		Cuerpo de Bombero	s					
Nombre del Área /	Mapa 1:50 000		Chinandega					
Nombre del Munici	ipio/Ciudad		El Viejo					
Localización	Latitud	12	39	41	1,399,696.29			
Localización	Longitud	87	10	2	481,841.90			
Nombre de la Estac	ión	Cuerpo de	Bomberos Volu	intarios El V	/ iejo			
Tipo		Principal 3		Estac	ión			
Número de Teléfon	10		8935438					
Nombre del Oficial	Jefe	Cmdt	e. Dionisio José	López Rubi				
Número de Vehícul	los de Emegencia	1						
Número de Ambula	incias	1 (Mal estado)						
Número de Bomber	ros	47	47					
Número de otros V	ehículos Especiales	1 (Mal estado)	1 (Mal estado)					
Número de Cistern:	as	2 (1 En mal estado)						
Número de Foto-1	Visión Completa *							
Número de Foto-2	Visión Cercana *							

Figure 7-2 A Sample of Filled Format for Fire Station

7-3 Processing of Collected Data

Once all the work items of collecting the data in the field were accomplished, the data were processed in the office of INETER. The collected data were taken into Microsoft Excel files and saved in the Excel file format. These digital data were frequently used for the verifications and corrections of the data on the formats filled in field survey. The format is useful as a reference in the future.

7-3-1 Transferring the Data to Database Format

Using the Microsoft Excel program, the tabulated data were transferred to a database format with .dbf extension which was compatible with the format used in ArcGIS. In this process, the geographic coordinates (Latitude and Longitude) was converted into UTM coordinates(Northing and Easting) with the parameters of UTM zone 16, Spheroid WGS84, and the unit of length was converted into meters. Also, in the .dbf format table, the path to photographs was input in a separate column which was designated to use as linkage to each facility on display. Generally, each site has two photos; one with its overall view (Photo of the Site), and the other to cover the surrounding area or to focus on the close view of the facility (Nearby photo). Sometimes, the names of the actual items in the database files were too long to fill into the GIS database format. In such a case, they were abbreviated. Hence, it was required to prepare a data dictionary of keywords used for particular items in ArcGIS to make correspond to true item names.

	icrosoft Excel - Alcaldia.dl							-1012
3)	Archivo Edición Ver I	nsertar <u>F</u> ormato	Herramientas Datos	s Ve <u>n</u> tana <u>?</u> Ado	be PDF		Escribà una pregunta	* - 6 3
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	A1 - fx	UNIQ NUM				2		
Ĩ	AB	C	D	E	F	G	H	-
ſ	UNIQ NUM CODIGO	MAPA 50K	DEPARTAMEN	MUNICIPIO	ESTE	NORTE	PERSON NUM	TELEFO NU
1	1 AL015 101	Tonala	Chinandega	Puerto Morazán	485231	1411620	- 25	883-8996
	2 AL015_101	Chinandega	Chinandega	El Viejo	481895	1399703	115	344-2219, 23
Ξ	3 AL015 101	Chinandega	Chinandega	Chinandega	485954	1396203	520	341-3016
	4 AL015_101	Chinandega	Chinandega	El Realejo	482050	1386516		887-6122, 88
1	5 AL015_101	Chinandega	Chinandega	Chichigalpa	497091	1389854	118	343-2416 34
	6 AL015_101	Nagarote	León	Nagarote	547046	1356011	150	313-2244
1	7 AL015_101	Nindirí	Masaya	Tisma	606909	1335710	18	552-8721
	8 AL015_101	Nindirí	Managua	Ticuantepe	586593	1329163	62	279-7012
וב	9 AL015_101	Nindirí	Masaya	Nindirí	595885	1327114	96	522-4911
1	10 AL015_101	Granada	Granada	Granada	613911	1318940	373	552-2147
2	11 AL015_101	Nandaime	Granada	Nandaime	603108	1299776		
3	12 AL015_101	Rivas	Rivas	Rivas	627570	1264859	137	563-3571, 58
	13 AL015_101	Rivas	Rivas	San Jorge	630775	1266500	16	563-4611
5	14 ALD15 101	Rivas	Rivas	Buenns Aires	629162	1268183	16	563-3336

Figure 7-3 Part of City Hall Database Format (dbf) File

7-3-2 Implementation of Data into GIS Database

Using the above dbf files, the geographic point data were created and their attributes were added using the ArcGIS software as shown in Figure 7-4). The projection system of the GIS data was kept as same as the one above mentioned.

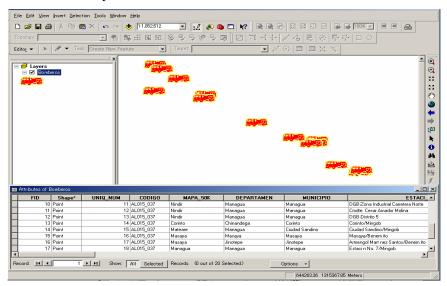


Figure 7-4 Geographic Data of Fire Department with Attached Attributes

7-3-3 Compilation of Background Data

The existing 1/250,000 scaled Digital Topographic Raster was used as background data. A total of five map sheets was required to cover the whole project area as indicated by the purple boundary lines in Figure 7-5.

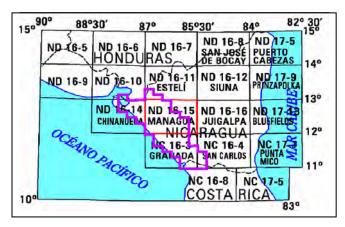
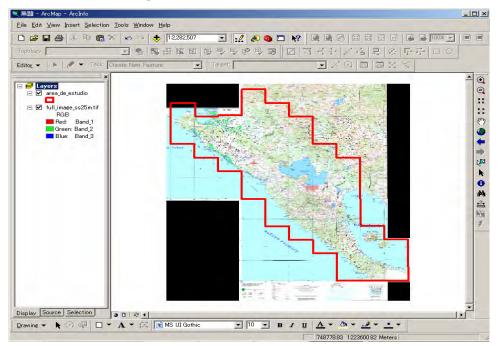


Figure 7-5 Topographic Map Sheets Covering the Entire Study Area

First, the digital raster data in the dgn format were converted into pdf using the software MicroStation/J and then into tiff format using the software Adobe Acrobat. After that, geo-rectification, mosaicing all map sheet images into one, clipping, and finally conversion into Geo-TIFF format were accomplished using the software ERDAS Imagine Version 8.7.

