

JAPAN INTERNATIONAL COOPERATION AGENCY (JICA)
NATIONAL DEVELOPMENT PLANNING AGENCY (BAPPENAS)
PROVINCIAL GOVERNMENT OF NANGGROE ACEH DARUSSALAM

**THE STUDY
ON
THE URGENT REHABILITATION AND RECONSTRUCTION
SUPPORT PROGRAM
FOR
ACEH PROVINCE AND AFFECTED AREAS
IN NORTH SUMATRA**

**(URGENT REHABILITATION AND RECONSTRUCTION PLAN
FOR BANDA ACEH CITY)**

IN THE REPUBLIC OF INDONESIA

**FINAL REPORT (2)
VOLUME III : APPENDICIES**

MARCH 2006

NIPPON KOEI CO., LTD.
YACHIYO ENGINEERING CO., LTD.
PASCO CORPORATION

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APPENDIX 1

QUICK IMPACT PROJECT (QIP) BY JICA FUND (SEPTAGE TREATMENT PLANT)

**APPENDIX-1 QUICK IMPACT PROJECT (QIP) BY JICA FUND
(SEPTAGE TREATMENT PLANT)**

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CHAPTER 1 DESIGN WORKS AND COST ESTIMATE

1.1 DESIGN CONDITIONS

1.1.1 Background of the Project

The purpose of the project is to rehabilitate septage treatment plant (*Instalasi Pengelolaan Lumpur Tinja*, IPLT) which was built in 1995 and was destroyed completely by the Tsunami. It is a matter of importance to implement and complete the project as earlier as possible to preserve the public hygiene. After the Tsunami, the septage which are regularly collected from septic tanks of houses and buildings in BAC, are disposed of to the sea directly without any treatment, resulting in environmental problems. Under the above situation, the Municipal Government of Banda Aceh City through the Government of Indonesia (GOI) requested to the JICA assistance for implementation of the project “Rehabilitation of Septage Treatment Plant” (the Project) in April 12th, 2005. In response to the request JICA decided to implement the Project.

1.1.2 Situation before Disaster

In BAC there was no centralized sewerage system including waste water treatment plant, and septic tank was the most common means of sewage treatment. It is estimated that 80 % of houses and buildings is relying on such on-site treatment facilities for their sewage treatment and disposal. The rest is dependent on pit latrines and other means. Septic tanks are normally connected to storm water drainages which are eventually linked to the main rivers/floodway within the city area. On the other hand domestic wastewater is directly drained to the urban drainages without any treatment.

Unfortunately there is no data available with regard to strength of septage, quality of effluent from septic tanks and domestic waste water, and quality of the river water which is receptacle of the effluent from the septic tanks and the domestic wastewater. However the river water might have been contaminated to certain extent judging from its turbidity.

Sanitation and Park Department of the City Municipality (DKP) is serving the collection, treatment and disposal of septage and has an IPLT which still exists nearby river-mouth of the Aceh River. It comprises two (2) lines of an imhoff tank, an anaerobic tank, a facultative tank and a maturation tank as show in Table 1.1.1 and Figure 1.1.1.

Table 1.1.1 Features of Main Components of IPLT

Components		Dimension (m)			Storage capacity (m ³)
		Bottom Length	Bottom Width	Depth	
1	Imhoff Tank				
2	Anaerobic Tank	10.8	3.0	3.5	392
3	Facultative Tank	48.0	13.9	1.35	1,078
4	Maturation Tank	15.8	7.0	1.35	221
5	Sludge Drying Bed	8.0	4.0	1.2	38

Source: Outline Plan of Pembuangan Air Limbah Domestic Kota Banda Ache, DPU

In addition to the above treatment facilities, the plant was provided with operation office. Before the disaster it is reported that DKP had operated three (3) vacuum cars and the private operators also had the same numbers of the vacuum cars.

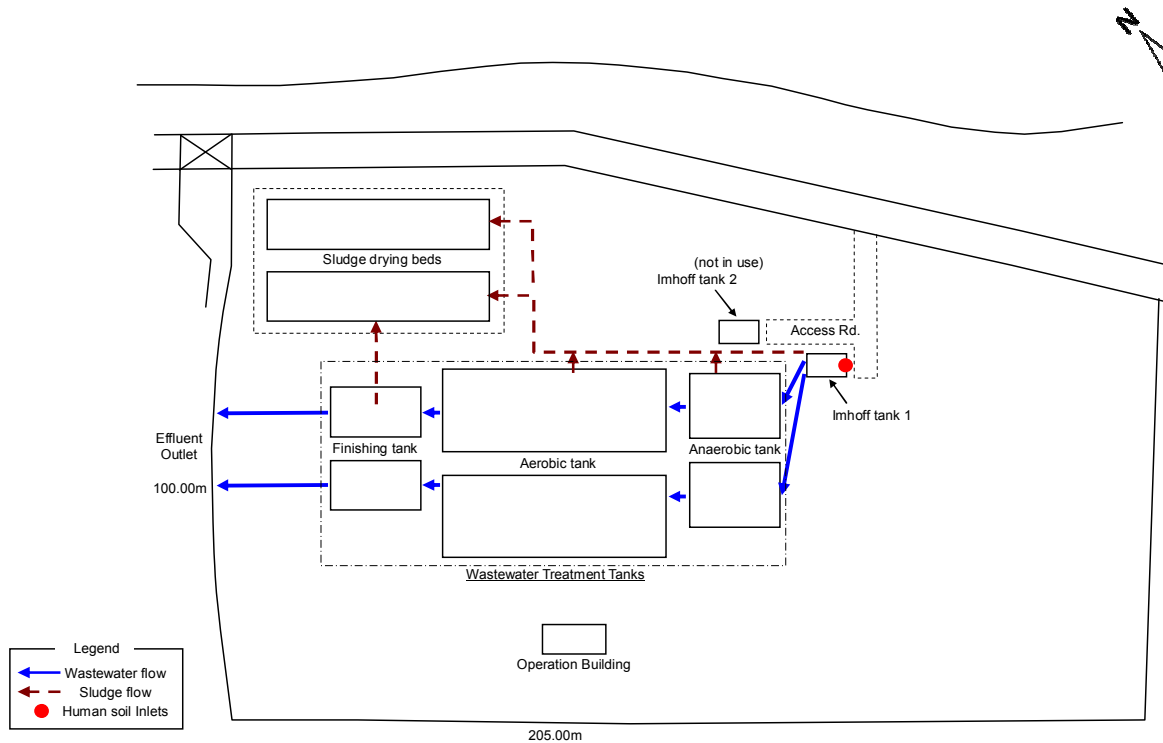


Figure 1.1.1 Schematic Layout of IPLT before Disaster

The treatment capacity was not exactly known. Given total storage capacity of 3,382 m³ by two lines and assumed retention time of 60 days, treatment capacity is assessed to be 50m³/day.

According to DKP one of Imhoff tanks remains out of operation even before the disaster, mainly because of sedimentation at the bottom of the tank. The septage contains plastic products, garbage, soils and other materials, and DKP had to engage a lot of manpower in desludging work. Outflow from the maturation tanks was discharged to the sea and dried sludge was disposed to the landfill site in the vicinity of the plant.

1.1.3 Situation after Disaster

According to the investigation by DKP and the JICA Study Team, the plant is no longer usable after the disaster. The main components were completely washed away/collapsed/destroyed. For instance there are a number of cracks in the anaerobic, facultative and maturation ponds, resulting in leakage of wastewater, and sludge drying bed is no longer in place. In addition DKP's three (3) vacuum cars were washed away.

The septage collected is at present directly discharged to the sea near the mouth of the Aceh River. There is therefore urgent need of rehabilitation or construction of the septage treatment plant.

1.1.4 Design Concept

In accordance with the principles of the request made by GOI, the IPLT is to be rehabilitated under the following concepts:

- ✓ The existing IPLT is required to be rehabilitated to urgently preserve urban and natural environment.
- ✓ Rehabilitation shall be directed to restore the existing IPLT with the same functions before the disaster damages.
- ✓ Rehabilitation also aims at improving operation and maintenance aspects.
- ✓ To use construction equipment and materials available from the local markets as much as possible.
- ✓ It is intended that construction works results in increasing job opportunity.

It is also planned not only rehabilitating the existing IPLT but also improving existing access road and providing drainage and traffic safety facilities.

1.2 DETAIL DESIGN

1.2.1 Design Criteria

(1) IPLT

The following planning criteria are set forth for IPLT:

- Treatment capacity : 50m³/day, to be the same as existing IPLT
- Treatment process : Same as existing
- Transportation of septage : 8 hours per day

(2) Access Road

The following planning criteria are set forth for access road:

(a) Design Standard

Indonesia Road Design Standard is adaptable in principle. Such other standards as British Standards, AASHTO, and Japanese Standards are also adopted where the Indonesian standards are not clear.

(b) Design Speed

The design speed of the access road is set at 30km/h except for the sections of sharp gradient, as the access road is classified as urban road in densely populated areas.

(c) Geometric Design

Road alignment is designed within the existing right-of-way in principle. Larger radius of

curvature shall be adopted to increase traffic volume and traffic safety level.

(d) Carriageway Width

Carriageway width from Km 0+00 to Km 0+550 are 4 m and from Km 0+550 to Km 2+00 are 6 m.

(e) Traffic Sign

Traffic signs are to be installed at critical road sections to secure traffic safety based on Indonesia Standard.

(f) Road Marking

Road marking is provided on the critical road sections.

(g) Passing Line

Passing line with 3.0m in width is provided.

(h) Pavement Design

AASHTO, among others, is adopted as the most popular international method.

The following procedures are adopted for the design as described in Figure. 1.2.1:

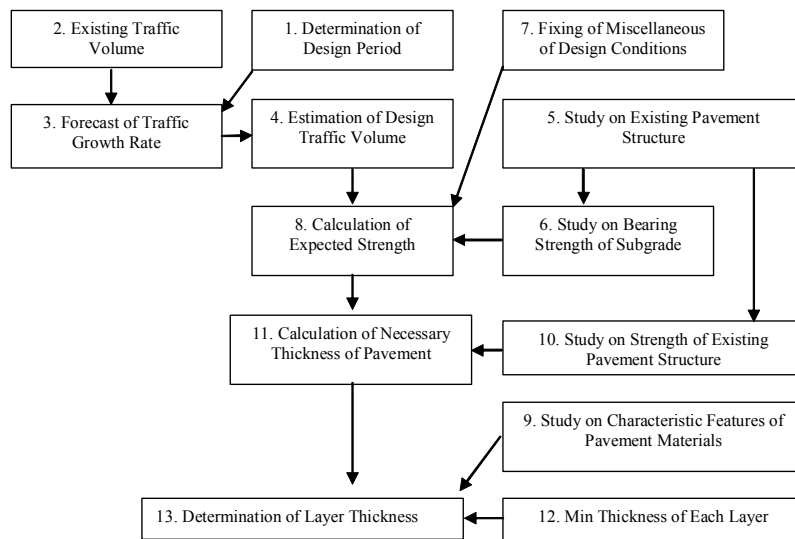


Figure 1.2.1 Asphalt Pavement Design Procedure by AASHTO

Necessary strength of the pavement, termed as Structural Number (SN), is calculated by the following formula:

$$\log_{10} W_{18} = Z_R * S_0 + 9.36 * \log_{10} (SN + 1) - 0.20 + \frac{\log_{10} \frac{\Delta PSI}{(4.2 - 1.5)}}{0.4 + \frac{1094}{(SN + 1)^{5.19}}} + 2.32 * \log_{10} M_R - 8.07$$

Where,

- W_{18} : passing number of equivalent single axle load of 18 kip (=8t)
- Z_R : coefficient for reliability = -1.037
(Corresponding to Reliable Probability: R=85%. 80~99% is for urban road)
- S_o : Overall Standard Deviation=0.45 (normally 0.45 is applied for flexible pavement)
- MR: Resilient Modulus
- Δ PSI: Loss of Performance Serviceability Index
(Ref. : initial value: $P_o=4.2$, terminal value: $P_t=2.5$ Loss= $P_o-P_t=1.7$)

1.2.2 Project Component

Prior to commencement of the rehabilitation design, the JICA Study Team made technical discussions with DKP to define the rehabilitation requirements and to find better operation practice than before the disaster. The rehabilitation design presented herein is therefore joint output of the DKP and the JICA Study Team.

As the same as the existing facilities, the whole treatment process shall be achieved by gravity flow, and no mechanical equipment will be used for any part of treatment process, excepting for lighting and deep well pump for a control house.

(1) Imhoff Tanks

According to DKP, existing imhoff tanks and their operation had the following problems:

- (a) They were originally constructed for the purpose of sludge decomposition but they were not properly functioning due to small storage capacity and mixture of solid waste such as soils, clothes, etc. in septage.
- (b) Before the disaster it was then used for separation of solid materials and septage.
- (c) It was necessary to input a huge amount of labor force to move out solid waste and sediment deposit at bottom of the tanks.

Through technical discussions with DKP, it is determined to rehabilitate the tanks with the following measures:

- ① Both the tanks will be modified only for a separation of septage and solid waste. The septage will be led to the anaerobic tanks through open ditch.
- ② In order to facilitate the removal of solid waste and soils, imhoff tank shall be provided with screen at its inlet and bottom outlet at its bottom. The bottom of the tank shall have sloped bottom for both longitudinal and cross-sectional directions.
- ③ The solid waste and sediment which passing through the screen shall be flushed out through the bottom outlet by using a head between the inlet and the bottom outlet. Such solid waste and sediments shall be conveyed to sludge drying bed.

- ④ Approach road shall be constructed to the tanks in order to allow discharge of septage by collection vehicle into inlet of the tank.

(2) Anaerobic, Aerobic and Finishing Ponds

The existing anaerobic, aerobic and finishing ponds are all lined with concrete at their bottom and slopes. But such concrete lining were cracked and destroyed at many places and also interconnecting conduits from one tank to another were also cut out at many locations. It is therefore determined to reconstruct all tanks completely with the following measures:

- (a) The locations and dimensions of tanks shall be the same as existing tanks in principle.
- (b) Existing concrete lining shall be removed entirely and bottom and slopes of new tanks shall be lined with a reinforced concrete with a thickness of 0.15 m.
- (c) Inflow into and outflow from the tank shall be controlled by overflow weir which shall be installed at outlet of the tank.

(3) Interconnection Conduits

All the ponds from anaerobic down to finishing ponds are interconnected by a gravity flow conduits. In view of maintenance, open conduit with pre-cast concrete cover is adopted. The dimension of the conduits is 30 cm in height and bottom wide.

(3) Sludge Drying Beds

All debris of damaged structure is removed and new sludge drying bed is determined to be constructed at the same location. Removal of debris and land grading are indispensable to start construction works. The new drying bed is designed with asphalt pavement on its surface which has a gentle slope towards edge of the bed to accelerate the process. The bed is connected to the anaerobic ponds by conduits to convey drain water for treatment.

(4) Outlet Work

Effluent from finishing tanks shall be discharged into the sea through outlet works. The outlet consists of a discharge pipe with a diameter of 30 cm and outlet structure equipped with a flap gate. The flap gate is to protect inflow of sea water into IPLT when high tide.

(5) Surrounding Dyke and Landscaping

IPLT shall be surrounded by earth embankment with average height of 2 m and at its entrance steel gate is provided. The embankment slopes are protected with sod facing and a number of trees are planted at the top surface of the embankment and several places within IPLT compound.

(6) Control house

New control house is built and has a floor area of approximately 90 m². It is divided into office, laboratory, and toilet. Water supply system is installed by means of deep well with pump.

(7) Access Road

The access road is rehabilitated for a length of approximately 2 km. It is of asphalt pavement structure and has a width of 4 m and 6 m and a wearing course thickness of a wearing course 5 cm.

Principal features of IPLT after rehabilitation works will be as given in Table 1.2.1.

