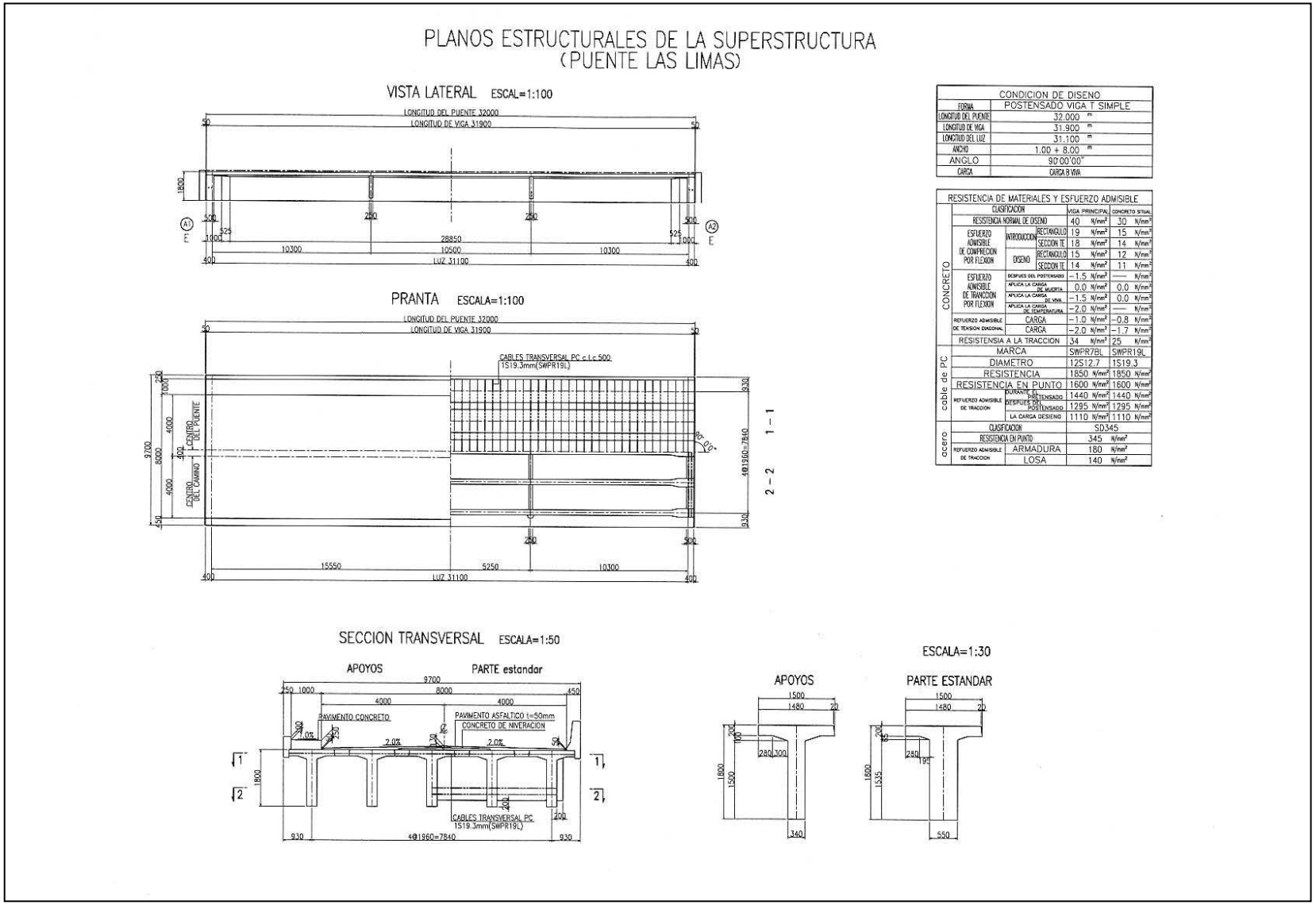
Anexo 6 Planos Estructurales de la Superestructura y Subestructura

(1) Puente Las Limas



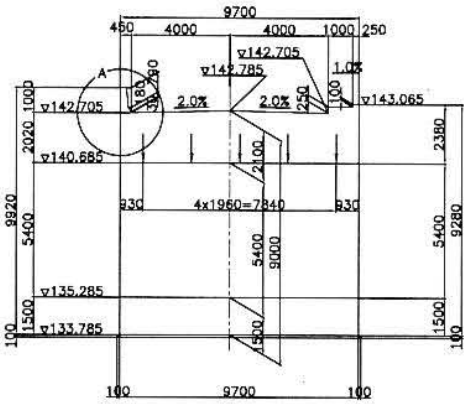
CONDICION DE DISEÑO	
FORMA	POSTENSADO VIGA T SIMPLE
LONGITUD DEL PUENTE	32.000 m
LONGITUD DE VIGA	31.900 m
LONGITUD DEL LUZ	31.100 m
ANCHO	1.00 + 8.00 m
ANGLO	90°00'00"
CARGA	CARGA B VIVA

RESISTENCIA DE MATERIALES Y ESFUERZO ADMISIBLE						
	CLASIFICACION			VIGA PRINCIPAL	CONCRETO S/TVAL	
CONCRETO	RESISTENCIA NORMAL DE DISEÑO			40 N/mm²	30 N/mm²	
	ESFUERZO ADMISIBLE DE COMPRECION POR FLEXION	INTRODUCCION	RECTANGULO	19 N/mm²	15 N/mm²	
			SECCION TE	18 N/mm²	14 N/mm²	
		DISEÑO	RECTANGULO	15 N/mm²	12 N/mm²	
			SECCION TE	14 N/mm²	11 N/mm²	
	ESFUERZO ADMISIBLE DE TRANCCION POR FLEXION	DESPUES DEL POSTENSADO		−1.5 N/mm²	—— N/mm²	
		APLICA LA CARGA DE MUERTA		0.0 N/mm²	0.0 N/mm²	
		APLICA LA CARGA DE VIVA		−1.5 N/mm²	0.0 N/mm²	
		APLICA LA CARGA DE TEMPERATURA		−2.0 N/mm²	—— N/mm²	
	REFUERZO ADMISIBLE DE TENSION DIAGONAL	CARGA		−1.0 N/mm²	−0.8 N/mm²	
		CARGA		−2.0 N/mm²	−1.7 N/mm²	
	RESISTENSIA A LA TRACCION			34 N/mm²	25 N/mm²	
cable de PC	MARCA			SWPR7BL	SWPR19L	
	DIAMETRO			12S12.7	1S19.3	
	RESISTENCIA			1850 N/mm²	1850 N/mm²	
	RESISTENCIA EN PUNTO			1600 N/mm²	1600 N/mm²	
	REFUERZO ADMISIBLE DE TRACCION	DURANTE EL PRETENSADO		1440 N/mm²	1440 N/mm²	
		DESPUES DEL POSTENSADO		1295 N/mm²	1295 N/mm²	
		LA CARGA DESIENO		1110 N/mm²	1110 N/mm²	
acero	CLASIFICACION			SD345		
	RESISTENCIA EN PUNTO			345 N/mm²		
	REFUERZO ADMISIBLE DE TRACCION	ARMADURA		180 N/mm²		
		LOSA		140 N/mm²		

