PART II MASTER PLAN

CHAPTER 7

BRIDGE CONDITION SURVEY AND DESIGN PRESUMPTION

Appendix 7.1.1-1 (1/34)

VISUAL INSPECTION REPORT (2/5) SUMMARY

	idae -		>				Date of Inspection	·	Dec. 13, 14, 19 CT Arenas		
ame of Br	idge : j	Pa1.1 Delpan Bridge (Upstrea	<u>am)</u>				Checker		LG Sta. Maria		
Damage No.	Span No,	Name of Member	Type of Damage	Rank of Damage	Nature	De Location/	scription of Damag		No. of	Remarks	
				ļ	Painting	Pattern Left and right side	Scale	Severity	Damages	(a, b, c)	No
1	1	RAILING (A1)	PD	S	deterioration	Left and right side	Small	Smalt	5 meters each side	c	-
2	1	RAILING (A1)	FR	M	Fracture Painting	railings	Remarkable	Medium	2 Locations	b	-
3	1	RAILING @ Center	PD	S	deterioration	Left and right side railings	Small	Smali	3 m. each side	c	-
4 ·	1	RAILING @ Center	FR	м	Fracture	Left and right side railings	Remarkable	Medium	2 Locations	b	-
5	1	RAILING (P1)	PD	S	Painting deterioration	Left and right side railings	Small	Small	4 m. each side	c	-
6	1	RAILING (P1)	FR	М	Fracture	Left and right side railings	Remarkable	Medium	2 Locations	b	-
7 8	1	CURB	CR SER	M	Crack Spalling	Median curb Right sidewalk	0.35 mm	Medium	1 Location	b	<u> </u>
9	1	PAVEMENT (Abutment)	CRPL	M	Local bump	Abutment A1,	0.25 m ² Concave	Medium	1 Location	b	
10	1	· · · · · · · · · · · · · · · · · · ·		·		Carriageway Asphalt concrete dec	>30 mm	Medium	1 Location	b	
11	1	PAVEMENT @ Center	CRPL	M	Crecks	overlav Pavement along Pier	.6 mm	Medium	Малу	b	<u> </u>
		PAVEMENT (P1)	CRPL	M	Cracks Spalling with	No, 1 Bottom of Girder at	6 mm	Medium	Many	b	
12	1.	BOX GIRDER	SER	н	Exposed Rebars	Abutment A1	0.50 m ²	High	1 Location	а	1, 1a
13	1	BOX GIRDER	HC	Н	Honeycombs	Bottom of Girder, Mic Span	>2.0 m	High	1 Location	a	2
14	1	BOX GIRDER	SER	н	Exposed rebars	Bottom of box girder	3.85 m ² Hairline cracks	High	Wide area	а	3,
15	1	BOX GIRDER	CR	S	Crack	Bottom of box girder Right side drain hole	0.13 mm	Small	Many	b	· ·
16	1	DRAINAGE SYSTEM (A1)	WLC	М	Clogging	Right drain hole at mic	Cioggea	Medium	Typical	b	5
17	1	DRAINAGE SYSTEM CL	WLC	М	Clogging	Right drain hole at mic	Cidgged	Medium	Typical	b	-
18	1	DRAINAGE SYSTEM P1	WLC	M	Clogging	span	Cioggeo	Medium	Typical	b	· ·
19	1	BEARING SHOE (A1)	co	M	Corrosion	Abutment A1	Spread over Whole shoe	Medium	All bearing shoes	b	6
20	1	BEARING SHOE (P1)	со	м	Corrosion	Pier No. 1	Spread over Whole shoe	Medium	All bearing shoes	b	7
21	1	ABUTMENT	D/D	м	Discoloration	Abutment A1	Whole abutment wall and backwall	Medium	1 Location	b	8
22	2	RAILING (P1)	PD	S	Painting deterioration	Left and right side railings	Small	Small	15 m each side	с	-
23	2	RAILING (P1)	FR	м	Fracture	Left and right side railings	Remarkable	Medium	1 Locaiion	b	-
24	2	RAILING (P2)	PD	s	Painting deterioration	Left and right side railings	Small	Small	15 m each side	с	
25	2	SIDEWALK	FR	М	Fracture	Right Sidewalk	Remarkable	Medium	1 Location	b	-
26	2	PAVEMENT @ Center	CRPL	M	Cracks	Asphalt Concrete Overlav	5 mm	Medium	Many	b	
27	2	PAVEMENT (P2)	CRPL	м	Cracks	Asphall Concrete	8 mm	Medium	Many	b	<u> </u>
28	2	DECK SLAB (sidewalk bottom)	CR	S	Cracks	Bottom of Sidewalk,	0.05 mm	Small	3 Locations	b	-
29	2	SIDE FACE OF WEB	SER	M	Spalling	Right Side Sideface of box girder	0.15 m ²	Medium	1 Location	b	-
30	2	BOTTOM OF BOX GIRDER	CR/SER	S/M	Cracks & Spall	web Bottom of box girder	Hairline cracks	Small to Medium	Many	b	9,10,
31		BOTTOM OF BOX GIRDER	FL/CR	S	Freelime & Cracks	Bottom of box girder	0.23 mm 0.05 mm	Small			
32		BEARING SHOE	co	s	Corrosion	Pier No. 2	Traces of corrosion on	Small	Many	b	12, 1
33		BEARING SHOE (PAD)	SER	S	Spalling	Pier No. 2, Concrete	steel bearing shoes	Small	Typical	b	14
34		PIER BODY	CR		Cracks	Riser Pier No. 2, Wall	0.08 m ²		1 Location	b	
35		FOUNDATION (CAP)	SER	M	Exposed rebars			Small to Medium	Many	b	15,1
36		RAILING (GERBER)	PD	S	Paint deterioration	Footing cap Left and right side	1.5 m²	Medium	2 locations 20 meters each	b	17, 1
37		RAILING @ Center	PD PD	S		railing Mid-span, left and	Small	Small	side	c	-
38		RAILING @ Center	FR		Paint deterioration	right side Mid-span, left and	Small	Small	20 m each side	C	-
39		RAILING (GERBER)	PD		Fracture	right side Left and right side	Remarkable	Medium	2 Locations	b	-
40		····· · · · · · · · · · · · · · · · ·		s	Paint deterioration	railings Left and right side	Small	Small	15 m each side	c	-
		RAILING (GERBER)	FR	M	Fracture	railings Railing over	Remarkable	Medium	2 Locations	b	-
41		RAILING (P3)	PD	S	Paint deterioration	pier No. 3	Small	Small	2 meters each side	C ·	-
42		RAILING (P3)	FR	М	Fracture	Railings left and right side	Remarkable	Medium	1 Location	b	-
43		SIDEWALK	SER	M	Spalling	Right sidewalk	0.30 m ²	Medium	2 Locations	b	
44		PAVEMENT (P2)	CRPL	М	Cracks	Asphalt concrete overlav	6 mm	Medium	Many	b	-
45		PAVEMENT (GERBER)	CRPL	М	Cracks	Asphalt concrete overlay	>5 mm	Medium	Many	b	-
46	3	PAVEMENT (GERBER)	CRPL	м	Cracks	Asphall concrete overlav	8 mm	Medium	Many	b	
47	3	PAVEMENT (P3)	CRPL	м	Cracks	Asphalt concrete overtay	10 mm	Medium	Many	b	· -
48		DECK SLAB (Sidewalk Bollom)	CR	S	Cracks	Bottom of Sidewalk, Right Side	Hairline cracks 0.05 mm	Small	Many	b	19
49		SIDE FACE OF WEB	SER	м	Spalling	Sideface of box girder web	0.3 m ²	Medium	1 Location	b	20
50		BOTTOM OF BOX GIRDER (Center)	SER	м	Spalling and exposed rebars	Bottom of box girder	0.25 m ²	Medium	Many	b	21,22,
51		BOTTOM OF BOX GIRDER (GERBER)	SER	н	Exposed rebars	Bottom of box girder	0.5 m ²	High	Many	а	24, 2
52		EXPANSION JOINT (GERBER)	M/I/S/D	м	Movement and settlement	Expansion joint gerber Hinge No. 1	Remarkable	Medium	Whole joint	b	26, 2
53	3 8	EXPANSION JOINT (GERBER)	SER	н	Exposed rebars	Gerber Hinge No. 1	>0.3 m ²	High	2 Locations	а	28, 2
54	3 6	EXPANSION JOINT (GERBER)	SER	н	Exposed rebars	Gerber Hinge No. 2	0.35 m ²	High	3 Locations	a	
55	3 1	BEARING SHOE (P3)	со	s	Corrosion	Pier No. 3	Traces of corrosion on	Small	Typicel		<u>30, 31,</u> 33
56	· · · · -	PIER BODY (P3)	CR	M	Cracks	Pier No. 3 Wall	steel bearing shoes 0.4 mm wide	Medium	Many	ь b	34
57		FOUNDATION P3 CAP	SER	M	Exposed rebars	Footing cap	1.2 m ²	Medium	1 Location	b	34
58		RAILING @ Center	PD	s	Paint deterioration	Mid-span	Small	Small	5 m		
59		RAILING (P4)	PD	s	Paint deterioration	Right side railings	Small	Small		c	
60		RAILING (P4)	FR	M	Fracture	Left and Right Side	Remarkable		3 m	c	• •
	4			(V)	i duture	Railings	INCITIZITY BOID	Medium	2 Locations	b	-

Appendix 7.1.1-1 (2/34)

