

4 CAFETERIA

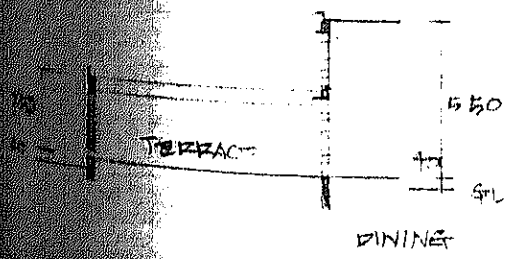
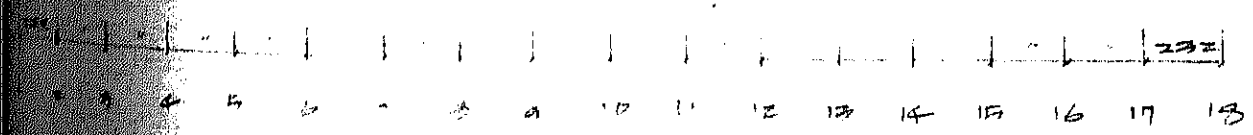
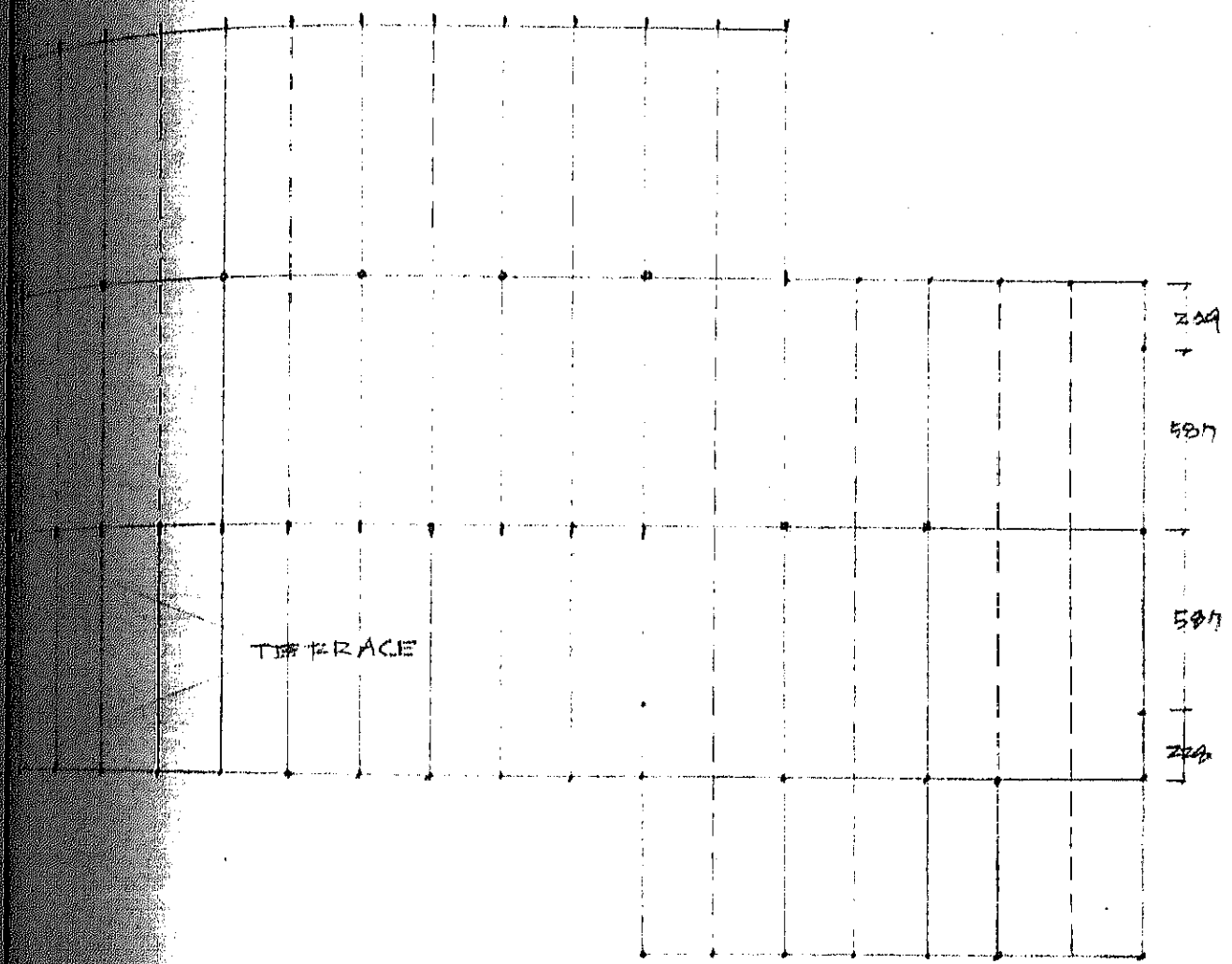
LOAD

(1) DEAD LOAD

ROOF	LIRE CONCRETE	10cm	200	} 460 <sup>kg/m<sup>2</sup></sup>
	R.C. SLAB	10	240	
	CEILING		20	
WALL	BRICK			} 490
	CEMENT MORTAR		50	
BEAM	200 x 400	1.40	1400	
	200 x 75	5.10	510	
COLUMN	200 x 75	5.10	510	
	200 x 200	1.00	1000	
	400	3.10	310	
	400 x 400	3.90	390	

