

**7. CORRESPONDENCE REGARDING
THE DEMOLITION OF
TEC'S OFFICE**

(1) FROM EU FIJI OFFICE TO TUVALU



EUROPEAN UNION
DELEGATION OF THE EUROPEAN COMMISSION FOR THE PACIFIC
Head of Delegation

TRANSMISSION BY TELEFAX (688.20.210)

005

05 JAN 2005

Mr Seve Paeniu
Permanent Secretary and
National Authorising Officer
Ministry of Finance & Economic Planning
Private Mail Bag
Funafuti
TUVALU

Dear Mr Paeniu,

Subject: Request to demolish the EU funded Electricity Corporation
Administration Building

We have been informed by Ms Pasemeta, that it is cheaper to demolish the above building and build a new one, rather than to refurbish it. We understand that JICA is assisting the government of Tuvalu in this matter and would need our permission in writing, to proceed.

Infrastructure built with EDF funding becomes the property of the beneficiary country, at the end of this project. Thus the building in question is the property of the government of Tuvalu and therefore it is up to the government to decide on this matter and our permission is not necessary.

We are nevertheless surprised that a building that is about 8 years old costs more to renovate than to demolish and build anew. We would like to have further information on this issue, in particular we would like to know what has been done in terms of basic maintenance since the building was built.

Yours sincerely,



MARIA J. RALHA
Head of Operations

cc: Mr Araki, JICA Fiji Office (fax:3302.452)
Mr James Conway, (fax:688.20.200)
Ms Pasemeta Talaapa

Attention : Mr Laasai

Fax # : 688 20207

(2) FROM TUVALU TO EU FIJI OFFICE



GOVERNMENT OF TUVALU
MINISTRY OF WORKS AND ENERGY

G.P.O. VAIAKU, FUNAFUTI, TUVALU

Tel: (688) 20060 / 20055 Fax: (688) 20207 Email: minwe@tuvalu.tv

Date: 21 January 2005

ref. WE : /

Ms Maria J. RALHA
Head of Operations
Delegation of the EU Commission for the Pacific
4th Floor Development Bank Centre
SUVA, Republic of Fiji

Dear Ms Ralha,

Subj/Re: Demolition of the Tuvalu Electricity Corporation Admin Bldg

Thank you for sending me a copy of your letter to Mr. Paeniu regarding the above matter.

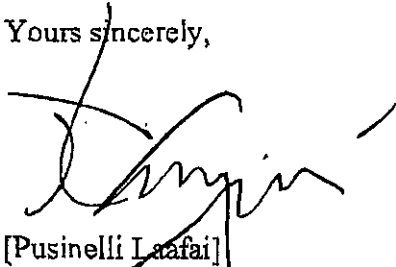
We can understand your reservations about the matter and feel obliged to offer the following reasons to really clarify the situation here:

1. The Cabinet has approved plans to construct a new power plant in the area of the existing powerhouse under grant aid from the Government of Japan.
2. The existing area reserved for the TEC is too small to construct the new powerhouse without relocating the existing fuel oil tanks, if the existing TEC office is left as it is.
3. The existing powerhouse will need to be running continuously while the project is on, therefore it is impossible to relocate the fuel oil tanks without causing inconvenience and great disruptions to normal services.
4. Also if the existing office is not removed, it will create great difficulties in implementing the project, as heavy equipment and machineries will need wide open access to the area behind the office for the construction of the new powerhouse.
5. The Cabinet has ruled out relocating the new powerhouse to a different site, considering the scarcity of land and the insurmountable difficulties associated with land tenure in Tuvalu (private landowners are being drilled in human right laws and have little qualms about threatening all sorts of eviction possibilities to people leasing lands).
6. We believe that it is more cost effective and environmentally friendly in the long term to rebuild the office than relocating the new powerhouse.

Given the above considerations I believe that you are now in a better position to appreciate our dilemma in this vital development.

Also, given the capacity and quality of our existing powerhouse, we cannot afford to delay this project by going through tedious negotiations with landowners, nor can we afford readily the costs of preparing a new site for the project.

Yours sincerely,



[Pusinelli Laafai]

Acting Secretary for Works and Energy

cc: .Mr. Hayashi Hiriyuki, JICA Tokyo
Mr. Araki, JICA Fiji
Mr. Mitsuhsa Nishikawa, Yachiyo Engineering Co. Ltd, Tokyo

(3) FROM JICA TO EU FIJI OFFICE



Ms. Maria J. RALHA
Head of Operations
Delegation of the EU Commission for the Pacific
4th Floor Development Bank Center
Suva, Republic of Fiji

Dear Ms. Ralha,

Re: Submission of Draft Basic Design Report on the Project for Upgrading of Electric Power Supply in Funafuti Atoll, TUVALU

Through the communications with Ms. Pasemeta Talaapa, Mr. Seve Paeniu and Mr. Pusinelli Laafai, you were informed that the Government of Tuvalu requested the Government of Japan to provide a grant aid for the titled project. Upon the request from the Government of Tuvalu, We, Japan International Cooperation Agency (JICA) are conducting a Basic Design Study to evaluate the necessity and urgency of the request and to formulate the scope and components of the Project. Now we are going to complete the study and prepare a draft basic design report.

It is our great pleasure to submit you a copy of the draft report as attached. we will briefly explain the summary of the report and inform you of an important issue which we request your kind understanding.

The main components of the Project are as follows;

1. Construction of a new power house where new diesel engine generators to be provided under the Project are installed
2. Procurement and installation of three (3) sets of diesel engine generators with output capacity of 600 kW each and their mechanical and electrical auxiliaries.
3. Upgrading of 11kV distribution cables and substation equipment such as Ring Main Units and Distribution Transformers including procurement of equipment and materials and installation work

Through the Basic Design Study, we recognized that the request from the Government of Tuvalu has high necessity and urgency if current power supply shortage and unreliable distribution network of Tuvalu are taken into consideration. We also recognized that the Government of Tuvalu is facing a difficulty to secure enough land to construct the new power house under limited land availability in Tuvalu. The Cabinet of Tuvalu ruled out relocating the new power house to a different site, considering the scarcity of land and the insurmountable difficulties associated with land tenure in Tuvalu. Since land acquisition for the new power house is difficult as described above, there is no alternative other than utilizing the existing power station area.

Through the field survey, it turned out that it is possible to construct the new power house inside the existing power station area if either the existing fuel tanks or TEC office is removed. We concluded that we had better to remove TEC office rather than the existing fuel oil tanks because of the following reasons:

1. The existing generating units will need to be running continuously while the Project is implemented. Therefore, the existing fuel oil tanks which supply fuel to the existing generating units are indispensable for continuous power supply and it is impossible to remove them. Relocation of the fuel tanks is also impossible, considering the land availability and power supply disruption during the relocation work.

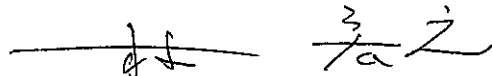
2. If the existing office is not removed, it will create great difficulties in implementing the Project, as heavy construction equipment and machines will need wide open access to the area behind the office for the construction of the new power house.

We respects EU's assistance to the construction of TEC office building which has provided convenient and comfortable working environment to TEC officials for the past nine years and we believe that the assistance contributed to improve power supply service in Tuvalu. However, power supply shortage in Tuvalu is getting worse and worse, a land for new power plants is much more important for the people of Tuvalu. Through discussions with officials of Tuvalu and the field survey, we concluded that demolition of the TEC office is inevitable.

We understand that reliable power supply system is a crucial infrastructure for economic development and improvement of living standards in developing countries.

Your understanding and consideration to the Project would be highly appreciated.

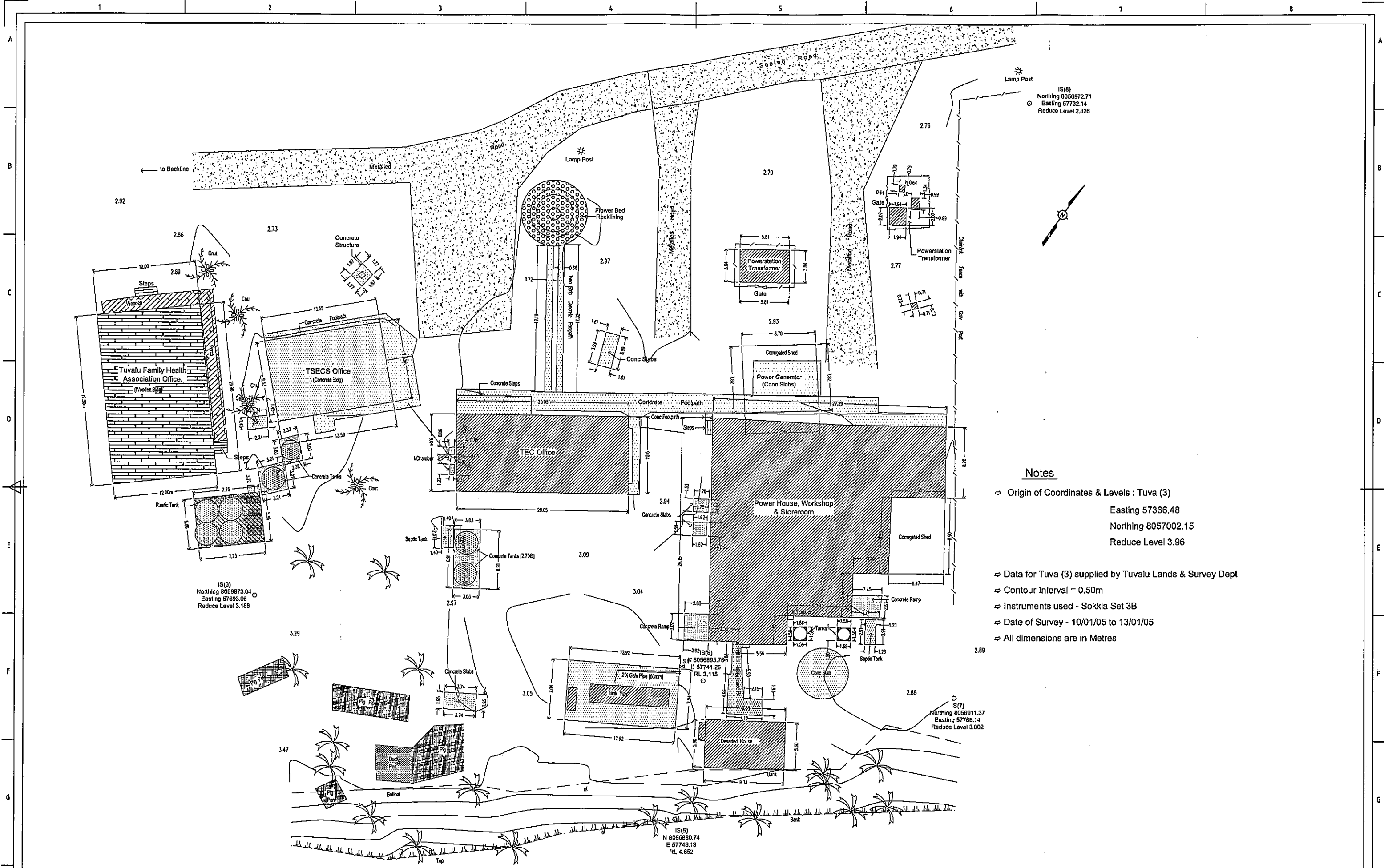
Yours sincerely,



Hayashi Hiroyuki
Traffic Infrastructure Team
Project Management Group II
Japan International Cooperation Agency