Damage	Span		Type of	Rank of		Des	cription of Damage			Demester	Photo
No.	No.	Name of Member	Damage	Damage	Nature	Location/ Pattern	Scale	Severity	No. of Damages	Remarks (a, b, c)	No.
62	4	PAVEMENT @ Center	CRPL	м	Cracks	Asphalt concrete overlav	7 mm	Medium	Many	Ъ	-
63	4	PAVEMENT (P4)	CRPL	м	Cracks	Asphalt concrete overlay	>5 mm	Medium	Many	b	-
64	4	BOTTOM OF BOX GIRDER (P4)	CR	S	Cracks	Bottom of box girder	0.13 mm	Small	Many	b	37, 38
65	4	BOTTOM OF BOX GIRDER (P4)	FL	м	Freelime	Bottom of box girder	0.25 m ²	Medium	Many	b	
66	4	BEARING SHOE	со	м	Corrosion	Pier No. 4	Spread over Whole bearing shoe	Medium	All bearing shoes	b	-
67	5	RAILING @ Center	PD	S	Paint deterioration	Mid-span, left and right side	Small	Small	10 m	. c	•
68	5	RAILING (A2)	PD	S	Paint deterioration	End of the span right side	Small	Small	10 m	с	-
69	5	RAILING (A2)	FR	м	Fracture	Railings	Remarkable	Medium	2 Locations	b	-
70	5	CURB	FR	Ň	Fracture	Medium curb	Remarkable	Medium	1 Location	b	·-
71	5	SIDEWALK	SER	М	Spalling	Sidewalk	0.15 m ²	Medium	1 Location	b	-
72	5	CURB & GUTTER	SER	м	Spalling	Curb and gutter	0.25 m ²	Medium	1 Location	b	
73	5	PAVEMENT @ Center	CRPL	М	Cracks	Asphalt concrete overlay	5 mm	Medium	Many	b	-
74	5	PAVEMENT A2	CRPL	м	Cracks	Asphalt concrete	8 mm	Medium	Many	b	

Name of Bridge : Pa1.2 Delpan Bridge (Downstream)

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Date of Inspection Inspector Checker

Dec. 13, 14, 19, 20, 2002 CT Arenas LG Sta. Maria

Damage	Span		Type of	Rank of		Desc	ription of Damag	e		Demonster	Dhata
No.	No.	Name of Member	Damage	Damage	Nature	Location/ Pattern	Scale	Severity	No. of Damages	Remarks (a, b, c)	Photo No.
1	1	RAILING (()	CR	S	Crecks	Reiling, Left Side	0.13 mm	Small	2 Locations	b	-
2	1	RAILING (P3)	CR	S	Cracks	Railing, Left Side	0.15 mm	Smail	1 Location	b	-
3	1	SIDEWALK	SER	м	Spalling	Sidewalk, Left Side	0.25 m ²	Medium	1 Location	b	-
4	2 -	SIDEWALK	EJ	S	Expansion Joint Separation	Sidewalk, Left Side	Small	Smali	1 Location	b	-
5	2	CURB & GUTTER	EJ	S	Expansion Joint Separation	Curb & Gutter	Small	Small	1 Location	b	-
6	2	BOT. OF BOX GIRDER	CR	S	Cracks	Bottom of Box Girder	<0.2 mm	Small	Малу	b	1, 2
7	2	PIER BODY	SER	· M	Exposed Rebars	Pier No. 2	1.5 m ²	Medium	1 Location	b	3
8	2	PIER BODY	SER	М	Spalling	Pier No. 2	2,0 m ²	Medium	1 Location	b	. 4
9	3	DECK SLAB (SW BOT)	CR	S	Cracks	Bottom of Sidewalk	0.12 mm	Small	Many	b	5
10	3	SIDE FACE OF WEB	CR	S	Cracks	Sideface of Web of Box Girder	0.10 mm	Small	3 Locations	b.	6
11 -	4	RAILING P4	CR	S	Cracks	Railing, Left Side	0.23 mm	Small	1 Location	b	-
12	4	BOT. OF BOX GIRDER	CR	S	Crecks	Bottom of Box Girder	0.15 mm	Small	5 Locations	b	7

Name of Bridge : Pa3_McArthur Bridge

Date of Inspection Inspector Checker

Dec. 10 - 13, 2002 CT Arenas LG Sta, Maria

Damage	Span		Type of	Rank of			cription of Damage			Domost-	Dhat
No.	No.	Name of Member	Damage	Damage	Nature	Location/ Pattern	Scale	Severity	No. of Damages	Remarks (a, b, c)	Photo No.
1	1	RAILING	Def	S	Deformation	Right Side Handrail		Small	1	с	-
2	1	SIDÉWALK	SER	S	Spalling	Right Side Sidewalk	0.08 m ²	Small	1	c	-
3	1	DECK SLAB	CR	S	Cracks	Bottom of deck	<0.3 mm	Small	Many	b	· 1
4	1	DECK SLAB	SER	м	Spall with Exposed Rebars	Bottom of deck	0.2 m ²	Medium	2	b	2
5	1	DECK SLAB	CR	S	Cracks	Bottom of deck	<0.3 mm	Small	Many	b	-
6	1	RIVETS	м	н	Missing	Bottom Flange of Steel Girders	9 pieces	High	Many	а	3a
7	1	WELDED PORTION OF STEEL PLATE	Def	н	Deformation	Girder Boltom Flange Splice Plate	Remarkable	High	4	а	3
8	1	MAIN GIRDER	Def	н	Deformation	Bottom Flange of Outside Girder	Remarkable	High	1	а	3
9	1	EXPANSION JOINT	NO	М	Noise	Expansion Joint Steel Plate Atop Carriageway	Loose Connection	Medium	1	b	-
10 -	1	BEARING SHOE (A1)	co	S	Corrosion	Abutment No.1	Whole Shoe	Small	All bearing shoes	b	· 4
11	1	BEARING SHOE (P1)	co	S	Corrosion	Pier No. 1	Whole Shoe	Small	All bearing shoes	b	5
12	1	UTILITIES (A1)	со	М	Corrosion	Abutment No.1, Utility Steel Ducts	Remarkable	Small	2 Locations	b	6
13	1	UTILITIES (A1)	D/D	М	Deterioration	Abutment No. 1 Utility Ducts	Remarkable	Medium	1	b	7
11	1	FOUNDATION (PIER CAP)	SER	S	Exposed Rebars	Pier No. 1 Foundation	0.2 m ²	Small	1 location	с	
15	2	CURB (🕼)	SER	S	Spalling	Centerline Concrete Curb	<0.1 m ²	Small	1	с	_
16	2	CURB (P2)	SER	S	Spalling	Centerline Curb	<0.1 m ²	Small	1	c	-
17	2	SIDEWALK	SER	S	Spalling	Sidewall, rightside	0.05 m ²	Small	1	с	-
18	2 .	DECK SLAB	CR	S	Cracks	Bottom of deck slab	<0.3 mm	Small	2 Locations	b	8, 9
19	2	DECK SLAB	HC	м	Honeycombs	Bottom of deck	0.1 m ²	Medium	3 Locations	b	10, 11
20	2	DECK SLAB	SER	м	Spall with Exposed Rebars	Bottom of deck	<0.3 m ²	Medium	1	b	12
21	2	BOLT (RIVETS)	м	н	Missing	Bottom Flange of Steel Girder	Dangerous >200 pieces	High	1 Girder	а	13
22	2	MAIN GIRDERS	D/CO	Н	Deformation/ Corrosion	Bottom Flange of Steel Girder, Out of Alignment	Dangerous	High	7 Girders	а	14
23	2	MAIN GIRDERS	D/CO	н	Deformation/ Corrosion	Girder Bottom Flange and web	tmax = 15 mm reduced tmin≃ 10 mm	High	7 Girders	а	15, 16
24	2	SWAY BRACINGS	м	м	Missing	Sway Brace Bottom Members	2 pieces	Medium	2 Locations	b	17
25	2	BEARING SHOE	со	S	Corrosion	Pier No. 2	Whole Shoe	Small	7 Bearing Shoes	b	18
26	2	BEARING SHOE	ВМ	S	Bearing Shoe Movement	Pier No. 2		Small	7 Bearing Shoes	b	19
27	2	UTILITIES	D/D	н	Deterioration	Bottom of Sidewalk	Remarkable	High	Many	а	20
28	2	PIER COPING	SER	S	Spalling	Pier No. 2	0.2 m ²	Small	1 Location	с	21
29	2	PIER BODY	SER	s	Spalling with Exposed Rebars	Pier No. 2	0.2 m ²	Small	1	с	22

Appendix 7.1.1-1 (3/34)

Damage	Span		Type of	Rank of			cription of Damage			Remarks	Photo
No.	No.	Name of Member	Damage	Damage	Nature	Location/ Pattern	Scale	Severity	No. of Damages	(a, b, c)	No.
30	3	SIDEWALK	SER	S	Spalling	Left Sidewalk	0.15 m ²	Small	1 Location	с	-
31	3	DECK SLAB	нс	м	Honeycomb	Bottom of Deck Slab	0.30 m ²	Medium	1	b	23
32	3	MAIN GIRDERS	со	S	Corrosion	Girder Bottom Flange and web	Rust Spots	Small	Many	C ·	-
33	3	SWAY BRACINGS	со	н	Corrosion	Sway Brace Members	Rust Spread over Whole Area	High	4 Locations	а	24
34	3	EXPANSION JOINT	NÔ	М	Noise	Expansion Joint Steel, Plate Over the Carriageway	Loose Connection	High	1	b	-
35	3	BEARING SHOE	со	S	Corrosion	Abutment No. 2	Whole Shoe	Small	All Bearing Shoes	b	25
36	3	ABUTMENT	SER	н	Spalling and Exposed Rebars	Abutment No. 2, Well	Abutment No. 2 Wall 3.5 m ²	High	1 Location	a	26