(2) LIVE LOAD

ROOF	SLAB	FLAKE	SEISMIC
	100	50	0



C.M.O.G. OF BEAM

$w = 0.51 \frac{1}{12}$

$l_x = 2.2 = 2.2$      $l_y = 5.36$      $\lambda = 2.31$      $w' = 0.34$



$C = 2.2 \times 2 \times 0.51 + 0.34 \times 5.36^2 / 12 = 2.25 + 0.41 = 3.6$

$I_x = 4.3 \times \dots + \dots / 12 = 4.39 + 1.22 = 5.6$

$I_y = 2.4 \times \dots + 0.34 \times 5.36^2 / 12 = 2.45 + 0.91 = 3.4$

$l_x = 3.32$      $l_y = 4.15$      $\lambda = 3.51$      $w' = 0.51$



$C = 6.0 \times 2 \times 0.51 + 0.51 \times 4.15^2 / 12 = 6.32 + 2.18 = 8.5$

$I_x = 9.6 \times \dots + \dots / 12 = 9.79 + 4.23 = 14.0$

$I_y = 4.12 \times \dots + 0.51 \times 4.15^2 / 12 = 4.20 + 2.04 = 6.4$

$l_x = 2.32$      $l_y = 4.96$      $\lambda = 3.43$      $w' = 0.51$



$C = 6.0 \times 2 \times 0.51 + 0.51 \times 4.96^2 / 12 = 6.32 + 2.69 = 9.0$

$I_x = 9.4 \times \dots + \dots / 12 = 9.59 + 4.04 = 13.6$

$I_y = 4.1 \times \dots + 0.51 \times 4.96^2 / 12 = 4.10 + 2.03 = 6.2$

$l = 0.15$      $w = 0.24$      $l = 2.32$



$C = 0.24 \times 0.15^2 / 12 = 1.3$     0.1

$I_x = \dots / 12 = 2.0$     0.2

$I_y = 0.24 \times 0.15 / 12 = 1.0$     0.3

$l_x = 2.32$      $\lambda = 1.0$      $w' = 0.44$



$C = 0.32 \times 0.51 + 0.44 \times 2.32^2 / 12 = 0.16 + 0.20 = 0.4$

$I_x = 0.63 \times \dots + \dots / 12 = 0.27 + 0.20 = 0.5$

$I_y = 0.68 \times \dots + 0.44 \times 2.32^2 / 12 = 0.31 + 0.41 = 0.9$


$l = 4.64$      $P = 3.4 + 6.4 = 9.8$      $w' = 0.51$



$C = 0.51 \times 0.51 + 9.8 \times 4.64^2 / 12 + 0.51 \times 4.64^2 / 12 = 17.7$

$I_x = 0.112 \times \dots + \dots / 12 + \dots / 12 = 14.3$


$I_y = 3.09 \times \dots + 9.8 / 12 + 0.51 \times 4.64^2 / 12 = 7.4$

500-10   $l = 464$   $P = 64 + 62 = 126$   $w' = 0.51$

$C = 271 \times 0.51 + 126 \times 464 / 3 + 0.51 \times 464^2 / 12 = 9.6$

$M_x = 271 \times \dots + \dots / 4 + \dots / 3 = 176$

$Q = 269 \times \dots + 126 / 2 + 0.51 \times 464 / 2 = 9.9$

600-10   $l = 696$   $P = 126$   $w' = 0.61$

$C = 401 \times 0.61 + 126 \times 696 \times 2 / 3 + 0.61 \times 696^2 / 12 = 244$

$M_x = 401 \times \dots + \dots \times 1/2 + \dots / 3 = 36.6$


$Q = 394 \times \dots + 126 + 0.61 \times 696 / 2 = 16.8$

601-2   $l = 464$   $P = 62 + 62 = 124$   $w' = 0.51$

$C = 271 \times 0.51 + 124 \times 464 / 3 + 0.51 \times 464^2 / 12 = 9.2$

$M_x = 271 \times \dots + \dots / 2 + \dots / 3 = 17.3$


$Q = 269 \times \dots + 124 / 2 + 0.51 \times 464 / 2 = 9.8$

5100   $l_x = 372$   $l_y = 796$   $\lambda = 3.53$   $w' = 0.61$

$C = 6.0 \times 0.61 + 0.61 \times 372^2 / 12 = 6.4$

$M_x = 9.4 \times \dots + \dots / 3 = 9.9$

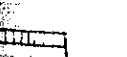
$Q = 4.1 \times \dots + 0.61 \times 372 / 2 = 4.6$