8. TOPOGRAPHIC SURVEY AND SOIL EXPLORATIONS

(1) TOPOGRAPHIC SURVEY



Notes

- ⇒ Origin of Coordinates & Levels : Tuva (3)
 Easting 57366.48
 Northing 8057002.15
 Reduce Level 3.96
- ⇒ Data for Tuva (3) supplied by Tuvalu Lands & Survey Dept
- ⇒ Contour Interval = 0.50m
- ⇒ Instruments used - Sokkia Set 3B
- ⇒ Date of Survey - 10/01/05 to 13/01/05
- ⇒ All dimensions are in Metres

WOOD & JEPSEN CONSULTANTS CIVIL ENGINEERS SURVEYORS PLANNERS PO Box 1248 SUVA Ph: (079) 3311077 10 HOLLAND ST. SUVA Fax: (079) 3363381 FJI E-MAIL: wood@wjc.com.fj										Client Engineered Designs		Project Survey of Funafuti Power Station Funafuti, Tuvalu		Surveyed by TW		Field by 458		Scale 1:200		Computer # TW	
Drawing Title Dimensions for Existing Structures										Checked by RGJ		Approved by - date 22/01/2005		Drawing No. 4744		Edition #		Sheet 1 OF 1			
										Rev No	Revision note	Date	Signature	Checked	Issued	No.	Name	Date	1	2	3

REPORT

YACHIYO ENGINEERING CO. LTD

**Proposed Funafuti Power Station
Re-development
Geotechnical Investigation**

Report prepared for:
YACHIYO ENGINEERING CO. LTD

Report prepared by:
TONKIN & TAYLOR INTERNATIONAL LTD

Distribution:	
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ENGINEERED DESIGNS	1 copy

February 2005

Job no: 750468

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Appendix A: Drawing No. 1: Trial Pit Locations,
Drawing No. 2a: Dynamic Cone Penetrometer test locations, Set B1
Drawing No. 2b: Dynamic Cone Penetrometer test Locations, Set B2

Appendix B: Site Photos
Appendix C: Laboratory Test Results

1 Introduction

This report presents the results of the geotechnical investigation for the proposed extensions of the existing power station, at Funafuti in the Tuvalu islands.

Tonkin & Taylor International (T&TI) in association with Engineered Designs (E-D) were engaged by Yachiyo Engineering Co. Ltd to carry out a geotechnical investigation of the site for the proposed extensions.

2 Objectives and scope

The objective of the investigation was to assess the subsurface conditions and determine allowable bearing capacities for foundation design, based upon field-testing and laboratory test results.

The scope of the field investigation undertaken by E-D comprised dynamic cone penetrometer tests (Scala), trial pits excavated by digger, and recovery of bulk samples.

The laboratory testing included determinations of water content, particle size analyses and solid densities.

The interpretation of the investigation results, assessment of subsurface conditions and determination of foundation design parameters has been undertaken by T&TI.

3 Field Investigation

The technicians from E-D carried out the geotechnical investigation of the site in January 2005, liaising with T&TI to ensure quality assurance during the investigations.

The first stage of work, involved 33 dynamic cone penetrometer tests carried out at the locations indicated on drawing no. 2a and 2b in Appendix A. Twenty-three tests were carried out in the area marked "Set B1" (drawing no. 2a), and 10 in the area marked "Set B2" (drawing no. 2b). All tests were carried out to "refusal", ranging between depths of 100 - 600 mm.

Following consultations with T&TI, E-D undertook a second stage of work which comprised putting down 3 trial pits excavated by a 12-tonne digger at the locations indicated on drawing no. 1, in Appendix A. All of the trial pits reached "refusal" at shallow depths, and the ground was too hard to reach the target depth of 5 m. Trial pits B1, B2 and B2A reached "refusal" at 0.20 m, 0.25 m, and 0.60 m depth respectively.

Site photos taken by E-D are contained within Appendix B.

4 Laboratory testing

Laboratory testing to determine the water content of the samples retrieved from trial pits B1 and B2 was carried out by E-D. T&TI have carried out determinations of solid density and particle size analyses on bulk samples retrieved from all three trial pits. Table 1 presents a summary of the water content results. Full test results are available in Appendix C.

Table 1: Water content results

TEST LOCATION	Depth	TEST NUMBER			AVERAGE
		1	2	3	
B1	100 mm	20.92 %	26.24 %	25.84 %	24.34 %
B2	100 mm	20.64 %	20.44 %	21.36 %	20.81 %
B2	200 mm	22.15 %	21.68 %	17.77 %	20.53 %

Table 2: Solid Density results

TEST LOCATION	Sample No.	Depth (m)	Solid Density (t/m ³)
B1	1	0.1	2.62
B2	1	0.1	2.48
B2	2	0.2	2.47
B2A	1	0.2	2.62

5 Site Conditions

5.1 Geology of Tuvalu

The islands of Tuvalu are atolls with low lying coral sand covering modern reef, built upon older volcanic seamounts¹. The islands are geologically young, having been formed during the last 3000 years¹

5.2 Subsurface conditions

The results of the field investigation show that the two main units underlie the site:

- Weathered residual soil
- Coral rock formation

A summary of the respective characteristics of these units is as follows:

¹ SOPAC Preliminary Report 38 (1991). Mapping survey and baseline study of coastal erosion on the islands of Tuvalu.

(2) SOIL EXPLORATIONS

5.2.1 Weathered residual soil

The investigations identified between 100 and 200 mm of weathered residual soil overlying the coral rock formation, comprising very fine-grained black silty sand, with occasional organics, including rootlets, wood, leaves and coral fragments greater than 100mm in diameter.

5.2.2 Coral rock formation

Cemented limestone coral rock formation was recorded at depths of 0.1, 0.2, and 0.2 m in trial pits B1, B2, and B2A respectively. The coral is vesicular, and visual inspections of the surface of the coral estimate the voids to be minor and in the order of 5%.

6 Geotechnical recommendations

6.1 Introduction

The opinions and recommendations presented in this section are based upon subsurface information obtained from the dynamic cone penetrometer tests and the 3 trial pit excavations, as well as our existing geotechnical database. The nature and continuity of the subsoil away from the tests locations is inferred, so it must be appreciated that actual conditions may vary from the assumed model.

6.2 Foundation Options

Shallow footings are considered suitable for the building development, and a geotechnical ultimate bearing capacity of 1MPa can be assumed for footings founded on the coral rock. This corresponds to an Ultimate Limit State (FOS=2) and Working Load bearing capacity (FOS=3) values of 500 and 300 kPa respectively, in accordance with NZS.4203:1992².

The above geotechnical ultimate capacities have been assessed using previous laboratory test results, and our knowledge of these materials.

Coral rock formations are susceptible to solution cavities and it will be essential to carry out proof drilling of the rock during construction to confirm the integrity of the foundation materials. We recommend a minimum depth of drilling of "2B" (B=footing width) be used to determine the minimum drilling depth.

² NZS.4203:1992, Code of practice for general structural design and design loadings for buildings.

7 Applicability

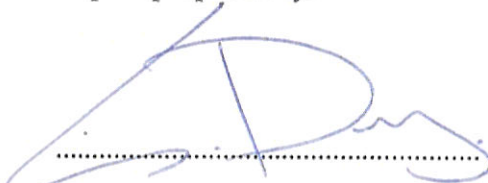
This report has been prepared for the benefit of Yachiyo Engineering Co. Ltd with respect to the particular brief given to us and it may not be relied upon in other contexts or for any other purpose without our prior review and agreement.

TONKIN & TAYLOR INTERNATIONAL LTD

Environmental and Engineering Consultants

Report prepared by:

Authorised for Tonkin & Taylor by:



Steven Drury



Chris Freer

PROJECT CO-ORDINATOR

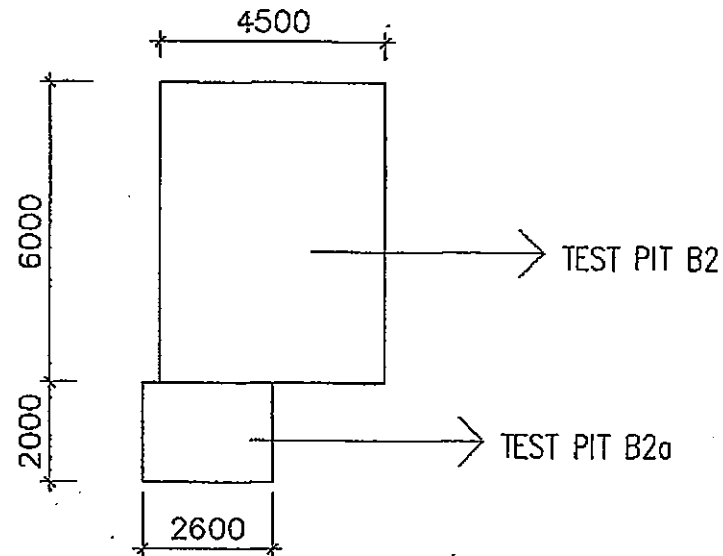
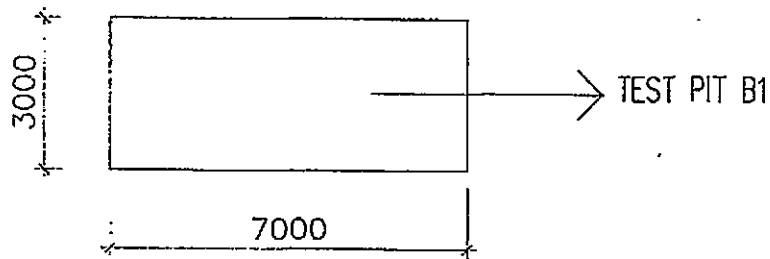
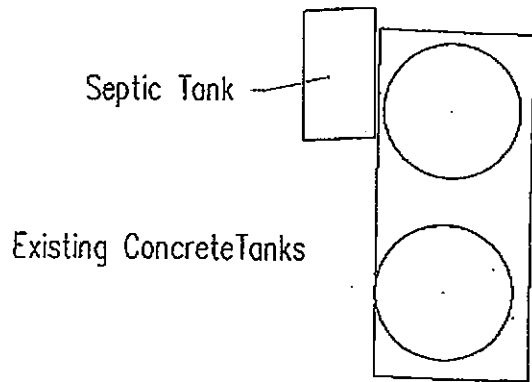
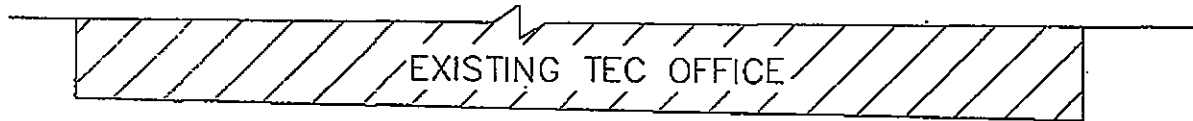
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Appendix A: Site Plans and Test Locations