Name of Bridge : Pa4. Quezon Bridge

Date of Inspection Inspector Checker

Nov. 27, 28, Dec. 2-5, 12-14, 16,18, 2002 CT Arenas LG Sta. Maria

Damaga	Ser		T	David C		Dee	cription of Damage	<u></u>			1
Damage No.	Span No.	Name of Member	Type of Damage	Rank of Damage	Nature	Location/ Pattern	Scale	Severity	No. of Damages	Remarks (a, b, c)	Photo No.
1	1	BOLTS	со	м	Corrosion	Joint No. B7 and at Mid-span	Remarkable	High	Many	а	
2	_ 1	BOLTS	м	м	Missing	Joint No. B7 and at Mid-span	7- 	High	Many	а	- 1, 1a
3	1	STEEL PLATE (BOLTED)	CO	Н	Corrosion	Joint A9	Remarkable	High	Whole Plate	a	2, 22
4	1	VERTICAL MEMBERS	CO	S	Corrosion	Joint B12	Surface Spots	Smali	Small	c	3, 3a
5	1	TIE	co	н	Corrosion	Joint B9	max. bf = 75 mm min. bf = 45 mm	High	Numerous	а	4, 4a
6	1	CROSS BEAM/JOINT C8	со	н	Corrosion	Cross Beam, Joint C8, End of Lateral Bracing	Remarkable	High	1	а	5
7	1	STRINGER	CO	Н	Corrosion	Stringer of Sidewalk	Remarkable	High	1 Location	а	6,6a
8	1	LATERAL BRACING (JOINT C9)	со	н	Corrosion	Lateral Bracing (Joint C9)	Remarkable	High	1 Location	а	7
9	1	ABUTMENT (ANCHORAGE)	co	н	Corrosion	Anchor Plate at Abutment A	Reduction of surface area by, 100 mm dia.	High	1	а	8
10	1	ABUTMENT	M	S	Missing	Rivet	1 pc.	Small	1 bolt	b	9
11	1	ABUTMENT BACKWALL	CR	S	Cracks	Abutment Backwall	Hairline Cracks	Small	Many	b	-
12	1	JOINT A3	co	н	Corrosion	Joint A3	max. t = 10 mm min. t = 8 mm	High	Whole Plate	а	10
13		JOINT B3	CO .	Н	Corrosion	Joint B3	Remarkable	High	Whole Plate	a	-
14		JOINT C3	co	H	Corrosion	Joint C3	Remarkable	High	Whole Plate	а	-
15	1	JOINT A4	CO	н	Corrosion	Joint A4	Remarkable	High	Whole Plate	а	
16	1	JOINT B4	co	Н	Corrosion	Joint B4	Remarkable	High	Whole Plate	a	11
17	1	JOINT C4	co	M	Corrosion	Joint C4	Remarkable	Medium	Whole Plate	b	-
18	1	JOINT A5	CO	н	Corrosion	Joint A5	Remarkable	High	Whole Plate	а	-
19	1	JOINT B5	CO	M	Corrosion	Joint B5	Remarkable max. I = 10 mm	Medium	Whole Plate	b	-
20	1	JOINT C5 JOINT A6	CO CO	н	Corrosion	Joint C5	min.t = 7 mm	High	Whole Plate	a	12, 12
22	1	JOINT B6	co	 M	Corrosion	Joint A6	Remarkable	High	Whole Plate	a	-
23	1	JOINT C6	co	<u>н</u>	Corrosion	Joint B6 Joint C6	Remarkable Remarkable	Medium	Whole Plate	b	
24	1	JOINT A7	co	M	Corrosion	Joint A7	Remarkable	High Medium	Whole Plate Whole Plate	a b	13
25	1	JOINT B7	co	M	Corrosion	Joint B7	Remarkable	Medium	Whole Plate	b	
26	1	JOINT C7	со	н	Corrosion	Joint C7	max.t=10 mm min.t=6 mm	High	Whole Plate	a	14, 18
27	1	JOINT A8	co	н	Corrosion	Joint A8	Remarkable	High	Whole Plate	а	-
28	. 1	JOINT B8	co	M	Corrosion	Joint B8	Remarkable	Medium	Whole Plate	b	-
29	1	JOINT C8	со	н	Corrosion	Joint C8	max. t = 10 mm min. t = 6.5 mm	High	Whole Plate	а	16
30	1	JOINT A9	co	Н	Corrosion	Joint A9	Remarkable	High	Whole Plate	а	-
31	1	JOINT B9	co	M	Corrosion	Joint B9	Remarkable	Medium	Whole Plate	b	-
32	1	JOINT C9	со	Н	Corrosion	Joint C9	max. bf = 75 mm min. bf = 50 mm	High	1 Joint	a	17
33	1	JOINT A10	D/CO	н	Deterioration/ Corrosion	Joint A10	Remarkable	High	Whole Plate	а	19, 19
34	1	JOINT B10	co	Н	Corrosion	Joint B10	Remarkable	High	Whole Plate	a	-
35	1	JOINT C10	со	Н	Corrosion	Joint C10	max. t = 10 mm min. t = 8 mm	High	Whole Plate	а	18
36		JOINT A11	D/CO	н	Deterioration/ Corrosion	Joint A11	max.t = 10 mm min.t = 7.5 mm	High	1 Whole Plate	а	20, 20a 21
37		JOINT B11	co	н	Corrosion	Joint B11	Remarkable	High	Whole Plate	а	-
38	. 1	JOINT C11	co	н	Corrosion	Joint C11	Remarkable	High	Whole Plate	а	-
39	1	JOINT A12	со	н	Corrosion	Joint A12	max. t = 10 mm min. t = 8.5 mm	High	Whole Plate	а	22, 22
40		JOINT B12	D/CO	н	Deterioration/ Corrosion	Tie @ Joint B12	max. bf = 75 mm min. bf = 65 mm	High	Both Ends	а	23
1		JOINT C12	- co	н_	Corrosion	Joint C12	Remarkable	High	Whole Plate	a	
2		JOINT A13	со	н	Corrosion	Joint A13	max. t = 10 mm min. t = 8.5 mm	High	Whole Plate	а	24, 24;
3		JOINT B13	<u></u>	M	Corrosion	Joint B13	Remarkable	Medium	Whole Plate	b	-
5		JOINT C13 JOINT A14	CO CO	<u>н</u>	Corrosion	Joint C13 Tie @ Joint A14	Remarkable max. bf = 75 mm	High	Whole Plate	a	-
6						-	min. bf = 60 mm	High	Both Ends	а	25
		JOINT B14	<u>co</u>	н	Corrosion	Joint B14	Remarkable	High	Whole Plate	а	-
7	1.	JOINT C14	со	н	Corrosion	Joint C14	max. l = 10 mm min. l = 9 mm max. l = 10 mm	High	Whole Plate	а	26
8		JOINT A15 JOINT B15	CO CO	H	Corrosion	Joint A15 Joint B15	min. t = 8 mm Remarkable	High	Whole Plate	a b	27

Name of Bridge : Pa5 Nagtahan Bridge

Name of Br	idge : <u>P</u> i	a5 Nagtahan Bridge					Date of Inspection Inspector Checker		Dec. 10 - 12, 2 CT Arenas LG Sta. Maria	002	
Damage No.	Span No.	Name of Member	Type of Damage	Rank of Damage	Nature	Des Location/ Pattern	cription of Damage Scale	Severity	No. of Damages	Remarks (a, b, c)	Photo No.
1	1	RAILING (A1)	Def/M	S	Deformation	Railing, Left Side	1 - Member	Small	1 Location	с	•
2	1	RAILING (🖢)	Def/M	S	Deformation	Railing, Left Side	1 - Member	Small	1 Location	с	-

Appendix 7.1.1-1 (4/34)

Damaga	6	1	Tumo of	Deally of	T	Des	cription of Damage	8		T	1
Damage No.	Span No.	Name of Member	Type of Damage	Rank of Damage	Nature	Location/ Pattern	Scale	Severity	No. of Damages	Remarks (a, b, c)	Photo No.
3	1	RAILING (P1)	L/M	S	Missing	Railing, Left Side	2 Bolts	Small	1 Location	с	1
4	1	CURB (A1)	SER	S	Spalling	Sidewalk Curb	< 0.10 m ²	Small	1 Location	c	2
5	1	CURB (¢)	EJ	м	Expansion Joint Separation	Sidewalk Curb	Abnormal Gap	Medium	1 Location	b	-
6	1	CURB (P1)	EJ	м	Expansion Joint	Sidewalk Curb	Abnormal Gap	Medium	1 Location	b	-
7	1	SIDEWALK (A1)	CR	s	Separation Cracks	Sidewalk	0.15 mm	Small	1 Location	c	-
8	1	SIDEWALK (C)	CR	S	Cracks	Sidewalk	0.2 mm	Small	1 Location	c	-
9	1	SIDEWALK (P1)	SER	S	Spalling	Finish @ Sidewalk	1.5 m ²	Small	1 Location	С	3
10	1	CURB & GUTTER (A1)	CR	S	Cracks	Curb and Gutter	0.1 mm	Small	1 Location	c	-
11 12	1	CURB & GUTTER (C)	CR	S	Cracks	Curb and Gutter	0.3 mm	Small	1 Location	c	4
		CURB & GUTTER (P1)	CR	S	Cracks	Curb and Gutter Pavement Bridge	0.25 mm Concave	Small	1 Location	C .	-
13	1	PAVEMENT	CRPL	S	Local Bump	Approach	30 mm	Small	1 Location	c	-
14	1	MEDIAN STRIP	SER	S	Exposed Rebars	Median Strip	< 0.1 m ²	Small	1 Location	c	
15	1	SURFACE OF DECKSLAB	SER	М	Exposed Rebars	Bottom Surface of Deck Slab	0.2 m ²	Medium	1 Location	b	5
16	1	BOLT	м	S	Missing	Bolt Nut (Girder No. 9) Abut A	1 pc.	Small	1 Location	c	6
17	1	BOTTOM CHORD	co	S	Corrosion	Bottom Chord, Girder No. 1	Surface Spois	Small	1 Location	c	7
18	1	VERTICAL MEMBER	DEF	S	Deformation	Vertical Member	Small	Small	1 Location	с	8
19	1	DIAGONAL MEMBER	со	S	Corrosion	Diagonal Member	Surface Spots	Small	Many	с	9
20	1	LATERAL BRACING	со	S	Corrosion	Lateral Bracing	Surface	Small	Many	b	10
21	1	EXPANSION JOINT	EJ	M	Expansion Joint	Abutment A	Spots Remarkable	Medium	1 Location	b	11
22	1	UTILITIES	co	н	Separation Corrosion	Abutment A	Remarkable	High	1 Location		12
					Spalling and	Abutment-Pier P1				а	
23 24	1	PIER A1 BACKWALL	SER	S	Exposed Rebars	Backwall Abutment-Pier P1	<1.0 m ²	Small	1 Location	c	13
			НС	s	Honeycombs	Backwall	0.1 m ²	Small	1 Location	С	14 15, 15a,
25	1	PIER BODY *	CR	M.	Cracks Spalling and	Pier No. 1	0.5 mm	Medium	1 Location	b	15, 15a, 15b
26	1	PIER BODY	SER	М	Exposed Rebars	Pier No. 1	1.0 m ²	Medium	1 Location	b	16
27		PIER A1 LOWER BODY	SER	М	Exposed Rebars	Pier No. 1	1.2 m²	Medium	1 Location	b	17
28	2	RAILING	D/M	S	Deformation	Railings	1 pc.	Small	1 Location	c	. •
29 30	2	CURB ((C) CURB (P2)	CR CR	S	Cracks	Sidewalk Curb	0.15 mm	Small	1 Location	c	-
31	2	SIDEWALK (C)	CR	s	Cracks	Sidewalk Curb Sidewalk	0.23 mm 0.12 mm	Small	1 Location 1 Location	C C	-
32	2	SIDEWALK (P2)	CR	s	Crecks	Sidewalk	0.26 mm	Small	1 Location	c	
33	2	CURB & GUTTER (C)	CR	S	Cracks	Curb and Gutter	0.1 mm	Small	1 Location	c	-
34	2	CURB & GUTTER (P2)	CR	S	Cracks	Curb and Gutter	0.13 mm	Small	1 Location	с	-
35	2	WELDED PORTION OF ST. PL	co	S	Corrosion	Welded Portion of Steel Plate	Surface Spots	Small	1 Location	с	- 1
36	2	BOTTOM CHORD (C)	co	М	Corrosion	Bottom Chord	Whole Member	Medium	Many	b	-
37		VERTICAL MEMBER (C)	CO	S	Corrosion	Vertical Members	Small	Small	Many	с	-
38	2	VERTICAL MEMBER (P2)	co	M	Corrosion	Vertical Members	Whole Member	Medium	Many	b	-
39 40	2		<u></u>	S	Corrosion	Diagonal Members	Small	Small	Many	c	-
40	2	DIAGONAL MEMBER (P2)	co co	S M	Corrosion Corrosion	Diagonal Members Lateral Bracings	Surface Spots Whole Member	Small Medium	Many	c b	- 18
42	2	LATERAL BRACING (P2)	co	M	Corrosion	Lateral Bracings	Whole Member	Medium	Many Many	b	- 18
43		PIER BODY	CR	M	Cracks	Pier No. 2	0.5 mm	Medium	1 Location	b	- 19
44	2	PIER A2 LOWER BODY	SER	M	Exposed Rebars	Footing, Pier No. 2	1.0 m ²	Medium	1 Location	b	20
45	3	RAILING	Def/M	s	Deformation	Railings	1 pc.	Small	1 Location	с	-
46	3	CURB (🖞)	CR	s	Cracks	Sidewalk Curb	0.25 mm	Small	1 Location	c	-
47		CURB (A2)	CR	s	Cracks	Sidewalk Curb	0.13 mm	Small	1 Location	c	-
48	3	SIDEWALK (🖢)	CR	s	Cracks	Sidewalk	0.10 mm	Small	1 Location	c	-
49	3	SIDEWALK (A2)	CR	s	Cracks	Sidewalk	0.12 mm	Small	1 Location	c	-
50	3	CURB & GUTTER (🖞)	CR	S	Cracks	Curb and Gutter	0,3 mm	Smail	1 Location	с	
51	3	CURB & GUTTER (A2)	CR	S	Cracks	Curb and Gutter	0.23 mm	Small	1 Location	c	· -
52	3	BOLT	м	S	Missing	Girder No. 9 @ Abutment B	1 pc.	Small	1 Location	с	-
53	3	DEFORMED VERTICAL	Def	S	Deformation	Vertical Member	Small	Small	1 Location	с	21
54	3	PIER A2	SER .	м	Spalling	Abulment-Pier 2	1.5 m ²	Medium	1 Location	b	22