As shown in Figure 7-6, the Digital Topographic Raster data covering the whole Study area was achieved with the pixel size of 25m. During geo-rectification, the overall RMS (Root Mean Square) error was kept less than 0.01 of one pixel (that is, less than 0.25m for the pixel size of 25m).





7-3-4 Display of Point Data and Background Data in ArcMap

As the final step, all infrastructures (points) data layers, the Digital Topographic Raster data, and the Study area boundary were displayed altogether in ArcMap of ArcGIS. Point data were assigned with symbols having suitable size and color, and also the photo linkage was confirmed. Then, using the Map Publisher Extension of ArcGIS, all the displayed data were converted into PMF (Portable Map Format). This PMF format is com-

patible with ArcReader of ArcGIS as shown in Figure 7-7. The figure on the right displays the whole Study area and the bottom right figure shows the disaster mitigation related facilities existing around the city LEON.

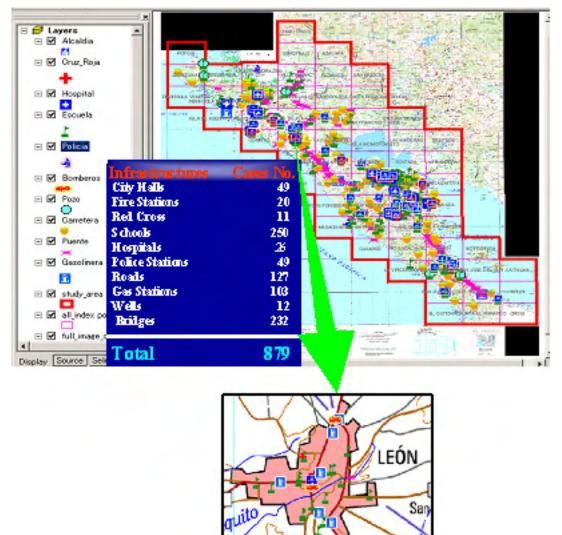


Figure 7-7 GIS Database of Infrastructures for Disaster Mitigation

The above system can display all the information by clicking the middle part of the featuring symbol. Similarly, the attached photo (image) will appear when either Hyperlink button or information button is hit and then a desired photo item is clicked.

Talchocote

8 PROCESSES OF HAZARD MAPPING

8-1 Earthquake

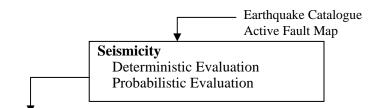
8-1-1 Basic Policy of Earthquake Hazard Mapping

Earthquake as a natural hazard includes various phenomena such as earthquake ground motion, rupture of surface ground, slope failure / landslide and liquefaction. Hazard potential of these phenomena shall be the subject of the hazard mapping. Among these phenomena, mapping of earthquake ground motions addresses one of the most fundamental aspects of future seismic hazard assessment. Firstly, the ground motions are directly related to the seismic forces acting on structures, and hence seismic zoning for ground motions provides an important initial indicator of seismic risk. Secondly, ground motions are directly related to soil failure such as slope failure and liquefaction, which can further lead to damage to structures. Zoning for ground motions is therefore an essential part of the information necessary to evaluate the overall nature of geotechnical hazards.

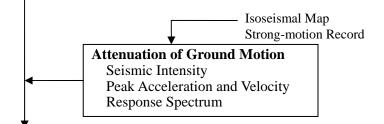
Earthquake ground motions are affected by several factors such as source, path and site effects. An assessment of ground motion therefore depends on the following:

- a) Regional seismicity;
- b) Attenuation of ground motion intensity;
- c) Local site effects on ground motion.

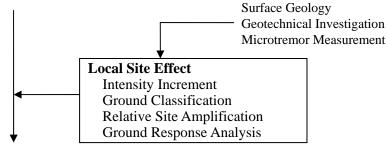
Figure 8-1 illustrates the fundamental flow of analysis and key parameters for the ground motion calculation.



Assignment of Hypothetical Earthquake



Ground Motion Intensity on Base Rock



Distribution of Ground Motion

Figure 8-1 Fundamental Flow of Analysis for the Ground Motion Calculation.

Source: TC4 Manual

8-1-2 Study on Regional Seismicity

(1) Earthquake Disaster History

Table 8-1 shows the recent observed major earthquake in Managua area. After Managua became the capital of Nicaragua, 8 number of earthquake disaster hit the region.

Year	Month	Day	Time (GMT)	Lat	Lon	М	Depth	Managua	Note
1853									Managua became capital of Nica- ragua.
1885	10	11	21:30 Local					VII	Strongly felt in Managua. Minor damage. 6 victims.
1898	4	29	22:30 Local	12.4	86.9			VII-VIII	Felt entire Nicaragua. Two events ?
1916	2	27	20:20:48	12.0	90.0	7.3	Shallow	IV	
1926	11	5	7:55:38	12.3	85.8	7.2	135 km	VII-VIII	Some victims in Managua. About half of the houses were damaged.
1931	3	31	16:02:21	13.2	85.7	5.5	Shallow	VIII	Death of 2000, several thousand injured. 35,000 homeless. \$30 mil- lion of loss.
1958	11	14	15:22:08	12.4	86.4	?	7 km	V-VI	
1968	1	4	10:03:57	12.1	86.3	4.6	5 km	VIII	Moderate damage in Managua. 300 houses in colonia centroamerica were inhabitable.
1972	12	23	6:29:42	12.4	86.1	6.2	5 km	VIII-IX	Destruction was 80 % in the center and 60 % in the peripheral area of the city. Death between 8000 and 9000. Material loss reached to \$1 billion.