Table 1.2.1 Features of Main Components of Septage Treatment Plant

Components		Dimension (m)			Storage capacity (m ³)
1	Imhoff Tank	Bottom Length	Bottom Width	Depth	
2	Anaerobic Tank	10.8	3.0	3.5	392
3	Facultative Tank	48.0	13.9	1.35	1,078
4	Maturation Tank	15.8	7.0	1.35	221
5	Sludge Drying Bed	8.0	4.0	1.2	38

1.3 DESIGN DRAWINGS

In total 38 drawings are prepared as Tender Drawing as shown in Table 1.3.1 and all compiled in Volume 3 of Tender Documents.

Table 1.3.1 List of Tender Document

No.	Title
1.	Location Map
2.	Layout Plan
3.	Preparation of Work
4.	Entrance Road
5.	Plan Cross Section
6.	Detail of Tanks
7.	Structures
8.	Standard Structure of Borehole
9.	Access Road
10.	Access Road Typical Cross Section
11.	Layout of Office Building
12.	Cross Section of Office Building
13.	Electrical Installation
14.	Air-Conditioner Installation Plan
15.	Plumbing Installation Plan
16.	Water Reservoir Unit
17.	Septic Tank
18.	Absorptive Plan
19.	Laboratory Waste Tank Plan
20.	Detail of Roof Structure
21.	Ceiling Plan
22.	Kozen Installation Plan
23.	Details of Window
24.	Details of Door
25.	Foundation Plan
26.	Laboratory Room

1.4 COST ESTIMATE

Construction cost of the restoration works is estimated at the price level of July 2005 as given in Table 1.4.1.

Table 1.4.1 Bill of Quantities

Bill Item No.	Pay Item No.	Item	Unit	Unit Price (US\$)	Quantity	Amount (US\$)
1.		General				
	1.1	Mobilization	L.S.	24,999.94	1.0	25,000
	1.2	Demobilization	L.S.	7,999.94	1.0	8,000
2.		Excavation, Filling and Disposal				
	2.1	Site Clearing and Removal of Debris	Cu.m	1.50	5,170.0	7,755
	2.2	Common Excavation	Cu.m	4.15	160.0	665
	2.3	Common Embankment	Cu.m	22.34	2,690.0	60,103
	2.4	Trimming of Sub-grade for Roads	Cu.m	6.94	3,730.0	25,878
3.		Cleaning of Existing Ditches				
	3.1	Cleaning of Existing Ditch	m	0.88	1,000.0	880
4.		Cleaning of Existing Tanks				
	4.1	Cleaning of Existing Imhoff Tanks	nos	791.21	1.0	791
	4.2	Cleaning of Existing Anaerobic Tanks	nos	1,837.53	2.0	3,675
	4.3	Cleaning of Existing Aerobic Tanks	nos	6,680.64	2.0	13,361
	4.4	Cleaning of Existing Finishing Tanks	nos	1,522.02	2.0	3,044
5.		Removal of Existing Asphalt Pavement and Drainage				
	5.1	Removal of Existing Asphalt Pavement	Sq.m	7.82	3,730.0	29,164
	5.2	Removal of existing U-shaped ditch	m	1.50	500.0	750
	5.3	Removal of existing crossing culvert	m	4.15	30.0	125
6.		Rehabilitation of Existing Tanks				
	6.1	Rehabilitation of Existing Imhoff Tanks	nos	2,885.75	2.0	5,771
	6.2	Rehabilitation of Existing Anaerobic Tanks	nos	16,569.11	2.0	33,138
	6.3	Rehabilitation of Existing Aerobic Tanks	nos	40,062.38	2.0	80,125
	6.4	Rehabilitation of Existing Finishing Tanks	nos	18,383.62	2.0	36,767
7.		Sub-base Course and Base Course				
	7.1	Sub-base course (t=250mm, crusher-run)	Sq.m	7.53	3,730.0	28,092
	7.2	Sub-base course (t=250mm, recycled crusher-run)	Sq.m	6.28	3,550.0	22,280
	7.3	Base course (t=100mm)	Sq.m	3.01	3,140.0	9,459
	7.4	Base course (t=150mm)	Sq.m	4.52	6,610.0	29,869
8.		Bituminous prime coat				
	8.1	Bituminous prime coat	Sq.m	1.28	7,280.0	9,308

9.		Bituminous tack coat				
	9.1	Bituminous tack coat	Sq.m	0.38	20,055.0	7,692
10.		Asphalt concrete pavement				
	10.1	Asphalt concrete surface course for carriageway (t=50mm)	Sq.m	16.92	20,055.0	339,407
11.		Drainage works				
	11.1	Type-A side ditch (Open Drain)	m	25.37	187.8	4,764
	11.2	Type-B cross culvert (D=600)	m	85.88	25.2	2,164
	11.3	Type-A Catchpit (H=2000)	nos	279.72	1.0	280
	11.4	Type-B Catchpit (H=1000)	nos	259.92	1.0	260
	11.5	Catchpit Cover	nos	15.86	2.0	32
	11.6	Outlet	nos	456.82	1.0	457
12		Road marking				
	12.1	Type-A Road marking (150mm wide)	m	1.53	4,062.0	6,233
	12.2	Type-A Road marking (450mm wide)	m	4.60	30.0	
13.		Traffic signs				
	13.1	Type-A Traffic sign	nos.	217.47	10.0	2,175
	13.3	Type-B Traffic sign	nos.	217.47	10.0	2,175
14.		Planting				
	14.1	Sodding	Sq.m	1.40	1,499.0	2,099
	14.2	Tree planting	nos.	24.48	150.0	3,672
	14.3	Flower Planting	nos.	13.99	500.0	6,995
	14.4	Gardening soils	Cu.m	4.25	100.0	425
15.		Site Office				
	15.1	Site Office	nos.	32071.00	1.0	32,071
16		Fenceing				
	16.1	Fenceing(include Gate)	m	224.64	300.0	67,392
Grand Total						912,291

Sub Total carried forward to Summary

CHAPTER 2 PREPARATION OF TENDER DOCUMENTS

2.1 TENDER CONDITIONS

2.1.1 Outline of the Contract Works

The Project aims at rehabilitating the access road with a length of 2 km and IPLT with a daily treatment capacity of 50 m³ in terms of septage in BAC. The contract works will include the supply of all labors, equipment, plants, materials and support services required for construction, inspection, testing, and commissioning of the following:

- site clearance (includes removal of debris);
- cleaning of all existing tanks;
- surrounding dykes and drainages;
- access road
- two (2) Imhoff tanks
- two(2) anaerobic tanks;
- two(2) Aerobic tanks;
- two(2) finishing tanks;
- sludge drying bed;
- control office

2.1.2 Details to Tender

JICA implements the Project in compliance to the request of the Banda Aceh City Government through GOI in accordance with the following pre-construction schedule:

- | | |
|---|-------------------------------|
| 1) Request of Banda Aceh Municipality to JICA | April 12 th , 2005 |
| 2) Submission of Draft Tender Documents to JICA | May 23 rd , 2005 |
| 3) Approval of Draft Tender Documents (JICA) | May 31 st , 2005 |
| 4) Distribution of Tender Documents | June 01 st , 2005 |
| 5) Tender Opening | June 16 th , 2005 |

2.1.3 Eligibility of Contractor and Required Documents

The Tender is determined to be evaluated in a numerical rating system, in which the Tender is divided into two categories: the first category is Tenderer's qualification including their financial status, construction experience and technical proposal for the contemplated contract works and the second category is tender price. Both the categories are evaluated by means of a numerical rating system respectively in addition to their formality and completeness and the first lowest tenderer is determined to be the one who gained the highest aggregate point of the both categories.

The following conditions and requirements are determined to be set forth in the Tenderer's

qualification requirements in order to procure reputable and reliable contractor in the contract works:

(1) Qualification of Tenderer

(a) Origin

The Tenderer shall be Japanese or Indonesian construction firm who has a license for construction business.

(b) Financial status including balance sheet and profit and loss statement in the last two (2) years be submitted

(c) Business license including registration of the firm be submitted

(d) Experience in Construction Works

The Tenderer shall have at least one (1) experience in construction works similar to the Project with contract of a value equivalent to US\$ 900,000 or more in the last ten (10) years.

(e) Type of Tenderer: single firm or joint operation by plural firms. In case of the joint operation, joint operation agreement shall be submitted together with the Tender.

(2) Technical Proposal

The Tenderers are requested to submit the following documents in accordance with the prescribed format:

(a) Proposed organization for the contract works

(b) Key personnel to be engaged

(c) Proposed constructional equipment

(d) Basic construction program

(e) Narrative construction plan

(3) Other Requirement

The Tenderers are encouraged to employ as many Ache people as possible in execution of the contract works.

2.2 TENDER DOCUMENTS

The Tender Documents will comprise three (3) volumes as listed up hereunder:

(1) VOLUME I	Section 1	Invitation for Tenders
	Section 2	Instructions to Tenderers

	Section 3	Tender Data
	Section 4	Bill of Quantities
	Section 5	Forms, Annexes and Enclosures
	Section 6	Conditions of Contract
	Part I:	General Conditions of Contract
	Part II:	Conditions of Particular Application
	Part III:	Appendix to Tender
(2) VOLUME II	Section 7	Technical Specifications
(3) VOLUME III	Section 8	Drawings

CHAPTER 3 TENDER RESULTS

3.1 TENDER EVALUATION CRITERIA

(1) Overall Criteria

The Tender submitted are to be evaluated in accordance with the manner mentioned in Sub-section 2.1.3 hereof. Allocation of points between the technical aspects and the tender price is determined to be as follows:

	<u>Weighting</u>
a. Stage 1 : Tenderer's Qualification	80 Points
b. Stage 2 : Tender Price	20 Points
<hr/> Full mark 100 Points	

It is pre-determined that the Stage 2 evaluation is to be conducted only for such Tenderers that gained the score 50 or more at the Stage 1 evaluation.

(2) Criteria for Stage 1: Evaluation of Tenderer's Qualification

The Tenderer's legal status, financial status, past experience and technical proposal for the contract works are evaluated also by means of the numerical rating system. There are 8 evaluation sub-items in total, and for each sub-item point is allocated as tabulated below:

		<u>Rating (Points)</u>
a. Company Profile	Enclosure No. 1	5
b. Particular Experience Record	Enclosure No. 2	10
c. Financial Capability	Enclosure No. 3	10
d. Joint Venture Agreement (if applicable)	Enclosure No. 4	-
e. Organisation and Key Personnel	Enclosure No. 5	10
f. List of Contractor's Equipment	Enclosure No. 6	10
g. Basic Programme of the Works	Enclosure No. 7	20
h. Outline of Construction Plan	Enclosure No. 8	15
<hr/> Total		80 Points

(3) Criteria for Stage 2: Price Evaluation

The lowest Tender price is to be given a full mark of 20, and score of the other Tenderers is decided to be calculated in accordance with the following equation:

$$\text{Score} = 20 \text{ points} \times \frac{\text{Lowest price}}{\text{Tenderder price}}$$

3.2 TENDER OPENING

(1) Tender schedule

- | | | |
|----|---|--|
| 1) | Submission of the Draft Tender documents (JICA) | May 23 rd , 2005 |
| 2) | Approval of the Draft Tender documents (JICA) | May 31 st , 2005 |
| 3) | Distribution of Tender Documents | June 01 st , 2005 |
| 4) | Inquiries from the Tenderers | June 06 th , 2005 |
| 5) | Answers from the Consultants (Addendum No.1) | June 10 th , 2005 |
| 6) | Date of Tender Opening | June 16 th , 2005 |
| 7) | Tender Evaluation | June 17 th -20 th , 2005 |
| 8) | Submission of Tender Evaluation Report | June 21 st , 2005 |

(2) Participants at Tender Opening Meeting

1) Client

- NAGAMI Kozo: Assistant Resident Representative Japan International Cooperation Agency (JICA) Indonesia Office
- SUGANO Yuichi: Japan International Cooperation Agency (JICA), Social Development Department

2) Counterpart

- Drs.H.T. Saifuddin, TA.M.Si: Head of Sanitation and Park Department, Banda Aceh City Government

3) Engineer

- INUZUKA Isao: Facility Plan/Infrastructures Designer, JICA URRP Study Team
- IZAWA Tetsuro: Cost Estimate/Procurement Plan 1, JICA URRP Study Team

4) Tenderer

- PT. TENAGA INTI MAKMU BEUSARE: Teuku Umar No.109 Street Banda Aceh
- PT. YUNIDA SWASTA: Prof. A. Mohd. Idrahim Street No.18
- PT. HATARI RAYA: Sri Ratu Safiatuddin Street No.12 Banda Aceh

3.3 TENDER EVALUATION

3.3.1 Formality and Completeness of Tenders Submitted

The formality and completeness of the tenders submitted were at first examined. The results are as presented in Table 3.3.1.

Table 3.3.1 Formality and Completeness of Tenders submitted

	PT. TENAGA INTI MAKMU BEUSARE	PT. YUNIDA SWASTA	PT. HATARI RAYA
Number of Tender	OK	OK	OK
Certificate of Register of companies	OK	OK	OK
Receipt of Tender Documents	OK	OK	OK
Affidavit of Site Inspection	OK	OK	OK
Form of Tender	OK	OK	OK
Priced Bill of Quantities	OK	OK	OK
Enclosure No 1 (Company Profile)	OK	OK	OK
Enclosure No 2 (Particular Experience Record)	OK	OK	OK
Enclosure No 3 (Financial Capability)	OK	OK	OK
Enclosure No 4 (Joint venture Agreement)	-	-	-
Enclosure No 5 (Organization & Key Personnel)	OK	NO	OK
Enclosure No 6 (List of Contractor's Equipment)	OK	OK	NO
Enclosure No 7 (Basic Programme of the Works)	OK	NO	NO
Enclosure No 8 (Outline of Construction Plan)	OK	NO	NO

3.3.2 Evaluation at Stage 1: Tenderer's Qualification

(1) Evaluation of Tenderer's Eligibility and Status

1) Company Profile and Line of Business (Enclosure No 1)

	PT. TENAGA INTI MAKMU BEUSARE	PT. YUNIDA SWASTA	PT. HATARI RAYA
Head Office	Teuku Umar No.109 Street Banda Aceh	Prof. A. Mohd. Idrahim Street No.18, Banda Aceh	Sri Ratu Safiatuddin Street No.12, Banda Aceh
Owner	Mr. Makmur,SE	Mr. Syahrul	Mr. Mawardi Yusuf
TEL	0651-40311	0651-92102	0651-7428673
FAX	651-49412	-	-

Business Registrations	Civil, Architecture, Environment System	Civil, Architecture, Environment System	Civil, Architecture.
------------------------	---	---	----------------------

2) Annual Turnover (Enclosure No 2)

		PT. TENAGA INTI MAKMU BEUSARE	PT. YUNIDA SWASTA	PT. HATARI RAYA
Projects with a value equivalent to US\$ 900,000 or more	1995	-	-	-
	1996	-	-	-
	1997	-	-	-
	1998	-	-	-
	1999	-	-	-
	2000	-	-	-
	2001	-	-	-
	2002	-	-	-
	2003	-	-	-
	2004	1	-	-
	Total	1	-	-

3) Financial Status (Enclosure No 3)

Unit : (US\$, million)

		Evaluated Scores														
		PT. TENAGA INTI MAKMU BEUSARE					PT. YUNIDA SWASTA					PT. HATARI RAYA				
		00	01	02	03	04	00	01	02	03	04	00	01	02	03	04
1	Total assets	0.68	0.74	0.90	0.95	1.00	0.35	0.41	0.47	0.51	0.54	0.30	0.33	0.36	0.42	0.44
2	Current assets	0.48	0.49	0.57	0.63	0.75	0.38	0.42	0.53	0.60	0.63	0.29	0.33	0.38	0.40	0.47
3	Total liabilities	0.06	0.06	0.07	0.08	0.10	0.04	0.05	0.05	0.06	0.07	0.04	0.05	0.05	0.06	0.06
4	Current liabilities	0.03	0.04	0.05	0.05	0.06	0.03	0.03	0.03	0.04	0.05	0.02	0.03	0.03	0.04	0.04
5	Profits before taxes	0.24	0.29	0.37	0.39	0.32	0.00	0.02	0.03	0.05	0.04	0.03	0.03	0.01	0.05	0.02
6	Profits after taxes	0.23	0.28	0.36	0.36	0.30	0.00	0.02	0.03	0.05	0.04	0.00	0.02	0.03	0.05	0.04
7	Net worth(1-3)	0.63	0.83	0.83	0.87	0.91	0.31	0.36	0.42	0.45	0.48	0.26	0.29	0.31	0.36	0.38
8	Working capital(2-4)	0.45	0.53	0.53	0.57	0.69	0.35	0.39	0.50	0.56	0.58	0.37	0.30	0.35	0.37	0.43

4) Joint Venture Agreement (Enclosure No 4)

There are no Tenderers with a joint operation.