Name of Bridge	Pa5	Nagtahan Bridge (Approach Spans)
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Date of Inspection Inspector Checker Feb. 17-21, 2003 AA Salvador / FAG Valderrama CT Arenas

Damage	Span		Type of	Rank of		Des	cription of Damage			Remarks	Photo
No.	No.	Name of Member	Damage	Damage	Nature	Location/ Pattern	Scale	Severity	No. of Damages	(a, b, c)	No.
1	1	LIGHTING POST	м	S	Missing	Lighting Post @ Span 1 (Downstream)	1 piece	Small	1 Location	с	1
2	1	PAVEMENT	CRPL	s	Rutting of Asphalt	Near Abut. 1 (Upstream)	Concave and Convex 35 mm	Medium	Whole Width	c	2
3	1	BOTTOM OF DECK SLAB	нс	м	Honeycomb	Whole Span @ (G8)	A=1.45m ² (max.)	Medium	No. 8	b	3
4	1	END DIAPHRAGM	нс	М	Honeycomb	Honeycomb @ End Diaphragm bet. G9-G10	A=1.25 m ²	Medium	1 Location	с	4
5	1	INTERMEDIATE DIAPHRAGM	HC / SER	s/s	Honeycomb	Honeycomb w/ exposed RSB @ Int. Diaphragm bet. G11-G12	A=0.15 m²/ A=0.1m²	Small	1 Location	с	5
6	2	BOTTOM OF DECK SLAB	HC	S	Honeycomb	Honeycomb bet. G3- G4 (Upstream)	A=0.30 m ²	Small	1 Location	с	6
7	3	BOTTOM OF DECK SLAB	нс	S	Honeycomb	Honeycomb bet. G2- G3 (Upstream)	A=0.10 m ²	Small	1 Location	с	7
8.	3	INTERMEDIATE DIAPHRAGM	нс	s	Honeycomb	Honeycomb bet. G2- G3 (Upstream)	A=0.20 m ²	Small	1 Location	с	8
9	3	PIER COPING	HC / SER	SIS	Honeycomb w/ Exposed RSB	Bottom of Coping No. 3	A=0.15 m2 / A=0.10m ²	Small	1 Location	c	9
10	3	PIER COPING	CR	S	Cracks	Cracks @ bot. and face of coping no. 3	W≃0.07 mm	Small	2 Locations	c	10

Appendix 7.1.1-1 (5/34)

Damage	Span		Type of	Rank of			cription of Damag	e		Remarks	Photo
No.	No.	Name of Member	Damage	Damage	Nature	Location/ Pattern	Scale	Severity	No. of Damages	(a, b, c)	No.
11	3	PIER COLUMN	HC	S	Honeycomb	Pier 3, col. 1and col. 2		Small	2 Locations	c	11, 11;
12	4	BOTTOM OF GIRDER	HC	S	Honeycomb	Bottom of Girder No. 8 Bottom of Coping No.		Small	1 Location	c	12
13	4	PIER COPING	НС	S	Honeycomb	Cracks @ bot. and	A=0.30 m ²	Small	1 Location	c	13
14	4	PIER COPING	нс	S	Cracks	face of coping no. 4	W=0.04 mm	Small	2 Locations	c	14
15	5	PIER COPING	CR	S	Vertical Cracks	Cracks below G9-G13, P5	0.18 to 0.3 mm	Small	Many	c	15, 15a
16	5	PIER COLUMN	CR	S	Horizontal Cracks	Cracks @ col. 2, Pier 5	W=0.30 mm	Small	Many	c	16
17	6	PIER COPING	CR	s	Vertical Cracks	Cracks @ coping near col. 3, P6	W=0.08 mm	Small	2 Locations	с	17
18	6	PIER COLUMN	SER	s	Spalling	Concrete Spalling @ Col. 3, P6	A=0,35 m ²	Small	1 Location	с	18
19	6	PIER COLUMN	CR	s	Horizontal Cracks	Crecks @ col. 2, Pier 6	W=0.30 mm	Small	Many	· c	19
20	7	PIER COLUMN	SER	s	Spalling w/ Exposed Rebars	Pier 7, col. 1 body	A=0.5 m ²	Small	1 Location	с	20
1	8	RAILING	М	S	Missing	Downstream side, 5.40 m from P8	1 piece	Medium	1 Location	b	21
2	8	CURB & GUTTER	SER	S	Spalling of Concrete	Spall of concrete @ curb near P8	A=0.09 m ²	Smalt	1 Location	с	22
23	8	EXPANSION JOINT	EJ	M.	Expansion joint Separation	Expansion @ pier no. 8	Gap=70 mm	Medium	Both Sides	b	23
24	13	FALL DOWN PREVENTION	SER	S	Spalling w/	Shear key bel. G15-	A=0.06 m ²	Small	1 Location	°C	23
25	13	DEVICE (SK) PIER COPING	SER	S	Exposed Rebars Spalling of Concrete	G16 (US) Spalling of concrete @	A=0.60 m ²	Small	1 Location	c	25
26	13	PIER COLUMN	HC	s	Honevcomb	coping no. 13 Honeycomb @ top of col.		Small			
27				-		1, pler no. 13 Spalling @ bot. of deck	A=0.65 m ²		1 Location	c	26
	14	BOTTOM OF DECK SLAB	SER	S	Spalling of Concrete	slab bet, G7-G8 Honeycomb @ bot.	A=0.40 m ²	Smali	1 Location	C	27
28	14	BOTTOM OF GIRDER	HC .	S	Honeycomb	of G9 (DS) Honeycomb @ end	A=0.30 m ²	Small	2 Locations	c	28
29	14	END DIAPHRAGM	нс	S	Honeycomb	diaphragm, bet. G11-G12	A=0.20 m ²	Small	1 Location	c	29
30	14	INTERMEDIATE DIAPHRAGM	SER	S	Spalling/Exposed Rebars	Exposed RSB @ int. dlaphragm bet. G12-G13	A=0.18 m ²	Small	1 Location	с	30
31	14	INTERMEDIATE DIAPHRAGM	нс	S	Honeycomb	Typical honeycomb of int. diaphragm from G5 to G8	A=0.15 m ²	Small	1 Location	с	31
-32	14	PIER COPING	SER	S.	Spalling/ Exposed RSB	Spalling with exposed RSB @ coping no. 14	A=0.05 m ²	Small	1 Location	с	32
33	14	PIER COLUMN	CR	S	Vertical Cracks	Typical cracks @ all columns @ pler no. 14	W<0.40 mm	Small	4 Locations	с	33
34	14	PAVEMENT	CRPL	S	Rutting of Asphalt	Rutting of asphalt 129.60 m from A2 (DS)	Concave and Convex <30mm	Small	1 Location	c	34
35	15	BOTTOM OF DECK SLAB	CR.	H.	Horizontal Cracks	Typical cracks @bot. of	W=1.0 mm (max.)	High	Typical from P15 - A;	а	34
36	15	BOTTOM OF GIRDER	CR	м	Horizontal Cracks	cantilever deck stab (SW) Typical cracks @ bot. of	W=0.20 mm	Medium	Typical from P15 - A:	b	36
37	15	EXPANSION JOINT	EJ	м	Expansion Joint	Girder No. 1 Expansion @ pler no. 15	Gap = 40 mm	Medium	1 Location	b	37
38		PIER COPING	SER	s	Separation Exposed RSB w/	Exposed RSB with concrete spall @ bot. of	A=0.24 m ²	Small	1 Location	c	38
39	15	PIER COLUMN	SER	s	Little Spalling Spalling/Exposed	coning 15 Spalling w/ exposed RSB	A=0.18/0.075	Small	2 Locations	c	39
40		PIER COLUMN	CR	s	Rebars Vertical Cracks	@ col. 2 & 4 @ pier 15 Typical cracks @ all	W=0.04 mm	Small	4 Locations	c	40
41	16	INTERMEDIATE DIAPHRAGM	нс	s	Honeycomb	columns @ pler 15 Honeycomb @ int.	A=0.06 m ²	Small	1 Location	c	41
42		BOTTOM OF GIRDER	SER	s	Spalling w/	diaphragm bet, G13-G14 Exposed RSB @ G12	A=0.04 m ²	Small	1 Location	c	41
43	19	EXPANSION JOINT	EJ	M	Exposed Rebars Expansion	(US) Expansion @ pier no. 19	Gap =30 mm	Medium	1 Location	с с	43
44		PIER COPING	CR	s	Separation Vertical Cracks	Cracks @ coping no. 19	W=0.07 mm	Small	1 Location	c	
45		PIER COPING	SER	s	Spalling/	Exposed RSB @ bot, of	A=0.05 m ²	Small	1 Location		44
46		PIER COPING	CR	s	Exposed Rebars Vertical Cracks	coping no. 19 Vertical cracks @ side	W=0.04 mm	Small	1 Location	c	45 46
47		PIER COPING	CR	S	Vertical Cracks	of coping no. 21 Vertical cracks @ side	W=0.035 mm	Small	1 Location	c	40
48	23	CURB & GUTTER	CR	s	Minor Damaged	of coping no, 22 Typical damaged of tiles	A=1.00m ²	Smell	Many	c	48
49		EXPANSION JOINT	EJ	M	of Tiles Expansion Joint Separation	Expansion @ abutment 2	Gap =45 mm	Medium	1 Location	c	40