Table 8-1 Major Earthquake in Managua Area

Grases J., 1990, Terremotos Destructores del Caribe 1502-1990, UNESCO-Red Latinoamericana y del Caribe de Centros de Ingenieria Sismica

(2) Tectonic Setting of the Region

Seismicity of Nicaragua is mainly caused by the Cocos Plate subducting under the North American Plate. The maximum observed earthquake generated by the subduction was 8.0. The general seismic and tectonic feature of Nicaragua is typical Benioff zone of earthquakes dipping inland from the trench. There is an obvious shallow seismic activity associated with the volcanic zone, where the more destructive earthquakes occurred. The Nicaragua Depression (Graben), lowland parallel to the coast line includes the majority of the volcanic centers in Nicaragua. This lowland is a basin covered with thick sedimentary deposits of alluvial and volcanic soil. The alignment of the volcanoes is offset at 3 places. The most interesting for the Study is near Managua offset. Within and near the boundary of the Graben, several fault system have been observed. Along the edges of the depression, dip-slip faults are observed. The most prominent faults are NE strike while NW, NS and EW striking faults are known. The 1972 Managua earthquake occurred on the NE striking Tiscapa fault and the motion was left lateral strike slip. Near Managua, there are several parallel NE striking faults and the 1931 earthquake also occurred on one of these faults close to the 1972 earthquake. Map of region around Managua and tectonic setting in Nicaragua is shown in Figure 8-2.

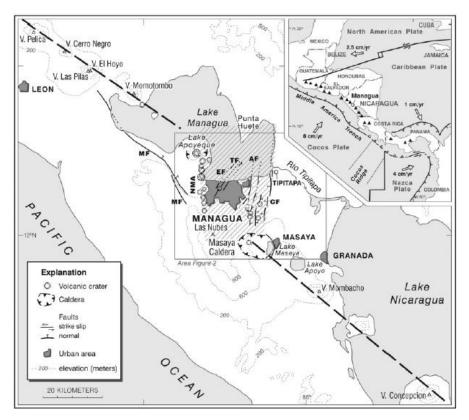
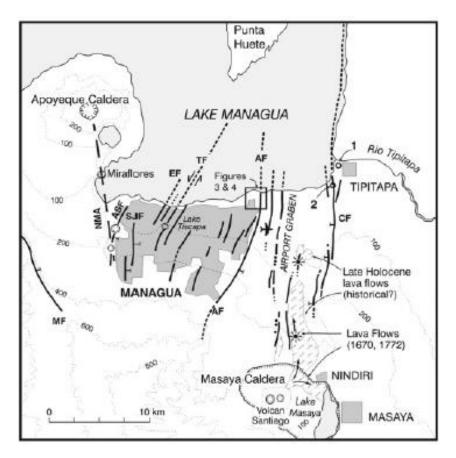


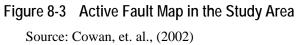
Figure 8-2 Map of Region around Managua and Tectonic Setting in Nicaragua

Source: Cowan, et. al., (2002)

(3) Active Fault in Managua Area

In the Managua area, where there is abundant evidence of recent faulting, a graben (Managua Graben) has formed at a 15-km discontinuity in the strike of the volcanic chain, from Apoyeque Caldera to Masaya Caldera, southeast of the metropolitan area. The Managua Graben and its strike-slip and oblique-normal faults show evidence of mild east-west extension and left-lateral strike slip along a north and northeast trend. The NNE–SSW trending Tiscapa fault and three adjacent parallel faults, all exhibited left-lateral and minor normal displacement during the 23 December 1972, M6.2 earthquake. Aggregate horizontal movements were in the range of 2 to 38 cm, down to the ESE. In 1931 an M 6.0 earthquake ruptured the Estadio fault, 1.5 km west of the Tiscapa fault, with a maximum observed normal slip of 10 cm and an undefined component of strike slip. The western margin of the Managua Graben is defined by cinder cones and collapse pits that extend south from the Apoyeque Caldera along a trend known as the Nejapa–Miraflores–Apoyeque alignment. The Asososca–Acahualinca and San Judas fault zones form east-facing escarpments at this margin of the graben, and Holocene displacements are documented at several localities. The eastern margin of the Managua Graben and Lake Managua are defined by the Cofradia fault. The Cofradia fault forms a prominent escarpment that extends south to the Masaya Caldera. Figure 8-3 is active fault map near Managua and the characteristics of these active faults are summarized in Table 8-2.





					Displacement		Age of Most		
Code	Fault Zone	Strike and Projected Length	Maximum Individual Reported	Maximum Scarp Height	Observed Sense of Movement	Slip Rate (mm/yr)	Recent Movement (kyr = 10 ³ yr)	Comments	References*
MF	Mateare	N50°W, ±318° 35 km to max. 70 km	Unknown	~200 m	Large component down to northeast	Unknown	Probably <20 kyr	NE-facing topographic escarpment in late Pleistocene ignimbrite deposits. No Holocene faulting reported and seismically quiescent since 1975 or longer.	1, 4, 8
NMA	Nejapa– Miraflores– Apoyeque Alignment	$\frac{\text{N10}^{\circ}\text{W},\ \pm5^{\circ}}{\sim}20\ \text{km}$	∼l m normal	~80 m	Down to east	Unknown	Probably <10 kyr	Arcuate scarps associated with volcanic craters defining the western margin of the Managua Graben.	2, 3, 8
ASF	Asososca– Acahualinca	N30°E, ±10° 5 km	~3 m normal	20 m	Mainly down to east, few scarps down to west	Unknown	<10 kyr	Arcuate east- and west-facing scarps that intersect the NMA to the south.	2, 3, 8
SJF	San Judas	N–S, ±5° 5 km to max. 10 km	1 m normal	15 m	Down to east	Unknown	Probably <5 kyr	Arcuate east-facing scrap that displaces Holocene soil. Minor cracking during 1972 earthquake.	2, 3, 8
EF	Estadio	N30°E, ±5° 2 km on land to max. 10 km beneath Lake Managua	0.5 m normal	∼1 m normal	Left-lateral strike-slip and down to east	Unknown	31 March 1931	Surface rupture along a 2-km section of the fault in 1931. Minor cracking during 1972 earthquake.	1, 5, 8
TF	Tiscapa	N24°E, ±7° 10 km to max. 20 km beneath Lake Managua	3.5 m normal	20 m normal	Left-lateral strike-slip and down to east	Probably 0–5	23 December 1972	Little geomorphic expression in former downtown area, but left-lateral offset of several meters on northeast wall of Laguna de Tiscapa. Farther SW an east-facing scarp up to 20 m high.	1, 2, 8
AF	Aeropuerto	N3°E, ±6° ~15 km plus extension beneath Lake Managua	0.5–1 m normal	10–12 m normal	Down to east Strike-slip may be important	Probably 0–5	A.D. 1650– 1810	Surface trace slightly convex to the east and the fault subvertical. Scarp has maximum height of 10–12 m west of the Airport.	6, 8
CF	Cofradia	N4°E, $\pm 13^{\circ}$ ~40 km	0.5–1 m normal	~15 m	Down to west	Probably 0–5	<5 kyr based on offset lake deposits near Tipitapa	Prominent escarpment extends south to the Masaya caldera and is asso- ciated with numerous hot springs.	7, 8