	PT. TENAGA INTI MAKMU BEUSARE	PT. YUNIDA SWASTA	PT. HATARI RAYA
Joint venture Agreement	Single	Single	Single

(2) Evaluation of Technical Proposal

1) Proposed Organization and Key Personnel (Enclosure No 5)

		PT. TENAGA INTI MAKMU BEUSARE	PT. YUNIDA SWASTA	PT. HATARI RAYA
Project Manager	Name	Ir.Twk. mansursyay	NO	Mr. Ridwan Dahilan, ST
	Age	51 years old	NO	35 years old
	nationality	Indonesia	NO	Indonesia
	Specialty	Civil Engineer	NO	Civil Engineer
	Experience	20Years	NO	6 Years
	Other language	English	NO	English
Site Manager	Name	Khairul Syahmega, ST	NO	Mr.Fuadi, ST
	Age	34 years old	NO	37 years old
	nationality	Indonesia	NO	Indonesia
	Specialty	Civil Engineer	NO	Civil Engineer
	Experience	5 Years	NO	6 Years
	Other language	English	NO	English
Organizational chart		OK	NO	OK

2) List of Contractor's Equipment (Enclosure No 6)

Name of Equipment	PT. TENAGA INTI MAKMU BEUSARE			PT. YUNIDA SWASTA			PT. HATARI RAYA		
	Capa	Number	Mobilisa	Capa	Number	Mobilisa	Capa	Number	Mobilisa
1. Excavator	0.8m3	2	July 1	0.8m3	1	July 1	-	-	-
2. Bulldozer	115Hp	1	July 1	140Hp	1	June 1	-	-	-
3. Vibratory Roller	110Hp	1	July 10	12 t	1	July 1	-	-	-
4. Motor Grader	100Hp	1	July 10	-	-	-	-	-	-
5. Flat Bed Truck	30 t	1	July 1	-	-	-	-	-	-
6. Dump Truck	15m3	2	July 1	15m3	5	July10	-	-	-
7. Dump Truck	4m3	8	July 1	4m3	8	July10	-	-	-

8. Generator Set	300Kv	1	July 10	-	-	-	-	-	-
9. Tandem Roller	12 t	1	July 15	12 t	1	July 1	-	-	-
10 PTR	12 t	1	July 15	-	-	-	-	-	-
11 Asphalt Sprayer	300 L	1	July 15	300 L	1	July10	-	-	-
12 Asphalt Finisher	5 m	1	July 15	5 m	1	July 1	-	-	-
13 Water Tanker	4,000L	1	July 10	-	-	-	-	-	-
14 Water Pump	6 Hp	1	July 10	-	-	-	-	-	-
15 Tamper	5 Hp	1	July 10	-	-	-	-	-	-
16 Concrete Mixer	0.3m3	3	July 10	-	-	-	-	-	-
17 Concrete Vibrator	5 Hp	1	July 10	-	-	-	-	-	-

3) Basic Programme of the Works (Enclosure No 7)

Item	PT. TENAGA INTI MAKMU BEUSARE	PT. YUNIDA SWASTA	PT. HATARI RAYA
1 Site clearance and Removal of debris	Acceptable	NO	NO
2 Cleaning of all existing tanks	Acceptable	NO	NO
3 Production of Pre-cast concrete block	NO	NO	NO
4 Access road for Base course	Acceptable	NO	NO
5 Repair work of each tank	Acceptable	NO	NO
6 All As pavement work	Acceptable	NO	NO
7 Facilities construction	Acceptable	NO	NO

4) Outline of Construction Plan (Enclosure No 8)

Item	PT. TENAGA INTI MAKMU BEUSARE	PT. YUNIDA SWASTA	PT. HATARI RAYA
1. Over-all plan	OK	NO	NO
2. Execution stage of works	OK	NO	NO
3. Materials quality control.	OK	NO	NO
4. Workmanship and Capacity	OK	NO	NO
5. Security Control method	NO	NO	NO

(3) Results of the Stage 1 Evaluation

All description and data given in the enclosures were evaluated according to the rating system as summarized table below: Each sub-item is classified into four (4) grades: excellent, good, fair and marginal, and a rating are allocated at 1.0, 0.8, 0.7 and 0.0 in order of category.

Table 3.3.2 Evaluated Scores at Stage 1: Tenderer's Qualification

		PT. TENAGA INTI MAKMU BEUSARE				PT. YUNIDA SWASTA				PT. HATARI RAYA			
		E	G	F	M	E	G	F	M	E	G	F	M
		1.0	0.8	0.7	0	1.0	0.8	0.7	0	1.0	0.8	0.7	0
Enclosure No 1	5Pt		4			4				4			
Enclosure No 2	10Pt		8				7				7		
Enclosure No 3	10Pt		8			8				8			
Enclosure No 4	-	-	-	-	-	-	-	-	-	-	-	-	
Enclosure No 5	10Pt		8					0		8			
Enclosure No 6	10Pt	10				8						0	
Enclosure No 7	20Pt			14				0				0	
Enclosure No 8	15Pt			10				0				0	
Total		62				27				27			

Legend E: Excellent G: Good F: Fair M: Marginal

Only PT. Tenaga Inti Makmu Breusare gained the score more than minimum requirement of 50 points, and is therefore pre-qualified, for stage 2 evaluation

3.3.3 Evaluation at Stage 2: Tender Price

The Tender prices of the Tenderers are as summarized in table below:

Table 3.3.3 Comparison of Tender Prices

Engineer's Estimation	Tender Prices (US\$)		
	PT. TENAGA INTI MAKMU BEUSARE	PT. YUNIDA SWASTA	PT. HATARI RAYA
<u>912,388</u>			
(100.00%)	866,000	1,094,000	917,000
	(94.91 %)	(119.90 %)	(100.01 %)

PT. Tenaga Inti Makmu Beusare offered the lowest price among the three (3) tenders received and is therefore given a full mark of 20 points. The Tender price of the other two Tenderers, PT. Yunida Swasta and PT. Hatari Raya were not evaluated since their Tenders were disqualified at the first stage.

Table 3.3.4 Evaluated Scores at Stage 2: Tender Prices

Evaluated Scores		
PT. TENAGA INTI MAKMU BEUSARE	PT. YUNIDA SWASTA	PT. HATARI RAYA
20 Point	-	-

3.3.4 Conclusion

The results of the tender evaluation are as tabulated below:

Table 3.3.5 Overall Result of Tender Evaluation

Tenderer	PT. TENAGA INTI MAKMU BEUSARE	PT. YUNIDA SWASTA	PT. HATARI RAYA
Technical Evaluation	62 Points	27 Points	27 Points
Price Evaluation	20 Points	-	-
Total	82 Points	27 Points	27 Points
Ranking	1	-	-

The Tender submitted by PT. Tenaga Inti Makmu Beusare is concluded to be the lowest responsive tender among there (3) Tenders received and is therefore recommended for award of the contract.

CHAPTER 4 CONSTRUCTION SUPERVISION WORKS

4.1 SCOPE OF CONSTRUCTION SUPERVISION WORKS

Scope of construction supervision works is mainly categorized into: (1) Progress Control, (2) Quality Control, (3) Cost Control and (4) Safety Control during the course of the following major activities on supervision works:

- Review and approval of construction drawings, methods, schedule and proposals submitted by the Contractor
- Modification of construction drawings, as required
- Review and approval of manufacturer's design and drawings, if necessary
- Inspection of the completed works
- Inspection and witness of final acceptance tests
- Certification of completed works
- Review and certification of statement of progress and final payments
- Preparation of monthly progress report, completion report and compilation of As-built Drawings.

4.2 ORGANIZATION OF CONSTRUCTION SUPERVISION TEAM

Organization of construction supervision team was set as shown in Figure 4.1.1.

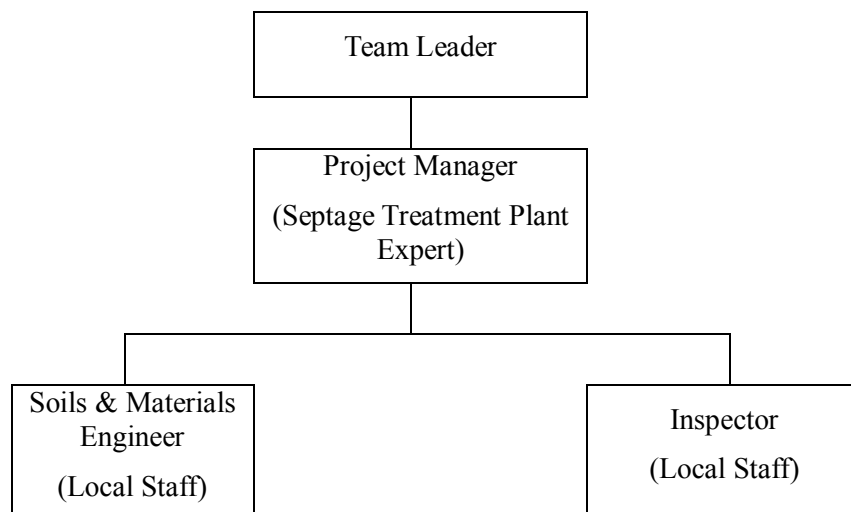


Figure 4.1.1 Organization of Construction Supervision Team

Team Leader of the JICA URRP Study Team is responsible for management of the construction supervision team.

Septage Treatment Plant Expert of the JICA URRP Study Team acted as a Project Manager who is

responsible for all construction supervision works.

Soils and Materials Engineer (Local Staff) was assigned for all construction materials including borrow pits, quarries, stockpiles, concrete and asphalt production, on-site manufacturing yard and off-site supplies.

Inspector (Local Staff) was also assigned for inspection works.

4.3 CONSTRUCTION SUPERVISION WORKS

4.3.1 Progress Control

Progress control was made based on the work program. Actual progress of works was checked at the site and compared to the planned progress. Regular review of the program was made to ensure that the planned progress of each project component could be made in each month.

The comparisons of the actual and planned progress were described in the monthly report together with explanations of reasons of delay if any, and details of measures taken or to be taken to avoid further delay. Monthly meetings were held to monitor respective works.

4.3.2 Quality Control

(1) Quality Control on Materials

Construction materials used for the project are divided into in-situ materials and procured materials. The in-situ materials are produced from the approved borrow pits, quarry sites, river deposits and beach areas, while procured materials are products manufactured at factories like cement, rebar, bitumen, piles, pipes, paint, hardware and road signs. All of these were confirmed as acceptable quality prior to construction.

(2) Quality Control on Workmanship

Quality control on workmanship was carried out on products made by the Contractor under the supervision of the Engineer in accordance with the Conditions of Contract, Specifications, Drawings and other relevant Contractual Documents.

The major work items subject to quality control on workmanship are as follows:

- Excavation works
- Embankment works
- Sand filling
- Slope protections consisting of rip-rap, gabions, masonry, etc.
- Instrumentation (for site office)
- Concrete works
- Metal works and painting

4.3.3 Cost Control

Each work item described in the Bill of Quantities was reviewed and adjusted as required. Cost control was made based on the measured work quantities and unit rates, lump sums and provisional sums in close cooperation with Inspector.

4.3.4 Safety Control

Safety is considered one of the key issues for any type of construction works. The safety control was fully taken during construction with establishment of safety control organization, meetings and monitoring.

4.3.5 Handover of the Project

The Project was completed with sufficient control on progress, quality, cost and safety on December 20, 2005, 8 days before the contacted completion date on December 28, 2005. Handover ceremony was conducted on the same day with the presence of Minister of Embassy of Japan, JICA Resident Representative, Mayor of Banda Aceh City, BRR and other related agencies.

Signing on handover agreement and signing on agreement on transfer of the Project were also undertaken in the ceremony.

APPENDIX 2

RECOVERY OF WATER SUPPLY SYSTEM IN BANDA ACEH CITY UNDER JAPAN'S NON-PROJECT TYPE GRANT AID

**APPENDIX-2 RECOVERY OF WATER SUPPLY SYSTEM IN BANDA ACEH CITY
UNDER JAPAN'S NON-PROJECT TYPE GRAND AID**

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CHAPTER 1 DESIGN WORKS AND COST ESTIMATE

1.1 DESIGN CONDITION

1.1.1 General

The Project aims at restoring water distribution network in the city to the pre-disaster situation in principle.

The detailed design was conducted in 3 months period from May to September 2005 by the JICA Study Team in close coordination and cooperation with BRR, PU, PDAM and other organizations/agencies concerned in pipe water supply in the Project area.

1.1.2 Scope of Design Works

The scope of work comprises (1) design works and cost estimate, and (2) preparation of technical report for restoration of water distribution network in Banda Aceh City.

It should also be noted that quantity of design discharge shall be forecasted based on the water demand for the year 2009, which is the end of rehabilitation and reconstruction program of Banda Aceh City as set forth by GOI.

1.1.3 Pipe Water Supply before Disaster

In Banda Aceh City public pipe water supply is managed and administered solely by PDAM.

There are two (2) sources of public water supply in the Project area: one is Lambaro Water Treatment Plant and the other is Siron Water Treatment Plant. According to hearing from PDAM staff and PDAM annual reports, the features of pipe water supply facilities before the disaster are as summarized in Table 1.1.1.

General layout of pipe water supply system is as shown in Figure 1.1.1. This layout is produced by cooperation of the JICA Study Team and PDAM after the disaster, and it is a benchmark data for contemplated design works.