Name of Bridge : Pa6 Pandacan Bridge

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Date of Inspection Inspector Checker

Dec. 6 - 7, 2002 CT Arenas LG Sta. Maria

Description of Damage Damage Span No. Type of Damage Rank of Remarks Photo Name of Member No. of No. Nature Damage Severity (a, b, c) No. Pattern Damages Painting Deterioration Surface Splitting 1 LIGHTING POST 1 PD s Light Post Small All с of Paint 2 1 DECK SLAB D/D м Discoloration Bottom of Deck Remarkable Medium Many b -Painting Deterioration Surface Splitting of Paint з 2 LIGHTING POST s PD Steel Light Post Small All с -2 4 DECK SLAB D/D м Discoloration Bottom of Deck Remarkable Medium Many b 1 5 2 BOTTOM OF PC GIRDER нс н Honeycombs Bottom of PC Girder 2 Locations 0.42 m^2 High а 2 6 2 s BOTTOM OF PC GIRDER SER Exposed Rebars Boltom of PC Girder 0.06 m² Small 1 с 3 7 2 END OF PC GIRDER н SER Exposed Rebars End of PC Girder 0.80 m² 4 High 1 а umn Tie Beam Pier P2 8 2 PIER BODY SER s Exposed Rebars 0.45 m² 5 Small 2 с Painting 9 3 LIGHTING POST PÐ s Surface Splitting Light Post All Small С -Deterioration of Paint 10 3 PAVEMENT CRPL s Cracks Carriageway 0.35 mm Small Many С -11 3 DECK SLAB CR М Cracks Bottom of Deck 0.3 mm Medium Many b 6 12 3 DECK SLAB HC М Honeycombs Diaphragm 0.2 m² Medium Many b -Column Tie Beam, Pier P3 з м 13 PIER BODY SER Exposed Rebars 1.4 m² High 2 b 7 Column Tie Beam, Pier P3 14 3 PIER BODY CR s Cracks 0.3 mm Small Many ¢ -Surface Splitting Painting Deterioration 15 4 LIGHTING POST PD s Light Post Small All с of Paint 16 4 DECK SLAB CR М Cracks Bollom of Deck 0.35 mm Medium Many 8 b 17 4 SIDE OF PC GIRDER SER М Exposed Rebars Side of PC Girder 0.25 m² Medium 1 b -18 4 PIER COPING SER s Exposed Rebars Coping of Pier P4 <0.2 m² Small Many ć 9 Column Tie Beam, 19 4 PIER BODY SER м Exposed Rebars 1.80 m² Medium 2 10 b Pier P4

Appendix 7.1.1-1 (6/34)

Damage	Span		Type of	Rank of		Desc	ription of Damage	1		Remarks	Photo
No.	No.	Name of Member	Damage	Damage	Nature	Location/ Pattern	Scale	Severity	No. of Damages	(a, b, c)	No.
20	4	PIER BODY	CR	М	Cracks	Column Tie Beam, Pier P4	0.33 mm	Medium	2	b	11
21	5	RAILING	м	s	Missing	Right Side Sidewalk	1 pc.	High	2 m	с	-
22	5	LIGHTING POST	PD	S	Painting Deterioration	Light Post	Surface Splitting	Small	All	c	-
23	5	ABUTMENT	CR	м	Cracks	Abutment Coping	0.3 mm	Medium	Many	с	-
24	5.	ABUTMENT BACKWALL	CR	м	Cracks	Abutment Backwali	0.3 mm	Medium	Many	c	12

Name of Bridge : Pa7 Lambingan Bridge

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Date of Inspection Inspector Checker

Dec. 4 - 5, 2002 CT Arenas LG Sta. Maria

Damage	Span		Type of	Rank of		Des	cription of Damag	e			
No.	No.	Name of Member	Damage	Damage	Nature	Location/ Pattern	Scale	Severity	No. of Damages	Remarks (a, b, c)	Photo No.
1	1	PAVEMENT	CRPL	S	Cracks	Roadway	0.3 mm	Small	Many	с	-
2	1	DRAINAGE SYSTEM	D/D	М	Deterioration	Span - 1	Remarkable	Medium	2 Locations	b	-
3	1.	BEARING SHOE (PAD)	Def	М	Deformation	Ext. Girder, Bearing Shoe at Abutment A1	Remarkable	Medium	1 Location	b	1a
4	1	ABUTMENT	SER	s	Exposed Rebars	Abutment A1	0.2 m ²	Small	1 Location	с	-
5	1	ABUTMENT	CR	s	Cracks	Abutment A1, Wall	0.3 mm	Small	2 places	с	1
6	1	PIER BODY	SER	s	Exposed Rebars	Pier P1	0.2 m ²	Small	1	с	2
7	1	PIER BODY	CR	M	Cracks	Pier P1, Wall	0.6 mm	Medium	Many	b	3
8	1	PIER BODY	CR	н	Cracks	Pier P1, Coping	>0.6 mm	Heavy	Many	а	4
9	2	DECK SLAB (BOTTOM OF DIAPHRAGM)	нс	М	Honeycombs	Bottom of Diaphragm	1.2 m²	Medium	3 Locations	b	5
10	2	DECK SLAB (DIAPHRAGM)	CR	М	Cracks	Diaphragm	0.35 mm	Medium	Many	b	6
11	2	DECK SLAB (DIAPHRAGM)	HC	М	Honeycombs	Diaphragm	0.25 m ²	Medium	Many	b	7
12	2	DECK SLAB (DIAPHRAGM)	CR	М	Cracks	Diaphragm	0.4 mm	Medium	Many	b	8
13	2	BOTTOM OF PC GIRDER	SER	м	Spalling	Girder No. 12	0.2 m ²	Medium	1	b	9
14	_ 2	BOTTOM OF PC GIRDER	SER	н	Exposed Rebars	Girder No. 12	0,75 m ²	Heavy	1	а	10
15	2	SIDE OF PC GIRDER	SER	М	Exposed Rebars	Girder No. 12	< 0.3 m ²	Medium	3 places	b	11
16	2	SIDE OF PC GIRDER	SER	М	Exposed Rebars	Girder No. 12	0.18 m ²	Medium	3 places	b	12
17	2	EXPANSION JOINT	SER	S	Spalling	Hinge	0.06 m ²	Small	1	c	-
18	2	DRAINAGE SYSTEM	D/D	М	Deterioration	Span - 2	Remarkable	Medium	8 spouls	b	-
19	2	UTILITIES	WLC	M .	Utilities	Waterlines	Remarkable	Medium	2 pipelines	b	-
20	2	PIER BODY	HC	s	Honeycombs	Pier No. 2	0.5 m ²	Small	1	с	13
21	2	PIER BODY	SER	s	Exposed Rebars	Pier No. 2	0.2 m ²	Small	. 1	с	
22	2	PIER BODY (CAP)	CR	м	Cracks	Pier No. 2	0.4 mm	Medium	Many	b	14
23	3	PAVEMENT	CRPL	s	Cracks	Roadway	0.3 mm	Small	Many	с	

Damage

No.

1

2

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12

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14

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16

17

18

19

20

21

22

23

24

25

26

27

Span No.