Table 8-2 Details of Active Faults

Source: Cowan, et. al., (2002)

(4) Improvement of Earthquake Catalogue

Following earthquake catalogues are collected and studied:

- a) Historical earthquake catalogue by INETER (1505 1992; hereafter called Catalog A)
- b) Instrumental earthquake catalogue by INETER (1993 2001; hereafter called Catalog B)
- c) INETER Boletines Sismologicos (1995 2003/09/309)
- d) ANSS composite earthquake catalog since 1898 present
- e) NEIC/USGS
- NEIC/USGS(PDE) 1979 present
- Significant Worldwide Earthquakes (2150B.C. 1994 A.D.)
- Mexicco, Central America, Carribean (1900 1979)
- f) CMT-solution catalog by Harvard University

Among these catalogues, a) and b) are the most comprehensive. Therefore these Catalogs A and B were selected as the base catalog. Other catalogues shown above were used for estimating the historical seismic disaster at the Managua study area in combination with candidate ground motion attenuation curves. Following correction and modification were performed on the base Catalogs A and B.

- 1) Locations and magnitudes of damaging earthquakes from 1520-1973 in the Catalog A were corrected by the Catalog by Leeds (1974).
- 2) The Instrumental Earthquakes from 1995 to the present in the above Catalog B were replaced by those in the Seismic Bulletins published by INETER.
- 3) The earthquakes designated as having an epicenter location error larger than 5 km were all excluded from the earthquakes occurred during 1995 to the present.
- 4) Volcanic events are separated by excluding the events within the particular radius from the location of historical volcano and focal depth less than 7 km.
- 5) Events following the main shocks of intermediate to large earthquakes (Mmix larger than 6.0 and depth shallower than 60) and clearly considered as the aftershocks of these main shocks were excluded.

Figure 8-4 illustrates the procedure of correction and modification of the catalogues. Figure 8-5 shows the hypocentral distribution of earthquakes by new earthquake catalog.

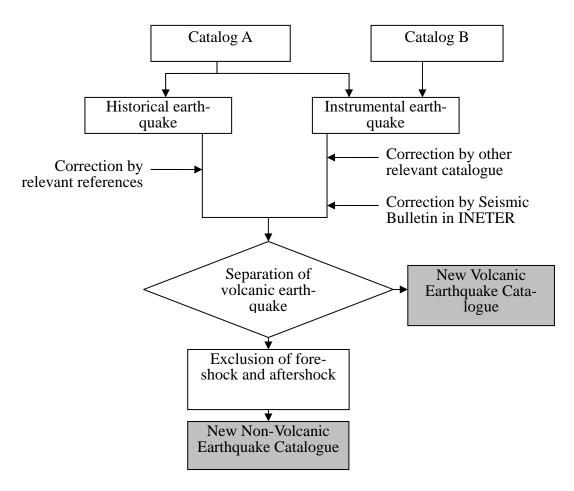


Figure 8-4 Procedure of Earthquake Catalog Correction

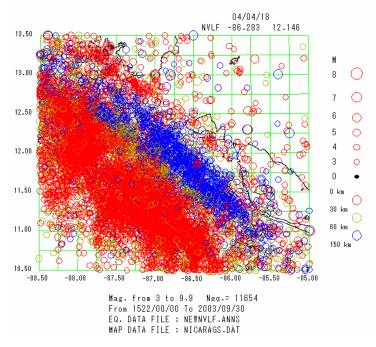


Figure 8-5 Hypocentral distribution of earthquakes in the New catalog

8-1-3 Determination of Scenario Earthquake

Based on the above studies and discussions with experts of the Counterpart, it was decided that the deterministic approach would be adopted for the simulation of the seismic motion caused by the active faults and volcanoes. Probabilistic base rock motion also adopted for 100 years return period. Following three (3) types of earthquakes are considered, and finally 5 kinds of scenario earthquakes are estimated and determined.

(1) Earthquake by Active Fault

Estadio and Tiscapa Fault are already moved recently. Following two faults those exist in the east of Managua city are modeled.

- Aeropuerto Fault
- Cofradia Fault

From Figure 8-3 which was compiled by USGS, the surface trace of the faults are assumed by two straight line each. The north extension of these two faults can not be observed at the north shoreline of Lake Managua. Therefore, the north end of these faults are probably in Lake Managua. From the view points of the scenario earthquake, we assumed that the north ends of these two faults are extended to the north shoreline of Lake Managua. The magnitudes of these scenario earthquakes are determined through the empirical formula by Donald L. Wells et. al (1994).

(2) Volcanic Earthquake

The largest earthquakes that occurred around following active volcanoes are modeled. Through the study of newly developed volcanic earthquake catalogue, it is found that the magnitude of volcanic earthquakes larger than 5.5 should be reduced 0.5 to 1.0 after the analysis by Dr. Katayama.

- Apoyeque	M=6.0	(1852 year)
- Masaya	M=6.0	(1772 year)

The location of the scenario earthquake models of the active faults and volcanoes are shown in Figure 8-6. The parameters of these scenarios are tabulated in Table 8-3.

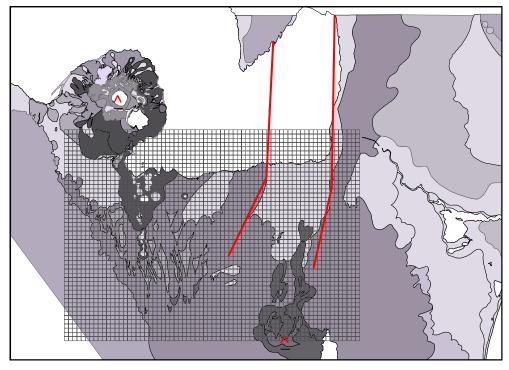


Figure 8-6 Location of Fault Model and Volcanic Model

				Activ		Vol	canic	
Name				puerto	Cofr	adia	Apoyeque	Masaya
Name			Northern Line	Southern Line	Northern Line	Southern Line		
	Latitude		12.300	12.156	12.332	12.152	12.242	11.984
North End	Longitude		-86.175	-86.183	-86.107	-86.111	-86.342	-86.161
	UTM Zone16,	Ν	1359877	1343917	1363451	1343494	1353401	1324923
	WGS84	Е	589675	588856	597041	596734	571566	591341
	Latitude		12	12	12	12	/	
South End	Longitude		-86	-86	-86	-86		
SOUTETIU	UTM Zone16,	Ν	1343917	1334844	1343494	1333711		
	WGS84	Е	588856	584720	596734	594564		
Length	(km)		16.0	10.0	20.0	10.0	0	0
Total Length	(km)		2	6.0	30).0	0	0
Width	(km)		8.0	5.0	10.0	5.0	0	0
Dip Angle	(deg.)		(90	90		0	0
Depth	(km)		8.0	5.0	10.0	5.0	0	0
Туре			oblique	e-normal	nor	mal	point source	point source
Magnitude	Mw		6.7		6.8		6.0	6.0

