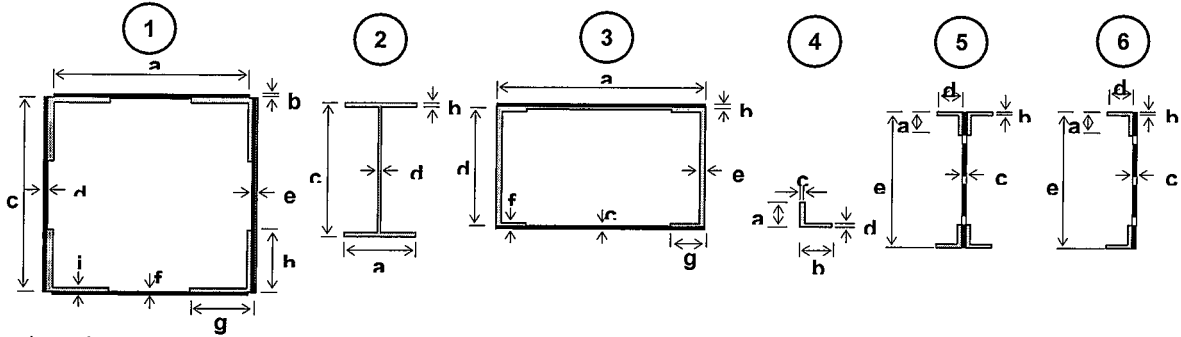


Appendix 21.1.4-1 (1/2)
SECTION PROPERTIES OF STEEL TRUSS MEMBERS



Legend :

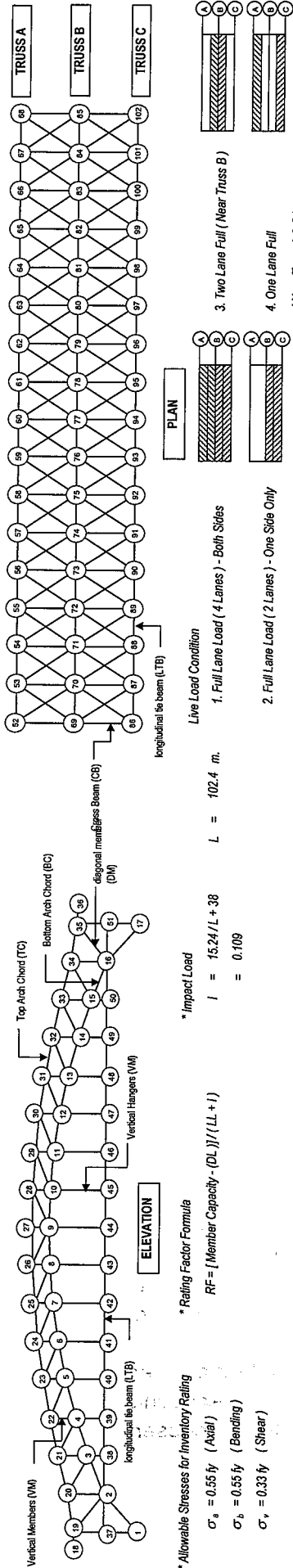
- A = Sectional area
- $A_{v,y}$ = Conventional shearing area along Y-axis
- I_x = Torsional moment of inertia
- I_y = Inertia moment about centroidal Y-axis
- I_z = Inertia moment about centroidal Z-axis

SECTION TYPE	MEMBER	LOCATION	GEOMETRIC DIMENSION								
			a (mm)	b (mm)	c (mm)	d (mm)	e (mm)	f (mm)	g (mm)	h (mm)	i (mm)
1	Top Arch Chord (Truss A&C)	GL1-GL6 / GL12 - GL17	560	11	660	14	13	8	175	200	14
1	Top Arch Chord (Truss A&C)	GL7-GL11	560	10	620	17	17	8	175	200	15
1	Top Arch Chord (Truss B)	GL1-GL17	560	15	620	14	14	8	150	200	14
1	Bottom Arch Chord (Truss A&C)	GL2-GL5 / GL13-GL16	560	10	690	20	20	8	150	200	15
1	Bottom Arch Chord (Truss A&C)	GL5-GL13	560	10	690	15	15	12	150	200	15
1	Bottom Arch Chord (Truss B)	GL2-GL6 / GL12-GL16	560	14	690	23	23	8	150	200	22
1	Bottom Arch Chord (Truss B)	GL6-GL12	560	13	690	15	23	8	150	200	15
2	Vertical Member (Truss A&C)	GL2-GL8 / GL10-GL16	310	610	20	14	-	-	-	-	-
2	Vertical Member (Truss A&C)	GL9	230	20	610	12	-	-	-	-	-
2	Vertical Member (Truss B)	GL2-GL8 / GL10-GL16	310	610	20	15	-	-	-	-	-
2	Vertical Member (Truss B)	GL9	230	20	610	12	-	-	-	-	-
3	Vertical Member (Truss A&C)	GL1 & GL17	610	12	12	410	15	12	90	-	-
3	Vertical Member (Truss B)	GL1 & GL17	610	15	15	410	15	12	90	-	-
2	Hangers (Truss A,B&C)	GL3-GL15	230	18	610	14	-	-	-	-	-
2	Diagonal Member (Truss A&C)	GL1-GL7 / GL11 - GL17	230	19	610	14	-	-	-	-	-
2	Diagonal Member (Truss A&C)	GL7-GL11	310	20	610	12	-	-	-	-	-
2	Diagonal Member (Truss B)	GL1-GL7 / GL11 - GL17	230	19	610	13	-	-	-	-	-
2	Diagonal Member (Truss B)	GL7-GL11	310	20	610	12	-	-	-	-	-
2	Cross Beam	GL1-GL17	300	25	900	15	-	-	-	-	-
5	Transverse Sway Braces (TC)	GL1-GL17	100	8	10	155	705	-	-	-	-
6	Diagonal Sway Braces (TC)	GL1-GL17	75	8	10	100	695	-	-	-	-
5	Transverse Sway Braces (BC)	GL1-GL17	100	8	10	155	765	-	-	-	-
6	Diagonal Sway Braces (BC)	GL1-GL17	75	8	10	100	745	-	-	-	-

Appendix 21.1.4-1 (2/2)
SECTION PROPERTIES OF STEEL TRUSS MEMBERS

SECTION TYPE	MEMBER	LOCATION	GEOMETRIC PROPERTIES					
			A (m ²)	A _{vy} (m ²)	I _x (m ⁴)	I _y (m ⁴)	I _z (m ⁴)	Y _M (m)
1	Top Arch Chord (Truss A&C)	GL1-GL6 / GL12 - GL17	0.0477	0.01730	0.000020	0.00307	0.00291	0.32500
1	Top Arch Chord (Truss A&C)	GL7-GL11	0.0528	0.02108	0.000030	0.00344	0.00310	0.32470
1	Top Arch Chord (Truss B)	GL1-GL17	0.0465	0.01736	0.000020	0.00296	0.00287	0.33700
1	Bottom Arch Chord (Truss A&C)	GL2-GL5 / GL13-GL16	0.0578	0.02760	0.000035	0.00390	0.00400	0.35180
1	Bottom Arch Chord (Truss A&C)	GL5-GL13	0.0531	0.02622	0.000026	0.00350	0.00400	0.34949
1	Bottom Arch Chord (Truss B)	GL2-GL6 / GL12-GL16	0.0729	0.03174	0.000070	0.00480	0.00510	0.36940
1	Bottom Arch Chord (Truss B)	GL6-GL12	0.0613	0.02070	0.000040	0.00400	0.00460	0.36920
2	Vertical Member (Truss A&C)	GL2-GL8 / GL10-GL16	0.0204	0.00798	0.000003	0.00010	0.01300	0.30500
2	Vertical Member (Truss A&C)	GL9	0.0160	0.00684	0.000001	0.00004	0.00099	0.30500
2	Vertical Member (Truss B)	GL2-GL8 / GL10-GL16	0.0209	0.00855	0.000003	0.00010	0.01130	0.30500
2	Vertical Member (Truss B)	GL9	0.0160	0.00684	0.000001	0.00004	0.00099	0.30500
3	Vertical Member (Truss A&C)	GL1 & GL17	0.0298	0.01200	0.000007	0.00171	0.00085	0.20500
3	Vertical Member (Truss B)	GL1 & GL17	0.0342	0.01200	0.000012	0.00187	0.00100	0.20500
2	Hangers (Truss A,B&C)	GL3-GL15	0.0163	0.00804	0.000001	0.00004	0.00095	0.30500
2	Diagonal Member (Truss A&C)	GL1-GL7 / GL11 - GL17	0.0168	0.00800	0.000002	0.00004	0.00098	0.30500
2	Diagonal Member (Truss A&C)	GL7-GL11	0.0204	0.00684	0.000003	0.00010	0.00130	0.30500
2	Diagonal Member (Truss B)	GL1-GL7 / GL11 - GL17	0.0162	0.00744	0.000001	0.00004	0.00097	0.30500
2	Diagonal Member (Truss B)	GL7-GL11	0.0192	0.00684	0.000002	0.00010	0.00126	0.30500
2	Cross Beam	GL1-GL17	0.0278	0.01275	0.000003	0.00011	0.00364	0.45000
5	Transverse Sway Braces (TC)	GL1-GL17	0.0150	0.00705	0.000001	0.00004	0.00116	0.38250
6	Diagonal Sway Braces (TC)	GL1-GL17	0.0096	0.00695	0.0000004	0.00001	0.00057	0.34750
5	Transverse Sway Braces (BC)	GL1-GL17	0.0150	0.00765	0.000001	0.00004	0.00140	0.38250
6	Diagonal Sway Braces (BC)	GL1-GL17	0.0101	0.00705	0.0000004	0.00001	0.00068	0.37250

BRIDGE NAME : QUEZON BRIDGE



* Allowable Stresses for Inventory Rating
 $\sigma_a = 0.55 f_y$ (Axial)
 $\sigma_b = 0.55 f_y$ (Bending)
 $\sigma_v = 0.33 f_y$ (Shear)