6000   $l_x = 232$   $l_y = 576$   $\lambda = 2.72$   $w' = 0.44$

$C = 2.0 \times 0.51 + 0.44 \times 232^2 / 12 = 2.4$

$M_x = 4.3 \times \dots + \dots / 3 = 4.8$

$Q = 2.4 \times \dots + 0.44 \times 232 / 2 = 2.4$

6100   $l = 232$   $w' = 0.44 + 0.51 \times 232 / 2 = 1.04$

$C = 1.04 \times 232^2 / 12 = 5.5$


$l = 228$   $l = 209$   
0.5                      0.4

$M_x = \dots / 3 = 0.7$

0.7                      0.6

$Q = 1.04 \times 232 / 2 = 1.2$


1.2                      1.1

Ex 11.1   $j = 5.43$   $w' = 1.07$

$$C = 10.4 \times 5.43^2 / 12 = 2.9$$

$$P_0 = 1.07 \times \dots / 12 = 4.4$$

$$Q = 10.4 \times 5.43 / 12 = 3.5$$

Ex 11.2   $j = 6.2$   $P = 6.2$   $w' = 0.61$

$$C = 10.1 \times 6.2 + 6.2 \times 6.2^2 / 12 - 0.61 \times 6.2^3 / 12 = 5.3$$

$$P_0 = 1.46 \times \dots + \dots / 12 + \dots / 12 = 9.7$$


$$Q = 1.75 \times \dots + 6.2 / 12 + 0.61 \times 6.2^2 / 12 = 4.2$$

$k = 4.64$   $P = 6.2$   $w' = 0.61 + 0.54 \times 0.75 = 0.92$

$$C = 10.1 \times 6.2 + 6.2 \times 6.2^2 / 12 - 0.92 \times 6.2^3 / 12 = 5.9$$

$$P_0 = 1.46 \times \dots + \dots / 12 + \dots / 12 = 10.7$$

$$Q = 1.75 \times \dots + 6.2 / 12 - 0.92 \times 6.2^2 / 12 = 6.0$$

Ex 11.3   $j = 7.64$   $w' = 1.092$

$$C = 6.0 \times 7.64 + 0.92 \times 7.64^2 / 12 = 7.6$$

$$P_0 = 9.4 \times \dots + \dots / 12 = 11.6$$

$$Q = 4.1 \times \dots + 0.92 \times 7.64 / 12 = 4.2$$

AXIAL FORCE OF COLUMN

ROOF		WALL		BEAM	COLUMN	P
				1.17	0.55	1.7
				1.39	0.55	1.9
0.01 x 1.16 x 3.93	2.35	1.02 x 3.76	0.51 4.34	3.16	3.23	15.0
0.01 x 2.72 x 3.93	4.71	2.63 x 1.02	0.43	3.52	3.23	13.6
0.01 x 2.72 x 7.96	9.42	0.22 x 2.04	0.23	6.90	3.09	20.2
0.01 x 4.64 x 7.96	18.84		0.40	10.03	1.83	30.7
0.01 x 1.16 x 3.93	2.35	1.02 x 3.67	0.41 5.34	2.27	3.23	16.2
0.01 x 2.72 x 3.93	4.71	2.63 x 1.02	2.34	2.64	3.23	15.9
0.01 x 1.16 x 2.64	1.19	0.22 x 2.04	0.23	1.21	0.84	7.3
0.01 x 2.72 x 2.64	3.17	0.43 x 3.66	0.31	1.56	0.84	7.7
0.01 x 2.72 x 2.64	3.17	0.43 x 2.04	0.23	4.26	0.84	10.1
0.01 x 2.72 x 3.93	4.71	0.95 x 4.72	0.39	1.45	0.84	11.8
0.01 x 4.64 x 6.76	16.07	2.23 x 3.62	0.02	7.97	0.84	32.8
0.01 x 3.46 x 6.76	12.00	0.43 x 4.54	4.34	0.84	0.84	26.1
0.01 x 1.16 x 4.06	2.41	0.43 x 2.70	0.01	1.29	0.84	8.1
0.01 x 3.93 x 3.93	7.06	0.22 x 1.02	0.24	6.34	3.23	23.8
0.01 x 2.72 x 3.93	3.51	0.43 x 3.07	0.34	10.00	1.72	36.1
0.01 x 4.64 x 8.06	19.07	0.43 x 7.66	7.28	12.86	1.72	38.4
0.01 x 4.64 x 8.06	22.92		0.59	3.88	1.77	25.8
0.01 x 2.72 x 7.96	9.42	1.02 x 3.66	0.20	5.18	0.95	28.8
0.01 x 1.16 x 3.93	2.35	0.43 x 1.02	0.12	8.19	1.77	10.3
0.01 x 2.72 x 3.93	4.71	0.43 x 2.04	0.23	1.90	0.84	4.3
0.01 x 1.16 x 1.05	0.62	0.43 x 2.04	0.23	1.90	0.84	4.3
0.01 x 1.16 x 3.93	2.35	0.43 x 3.70	0.40	1.29	0.84	8.3



## 1.4 SEISMIC FORCE

ROOF	$0.46 \times 2734 \times 24.07$	303.3
WALL	$0.10 \times 103.8$	10.4
	$0.42 \times 206 \times 10$	86
	$0.52 \times 2173 \times 40.6$	60.3
BEAM	$0.24 \times 449$	15.3
	$0.11 \times 199.0$	101.5
COLUMN	$0.51 \times 224 \times 26$	444
	$0.21 \times 293 \times 6$	55

$$Q_c = 554.3 \times 0.1 = 55.43$$

TERRACE COLUMN	$0.19 \times 1.45 \times 10$	30
	$0.24 \times 92.4$	22.2
		25.0

$$Q_T = 25.0 \times 0.1 = 2.50$$

KITCHEN		
ROOF	$0.46 \times (11.6 \times 21.47 + 466 \times 13.51)$	143.4
WALL	$0.10 \times 62.8$	6.3
	$0.42 \times 222 \times (2 + 23.0)$	44.1
	$0.25 \times 50.6$	43.1
BEAM	$0.21 \times 35.2$	24.7
	$0.57 \times 103.2$	52.6
COLUMN	$0.61 \times 2.5$	3.4
	$0.19 \times 22 \times 21$	8.7
	$0.24 \times 3.2 \times 7$	2.6
		339.5

$$Q_k = 339.5 \times 0.1 = 33.95$$

$$I_Q = 91.88$$



STIFFNESS RATIO

	b x D	J <sub>100</sub>	l	K <sub>100</sub>	φ	ℓ <sub>c</sub>
C	20 x 75	234	520	16.7		1.67
	75 x 20	137	"	2.3		0.23
	20 x 60	502	"	8.5		2.86
	60 x 20	11.0	"	1.9		0.19
	40 φ	12.6	"	2.1		0.21
	40 x 40	213	440	4.0		0.40
	20 x 20	51	"	1.2		0.12
G			290	1.8		0.18
	20 x 50	297	332	12.6	1.27 1.24 1.50	1.16 0.94
			536	5.0	1.89	1.01
			587	4.0	1.58 1.42	0.79 0.46
	20 x 75	245	465	20.4	2.10 1.70	2.35 1.19
			725	11.9	1.54 1.95	1.19 2.72
			815	11.6	1.00 1.97	1.35 2.28
	20 x 90	170	444	36.7	1.63	5.98
			645	24.4	1.88	4.58
	GG	30 x 90	215	332	21.6	
			464	44.8		4.58
			696	30.5		3.05
			796	26.7		2.67
			815	26.1		2.61

K<sub>0</sub> = 10<sup>3</sup>

1.60	1.60	1.60	1.60	1.60	
0.23	0.23	0.23	0.23	0.23	5.90
9.16	9.16	9.16	9.16	9.16	

5.94	5.94	5.94	0.23		
0.21	0.21		1.60	1.60	1.60
4.58	4.58	4.58	0.25	0.12	0.12
			9.16	9.16	9.16