Table 1.1.1 Principal Features of Water Supply System Existed before Disaster

Main Facilities		Lambaro System	Siron System
Raw water pumping station	Number	5	2
	Capacity of each	[Q:208 l/s, h;20m] x3 [Q:147 l/s, h;20m] x2	Q:22 l/s, h:15m
Water treatment plant	Production capacity	37,584 m ³ /d	1,728 m ³ /d
	Treatment process	Rapid sand filtration	Rapid sand filter
	Number of treatment line	2	1
	Main treatment component	Pre-sedimentation, clarifier, rapid sand filter, clear water reservoir	Flocculator and Coagulator chamber, sedimentation tank, rapid sand filter, clear water reservoir
Treated water pumps	Number of pump	5	3
	Capacity of each	Q:147 l/s, h:60m	Q:20 l/s, h:50m
Distribution pipelines	Pipe materials	Steel pipe (SP) for D300 – 600	
		Polyvinyl chloride (PVC) for D100 - 250	
	Pipe length	D600 = 7,566m	
		D500 = 6,053m	
		D400 = 1,451m	
		D300 = 754m	
		D250 = 6,389	
		D200 = 4,090m	
		D150 = 31,789m	
D100 = 29,255m			
Water meter	Number	24,411	

Source: PDAM

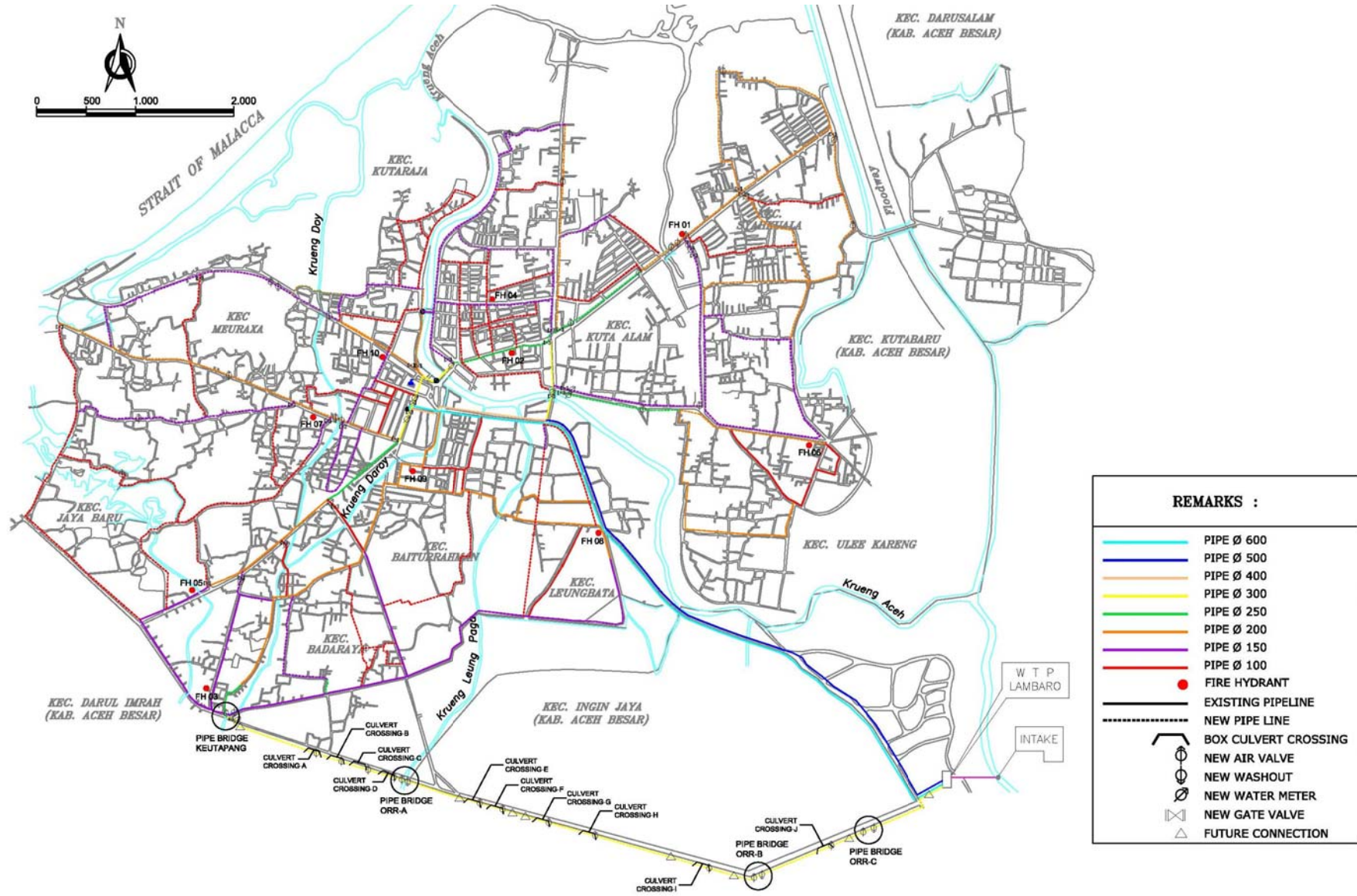


Figure 1.1.1 General Layout of Pipe Water Supply System before the 2004 Disaster

(1) Water Treatment Plants

A schematic layout of Lambaro and Siron WTPs are as shown in Figures 1.1.2 and 1.1.3 respectively. Both the plants depend on unregulated surface runoff of the Aceh River for raw water intake.

The Lambaro WTP has a raw water pumping station on the left bank of the Aceh River with 5 pumping units. The capacity of each pump is as presented in Table 1.1.1. The treatment plant has a daily production capacity of 37,584 m³ and is divided into two (2) lines. The first line was constructed in 1974 and the second in 1998. The first line appears to be timeworn.

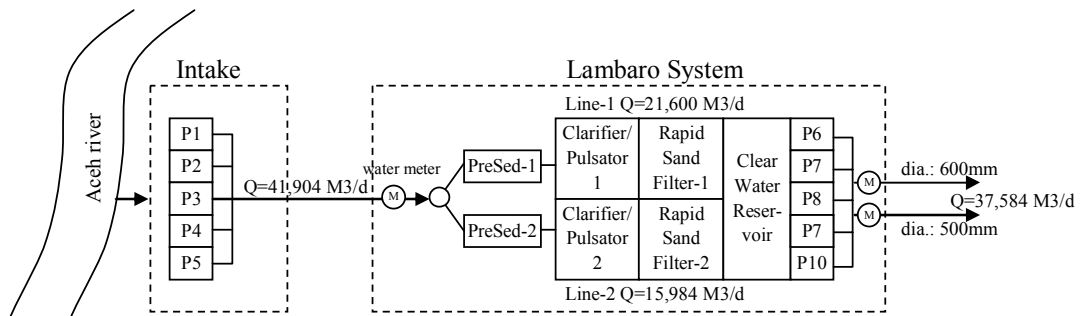


Figure 1.1.2 Schematic Layout of Lambaro WTP

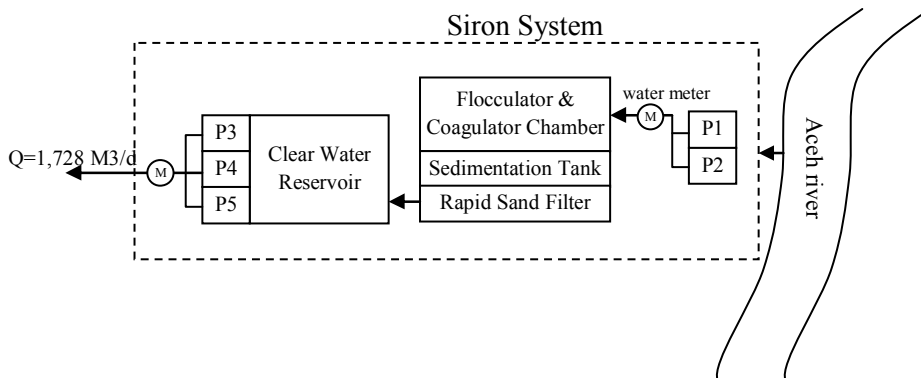


Figure 1.1.3 Schematic Layout of Siron WTP

The Siron WTP is located on the right bank of the Aceh River and had a daily production capacity of 3,460m³.

(2) Distribution Facilities

The distribution systems of the Lambaro and Siron supply systems are basically the same each other. The system consists of treated water pumping facilities and primary, secondary and tertiary distribution pipelines. There is no service reservoir in distribution system, and the treated water is distributed directly from clear water reservoir in the treatment plant by means of pressured flow by treated water pumping facilities.

The Lambaro system is equipped with 5 treated water pumping units and has two (2) primary distribution mains, while the Siron system is provided with 3 treated water pumping units and a single primary main.

Total length of the distribution pipelines was approximately 87 km and there were 24,400 water meters. Pipe materials were steel for pipe diameter larger than 500 mm and polyvinyl for pipes less than 350 mm in diameter.

(3) Status of Water Supply

Pipe water supply system had a daily water supply capacity of 40,000 m³ before the 2004 disaster and had individual connection of 24,411 and public tap of 100. The historical water supply record and layout of distribution network reveals the following:

- ① Assuming the average family member of each household was 5, total connected people were 138,984 in 2004, meaning service connection ratio of about 57%. The rest of population was deemed to be relied on groundwater and/or other water resources for its water supply.
- ② The average water consumption per capita was 136 liter per day.
- ③ Un-accounted for water was relatively high, calculated at 49%.
- ④ The distribution pipelines appear to be not systematic and rational, resulting in uneven distribution of pressure and water through out the service area.
- ⑤ There was no major industry or bulk water consumer in the Project area, though there were a large number of retail shops throughout the Project area..

1.1.4 Pipe Water Supply after Disaster

(1) Physical Damages

① Water treatment plants

Fortunately, there is no serious structural and mechanical damage in Lambaro WTP, so that the plant has started its production soon after the disaster. Installment in auto-desludging equipment in pre-sedimentation and clarifier are however out of order in the first treatment line. On the other hand the Siron WTP had lost its production capacity at all.

② Distribution Network

The distribution pipelines were destroyed in many locations and areas especially in the areas along the coast (Kecamatan Meraxa, Kuta Raja, Jaya Baru and Syiah Kuala) where the

houses and infrastructure were completely destroyed and/or heavily damaged. It is still not possible to distribute the treated water to such Kecamatan areas.

As of September 2005, pipe water service is provided to the areas where distribution pipelines are not damaged and/or restored already, and those 3 Kecamatan areas are served by public taps which are regularly filled up by water tankers.

(2) Administrative Damages

According to PDAM, out of 173 PDAM staff, 29 staffs were killed and/or missing, hampering operation of water supply system, billing and collecting tariff.

In addition various data and information on pipe water service were destroyed, in particular distribution pipelines record, tariff billing and collection, etc.

1.1.5 Available Data and Information

(1) Topography and Geology

There are very limited topographic and geological data and information made available for detail design.

Topographic map in a scale of 1 to 2000 is available but it is without contour. URRP has produced GIS by using latest satellite images after the 2004 disaster and is very useful tool for the current design works. Unfortunately elevation data was not made available during the design period.

Sub-surface and soil characteristics data were also hardly available. It is therefore necessary to conduct detailed topographic mapping along the proposed pipelines and sub-surface geological survey at major structure sites such as pipe bridges before execution of actual construction works.

(2) Data on Pre-disaster Pipe Water Supply Facilities

For the purpose of completing the detailed design, it is required to redevelop distribution pipe network map including diameters of the pipes installed before the disaster. The JICA Study Team in cooperation with PDAM attempted such redevelopment before commencement of the design. At first PDAM staff concerned located distribution pipes with diameters on topographic maps in a scale of 1 to 2,000 based on their knowledge. The JICA Study Team and PDAM then conducted field survey to confirm such information from street to street and physical condition of the primary and secondary pipe lines by visual inspection. The distribution network map shown in Figure 1.1.1 is output of these elaborations.

1.2 DETAIL DESIGN

1.2.1 Design Criteria

In July 6, 2005 there was a meeting at Meeting Room of Urban and Rural Development, Western Region, DG of Human Resettlement, MPW in Jakarta to discuss about restoration works of the

distribution system among the representatives of JICA Study Team, Ciptakarya, BRR and other parties concerned and the following are mutually agreed among the attendants:

- (1) Service population should be 80% of population in Banda Aceh City in 2009.
- (2) Elevated tank should not be reconstructed, since it is observed that it does not function in many areas in Indonesia as wells as Banda Aceh City.
- (3) Minimum water pressure and other design criteria/condition should follow PU Guidelines.

Based on the above the following design criteria are determined to be adopted:

- (1) Design discharge : Maximum hourly, to be 1.5 times of the average daily demand
- (2) Required minimum pressure : 5 m
- (3) Hydraulic analysis : EPANET-2
- (4) Boosting capacity of Transmission pumps : 6 bars (to be the same capacity of pumps existing at Lambaro WTP)
- (5) Standards for pipe : BS, AWWA, JIS and their equivalent

1.2.2 Water Demand Forecast

(1) Forecast Population

Though the Project aims at restoring the pipe water distribution network basically to the situation before the 2004 disaster, it is envisaged to redevelop the city with disaster preparedness and more upgraded infrastructure than the ones existed before the disaster. The JICA study Team has worked out a spatial plan and land use plan of the Banda Aceh City as explained and presented in detail in the Main Report. This land use plan is fundamental for laying out pipelines.

Population in the Project area is another important element in forecasting pipe water demand in the Project area. The Main Report also presents the future population forecast in the Project area on a basis of Desa (village) unit and it is summarized in Table 1.2.1.

Table 1.2.1 Projected Population by Desa

NO	KECAMATAN	DESA/KELURAHAN	ID #	POPULATION FORECAST					NO	KECAMATAN	DESA/KELURAHAN	ID #	POPULATION FORECAST				
				2005	2006	2007	2008	2009					2005	2006	2007	2008	2009
1	MEURAXA	Gampong Pie	117101001	94	93	93	92	92	5	JAYA BARU	Lampho Daya	117101101	513	516	519	522	525
		Deah Glumpang	117101002	332	330	328	326	325			Emperon	117101102	873	872	871	871	870
		Ulee Lheu	117101003	784	787	790	793	796			Lanjamee	117101103	413	413	414	415	416
		Lambung	117101004	239	239	238	238	237			Bitai	117101104	369	370	372	373	375
		Alue Deah Tengah	117101005	219	220	220	220	221			Lantemen Barat	117101105	2,368	2,370	2,372	2,374	2,376
		Deah Baro	117101006	202	202	203	203	203			Lantemen Timur	117101106	745	744	742	741	740
		Cot Lamkewueh	117101007	175	174	173	173	172			Ulee Patatah	117101107	157	158	158	158	159
		Blang Oi	117101008	573	576	578	581	584			Geuceu Menara	117101108	2,702	2,702	2,702	2,702	2,702
		Gampong Blang	117101009	84	85	86	87	88			Punge Blang Cut	117101109	3,222	3,230	3,238	3,246	3,254
		Lamjabat	117101010	169	168	167	166	165			Total	11,362	11,375	11,388	11,402	11,417	
		Asoe Nanggroe	117101011	169	169	168	168	167			Lhong Raya	117101201	1,893	2,080	2,277	2,486	2,706
		Punge Ujong	117101012	504	506	508	509	511			Lampuoet	117101202	2,566	2,787	3,042	3,346	3,729
		Lampaseh Aceh	117101013	418	419	421	423	424			Mibo	117101203	1,509	1,996	2,467	2,908	3,290
		Punge Jurong	117101014	1,041	1,038	1,034	1,031	1,027			Lam Ara	117101204	1,637	2,049	2,501	3,001	3,567
		Surien	117101015	313	316	318	320	323			Lhong Cut	117101205	1,831	2,516	3,217	3,923	4,614
		Gampong Baro	117101016	345	345	346	347	348			Penjeurat	117101206	1,754	2,455	3,191	3,962	4,805
Total	5,661	5,667	5,671	5,677	5,683	5,683	Geuceu Kaye Jato	117101207	1,041	1,082	1,118	1,145	1,162				
2	BAITURRAHMAN	Desa Ateuk Jawo	117102001	4,366	4,411	4,461	4,519	4,586	Geuceu Inem	117101208	2,076	2,312	2,558	2,813	3,072		
		Desa Ateuk Deah Tanoh	117102002	1,382	1,382	1,383	1,384	1,386	Geuceu Komplek	117101209	2,488	2,683	2,884	3,090	3,297		
		Kel. Ateuk Pahlawan	117102003	4,727	4,740	4,753	4,768	4,785	Lamlagang	117101210	4,430	4,455	4,482	4,511	4,542		
		Desa Ateuk Munjeng	117102004	1,643	1,630	1,657	1,665	1,674	Total	21,225	24,415	27,737	31,185	34,784			
		Desa Neusu Aceh	117102005	966	975	986	998	1,012	Batoh	117102101	4,521	5,640	6,863	8,212	9,830		
		Kel. Setui	117102006	5,017	5,022	5,031	5,038	5,047	Landom	117102102	1,625	2,415	3,240	4,093	5,058		
		Kel. Sukaramai	117102007	4,716	4,724	4,733	4,742	4,753	Cot mesjid	117102103	2,715	2,906	3,104	3,310	3,519		
		Kel. Neusu Jaya	117102008	3,578	3,586	3,595	3,605	3,616	Desa Lueng Bata	117102104	3,232	3,308	3,389	4,138	4,505		
		Kel. Peniti	117102009	7,888	7,890	7,895	7,898	7,901	Blang Cut	117102105	1,997	2,497	3,029	3,598	4,203		
		Kel. Kampong Baro	117102010	2,611	2,633	2,658	2,686	2,720	Lampaloh	117102106	533	619	711	807	908		
Total	36,894	37,013	37,152	37,303	37,480	37,480	Sukadama	117102107	2,137	2,183	2,231	2,281	2,334				
3	KUTA ALAM	Peunayong	117103001	2,858	2,872	2,887	2,903	2,919	Pantenek	117102108	1,146	1,498	1,860	2,229	2,597		
		Laksana	117103002	6,630	6,669	6,689	6,709	6,729	Lamseupeng	117102109	2,731	2,832	2,941	3,059	3,189		
		Keuramat	117103003	6,214	6,225	6,236	6,247	6,258	Total	20,637	24,098	27,788	31,727	36,143			
		Kuta Alam	117103004	4,183	4,186	4,189	4,192	4,195	Ie Masen Kaye Adang	117104001	3,254	3,297	3,345	3,399	3,461		
		Beurawe	117103005	6,102	6,130	6,202	6,260	6,325	Pineung	117104002	3,753	3,777	3,803	3,833	3,865		
		Kota Baro	117103006	1,450	1,465	1,481	1,499	1,517	Lamgugop	117104003	8,639	8,745	8,867	9,010	9,178		
		Bandar Baru	117103007	6,605	6,656	6,713	6,775	6,845	Kopelma Darussalam	117104004	5,926	5,982	6,045	6,120	6,209		
		Mulia	117103008	3,098	3,157	3,223	3,299	3,386	Rukoh	117104005	8,866	8,918	8,977	9,046	9,126		
		Lampulo	117103009	2,372	2,444	2,524	2,615	2,719	Jeulingke	117104006	4,116	4,180	4,252	4,334	4,431		
		Lamdingin	117103010	1,581	1,646	1,718	1,798	1,891	Tibang	117104007	912	981	1,059	1,150	1,256		
Lambaro Skep	117103011	2,394	2,458	2,530	2,609	2,700	Alue Naga	117104008	483	557	641	739	854				
Total	43,507	43,928	44,392	44,906	45,484	45,484	Deah Raya	117104009	35	64	97	135	180				
4	ULEE KARENG	Pango Raya	117104101	1,605	2,400	3,250	4,157	5,222	Total	35,984	36,501	37,086	37,766	38,560			
		Pango Deah	117104102	695	1,219	1,734	2,222	2,656	Kel. Keudah	117103101	602	622	644	672	701		
		Ilie	117104103	2,580	3,301	4,079	4,922	5,942	Kel. Pelanggahan	117103102	987	1,026	1,070	1,121	1,180		
		Lamteh	117104104	2,198	2,472	2,765	3,079	3,416	Gp. Pande	117103103	220	252	289	332	383		
		Lambhuk	117104105	4,146	4,671	5,199	5,719	6,210	Gp. Jawa	117103104	1,247	1,370	1,511	1,674	1,867		
		Doy	117104106	2,074	2,393	2,751	3,161	3,647	Kel. Merduati	117103105	1,463	1,497	1,535	1,578	1,629		
		Lam Glumpang	117104107	2,482	2,703	2,943	3,205	3,496	Lampaseh Kota	117103106	858	892	932	977	1,031		
		Ceurih	117104108	2,855	3,249	3,663	4,092	4,533	Total	5,377	5,659	5,981	6,354	6,791			
		Ie Masen Ulee Kareng	117104109	1,561	1,829	2,088	2,329	2,536	GRAND TOTAL	200,843	212,893	225,667	239,206	254,000			
		Total	20,196	24,237	28,472	32,886	37,658	37,658									

