

JAPAN INTERNATIONAL COOPERATION AGENCY (JICA)



MINISTRY OF PUBLIC WORKS REPUBLIC OF INDONESIA

DETAILED DESIGN STUDY OF NORTH JAVA CORRIDOR FLYOVER PROJECT IN THE REPUBLIC OF INDONESIA

FINAL REPORT EXECUTIVE SUMMARY

DECEMBER 2006





PREFACE

In response to a request from the Government of the Republic of Indonesia, the Government of Japan decided to conduct the "Detailed Design Study of North Java Corridor Flyover Project" and entrusted it to the Japan International Cooperation Agency (JICA).

JICA selected and dispatched a Study Team headed by Mr. Mitsuo Kiuchi of Katahira & Engineers International to the Republic of Indonesia, two times between October 2005 and December 2006.

The team held discussions with the officials concerned of the Ministry of Public Works as well as other officials concerned, and conducted field surveys in the Java Island. Upon returning to Japan, the team prepared this final report to summarize the results of the study.

I hope that this report will contribute to the development in the Republic of Indonesia, and to the enhancement of friendly relationship between our two countries.

Finally, I wish to express my sincere appreciation to the officials concerned of the Government and those concerned in the Republic of Indonesia for the close cooperation they extended to the study.

December 2006,

Kazuhisa MATSUOKA, Vice President Japan International Cooperation Agency Mr. Kazuhisa MATSUOKA, Vice President Japan International Cooperation Agency

December 2006

Dear Sir,

Letter of Transmittal

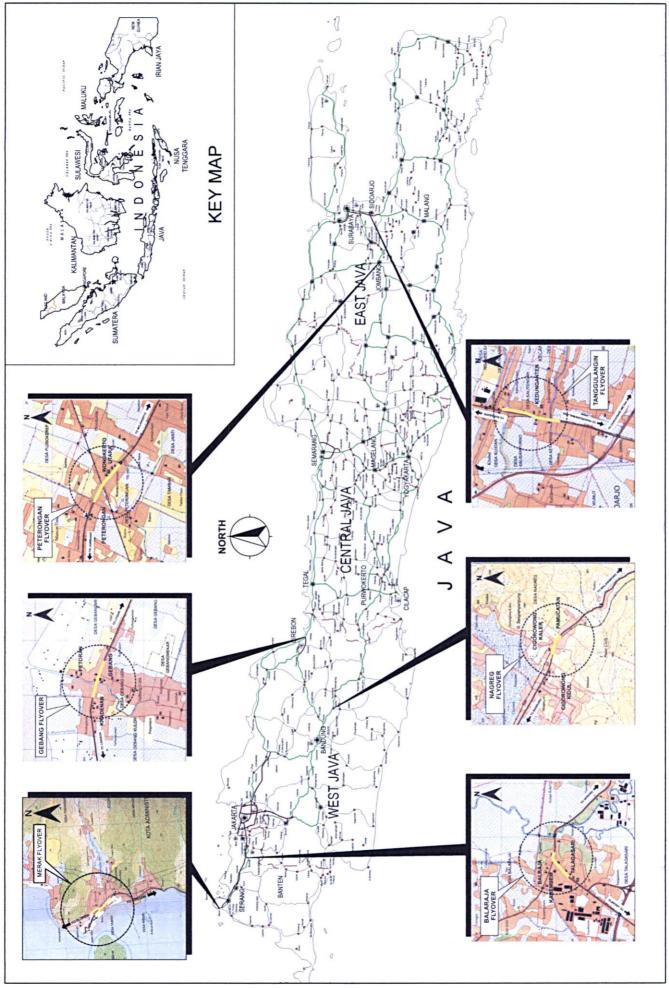
We are pleased to submit herewith the Final Report of the "Detailed Design Study of North Java Corridor Flyover Project". The report compiles the results of the Study and includes the advices and suggestions of the authorities concerned of the Government of Japan and your agency as well as the comments made by the Ministry of Public Works and other authorities concerned in the Republic of Indonesia.

The report studies the detailed design for flyovers at six priority locations along North Java Corridor in order to eliminate traffic bottlenecks. It presents the detailed design, cost estimate, construction planning, implementation planning and draft tender document under consideration of resettlement, ROW acquisition and environmental impact. We hope this report will contribute to the implementation of this Loan Project.

We wish to take this opportunity to express our sincere gratitude to your agency and the Ministry of Foreign Affairs. We also wish to express our deep gratitude to the Ministry of Public Works as well as other Governmental Agencies concerned in the Republic of Indonesia for the close cooperation and assistance extended to us during the Study.

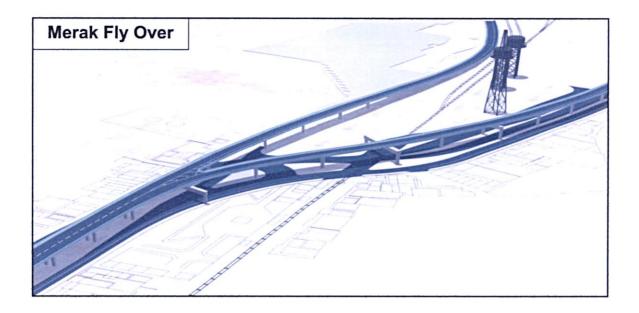
Very truly yours,

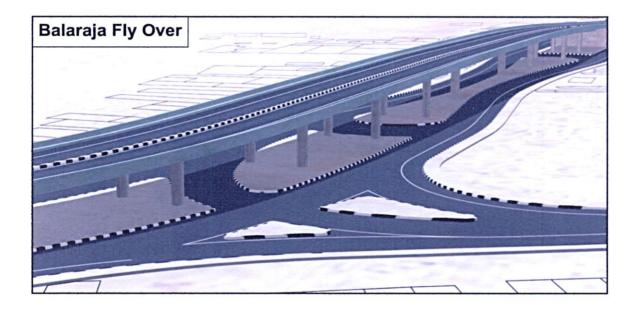
Mitsuo Kiuchi Team Leader, Detailed Design Study of North Java Corridor Flyover Project in the Republic of Indonesia

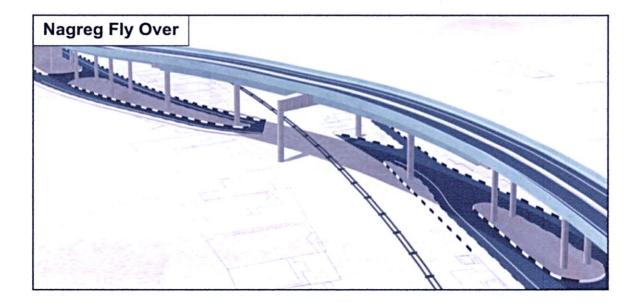


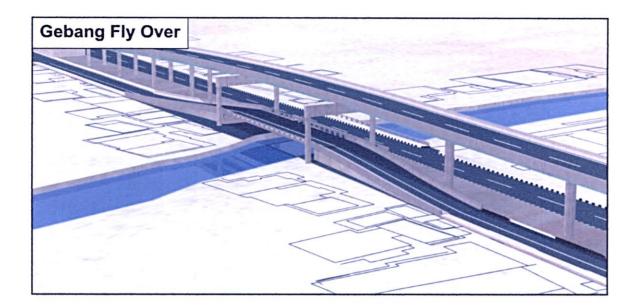
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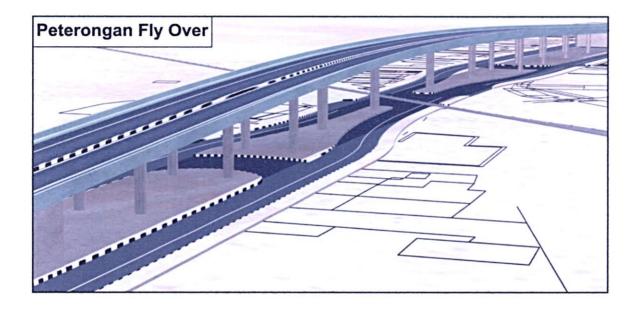
LOCATION MAP OF FLYOVERS FOR NORTH JAVA CORRIDOR PROJECT

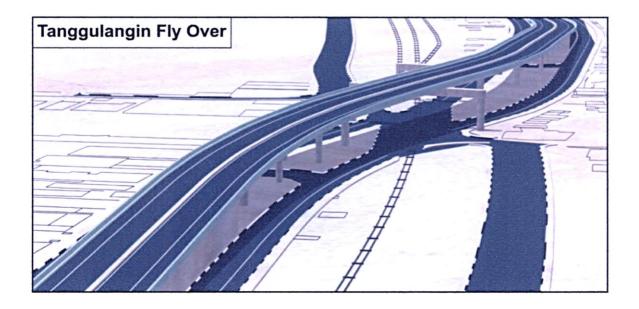












BACKGROUND

With the increasing traffic volume along North Java Corridor, transport efficiency is rapidly decreasing due to traffic bottlenecks particularly at urban intersections, railway crossings, etc. To cope with these problems, the Government of Indonesia has decided to construct flyovers at six priority locations (Merak, Balaraja, Nagreg, Gebang, Peterongan and Tanggulangin Flyovers). JBIC's STEP Loan was provided for the project.

OBJECTIVES OF THE STUDY

The objectives of the study are to undertake the detailed design based on the engineering surveys and to prepare the construction plan, cost estimates and draft tender documents.

PROJECT SITE CHARACTERISTICS

- The project site is located in the busy urban area with heavy traffic.
- The project site is narrow and a detour road is not available.
- Four flyovers are to be built over the railway.
- Gebang and Tanggulangin Flyovers are located at the deep soft ground area.
- Merak Flyover is located at loose sandy layer which would cause liquefaction during earthquake.
- All flyovers are located within seismic zone.

DESIGN CONCEPT

- a) The following Japanese technologies shall be adopted:
 - Fast construction method to minimize traffic congestion.
 - Efficient construction method applicable to narrow construction area under urban environment.
 - Construction method to realize efficient traffic management during construction.
 - Anti-earthquake technology.
 - Soft ground treatment technique and treatment against liquefaction.
 - Steel bridges for safe, fast and easy construction over the railway.
- b) To cope with external condition changes compared with the time of project appraisal by JBIC such as domestic construction price increase (1.4 times), Japan's steel material price increase (1.2 times) and Yen value depreciation (10%), cost reduction measures shall be focused in the detailed design.
- c) STEP Loan requirement on Japan portion which shall not be less than 30% of the total amount of contract shall be satisfied.
- d) Minimize land acquisition and relocation of families.

	-	Total Length (m)	Width of Flyover	Approach Section (m)	E	Bridge Length (m)
	Flyover	(Approach + Bridge)	(m) ໌	(Type of Embankment)	Total	Steel Bridge	PC Bridge
	National Road (Pulorida Side)	445.5	6.75	160.5 (MSE)	285.0	125.0	160.0
Merak	National Road (Jakarta Side)	262.5	9.00	202.5 (MSE)	60.0	-	60.0
	Terminal Exit	346.9	7.00	176.9 (MSE)	170.0	60.0	110.0
Balaraja	I	520.0	13.00	299.0 (MSE)	221.0	81.0	140.0
Nagreg		734.0	13.00	510.0 (MSE)	224.0	104.0	120.0
Gebang		760.0	9.00	375.0 (LWE)	385.0	225.0	160.0
Peterongan		615.0	13.00	353.0 (MSE)	262.0	82.0	180.0
Tanggu	langin	530.0	13.00	330.0 (LWE)	200.0	100.0	100.0
Total		4,213.9	-	2,406.9	1,807.0	777.0	1,030.0

SCOPE OF CIVIL WORKS

Note: MSE = Mechanically Stabilized Earth LWE = Light Weight Embankment

ESTIMATED COST

Flyover	Civil Works	Utility Relocation	Тах	Total
Merak	69.42	0.83	7.02	77.27
Balaraja	40.55	3.92	4.45	48.92
Nagreg	54.89	10.44	6.53	71.86
Gebang	62.63	0.69	6.33	69.65
Peterongan	46.63	2.25	4.89	53.77
Tanggulangin	54.23	0.51	5.47	60.21
Total (Million Yen)	328.35 (4,378)	18.64 (248)	34.69 (463)	381.68 (5,089)

Unit: Billion Rp

ECONOMIC EVALUATION RESULTS

Flyover	EIRR (%)	NPV (Billion Rp)	B/C Ratio
Merak	14.5%	17,1	1.30
Balaraja	23.0%	63,4	2.74
Nagreg	21.0%	71,1	2.33
Gebang	21.9%	80,8	2.56
Peterongan	17.3%	23,8	1.59
Tanggulangin	13.6%	8,1	1.18

Note: Discount rate at 12% per annum.

OPERATION AND EFFECT INDICATORS

The following operation and effect indicators were prepared and high positive effects were confirmed at each flyover:

- Operation Indicator Average Daily Traffic (veh/day)
 - Effect Indicators Travel speed (km/hr)
 - Travel time reduction (veh hr/day)
 - Travel cost reduction (1,000 Rp/day)
 - Max. queue length at railway crossing (m)

LOAN AMOUNT VS ESTIMATED COST

JBIC loan was originally planned to cover 100% of construction cost (excluding tax). Due to drastic construction price increases, Yen depreciation, etc., shortage of loan is inevitable.

Estimated Cost Without Tax (Million Yen)

Civil Works	4,293
Utility Relocation	248
(Note: Utility relocation was decided to be	
implemented using local fund.)	

Available Loan Amount (Million Yen)

Base Cost	2,993
Escalation	578
 Unused Balance of D/D 	200
Total	3,771

Shortage of loan is estimated at 522 Million yen (39.2 Billion Rp.)

PROJECT IMPLEMENTATION

The implementing agency is the Directorate General of Highways, Ministry of Public Works. The project is divided into three contract packages:

- Package 1: Merak and Balaraja Flyovers (Bantan Province)
- Package 2: Nagreg and Gebang Flyovers (West Java Province)
- Package 3: Peterongan and Tanggulangin Flyovers (East Java Province)

The Consultant for the construction supervision will be employed.

IMPLEMENTATION SCHEDULE

			2005	2006	2007	2008
Detailed	Design by	JICA				
Selection	of Supervi	sion Consultant				
Land Acc	quisition					
Selection	of Contrac	tor				
Utility Re	elocation by	Local Fund				
Consulta	ncy Service	es				
Construc	tion					
Annual Fund	JBIC Loan +	Consultancy Services			176	165
Require- ment	Local Fund	Construction of Flyover			1,928	2,794
(Million	Local Fund	Public Utility Relocation			273	-
Yen)		TOTAL			2,377	2,959

PREPARATION FOR IMPLEMENTATION

- Updated UPL and UKL were approved by respective local environmental agency, except Nagreg Flyover for which DGH is following up.
- Public hearing/socialization was undertaken at each flyover location with no major objection against the project.
- ROW acquisition for Balaraja and Gebang Flyovers has been completed. ROW acquisition of remaining four flyovers is ongoing and scheduled to be completed by the end of 2006.
- ROW acquisition at Nagreg Flyover is being delayed due to prolonged negotiation on land/compensation value between the land acquisition committee and the affected people.

CONCLUSION

The Project was evaluated technically, economically, financially and environmentally feasible.

Technical Feasibility: the project utilizes Japanese technologies in line with STEP Loan condition. Construction will be implemented by Japanese Contractor or Japanese Contractor in joint venture with Indonesian Contractor (Japanese contractor as a lead firm) who can execute the work efficiently. Japan portion is estimated to be 31.1% which satisfies STFP Loan requirement.

Economic Feasibility: all flyovers were evaluated economically feasible.

Financial Feasibility: although the project requires additional local counterpart fund which is, however, not extensive and manageable by DGH.

Environmental Feasibility: the project is not environmentally critical, thus EIA (AMDAL) is not required for this project. The project should be implemented and operated in accordance with requirements of UPL and UKL.

RECOMMENDATION

- The project must be implemented under the severe urban environment. Construction must be undertaken without major traffic disruption and be completed within the limited time frame. The construction plan prepared under this study should be carefully studied by contractors and supervision consultant and implemented.
- Various technologies were adopted in the study which can be applicable to other similar projects. Such technologies should be positively considered for wide application.
- Options were presented to cover shortage of loan. DGH should further study options and decision should be made as early as possible.
- PT. KAI required closure of an at-grade road at railway crossing, however, it should be done at later stage with proper provisions for local traffic and pedestrians.
- 5) Effect of mudflows from natural gas well near Tanggulangin Flyover should be closely monitored, particularly traffic flow changes, diverted to the national road from the toll road. The effects of mudflows are getting worse, DGH should decide whether construction of this flyover be implemented or not.

EXECUTIVE SUMMARY

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Exchange Rates Used in the Study:

US\$ 1.00 = Indonesian Rupiah 9,110 US\$ 1.00 = Japanese Yen 116 Japanese Yen 1.00 = Indonesian Rupiah 75.0

1. INTRODUCTION

1.1 BACKGROUND

North Java Corridor Road connects major industrial cities of Java Island such as Jakarta, Surabaya, Semarang, etc. and is vitally supporting the country's socioeconomic and industrial activities.

With the increasing traffic volume along North Java Road, transport efficiency is rapidly decreasing due to traffic bottlenecks formed particularly at intersections in urban sections, railway crossings and along urban sections where many street stalls are concentrated along the road sides, which are affecting sound socio-economic and industrial development.

To cope with the above problems, the Government of Indonesia (GOI) has decided to construct flyovers at six priority locations along North Java Corridor in order to eliminate traffic bottlenecks and to achieve smooth traffic movements. The project was appraised by the Japan Bank for International Cooperation (JBIC) and the loan agreement between GOI and JBIC applying the Special Term for Economic Partnership (STEP) was signed in March 2005.

In connection with the implementation of this STEP Loan, GOI requested the Government of Japan (GOJ) to provide the technical assistance for the detailed design of the project. In response to the request of GOI, GOJ has decided to conduct the Detailed Design Study of the North Java Corridor Flyover Project in Indonesia (the Study), and exchanged Notes Verbales with GOI concerning implementation of the Study. Accordingly, the Japan International Cooperation Agency (JICA), the official agency responsible for the implementation of the technical cooperation projects of GOJ decided to undertake the Study in close cooperation with concerned authorities of GOI.

On the part of GOI, the Directorate General of Highways (DGH), Ministry of Public Works acted as the counterpart agency to the Japanese study team and as the coordinating body in relation with other concerned governmental and non-governmental organizations for the smooth implementation of the Study.

1.2 OBJECTIVES OF THE STUDY

The objectives of the Study are:

- 1) To review previous studies and plans related to the project, analyze the most effective and efficient roads development of the project,
- 2) To carry out necessary engineering surveys,
- 3) To complete a detailed design of execution of the project,
- 4) To carry out construction planning and cost estimate, and
- 5) To prepare draft tender documents for execution of the project.

1.3 STUDY AREA

The study area shall cover the following construction sites of flyover along main roads in Java Island (refer to the location map):

Banten Province	West Java Province	East Java Province
Merak FlyoverBaralaja Flyover	Nagreg FlyoverGebang Flyover	Peterongan FlyoverTanggulangin Flyover

1.4 COMPOSITION OF REPORT

The final report is organized with the following:

- Executive Summary
- Main Text
- Drawings
- Draft PQ and Tender Documents
- Design Analysis Report
- Quantity Calculations Report
- Cost Analysis Report

2. OBJECTIVE OF THE PROJECT

The objective of the project defined by the Loan Agreement is

"To provide flyovers as the most appropriate countermeasures to achieve sound improvement of road transportation network and substantial enhancement of physical distribution along North Java Corridor and in the inland areas of Java Island for vitalization of socio-economic activities in the pertinent regions through the establishment of smooth and reliable traffic network."