1.60	0.91	2.45	1.42		
0.31	0.31	5.98	0.48	0.48	1.15
4.58	4.58	4.58	3.05		

0.24	2.45	3.40	1.36	3.46	
0.18	0.12	0.12	0.12	0.12	2.12
4.58	4.58	4.58	9.16	4.58	

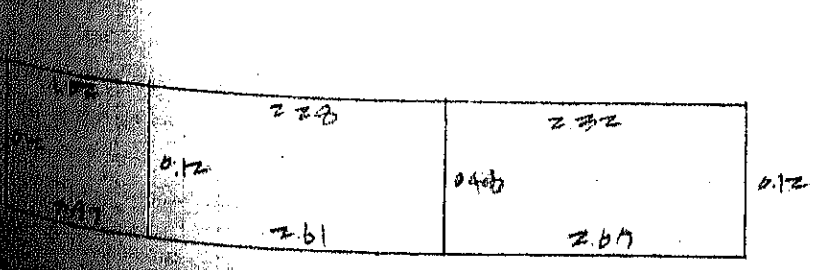
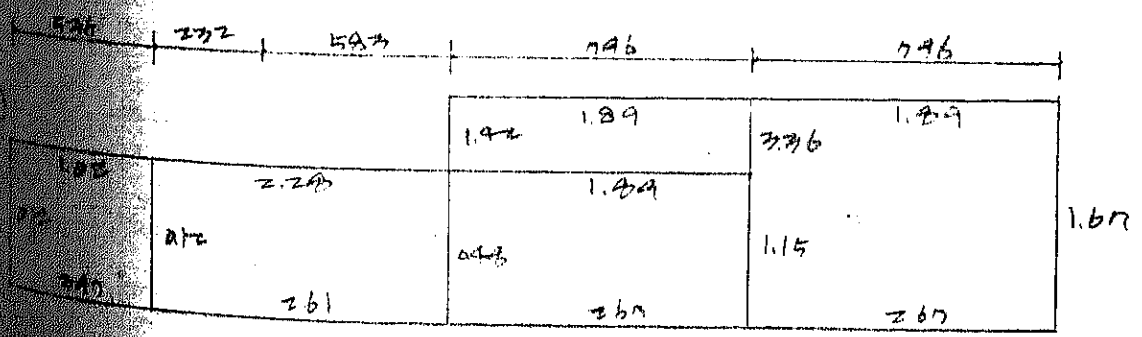
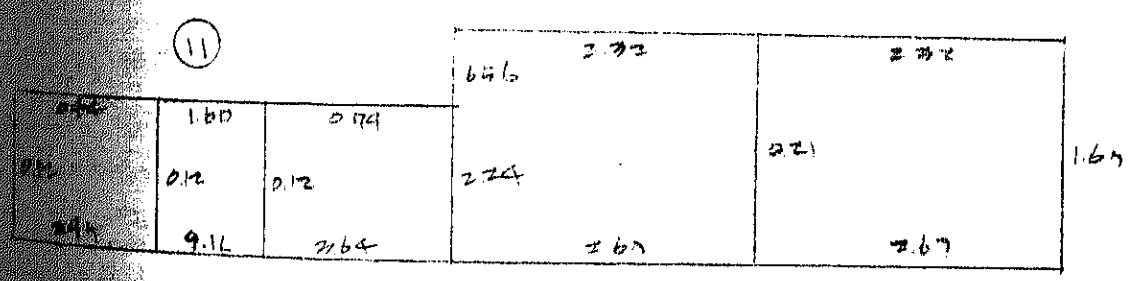
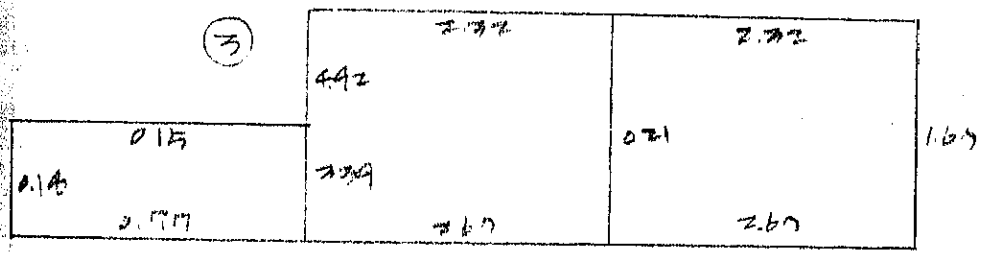
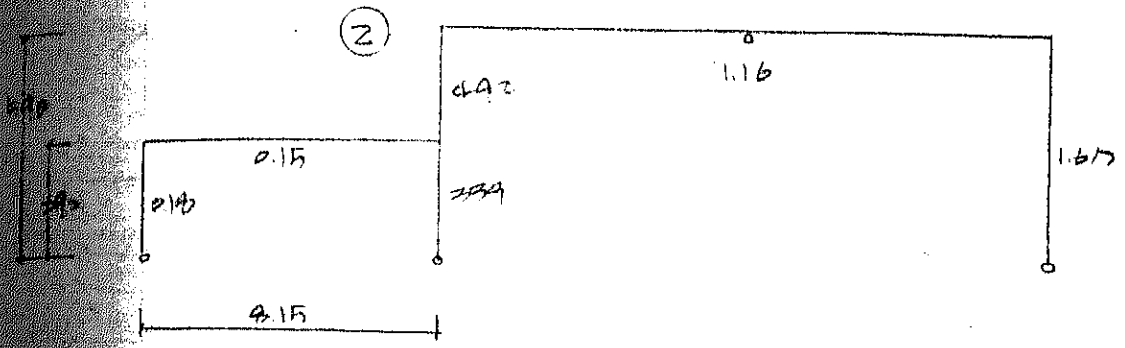
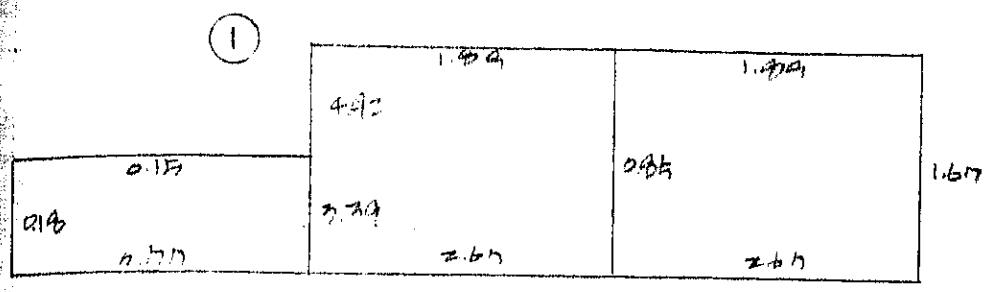
1.60	1.60	1.60		1.60	1.60
0.12	0.12	0.12	4.40	0.12	0.12
9.16	9.16	9.16		9.16	9.16

1.60	1.60	1.60	5.90
0.23	0.23	0.23	
9.16	9.16		

ⓑ

0.53	0.53	0.53	
0.16	0.16	0.16	2.90
2.71	2.71	2.71	

2.71	2.71	1
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4-6 CALCULATION OF STRESS

ⓐ

0.85	1.791	3.157	56.95	10.333
0.19	0.21	0.18	0.30	0.612
1.4	1.5	1.3	1.5	2.6
0.55	0.55	0.55	0.55	0.45 - 0.05

0.35

6.50	2.67	1.33
0.205	0.081	0.06
	0.15	0.12
2.5	1.0	0.9
0.45	0.55	0.55

ⓑ

6.95	1.791	4.01	4.11
0.19	0.21	0.607	0.925
1.4	1.5	3.4	5.6
0.55	0.55	0.45 - 0.05	0.45 - 0.05

0.46

9.19	19.39	22.00	3.98
0.260	0.445	0.450	0.862
3.3	5.5	5.5	10.6
0.60	0.55	0.55	0.55

ⓑ

5.87	5.767	44.33	54.33	20.25
0.255	0.117	0.116	0.116	0.14
3.1	1.4	1.4	1.5	0.20
0.45 - 0.10	0.21	0.21	0.21	1.4
2.94	0.55	0.55	0.55	0.55
1.6	0.55	0.55	0.55	0.55
0.55	0.55	0.55	0.55	0.55