 Table 8-3
 Parameter of Scenario Earthquake

(3) Probabilistic Base Rock Motion

An analysis to establish a probabilistic scenario earthquake of Managua City is performed. For this study, the newly improved earthquake catalogue is used and analyzed. The target earthquakes are the earthquake occurring in the land area and frontal sea of Nicaragua having focal depths ranging from 0 to 90 km and magnitudes larger than 3.5 excluding those of volcanic origin and known active faults.

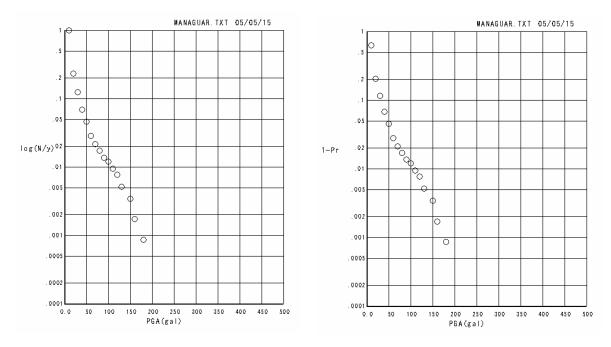
In order to obtain the probabilistic base rock motion in Managua City, following process are applied.

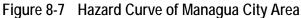
(a) B-value analysis

From the analysis, generally standard value of approximately 1.0 is obtained in case that events having moment magnitude less than 4.5 are excluded. Therefore, for the probabilistic analysis, events of Mw<4.5 are eliminated.

(b) Generation of Hazard Curve which predicts the relationship between the probability and the events causing a particular level of PGA

The events, which are assumed to cause potential earthquakes associated with existing causative faults in the study area, are already taken into consideration in the previous section (a). Therefore, those historical earthquakes clearly considered as associated with existing active faults systems within the study area including the main shock and aftershocks of the 1972 Managua Earthquake are excluded in hazard curve generation. From the hazard curve shown in Figure 8-7; the maximum acceleration of 100-year-return period is estimated as 110 gals.





(c) Generation of the maximum probabilistic acceleration on the bed rock

The probabilistic ground motion time-histories are generated using a Magnitude-Frequency relationship of events corresponding with the above hazard curve and other random variables similarly as described in the reference by Katayama and others (2001) for the bed rock motions.

From the generated time-histories, the maximum acceleration expected at the bed rock of Managua city center is defined as the geometric average of peak accelerations with the standard deviation. Thus, the maximum probabilistic acceleration on the bed rock is computed as 110 gals with the standard deviation of 28 gals.

8-1-4 Selection of Attenuation Formula

Some empirical attenuation laws for maximum ground motions are tested whether they can properly explain the past historical disaster and recorded ground motion intensities at Managua city. In selection of attenuation laws, laws which were used in the past for the seismic hazard of Managua, and laws which are the most current and considered as conceivably appropriate in light of characteristics of earthquake source region in and around Nicaragua including Managua city were selected.

The following three types of attenuation laws and application procedures were selected:

1) Sherdan and Turnner (GSHAP 1999)

This attenuation law has been used in the seismic hazard analysis of Central America and also of Managua city and the result is shown in INETER's Web site. This attenuation law consists from two different empirical formula; one is adopted from the paper by Joyner and Boore(1981) which is applied to the events occurring in the upper crust of continental slab and the other is considered as built by the authors and applied to the events occurring at the interface between the subducting plate and continental crust and also to the events within subducting plate (deep earthquakes of depth up to 250 km).

2) Campbell (1989)

This formula is built with the aim to be applied to small to medium magnitude earthquakes of very shallow depth defined by the local magnitude ML and therefore considered a good candidate attenuation law for those earthquakes of volcanic origin which usually occur in very shallow depth and with small magnitude such as those detected in Managua. This law assumes the point source in calculating the source-site distance.

3) Combined formula of Joyner and Boore (1981) and Young et al (1997)

This formula is newly developed by the following scheme of combined formula proposed by Sherdan and Turnner (GSHAP 1999). But the first and second attenuation law were replaced by those given by Joyner and Boore (1981) and Young et al (1997), which are tractable in professional papers for the method and data used. Especially the latter empirical law has been built using some strong records obtained at earthquakes occurred in Central and South America. The application rule of this attenuation law is similar to the previous one by Sherdan and Turnner (GSHAP 1999).

Using these laws, expected maximum acceleration and MM Intensity at Managua by historical earthquakes from the Significant Earthquake Catalog compiled by ANSS (Advanced Seismic System; USGS) were calculated and compared with the reported disaster records. Detailed procedure and result of the study are presented in the 2-1 Earthquake in Manual 2 Hazard Mapping. Non-volcanic and volcanic events were respectively calculated their maximum accelerations by these candidate laws. Finally, it is concluded that the new combined Joyner-Boore and Young law is considered as appropriately applicable to the recorded accelerations of both non-volcanic and volcanic events. The equation is expressed as follows:

For the events whose focal depth is shallower than 15 km,

$$\log A = -1.02 + .249 M w - \log \sqrt{(D^2 + 7.3^2)} - .00255 \sqrt{(D^2 + 7.3^2)},$$

and for the events whose focal depth is deeper than 15 km,

$$A = 0.2418 + 1.414 Mw - 2.552 \ln(R_1 + 1.7818 e^{.554 Mw}) + .00607 FD$$
$$+ .3846Z_{T}$$

where D: horizontal distance to surface projection of fault (km)

$$Z_{T} = 0 \quad \text{for interface events}$$

$$Z_{T} = 1 \quad \text{for intra-slab events}$$

$$RD = 0.5 * \sqrt{10^{(Mw-4)}}$$

$$if \quad RD > 37; \quad RD = 37$$

$$R_{1} = \sqrt{(D^{2} + FD^{2})}$$

$$if \quad R < RD; \quad R_{1} = RD$$

$$if \quad R \ge RD \qquad R_{1} = R - RD$$

Here, conversion rule of Magnitudes used to finally obtain the Moment Magnitude Mw is as follows:

From ML to Ms:
$$Ms = (0.8ML - 0.01ML^2 - 0.8)/0.63$$

From Mb to Ms: $Ms = 1.74Mb - 3.95$
From Ms to Mw: if $Ms \le 6.6$ then $Mw = 2/3Ms + 2.43$,
and if $Ms > 6.6$ then $Mw = Ms$.