* Rating Factor Formula
 $RF = [\text{Member Capacity} - (DL)] / (LL + I)$

* Impact Load
 $I = 15.24 / (L + 38)$
 $L = 102.4 \text{ m.}$
 $I = 0.109$

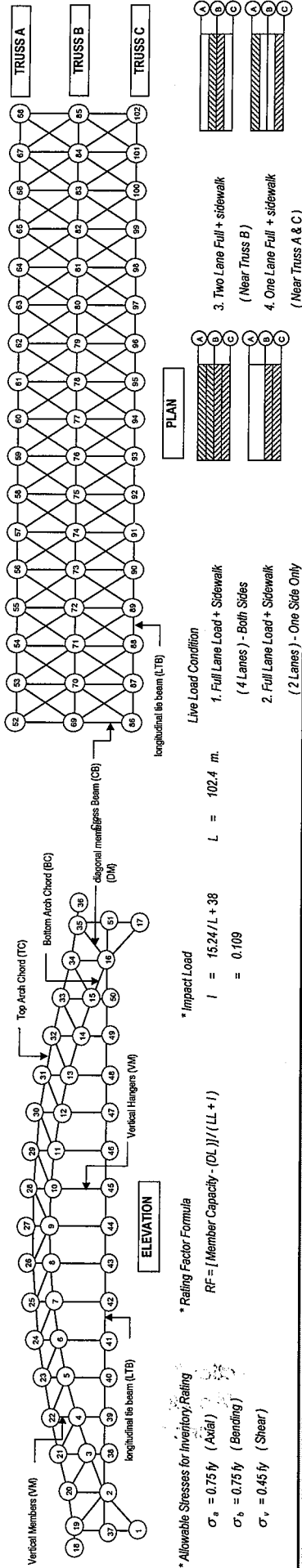
Live Load Condition
 1. Full Lane Load (4 Lanes) - Both Sides
 2. Full Lane Load (2 Lanes) - One Side Only
 3. Two Lane Full (Near Truss B)
 4. One Lane Full (Near Truss A & C)

LOAD RATING - INVENTORY LEVEL (ASR)

LOCATION	MEMBER I.D.	SECTION PROPERTIES				ALLOWABLE STRESSES (SERVICEABILITY LIMIT)				DEAD LOAD				MEMBER CAPACITY				LIVE LOAD plus IMPACT				RATING FACTOR (RF)		EQUIV. LL (HS20)		
		A_{gross} (in ²)	A_{net} (in ²)	$\sigma_{a,allow}$ (MPa)	$\sigma_{b,allow}$ (MPa)	f_y (MPa)	σ_a (MPa)	σ_b (MPa)	σ_v (allow) (MPa)	P_{DL} (KN)	M_{DL} (KN-m)	$\sigma_{b,DL}$ (MPa)	$\sigma_{v,DL}$ (MPa)	P_{imp} (KN)	M_{imp} (KN-m)	$\sigma_{b,imp}$ (MPa)	$\sigma_{v,imp}$ (MPa)	Axial	Bending	Shear	Axial	Bending	Shear			
TOP CHORD (TRUSS A)	TC 18-19	0.04641	0.01946	0.338	0.00308	228	125.4	125.4	75.24	0.74	0.02	7.31	0.80	7.27	0.37	5694	1143	1464	1.11	1.11	1.11	5136.16	1024.213	1314.2432	32775	
	TC 19-20	0.04709	0.01946	0.351	0.0034	228	125.4	125.4	75.24	560.7	11.91	41.47	4.28	20.38	1.05	5905	1215	1464	1.11	1.11	1.11	45.77	217.76751	943.78039	1465	
	TC 20-21	0.04709	0.01946	0.351	0.0034	228	125.4	125.4	75.24	979.48	20.80	27.02	2.79	19.48	1.00	5905	1215	1464	1.11	1.11	1.11	25.73	139.66516	651.61441	823	
	TC 21-22	0.04709	0.01946	0.351	0.0034	228	125.4	125.4	75.24	1111.57	23.61	56.67	5.85	17.09	0.88	5905	1215	1464	1.11	1.11	1.11	24.3	89.9	1044.3	776	
	TC 22-23	0.04709	0.01946	0.351	0.0034	228	125.4	125.4	75.24	1421.56	30.19	77.11	7.96	15.27	0.78	5905	1215	1464	1.11	1.11	1.11	17.5	58.5	1156.7	560	
	TC 23-24	0.04709	0.01946	0.351	0.0034	228	125.4	125.4	75.24	2090.71	44.40	74	7.64	25.75	1.32	5905	1215	1464	1.11	1.11	1.11	9.0	64.6	439.9	289	
	TC 24-25	0.05115	0.02363	0.351	0.0034	228	125.4	125.4	75.24	2576.93	50.38	37.34	3.85	13.54	0.57	6414	1215	1778	1.11	1.11	1.11	6.8	218.1	3789.6	216	
	TC 25-26	0.05115	0.02363	0.351	0.0034	228	125.4	125.4	75.24	2918.15	57.07	43.07	4.45	14.74	0.62	6414	1215	1778	1.11	1.11	1.11	5.0	73.5	1120.1	161	
	TC 26-27	0.05115	0.02363	0.351	0.0034	228	125.4	125.4	75.24	2951.66	57.71	167.78	17.32	34.32	1.45	6414	1215	1778	1.11	1.11	1.11	4.8	22.4	381.8	154	
	TC 27-28	0.05959	0.02363	0.347	0.00336	228	125.4	125.4	75.24	2915.47	58.32	167.81	17.33	34.34	1.45	6344	1214	1778	1.11	1.11	1.11	4.7	22.4	381.8	151	
	TC 28-29	0.04947	0.02363	0.339	0.0032	228	125.4	125.4	75.24	2915.47	58.93	43.02	4.56	14.74	0.62	6204	1184	1778	1.11	1.11	1.11	4.8	71.7	1120.1	152	
	TC 29-30	0.05115	0.02363	0.351	0.0034	228	125.4	125.4	75.24	2571.7	50.28	37.27	3.85	13.52	0.57	6414	1215	1778	1.11	1.11	1.11	6.8	219.0	3882.0	217	
	TC 30-31	0.04709	0.01946	0.351	0.0034	228	125.4	125.4	75.24	2087.08	44.32	73.67	7.61	25.68	1.32	5905	1215	1464	1.11	1.11	1.11	9.1	64.8	442.9	290	
	TC 31-32	0.04653	0.01946	0.347	0.00311	228	125.4	125.4	75.24	1420.35	30.53	76.76	8.56	15.1	0.78	5835	1124	1464	1.11	1.11	1.11	3.25	54.1	1188.3	552	
	TC 32-33	0.04709	0.01946	0.351	0.0034	228	125.4	125.4	75.24	1110.45	23.58	57.4	5.93	17.51	0.90	5905	1215	1464	1.11	1.11	1.11	1.22	47.2	54.1	1188.3	552
	TC 33-34	0.04709	0.01946	0.351	0.0034	228	125.4	125.4	75.24	980.63	20.82	25.24	2.61	18.77	0.96	5905	1215	1464	1.11	1.11	1.11	2.06	25.7	160.3	777	
	TC 34-35	0.04709	0.01946	0.351	0.0034	228	125.4	125.4	75.24	549.45	11.67	40.81	4.21	19.87	1.02	5905	1215	1464	1.11	1.11	1.11	1.42	46.9	221.5	1501	
	TC 35-36	0.04653	0.02363	0.347	0.00311	228	125.4	125.4	75.24	0.56	0.01	7.29	0.81	7.27	0.31	5835	1124	1778	1.11	1.11	1.11	5263.02	1007.2753	1597.2721	32233	
	TOP CHORD (TRUSS B)	TC 18-19	0.04724	0.01946	0.335	0.00332	228	125.4	125.4	75.24	1.4	0.03	13.84	1.40	13.77	0.71	5924	1243	1464	1.11	1.11	1.11	5342.58	1108.5954	1308.3797	35475
		TC 19-20	0.04892	0.01946	0.347	0.0035	228	125.4	125.4	75.24	888.47	18.16	91.57	9.08	40.96	2.10	6135	1265	1464	1.11	1.11	1.11	25.87	61.178427	292.44922	828
		TC 20-21	0.04892	0.01946	0.347	0.0035	228	125.4	125.4	75.24	1755.9	38.89	47.1	4.67	27.77	1.43	6135	1265	1464	1.11	1.11	1.11	11.17	98.097426	887.50038	357
		TC 21-22	0.04892	0.01946	0.347	0.0035	228	125.4	125.4	75.24	2244.57	45.88	76.21	7.56	32.15	1.65	6135	1265	1464	1.11	1.11	1.11	7.87	80.359185	569.07464	252
		TC 22-23	0.04892	0.01946	0.347	0.0035	228	125.4	125.4	75.24	2855.48	56.37	86.82	8.61	23.69	1.22	6135	1265	1464	1.11	1.11	1.11	5.10	60.105864	1939.4493	163
		TC 23-24	0.04892	0.01946	0.347	0.0035	228	125.4	125.4	75.24	3775.63	77.18	88.25	8.75	38.13	1.96	6135	1265	1464	1.11	1.11	1.11	2.62	58.76975	385.15119	84
		TC 24-25	0.0584	0.01946	0.344	0.00422	228	125.4	125.4	75.24	4272.87	73.17	52.41	4.27	18.02	0.93	7323	1638	1464	1.11	1.11	1.11	0.40	267.55046	3623.7365	89
		TC 25-26	0.06976	0.03058	0.347	0.0047	228	125.4	125.4	75.24	4660.21	66.80	62.1	4.58	18.62	0.61	8748	1699	2301	1.11	1.11	1.11	3.43	60.473988	666.26157	101

LOCATION	MEMBER I.D.	SECTION PROPERTIES				ALLOWABLE STRESSES (SERVICEABILITY LIMIT)				DEAD LOAD						MEMBER CAPACITY				LIVE LOAD plus IMPACT			RATING FACTOR (RF)			EQUIV. LL (HS20)
		A _{gross} (m ²)	A _{steel} (m ²)	C _{pa} (m)	I _{bending} (m ⁴)	fy (MPa)	σ _s (allow) (MPa)	σ _s (allow) (MPa)	σ _s (allow) (MPa)	P _{br} (KN)	σ _{axial} (MPa)	M _{br} (KN-m)	σ _{bending} (MPa)	V _{br} (KN)	σ _{shear} (MPa)	P _{cap} (KN)	M _{cap} (KN-m)	V _{cap} (KN)	F _u (KN)	M _L (KN-m)	V _L (KN)	Axial	Bending	Shear		
																									RF = (Cap - DL) / (LL+I)	
LONGITUDINAL TIE BEAM	LTB 96-91	0.00808	0.003	0.15	0.00013	228	125.4	125.4	75.24	6.89	0.85	14.62	16.87	7.5	2.50	1013	109	226	7.82	2.89	0.51	128.77	36.26	427.94	1160	
	LTB 91-92	0.00808	0.003	0.15	0.00013	228	125.4	125.4	75.24	6.9	0.85	6.82	7.87	5.59	1.86	1013	109	226	7.83	0.71	0.08	128.58	143.57	2836.79	4115	
	LTB 92-93	0.00808	0.003	0.15	0.00013	228	125.4	125.4	75.24	6.87	0.85	7.46	8.61	5.78	1.93	1013	109	226	7.82	2.43	0.34	128.77	41.69	640.01	1334	
	LTB 93-94	0.00808	0.003	0.15	0.00013	228	125.4	125.4	75.24	6.85	0.85	18.72	21.60	8.01	2.67	1013	109	226	7.82	5.68	0.50	128.77	15.85	436.43	507	
	LTB 94-95	0.00808	0.003	0.15	0.00013	228	125.4	125.4	75.24	6.84	0.85	18.69	21.57	8.03	2.68	1013	109	226	7.80	5.68	0.50	128.96	15.86	436.39	507	
	LTB 95-96	0.00808	0.003	0.15	0.00013	228	125.4	125.4	75.24	6.83	0.85	7.67	8.85	5.67	1.89	1013	109	226	7.80	2.41	0.31	128.96	41.99	703.94	1344	
	LTB 96-97	0.00808	0.003	0.15	0.00013	228	125.4	125.4	75.24	6.83	0.85	7.03	8.11	5.48	1.83	1013	109	226	7.80	0.69	0.06	128.96	147.90	3973.49	4127	
	LTB 97-98	0.00808	0.003	0.15	0.00013	228	125.4	125.4	75.24	6.79	0.84	14.68	16.94	7.48	2.49	1013	109	226	7.79	2.61	0.51	128.15	36.08	427.98	1155	
	LTB 98-99	0.00808	0.003	0.15	0.00013	228	125.4	125.4	75.24	6.67	0.83	14.72	16.98	6.13	2.04	1013	109	226	7.76	2.63	0.20	128.71	35.76	1100.49	1144	
	LTB 99-100	0.00808	0.003	0.15	0.00013	228	125.4	125.4	75.24	9.78	1.21	10.27	11.85	5.83	1.94	1013	109	226	8.95	1.35	0.10	112.17	72.77	2203.99	2328	
	LTB 100-101	0.00988	0.0048	0.15	0.00014	228	125.4	125.4	75.24	33.29	3.37	10.08	10.80	7.79	1.62	1239	117	361	16.73	2.83	0.58	72.07	36.55	613.00	1170	
	LTB 101-102	0.00988	0.0048	0.15	0.00014	228	125.4	125.4	75.24	17.8	1.80	39.39	42.20	11.16	2.33	1239	117	361	5.81	6.96	1.18	210.22	11.15	297.85	357	