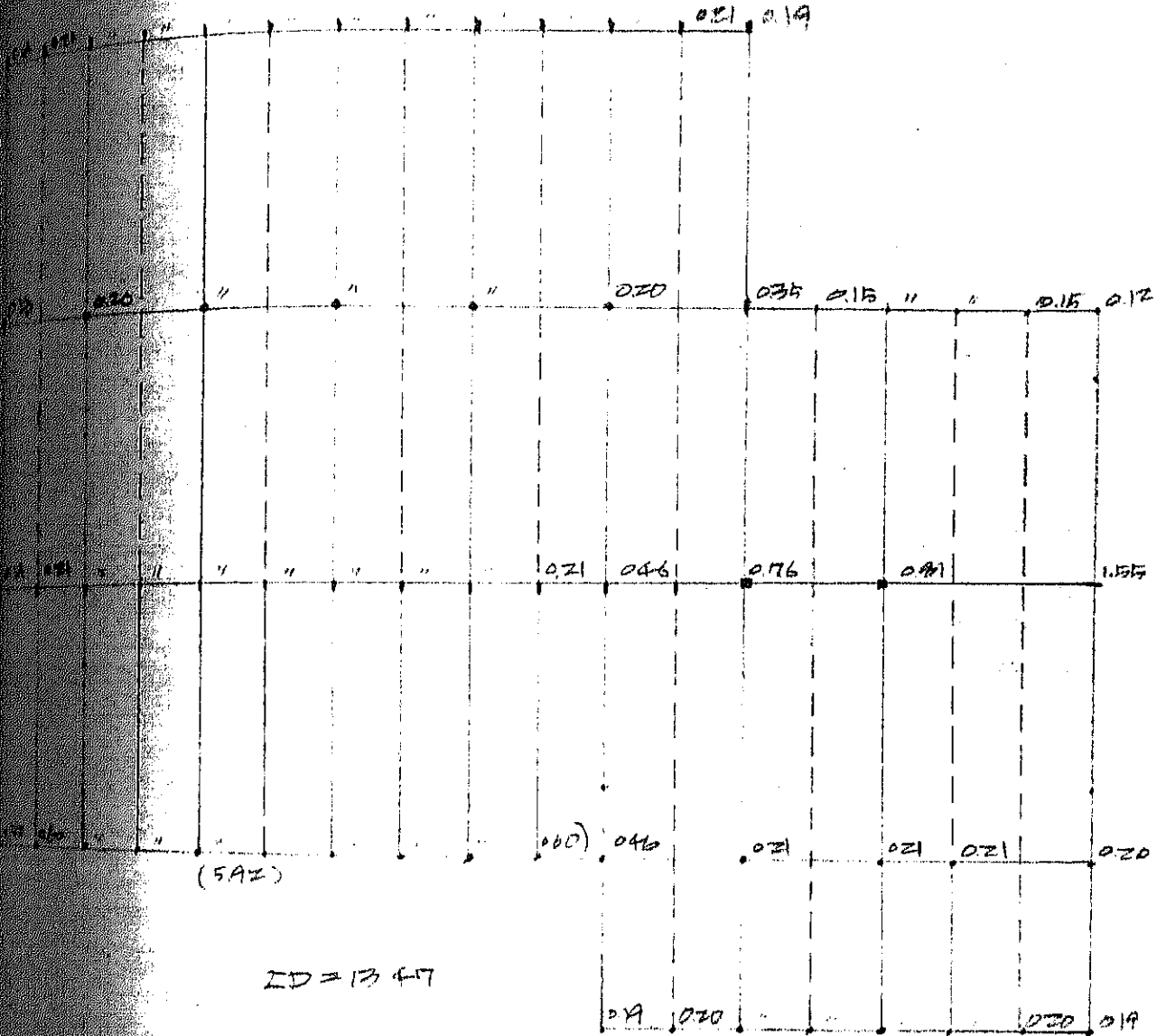
0.255  
↓  
0.46

Ⓐ

1.777	3.657
0.126	0.112
1.3	1.4
0.19	0.20
0.55	0.55

$R_c = 55.43$

$ID_0 = 7.52$



$ID = 13.47$

$ID = 9.10$

①

	0.277 0.661	4.45 0.59	1.13 0.87
	4.1	2.0	3.0
1.21	0.40 -0.40		
0.04	0.044 0.902		
1.15	1.1	0.55	0.55
1.0	0.97		

②

	0.133 0.307	0.69 0.29
	0.9	0.8
0.23	0.22 -0.40	
0.04	0.044 0.069	
0.5	0.16	0.4

③

	0.261 0.449	3.09 0.20	1.39 0.93
	4.6	0.7	3.2
1.15	0.38 -0.40		
0.04	0.044 0.902		
1.15	1.1	0.55	0.55
1.0	0.97		

④

2.50	2.760	9.48	19.33
0.107	0.114	0.4145	0.112
0.19	0.20	0.175	0.20
1.5	1.6	6.0	1.6
0.55	0.55	0.55	0.55

⑤

	0.277 1.195	3.09 0.20	1.39 0.93
	4.4	0.7	3.2
1.28	0.38 -0.40		
0.04	0.044 0.902		
1.15	1.1	0.55	0.55
1.0	0.97		