3. TRAFFIC AND ENGINEERING SURVEYS UNDERTAKEN

3.1 TRAFFIC SURVEY

The following traffic surveys were undertaken at each flyover location for two (2) consecutive days:

- 24-hour Traffic Count
- 14-hour Roadside OD Survey
- 14-hour Intersection Traffic Count
- Travel Speed Survey
- Vehicle Queue Length at Railway Crossing

At Balaraja Flyover location, the following additional surveys were undertaken:

- U-turn Traffic Count
- U-turn Traffic OD Survey

3.2 ENGINEERING SURVEY

Topographic Survey covering the following was undertaken at each flyover location:

- Road Survey
- Structure Survey
- Public Utility Survey
- River Survey (Gebang Flyover only)

Geotechnical Survey at each flyover location consisting of the following was undertaken:

Boring	(96 holes, L=2,937m)
 Sampling 	(361 samples)
• SPT	(1,445)
 Laboratory Tests 	(2,252 tests)
 Soil Tests for Pavement Design 	(26 test pits)

Meteorological and hydrological data collection at each flyover location was undertaken.

4. PRESENT AND FUTURE TRAFFIC

4.1 PRESENT TRAFFIC CONDITION

Traffic survey results are summarized in **Table 4-1**. Existing traffic problems are summarized hereunder.

Merak Flyover

- Heavy roadside friction due to roadside business activities including illegal stalls/vendors within the road right-of-way, particularly at the opposite side of the Ferry Terminal Waiting Area, causing traffic congestion and disturbing traffic flow.
- Illegal parking of a lot of mini-taxis waiting for passengers within the carriageway of the national road.
- Traffic congestion at the intersection at the exit of the Ferry Terminal. Traffic from the exit of the Ferry Terminal concentrates at the intersection soon after a ferry boat arrives and conflicts with the traffic on the national road. The intersection is not channelized, neither signalized, which aggravates traffic congestion.
- Traffic queue is formed at the railway crossing during train passing (maximum queue length is 115m at Pulorida side).
- Due to above conditions, travel speed of this section is reduced to 19.5km/hour from 35km/hour of adjacent section.

Balaraja Flyover

- Heavy local traffic concentrates at this section.
- Heavy roadside friction due to roadside business activities.
- At the intersection between the national road and the intersecting road going to Kresek, right turn from the intersecting road to the national road is prohibited (or closed). Right turn traffic utilizes U-turn slot along the national road where traffic on the national road is heavily disturbed.

			Daily Traffi	c Volume (4-wheel or	more) (Boi	Daily Traffic Volume (4-wheel or more) (Both Direction)	_	Peak Hour	Daily Traffic Volume (3	Traffic	Traffic Characteristics By Direction	cs By Direc	tion			Travel Speed (km/hr)	ed (km/hr)			Rai	Railway Crossing	6u
				Mini					Ratio	wheels or	Through	Ľ	ocal Traffic		Morn	ina	Davi	time	Eve	venina	No. of	Queue	ne
		Car Jeep	Pick-up	Bus (oplet)	Medium Bus	Large Bus	Truck Trailer	Total	(%) (By Direction)	less) (Both Direction)	Traffic	4-wheels or more		Total	Flyover Section		Flyover Section	Outside F.O. Section		Outside F.O. Section	Train Passing	Length (m) Max Ave	(m) Average
	Pulorida side	1,306	729	2,016	180	101	1,563	5,895	10 6	E 162	2,080	878	002 C	7 86 7	0 01	25.0	100	2E 4	10.6	3E 0		11	02
Merak	(or from Pulorida)	(22%)	(12%)	(34%)	(3%)	(2%)	(27%)	(100%)	0.01	001 fr	(20%)	(%0£)	2,103	100 %	6.6	6.00	02	tion	19.0	0.00	ų	2	8
	Cilegon	4,558	1,633	5,888	581	1,958	4,410	19,028	7 3	10,100	2,344	663	7 267	2 26.0	1	110	0.00	26.0	2.00	7 30	>	6	ę
	(or from Cilegon)	(24%)	(%6)	(31%)	(3%)	(10%)	(23%)	(100%)	t D	12, 103	(%02)	(%0£)	100'7	0000		t. t.	0.02	e 	20.1	1.00		6	f
	Selang side	2,083	1,552	8,443	355	523	2,112	15,068	ά	22 REQ	3,360	4,490	11 731	16 221	۳. ۲	732	57	02	80	с С			
Balaraia	(or from Selang)	(14%)	(10%)	(26%)	(2%)	(%E)	(14%)	(100%)	5	000	(43%)	(27%)	2		5	500	2	2	5	2			
	Tangerang side	2,091	1,591	4,527	466	523	2,240	11,438	0.7	18 000	2,100	3,640	001 11	14 760	81	C 1 C	81	100	80	73 1			
	(or from Tangerang)	(18%)	(14%)	(40%)	(4%)	(%)	(20%)	(100%)	ò	000.01	(37%)	(63%)	271.	n n n n n n n n n n n n n n n n n n n	ç F	7:1-7	ç F	1.00	0				
	Bandung side	7,487	2,879	3,481	88	1,362	3,688	18,985	7 4	8 805	5,920	3,660	4 307	7 967	23.0	49.0	100	56.6	23.0	0 07		430	25.8
Nadred	(or from Bandung)	(39%)	(15%)	(18%)	(%0)	(%2)	(19%)	(100%)	t	200	(62%)	(38%)	ioc'r	100'1	0.03	Die F		0.00	6.04	2	18	2	002
Ro Rep	Malangbong	5,765	2,752	1,755	839	1,295	3,304	15,710	с С	q 237	5,310	2,950	4 588	7 538	28.8 8	30 F	31.2	50 S	30.9	37 G	2	000	121
	(or from Malangbong)	(37%)	(18%)	(11%)	(2%)	(%8)	(21%)	(100%)	2	010	(64%)	(36%)	2000 t	2000	2	2	<u>i</u>	2.20	2.00	5		0	2
	Cirebon side	4,636	1,619	1,823	62	2,923	9,137	20,200	6.1	90C 0	U	9,840	3 474	13 211	27 Q	0 77 0	8 1/6	7 2 7	28 G	0.51			
Gehand	(or from Cirebon)	(23%)	(%8)	(%6)	(%0)	(14%)	(45%)	(100%)	ò	9,230	- -	(100%)	- 110	- 10,01	E: 77	D. #	0.42		0.02	P. 2			
B. 1999 0	Losari side	3,466	2,992	1,394	108	3,468	8,145	19,573	7.6	4 995	7,240	3,400	3 806	7 206	23.7	44.9	0.3.9	43.8	27.1	44.2			
	(or from Losari)	(18%)	(15%)	(%2)	(1%)	(18%)	(42%)	(100%)			(%89)	(32%)	0						:				
	Jombang side	5,686	2,213	298	68	1,356	5,339	15,529	7.8	18 533	5,370	2,360	8 877	11 187	9 92	46.5	27.2	50.4	27.4	48.7		270	08
Peterondan	(or from Jombang)	(37%)	(14%)	(%9)	(%0)	(%6)	(34%)	(100%)			(%69)	(31%)					1		:		31) i	8
	Mojokerto side	6,568	2,527	1,059	152	1,332	5,270	16,908	7.8	18.433	6,250	2,330	9.607	11.937	27.2	53.8	21.7	53.0	29.3	50.7	5	300	162
	(or from Mojokerto)	(39%)	(15%)	(%9)	(1%)	(%8)	(31%)	(100%)			(%22)	(27%)	<u>.</u>										
	Porong side	5,622	2,578	3,724	30	25	3,706	15,685	7 1	50.692	5,060	3,060	34 999	38.059	60.0	62.5	525	60.6	54.8	59.0		110	42
Tanoqulanoin	(or from Porong)	(36%)	(16%)	(24%)	(%0)	(%0)	(24%)	(100%)			(62%)	(38%)									28		!
	Sidoarjo side	5,679	2,753	3,802	54	25	3,656	15,969	0 5	66 702	4,900	2,750	70 20E	24 12E	40 E	67.2	V 6V	603	10.4	50.2	ì	16.0	E G
	(or from Sidoarjo)	(36%)	(17%)	(24%)	(%0)	(%0)	(23%)	(100%)	0.0	00,000	(64%)	(36%)	70,000	01,100	r. 0+	c.10	t.0	7.00	t.0.t	0.50		100	8

Table 4-1 SUMMARY OF TRAFFIC SURVEY RESULTS

- There is another U-turn slot along the national road where a turning radius is small, thus buses and tracks cannot make smooth U-turn which is severely affecting traffic on the national road.
- Illegal parking of a lot of mini-buses and mini-taxis waiting for passengers along the national road.
- Due to the effects of the above problems, travel speed at this section is reduced to 5 to 10km/hour from 30 to 35km/hour of the adjacent section.

Nagreg Flyover

- Heavy roadside friction due to vegetable/fruit stands within the road right-of-way.
- Traffic queue is formed at the railway crossing during the train passing (maximum queue length is 430m at Bandung side). Number of train passing is 18 times a day.
- Travel speed of this section is reduced to 24 to 30km/hour from 40 to 50km/hour of adjacent section.

Gebang Flyover

- A fishing port is located near the site. There are many stalls/vendors occupying shoulders and sometimes outer carriageway lane, which drastically reduce traffic capacity and disturb smooth traffic flow.
- Slow moving vehicles and pedestrians/shoppers are also causing heavy roadside friction.
- There is one T-shaped intersection accessing to the public market. Although traffic going to the public market is still light, this intersection will be a traffic bottleneck in the near future.
- Due to the effects of the above conditions, travel speed of th is section is reduced to 23 to 27km/hour from 44 to 45km/hour of the adjacent section.

Peterongan Flyover

- Due to roadside development and high composition of local traffic, travel speed of this section is reduced to 22 to 30 km/hour from 46 to 54 km/hour of adjacent section.
- Traffic queue is formed at the railway crossing during train passing (maximum queue length is 300m at Mojokerto side). Number of train passing is 31 times per day.

Tanggulangin Flyover

- Due to high composition of local traffic, particularly motorbikes, travel speed is slightly reduced to 43 to 60km/hour from 57 to 62km/hour of adjacent section.
- Traffic queue is formed at the railway crossing during train passing (maximum queue length is 160m at the Sidoarjo side). Number of train passing is 28 times per day.

4.2 FLYOVER TRAFFIC

During the OD survey, drivers were interviewed and asked whether they will stop within the flyover section or not. Those who answered "yes" are considered "local traffic", and those who answered "no" are considered "through traffic". It is assumed that "through traffic" will utilize a flyover and "local traffic" will utilize an at-grade road.

4.3 FUTURE TRAFFIC

By applying annual traffic growth rate by vehicle type which was used by the Feasibility Study, future traffic volume was estimated.

					Unit: veh/day
				Year	
			2005	2015	2025
	National Road	Flyover	-	3,123	4,413
Morok	National Road	At-grade	6,292	6,873	9,908
Merak	Ferry Terminal Exit	Flyover	-	3,189	4,231
	Road	At-grade	2,998	1,294	1,788
Delere	0	Flyover	-	8,446	12,120
Balaraj	d	At-grade	14,607	14,863	21,517
Nogroe		Flyover	-	17,599	24,519
Nagreg	J	At-grade	17,783	10,570	15,138
Geban	g	Flyover	-	11,488	16,448
(Cirebon-bound Direction)		At-grade	10,338	5,020	7,267
Dotoro	200	Flyover	-	18,125	25,458
Petero	nyan	At-grade	15,864	6,961	9,818
Tanga	Jongin	Flyover	-	15,359	21,665
ranggi	ulangin	At-grade	15,572	9,370	13,551

ESTIMATED FUTURE TRAFFIC VOLUME

4.4 LEVEL OF SERVICE OF EXISTING ROAD WITHOUT FLYOVER

Approximate year when traffic volume will reach to traffic capacity of the existing road in case of without flyover is summarized below.

Flyover	Approx. Year when Traffic Volume of Existing Road Reaches to Capacity
Merak (National Road, Pulorida Side)	2016
Balaraja	2015
Nagreg	2012
Gebang (Cirebon-bound Direction)	2012
Peterongan	2015
Tanggulangin	2014

4.5 NUMBER OF LANES REQUIRED

E	lyover	No. of	V/C Ratio of Flyover in	
		Flyover	At-grade	2025
	From Pulorida	1-lane 1-way	1-lane	0.43
Merak	From Pulorida after merging with Exit Ramp	2-lane 1-way	1-lane	0.58
From Tangerang		1-lane 1-way	2-lane	0.51
Balaraja	From Serang	1-lane 1-way	2-lane	0.63
Nagrog	From Bandung	1-lane 1-way	1-lane	0.85
Nagreg	From Malangbong	1-lane 1-way	1-lane	0.91
Gebang From Losari		2-lane 1-way	1-lane	0.80
Deteronar	From Morokerto	1-lane 1-way	1-lane	1.18
Peterongan	From Jombang	1-lane 1-way	1-lane	1.07
Topaqulopain	From Porong	1-lane 1-way	1-lane	0.95
Tanggulangin	From Sidoardjo	1-lane 1-way	1-lane	1.13

The number of lanes required is summarized below.

Traffic volume of Peterongan and Tanggulangin Flyovers will reach to its traffic capacity around year 2025. Widening of flyover will be required.

5. **PROJECT SITE SETTING**

The project site setting is summarized in **Table 5-1**.

6. DESIGN STANDARDS AND CRITERIA

1) Highway Design

The following Indonesian highway design standards and criteria were adopted:

- Standard Specifications for Geometric Design of Urban Roads, RSWI, T-14-2005
- Standard Specifications for Geometric Design of Urban Roads, 1992

In case that there are some lacking items or from the standpoint of economic consideration, other standards listed below were referred:

- A Policy on Geometric Design of Highways and Streets, 2004 (AASHTO)
- Road Structure Ordinance, Japan Road Association, 2004 (JRA)

 Table 6-1 shows geometric design standards of flyovers and service roads.

2) Intersection Design

The same design standards mentioned in (1) above were used.

SETTING	
PROJECT SITE	
TABLE 5-1	

		Existin	Existing Road	Deilway			Goological		Critical .
-	Flyover	Flyover Section	Adjacent Section	Crossing	Topography	Land Use	Condition	Right-of-Way	Underground Utilities
Merak	Pulorida Side	2-lane 2-way (13.95m)	2-lane 2-way (13.95m)	Yes	 Narrow coastal plain followed by mountain slope 	 Right side is commercial area Left side is Ferry Terminal Waiting Area 	 Liquefaction layer (6 ~ 8m thickness) 	 No action yet at the start of the Study 	 Water pipeline for the Power Plant
	Jakarta Side	4-lane Divided (26.0m)	4-lane Divided (26.0m)	1		 Commercial/residential area Vacant area 			
Balaraja		4-lane Divided (18.4m)	2-lane 2-way (12.0m)	1	 Mostly flat Jakarta side with slope of about 5% 	 Commercial/residential Industrial area near the flyover 	 Hard layer 8 ~ 14m from ground surface 	 New ROW acquired Standard 29.1m Narrow section 18.0m 	 Gas pipeline Many electrical, communication cables
Nagreg		2-lane 2-way (12.0m)	Bandung Side 4-lane (18.0m) Malambong Side 2-lane (12.0m)	Yes	 Bandung Side with slope of about 5% Flat after railway 	 Residential/commercial area Vegetable selling stalls 	 Hard layer 20~30m from ground surface 	 New ROW acquisition started Standard 29.1m Narrow section 18.0m 	Oil pipelines (2 lines)
Gebang		4-lane Divided (20.5m)	4-lane Divided (20.5m)		 Flat About 1km from the sea 	 Residential commercial area Many vendors along the road 	 Soft ground with 10~15m thickness 	 Cirebon-bound direction completed with 13.3~16.0m in width 	Water pipeline
Peterongan	ıgan	4-lane Divided (15.5m)	4-lane Divided (15.5m)	Yes	• Flat	 Residential/commercial area 	 Hard layer 14~16m from ground surface 	 No action yet at the start of the Study 	Water pipelines
Tanggulangin	llangin	4-lane Divided (25.0m)	4-lane Divided (25.0m)	Yes	• Flat	 Residential/commercial area One side is railway land 	 Soft ground with 30~32m thickness 	 No action yet at the start of the Study 	Water pipelines