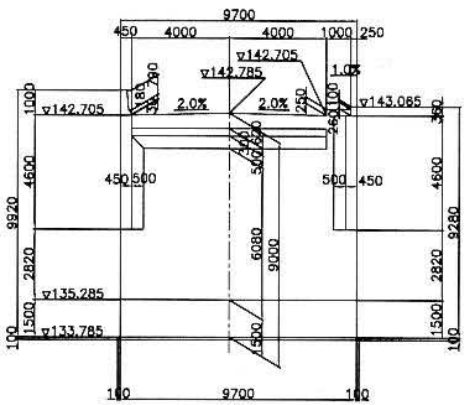
A1DISPOSION GENERAL DEL ESTRIBO (PUENTE LAS LIMAS)

ESCALA=1:100

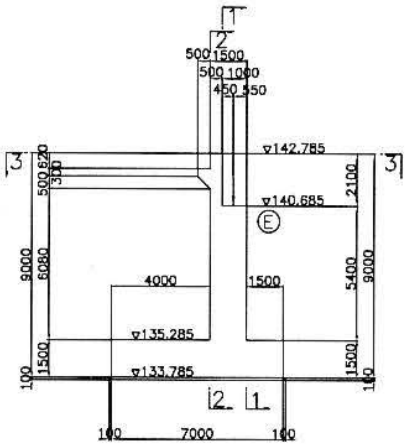
1–1



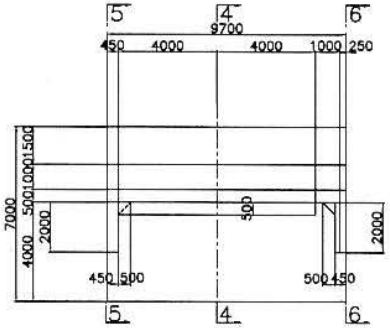
2–2



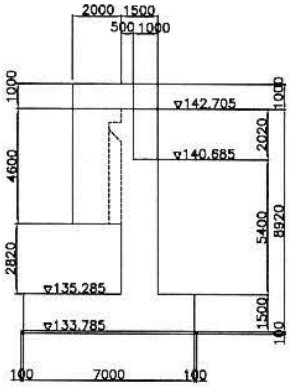
4–4



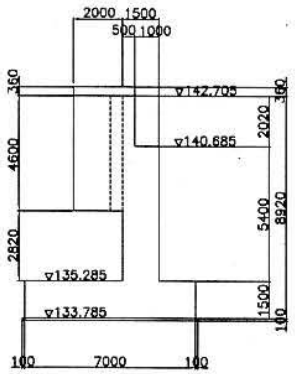
3–3



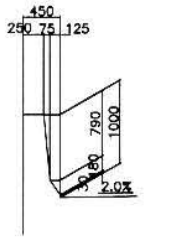
5–5



6–6

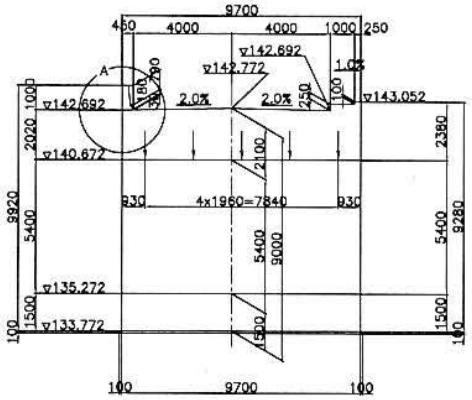


DETALLE"A" SECALA=1:30

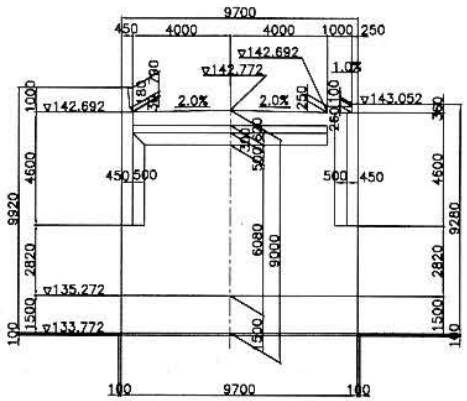


A2 DISPOSION GENERAL DEL ESTRIBO (PUENTE LAS LIMAS) ESCALA=1:100

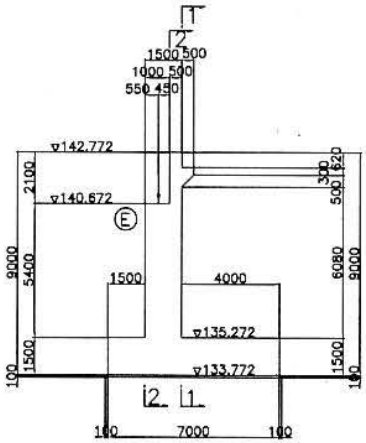
1–1



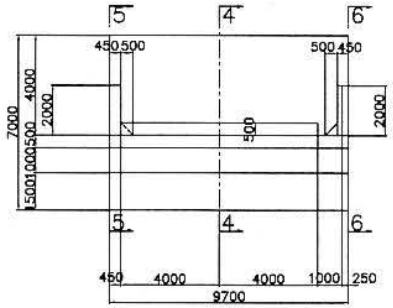
2–2



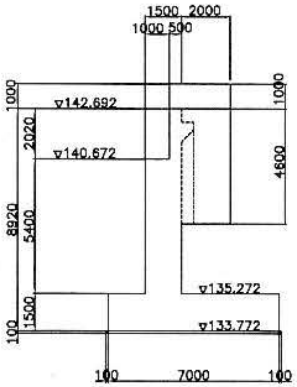
4–4



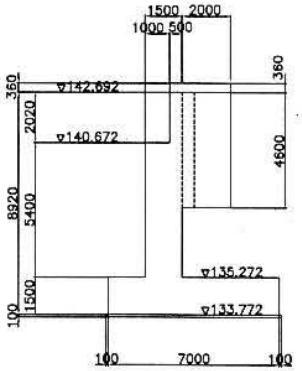