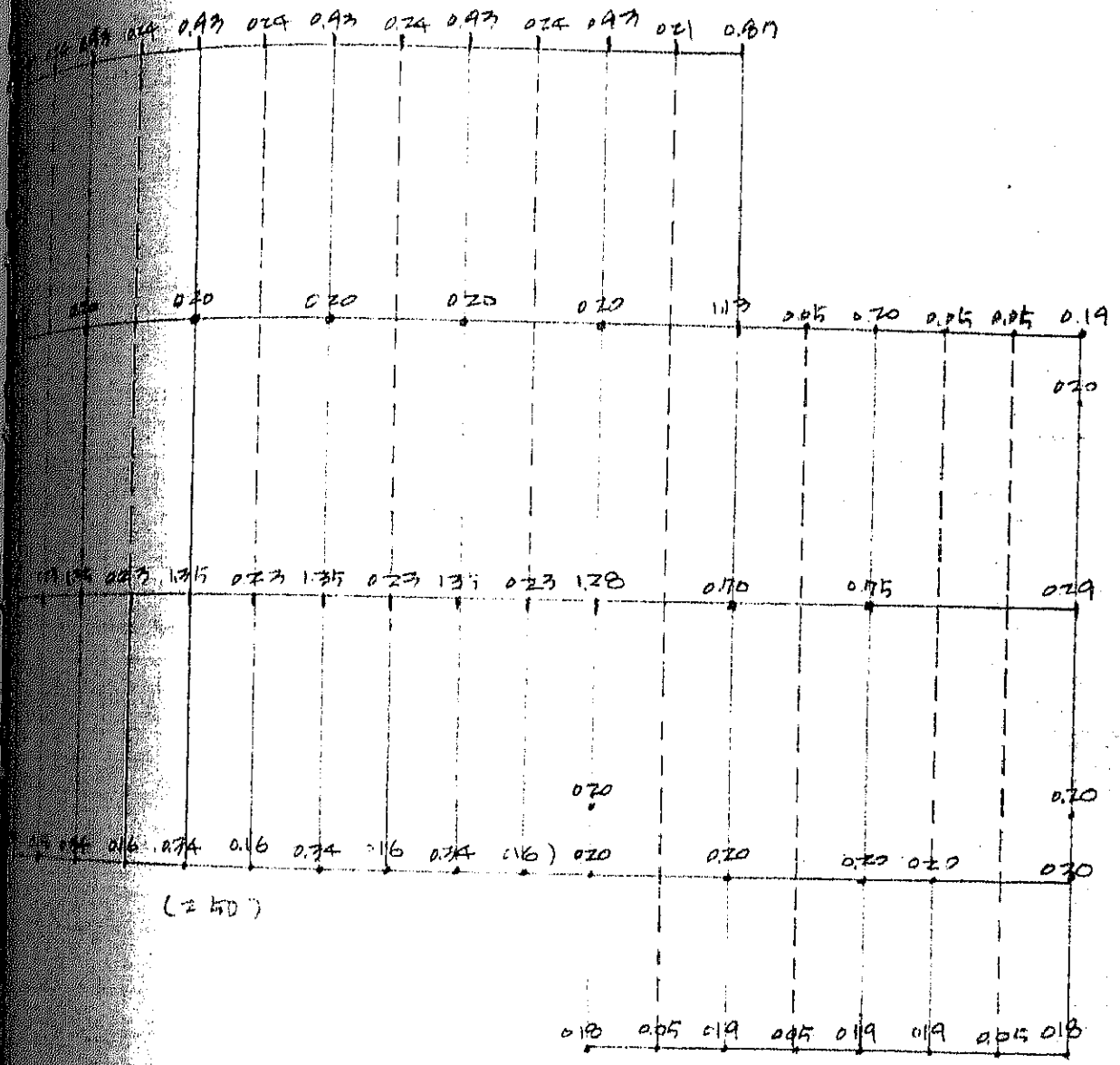
⑥

	2.17 0.122	0.84 0.794	1.13 0.87
	2.4	3.8	3.0
0.70	0.45 0.05	0.40 -0.10	
0.04	0.044 0.902	1.64 0.676	1.22
1.15	1.1	0.7	0.55
1.0	0.97		



$$\Sigma D_2 = 20.26$$

$$\Sigma D_2 = 16.53 + 2.50 = 19.03$$



(2.50)

$$ID = 24.53$$

$$13.50$$

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$$27.02$$

$$\Sigma D_2 = 4.26$$

$$Q_k = 33.95$$

$$\Sigma D = 91.33$$







# PRESS DIAGRAM OF VERTICAL LOAD

0.9 0.4 1/2 0.1	0.1
0.1	0.1
0.1	0.1

0.9 0.4 1/2 0.1	0.1	0.1	0.1
0.1	0.1	0.1	0.1
0.1	0.1	0.1	0.1

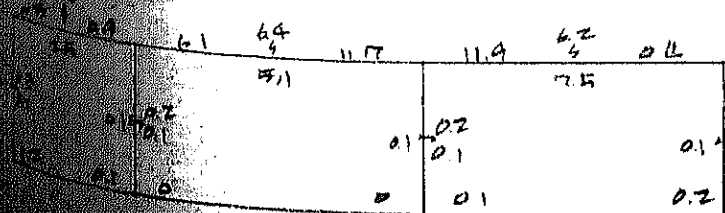
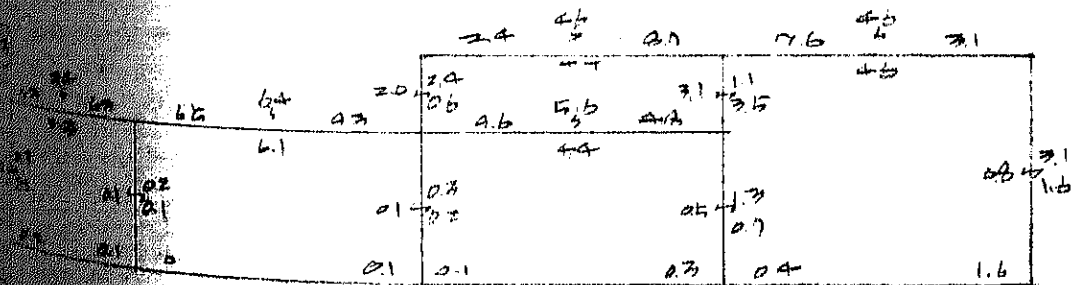
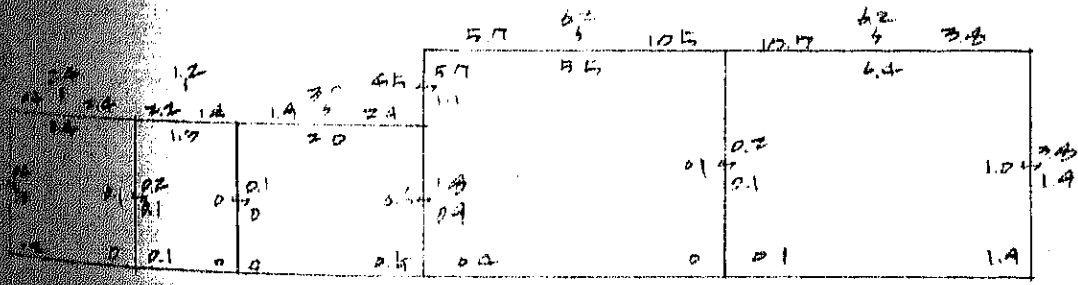
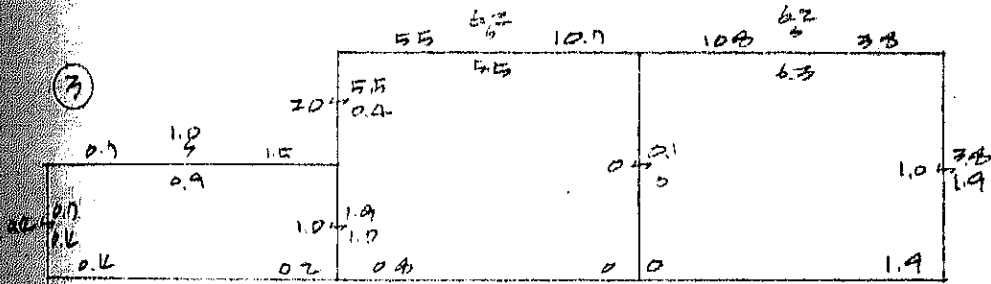
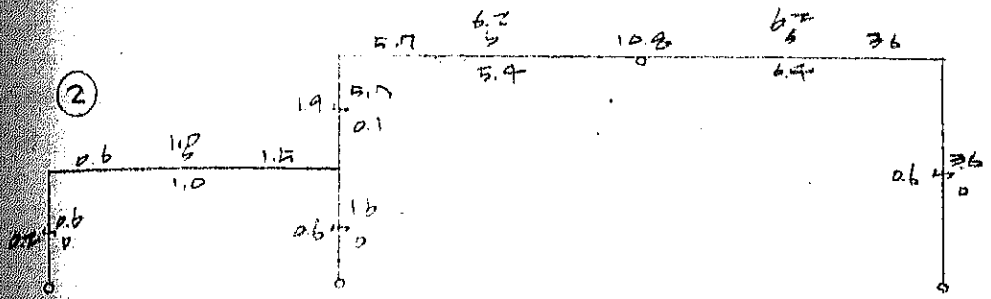
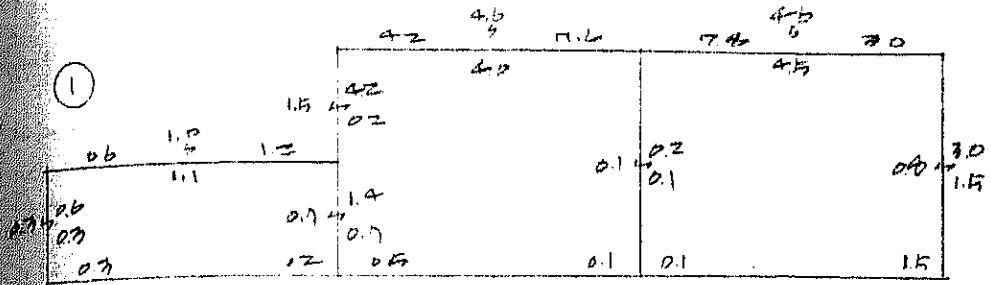
0.9 0.4 1/2 0.1	0.1	0.1	0.1
0.1	0.1	0.1	0.1
0.1	0.1	0.1	0.1