Road Function Road Function Design Speed based on Existing Alignment Minimum Radius of Curvature : Rmin Existing Alignment (Based on SAPROF Drawing) Existing Alignment (Based on SAPROF Drawing) Existing (Based on Saprof Ore side) Minimum Length of Horizontal Curve (Rmin) (Based Ore Eadopted for Flower Minimum (Midth (Based Ore Eadopted for Flower Minimum (Midth (Based Ore Eadopted for Flower Crest					balal aja	nagreg	Gebang	Peterongan	rariggurarigir
Design Speed based on Existing Alignment Minimum Radius of Curvature : Rmin Minimum Radius of Curvature : Rmin (Based on SAPROF Drawing) Design Vehicle Type (Based on SAPROF Drawing) Design Vehicle Type (Construction) Design Vehicle Type (Based on SAPROF Drawing) Design Vehicle Type (Construction) Design Speed (Vr) (Number of Lane Total Flyover Width (Construction) Total Roadway Width (Construction) Median Median Marginal Strip (On (Construction) Marginal Strip (On (Construction) Cross Slope (Minimum Length of Horizontal Curve (Lh Minimum (em Minimum Spiral Curve Length of Grade (Loc) Stopping Sight Distance (Ss) (Softical Length of Grade (Loc) Stopping Sight Distance (Ss) (Softical Length of Pavement Design Speed (Vr) (Diminum Radius of Vertical Curve Design Speed (Vr) (Critical Length of Grade (Loc) Stopping Sight Distance (Ss) (Dissign Speed (Vr) Design Speed (Vr) (Dissign Speed (Vr) Design Speed (Vr)			Arterial		Arterial	Arterial	Arterial	Arterial	Arterial
Minimum Radius of Curvature : Rmin (Based on SAPROF Drawing) (Based on SAPROF Drawing) (Based on SAPROF Drawing) (Based on SAPROF Drawing) (Construction)	km/hr		40		40	50	80	80	80
(Based on SAPROF Drawing) (Based on SAPROF Drawing) (General Design Vehicle Type Type of Pavement Type of Pavement Type of Pavement Design Vehicle Type Type of Pavement Design Vehicle Type Number of Lane Total Flyover Width Median Total Roadway Width Median Marginal Strip (One Minimum Length of Horizontal Curve (R Marginal Strip (One Minimum Length of Horizontal Curve (L Maximum (ema Vertical Minimum Radius of Horizontal Curve (L Minimum Length of Florizontal Curve (L Maximum (ema Super-elevation Maximum (ema Minimum Radius of Vertical Length (Ls mi Maximum (ema Super-elevation Maximum (ema Maximum (ema Super-elevation Maximum (ema Maximum (ema Super-elevation Maxi	ш		65		75	55	8	500	270
Design Vehicle Type Ceneral Type of Pavement Type of Pavement Type of Pavement Number of Lane Number of Lane Traffic Lane Width Trotal Roadway Width Traffic Lane Width Total Roadway Width Anotation Total Roadway Width Median Total Roadway Width Median Marginal Stip (One: Minimum Length of Horizontal Curve (Rm Minimum Spiral Curve Length (La min Minimum Spiral Curve Length (La min Maximum Grade (De adopted for Flyover Official Longth of Curve Length (La min Minimum Radius of Horizontal Curve Minimum Radius of Vertical Length (Crower Curve Minimum Radius of Vertical Curve Maximum Grade (Cmax) Super-elevation Maximum Grade (Curve Minimum Radius of Vertical Length (Crower Crower Minimum Radius of Vertical Curve Minimum Radius of Vertical Curve Design Speed (Vr) Type of Pavement Type of Pavement	ш		106		85	150	8	800	250
Gener Type of Pavement Gener Design Speed (Vr) Number of Lane Indian Indian Indian Indian Indian Median Marginal Strip (One: Minimum Length of Horizontal Curve (Rm Minimum Spiral Curve Length of Horizontal Curve (Rm Minimum Spiral Curve Length of Curve (Rm Minimum Spiral Curve Length of Curve (Rm Minimum Spiral Curve (Rm Minimum Radius of Horizontal Curve (Rm Minimum Radius of Portical Length of Grade (Cc) Stopping Sight Distance (Ss) Minimum Radius of Vertical Curve Index of Pavement Type of Pavement	=		WB-15		WB-15	WB-15	WB-15	WB-15	WB-15
G Design Speed (Vr) Mumber of Lane Number of Lane Number of Lane Total Flyover Width Total Roadway Width Total Roadway Width Total Roadway Width Total Width Median Marginal Strip (One: Minimum Length of Horizontal Curve (Rm Marginal Strip (One: Minimum Length of Horizontal Curve (Rm Marginal Strip (One: Minimum Spiral Curve Length of Horizontal Curve (Rm Maximum (emax) Super-elevation Runoff (L) Minimum Spiral Curve Length (Ls min Minimum Radius of Vertical Curve Cross Slope Minimum Radius of Vertical Curve Design Speed (Vr) Stopping Sight Distance (Ss) Minimum Radius of Vertical Curve Minimum Radius of Vertical Curve Design Speed (Vr) Design Speed (Vr) Type of Pavement	П		ACP		ACP	ACP	ACP	ACP	ACP
Image: Conservent of Lane Total Flyover Width Total Roadway Width Total Roadway Width Traffic Lane Width Total Note: Width Median Inaffic Lane Width Anninnum Length of Horizontal Curve (Rm Minimum Length of Horizontal Curve (In r Minimum Spiral Curve (Emm) Super-elevation Maximum (emax) Super-elevation Runoff (A) Minimum Spiral Curve Length of Grade (In r Minimum Spiral Curve Length of Cade (Gmax) Stopping Sight Distance (Ss) Minimum Radius of Vertical Curve Minimum Radius of Vertical Curve Concepting Sight Distance (Ss) Minimum Radius of Vertical Curve Minimum Radius of Vertical Curve Minimum Radius of Vertical Curve	km/hr		40		40	50	09	09	90
Total Flyover Width Total Roadway Width Total Roadway Width Traffic Lane Width Shoulder Width Shoulder Width Shoulder Width Shoulder Width Median Minimum Length of Horizontal Curve (Rr Minimum Length of Horizontal Curve (In r Minimum Spiral Curve (Entron) Super-elevation Runoff (A) Widening on Curve Minimum Spiral Curve Length of Grade (Lc) Stopping Sight Distance (Ss) Minimum Radius of Vertical Curve Coneiding Sight Distance (Ss) Minimum Radius of Vertical Curve Minimum Radius of Vertical Curve Design Speed (Vr) Type of Pavement Number of Lane	н	1 (One way) From Pulorida F	1 (One way) From Harbour	2 (One way) To Jakarta	2 (Two way)	2 (Two way)	2 (One way)	2 (Two way)	2 (Two way)
Total Roadway Width Traffic Lane Width Traffic Lane Width Shoulder Width Shoulder Width Median Minimum Radius of Horizontal Strip (One. Minimum Length of Horizontal Curve (Hr Minimum Spiral Curve (Gmax) General Stopping Sight Distance (Ss) Minimum Radius of Vertical Curve Minimum Radius of Vertical Curve	E	6.75	7.00	00.6	13.00	13.00	6.00	13.00	13.00
Closs 5 Traffic Lane Width Closs 5 Shoulder Width Median Anarginal Strip (One: Minimum Length of Horizontal Curve (Rm Minimum Length of Horizontal Curve (Rm Minimum Spiral Curve Length (I smin Maximum (emax) Super-elevation Maximum (emax) Super-elevation Maximum (emax) Super-elevation Maximum (emax) Super-elevation Maximum (emax) Side to be adopted for Flyover Critical Length of Grade (Lc) Stopping Sight Distance (Ss) Minimum Radius of Vertical Curve Minimum Radius of Vertical Curve Design Speed (Vr) Type of Pavement Type of Pavement		5.75	6.00	8.00	5.75 + 5.75	5.75 + 5.75	8.00	5.75 + 5.75	5.75 + 5.75
Citos Shoulder Width Median Total Width Median Total Width Minimum Radius of Horizontal Curve (Rm Minimum Radius of Horizontal Curve (Rm Minimum Length of Horizontal Curve (Rm Minimum Spiral Strip (One. Minimum Length of Horizontal Curve (Rm Maximum (emax Minimum Spiral Curve Length (Ls min Maximum Grade (Gmax) Minimum Spiral Curve Length (Ls min Minimum Grade (Gmax) Minimum Radius of Vertical Length of Grade (Lc C) Slopping Sight Distance (Ss) Minimum Radius of Vertical Curve Design Speed (Vr) Type of Pavement Type of Pavement	ш	3.50	3.50	7.00	3.50	3.50	7.00	3.50	3.50
Median Total Width Median Marginal Strip (One: Minimum Radius of Horizontal Curve (Rm Minimum Length of Horizontal Curve (Rm Minimum Length of Horizontal Curve (Rm Minimum Carlied (A) Nuper-elevation Maximum (emax Ninimum Spiral Curve Length of Horizontal Curve (Rm Maximum (emax Minimum Spiral Curve Length of Carle (Lo Super-elevation Minimum Spiral Curve Length of Grade (Co Critical Length of Grade (Lo Stopping Sight Distance (Ss) Minimum Radius of Vertical Curve Minimum Radius of Vertical Curve Minimum Radius of Vertical Curve Design Speed (Vr) Type of Pavement	ш	2.00	2.25	0.50	2.00	2.00	0.50	2.00	2.00
Marginal Strip (One: Minimum Radius of Horizontal Curve (Rm Minimum Length of Horizontal Curve (Rm Minimum Length of Horizontal Curve (Rm Minimum Length of Horizontal Curve (Rm Minimum Spiral Curve Length (Ls min Minimum Radius of Vertical Length of Grade (Cc) Stopping Sight Distance (Ss) Minimum Radius of Vertical Curve Design Speed (Vr) Type of Pavement	Е				1.00	1.00	-	1.00	1.00
Cross Slope Minimum Radius of Horizontal Curve (Rm Minimum Length of Horizontal Curve (Rm Minimum Spiral Curve Length (Ls min Minimum Radius of Vertical Length of Grade (Cr Stopping Sight Distance (Ss) Minimum Radius of Vertical Curve Design Speed (Vr) Type of Pavement	ш	0.25	0.25	0.25	0.25	0.25	0.25	0.25	0.25
Minimum Radius of Horizontal Curve (Rr Minimum Length of Horizontal Curve (Lh r Maximum Gradiel Curve Length (Ls min Minimum Spiral Curve Length (Ls min Maximum Grade (Gmax) Ordicatical Length of Curve Length (Ls min Maximum Grade (Gmax) Stopping Sight Distance (Ss) Minimum Radius of Vertical Curve Ordicatical Length of Grade (Lc) Stopping Sight Distance (Ss) Minimum Radius of Vertical Curve Design Speed (Vr) Type of Pavement	%		2.0		2.0	2.0	2.0	2.0	2.0
Minimum Length of Horizontal Curve (Lh r Super-elevation Maximum (emax Maximum (emax) Anoint Nunoff (A) Widening on Curve Runoff (A) Minimum Spiral Curve Length (Ls min Maximum Grade (Gmax) Critical Length of Grade (Lo Stopping Sight Distance (Ss) Minimum Radius of Vertical Curve Minimum Radius of Vertical Curve Design Speed (Vr) Type of Pavement Type of Pavement	ш		55		55	06	135	135	135
Super-elevation Maximum (emax Runoff (Δ) Aninimum Spiral Curve Length (Ls min Maximum Grade ($Gmax$) Oefficial Critical Length of Grade ($Cmax$) Stopping Sight Distance (Ss) Minimum Radius of Vertical Curve Design Speed (Vr) Type of Pavement Type of Pavement	ш		70		70	85	105	105	105
Hold Runoff (Δ) Hold Widening on Curve Widening on Curve Widening on Curve Minimum Spiral Curve Length (Ls min Maximum Grade (Gmax) Stopping Sight Distance (Ss) Stopping Sight Distance (Ss) Minimum Radius of Vertical Curve Design Speed (Vr) Design Speed (Vr) Type of Pavement	%		6.0		6.0	6.0	6.0	6.0	6.0
⊥ Widening on Curve Minimum Spiral Curve Length (Ls min Maximum Grade (Gmax) Grade to be adopted for Flyover Critical Length of Grade (Lc) Stopping Sight Distance (Ss) Minimum Radius of Vertical Curve Design Speed (Vr) Type of Pavement Number of Lane	Ш		1/143		1/143	1/150	1/167	1/167	1/167
Minimum Spiral Curve Length (Ls min Maximum Grade (Gmax) Öefficied Carade to be adopted for Flyover Critical Length of Grade (Lc) Stopping Sight Distance (Ss) Minimum Radius of Vertical Curve Design Speed (Vr) Type of Pavement Type of Pavement Number of Lane Number of Lane	Е	3) 0.25	0.00	0.00	3) 0.75	3) 0.5	0.00	0.00	3) 0.25
Maximum Grade (Gmax) Critical Length of Grade (Icc) Stopping Sight Distance (Ss) Minimum Radius of Vertical Curve Celeferal Design Speed (Vr) Type of Pavement Number of Lane Roadway Mirthh (One side)	E		22		22	28	33	33	33
Grade to be adopted for Flyover Oritical Length of Grade (Lc) Oritical Length of Grade (Lc) Stopping Sight Distance (Ss) Minimum Radius of Vertical Curve Image: Stopping Stoped (Vr) Type of Pavement Number of Lane Number of Lane	%		8.0		8.0	8.0	7.0	7.0	7.0
Official Length of Grade (Lc.) Official Length of Grade (Lc.) Stopping Stight Distance (Ss.) Minimum Radius of Vertical Curve Design Speed (Vr.) Type of Pavement Number of Lane Doction Number of Lane	%		5.0		5.0	5.0	5.0	5.0	5.0
Bit Stopping Sight Distance (Ss) Minimum Radius of Vertical Curve Design Speed (Vr) Design Speed (Vr) Number of Lane Dractwarv Mirthh (One side)	ш		400 (8.0 %)		400 (8.0 %)	400 (8.0 %)	400 (7.0 %)	400 (7.0 %)	400 (7.0 %)
Minimum Radius of Vertical Curve	E		50		50	65	85	85	85
Design Speed (Vr) Od Type of Pavement Od Number of Lane D Provider (Tope side)	εE		450 450		450 450	800	1,400	1,400	1,400
on Genera	hm/hr		UV		90 VV	07	ODO!	ODDI-	000/1
I9D UC	=		ACP		ACP	ACP	ACP	ACP	ACP
uc					2	2			D.
uo	"		-		2	-	-	1	-
ļ	E		5.50		6.00	5.50	$5.00 \sim 5.50$	5.50	5.50
26C	Е		3.50		3.00	3.50	3.50	3.50	3.50
	ш		2.00		0	2.00	$1.50 \sim 2.00$	2.00	2.00
Cro	ш		1.50		1.55	2.05	1.50	1.50	1.50
Cross Slope	%		2.00		2.00	2.00	2.00	2.00	2.00
R.O.W. Width m	E		12.5 ~ 27.6	-). 18.7 ~ 29.1	2). 19.1 ~ 29.1	1). 13.3 ~ 16.0	20.1 ~ 28.0	19.5 ~ 28.0

Table 6-1 GEOMETRIC DESIGN STANDARDS OF FLYOVERS AND SERVICE ROADS

3) Pavement Design

The following pavement design standards in Indonesia for flexible pavement and rigid pavement were adopted:

- Guide for Flexible Pavement Design (Pedoman Penetuan Tebal Perkerasan Jalan Raja, No. 01/PD/b/1983) published by Bina Marga
- Guide for Rigid Pavement Design (Pedoman Perencanaan Perkerasan Kaku, No. 009/T/BNKT/1988) published by Bina Marga
- Road Design System (RDS) ver.5, one of the pavement design softwares developed by Bina Marga. This is usually used in the design of pavements of national and provincial roads.
- 4) Bridge Design

The following design codes and standards were followed:

- Bridge Design Code, Draft, Volume 1 and Volume 2 Bridge Management System 1992, Direktorat Jenderal Bina Marga Departemen Pekerjaan Umum
- Bridge Design Manual, Draft, Volume 1 and Volume 2 Bridge Management System 1992, Direktorat Jenderal Bina Marga Departemen Pekerjaan Umum
- Pembebanan untuk jembatan, RSNI4 (*Loading for Bridges*)
- Standar perencanaan ketahanan gempa untuk jembatan, SNI (*Design Standard of Earthquake Resistance for Bridges*)
- Perencanaan struktur beton untuk jembatan, RSNI (*Design of Concrete Structure for Bridge*)
- Perencanaan struktur baja untuk jembatan, ASNI4 (*Design of Steel Structure for Bridge*)
- AASHTO LRFD Bridge Design Specifications, 3rd Edition

For design requirements not covered by the above Codes and Standards, the following references will be used as required:

- Japanese Specifications for Highway Bridges
- AS S100 Bridge Design, Australian Standard, 2004
- FHWA-IF-99-025, "Drilled Shafts: Construction Procedures and Design Methods", 1999
- FHWA-NHI-00-043, "Mechanically Stabilized Earth Walls and Reinforced Soil Slopes, Design and Construction Guidelines", 2001
- NCHRP Report 529, "Guidelines and Recommended Standard for Geofoam Applications in Highway Embankments", Transport Research Board, 2004

5) Drainage Design

The following Indonesian drainage design standards and criteria were followed:

- Manual of Design for Road Surface Drainage, 1990, Directorate General of Highways, Directorate of Freeway and Urban Road
- Guidelines of Design for Road Surface Drainage, 1994, Council of Indonesian National Standard
- Design of Road Drainage System, 2005, Department of Settlement and Infrastructure Region
- Calculation Method of Overflow Debit, 1991, Council of Indonesian National Standard

In case there were some lacking information, other standards listed below were referred:

- Highway Engineering, Seventh Edition, Paul H. Wright and Karen Dixon, 2003, John Wiley and Sons, Inc.
- Hydrology Analysis, Sri Harto Br, 1993, Gramedia Pustaka Utama, Jakarta
- Hydraulic for Open Channel, Ven Te Chow, 1992, Erlangga, Jakarta
- Hydrology for Irrigation, Suyono Sosrodarsono, 1993, Pradrya Paramita, Jakarta
- 6) Railway Crossing Requirements

According to the Ministry of Transportation Decree No. KM52, 2000, horizontal and vertical clearance for permanent structures is as follows:

Horizontal Clearance :	10.0m from the rail to surface of pier or permanent structure for each side	
Vertical Clearance :	6.5m from the top of the rail	
ording to PT. KAL clear	ance can be reduced to the following during	

According to PT. KAI, clearance can be reduced to the following during construction:

Horizontal Clearance	:	3.0m from the centerline of the railway for each side
Vertical Clearance	:	5.0m from the top of the rail

7. DESIGN POLICY

7.1 Characteristics of the Project

This project must be implemented under the following conditions:

- The project site is located in the busy urban area with concentration of vehicular traffic as well as pedestrians.
- The project site is narrow and a detour road is not available.
- Commercial and business activities are active along the project site.
- Four flyovers are to built over the existing railway.
- All project sites are located within the seismic zone.
- Gebang and Tanggulangin Flyovers are located at the deep soft ground area.
- Merak Flyover is located at loose sandy layer which would cause liquefaction during the earthquake.

7.2 Japanese Technologies Utilized

The Project is financed under JBIC's STEP Loan. To cope with conditions mentioned in 7.1 above, Japanese technologies were fully utilized on the following:

- Fast construction method to minimize traffic congestion as well as adverse economic impacts during construction.
- Efficient construction method applicable to narrow construction area under urban environment.
- Construction method to realize efficient traffic management during construction.
- Anti-earthquake technology.
- Soft ground treatment technique and treatment against liquefaction.
- Steel bridges for safe, fast and easy construction over the existing railway where the alignment is curved.

 Table 7-1
 summarizes
 Japanese
 technologies
 adopted
 for
 this
 project
 which
 are
 eligible
 to
 STEP
 Loan
 technical
 requirements.

			Japanese Teo	chnology	Adopted		
Objectives	Large Diameter Single Pile	Steel and Concrete Composite Pier	of Super-	PC Deck Slab	Curved Steel Bridge	Soft Soil Improve- ment Around Single Pile	Light Weight Embank- ment
1. Fast Construction	0	0	Δ	0	0	-	0
2. Efficient construction at narrow area	0	0	Δ	_	0	_	0
3. Efficient traffic management	0	0	Δ	-	0	_	0
4. Improved seismic resistance	-	0	0	-		0	0
5. Efficient countermeasure against soft ground in urban area	_	_	_	_	_	0	0
6. Safe, fast and easy construction over railway	-	_	_	_	0	-	-

TABLE 7-1 JAPANESE TECHNOLOGY ADOPTED FOR THIS PROJECT

			Japanese Te	chnology	Adopted		
Objectives	Large Diameter Single Pile	Steel and Concrete Composite Pier	of Super-	PC Deck Slab	Curved Steel Bridge	Soft Soil Improve- ment Around Single Pile	Light Weight Embank- ment
Applied section	 Section with narrow road ROW Section near railway crossing to satisfy required horizontal clearance 	Pier with large diameter single pile	ments	• All bridges	• Over the Railway	• Soft ground section	 Approach section at soft ground

7.3 MEASURES TO COPE WITH CONDITION CHANGES

From the time of project appraisal in October 2004 to the present, there are some drastic changes as follows:

- Domestic construction prices increased by 1.4 times due mainly to fuel price increase made in 2005.
- Japan's steel material price increased by 1.2 times.
- Yen Value depreciated by about 10%.
- Gebang and Tanggulangin Flyover locations were found to be soft ground area. Liquification layer exists at Merak Flyover.
- Public utilities relocation/protection cost was not estimated at the time of project appraisal.

Domestic price increase and Japanese yen value depreciation alone impacted about 43% price escalation, whereas price escalation during the project appraisal was assumed to be 19.1%. In view of such conditions, measures for cost reduction were considered as follows:

MEASURES FOR COST REDUCTION

- To reduce bridged length as much as possible (height of abutment was targeted between 6.5 m to 7.0 m)
- To reduce steel bridge length as much as possible (steel bridges are only adopted for limited sections such as railway crossing, and where single column pier with single pile is required at narrow construction space.
- To use short span length as much as possible (it was found that the shorter span length is more economical even at soft ground.)
- To study reduction of bridge width from 13.0m to 11.5m

7.4 CANDIDATE ITEMS FOR JAPAN PORTION

Requirement of STEP Loan on Japan portion is that "total cost of goods procured from Japan and Indonesia-Japan J.V. companies shall be not less than 30% of total amount of contract(s)". Candidate items for Japan portion is shown in **Table 7-2**.

	Item		Judgement	Condition
Steel	Steel Material		Yes	Procured in Japan
Bridge	Shipping (Japa	an Indonesia)	Yes	
		In Japan	Yes	Fabricated in Japan
	Fabrication	In Indonesia	Yes	 Fabricated by Indonesia-Japan J.V. company
		In Indonesia	No	 Local company other than above
Local Transpo		rtation	No	
	Erection		No	
PC Bridge	PC wire/tendo	on, anchor	Yes	 Procured in Japan Procured from Indonesia- Japan J.V. company
	Admixture for	concrete	Yes	• Same as above
Pier	Steel coping		Yes	Same as steel bridge
	Inner ribbed c	asing for pile head	Yes	Same as steel bridge
Large Diameter Bored Pile	Inner ribbed c	asing for pile head	Yes	Same as steel bridge
Miscellan	Bearing shoe		Yes	Same as steel bridge
eous Bridge Parts	Fall-down Prev	vention Devices	Yes	Same as steel bridge
Drainage	Precast concrete pipe		Yes	 Procured from Indonesia- Japan J.V. company
	Precast catch	basin	Yes	Same as above
	Mechanically	Strip	Yes	Same as steel bridge
Approach Embank- ment	Stabilized Earth Wall	Concrete Panel	Yes	Procured from Indonesia- Japan J.V. company
	Light Weight E	mbankment	No	

TABLE 7-2 CANDIDATES OF JAPAN PORTION

8. BRIDGE TYPE SELECTION

8.1 BRIDGE TYPE SELECTION PROCEDURE

Bridge type selection procedure is shown in Figure 8-1.