BRIDGE NAME : QUEZON BRIDGE



* Allowable Stresses for Inventory Rating
 $\sigma_a = 0.75 f_y$ (Axial)
 $\sigma_b = 0.75 f_y$ (Bending)
 $\sigma_v = 0.45 f_y$ (Shear)

* Rating Factor Formula

$$RF = \frac{[Member Capacity - (DL)] / (LL + I)}{L}$$

$$I = 15.24 / L + 38$$

$$L = 102.4 \text{ m}$$

Live Load Condition
 1. Full Lane Load + Sidewalk
 (4 Lanes) - Both Sides
 2. Full Lane Load + Sidewalk
 (2 Lanes) - One Side Only

MEMBER LOCATION	SECTION PROPERTIES				ALLOWABLE STRESSES (SERVICEABILITY LIMIT)				DEAD LOAD				MEMBER CAPACITY				LIVE LOAD plus IMPACT				RATING FACTOR (RF)		EQUIV. LL (HS20)		
	A _{gross} (m ²)	A _{shear} (m ²)	C _{xx} (m)	I _{bending} (m ⁴)	f _y (MPa)	σ _s (allow) (MPa)	σ _s (allow) (MPa)	σ _v (allow) (MPa)	P _{DL} (KN)	σ _{axial} (MPa)	M _{DL} (KN-m)	σ _{bending} (MPa)	V _{DL} (KN)	σ _{shear} (MPa)	P _{exp} (KN)	M _{exp} (KN-m)	V _{exp} (KN)	F _{LL} (KN)	M _{LL} (KN-m)	V _{LL} (KN)	RF Axial	RF Bending		RF Shear	32 TONS
TOP CHORD (TRUSS A)	TC 18-19	0.04641	0.01946	0.338	0.00308	228	171	171	102.6	0.74	0.02	7.31	0.80	7.27	0.37	7765	1558	4659	1.11	1.11	1.11	7004.10	1399.0519	4196.3002	44770
	TC 19-20	0.04709	0.01946	0.351	0.0034	228	171	171	102.6	560.7	11.91	41.47	4.28	20.36	1.05	8052	1656	4831	205.45	14.77	3.98	36.47	109.36998	1208.9036	1167
	TC 20-21	0.04709	0.01946	0.351	0.0034	228	171	171	102.6	979.48	20.80	27.02	2.79	19.48	1.00	8052	1656	4831	388.26	12.04	2.47	19.74	135.34468	1946.5362	632
	TC 21-22	0.04709	0.01946	0.351	0.0034	228	171	171	102.6	1111.57	23.61	56.67	5.85	17.09	0.88	8052	1656	4831	413.98	16.57	1.81	16.8	96.5	2654.4	537
	TC 22-23	0.04709	0.01946	0.351	0.0034	228	171	171	102.6	1421.56	30.19	77.11	7.96	15.27	0.78	8052	1656	4831	545.49	25.95	1.35	12.2	60.9	3551.1	389
	TC 23-24	0.04709	0.01946	0.351	0.0034	228	171	171	102.6	2090.71	44.40	74	7.64	25.75	1.32	8052	1656	4831	820.79	25.91	5.61	7.3	61.1	856.7	232
	TC 24-25	0.05115	0.02363	0.351	0.0034	228	171	171	102.6	2576.93	50.38	37.34	3.85	13.54	0.57	8747	1656	5248	1036.69	11.31	0.53	6.0	143.2	9837.3	190
	TC 25-26	0.05115	0.02363	0.351	0.0034	228	171	171	102.6	2919.15	57.07	43.07	4.45	14.74	0.62	8747	1656	5248	1208.36	20.20	1.67	4.8	79.9	3126.4	154
	TC 26-27	0.05115	0.02363	0.351	0.0034	228	171	171	102.6	2951.66	57.71	167.78	17.32	34.32	1.45	8747	1656	5248	1236.85	76.97	8.77	4.7	19.3	594.6	150
	TC 27-28	0.05059	0.02363	0.347	0.00346	228	171	171	102.6	2950.41	58.32	167.81	16.83	34.34	1.45	8851	1705	5191	1236.32	76.98	8.77	4.6	20.0	588.0	148
	TC 28-29	0.04947	0.02363	0.339	0.0033	228	171	171	102.6	2915.47	58.93	43.02	4.42	14.74	0.62	8459	1656	5076	1206.88	20.18	1.67	4.6	80.4	3023.4	147
	TC 29-30	0.05115	0.02363	0.351	0.0034	228	171	171	102.6	2571.7	50.28	37.27	3.85	13.52	0.57	8747	1656	5248	1034.65	11.26	0.52	6.0	143.8	10046.6	191
	TC 30-31	0.04709	0.01946	0.351	0.0034	228	171	171	102.6	2087.08	44.32	73.67	7.61	25.68	1.32	8052	1656	4831	819.48	25.76	5.58	7.3	61.4	861.9	233
TC 31-32	0.04653	0.01946	0.347	0.00311	228	171	171	102.6	1420.35	30.53	76.76	8.56	15.1	0.78	7957	1533	4774	545.28	25.80	1.29	12.0	56.4	3700.8	384	
TC 32-33	0.04709	0.01946	0.351	0.0034	228	171	171	102.6	1110.45	23.58	57.4	5.93	17.51	0.90	8052	1656	4831	413.85	16.86	1.97	16.8	94.8	2439.6	537	
TC 33-34	0.04709	0.01946	0.351	0.0034	228	171	171	102.6	980.63	20.82	25.24	2.61	18.77	0.96	8052	1656	4831	359.11	11.11	2.21	19.7	146.9	2181.6	630	
TC 34-35	0.04709	0.01946	0.351	0.0034	228	171	171	102.6	549.45	11.67	40.81	4.21	19.87	1.02	8052	1656	4831	201.43	14.60	3.81	37.2	110.7	1261.8	1192	
TC 31-32	0.04653	0.02363	0.347	0.00311	228	171	171	102.6	0.56	0.01	7.29	0.81	7.27	0.31	7957	1533	4774	1.11	1.11	1.11	7177.03	1375.9486	4299.9602	44030	
TOP CHORD (TRUSS B)	TC 18-19	0.04724	0.01946	0.335	0.00332	228	171	171	102.6	1.4	0.03	13.84	1.40	13.77	0.71	8078	1695	4847	1.11	1.11	1.11	7285.79	1516.261	4359.8097	48520
	TC 19-20	0.04892	0.01946	0.347	0.0035	228	171	171	102.6	888.47	18.16	91.57	9.08	40.96	2.10	8365	1725	5019	293.14	23.12	6.16	25.51	70.827631	807.69263	816
	TC 20-21	0.04892	0.01946	0.347	0.0035	228	171	171	102.6	1755.9	35.89	47.1	4.67	27.77	1.43	8365	1725	5019	547.47	17.66	2.77	12.07	95.003638	1801.0682	386
	TC 21-22	0.04892	0.01946	0.347	0.0035	228	171	171	102.6	2244.57	45.88	76.21	7.56	32.15	1.65	8365	1725	5019	656.79	22.59	3.44	9.32	72.970969	1451.1988	298
	TC 22-23	0.04892	0.01946	0.347	0.0035	228	171	171	102.6	2855.48	58.37	86.82	8.61	23.69	1.22	8365	1725	5019	829.64	29.10	1.04	6.64	56.288658	4783.9905	213
	TC 23-24	0.04892	0.01946	0.347	0.0035	228	171	171	102.6	3775.63	77.18	88.25	8.75	38.13	1.96	8365	1725	5019	1167.24	29.31	5.47	3.93	55.835374	911.42495	126
TC 24-25	0.0584	0.01946	0.344	0.00422	228	171	171	102.6	4272.87	73.17	52.41	4.27	18.02	0.93	9986	2098	5992	1420.75	11.33	0.85	4.02	180.53308	6998.5375	129	
TC 25-26	0.06976	0.03058	0.347	0.0047	228	171	171	102.6	4660.21	66.80	62.1	4.58	18.62	0.61	11929	2316	7157	1671.46	36.72	4.04	4.35	61.392717	1769.1597	139	