0.9 0.4 1/2 0.1	0.1	0.1	0.1
0.1	0.1	0.1	0.1
0.1	0.1	0.1	0.1

0.9 0.4 1/2 0.1	0.1
0.1	0.1
0.1	0.1

0.9 0.4 1/2 0.1	0.1
0.1	0.1
0.1	0.1

0.9 0.4 1/2 0.1	0.1	0.1
0.1	0.1	0.1
0.1	0.1	0.1



DESIGN OF SECTION

BEAM

SECTION	$P_{OF}$	$M_C$	$M_{FE}$	$Q$	$M_{FE}$	$M_C$	$Q$
L	0	0.3	0.5	0.9	0.5	0.1	0.9
E	2.6	0.7	1.3	1.7	1.3	0	1.1
D	2.6	1.0	1.6	4.3	1.8		2.1

$b \times D = 20 \times 50$        $\beta = 39.3$        $Q_0 = 9.9$   
 $Q$     2.2    0.8    1.5                    1.5    0.1  
 $n$     2-D16   2-D16   2-D16                    2-D16   2-D16

SECTION	$P_{OF}$	$M_C$	$M_{FE}$	$Q$	$P_{OF}$	$M_C$	$M_{FE}$	$Q$
L	0.1	0.1	0.2		1.5	1.0	1.0	
E	3.3	0.8	2.2		1.5	0.6	0.3	
D	3.4	0.9	4.6		3.0	1.6	1.6	

$b \times D = 20 \times 42$        $\beta = 33.3$        $Q_0 = 5.3$   
 $Q$     2.3    0.9                    3.1    1.6  
 $n$     2-D16   2-D16                    2-D16   2-D16

COLUMN

SECTION	$P_{OF}$	$M_C$	$M_{FE}$	$Q$	$P_{OF}$	$M_C$	$M_{FE}$	$Q$	$P_{OF}$	$Q$	
L	0.7	0.6	1.0	7.4	2.0	2.2	1.2	4.3	12.0	0.3	3.4
E	2.0	0.5	1.4	0.7	1.0	0	0.9	1.8	0.5	2.8	1.0
D	2.0							3.0		3.1	

$b \times D = 28 \times 74$                      $28 \times 50$                      $28 \times 74$   
 $\beta = 60.3$        $Q_0 = 10.1$   
 $Q$     2.0    0.1    0.9                    5.1    3.8    3.6    10.0    1.7  
 $n$     2-D19   4-D19                    2-D19   2-D19    2-D19   4-D19   2-D19

$\beta = 1200 / 60.3 \times 12 = 10.2$

	$\pi_{0E}$	$\pi_C$	$\pi_{0E}$	$\pi$	$\pi_E$	$\pi_C$	$\pi$	$\pi_{0E}$	$\pi_C$	$\pi_{0E}$	$\pi$
L	0	0.3	0.5	0.9	0.4	0.1	0.9	0	0.7	2.4	0.9
E	2.0	0.9	3.0	2.5	2.0	0	1.7	2.0	0.5	1.0	1.3
F	3.0	1.2	3.5	5.9	2.4		4.3	2.0	1.2	3.4	3.5

$b \times D = 2.03 \times 5.9$

A	3.1	1.0	2.1		2.0	0.1		1.7	1.0	2.9	
a	2.016	2.016	2.016		2.016	2.016		2.016	2.016	2.016	

	$\pi_{0E}$	$\pi_C$	$\pi_{0E}$	$\pi$
L	4.0	6.5	2.4	4.2
E	1.3	1.2	4.2	1.3
F			6.6	6.3

$b \times D = 2.03 \times 7.4$

$q = 6000 / 60.3 \times 1.8 = 6.7$

A	3.3	4.4	3.1	
a	2.016	2.016	2.016	

	$\pi_{0E}$	$\pi_C$	$\pi_{0E}$	$\pi$	$\pi_{0E}$	$\pi_C$	$\pi_{0E}$	$\pi$	$\pi_{0E}$	$\pi_C$	$\pi_{0E}$	$\pi$
L	2.2	6.3	6.5	6.0	4.4	3.5	2.3	3.9	24.6	21.3	6.0	16.3
E	0.3	2.0	2.9	2.9	1.7	2.0	6.2	3.5	4.7	3.2	21.0	3.7
F	0.0		11.5	11.4	15.1	5.5		15.9			27.0	

$b \times D = 2.0 \times 17.5$

$2.3 \times 9.0 \quad j = 9.4 \quad 2.0 \times 9.0$   
 $1.07 = 12.5$

A	6.1	5.2	6.4		6.0	2.5	15.0		16.6	14.3	12.1	
a	4.016	3.016	4.016		2.016	2.016	4.016		4.016	3.016	4.016	