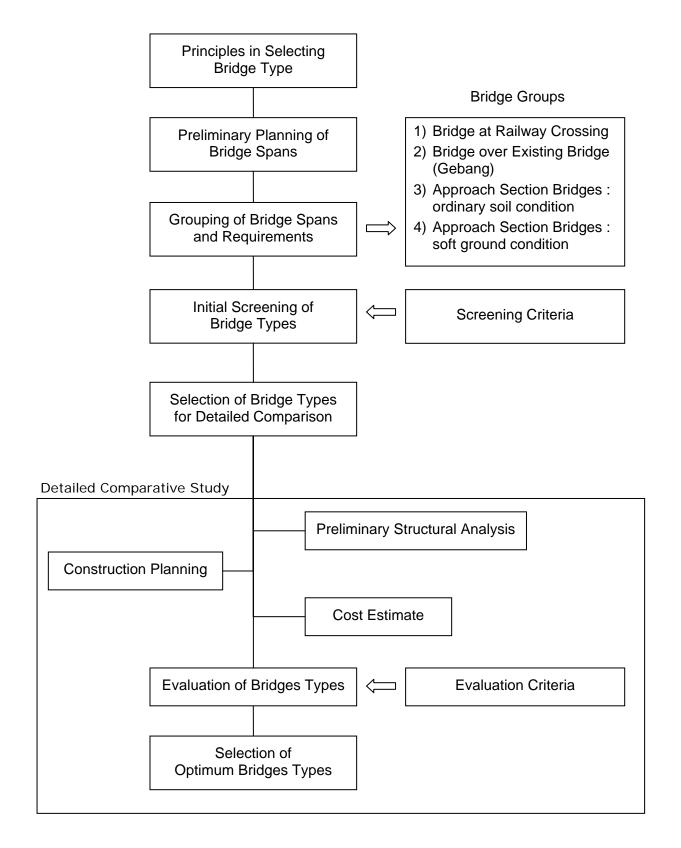


FIGURE 8-1 BRIDGE TYPE SELECTION PROCEDURE

8.2 PRINCIPLES IN SELECTING BRIDGE TYPE

Flyovers will be constructed in the urban areas with high traffic volume and narrow construction sites. Bridge type must be selected in due consideration of such conditions. Principles in selecting bridge type were established as follows:

Principles in Selecting Bridge Type

Primary Principle

- Must be economical.
- Fast construction is possible.
- Traffic disturbance can be minimized.
- Bridge system must be strong against earthquake. (integration of superstructure and substructure should-be achieved as much as possible)

Secondary Principles

- Maintenance is easy and less costly.
- Aesthetic consideration. (match with urban scenery)
- Introduction of new technology.

Special Consideration

• STEP Loan requirement must be satisfied.

8.3 BRIDGE GROUPS

Based on the preliminary planning of bridge spans and location conditions, bridges were grouped into 4 as shown in **Table 8-1**.

	Bridge Group	Characteristics	Approximate Share in Total Bridge Length
1.	Approach Bridge (Standard Soil Condition)	 Any span length will applicable, however, shorter span length is usually more. Almost straight alignment Economical span length is usually 20m ~ 30m. 	35%
2.	Approach Bridge (Soft Soil Condition)	 Any span length will be applicable. Almost straight alignment Economical span length need to be determined. 	25%
3.	Railway Crossing	 Span length = 25 m ~ 35 m Curved Alignment 	35%
4.	Over the Existing Bridge (Gebang Flyover	 Span length = 35 m ~ 45 m Almost straight alignment 	5%

TABLE 8-1 BRIDGE GROUP

8.4 BRIDGE TYPE SELECTED

Through the initial screening and the detailed comparative study, bridge type was selected for each bridge group as shown below:

	Bridge Group	Bridge Type Selected	Remarks
1.	Approach Bridge (Standard Soil Condition)	PC Double Girder	Table 8-2
2.	Approach Bridge (Soft Soil Condition)	PC Double Girder	Same as 1 above
3.	Railway Crossing	Small Size Steel Box Girder	Table 8-3
4.	Over the Existing Bridge	Small Size Steel Box Girder	Same as 3 above

9. DETAILED DESIGN

9.1 FLYOVER LAYOUT

Flyover layout is summarized in **Table 9-1**. General elevation and plan of flyovers are presented in the succeeding pages.

9.2 HIGHWAY DESIGN

Horizontal alignment and vertical alignment are summarized in **Table 9-2** and **9-3** respectively.

Typical cross sections are shown in Figure 9-1 to 9-6.

9.3 STRUCTURE DESIGN

Typical details of structure are shown in the following figures:

- Figure 9-7 General Dimension of Steel Superstructure (1)
- Figure 9-8 General Dimension of Steel Superstructure (2)
- Figure 9-9 Pier Layout: Portal type
- Figure 9-10 Arrangement of PC Cables
- Figure 9-11 Typical Cross Section of PC Bridge
- Figure 9-12 Typical Two Column Pier
- Figure 9-13 Typical One Column Pier
- Figure 9-14 Composite Column Socket Type Connection

		1	Point		40		φ	ŧ	5	10	e	2	Ø	ø	8		ed.
	IR				Girder Height = 1.20m	Girder Height = 1.50m Cirder Heinht = 2.00m	stematic form work tressed concrete. day time.	Superstructure work can start after completion of column, since no coping survived forst setting concete and systematic form work, duration of. Obstruction may be reduced.	with stable flyover	Integration of girder and piler. This system is tuily controlly with Bino Marga Requirement "Bridge Type A" (Integrated Bridge System).	efore after strong column where	New concept of preteresand concrete sido and integrated with slab and grider by Double T-Type Grider System. Camity System is also verified in Japan as appropriate seismic system in earthquake comity of the statem of the	iew in urban area. same as precast	 TPC strongs/ cnoices is both milding facts is devised by all be upportese contents 			As STEP from Project, this acteme is the best option to opply seismically most stokle and integrated prove system unput optioners includogy, stok accordents out and actem and state based pair (tage stac). For itselfs construction high performance concrete all be recommended.
	OUBLE GIRDE			LM Cost Ratio	0.99	c0.1	n-situ using sy hod. is particial pres time but also	xmpletion of co stematic form v	smooth curve	Marga Requirem	integrated, ther located top of	slab and integra 1. is appropriate s	relived. Good v concrete will be	ders & deck sic uction is suitab tem is one of 1		ded	option to apply plogy, such as high performanc
	SCHEME 9 PC DOUBLE GIRDER		Evaluation	Cost /LM	68.10	15.15	dane by cast—i all—staging meti nsioned, girders d not only night	in start after co concrete and syn educed.	possible. exactly with the	nd pier. mply with Bina (em).	ee. Structurally f column will be repair.	essed concrete be Girder System rified in Japan o	is impression of increte, view of concrete colour	& both main gir for fast constr e structural sys ient.		Recommended	ame is the best Japanese Techni er construction 1
TH = 20m	SCH			Cost M Rp /span	1,361.9	1,839.8	 All concrete works are done by coat-in-situ using systematic form work (movable type), or typ of versioning method. Dacks stabilis foot-tensioned, priders is porticial prestressed concrete. Traffic will be disturbed not only night time but also day time. 	perstructure work co required. / using fast setting o nstruction may be re	Curved girders can be possible. Flyover can construct exactly with the smooth curve with stable flyover system.	tegration of girder or is system is fully co itegrated Bridge Syst	Almost maintenance free. Structurally integrated, therefore after strong earthquake, damage of column will be located top of column where accessible and easy to repair.	w concept of prestr der by Double T-Typ iis system is also ver untry.	 Looks stender and gives impression of relived. Good view in urban area. Using high strength concrete, view of concrete will be same as precast girder, solid and clean concrete colour. 	strands/ anchors & gh strength concrete asmically highly stabl STEP Loan requirem		Ř	an Project, this sche Iyover system using large size). For faste
LE LENG					20m Span	30m Snan	1.11	Faster – Su than other is schemes – B)	Easy - C	Easy I I	Good - Al	Yes I I	Good I Lo	Higher Japan's - PC Contents than - Hi other types - Se			 As STEP Lo integrated f bored pile (
r; PII		Ĩ.	Point		40			0	en e	<u>ه</u>	-		3	m	78		
YOVER AT STANDARD SOIL CONDITION, BALARAJA Flyover ; PILE LENGTH = 20m	Standard)		LM : linear meter	Remarks	Girder Height = 1.25m	Girder Height = 1.60m Girder Heicht = 1.70m	 Erection of Grider by truck crone during night time. Precest PC panel is used as forms for dek deb concreting between girders. Andrany roms are used for contilevered deck aloh. Several steps of concreting for coping, diophragm and do are involved. 	substructure.	however its fitting curvature.	irement)	Expansion joint can be eliminated by connecting deck sido of neighbouring spons. Abons.		Commonly seen at flyover in the country, but not appreciated especially for STEP Loan Project Program.	PC strands/ anchors & girder failing prevention devices will be Japanese contents Difficult to comply with STEP Loan Requirement on Japanese Technology application.			ment method, truction concept.
IDITION, I	ER (Indonesia			LM Cost Ratio	1.01	0.08	ing night time. or deck slab cor red deck slab. 9. diaphragm an	construction of a girder erection.	n of deck slab, g is needed for	ed. Bina Marga requ	connecting dech nd girder falling	nighly durable.	ntry, but not ap	prevention devic equirement on J		ended	stalled by segme of faster constru
SOIL CON	SCHEME 6' PC I-GIRDER (Indonesia Standard)		Evaluation	Cost /LM	69.34	71.00	truck crane dur used as forms f sed for cantileve creting for copin	foctured during disturbed during	cantilever portio cantilever. ared type) coping	and pier difficult on devices need Bridge Type A (e eliminated by bearing pads ar	concrete is not be used as a forn flyover in the cou		& girder falling th STEP Loan R		Not Recommended	scasted and be in ffective in terms
ANDARD	SCHEME			Cost M Rp /span	1,386.8	1,/94.5	Erection of Girder by truck crone during night time. Precast PC ponel is used as forms for deck sido co Ordinary forms are used for contilevered deck sido. Several steps of concreting for coping, diophrogm an	Girders can be manufactured during construction of substructure. Night time traffic is disturbed during girder erection.	 Adjusted by width of cantilever portion of deck slab, however its difficult due to short cantilever. Tropezoidal shape (flared type) coping is needed for fitting curvature. 	 Integration of girder and pier difficult. Girder failing prevention devices needed. Not ideal system for Bridge Type A (Bina Marga requirement). 	ansion joint can t ns. d maintenance of	Conventional type. Slab with reinforced concrete is not highly durable. Precast panel can be used as a form.	ommonly seen at flyover in th or STEP Loan Project Program.	strands/ anchors icult to comply wi lication.		No	Unless coping and stab are precasted and be installed by segment method, precasting glider only is not effective in terms of faster construction concept.
ER AT ST		Their French of Supermeterioreterio en el			20m Span	Tork Shan		similar to - Girc T-girder - Nigl	Rather – Adj difficult for – Adj sharp curve – Adj section – Troj	Difficult - Inte - Gird - Not	1 1	111	1	Less Japan's - PC Contents - Diff opp			 Unless copin- precasting gi
YOVI		5 1	Point	Sp	40 40	N P	<u> </u>	10 1-1-0	4 Se filige Se filige	ö	Fair	۶ «	5 Fair	S E	8		
APPROACH SECTION OF FL	SCHEME 6 PC T-GIRDER	That Project of Departmentions Project Brown Sam 2020 Brown	LM : linear meter	Remarks	Girder Height = 1.40m	Girder Height = 1.60m Girder Height = 2.00m	creting between d slab are involved.	ubstructure. cted.	fitting curvature.	irement).	slab of neighbouring prevention devices.	girders which also	preciated especially	PC strands/ anchors and girder falling prevention devices will be japanese. Difficult to comply with STEP Loan Requirement contents on Japanese Technology application.			ment method, struction concept.
PPROAC			E	LM Cost Ratio	1.00	1.00	ing night time. for deck slab con yant of deck slab. g. diaphragm and	construction of s girder erection. struction is expe	on of deck slab. g to be used for	ed. Bina Marga requi	connecting deck id girder falling p nighly durable. panel between g		try, but not appr a prevention devic quirement content	ig prevention dev equirement conte		lended	installed by segmined faster constru
			Evaluation	Cost ALM	68.93	68.96 72 38	of Girder by truck crane during night time PC panel is used as forms for deck slab c which will also constitute a pant of deck slab forms are used for cantilevened deck alab. steps of concreting for cospin, albohragm	lactured during co disturbed during g iring coping const	f cantilever portic lared type) copin;	and pier difficult. on devices needed Bridge Type A (B	be eliminated by f bearing pads ar	concrete is not with PC precast deck slab concr	lyover in the cou ct Program.	s and girder fallir rith STEP Loan R. n.		Not Recommended	precasted and be effective in term
TABLE 8-2				Cost M Rp /span	1,378.5	1,/49.0	 Erection of Girder by truck crone during night time. Precession of Girder by truck crone during night time. Precession of the second truck of the second second	 Girders can be manufactured during construction of substructure. Night time traffic is disturbed during girder erection. Traffic disturbance during coping construction is expected. 	 Adjusted by width of contilever portion of deck slab. Tropezolidal shape (flared type) coping to be used for fitting curvature. 	 Integration of girder and pier difficult. Girder falling prevention devices needed. Not ideal system for Bridge Type A (Bina Marga requirement). 	 Expansion joint can be eliminated by connecting deck slab of neighbouring spans. Need maintenance of bearing pods and girder falling prevention devices. 	 Slab with reinforced concrete is not highly durable. Composite deck slab with PC precast panel between girders which clea works as a form for deck slab concreting. 	 Commonly seen at flyower in the country, but not appreciated especially for STEP Loon Project Program. 	 PC strands/ anchors Difficult to comply wir Technology application 		No	 Unless coping and slob are precasted and be installed by segment method, precasting girder only is not effective in terms of faster construction concept.
				Span Length		The Soon	Easy	Need time for coping works	Rather difficult for sharp curve section	Difficult	Fair	2	Fair	Less Japan's Contents			Unless precas
			Point		A0		9	12	ي م	10 Ice	n	2 C	10	tion 5	100		
	DESCRIPTION	SECTION	Criteria	and the second s	Construction Cost / Economic Aspect (include substructure and pile	(uo	Construction Difficulty / Effective Traffic Managment	Construction Period (Fast Construction)	Applicability to Horizontal Curvature al	Applicability to Integrated Pier and Earthquake Resistance	ance	Introduction of New Technology	g	STEP Loan Requirement Consideration (Japanese Contents)	Total Point	Evaluation	Remarks
					Construction	Toundati	Constru Effective	Constru. (Fast Co	Structural	Aspect	Maintenance	Introduc	Aesthetics	STEP Lo.			
			No.			9		e		4	2	g	7				

			Point			36	10	10	5	10	2	4	œ	4	88		action
-3 RAILWAY CROSSING AT MERAK, NAGREG, PETERONGAN AND TANGGULANGIN FLYOVER	SCHEME 2 SMALL SIZE STEEL BOX GIRDER	Controls Table from the second secon	Evaluation	Span Length Cost (M Rp)/span Cost / LM Cost Ratio Remarks	25m Span 3,699.5 148.0 1.02 Girder Height = 1.40m	35m Span 6,937.5 198.2 1.09 Girder Height = 1.80m	Best Suitable for curved girder and stable during erection, especially above railway.	Good Less number of steel members for erection.	Best No need intermediate diaphragm and most ideal structure system.	Easy to integrate beetween box-girder and box-pier coping.	Good Appropriate slab system and less maintenance.	Good Small size box girder and less number of girders with prestressed concrete slab. Rigid connection of girder and pier.	Good Most simple and appreciated view underneath.	Good Slightly heavier weight than scheme 1 (5%).		Recommend	The best scheme for curve bridge over railway, and if bridge type which is PC 2-Girder is applied to approach section
			Point	0		6	<u></u> б	2	ш v	10	8	ю м	4	ю м	83	ш	-
	SCHEME 1 STEEL I-GIRDER	E & LIOIS Tau mer, rasmit	Evaluation	Span Length Cost (M Rp)/span Cost / LM Cost Ratio Remarks	25m Span 3,620.9 144.8 1.00 Girder Height = 1.60m	35m Span 6,390.7 182.6 1.00 Girder Height = 2.00m	Fair Eosy	Fair Needs longer construction period than scheme 2 due to increased small steel members.	Best Need intermediate cross beam and full lower lateral bracing is required for curve section.	Good Easy to integrate between steel I girder ad box pier coping.	Good Presstresed concrete deck slab is durable and less maintenance.	Fair Rigid connection of girder and pier.	Bad Not oppropriated for urban flyover.	Fair Slightly heavier weight than straight girder for additional bracing member against tortional moment for curve girder.		Not Recommend	Rather complicated erection condition due to curved I-girder above railway
Е 8-		S E C T				04	 10 T	12 12	е 2	10	ю м	ي د	10 E	ш s	100	2	_
TABLE 8-3	DESCRIPTION					1 Construction Cost / Economic Aspect	Construction Difficulty / Effective Traffic Management	 Construction Period (Fast Construction) 	Applicability to Horizontal Curvature	Aspect Applicability to Integrated Pier and Earthquake Resistance	4 Maintenance	5 Introduction of New Technology	6 Aesthetics	7 STEP Loan Requirement Consideration (Japanese Contents)	Total Point	Evaluation	Remarks

ī	- - 		ſ							
		Width of	A	Approacn section			Bridges			
3	Length (Annroach	FIJOVE	Length (m)	Type of Embankment	Bridge	Span Composition and Type of		Substructure	Substructure / Foundation	
ンT	+ Bridge)				Length	Superstructure	Abutment and		Pier and Foundation	u
)						Foundation	Two Column with Two Piles	Single Column with Single Piles	Portal Type
	445.5 m	6.75 m	160.5 m	Mechanically Stabilized Embankment with Soil Improvement	285.0 m	285.0 m PC Void Slab 4span@20m=80m, 4span@20m=80m Steel Box	1-Abut (Integral Abutment) Bored Pile		Bored Pile Φ=2500 mm N=9	Bored Pile Φ=2500 mm, N=4.ΣL=108m
						5span@25m=125m	Φ2-1500,L = 30m		ΣL=201m	Φ=1800 mm, N=2,ΣL=36m
	262.5 m	9.0 m	202.5 m	Mechanically Stabilized Embankment with Soil Improvement	60.0 m	PC Void Slab 3span@20m=60m		Bored Pile Φ=1800 mm N=6, ΣL=152		
Ferry Terminal	346.9 m	7.0 m	176.9 m	Mechanically Stabilized Embankment	170.0 m	170.0 m PC Void Slab 3span@20m=60m	- = 30m egral	ε	Bored Pile Φ=2500 mm,	Bored Pile Φ=2500 mm, N=2
						Steel bux 25m+30m +30m +25m =110m	вогед гле Ф2-1500,L = 34m		N=4,2L=10011	
	520.0 m	13.0 m	159.0 m 140.0 m 299.0 m	Mechanically Stabilized Embankment	221.0 m	221.0 m PRC Double 3span@20m=60m, 4span@20m=80m Steel Box 25m+31m +25m =81m	2-Abut (Integral Abutment) Bored Pile Φ3-1800,L = 20m Φ3-1800,L = 20m	Bored Pile Φ=1500 mm N=12,ΣL=298 m	Bored Pile Φ=2500 mm N=3, ΣL=79m	
	734.0 m	13.0 m	355.5 m 154.5 m 510.0 m	Mechanically Stabilized Embankment	224.0 m	224.0 m PRC Double 4span@20m=80m, 2span@20m=40m Steel Box 25m+27m +27m +25m =104m	2-Abut (Integral Abutment) Bored Pile Ф3-1800,L = 30m Ф3-1800,L = 30m	Bored Pile Φ=1500 mm N=8 ΣL=288 m	Bored Pile Φ=2500 mm N=4, ΣL=169m	Bored Pile Φ=2500 mm N=2 ΣL=52m
	760.0 m	9.0 m	168.0 m 207.0 m 375.0 m	Light Weight Embankment	385.0 m	PRC Double 4span@20m=80m, 4span@20m=80m Steel Box 27m+36m +27m =90m, 5span@27m=135m	2-Abut (Integral Abutment) Bored Pile Φ2-1800,L = 31m Φ2-1800,L = 31m	Bored Pile Φ=1500 mm N=12 ΣL=428 m	Bored Pile Φ=2500 mm N=7 ΣL=255m	Bored Pile Φ=2500 mm N=4 ΣL=140m
	615.0 m	13.0 m	158.0 m 195.0 m 353.0 m	Mechanically Stabilized Embankment	262.0 m	PRC Double 4span@20m=80m,5span@20m=100m Steel Box 25m+32m +25m =82m	2-Abut (Integral Abutment) Bored Pile Φ3-1800,L = 18m Φ3-1800,L = 18m	Bored Pile Φ=1500 mm N=14 ΣL=324 m	Bored Pile Φ=2500 mm N=4 ΣL=132m	
	530.0 m	13.0 m	162.0 m 168.0 m 330.0 m	Light Weight Embankment		200.0 m PRC Double 2span@20m=40m, 3span@20m=60m Steel Box 25m+25m +25m +25m =100m	2-Abut (Integral Abutment) Bored Pile Φ3-1800,L = 40m Φ3-1800,L = 39m	Bored Pile Φ=1500 mm N=8 ΣL=384 m	Bored Pile Φ=2500 mm N=2 ΣL=100m	Bored Pile Φ=2500 mm N=4 ΣL=187m