3–3



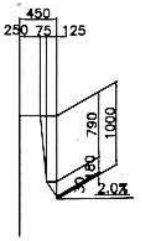
5–5



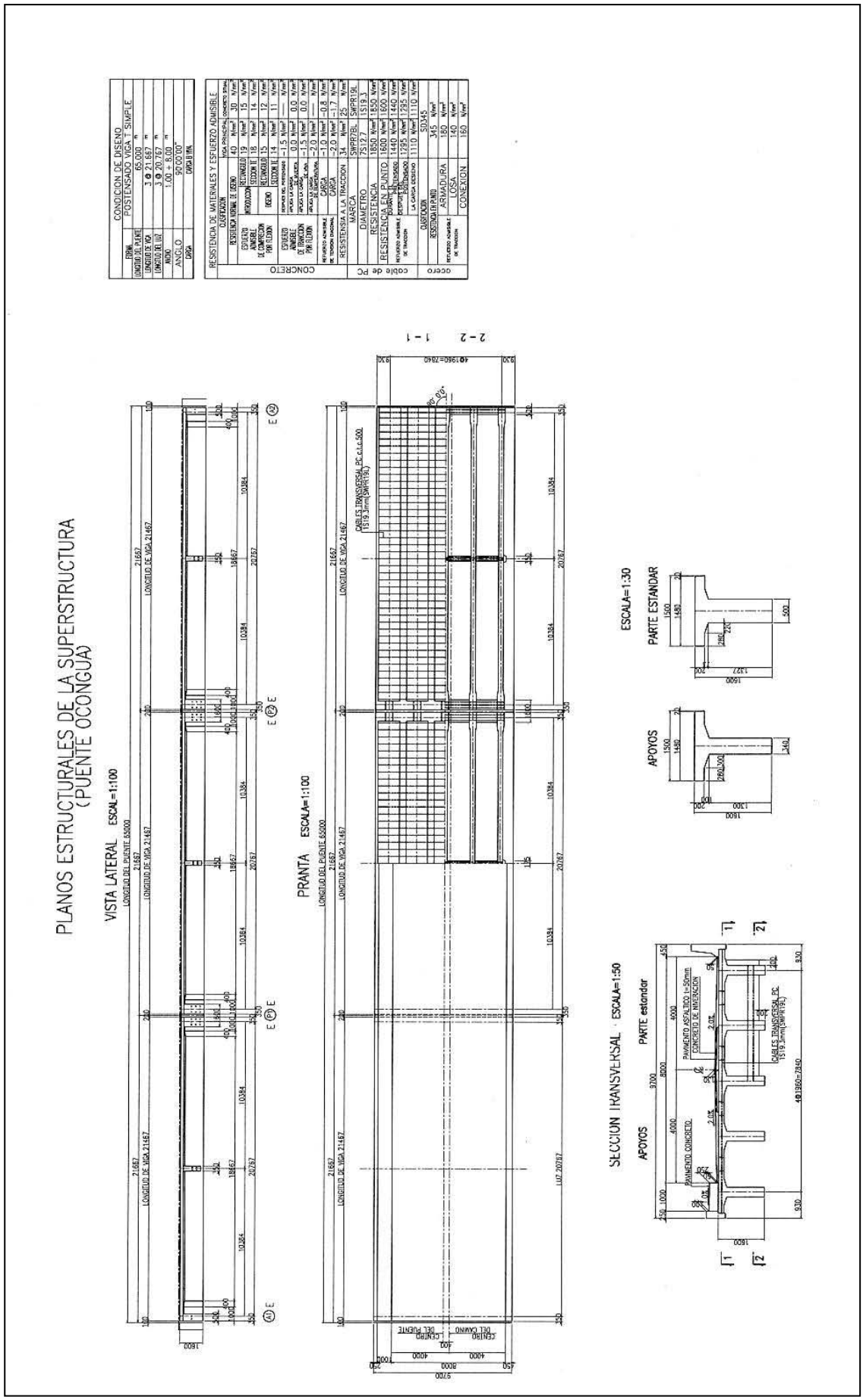
6–6



DETALLE"A" SECALA=1:30

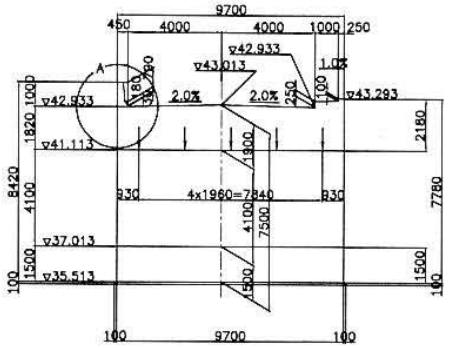


(2) Puente Ocongua

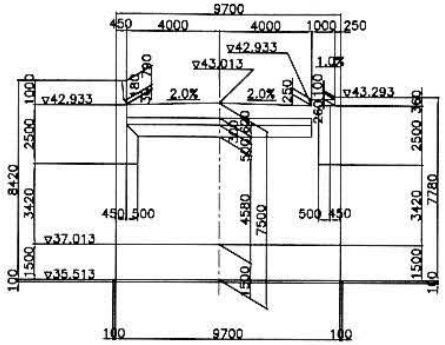


A1 DISPOSION GENERAL DEL ESTRIBO (PUENTE OCONGUA) ESCALA=1:100

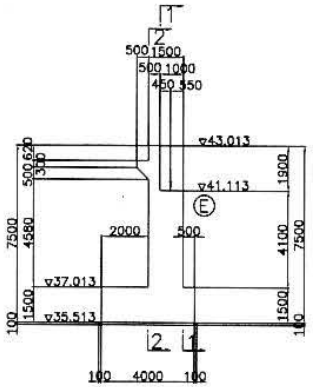
1–1



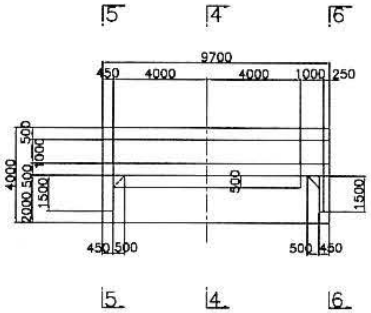
2–2



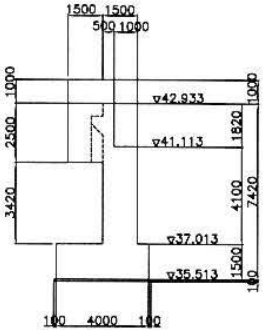
4–4



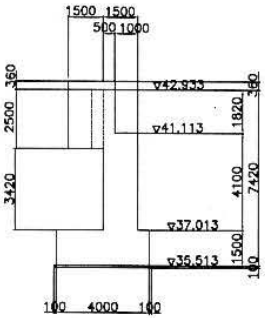
3–3



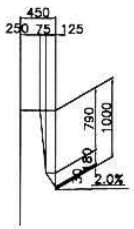
5–5



6–6



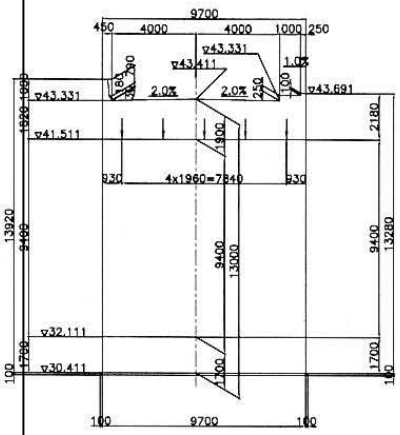
DETALLE"A" ESCALA=1:30



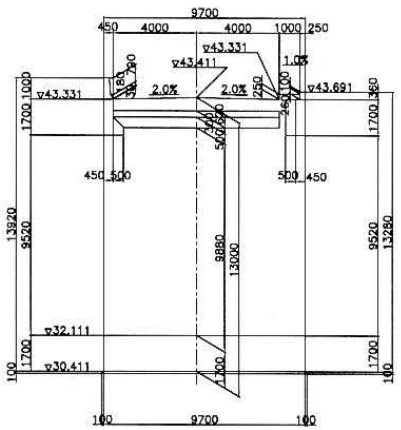
A2DISPOSION GENERAL DEL ESTRIBO (PUENTE OCONGUA)