1

1

1

1

1

1

1

1

1

1

1

2

2

2

2

2

2

2

2

2

2

3

3

3

3

3

3

BOTTOM OF BOX

FOUNDATION

SIDEWALK

PAVEMENT

SURFACE OF WEB

SURFACE OF WEB

EXPANSION JOINT

BEARING SHOE

PIER BODY (COLUMN)

CR

SER

M/I/S/D

EJ

CRPL

CR

CR

EJ

со

s

М

s

м

s

М

н

ş

м

Cracks

Exposed Rebars

Inclination

Almost closed

Cracks

Cracks

Cracks

Slightly disturbed

Corrosion

Name of Bridge :	Pa8 Makati-Mandaluyong Bridge
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Ε	as Makati-Mandaluyong Brid	ge				Checker		LG Sta. Maria	
n	I	Type of	Rank of	1	Des	cription of Damage)		
	Name of Member	Damage	Damage	Nature	Location/ Pattern	Scale	Severity	No. of Damages	Remarks (a, b, c)
	SIDEWALK	EJ	м	Almost closed	Left and Right Sidewalk	1.5 m	Medium	2	b
_	SIDEWALK	CR	S	Cracks	Left and Right Sidewalk	less than 0.3 mm	Small	Many	c
	PAVEMENT	CRPL	S	Cracks	Carriageway	less than 5 mm	Small	Малу	c
	SIDEFACE OF WEB	CR	н	Cracks	Sideface of web near P1 Downstream	0.4 - 0.65 mm	High	Many Horizonial Cracks	а
	SIDEFACE OF WEB	CR	н	Cracks	Sideface of web near P1 Upstream	0.6~1.5 mm	High	Many Horizontal Cracks	а
	BOTTOM OF PC GIRDER	CR	н	Cracks	Bottom of Box Girder	>0.6 mm	High	1 location	а
	BOTTOM OF PC GIRDER	CR	м	Cracks	Bollom of Box Girder	0.2-0.3 mm	Medium	Numerous	b
	EXPANSION JOINT	EJ	м	Almost closed	Beginning of Bridge	Medium	Medium	1	b
	BEARING SHOE	CO	М	Corrosion	Pier No. 1	Whole	Medium	Whole Shoes	b
	PIER BODY (COLUMN)	SER	S	Exposed Rebars	Pier No. 2	0.45m ²	Small	1 column	с
	FOUNDATION	M/I/S/D	S	Inclination	Pier No. 2	Small	Small	1	с
_	RAILING	B/R	S	Rupture	Railings	Small	Small	1	с
	SIDEWALK	CR	S	Cracks	Left and Right Sidewalk	less than 0.3 mm	Small	Many	с
	SIDEWALK	FR	S	Fracture	Sidewalk	less than 0.1 m ²	Small	1 side	с
	SIDEWALK	B/R	S	Break	Sidewalk	less than 0.1 m ²	Small	1 location	с
	END DIAPHRAGM	HC .	М	Honeycombs	Bottom Bet. G5 & G6	0.2 m ²	Medium	2	. b .
	END DIAPHRAGM	нс	н	Honeycombs	Bottom Bet. G4 & G5	1.5 m²,	High	1	а
	BOTTOM OF BOX	SER	M	Exposed Rebars	Boltom of Deck	0.2 m ²	Medium	2	b

Bottom of Deck

Pier No. 3

Pier No. 3

Left and Right Sidewalk

Carriageway

Surface of Web

Surface of Web

End of Bridge

Pier No. 4

0.03 mm

1.5 m²

Small (0.3°)

1.5 m

less than 5 mm

0.3 mm

0.8 mn

Small

Whole

Small

Medium

Small

Medium

Small

Medium

High

Small

Medium

Many

1 Location

1

1.5 m

Many

1

Many

Whole Shoes

Date of Inspection Inspector

Nov. 26, 28, Dec. 2, 3, 2002 CT Arenas

Photo

No.

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1, 2

3, 4

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Appendix 7.1.1-1 (7/34)

Damage	Span		Type of	Rank of		Des	cription of Damage			Remarks	Photo
No.	No.	Name of Member	Damage	Damage	Nature	Location/ Pattern	Scale	Severity	No. of Damages	(a, b, c)	No.
28	3	PIER COPING	SER	S	Exposed Rebars	Pier No. 4	0.2 m ²	Small	1	с	17

Name of Bridge : Pa8 Makati-Mandaluyong Bridge (Approach Spans)

Date of Inspection Inspector Checker

Scale

Progressive Deterioration

Feb. 17-20, 2003 AA Salvador CT Arenas

Remarks

(a, b, c)

с

No. of

Damages

Both Sides

Severity

Small

Photo

No.

1

Description of Damage Span No. Damage Type of Damage Rank of Name of Member No. Damage Nature Pattern 1 1 RAILING P/D s. Paint Deterioration Concrete Railing 1 2

2	1	RAJLING	P/D	s	Paint Deterioration	Concrete Railing	Progressive Deterioration	Small	Both Sides	с	-
3	1	PAVEMENT	CRPL	s	Local Bumps Asphalt	Expansion Joint Approach	Concave > 20 mm Transverse	Small	Whole Width	с	-
4	1	DECK SLAB	CR	M	Cracks	Deck Slab Surface	0.5 mm	Medium	Many	b	2
5	1	EXPANSION JOINT	EJ	S	Clogged	Expansion @ A1	Full of dirt & fine aggregate	Small	Full Length	c	3
6	1	DRAINAGE SYSTEM	со/м	M/M	Missing/Corrosion	Whole Span	Spread to Whole Member	Medium	2 Location	b	4, 5
7	1	ABUTMENT BACKWALL	CR	н	Horizontal Cracks	Abutment Wall A1	>0.6 mm	High	1 Location	а	6
8	2	RAILING	P/D	s	Paint Deterioration	Concrete Relling	Progressive Deterioration	Small	Both Sides	С	-
9	2	DECK SLAB	CR	M	Cracks	Deck Slab Surface	0.35 mm	Medium	Many	b	-
10	2	DRAINAGE SYSTEM	со	M	Corrosion	Whole Span	Spread over Whole Member	Medium	Many	b	
11	3	RAILING	P/D	s	Paint Deterioration	Concrete Railing	Progressive Deterioration	Small	Both Sides	c	-
12	3	DECK SLAB	CR	M	Cracks	Deck Slab Surface	0.4 mm	Medium	Many	b	-
13	3	DRAINAGE SYSTEM	co	М	Corrosion	Whole Span	Spread over Whole Member	Medium	2 Location	b	-
14	4	RAILING	P/D	S	Paint Deterioration	Concrete Railing	Progressive Deterioration	Small	Both Sides	c	1.2
15	4	DECK SLAB	CR	М	Cracks	Deck Slab Surface	0.35	Medium	Many	b	7
16	4	EXPANSION JOINT	EJ	м	Clogged & Exp. Jt. Separation	Expansion @ P4	Remarkable 25 mm. Vert.	Medium	1 Location	b	8
17	4	DRAINAGE SYSTEM	co	м	Corrosion	Whole Span	Spread over Whole Member	Medium	2 Location	b	-
18	8	RAILING	P/D	S	Paint Deterioration	Concrete Railing	Progressive Deterioration	Small	Both Sides	c	9
19	8	DECK SLAB	CR	M	Cracks	Deck Slab Surface	0.45	Medium	4 Locations	b	10
20	8	BOT. FLAT SLAB	CR	· S	Cracks	Bot. of Flat Slab	0.3 mm	Small	1 Location	с	11
21	8	BOT. FLAT SLAB	CR/FL	S	Crack with Freelime	Bottom of Flat Slab	0.2 mm/0.1 m ²	Small	1 Location	c	12, 13
22	8	DRAINAGE SYSTEM	со	м	Corrosion	Whole Span	Spread over Whole Member	Medium	2 Location	b	· -
23	8	ABUT. BACKWALL, P7	SER	S	Spall w/ Exposed Rebars	Abut. P7 Wall	0.25 m ²	Small	1 Location	° C	14
24	9	RAILING	P/D	S	Paint Deterioration	Concrete Railing	Progressive Deterioration	Small	Both sides	c	-
25	9	DECK SLAB	CR	S	Cracks	Deck Slab Surface	0.2 mm	Small	Many	с	-
26	9	DRAINAGE SYSTEM	со	м	Corrosion	Whole Span	Spread over Whole Member	Medium	2 Location	b	-
27	10	RAILING	P/D	S	Paint Deterioration	Concrete Railing	Progressive Deterioration	Small	Both sides	с	-
28	10	DECK SLAB	CR	M	Cracks	Deck Slab Surface	0.35 mm	Medium	Many	b	-
29	10	EXPANSION JOINT	EJ	s	Clogged	Pier P11	Full of Fine Aggregates	Small	Whole Length	с	15
30	10	DRAINAGE SYSTEM	со	. M .	Corrosion	Whole Span	Spread over Whole Member	Medium	1 Location	b	-
31	10	BEARING SHOE	м	м	Missing	Elastomeric Pad @ P10	2 pcs.	Medium	2 Location	b	-
32	11	RAILING	P/D	S	Paint Deterioration	Concrete Railing	Progressive Deterioration	Small	Both sides	c	-
33	11	DECK SLAB	CR	s	Cracks	Deck Slab Surface	0.15 mm	Small	Many	с	-
34	11	DRAINAGE SYSTEM	co	м	Corrosion	Whole Span	Spread over Whole Member	Medium	1 Location	b	-
35		RAILING	P/D	s	Paint Deterioration	Concrete Railing	Progressive Deterioration	Sməll	Both sides	с	-
36	12	DECK SLAB	CR	S	Cracks	Deck Slab Surface	0.25 mm	Smali	Many	с	-
37	12	DRAINAGE SYSTEM	со	M	Corrosion	Whole Span	Spread over Whole Member	Medium	1 Location	b	-

Name of Bridge : Pa9.1_GUADALUPE BRIDGE (STEEL TRUSS)