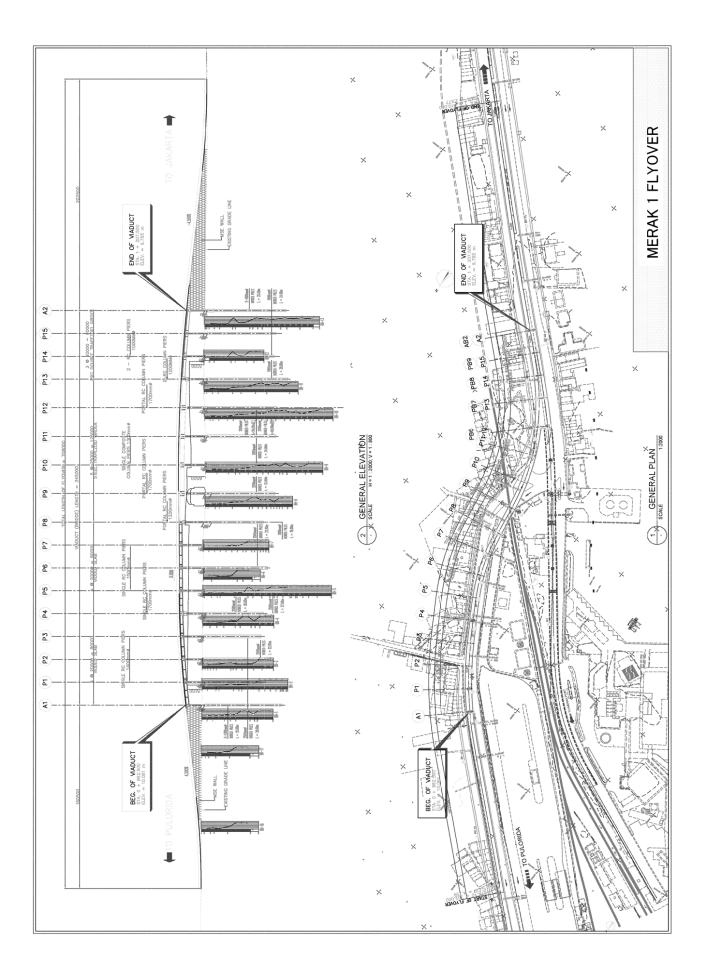
TABLE 9-1 SUMMARY OF FLYOVER LAYOUT

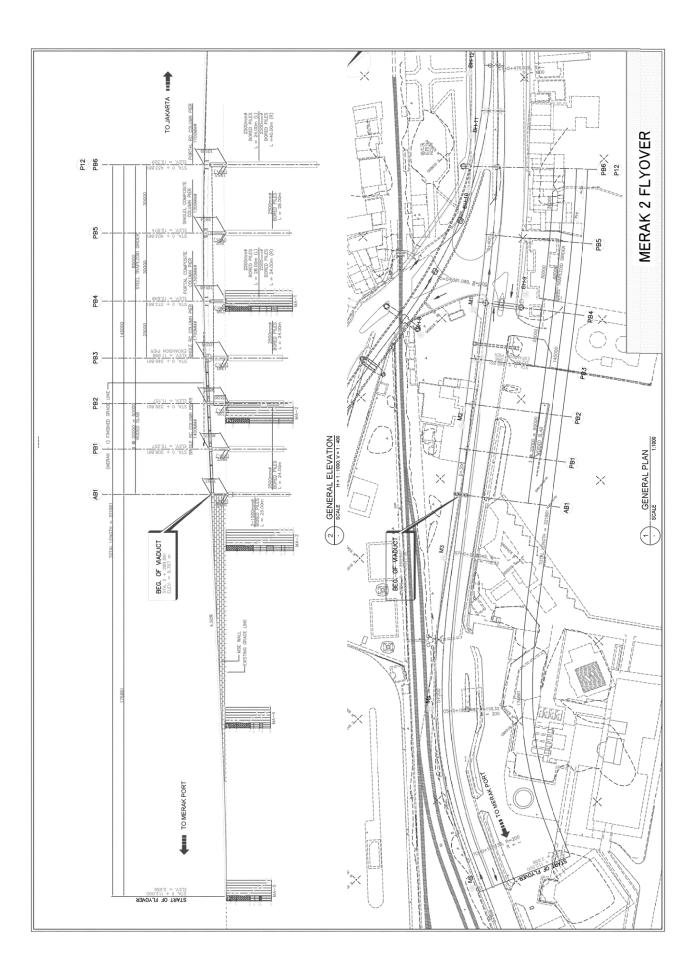
		Design Red	quirement	Horizonta	al Alignment Ado	opted		Remarks
		Design Speed	Min. Radius	Flyover Centerline	No. of curves	Min. Radius Adopted	Super-elevation	
Merak	Along National Road (Pulorida Side)	40 km/h	55m	 About 10m from right edge of existing ROW 	4	150m	6.0%	S-curve at railway crossing
	Ferry Terminal Exit Ramp	40 km/h	55m	 About 4.0 m from left side boundary between ASDP and railway land for the first 100 m. 	2	200m	5.5%	-
	Along National Road (Jakarta Side)	40 km/h	55m	 About 0.5 m left side of the existing road centerline. 	1 1500m		2.0%	-
Balaraja		40 km/h	55m	- Centerline of acquired new ROW	4	75m	5.7%	Sharp curve
Nagreg		50 km/h	90m	- Centerline of being acquired new ROW	7	150m	5.3%	S-curve at railway crossing
Gebang		60 km/h	135m	- Left edge of flyover almost following existing road centerline	6	1200m	2%(Normal)	
Peterongan		60 km/h	135m	- Almost following existing road centerline	3	800m	2.5%	
Tanggulangin		60 km/h	135m	- Almost following existing road centerline	6	250m	5.0%	S-curve at railway crossing

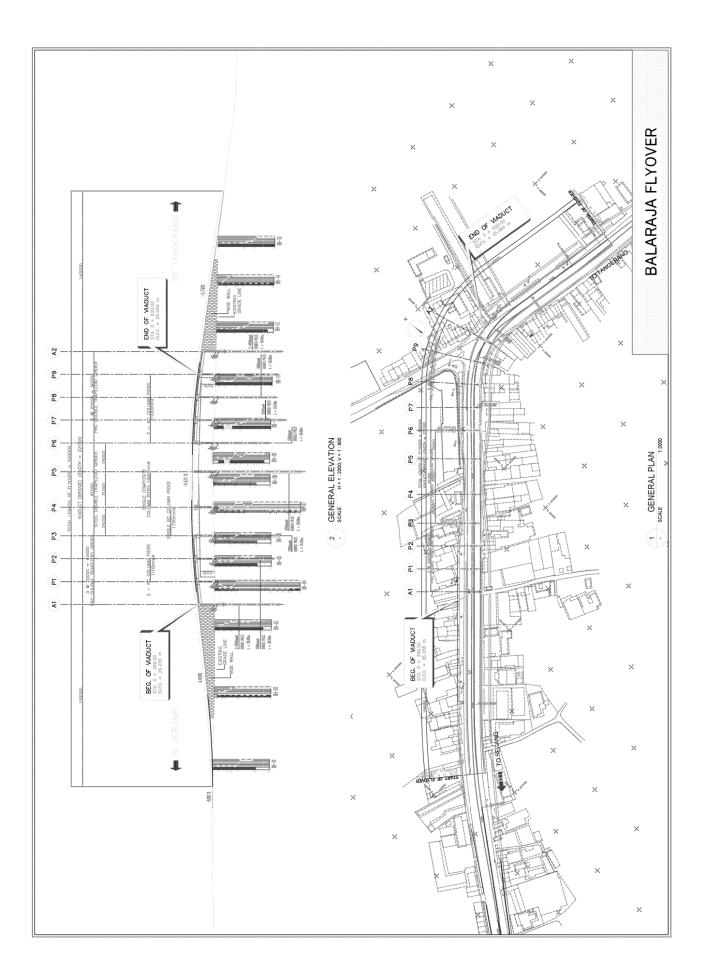
TABLE 9-2 HORIZONTAL ALIGNMENT

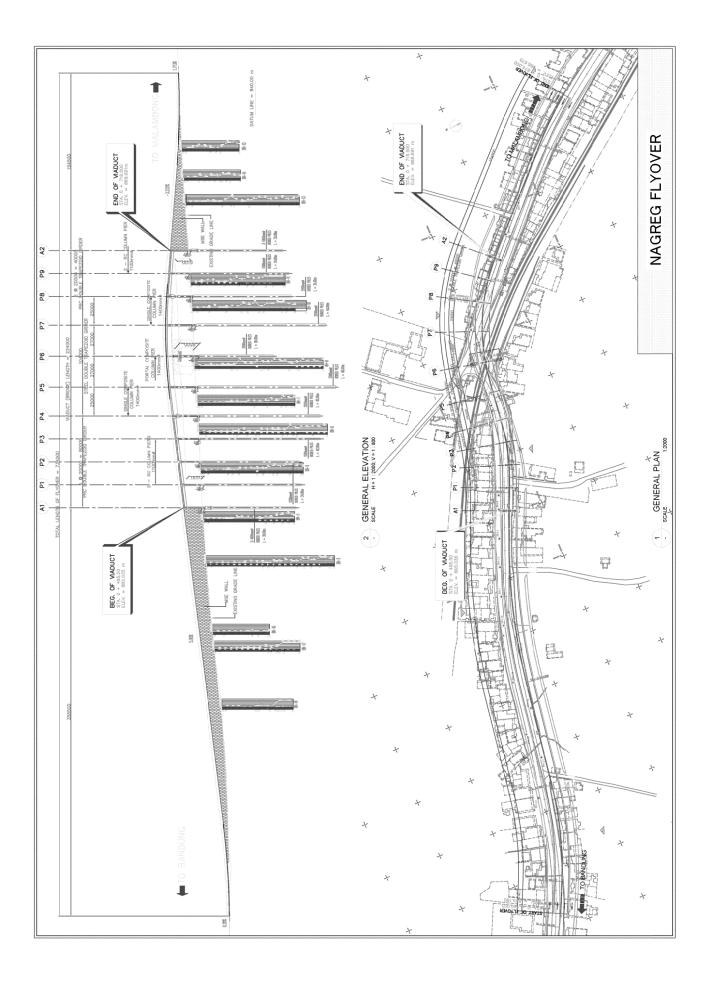
TABLE 9-3 VARTICAL ALIGNMENT

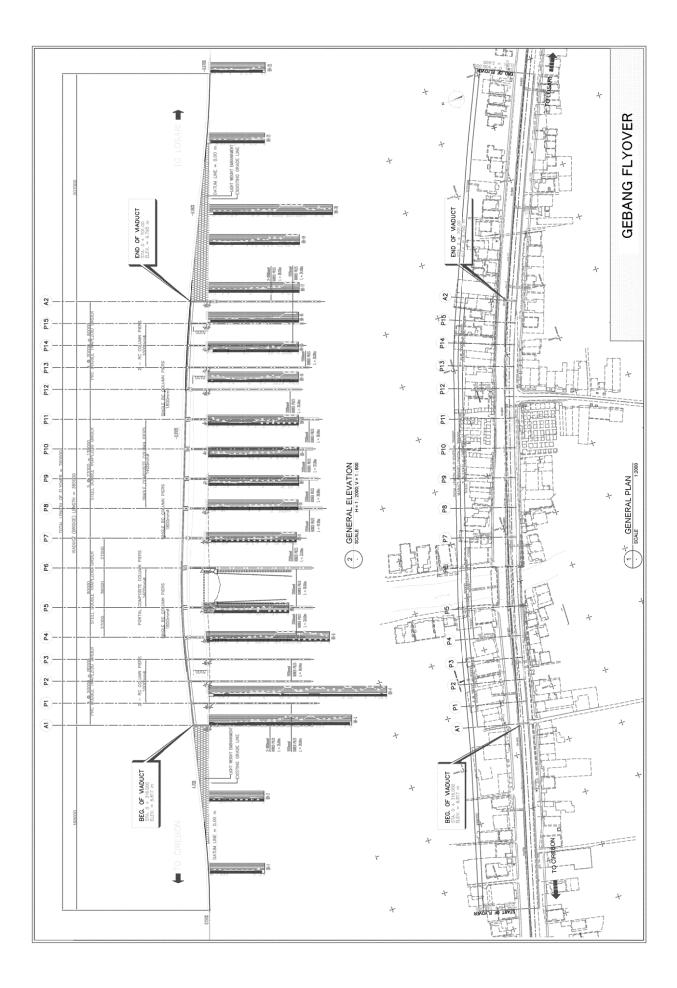
			[Design Requ	irement			Vertical Alignment Adopted					
		Design	Max Gradient	Min. F	Radius	Vertical Clearance	Max	Min	. Radius	Vertical Clearance			
		Speed		Sag	Crest	-	Gradient	Sag	Crest				
Merak	Along National Road			450m 450m		Over Railway 6.5m Over At grade 5.1m	4.5%	2381m	1651m	0+880 – 1+020 (clearance 5.1m) 1+020 – 1+070 (clearance 6.5m) 1+070 – 1+167.5 (clearance 5.1m)			
	Ferry Terminal Exit	40 km/h	8.0%	450m	450m	Over At grade 5.1m	4.5%	1431m	1451m	0+328 – 0+407 (clearance 5.1m)			
Balaraja		40 km/h	8.0%	450m	450m	Over At grade 5.1m	5.73%	1765m	1521m	0+420 – 0+600 (clearance 5.1m)			
Nagreg	Nagreg		8.0%	700m	800m	Over Railway 6.5m Over At grade 5.1m	5.0%	1618m	1215m	0+520 - 0+610 (clearance 5.1m) 0+610 - 0+640 (clearance 6.5m) 0+640 - 0+710 (clearance 5.1m)			
Gebang	Gebang		7.0%	1000m	1400m	Over At grade 5.1m	4.7%	1760m	1783m	0+370 - 0+680 (clearance 5.1m)			
Peterongan		60 km/h 7.0%		1000m	1400m	Over Railway 6.5m Over At grade 5.1m	4.6%	1895m	1796m	0+360 - 0+444 (clearance 5.1m) 0+444 - 0+484 (clearance 6.5m) 0+484 - 0+545 (clearance 5.1m)			
Tanggulangin		60 km/h	7.0%	1000m	1400m	Over Railway 6.5m Over At grade 5.1m	5.0%	1626m	1400m	0+550 – 0+730 (clearance 5.1m) 0+600 – 0+680 (clearance 6.5m) 0+680 – 0+730 (clearance 5.1m)			

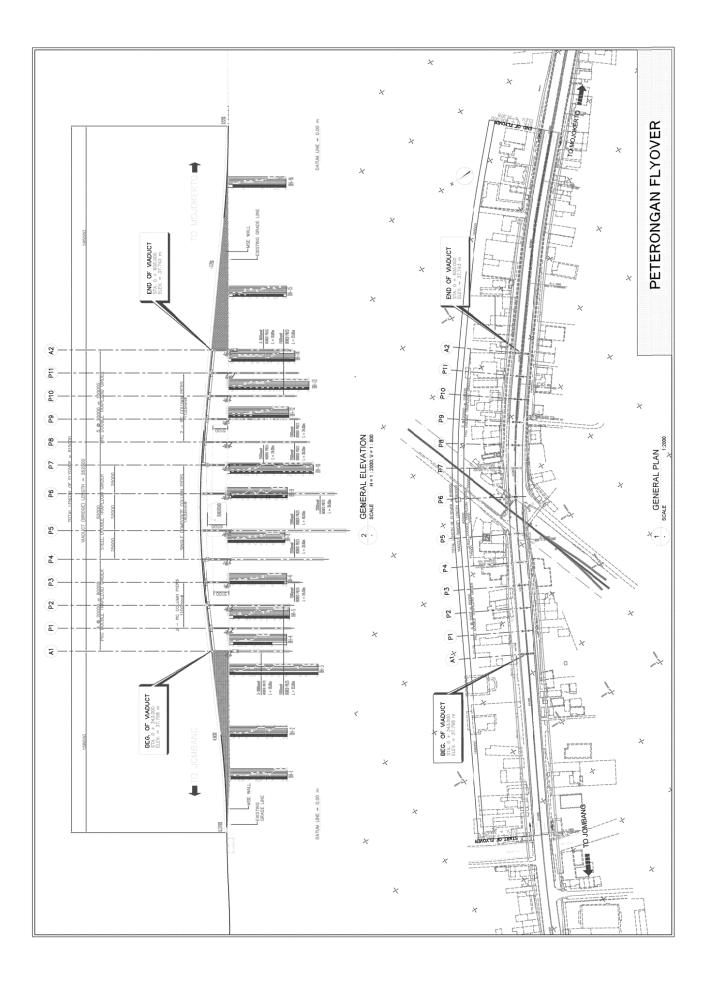


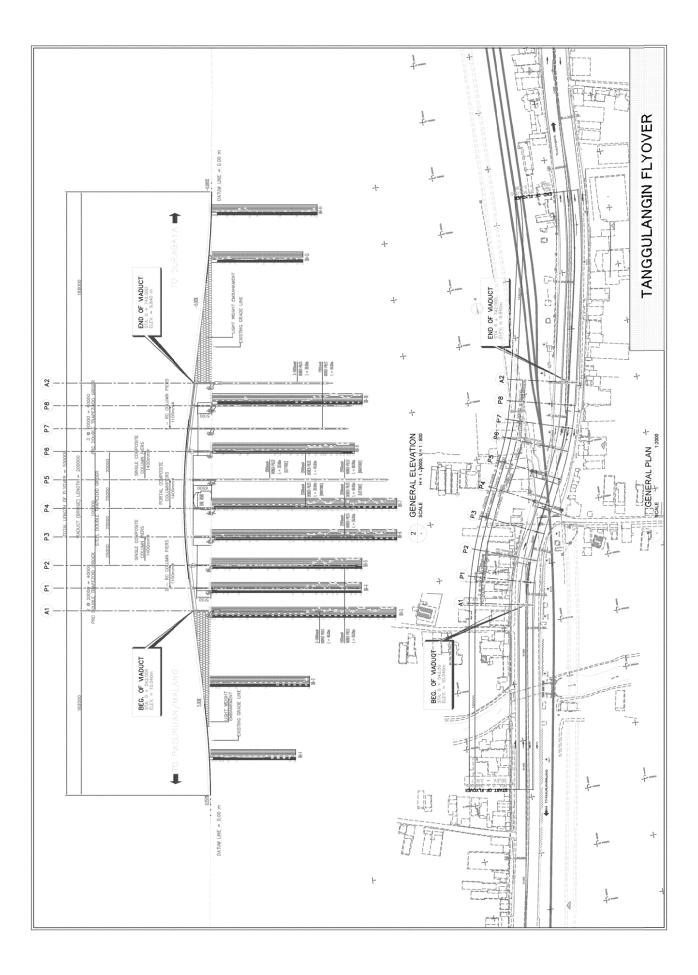












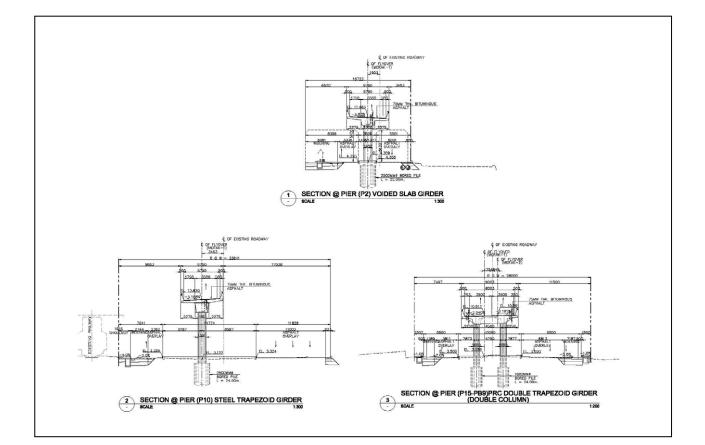


FIGURE 9-1 TYPICAL CROSS SECTION (MERAK)