When the type of M is unknown, M is assumed as Ms.

Log a= -1.02 + 0.249 M - log R - 0.00255 R Where a: peak ground acceleration (G) M: moment magnitude $R^2 = d^2 + 7.3^2$

d: horizontal distance to surface projection of fault (km)

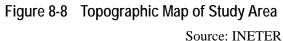
8-1-5 Ground Model for Subsurface Amplification

(1) Topography of Study Area

Most of the city of Managua is on a surface dipping a gentle degree towards the north. A several north flowing drains flow into Lake Managua. All are small and maximum depth of valley is less than 20 m in the city. In southern part of the Study area, deeper valley is common in the hill area lying west of Masaya Caldera. The gently north dipping surface of Managua city is interrupted in several places by low hills. Most of them are clearly recent volcanic origin. Tiscapa in the center of the city, the hill enclosing Lake Asosoca on the west, and the Nejapa pits southwest of Managua are examples. This surface appears to be graded to Masaya Caldera and other volcanic centers.

The southern part of Study area shows steep hilly topography continued from the Mateare mountains which show quite clear lineament of Mateare fault ranging NW to SE. Town of El Crucero is located on the crossing point of the Mateare Mountains and the Managua mountains and on relatively gentle hilly topography around 900 m elevation. The Managua Mountains stretch towards east from the town El Crucero, also show clear lineament and peter out in Managua Graben near Masaya. Ridges of east part of the Managua Mountains are very thin and steep. Figure 8-8 is the existing topographical map of the Study area.

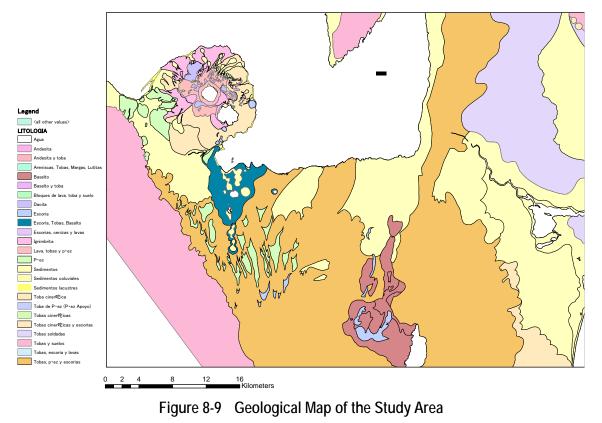




(2) Geology of Managua Region

The geologic material in the Managua and surrounding region originates as deposits from various volcanic centers. Four volcanic centers are identified to form the bulk of the material visible on the surface of the region. These are 1) Masaya, 2) Apoyeque, 3) Asosoca, Tiscapa, Nejapa-Ticomo and associated cones, and 4) Talpetas and Miraflores. Many research works have been conducted to identify the origin and the chronologic relationship of these volcanic deposits.

On the other hand, from the engineering point of view, the geology in the area is relatively homogeneous and no clear difference can be seen by its origins. Geology of the area can be grouped according to their physical characteristics. Five categories of materials may be differentiating. They are 1) Loose tephra, 2) Volcanic mudflow and ash flow deposits, 3) Hard volcanic rock 4) Alluvium, and 5) Residual soil and top soil. Figure 8-9 is a geological map of Managua area prepared by INETER



Source: INETER

(3) Compilation of Existing Geological Data

Geological data is compiled based on existing 170 boreholes data. The almost all borehole data is located on the central Managua city area. In the meanwhile, there is not any borehole data on the other area. Therefore, ground characteristics of the area except Managua city area is estimated based on the Managua city area's data.

Four geological profiles are prepared in East-West direction and three profiles in South-North direction based on the existing surface geological map with the existing borehole data. Location of these profiles is shown on Figure 8-10. The grid system that is determined on the preceding paragraph and location of the existing boreholes are also shown on the Figure. Examples of the profiles are shown on Figure 8-11.

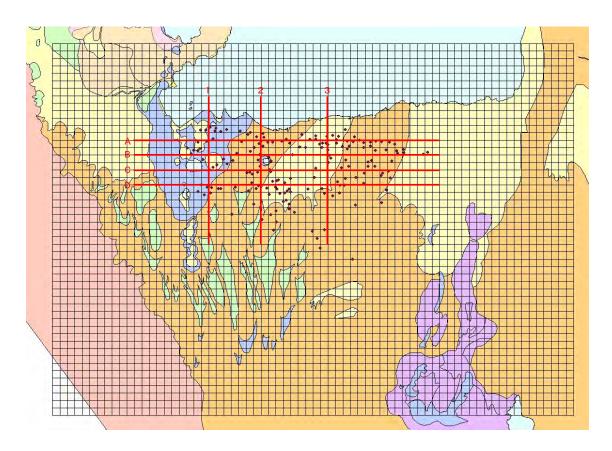


Figure 8-10 Location of Geological Profiles

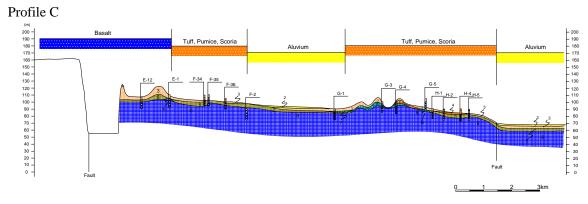


Figure 8-11 Example of Geological Profile (Profile C)