**Test Pit Logs and Dynamic Cone
Penetrometer Results**

TEST PIT LOGS



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		FUNAFUTI POWER PROJECT		TEST PIT LOCATION		SVC	HK	170/05	JAN 05	GT
						Scale	Drawing No.		Revision	
						1:150	1		<input type="checkbox"/> <input type="checkbox"/> <input type="checkbox"/> <input type="checkbox"/>	

PROJECT: Funafuti Power Project. **LOCATION:** Funafuti, Tuvalu. **JOB NUMBER:** 170/04
EXPOSURE TYPE: Trial Pit **HOLE STARTED:** 13/01/2005 **LOGGED BY:** SVC
EQUIPMENT: 12 Tonne Digger **HOLE FINISHED:** 13/01/2005 **CHECKED BY:** V.K
Dimension: 7.0 x 3.0(M)

GEOLOGICAL			ENGINEERING DESCRIPTION						
GEOLOGICAL UNIT, GENERIC NAME, ORIGIN, MINERAL COMPOSITION	WATER	TEST	R.L. (m) DEPTH (m)	GRAPHICAL LOG	CLASSIFICATION SYMBOL			DEFECT SPACINGS (mm)	SOIL DESCRIPTION MAIN: minor components, strength, colour, structure, weathering. ROCK DESCRIPTION SUBSTANCE: Rock Type, Particle size, Colour, minor components DEFECTS: Type, Inclination, thickness, roughness, filling
					SM	M	L		
WEATHERED RESIDUAL SOIL?									SILTY SAND, Very fine grained, black with occasional organic fragments; rootlets, wood, leaves.
CORAL ROCK FORMATION									Coral rock (formation), cemented, creamy white, Vesicular (<5% Surface) with some shell fossils)
			1.00						
			2.00						
			3.00						
			4.00						
			6.00						
END OF TEST PIT at 0.20m. Unable to penetrate further. (Coral Rock Formation)									

PROJECT: Funafuti Power Project. LOCATION: Funafuti, Tuvalu. JOB NUMBER: 170/04
 EXPOSURE TYPE: Trial Pit HOLE STARTED: 13/01/2005 LOGGED BY: SVC
 EQUIPMENT: 12 Tonne Digger HOLE FINISHED: 13/01/2005 CHECKED BY: V.K
 Dimension: 6.0 x 4.5(M)

GEOLOGICAL				ENGINEERING DESCRIPTION						
GEOLOGICAL UNIT, GENERIC NAME, ORIGIN, MINERAL COMPOSITION	WATER	TEST	R.L. (m) DEPTH (m)	GRAPHICAL LOG CLASSIFICATION SYMBOL	MOISTURE/WEATHERING	STRENGTH/ DENSITY	DEFECT SPACING (mm)	SOIL DESCRIPTION MAIN: minor components, strength, colour, structure, weathering. ROCK DESCRIPTION SUBSTANCE: Rock Type, Particle size, Colour, minor components DEFECTS: Type, inclination, thickness, roughness, filling		
WEATHERED RESIDUAL SOIL?								SILTY SAND, Very fine grained, black with occasional rootlets and coral fragments < 50mm dia, sub angular to angular, hard.		
CORAL ROCK FORMATION			1.00					200- grades brownish with coral fragments > 100mm diameter, subangular to angular, hard.		
			2.00					250- Coral rock (formation), cemented, creamy white, Vesicular (< 5% Surface) with some shell fossils. A dark orange stain was noted on few of the fragments.		
			3.00					END OF TEST PIT at 0.25m. Unable to penetrate further. (Coral Rock Formation)		
			4.00							
			5.00							
			6.00							



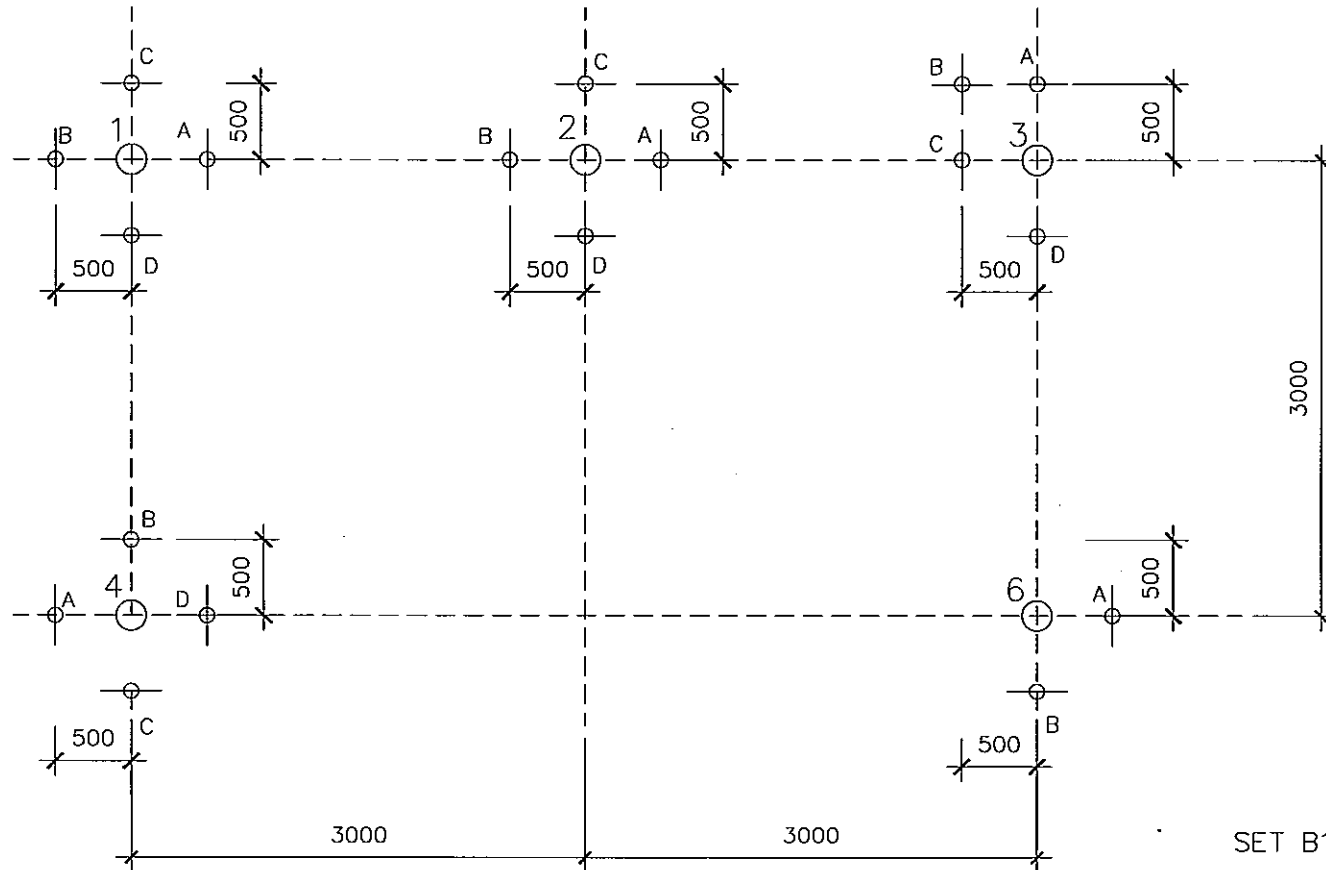
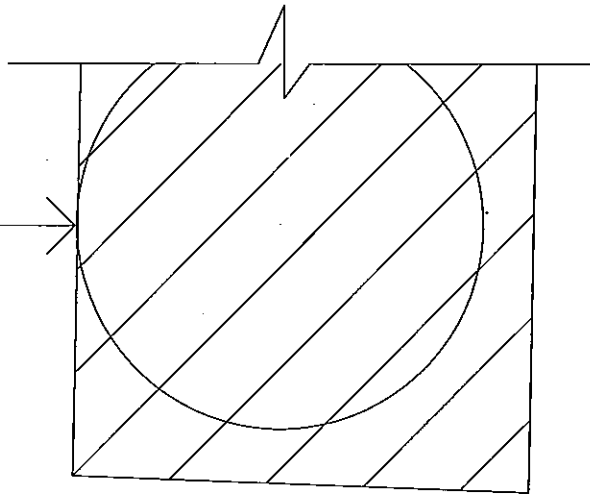
TEST PIT NUMBER: B2A

PROJECT: Funafuti Power Project. LOCATION: Funafuti, Tuvalu. JOB NUMBER 170/04
 EXPOSURE TYPE: Trial Pit HOLE STARTED: 13/01/2005 LOGGED BY: SVC
 EQUIPMENT: 12 Tonne Digger HOLE FINISHED: 13/01/2005 CHECKED BY: V.K
 Dimension: 2.60 x 2.0(M)

GEOLOGICAL			ENGINEERING DESCRIPTION						
GEOLOGICAL UNIT, GENERIC NAME, ORIGIN, MINERAL COMPOSITION	TEST	WATER	R.L. (m) DEPTH (m)	GRAPHICAL LOG	CLASSIFICATION SYMBOL	MOISTURE/WEATHERING	STRENGTH/ DENSITY	DEFECT SPACINGS (mm)	SOIL DESCRIPTION MAIN: minor components, strength, colour, structure, weathering. ROCK DESCRIPTION SUBSTANCE: Rock Type, Particle size, Colour, minor components DEFECTS: Type, Inclination, thickness, roughness, filling
WEATHERED RESIDUAL SOIL?									SILTY SAND, Very fine grained, black with occasional rooflets and coral fragments < 50mm dia, sub angular to angular, hard.
CORAL ROCK FORMATION									100- grades grey with coral fragments > 100mm diameter, subangular to angular, hard.
			1.00						600- Coral rock (formation), cemented, creamy white, Vesicular (< 5% Surface) with some shell fossils.
			2.00						
			3.00						
			4.00						
			5.00						END OF TEST PIT at 0.60m. Unable to penetrate further. (Coral Rock Formation)

**DYNAMIC CONE
PENETROMETER
RESULTS FOR
SET B1**

Existing Concrete Tanks



Engineered Designs

Consulting Civil & Structural Engineers
 Suite A10-A12 Grantham Plaza
 Grantham Road, Suva, Fiji.
 PO Box 117, Suva
 Tel : 338 3788
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Project Title
FUNAFUTI POWER PROJECT

Drawing Title
DYNAMIC CONE PENETROMETER TEST LOCATION

Scale
 1:50

Test By
 SVC

Drawn
 HK

Job No
 170/05

Date
 JAN 05

Division
 GT

Drawing No
 2a

Revision

SET B1



Suite A10 - A12 Grantham Plaza
 PO Box 117 Suva Fiji
 Telephone: [878] 3383788
 Fax: [878] 3370354
 Email: edesigns@connect.com.fj

Site: Funafuti, Tuvalu.