LOCATION	MEMBER I.D.	SECTION PROPERTIES				ALLOWABLE STRESSES (SERVICEABILITY LIMIT)				DEAD LOAD				MEMBER CAPACITY				LIVE LOAD plus IMPACT				RATING FACTOR (RF)				EQUIV. LL (HS20)	
		A _{gross} (m ²)	A _{net} (m ²)	C _{pa} (m)	I _{bending} (m ⁴)	f _y (MPa)	σ _s (allow) (MPa)	σ _s (allow) (MPa)	σ _s (allow) (MPa)	P _{ax} (KN)	σ _{ax} (MPa)	M _{cap} (KN-m)	V _{cap} (KN)	P _{ax} (KN)	M _{cap} (KN-m)	V _{cap} (KN)	F _{LL} (KN)	M _L (KN-m)	V _L (KN)	Axial	Bending	Shear	Axial	Bending	Shear		RF = (Cap - DL) / (LL+I)
	LTB 89-90	0.00808	0.003	0.15	0.00013	228	171	171	102.6	6.94	0.86	14.66	16.92	6.09	2.03	829	148	829	8.48	3.64	0.28	162.11	36.73	2969.36	1175		
	LTB 90-91	0.00808	0.003	0.15	0.00013	228	171	171	102.6	6.89	0.85	14.62	16.87	7.5	2.50	829	148	829	8.62	3.64	0.81	159.41	36.74	1015.16	1176		
	LTB 91-92	0.00808	0.003	0.15	0.00013	228	171	171	102.6	6.9	0.85	6.82	7.87	5.69	1.86	829	148	829	8.62	1.57	0.13	159.40	89.81	6189.92	2874		
	LTB 92-93	0.00808	0.003	0.15	0.00013	228	171	171	102.6	6.87	0.85	7.46	8.61	5.78	1.93	829	148	829	8.61	2.98	0.35	159.61	47.20	2320.88	1510		
	LTB 93-94	0.00808	0.003	0.15	0.00013	228	171	171	102.6	6.85	0.85	18.72	21.60	8.01	2.67	829	148	829	8.61	9.33	0.99	159.62	13.87	832.14	444		
	LTB 94-95	0.00808	0.003	0.15	0.00013	228	171	171	102.6	6.84	0.85	18.69	21.57	8.03	2.68	829	148	829	8.60	9.33	1.00	159.82	13.88	822.88	444		
	LTB 95-96	0.00808	0.003	0.15	0.00013	228	171	171	102.6	6.83	0.85	7.67	8.85	5.67	1.89	829	148	829	8.60	2.93	0.30	159.82	48.02	2750.81	1537		
	LTB 96-97	0.00808	0.003	0.15	0.00013	228	171	171	102.6	6.83	0.85	7.03	8.11	5.48	1.83	829	148	829	8.59	1.53	0.09	160.03	92.28	9286.12	2953		
	LTB 97-98	0.00808	0.003	0.15	0.00013	228	171	171	102.6	6.79	0.84	14.68	16.94	7.48	2.49	829	148	829	8.58	3.67	0.81	160.24	36.39	1015.19	1164		
	LTB 98-99	0.00808	0.003	0.15	0.00013	228	171	171	102.6	6.67	0.83	14.72	16.98	6.13	2.04	829	148	829	8.42	3.67	0.30	163.21	36.38	2749.27	1164		
	LTB 99-100	0.00808	0.003	0.15	0.00013	228	171	171	102.6	9.78	1.21	10.27	11.85	5.83	1.94	829	148	829	8.01	1.75	0.14	171.17	78.75	5712.11	2520		
	LTB 100-101	0.00988	0.0048	0.15	0.00014	228	171	171	102.6	33.29	3.37	10.08	10.80	7.79	1.62	1014	160	1014	10.35	5.15	0.93	159.96	29.01	1080.24	928		
	LTB 101-102	0.00988	0.0048	0.15	0.00014	228	171	171	102.6	17.8	1.80	39.39	42.20	11.16	2.33	1014	160	1014	3.66	13.22	2.14	466.97	9.09	466.58	291		

Appendix 21.1.4-3 (1/5)
CAPACITY-DEMAND RATIO OF ABUTMENT FOUNDATION

ABUTMENT ON TIMBER PILES

Section Properties

Section Dimensions

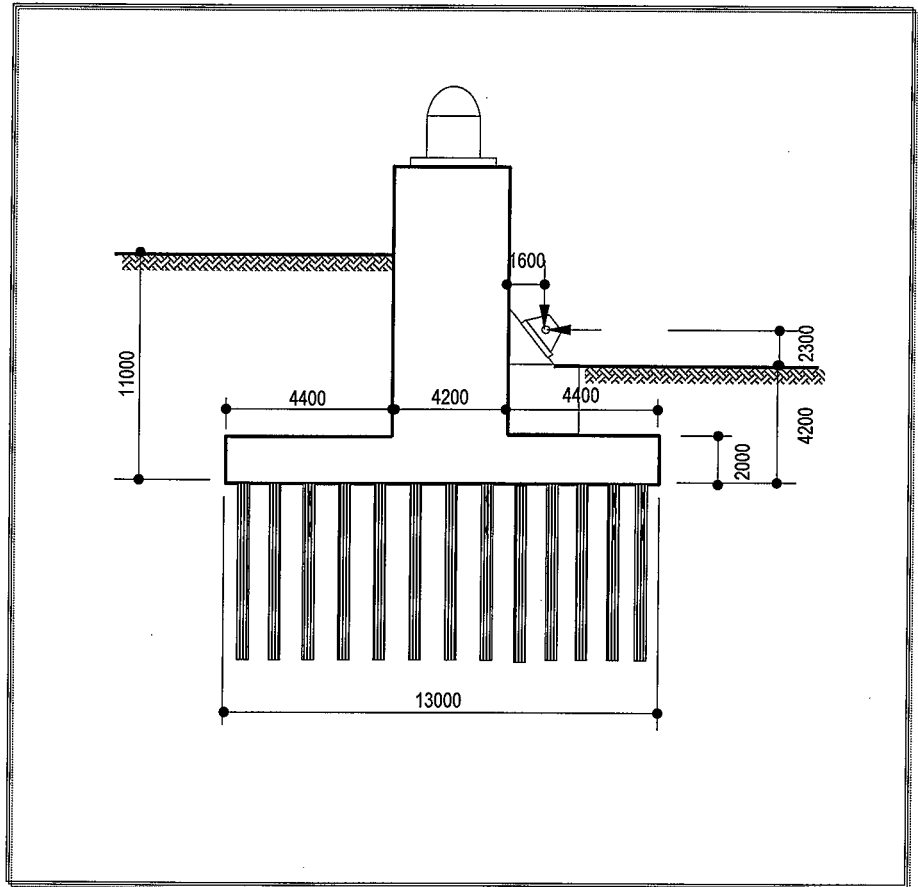
Footing Foundation

- base thickness (T_f) = 2000 mm
- length of base (L_b) = 13000 mm
- length of toe (L_t) = 4400 mm
- length of heel (L_h) = 4400 mm

Height of Soil

- Passive Soil (H_{ps}) = 4200 mm
- Active Soil (H_{as}) = 11000 mm

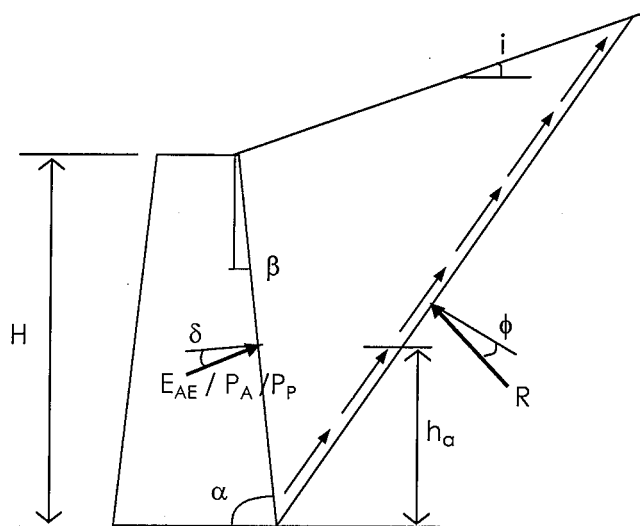
Length of Abut. (L) = 27000 mm



Abutment Diagram Detail

Appendix 21.1.4-3 (2/5)
CAPACITY-DEMAND RATIO OF ABUTMENT FOUNDATION

Earth Pressures



Earth Pressure Parameters

The earth pressures are calculated based on the following :

- (1) for the dynamic earth pressure due to earthquake, the psuedo-static Mononobe-Okabe formula is used, and
- (2) for the static earth pressures, the Coulomb equation for both the active and passive earth pressures are used.

(a) COMBINED STATIC AND SEISMIC EARTH PRESSURE (Mononobe-Okabe Principle)

$$P_{AE} = 1/2 \gamma H^2 (1-K_v) K_{AE}$$

$$K_{AE} = \frac{\cos^2 (\phi - \theta - \beta)}{\phi \cos \theta \cos^2 \beta \cos (\delta + \beta + \theta)} \quad \phi = \left[1 + \sqrt{\frac{\sin (\phi + \delta) \sin (\phi - \theta - i)}{\cos (\delta + \beta + \theta) \cos (i - \beta)}} \right]^2$$

K_{AE} has two components (seismic and static). The seismic component ($K_{AE} - K_A$) is assumed to be an inverted triangular force (max. at the ground surface) acting at a height of 0.6H.

(b) STATIC ACTIVE EARTH PRESSURE

$$P_A = 1/2 \gamma H^2 K_A$$

$$K_A = \frac{\sin^2 (\alpha + \phi)}{\phi \sin^2 \alpha \sin (\alpha - \delta)} \quad \phi = \left[1 + \sqrt{\frac{\sin (\phi + \delta) \sin (\phi - i)}{\sin (\alpha - \delta) \sin (\alpha + i)}} \right]^2$$

The K_A component is the familiar triangular distribution acting at H/3.

(c) STATIC PASSIVE EARTH PRESSURE

$$P_p = 1/2 \gamma H^2 K_p$$

$$K_p = \frac{\sin^2 (\alpha - \phi)}{\phi \sin^2 \alpha \sin (\alpha + \delta)} \quad \phi = \left[1 - \sqrt{\frac{\sin (\phi + \delta) \sin (\phi + i)}{\sin (\alpha + \delta) \sin (\alpha + i)}} \right]^2$$

where :

- γ = unit weight of soil
- H = height of soil face
- ϕ = angle of friction of soil
- θ = arc tan ($k_h / 1 - k_v$)
- δ = angle of friction between soil and abutment
- P_{AE} = total force (active and seismic)
- P_A = static active earth presure
- P_p = static passive earth presure

- k_h = horizontal acceleration coefficient
- k_v = vertical acceleration coefferient
- i = backfill slope angle
- β = slope of soil face
- α = slope of wall

Appendix 21.1.4-3 (3/5)
CAPACITY-DEMAND RATIO OF ABUTMENT FOUNDATION

Soil Parameters

The following data are used to calculate the earth pressures based on the above formula :

$A = 0.40$ (seismic acceleration coefficient)
 $K_h = 0.5A$ (K_h is taken as $1/2 A$)
 $K_h = 0.20$ (horizontal acceleration coefficient)
 $K_v = 0.00$ (vertical acceleration coefficient)

Check horizontal acceleration coefficient (AASHTO C6.3.2 1-A)

$K_h = \leq (1 - K_v) \tan (\phi - i)$
 $0.20 \leq 0.58$ **OK!**

ϕ	30.0	deg.	
θ	11.31	deg.	
β	0	deg.	* vertical wall
α	90	deg.	* vertical wall
δ	22.5	deg.	
i	0	deg.	* horizontal backfill
γ_s	18.00	KN/m ³	(Unit Wt. of Soil)
γ_c	24.50	KN/m ³	(Unit Wt. of Concrete)
h	600.00	mm	* equiv. height for surcharge

Calculated Pressures and Forces

The earth pressure coefficients and earth pressures calculated based on the above formula are:

Seismic Active Earth Pressure

ϕ	2.412	
K_{AE}	0.457	
P_{AE}	332.83	KN/m (for stem)
P_{AE}	497.19	KN/m (for wall)