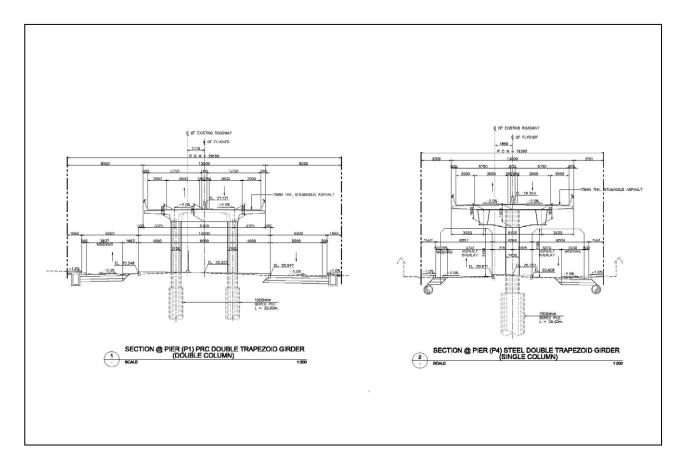


FIGURE 9-2 TYPICAL CROSS SECTION (BALARAJA)

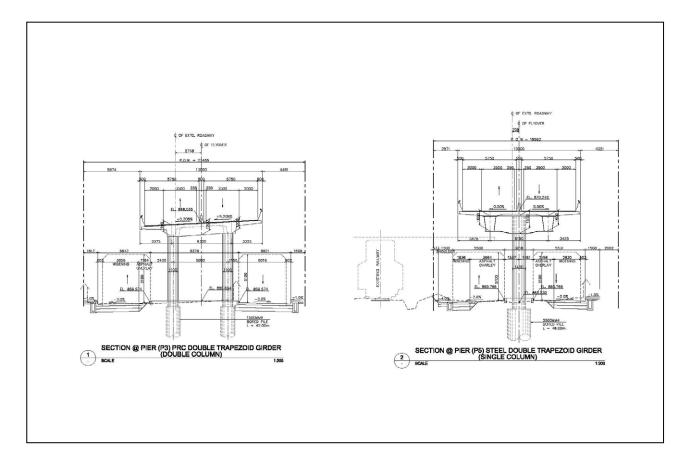


FIGURE 9-3 TYPICAL CROSS SECTION (NAGREG)

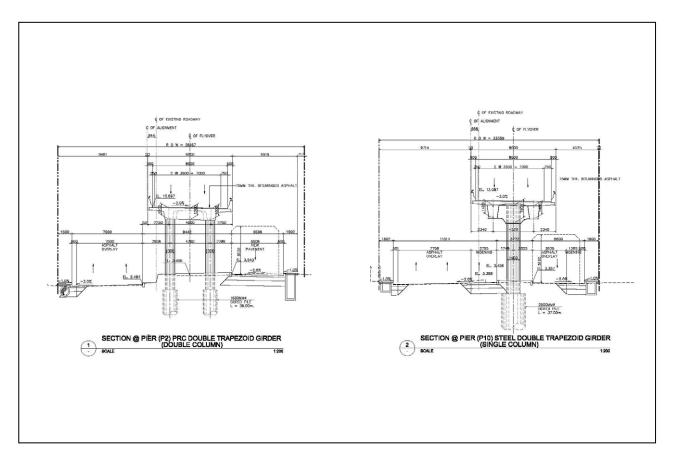


FIGURE 9-4 TYPICAL CROSS SECTION (GEBANG)

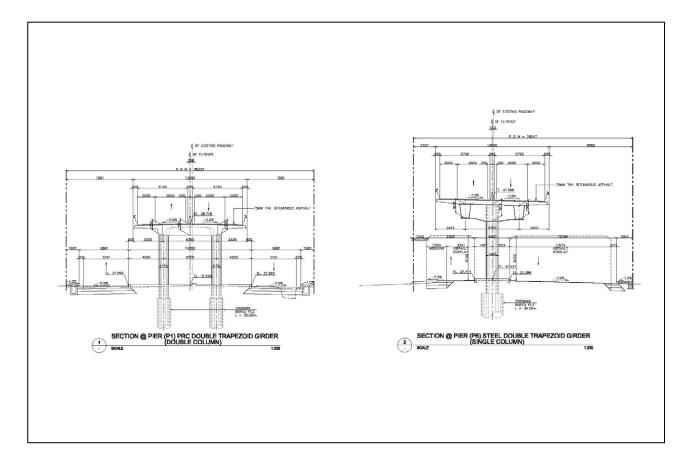


FIGURE 9-5 TYPICAL CROSS SECTION (PETERONGAN)

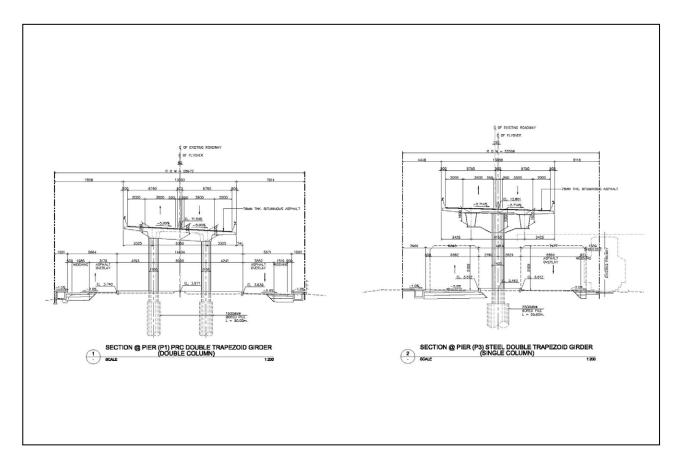


FIGURE 9-6 TYPICAL CROSS SECTION (TANGGULANGIN)

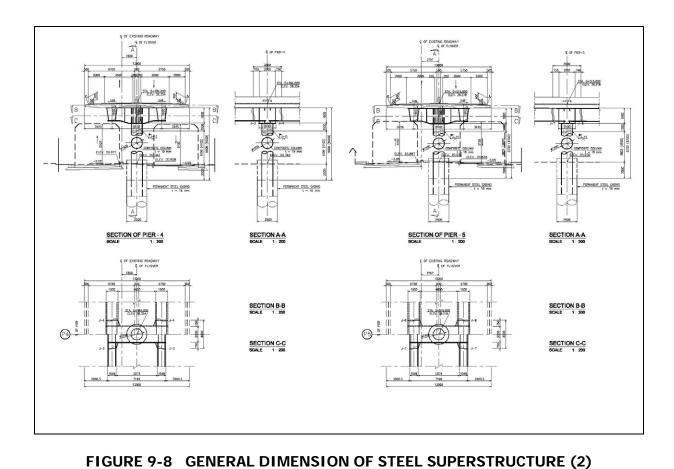
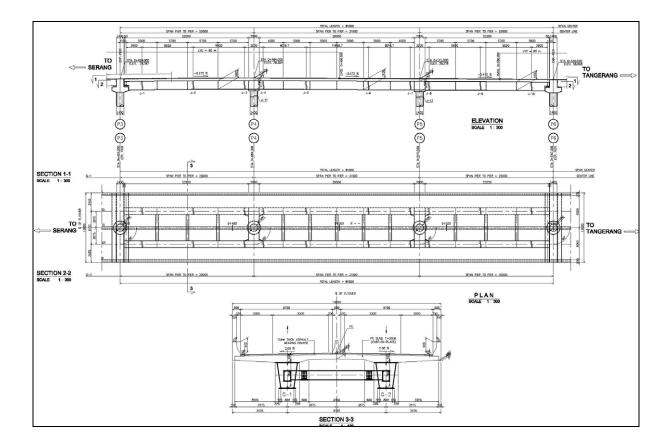
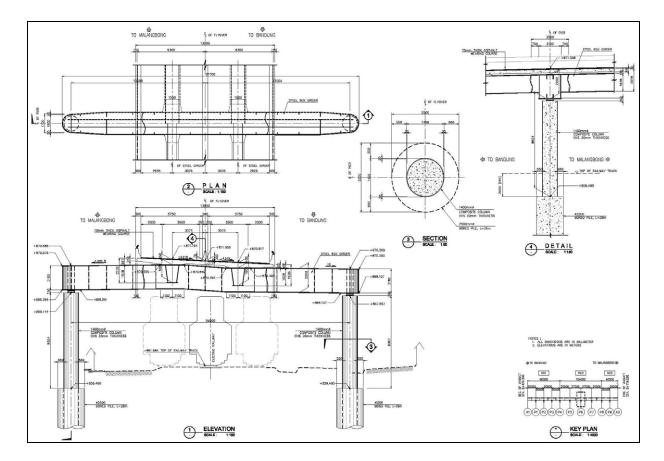


FIGURE 9-7 GENERAL DIMENSION OF STEEL SUPERSTRUCTURE (1)



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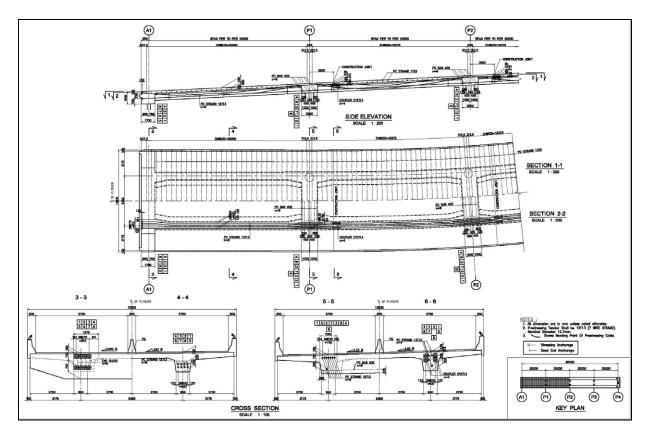
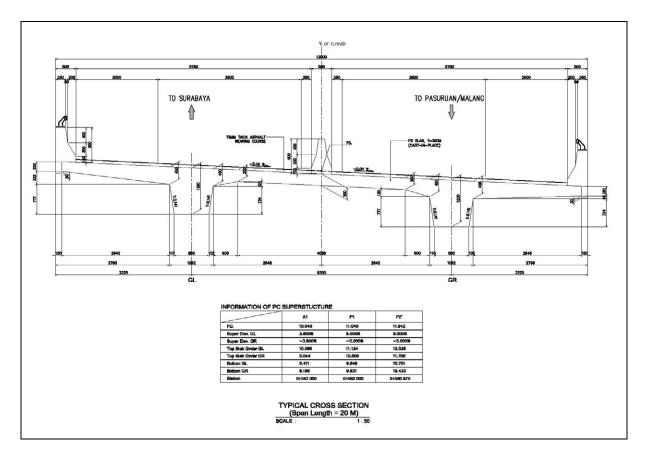


FIGURE 9-10 ARRANGEMENT OF PC CABLES





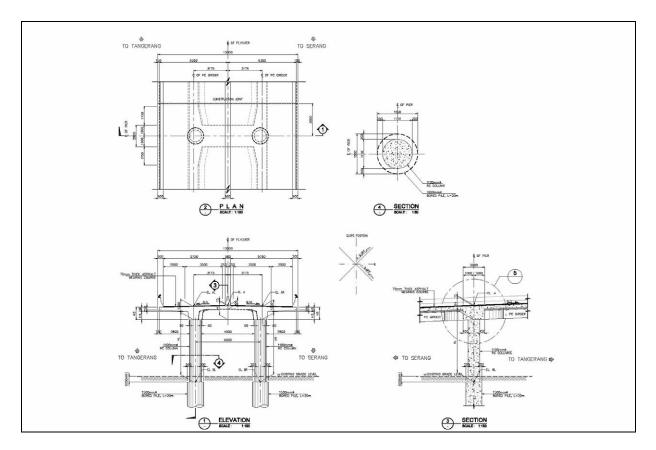


FIGURE 9-12 TYPICAL TWO COLUMN PIER

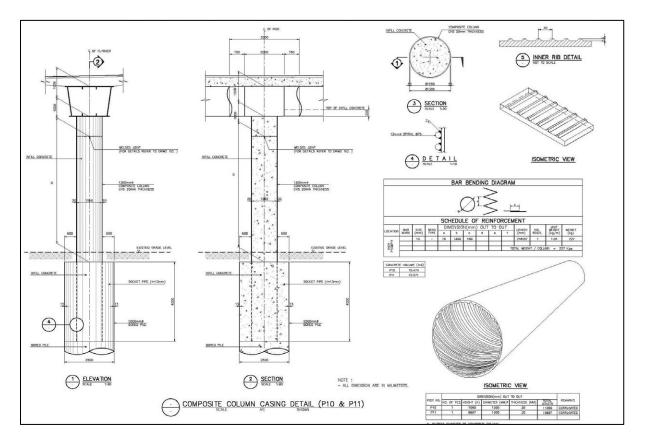


FIGURE 9-13 TYPICAL ONE COLUMN PIER

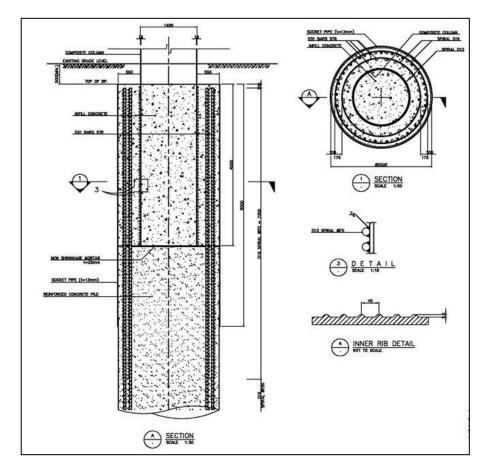


FIGURE 9-14 COMPOSITE COLUMN SOCKET TYPE CONNECTION

9.4 APPROACH EMBANKMENT DESIGN

Approach embankment type was selected focusing on the following:

- Fast construction method to achieve shorter construction period.
- Minimize traffic disturbance during construction (narrow construction space is required).

There are three types of soil conditions as follows:

- Ordinary soil condition (Balaraja, Nagreg, Peterongan)
- Soft soil condition (Gebang, Tanggulangin)
- Ordinary soil with liquefaction layer (Merak)

Embankment type selected for each soil condition is as follows:

- Ordinary soil condition: Mechanically stabilized Earth (MSE) Wall with back fill.
- Soft soil condition: Light Weight Embankment using expanded polystyrene (EPS) block.
- Ordinary soil with liquefaction layer: MSE with soil improvement.

Isometric view of EPS block is shown in Figure 9-15

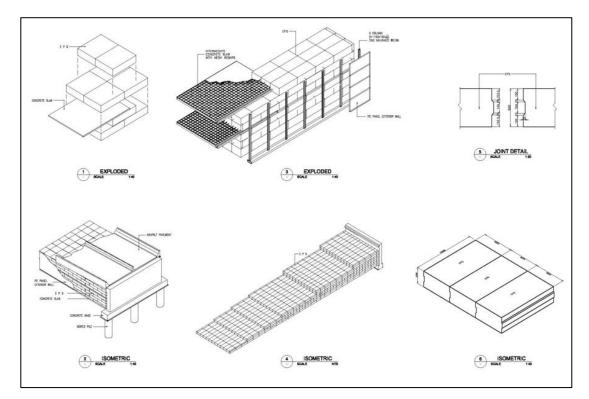


FIGURE 9-15 ISOMETRIC VIEW OF EPS BLOCK

10. PUBLIC UTILITY RELOCATION/PROTECTION PLAN

Following public utility surveys were undertaken:

- Location of overhead utilities such as electric/telecommunication posts were surveyed during the topographic survey.
- As-built drawings of underground utilities were collected from concerned public utility companies and agencies.
- Trial diggings were undertaken to confirm the kind of utilities, depth and sizes.

Although above surveys were undertaken, exact locations of underground utilities were still uncertain, since some of them are located under the existing pavement and as-built drawings are not always accurate. Prior to the start of construction work, exact locations should be confirmed by respective contractor.

<u>OVERHEAD PUBLIC UTILITIES</u>: all overhead public utilities within the project site shall be relocated.

<u>UNDERGROUND PUBLIC UTILITIES</u>: critical underground utilities are as follows:

Flyover	Type of Utility	Name of Utility Company	Relocation is Possible or Not	Measures to be Taken
Merak	Water Pipe (¢200) for Power Plant	PT. PLTU SURALAYA (Power Indonesia)	No	 If it hits flyover foundation, location of foundation to be adjusted
Balaraja	Gas Pipe (¢200)	PN. GAS NEGARA	No	 Protection If it hits flyover foundation, location of foundation to be adjusted
	Oil Pipe (∳400)	PT. PERTAMINA	Yes	Relocation
Nagreg	Oil Pipe (¢250)	PT. PERTAMINA	Yes	 Protection, since it is located deep from the ground surface.
Gebang		(No critical u	Inderground u	utilities)
Peterongan		(No critical u	Inderground u	utilities)
Tanggulangin	Water Pipe (¢400)	PDAM, Surabya	Yes	Relocation

CRITICAL UNDERGROUND UTILITIES

11. CONSTRUCTION PLAN

Construction planning for the work will require due consideration of the following:

- 1) Appropriate and well considered traffic management plan to minimize traffic congestion.
- 2) Due safety for motorist, pedestrian and other road users, protection for existing adjacent houses and operating railway lines.
- 3) Relocation and protection of the overhead and underground utilities.
- 4) Least time consuming construction methodology.

11.1 CONSTRUCTION SCHEDULE

Non-working day ratio (P=0.29) due to Holiday, Sunday and Rainfall over 10mm/day are considered for construction plan.

Detailed construction schedules for each flyover are prepared. Summarized construction schedules for Merak Flyover and Balaraja Flyover are shown in **Figure 11-1**.

