA : C.E do km: 204+00 L/E "Cache"

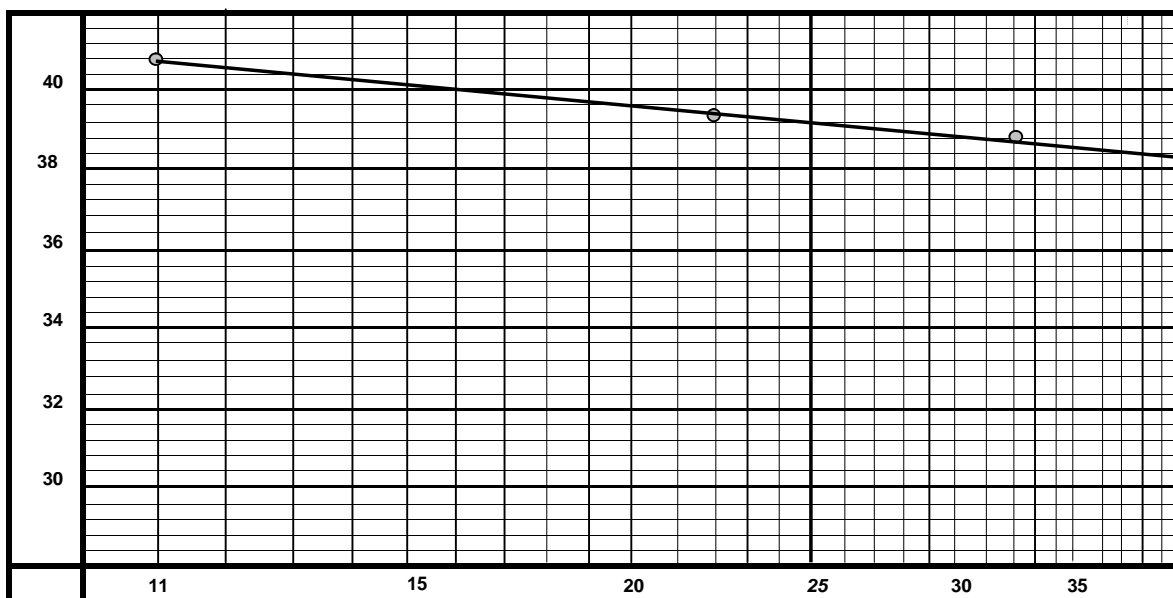
AMOSTRA Nº : NAM/0019/06 EFECTUADO POR : Rafael VERIFICADO POR: Rafael DATA: 2006/6/11

DESCRIÇÃO DO SOLO : Areia argilosa Castanho Escuro com Cascalho, "Laterite"

1. DETERMINAÇÃO DO LIMITE DE LIQUIDEZ

Nº da Cápsula	Nº	8	9	10	11				
Peso Solo Húmido + Cápsula (P0)	gr	24.70	23.09	25.60	23.7				
Peso Solo Seco + Cápsula (P1)	gr	22.20	21.10	22.90	21.5				
Peso da Cápsula (P2)	gr	16.04	16.03	15.96	15.74				
Peso da Água P3 = (P0) - (P1)	gr	2.50	1.99	2.70	2.2				
Peso do Solo Seco P4 = (P1) - (P2)	gr	6.16	5.07	6.94	5.7				
Teor de Humidade W = P3 / P4 x 100	gr	40.6	39.3	38.9	38.2				
Nº de Pancadas	Nº	11	22	32	40				

Teor de Humidade %



Número de Pancadas

2. DETERMINAÇÃO DO LIMITE DE PLASTICIDADE

Nº da Cápsula	Nº	12	13	14				
Peso do Solo Húmido + Cápsula (P0)	gr	25.42	25.50	26.7				
Peso Solo Seco + Cápsula (P1)	gr	23.80	23.90	24.9				
Peso da Cápsula (P2)	gr	16.04	16.13	16.14				
Peso da Água P3 = (P0) - (P1)	gr	1.62	1.60	1.8				
Peso do Solo Seco P4 = (P1) - (P2)	gr	7.76	7.77	8.76				
Teor de Humidade W = P3 / P4 x 100	gr	20.9	20.6	20.5				

3. DETERMINAÇÃO DO LIMITE DE RETRAÇÃO

Nº do Moulde	Nº	1			
Comprimento do Moulde (L1)	mm	15.20			
Distância Retraída (L2)	mm	1.30			
Retração Linear L.S. = (L2) / (L1) x 100	%	8.6			

4. RESUMO RESULTADOS

Limite de Liquidez	39	%
Limite Plasticidade	21	%
Indice Plasticidade	18	%
Limite Retração	8.6	%

CONSULTOR / EMPREITEIRO : Téc. Engs. Consultores

PROJECTO : N13 Nampula/Cuamba

CAMADA ENSAIADA : C.E do km: 256+50 L/D

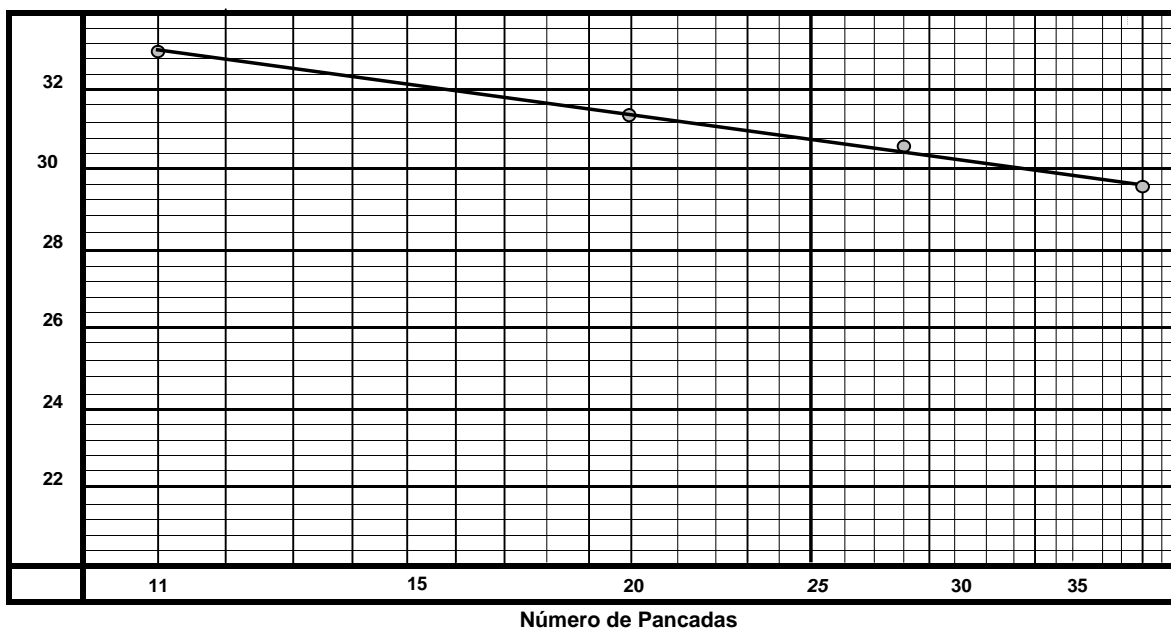
AMOSTRA Nº : NAM/0020/06 EFECTUADO POR : Rafael VERIFICADO POR: Rafael DATA: 22/11/2006

DESCRIÇÃO DO SOLO : Areia argilosa Castanho Escuro com Laterite

1. DETERMINAÇÃO DO LIMITE DE LIQUIDEZ

Nº da Cápsula	Nº	1	2	3	4				
Peso Solo Húmido + Cápsula (P0)	gr	25.10	25.67	24.80	24.6				
Peso Solo Seco + Cápsula (P1)	gr	22.80	23.30	22.70	22.6				
Peso da Cápsula (P2)	gr	15.80	15.74	15.84	15.81				
Peso da Água P3 = (P0) - (P1)	gr	2.30	2.37	2.10	2				
Peso do Solo Seco P4 = (P1) - (P2)	gr	7.00	7.56	6.86	6.79				
Teor de Humidade W = P3 / P4 x 100	gr	32.9	31.3	30.6	29.5				
Nº de Pancadas	Nº	11	20	28	38				

Teor de Humidade %



2. DETERMINAÇÃO DO LIMITE DE PLASTICIDADE

Nº da Cápsula	Nº	5	6	7				
Peso do Solo Húmido + Cápsula (P0)	gr	26.50	25.20	24.9				
Peso Solo Seco + Cápsula (P1)	gr	25.10	24.00	23.7				
Peso da Cápsula (P2)	gr	15.93	15.95	15.91				
Peso da Água P3 = (P0) - (P1)	gr	1.40	1.20	1.2				
Peso do Solo Seco P4 = (P1) - (P2)	gr	9.17	8.05	7.79				
Teor de Humidade W = P3 / P4 x 100	gr	15.3	14.9	15.4				

3. DETERMINAÇÃO DO LIMITE DE RETRAÇÃO

Nº do Moulde	Nº	2			
Comprimento do Moulde (L1)	mm	15.20			
Distância Retraída (L2)	mm	1.10			
Retração Linear L.S. = (L2) / (L1) x 100	%	7.2			

4. RESUMO RESULTADOS

Limite de Liquidez	31	%
Limite Plasticidade	15	%
Indice Plasticidade	15	%
Limite Retração	7.2	%

CONSULTOR / EMPREITEIRO : Téc. Engs. Consultores

PROJECTO : N13 Nampula/Cuamba

CAMADA ENSAIADA : C.E do km: 332+10 L/D

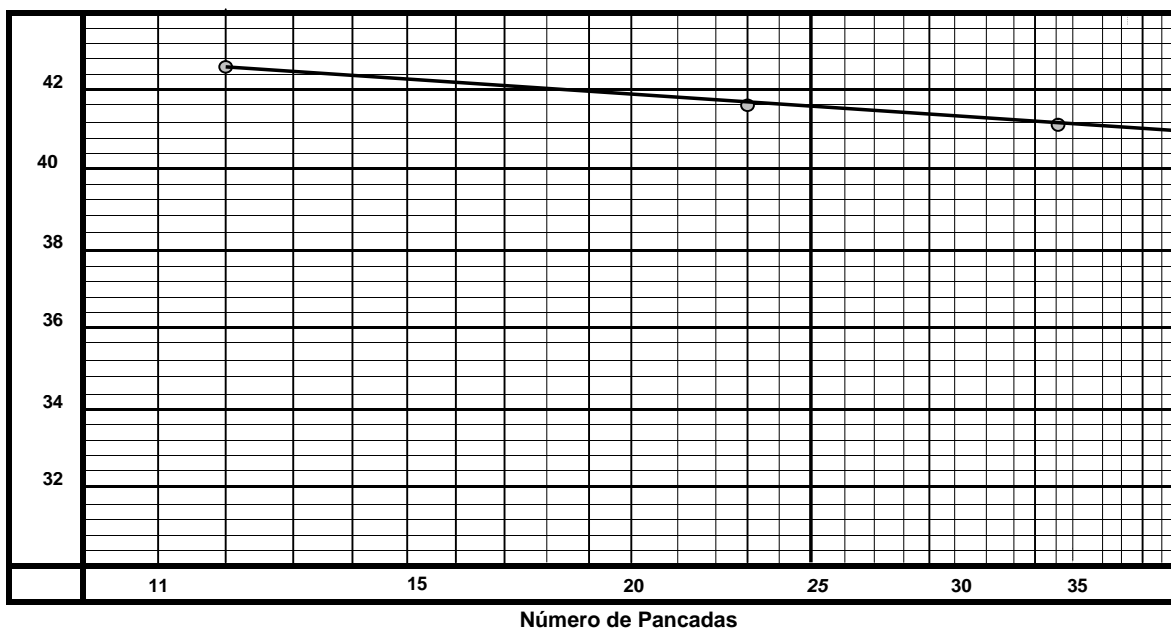
AMOSTRA Nº : NAM/0021/06 EFECTUADO POR : Rafael VERIFICADO POR: Rafael DATA: 22/11/2006

DESCRIÇÃO DO SOLO : Areia argilosa Castanho Claro com Laterite

1. DETERMINAÇÃO DO LIMITE DE LIQUIDEZ

Nº da Cápsula	Nº	8	9	10	11				
Peso Solo Húmido + Cápsula (P0)	gr	26.10	23.50	24.06	24				
Peso Solo Seco + Cápsula (P1)	gr	23.10	21.30	21.70	21.6				
Peso da Cápsula (P2)	gr	16.04	16.03	15.96	15.74				
Peso da Água P3 = (P0) - (P1)	gr	3.00	2.20	2.36	2.4				
Peso do Solo Seco P4 = (P1) - (P2)	gr	4.06	8.27	5.74	5.86				
Teor de Humidade W = P3 / P4 x 100	gr	42.5	41.7	41.1	41.0				
Nº de Pancadas	Nº	12	23	34	40				

Teor de Humidade %



2. DETERMINAÇÃO DO LIMITE DE PLASTICIDADE

Nº da Cápsula	Nº	12	13	14				
Peso do Solo Húmido + Cápsula (P0)	gr	26.30	28.40	27.7				
Peso Solo Seco + Cápsula (P1)	gr	24.40	26.20	25.6				
Peso da Cápsula (P2)	gr	16.04	16.13	16.14				
Peso da Água P3 = (P0) - (P1)	gr	1.90	2.20	2.1				
Peso do Solo Seco P4 = (P1) - (P2)	gr	8.36	10.07	9.46				
Teor de Humidade W = P3 / P4 x 100	gr	22.7	21.8	22.2				

3. DETERMINAÇÃO DO LIMITE DE RETRAÇÃO

Nº do Moulde	Nº	1			
Comprimento do Moulde (L1)	mm	15.20			
Distância Retraída (L2)	mm	1.40			
Retração Linear L.S. = (L2) / (L1) x 100	%	9.2			

4. RESUMO RESULTADOS

Limite de Liquidez	42	%
Limite Plasticidade	22	%
Índice Plasticidade	19	%
Limite Retração	9.2	%

CONSULTOR / EMPREITEIRO :	TÉC. ENGS. CONSULTORES	DATA:	2006/6/11
PROJECTO :	N13 NAMPULA/CUAMBA		
CAMADA ENSAIADA :	C.E Km: 79+00 L/E	VERIF. POR:	
AMOSTRA Nº :	NAM/0017/06	EFFECT. POR :	RAFAEL
DESCRIÇÃO DO SOLO :	AREIA ARGILOSA CASTANHA CLARA COM LATERITE		

DETERMINAÇÃO DO PESO ESPECÍFICO

Nº do Picnómetro		Nº				
Peso da Brita + Picnómetro (P1)		gr				
Peso da Brita + Água + Picnómetro (P2)		gr				
Peso do Picnómetro + Água (P3)		gr				
Peso da Brita (P4)		gr				
Peso da Brita Ps= (P1)-(P4)		gr				
Peso da Água - Volume da Brita P6= (P2)-(P1)		gr				
Peso da Água P7= (P3)-(P4)		gr				
Volume da Brita Vs=P7-P6		cc				
Peso Específico SG=Ps/Vs		-				

DETERMINAÇÃO DA ABSORÇÃO DE ÁGUA

Nº Da Cápsula		Nº				
Peso da Cápsula + Brita Seca (P1)		gr				
Peso da Cápsula + Brita Molhada * (P2)		gr				
Peso da Cápsula (P3)		gr				
Peso da Água P4=(P2)-(P1)		gr				
Peso da Brita Seca P5= (P1)-(P3)		gr				
Absorção de Água W=P4/P5x100		%				

DETERMINAÇÃO DA BARIDADE APARENTE

Nº Da Molde		Nº				
Peso da Brita + Molde (P1)		gr				
Peso do Molde (P2)		gr				
Peso da Brita (P3)=(P1)-(P2)		gr				
Volume do Molde V		cc				
Baridade da Brita Bs=P3/V		gr/cc				

DETERMINAÇÃO DO TEOR DE HUMIDADE

Nº Da Cápsula		Nº	1	2	3	
Peso Do Solo Húmido + Cápsula (P1)		gr	326.1	315.2	293	
Peso Do Solo Seco + Cápsula (P2)		gr	311.1	301.4	284.9	
Peso da Cápsula (P3)		gr	84.3	84.5	84.5	
Peso da Água P4=(P1)-(P2)		gr	15	13.8	8.1	
Peso do Solo Seco P5= (P2)-(P3)		gr	226.8	216.9	200.4	
Teor de Humidade W=P4/P5x100		%	6.6	6.4	4.0	
			MÉDIA =		5.7 %	

CONSULTOR / EMPREITEIRO :	TÉC. ENGS. CONSULTORES	DATA:	<u>2006/6/11</u>
PROJECTO :	N13 NAMPULA/CUAMBA	VERIF. POR:	
CAMADA ENSAIADA :	C.E Km: 124+00 L/D "NAMIGONHA"	EFFECT. POR :	RAFAEL
AMOSTRA Nº :	NAM/0018/06		
DESCRIÇÃO DO SOLOAREIA ARGILOSA CASTANHA CLARA COM CASCALHO "LATERITE"			

DETERMINAÇÃO DO PESO ESPECÍFICO

Nº do Picnómetro		Nº				
Peso da Brita + Picnómetro	(P1)	gr				
Peso da Brita + Água + Picnómetro	(P2)	gr				
Peso do Picnómetro + Água	(P3)	gr				
Peso da Brita	$P_s = (P1) - (P4)$	gr				
Peso da Água - Volume da Brita	$P_6 = (P2) - (P1)$	gr				
Peso da Água	$P_7 = (P3) - (P4)$	gr				
Volume da Brita	$V_s = P_7 - P_6$	cc				
Peso Específico	$SG = P_s / V_s$	-				

DETERMINAÇÃO DA ABSORÇÃO DE ÁGUA

Nº Da Cápsula		Nº				
Peso da Cápsula + Brita Seca	(P1)	gr				
Peso da Cápsula + Brita Molhada *	(P2)	gr				
Peso da Cápsula	(P3)	gr				
Peso da Água	$P_4 = (P2) - (P1)$	gr				
Peso da Brita Seca	$P_5 = (P1) - (P3)$	gr				
Absorção de Água	$W = P_4 / P_5 \times 100$	%				

DETERMINAÇÃO DA BARIDADE APARENTE

Nº Da Molde		Nº				
Peso da Brita + Molde	(P1)	gr				
Peso do Molde	(P2)	gr				
Peso da Brita	$(P3) = (P1) - (P2)$	gr				
Volume do Molde	V	cc				
Baridade da Brita	$B_s = P_3 / V$	gr/cc				

DETERMINAÇÃO DO TEOR DE HUMIDADE

Nº Da Cápsula		Nº	4	5	6	
Peso Do Solo Húmido + Cápsula	(P1)	gr	389.4	394.8	351.2	
Peso Do Solo Seco + Cápsula	(P2)	gr	384.3	389.2	347	
Peso da Cápsula	(P3)	gr	84.4	83.7	84	
Peso da Água	$P_4 = (P1) - (P2)$	gr	5.1	5.6	4.2	
Peso do Solo Seco	$P_5 = (P2) - (P3)$	gr	299.9	305.5	263	
Teor de Humidade	$W = P_4 / P_5 \times 100$	%	1.7	1.8	1.6	
			MÉDIA = 1.7 %			

CONSULTOR / EMPREITEIRO : TÉC. ENGS. CONSULTORES	DATA: <u>2006/6/11</u>
PROJECTO: 13 NAMPULA/CUAMBA " MALEMA"	
CAMADA ENSAIADA : C.E Km: 204+00 L/E "CACHE"	VERIF. POR: _____
AMOSTRA Nº : NAM/0019/06	EFFECT. POR : RAFAEL
DESCRIÇÃO DO SOLO : AREIA ARGILOSA CASTANHA ESCURA COM CASCALHO "LATERITE"	

DETERMINAÇÃO DO PESO ESPECÍFICO

Nº do Picnómetro		Nº				
Peso da Brita + Picnómetro (P1)		gr				
Peso da Brita + Água + Picnómetro (P2)		gr				
Peso do Picnómetro + Água (P3)		gr				
Peso da Brita + Picnómetro (P4)		gr				
Peso da Brita	$P_s = (P1) - (P4)$	gr				
Peso da Água - Volume da Brita	$P_6 = (P2) - (P1)$	gr				
Peso da Água	$P_7 = (P3) - (P4)$	gr				
Volume da Brita	$V_s = P_7 - P_6$	cc				
Peso Específico	$SG = P_s / V_s$	-				

DETERMINAÇÃO DA ABSORÇÃO DE ÁGUA

Nº Da Cápsula		Nº				
Peso da Cápsula + Brita Seca (P1)		gr				
Peso da Cápsula + Brita Molhada * (P2)		gr				
Peso da Cápsula (P3)		gr				
Peso da Água	$P_4 = (P2) - (P1)$	gr				
Peso da Brita Seca	$P_5 = (P1) - (P3)$	gr				
Absorção de Água	$W = P_4 / P_5 \times 100$	%				

DETERMINAÇÃO DA BARIDADE APARENTE

Nº Da Molde		Nº				
Peso da Brita + Molde (P1)		gr				
Peso do Molde (P2)		gr				
Peso da Brita (P3) = (P1) - (P2)		gr				
Volume do Molde	V	cc				
Baridade da Brita	$B_s = P_3 / V$	gr/cc				

DETERMINAÇÃO DO TEOR DE HUMIDADE

Nº Da Cápsula		Nº	7	8	9	
Peso Do Solo Húmido + Cápsula (P1)		gr	382.3	401	362	
Peso Do Solo Seco + Cápsula (P2)		gr	376.9	394.8	356.1	
Peso da Cápsula (P3)		gr	84.6	85	84.5	
Peso da Água	$P_4 = (P1) - (P2)$	gr	5.4	6.2	5.9	
Peso do Solo Seco	$P_5 = (P2) - (P3)$	gr	292.3	309.8	271.6	
Teor de Humidade	$W = P_4 / P_5 \times 100$	%	1.8	2.0	2.2	
			MÉDIA = 2.0 %			

CONSULTOR / EMPREITEIRO : TÉC. ENGS. CONSULTORES	DATA: <u>2006/6/11</u>
PROJECTO: M3 NAMPULA/CUAMBA "MALEMA"	
CAMADA ENSAIADA : C.E Km: 256+50 L/D 100m DO EIXO	VERIF. POR: _____
AMOSTRA Nº : NAM/0020/06	EFFECT. POR : RAFAEL
DESCRIÇÃO DO SOLO : AREIA ARGILOSA CASTANHA ESCURA COM CASCALHO "LATERITE"	

DETERMINAÇÃO DO PESO ESPECÍFICO

Nº do Picnómetro		Nº				
Peso da Brita + Picnómetro (P1)		gr				
Peso da Brita + Água + Picnómetro (P2)		gr				
Peso do Picnómetro + Água (P3)		gr				
Peso da Brita (P4)		gr				
Peso da Brita $P_s = (P1) - (P4)$		gr				
Peso da Água - Volume da Brita $P_6 = (P2) - (P1)$		gr				
Peso da Água $P_7 = (P3) - (P4)$		gr				
Volume da Brita $V_s = P_7 - P_6$		cc				
Peso Específico $SG = P_s / V_s$		-				

DETERMINAÇÃO DA ABSORÇÃO DE ÁGUA

Nº Da Cápsula		Nº				
Peso da Cápsula + Brita Seca (P1)		gr				
Peso da Cápsula + Brita Molhada * (P2)		gr				
Peso da Cápsula (P3)		gr				
Peso da Água $P_4 = (P2) - (P1)$		gr				
Peso da Brita Seca $P_5 = (P1) - (P3)$		gr				
Absorção de Água $W = P_4 / P_5 \times 100$		%				

DETERMINAÇÃO DA BARIDADE APARENTE

Nº Da Molde		Nº				
Peso da Brita + Molde (P1)		gr				
Peso do Molde (P2)		gr				
Peso da Brita $(P3) = (P1) - (P2)$		gr				
Volume do Molde V		cc				
Baridade da Brita $B_s = P_3 / V$		gr/cc				

DETERMINAÇÃO DO TEOR DE HUMIDADE

Nº Da Cápsula		Nº	10	11	12		
Peso Do Solo Húmido + Cápsula (P1)		gr	484.5	545.3	446.3		
Peso Do Solo Seco + Cápsula (P2)		gr	475.9	537	438.7		
Peso da Cápsula (P3)		gr	84.5	84.5	84.4		
Peso da Água $P_4 = (P1) - (P2)$		gr	8.6	8.3	7.6		
Peso do Solo Seco $P_5 = (P2) - (P3)$		gr	391.4	452.5	354.3		
Teor de Humidade $W = P_4 / P_5 \times 100$		%	2.2	1.8	2.1		
			MÉDIA = 2.1 %				

CONSULTOR / EMPREITEIRO : TÉC. ENGS. CONSULTORES	DATA: <u>2006/6/11</u>
PROJECTO I13 NAMPULA/CUAMBA "CUAMBA"	
CAMADA ENSAIADA : <u>C.E Km: 332+10 L/D</u>	VERIF. POR: _____
AMOSTRA Nº : <u>NAM/0021/06</u>	EFFECT. POR : <u>RAFAEL</u>
DESCRIÇÃO DO SOLO : <u>AREIA ARGILOSA CASTANHA ESCURA COM CASCALHO "LATERITE"</u>	

DETERMINAÇÃO DO PESO ESPECÍFICO

Nº do Picnómetro		Nº				
Peso da Brita + Picnómetro (P1)		gr				
Peso da Brita + Água + Picnómetro (P2)		gr				
Peso do Picnómetro + Água (P3)		gr				
Peso da Brita (P4)		gr				
Peso da Brita	$P_s = (P1) - (P4)$	gr				
Peso da Água - Volume da Brita	$P_6 = (P2) - (P1)$	gr				
Peso da Água	$P_7 = (P3) - (P4)$	gr				
Volume da Brita	$V_s = P_7 - P_6$	cc				
Peso Específico	$SG = P_s / V_s$	-				

DETERMINAÇÃO DA ABSORÇÃO DE ÁGUA

Nº Da Cápsula		Nº				
Peso da Cápsula + Brita Seca (P1)		gr				
Peso da Cápsula + Brita Molhada *	(P2)	gr				
Peso da Cápsula (P3)		gr				
Peso da Água	$P_4 = (P2) - (P1)$	gr				
Peso da Brita Seca	$P_5 = (P1) - (P3)$	gr				
Absorção de Água	$W = P_4 / P_5 \times 100$	%				