 Date of Inspection
 Dec.3,4,9&10, 2002

 Inspector
 J. Abadam/R. Abad

 Checker
 J.B. Agnes

Damage	Span		Type of	Rank of		Desc	cription of Damage			Remarks	Photo
No.	No.	Name of Member	Damage	Damage	Nature	Location/ Pattern	Scale	Severity	No. of Damages	(a, b, c)	No.
1	1	Vertical Chord @ Girder 1,2&3	со	м	Corrosion	Panel 36 @ Bay 2	Reduction of Cross Section	Moderate	Оле	b	225
2	1	Vertical Chord @ Girder 3	DEF	S	Deformed	Panel 28 @ Bay 3		Slight	One	с	
3	1	Top Chord @ Girder	M	S	Missing Rivet	@ Bay 2	1 Piece	Slight	One	c	
4	1	Vertical Chord	со	s	Corrosion	Near Abutment A @ Bay 6	Surface Rust	Slight	One	c	
5	1	Lateral Bracing	B/R	M	Broken by Vehicle	Panel 34 @ Bay 4	1 Portion	Moderate	One	b	226
6	1	Bottom Chord @ G-6	DEF	s	Uneven Thickness	Panel 24 & 23 @ Bay 5		Slight	One	c	228
7	1	Sway Bracing	DEF	S	Deformed	Panel 35 @ Bay 1		Slight	One	c	
8	1	Lateral Bracing	B/R	M	Broken	Panel 33 @ Bay 6	1 Portion	Moderate	One	b	227
9	1	Abut. Coping / Abut. A	CR	м	Random Cracks	Below Girder 3	t = 0.483 mm	Moderate	Many	b	204
10	1	Abutment Coping / Abut. A	CR	S	Crack	@ Coping Face	t = 0.254 mm	Slight	One	с	205
11	1	Abutment Coping / Abut, A	SER	S	Spati	@ Coping Face	$A = 0.1 m^2$	Slight	One	c	206
12	1	Abutment Coping / Abut. A	CR	S	Crack	@ Coping Face	t = 0.173 mm	Slight	One	c	207
13	1	Abutment Coping / Abut. A	CR	s	Vertical and Horizontal Cracks	@ Coping Face	l = 0.300 mm	Slight	Many	c	208
14	1	Abutment Coping / Abut. A	CR	M	Vertical Cracks	@ Coping Face	t = 0.350 m	Moderate	Two	b	209
15	1	Abutment Coping / Abut. A	CR	M	Vertical Crack	@ Coping Face	t = 0.50 mm	Moderate	One	b	210
16	. 1	Abutment Coping / Abut, A	CR	S	Vertical Crack	@ Coping Face	t = 0.300 mm	Slight	One	c	211
17	1	Abutment Coping / Abut. A	CR	S	Random Cracks	@ Coping Face	1 = 0,102 mm	Slight	Many	c	212
18	2	Lateral Bracing	CO	S	Corrosion	Panel 22 @ Bay 3	Surface Rust	Slight	One	c	224
19	2	Girder 9	M	S	Missing Rivet	Panel 21 @ Bay 2	One	Slight	One	C .	223
20	2	Vertical Chord	DEF	S	Deformed	Panel 20 @ Bay 2		Slight	One	C	222

Appendix 7.1.1-1 (8/34)

Damage No.	Span No.	Name of Member	Type of Damage	Rank of Damage	Nature	Location/	scription of Damag Scale	e Severity	No. of	Remarks (a, b, c)	Phot No.
04	-	Detter Obert O Obert		-		Pattern			Damages	(=, =, =, =,	
21 22	2	Bottom Chord @ Girder 9	DEF	<u>s</u>	Deformed	Panel 34 @ Bay 8		Slight	Several	c	240
22	2	Sway Bracing Sway Bracing	DEF DEF	s S	Deformed Deformed	Panel 19 @ Bay 1	12.0	Slight	One	c	
24	2		DEF			Panel 20 @ Bay 1	1 Portion	Slight	One	c	
25	2	Sway Bracing Sway Bracing	DEF	S	Deformed	Panel 22 @ Bay 1	1 Portion	Slight	One	c	<u> </u>
26	2	Sway Bracing	DEF	S S	Deformed	Panel 19 @ Bay 2 Panel 23 @ Bay 2	1 Portion	Slight	One	C	
27	2	Sway Bracing	DEF	s	Deformed		1 Portion	Slight	One	<u>с</u>	
28	2	Sway Bracing	DEF	S	Deformed Deformed	Panel 23 @ Bay 7	1 Portion	Slight	One	c	233
29	2	Sway Bracing	DEF	S		Panel 24 @ Bay 7	1 Portion	Slight	One	c	235
30	2	Sway Bracing	DEF	S	Deformed	Panel 28 @ Bay 7	1 Portion	Slight	One	C	237
31	2	Sway Bracing	DEF	S	Deformed	Panel 32 @ Bay 7	1 Portion	Slight	One	C.	238
32	2	Lateral Bracing	B/R	<u>N</u>	Deformed	Panel 34 @ Bay 7	1 Portion	Slight	One	c	239
33	2		DEF	S	Broken Deformed	Panel 35 @ Bay 7	Remarkable Damage	Moderate	One	b	245
34	2	Sway Bracing	DEF			Panel 12 @ Bay 8	1 Portion	Slight	One	c	230
35	2	Sway Bracing		<u> </u>	Deformed	Panel 12 @ Bay 8	1 Portion	Slight	One	C	229
36	2	Sway Bracing	DEF DEF		Deformed	Panel 17 @ Bay 8	Unusual Deflection	Moderate	One	b	231
37	2	Sway Bracing	DEF		Deformed	Panel 23 @ Bay 8	1 Portion	Slight	One	c c	234
38	2	Sway Bracing		<u> </u>	Deformed	Panel 24 @ Bay 8	2 Portions	Slight	Two	c	236
		Sway Bracing	DEF	<u> </u>	Deformed	Panel 35 @ Bay 9	1 Portion	Slight	One	С	242
39	2	Sway Bracing	DEF	S	Deformed	Panel 37 @ Bay 8	1 Portion	Slight	One	c	244
40	. 2	Sway Bracing	DEF	M	Deformed	Panel 21 @ Bay 9	Unusual Deflection	Moderate	Оле	b	232
41	2	Sway Bracing	DEF	M	Deformed	Panel 35 @ Bay 8	Unusual Deflection	Moderate	One	b	241
42	2	Sway Bracing	CO	<u>S</u>	Corrosion	Panel 36 @ Bay 9	Surface Rust	Slight	One	c	243
43	2.	Sway Bracing	DEF	M	Deformed	Panel 36 @ Bay 9	Remarkable Damage	Moderate	One	b	
44	2	Sway Bracing	M	<u> </u>	Missing Rivet	Panel 15 @ Bay 1	Three Location	Heavy	Twelve	а	
45	2	Sway Bracing	M	н	Missing Rivet	Panel 19 @ Bay 1	Two Location	Heavy	Eight	а	
46	2	Sway Bracing	DEF	S	Deformed	Panel 20 @ Bay 1		Slight	One	с	
47	2	Sway Bracing	DEF	S	Deformed	Panel 23 @ Bay 1		Slight	One	С	
48	2	Sway Bracing	М	S	Missing Rivet	Panel 15 @ Bay 2	1 Piece	Slight	One	С	
49	2	Sway Bracing	DEF	S	Deformed	Panel 17 @ Bay 2		Slight	One	c	221
50	2	Sway Bracing	М	Н	Missing Rivet	Panel 20 @ Bay 2	Four Pieces	Heavy	Twenty Four	а	
51	2	Sway Bracing	DEF	S	Deformed	Panel 21 @ Bay 3		Slight	One	c	
52	2	Pier 1 (Wall)	SER	S	Exposed Rebar	Bottom of Center Column @ Upstream	A = 0.45 m ²	Slight	One	с	167
53	2	Pier 1 (Wall)	SER	s	Exposed Rebar	Side Bottom of Column @	A = 0.40 m ²	Slight	One	с	168
54	3	Girder 8	м	м	Loose Botls/Missing	Downstream Side Panel 10 @ Bay 8	Three Pieces	Moderate	three	b	
55	3	Top Chord @ Girder 7	M	s	Rivet Missing Rivet Head	@ Bay 8					
		_					One Piece	Slight	one	c	
56	3	Top Chord @ Girder 7	M	S	Missing Bolt	Panel 8 @ Bay 7	One Piece	Slight	one	С	218
57	3	Top Chord @ Girder 7& 8	М	м	Missing Bolt	Panel 6 @ Bay 7	Two Piece	Moderate	two	b	217
58	3	Top Chord @ Girder 8	м	S	Missing Bolt	Girder 7, Panel 8 @ Bay 7	One Piece	Slight	one	C ·	
59	3	Top Chord @ Girder 8	м	м	Missing Rivel Head	@ Bay8	Two Piece	Moderate	Two	b	
60	3	Top Chord @ Girder 2	co	M ·	Corrosion	Panel 4 & 6 @ Bay 1	Reduction of Cross Section	Moderate	One	b	220
61	3	Stringer	M	S	Missing Bolt	@ Panel 3	One Piece	Slight	One	с	23A
62	3	Stringer	L/M	s	Loosed Bolt and	@ Panel 3	One Piece	Slight	One	 c	23A 22A
					Nut	_					
63	3	Sway Bracing	M	<u>M</u>	Missing Rivet	@ Panel 3	One Piece	Slight	Two	b	27A
64	3	Cross Beam	DEF	<u>s</u>	Deformed	@ Panel 8		Slight	One	C	30
65	3	Sway Bracing	DEF	<u> </u>	Deformed	Panel 9	One Location	Slight	Four	c	31
66	3	Sway Bracing	M	<u>M</u>	Missing Rivet	Panel 1	One Piece	Moderate	Four	b	
67 68	3	Sway Bracing	M CO	M S	Missing Rivet Not	Panel 2	One Piece	Moderate	Two	b	26A
69	3	Sway Bracing Sway Bracing	DEF	M	Welded/Corrosion Deformed	Panel 5 Panel 6	Remarkable Deflection	Slight Moderate	c b	c b	28A 29
70							Dencedori				
	3	Sway Bracing	DEF	<u>S</u>	Deformed	Panel 10		Slight	One	C	32
71	3	Sway Bracing	<u>M</u>	<u>s</u>	Missing Rivet	Panel 2 @ Bay 6	One Piece	Slight	One	c	214
72	3	Sway Bracing	M	<u> </u>	Missing Rivet	Panel 2 @ Bay 6	One Piece	Slight	one	C	215
73	3	Sway Bracing	<u>M</u>	S	Missing Rivet	Panel 2 @ Bay 6	One Piece	Slight	One	C	216
74	3	Stringer	<u></u>	<u>s</u>	Corrosion	@ Panel 3	Surface Rust	Slight	One	c	21A
	3	Stringer	M	M	Missing Rivet	@ Panel 10	One Piece	Moderate	Two	b	33
76 77	3	Sway Bracing	DEF	S	Deformed	@ Panel 11		Slight	One	c	34
78	3	Stringer	DEF	M	Deformed	@ Panel 11		Moderate	Two	b	35
79	3	Abutment B	SER	S	Spall	Backwall	A = 0.14 m ²	Slight	One	c	24A
	3	Pier 2 (Wall)	CR	<u> H </u>	Vertical Crack	Upstream Side of Wall	l = 1.753 mm	Heavy	One	a	65
80	3	Pier 2 (Wall)	CR	. Н	Vertical Crack	Upstream Side of Wall	t = 1.061 mm	Heavy	One	a	66
81	3	Pier 2 (Wall)	CR	н	Vertical Crack	Construction Jopint along Upstream Side	t = 1,194 mm	Heavy	One	a	67
82	3	Pier 2 (Wall)	CR	н	Vertical Crack	Center Face of Pier @	l = 0.635 mm	Heavy	One		
83	3	Pier 2 (Wall)	CR	н	Vertical Crack	Downstream Side Downstream Side of Pler Wall	l = 2.794 mm	Heavy	One	a	68 69
84	3	Pier 2 (Wall)	D/D	S	Cavity	Bottom Face of Pler Wall @ Downstream Side	A = 0.44 m ²	Slight	One	c	70-71
85	3	Extended Coping	CR	н	Vertical Crack	Face of Coping @ Pier 2	l = 2.784 mm	Heavy	One	а	50
86	3	Extended Coping	CR	IVI	Crack	Face of Coping along Bay 5	t = 0,203 mm	Moderate	Оле	b	52
87	3	Extended Coping	CR	н	Vertical & Horizontal	Face of Coping along Bay	t = 0.584 mm	Heavy	One	а	53