ESCALA=1:100

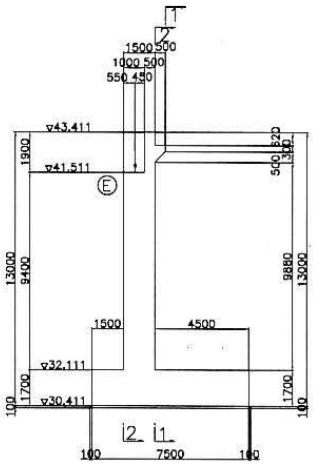
1–1



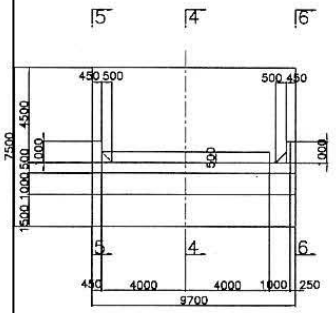
2–2



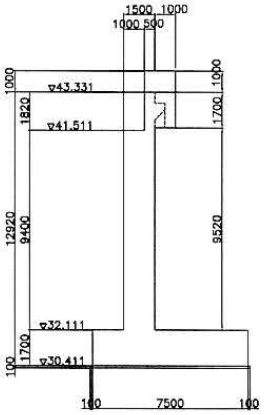
4–4



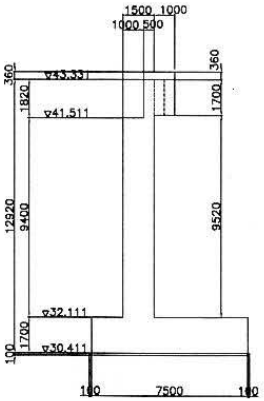
3–3



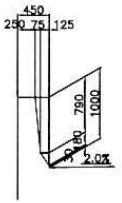
5–5



6–6



DETALLE"A" SECALA=1:30

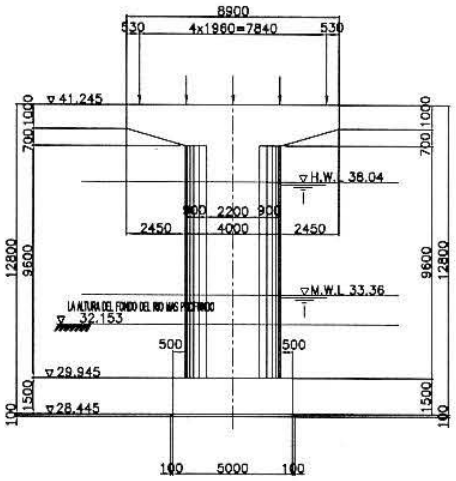


DISPOSION GENERAL DEL PILAR (PUENTE EL OCONGUA)