(2) Water Demand Forecast

① Population in the Project area

Population in the Project area is as stated in Table 1.2.1. According to the last census average family size is set at 5 people per household throughout the planning horizon.

② Pipe water service ratio

The pipe water service ratio is estimated at 57 % in 2004. According to PDAM medium term plan and the PU Guidelines it is planned to increase year after year and to reaches at 57 % in 2005 to 80 % in 2009.

It is also planned that the pipe water will be served through individual connection and public taps. The rates of individual and public taps connection are assumed at 90 % and 10 % respectively.

③ Unit water consumption

Water demand is classified into domestic use and non-domestic uses. It is also assumed that quantity of the non-domestic use is equivalent to 20 % of the domestic use and this rate is constant throughout the planning horizon.

Per capita water consumption in the domestic use is 150 liter per day on average and remains constant through out the planning horizon.

④ Un-accounted for water

After the disaster the un-accounted for water is estimated to amount to about 50 % of total water produced. It is assumed that the rate would gradually reduce year after year: 50 % in 2005 to 30 % in 2009.

Those basic parameters are as summarized in Table 1.2.2.

Table 1.2.2 Basic Parameters Applied for Water Demand Forecast

Description		unit	2005	2006	2007	2008	2009	PU guideline
Connection	Served population	%	58	60	65	70	80	
	Average family size		1:5					1:(5-6)
	House connection	%	90					80-90
	Public tap	%	10					10-20
Unit consumption	House connection	lpcd	150					150
	Public tap	lpcd	40					30-50
	Non-Domestic	%	20					15-30
UFW		%	50	45	40	35	30	30-50
Maximum Daily Demand Factor			1.1					1.1-1.25
Peak Hourly Demand Factor			1.5					1.5-2.0

note: PU guideline for house connection per capita consumption applied figure for the city with 100,000-500,000 population.

⑤ Forecast Water Demand

The water demand is forecasted for the respective desa annually during the period from 2005 to 2009 and for the year 2015 as reference for a long-term pipe water supply planning. Table 1.2.3 presents the net amount of the domestic and non-domestic demands in the year 2009. The average daily gross water demand including the amount of the un-accounted for water is forecasted at 29,146 m³ in 2005, 30,894 m³ in 2006, 34,269 m³ in 2007, 37,705 m³ in 2008, and 44,062 m³ in 2009.

Table 1.2.3 Water Demand and Supply Capability

Description		unit	2005	2006	2007	2008	2009	2015
Population		person	200,843	212,893	225,767	239,206	254,000	360,304
Served Population	Total	person	116,489	127,736	146,749	167,444	203,200	288,243
	House Conn.	person	104,840	114,962	132,074	150,700	182,880	259,419
	Public Tap	person	11,649	12,774	14,675	16,744	20,320	28,824
Net Demand	House Conn.	m ³ /day	15,726	17,244	19,811	22,605	27,432	38,913
	Public Tap	m ³ /day	466	511	587	670	813	1,153
	Non-Domestic	m ³ /day	3,238	3,551	4,080	4,655	5,649	8,013
	TOTAL	m ³ /day	19,430	21,306	24,478	27,930	33,894	48,079
UFW	Rate		50%	45%	40%	35%	30%	30%
	Amount	m ³ /day	9,715	9,588	9,791	9,775	10,168	14,424
Gross Demand		m ³ /day	29,146	30,894	34,269	37,705	44,062	62,503
Supply Capacity	Lambaro	m ³ /day	37,584	37,584	37,584	37,584	37,584	37,584
	Siron	m ³ /day	1,728	1,728	1,728	1,728	1,728	1,728
	Total	m ³ /day	39,312	39,312	39,312	39,312	39,312	39,312
Balance		m ³ /day	10,166	8,418	5,043	1,607	-4,750	-23,191

⑥ Design Discharge for Distribution Network

The daily average demand by desa in 2009 is as given in Table 1.2.4. These figures are basically employed as design discharge of distribution pipelines after applying hourly peak factor.

Table 1.2.4 Net Water Demand by Desa

KECAMATAN	DESA/KELURAHAN	ID #	water demand (m3/day)	KECAMATAN	DESA/KELURAHAN	ID #	water demand (m3/day)	
MEURAXA	Gampong Pie	117101001	12.28	JAYA BARU	Lampoh Daya	117101101	70.06	
	Deah Glumpang	117101002	43.37		Emperon	117101102	116.09	
	Ulee Lheu	117101003	106.22		Lanjamee	117101103	55.51	
	Lambung	117101004	31.63		Bitai	117101104	50.04	
	Alue Deah Tengoh	117101005	29.49		Lamtemen Barat	117101105	317.05	
	Deah Baro	117101006	27.09		Lamtemen Timur	117101106	98.75	
	Cot Lamkeweuh	117101007	22.95		Ulee Patah	117101107	21.22	
	Blang Oi	117101008	77.93		Geuceu Menara	117101108	360.55	
	Gampong Blang	117101009	11.74		Punge Blang Cut	117101109	434.21	
	Lamjabat	117101010	22.02		Total	1,523.48		
	Asoe Nanggroe	117101011	22.28		BANDA RAYA	Lhong Raya	117101201	361.09
	Punge Ujong	117101012	68.19			Lampuoet	117101202	497.60
	Lampaseh Aceh	117101013	56.58			Mibo	117101203	439.02
	Punge Jurong	117101014	137.04			Lam Ara	117101204	475.98
	Surien	117101015	43.10			Lhong Cut	117101205	615.69
	Gampong Baro	117101016	46.44			Penjeurat	117101206	641.18
Total	758.34		Geuceu Kaye Jato	117101207		155.06		
			Geuceu Inem	117101208		409.93		
			Geuceu Komplek	117101209		439.95		
			Lamlagang	117101210		606.08		
BAITURRAHMAN	Ateuk Jawo	117102001	611.96	Total	4,641.58			
	Ateuk Deah Tanoh	117102002	184.95	LUENG BATA	Batoh	117102101	1,311.72	
	Ateuk Pahlawan	117102003	638.51		Lamdom	117102102	674.94	
	Ateuk Munjeng	117102004	223.38		Cot mesjid	117102103	469.58	
	Neusu Aceh	117102005	135.04		Desa Lueng Bata	117102104	601.15	
	Setui	117102006	673.47		Blang Cut	117102105	560.85	
	Sukaramai	117102007	634.24		Lampaloh	117102106	121.16	
	Neusu Jaya	117102008	482.52		Sukadamai	117102107	311.45	
	Peniti	117102009	1,054.31		Panteriek	117102108	346.54	
	Kampong Baro	117102010	362.96		Lamseupeng	117102109	425.67	
Total	5,001.33		Total		4,823.06			
KUTA ALAM	Peunayong	117103001	389.51	SYIAH KUALA	Ie Masen Kaye Adang	117104001	461.84	
	Laksana	117103002	897.92		Pineung	117104002	515.75	
	Keuramat	117103003	835.07		Lamgugop	117104003	1,224.71	
	Kuta Alam	117103004	559.78		Kopelma Darussalam	117104004	828.53	
	Beurawe	117103005	844.01		Rukoh	117104005	1,217.64	
	Kota Baro	117103006	202.43		Jeulingke	117104006	591.27	
	Bandar Baru	117103007	913.40		Tibang	117104007	167.60	
	Mulia	117103008	451.83		Alue Naga	117104008	113.96	
	Lampulo	117103009	362.82		Deah Raya	117104009	24.02	
	Lamdingin	117103010	252.34		Total	5,145.31		
Lambaro Skep	117103011	360.29	KUTA RAJA	Keudah	117103101	93.54		
Total	6,069.38			Pelanggahan	117103102	157.46		
				Gp. Pande	117103103	51.11		
				Gp. Jawa	117103104	249.13		
				Merduati	117103105	217.37		
				Lampaseh Kota	117103106	137.58		
				Total	906.19			
ULEE KARENG	Pango Raya	117104101		696.82	GRAND TOTAL	Total	33,893.76	
	Pango Deah	117104102		354.42				
	Ilie	117104103	792.90					
	Lamteh	117104104	455.83					
	Lambhuk	117104105	828.66					
	Doy	117104106	486.66					
	Lam Glumpang	117104107	466.51					
	Ceurih	117104108	604.88					
	Ie Masen Ulee Kareng	117104109	338.40					
Total	5,025.08							

1.2.3 Network Analysis under Pre-disaster Condition

It is considered to be important to assess whether distribution network existed before the 2004 disaster is capable of properly conveying the design discharge (the forecast water demand in the year 2009) with required residual pressure with booster pumping facilities existing. For this purpose of assessment, the distribution pipelines existed before the disaster is aligned into network as given in Figure 1.2.1 and the design discharge is assigned to the respective section as presented in Table 1.2.5.

Table 1.2.5 Design Discharge for Hydraulic Analysis

Water Demand Nodes	Population Served	Water Demand (L/sec)
<u>Kec. Meuraxa</u> <i>Node No.: 91, 92, 95, 98, 99, 100</i>	4,546	11.41
<u>Kec. Baiturrahman</u> <i>Node No.: 3, 56, 57, 58, 59, 60, 61, 62, 66, 67, 68, 77, 83, 84, 85, 86, 87, 88, 89, 90, 111</i>	29,984	75.25
<u>Kec. Kuta Alam</u> <i>Node No.: 6, 7, 8, 9, 10, 11, 12, 13, 14, 15, 16, 17, 18, 18, 19, 20, 21, 22, 28, 29, 34, 49, 50, 51, 107, 108, 109, 110</i>	35,587	91.32
<u>Kec. Ulee Kareng</u> <i>Node No.: 38, 39, 40, 41, 42, 43, 44, 45, 46, 47, 48, 125</i>	30,126	75.61
<u>Kec. Java Baru</u> <i>Node No.: 81, 82, 94, 96, 97, 115, 116, 117, 126</i>	9,134	22.92
<u>Kec. Banda Raya</u> <i>Node No.: 69, 70, 71, 72, 73, 74, 75, 76, 78, 79, 80, 112, 113, 114, 129</i>	27,827	69.84
<u>Kec. Lueng Bata</u> <i>Node No.: 1, 2, 4, 5, 55, 63, 64, 6, 123, 124, 131</i>	28,914	72.57
<u>Kec. Sviah Kuala</u> <i>Node No.: 30, 31, 32, 33, 35, 36, 37, 52, 53, 54</i>	30,848	77.42
<u>Kec. Kuta Raja</u> <i>Node No.: 23, 24, 25, 26, 27, 101, 102, 103, 104, 105, 106, 118</i>	5,433	13.64
TOTAL	203,200	509.98

The results of network analysis are shown in Table 1.2.6 and in Figure 1.2.2 showing residual pressure. The following are the conclusion:

- ① The design discharge is able to be distributed with the required residual head only to the central part of the Project area. It is found that several sections subject to negative pressure.
- ② The eastern and western parts of the Project area are unable to receive the required quantity of water mainly due to complicated network and smaller size of pipe in many sections.