DETERMINAÇÃO DA BARIDADE APARENTE

Nº Da Molde		Nº				
Peso da Brita + Molde (P1)		gr				
Peso do Molde (P2)		gr				
Peso da Brita (P3)	$(P3) = (P1) - (P2)$	gr				
Volume do Molde	V	cc				
Baridade da Brita	$B_s = P_3 / V$	gr/cc				

DETERMINAÇÃO DO TEOR DE HUMIDADE

Nº Da Cápsula		Nº	13	14	15	
Peso Do Solo Húmido + Cápsula (P1)		gr	427	406.2	342.8	
Peso Do Solo Seco + Cápsula (P2)		gr	418.5	396.9	335.1	
Peso da Cápsula (P3)		gr	84.1	84.8	84.4	
Peso da Água	$P_4 = (P1) - (P2)$	gr	8.5	9.3	7.7	
Peso do Solo Seco	$P_5 = (P2) - (P3)$	gr	334.4	312.1	250.7	
Teor de Humidade	$W = P_4 / P_5 \times 100$	%	2.5	3.0	3.1	
			MÉDIA = 2.9 %			

LABORATÓRIO PROVINCIAL Nampula

5

CONSULTOR / EMPREITEIRO:	<u>Téc. Eng. Comiteza</u>	DATA:	<u>06/11/00</u>
PROJECTO:	<u>A13 Nampula/Cuacando</u>		<u>1 Cuacando</u>
CAMADA ENSAIADA	<u>C.E. do Mac: 32210 L11</u>		
AMOSTRA Nº:	<u>HAR002/100</u>	ENSAIO EFECTUADO POR:	<u>Rafael</u>
DESCRIÇÃO DO SOLO:	<u>Arg. orgânica, castanha, grupo C, encolado</u> <u>"laterite"</u>		

DETERMINAÇÃO DO PESO ESPECÍFICO

Nº do Pícnómetro	Nº						
Peso da Brita + Pícnómetro	(P1)	gr					
Peso do Brita + Água + Pícnómetro	(P2)	gr					
Peso Pícnómetro + Água	(P3)	gr					
Peso da Brita	$P_4 = (P1) - (P3)$	gr					
Peso Água - Volume da Brita	$P_5 = (P2) - (P1)$	gr					
Peso da Água	$P_7 = (P3) - (P4)$	gr					
Volume da Brita	$V_8 = P_7 - P_6$	cc					
Peso Específico	$SG = P_6 / V_8$	-					

DETERMINAÇÃO DA ABSORÇÃO DE ÁGUA

Nº da Cápsula	Nº						
Peso da Cápsula + Brita Seca	(P1)	gr					
Peso da Cápsula + Brita Molhada *	(P2)	gr					
Peso da Cápsula	(P3)	gr					
Peso da Água	$P_4 = (P2) - (P1)$	gr					
Peso da Brita Seca	$P_6 = (P1) - (P3)$	gr					
Absorção de Água	$W = P_4 / P_6 \times 100$	%					

DETERMINAÇÃO DA BARIDADE APARENTE

Nº do Molde	Nº						
Peso da Brita + Molde	(P1)	gr					
Peso do Molde	(P2)	gr					
Peso da Brita	$P_3 = (P1) - (P2)$	gr					
Volume do Molde	V	cc					
Baridade da Brita	$B_s = P_3 / V$	gr/cc					

DETERMINAÇÃO DO TEOR DE HUMIDADE

Nº da Cápsula	Nº		13	14	15		
Peso do Solo Húmido + Cápsula	(P1)	gr	422,00	406,20	342,80		
Peso do Solo Seco + Cápsula	(P2)	gr	416,50	396,90	335,10		
Peso da Cápsula	(P3)	gr	84,10	84,80	84,40		
Peso da Água	$P_4 = (P1) - (P2)$	gr	8,50	9,30	7,70		
Peso do Solo Seco	$P_6 = (P2) - (P3)$	gr	332,40	312,10	250,70		
Teor de Humidade	$W = P_4 / P_6 \times 100$	%	2,5	3,0	3,1		<u>médias 2,9%</u>

Appendix B-6
QUARRY TEST

CONSULTOR / EMPREITEIRO :	<u>TÉC. ENGS. CONSULTORES</u>	DATA:	<u>29/11/2006</u>
PROJECTO :	<u>N13 NAMPULA/CUAMBA-NAMIALO</u>		
CAMADA ENSAIADA :	<u>PEDREIRA DE NAMIALO Km: 92+00 L/E ; 0.5 mm</u>	VERIF. POR:	
AMOSTRA Nº :	<u>NAM/Q/01</u>	EFFECT. POR :	<u>RAFAEL</u>
DESCRIÇÃO DO SOLO :			

DETERMINAÇÃO DO ACV

Nº da Amostras		Nº	1			
Peso Total da Amostra Seca Após o Ensaio	(P1)	gr	2039.7	2670.6		
Peso da Fracção Passada no Peneiro 2.36	(P2)	gr	562.0	761.7		
ACV da Amostra Seca	$ACV=(P2)/(P1)*100$	%	27.6	28.5		
ACV Médio		%	28.0			

DETERMINAÇÃO DO 10% FACT

Nº da Amostras		Nº				
Força Aplicada em KN		KN				
Peso Total da Amostra Seca Após o Ensaio	(P1)	gr				
Peso da Fracção Passada no Peneiro 2.36	(P2)	gr				
% FACT da Amostra Seca	$10\%FACT=(P2)/(P1)*100$	KN				
% FACT Seco Médio		KN				
Peso Total Da Amostra Seca Após o Ensaio Satur. W=P4/P5x100		gr				
Peso da Fracção Passada no Peneiro	(P2)	gr				
10% FACT da Amostra Saturada	$10\%FACT=(P2)/(P1)*100$	KN				
10% FACT da Amostra Média		KN				
% Perda de Resistência Média	$10\%FACT/10\%FACTx100$	%				

DETERMINAÇÃO DO INDICE DE LAMELAÇÃO

Amostra Nº		Nº				
Peso Total da Amostra - 26.6mm+19.0mm	(P1)	gr				
Peso da Amostra Passada - 25.5mm+19.0mm	(P2)	gr				
Índice Lamentação - 25.5mm+19.0mm	$IL=(P2)/(P1)*100$	%				
Índice Lamentação Médio - 25.5mm+19.0mm		%				
Peso Total da Amostra - 19.0mm + 13.2mm		gr				
Peso Da Amostra Passada - 19.0mm + 13.2mm		gr				
Índice de Lamelação - 19.0mm + 13.2mm		%				
Índice de Lamelação Médio - 19.0mm + 13.2mm						
Peso Total da Amostra - 13.2mm + 9.5mm		gr				
Peso da Amostra Passada - 13.2mm + 9.5mm		gr				
Índice de Lamelação - 13.2mm + 9.5mm		%				
Índice de Lamelação Médio - 13.2mm + 9.5mm		%				
Peso Total da Amostra - 9.5mm + 6.7mm		gr				
Peso da Amostra Passada - 9.5mm + 6.7mm		gr				
Índice de Lamelação - 9.5mm + 6.7mm		%				
Índice de Lamelação Médio - 9.5mm + 6.7mm						

CONSULTOR / EMPREITEIRO :	TÉC. ENGS. CONSULTORES	DATA:	29/11/2006
PROJECTO	13 NAMPULA/CUAMBA - CARAMACHA-2		
CAMADA ENSAIADA :	PEDREIRA DO Km: 60+30 L/D	VERIF. POR:	
AMOSTRA Nº :	NAM/Q/02	EFFECT. POR :	RAFAEL
DESCRIÇÃO DO SOLO :			

DETERMINAÇÃO DO ACV

Nº da Amostras		Nº	2	
Peso Total da Amostra Seca Após o Ensaio	(P1)	gr	3055.6	1711.9
Peso da Fracção Passada no Peneiro	2.36 (P2)	gr	118.3	672.5
ACV da Amostra Seca	ACV=(P2)/(P1)*100		3.9	39.3
ACV Médio			21.6	

DETERMINAÇÃO DO 10% FACT

Nº da Amostras		Nº		
Força Aplicada em KN		KN		
Peso Total da Amostra Seca Após o Ensaio	(P1)	gr		
Peso da Fracção Passada no Peneiro	2.36 (P2)	gr		
% FACT da Amostra Seca	10%FACT=(P2)/(P1)x100		KN	
% FACT Seco Médio			KN	
Peso Total Da Amostra Seca Após o Ensaio Satur. W=P4/P5x100		gr		
Peso da Fracção Passada no Peneiro	(P2)	gr		
10% FACT da Amostra Saturada	10% FACT=(P2)/(P1)*100		KN	
10% FACT da Amostra Média			KN	
% Perda de Resistência Média	10% FACT/10% FACTx100		%	

DETERMINAÇÃO DO INDICE DE LAMELAÇÃO

Amostra Nº		Nº		
Peso Total da Amostra - 26.6mm+19.0mm	(P1)	gr		
Peso da Amostra Passada - 25.5mm+19.0mm	(P2)	gr		
Índice Lamentação - 25.5mm+19.0mm	IL=(P2)/(P1)x100		%	
Índice Lamentação Médio - 25.5mm+19.0mm			%	
Peso Total da Amostra - 19.0mm + 13.2mm		gr		
Peso Da Amostra Passada - 19.0mm + 13.2mm		gr		
Índice de Lamelação - 19.0mm + 13.2mm			%	
Índice de Lamelação Médio - 19.0mm + 13.2mm			%	
Peso Total da Amostra - 13.2mm + 9.5mm		gr		
Peso da Amostra Passada - 13.2mm + 9.5mm		gr		
Índice de Lamelação - 13.2mm + 9.5mm			%	
Índice de Lamelação Médio - 13.2mm + 9.5mm			%	
Peso Total da Amostra - 9.5mm + 6.7mm		gr		
Peso da Amostra Passada - 9.5mm + 6.7mm		gr		
Índice de Lamelação - 9.5mm + 6.7mm			%	
Índice de Lamelação Médio - 9.5mm + 6.7mm			%	

CONSULTOR / EMPREITEIRO : TÉC. ENGS. CONSULTORES	DATA: 29/11/2006
PROJECTO : N13 NAMPULA/CUAMBA-RIBANÉ	
CAMADA ENSAIADA : PEDREIRA RIBANÉ-CUAMBA Km: 10+00 L/E	VERIF. POR: _____
AMOSTRA Nº : NAM/Q/03	EFFECT. POR : RAFAEL
DESCRIÇÃO DO SOLO : _____	

DETERMINAÇÃO DO ACV

Nº da Amostras	Nº	3	
Peso Total da Amostra Seca Após o Ensaio (P1)	gr	3123.5	2896.3
Peso da Fracção Passada no Peneiro 2.36 (P2)	gr	1241.3	1047.3
ACV da Amostra Seca $ACV=(P2)/(P1)*100$	%	39.7	36.2
ACV Médio	%	38.0	

DETERMINAÇÃO DO 10% FACT

Nº da Amostras	Nº		
Força Aplicada em KN _____	KN		
Peso Total da Amostra Seca Após o Ensaio (P1)	gr		
Peso da Fracção Passada no Peneiro 2.36 (P2)	gr		
% FACT da Amostra Seca $10\%FACT=(P2)/(P1)*100$	KN		
% FACT Seco Médio	KN		
Peso Total Da Amostra Seca Após o Ensaio Satur. $W=P4/P5*100$	gr		
Peso da Fracção Passada no Peneiro _____ (P2)	gr		
10% FACT da Amostra Saturada $10\%FACT=(P2)/(P1)*100$	KN		
10% FACT da Amostra Média	KN		
% Perda de Resistência Média $10\%FACT/10\%FACT*100$	%		

DETERMINAÇÃO DO ÍNDICE DE LAMELAÇÃO

Amostra Nº	Nº		
Peso Total da Amostra - 26.6mm+19.0mm (P1)	gr		
Peso da Amostra Passada - 25.5mm+19.0mm (P2)	gr		
Índice Lamentação - 25.5mm+19.0mm $IL=(P2)/(P1)*100$	%		
Índice Lamentação Médio - 25.5mm+19.0mm	%		
Peso Total da Amostra - 19.0mm + 13.2mm	gr		
Peso Da Amostra Passada - 19.0mm + 13.2mm	gr		
Índice de Lamelação - 19.0mm + 13.2mm	%		
Índice de Lamelação Médio - 19.0mm + 13.2mm	%		
Peso Total da Amostra - 13.2mm + 9.5mm	gr		
Peso da Amostra Passada - 13.2mm + 9.5mm	gr		
Índice de Lamelação - 13.2mm + 9.5mm	%		
Índice de Lamelação Médio - 13.2mm + 9.5mm	%		
Peso Total da Amostra - 9.5mm + 6.7mm	gr		
Peso da Amostra Passada - 9.5mm + 6.7mm	gr		
Índice de Lamelação - 9.5mm + 6.7mm	%		
Índice de Lamelação Médio - 9.5mm + 6.7mm	%		

CONSULTOR / EMPREITEIRO :	TÉC. ENGS. CONSULTORES	DATA:	29/11/2006
PROJECTO N13 NAMPULA/CUAMBA-MALEMA/NPL			
CAMADA ENSAIADA :	PEDREIRA DE MALEMA Km: 11+50 L/E	VERIF. POR:	
AMOSTRA Nº :	NAM/Q/04	EFFECT. POR :	RAFAEL
DESCRIÇÃO DO SOLO :			

DETERMINAÇÃO DO ACV

Nº da Amostras		Nº	4		
Peso Total da Amostra Seca Após o Ensaio	(P1)	gr	3037.9	3046	
Peso da Fracção Passada no Peneiro 2.36	(P2)	gr	848.0	864.9	
ACV da Amostra Seca	$ACV=(P2)/(P1)*100$	%	27.9	28.4	
ACV Médio		%	28.2		

DETERMINAÇÃO DO 10% FACT

Nº da Amostras		Nº			
Força Aplicada em KN		KN			
Peso Total da Amostra Seca Após o Ensaio	(P1)	gr			
Peso da Fracção Passada no Peneiro 2.36	(P2)	gr			
% FACT da Amostra Seca	$10\%FACT=(P2)/(P1)*100$	KN			
% FACT Seco Médio		KN			
Peso Total Da Amostra Seca Após o Ensaio Satur. W=P4/P5x100		gr			
Peso da Fracção Passada no Peneiro	(P2)	gr			
10% FACT da Amostra Saturada	$10\%FACT=(P2)/(P1)*100$	KN			
10% FACT da Amostra Média		KN			
% Perda de Resistência Média	$10\%FACT/10\%FACT*100$	%			

DETERMINAÇÃO DO INDICE DE LAMELAÇÃO

Amostra Nº		Nº			
Peso Total da Amostra - 26.6mm+19.0mm	(P1)	gr			
Peso da Amostra Passada - 25.5mm+19.0mm	(P2)	gr			
Índice Lamentação - 25.5mm+19.0mm	$IL=(P2)/(P1)*100$	%			
Índice Lamentação Médio - 25.5mm+19.0mm		%			
Peso Total da Amostra - 19.0mm + 13.2mm		gr			
Peso Da Amostra Passada - 19.0mm + 13.2mm		gr			
Índice de Lamelação - 19.0mm + 13.2mm		%			
Índice de Lamelação Médio - 19.0mm + 13.2mm		%			
Peso Total da Amostra - 13.2mm + 9.5mm		gr			
Peso da Amostra Passada - 13.2mm + 9.5mm		gr			
Índice de Lamelação - 13.2mm + 9.5mm		%			
Índice de Lamelação Médio - 13.2mm + 9.5mm		%			
Peso Total da Amostra - 9.5mm + 6.7mm		gr			
Peso da Amostra Passada - 9.5mm + 6.7mm		gr			
Índice de Lamelação - 9.5mm + 6.7mm		%			
Índice de Lamelação Médio - 9.5mm + 6.7mm		%			

CONSULTOR / EMPREITEIRO :	TÉC. ENGS. CONSULTORES	DATA:	2006/1/12
PROJECTO :	N13 NAMPULA/CUAMBA		
CAMADA ENSAIADA :	PEDREIRA: CUAMBA/LICHINGA Km: 36+00 L/D	VERIF. POR:	
AMOSTRA Nº :	NAM/Q/05	EFFECT. POR :	RAFAEL
DESCRIÇÃO DO SOLO :			

DETERMINAÇÃO DO ACV

Nº da Amostras		Nº	5		
Peso Total da Amostra Seca Após o Ensaio	(P1)	gr	3470.5	2168	
Peso da Fracção Passada no Peneiro 2.36	(P2)	gr	741.7	520.8	
ACV da Amostra Seca	$ACV=(P2)/(P1)*100$	%	21.4	24.0	
ACV Médio		%	22.7		

DETERMINAÇÃO DO 10% FACT

Nº da Amostras		Nº			
Força Aplicada em KN		KN			
Peso Total da Amostra Seca Após o Ensaio	(P1)	gr			
Peso da Fracção Passada no Peneiro 2.36	(P2)	gr			
% FACT da Amostra Seca	$10\%FACT=(P2)/(P1)*100$	KN			
% FACT Seco Médio		KN			
Peso Total Da Amostra Seca Após o Ensaio Satur. W=P4/P5x100		gr			
Peso da Fracção Passada no Peneiro	(P2)	gr			
10% FACT da Amostra Saturada	$10\%FACT=(P2)/(P1)*100$	KN			
10% FACT da Amostra Média		KN			
% Perda de Resistência Média	$10\%FACT/10\%FACT*100$	%			

DETERMINAÇÃO DO INDICE DE LAMELAÇÃO

Amostra Nº		Nº			
Peso Total da Amostra - 26.6mm+19.0mm	(P1)	gr			
Peso da Amostra Passada - 25.5mm+19.0mm	(P2)	gr			
Indice Lamentação - 25.5mm+19.0mm	$IL=(P2)/(P1)*100$	%			
Indice Lamentação Médio - 25.5mm+19.0mm		%			
Peso Total da Amostra - 19.0mm + 13.2mm		gr			
Peso Da Amostra Passada - 19.0mm + 13.2mm		gr			
Indice de Lamelação - 19.0mm + 13.2mm		%			
Indice de Lamelação Médio - 19.0mm + 13.2mm		%			
Peso Total da Amostra - 13.2mm + 9.5mm		gr			
Peso da Amostra Passada - 13.2mm + 9.5mm		gr			
Indice de Lamelação - 13.2mm + 9.5mm		%			
Indice de Lamelação Médio - 13.2mm + 9.5mm		%			
Peso Total da Amostra - 9.5mm + 6.7mm		gr			
Peso da Amostra Passada - 9.5mm + 6.7mm		gr			
Indice de Lamelação - 9.5mm + 6.7mm		%			
Indice de Lamelação Médio - 9.5mm + 6.7mm		%			

Appendix B-7
UCS TEST RESULTS SOIL – CEMENT
MIXTURES

TÉCNICA - ENGENHEIROS CONSULTORES, LDA

AV. 25 DE SETEMBRO, NR. 2526, 1ST FLOOR
MAPUTO-MOCAMBIQUE

TELEPHONE: 21322185
FAX: 21322186
E-MAIL: tec@tec.co.mz

UCS SOILCEMENT

ORDEM	km	Nº	PROVETE	COMPOSIÇÃO (%)	ENERGIA	DATA /HORA DA MOLDAGEM	CARGA (kN)	RESISTÊNCIA (kPa)*
1	204.30	03	1º provete	95late. + 5cimento	5x25	17,01,07 - 23,01,07/10:00	15.4	1923
2	204.30	03	2º provete	95late. + 5cimento	5x25	17,01,07 - 23,01,07/10:15	15.1	1885
3	204.30	03	3º provete	95late. + 5cimento	5x25	17,01,07 - 23,01,07/11:16	15.7	1960
4	204.30	03	1º provete	96late. + 4cimento	5x25	17,01,07 - 23,01,07/11:15	10.5	1211
5	204.30	03	2º provete	96late. + 4cimento	5x25	17,01,07 - 23,01,07/11:16	13.1	1635
6	204.30	03	3º provete	96late. + 4cimento	5x25	17,01,07 - 23,01,07/10:15		
7	204.30	03	1º provete	97late. + 3cimento	5x25	17,01,07 - 23,01,07/08:00	8.6	1074
8	204.30	03	2º provete	97late. + 3cimento	5x25	17,01,07 - 23,01,07/08:15	6.8	849
9	256.50	03	3º provete	97late. + 3cimento	5x25	17,01,07 - 23,01,07/08:30	8.5	1061
10	256.50	04	1º provete	95late. + 5cimento	3x25	17,01,07 - 23,01,07/13:30	19.5	2434
11	256.50	04	2º provete	95late. + 5cimento	5x25	17,01,07 - 23,01,07/12:30	20.5	2559
12	256.50	04	3º provete	96late. + 4cimento	5x25	17,01,07 - 23,01,07/12:15	18.9	2360
13	256.50	04	1º provete	96late. + 4cimento	5x25	17,01,07 - 23,01,07/11:30	11.0	1373
14	256.50	04	2º provete	96late. + 4cimento	5x25	17,01,07 - 23,01,07/sem hora	19.3	2409
15	256.50	04	3º provete	97late. + 3cimento	5x25	17,01,07 - 23,01,07/10:00	17.2	2147
16	256.50	04	1º provete	97late. + 3cimento	5x25	17,01,07 - 23,01,07/09:30	20.2	2522
17	256.50	04	2º provete	97late. + 3cimento	5x25	17,01,07 - 23,01,07/09:15	19.3	2409
18	256.50	04	3º provete	96late. + 4cimento	5x25	17,01,07 - 23,01,07/09:00	9.5	1186
19	332.10	05	1º provete	95late. + 5cimento	5x25	18,01,07/11:10	18.4	2297
20	332.10	05	2º provete	95late. + 5cimento	5x25	18,01,07/11:55	15.3	1910
21	332.10	05	3º provete	95late. + 5cimento	5x25	sem data/11:40	13.1	1635
22	332.10	05	1º provete	96late. + 4cimento	5x25	sem data/10:31	6.0	749
23	332.10	05	2º provete	96late. + 4cimento	5x25	18,01,07/10:35	6.8	849
24	332.10	05	3º provete	97late. + 3cimento	5x25	sem data/09:45		
25	332.10	05	1º provete	97late. + 3cimento	5x25	18,01,07/09:40	10.8	1348
26	332.10	05	2º provete	97late. + 3cimento	5x25	18,01,07/09:25	9.6	1199
27	332.10	05	3º provete	96late. + 4cimento	5x25	18,01,07/10:55	8.7	1086.1

Appendix B-8
CBR TEST RESULTS MATERIAL
MIXTURES

CONSULTOR / EMPREITEIRO : TÉC. ENGS. CONSULTORES	DATA: <u>16-01-06</u>	ENSAIO DE COMPACTAÇÃO	
PROJECTO : N13 NAMPULA/CUAMBA	VERIF. POR: _____	Barid. Seca Máx. <u>2.152</u>	gr/cm3
CAMADA ENSAIADA : C.E DO Km: 79+00 L/E NAMIALO	EFFECT. POR : RAFAEL	Teor Ópt. Água <u>16.6</u>	%
AMOSTRA Nº : NAM/0016/07	DESCRICÃO DO SOLO : AREIA ARGILOSA CASTANHO CLARO COM CASCALHO		

DETERMINAÇÃO DO TEOR DE HUMIDADE

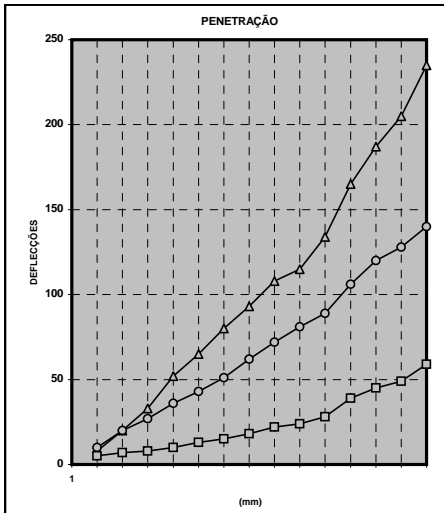
Nº da Cápsula/Molde	Nº	Hygroscópico			Depois da Mistura			Ótimo % (a)	16.6
		15	16	17	15	16	17		
Peso do Solo Húmido + Cápsula (P0)	gr	301.31	325.11	340.57				Hygroscópico % (b)	5.5
Peso do Solo Seco + Cápsula (P1)	gr	272.20	289.80	303.90				Volume de Água =	
Peso da Cápsula (P2)	gr	84.40	84.50	84.30				Solo seco x(100+a) - Solo seco =	
Peso da Água (P3)=(P0)-(P1)	gr	29.11	35.31	36.67				100	
Peso do Solo Seco (P4)=(P1)-(P2)	gr	187.80	205.30	219.60				=	1894
Teor de Humidade W=(P3)/(P4)x100	%	5.5	15.5	17.2					

Água a Adicionar

Moldagem				
Compacção	Nº	55x10x5	25x10x5	55x5,5x3
Nº do Molde	Nº	4	5	6
Peso do Molde + Solo Húmido (M0)	gr	11490	11250	10937
Peso do Molde (M1)	gr	6218	6239	6175
Peso do Solo Húmido (M2)=(M0)-(M1)	gr	5272	5011	4762
Volume do Molde V	cc	2104	2104	2104
Baridade Húmida do Solo Dh=(M2)/V	gr/cc	2.506	2.382	2.263
Teor Humidade da Moldagem W	%	16.5	16.5	16.5
Baridade Seca do Solo Ds=Dh x 100 / (100+W)	gr/cc	2.151	2.044	1.943
% de Compacção Ds/Ds Máx. x 100	%	99.0	95.0	90.2
Peso Molde+Solo Após Embebição (M3)	gr			
Peso Solo Húmido Final (M4)=(M3)-(M1)	gr			
Teor Humidade Final Wf=[M4-(Ds x V)]/(Ds x V)	%			