Page 1

Job Name: Funafuti Power Project.

Job No: 170/05

TEST METHOD USED: NZS 4402 : 1988 Test 6.5.2 Dynamic Cone Penetrometer

Depth from ground surface to commencement of penetration: 0.00m

SOIL DESCRIPTION

MAIN \ minor components, strength, colour
 structure, weathering.

REFER TO TEST PIT LOGS

Vertical Distance driven (mm)	Number of blows	DYNAMIC CONE PENETROMETER -- SCALA	REMARKS
TEST 1			
50	1		
100	3		
150	20		
200	>30		
250			
300			
350			
400			
450			
500			

Vertical Distance driven (mm)	Number of blows	DYNAMIC CONE PENETROMETER -- SCALA	REMARKS
TEST 1A			
50	4		
100	>30		
150			
200			
250			
300			
350			
400			
450			
500			

Vertical Distance driven (mm)	Number of blows	DYNAMIC CONE PENETROMETER -- SCALA	REMARKS
TEST 1B			
50	4		
100	>30		
150			
200			
250			
300			
350			
400			
450			
500			

TESTED BY: SVC

DATE: 11/01/05

AUTHORISED BY: VK



Suite A10 - A12 Grantham Plaza
 PO Box 117 Suva Fiji
 Telephone:[679] 3383788
 Fax:[679] 3370854
 Email: edesigns@connect.com.fj

Site: Funafuti, Tuvalu. Page 2
 Job Name: Funafuti Power Project. Job No: 170/05
 TEST METHOD USED: NZS 4402 : 1988 Test 6.5.2 Dynamic Cone Penetrometer

Depth from ground surface to commencement of penetration: 0.00m

SOIL DESCRIPTION MAIN \ minor components, strength, colour structure, weathering.	REFER TO TEST PIT LOGS
--	-------------------------------

Vertical Distance driven (mm)	Number of blows	DYNAMIC CONE PENETROMETER --- SCALA	REMARKS
TEST 1C			
50	2		
100	3		
150	5		
200	4		
250	1		
300	1		
350	8		
400	8		
450	13		
500	>30		

Vertical Distance driven (mm)	Number of blows	DYNAMIC CONE PENETROMETER --- SCALA	REMARKS
TEST 1D			
50	2		
100	>30		
150			
200			
250			
300			
350			
400			
450			
500			

TESTED BY: SVC

DATE: 11/01/05

AUTHORISED BY: VK



Suite A10 - A12 Grantham Plaza
 PO Box 117 Suva Fiji
 Telephone:[679] 3383788
 Fax:[679] 3370354
 Email: edesigns@connect.com.fj

Site: Funafuti, Tuvalu.

Page 3

Job Name: Funafuti Power Project.

Job No: 170/05

TEST METHOD USED: NZS 4402 : 1988 Test 6.5.2 Dynamic Cone Penetrometer

Depth from ground surface to commencement of penetration: 0.00m

SOIL DESCRIPTION

MAIN \ minor components, strength, colour
 structure, weathering.

REFER TO TEST PIT LOGS

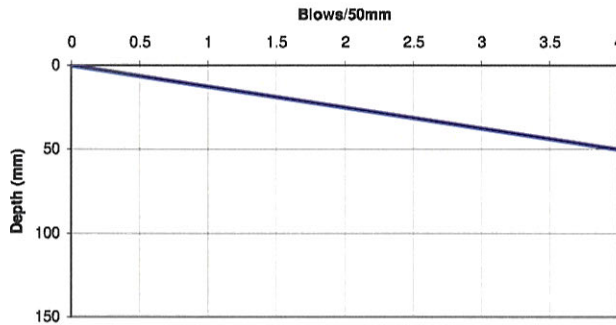
Vertical Distance driven (mm)	Number of blows
-------------------------------	-----------------

DYNAMIC CONE PENETROMETER --- SCALA

REMARKS

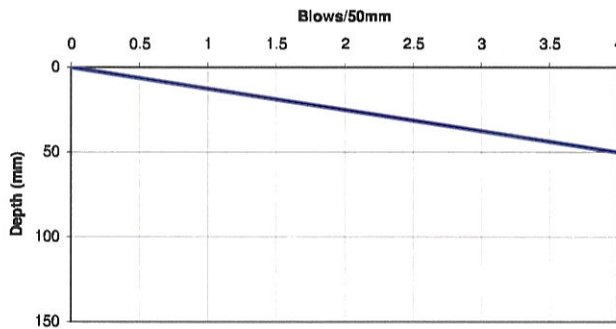
TEST 2

50	4
100	>30
150	
200	
250	
300	
350	
400	
450	
500	



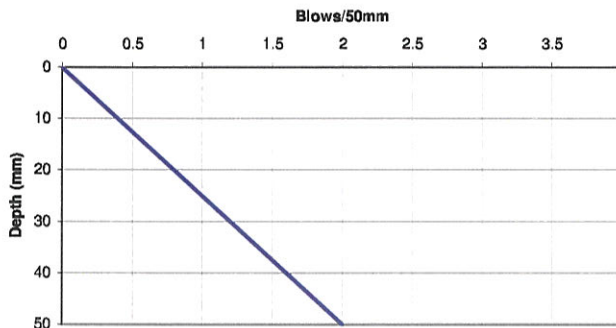
TEST 2A

50	4
100	>30
150	
200	
250	
300	
350	
400	
450	
500	



TEST 2B

50	2
100	>30
150	
200	
250	
300	
350	
400	
450	
500	



TESTED BY: SVC

DATE: 11/01/05

AUTHORISED BY: VK

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Site: Funafuti, Tuvalu.

Page 4

Job Name: Funafuti Power Project.

Job No: 170/05

TEST METHOD USED: NZS 4402 : 1988 Test 6.5.2 Dynamic Cone Penetrometer

Depth from ground surface to commencement of penetration: 0.00m

SOIL DESCRIPTION

MAIN\ minor components, strength, colour structure, weathering.

REFER TO TEST PIT LOGS

Vertical Distance driven (mm)	Number of blows	DYNAMIC CONE PENETROMETER --- SCALA	REMARKS
TEST 2C			
50	1		
100	>30		
150			
200			
250			
300			
350			
400			
450			
500			

Vertical Distance driven (mm)	Number of blows	DYNAMIC CONE PENETROMETER --- SCALA	REMARKS
TEST 2D			
50	2		
100	>30		
150			
200			
250			
300			
350			
400			
450			
500			

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DATE: 11/01/05

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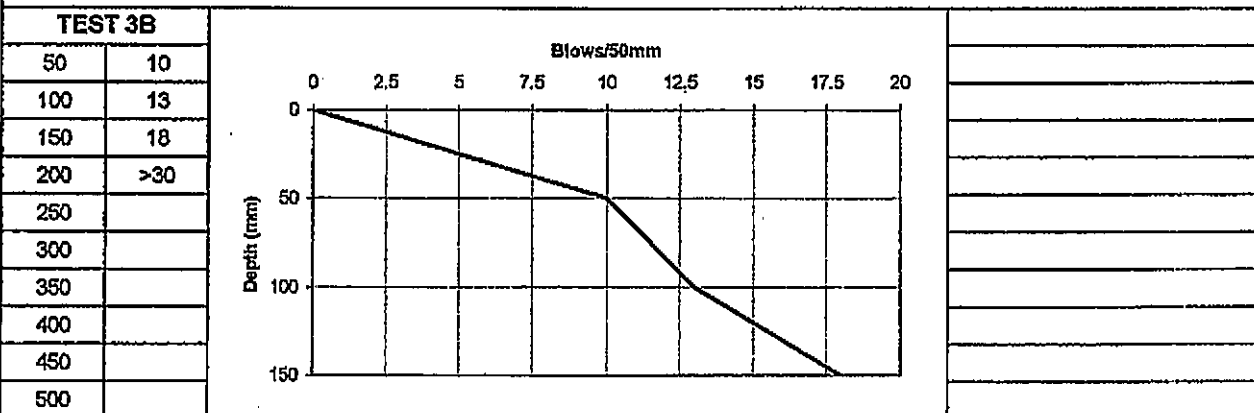
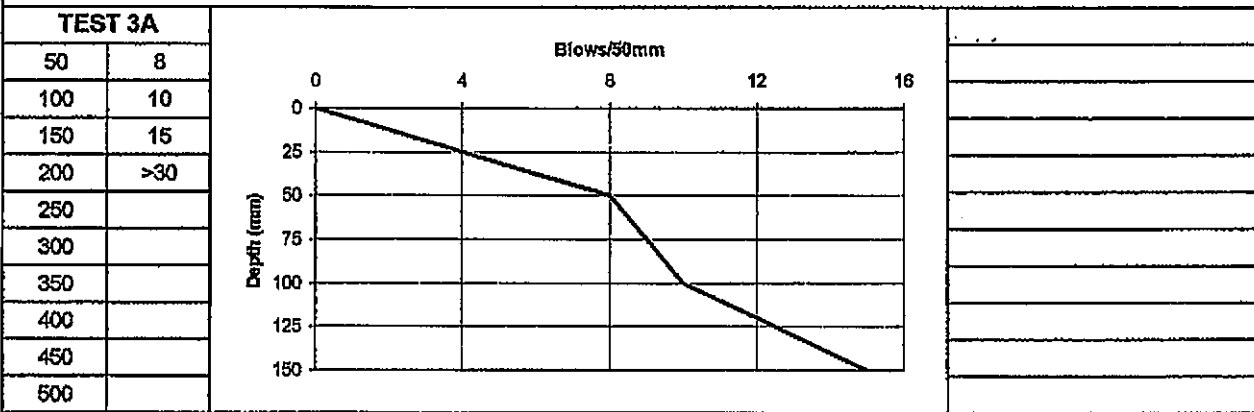
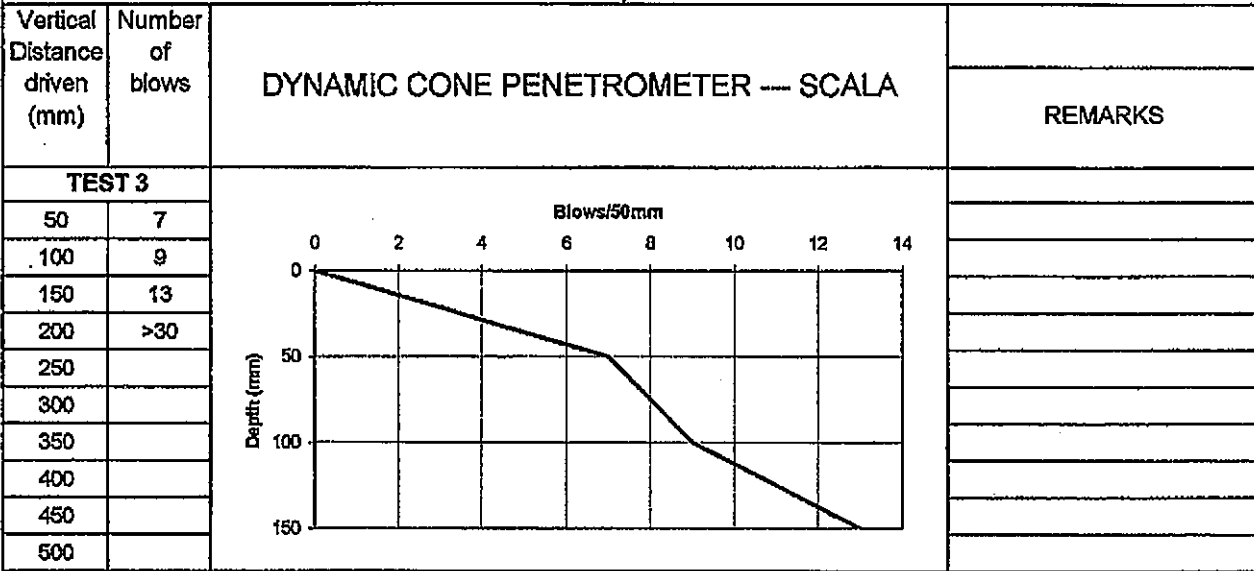
Site: Funafuti, Tuvalu. Page 5