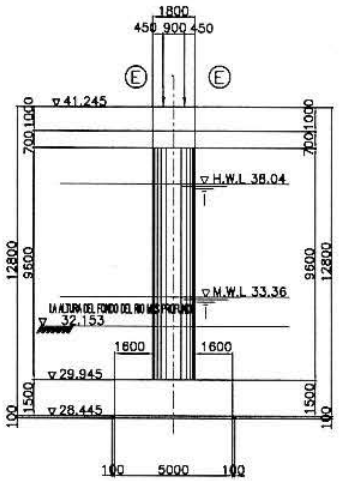
ESCALA=1:100

P1 VISTA

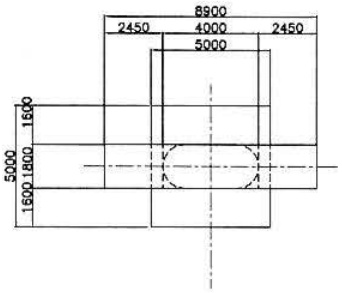
VISTA DE CORTE



VISTA LATERAL

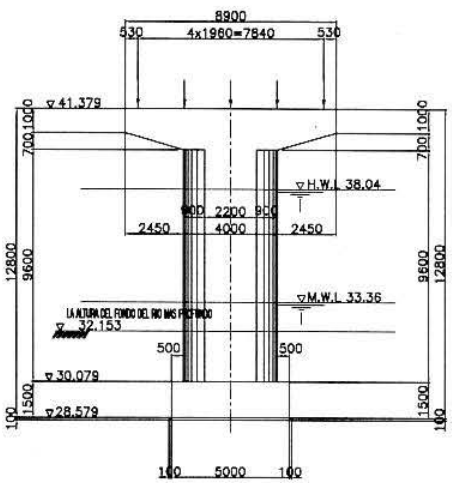


PLANTA

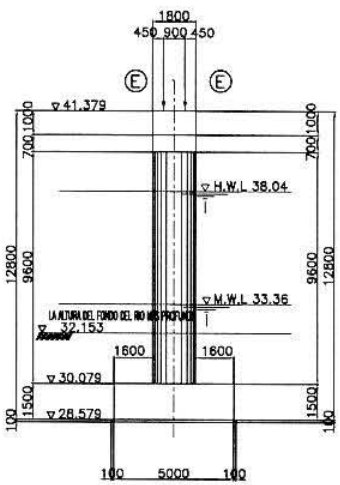


P2 VISTA

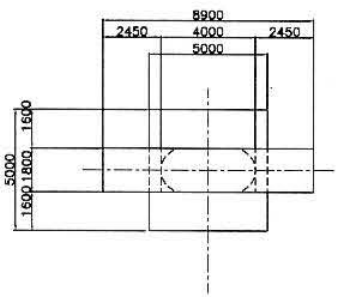
VISTA DE CORTE



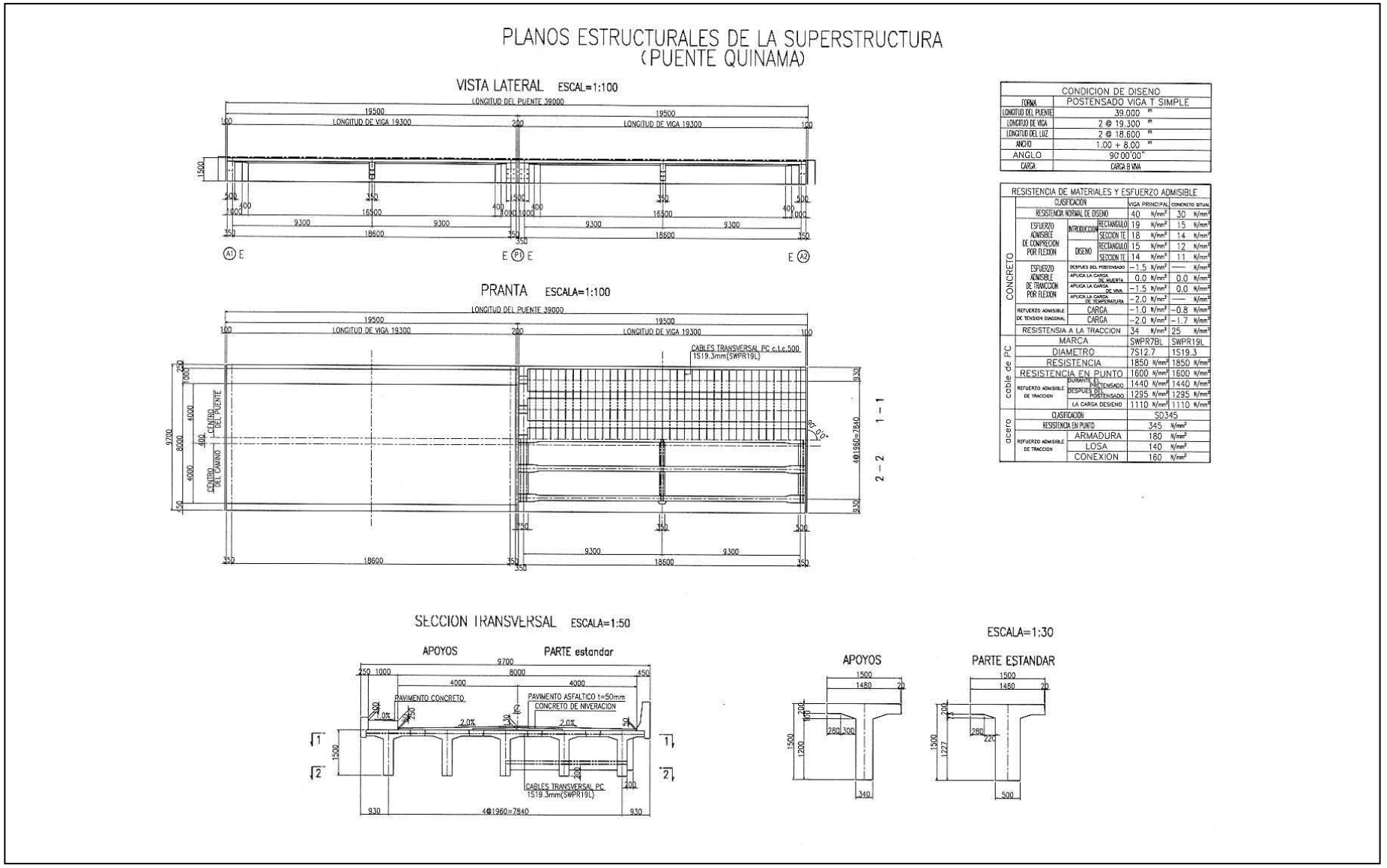
VISTA LATERAL



PLANTA



(3) Puente Quinama

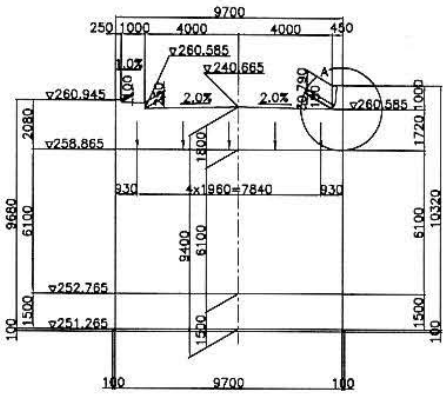


CONDICION DE DISEÑO	
FORMA	POSTENSADO VIGA T SIMPLE
LONGITUD DEL PUENTE	39.000 m
LONGITUD DE VIGA	2 @ 19.300 m
LONGITUD DEL LUZ	2 @ 18.600 m
ANCHO	1.00 + 8.00 m
ANGLO	90°00'00"
CARGA	CARGA B VNA

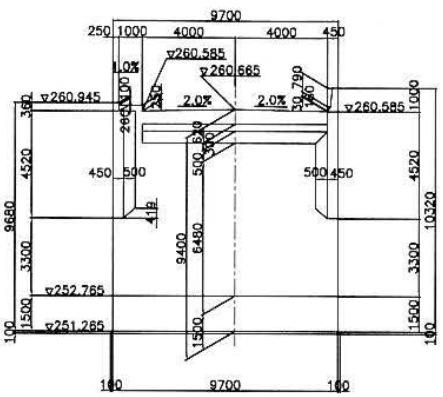
RESISTENCIA DE MATERIALES Y ESFUERZO ADMISIBLE					
	CLASIFICACION			VIGA PRINCIPAL	CONCRETO SITIAL
CONCRETO	RESISTENCIA NORMAL DE DISEÑO			40 N/mm²	30 N/mm²
	ESFUERZO ADMISIBLE DE COMPRECION POR FLEXION	INTRODUCCION	RECTANGULO	19 N/mm²	15 N/mm²
			SECCION TE	18 N/mm²	14 N/mm²
		DISEÑO	RECTANGULO	15 N/mm²	12 N/mm²
			SECCION TE	14 N/mm²	11 N/mm²
	ESFUERZO ADMISIBLE DE TRACCION POR FLEXION	DESPUES DEL POSTENSADO		−1.5 N/mm²	—— N/mm²
		APLICA LA CARGA DE MUERTA		0.0 N/mm²	0.0 N/mm²
		APLICA LA CARGA DE VIVA		−1.5 N/mm²	0.0 N/mm²
		APLICA LA CARGA DE TEMPERATURA		−2.0 N/mm²	—— N/mm²
	REFUERZO ADMISIBLE DE TENSION DIAGONAL	CARGA		−1.0 N/mm²	−0.8 N/mm²
		CARGA		−2.0 N/mm²	−1.7 N/mm²
	RESISTENSIA A LA TRACCION			34 N/mm²	25 N/mm²
cable de PC	MARCA			SWPR7BL	SWPR19L
	DIAMETRO			7S12.7	1S19.3
	RESISTENCIA			1850 N/mm²	1850 N/mm²
	RESISTENCIA EN PUNTO			1600 N/mm²	1600 N/mm²
	REFUERZO ADMISIBLE DE TRACCION	DURANTE EL PRETENSADO		1440 N/mm²	1440 N/mm²
		DESPUES DEL POSTENSADO		1295 N/mm²	1295 N/mm²
		LA CARGA DESIENO		1110 N/mm²	1110 N/mm²
acero	CLASIFICACION			SD345	
	RESISTENCIA EN PUNTO			345 N/mm²	
	REFUERZO ADMISIBLE DE TRACCION	ARMADURA		180 N/mm²	
		LOSA		140 N/mm²	
		CONEXION		160 N/mm²	