$\Delta Q = 16.3 - 12.5 = 3.8$

$\Delta Q / j = 20.7$

$PW = 0.41$

$q = 16000 / 60.3 \times 1.2 = 19.6$

$0.13 \pi = 2.55 / (2.0 \times 0.41) = 22 \rightarrow 17 \text{ @}$



13	ROE	TC	TE	Q	TC	TE	Q
L	0.7	11.4	0.5	0.8	11.4	0.6	0.5
E	0.75	0.8	2.0	1.2	2.0	0.4	0.9
S	0.8						

$g = 10/143 \times 12 = 8.4$

$b \times D = 2A \times 50 \quad j = 74.3$

A	1.7	7.7	0.9	7.7	5.8
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A	2-022	2-022	2-022	2-022	2-022
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13	ROE	TC	TE	Q	TC	TE	ROE	Q
L	0.2	0.2	0.2	0.9	0.4	0.3	0.1	0.9
E	0.6	2.0	1.0	2.3	1.0	0.3	1.6	1.1
S	0.6	3.0	1.2	7.5	1.4	0.6	1.7	3.1

$b \times D = 2A \times 50$

A	5.8	2.5	1.2	1.2	0.5	1.4
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A	1-016	2-016	2-016	2-016	2-016	2-016
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13	ROE	TC	TE	Q	TC	TE	Q
L	0.1	0.3	0.4	0.4	0.4	0.1	0.9
E	0.1	0.1	2.0	2.1	2.0	0	1.7
S	0.8	1.2	2.4	5.0	2.4		4.3

$b \times D = 2A \times 50$

A	0.2	1.0	2.0	2.0	0.1
---	-----	-----	-----	-----	-----

A	2-016	2-016	2-016	2-016	2-016
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FLUX	$\Gamma_{OE}$	$\Gamma_C$	$\Gamma_{IE}$	$\Omega$	$\Gamma_{IE}$	$\Gamma_C$	$\Gamma_{OE}$	$\Omega$
L	4.2	4.0	7.6	4.6	7.4	4.5	3.0	4.0
E	12.7	4.4	3.7	1.9	2.7	2.7	3.0	1.3
S	16.5	4.4		2.4		7.2	11.0	7.2

$b \times b = 2.4 \times 7.4$

A	6.4	4.4	6.3	6.5	4.0	6.1
M	4-D1A	2-D1A	4-D1A	4-D1A	2-D1A	4-D1A

FLUX	$\Gamma_{OE}$	$\Gamma_C$	$\Gamma_{IE}$	$\Omega$	$\Gamma_{IE}$	$\Gamma_C$	$\Gamma_{OE}$	$\Omega$
L	5.7	5.4	10.4	6.2	10.3	6.4	3.6	6.2
E	2.7	1.4	0	0.2	0	2.4	4.7	0.6
S								4.3

$b \times b = 2.4 \times 7.4$

$g = 6.200 / 60.7 \times 12 = 2.6$

A	4.7	4.4	4.2	4.4	5.3	4.6
M	2-D1A	2-D1A	4-D1A	4-D1A	2-D1A	2-D1A

FLUX	$\Gamma_{OE}$	$\Gamma_C$	$\Gamma_{IE}$	$\Omega$	$\Gamma_{IE}$	$\Gamma_C$	$\Gamma_{OE}$	$\Omega$
L	5.5	5.4	10.7	6.2	10.6	6.3	2.8	6.2
E	17.1	6.6	1.0	1.9	1.0	3.3	3.5	1.2
S	19.6	12.1		10.0		10.1	12.3	

$b \times b = 2.4 \times 7.4$

A	10.8	6.7	4.9	9.0	5.5	6.8
M	4-D1A	2-D1A	4-D1A	4-D1A	2-D1A	4-D1A

FLUX	$\Gamma_{OE}$	$\Gamma_C$	$\Gamma_{IE}$	$\Omega$	$\Gamma_{IE}$	$\Gamma_C$	$\Gamma_{OE}$	$\Omega$
L	5.7	5.5	10.5	6.2	10.7	6.4	3.3	6.2
E	6.7	2.4	1.0	1.0	1.0	3.8	3.5	1.2
S	12.4	4.4				10.2	12.3	
A	6.4	4.6	4.7	2.9	5.6	6.8		
M	4-D1A	2-D1A	4-D1A	4-D1A	2-D1A	4-D1A		

CODE	MOE	MC	ME	Q	FE	FC	MOE	Q
L	2.4	4.4	8.0	4.6	5.3	4.3	3.1	4.6
B	1.4	0.1	2.0	0.5	2.0	3.0	6.0	1.3
P	4.2					3.6	11.1	

$b \times D = 2.0 \times 7.5$

A	2.3	3.6	9.2		6.3	4.2	6.1	
M	2-D19	2-D19	4-D19		4-D19	2-D19	4-D19	

CODE	MOE	MC	ME	Q	FE	FC	MOE	Q			
L	0.4	2.4	2.4	3.4	3.3	1.3	1.2	1.4	2.0	2.4	3.0
B	2.4	0.4	1.1	0.7	2.1	0	1.3	1.1	6.9	14.4	2.0
P	3.2			3.8	4.3		4.3	3.0	8.9	17.3	8.4

$b \times D = 2.6 \times 5.0$

A	2.7	3.1	3.1		3.6	1.7		2.5	7.5	14.7	
M	2-D16	2-D16	2-D16		2-D16	2-D16		2-D16	4-D16	4-D22	

CODE	MOE	MC	ME	Q	FE	FC	MOE	Q				
L	0.7	7.7	6.3	3.2	6.5	6.1	4.7	6.4	9.6	4.4	4.3	11.6
B	3.0	1.0	1.0	0.7	2.2	3.1	4.3	1.3	6.5	6.3	20.1	3.4
P	2.7				9.2	17.6		16.1	11.2	24.9	12.4	

$b \times D = 2.8 \times 5.0$

$2.2 \times 7.2$

$2.4 \times 7.4$

A	1.6	2.4	8.0		5.4	5.1	9.7		3.4	6.2	13.8	
M	2-D19	2-D19	4-D19		2-D19	2-D19	4-D19		4-D19	3-D19	4-D22	

CODE	MOE	MC	ME	Q	FE	FC	MOE	Q				
L	0.4	2.5	5.4	3.4	6.1	5.1	11.7	6.4	11.9	7.5	0.4	6.2
B	3.0	1.0	1.0	0.7	2.2	1.4	5.9	1.0	6.0	1.2	3.2	1.2
P	3.0						17.6		17.9		3.6	
A	1.6	2.1	7.5		5.1	4.2	9.7		9.9	6.2	2.0	
M	2-D19	2-D19	4-D19		2-D19	2-D19	4-D19		4-D19	3-D19	2-D19	