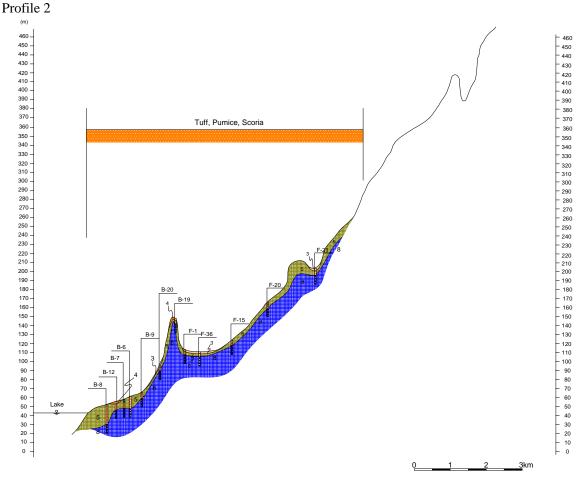


Figure 8-12 Example of Geological Profiles (Profile 2)

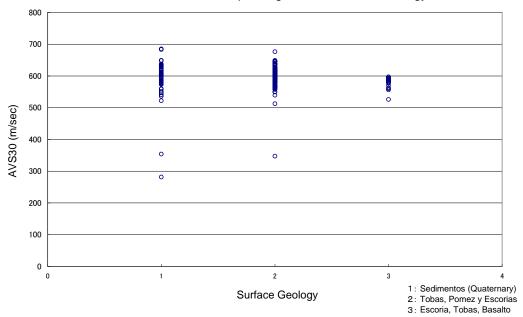
(4) Ground Classification

Ground classification is used to calculate the amplification of ground surface. In this study, the ground type is classified by Average Shear-wave Velocity over the upper 30 m (AVS30). AVS30 is authorized on "Recommended Provisions for Seismic Regulations for New Buildings and Other Structures," (1997 edition, FEMA-302, 303; BSSC, 1997) by NEHRP (National Earthquake Hazards Reduction Program) in U.S.A. Table 8-4 shows ground type classification of NEHRP.

Site Class	Average S Wave Velocity Over Upper 30m			
А	>1500m/sec			
В	760 - 1500m/sec			
С	360 - 760m/sec			
D	180 - 360m/sec			
Е	<180m/sec			

Distribution of AVS30 is compiled depending on each type of surface geology, each

type of soil that is observed on the boring core samples and each depth of distribution of Shear-wave velocity. Figure 8-13 shows distribution of the AVS30 of borehole point on each type of surface geology.



Distribution of AVS30 depending on Each Surface Geology

Figure 8-13 Distribution of AVS30 on Each Type of Surface Geology

As the result of the study, the following points are recognized.

- As the distribution of the average Shear-wave velocity of each layer that is specified as seven layers based on the existing borehole data, there are not distinctive features except few locations.
- AVS30 of almost all existing borehole data shows from Vs=500m/sec to 700m/sec, which is corresponded to C class in NEHRP classification (refer to Table 8-4). Therefore, AVS30 will be irrelevant to surface geological type in the study area.
- There are locally some boreholes showing Vs of 300m/sec (it is corresponded to D class in NEHRP classification) on old crater at 1 km north from Asososca Lake.
- The layer that is considered as base rock is found on average 10m depth on almost all borehole points. The base rock layer at the borehole point showing Vs = 300m/sec is found deeper than 30m. Therefore, Vs of this point is lower than other borehole points.
- The cross section shows that the slope angle increases from the Managua Lake to the mountain area. However AVS30 is not changed by the landform.
- No relevance was found between distribution of surface geology and geological stratigraphy of existing borehole data.

Thus, the preparation of the ground model by the distribution of the surface geology is found difficult. Therefore, the ground type is not classified to apply for the NEHRP classification by the geological information, but by geographic information.

(5) Ground Model

Ground type is classified five types as Lower plane, Middle plane, Hill area, Mountain

area and Crater area by Satellite image.

Almost all existing borehole points are located on the Hill area. AVS30 of these borehole points are varied from 500 to 700m/sec, which is corresponded to C class (Vs=360 to 760m/sec) in NEHRP classification (refer to Table 8-4). Therefore, the Hill area is defined as C class. There are some borehole points showing Vs of approximately 300m/sec. These Vs is classified as D class (Vs=180 to 360m/sec) in NEHRP classification. These points are located on old crater that is estimated by the geographic and geological information. Therefore, the Crater area is defined as D class. In addition, old crater feature on north part of the Masaya Lake is found by the satellite image. This area is covered by volcanic materials such as tuff, pumice and scoria on the surface unlike other old craters that are covered by Quaternary sediments. The old crater feature feature classification is defined as D class in here. Propriety of the old crater feature classification shall be considered with the local specialists and engineers.

In the Mountain area, bedrock is considered to be found at a shallow depth because sediment will covers on the Mountain area thinly. Therefore, the Mountain area is defined as B class (760 to 1500m/sec).

There are only two borehole points on the Lower plane (D-6 and D-7). AVS30 of both borehole points are 630m/sec and 685m/sec. These Vs are almost same as Vs of the other borehole points on the Hill area. Therefore, the Lower plane is defined as C class.

There are not any borehole data on the Middle plane. It is difficult to consider that sediment layer on the area is thicker than the Hill area's. Therefore, the Middle plane is defined as C class. Figure 8-14 shows the ground model of the study area.

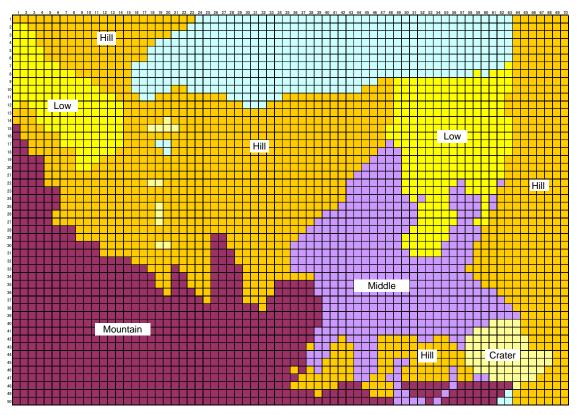
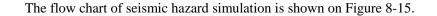