Static Active Earth Pressure

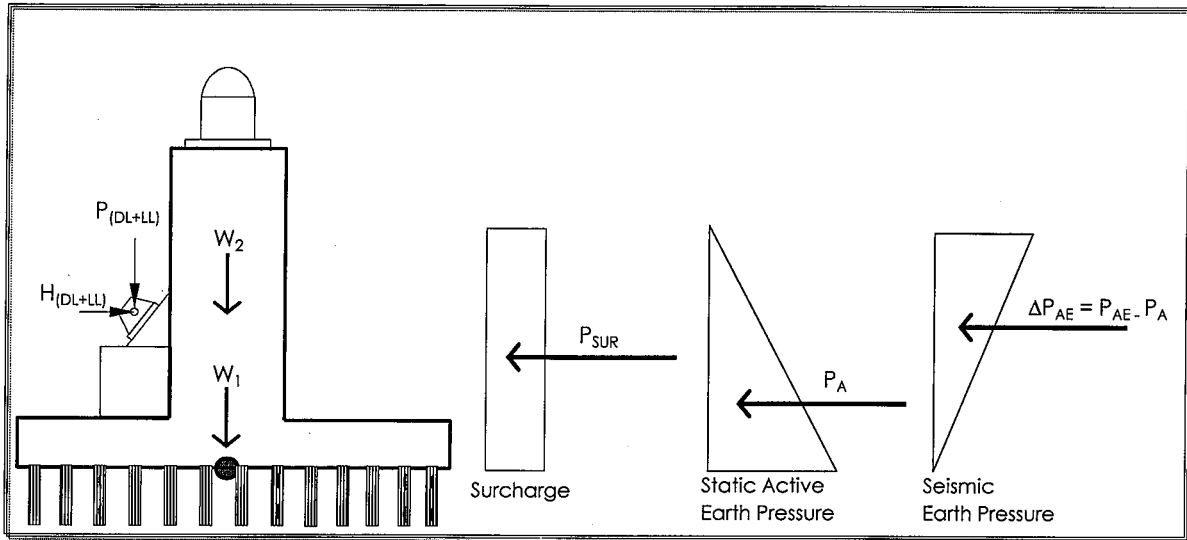
ϕ	2.740	
K_A	0.296	
P_A	215.99	KN/m (for stem)
P_A	322.66	KN/m (for wall)

Static Passive Earth Pressure

ϕ	0.119	
K_p	6.830	
P_p	1084.41	KN/m

Appendix 21.1.4-3 (4/5)
CAPACITY-DEMAND RATIO OF ABUTMENT FOUNDATION

Design Loads



Forces on Abutment

The horizontal and vertical forces at different load conditions are presented in Table 1.1 and the load combinations shown in Table 1.2.

Table 1.1 Forces on Abutment

Forces		F_v (kN)	F_h (kN)	x (m)	y (m)	$F_v x$ (kN-m)	$F_h y$ (kN-m)
1.0 Dead Load (DL)							
Abutment	W_1	18860.24		0.00		0.00	
Footing	W_2	16848.00		0.00		0.00	
superstructure	P_{DL}	12790.00		3.70		47323.00	
soil@heel	P_{soil}	19224.00		-4.30		-82663.20	
soil@toe	P_{soil}	4698.00		4.30		20201.40	
active	P_A		8711.79		3.67		31943.24
passive	P_P		29278.98		-1.40		-40990.58
3.0 Live Load (LL)							
surcharge	P_{SUR}	1283.04	950.38	-4.30	5.50	-5517.07	5227.08
superstructure, vert	P_{LL}	2209.00		3.70		8173.30	
superstructure, hor	H_{LL}		3269.00		6.50		21248.50
4.0 Earthquake (EQ)							
seismic earth pressure	ΔP_{AE}		4712.27		6.60		31100.96
seismic-wall	$k_h W_1$		3772.05		5.50		20746.26
superstructure	H_{EQ}		5614.00		6.50		36491.00
5.0 Buoyancy (B)							
buoyant force	B_F	0.00		0.00		0.00	

Table 1.2 Load Combinations (Service Load)

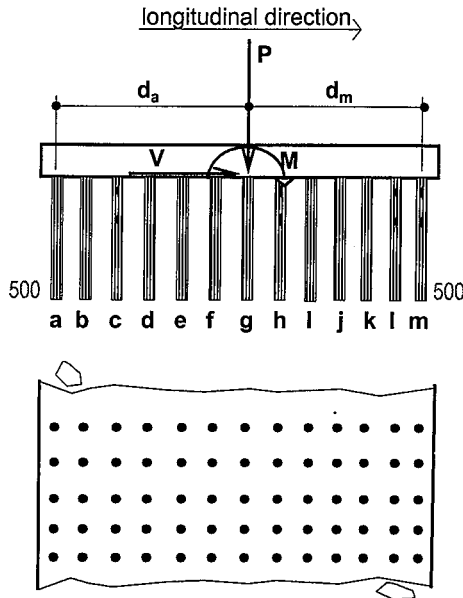
Load Case	ΣF_v (kN)	ΣF_h (kN)		ΣRM	ΣOM
CASE I (Normal Stage)	51990.28	42210.15		12482.57	17428.23
CASE VII (E.Q. Stage)	72420.24	22810.11		15138.80	120281.46

where : CASE I (N. Stage) = DL + LL+I + B + E
CASE VII (E.Q. Stage) = DL + E + B + EQ

CAPACITY-DEMAND RATIO OF ABUTMENT FOUNDATION

PROJECT TITLE: PASIG-MARIKINA RIVER BR. INSPECTION, QUEZON BRIDGE
 ITEM : ANALYTICAL ASSESSMENT OF BRIDGE STRUCTURAL SOUNDNESS

TIMBER PILE REACTIONS (A1 AND A2)



Forces	Case I (Old Code)	Case VIIa (New Code)	Units
P	51990.3	72420.2	KN
M	29910.81	105142.66	KN-m
V	42210.15	22810.11	KN

FOOTING DIMENSIONS:

S = 13.00 m
 L = 27.00 m

$$P_p = P_t / n + Mx / \Sigma x^2 \quad \text{-- Gen. Equation}$$

where :

- P_p = load per pile (KN or Tons)
- M = moment about axis considered (KN-m)
- x = distance from axis to any pile (m.)
- Σx^2 = summation of moment of Inertia

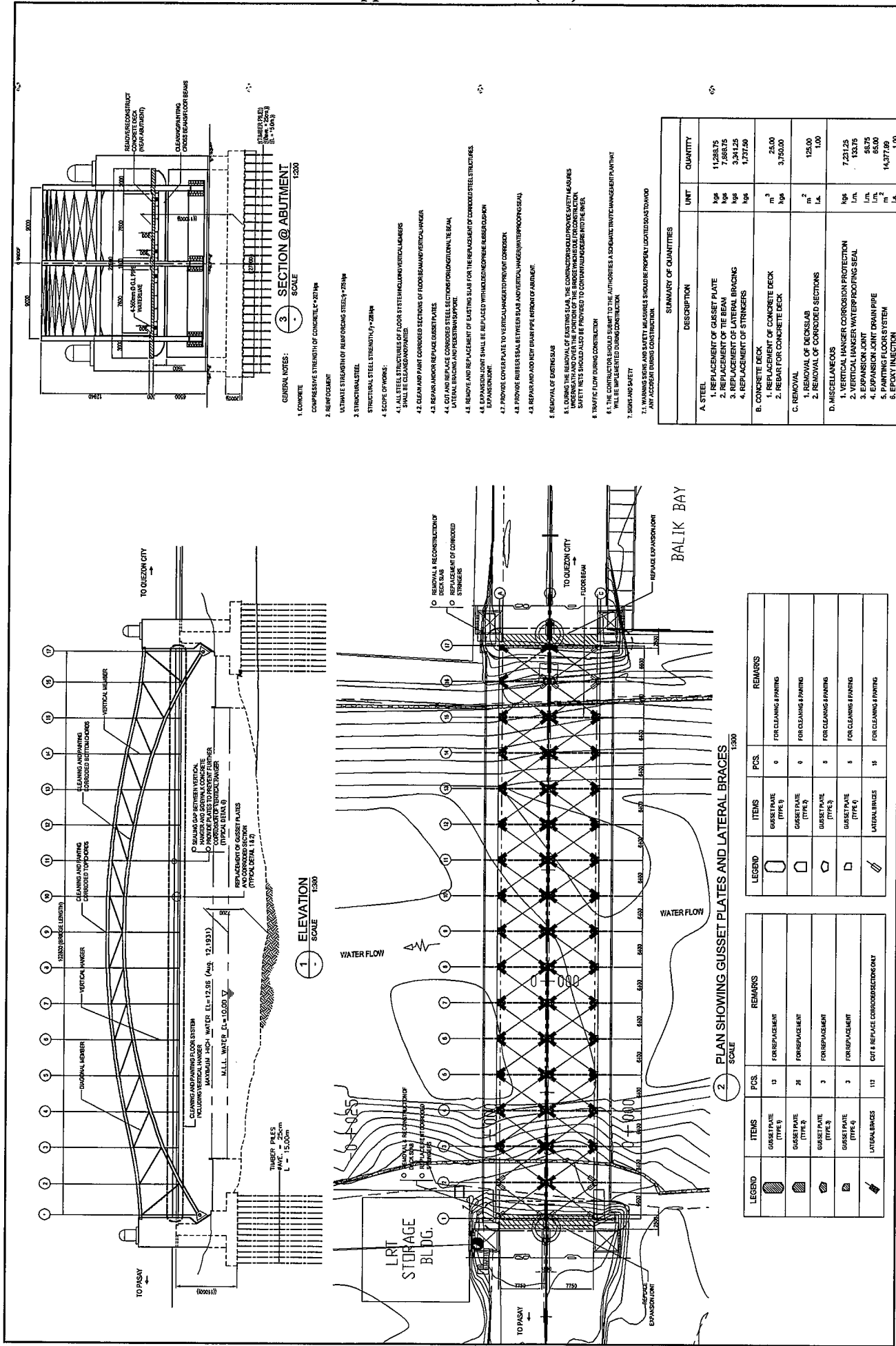
Location	No. of Piles (n)	Pile Distance (x)	nx^2	Case I (KN)	Case I (tons)	Case VIIa (KN)	Case VIIa (tons)
At row a'	0	7.75	0	195.29	19.91	372.15	
At row a	27	6.00	972	184.64	18.82	334.70	34.12
At row b	27	5.00	675	178.55	18.20	313.31	31.94
At row c	27	4.00	432	172.47	17.58	291.91	29.76
At row d	27	3.00	243	148.12	15.10	270.52	27.58
At row e	27	2.00	108	148.12	15.10	249.12	25.39
At row f	27	1.00	27	148.12	15.10	227.72	23.21
At row g	27	0.00	0	148.12	15.10	206.33	21.03
At row h	27	-1.00	27	148.12	15.10	184.93	18.85
At row i	27	-2.00	108	148.12	15.10	163.53	16.67
At row j	27	-3.00	243	148.12	15.10	142.14	14.49
At row k	27	-4.00	432	148.12	15.10	120.74	12.31
At row l	27	-5.00	675	148.12	15.10	99.34	10.13
At row m	27	-6.00	972	148.12	15.10	77.95	7.95
At row m'	0	-7.75	0	148.12	15.10		