Table 1.2.6

Hydraulic Analysis of Pre-Disaster Distribution Network

* Version 2.0 *

Input File: Existing.NET

Link - Node

Link ID	Start Node	End Node	Length m	Diameter mm
1	1	5	5,908	500
2	5	4	145	500
3	1	2	4,906	600
4	2	3	2,140	600
5	2	55	971	150
6	5	6	191	400
7	4	3	998	400
8	3	60	262	400
9	3	61	610	100
10	61	62	466	100
11	61	58	341	100
12	58	59	100	100
13	59	56	216	100
14	58	57	256	100
15	62	57	572	200
16	57	56	146	200
17	62	68	837	200
18	56	55	705	200
19	55	63	668	200
20	63	65	541	200
21	63	64	1,094	100
22	65	64	1,736	150
23	64	66	1,642	150
24	66	67	369	150

Node Results:

Node ID	Demand LPS	Pressure m
1	-401.91	Reservoir
2	2.21	46.72
3	2.21	46.58
4	2.21	44.52
5	2.21	44.50
6	2.21	43.42
7	2.21	40.80
8	3.21	8.46
9	3.21	8.18
10	3.21	8.53
11	3.21	7.64
12	3.21	7.44
13	3.55	8.28
14	3.55	10.98
15	3.55	8.85
16	3.55	16.29
17	3.55	14.56
18	3.55	13.89
19	3.55	20.12
20	3.55	19.20
21	3.55	13.52
22	3.55	8.26
23	3.21	12.59
24	3.21	12.15

Link Results:

Link ID	Velocity m/s
1	0.81
2	0.17
3	0.86
4	0.71
5	2.23
6	1.52
7	0.29
8	1.16
9	2.09
10	0.93
11	0.88
12	0.26
13	0.02
14	0.34
15	0.62
16	0.61
17	0.77
18	0.69
19	0.50
20	0.31
21	0.48
22	0.46
23	0.58
24	0.14

25	67	68	997	150
26	67	69	664	200
27	60	83	380	250
28	83	77	949	250
29	83	84	216	250
30	84	88	185	250
31	88	85	181	250
32	84	86	739	100
33	85	86	985	100
34	85	87	72	200
35	87	89	88	200
36	89	93	223	200
37	93	92	183	200
38	86	93	738	100
39	92	91	204	150
40	88	87	1,536	150
41	89	90	467	150
42	90	111	420	100
43	111	91	946	100
44	68	77	635	100
45	6	7	78	300
46	7	19	520	250
47	7	40	1,542	150
48	7	48	1,301	200
49	48	40	243	200
50	40	41	332	150
51	41	42	978	150
52	41	43	1,004	100
53	43	45	502	100
54	45	42	502	100
55	48	43	2,775	100
56	42	46	81	100
57	46	47	433	250
58	47	39	1,741	100
59	39	38	1,026	150
60	42	38	1,168	100
61	39	36	274	100

25	3.21	12.04
26	3.21	11.97
27	3.21	11.97
28	3.21	18.11
29	3.21	6.72
30	3.21	19.59
31	3.21	19.26
32	3.21	18.89
33	3.21	18.47
34	3.21	19.24
35	2.31	5.89
36	4.14	3.00
37	1.46	4.66
38	3.23	4.61
39	3.84	3.21
40	4.51	27.34
41	3.62	18.37
42	4.08	5.59
43	4.85	7.22
44	3.61	4.98
45	4.23	5.32
46	5.21	3.26
47	4.84	3.17
48	3.59	28.92
49	3.21	9.00
50	3.21	19.49
51	1.69	20.25
52	4.14	18.21
53	3.53	15.53
54	1.15	16.49
55	2.21	9.95
56	2.21	8.65
57	2.21	8.48
58	2.21	8.96
59	2.21	8.93
60	2.82	45.62
61	2.21	12.79

25	0.59
26	0.34
27	2.92
28	1.44
29	1.41
30	1.25
31	1.06
32	0.56
33	0.32
34	1.46
35	1.54
36	1.11
37	1.10
38	0.43
39	0.76
40	0.34
41	0.56
42	0.81
43	0.28
44	1.26
45	2.67
46	2.60
47	0.88
48	1.10
49	0.84
50	1.53
51	1.03
52	0.65
53	0.60
54	0.04
55	0.57
56	1.24
57	0.09
58	0.04
59	0.31
60	0.17
61	0.17

62	36	35	1,099	100	62	2.82	4.98	62	0.35
63	38	35	616	150	63	2.21	7.69	63	0.42
64	40	52	1,818	150	64	1.60	6.04	64	0.59
65	19	30	1,154	250	65	1.60	7.30	65	0.45
66	30	32	246	200	66	2.21	1.08	66	0.55
67	32	52	307	200	67	2.21	0.99	67	0.39
68	30	31	70	100	68	2.21	4.21	68	0.22
69	31	50	994	100	69	5.58	0.49	69	0.19
70	19	50	448	200	70	7.97	-1.38	70	0.46
71	50	28	497	200	71	7.96	-1.35	71	0.31
72	19	20	41	250	72	7.97	-1.98	72	1.79
73	20	16	346	250	73	7.97	-1.22	73	1.24
74	20	49	517	150	74	7.97	-1.29	74	1.31
75	49	8	145	150	75	7.97	-1.27	75	0.67
76	8	9	142	100	76	5.58	-0.22	76	0.29
77	9	10	502	100	77	3.25	21.31	77	0.12
78	8	11	506	150	78	3.25	16.13	78	0.36
79	11	12	165	150	79	5.58	0.87	79	0.28
80	12	107	143	150	80	3.25	7.34	80	0.27
81	107	108	618	150	81	3.25	6.93	81	0.19
82	107	22	269	100	82	3.86	6.80	82	0.39
83	12	13	252	100	83	3.25	30.53	83	0.37
84	11	10	199	100	84	3.55	28.52	84	0.43
85	10	13	174	100	85	3.55	25.77	85	0.24
86	13	22	168	100	86	3.55	23.53	86	0.01
87	22	21	369	100	87	3.55	24.44	87	0.83
88	14	16	461	100	88	3.55	27.37	88	0.76
89	14	17	230	100	89	3.55	22.82	89	0.90
90	15	18	239	100	90	3.55	20.72	90	1.04
91	16	17	238	250	91	3.55	16.77	91	1.05
92	17	18	162	250	92	3.55	18.32	92	0.83
93	18	21	173	250	93	3.55	20.32	93	0.59
94	21	23	183	250	94	3.86	5.62	94	0.57
95	23	24	181	250	95	1.86	15.31	95	0.43
96	21	7	1,119	100	96	1.86	15.31	96	1.14
97	23	105	922	150	97	1.86	13.93	97	0.22
98	24	25	231	300	98	1.86	12.98	98	0.25

99	25	26	364	300
100	26	27	81	300
101	26	103	370	100
102	103	104	111	100
103	104	105	155	100
104	25	105	684	200
105	105	106	220	200
106	27	101	519	150
107	101	102	106	100
108	102	103	611	100
109	102	106	1,019	150
110	15	13	78	100
111	14	10	87	100
112	52	53	757	200
113	35	53	1,261	150
114	53	54	1,426	200
115	35	37	1,980	100
116	33	34	556	100
117	34	28	594	150
118	28	51	1,163	200
119	11	109	589	100
120	107	108	601	100
121	49	29	517	150
122	29	109	650	150
123	109	110	413	150
124	110	118	1,267	150
125	66	69	443	150
126	69	71	385	150
127	71	113	1,100	150
128	113	114	161	150
129	114	115	1,156	150
130	114	79	1,206	150
131	79	80	237	150
132	115	81	792	150
133	81	82	780	150
134	79	75	404	150
135	71	70	151	250

99	1.86	13.69
100	1.86	13.98
101	3.21	12.07
102	3.21	11.64
103	3.21	11.83
104	3.21	11.83
105	3.21	12.30
106	3.21	12.27
107	3.21	7.40
108	3.18	7.00
109	3.21	5.86
110	3.21	7.76
111	3.55	14.96
112	7.97	-0.65
113	5.58	-0.59
114	5.57	0.51
115	2.03	5.52
116	1.86	13.41
117	2.03	9.04
118	1.41	5.51

99	0.11
100	0.03
101	0.30
102	0.04
103	0.37
104	0.21
105	0.14
106	0.05
107	0.43
108	0.15
109	0.06
110	0.59
111	1.21
112	0.59
113	0.79
114	0.04
115	0.19
116	0.20
117	0.27
118	0.05
119	0.19
120	0.15
121	0.46
122	0.28
123	0.18
124	0.08
125	0.32
126	0.60
127	0.20
128	0.68
129	0.57
130	0.43
131	1.50
132	0.57
133	0.09
134	0.75
135	0.13

136	70	72	1,071	100
137	72	73	191	150
138	68	73	872	200
139	73	74	101	200
140	74	75	104	200
141	74	78	623	100
142	113	112	391	250
143	112	76	173	150
144	76	75	809	200
145	77	78	625	250
146	78	80	495	200
147	80	81	378	200
148	111	82	984	100
149	82	94	212	100
150	94	117	1,887	100
151	115	117	2,282	100
152	91	96	738	150
153	96	97	1,059	150
154	97	116	370	100
155	117	116	510	100
156	92	95	1,151	200
157	95	99	802	200
158	99	100	599	200
159	96	95	613	100
160	116	100	919	100
161	101	98	1,564	200
162	98	99	1,441	200
163	95	98	999	100
164	42	44	201	100
165	44	45	400	75
166	32	33	131	100
167	110	108	445	150

136	0.21
137	0.54
138	0.68
139	0.12
140	0.17
141	1.21
142	0.06
143	0.29
144	0.34
145	1.17
146	1.42
147	0.48
148	0.65
149	0.37
150	0.13
151	0.25
152	0.44
153	0.34
154	0.53
155	0.63
156	0.56
157	0.42
158	0.14
159	0.02
160	0.34
161	0.23
162	0.22
163	0.30
164	0.35
165	0.20
166	0.21
167	0.08

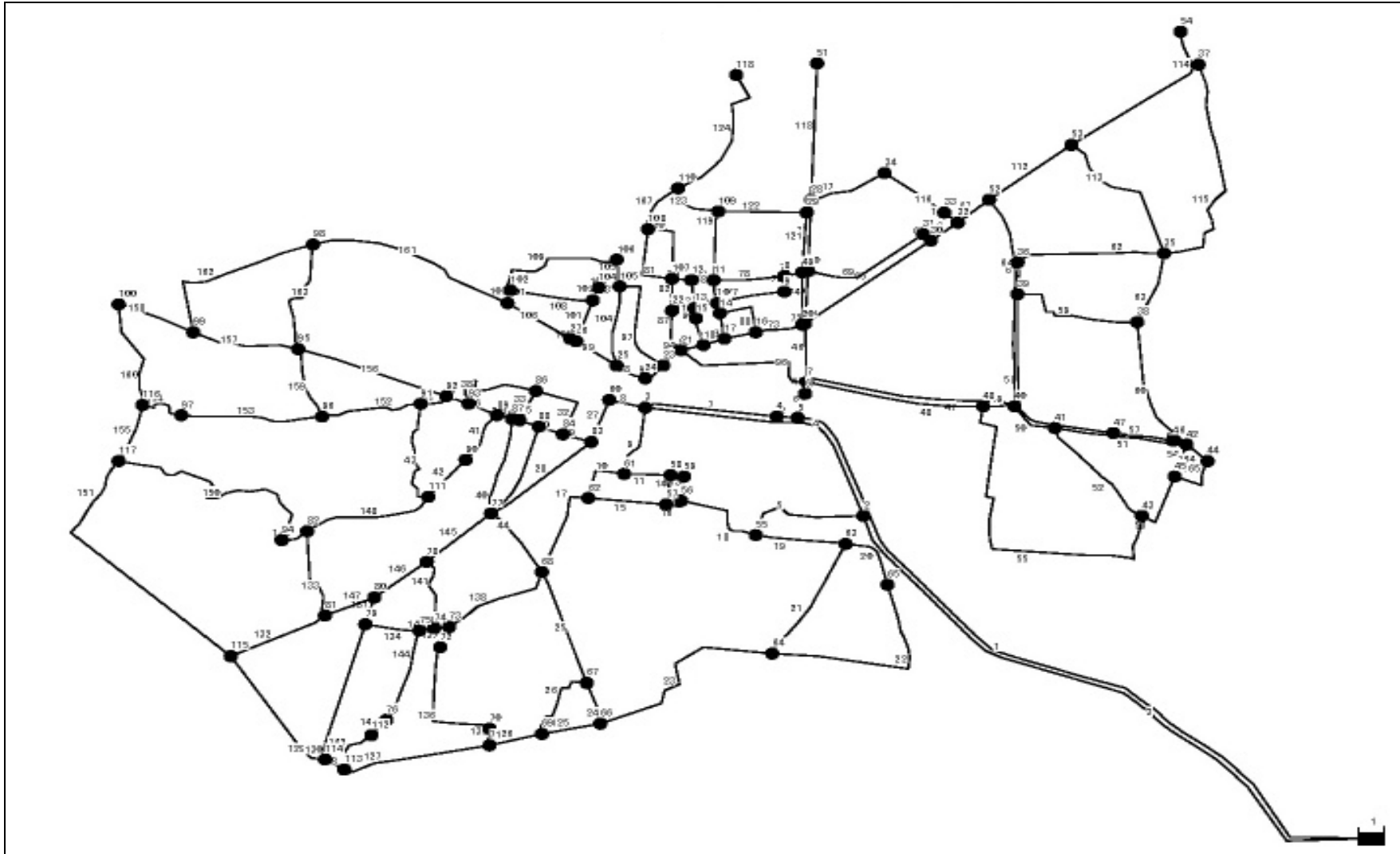


Figure 1.2.1 Water Distribution Link and Nodes, Pre-disaster Case

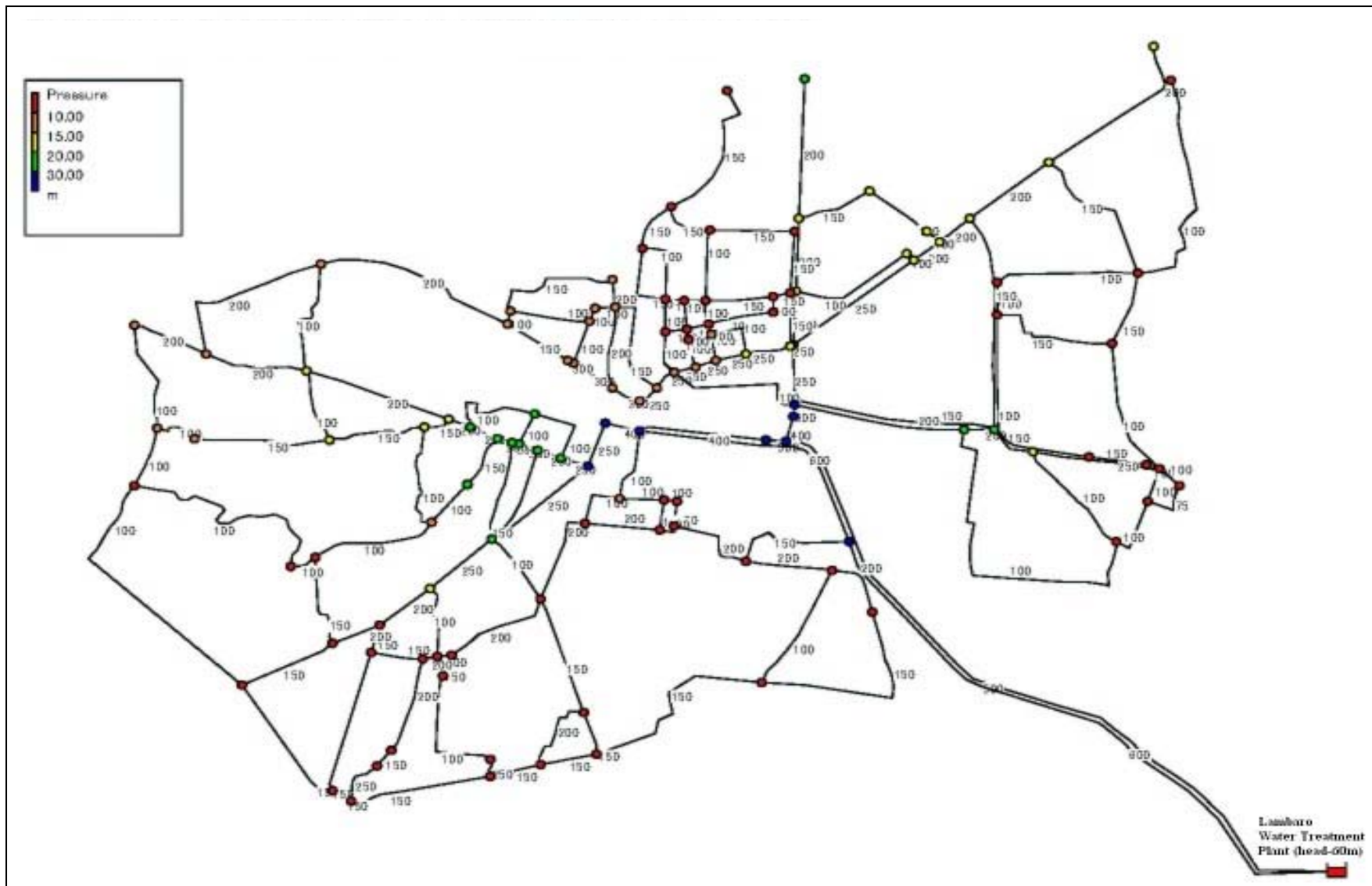


Figure 1.2.2 Result of Hydraulic Analysis, Pre-disaster Case

1.2.4 Restoration Design of Distribution System

(1) Basic Approach

Taking into consideration of the hydraulic characteristics and damages of the pre-disaster distribution network, the restoration works are determined to include the following principles:

- ① A complete new distribution network will be installed in the area of Kecamatan Jaya Baru, Muaraxa and Ulee Kareng where distribution pipelines were completely destroyed and/or severely damaged.
- ② Aiming at achieving rational and efficient control and flexibility in water distribution, the water service area is proposed to be divided into four (4) zones as shown in Figure 1.2.3.
- ③ Aiming also at minimizing head loss and efficient distribution of water throughout the network, it is determined to adopt a loop system. The system will also allow changing direction of flow in case of emergency.
- ④ New secondary pipelines will be located in the southern part of the Project area in compliance with the proposed expansion of the city structure. It is proposed to accommodate new housing and city government offices in future in such area. The new secondary pipeline will eventually contribute efficient water supply to the southern and western parts (Zones 1 and 2).
- ⑤ It is proposed to replace existing pipes which are identified to be defective in terms of hydraulic design with new pipes. Such replacement mainly occurs in Zone 4.
- ⑥ It is important to increase the number of water meter as many as possible in order to reduce un-accounted for water and increase collection of tariff. It is proposed to procure 5,000 sets of PDAM standard type connection unit under the Project, and their installation are determined to be executed directly by PDAM keeping pace with progress of rehabilitation and reconstruction of residential houses and commercial units in the disaster affected areas.