Job Name: Funafuti Power Project. Job No: 170/05

TEST METHOD USED: NZS 4402 : 1988 Test 6.5.2 Dynamic Cone Penetrometer

Depth from ground surface to commencement of penetration: 0.00m

SOIL DESCRIPTION
 MAIN \ minor components, strength, colour
 structure, weathering. REFER TO TEST PIT LOGS



TESTED BY: SVC DATE: 11/01/05 AUTHORISED BY: VK



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Site: Funafuti, Tuvalu. Page 6

Job Name: Funafuti Power Project. Job No: 170/05

TEST METHOD USED: NZS 4402 : 1988 Test 6.5.2 Dynamic Cone Penetrometer

Depth from ground surface to commencement of penetration: 0.00m

SOIL DESCRIPTION

MAIN \ minor components, strength, colour
 structure, weathering.

REFER TO TEST PIT LOGS

Vertical Distance driven (mm)	Number of blows	DYNAMIC CONE PENETROMETER — SCALA	REMARKS
TEST 3C			
50	11		
100	15		
150	18		
200	>30		
250			
300			
350			
400			
450			
500			

Vertical Distance driven (mm)	Number of blows	DYNAMIC CONE PENETROMETER — SCALA	REMARKS
TEST 3D			
50	15		
100	>30		
150			
200			
250			
300			
350			
400			
450			
500			

TESTED BY: SVC

DATE: 11/01/05

AUTHORISED BY: YK



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Site: Funafuti, Tuvalu.		Page 7
Job Name: Funafuti Power Project.		Job No: 170/05
TEST METHOD USED: NZS 4402 : 1988 Test 6.5.2 Dynamic Cone Penetrometer		
Depth from ground surface to commencement of penetration: 0.00m		
SOIL DESCRIPTION MAIN \ minor components, strength, colour structure, weathering.		REFER TO TEST PIT LOGS
Vertical Distance driven (mm)	Number of blows	DYNAMIC CONE PENETROMETER --- SCALA
		REMARKS
TEST 4		
50	0.5	
100	0.5	
150	10	
200	15	
250	17	
300	25	
350	>30	
400		
450		
500		
TEST 4A		
50	0.5	
100	0.5	
150	15	
200	18	
250	20	
300	21	
350	22	
400	23	
450	>30	
500		
TEST 4B		
50	1	
100	0.5	
150	0.5	
200	0.25	
250	0.25	
300	0.25	
350	0.25	
400	2	
450	4	
500	4	
550	14	
600	>30	
TESTED BY: SVC		DATE: 11/01/05
		AUTHORISED BY: VK



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Site: Funafuti, Tuvalu.

Page 8

Job Name: Funafuti Power Project.

Job No: 170/05

TEST METHOD USED: NZS 4402 : 1988 Test 6.5.2 Dynamic Cone Penetrometer

Depth from ground surface to commencement of penetration: 0.00m

SOIL DESCRIPTION

MAIN \ minor components, strength, colour structure, weathering.

REFER TO TEST PIT LOGS

Vertical Distance driven (mm)	Number of blows	DYNAMIC CONE PENETROMETER --- SCALA	REMARKS
TEST 4C			
50	3		
100	4		
150	>30		
200			
250			
300			
350			
400			
450			
500			

Vertical Distance driven (mm)	Number of blows	DYNAMIC CONE PENETROMETER --- SCALA	REMARKS
TEST 4D			
50	4		
100	17		
150	>30		
200			
250			
300			
350			
400			
450			
500			

TESTED BY: SVC

DATE: 11/01/05

AUTHORISED BY: VK



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Site: Funafuti, Tuvalu.

Page 9

Job Name: Funafuti Power Project.

Job No: 170/05

TEST METHOD USED: NZS 4402 : 1988 Test 6.5.2 Dynamic Cone Penetrometer

Depth from ground surface to commencement of penetration: 0.00m

SOIL DESCRIPTION

MAIN \ minor components, strength, colour structure, weathering.

Grey, mottled with light black SAND with medium to coarse GRAVEL, dry tightly packed subangular to subrounded.

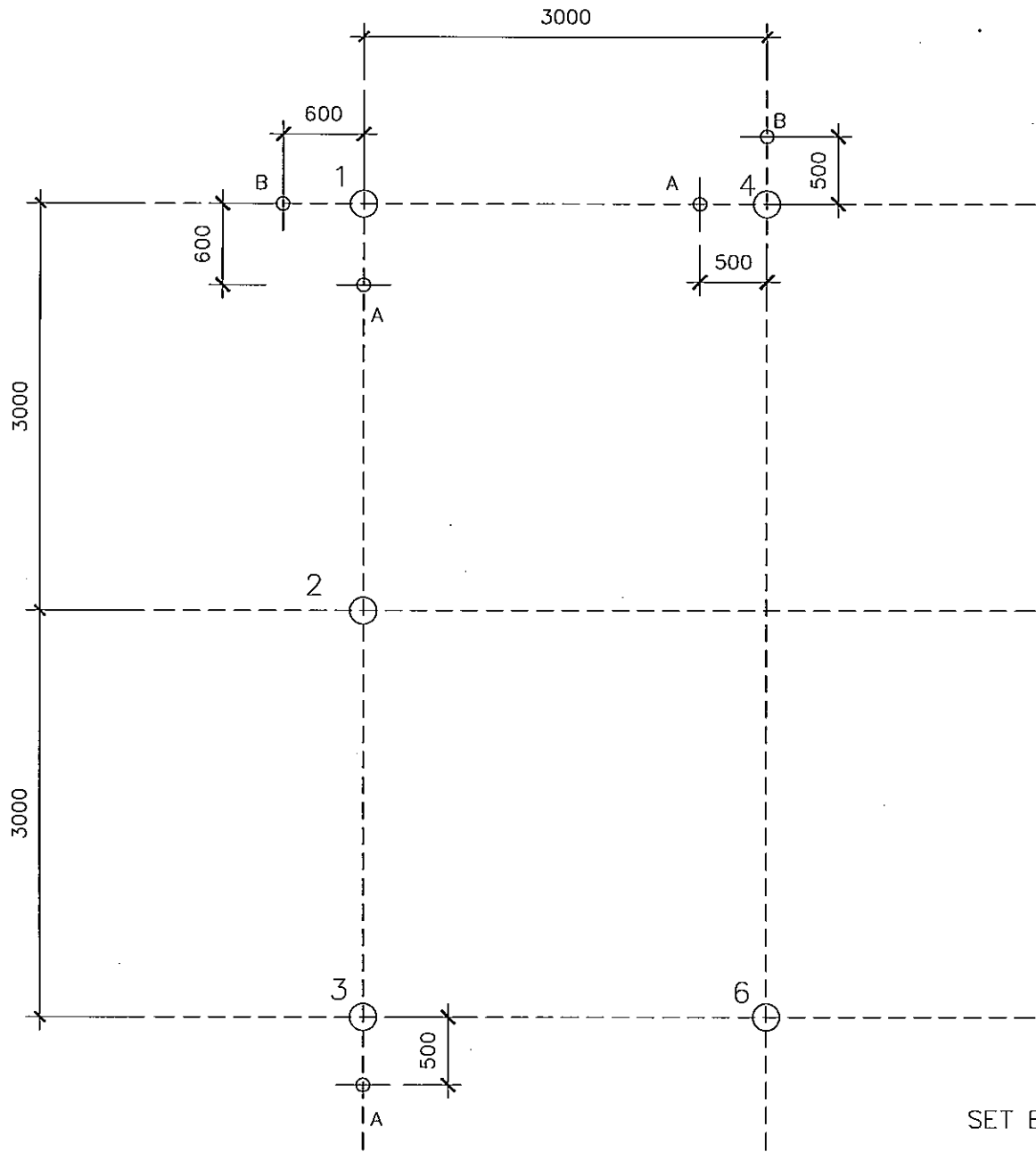
Vertical Distance driven (mm)	Number of blows	DYNAMIC CONE PENETROMETER --- SCALA		REMARKS
TEST 6				
50	4			
100	>30			
150				
200				
250				
300				
350				
400				
450				
500				
TEST 6A				
50	4			
100	>30			
150				
200				
250				
300				
350				
400				
450				
500				
TEST 6B				
50	4			
100	>30			
150				
200				
250				
300				
350				
400				
450				
500				

TESTED BY: SVC


DATE: 11/01/05

AUTHORISED BY: VK

**DYNAMIC CONE
PENETROMETER
RESULTS FOR
SET B2**



SET B2

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<p>Copyright of this drawing shall remain the property of Engineered Designs. Figured dimensions shall take preference over scaled. Precedence shall be given to the largest scale drawing. The contractor shall verify all dimensions prior to preparation of shop drawings or commencement of any work. The Engineer shall be notified immediately of any discrepancies.</p>									
Project Title FUNAFUTI POWER PROJECT									
Drawing Title DYNAMIC CONE PENETROMETER TEST LOCATION									
Scale 1:50	Test By SVC <hr/> Drawn HK								
Job No 170/05	Date JAN 05								
Division GT									
Drawing No 2b	Revision <table border="1" data-bbox="1892 1420 2072 1508"> <tr> <td> </td> <td> </td> <td> </td> <td> </td> </tr> <tr> <td> </td> <td> </td> <td> </td> <td> </td> </tr> </table>								



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Site: Funafuti, Tuvalu.