Total n = 351 $\Sigma x^2 = 4914$

	Case I		Case VIIa
P_{max} =	-19.91 tons ... <i>Compression</i>	0.00	tons ... <i>Tension</i>
P_{min} =	15.10 tons ... <i>Tension</i>	0.00	tons ... <i>Tension</i>

Allowable Timber Pile Capacity = 20 tons
 Ultimate Timber Pile Capacity = 24 tons

SUMMARY OF CAPACITY / DEMAND RATIO			
Location	Capacity	Demand	C / D
	(tons)	(tons)	
(Old Code)Case I, P_a	20	19.91	1.00
(New Code) Case VII, P_a	24	34.12	0.70



SECTION @ ABUTMENT
SCALE 1:200

- GENERAL NOTES:
1. CONCRETE
COMPRESSIVE STRENGTH OF CONCRETE $f_c = 28.7 \text{ MPa}$
 2. REINFORCEMENT
ULTIMATE STRENGTH OF REINFORCING STEEL $f_y = 729 \text{ MPa}$
 3. STRUCTURAL STEEL
STRUCTURAL STEEL STRENGTH $f_y = 276 \text{ MPa}$
 4. SCOPE OF WORK:
4.1. ALL STEEL STRUCTURES OF FLOOR SYSTEM AND VERTICAL MEMBERS SHALL BE CLEAN AND PAINT.
4.2. CLEAN AND PAINT CORRODED SECTIONS OF FLOOR BEAM AND VERTICAL MEMBER.
4.3. REPAIR AND/OR REPLACE GUSSET PLATES.
4.4. CUT AND REPLACE CORRODED STEEL SECTIONS FOUND ON COLUMN, TIE BEAM, LATERAL BRACING AND FEEDER TRANSUPPORT.
4.5. REMOVE AND REPAIR EXISTING SLAB FOR THE REPLACEMENT OF CORRODED STEEL STRUCTURES.
4.6. EXPANSION JOINT SHALL BE REPLACED WITH UNIDIRECTIONAL REINFORCEMENT.
4.7. PROVIDE CONCRETE TO VERTICAL MEMBERS UP TO CORROSION.
4.8. PROVIDE RUBBER SEAL BETWEEN SLAB AND VERTICAL MEMBER (WATERPROOFING SEAL).
4.9. REPAIR AND NEW DRAIN PIPE WITHIN OF ABUTMENT.
5. REMOVAL OF EXISTING SLAB
5.1. DURING THE REMOVAL OF EXISTING SLAB, THE CONTRACTOR SHALL PROVIDE SAFETY GUARDS UNDERneath AND OVER THE PORTION OF THE BRIDGE WHERE THE CONSTRUCTION SAFETY NETS SHOULD BE PROVIDED TO CONTAIN DEBRIS INTO THE RIVER.
6. TRAFFIC FLOW DURING CONSTRUCTION
6.1. THE CONSTRUCTION SHOULD SUBMIT TO THE AUTHORITIES A SCHEMATIC TRAFFIC MANAGEMENT PLAN THAT WILL BE IMPLEMENTED DURING CONSTRUCTION.
7. SURVEY AND SAFETY
7.1. SURVEYING AND SAFETY MEASURES SHOULD BE PROPERLY LOCATED SO AS TO AVOID ANY ACCIDENT DURING CONSTRUCTION.

SUMMARY OF QUANTITIES		
DESCRIPTION	UNIT	QUANTITY
A. STEEL		
1. REPLACEMENT OF GUSSET PLATE	Kg	11,287.75
2. REPLACEMENT OF THE BEAM	Kg	7,888.75
3. REPLACEMENT OF LATERAL BRACING	Kg	3,341.25
4. REPLACEMENT OF STRINGERS	Kg	1,737.50
B. CONCRETE DECK		
1. REPLACEMENT OF CONCRETE DECK	m ³	25.00
2. REPAIR FOR CONCRETE DECK	Kg	3,750.00
C. REMOVAL		
1. REMOVAL OF DECK SLAB	m ²	125.00
2. REMOVAL OF CORRODED SECTIONS	Lt.	1.00
D. MISCELLANEOUS		
1. VERTICAL HANGER CORROSION PROTECTION	Kg	7,231.25
2. VERTICAL HANGER WATERPROOFING SEAL	Lt.	133.75
3. EXPANSION JOINT DRAIN PIPE	Lt.	58.75
4. PAINTING FLOOR SYSTEM	Lt.	85.00
5. EPOXY INJECTION	Lt.	14,377.00
		1.00

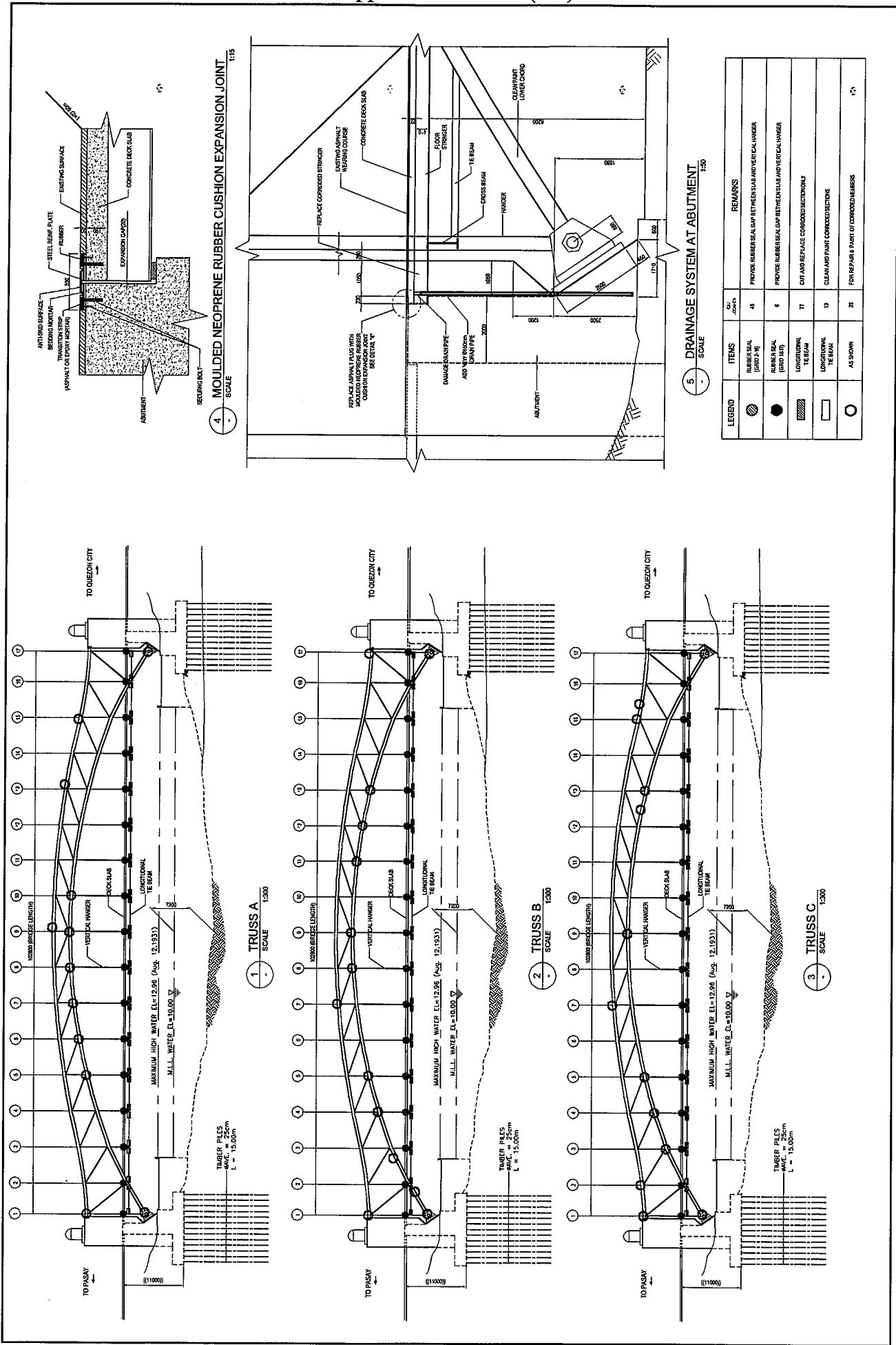
ELEVATION
SCALE 1:300

PLAN SHOWING GUSSET PLATES AND LATERAL BRACES
SCALE 1:300

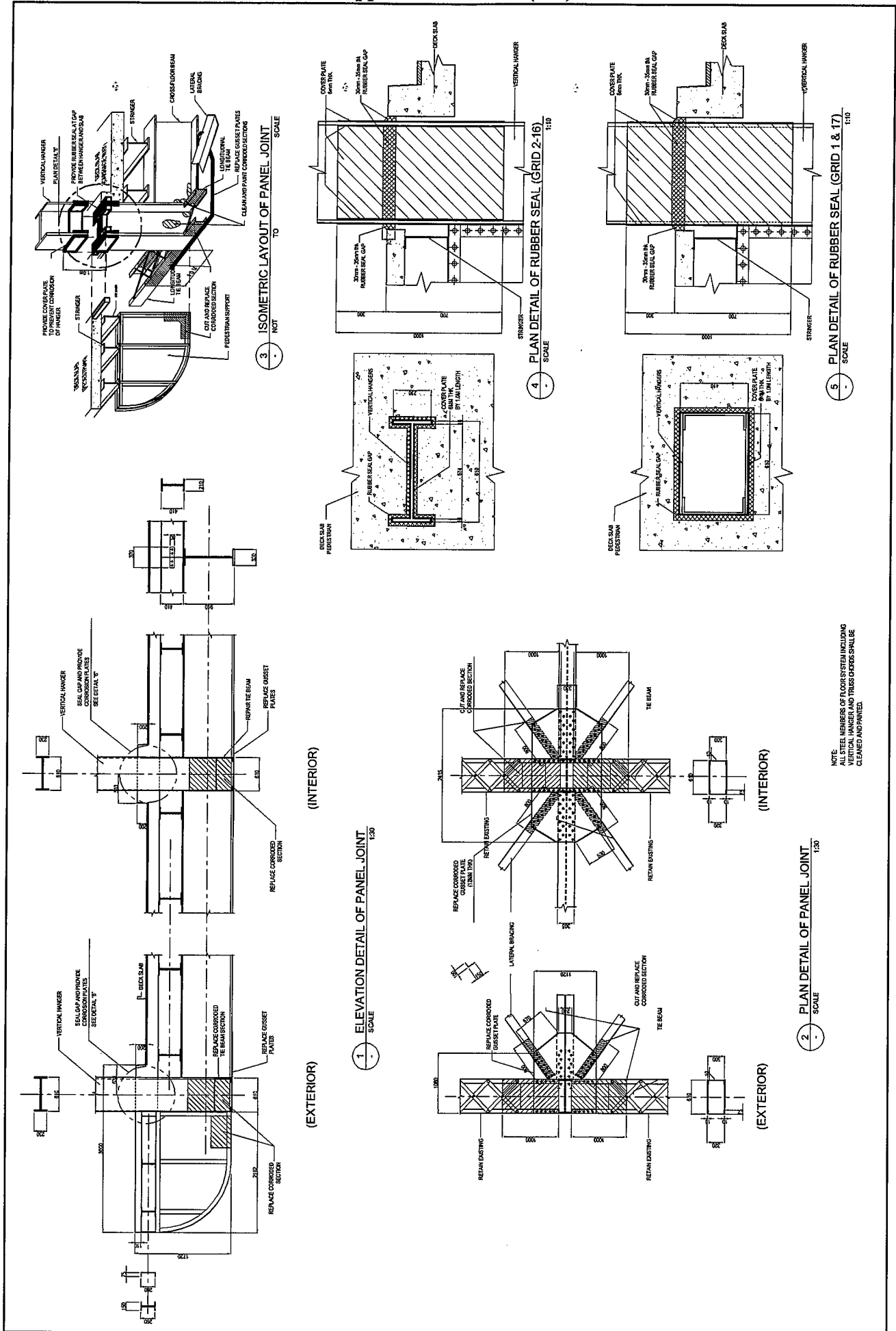
LEGEND	ITEMS	PCS	REMARKS
	GUSSET PLATE (TYPE 1)	0	FOR CLEANING PAINTING
	GUSSET PLATE (TYPE 2)	9	FOR CLEANING PAINTING
	GUSSET PLATE (TYPE 3)	5	FOR CLEANING PAINTING
	GUSSET PLATE (TYPE 4)	5	FOR CLEANING PAINTING
	LATERAL BRACES	15	FOR CLEANING PAINTING