Expansão

Nº do Molde	1	2	3
Leitura Inicial			
Leitura Final			
% Expansão			



Nº Molde	4 △		5 ○		6 □	
	100.0		95.0		90	
% Mod AASHTO						
Penetração	Leitura	Corrig.	Leitura	Corrig.	Leitura	Corrig.
0.0						
0.5	8		10		5	
1.0	20		20		7	
1.5	33		27		8	
2.0	52		36		10	
2.5*	65	65	43	43	13	13
3.0	80		51		15	
3.5	93		62		18	
4.0	108		72		22	
4.5	115		81		24	
5.0*	134	134	89	89	28	28
6.0	165		106		39	
7.0	187		120		45	
8.0	205		128		49	
9.0	235		140		59	
Factor do Anel	0.012 KN					
CBR	2.5mm=13.344KN		5.0mm=20.016KN		7.5mm=25.354KN	
2,5mm *	5.8		3.9		1.2	
5,0mm **	8.0		5.3		1.7	
% Compacção						
CBR 2,5mm						
CBR 5,0mm						

CONSULTOR / EMPREITEIRO : TÉC. ENGS. CONSULTORES	DATA: <u>15-01-06</u>	ENSAIO DE COMPACTAÇÃO	
PROJECTO : N13 NAMPULA/CUAMBA	VERIF. POR: _____	Barid. Seca Máx. <u>2.287</u>	gr/cm3
CAMADA ENSAIADA : C.E DO Km: 124+20 L/D LAT.02	EFFECT. POR : RAFAEL	Teor Ópt. Água <u>7.6</u>	%
AMOSTRA Nº : NAM/0017/07	DESCRICÃO DO SOLO : AREIA ARGILOSA CASTANHO CLARA COM CASCALHO		

DETERMINAÇÃO DO TEOR DE HUMIDADE

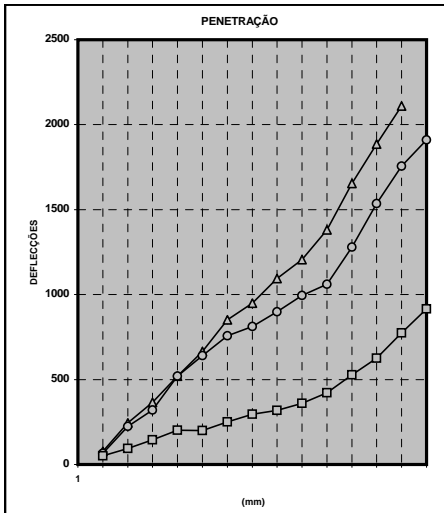
		Hygroscópico		Depois da Mistura			Água a Adicionar	
Nº da Cápsula/Molde	Nº			10	11	12	Ótimo % (a)	7.6
Peso do Solo Húmido + Cápsula (P0)	gr			467.10	395.60	396.57	Hygroscópico % (b)	0.7
Peso do Solo Seco + Cápsula (P1)	gr			440.50	373.20	375.60	Volume de Água =	
Peso da Cápsula (P2)	gr			84.50	84.50	84.40	Solo seco x(100+a) - Solo seco =	
Peso da Água (P3)=(P0)-(P1)	gr			26.60	22.40	20.97	100	
Peso do Solo Seco (P4)=(P1)-(P2)	gr			356.00	288.70	291.20	=	1233
Teor de Humidade W=(P3)/(P4)x100	%		0.7	7.5	7.8	7.2		

Moldagem

		55x10x5	25x10x5	55x5,5x3
Nº do Molde	Nº	4	5	6
Peso do Molde + Solo Húmido (M0)	gr	11376	11148	10830
Peso do Molde (M1)	gr	6218	6239	6175
Peso do Solo Húmido (M2)=(M0)-(M1)	gr	5158	4909	4655
Volume do Molde V	cc	2104	2104	2104
Baridade Húmida do Solo Dh=(M2)/V	gr/cc	2.452	2.333	2.212
Teor Humidade da Moldagem W	%	7.5	7.5	7.5
Baridade Seca do Solo Ds=Dh x 100 / (100+W)	gr/cc	2.280	2.170	2.058
% de Compactação Ds/Ds Máx. x 100	%	99.7	94.9	90.0
Peso Molde+Solo Após Embebição (M3)	gr			
Peso Solo Húmido Final (M4)=(M3)-(M1)	gr			
Teor Humidade Final Wf=[M4-(Ds x V)]/(Ds x V)	%			

Expansão

Nº do Molde	1	2	3
Leitura Inicial			
Leitura Final			
% Expansão			



Nº Molde % Mod AASHTO	4 △		5 ○		6 □	
	100.0		95.0		90	
Penetração	Leitura	Corrig.	Leitura	Corrig.	Leitura	Corrig.
0.0						
0.5	78		65		50	
1.0	245		225		95	
1.5	365		320		146	
2.0	520		520		202	
2.5*	665	665	640	640	200	200
3.0	852		758		250	
3.5	950		812		295	
4.0	1095		898		318	
4.5	1205		995		360	
5.0*	1380	1380	1060	1060	422	422
6.0	1655		1280		528	
7.0	1885		1535		625	
8.0	2110		1755		775	
9.0			1910		915	
Factor do Anel	0.012 KN					
CBR	2.5mm=13.344KN		5.0mm=20.016KN		7.5mm=25.354KN	
2,5mm *	59.8		57.6		18.0	
5,0mm **	82.7		53.0		25.3	
% Compactação						
CBR 2,5mm						
CBR 5,0mm						

CONSULTOR / EMPREITEIRO : TÉC. ENGS. CONSULTORES	DATA: <u>16-01-07</u>	ENSAIO DE COMPACTAÇÃO	
PROJECTO : N13 NAMPULA/CUAMBA		Barid. Seca Máx. <u>2.152</u>	gr/cm ³
CAMADA ENSAIADA : C.E DO Km: 79+00 L/E NAMIALO	VERIF. POR:	Teor Ópt. Água <u>16.6</u>	%
AMOSTRA Nº : NAM/0016/07	EFFECT. POR : RAFAEL		
DESCRIÇÃO DO SOLO : AREIA ARGILOSA CASTANHO CLARO COM CASCALHO			

DETERMINAÇÃO DO TEOR DE HUMIDADE

Água a Adicionar

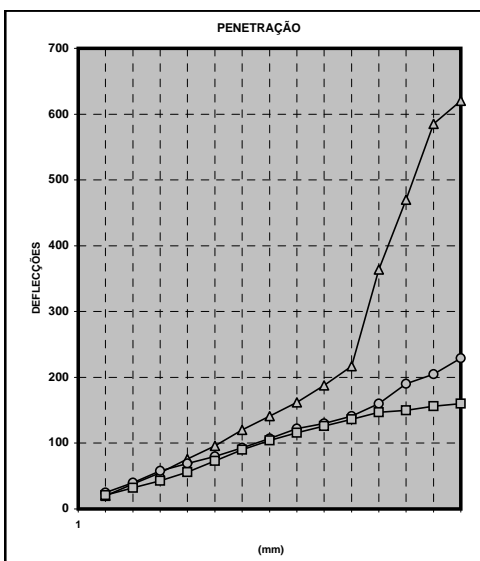
Nº da Cápsula/Molde	Nº	Hygroscópico		Depois da Mistura			Ótimo % (a)	16.6
		1	2	3	Hygroscópico % (b)	5.5		
Peso do Solo Húmido + Cápsula (P0)	gr		297.96	332.26	238.43			
Peso do Solo Seco + Cápsula (P1)	gr		267.70	296.20	217.20			
Peso da Cápsula (P2)	gr		84.30	84.10	84.50			
Peso da Água (P3)=(P0)-(P1)	gr		30.26	36.06	21.23			
Peso do Solo Seco (P4)=(P1)-(P2)	gr		183.40	212.10	132.70			
Teor de Humidade $W=(P3)/(P4) \times 100$	%	5.5	16.5	17.0	16.0			1894

Moldagem

Expansão

Compactação		Moldagem		
Nº do Molde	Nº	55x10x5	25x10x5	55x5,5x3
Peso do Molde + Solo Húmido (M0)	gr	11490	11250	10937
Peso do Molde (M1)	gr	6218	6239	6175
Peso do Solo Húmido (M2)=(M0)-(M1)	gr	5272	5011	4762
Volume do Molde V	cc	2104	2104	2104
Baridade Húmida do Solo $D_h=(M2)/V$	gr/cc	2.506	2.382	2.263
Teor Humidade da Moldagem W	%	16.5	16.5	16.5
Baridade Seca do Solo $D_s=D_h \times 100 / (100+W)$	gr/cc	2.154	2.044	1.943
% de Compactação $D_s/D_s \text{ Máx.} \times 100$	%	100.0	95.0	90.2
Peso Molde+Solo Após Embebição (M3)	gr	11492	11258	10960
Peso Solo Húmido Final (M4)=(M3)-(M1)	gr	5274	5019	4785
Teor Humidade Final $W_f=[M4-(D_s \times V)]/(D_s \times V)$	%	16.4	16.6	17.3

Nº do Molde	1	2	3
Leitura Inicial			
Leitura Final			
% Expansão			



Nº Molde	4 Δ		5 \circ		6 \square	
	% Mod AASHTO 100.0		95.0		90	
Penetração	Leitura	Corrig.	Leitura	Corrig.	Leitura	Corrig.
0.0						
0.5	20		25		21	
1.0	38		40		32	
1.5	55		58		43	
2.0	76		69		56	
2.5*	96	96	80	80	73	73
3.0	120		93		90	
3.5	141		107		104	
4.0	162		122		116	
4.5	188		130		126	
5.0*	217	217	141	141	136	136
6.0	364		160		147	
7.0	470		190		150	
8.0	585		205		156	
9.0	620		229		160	
Factor do Anel			0.012 KN			
CBR	2.5mm=13.344KN		5.0mm=20.016KN		7.5mm=25.354KN	
2,5mm *	8.6		7.2		6.6	
5,0mm **	13.0		8.5		8.2	
% Compactação						
CBR 2,5mm						
CBR 5,0mm						

CONSULTOR / EMPREITEIRO : TÉC. ENGS. CONSULTORES	DATA: <u>15-01-07</u>	ENSAIO DE COMPACTAÇÃO	
PROJECTO : N13 NAMPULA/CUAMBA		Barid. Seca Máx. <u>2.287</u>	gr/cm ³
CAMADA ENSAIADA : C.E DO Km: 124+20 L/D	VERIF. POR: _____	Teor Ópt. Água <u>7.6</u>	%
AMOSTRA Nº : NAM/0017/07	EFFECT. POR : RAFAEL		
DESCRIÇÃO DO SOLO : AREIA ARGILOSA CASTANHO CLARA COM CASCALHO			

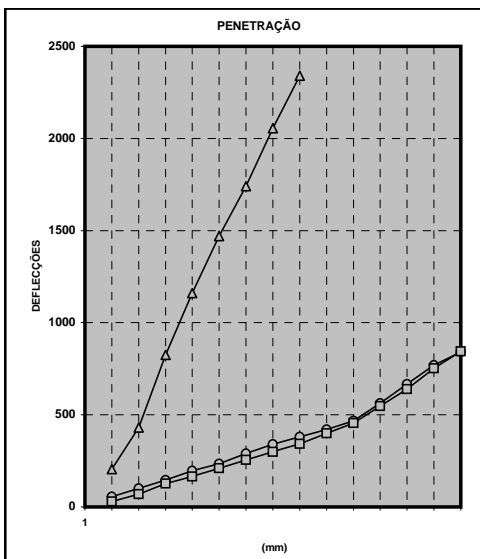
DETERMINAÇÃO DO TEOR DE HUMIDADE

Nº da Cápsula/Molde	Nº	Hygroscópico	Depois da Mistura			Ótimo % (a)	7.6
			14	18	12		
Peso do Solo Húmido + Cápsula (P0)	gr		392.10	377.90	412.12	Hygroscópico % (b)	0.7
Peso do Solo Seco + Cápsula (P1)	gr		372.60	357.20	388.40	Volume de Água =	
Peso da Cápsula (P2)	gr		84.80	84.20	84.40	Solo seco x(100+a) - Solo seco =	
Peso da Água (P3)=(P0)-(P1)	gr		19.50	20.70	23.72	100	
Peso do Solo Seco (P4)=(P1)-(P2)	gr		287.80	273.00	304.00	=	1233
Teor de Humidade W=(P3)/(P4)x100	%	0.7	6.8	7.6	7.8		

Água a Adicionar

Moldagem				
Compacção	Nº	55x10x5	25x10x5	55x5,5x3
Nº do Molde	Nº	1	2	3
Peso do Molde + Solo Húmido (M0)	gr	11374	11108	10856
Peso do Molde (M1)	gr	6196	6189	6196
Peso do Solo Húmido (M2)=(M0)-(M1)	gr	5178	4919	4660
Volume do Molde V	cc	2104	2104	2104
Baridade Húmida do Solo Dh=(M2)/V	gr/cc	2.461	2.338	2.215
Teor Humidade da Moldagem W	%	7.4	7.4	7.4
Baridade Seca do Solo Ds=Dh x 100 / (100+W)	gr/cc	2.291	2.177	2.062
% de Compacção Ds/Ds Máx. x 100	%	100.2	95.2	90.2
Peso Molde+Solo Após Embebição (M3)	gr	11418	11208	10985
Peso Solo Húmido Final (M4)=(M3)-(M1)	gr	5222	5015	4789
Teor Humidade Final Wf=[M4-(Ds x V)]/(Ds x V)	%	8.3	9.5	10.4

Expansão			
Nº do Molde	1	2	3
Leitura Inicial			
Leitura Final			
% Expansão			



Nº Molde	1 Δ		2 \circ		3 \square	
	% Mod AASHTO		95.0		90	
Penetração	Leitura	Corrig.	Leitura	Corrig.	Leitura	Corrig.
0.0						
0.5	205		55		30	
1.0	430		100		70	
1.5	825		146		127	
2.0	1160		196		165	
2.5*	1470	1470	235	235	210	210
3.0	1740		291		255	
3.5	2055		341		298	
4.0	2340		380		343	
4.5			420		400	
5.0*		1380	468	468	455	455
6.0			563		548	
7.0			667		639	
8.0			769		753	
9.0			840		845	
Factor do Anel			0.012 KN			
CBR	2.5mm=13.344KN		5.0mm=20.016KN		7.5mm=25.354KN	
2,5mm *	132.2		21.1		18.9	
5,0mm **	11.0		28.1		27.3	
% Compacção						
CBR 2,5mm						
CBR 5,0mm						

APÊNDICE-C

Breakdown of the Project Cost by Pavement Type

Summary of the Project Cost

Appendix-C

ALT1-1 Design speed = 80kph (Currency: USD)

No.	Description	Section			Total	% of (1-10)	
		Nampula - Ribau	Ribau - Malema	Malema - Cuamba			
		131.85 km	102.87 km	112.91 km	347.63 km		
0	Compensation	443,675	346,158	379,942	1,169,775		
1	Preliminary & general	18,992,194	16,918,330	20,497,596	56,408,120	28.7%	
2	Earthworks	5,742,767	4,589,201	3,032,348	13,364,316	6.8%	
3	Pavement	33,541,268	26,941,386	33,951,207	94,433,862	48.0%	
4	Drainage	4,571,881	5,557,400	6,949,201	17,078,482	8.7%	
5	Road furniture	175,198	176,688	292,253	644,139	0.3%	
6	Miscellaneous	12,000	22,450	113,962	148,412	0.1%	
7	Bridge	0	2,043,004	3,498,934	5,541,938	2.8%	
8	Temporary construction road	1,262,692	1,028,483	1,059,032	3,350,207	1.7%	
9	Dayworks	1,114,523	992,822	1,202,865	3,310,209	1.7%	
10	Social issues	810,974	722,419	875,255	2,408,648	1.2%	
Total (1-10)		66,223,498	58,992,183	71,472,652	196,688,334	100.0%	
11	Contingency	10%	6,622,350	5,899,218	7,147,265	19,668,833	
Total construction cost (1-11)			72,845,848	64,891,402	78,619,917	216,357,167	
12	Engineering cost	8%	5,827,668	5,191,312	6,289,593	17,308,573	
Total project cost (1-12)			78,673,516	70,082,714	84,909,510	233,665,740	
13	VAT	17%	13,374,498	11,914,061	14,434,617	39,723,176	
Total project cost with VAT			92,048,014	81,996,775	99,344,127	273,388,916	
14	Total (13) + (0) Compensation		92,491,689	82,342,933	99,724,069	274,558,691	

	Section 1	Section 2	Section 3	Total
Unit construction cost (1-11)	\$552,490 /km	\$630,810 /km	\$696,306 /km	\$622,378 /km
Unit project cost (1-12)	\$596,690 /km	\$681,275 /km	\$752,011 /km	\$672,168 /km
Unit project cost with VAT	\$698,127 /km	\$797,091 /km	\$879,852 /km	\$786,436 /km
Unit project cost +VAT + Compen.	\$701,492 /km	\$800,456 /km	\$883,217 /km	\$789,801 /km
Unit construction cost (0-10)	\$505,629 /km	\$576,828 /km	\$636,371 /km	\$569,163 /km

SC: A100 => =>
 BC: G200 G150 =>
 SBC: G175 G150 =>

ALT1-2 Design speed = 80kph (Currency: USD)

No.	Description	Section			Total	% of (1-10)	
		Nampula - Ribau	Ribau - Malema	Malema - Cuamba			
		131.85 km	102.87 km	112.91 km	347.63 km		
0	Compensation	443,675	346,158	379,942	1,169,775		
1	Preliminary & general	17,774,842	13,211,335	15,710,452	46,696,629	28.7%	
2	Earthworks	5,044,228	4,423,641	2,876,294	12,344,162	7.6%	
3	Pavement	31,335,818	18,263,922	22,687,549	72,287,289	44.4%	
4	Drainage	4,571,881	5,557,400	6,949,201	17,078,482	10.5%	
5	Road furniture	175,198	176,688	292,253	644,139	0.4%	
6	Miscellaneous	12,000	22,450	113,962	148,412	0.1%	
7	Bridge	0	2,043,004	3,498,934	5,541,938	3.4%	
8	Temporary construction road	1,262,692	1,028,483	1,059,032	3,350,207	2.1%	
9	Dayworks	1,043,085	775,283	921,940	2,740,308	1.7%	
10	Social issues	758,993	564,129	670,842	1,993,964	1.2%	
Total (1-10)		61,978,736	46,066,335	54,780,459	162,825,531	100.0%	
11	Contingency	10%	6,197,874	4,606,634	5,478,046	16,282,553	
Total construction cost (1-11)			68,176,610	50,672,969	60,258,505	179,108,084	
12	Engineering cost	8%	5,454,129	4,053,838	4,820,680	14,328,647	
Total project cost (1-12)			73,630,739	54,726,806	65,079,185	193,436,730	
13	VAT	17%	12,517,226	9,303,557	11,063,461	32,884,244	
Total project cost with VAT			86,147,964	64,030,364	76,142,647	226,320,974	
14	Total (13) + (0) Compensation		86,591,639	64,376,521	76,522,589	227,490,749	

	Section 1	Section 2	Section 3	Total
Unit construction cost (1-11)	\$517,077 /km	\$492,592 /km	\$533,686 /km	\$515,226 /km
Unit project cost (1-12)	\$558,443 /km	\$532,000 /km	\$576,381 /km	\$556,444 /km
Unit project cost with VAT	\$653,379 /km	\$622,440 /km	\$674,366 /km	\$651,040 /km
Unit project cost +VAT + Compen.	\$656,744 /km	\$625,805 /km	\$677,731 /km	\$654,405 /km
Unit construction cost (0-10)	\$473,435 /km	\$451,176 /km	\$488,534 /km	\$471,752 /km

SC: A50 => =>
 BC: C150 => =>
 SBC: C200 => =>

Summary of the Project Cost

Appendix-C

ALT1-3 Design speed = 80kph (Currency: USD)

No.	Description		Section 1	Section 2	Section 3	Total	% of (1-10)
			Nampula - Ribaua 131.85 km	Ribaua - Malema 102.87 km	Malema - Cuamba 112.91 km		
0	Compensation		443,675	346,158	379,942	1,169,775	
1	Preliminary & general		12,059,163	10,274,299	12,482,967	34,816,429	28.7%
2	Earthworks		5,263,486	4,406,071	3,445,411	13,114,968	10.8%
3	Pavement		17,481,829	11,275,205	14,419,277	43,176,311	35.6%
4	Drainage		4,571,881	5,557,400	6,949,201	17,078,482	14.1%
5	Road furniture		175,198	176,688	292,253	644,139	0.5%
6	Miscellaneous		12,000	22,450	113,962	148,412	0.1%
7	Bridge		0	2,043,004	3,498,934	5,541,938	4.6%
8	Temporary construction road		1,262,692	1,028,483	1,059,032	3,350,207	2.8%
9	Dayworks		707,670	602,929	732,541	2,043,140	1.7%
10	Social issues		514,931	438,716	533,027	1,486,675	1.2%
Total (1-10)			42,048,851	35,825,246	43,526,604	121,400,701	100.0%
11	Contingency	10%	4,204,885	3,582,525	4,352,660	12,140,070	
Total construction cost (1-11)			46,253,736	39,407,770	47,879,265	133,540,771	
12	Engineering cost	8%	3,700,299	3,152,622	3,830,341	10,683,262	
Total project cost (1-12)			49,954,035	42,560,392	51,709,606	144,224,032	
13	VAT	17%	8,492,186	7,235,267	8,790,633	24,518,085	
Total project cost with VAT			58,446,220	49,795,659	60,500,239	168,742,118	
14	Total (13) + (0) Compensation		58,889,896	50,141,816	60,880,181	169,911,893	
			Section 1	Section 2	Section 3	Total	
Unit construction cost (1-11)			\$350,806 /km	\$383,083 /km	\$424,048 /km	\$384,146 /km	SC: DS => =>
Unit project cost (1-12)			\$378,870 /km	\$413,730 /km	\$457,972 /km	\$414,878 /km	BC: G250 G200 =>
Unit project cost with VAT			\$443,278 /km	\$484,064 /km	\$535,827 /km	\$485,407 /km	SBC: G400 G375 =>
Unit project cost +VAT + Compen.			\$446,643 /km	\$487,429 /km	\$539,192 /km	\$488,772 /km	
Unit construction cost (0-10)			\$322,279 /km	\$351,622 /km	\$388,863 /km	\$352,589 /km	

ALT1-4 Design speed = 80kph (Currency: USD)

No.	Description		Section 1	Section 2	Section 3	Total	% of (1-10)
			Nampula - Ribaua 131.85 km	Ribaua - Malema 102.87 km	Malema - Cuamba 112.91 km		
0	Compensation		443,675	346,158	379,942	1,169,775	
1	Preliminary & general		16,868,459	11,636,635	13,926,648	42,431,742	28.7%
2	Earthworks		5,263,486	4,128,457	2,782,879	12,174,823	8.2%
3	Pavement		28,954,386	14,802,667	18,525,704	62,282,757	42.1%
4	Drainage		4,571,881	5,557,400	6,949,201	17,078,482	11.5%
5	Road furniture		175,198	176,688	292,253	644,139	0.4%
6	Miscellaneous		12,000	22,450	113,962	148,412	0.1%
7	Bridge		0	2,043,004	3,498,934	5,541,938	3.7%
8	Temporary construction road		1,262,692	1,028,483	1,059,032	3,350,207	2.3%
9	Dayworks		989,895	682,875	817,260	2,490,031	1.7%
10	Social issues		720,290	496,889	594,673	1,811,852	1.2%
Total (1-10)			58,818,287	40,575,549	48,560,546	147,954,382	100.0%
11	Contingency	10%	5,881,829	4,057,555	4,856,055	14,795,438	
Total construction cost (1-11)			64,700,116	44,633,104	53,416,600	162,749,820	
12	Engineering cost	8%	5,176,009	3,570,648	4,273,328	13,019,986	
Total project cost (1-12)			69,876,125	48,203,752	57,689,928	175,769,806	
13	VAT	17%	11,878,941	8,194,638	9,807,288	29,880,867	
Total project cost with VAT			81,755,067	56,398,390	67,497,216	205,650,673	
14	Total (13) + (0) Compensation		82,198,742	56,744,547	67,877,158	206,820,448	
			Section 1	Section 2	Section 3	Total	
Unit construction cost (1-11)			\$490,710 /km	\$433,879 /km	\$473,090 /km	\$468,170 /km	SC: DS => =>
Unit project cost (1-12)			\$529,967 /km	\$468,589 /km	\$510,937 /km	\$505,623 /km	BC: C150 => =>
Unit project cost with VAT			\$620,061 /km	\$548,249 /km	\$597,797 /km	\$591,579 /km	SBC: C300 => =>
Unit project cost +VAT + Compen.			\$623,426 /km	\$551,614 /km	\$601,162 /km	\$594,944 /km	
Unit construction cost (0-10)			\$449,465 /km	\$397,800 /km	\$433,447 /km	\$428,974 /km	

Summary of the Project Cost

Appendix-C

ALT1-5 Design speed = 80kph (Currency: USD)