A1DISPOSION GENERAL DEL ESTRIBO ESCALA=1:100
(PUENTE QUINAMA)

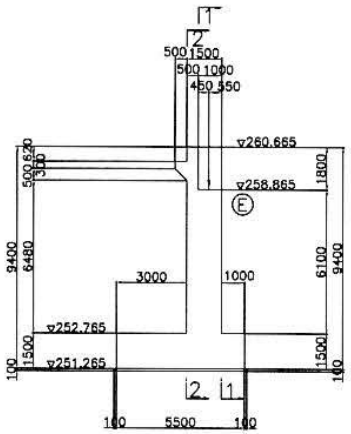
1–1



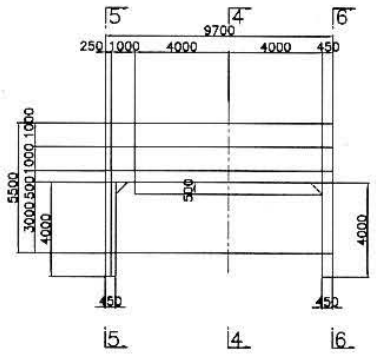
2–2



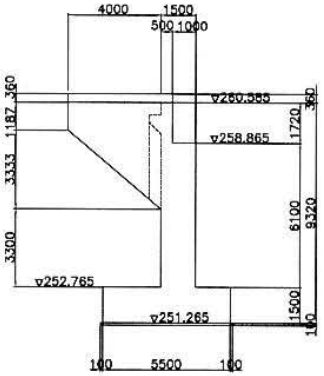
4–4



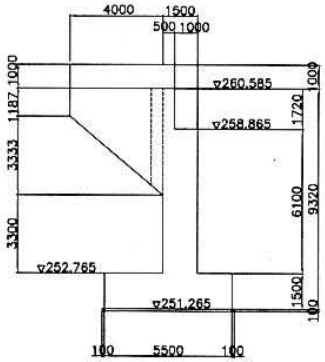
3–3



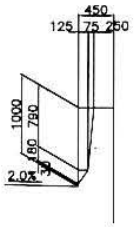
5–5



6–6

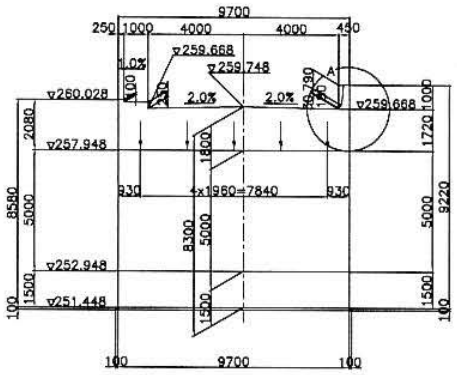


DETALLE"A" SECALA=1:30

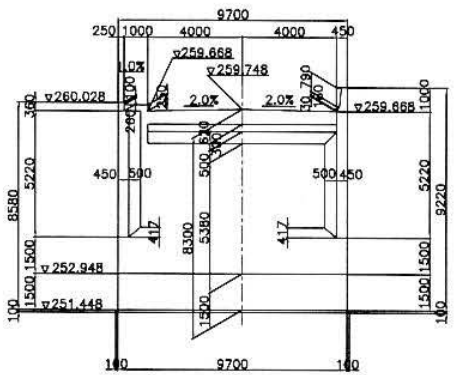


A2 DISPOSION GENERAL DEL ESTRIBO ESCALA=1:100
(PUENTE QUINAMA)

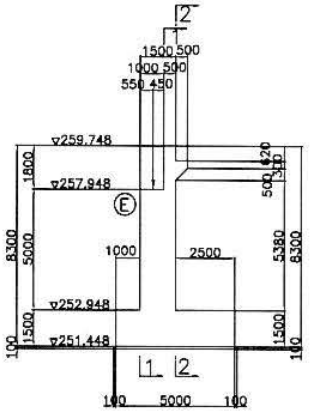
1–1



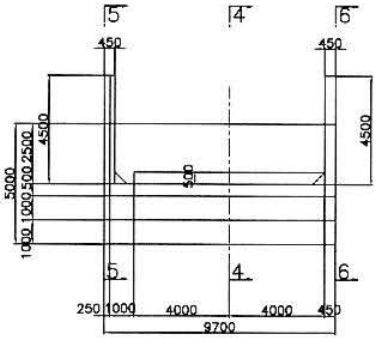
2–2



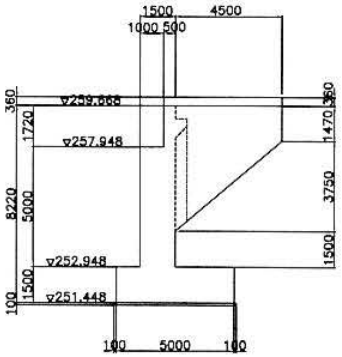
4–4



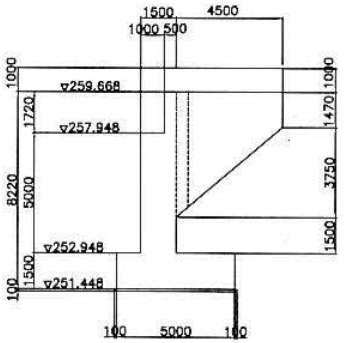
3–3



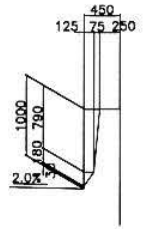
5–5



6–6



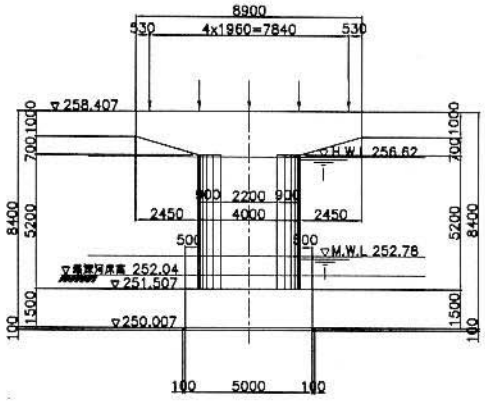
DETALLE"A" SECALA=1:30



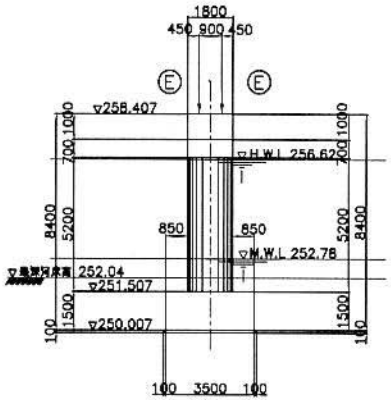
DISPOSION GENERAL DEL PILAR
(PUENTE EL QUINAMA)