LEGEND	ITEMS	PCS	REMARKS
	GUSSET PLATE (TYPE 1)	13	FOR REPLACEMENT
	GUSSET PLATE (TYPE 2)	24	FOR REPLACEMENT
	GUSSET PLATE (TYPE 3)	3	FOR REPLACEMENT
	GUSSET PLATE (TYPE 4)	3	FOR REPLACEMENT
	LATERAL BRACES	113	CUT & REPLACE CORRODED/CRACKING

QUEZON BRIDGE PROPOSED REHABILITATION

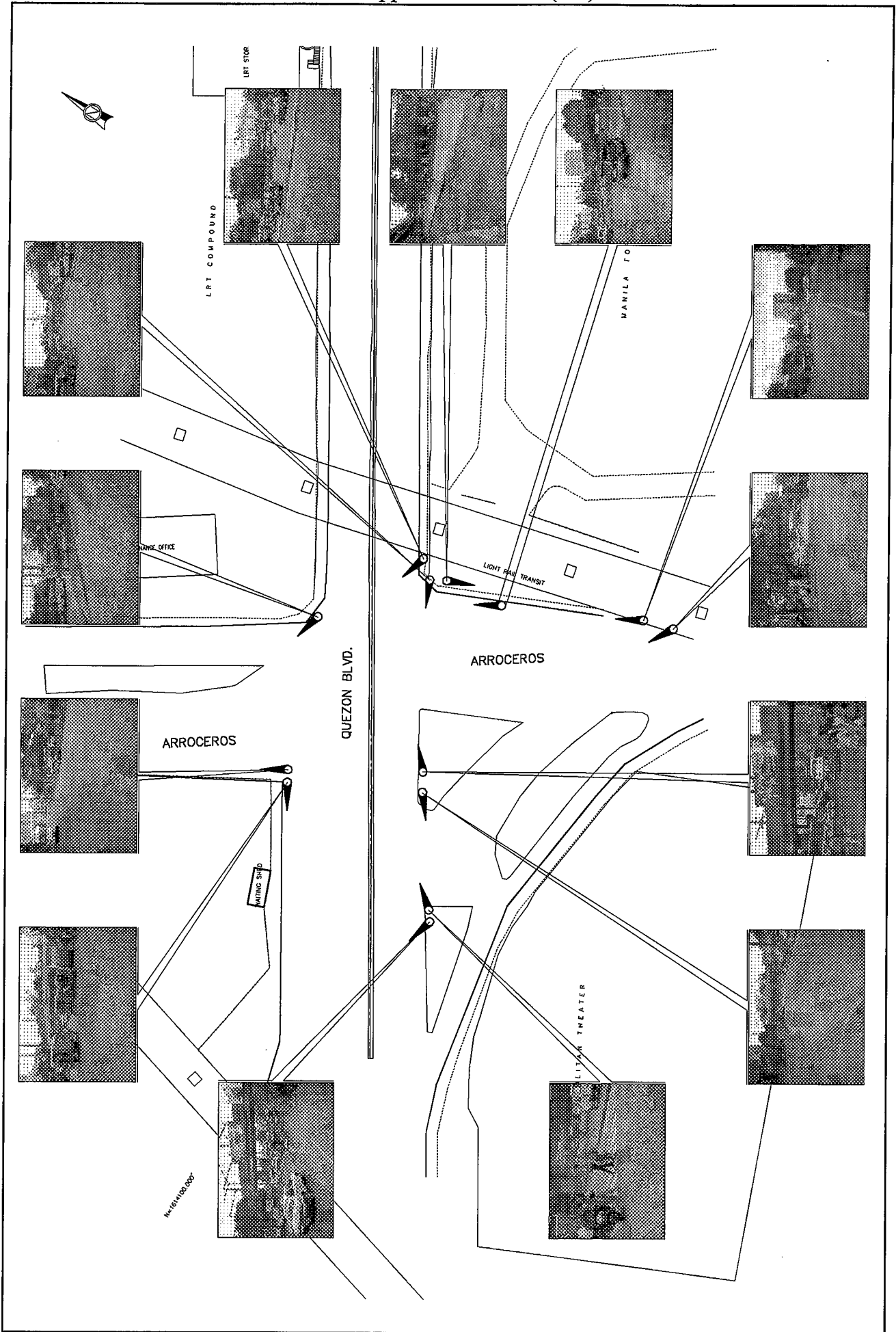


QUEZON BRIDGE PROPOSED REHABILITATION

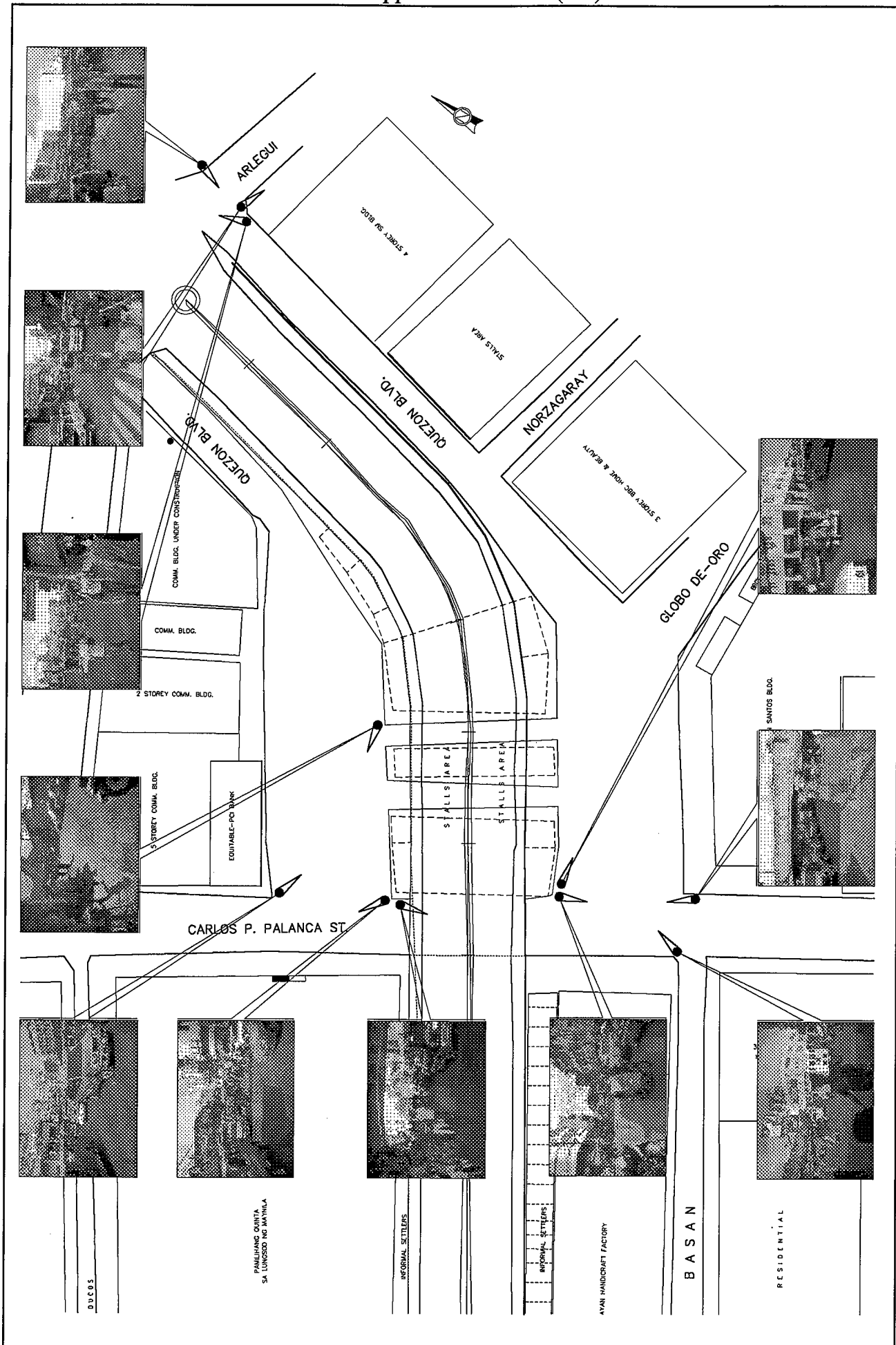


NOTE: ALL STEEL MEMBERS OF FLOOR SYSTEM INCLUDING VERTICAL HANGER AND TRUSS JACKS SHALL BE CLEANED AND PAINTED.