Figure 8-14 Ground Model of the Study Area

8-1-6 Development of Simulation Software



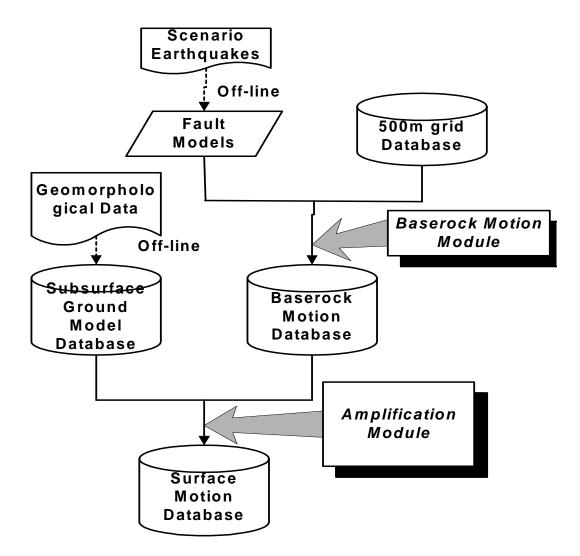


Figure 8-15 Flow Chart of Seismic Hazard Simulation

In the chart, "Base rock Motion Module" and "Amplification Module" are called as simulation software. Base rock Motion Module calculates attenuation. Meanwhile, Amplification Module calculates amplification of the ground surface.

As described in Section 8-1-4, the following formulas are used to calculate attenuation.

In case of earthquake by active fault or volcano, the following formula (Joyner and Boore (1981)) is applied.

$$\log(A) = -1.02 + 0.249M - \log\sqrt{D^2 + 7.3^2} - 0.00255\sqrt{D^2 + 7.3^2}$$

A: acceleration (g)

M : moment magnitude

D: closest horizontal distance to the surface projection of the rupture plane (km)

In case of probabilistic earthquake, the attenuation formula is not applied. Bed rock motion for amplification calculation is equally set as 138 gals for all grids.

As determined in the ground modeling, amplification of ground surface recommended by NEHRP is applied as shown in Table 8-5.

	Mapped Maximum Considered Earthquake Spectral Response Acceleration at Short Periods								
Site Class	Ne	sponse Acc		Short Fend	Jus				
	$S_S \leq \ 0.25$	S _S =0.50	S _S =0.75	S _S =1.00	$S_S \geq 1.25$				
Α	0.8	0.8	0.8	0.8	0.8				
В	1.0	1.0	1.0	1.0	1.0				
С	1.2	1.2	1.1	1.0	1.0				
D	1.6	1.4	1.2	1.1	1.0				
E	2.5	1.7	1.2	0.9	0.9				
F	а	а	а	а	а				

 Table 8-5
 Amplification of Ground Surface

NOTE: Use straight line interpolation for intermediate values of S_s . a: Site-specific geotechnical investigation and dynamic site response analyses shall be performed.

8-1-7 Preparation of Earthquake Hazard Map

(1) Assumed User

The output of the hazard map is mainly peak ground acceleration (PGA). This unit of physical vibration of the ground is generally used for the seismic code for the construction design and damage/risk estimation of building and infrastructure. Therefore, we assumed the end user of the earthquake hazard map shall be scientists, researchers and engineers who are involved in the building design, construction, city planning and disaster management.

(2) Size of Map

The earthquake hazard map will be distributed to the assumed end users. However, the map will not be used for the actual determination of the seismic code, city planning nor disaster management plan. In order to conduct these actual works, more detailed information, such as location of the objective building, location of disaster prevention resources, land use map and existing city planning map are necessary. Therefore, the role of the map is to inform the end users that the PGA distribution map is available in GIS database in INETER and these users can utilize the database for their objectives.

From this viewpoint, the scale of the map is not necessary to be large. We determine the size of the map to be 17x22 inches. Accordingly, the scale of the map is set approximately 1/100,000.

(3) Contents of Hazard Map

Based on the above assumption of the end users, following contents are determined through the discussion with the counterpart.

- Distribution of peak ground acceleration (PGA)
- Earthquake source parameters
- Adopted attenuation formula
- Meaning of PGA in terms of seismic intensity.

The meaning of PGA is not easy to understand for the end users except the seismic design engineer. Therefore, a description of the PGA in terms of the seismic intensity (MMI) is presented in the map. The seismic intensity (MMI) scale is based on human observation of shaking and damage during an earthquake. The relation of seismic intensity and physical parameters of seismic waves, such as acceleration, velocity and spectral intensity, is studied by many researchers. In the map, the consideration of Trifunac and Brady (1975) is adopted. The relation between PGA and MMI is shown in Figure 8-16.

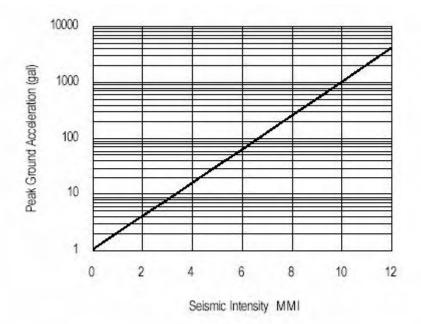


Figure 8-16 Relation between Peak Ground Acceleration (PGA) and Seismic Intensity (MMI) Source: Trifunac and Brady (1975)

(4) Example of Hazard Map

Figure 8-17 shows an example of earthquake hazard map which is in case of Cofradia Fault.

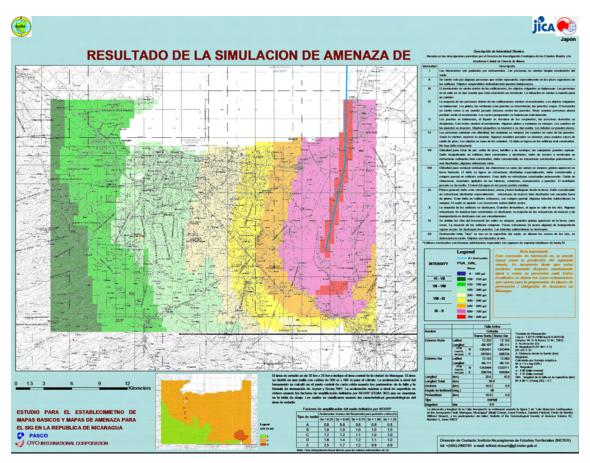


Figure 8-17 Example of Earthquake Hazard Map (Cofradia Fault)

In addition to the map shown in Figure 8-17, four other scenarios are mapped and printed 500 copies each, thus 2,500 sheets total.

Reference

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