ITEMS		MONTH								1	1	2	3		4		5	6		7	:	8	9		10	- 1	1	12
	Utilities Re	elocation & Pro	otection	+	By	Oth	e <u>rs</u> .		- •					AY	÷÷:													
	Mobilizatio	on							+		•			15	l · . ·													
PREPARATIO	N Purchasing								•		_	-	•	Η̈́́́	·													
	Clearing &	Grubbing							•		_	-	,	REBALAN HOLIDAY	[.∵.													
	Temporary	Road									1				÷::													
	Pile Found	ation	(Ø1500,Ø18	300,Ø	2500),L=2	0~4	0m)							¦∴.	2 te	ams											
	Substructu	re	(RC Colum	19 no	os, Co	ompo	osit C	Colur	n 4 n	os)					÷.		3 tea	ams						-				
BRIDGE		PC Girder	14 spans												l 🖓			2	tear	s								
	Superstructure	Steel Girder	-	Deck	s Slal	5)									÷.:	Ĩ					2 te	ams		_	1			
	Finishing V	Works													l · ː ·									1	*			_
	APPROA	CH ROAD												Ĭ-	÷.									—				_
	AT GRA	DE ROAD												۳-	÷				_	-			_	—				
	MISCELI	LANEOUS													÷.:									=				

CONSTRUCTION SCHEDULE

(BALARAJA FLYOVER)

(DALARAJA		-)																													
ITEMS		MONTH									1		2	3	3	4	4		5		6		7		8		9	1	0	11	12
	Utilities Re	location & Pro	tection	♦	B	Oth	ers		-)	•						AY															
	Mobilizatio	n								•		•				틩	· · .														
PREPARATION	Purchasing									-	_			•		AN HOLIDAY	÷														
	Clearing &	Grubbing								•	_	_		•		VIV	÷														
	Temporary	Road										¥—				REBAL/	÷														
	Pile Founda	ation	(Ø1500,Ø180)0,Ø	2500),L=2	20~2	29m))									1 t	eam	•											
	Substructur	e	(RC Colum 9	nos	, Co	npos	it C	olun	n 2	nos)						•	÷.	2 t	eam	s											
BRIDGE	Superstructure	PC Girder	7 spans														· : ·		,			11	eam					•			
	Superstructure	Steel Girder	3 spans (PC I	Deck	Slal	5)											÷					11	eam		•			Γ			
	Finishing V	Vorks															÷								¥	_		Y			_
	APPROA	CH ROAD													K		÷					_	_		_	_					
	AT GRAI	DE ROAD													Ý		÷	-	_	_	-	_	_	_	-	-	_	-		_	
	MISCELI	ANEOUS															÷		•	-	-	_		-	_	-					

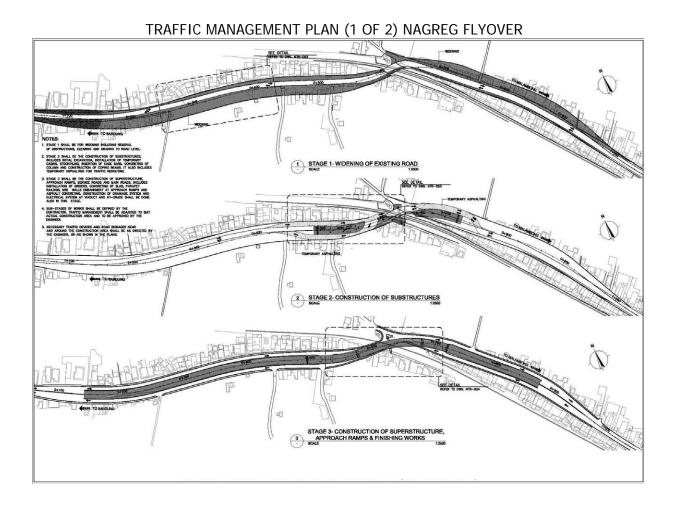
FIGURE 11-1 CONSTRUCTION SCHEDULE (MERAK, BALARAJA FLYOVER)

11.2 TRAFFIC MANAGEMENT PLAN

A plan for moving traffic through or around a construction zone must be developed for the project to assure that adequate consideration is given to the safety and convenience of motorist, pedestrians and all other road users, during the implementation of the project.

The detailed traffic management plans of each flyover for each construction stage are prepared.

The traffic management plan for Nagreg Flyover are shown in **Figure 11-2**.



TRAFFIC MANAGEMENT PLAN (2 OF 2) NAGREG FLYOVER

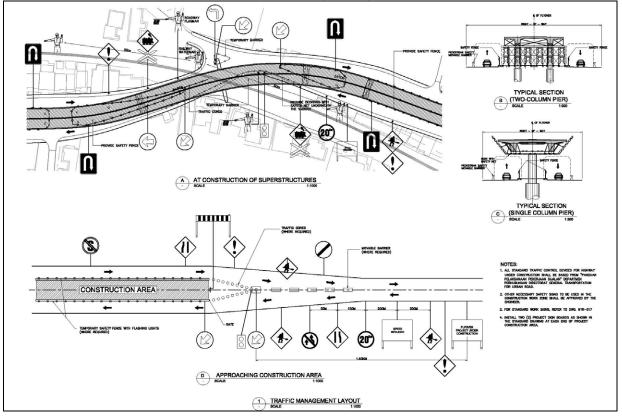


FIGURE 11-2 TRAFFIC MANAGEMENT PLAN (NAGREG FLYOVER)

12. COST ESTIMATE

The derivation of Unit Cost for each item in the BOQ is based on the PU Guide Book (BAHAN BACAAN DAN REFERENSI, ANALISA HARGA SATUAN).

The Unit Cost Analysis/development for pay items which are not included in the PU Guide Book are based on the Civil Works Cost Estimation Standard for Steel Bridges, (Ministry of Land and Transportation, Japan) and Cost Estimation Standard (Japan Construction Cost Investigation Association / Ministry of Land and Transportation, Japan).

12.1 Labor Cost

The basic labor cost is determined in accordance with the Indonesian Guide Books, which are published by each province. (PATOKAN HARGA SATUAN BAHAN DAN UPAH PEKERJAAN BIDANG PEMBORONGAN, DKI JAKARTA, BANTEN, WEST JAVA, EAST JAVA).

Cost of Social Charge, Bonus & Leaves are considered in the Unit Rates for Labour.

12.2 Material Cost

The monetary data used in establishing the Unit Cost of Major Items is based on the Indonesian Guide Book (PATOKAN HARGA SATUAN BAHAN DAN UPAH PEKERJAAN BIDANG PEMBORONGAN). These are for construction material, equipment rental and labor wages. The unit cost of materials is based on current market prices. This analysis is applied for local currency and for the construction components originating in Japan, an exchange rate of Rupiah 1.0 to Japanese Yen 0.0133 (1 Rp. = ± 0.0133) is applied.

12.3 Equipment Cost

Equipment Cost are derived based on PU Guide Book (BAHAN BACAAN DAN REFERENSI, ANALISA HARGA SATUAN). Equipment rental rates are calculated using the formula in the PU Guide Book, which include maintenance cost, fuel & lubricants and operation costs. Equipment prices are updated from the quotations from the manufactures. For the items which are not available in the PU Guide Book, the Construction Machines Depreciation Guide Book of Japan (Japan Construction Machine Association) is adopted.

12.4 Site Investigation

Current market prices for basic materials, labor and equipment are confirmed from the data gathered during the site visits and are incorporated in the estimates.

The locations of Concrete Batching Plant, Asphalt Mixing Plant and Quarry are also identified during the site visits.

12.5 Japan Component

The total costs of goods procured from Japan shall not be less than thirty percent (30%) of the total price of the contract.

The goods procured from the eligible local manufacturing companies invested by Japanese companies can be regarded and counted as Japanese origin if such companies satisfy the condition stated in the Loan Agreement.

12.6 Total Construction Cost

Total Construction Cost and Japan component are summarized in **Table 12-1** and **Table 12-2**.

33.04	3.00	30.04	669.02	60.82	608.20	418.28	189.92	PETERONGAN	S
10.12	0.92	9.20	899.39	81.76	817.63	527.03	290.59	GEBANG	4
153.08	13.92	139.16	786.70	71.52	715.18	464.84	250.34	NAGREG	3
57.46	5.22	52.23	583.92	53.08	530.83	371.02	159.81	BALARAJA	5
12.13	1.10	11.03	1,006.70	91.52	915.18	586.78	328.40	MERAK	1
(Million Yen)	(Million Yen)	(Million Yen)	(Million Yen)	(Million Yen)	(Million Yen)	(Million Yen)	(Million Yen)		
WITH VAT	VAT (10%)	LOCAL PORTION	WITH VAT	VAT (10%)	SUB TOTAL	LOCAL PORTION	JAPAN PORTION	DESCRIPTION	NO
TOTAL	ELOCATION	UTILITIES RI	TOTAL		UCTION COST	TOTAL CONSTR			
)				NT (YEN)	AN COMPONEN	OST AND JAP/	VISTRUCTION C	BLE 11-2 TOTAL CON	TAI
	31.11%	TES RELOCATION	NCLUDING UTILIT	JAPAN PORTION II					
					100.0%	67.1%	32.9%	PORTION PERCENTAGE	
20,500.54	1,863.69	18,636.85	354,141.82	32,194.71	321,947.11	215,998.20	105,948.91	TOTAL	
563.74	51.25	512.49	58,212.70	5,292.06	52,920.64	38,401.94	14,518.70	TANGGULANGIN	9
2,478.18	225.29	2,252.89	50,176.58	4,561.51	45,615.07	31,371.24	14,243.83	PETERONGAN	5
758.96	69.00	689.96	67,454.10	6,132.19	61,321.91	39,527.32	21,794.59	GEBANG	4
11,480.85	1,043.71	10,437.14	59,002.17	5,363.83	53,638.34	34,862.98	18,775.36	NAGREG	3
4,309.15	391.74	3,917.41	43,793.70	3,981.25	39,812.45	27,826.34	11,986.11	BALARAJA	2
909.60	82.70	826.96	75,502.57	6,863.87	68,638.70	44,008.38	24,630.32	MERAK	1
(Million Rupiah)	(Million Rupiah)	(Million Rupiah)	(Million Rupiah)	(Million Rupiah)	(Million Rupiah)	(Million Rupiah)	(Million Rupiah)		
WITH VAT	VAT (10%)	LOCAL PORTION	WITH VAT	VAT (10%)	SUB TOTAL	LOCAL PORTION	JAPAN PORTION	DESCRIPTION	NO
TOTAL	ELOCATION	UTILITIES RI	TOTAL		UCTION COST	TOTAL CONSTR			
NN)				IT (RUPIAH)	N COMPONEN	OST AND JAPA	STRUCTION CO	3LE 11-1 TOTAL CON	TAF
	(UNIT: Million Rupiah) TOTAL GRAND TOTAL WITH VAT GRAND TOTAL WITH VAT GRAND TOTAL (Million Rupiah) (Million Rupiah) 909.66 76,412.23 11,480.85 76,412.23 11,480.85 76,412.23 758.96 68,213.06 758.96 68,213.06 758.96 68,213.06 758.96 68,213.06 758.96 68,213.06 758.96 68,213.06 758.96 68,213.06 758.97 70,483.03 20,500.54 374,642.36 20,500.54 374,642.36 70,500.54 374,642.36 7011 20,500.54 7011 10112 7011 1018.83 710.14 701.37 710.12 909.51 710.12 909.51 710.12 909.51 710.12 909.51 710.12 909.51	TOTAL n WITH VA' h) WITH VA' n WITH VA' n WITH VA' 71 WITH VA' 71 11,480 72 909 71 11,480 73 2,478 29 2,478 29 2,478 29 2,478 30 00 71 11,480 73 20,500 69 20,500 69 20,500 70 112 10 112 10 12 92 153 92 153	ELOCATION TOTAL VAT (10%) WITH VA' (Million Rupiah) (Million Rupia) (Million Rupia) (Million Rupia) 82.70 909 82.70 909 391.74 4,309 1,043.71 11,480 758 69.00 758 69.00 758 563 1,043.71 11,480 758 51.25 51.25 2,478 758 20,500 1,863.69 20,500 1,863.69 20,500 1,863.69 20,500 1,863.69 20,500 1,863.69 20,500 1,863.69 20,500 1,863.69 20,500 1,863.69 20,500 1,863.69 20,500 1,863.69 20,500 1,863.69 20,500 1,10% MITH VA' (Million Yen) (Million Yen) (Million Yen) 112 (Million Yen) 112 (Million Yen) 113 1,302 153 1,302 153 1,302 33 3,000 33	ELOCATION TOTAL VAT (10%) WITH VA' (Million Rupiah) (Million Rupia) (Million Rupia) (Million Rupia) 82.70 909 82.70 909 391.74 4,309 1,043.71 11,480 758 69.00 758 69.00 758 563 1,043.71 11,480 758 69.00 758 563 1,043.71 11,480 758 69.00 758 563 1,043.71 11,480 758 751.25 51.25 563 1,863.69 20,500 12 1,863.69 20,500 12 1,863.69 20,500 12 1,106 WITH VA' 11 VAT (10%) WITH VA' 12 (Million Yen) (Million Yen) 12 (Million Yen) 13.32 522 13.92 5.22 57 57 11.10 11.10 12 13.30 33 33 <	TOTAL UTILITIES RELOCATION TOTAL NITH VAT UUTILITIES RELOCATION ATIL NITH VAT LOCAL PORTION VAT (10%) WITH VAT NITH VAT LOCAL PORTION VAT (10%) WITH VAT Notilition Rupiah) (Million Rupiah) (Million Rupiah) (Million Rupiah) 87 75,502.57 826,96 82.70 909 25 43,793.70 3,917.41 391.74 4,309 26 67,454.10 689.96 69.00 758 21 50,176.58 2.252.89 22,478 563 21 50,176.58 2.252.89 21,269 563 21 50,176.58 2.252.99 2,478 21 50,176.58 2.252.99 2,478 21 50,176.58 2.12.49 51.25 563 21 354,141.82 18,636.85 1,863.69 20,500 21 354,141.82 18,636.85 1,863.69 20,500 21 354,141.82 18,636.85	TOTAL UTILITIES RELOCATION TOTAL NUTH VAT UUTILITIES RELOCATION MITH VAT NUTH VAT LOCAL PORTION VAT (10%) WITH VAT NUTH VAT LOCAL PORTION VAT (10%) WITH VAT Nutrinon Rupiah) (Million Rupiah) (Million Rupiah) (Million Rupiah) 87 75,502.57 826.96 82.70 909 83 59,002.17 10,437.14 11,480 758 83 59,002.17 10,437.14 11,480 758 83 59,002.17 10,437.14 11,480 758 83 59,002.17 10,437.14 11,480 758 83 59,002.17 10,437.14 1,043.71 11,480 83 59,002.17 10,437.14 1,043.71 11,480 83 59,002.17 10,437.14 1,043.71 11,480 83 59,002.17 10,437.14 1,043.71 11,480 83 50,176.58 2,252.89 2,523 503 854	TOTAL UTILITIES RELOCATION TOTAL NUTH VAT UUTILITIES RELOCATION MITH VAT NUTH VAT LOCAL PORTION VAT (10%) WITH VAT NUTH VAT LOCAL PORTION VAT (10%) WITH VAT Nutrinon Rupiah) (Million Rupiah) (Million Rupiah) (Million Rupiah) 87 75,502.57 826.96 82.70 909 83 59,002.17 10,437.14 11,480 758 83 59,002.17 10,437.14 11,480 758 83 59,002.17 10,437.14 11,480 758 83 59,002.17 10,437.14 11,480 758 83 59,002.17 10,437.14 1,043.71 11,480 83 59,002.17 10,437.14 1,043.71 11,480 83 59,002.17 10,437.14 1,043.71 11,480 83 59,002.17 10,437.14 1,043.71 11,480 83 50,176.58 2,252.89 2,523 503 854	TOTAL UTILITIES RELOCATION TOTAL NUTH VAT UUTILITIES RELOCATION MITH VAT NUTH VAT LOCAL PORTION VAT (10%) WITH VAT NUTH VAT LOCAL PORTION VAT (10%) WITH VAT Nutrinon Rupiah) (Million Rupiah) (Million Rupiah) (Million Rupiah) 87 75,502.57 826.96 82.70 909 83 59,002.17 10,437.14 11,480 758 83 59,002.17 10,437.14 11,480 758 83 59,002.17 10,437.14 11,480 758 83 59,002.17 10,437.14 11,480 758 83 59,002.17 10,437.14 1,043.71 11,480 83 59,002.17 10,437.14 1,043.71 11,480 83 59,002.17 10,437.14 1,043.71 11,480 83 59,002.17 10,437.14 1,043.71 11,480 83 50,176.58 2,252.89 2,523 503 854	IL CONSTRUCTION COST TOTAL I CONSTRUCTION COST TOTAL ON INDUCTION COST TOTAL I TOTAL CONSTRUCTION COST TOTAL ONTION EXENTED NITH VAT I TOTAL CONSTRUCTION COST TOTAL ONTION Regula ONTION Regula ONTION Regula VAT (10%) VITH VAT ONTION Regula ONTION Regula ONTION Regula VAT (10%) VITH VAT LOTAL CONSTRUCTION COST TOTAL A SIGN S SUB TOTAL ONTION Regula ONTION Regula ONTION Regula ONTION Regula ONTION Regula ONTION Regula ONTION Regula ONTION Regula ONTION Regula ONTION Regula ONTION Regula ONTION Regula SIGN SiGN SiGN SiGN SiGN SiGN SiGN SiGN Si

783.69 4,995.23

7.52 273.34

0.68 24.85

6.83 248.49

776.17 4,721.89

70.56 429.26

705.61 4,292.63

512.03

193.58

TANGGULANGIN

9

31.11%

JAPAN PORTION INCLUDING UTILITIES RELOCATION

100.0%

2,879.98 67.1%

1,412.65 32.9%

PORTION PERCENTAGE

TOTAL

(NOTE); EXCHANGE RATE 1 Yen = 75 Rupiah

13. PREPARATION OF DRAFT P/Q AND TENDER DOCUMENTS

Draft Prequalification Documents and Tender Documents consisting of the following were prepared.

Prequalification Documents

- Glossary (Definition)
- Invitation for Prequalification
- Instructions to Applicants
- Application Data Sheet
- Prequalification Criteria
- Application Forms
- Scope of Contract

Tender Documents

- Invitation for Bids
- Instruction to Bidders
- Bidding Data
- General Conditions of Contract (FIDIC, 1999)
- Conditions of Particular Application
- Technical Specifications
- Drawings
- Bid Form, Appendices to Bid, Bid Security forms
- Bill of Quantities
- Schedule of Supplementary Informations
- Form of Agreement and Sample Forms of Securities
- Disputes Resolution Procedure
- Evaluation Procedure of Bid Proposals
- Post Qualification

14. UPDATING OF UPL AND URL

14.1 Social Survey

The public hearings (socialization) and negotiation with the affected families have been completed before the Study commenced at Balaraja and Gebang Flyovers and these activities were on-going at Nagreg Flyover, therefore, the social survey was undertaken at the remaining three flyovers, namely Merak, Peterongan and Tanggulangin Flyovers.