(2) Restoration of Distribution Network

The proposed distribution network is almost identical to the ones existed before the 2004 disaster. But it is more effective and efficient in terms of water distribution and control and stronger against possible disaster. The lengths of pipe lines by diameters are as given in Table 1.2.7.

Table 1.2.7 Pipe ID Table (1/2)

Table with 18 columns (No, PIPE ID, DIAMETER, LENGTH, DRAWING) and 18 rows, representing pipe data for various zones (ZONE 1 to ZONE 3). The table is divided into sections for ZONE 1, ZONE 2, and ZONE 3, with each section containing a list of pipe IDs, diameters, lengths, and drawing references.

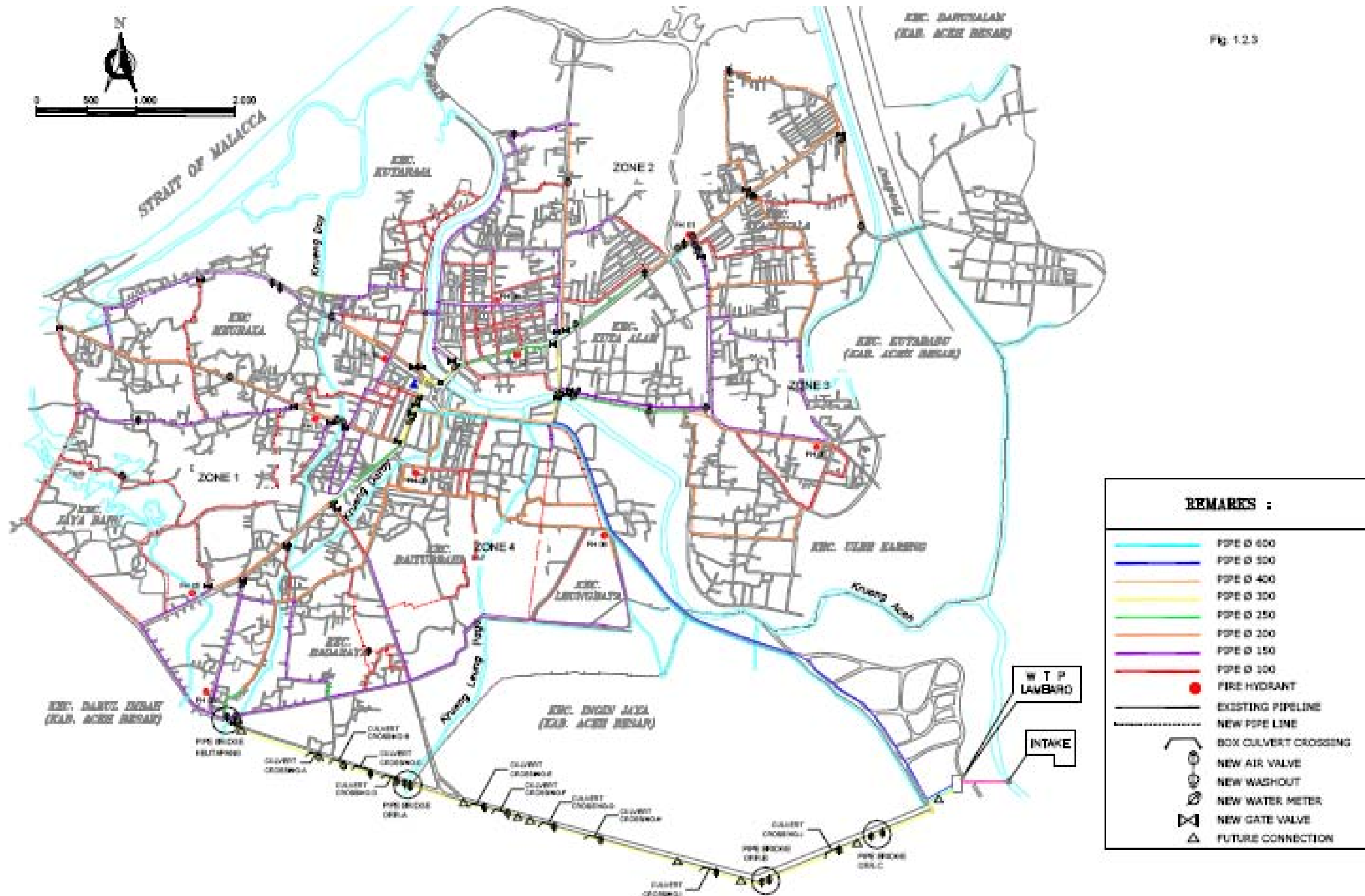


Fig. 1.2.3

Figure 1.2.3 Water Supply Distribution Network Plan

(a) Pipe materials

Pipe materials are selected through technical discussion between the JICA Study Team, PU and PDAM and in due consideration of strength, durability and easiness in installation and transportation. It is proposed to use Ductile Iron Pipe (DIP) for pipes with a diameter of 400 and more, and High Density Polyethylene Pipe (HDPE) for the others.

(b) Fire hydrant

Ten (10) fire hydrants are proposed to be installed by the Project at the locations shown in Figure 1.2.3. The location is determined based on consultation to PDAM, Dinas PU and Department of Fire Fighting, and take into account population density, land use condition, administrative services, etc.

(c) Public stand pipe

It is proposed to install 33 public stand pipes by the Project, especially in most affected three (3) Kecamatan: Baru Jaya, Muaraxa and Ulee. A large portion of lands in these areas are still under water and no land restoration works are yet taken, though temporary housing construction has been taken place at many locations. In order to expedite return of internally displaced people from their temporary shelters, it is considered to be matter of importance to extend safe pipe water service. There are a number of villages in such areas but the public stand pipe is determined to be installed at a rate of one location per 355 inhabitants. As a result, the number and location of public stand pipe are as follows:

Table 1.2.8 Distribution of Public Stand Pipe

Kecamatan	Desa	Unit
1. Meraxa	except desa Punge Jurong	15 (one per desa)
2. Kuta Raja	Gumpong Jawa	4
	Gumpong Pande	1
	Peulanggahan	3
	Kedua	2
3. Jaya Baru	Lampoh Daya	1
	Lanjamee	1
	Bitai	1
	Ulee Patah	1
	Geuceu Menara	1
4. Syiah Kuala	Tibang	3
TOTAL		33

(3) Network Analysis for Proposed Restoration

A trial and error method is carried out to identify the optimal network in terms of hydraulic behavior and restoration cost. The hydraulic calculation was conducted by using a computer software ESPANET-2. The design discharge is the same as that applied for the hydraulic analysis of the pre-disaster disaster network. The results of calculation are as shown in Table 1.2.9.

A schematic distribution network of the optimum plan is as shown in Figure 1.2.4, which also indicates the residual water pressure.

1.2.5 Design of Pipe Laying Works and Valve Chambers

(1) Pipe Laying Works

Pipe laying works basically follows the PDAM design standards. In principle, the pipe is designed to be buried within the right of way of existing road, and characterized by types of backfilling of trench there are two (2) types in installation as shown in Figure 1.2.5. Minimum coverage of backfill is determined to be 60cm above crown of the pipes.

Pipes are designed to be laid in trench and backfilled with selected material. Where there is pavement, such pavement is to be reinstated with the same materials as existing after backfilling of the trench.

Where the pipes are located within a road carriage both longitudinal and crossing directions, they are designed to be protected by surrounding concrete.

* Table 1.2.9 *

Hydraulic Analysis of Proposed Restoration Network

* Version 2.0 *

Link - Node Table:

Link ID	Start Node	End Node	Length m	Diameter mm
1	1	5	5,908	500
2	5	4	145	400
3	122	1	6,546	600
4	130	131	302	100
5	55	130	971	200
6	5	6	191	400
7	4	60	1,260	400
8	3	60	262	600
9	3	61	610	200
10	61	62	466	200
11	61	58	341	100
12	58	59	100	100
13	59	56	216	100
14	58	57	256	100
15	62	57	572	200
16	57	56	146	200
17	62	68	837	200
18	56	55	705	200
19	124	55	300	200
20	63	131	200	200
20b	65	131	341	200
21	63	64	1,094	100
22	65	64	1,736	150
23	64	66	1,642	150
24	66	67	369	150

Node Results:

Node ID	Demand LPS	Pressure m
1	-509.88	Reservoir
3	2.87	41.97
4	3.00	40.00
5	2.28	40.01
6	9.38	39.01
7	2.87	36.65
8	5.48	24.90
9	4.18	24.65
10	4.18	25.62
11	1.58	24.80
12	4.18	24.68
13	4.62	25.57
14	4.62	28.42
15	4.62	26.43
16	4.62	33.81
17	4.62	33.90
18	4.62	34.23
19	4.62	33.43
20	4.62	33.23
21	4.62	34.97
22	4.62	25.65
23	4.62	34.95
24	4.18	35.37
25	2.09	36.27
26	2.08	33.63

Link Results:

Link ID	Velocity m/s
1	0.90
2	0.17
3	0.91
4	0.14
5	0.30
6	1.22
7	0.14
8	0.70
9	1.40
10	1.13
11	0.73
12	0.38
13	0.94
14	0.75
15	0.53
16	0.62
17	0.49
18	0.77
19	0.37
20	0.16
20b	0.12
21	0.12
22	0.10
23	0.03
24	0.32

25	67	68	997	150
26	67	69	664	100
27	60	83	380	300
28	83	77	949	250
29	83	84	216	200
30	84	88	185	200
31	88	85	181	200
32	84	121	400	100
33	85	86	985	150
34	85	87	72	200
35	87	89	88	200
36	89	93	223	200
37	93	92	183	200
38	86	93	738	100
39	92	91	204	150
40	88	87	1,536	150
41	89	90	467	150
42	90	111	420	100
43	111	91	946	100
44	68	77	635	100
45	6	7	78	250
46	7	19	520	300
47	7	128	1,542	150
48	7	48	1,301	250
49	48	40	243	200
50	40	41	332	150
51	41	42	978	200
52	41	43	1,004	100
53	43	45	502	100
54	45	42	502	100
55	48	43	2,775	200
56	47	125	514	150
57	113	46	6	150
58	47	39	1,741	150
59	39	38	1,026	150
60	38	125	1,168	150
61	36	39	274	200

27	2.30	37.67
28	4.18	27.97
29	4.18	23.68
30	4.18	29.34
31	4.17	29.02
32	4.18	28.49
33	4.17	27.92
34	4.17	28.79
35	3.00	23.36
36	4.21	22.70
37	1.76	25.52
38	4.19	22.91
39	4.99	22.62
40	5.86	33.97
41	4.71	32.21
42	1.35	31.71
43	5.52	30.13
44	4.40	30.73
45	4.07	30.65
46	0.00	33.81
47	4.64	22.13
48	4.67	34.70
49	4.20	25.25
50	4.20	30.77
51	2.20	28.91
52	5.38	27.58
53	4.69	25.32
54	1.50	25.65
55	2.87	19.63
56	2.87	24.34
57	2.87	25.08
58	2.87	28.03
59	1.43	28.44
60	3.66	41.65
61	2.87	31.84
62	3.66	24.63
63	2.87	17.80

25	0.59
26	0.23
27	1.36
28	0.98
29	1.41
30	1.23
31	0.98
32	0.42
33	0.33
34	1.02
35	1.05
36	0.62
37	0.60
38	0.49
39	0.56
40	0.31
41	0.49
42	0.52
43	0.27
44	1.05
45	2.93
46	1.41
47	0.47
48	0.62
49	0.54
50	0.63
51	0.22
52	0.06
53	0.36
54	0.26
55	0.28
56	0.13
57	1.49
58	0.14
59	0.14
60	0.20
61	0.15

62	36	35	1,099	200
63	38	35	616	200
64	52	128	1,818	150
65	19	30	1,154	250
66	30	32	246	250
67	32	52	307	250
68	30	31	70	100
69	31	50	994	100
70	19	50	448	200
71	50	28	497	200
72	19	20	41	250
73	20	16	346	250
74	20	49	517	150
75	49	8	145	150
76	8	9	142	100
77	9	10	502	100
78	8	11	506	150
79	11	12	165	150
80	12	107	143	150
81	107	108	618	150
82	107	22	269	100
83	12	13	252	100
84	11	10	199	100
85	10	13	174	100
86	13	22	168	100
87	22	21	369	100
88	14	16	461	100
89	14	17	230	100
90	15	18	239	100
91	16	17	238	250
92	17	18	162	250
93	18	21	173	250
94	21	23	183	300
95	23	24	181	300
96	21	7	1,119	100
97	23	105	992	150
98	24	25	231	300

64	2.08	19.74
65	2.08	17.68
66	2.87	20.05
67	2.87	20.62
68	2.87	25.15
69	7.25	19.85
70	9.84	19.95
71	10.35	19.96
72	10.35	20.34
73	10.35	22.54
74	10.35	22.78
75	10.35	23.30
76	2.08	38.02
77	4.35	31.30
78	4.22	29.48
79	7.25	24.41
80	3.22	26.12
81	2.11	26.13
82	5.02	23.94
83	4.22	37.68
84	2.30	34.08
85	4.61	30.13
86	2.30	31.78
87	4.61	29.42
88	2.30	31.90
89	4.61	28.68
90	4.61	27.01
91	3.61	26.16
92	4.61	27.05
93	4.61	27.74
94	3.02	23.46
95	2.41	27.28
96	2.41	26.02
97	2.09	43.48
98	2.41	28.47
99	2.41	27.06
100	2.41	27.34

62	0.29
63	0.33
64	0.47
65	0.81
66	0.69
67	0.56
68	0.21
69	0.32
70	0.86
71	0.64
72	0.58
73	0.07
74	1.15
75	0.58
76	0.27
77	0.26
78	0.15
79	0.22
80	0.15
81	0.07
82	0.40
83	0.38
84	0.41
85	0.10
86	0.15
87	1.14
88	0.77
89	1.12
90	1.31
91	0.15
92	0.42
93	0.73
94	0.66
95	0.77
96	0.31
97	0.18
98	0.83