Page 1

Job Name: Funafuti Power Project.

Job No: 170/05

TEST METHOD USED: NZS 4402 : 1988 Test 6.5.2 Dynamic Cone Penetrometer

Depth from ground surface to commencement of penetration: 0.00m

SOIL DESCRIPTION

MAIN \ minor components, strength, colour
structure, weathering.

REFER TO TEST PIT LOGS

Vertical Distance driven (mm)	Number of blows	DYNAMIC CONE PENETROMETER -- SCALA		REMARKS
TEST 1				
50	1			
100	>30			
150				
200				
250				
300				
350				
400				
450				
500				
TEST 1A				
50	1			
100	1			
150	1			
200	1			
250	1			
300	1			
350	1			
400	>30			
450				
500				
TEST 1B				
50	1			
100	1			
150	1			
200	1			
250	1			
300	1			
350	>30			
400				
450				
500				
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Site: Funafuti, Tuvalu.

Page 2

Job Name: Funafuti Power Project.

Job No: 170/05

TEST METHOD USED: NZS 4402 : 1988 Test 6.5.2 Dynamic Cone Penetrometer

Depth from ground surface to commencement of penetration: 0.00m

SOIL DESCRIPTION

MAIN \ minor components, strength, colour
 structure, weathering.

REFER TO TEST PIT LOGS

Vertical Distance driven (mm)	Number of blows	DYNAMIC CONE PENETROMETER --- SCALA	REMARKS
TEST 2			
50	1		
100	1		
150	1		
200	2		
250	>30		
300			
350			
400			
450			
500			

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Site: Funafuti, Tuvalu.

Page 3

Job Name: Funafuti Power Project.

Job No: 170/05

TEST METHOD USED: NZS 4402 : 1988 Test 6.5.2 Dynamic Cone Penetrometer

Depth from ground surface to commencement of penetration: 0.00m

SOIL DESCRIPTION

MAIN \ minor components, strength, colour structure, weathering.

REFER TO TEST PIT LOGS

Vertical Distance driven (mm)

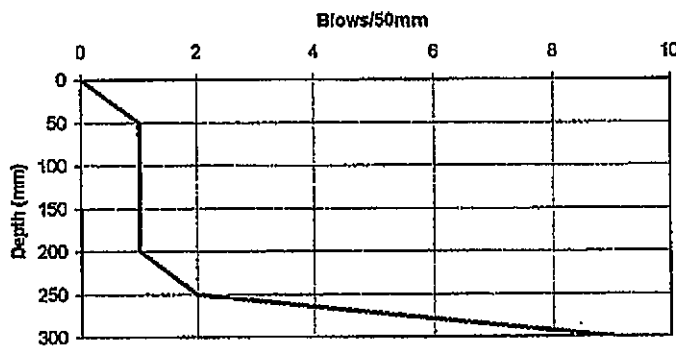
Number of blows

DYNAMIC CONE PENETROMETER -- SCALA

REMARKS

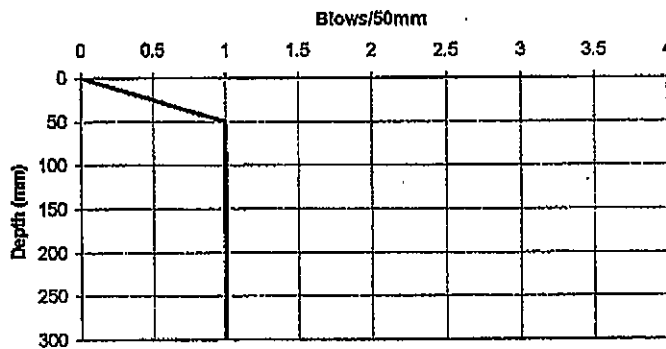
TEST 3

50	1
100	1
150	1
200	1
250	2
300	9
350	>30
400	
450	
500	



TEST 3A

50	1
100	1
150	1
200	1
250	1
300	1
350	>30
400	
450	
500	



TESTED BY: SVC

DATE: 11/01/05

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Site: Funafuti, Tuvalu. Page 6
 Job Name: Funafuti Power Project. Job No: 170/05
 TEST METHOD USED: NZS 4402 : 1988 Test 6.5.2 Dynamic Cone Penetrometer

Depth from ground surface to commencement of penetration: 0.00m

SOIL DESCRIPTION
 MAIN \ minor components, strength, colour
 structure, weathering. REFER TO TEST PIT LOGS

Vertical Distance driven (mm)	Number of blows	DYNAMIC CONE PENETROMETER --- SCALA	REMARKS																						
TEST 6		<p>The graph plots 'Blows/50mm' on the x-axis (0 to 20) against 'Depth (mm)' on the y-axis (0 to 400). The data points from the table are plotted as follows:</p> <table border="1"> <tr><th>Depth (mm)</th><th>Blows/50mm</th></tr> <tr><td>50</td><td>1</td></tr> <tr><td>100</td><td>1</td></tr> <tr><td>150</td><td>8</td></tr> <tr><td>200</td><td>8</td></tr> <tr><td>250</td><td>9</td></tr> <tr><td>300</td><td>10</td></tr> <tr><td>350</td><td>17</td></tr> <tr><td>400</td><td>18</td></tr> <tr><td>450</td><td>>30</td></tr> <tr><td>500</td><td></td></tr> </table>	Depth (mm)	Blows/50mm	50	1	100	1	150	8	200	8	250	9	300	10	350	17	400	18	450	>30	500		
Depth (mm)	Blows/50mm																								
50	1																								
100	1																								
150	8																								
200	8																								
250	9																								
300	10																								
350	17																								
400	18																								
450	>30																								
500																									
50	1																								
100	1																								
150	8																								
200	8																								
250	9																								
300	10																								
350	17																								
400	18																								
450	>30																								
500																									



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Site: Funafuti, Tuvalu.

Page 4

Job Name: Funafuti Power Project.

Job No: 170/05

TEST METHOD USED: NZS 4402 : 1988 Test 6.5.2 Dynamic Cone Penetrometer

Depth from ground surface to commencement of penetration: 0.00m

SOIL DESCRIPTION

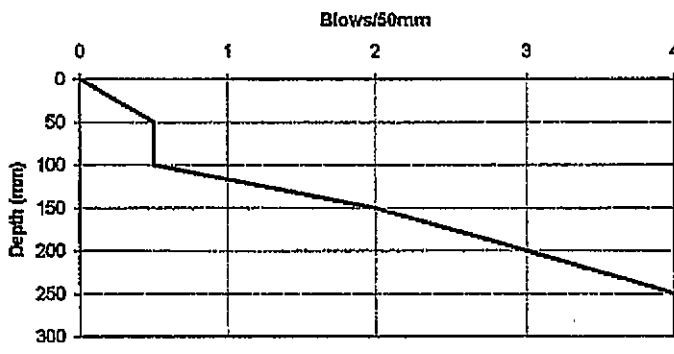
MAIN \ minor components, strength, colour structure, weathering.

REFER TO TEST PIT LOGS

Vertical Distance driven (mm)	Number of blows	DYNAMIC CONE PENETROMETER --- SCALA	REMARKS
-------------------------------	-----------------	-------------------------------------	---------

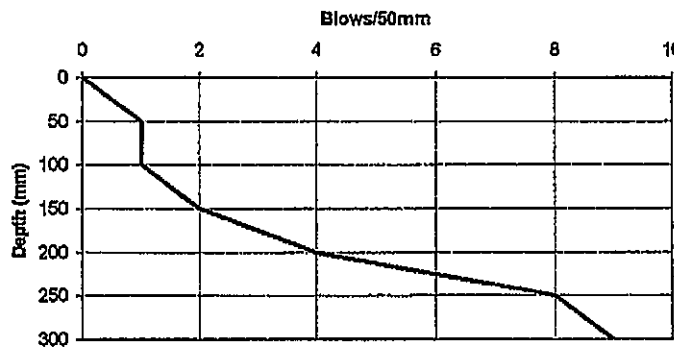
TEST 4

Vertical Distance driven (mm)	Number of blows
50	0.5
100	0.5
150	2
200	3
250	4
300	4
350	4
400	>30
450	
500	



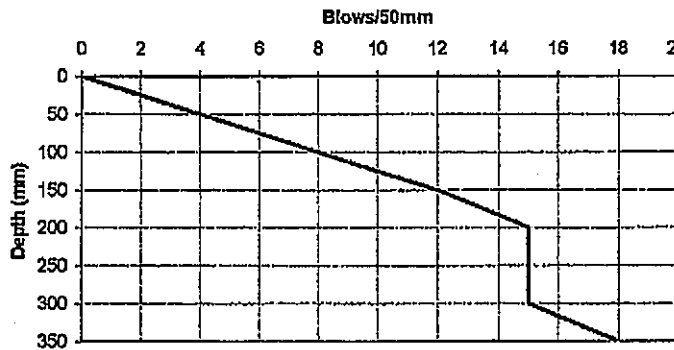
TEST 4A

Vertical Distance driven (mm)	Number of blows
50	1
100	1
150	2
200	4
250	8
300	9
350	>30
400	
450	
500	



TEST 4B

Vertical Distance driven (mm)	Number of blows
50	4
100	8
150	12
200	15
250	15
300	15
350	18
400	>30
450	
500	