ESCALA=1:100

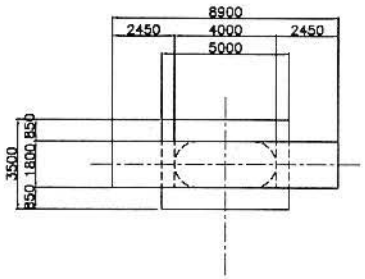
VISTA DE CORTE



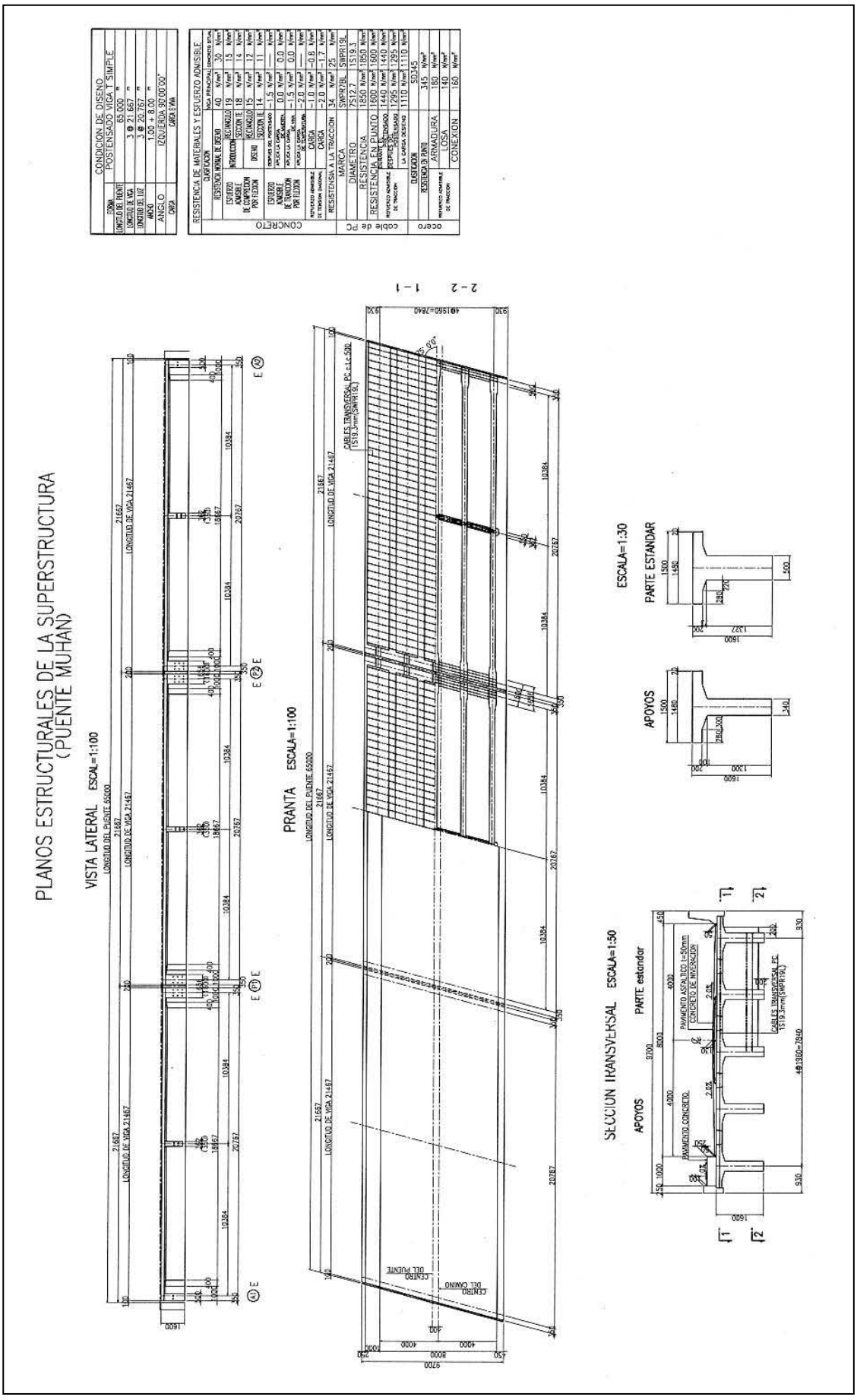
VISTA LATERAL



PLANTA



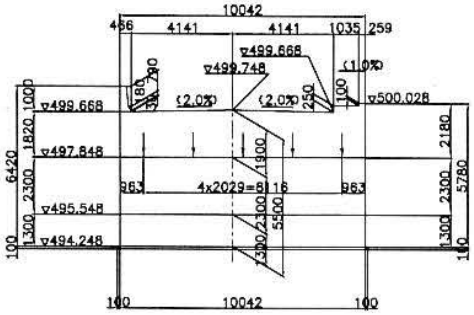
(4) Puente Muhan



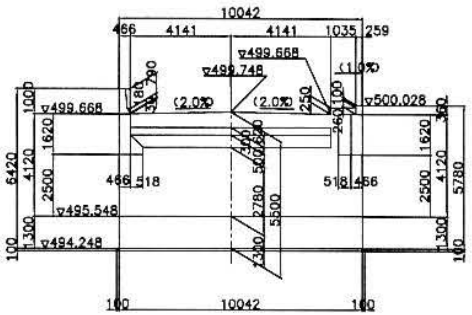
A1DISPOSION GENERAL DEL ESTRIBO (PUENTE MUHAN)