99	25	26	364	200
100	76	26	293	150
101	26	103	370	100
102	103	104	111	100
103	104	105	100	100
104	25	105	684	200
105	105	106	220	200
106	101	26	590	200
107	101	102	106	100
108	102	103	611	100
109	106	127	255	150
109b	102	127	764	150
110	15	13	78	100
111	14	10	87	100
112	52	53	757	250
113	35	53	1,261	200
114	53	37	1,091	200
115	35	37	1,980	200
116	33	34	556	100
117	34	28	594	150
118	28	51	1,163	200
119	11	109	589	100
120	107	108	601	100
121	49	29	517	150
122	29	109	650	150
123	109	110	413	150
124	110	118	1,267	150
125	66	69	443	150
126	69	71	385	150
127	71	113	1,100	150
128	129	114	121	150
129	114	115	1,156	150
130	114	79	1,206	150
131	79	80	237	150
132	126	81	528	150
132b	115	126	264	150
133	81	82	780	100

101	4.72	32.61
102	4.18	32.66
103	4.18	33.17
104	4.72	33.22
105	4.72	34.89
106	4.17	34.67
107	4.18	24.74
108	4.13	24.65
109	1.57	23.48
110	4.18	25.60
111	4.61	24.46
112	10.35	29.45
113	8.19	30.70
114	7.74	29.09
115	3.14	27.45
116	3.91	27.45
117	3.14	26.25
118	1.83	25.62
119	0.32	38.79
120	2.30	37.79
121	2.00	37.71
122	1.43	46.43
123	2.28	23.16
124	1.30	23.91
125	1.35	25.72
126	3.51	30.24
127	0.00	38.84
128	0.00	37.16
129	0.00	33.65
130	10.06	23.39
131	0.00	23.55

99	0.89
100	0.54
101	0.36
102	0.12
103	0.72
104	0.55
105	0.31
106	0.43
107	0.14
108	0.05
109	0.27
109b	0.32
110	0.73
111	1.30
112	0.62
113	0.43
114	0.24
115	0.28
116	0.22
117	0.33
118	0.32
119	0.07
120	0.05
121	0.33
122	0.10
123	0.04
124	0.34
125	0.20
126	0.11
127	0.88
128	1.08
129	0.31
130	0.33
131	0.68
132	0.31
132b	0.07
133	0.35

134	79	75	404	150
135	71	70	151	250
136	70	72	1,071	150
137	72	73	191	150
138	68	73	872	200
139	73	74	101	200
140	74	75	104	200
141	74	78	623	100
142	112	129	391	250
143	112	75	982	200
144	94	126	357	100
145	77	78	625	250
146	78	80	495	200
147	80	81	378	200
148	111	82	984	100
149	82	94	212	100
150	94	117	1,887	100
151	115	117	2,282	100
152	91	96	378	150
153	116	96	1,429	150
154	113	129	40	150
155	117	116	510	100
156	92	95	1,151	200
157	95	99	802	200
158	99	100	599	200
159	96	95	613	100
160	116	100	919	100
161	101	98	1,564	150
162	98	99	1,441	150
163	95	98	999	100
164	42	44	201	100
165	44	45	400	100
166	32	33	131	100
167	110	108	445	150
168	97	25	300	300
169	51	118	655	150
170	37	54	335	200

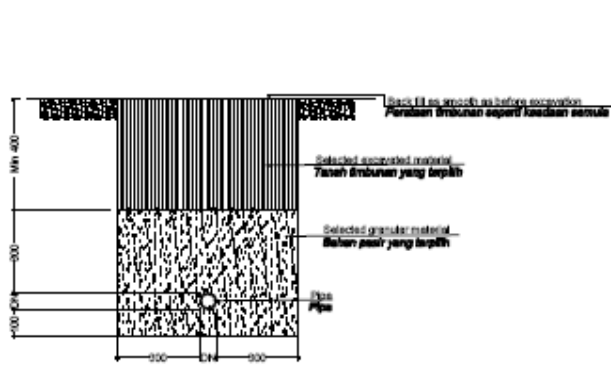
134	0.60
135	0.06
136	0.38
137	0.96
138	0.33
139	0.55
140	0.70
141	0.72
142	0.65
143	0.69
144	0.41
145	0.72
146	0.81
147	0.33
148	0.21
149	0.09
150	0.06
151	0.14
152	0.24
153	0.23
154	0.15
155	0.31
156	0.14
157	0.04
158	0.15
159	0.29
160	0.29
161	0.56
162	0.32
163	0.23
164	0.46
165	0.10
166	0.31
167	0.14
168	1.50
169	0.44
170	0.09

171	1	46	6,968	300
172	86	27	300	150
173	27	76	123	150
174	60	97	198	300
175	76	119	234	100
176	119	97	220	100
177	86	121	332	100
178	121	120	225	100
179	120	119	200	100
180	27	120	234	100
181	59	122	557	100
182	106	127	902	100
183	122	3	500	600
184	123	130	1,316	100
185	123	124	1,074	100
186	63	124	368	200
187	54	53	2,799	200
188	46	129	40	250

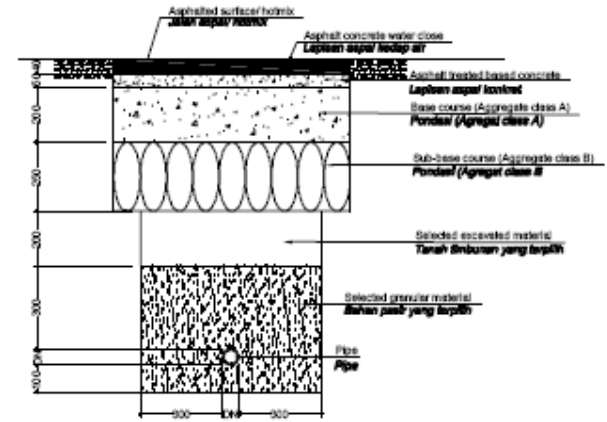
171	1.06
172	0.54
173	0.59
174	1.65
175	0.38
176	1.04
177	0.31
178	0.15
179	0.62
180	0.18
181	1.51
182	0.11
183	0.87
184	0.08
185	0.21
186	0.28
187	0.14
188	0.99



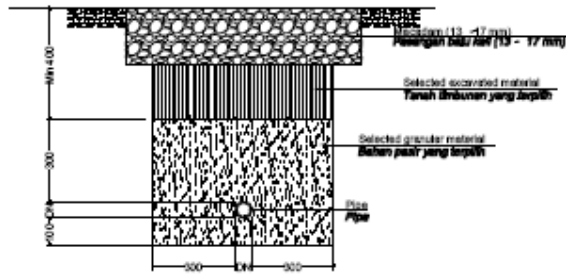
Figure 1.2.4 Result of Hydraulic Analysis, Proposed Restoration Network



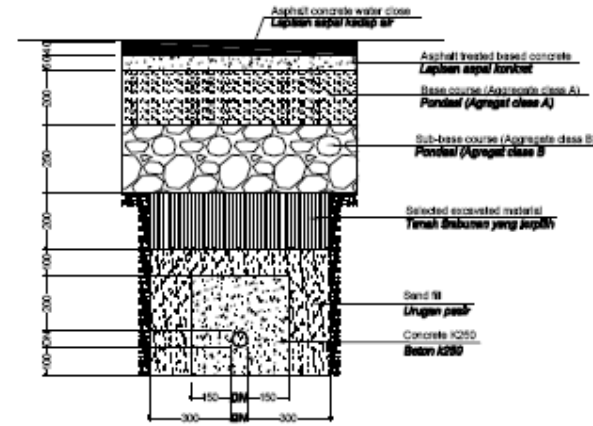
Type I (Beneath Road Shoulder)
Tipe I (Dibawah Bahu Jalan)



Type II (Beneath Asphalt Pavement)
Tipe II (Dibawah Jalan Aspal)



Type III (Beneath Macadam Pavement)
Tipe III (Dibawah Jalan Kerikil)



Type IV (Road Crossing)
Tipe IV (Penyeberangan Jalan)

Fig. 1.2.4

Figure 1.2.5 Pipe Laying

(2) Valve Chambers

Pipelines associate with 31 air valves, 28 section valves, and 7 washouts as shown in Table 1.2.10.

Table 1.2.10 List of Valves

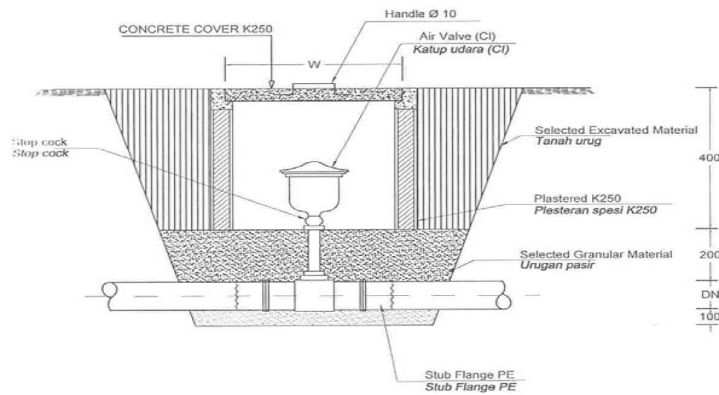
Valves	Diameter	Zone 1	Zone 2	Zone 3	Zone 4	Total
Gate valve	300	3	-	-	1	4
	250	-	-	2	1	3
	200	-	2	2	2	6
	150	1	3	3	-	7
	100	-	6	-	2	8
Air valve	300	13	-	-	-	13
	250	1	-	3	1	5
	200	-	2	2	1	5
	150	-	3	1	2	6
	100	1	1	-	-	2
Wash-out	300	3	-	-	-	3
	250	1	-	1	-	2
	200	-	1	-	-	1
	150	-	1	-	-	1
	100	-	-	-	-	-
Water meter	300	1	2	-	2	5
	250	-	-	-	1	1
	200	-	-	-	2	2
	150	1	-	1	1	3
	100	-	1	-	1	2

① Air valves

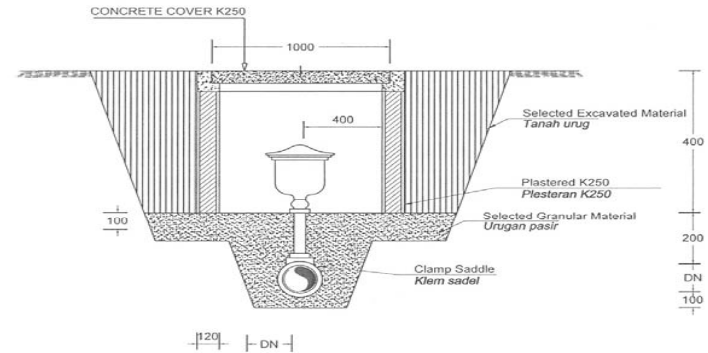
There two (2) types of air valves: one is a single vent and the other is double vents. The air valves are designed to be installed in concrete chambers excepting pipe bridges sites. The design of the chamber is as shown in Figure 1.2.6.

② Section valves

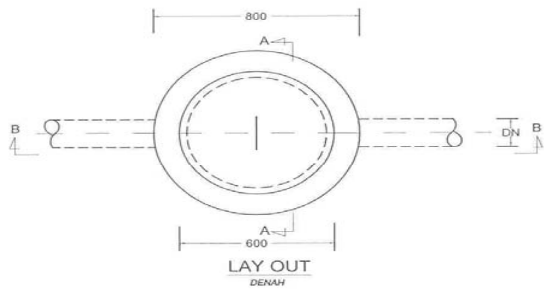
The section valves are to be sited at 28 locations, and their locations are determined in conjunction with zoning of the network. All the valves are to be encased in concrete chamber of which design is shown in the Figure 1.2.7.



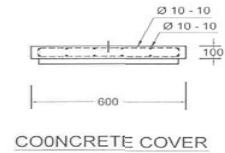
PROFILE
POTONGAN B-B



PROFILE
POTONGAN A-A



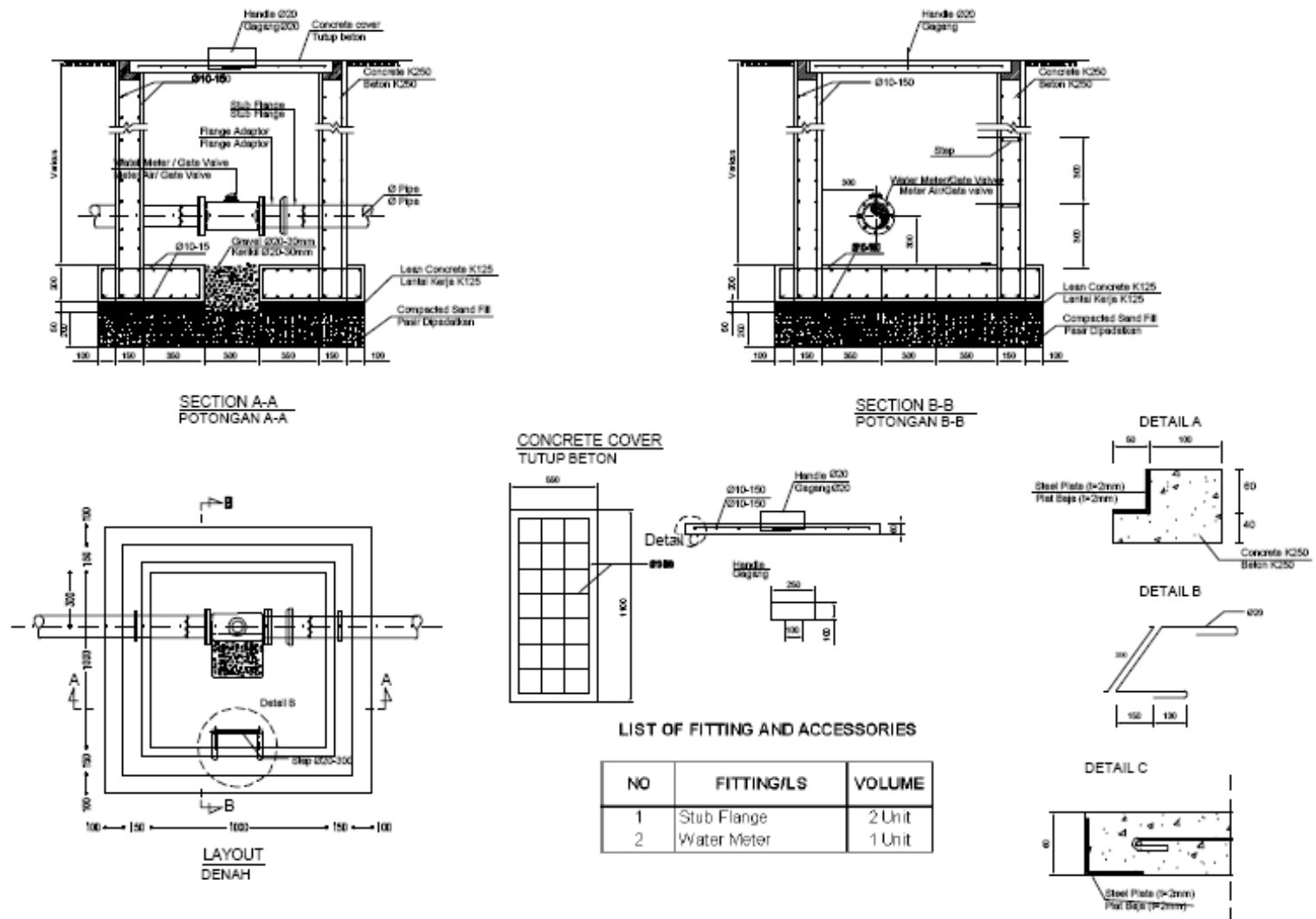
LAY OUT
DENAH



TYPE	APPLICABLE DIAMETER	W (mm)
A	300 , 250	1000
B	200 , 150 , 100	800

Note : The detailed location of the air valve will determined upon completion of the pipeline route survey

Figure 1.2.6 Air Valve Chamber



LIST OF FITTING AND ACCESSORIES

NO	FITTING/LS	VOLUME
1	Stub Flange	2 Unit
2	Water Meter	1 Unit

Figure 1.2.7 Standard Gate Valve and Water Meter Chamber