No.	Description		Section 1	Section 2	Section 3	Total	% of (1-10)
			Nampula - Ribaua 131.85 km	Ribaua - Malema 102.87 km	Malema - Cuamba 112.91 km		
0	Compensation		443,675	346,158	379,942	1,169,775	
1	Preliminary & general		20,433,391	17,530,159	21,204,095	59,167,645	28.7%
2	Earthworks		5,993,063	4,589,201	3,018,632	13,600,896	6.6%
3	Pavement		36,728,942	28,400,902	35,650,275	100,780,118	48.8%
4	Drainage		4,571,881	5,557,400	6,949,201	17,078,482	8.3%
5	Road furniture		175,198	176,688	292,253	644,139	0.3%
6	Miscellaneous		12,000	22,450	113,962	148,412	0.1%
7	Bridge		0	2,043,004	3,498,934	5,541,938	2.7%
8	Temporary construction road		1,262,692	1,028,483	1,059,032	3,350,207	1.6%
9	Dayworks		1,199,097	1,028,726	1,244,324	3,472,147	1.7%
10	Social issues		872,514	748,544	905,423	2,526,481	1.2%
Total (1-10)			71,248,778	61,125,558	73,936,130	206,310,465	100.0%
11	Contingency	10%	7,124,878	6,112,556	7,393,613	20,631,047	
Total construction cost (1-11)			78,373,656	67,238,113	81,329,743	226,941,512	
12	Engineering cost	8%	6,269,892	5,379,049	6,506,379	18,155,321	
Total project cost (1-12)			84,643,548	72,617,162	87,836,122	245,096,833	
13	VAT	17%	14,389,403	12,344,918	14,932,141	41,666,462	
Total project cost with VAT			99,032,951	84,962,080	102,768,263	286,763,294	
14	Total (13) + (0) Compensation		99,476,626	85,308,238	103,148,205	287,933,069	
			Section 1	Section 2	Section 3	Total	
Unit construction cost (1-11)			\$594,415 /km	\$653,622 /km	\$720,306 /km	\$652,825 /km	SC: A100 => =>
Unit project cost (1-12)			\$641,969 /km	\$705,912 /km	\$777,930 /km	\$705,051 /km	BC: G150 => =>
Unit project cost with VAT			\$751,103 /km	\$825,917 /km	\$910,179 /km	\$824,910 /km	SBC: C175 C150 =>
Unit project cost +VAT + Compen.			\$754,468 /km	\$829,282 /km	\$913,544 /km	\$828,275 /km	
Unit construction cost (0-10)			\$543,743 /km	\$597,567 /km	\$658,189 /km	\$596,842 /km	

ALT1-6 Design speed = 80kph (Currency: USD)

No.	Description		Section 1	Section 2	Section 3	Total	% of (1-10)
			Nampula - Ribaua 131.85 km	Ribaua - Malema 102.87 km	Malema - Cuamba 112.91 km		
0	Compensation		443,675	346,158	379,942	1,169,775	
1	Preliminary & general		15,060,428	11,578,742	14,002,471	40,641,641	28.7%
2	Earthworks		4,957,126	4,285,016	3,306,783	12,548,925	8.9%
3	Pavement		24,947,695	14,508,004	18,182,678	57,638,377	40.7%
4	Drainage		4,571,881	5,557,400	6,949,201	17,078,482	12.1%
5	Road furniture		175,198	176,688	292,253	644,139	0.5%
6	Miscellaneous		12,000	22,450	113,962	148,412	0.1%
7	Bridge		0	2,043,004	3,498,934	5,541,938	3.9%
8	Temporary construction road		1,262,692	1,028,483	1,059,032	3,350,207	2.4%
9	Dayworks		883,794	679,478	821,710	2,384,982	1.7%
10	Social issues		643,086	494,417	597,911	1,735,414	1.2%
Total (1-10)			52,513,900	40,373,682	48,824,934	141,712,517	100.0%
11	Contingency	10%	5,251,390	4,037,368	4,882,493	14,171,252	
Total construction cost (1-11)			57,765,290	44,411,050	53,707,428	155,883,768	
12	Engineering cost	8%	4,621,223	3,552,884	4,296,594	12,470,701	
Total project cost (1-12)			62,386,514	47,963,934	58,004,022	168,354,470	
13	VAT	17%	10,605,707	8,153,869	9,860,684	28,620,260	
Total project cost with VAT			72,992,221	56,117,803	67,864,706	196,974,729	
14	Total (13) + (0) Compensation		73,435,896	56,463,960	68,244,648	198,144,504	
			Section 1	Section 2	Section 3	Total	
Unit construction cost (1-11)			\$438,114 /km	\$431,720 /km	\$475,666 /km	\$448,419 /km	SC: DS => =>
Unit project cost (1-12)			\$473,163 /km	\$466,258 /km	\$513,719 /km	\$484,292 /km	BC: G250 G225 =>
Unit project cost with VAT			\$553,600 /km	\$545,522 /km	\$601,051 /km	\$566,622 /km	SBC: C325 => =>
Unit project cost +VAT + Compen.			\$556,965 /km	\$548,887 /km	\$604,416 /km	\$569,987 /km	
Unit construction cost (0-10)			\$401,650 /km	\$395,838 /km	\$435,788 /km	\$411,018 /km	

Summary of the Project Cost

Appendix-C

ALT2-1

Design speed = 100kph

(Currency: USD)

No.	Description		Section 1	Section 2	Section 3	Total	% of (1-10)
			Nampula - Ribaué	Ribaué - Malema	Malema - Cuamba	347.51 km	
0	Compensation		444,281	345,989	379,101	1,169,371	
1	Preliminary & general		19,757,362	18,038,731	20,920,764	58,716,857	28.7%
2	Earthworks		6,312,582	7,126,371	4,213,892	17,652,846	8.6%
3	Pavement		34,748,616	26,923,115	33,877,656	95,549,387	46.7%
4	Drainage		4,618,866	5,711,323	6,848,555	17,178,744	8.4%
5	Road furniture		176,356	176,578	294,372	647,305	0.3%
6	Miscellaneous		12,000	22,450	113,962	148,412	0.1%
7	Bridge		0	2,043,004	3,498,934	5,541,938	2.7%
8	Temporary construction road		1,262,692	1,028,483	1,059,032	3,350,207	1.6%
9	Dayworks		1,159,425	1,058,571	1,227,697	3,445,693	1.7%
10	Social issues		843,647	770,261	893,325	2,507,232	1.2%
Total (1-10)			68,891,546	62,898,888	72,948,187	204,738,621	100.0%
11	Contingency	10%	6,889,155	6,289,889	7,294,819	20,473,862	
Total construction cost (1-11)			75,780,701	69,188,776	80,243,006	225,212,483	
12	Engineering cost	8%	6,062,456	5,535,102	6,419,440	18,016,999	
Total project cost (1-12)			81,843,157	74,723,878	86,662,447	243,229,482	
13	VAT	17%	13,913,337	12,703,059	14,732,616	41,349,012	
Total project cost with VAT			95,756,494	87,426,938	101,395,062	284,578,494	
14	Total (13) + (0) Compensation		96,200,775	87,772,927	101,774,163	285,747,865	

	Section 1	Section 2	Section 3	Total	
Unit construction cost (1-11)	\$573,966 /km	\$672,912 /km	\$712,258 /km	\$648,075 /km	SC: A100 => =>
Unit project cost (1-12)	\$619,883 /km	\$726,745 /km	\$769,239 /km	\$699,921 /km	BC: G250 G150 =>
Unit project cost with VAT	\$725,263 /km	\$850,291 /km	\$900,009 /km	\$818,907 /km	SBC: G175 G150 =>
Unit project cost +VAT + Compen.	\$728,628 /km	\$853,656 /km	\$903,374 /km	\$822,272 /km	
Unit construction cost (0-10)	\$525,152 /km	\$615,103 /km	\$650,872 /km	\$592,524 /km	

Breakdown of the Project Cost (BQ_ALTI1-1_80kph)

Item	Description	Unit	Section 1 (Namputa - Ribau)			Section 2 (Ribau - Malema)			Section 3 (Malema - Cuumba)			Section 4 (Cuumba - Amont 3)			Total (Section 1-3) 347.63 (km)		
			Qty	Material coefficient	Transport coefficient	Unit Price 1	Amount 1	Qty	Material coefficient	Transport coefficient	Unit Price 2	Amount 2	Qty	Material coefficient		Transport coefficient	Unit Price 3
0	Compensation	km	131.85			3,365.00	443,675.25				3,365.00	346,157.55				3,365.00	379,942.15
1	Preliminary & general works	L.S.	1			18,992,194.41	18,992,194.41	1			16,918,330.22	16,918,330.22	1			20,497,595.68	20,497,595.68
2	Earthworks																
	2.1	Excavation	619,354			3.12	1,934,242.54	523,493			3.12	1,654,868.64	338,605			3.12	1,057,463.42
	2.2	Fill (use excavated soil)	619,354			3.16	1,954,190.28	523,493			3.16	1,651,278.95	338,605			3.16	1,068,368.98
	2.3	Fill (borrow material)	380,845			4.87	1,854,334.31	267,530			4.87	1,302,603.57	186,181			4.87	906,515.29
	2.4	Disposal of surplus soil				3.50	0.00				3.50	0.00				3.50	0.00
						Total S1-2	5,742,767.13				Total S2-2	4,589,201.16				Total S3-2	3,032,347.68
3	Pavement																
	3.1.1	Replacement of subgrade (t = 150mm)	sqm			2.47	0.00				2.47	0.00				2.47	0.00
	3.1.2	Replacement of subgrade (t = 200mm)	sqm			2.87	0.00				2.87	0.00				2.87	0.00
	3.2	Granular sub base course (t = 150mm) (Soaked CBR > 30%)	sqm	80.9%	69.7%	0.89	0.00	1,031,870	80.9%	23.7%	0.45	466,808.09	1,145,540	80.9%	26.0%	0.47	543,425.71
	3.3	Granular sub base course (t = 175mm) (Soaked CBR > 30%)	sqm	80.9%	69.7%	1.04	1,388,237.41		80.9%	23.7%	0.53	0.00		80.9%	26.0%	0.55	0.00
	3.4	Granular sub base course (t = 225mm) (Soaked CBR > 30%)	sqm	80.9%	69.7%	1.34	0.00		80.9%	23.7%	0.68	0.00		80.9%	26.0%	0.71	0.00
	3.5.1	Granular sub base course (t = 325mm) (Soaked CBR > 30%)	sqm	80.9%	69.7%	1.93	0.00		80.9%	23.7%	0.98	0.00		80.9%	26.0%	1.03	0.00
	3.5.2	Granular sub base course (t = 375mm) (Soaked CBR > 30%)	sqm	80.9%	69.7%	2.23	0.00		80.9%	23.7%	1.13	0.00		80.9%	26.0%	1.19	0.00
	3.5.3	Granular sub base course (t = 400mm) (Soaked CBR > 30%)	sqm	80.9%	69.7%	2.38	0.00		80.9%	23.7%	1.21	0.00		80.9%	26.0%	1.27	0.00
	3.6	Cemented sub base course (t = 150mm) (7day UCS 0.75 - 1.5MPa)	sqm	80.9%	69.7%	3.68	0.00		80.9%	23.7%	1.87	0.00		80.9%	26.0%	1.96	0.00
	3.7	Cemented sub base course (t = 175mm) (7day UCS 0.75 - 1.5MPa)	sqm	80.9%	69.7%	4.30	0.00		80.9%	23.7%	2.18	0.00		80.9%	26.0%	2.28	0.00
	3.8	Cemented sub base course (t = 200mm) (7day UCS 0.75 - 1.5MPa)	sqm	80.9%	69.7%	4.91	0.00		80.9%	23.7%	2.49	0.00		80.9%	26.0%	2.61	0.00
	3.9.1	Cemented sub base course (t = 225mm) (7day UCS 0.75 - 1.5MPa)	sqm	80.9%	69.7%	5.52	0.00		80.9%	23.7%	2.80	0.00		80.9%	26.0%	2.94	0.00
	3.9.2	Cemented sub base course (t = 250mm) (7day UCS 0.75 - 1.5MPa)	sqm	80.9%	69.7%	6.14	0.00		80.9%	23.7%	3.11	0.00		80.9%	26.0%	3.26	0.00
	3.9.3	Cemented sub base course (t = 300mm) (7day UCS 0.75 - 1.5MPa)	sqm	80.9%	69.7%	7.36	0.00		80.9%	23.7%	3.73	0.00		80.9%	26.0%	3.92	0.00
	3.9.4	Cemented sub base course (t = 325mm) (7day UCS 0.75 - 1.5MPa)	sqm	80.9%	69.7%	7.98	0.00		80.9%	23.7%	4.04	0.00		80.9%	26.0%	4.24	0.00
	3.10	Granular base course (t = 150mm) (Soaked CBR > 80%)	sqm	80.9%	69.7%	2.59	0.00	1,031,870	80.9%	23.7%	1.31	1,356,705.44	1,145,540	80.9%	26.0%	1.38	1,579,382.69
	3.11.1	Granular base course (t = 200mm) (Soaked CBR > 80%)	sqm	80.9%	69.7%	3.46	4,611,082.42		80.9%	23.7%	1.75	0.00		80.9%	26.0%	1.84	0.00
	3.11.2	Granular base course (t = 225mm) (Soaked CBR > 80%)	sqm	80.9%	69.7%	3.89	0.00		80.9%	23.7%	1.97	0.00		80.9%	26.0%	2.07	0.00
	3.11.3	Granular base course (t = 250mm) (Soaked CBR > 80%)	sqm	80.9%	69.7%	4.32	0.00		80.9%	23.7%	2.19	0.00		80.9%	26.0%	2.30	0.00
	3.12	Cemented base course (t = 150mm) (7day UCS 1.5 - 3.0MPa)	sqm	80.9%	69.7%	5.07	0.00		80.9%	23.7%	2.57	0.00		80.9%	26.0%	2.69	0.00
	3.13	Cemented base course (t = 180mm) (7day UCS 1.5 - 3.0MPa)	sqm	80.9%	69.7%	6.08	0.00		80.9%	23.7%	3.08	0.00		80.9%	26.0%	3.23	0.00
	3.14	Cemented base course (t = 200mm) (7day UCS 1.5 - 3.0MPa)	sqm	80.9%	69.7%	6.76	0.00		80.9%	23.7%	3.43	0.00		80.9%	26.0%	3.59	0.00
	3.15	Prime coat	sqm			1.59	1,912,678.25	929,010	116.2%	2.33	2,166,575.81	1,032,660	150.1%	1.66	3.01	3,110,895.48	7,190,149.53
	3.16	Tack coat	sqm			0.88	1,054,975.00	929,010	116.2%	1.29	1,195,017.15	1,032,660	150.1%	1.66	1.71	1,715,875.08	3,965,867.23
	3.17.1	Asphalt concrete (t = 50mm)	sqm	36.4%	79.3%	10.22	0.00		36.4%	11.71	0.00		36.4%	13.07	0.00	0.00	
	3.17.2	Asphalt concrete (t = 100mm)	sqm	36.4%	79.3%	20.45	24,574,296.35	929,010	116.2%	23.42	21,756,279.39	1,032,660	150.1%	26.15	27,001,628.32	73,332,203.07	
	3.18	Double bituminous surface treatment (DBST)	sqm	36.4%	79.3%	4.64	0.00		36.4%	5.31	0.00		36.4%	5.93	0.00	0.00	
	3.19	Ota seal for sidewalk	sqm	36.4%	79.3%	2.78	0.00		36.4%	3.19	0.00		36.4%	3.56	0.00	0.00	
	3.20	Granular sub base course for sidewalk (t = 100mm; Soaked CBR > 30%)	sqm	80.9%	69.7%	0.59	0.00		80.9%	23.7%	0.30	0.00		80.9%	26.0%	0.32	0.00
						Total S1-3	33,541,268.43			Total S2-3	26,941,385.88			Total S3-3	33,951,207.29	94,433,861.59	

Breakdown of the Project Cost (BQ_ALTI1-1_80kph)

Item	Description	Unit	Section 1 (Nampula - Ribaué)			Section 2 (Ribaué - Malema)			Section 3 (Malema - Cuimba)			Section 4 (Cuimba - Vila Fontes)																											
			Qty	Material coefficient	Transport coefficient	Unit Price 1	Amount 1	Q'ty	Material coefficient	Transport coefficient	Unit Price 2	Amount 2	Q'ty	Material coefficient	Transport coefficient	Unit Price 3	Amount 3	Total (Section 1-3)																					
4	Drainage	m				3.35	0.00																																
																			4.1	Unlined side ditch																			
																			4.2	Concrete lined side ditch	98,800	49.0%	79.3%	21.36	2,110,027.74	97,400	49.0%	116.2%	25.65	2,498,691.44	91,000	49.0%	150.1%	29.60	2,693,772.67	7,302,491.85			
																			4.3	U-shaped side ditch (open)	1,170	49.0%	79.3%	38.43	44,958.42	240	49.0%	116.2%	46.16	11,077.94	1,260	49.0%	150.1%	53.26	67,109.50	123,145.86			
																			4.4	U-shaped side ditch (covered)	16,240	49.0%	79.3%	47.79	776,124.13	3,010	49.0%	116.2%	57.41	172,796.17	15,686	49.0%	150.1%	66.24	1,039,072.14	1,987,992.44			
																			4.5	Cross culvert	252	49.0%	79.3%	292.41	73,687.76	72	49.0%	116.2%	351.25	25,290.05	384	49.0%	150.1%	405.31	155,637.55	254,615.36			
																			4.6	Box culvert (1.0*1.0)	179	49.0%	79.3%	7,226.46	1,293,537.03	139	49.0%	116.2%	8,680.37	1,206,899.35	179	49.0%	150.1%	10,016.46	1,792,945.94	4,293,082.32			
4.7	Box culvert (2.0*2.0)	No	49.0%	79.3%	22,795.51	273,546.12	60	49.0%	116.2%	27,382.42	1,642,944.94	38	49.0%	150.1%	31,596.40	1,200,663.30	3,117,154.35																						
						Total S1-4	4,571,881.20			Total S2-4	5,557,399.89			Total S3-4	6,949,201.11		17,078,482.20																						
5	Road furniture	m2																																					
																			5.1	Road sign (Warning) A=0.62m2/no	54	79.3%	232.05	12,530.65	16	79.3%	116.2%	340.03	5,440.42	64	79.3%	150.1%	439.23	28,110.41	46,081.48				
																			5.2	Road sign (Regulatory) A=1.3m2/no	54	79.3%	232.05	12,530.65	14	79.3%	116.2%	340.03	4,760.37	44	79.3%	150.1%	439.23	19,325.91	36,619.92				
																			5.3	Road marking (continuous) W=100mm	283,242	79.3%	0.52	147,164.99	217,356	79.3%	116.2%	0.76	165,482.25	244,095	79.3%	150.1%	0.98	240,056.37	552,703.62				
																			5.4	Road marking (continuous) W=400mm	1,430	79.3%	2.08	2,971.96	330	79.3%	116.2%	3.05	1,004.97	1,210	79.3%	150.1%	3.93	4,759.92	8,736.85				
																								Total S1-5	175,198.24			Total S2-5	176,688.02			Total S3-5	292,252.01		644,138.87				
6	Miscellaneous work	m																																					
																			6.1	Concrete kerb	0	49.0%	16.21	0.00	0	49.0%	116.2%	19.48	0.00	4,260	49.0%	150.1%	22.47	95,742.93	95,742.93				
																			6.2	Demolishing existing concrete (plain)	754			12.49	9,413.99	826			12.49	10,312.94	1,183			12.49	14,770.22	34,497.15			
																			6.3	Demolishing existing concrete (reinforced)	114			22.69	2,586.29	535			22.69	12,137.41	28			22.69	635.23	15,358.93			
																			6.4	Removal of Bailey bridge (W=4m)				90.75	0.00			90.75	0.00	31			90.75	2,813.16	2,813.16				
																								Total S1-6	12,000.28			Total S2-6	22,450.35			Total S3-6	113,901.54		148,412.17				
7	Bridge	No																																					
																				No. 12 Monupo bridge	L=25m																		
																				No. 19 Lalaua bridge	L=30m																		
																				No. 24 Nualaleia bridge	L=48m																		
																				No. 27 Mutivasse bridge	L=30m																		
																				No. 30 Nuanuela bridge	L=30m																		
																				No. 34 Lutto bridge	L=94m																		
																				Temporary construction road	km	124.00			10,183.00	1,262,692.00	101.00			10,183.00	1,028,483.00	104.00			10,183.00	1,059,032.00	3,350,207.00		
																									Total S1-7	0.00			Total S2-7	2,043,003.87			Total S3-7	3,498,934.03		5,541,937.90			
																									Total S1 (2-8)	45,305,807.27			Total S2 (2-8)	40,388,612.17			Total S3 (2-8)	48,896,936.26		134,561,355.70			
																				9	Dayworks	L.S.			1,114,522.86	1,114,522.86			992,821.86	992,821.86			1,202,864.63	1,202,864.63		3,310,209.35			
																				10	Soil issues	L.S.			810,973.95	810,973.95			722,419.16	722,419.16			875,255.16	875,255.16		2,408,648.27			
					Total S1 (1-10)	66,223,498.49			Total S2 (1-10)	58,992,183.40			Total S3 (1-10)	71,472,651.74		196,688,333.63																							
11	Contingency cost	L.S.			6,622,349.85	6,622,349.85			5,899,218.34	5,899,218.34			7,147,265.17	7,147,265.17		19,668,883.36																							
					72,845,848.34	72,845,848.34			64,891,401.74	64,891,401.74			78,619,916.91	78,619,916.91		216,357,166.99																							
					12,383,794.22	12,383,794.22			11,031,538.30	11,031,538.30			13,365,385.87	13,365,385.87		36,780,718.39																							
					Total Construction S1 (1-11)	85,229,642.56			Total Construction S2 (1-11)	75,922,940.04			Total Construction S3 (1-11)	91,985,302.78		253,137,885.38																							
12	Engineering cost	L.S.			5,827,667.87	5,827,667.87			5,191,312.14	5,191,312.14			6,289,593.35	6,289,593.35		17,308,573.36																							
					990,703.54	990,703.54			882,523.06	882,523.06			1,069,230.87	1,069,230.87		2,942,457.47																							
					Total engineering S1	6,818,371.40			Total engineering S2	6,073,835.20			Total engineering S3	7,358,824.22		20,251,030.83																							
					78,673,516.21	78,673,516.21			70,082,713.88	70,082,713.88			84,909,510.26	84,909,510.26		233,665,740.35																							
					13,374,497.76	13,374,497.76			11,914,061.36	11,914,061.36			14,434,616.74	14,434,616.74		39,723,175.86																							
					Total project S1 (1-12)	92,048,013.96			Total project S2 (1-12)	81,996,775.24			Total project S3 (1-12)	99,344,127.01		273,388,916.21																							

Breakdown of the Project Cost (BQ_ALTI-2_80kph)

Item	Description	Unit	Section 1 (Nampula - Ribaué)			Section 2 (Ribaué - Malema)			Section 3 (Malema - Cuamba)			Section 4 (Cuamba - Vila Fontes)						
			Qty	Material coefficient	Transport coefficient	Unit Price 1	Amount 1	Qty	Material coefficient	Transport coefficient	Unit Price 2	Amount 2	Qty	Material coefficient	Transport coefficient	Unit Price 3	Amount 3	Total (Section 1-3)
0	Compensation	km	131.85			3,365.00	443,675.25	102.87			3,365.00	346,157.55	112.91			3,365.00	379,942.15	1,109,774.95
1	Preliminary & general works	L.S.	1			17,774,841.78	17,774,841.78	1			13,211,334.61	13,211,334.61	1			15,710,452.47	15,710,452.47	46,696,628.86
2	Earthworks																	
2.1	Excavation	cum	748,107			3.12	2,336,338.16	551,124			3.12	1,721,160.25	371,024			3.12	1,160,581.75	5,218,080.17
2.2	Fill (use excavated soil)	cum	748,107			3.16	2,360,432.69	551,124			3.16	1,738,910.49	371,024			3.16	1,172,550.77	5,271,893.95
2.3	Fill (borrow material)	cum	71,361			4.87	347,456.71	197,899			4.87	963,570.23	111,555			4.87	543,161.30	1,854,188.24
2.4	Disposal of surplus soil	cum				3.50	0.00				3.50	0.00				3.50	0.00	0.00
						Total SI-2	5,044,227.56				Total S2-2	4,423,640.97				Total S3-2	2,876,293.82	12,344,162.35
3	Pavement																	
3.1.1	Replacement of subgrade (t = 150mm)	sq.m				2.47	0.00				2.47	0.00				2.47	0.00	0.00
3.1.2	Replacement of subgrade (t = 200mm)	sq.m	1,333,600			2.87	3,831,966.24				2.87	0.00				2.87	0.00	3,831,966.24
3.2	Granular sub base course (t = 150mm) (Soaked CBR > 30%)	sq.m		80.9%	69.7%	0.89	0.00				0.89	0.00				0.89	0.00	0.00
3.3	Granular sub base course (t = 175mm) (Soaked CBR > 30%)	sq.m		80.9%	69.7%	1.04	0.00				1.04	0.00				1.04	0.00	0.00
3.4	Granular sub base course (t = 225mm) (Soaked CBR > 30%)	sq.m		80.9%	69.7%	1.34	0.00				1.34	0.00				1.34	0.00	0.00
3.5.1	Granular sub base course (t = 325mm) (Soaked CBR > 30%)	sq.m		80.9%	69.7%	1.93	0.00				1.93	0.00				1.93	0.00	0.00
3.5.2	Granular sub base course (t = 375mm) (Soaked CBR > 30%)	sq.m		80.9%	69.7%	2.23	0.00				2.23	0.00				2.23	0.00	0.00
3.5.3	Granular sub base course (t = 400mm) (Soaked CBR > 30%)	sq.m		80.9%	69.7%	2.38	0.00				2.38	0.00				2.38	0.00	0.00
3.6	Cemented sub base course (t = 150mm) (7day UCS 0.75 - 1.5MPa)	sq.m		80.9%	69.7%	3.68	0.00				3.68	0.00				3.68	0.00	0.00
3.7	Cemented sub base course (t = 175mm) (7day UCS 0.75 - 1.5MPa)	sq.m		80.9%	69.7%	4.30	0.00				4.30	0.00				4.30	0.00	0.00
3.8	Cemented sub base course (t = 200mm) (7day UCS 0.75 - 1.5MPa)	sq.m	1,333,600			4.91	6,547,064.56	1,031,870			4.91	2,568,431.81	1,145,540			4.91	2,989,990.78	12,105,487.15
3.9.1	Cemented sub base course (t = 225mm) (7day UCS 0.75 - 1.5MPa)	sq.m		80.9%	69.7%	5.52	0.00				5.52	0.00				5.52	0.00	0.00
3.9.2	Cemented sub base course (t = 250mm) (7day UCS 0.75 - 1.5MPa)	sq.m		80.9%	69.7%	6.14	0.00				6.14	0.00				6.14	0.00	0.00
3.9.3	Cemented sub base course (t = 300mm) (7day UCS 0.75 - 1.5MPa)	sq.m		80.9%	69.7%	7.36	0.00				7.36	0.00				7.36	0.00	0.00
3.9.4	Cemented sub base course (t = 325mm) (7day UCS 0.75 - 1.5MPa)	sq.m		80.9%	69.7%	7.98	0.00				7.98	0.00				7.98	0.00	0.00
3.10	Granular base course (t = 150mm) (Soaked CBR > 80%)	sq.m		80.9%	69.7%	2.59	0.00				2.59	0.00				2.59	0.00	0.00
3.11.1	Granular base course (t = 200mm) (Soaked CBR > 80%)	sq.m		80.9%	69.7%	3.46	0.00				3.46	0.00				3.46	0.00	0.00
3.11.2	Granular base course (t = 225mm) (Soaked CBR > 80%)	sq.m		80.9%	69.7%	3.89	0.00				3.89	0.00				3.89	0.00	0.00
3.11.3	Granular base course (t = 250mm) (Soaked CBR > 80%)	sq.m		80.9%	69.7%	4.32	0.00				4.32	0.00				4.32	0.00	0.00
3.12	Cemented base course (t = 150mm) (7day UCS 1.5 - 3.0MPa)	sq.m	1,333,600			5.07	6,756,961.22	1,031,870			5.07	2,650,774.86	1,145,540			5.07	3,085,848.87	12,493,584.96
3.13	Cemented base course (t = 180mm) (7day UCS 1.5 - 3.0MPa)	sq.m		80.9%	69.7%	6.08	0.00				6.08	0.00				6.08	0.00	0.00
3.14	Cemented base course (t = 200mm) (7day UCS 1.5 - 3.0MPa)	sq.m		80.9%	69.7%	6.76	0.00				6.76	0.00				6.76	0.00	0.00
3.15	Prime coat	sq.m	1,201,770			1.59	1,912,678.25	929,010			1.59	2,166,575.81	1,032,660			1.59	3,110,895.48	7,190,149.53
3.16	Tack coat	sq.m				0.88	0.00				0.88	0.00				0.88	0.00	0.00
3.17.1	Asphalt concrete (t = 50mm)	sq.m	1,201,770			10.22	12,287,147.68	929,010			10.22	10,878,139.70	1,032,660			10.22	13,500,814.16	36,666,101.53
3.17.2	Asphalt concrete (t = 100mm)	sq.m		36.4%	79.3%	20.45	0.00				20.45	0.00				20.45	0.00	0.00
3.18	Double bituminous surface treatment (DBST)	sq.m		36.4%	79.3%	4.64	0.00				4.64	0.00				4.64	0.00	0.00
3.19	Ona seal for sidewalk	sq.m		36.4%	79.3%	2.78	0.00				2.78	0.00				2.78	0.00	0.00
3.20	Granular sub base course for sidewalk (t = 100mm, Soaked CBR > 30%)	sq.m		80.9%	69.7%	0.59	0.00				0.59	0.00				0.59	0.00	0.00
						Total SI-3	31,335,817.94				Total S2-3	18,263,922.18				Total S3-3	22,687,549.29	71,287,289.41