ESCALA=1:100

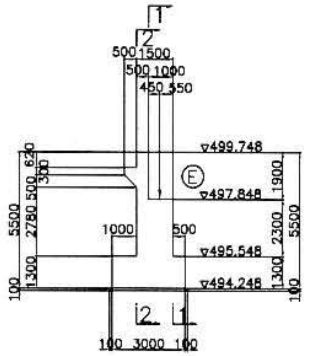
1−1



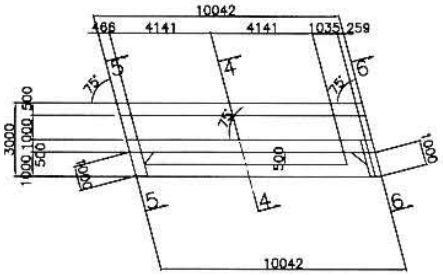
2−2



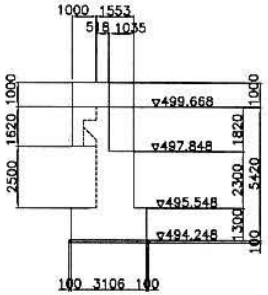
4−4



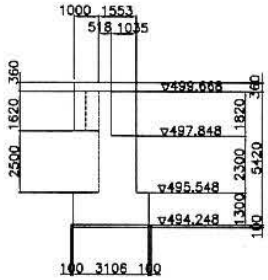
3−3



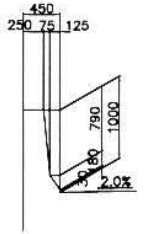
5−5



6−6

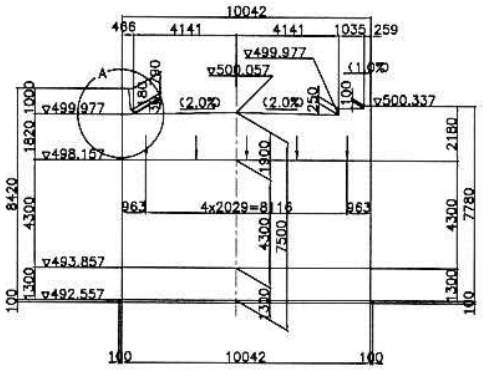


DETALLE"A" SECALA=1:30

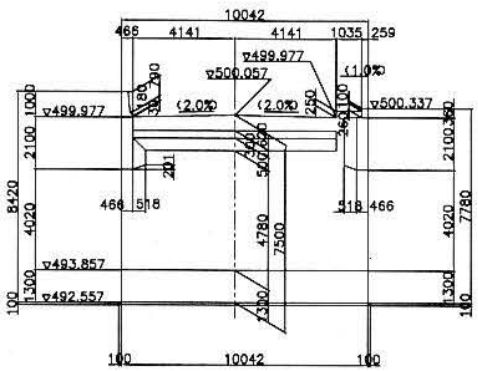


A2 DISPOSION GENERAL DEL ESTRIBO ESCALA=1:100
(PUENTE MUHAN)

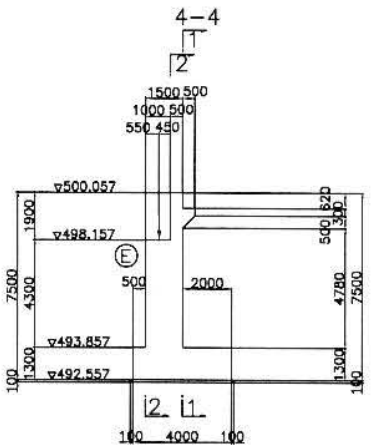
1−1



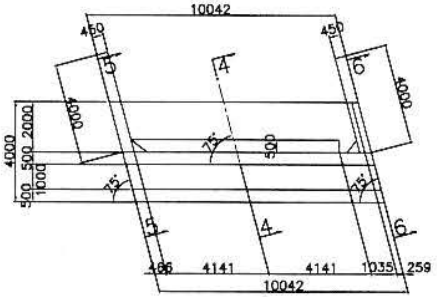
2−2



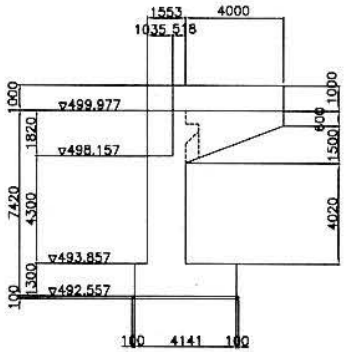
4−4



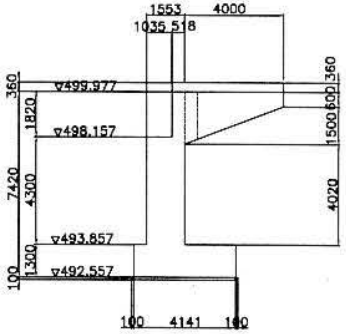
3−3



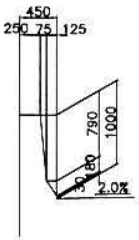
5−5



6−6



DETALLE"A" SECALA=1:30

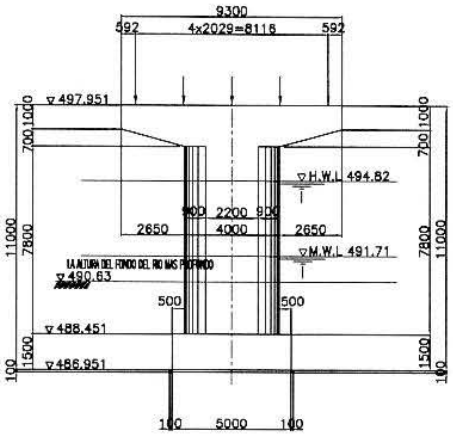


DISPOSION GENERAL DEL PILAR (PUENTE EL MUHAN)

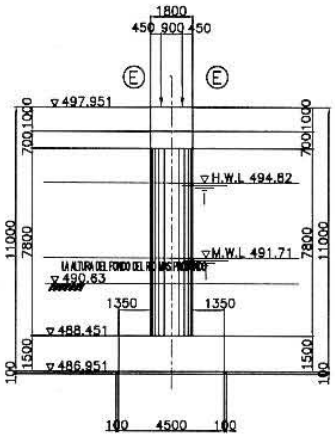
ESCALA=1:100

P1 VISTA

VISTA DE CORTE

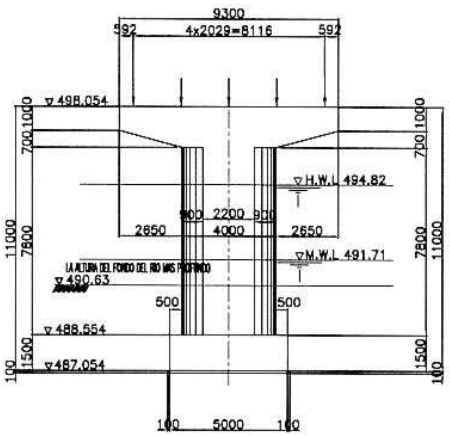


VISTA LATERAL

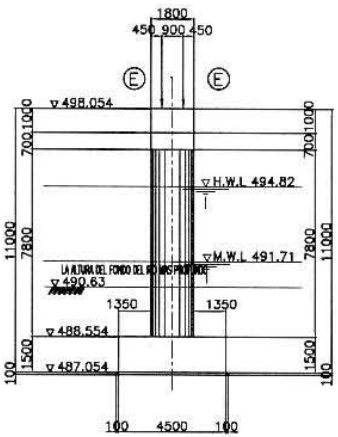


P2 VISTA

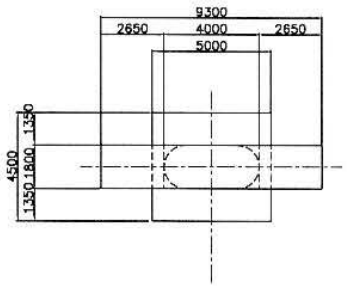
VISTA DE CORTE



VISTA LATERAL



PLANTA



PLANTA