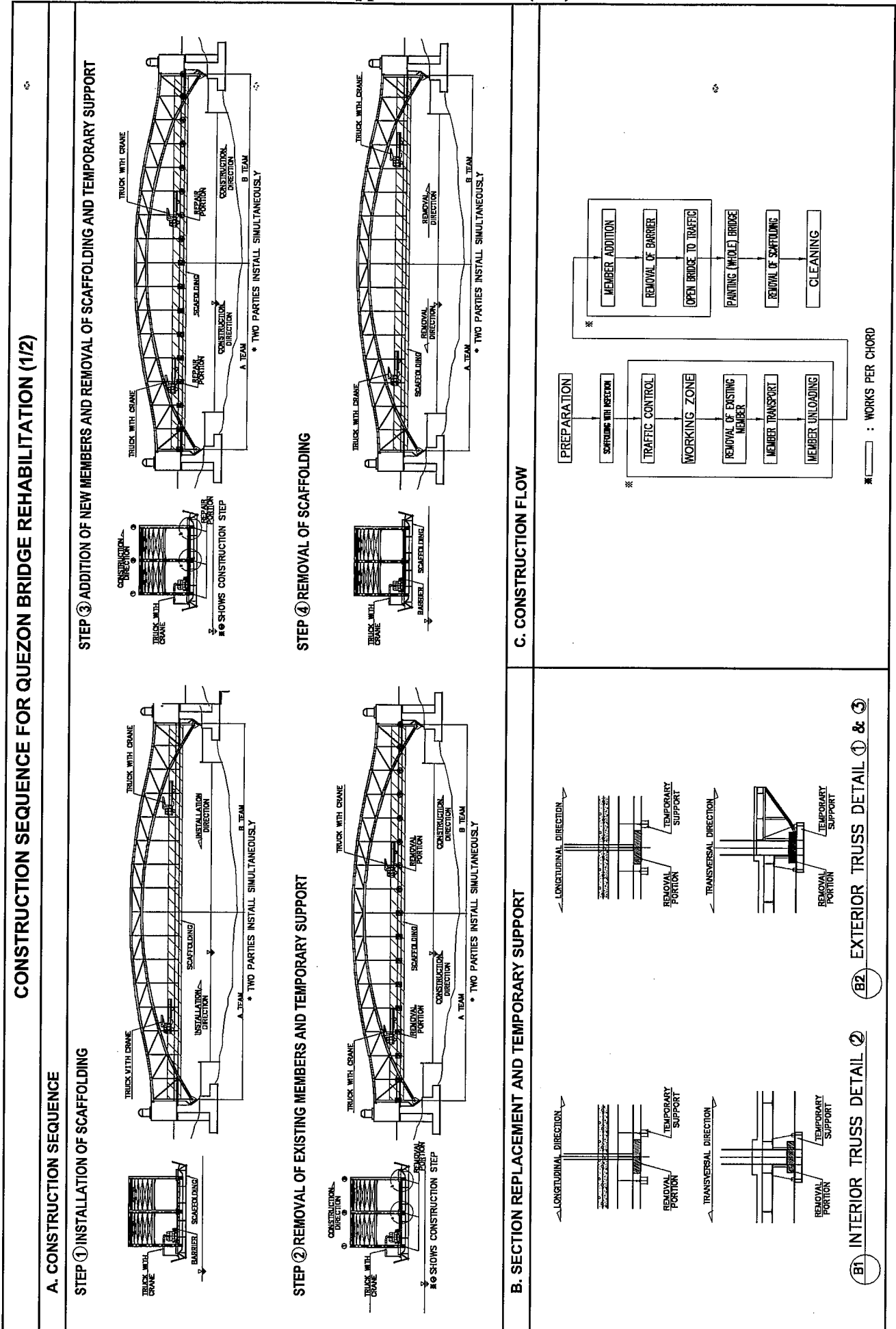
QUEZON BRIDGE PROPOSED REHABILITATION



APPROACH 1 SITE OCULAR INSPECTION (QUEZON BRIDGE)

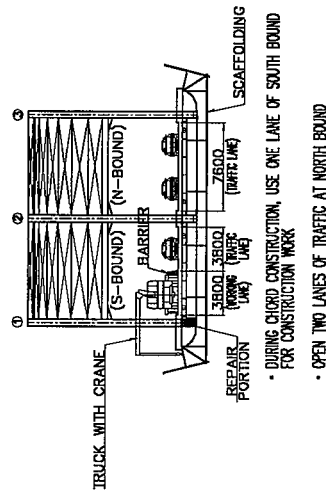


APPROACH 2 SITE OCULAR INSPECTION (QUEZON BRIDGE)

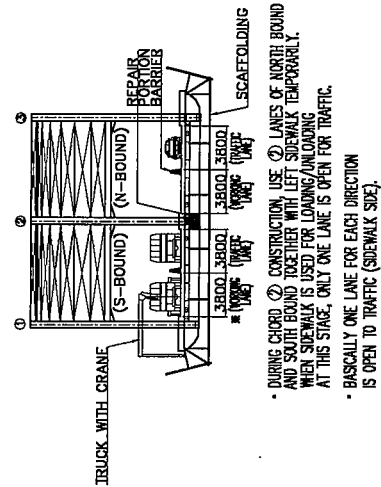


CONSTRUCTION SEQUENCE FOR QUEZON BRIDGE REHABILITATION (2/2)

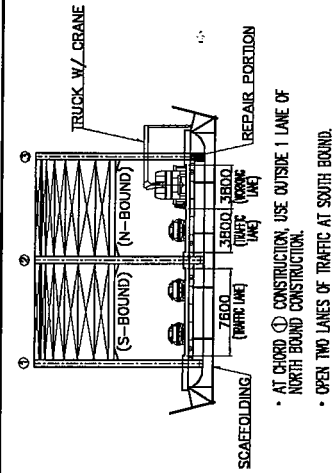
D. TRAFFIC MANAGEMENT DURING CONSTRUCTION



D1 TRUSS CHORD 1 CONSTRUCTION

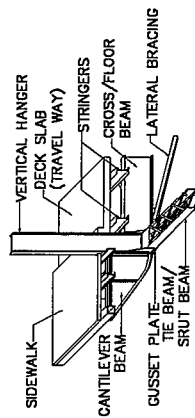


D2 TRUSS CHORD 2 CONSTRUCTION

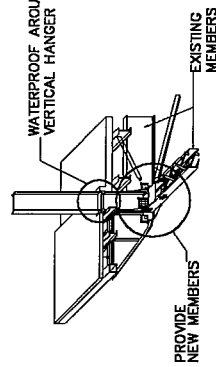


D3 TRUSS CHORD 3 CONSTRUCTION

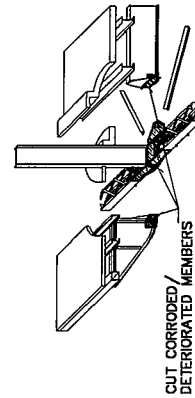
E. DETAILS OF REPAIR WORKS



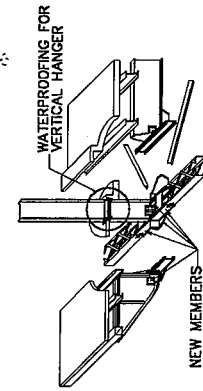
E1 PRESENT CONDITION



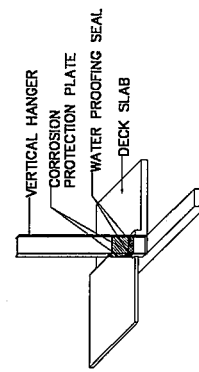
E2 CONDITION AFTER REPAIR



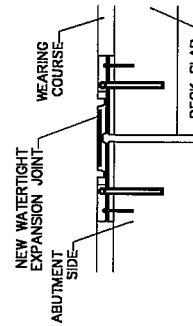
E3 CUTTING OF DETERIORATED MEMBER/PARTS



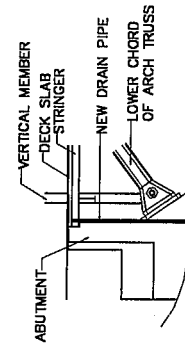
E4 REPLACEMENT OF NEW MEMBER



E5 VERTICAL HANGER WATER PROOFING



E6 EXPANSION JOINT DETAILS
PROVIDE WATERPROOF AT EXPANSION JOINT



E7 PROVISION OF NEW DRAIN PIPE
PREVENT WATER LEAKAGE TO THE ARCH SUPPORT

Construction Cost for Rehabilitation of Quezon Bridge

Description	Unit	Quantity	Unit Price	Cost	Components		Taxes
					Foreign	Local	
A. Steel Structures (Fabricate & Transport)							
Steel I Girder	kgs	24,217.00	150.00	3,632,550.00	2,688,087.00	399,580.50	544,882.50
B. Scaffoldings/Temporary Works							
Scaffoldings/Temporary Works	sq. m.	2,625.00	1,200.00	3,150,000.00	2,142,000.00	567,000.00	441,000.00
C. Siteworks							
Bridge Survey at Site	days	120.00	83,000.00	9,960,000.00	7,370,400.00	1,095,600.00	1,494,000.00
Removal of Existing Member	place	45.00	200,000.00	9,000,000.00	6,660,000.00	990,000.00	1,350,000.00
Installation	place	45.00	134,000.00	6,030,000.00	4,462,200.00	663,300.00	904,500.00
Section Repair	place	180.00	50,000.00	9,000,000.00	6,660,000.00	990,000.00	1,350,000.00
Painting Works	sq. m.	8,820.00	4,020.00	35,456,400.00	26,237,736.00	3,900,204.00	5,318,460.00
Expansion Joint	l.m.	58.75	130,000.00	7,637,500.00	5,651,750.00	840,125.00	1,145,625.00
Expansion Joint Drain Pipe	l.m.	65.00	950.00	61,750.00	45,695.00	6,792.50	9,262.50
Epoxy Injection	l.s.	1.00	1,250,000.00	1,250,000.00	925,000.00	137,500.00	187,500.00
			Sub-total	78,395,650.00	58,012,781.00	8,623,521.50	11,759,347.50
D. Total Direct cost							
				85,178,200.00	62,842,868.00	9,590,102.00	12,745,230.00
E. Indirect Cost							
Traffic Management							
Temporary Facilities							
Mobilization/Demobilization							
40% of Total Direct Cost				34,071,280.00	25,137,147.20	3,636,040.80	5,098,092.00
F. Total Construction Cost							
				119,249,480.00	87,980,015.20	13,426,142.80	17,843,322.00

Roadway Improvement Cost (Quezon Bridge)

Item No.	Description	Unit	Quantity	Unit Cost	Amount	Component		Tax
						Foreign	Local	
Earthworks								
101(3)a	Removal of Island	m ³	42.80	114.15	4,885.49	3,175.57	1,025.95	683.97
Miscellaneous								
	Concrete Median	m ²	506.00	272.93	138,103.59	89,767.33	29,001.75	19,334.50
600(1)	Concrete Curb	l.m.	170.00	562.46	95,618.71	62,152.16	20,079.93	13,386.62
612(1)	Pavement Markings	m ²	146.00	862.13	125,870.25	81,815.66	26,432.75	17,621.84
	Contingencies	l.s.	1.00	18,223.90	18,223.90	13,667.93	2,733.59	1,822.39
Total					382,701.95	250,578.65	79,273.97	52,849.32
% Component					100%	65%	21%	14%