Breakdown of the Project Cost (BQ_ALTI-2_80kph)

Item	Description	Unit	Section 1 (Nampula - Ribaué)			Section 2 (Ribaué - Makema)			Section 3 (Makema - Cuamba)			Section 4 (Cuamba - Vila Fontaine)			Total (Section 1-3) (km)				
			Qty	Material coefficient	Transport coefficient	Unit Price 1	Amount 1	Q'ty	Material coefficient	Transport coefficient	Unit Price 2	Amount 2	Q'ty	Material coefficient		Transport coefficient	Unit Price 3	Amount 3	
4	Drainage	m	4.1			3.35	0.00			3.35	0.00				3.35	0.00	0.00		
			4.2	98,800	49.0%	79.3%	21.36	2,110,027.74	97,400	49.0%	116.2%	25.65	2,498,691.44	91,000	49.0%	150.1%	29.60	2,693,772.67	
			4.3	1,170	49.0%	79.3%	38.43	44,958.42	240	49.0%	116.2%	46.16	11,077.94	1,260	49.0%	150.1%	53.26	67,109.50	
			4.4	16,240	49.0%	79.3%	47.79	776,124.13	3,010	49.0%	116.2%	57.41	172,796.17	15,686	49.0%	150.1%	66.24	1,039,072.14	
			4.5	232	49.0%	79.3%	292.41	73,687.76	72	49.0%	116.2%	351.25	25,590.05	384	49.0%	150.1%	405.31	155,637.55	
			4.6	179	49.0%	79.3%	7,226.46	1,293,537.03	139	49.0%	116.2%	8,680.57	1,206,599.35	179	49.0%	150.1%	10,016.46	1,792,945.94	
			4.7	12	49.0%	79.3%	22,795.51	273,546.12	60	49.0%	116.2%	27,382.42	1,642,944.94	38	49.0%	150.1%	31,596.40	1,200,663.30	
					Total SI-4	4,571,881.20			Total S2-4	5,557,399.89				Total S3-4	6,949,201.11	17,078,482.20			
5	Road furniture	m2	5.1	54			232.05	12,530.65	16			340.03	5,440.42	64			28,110.41		
			5.2	54		79.3%	232.05	12,530.65	14		116.2%	340.03	4,760.37	44		439.23	19,325.91		
			5.3	283,242		79.3%	0.52	147,164.99	217,356		116.2%	0.76	165,482.25	244,095		0.98	240,056.37		
			5.4	1,430		79.3%	2.08	2,971.96	330		116.2%	3.05	1,004.97	1,210		3.93	4,759.92		
						Total SI-5	175,198.24			Total S2-5	176,688.02					Total S3-5	292,252.61	644,138.87	
6	Miscellaneous work	m	6.1	0	49.0%	16.21	0.00	0	49.0%	116.2%	19.48	0.00	4,260	49.0%	150.1%	22.47	95,742.93		
			6.2	754			12.49	9,413.99	826			12.49	10,312.94	1,183		12.49	14,770.22		
			6.3	114		79.3%	22.69	2,586.29	535		116.2%	22.69	12,137.41	28		22.69	635.23		
			6.4			79.3%	90.75	0.00			116.2%	90.75	0.00	31		90.75	2,813.16		
			Total SI-6	12,000.28			Total S2-6	22,450.35					Total S3-6	113,961.54	148,412.17				
7	Bridge	No	No. 12 Monapo bridge	L=25m													0.00		
			No. 19 Lalaua bridge	L=30m														0.00	
			No. 24 Nanaleia bridge	L=48m														0.00	
			No. 27 Muivasse bridge	L=30m														0.00	
			No. 30 Namueia bridge	L=30m														0.00	
			No. 34 Lurio bridge	L=94m														0.00	
			Temporary construction road	km	124.00			10,183.00	1,262,692.00	101.00			10,183.00	1,028,483.00	104.00			10,183.00	1,059,032.00
			Total (2-8)					Total SI (2-8)	42,401,817.22				Total S2 (2-8)	31,515,588.28				Total S3 (2-8)	37,477,224.40
			Dayworks	L.S.	1			1,043,084.70	1,043,084.70	1			775,283.47	775,283.47	1			921,939.72	921,939.72
			Social issues	L.S.	1			758,992.53	758,992.53	1			564,129.03	564,129.03	1			670,842.32	670,842.32
			Contingency cost	L.S.	1			6,197,873.62	6,197,873.62	1			4,606,633.54	4,606,633.54	1			5,478,045.89	5,478,045.89
Total construction cost (1-11)	L.S.	1			68,176,609.86	68,176,609.86	1			50,672,968.92	50,672,968.92	1			60,238,504.80	60,238,504.80			
VAT (17% of total construction cost (1-11))	L.S.	1			11,590,023.68	11,590,023.68	1			8,614,404.72	8,614,404.72	1			10,243,945.82	10,243,945.82			
Total construction cost (1-11) inclu. VAT	L.S.	1			79,766,633.54	79,766,633.54	1			59,287,373.64	59,287,373.64	1			70,502,450.62	70,502,450.62			
Engineering cost	L.S.	1			5,454,128.79	5,454,128.79	1			4,053,837.51	4,053,837.51	1			4,820,680.38	4,820,680.38			
VAT (17% of engineering cost)	L.S.	1			927,201.89	927,201.89	1			689,152.38	689,152.38	1			819,515.67	819,515.67			
Total engineering cost inclu. VAT	L.S.	1			6,381,330.68	6,381,330.68	1			5,742,989.89	5,742,989.89	1			6,640,196.05	6,640,196.05			
Total project cost (1-12)	L.S.	1			73,630,738.65	73,630,738.65	1			54,726,806.44	54,726,806.44	1			65,079,185.18	65,079,185.18			
VAT (17% of total project cost (1-12))	L.S.	1			12,517,225.57	12,517,225.57	1			9,303,557.09	9,303,557.09	1			11,063,461.48	11,063,461.48			
Total project cost (1-12) inclu. VAT	L.S.	1			86,147,964.22	86,147,964.22	1			64,030,363.53	64,030,363.53	1			76,142,646.67	76,142,646.67			

Breakdown of the Project Cost (BQ_ALTI-3_80kph)

Item	Description	Unit	Section 1 (Nampula - Ribuae)			Section 2 (Ribuae - Malema)			Section 3 (Malema - Cuimba)			Section 4 (Cuimba - Morrumbulo)			Total (Section 1-3) (km)		
			Qty	Material coefficient	Transport coefficient	Unit Price 1	Amount 1	Qty	Material coefficient	Transport coefficient	Unit Price 2	Amount 2	Qty	Material coefficient		Transport coefficient	Unit Price 3
0	Compensation	km	131.85			3,365.00	443,675.25										
1	Preliminary & general works	L.S.	1			12,059,162.74	12,059,162.74	1									
2	Earthworks																
	2.1	Excavation	832,667			3.12	2,600,419.04	695,594									
	2.2	Fill (use excavated soil)	728,531			3.16	2,298,666.35	582,281									
	2.3	Fill (borrow material)				4.87	0.00										
	2.4	Disposal of surplus soil	104,136			3.50	364,400.88	113,313									
						Total S1-2	5,263,486.28										
3	Pavement																
	3.1.1	Replacement of subgrade (t = 150mm)	sq.m			2.47	0.00										
	3.1.2	Replacement of subgrade (t = 200mm)	sq.m			2.87	0.00										
	3.2	Granular sub base course (t = 150mm) (Soaked CBR > 30%)	sq.m	80.9%	69.7%	0.89	0.00										
	3.3	Granular sub base course (t = 175mm) (Soaked CBR > 30%)	sq.m	80.9%	69.7%	1.04	0.00										
	3.4	Granular sub base course (t = 225mm) (Soaked CBR > 30%)	sq.m	80.9%	69.7%	1.34	0.00										
	3.5.1	Granular sub base course (t = 325mm) (Soaked CBR > 30%)	sq.m	80.9%	69.7%	1.93	0.00										
	3.5.2	Granular sub base course (t = 375mm) (Soaked CBR > 30%)	sq.m	80.9%	69.7%	2.23	0.00	1,031,870									
	3.5.3	Granular sub base course (t = 400mm) (Soaked CBR > 30%)	sq.m	80.9%	69.7%	2.38	3,173,114.09										
	3.6	Cemented sub base course (t = 150mm) (7day UCS 0.75 - 1.5MPa)	sq.m	80.9%	69.7%	3.68	0.00										
	3.7	Cemented sub base course (t = 175mm) (7day UCS 0.75 - 1.5MPa)	sq.m	80.9%	69.7%	4.30	0.00										
	3.8	Cemented sub base course (t = 200mm) (7day UCS 0.75 - 1.5MPa)	sq.m	80.9%	69.7%	4.91	0.00										
	3.9.1	Cemented sub base course (t = 225mm) (7day UCS 0.75 - 1.5MPa)	sq.m	80.9%	69.7%	5.52	0.00										
	3.9.2	Cemented sub base course (t = 250mm) (7day UCS 0.75 - 1.5MPa)	sq.m	80.9%	69.7%	6.14	0.00										
	3.9.3	Cemented sub base course (t = 300mm) (7day UCS 0.75 - 1.5MPa)	sq.m	80.9%	69.7%	7.36	0.00										
	3.9.4	Cemented sub base course (t = 325mm) (7day UCS 0.75 - 1.5MPa)	sq.m	80.9%	69.7%	7.98	0.00										
	3.10	Granular base course (t = 150mm) (Soaked CBR > 80%)	sq.m	80.9%	69.7%	2.59	0.00										
	3.11.1	Granular base course (t = 200mm) (Soaked CBR > 80%)	sq.m	80.9%	69.7%	3.46	0.00	1,031,870									
	3.11.2	Granular base course (t = 225mm) (Soaked CBR > 80%)	sq.m	80.9%	69.7%	3.89	0.00										
	3.11.3	Granular base course (t = 250mm) (Soaked CBR > 80%)	sq.m	80.9%	69.7%	4.32	5,763,853.02										
	3.12	Cemented base course (t = 150mm) (7day UCS 1.5 - 3.0MPa)	sq.m	80.9%	69.7%	5.07	0.00										
	3.13	Cemented base course (t = 180mm) (7day UCS 1.5 - 3.0MPa)	sq.m	80.9%	69.7%	6.08	0.00										
	3.14	Cemented base course (t = 200mm) (7day UCS 1.5 - 3.0MPa)	sq.m	80.9%	69.7%	6.76	0.00										
	3.15	Prime coat	sq.m		79.3%	1.59	1,912,678.25	929,010									
	3.16	Tack coat	sq.m		79.3%	0.88	1,054,975.00	929,010									
	3.17.1	Asphalt concrete (t = 50mm)	sq.m	36.4%	79.3%	10.22	0.00										
	3.17.2	Asphalt concrete (t = 100mm)	sq.m	36.4%	79.3%	20.45	0.00										
	3.18	Double bituminous surface treatment (DBST)	sq.m	36.4%	79.3%	4.64	5,577,208.33	929,010									
	3.19	Ona seal for sidewalk	sq.m	36.4%	79.3%	2.78	0.00										
	3.20	Granular sub base course for sidewalk (t = 100mm, Soaked CBR > 30%)	sq.m	80.9%	69.7%	0.59	0.00										
						Total S1-3	17,481,828.69										
						Total S2-3	11,275,205.11										
						Total S3-3	14,419,277.09										
						Total S3-3	13,114,968.46										

Breakdown of the Project Cost (BQ_ALTI-3_80kph)

Item	Description	Unit	Section 1 (Nampula - Ribaué)			Section 2 (Ribaué - Makema)			Section 3 (Makema - Cuimba)			Section 4 (Cuimba - Vila Fontes)			Total (Section 1-3) (km)					
			Qty	Material coefficient	Transport coefficient	Unit Price 1	Amount 1	Q'ty	Material coefficient	Transport coefficient	Unit Price 2	Amount 2	Q'ty	Material coefficient		Transport coefficient	Unit Price 3	Amount 3		
4	Drainage																			
		4.1	Unlined side ditch	m			3.35	0.00								3.35	0.00	0.00		
		4.2	Concrete lined side ditch	m	98,800	49.0%	79.3%	21.36	2,110,027.74	97,400	49.0%	79.3%	25.65	2,498,691.44	91,000	49.0%	79.3%	2,693,772.67	7,302,491.85	
		4.3	U-shaped side ditch (open)	m	1,170	49.0%	79.3%	38.43	44,958.42	240	49.0%	79.3%	46.16	11,077.94	1,260	49.0%	79.3%	67,109.50	123,145.86	
		4.4	U-shaped side ditch (covered)	m	16,240	49.0%	79.3%	47.79	776,124.13	3,010	49.0%	79.3%	57.41	172,796.17	15,686	49.0%	79.3%	1,039,072.14	1,987,992.44	
		4.5	Cross culvert	m	232	49.0%	79.3%	292.41	73,687.76	72	49.0%	79.3%	351.25	25,590.05	384	49.0%	79.3%	155,637.55	254,615.36	
		4.6	Box culvert (1*1.0)	No	179	49.0%	79.3%	7,226.46	1,293,537.03	139	49.0%	79.3%	8,680.57	1,206,599.35	179	49.0%	79.3%	1,792,945.94	4,293,082.32	
4.7	Box culvert (2.0*2.0)	No	12	49.0%	79.3%	22,795.51	273,546.12	60	49.0%	79.3%	27,382.42	1,642,944.94	38	49.0%	79.3%	1,200,663.30	3,117,154.35			
						Total SI-4	4,571,881.20				Total S2-4	5,557,399.89			Total S3-4	6,949,201.11	17,078,482.20			
5	Road furniture																			
		5.1	Road sign (Warning) A=0.62m2/wo	m2	54			232.05	12,530.65	16			340.03	5,440.42	64			28,110.41	46,081.48	
		5.2	Road sign (Regulatory) A=1.13m2/wo	m2	54			232.05	12,530.65	14			340.03	4,760.37	44			19,325.91	36,616.92	
		5.3	Road marking (continuous) W=100mm	m	283,242			0.52	147,164.99	217,356			0.76	165,482.25	244,095			0.98	240,056.37	552,705.62
		5.4	Road marking (continuous) W=400m	m	1,430			2.08	2,971.96	330			3.05	1,004.97	1,210			3.93	4,759.92	8,756.85
							Total SI-5	175,198.24					Total S2-5	176,688.02			Total S3-5	292,252.61	644,138.87	
6	Miscellaneous work																			
		6.1	Concrete kerb	m	0	49.0%	79.3%	16.21	0.00	0	49.0%	79.3%	19.48	0.00	4,260	49.0%	79.3%	95,742.93	95,742.93	
		6.2	Demolishing existing concrete (plain)	m3	754			12.49	9,413.99	826			12.49	10,312.94	1,183			14,770.22	34,497.15	
		6.3	Demolishing existing concrete (reinforced)	m3	114			22.69	2,586.29	535			22.69	12,137.41	28			635.23	15,358.93	
		6.4	Removal of Bailey bridge (W=4m)	m				90.75	0.00				90.75	0.00	31			2813.16	2,813.16	
					Total SI-6	12,000.28					Total S2-6	22,450.35			Total S3-6	113,961.54	148,412.17			
7	Bridge																			
		No. 12 Monapo bridge	L=25m	No																
		No. 19 Lalaua bridge	L=30m	No																
		No. 24 Nuaaleia bridge	L=48m	No																
		No. 27 Mutivasse bridge	L=30m	No																
		No. 30 Nuanua la bridge	L=30m	No																
		No. 34 Lurio bridge	L=94m	No																
							Total SI-7	0.00					Total S2-7	2,043,003.87			Total S3-7	3,498,934.03	5,541,937.90	
							Total SI (2-8)	28,767,086.68					Total S2 (2-8)	24,509,301.35			Total S3 (2-8)	29,778,069.46	83,054,457.49	
		8	Temporary construction road	km	124.00			10,183.00	1,262,692.00	101.00			10,183.00	1,028,483.00	104.00			10,183.00	1,059,032.00	3,380,207.00
9	Dayworks	L.S.	1			707,670.33	707,670.33	1			602,928.81	602,928.81	1			732,540.51	2,043,139.65			
10	Social issues	L.S.	1			514,930.85	514,930.85	1			438,716.49	438,716.49	1			533,027.44	1,486,674.79			
					Total SI (1-10)	42,048,850.60					Total S2 (1-10)	35,825,245.78			Total S3 (1-10)	43,526,604.12	121,400,700.51			
11	Contingency cost [10% of Total (1-10)]	L.S.	1			4,204,885.06	4,204,885.06	1			3,582,524.58	3,582,524.58	1			4,332,660.41	12,140,070.05			
	Total construction cost (1-11)	L.S.	1			46,253,735.66	46,253,735.66	1			39,407,770.36	39,407,770.36	1			47,879,264.54	135,540,770.56			
	VAT (17% of total construction cost (1-11))	L.S.	1			7,863,135.06	7,863,135.06	1			6,699,320.96	6,699,320.96	1			8,139,474.97	22,701,931.00			
	Total construction cost (1-11) inclu. VAT	L.S.	1			54,116,870.73					Total Construction S2 (1-11)	46,107,091.32			Total Construction S3 (1-11)	56,018,739.51	156,242,701.56			
12	Engineering cost [8% of total construction cost (1-11)]	L.S.	1			3,709,298.85	3,709,298.85	1			3,152,621.63	3,152,621.63	1			3,830,341.16	10,683,261.64			
	VAT (17% of engineering cost)	L.S.	1			629,050.81	629,050.81	1			535,945.68	535,945.68	1			651,158.00	1,816,154.48			
	Total engineering cost inclu. VAT	L.S.	1			4,329,349.66					Total engineering S2	3,688,567.31			Total engineering S3	4,481,499.16	12,499,416.12			
	Total project cost (1-12)	L.S.	1			49,954,034.52	49,954,034.52	1			42,560,391.99	42,560,391.99	1			51,709,605.70	144,224,032.21			
	VAT (17% of total project cost (1-12))	L.S.	1			8,492,185.87	8,492,185.87	1			7,235,266.64	7,235,266.64	1			8,790,632.97	24,518,085.47			
	Total project cost (1-12) inclu. VAT	L.S.	1			58,446,220.39					Total project S2 (1-12)	49,795,658.63			Total project S3 (1-12)	60,500,238.67	168,742,117.68			

Breakdown of the Project Cost (BQ_ALTI-4_80kph)

Item	Description	Unit	Section 1 (Nampula - Ribaué)			Section 2 (Ribaué - Malema)			Section 3 (Malema - Cuamba)			Section 4 (Cuamba - Vila Fontes)						
			Qty	Material coefficient	Transport coefficient	Unit Price 1	Amount 1	Qty	Material coefficient	Transport coefficient	Unit Price 2	Amount 2	Qty	Material coefficient	Transport coefficient	Unit Price 3	Amount 3	Total (Section 1-3)
0	Compensation	km	131.85			3,365.00	443,675.25				3,365.00	346,157.55				3,365.00	379,942.15	1,109,774.95
1	Preliminary & general works	L.S.	1			16,868,458.71	16,868,458.71	1			11,636,635.48	11,636,635.48	1			13,926,647.54	13,926,647.54	42,431,741.73
2	Earthworks																	
2.1	Excavation	cum	832,667			3.12	2,600,419.04				3.12	1,910,735.72				3.12	1,378,054.98	5,889,209.74
2.2	Fill (use excavated soil)	cum	728,531			3.16	2,298,666.35				3.16	1,930,441.04				3.16	1,277,114.34	5,506,221.73
2.3	Fill (borrow material)	cum				4.87	0.00				4.87	287,280.74				4.87	0.00	287,280.74
2.4	Disposal of surplus soil	cum	104,136			3.50	364,400.88				3.50	0.00				3.50	127,709.67	492,110.56
						Total SI-2	5,263,486.28				Total S2-2	4,128,457.50				Total S3-2	2,782,879.00	12,174,822.77
3	Pavement																	
3.1.1	Replacement of subgrade (t = 150mm)	sq.m				2.47	0.00				2.47	0.00				2.47	0.00	0.00
3.1.2	Replacement of subgrade (t = 200mm)	sq.m	1,333,600			2.87	3,831,966.24				2.87	0.00				2.87	0.00	3,831,966.24
3.2	Granular sub base course (t = 150mm) (Soaked CBR > 30%)	sq.m		80.9%		69.7%	0.89	0.00			80.9%	23.7%				80.9%	26.0%	0.00
3.3	Granular sub base course (t = 175mm) (Soaked CBR > 30%)	sq.m		80.9%		69.7%	1.04	0.00			80.9%	23.7%				80.9%	26.0%	0.00
3.4	Granular sub base course (t = 225mm) (Soaked CBR > 30%)	sq.m		80.9%		69.7%	1.34	0.00			80.9%	23.7%				80.9%	26.0%	0.00
3.5.1	Granular sub base course (t = 325mm) (Soaked CBR > 30%)	sq.m		80.9%		69.7%	1.93	0.00			80.9%	23.7%				80.9%	26.0%	0.00
3.5.2	Granular sub base course (t = 375mm) (Soaked CBR > 30%)	sq.m		80.9%		69.7%	2.23	0.00			80.9%	23.7%				80.9%	26.0%	0.00
3.5.3	Granular sub base course (t = 400mm) (Soaked CBR > 30%)	sq.m		80.9%		69.7%	2.38	0.00			80.9%	23.7%				80.9%	26.0%	0.00
3.6	Cemented sub base course (t = 150mm) (7day UCS 0.75 - 1.5MPa)	sq.m		80.9%		69.7%	3.68	0.00			80.9%	23.7%				80.9%	26.0%	0.00
3.7	Cemented sub base course (t = 175mm) (7day UCS 0.75 - 1.5MPa)	sq.m		80.9%		69.7%	4.30	0.00			80.9%	23.7%				80.9%	26.0%	0.00
3.8	Cemented sub base course (t = 200mm) (7day UCS 0.75 - 1.5MPa)	sq.m		80.9%		69.7%	4.91	0.00			80.9%	23.7%				80.9%	26.0%	0.00
3.9.1	Cemented sub base course (t = 225mm) (7day UCS 0.75 - 1.5MPa)	sq.m		80.9%		69.7%	5.52	0.00			80.9%	23.7%				80.9%	26.0%	0.00
3.9.2	Cemented sub base course (t = 250mm) (7day UCS 0.75 - 1.5MPa)	sq.m		80.9%		69.7%	6.14	0.00			80.9%	23.7%				80.9%	26.0%	0.00
3.9.3	Cemented sub base course (t = 300mm) (7day UCS 0.75 - 1.5MPa)	sq.m	1,333,600			69.7%	7.36	9,820,596.84			69.7%	3.73	3,852,647.72	1,145,540		69.7%	3.92	4,484,986.17
3.9.4	Cemented sub base course (t = 325mm) (7day UCS 0.75 - 1.5MPa)	sq.m		80.9%		69.7%	7.98	0.00			80.9%	23.7%				80.9%	26.0%	0.00
3.10	Granular base course (t = 150mm) (Soaked CBR > 80%)	sq.m		80.9%		69.7%	2.59	0.00			80.9%	23.7%				80.9%	26.0%	0.00
3.11.1	Granular base course (t = 200mm) (Soaked CBR > 80%)	sq.m		80.9%		69.7%	3.46	0.00			80.9%	23.7%				80.9%	26.0%	0.00
3.11.2	Granular base course (t = 225mm) (Soaked CBR > 80%)	sq.m		80.9%		69.7%	3.89	0.00			80.9%	23.7%				80.9%	26.0%	0.00
3.11.3	Granular base course (t = 250mm) (Soaked CBR > 80%)	sq.m		80.9%		69.7%	4.32	0.00			80.9%	23.7%				80.9%	26.0%	0.00
3.12	Cemented base course (t = 150mm) (7day UCS 1.5 - 3.0MPa)	sq.m	1,333,600			69.7%	5.07	6,756,961.22			69.7%	2.57	2,650,774.86	1,145,540		69.7%	2.69	3,085,848.87
3.13	Cemented base course (t = 180mm) (7day UCS 1.5 - 3.0MPa)	sq.m		80.9%		69.7%	6.08	0.00			80.9%	23.7%				80.9%	26.0%	0.00
3.14	Cemented base course (t = 200mm) (7day UCS 1.5 - 3.0MPa)	sq.m		80.9%		69.7%	6.76	0.00			80.9%	23.7%				80.9%	26.0%	0.00
3.15	Prime coat	sq.m	1,201,770			79.3%	1.59	1,912,678.25			79.3%	2.33	2,166,575.81	1,032,660		79.3%	3.01	3,110,895.48
3.16	Tack coat	sq.m	1,201,770			79.3%	0.88	1,054,975.00			79.3%	1.29	1,195,017.15	1,032,660		79.3%	1.66	1,715,875.08
3.17.1	Asphalt concrete (t = 50mm)	sq.m		36.4%		79.3%	10.22	0.00			36.4%	11.71	0.00			36.4%	13.07	0.00
3.17.2	Asphalt concrete (t = 100mm)	sq.m		36.4%		79.3%	20.45	0.00			36.4%	23.42	0.00			36.4%	26.15	0.00
3.18	Double bituminous surface treatment (DBST)	sq.m	1,201,770			79.3%	4.64	5,577,208.33			79.3%	5.31	4,937,651.35	1,032,660		79.3%	5.93	6,128,098.66
3.19	Ona seal for sidewalk	sq.m		36.4%		79.3%	2.78	0.00			36.4%	3.19	0.00			36.4%	3.56	0.00
3.20	Granular sub base course for sidewalk (t = 100mm, Soaked CBR > 30%)	sq.m		80.9%		69.7%	0.59	0.00			80.9%	23.7%				80.9%	26.0%	0.00
						Total SI-3	28,954,385.87				Total S2-3	14,802,666.89				Total S3-3	18,525,704.27	62,282,757.03