TESTED BY: SVC

DATE: 11/01/05

AUTHORISED BY: VK

Appendix B: Site Photos

Appendix B: Site Photos



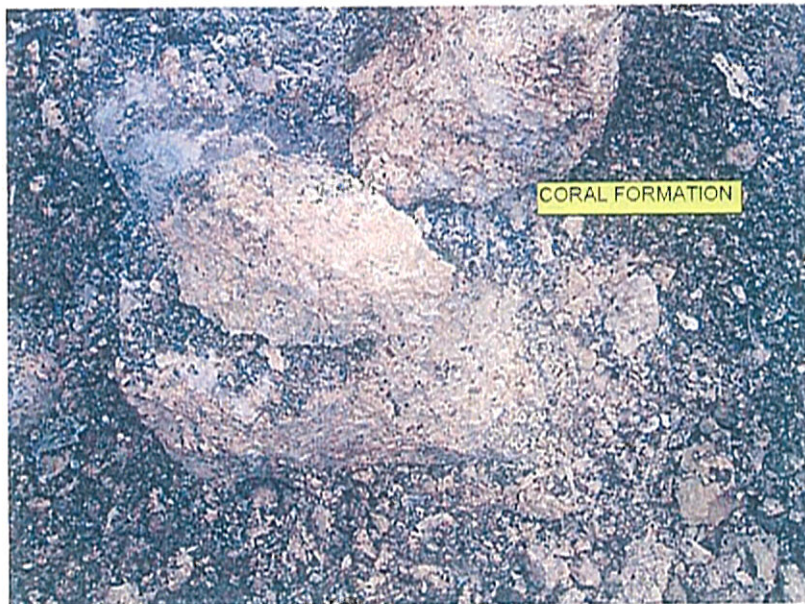
Photo(1): Area B2 (Dynamic Cone penetrometer test locations)



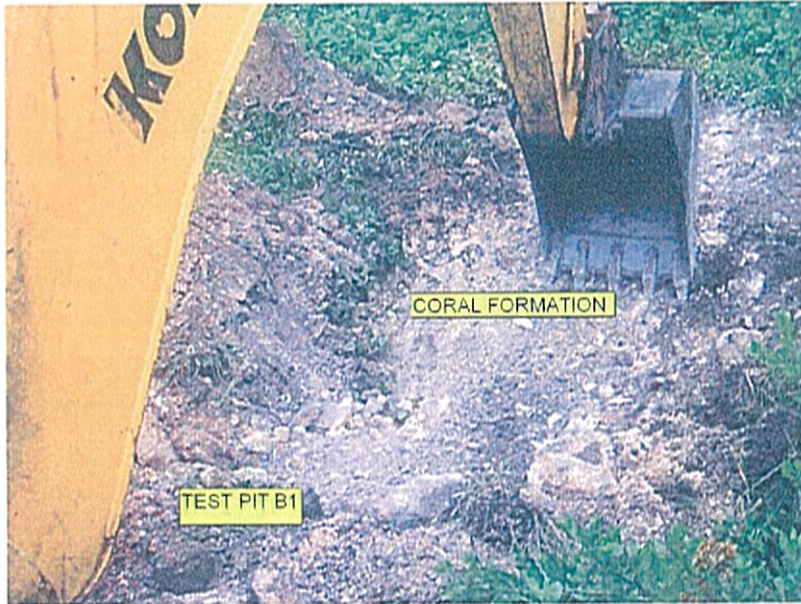
Photo(2): Area B1 (Dynamic Cone penetrometer test locations)



Photo(3)



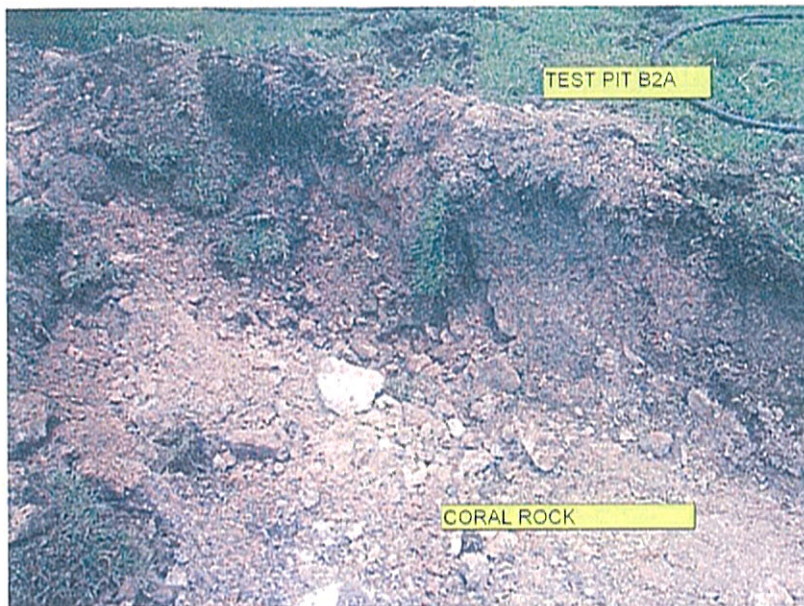
Photo(4): Coral Formation



Photo(5): Test Pit B1 / Coral Formation



Photo(6): Test Pits B1, B2 and B2A



Photo(7): Test Pit B2A / Coral rock

Appendix C: Laboratory Test Results

Site: Funafuti, Tuvalu.			Page: 1	
Job Name: Funafuti Power Project.			Job Number: 170/2004	
Test Method Used: NZS 4402 : 1986 Test 2.1 Determination of Water Content (delete non-applicable) NZS 4407 : 1991 Test 3.1 Water Content of Aggregate NZS 3111 : Aggregate Moisture Content				
WATER CONTENT				
Date in oven	20-Jan-05	20-Jan-05	20-Jan-05	Average moisture content
Sample No.	B1	B1	B1	
Depth(mm)	100	100	100	
Tin No.	B02	B04	B05	
Tin mass M1 (g)	120.28	118.76	119.32	
Tin + wet mass M2 (g)	228.54	204.87	202.10	
Tin + dry mass M3 (g)	209.81	186.97	185.10	
Water content ω (%)	20.92	26.24	25.84	24.34
Date in oven	20-Jan-05	20-Jan-05	20-Jan-05	Average moisture content
Sample No.	B2	B2	B2	
Depth(mm)	100	100	100	
Tin No.	A01	A03	A05	
Tin mass M1 (g)	96.33	93.98	94.57	
Tin + wet mass M2 (g)	176.06	160.09	169.80	
Tin + dry mass M3 (g)	162.42	148.87	156.56	
Water content ω (%)	20.64	20.44	21.36	20.81
Date in oven	20-Jan-05	20-Jan-05	20-Jan-05	Average moisture content
Sample No.	B2	B2	B2	
Depth(mm)	200	200	200	
Tin No.	A07	A08	A09	
Tin mass M1 (g)	96.59	96.25	104.57	
Tin + wet mass M2 (g)	169.72	201.16	179.72	
Tin + dry mass M3 (g)	156.46	182.47	168.38	
Water content ω (%)	22.15	21.68	17.77	20.53
TESTED BY: SVC			DATE: 20/01/05	



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Plate No.:

Page of

Site : Funafuti Power Station, Funafuti, Tuvalu Islands

Job No.: 750468

Test Method Used: NZS 4402:1986 Test 2.7.1 Determination of Solid Density

SOLID DENSITY TEST RESULTS

Table 1: Solid Density

BH/Pit No.	B1	B2	B2	B2A
Sample No.	1	1	2	1
Depth (m)	0.1	0.1	0.2	0.2
Solid Density (t/m ³)	2.62	2.48	2.47	2.62

Tested by: ST

Date: 14/2/05

Checked by:

970

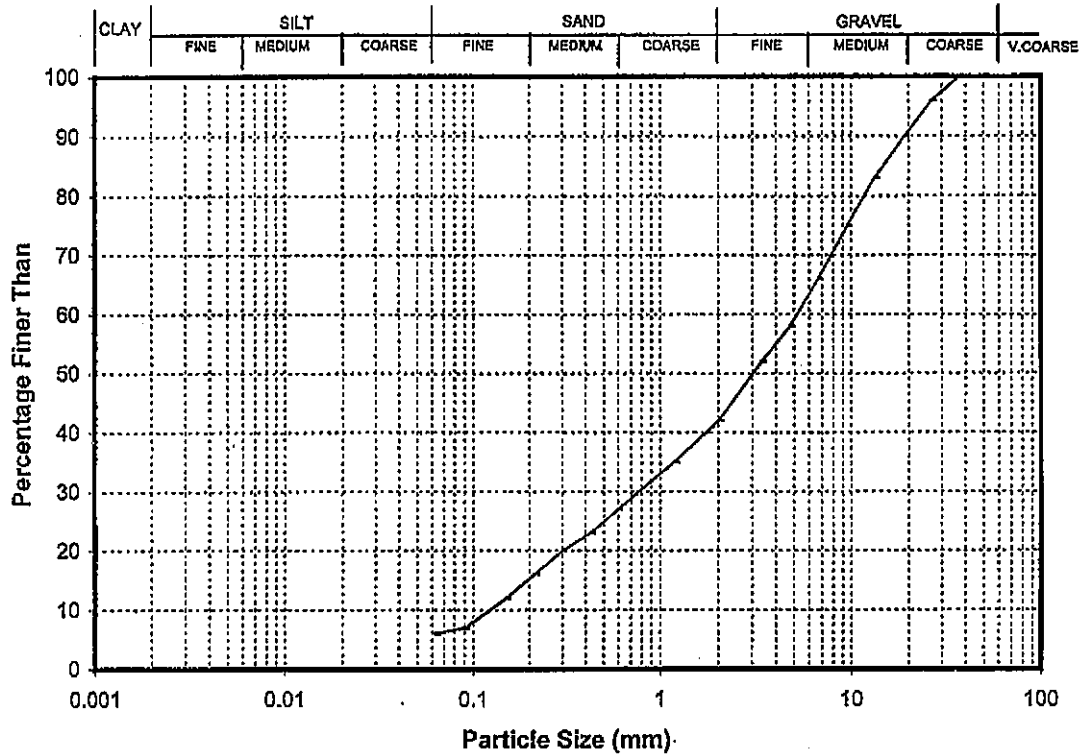
Date: 14/2/05



Plate No.:
 Site : Funafuti Power Station, Funafuti, Tuvalu Islands
 BH/Test Pit No.: B1 Sample No. : 1
 Test Method Used : NZS 4402 : 1986 Test 2.8.1 Wet Sieve

Page of
 Job No. : 750468
 Depth : 0.1 (m)

PARTICLE SIZE ANALYSIS



Sieve (mm)	Total % Passing
63.0	---
53.0	---
37.5	100
26.5	96
19.0	90
13.2	83
9.50	75
6.70	66
4.75	58
3.35	52

Sieve (mm)	Total % Passing
2.00	42
1.18	35
0.600	27
0.425	23
0.300	20
0.212	16
0.150	12
0.090	7
0.063	6

Sample history : As received at natural water content.

Description : CORAL mixed with sand, loose, medium to dark brown with grey/white.

Remarks: Percentage passing the finest sieve was obtained by difference.