Number of respondents were as follows:

	Merak	Peterongan	Tanggulangin
No. of Respondents	165	118	88

Some of the results are summarized hereunder:

FAMILY STATUS

			Merak	Peterongan	Tanggulangin
1.	No. o	f Families in One House			
	1.1	One (1)	145	75	59
	1.2	Two (1)	3	17	16
	1.3	Three (3)	1	2	3
	1.4	Four (4)	-	-	-
	1.5	No answer	16	24	10
2.	No. o	f Persons in One Family			
	2.1	Two (2)	2	16	3
	2.2	Three (3)	16	10	15
	2.3	Four (4)	22	23	22
	2.4	Five (5)	41	13	18
	2.5	Six (6)	36	4	8
	2.6	More than six	24	28	13
	2.7	No answer	24	24	9
3.	Month	hly Family Income			
	3.1	< Rp 500,000	22	25	18
	3.2	500,000 - 1,000,000	62	27	26
	3.3	1,000,000 - 2,000,000	37	19	24
	3.4	2,000,000 - 3,000,000	21	9	4
	3.5	> 3,000,000	14	14	3
	3.6	No answer	9	24	13
4.	Month	hly Family Income			
	4.1	< Rp 500,000	30	20	18
	4.2	500,000 - 1,000,000	69	29	26
	4.3	1,000,000 - 2,000,000	30	20	24
	4.4	2,000,000 - 3,000,000	17	12	4
	4.5	> 3,000,000	10	13	3
	4.6	No answer	9	24	13

STATUS OF HOUSE

			Merak	Peterongan	Tanggulangin
1.	Own	ership of House Land			
	1.1	Owned	49	80	64
	1.2	Rental	72	6	3
	1.3	Company Land	-	-	2
	1.4	Parent's Land	-	3	14
	1.5	Government Land	39	3	-
	1.6	No Answer	5	26	5
2.	Own	ership of House			
	2.1	Owned	103	80	70
	2.2	Rental	42	6	1
	2.3	Company House	1	3	1
	2.4	Parent's House	-	3	10
	2.5	Government House	8	3	-
	2.6	No Answer	11	26	6

			Merak	Peterongan	Tanggulangin
1.	Distar	nce from Home to Workplace			
	1.1	0-3 km	119	79	57
	1.2	3-5 km	5	4	4
	1.3	5-7 km	-	5	3
	1.4	7-9 km	-	1	-
	1.5	More than 9 km	15	5	16
	1.6	No Answer	26	24	8
2.	Time	Required to Workplace			
	2.1	0 minute	12	65	20
		1 – 20 minutes	62	13	15
	2.3	20 – 30 minutes	2	9	39
	2.4	More than 30 minutes	21	7	6
	2.5	No Answer	18	24	28
3.	Mean	s of Transportaiton			
	3.1	Walking	58	55	12
	3.2	Bicycle	6	2	3
	3.3	Motorcycle	49	13	19
	3.4	Motorbike Taxi (Ojek)	3	-	1
	3.5	Private Car	27	6	3
	3.6	Bus / Mini Bus	11	4	6
	3.7	Train	-	-	1
	3.8	Pedicab (Becak)	-	-	-
	3.9	Government car	1	-	-
	3.10	Others	-	-	14
	3.11	No Answer	10	28	29

KNOWLEDGE AND OPINION ON THE PROJECT

			Merak	Peterongan	Tanggulangin
1.	Knowledge abo	out the Project			
	1.1 Have kn	own	47	65	42
	1.2 Do not k	now	112	53	34
	1.3 No answ	er	6	-	12
2.	Source of Infor	mation			
	2.1 Officer of	f Desa/Kecamatan	14	61	10
	2.2 Neighbo	r	29	10	6
	2.3 Radio /	ΓV	1	-	-
	2.4 Newspa	ber	-	1	3
	2.5 Others		3	46	18
	2.6 No Answ	ver	118	-	51
3.	Opinion on the	Project			
	3.1 Give Ber	nefit	20	36	17
	3.2 Harming		122	47	41
	3.3 No Chan	ge	14	35	24
	3.4 No Answ	ver	9	-	6

			Merak	Peterongan	Tanggulangin
4.	Rease	on Why the Project is Harming			
	4.1	Increase noise	-	5	1
	4.2	Increase air pollution	-	35	2
	4.3	Land and/or house be taken	114	46	34
	4.4	Decrease income	1	-	-
	4.5	Less of business	1	-	-
5.	Reas	on Why Project is Benefecial			
	5.1	Smooth Traffic Attained	26	13	7
	5.2	Faster Travel	1	5	5
	5.3	Increase of Land Price	2	15	5
	5.4	New Business Opportunity	1	-	-
	5.5	Obtaining Compensation	1	-	-
	5.6	Others	-	3	-
6.	•	ctation from the Project	76	3	2
	6.1	Job opportunity during construction	70	3	2
	6.2	Obtain New Livelihood	35	9	12
	6.3	No expectation	46	80	51
	6.4	Smooth Traffic	1	-	-
	6.5	Getting Compensation	2	-	16
	6.6	Others	-	26	7
	6.7	No Answer	4	-	-

METHOD OF COMPENSATION

	Method	Merak	Peterongan	Tanggulangin
1.	Money	153	94	61
2.	Alternative Land	5	-	4
3.	Up to the Government	1	24	16
4.	Do not know yet	2	-	-
5.	Business Place	-	-	-
6.	No Answer	4	-	7

14.2 Updating UKL and UPL

Original UKL and UPL

According to the decree of Ministry of Environment No. 17/2001, EIA (AMDAL) is not required for construction of flyover of less than 2 km in length, instead, the Environmental Management Plan (UKL) and the Environmental Monitoring Plan (UPL) are required. All flyovers have the length of less than 2 km, thus EIA is exempted, but UKL and UPL are required.

Based on the Feasibility Study undertaken in year 2003, original UKL and UPL were prepared and approved by the respective local environmental agency as follows:

Flyover	Original UKL & UPL Approved on :
Merak	October 8. 2003
Balaraja	October 6, 2003
Nagreg	February 16, 2005
Gebang	October 3. 2003
Peterongan	October 8. 2003
Tanggulangin	October 8. 2003

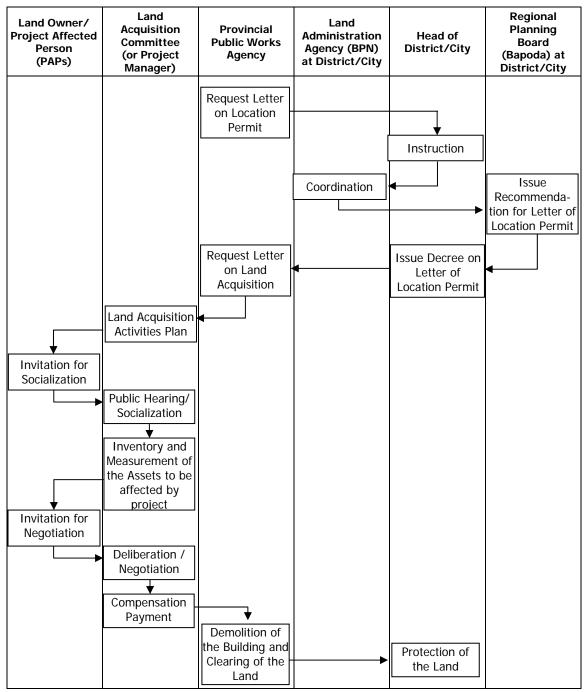
Updating of UKL and UPL

Based on the Basic Design of the project, original UKL and UPL were updated. Basic concept, nature of the Project and project site condition are almost the same as those of the feasibility study stage, updating was focused on the revision of the scope of work of the Project. Updated UKL and UPL were submitted to the respective local environmental agency and approved on the date as follows:

Flyover	Updated UKL & UPL Approved on	
Merak	June 22. 2006	
Balaraja	June 13, 2006	
Nagreg	November 24, 2006	
Gebang	July 6, 2006	
Peterongan	June 13, 2006	
Tanggulangin	June 16, 2006	

15. DRAFT ROW ACQUISITION AND RESETTLEMENT PLAN

Right-of-way acquisition process is shown below:



ROW ACQUISITION PROCESS

Note: If land area to be acquired is less than 1 ha. Project Manager can undertake land acquisition.

ROW acquisition of Balaraja, Nagreg and Gebang Flyovers has started prior to the start of the study. ROW acquisition of Merak, Peterongan and Tanggulangin is being implemented simultaneously with the detailed design. Present status of ROW acquisition is shown in **Table 15-1**.

So far, all project affected peopled preferred to be compensated by money. Most of the case, only a partial of a house/store/building is affected, therefore, people still stay in the same place. Two school buildings were affected at Balaraja Flyover. The Local Government has already built alternate school building within the same school compound.

TABLE 15-1 PRESET STATUS OF ROW ACQUISITION

In the field of the f	Instruction Land Acquisition Tended (pillion)	No.	Project Name	a sinisimore a secondaria de la seconda de l					Journal of	-					
				Land Acquistion Required (m2)		Public Hearing	Measuring Affected	Community		Demolition	for ROW Acquisition	APRD-II	(billion) APRD-I	APRN	Remarks
Operational constant Safano Complete Operational constant MV MV Patholic constant Pa	Mode -1 Mode Florent SP0.00 Campelene OC MV XV ZeBulon YV ZeBulon Y				Assets	(Socialization)	Assets	Consultation		Clearing	inclusion				
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16. PROJECT IMPLEMENTATION PLAN

The implementing agency is the Directorate General of Highway (DGH), Ministry of Public Works. Project implementing organization is shown in **Figure 16-1**.

Implementation Schedule is shown in Table 16-1

		20	05		20	06		20	07			20	80	
Detailed Design	by JICA		_											
Selection of Sup	ervision Consultant													
Land Acquisition														
Selection of Con	tractor						(8 m	onth	5)					
Utility Relocation by Local Fund									nths)				
Consultancy Ser Construction Su														
	Package - 1									(12	mont	hs)		
Construction	Package - 2									(12	moni	hs)		
	Package -3									(12	moni	hs)		

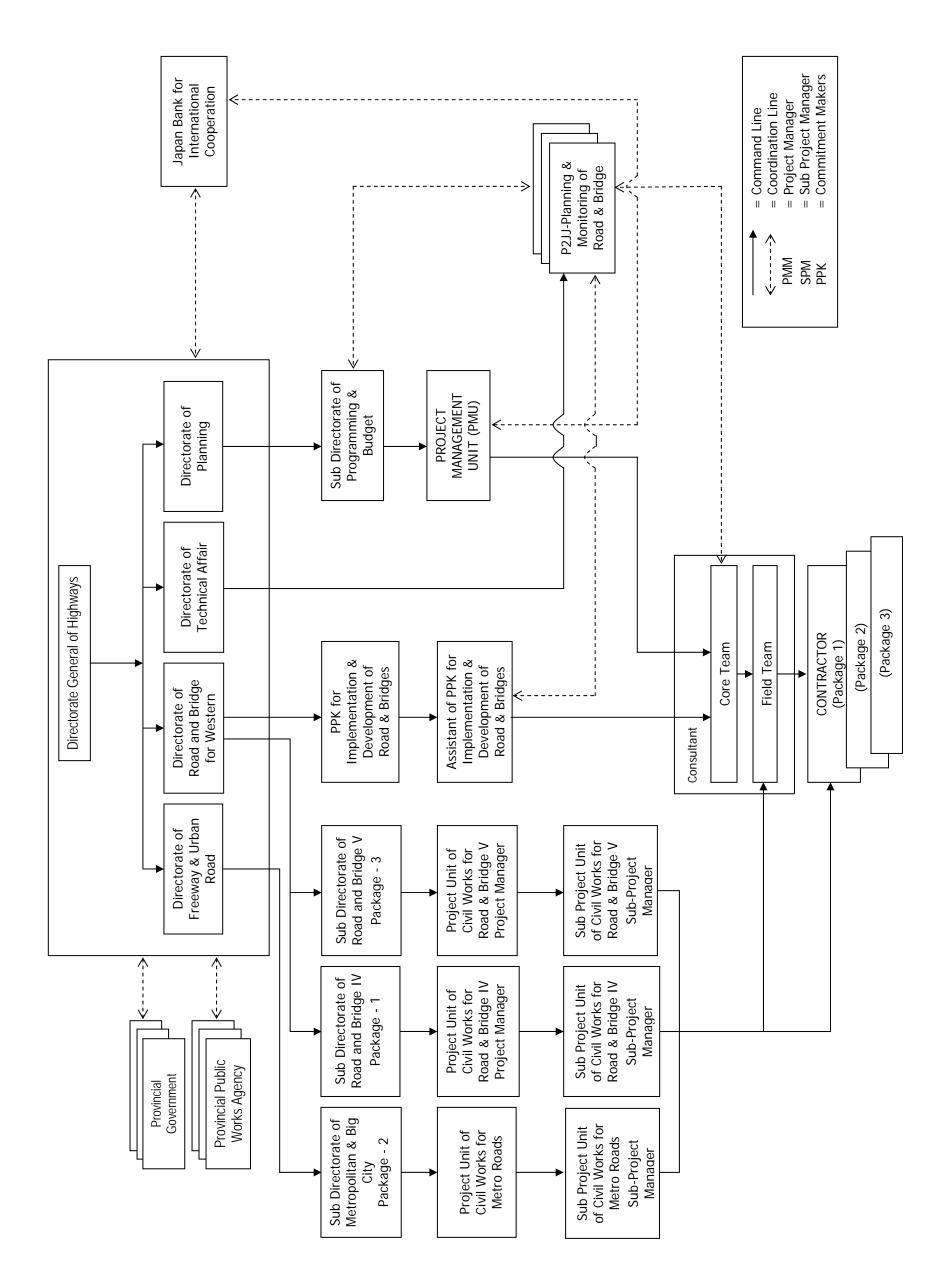
TABLE 16-1 IMPLEMENTATION SCHEDULE

Annual fund requirement by source of fund for construction is estimated as shown below:

ANNUAL FUND REQUIREMENT BY SOURCE OF FUND

	UNIT : Million Y					
	Fund Source		Year			
	Fund Source	2007	2008	Total		
Α.	Construction of Flyover					
A-1.	Consultancy	176	165	341		
	- JBIC Loan	160	150	310		
	- Local Counterpart Fund (Tax)	16	15	31		
A.2	Civil Work	1,928	2,794	4,722		
		1,510	2,261	3,771		
	- Local Counterpart Fund					
	Local Portion	243	279	522		
	Тах	175	254	429		
	Total	418	533	951		
В.	Utility Relocation					
	- Local Fund	273	-	273		

Note: 1 = 75 Rp.





17. FLYOVER/BRIDGE MAINTENANCE PLAN

In recognition of the current inappropriate state of flyovers/bridge in Indonesia, and lack of appropriate management practices, effective and efficient flyover/bridge asset management system comprising of the following should be established:

Basic Components of a Flyover Management System

- A comprehensive, up-to-date inventory of Flyover.
- A system for inspection and a standard means of recording inspection results.
- A system for recording all flyovers and their associated cost.
- Procedures to establish priorities for maintenance, rehabilitation and improvement.
- A system of identifying, practical deterioration models and treatment options to minimize deterioration of the flyover structural component.
- System for production of reports for evaluating and supporting management decisions.

For better management of flyover/bridge, following should be undertaken:

For Better Management of Flyover/Bridge Assets

- Establish effective and efficient bridge asset management system.
- Develop flyover inventory system based on the typical data for a flyover/bridge inventory.
- Conduct initial condition assessments on all flyover/bridges.
- Determine from the initial condition surveys those flyover/bridge that require a detailed report from inspector. If further detailed inspection is required this should be undertaken by a qualified structural engineer.
- Based on the inspection reports received determine the nature of the problems and possible solutions.
- Undertake comparison between improvement and rehabilitation to establish the most appropriate actions to be taken.
- Report on the findings of the evaluation and put forward recommendations that are based on sound condition and economic assessment, social assessment.
- Monitor on a regular basis the condition of the flyover/bridge assets and ongoing management actions.

18. PROJECT EVALUATION AND RECOMMENDATIONS

18.1 **PROJECT EVALUATION**

1) Operation and Effect Indicators

Operation and effect indicators were prepared and high positive effect were confirmed at each flyover. Example of operation and effect indicators are shown in **Table 18-1**.

TABLE 17-1 OPERATION AND EFFECT INDICATOR: NAGREG FLYOVER

	Operation / Effect Indicator			Year	
	Operation / Effect find	2005	2008	2018	
1)	Daily Traffic Volume	At-grade	17,783	7,672	11,853
	(veh/day)	Flyover	-	12,868	19,638
2)	Travel Speed (km/hr)	At-grade	27.9	39.1	25.8
2)	navel speed (km/m)	Flyover	-	41.1	29.9
3)	3) Travel Time Reduction (veh-hr/day)		-	293	1,752
4)	Travel Cost Savings (1,000	Rp/day)	-	16,213	75,698
5)	Maximum Traffic Queue Leng Railway Crossing during Trai		430	185	288

2) <u>Economic Evaluation</u>

Economic evaluation results are shown in **Table 18-2**. All flyovers were evaluated economically feasible.

Flyover	Economic Internal Rate of Return- EIRR	Net Present Value (Million Rp.)	Benefit Cost Ratio
Merak	14.5%	17,102	1.30
Balaraja	23.0%	63,371	2.74
Nagreg	21.0%	71,085	2.33
Gebang	21.9%	80,788	2.56
Peterongan	17.3%	23,833	1.59
Tanggulangin	13.6%	8,101	1.18

TABLE 17-2 SUMMARY OF COST BENEFIT ANALYSIS

NPV and BCR are based on Discounted Rate 12%

3) Loan amount VS Estimated Cost

JBIC Loan consists of the following:

onsists of the following.	Unit: Million Yen
Base cost for civil work	2,993
Price escalation (19%)	578
Contingency (5%)	178
Total	3,749

In addition to above, the consultancy cost for the detailed design (200 Million Yen) is available. Contingency should be kept to cope with some changes during construction. Available amount of loan is as follows:

Available Amount of Loan (Million Yen)			
Base cost for civil work	2,993		
Price escalation (19%)	578		
 Unused cost for Detailed Design 	200		
Total	3,771		

Estimated cost and shortage of loan amount is as follows:

		Un	it: Million Yen
	Estim	nated Cost	
	(Japan Portion + Local Portion)	(Tax)	(Total)
Civil Work	4,293	429	4,722
Available JBIC Loan	3,771	-	-
Shortage of JBIC Loan	522	-	-

Note: DGH decided to implement public utilities relocation (273 Million Yen or 20.5 B. Rp.) by using local fund prior to the start of flyover construction.

Options to cover shortage of loan are as follows:

Option	Measures to Cover Shortage of Loan	Remarks
Option – 1	The shortage is covered by the local counterpart fund	 This option is in accordance with the condition of Loan Agreement
Option – 2	The scope of work is to be slimed down (such as overlay of an at – grade road, lighting for an at –grade road). Such work is to be done after completion of the flyover by local fund. Work which can be slimed down is limited, thus the local counterpart fund is still needed.	 Amount of scope down is not extensive. Scope down to be studied waiting for bid result.
Option – 3	Defer implementation of one of flyovers, which is to be constructed by new loan or local fund.	 Preparation for implementation such as socialization and ROW acquisition is being undertaken by respective local government This option is possible when ROW acquisition of a certain flyover is not successful in time.
Option – 4	Defer implementation of Exit Ramp of Merak Flyover which is to be constructed by local fund. Cost reduction of this option is not enough, thus local counterpart fund is still needed.	 MoT and ASDP will not agree on this option.

In view of above, Option – 1 is recommended.

	Estimated Cost Excluding Utility Relocation	Amount covered by Loan	Local Counterpart Fund Required
Japan Portion and Local Portion	4,293	3,771	522 (39.2 Billion Rp.)
Тах	429	-	429 (32.2 Billion Rp.)
Total	4,722	3,771 (80%)	951 (71.3 Billion Rp.) (20%)

If Option – 1 is selected, amount and share of local counterpart fund will be as follows:

Note: At the time of the project appraisal, tax portion was estimated at 35.7 Billion Rp.

18.2 CONCLUSION

The Project was evaluated technically, economically, financially and environmentally feasible.

<u>Technical Feasibility</u>: the project utilizes Japanese technologies in line with STEP Loan condition. Construction will be implemented by Japanese Contractor or Japanese Contractor in joint venture with Indonesian Contractor (Japanese contractor as a lead firm) who can execute the work efficiently. Japan portion is estimated to be 31.1% which satisfies STEP Loan requirement.

<u>Economic Feasibility</u>: all flyovers were evaluated economically feasible.

<u>Financial Feasibility</u>: although the project requires additional local counterpart fund which is, however, not extensive and manageable by DGH.

Environmental Feasibility: the project is not environmentally critical, thus EIA (AMDAL) is not required for this project. The project should be implemented and operated in accordance with requirements of UPL and UKL.

18.3 **RECOMMENDATIONS**

- 1) The project must be implemented under the severe urban environment. Construction must be undertaken without major traffic disruption and be completed within the limited time frame. The construction plan prepared under this study should be carefully studied by contractors and supervision consultant and implemented.
- 2) Various technologies were adopted in the study which can be applicable to other similar projects. Such technologies should be positively considered for wide application.
- 3) Options were presented to cover shortage of loan. DGH should further study options and decision should be made as early as possible.
- 4) PT. KAI required closure of an at-grade road at railway crossing, however, it should be done at later stage with proper provisions for local traffic and pedestrians.
- 5) Effect of mudflows from natural gas well near Tanggulangin Flyover should be closely monitored, particularly traffic flow changes, diverted to the national road from the toll road. The effects of mudflow are getting worse, DGH should decide whether construction of this flyover be implemented or not.

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