Breakdown of the Project Cost (BQ_ALTI-4_80kph)

Item	Description	Unit	Section 1 (Nampula - Ribaué)			Section 2 (Ribaué - Makema)			Section 3 (Makema - Cuimba)			Section 4 (Cuimba - Vila Fontes)			Total (Section 1-3) (km)				
			Qty	Material coefficient	Transport coefficient	Unit Price 1	Amount 1	Qty	Material coefficient	Transport coefficient	Unit Price 2	Amount 2	Qty	Material coefficient		Transport coefficient	Unit Price 3	Amount 3	
4	Drainage	m	4.1			3.35	0.00			3.35	0.00				3.35	0.00	0.00		
			4.2	98,800	49.0%	79.3%	21.36	2,110,027.74	97,400	49.0%	116.2%	25.65	2,498,691.44	91,000	49.0%	150.1%	29.60	2,693,772.67	
			4.3	1,170	49.0%	79.3%	38.43	44,958.42	240	49.0%	116.2%	46.16	11,077.94	1,260	49.0%	150.1%	53.26	67,109.50	
			4.4	16,240	49.0%	79.3%	47.79	776,124.13	3,010	49.0%	116.2%	57.41	172,796.17	15,686	49.0%	150.1%	66.24	1,039,072.14	
			4.5	232	49.0%	79.3%	292.41	73,687.76	72	49.0%	116.2%	351.25	25,290.05	384	49.0%	150.1%	405.31	155,637.55	
			4.6	179	49.0%	79.3%	7,226.46	1,293,537.03	139	49.0%	116.2%	8,680.57	1,206,599.35	179	49.0%	150.1%	10,016.46	1,792,945.94	
			4.7	12	49.0%	79.3%	22,795.51	273,546.12	60	49.0%	116.2%	27,382.42	1,642,944.94	38	49.0%	150.1%	31,596.40	1,200,663.30	
					Total SI-4	4,571,881.20				Total S2-4	5,557,399.89				Total S3-4	6,949,201.11			
5	Road furniture	m2	5.1	54			232.05	12,530.65	16			340.03	5,440.42	64			28,110.41		
			5.2	54		79.3%	232.05	12,530.65	14		116.2%	340.03	4,760.37	44		439.23	19,325.91		
			5.3	283,242		79.3%	0.52	147,164.99	217,356		116.2%	0.76	165,482.25	244,095		0.98	240,056.37		
			5.4	1,430		79.3%	2.08	2,971.96	330		116.2%	3.05	1,004.97	1,210		3.93	4,759.92		
						Total SI-5	175,198.24					Total S2-5	176,688.02				Total S3-5	292,252.61	644,138.87
6	Miscellaneous work	m	6.1	0	49.0%	16.21	0.00	0	49.0%	116.2%	19.48	0.00	4,260	49.0%	150.1%	22.47	95,742.93		
			6.2	754			12.49	9,413.99	826			12.49	10,312.94	1,183		12.49	14,770.22		
			6.3	114			22.69	2,586.29	535			22.69	12,137.41	28		22.69	635.23		
			6.4				90.75	0.00				90.75	0.00	31		90.75	2,813.16		
			Total SI-6	12,000.28					Total S2-6	22,450.35				Total S3-6	113,961.54	148,412.17			
7	Bridge	No	No. 12 Monapo bridge	L=25m													0.00		
			No. 19 Lalaua bridge	L=30m														0.00	
			No. 24 Nanaleia bridge	L=48m														0.00	
			No. 27 Muivasse bridge	L=30m														0.00	
			No. 30 Namueia bridge	L=30m														0.00	
			No. 34 Lurio bridge	L=94m														0.00	
																			0.00
																			0.00
																			0.00
																			0.00
																			0.00
8	Temporary construction road	km	124.00			10,183.00	1,262,692.00	101.00			10,183.00	1,028,483.00	104.00			10,183.00	1,059,032.00		
						Total S1 (2-8)	40,239,643.87				Total S2 (2-8)	27,759,149.52				Total S3 (2-8)	33,221,964.56	101,220,757.94	
9	Dayworks	L.S.	1			989,895.24	989,895.24	1			682,875.08	682,875.08	1			817,260.33	2,499,030.65		
						720,289.63	720,289.63	1			496,888.78	496,888.78	1			594,673.17	1,811,851.57		
10	Social issues	L.S.	1			58,818,287.45	58,818,287.45	1			4,057,554.88	4,057,554.88	1			4,856,054.56	14,795,438.19		
						64,700,116.19	64,700,116.19	1			44,633,103.73	44,633,103.73	1			53,416,600.15	162,749,820.08		
11	Contingency cost	L.S.	1			10,959,019.75	10,959,019.75	1			7,587,627.63	7,587,627.63	1			9,080,822.03	27,667,469.41		
						5,881,828.74	5,881,828.74	1			4,057,554.88	4,057,554.88	1			4,856,054.56	14,795,438.19		
12	Total construction cost (1-11) inclu. VAT	L.S.	1			5,176,009.30	5,176,009.30	1			3,570,648.30	3,570,648.30	1			4,273,328.01	13,019,985.61		
						879,921.58	879,921.58	1			607,010.21	607,010.21	1			726,465.76	2,213,397.55		
	Total engineering cost inclu. VAT	L.S.	1			6,055,930.88	6,055,930.88	1			4,177,658.51	4,177,658.51	1			4,999,793.77	15,233,383.16		
					69,876,125.49	69,876,125.49	1			48,203,752.03	48,203,752.03	1			57,689,928.16	175,769,805.68			
	Total project cost (1-12)	L.S.	1			11,878,941.33	11,878,941.33	1			8,194,637.85	8,194,637.85	1			9,807,287.79	29,880,866.97		
					81,755,066.82	81,755,066.82	1			56,398,389.88	56,398,389.88	1			67,497,215.95	205,650,672.65			

Breakdown of the Project Cost (BQ_ALTI-5_80kph)

Item	Description	Unit	Section 1 (Nampula - Ribuae)			Section 2 (Ribuae - Malema)			Section 3 (Malema - Cuimba)			Section 4 (Cuimba - Vila Fontes)						
			Qty	Material coefficient	Transport coefficient	Unit Price 1	Amount 1	Qty	Material coefficient	Transport coefficient	Unit Price 2	Amount 2	Qty	Material coefficient	Transport coefficient	Unit Price 3	Amount 3	Total (Section 1-3)
0	Compensation	km	131.85			3,365.00	443,675.25				3,365.00	346,157.55				3,365.00	379,942.15	1,109,774.95
1	Preliminary & general works	L.S.	1			20,433,391.01	20,433,391.01	1			17,530,159.23	17,530,159.23	1			21,204,094.99	21,204,094.99	59,167,645.23
2	Earthworks																	
	2.1	Excavation	591,229			3.12	1,846,408.17	523,493			3.12	1,634,868.64	338,605			3.12	1,057,463.42	4,538,740.22
	2.2	Fill (use excavated soil)	591,229			3.16	1,865,450.08	523,493			3.16	1,651,728.95	338,605			3.16	1,068,368.98	4,585,548.01
	2.3	Fill (borrow material)	468,516			4.87	2,281,204.40	267,530			4.87	1,302,603.57	183,564			4.87	892,799.32	4,476,607.29
	2.4	Disposal of surplus soil				3.50	0.00				3.50	0.00				3.50	0.00	0.00
						Total S1-2	5,993,062.65				Total S2-2	4,589,201.16				Total S3-2	3,018,631.71	13,600,895.52
3	Pavement																	
	3.1.1	Replacement of subgrade (t = 150mm)	sq.m			2.47	0.00				2.47	0.00				2.47	0.00	0.00
	3.1.2	Replacement of subgrade (t = 200mm)	sq.m			2.87	0.00				2.87	0.00				2.87	0.00	0.00
	3.2	Granular sub base course (t = 150mm) (Soaked CBR > 30%)	sq.m	80.9%		0.89	0.00			80.9%	0.45	0.00			80.9%	0.47	0.00	0.00
	3.3	Granular sub base course (t = 175mm) (Soaked CBR > 30%)	sq.m	80.9%		1.04	0.00			80.9%	0.53	0.00			80.9%	0.55	0.00	0.00
	3.4	Granular sub base course (t = 225mm) (Soaked CBR > 30%)	sq.m	80.9%		1.34	0.00			80.9%	0.68	0.00			80.9%	0.71	0.00	0.00
	3.5.1	Granular sub base course (t = 325mm) (Soaked CBR > 30%)	sq.m	80.9%		1.93	0.00			80.9%	0.98	0.00			80.9%	1.03	0.00	0.00
	3.5.2	Granular sub base course (t = 375mm) (Soaked CBR > 30%)	sq.m	80.9%		2.23	0.00			80.9%	1.13	0.00			80.9%	1.19	0.00	0.00
	3.5.3	Granular sub base course (t = 400mm) (Soaked CBR > 30%)	sq.m	80.9%		2.38	0.00			80.9%	1.21	0.00			80.9%	1.27	0.00	0.00
	3.6	Cemented sub base course (t = 150mm) (7day UCS 0.75 - 1.5MPa)	sq.m	80.9%		3.68	0.00	1,031,870		80.9%	1.87	1,926,323.86	1,145,540		80.9%	1.96	2,242,493.09	4,168,816.95
	3.7	Cemented sub base course (t = 175mm) (7day UCS 0.75 - 1.5MPa)	sq.m	80.9%		4.30	5,728,681.49			80.9%	2.18	0.00			80.9%	2.28	0.00	5,728,681.49
	3.8	Cemented sub base course (t = 200mm) (7day UCS 0.75 - 1.5MPa)	sq.m	80.9%		4.91	0.00			80.9%	2.49	0.00			80.9%	2.61	0.00	0.00
	3.9.1	Cemented sub base course (t = 225mm) (7day UCS 0.75 - 1.5MPa)	sq.m	80.9%		5.52	0.00			80.9%	2.80	0.00			80.9%	2.94	0.00	0.00
	3.9.2	Cemented sub base course (t = 250mm) (7day UCS 0.75 - 1.5MPa)	sq.m	80.9%		6.14	0.00			80.9%	3.11	0.00			80.9%	3.26	0.00	0.00
	3.9.3	Cemented sub base course (t = 300mm) (7day UCS 0.75 - 1.5MPa)	sq.m	80.9%		7.36	0.00			80.9%	3.73	0.00			80.9%	3.92	0.00	0.00
	3.9.4	Cemented sub base course (t = 325mm) (7day UCS 0.75 - 1.5MPa)	sq.m	80.9%		7.98	0.00			80.9%	4.04	0.00			80.9%	4.24	0.00	0.00
	3.10	Granular base course (t = 150mm) (Soaked CBR > 80%)	sq.m	80.9%		2.59	3,458,311.81	1,031,870		80.9%	1.31	1,356,705.44	1,145,540		80.9%	1.38	1,579,382.69	6,394,399.94
	3.11.1	Granular base course (t = 200mm) (Soaked CBR > 80%)	sq.m	80.9%		3.46	0.00			80.9%	1.75	0.00			80.9%	1.84	0.00	0.00
	3.11.2	Granular base course (t = 225mm) (Soaked CBR > 80%)	sq.m	80.9%		3.89	0.00			80.9%	1.97	0.00			80.9%	2.07	0.00	0.00
	3.11.3	Granular base course (t = 250mm) (Soaked CBR > 80%)	sq.m	80.9%		4.32	0.00			80.9%	2.19	0.00			80.9%	2.30	0.00	0.00
	3.12	Cemented base course (t = 150mm) (7day UCS 1.5 - 3.0MPa)	sq.m	80.9%		5.07	0.00			80.9%	2.57	0.00			80.9%	2.69	0.00	0.00
	3.13	Cemented base course (t = 180mm) (7day UCS 1.5 - 3.0MPa)	sq.m	80.9%		6.08	0.00			80.9%	3.08	0.00			80.9%	3.23	0.00	0.00
	3.14	Cemented base course (t = 200mm) (7day UCS 1.5 - 3.0MPa)	sq.m	80.9%		6.76	0.00			80.9%	3.43	0.00			80.9%	3.59	0.00	0.00
	3.15	Prime coat	sq.m			1.59	1,912,678.25	929,010			2.33	2,166,575.81	1,032,660			3.01	3,110,895.48	7,190,149.53
	3.16	Tack coat	sq.m			0.88	1,054,975.00	929,010			1.29	1,195,017.15	1,032,660			1.66	1,715,875.08	3,965,867.23
	3.17.1	Asphalt concrete (t = 50mm)	sq.m	36.4%		10.22	0.00			36.4%	11.71	0.00			36.4%	13.07	0.00	0.00
	3.17.2	Asphalt concrete (t = 100mm)	sq.m	36.4%		20.45	24,574,298.35	929,010		36.4%	23.42	21,756,279.39	1,032,660		36.4%	26.15	27,001,628.32	73,332,203.07
	3.18	Double bituminous surface treatment (DBST)	sq.m	36.4%		4.64	0.00			36.4%	5.31	0.00			36.4%	5.93	0.00	0.00
	3.19	Ona seal for sidewalk	sq.m	36.4%		2.78	0.00			36.4%	3.19	0.00			36.4%	3.56	0.00	0.00
	3.20	Granular sub base course for sidewalk (t = 100mm, Soaked CBR > 30%)	sq.m	80.9%		0.59	0.00			80.9%	0.30	0.00			80.9%	0.32	0.00	0.00
						Total S1-3	36,728,941.90				Total S2-3	28,400,901.65				Total S3-3	35,650,274.66	100,780,118.20

Breakdown of the Project Cost (BQ_ALTI-5_80kph)

Item	Description	Unit	Section 1 (Nampula - Ribaué)			Section 2 (Ribaué - Makema)			Section 3 (Makema - Cuamba)			Section 4 (Cuamba - Vila Fontana)								
			Qty	Material coefficient	Transport coefficient	Unit Price 1	Amount 1	Q'ty	Material coefficient	Transport coefficient	Unit Price 2	Amount 2	Q'ty	Material coefficient	Transport coefficient	Unit Price 3	Amount 3	Total (Section 1-3)		
4	Drainage	4.1				3.35	0.00													
		4.2	Unlined side ditch	m	98,800	49.0%	79.3%	21.36	2,110,027.74	97,400	49.0%	116.2%	3.35	0.00						
		4.3	Concrete lined side ditch	m	1,170	49.0%	79.3%	38.43	44,958.42	240	49.0%	116.2%	25.65	2,498,691.44	91,000	49.0%	150.1%	2,693,772.67	7,302,491.85	
		4.4	U-shaped side ditch (open)	m	16,240	49.0%	79.3%	47.79	776,124.13	3,010	49.0%	116.2%	46.16	11,077.94	1,260	49.0%	53.26	67,109.50	123,145.86	
		4.5	U-shaped side ditch (covered)	m	232	49.0%	79.3%	292.41	73,687.76	72	49.0%	116.2%	351.25	25,590.05	384	49.0%	405.31	155,637.55	254,015.36	
		4.6	Cross culvert	No	179	49.0%	79.3%	7,226.46	1,293,537.03	139	49.0%	116.2%	8,680.57	1,206,599.35	179	49.0%	10,016.46	1,792,945.94	4,293,082.32	
		4.7	Box culvert (2.0*2.0)	No	12	49.0%	79.3%	22,795.51	273,546.12	60	49.0%	116.2%	27,382.42	1,642,944.94	38	49.0%	31,596.40	1,200,663.30	3,117,154.35	
						Total SI-4	4,571,881.20				Total S2-4	5,557,399.89				Total S3-4	6,949,201.11	17,078,482.20		
5	Road furniture	5.1	Road sign (Warning) A=0.62m2/wo	m2	54			232.05	12,530.65	16			340.03	5,440.42	64			28,110.41	46,081.48	
		5.2	Road sign (Regulatory) A=1.13m2/wo	m2	54			232.05	12,530.65	14			340.03	4,760.37	44			19,325.91	36,616.92	
		5.3	Road marking (continuous) W=100mm	m	283,242			0.52	147,164.99	217,356			0.76	165,482.25	244,095			240,056.37	552,705.62	
		5.4	Road marking (continuous) W=400m	m	1,430			2.08	2,971.96	330			3.05	1,004.97	1,210			4,759.92	8,756.85	
							Total SI-5	175,198.24					Total S2-5	176,688.02			Total S3-5	292,252.61	644,138.87	
6	Miscellaneous work	6.1	Concrete kerb	m	0	49.0%	79.3%	16.21	0.00	0	49.0%	116.2%	19.48	0.00	4,260	49.0%	150.1%	95,742.93	95,742.93	
		6.2	Demolishing existing concrete (plain)	m3	754			12.49	9,413.99	826			12.49	10,312.94	1,183			14,770.22	34,497.15	
		6.3	Demolishing existing concrete (reinforced)	m3	114			22.69	2,586.29	535			22.69	12,137.41	28			635.23	15,558.93	
		6.4	Removal of Bailey bridge (W=4m)	m				90.75	0.00				90.75	0.00	31			2,813.16	2,813.16	
					Total SI-6	12,000.28					Total S2-6	22,450.35			Total S3-6	113,961.54	148,412.17			
7	Bridge	No. 12 Monapo bridge	L=25m	No																
		No. 19 Lalaua bridge	L=30m	No																
		No. 24 Nanaleia bridge	L=48m	No																
		No. 27 Muivasse bridge	L=30m	No																
		No. 30 Namueia bridge	L=30m	No																
		No. 34 Lurio bridge	L=94m	No																
							Total SI-7	0.00					Total S2-7	2,043,003.87			Total S3-7	3,498,934.03	5,541,937.90	
		8	Temporary construction road	km	124.00			10,183.00	1,262,692.00	101.00			10,183.00	1,028,483.00	104.00			10,183.00	1,059,032.00	3,380,207.00
							Total (2-8)	48,743,776.26					Total S2 (2-8)	41,818,127.94			Total S3 (2-8)	50,582,287.67	141,144,191.87	
		9	Dayworks	L.S.	1			1,199,096.90	1,199,096.90	1			1,028,725.95	1,028,725.95	1			1,244,324.28	3,472,147.12	
10	Social issues	L.S.	1			872,513.60	872,513.60	1			748,544.49	748,544.49	1			905,422.95	2,526,481.03			
					Total (1-10)	71,248,777.76					Total S2 (1-10)	61,125,557.61			Total S3 (1-10)	73,936,129.88	206,510,465.25			
11	Contingency cost [10% of Total (1-10)]	L.S.	1			7,124,877.78	7,124,877.78	1			6,112,555.76	6,112,555.76	1			7,393,612.99	20,631,046.52			
	Total construction cost (1-11)	L.S.	1			78,373,655.54	78,373,655.54	1			67,238,113.37	67,238,113.37	1			81,329,742.87	226,941,511.77			
	VAT (17% of total construction cost (1-11))	L.S.	1			13,323,521.44	13,323,521.44	1			11,430,479.27	11,430,479.27	1			13,826,056.29	38,580,057.00			
	Total construction cost (1-11) inclu. VAT	L.S.	1			91,697,176.98					Total Construction S2 (1-11)	78,668,592.64			Total Construction S3 (1-11)	95,155,799.16	265,521,568.78			
12	Engineering cost [8% of total construction cost (1-11)]	L.S.	1			6,269,892.44	6,269,892.44	1			5,379,049.07	5,379,049.07	1			6,506,379.43	18,155,320.94			
	VAT (17% of engineering cost)	L.S.	1			1,065,881.72	1,065,881.72	1			914,438.34	914,438.34	1			1,106,084.50	3,086,404.56			
	Total engineering cost inclu. VAT	L.S.	1			7,335,774.16					Total engineering S2	6,293,487.41			Total engineering S3	7,612,463.93	21,241,725.50			
	Total project cost (1-12)	L.S.	1			84,643,547.98	84,643,547.98	1			72,617,162.44	72,617,162.44	1			87,836,122.30	245,096,832.72			
	VAT (17% of total project cost (1-12))	L.S.	1			14,389,403.16	14,389,403.16	1			12,344,917.61	12,344,917.61	1			14,932,140.79	41,666,461.56			
	Total project cost (1-12) inclu. VAT	L.S.	1			99,032,951.14					Total project S2 (1-12)	84,962,080.05			Total project S3 (1-12)	102,768,265.09	286,763,294.28			

Breakdown of the Project Cost (BQ_ALTI-6_80kph)