Entered by : ST

Date : 14/2/05

Checked by : GJD

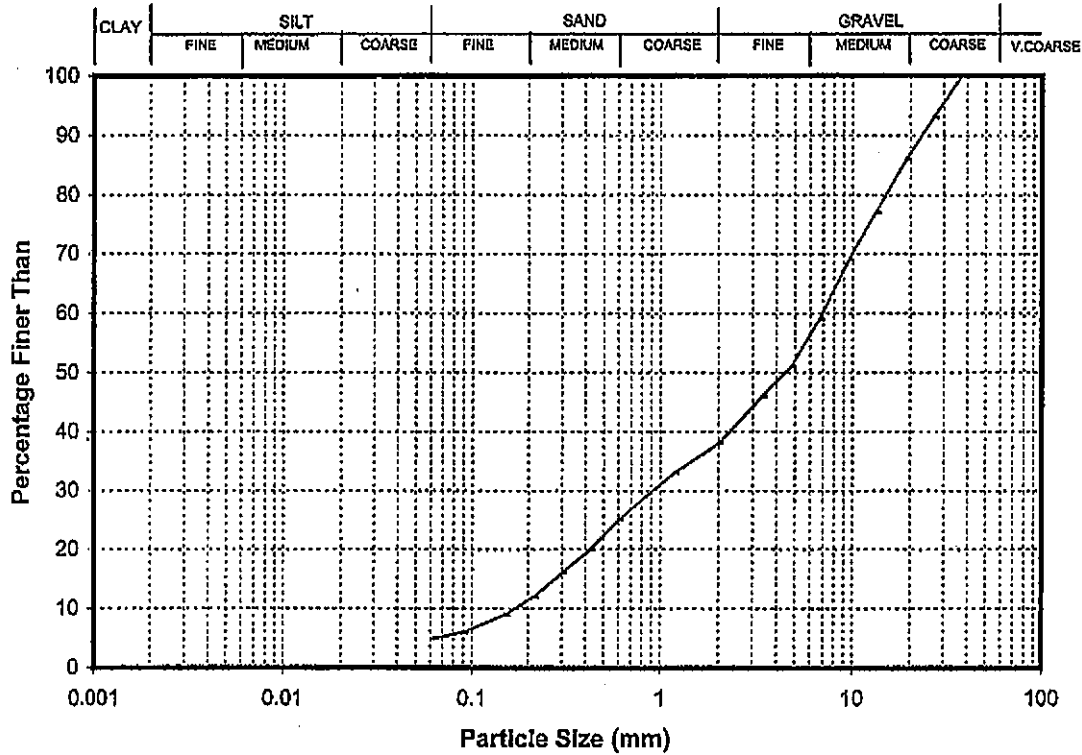
Date : 14/2/05



Plate No.:
 Site : **Funafuti Power Station, Funafuti, Tuvalu Islands**
 BH/Test Pit No.: B2 Sample No. : 1
 Test Method Used : NZS 4402 : 1986 Test 2.8.1 Wet Sieve

Page of
 Job No. : **750468**
 Depth : 0.1 (m)

PARTICLE SIZE ANALYSIS



Sieve (mm)	Total % Passing
63.0	---
53.0	---
37.5	100
26.5	93
19.0	86
13.2	77
9.50	69
6.70	59
4.75	51
3.35	46

Sieve (mm)	Total % Passing
2.00	38
1.18	33
0.600	25
0.425	20
0.300	16
0.212	12
0.150	9
0.090	6
0.063	5

Sample history : As received at natural water content.

Description : CORAL mixed with sand, loose, medium to dark brown with grey/white.

Remarks: Percentage passing the finest sieve was obtained by difference.

Entered by : *ST*

Date : *14/2/05*

Checked by : *970*

Date : *14/2/05*



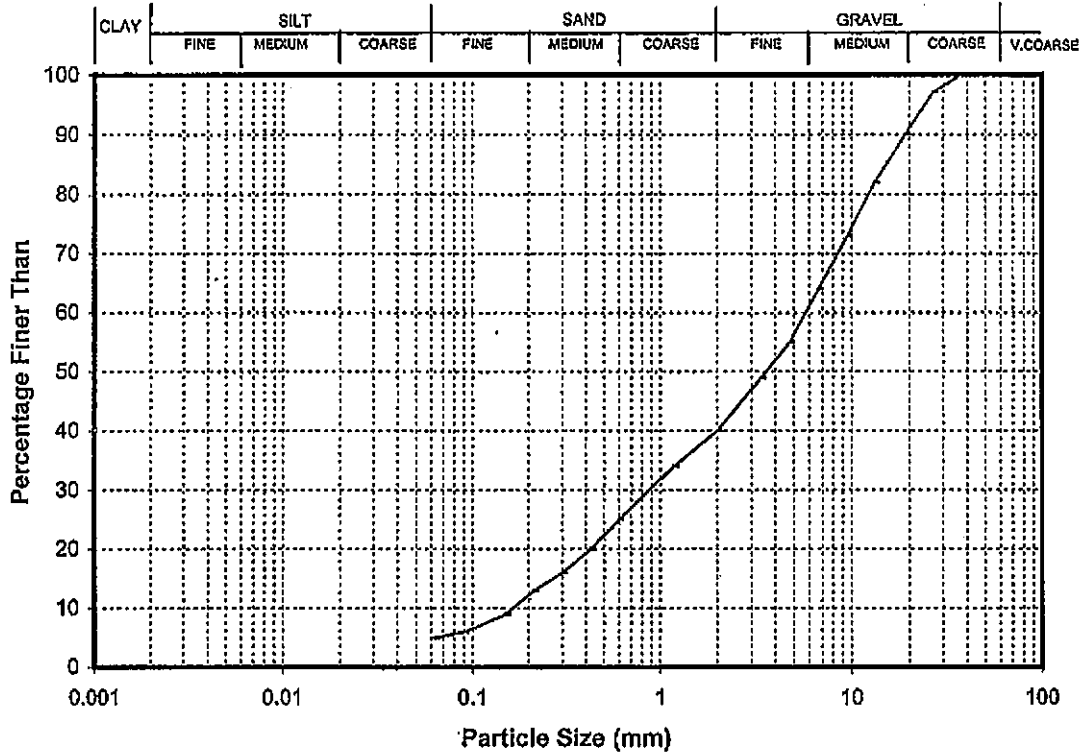
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Form No.: S5
 Form Date: JANUARY 2004
 File: M:\sieve\750468\BH2_2_0.2m

Plate No.:
 Site : Funafuti Power Station, Funafuti, Tuvalu Islands
 BH/Test Pit No.: B2 Sample No. : 2
 Test Method Used : NZS 4402 : 1986 Test 2.8.1 Wet Sieve

Page of
 Job No. : 750468
 Depth : 0.2 (m)

PARTICLE SIZE ANALYSIS



Sieve (mm)	Total % Passing
63.0	---
53.0	---
37.5	100
26.5	97
19.0	90
13.2	82
9.50	73
6.70	64
4.75	55
3.35	49

Sieve (mm)	Total % Passing
2.00	40
1.18	34
0.600	25
0.425	20
0.300	16
0.212	13
0.150	9
0.090	6
0.063	5

Sample history : As received at natural water content.

Description : CORAL mixed with sand, loose, medium to dark brown with grey/white.

Remarks: Percentage passing the finest sieve was obtained by difference.

Entered by : ST

Date : 14/2/05

Checked by : 970

Date : 14/2/05



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Form No.: S5

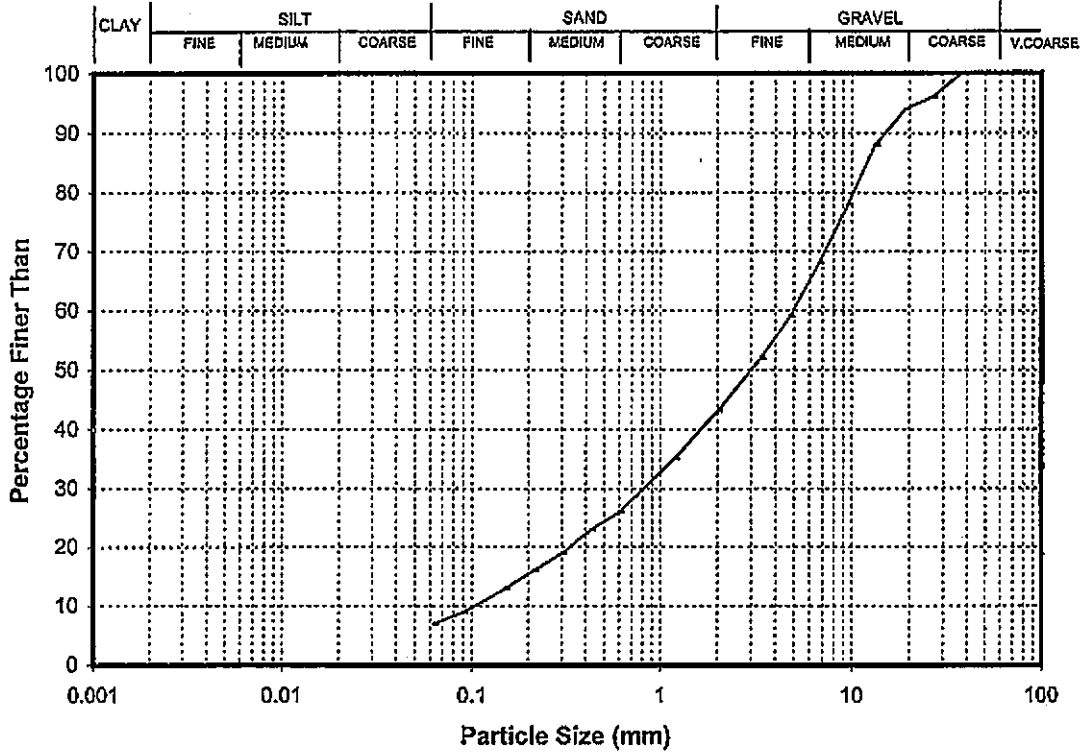
Form Date: JANUARY 2004

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Plate No.:
 Site : **Funafuti Power Station, Funafuti, Tuvalu Islands**
 BH/Test Pit No.: B2A Sample No. : 1
 Test Method Used : NZS 4402 : 1986 Test 2.8.1 Wet Sieve

Page of
 Job No. : 750468
 Depth : 0.2 (m)

PARTICLE SIZE ANALYSIS



Sieve (mm)	Total % Passing
63.0	---
53.0	---
37.5	100
26.5	96
19.0	94
13.2	88
9.50	78
6.70	68
4.75	59
3.35	52

Sieve (mm)	Total % Passing
2.00	43
1.18	35
0.600	26
0.425	23
0.300	19
0.212	16
0.150	13
0.090	9
0.063	7

Sample history : As received at natural water content.

Description : CORAL mixed with sand, loose, medium to dark brown with grey/white.

Remarks: Percentage passing the finest sieve was obtained by difference.

Entered by : ST

Date : 14/2/05

Checked by : 990

Date : 14/2/05