Item	Description	Unit	Section 1 (Nampula - Ribaué)			Section 2 (Ribaué - Malema)			Section 3 (Malema - Cuamba)			Section 4 (Cuamba - Vila Fontes)								
			Qty	Material coefficient	Transport coefficient	Unit Price 1	Amount 1	Qty	Material coefficient	Transport coefficient	Unit Price 2	Amount 2	Qty	Material coefficient	Transport coefficient	Unit Price 3	Amount 3	Total (Section 1-3)		
0	Compensation	km	131.85			3,365.00	443,675.25				3,365.00	346,157.55				3,365.00	379,942.15	1,109,774.95		
1	Preliminary & general works	L.S.	1			15,060,427.59	15,060,427.59	1			11,578,742.15	11,578,742.15	1			14,002,471.45	14,002,471.45	40,641,641.19		
2	Earthworks																			
	2.1	Excavation	768,261			3.12	2,399,279.10	678,194			3.12	2,117,999.86	516,659			3.12	1,613,526.06	6,130,805.02		
	2.2	Fill (use excavated soil)	768,261			3.16	2,424,022.74	599,218			3.16	1,890,657.03	333,295			3.16	1,051,614.83	5,566,294.60		
	2.3	Fill (borrow material)	27,485			4.87	133,824.47			4.87	0.00				4.87	0.00	133,824.47			
	2.4	Disposal of surplus soil				3.50	0.00	78,976			3.50	276,359.03	183,564			3.50	641,641.73	918,000.77		
						Total S1-2	4,957,126.31				Total S2-2	4,285,015.92				Total S3-2	3,306,782.62	12,548,924.85		
3	Pavement																			
	3.1.1	Replacement of subgrade (t = 150mm)	sq.m			2.47	0.00				2.47	0.00				2.47	0.00	0.00		
	3.1.2	Replacement of subgrade (t = 200mm)	sq.m			2.87	0.00				2.87	0.00				2.87	0.00	0.00		
	3.2	Granular sub base course (t = 150mm) (Soaked CBR > 30%)	sq.m	80.9%	69.7%	0.89	0.00			80.9%	23.7%	0.45	0.00		80.9%	26.0%	0.00	0.00		
	3.3	Granular sub base course (t = 175mm) (Soaked CBR > 30%)	sq.m	80.9%	69.7%	1.04	0.00			80.9%	23.7%	0.53	0.00		80.9%	26.0%	0.00	0.00		
	3.4	Granular sub base course (t = 225mm) (Soaked CBR > 30%)	sq.m	80.9%	69.7%	1.34	0.00			80.9%	23.7%	0.68	0.00		80.9%	26.0%	0.00	0.00		
	3.5.1	Granular sub base course (t = 325mm) (Soaked CBR > 30%)	sq.m	80.9%	69.7%	1.93	0.00			80.9%	23.7%	0.98	0.00		80.9%	26.0%	0.00	0.00		
	3.5.2	Granular sub base course (t = 375mm) (Soaked CBR > 30%)	sq.m	80.9%	69.7%	2.23	0.00			80.9%	23.7%	1.13	0.00		80.9%	26.0%	0.00	0.00		
	3.5.3	Granular sub base course (t = 400mm) (Soaked CBR > 30%)	sq.m	80.9%	69.7%	2.38	0.00			80.9%	23.7%	1.21	0.00		80.9%	26.0%	0.00	0.00		
	3.6	Cemented sub base course (t = 150mm) (7day UCS 0.75 - 1.5MPa)	sq.m	80.9%	69.7%	3.68	0.00			80.9%	23.7%	1.87	0.00		80.9%	26.0%	0.00	0.00		
	3.7	Cemented sub base course (t = 175mm) (7day UCS 0.75 - 1.5MPa)	sq.m	80.9%	69.7%	4.30	0.00			80.9%	23.7%	2.18	0.00		80.9%	26.0%	0.00	0.00		
	3.8	Cemented sub base course (t = 200mm) (7day UCS 0.75 - 1.5MPa)	sq.m	80.9%	69.7%	4.91	0.00			80.9%	23.7%	2.49	0.00		80.9%	26.0%	0.00	0.00		
	3.9.1	Cemented sub base course (t = 225mm) (7day UCS 0.75 - 1.5MPa)	sq.m	80.9%	69.7%	5.52	0.00			80.9%	23.7%	2.80	0.00		80.9%	26.0%	0.00	0.00		
	3.9.2	Cemented sub base course (t = 250mm) (7day UCS 0.75 - 1.5MPa)	sq.m	80.9%	69.7%	6.14	0.00			80.9%	23.7%	3.11	0.00		80.9%	26.0%	0.00	0.00		
	3.9.3	Cemented sub base course (t = 300mm) (7day UCS 0.75 - 1.5MPa)	sq.m	80.9%	69.7%	7.36	0.00			80.9%	23.7%	3.73	0.00		80.9%	26.0%	0.00	0.00		
	3.9.4	Cemented sub base course (t = 325mm) (7day UCS 0.75 - 1.5MPa)	sq.m	80.9%	69.7%	7.98	0.00	10,638,979.91	1,031,870	80.9%	23.7%	4.04	4,173,701.69	1,145,540	80.9%	26.0%	4.24	4,858,735.02	19,671,416.62	
	3.10	Granular base course (t = 150mm) (Soaked CBR > 80%)	sq.m	80.9%	69.7%	2.59	0.00			80.9%	23.7%	1.31	0.00		80.9%	26.0%	1.38	0.00		
	3.11.1	Granular base course (t = 200mm) (Soaked CBR > 80%)	sq.m	80.9%	69.7%	3.46	0.00			80.9%	23.7%	1.75	0.00		80.9%	26.0%	1.84	0.00		
	3.11.2	Granular base course (t = 225mm) (Soaked CBR > 80%)	sq.m	80.9%	69.7%	3.89	0.00	1,031,870	1,031,870	80.9%	23.7%	1.97	2,035,658.15	1,145,540	80.9%	26.0%	2.07	2,369,074.04	4,404,132.19	
	3.11.3	Granular base course (t = 250mm) (Soaked CBR > 80%)	sq.m	80.9%	69.7%	4.32	5,763,853.02			80.9%	23.7%	2.19	0.00		80.9%	26.0%	2.30	0.00	5,763,853.02	
	3.12	Cemented base course (t = 150mm) (7day UCS 1.5 - 3.0MPa)	sq.m	80.9%	69.7%	5.07	0.00			80.9%	23.7%	2.57	0.00		80.9%	26.0%	2.69	0.00		
	3.13	Cemented base course (t = 180mm) (7day UCS 1.5 - 3.0MPa)	sq.m	80.9%	69.7%	6.08	0.00			80.9%	23.7%	3.08	0.00		80.9%	26.0%	3.23	0.00		
	3.14	Cemented base course (t = 200mm) (7day UCS 1.5 - 3.0MPa)	sq.m	80.9%	69.7%	6.76	0.00			80.9%	23.7%	3.43	0.00		80.9%	26.0%	3.59	0.00		
	3.15	Prime coat	sq.m		79.3%	1.59	1,912,678.25	929,010			116.2%	2.33	2,166,575.81	1,032,660		150.1%	3.01	3,110,895.48	7,190,149.53	
	3.16	Tack coat	sq.m		79.3%	0.88	1,054,975.00	929,010			116.2%	1.29	1,195,017.15	1,032,660		150.1%	1.66	1,715,875.08	3,965,867.23	
	3.17.1	Asphalt concrete (t = 50mm)	sq.m	36.4%	79.3%	10.22	0.00			36.4%	116.2%	11.71	0.00		36.4%	150.1%	13.07	0.00		
	3.17.2	Asphalt concrete (t = 100mm)	sq.m	36.4%	79.3%	20.45	0.00			36.4%	116.2%	23.42	0.00		36.4%	150.1%	26.15	0.00		
	3.18	Double bituminous surface treatment (DBST)	sq.m	36.4%	79.3%	4.64	5,577,208.33	929,010			36.4%	116.2%	5.31	4,937,651.35	1,032,660	36.4%	150.1%	5.93	6,128,098.66	16,642,958.35
	3.19	Ona seal for sidewalk	sq.m	36.4%	79.3%	2.78	0.00			36.4%	116.2%	3.19	0.00		36.4%	150.1%	3.56	0.00		
	3.20	Granular sub base course for sidewalk (t = 100mm, Soaked CBR > 30%)	sq.m	80.9%	69.7%	0.59	0.00			80.9%	23.7%	0.30	0.00		80.9%	26.0%	0.32	0.00		
						Total S1-3	24,947,694.50				Total S2-3	14,508,004.16				Total S3-3	18,182,678.28	57,638,376.94		

Breakdown of the Project Cost (BQ_ALTI-6_80kph)

Item	Description	Unit	Section 1 (Nampula - Ribaué)			Section 2 (Ribaué - Makema)			Section 3 (Makema - Cuimba)			Section 4 (Cuimba - Vila Fontes)			Total (Section 1-3) (km)				
			Qty	Material coefficient	Transport coefficient	Unit Price 1	Amount 1	Q'ty	Material coefficient	Transport coefficient	Unit Price 2	Amount 2	Q'ty	Material coefficient		Transport coefficient	Unit Price 3	Amount 3	
4	Drainage	m	4.1			3.35	0.00			3.35	0.00				3.35	0.00	0.00		
			4.2	98,800	49.0%	79.3%	21.36	2,110,027.74	97,400	49.0%	116.2%	25.65	2,498,691.44	91,000	49.0%	150.1%	29.60	2,693,772.67	
			4.3	1,170	49.0%	79.3%	38.43	44,958.42	240	49.0%	116.2%	46.16	11,077.94	1,260	49.0%	150.1%	53.26	67,109.50	
			4.4	16,240	49.0%	79.3%	47.79	776,124.13	3,010	49.0%	116.2%	57.41	172,796.17	15,686	49.0%	150.1%	66.24	1,039,072.14	
			4.5	232	49.0%	79.3%	292.41	73,687.76	72	49.0%	116.2%	351.25	25,590.05	384	49.0%	150.1%	405.31	155,637.55	
			4.6	179	49.0%	79.3%	7,226.46	1,293,537.03	139	49.0%	116.2%	8,680.57	1,206,599.35	179	49.0%	150.1%	10,016.46	1,792,945.94	
			4.7	12	49.0%	79.3%	22,795.51	273,546.12	60	49.0%	116.2%	27,382.42	1,642,944.94	38	49.0%	150.1%	31,596.40	1,200,663.30	
					Total SI-4	4,571,881.20				Total S2-4	5,557,399.89				Total S3-4	6,949,201.11			
5	Road furniture	m2	5.1	54			232.05	12,530.65	16			340.03	5,440.42	64			28,110.41		
			5.2	54		79.3%	232.05	12,530.65	14		116.2%	340.03	4,760.37	44		439.23	19,325.91		
			5.3	283,242		79.3%	0.52	147,164.99	217,356		116.2%	0.76	165,482.25	244,095		0.98	240,056.37		
			5.4	1,430		79.3%	2.08	2,971.96	330		116.2%	3.05	1,004.97	1,210		3.93	4,759.92		
								Total SI-5	175,198.24				Total S2-5	176,688.02				Total S3-5	292,252.61
6	Miscellaneous work	m	6.1	0	49.0%	16.21	0.00	0	49.0%	116.2%	19.48	0.00	4,260	49.0%	150.1%	22.47	95,742.93		
			6.2	754			12.49	9,413.99	826			12.49	10,312.94	1,183		12.49	14,770.22		
			6.3	114			22.69	2,586.29	535			22.69	12,137.41	28		22.69	635.23		
			6.4				90.75	0.00				90.75	0.00	31		90.75	2,813.16		
					Total SI-6	12,000.28				Total S2-6	22,450.35				Total S3-6	113,961.54			
7	Bridge	No	No. 12 Monapo bridge	L=25m													0.00		
			No. 19 Lalaua bridge	L=30m														0.00	
			No. 24 Nanaleia bridge	L=48m														0.00	
			No. 27 Muivasse bridge	L=30m														0.00	
			No. 30 Namueia bridge	L=30m														0.00	
			No. 34 Lurio bridge	L=94m														0.00	
								Total SI-7	0.00				Total S2-7	2,043,003.87				Total S3-7	3,498,934.03
																			0.00
																			0.00
																			0.00
8	Temporary construction road	km	124.00			10,183.00	1,262,692.00	101.00			10,183.00	1,028,483.00	104.00			10,183.00	1,059,032.00		
								Total S2 (2-8)	27,621,045.21								Total S3 (2-8)	33,402,842.19	
9	Dayworks	L.S.	1			883,794.18	883,794.18	1			679,477.71	679,477.71	1			821,709.92	821,709.92		
																		2,384,981.81	
10	Social issues	L.S.	1			643,086.01	643,086.01	1			494,416.71	494,416.71	1			597,910.88	597,910.88		
																		1,735,413.59	
11	Contingency cost	L.S.	1			5,251,390.03	5,251,390.03	1			4,037,368.18	4,037,368.18	1			4,882,493.44	4,882,493.44		
																		14,171,251.65	
12	VAT (17% of total construction cost (1-11))	L.S.	1			57,765,290.33	57,765,290.33	1			44,411,049.96	44,411,049.96	1			53,707,427.88	53,707,427.88		
																		155,883,768.17	
																	26,500,240.59		
																	91,302,627.74		
																	62,837,696.62		
																	4,296,594.23		
																	730,421.02		
																	2,120,019.25		
																	5,027,015.25		
																	14,590,270.70		
																	168,354,469.63		
																	28,620,259.84		
																	67,864,706.87		
																	196,974,229.46		

APÊNDICE-D

Short Report for the First Stakeholder Meetings

16th November, 2006

Short Report for the First Stakeholder Meetings along the Road from Nampula-Cuamba

ANE UASMA/JICA Study Team

I. Objectives of the meetings

Holding public consultation is prescribed in the Mozambique's EIA law and procedures, and also in JICA's environmental and social consideration guidelines. Hence ANE in cooperation with JICA Study Team held the first meetings in relevant districts and municipality along the N-13 road.

II. Outline of Stakeholder Meeting

1. Schedule

Conducted meetings are as follows.

Most of attendances are village leaders and personnel's in district office.

Table 1 Schedule of Stakeholder Meetings

Date	Time	Venue	Attendance/Num.	Contact/Key Person
6 th Nov. (Mon)	0930-1130	District meeting hall Rapale, Nampula District	Nampula	Secretary of district governor Ms. Flora MEQUE Cell: 82-3800980
			Mecuburi	40 District governor Mr. Manuel Joao GABRIEL Cell: no service area Post Administrator for Namina Mr. Johane CASTRO (Namina) Cell: no service area
7 th Nov. (Tue)	0900-1100	Ines Complex (Private) Ribae, Ribae District	Ribae	47 Secretary of district governor Mr. Armand MAILANHIHA Cell: 82-3858194
8 th Nov. (Wed)	0900-1100	Maleya Complex Malema, Malema District	Malema	44 District governor Mr. Cristene Joquim Consula Cell: 82-2571784 Secretary of district governor Mr. Jaime Braz SAIDE Cell: 82-587594
	1400-1600	District meeting hall Cuamba, Cuamba District	Cuamba	34 Secretary of district governor Mr. Angelo SABITE Cell: 82-7082940
16 th Nov. (Thu)	0930-1130	Nampula Municipality	Nampula Municipality	37
			Total	202

Attendance from ANE, JICA Study Team and other organizations

Date	Venue	Attendance of Organizer
6 th Nov. (Mon)	District meeting hall Rapale, Nampula District	ANE Nampula ; Mr. Nyaca (Master of Ceremony), Mr. Gabriel UASMA: Ms. Deolinda Nunes, Ms. Angelina Balate JICA: Mr. Hironori KUROKI, Mr. Manabu SHIMIZU Ministry of Public Works and Housing in Nampura Province Mr. Antonio Alberto Mahave (Director)
7 th Nov. (Tue)	Ines Complex (Private) Ribae, Ribae District	ANE Nampula ; Mr. Gabriel (Master of Ceremony) UASMA: Ms. Deolinda Nunes, Ms. Angelina Balate JICA: Mr. Hironori KUROKI
8 th Nov. (Wed)	Maleya Complex	ANE Nampula ; Mr. Gabriel (Master of Ceremony)

Date	Venue	Attendance of Organizer
	Malema, Malema District	UASMA: Ms. Deolinda Nunes, Ms. Angelina Balate JICA: Mr. Hironori KUROKI
	District meeting hall Cuamba, Cuamba District	ANE Niassa (Master of Ceremony) UASMA: Ms. Deolinda Nunes, Ms. Angelina Balate JICA: Mr. Hironori KUROKI
16 th Nov. (Thu)	Nampula Municipality	ANE Nampula ; Mr. Nyaca (Master of Ceremony) UASMA: Ms. Deolinda Nunes, Ms. Angelina Balate JICA: Mr. Hironori KUROKI

2. Agenda and Presentation

ANE in cooperation with JICA Study Team carried out presentation in English, Portuguese and local language Makua through translator.

Table 2 Agenda and Presentation

Agenda	Person in charge	Details of Presentation
1. Opening the ceremony	- District governor / Representative of district - ANE, UASMA, JICA Study Team	
2. Explanation of Project Outline	ANE or JICA Study Team	ANE and JICA explained we are in feasibility study, not construction stage strongly
3. Predicted Negative and Positive Impact, Collection information	ANE or JICA Study Team	- Positive/Negative impact - What is EIA? - Scopes for EIA in this time
4. Discussion	All participants	- Collection of natural and social information - Collection present problems for using the road - Opinion about improvement of the road
5. Closing the ceremony	-MC (Mr. Notece: Makuwa speaker is required) - ANE (UASMA/JICA Study Team) - District governor	Explanation of next schedule

3. Major Opinion and Interests from Participants

Q-1: From when is this road construction start?

A-1: Feasibility study will complete in September 2007. And Mozambique government will contact with donors for borrowing finances of construction road. Thus these steps are required for construction of the road.

Q-2: When can residents know information and new alignment regarding resettlement?

A-2: ANE in cooperation with JICA Study Team will be able to show you rough alignment in the next stakeholder meeting in April to June 2007.

Q-3: Can affected residents by construction of the road receive compensation?

A-3: Mozambique government will compensate any structures, crops, agricultural field and tress based on the land law and relevant guidelines. With regard to compensation, Ministry of public works & housing will evaluate structures, and Ministry of agriculture will do about farming land and crops.

Q-4: How many meters of the road width?

A-4: ANE is planning classification of the road. ANE can inform you road structure and rough

alignments in the next meeting.

Opinions from Participants

Op.-1 There is dangerous points on existing road such as crossing points with railway in curve.

Information from participants

Info.-1 : 5 elephants were confirmed 40km from Nampula in January 2006.

Info-2 : Some grave yard are located along the road (near Lulio River, Lalawa, Mutuali in Malema)

Info-3 : Some preying trees are located along the road (none at least/ Ribaue, Nampula Municipality)

Conclusion

All participants agreed with road improvement project by applause. (Nampula, Ribaue, Malema and Cuamba)

III Pictures



In Nampula District

Date: 6th Nov. 2006

Time: 0930-1130

Participants: 40

Venue: Rapale



In Ribaue District

Date: 7th Nov. 2006

Time: 0900-1100

Participants: 47

Venue: Ribaue



In Malema District

Date: 8th Nov. 2006

Time: 0900-1100

Participants: 44

Venue: Malema Meleya Comlex



In Cuamba District

Date: 8th Nov. 2006

Time: 1400-1600

Participants: 34

Venue: Cuamba



In Namupa Municipality

Date: 16th Nov. 2006

Time: 0940-1140

Participants: 37

Venue: Nampula Municipality
hall

NAMPULA/CUAMBA ROAD STAKE HOLDERS MEETINGS

Meeting Record, prepared by ANE-UASMA

(These meetings have been held in Nampula, Ribaué, Malema, Cuamba district and Nampula Municipality. Makuwa local language was used in all meetings.)

NAMPULA-MECUBURI DISTRICTS (6TH November, 2006)

ANE provincial delegate has opened a meeting, explaining to all the participants the objectives of the meeting.

He has also explained about the on going feasibility study in that section of road.

Major Participants;

After this presentation, he gave time for each participant to present him selves and among them, there were:

- Provincial Director from Public works and Housings Traditional Leaders;
- Chefes de Posto;
- Secretários de Bairros ;
- Agriculture authorities representatives;
- Education representatives.

After all presentations, the Provincial Director from Public works, as the Government representative has appreciated the initiative and has also appreciated the presence of all participants in the meeting. He has said the rehabilitation of this section of road will facilitate the circulation of people and goods from the Districts of Rapale and Namina to Nampula.

The Director has explained to the participants that the study will be helpful on knowing the problems that may occur during the construction period, and this is the reason why this meeting is taking place, to involve all the communities, and to have the contributions of all the stakeholders.

He also mentioned that the presence of all stakeholders is of high importance because the meeting is also about social aspects regarded to the project and the collaboration of all stakeholders in giving the relevant information will lead to good results of the study.

The Presentation of the study

After the intervention of the Public works Director, the speech was given to the study team to proceed with the presentation of the study.

The presentation was made in English, with translation to Portuguese and to Makuwa, the local language.

The Concerns of the participants was as follows:

1. What will happen to people who are going to loose their properties.
2. All the people with “machambas” in the road reserve zone should be informed to abandon the area before the harvest period;

3. What is the extension of the roads reserve zone that people should avoid to occupy;
4. The new road will be paved?
5. The new alignment of the road will also pass through the railway like the present one?

After the intervention of the participants, the provincial director has stressed once again the importance of giving the relevant information like sacred trees, cultural heritage, graveyards, animal passing zones (elephants, lions, etc)

The participants were asked to spread the information for all the communities about the project and about the discussions on the meeting. They were also asked to be aware of the new constructions after the project has started.

One of the participants has mentioned one case of elephants coming from Gilé to Mecuburi, but it happened just once.

RIBAUE DISTRICT (7th November, 2006)

Major Participants:

Local leaders;

Suburb leaders;

Religious Leaders;

In this district the participants were also concerned about the people who may be displaced when the project starts and they have requested that people should be advised before the construction.

The Ribaué Administrative Post head said that it would be better to increase the width of the road, and he also sown his concern about the bad conditions of the road (“the road has too many potholes, we are asking to the project to consider the possibility of closing this potholes).

He has also called attention to the existing improved houses built with bricks and cement that has to be taken in consideration before resettlement of people living in those houses.

QUESTIONS TO BE TAKEN IN CONSIDERATION

About the places with cultural heritage, the leader of Mathai (one of the community leader) said: There is a problem with a strange wind close to one mountain.

The Suburb leader of Napasso said: “Last year there was a lion that used to be next to the road in October. Every year we see lions passing through, in October.

MALEMA DISTRICT (8th November, 2006)

Major Participants:

Judicial court of Malema ;

Administrative Post Head;

Local Leader;

Agriculture Director;

Traders;

Teachers;

Mozambican Women Organization Representative

QUESTIONS

- When does the construction will start?

QUESTIONS TO BE CONSIDERED

There is a graveyard in Lalaua/Mutuáli;

In the curve to Lúrio River, there is also a graveyard;

In the Chigulo zone, there are elephants;

In Malema, there are too many people next to the road;

In Mutuáli, there is a school and houses close to the road.

CUAMBA DISTRICT (8th November, 2008)

Participants: see attendance list with signature

Questions arose

1. Cabo António: We are pleased with this initiative and we promise that we will give you all the relevant information;
2. Agostinho, Adjunto do Cabo de Ncuapa: We are very happy and we promise to cooperate with the team;
3. Rafael- Cabo Chovela (In the border between Nampula and Niassa province) said: I am very happy with the project and I promise that I will take the message to my community;
4. Baptista Xavier- Lurio Administrative Post: Does the project affected people will be compensated?
5. There are too many graveyards along the road, what will be done regarding to this?
6. When the projects starts, it will be important to revise all the existing box culverts because many of them were doesn't respect the required standard;
7. Municipality: WE will collaborate with the team and ANE in the project implementation period
8. MICOA: From Lurio to Cuamba, there is no elephants;
9. From Cuamba to Ncuapa there are people making bricks, the team has to take this in consideration.

NAMPULA MUNICIPALITY (16th November, 2006)

Major Participants;

- Commission agent ANE – Nampula,
- Coordinating of the ANE,
- Heads of the administrative ranks;
- Councilmen;
- Leader communitarian

- Secretaries of the Quarters;
- Representative of the Municipal Chairman of the board;
- Consultants of JICA
- Technician and Engineers of the ANE;
- Community in general

QUESTIONS

- What one will become with the cultural heritage along the roads the sanctuaries, monuments?
- I think that number of accident will be increase due to speeding
- Some structures are located along the road now. Which is a main organization which has responsibility for compensation and relocation?
- Some graveyards and sanctuaries are located along the road. Such relocation of facilities and places are too difficult. Do you have some alternatives for it?

Answers clarified for the ANE:

- It is predicted that number of accident will be reduce due to improvement of dangerous points such as narrow road and shape corners. Main reason for accidents is speeding without respect traffic regulations.
- The compensations should be done by the Government. Concrete alignments with options will be reported in the next meeting.

Final aspects of the meeting:

It is not possible that it has one project of this dimension without that has impacts it is noticed along the roads, that people they construct houses for authorization of the state or for ignorance they are used to advantage of these lands for construct, for this reason exist must themselves be made a minute study to give to who of opportunist right indemnification, and it that when discovers that one exists project they do not make - to transfer for the owners of the houses.

As communities the long one of the way must be given chance in the participation of the construction of the road, this is part of the politics of development of the proper community, and, so thus the construction will give to greater value.

The population in the rainy weather must itself be prevented to make compensations to prevent the movement at this time.

To prevent constructions of tents, houses, crops to the long one of the not authorized roads and, for the culture zones they must be informed anticipated harvesting its products.

REPÚBLICA DE MOÇAMBIQUE
PROVÍNCIA DE NAMPULA
GOVERNO DO DISTRITO DE MECUBÚRI
SECRETARIA DO POSTO ADMINISTRATIVO DE NAMINA

LISTA NOMINAL DOS PARTICIPANTES DA REUNIÃO DE RAPALI

- ✓1- Johane Cumbane Castro, Chefe do Posto Administrativo
- ✓2- Augusto Tepanheque, Régulo Canhaua
- ✓3- Jorge João, Regulo Moquino
- ✓4- Representante do Régulo Mirapue
- 5- Luis Nicolau Vachaneque, Secretário do Bairro
- ✓6- José Selso Aivano, " " "
- ✓7- Tepanheque Névia, " " "
- ✓8- Afonso Sucar " " "
- ✓9- Zacarias Uawa, Secretário do Bairro
- ✓10- António Mualicohan, Chehe
- ✓11- José Zacarias, Ancião
- ✓12- Armando Parte, Secretário do Bairro
- ✓13- Fernando Sirola, Secretário do Bairro
- ✓14- Abreu Tomás Niquino, " " "
- 15- Alfredo Ernesto " " "
- ✓16- Aquimo, Chehe
- ✓17- Ernesto Motocoua, Ancião
- ✓18- Daniel Raça, Secretário do Bairro
- ✓19- Daniel Chacobe " " "
- ✓20- Jorge Paulo " " "
- ✓20 - Afonso Paial, Secretario do Bairro
- ✓22 - Paulo Murãesa, Chehe
- ✓23 - Armando Murereque, Ação

Namina, aos 6 de Novembro de 2006.-



/Técnico Superior de Adm/ção Pública N2/

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Nampula-Cuamba Road Feasibility Study - Stakeholder Meeting -

Vonuc Nampula District / RAPALE

Date 6th Nov. 2006

List of Attendance			
No	Nome	Posição	Contacto
No	NAME	POSITION	CONTACT NUMBER (CELL)
1	Luis Ernesto	Regulo - Anchiã -	
2	Eduardo Fernaldo	Cabo de Muzica Anchiã	
3	Amade Carroura	Lider Comunitario	
4	Estevão Matias	Lider Comunitario	
5	Carlos Mapueda	Lider Comunitario	
6	Trasicio Mutamuro	Cabo	
7	Wicas Manuel	Lider Comunitario	
8	Mamuel Gialop	Lider de massa	
9	Marcelino Joia	Lider - Muthinga	
10	Mariane da Silva	Lider - Dourado	
11	Grigias Rufina	Cabo	Muito malama
12	Alfredo C. Machoie	Chefe de Equipa Técnica	826737660
13	Bartolomeu Jo. Yecatebeite	Tecnico OPH no distrito	825813134
14	Mariane H. de Jesus	Tecnico OPH - Distrito	
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Nampula-Cuamba Road Feasibility Study - Stakeholder Meeting -

Venue RAPALE

Date 6th Nov. 2006

No	Nome NAME	Posição POSITION	Contacto CONTACT NUMBER (CELL)
1	Adriano R. Muetira	Chefe de Posto	Rapale Sede
2	Latifo V. Salimio	Chefe do Posto	826471720
3	Johannell. Castro	Chefe do Posto Adm	Nammina/Sede
4	Paulo Mecanilae	cheke	Nammina
5	Jose Celso Aivana	Secretario do Bairro	Nammina
6	Antonio Munali estor	Cheke	Nammina
7	Augusto Teponlegu	Regulo	Nammina
8	Daniel Jacobo	Secretaria - Bairro	Nammina
9	Armando Porto	Secretaria - Bairro	Nammina
10	Teponlegu Neria	Secretaria - Bairro	Nammina
11	José Albino António	Cheke	Nammina
12	Adonso Lucas	Secretaria - Bairro	Nammina
13	Afonso Paial	Secretaria do Bairro	Nammina
14	Jorge Fausto	Secretario B	Nammina
15	Alcides L. S.	Secretaria Bairro	Nammina
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Nampula-Cuamba Road Feasibility Study - Stakeholder Meeting -

Venue RAPALE

Date 6th Nov. 2006

No	Nome	Posição	Contacto
No	NAME	POSITION	CONTACT NUMBER (CELL)
1	Afonso E. Miranda	CHEFE DO POSTO	82.4014349
2	Dauda Mussa	Chefe do Posto	829197030
3	Jose Zacarias	Pastor	Namima
4	João João	Regulo Namima	
5	Abreu Tomas Niquino	Secretario do Bairro	Namima
6	Fernando Sirela	Secretario do Bairro	Namima
7	Ernesto Motuca	conselheiro do Bairro	Namima
8	Daniel	praça	Namima
9	Afonso Avarague	animador	Namima
10	Rogal Vaino	Secretario do Bairro	Namima
11	António Melora	Dir. de Prov. OPA	82804720
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Nampula-Cuamba Road Feasibility Study - Stakeholder Meeting -

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Venue RIBAUE

Date 7th Nov. 2006

ASSINATURA

List of Attendance

~~Sign~~ ~~ATT~~

No	NAME	POSITION	CONTACT NUMBER (CELL)
1	Ernesto Manuel	cabo de Terra	817
2	Eugenio Perreira	cabo de terra	Eugenio
3	Manuel ZIMBENS	cabo de Terra	Manuel
4	Alberto Pereira	cabo de Terra	Alberto
5	Cabriel Cesarim	Secretario	Cabriel
6	Eduardo Campa	Secretario	Eduardo
7	Jorge Nabussu	Chefe de pesquisa	
8	Manuel Correia	delegado de mesa	
9	JOAQUIM Colete	Adjuvado do	Secretario
10	Bernardo J. Nunes	C/Localidade Navi	Junco
11	Samuel Maneluz	C/Localid. Patlora	77
12	Samuel C. Nhamo	C/loc. Nhamo	Manuel
13	GUILLERMINA PEDRO	chefe do Posto IAP4	Junco
14	Francisco dos Santos	Secretario de	Quithela
15	Alberto Vachegu	2º Secretario	Quithela
16	Mario Vachegu	chefe	Quithela
17	Rafael Lamogem	Cabo de terra	Varamcha
18	Alberto Rocherim	Secretario	Moliflha "B"
19	CARLOS ASSON	selula	Moliflha "A"
20	António P. João	Selula	Murrapauia "A"
21	Rafael Cheli	secretario de mesa	B. NOVO
22	Constantino Colete	Secretario	Junta

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Nampula-Cuamba Road Feasibility Study - Stakeholder Meeting -

Venue BARALE RIBAVE

Date 7th Nov. 2006

No	Nome	Posição	Assinatura
No	NAME	POSITION	CONTACT NUMBER (CELL)
1	Alto Zito Mungo	líder	Namiconha
2	gacinto geral	Regulador	Retace
3	Goncalves Francisco	Líder de	Mathatya
4	Pedro Gasolina	Líder Comunitário	Mestizela
5	Zhao James	Líder Comunitário	Naze
6	Jooceim S. Simone	Secretário Municipal	Huatala
7	Mário Baptista	1 ^o Secretário	Mathatya
8	Bernardo Chave	Secretário de Saúde	Saua-Saua
9	Rodrigues Francisco	Cabo de Terra	Saua-Saua
10	Samuel António	Chefe de Povoação	Napasso
11	JOSÉ Alberto	Secretário de Napasso	Napasso
12	Ernesto Mussaruni	Secretário Municipal	Namiconha
13	Bastola Salumede	Cabo de Terra	Modipile
14	Joo Namud	Secretário	Nachilapa
15	Feliciano Pedro	Secretário	Namiconha
16	Agostinho Viago	Secretário Bairro	Hansoace
17	Fernando PRivo	Secretário	Modipile
18	Julio Suetiro	Secretário	Nachilapa
19	Celestino Paulo	Assunto Social	Nachilapa
20	Daniel Ernesto	Chefe da Povoação	Nachilapa

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Nampula-Cuamba Road Feasibility Study - Stakeholder Meeting -

Venue Ribaue

Date 7th Nov. 2006

No	NAME	POSITION	CONTACT NUMBER (CELL)
1	Mário João	Secretário	Napass
2	Mário Afonso	camponês	gestor
3	Armando Mankhika	Chefe Gabinete	Construção Administrativa
4	Carlo de Sousa	Dir. Dist. Plan. e Infra	8255 93450
5	João Manuel da Costa	Secretário de Administração	
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Nampula-Cuamba Road Feasibility Study - Stakeholder Meeting -

Venue NALEMA

Date 8th Nov. 2006

List of Attendance			
No	NAME	POSITION	CONTACT NUMBER (CELL)
1	Jeliani Macule	Juiz Residente TJD	Tribunal - Malau
2	AMIRIO FERREIRA	Supervisor TDM - mca	82 9232260
3	Pacia Tenaba Nave	Directora EPOCabela	
4	Domício Frederico	chefe de posto Medica	82 9632650
5	Luís Alimissia	membro do partido	Frelimo
6	Fernando Alupi	comerciante	
7	António Tábua	Regulo - Avava	
8	João Pereira	comerciante	
9	LUIS ABEL MOFATE		comerciante
10	Daquiles Siana		Cafumes
11	Armando Almeida	chefe de sala de aula	musse
12	Agostinho Vinha	organizador	
13	Luís Mário Ipo		
14	Augusto Abreu	ADMINISTRADOR	Luís control
15	Maurício	Bautismo	Luís control
16	António Sobá		
17	Essefania	Jurista	
18	Jaime	Atacado	NE Godigunda
19	Luís	MANUEL	campesão NATALIA
20	Cecília Formafina	membro do partido	Frelimo

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Nampula-Cuamba Road Feasibility Study - Stakeholder Meeting -

Venue MALEMA

Date 8th Nov. 2006

No	NAME	POSITION	CONTACT NUMBER (CELL)
1	Antonio Sergio Pinto	Admin ^o / Eng ^o	
2	Nelson Pinheiro	TECNICO	820125820
3	Leociano Cassalupa		
4	Gissa Malla		
5	Bernardino Lapereira	D.Agricultura Malema	824201331
6	Mutuali gaidade Alcosobalar		
7	Henrique Amunonyo Humana		
8	AGOSTINHO ABALDO	NAMECUNA	
9	Francisco de Jesus Filadelfo		
10	DANIEL Nampula	COMERCANTE	823845830
11	Manuel Antonio	Ketacusee	
12	Jaime Paulo Silva	Sec. Permanente	825875394
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Nampula-Cuamba Road Feasibility Study - Stakeholder Meeting -

Venue MALEMA

Date 8th Nov. 2006

No	NAME	POSITION	CONTACT NUMBER (CELL)
1	FERNANDO SARAIVA	CONSERVADOR	826439580
2	JONSECA B. SARAIVA	COMERCIANTE	825906276
3	Eusébio Francisco	Parten da Imunio	829088980
4	Abdul Alhuda	Comerciante	
5	Goncalves Leite	comerciante	
6	Jonjando	Empresario	
7	Xavier Pacheco	carpinteiro	M. P. P.
8	crisina matens	secretaria F. A. M.	
9	Angelina	Manuel Mucopa	
10	Nammosa	leitor Secretario de M. P. P.	
11	Júlio Albino	chef da localidade de Nasic	
12	Estanislau Francisco	Pregado Nampula	
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Nampula-Cuamba Road Feasibility Study - Stakeholder Meeting -

Venue CUAMBA

Date 8th NOV. 2006

No	Nome	Posição	Assinatura
No	NAME	POSITION	CONTACT NUMBER (CELL)
1	Francisco A. Simbine	Coordenador	823269620
2	Adelino Chuau	chefe Bonongue	Administrador
3	Bartolomeu Xavier	chefe cumbrina	administrador
4	Miguel's Rapade	chefe mucoma	administrador
5	Fabiano José	chefe Macaue	Administrador
6	Rafael Jesus	Cabo Chetola	Administrador
7	Alberto Usseuane	Regedor Hececa	825279255
8	Batone Sencuá	Cabo	Hececa
9	Agostinho Assibo	Adjunto Batone	Hececa
10	Maeca	Chefe	Hececa
11	Uniare	Chefe	Nampula
12	André Simão	SPD	827082940
13	Jaime Lourenço	Director, Instituto N.A. SIDA	825964894
14	João Mutipia	?/ Director disto Agrícola	cuamba
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Nampula-Cuamba Road Feasibility Study - Stakeholder Meeting -

Venue CUAMBA

Date 8th Nov. 2006

No	NAME	POSITION	CONTACT NUMBER (CELL)
1	Eusébio SAÍSSO	CHEFE do Posto Local	[Signature]
2	MARIANA JUÍSSA	RAINHA	MARIANA JUÍSSA
3	Maurício Félix Quilica	estatística	[Signature]
4	Alberto Frenco	CH. M. M. E.	[Signature]
5	Estevão Jonas Mastala	Técnico do R.N.C.	[Signature]
6	ALEXANDRE P. JOSÉ	MEN. AMBIENTE	[Signature]
7	Umberto B. JSSP	D. D. I. C. - C. U. B.	[Signature]
8	Patrício Paulo	IGOPH Curupa	[Signature]
9	Diogo António Teta	DRM e C. S. A.	[Signature]
10	Helino Luqui	Comun. P. R. T.	[Signature]
11	Maurício Teta	cola	[Signature]
12	Teresa Jorge Vinturo	D. D. A. Aud. Com. Local	[Signature]
13	ARLINDO C. MACHADO	D. AREA FISCAL	[Signature]
14	Francoise Ines Alves	D. S. E.	[Signature]
15	Poncio Cavete	Pres. Ass. Eleitoral	[Signature]
16	Maria da Graça A.	D. D. Juventude e D.	[Signature]
17	Diogo Vitor	PM	[Signature]
18	Luciano Filipe	A. B. E. C.	[Signature]
19	Maria A. F. Lázaro	Vereador C. M. C.	[Signature]
20	Steven Tapira	Repórter	[Signature]

Nampula-Cuamba Road Feasibility Study - Stakeholder Meeting -

Venue Nampula Municipality

Date 16th November 2006

No	NAME	Posição POSITION	ASSINATURA CONTACT NUMBER (CELL)
1	Atumane Victor	chefe da S. Liberdade Central	876837200
2	João Maliciana	secretário do Bairro	Muhala 829501020
3	Samuel Fagundes	secretário de bairro	Furrapanina
4	António M. Fidalgo	representante chefe	Nampulo
5	António Wagera	representante do chefe do posto de Muhala	825401258
6	Rafael C. António	chefe do posto Natikini	Natikini 825278374
7	Francisco A. S. João	chefe do posto Coastal	827434090
8	Paul A. S. António	chefe do posto 255	828337070
9	gestora g. Santos	chefe do posto Muhala	824374500
10	Fernando Mucaguiça	Secret. subst. B/Município	825279321
11	Francisco Coque	Líder Natikini	826451470
12	Josefa C. V. António	Vereadora	826015360
13	Tomás Naveira	Vereador	825806129
14	Bartholomeu Rafael	Vereador	825281182
15	Luísa F. António	Vereadora	823834750
16	Bláscas F. António	Vereador	826685530
17	Fátima Estrela	Vereadora	827170730
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Nampula-Cuamba Road Feasibility Study - Stakeholder Meeting -

Venue Nampula Municipality

Date 16/11/2006

No	NAME	POSICÃO POSITION	ASSINATURA CONTACT NUMBER (CELL)
1	Sociedade Alkalala	chefe D. B. B. B.	[Signature]
2	Maria Luísa Bile	11 11 Limocino	Maria Bile
3	Madalena Simola	11 11 25 de Setembro	[Signature]
4	Júlio Mualala	11 11 Liberdade	[Signature]
5	Clara Tris	Estudante UCM	Clara Tris
6	CESÓDIO VOABIL	ASSESSOR TÉCNICO DPCA	[Signature]
7	Francisco Mucurusa	Intendente Bairro Liberdade	[Signature]
8	Leandro Tocaati	11 00 B. Militar	Melhor Tocaati
9	Manuel Bastião Mualala	Ch. Director Soc. Militar	[Signature]
10	ALDO LOPES LAVIEIRA	SECRETÁRIO DO B. DE MAIO	Aldo Lopes Lavieira
11	Ernesto J. Namuato	Sec. Técnico do B. Mualala	Ernesto J. Namuato
12	Carla R. Baga	Chefe do B. Presidente	[Signature]
13	Rafael N. Tarcisio	Vendedor I.A.7	[Signature]
14	José Sabonete	Secretário do Bairro Mualala	[Signature]
15	Zefrino M. Gnlhe	D. S. Adm Mualala	[Signature]
16	Alexandre R. Costa	Delegado	[Signature]
17	Agostinho V. Nolutu	Coordenador - ANE	[Signature]
18	Angélica Balato	Contribuinte ANE	Balato
19	António Moleira	Membro Conselho	[Signature]
20	Deolinda Nunes	Coordenadora - ANE	Nunes

**EIA FOR THE REHABILITATION OF NAMPULA – CUAMBA ROAD (N13), IN
NAMPULA AND NIASSA PROVINCES**

Public Meeting - Nampula

Note

This meeting has been attended by local consultant the CONSULTEC which contracted with ANE regarding EIA activities. This meeting is the second stakeholder meeting in this JICA's feasibility study, and at the same time, prescribed preliminary public consultation based on the Mozambican EIA law.

Venue: NAMPULA – Municipal Council

Date: 20th August, 2007

Time: 08.30 h – 11:00 h

PARTICIPANTS:

PROPONENT: Bento Mualoja (Director of Public Works), Alexandre Nhaca (ANE Delegate), Angelina Balate (UASMA)

NAMPULA MUNICIPALITY: José Valente

CONSULTANTS: Madalena Dray, Vera Ribeiro

JICA: Hironori Kuroki

FACILITATOR: Madalena Dray

PARTICIPANTS: As per attached list.

WELCOMING

The Director of Public Works in Nampula welcomed the participants and gave a brief presentation about the purpose of the meeting and solicited the participation of all.

PRESENTATION CONSULTEC (Madalena Dray)

- Objectives of the meeting,
- Meeting's agenda
- Project definition
- Environmental Impact Assessment Process & Public Participation Process
- Project Description
- Project's Timeframe
- Project's alternatives
- Project details
- Construction phase
- Post-construction phase
- Description of the area's environment: Physical, Biological and Socio-economic environment
- Main potential impacts during construction phase & post-construction phase
- Interested & affected parties
- Objectives of the meeting

After the presentation, the facilitator opened the meeting for comments and/or questions:

Questions / suggestions	Made by
<p>1. This project seems to have taken into consideration few alternatives. The question about road safety, especially within towns, should be taken into consideration, since the project did not include an alternative to bypass main towns.</p> <p>2. In the past, we have experienced many contractors who promise employment opportunities to local people, and then fail to pay them. The signing of contracts with local workforce should be implemented and enforced.</p>	<p>Augusto Ferro – Nampula District Administrator</p>
<p>3. Local committees shall be established, both by contractors and local</p>	<p>Madalena Dray -</p>

<p>communities, to ensure that contracts are signed and followed.</p>	<p>Consultec</p>
<p>4. The rehabilitation of this road will bring about not only an increase in traffic, but also crime rates are likely to increase. The Ministry of Interior should therefore be involved in the process.</p> <p>5. Programmes should be developed during pre-construction phase and post-construction phase in order to educate communities on possible impacts such as cultural clashes with workers, increase in HIV/AIDS, road safety and security in general, links to markets, etc.</p> <p>6. Communities should also be warned early on that they shall not build any more infrastructures or establish machambas (agricultural plots) along the road.</p> <p>7. What can be done in order for local communities to respect the 30m of ROW required along roads? This is a very important issue, as the speed limit will substantially increase with the road's rehabilitation, and hence the risk of accidents.</p> <p>8. In terms of resettlement, an information dissemination programme should be developed early on in the project, especially with possibly affected families, and this programme should be carried out in an integrated manner, together with the social services and other relevant institutions.</p>	<p>António Murserene - Executive Director OLIPA</p>
<p>9. First contacts with District Directorates have already been done, and later on there will be some work towards identifying and demarcating suitable areas for the establishment of markets and fares, as well as parking spots.</p> <p>We are only doing a preliminary study, but will later identify all possibly affected houses, machambas and other infrastructure. The executive project will be ready in 2008, and only then will we be able to identify the exact number of infrastructures that will be affected.</p> <p>We are now working with an aerial photograph of the area, which shall be distributed amongst district administrations and local leaders, in order to avoid the construction of more houses or the establishment of machambas along the road's ROW between now and when the project starts. This aerial photograph will also be used when working with the owners of possibly affected houses and agricultural plots.</p>	<p>Madalena Dray - Consultec</p>
<p>10. It is necessary to ensure the management of funds, as many projects in</p>	<p>Arnaldo Chalala -</p>

<p>this country are not followed due to the mismanagement of funds and lack of transparency.</p> <p>11. Due to the risk of increase in HIV/AIDS, I would suggest that a hospital unit be built on the camp site.</p> <p>12. In relation to noise and dust, will there be any mechanism to ensure that the generated dust will not negatively impact on people health as dust can generate several diseases as tuberculosis?</p> <p>13. There is usually agglomeration of people and markets around camping sites, and this should be controlled by introducing some form of policing.</p>	<p>Renamo Party</p>
<p>14. There are many dust controlling measures which are used both internationally and nationally such as water aspersion, especially during the dry season. For piles of sand, the measure taken includes planting grass to avoid dust emissions.</p>	<p>Madalena Dray -Consultec</p>
<p>15. There should be a strong coordination between the different municipal councils and districts for the resettlement process. Construction is due to start in 2009, so 2008 will be a preparatory year.</p> <p>16. There are lots of machambas and infrastructures within the 30m ROW, which is dangerous in terms of road safety. There is a need for us, government, to work towards the implementation of the law.</p> <p>17. In terms of sacred places, the administrators and municipal councils shall identify these areas, by working with local communities.</p> <p>18. Whenever possible, the road crossing main towns should be avoided. One of the reasons behind it being general disrespect for speed limits and hence increased accident rates.</p>	<p>Alexandre Nhaca – ANE delegate</p>
<p>19. The fact that the road will cross towns is a main worry, as this will not only be a national road but in fact a national corridor, where vehicle traffic will substantially increase to approximately 936 vehicles/day by 2026. The road will be the main route used for the export of agricultural products. Today, even with the small maintenance works carried out on the road, traffic is already high, and speeds are often above 100km/hr. Therefore, I believe that Alternative B should be implemented in town sections, and Alternative A should be left to areas outside major towns.</p>	<p>Provincial Director of Public Works - Nampula</p>

<p>20. Communities must be educated on road safety. There is an area just outside Malema which has a propensity for accidents. There are lots of domestic animals such as chickens, pigs and goats crossing the road. This situation should be carefully analysed and the Ministry of Interior should be involved in the process.</p>	
<p>22. ANE already has a programme of work for HIV/AIDS. This entails contracting local institutions which are already working on HIV/AIDS at the local level, so that they develop a programme with those communities that live within the road's area of influence.</p>	<p>Angelina Balate ANE - UASMA</p>
<p>23. I would like to know exactly when the road starts and when it ends. 24. The question of clearance width, as in Natiquir Locality even the Administrative post is within 5 metres from the road.</p>	<p>Camponês – Chief of Natiquir Administrative Post</p>
<p>25. Its important that each administrative post and municipal council have information regarding where the project starts and where it ends, as well as the project's timeframe and planned activities (at least information regarding the section of the road which falls within each administrative post/municipality). 26. Once people find out about compensations, they might start building or cultivating along the road in order to receive some form of compensation. It is therefore important to identify each infrastructure and machambas along the road as soon as possible. 27. Proper signalling will be needed, especially on those deviations which are usually areas where accident rates are high. 28. Would like to ask contractors to contract local labour whenever possible. 29. Roads should be built for the future, and not only for a period of 3 or 4 years.</p>	<p>José Valente - Municipal Council Nampula</p>
<p>30. The N13 starts at the crossing between National Road 1 towards Cuamba. 31. We need much more than 12 meters width clearence in urban areas for many reasons, one of them is that the number of vehicles is increasing, and this always happens faster in urban areas.</p>	<p>Alexandre Nhaca – ANE delegate</p>

<p>32. The Alternative A route would cause many inconveniences to Malema town. For example, the road ends exactly where the District Administration office is located. There will be no road safety, and therefore I agree that the project should consider another alternative for those areas inside towns.</p>	<p>Tadeu Mariano - Director of Economic Activities, Malema District Administration</p>
<p>33. I am worried about the rights of those farmers who will lose their agricultural plots, as well as food security. The farmers and their families will lose cultivated areas; therefore dissemination of information regarding the project should start immediately if not Today. I would like to ask the relevant people to come up with adequate methodologies to inform local families about the project.</p>	<p>Calisto Ribeiro - ORAM</p>
<p>34. ANE and JICA already have stipulated resettlement guidelines.</p> <p>These guidelines entail a survey of the houses, agricultural plots and other infrastructure located along the road, which will be done as soon as the executive project is complete, in 2008. A survey of all affected families will be followed, as well as a survey of all the crops and fruit trees. The latter will be then analysed according to the provincial compensation table, which stipulates prices for different crops and fruit trees. There will be a team working together affected families, and later on a Resettlement Action Plan will be implemented.</p> <p>It is however important to start as early as possible the process of information dissemination, so as to avoid further construction of houses or agricultural plots along the road.</p> <p>This process will be roughly detailed during 2008.</p>	<p>Madalena Dray - Consultec</p>
<p>35. It seems the study is based on two main alternatives, A and B, without any other proposed alternative. Both these alternatives seem to be very focused on the costs rather than on the benefits. Most of the roads in Mozambique were designed during the colonial period, and therefore served the interests of the then District Administrators, which included, amongst other things, information regarding visitors, proximity to the military school, etc. Today we live in a completely different context, and it is important to re-think the design of our roads. The rehabilitation of the road will bring</p>	<p>António Murserene - Executive Director OLIPA</p>

<p>about an increase in traffic, hence we should include the possibility of avoiding towns.</p>	
<p>36 Would like to know if anybody has information regarding the existence of mines in the areas along the road</p>	<p>Hironori Kuroki – JICA</p>
<p>37. Where does the road start and end? 38. Wouldn't it be better if we could use the road that goes through Rua Chave? If the road started in Rua Chave (5000 Road) instead, we could have two roads coming into Nampula town, therefore improving the traffic conditions.</p>	<p>Arnaldo Chalala - Renamo Party</p>
<p>38. If traffic congestion is bad in Trabalho Avenue, then the idea of using 5000 Road is a good one. However, this project has been designed a long time ago, and it has to start in FAINA. The Municipal council should look into possible alternatives, if they think this is necessary. We can then make a proposal, but I don not think it is a possible alternative. This is a national road, which is part of the Nacala Corridor, and the main worry now is to find a road design that may improve traffic in towns, but this alternative is not included. Perhaps with more discussions and suggestions, we could come up with another feasible alternative.</p>	<p>Alexandre Nhaca – ANE delegate</p>

Once all comments were made, the facilitator ended the meeting.

PUBLICIDADE

Sexta-feira
17 de Agosto de 2007

VENDA DE VIATURA

- Marca – Musso
- Matrícula – MMA-53-62
- Ano de fabrico – 1997
- Estado: Operacional

O carro poderá ser visto na Rua de Mukumbura, nº 434, das 12.30 às 14.00 horas.

Os interessados deverão apresentar as suas propostas até ao dia 24 de Agosto de 2007, por carta fechada com a seguinte indicação:

GAPI, SARL – Serviços Administrativos
Viatura MUSSO – Matrícula MMA-53-62.

12980

AVISO

Humula Limitada informa a todas as instituições e o público em geral o Furto de vendas a crédito n.ºs 651 a 700. Vendas a Dinheiro furtadas n.ºs Humula Limitada informa ainda que não tem responsabilidade pelo uso indevido das referidas vendas.

A Administração



Intermon Oxfam

ONG espanhola dedicada à cooperação internacional e para seu escritório em Maputo (Moçambique).

COORDENADOR DE ÁGUA E SANEAMENTO

- Titular – universitária ou equivalente em Engenharia, valor em Engenharia, Água, Higiene e Saneamento (Watsan) ou Meio Ambiente
- Experiência mínima de 2 anos em trabalhar na área de emergência ou pós-emergência em zonas rurais;
- Conhecimentos e experiência em Engenharia de Água (sistemas de água, assim como de análises e tratamento da mesma);
- Inglês bem falado e no mínimo razoavelmente escrito, e fluente
- Disponibilidade para ir ao terreno;
- Experiência em gestão de equipas;
- Capacidade para a comunicação. Sensibilidade às diferenças culturais em contextos culturais distintos;
- Conhecer e respeitar os princípios humanitários e este humanitário.

As pessoas interessadas podem enviar os "currículos" para humanitarianstaff@intermonoxfam.org, ou para Intermon Oxfam Maputo, indicando a referência "Ref. Coordenador Watsan N.º 1".

O prazo de apresentação da solicitação finalizará no próximo dia 20 de Agosto de 2007.

Só se contactarão os candidatos ou candidatas pré-seleccionados a partir da data do término de apresentação das candidaturas.

II Na Intermon Oxfam pautamos pelo princípio de equidade entre mulheres e homens em todos os postos.



**CONVITE
PROCESSO DE PARTICIPAÇÃO PÚBLICA**

O Governo de Moçambique, através da Administração Nacional de Estradas (ANE) pretende reabilitar a estrada entre Nampula e Cuamba (N13), nas Províncias de Nampula e Niassa respectivamente.

Neste âmbito, a ANE seleccionou a CONSULTTEC – Consultores Associados, para a realização do referido Estudo de Impacto Ambiental (EIA).

Assim, convidam-se pessoas interessadas a participar numa reunião pública de apresentação do projecto e EIA a realizar.

Data: 20 de Agosto de 2007 (Segunda-feira)

Hora: 08:30 - 11:30

Local: Nampula, Conselho Municipal

Para quaisquer esclarecimentos, por favor contactar Consulttec Lda., Eduardo Langa, através do endereço elanga@consulttec.co.mz ou pelo telefone +258 21 491555.

12743



Announcement for the Public Consultation on the Newspaper (17th August 2007)

Estudo de Impacto Ambiental (EIA) da estrada Nampula-Cuamba

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Estudo de Impacto Ambiental (EIA) da estrada Nampula-Cuamba

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Data: 20/08/2004