Name of Bridge : Pa9.1 GUADALUPE BRIDGE (DOWNSTREAM)

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 Date of Inspection
 Dec. 3,4,9&10, 2002

 Inspector
 R. Abad / J. Abadam

 Checker
 J. Agnes

Description of Damage Span No. Type of Damage Damage Rank of Photo No. Remarks Name of Member No. of No. Damage Nature Severity Scale (a, b, c) Pattern Damage SER 1 1 Railing s Exposed Rebar 4.5 m from Abut. A A = 0.10 m² Slight One с 1 2 Cantilever Slab & Rail Post SER 1 м Exposed Rebar Side of Cantilever Slab A = 0.12 m² Moderate Two b 2 3 s 14.5 m from Abutmer 1 Curb SER Spall A = 0.03 m² Slight One с 3 Α 4 CR 1 PSC Girder G-1 М Crack Pier 1 Face of Girder l = 0.254 mm Moderate One b 121 Bot of Slab @ Bay 1, 2.0 m fr. Pier 1 Pier 1 Face of 5 1 CR s Deck Slab Random Cracks t = 0.173 mm Slight Many с 122 6 End Diaphragm Vertical and 1 CR н t = 0.610 mm Heavy Three а 123 Horizontal Cracks Diaphragm @ Bay 1 7 1 CR М Pier 1 (Wall) Vertical Crack t = 0.330 mm 124 Below Girder 2 Moderate One b 8 1 Pier 1 (Wall) CR s Vertical Crack Below Girder 2 l = 0,152 mm Slight 125 One С 9 1 Pier 1 (Wall) CR s Vertical Crack t = 0.102 mm Below Girder 3 126 Slight One с

Appendix 7.1.1-1 (9/34)

Damage	Span	Name of Member	Type of	Rank of			scription of Damag	e	No of	Remarks	Pho
No.	No.	Name of Member	Damage	Damage	Nature	Location/ Pattern	Scale	Severity	No. of Damages	(a, b, c)	No
10	1	Pier 1 (Wall)	CR	s	Vertical Crack	Below Girder 3	t = 0.152 mm	Slight	One	с	12
11	1	Pier 1 (Wall)	CR	M	Vertical Crack	Below Girder 3	t = 0.305 mm	Moderate	One	b	128
12	1	End Diaphragm	CR		Vertical Crack	Face of Diaphragm @			+	-	
				н		Bay 2 Face of Diephragm @	(= 0.010 mm	Heavy	One	а	129
13	1	End Diaphragm	CR	м	Vertical Crack	Bay 2	1 - 0.432 1111	Moderate	One	b	130
14	1	End Diaphragm	CR	S	Vertical Crack	Face of Diaphragm @ Bay 3	1 - 0.254 min	Slight	One	c	131
15	1	End Diaphragm	CR	S	Vertical Crack	Face of Diaphragm @ Bay 3	t = 0.229 mm	Slight	One	c	132
16	1	PSC Girder G-4	CR	S	Random Cracks	Face of Girder	t = 0.173 mm	Slight	Many	с	133
17	1	PSC Girder G-4	SER	s	Exposed Rebar	Face of Girder	A = 0.01 m ²	Slight	Two	с	134
18	1	PSC Girder G-4	CR	s	Crack	Face of Girder	t = 0.173 mm	Slight	One	c	135
19	1	Cantilever Slab	SER	s	Exposed Rebar						1
	· · · · ·					Bottom of Slab Bearing Shoe @	A = 0.03 m ²	Slight	One	с 	130
20	1	Bearing Shoe	SER	S	Spall	Girder 4	A = 0.052 m ²	Slight	One	c	137
21	1	PSC Girder G-4	SER	S	Spall	Top of Rocker @ Girder	A = 0.05 m ²	Slight	One	с	13
22	1	PSC Girder G-1	SER	S	Spall	Face of Girder	A = 0.02 m ²	Slight	One	c	146
23	2	Pier 2 (Wall)	SER	S	Exposed Rebar	Face of Pier	A = 0.09 m ²	Slight	Two	с	77
24	2	Pier 2 (Wall)	CR	н	Verticaland	1.5 m from Top of	t = 0.610 mm	Heavy	Two	a	78
25	2	Fender	FR	м	Horizontal Cracks Fracture	Footing Downstream Side	Whole				
								Moderate	One	b	79
26	2	Pier 2 Footing	CRPL	M	Pothole	Top of Footing	A = 0.07 m ²	Moderate	ь	c	80
27	2	Pier 2 Footing	SER	S	Spall	Edge of Footing	A = 0.03 m ²	Slight	One	c	81
28	2	PSC Girder G-1	SER	S	Exposed Rebar	Bottom of Girder	A = 0.05 m ²	Slight	One	с	82
29	2	PSC Girder G-1	SER	s	Exposed Rebar	Face of Girder	A = 0.01 m ²	Slight	One	с	83
30	2	PSC Girder G-1	SER	s	Exposed Rebar	Face of Girder	A = 0.01 m ²	Slight	One	c	84
31	2			s							
		End Diaphragm @ Bay 1	CR		Vertical Crack	Front Face Boltom of Cantilever	t = 0,254 mm	Slight	One	c	85
32	2	Cantilever Slab	CR	S	Random Cracks	Slab	t = 0.229 mm	Slight	Four	с	86
33	2	PSC Girder G-1	CR	Н	Horizontal Crack	Face of Girder near Gerber	t = 0.406 mm	Heavy	One	a	91
34	2	PSC Girder G-1	CR	н	Vertical Crack	Face of Girder near Gerber	l = 3.048 mm	Heavy	One	а	92
35	2	PSC Girder G-1	CR	M	Diagonal Crack	Face of Girder near	t = 0.381 mm	Moderate	Two	b	93
36	2	PSC Girder G-1	CR			Gerber Face of Girder near					
				M	Vertical Crack	Gerber Face of Girder near	t = 0.305 mm	Moderate	One	b	94
37	2	PSC Girder G-1	CR	M	Vertical Crack	Gerber	l = 0.254 mm	Moderate	One	b	95
38	2	PSC Girder G-1	CR	М	Random Cracks	Face of Girder near Gerber	t = 0.254 mm	Moderate	Three	b	96
39	2	Extended Coping	CR	м	Vertical Crack	Face of Coping	t = 0.406 mm	Moderate	One	b	97
40	2	PSC Girder G-1	SER	s	Exposed Rebar	Bottom of Girder	$A = 0.03 m^2$	Slight	One	с	98
41	2	PSC Girder G-1	SER	s	Exposed Rebar	Bottom End of Girder	A = 0.01 m ²		One		
						@ Pier 2 Gerber Hinge		Slight		c	99
42	2	End Diaphragm	HC	М	Honeycomb	@ Bay 1	A = 0.20 m ²	Moderate	One	b	100
43	2	PSC Girder G-1	CR	M :	Diagonal Crack	Face of Girder	t = 0.254 mm	Moderate	One	b	101
44	2	End Diaphragm	CR	S	Random Cracks	Bottom of Diap. @ Bay 1	t = 0.076 mm	Slight	Тwo	с	102
45	2	PSC Girder G-2	CR	s	Vertical Crack	Face of Girder	t = 0.102 mm	Slight	One	c	103
46	2	End Diaphragm	CR	s	Crack	Bottom of Disphragm	t = 0.127 mm	Slight	One	с	104
47	2	End Diaphragm	CR	s	Crack	@ Bav 2 Bottom of Diaphragm	t = 0.102 mm	Slight		· · · ·	105
						@ Bav 2 Bollom of Diaphragm			One	c	
48	2	End Diaphragm	CR	м	Crack	@ Bay 2 Gerber Hinge	t = 0.305 mm	Moderate	Three	b	106
49	2	End Diaphragm	HC	М	Honeycomb	@ Bay 2	A = 0.20 m ²	Moderate	One	ъ	107
50	2	PSC Girder G-3	SER	S	Exposed Rebar	Bottom of Girder @ Gerber Hinge	A = 0.01 m ²	Slight	One	с	108
51	2	End Diaphragm	HC	s	Honeycomb	Gerber Hinge @ Bay 3	A = 0.09 m ²	Slight	One	с	109
52	2	End Diaphragm	НС	м	Honeycomb	Gerber Hinge	A = 0.23 m ²	Moderate	One	b	110
53	2	End Diaphragm	SER	s	Exposed Rebar	@ Bay 3 Gerber Hinge					
						@ Bay 3	A = 0.03 m ²	Slight	One	c	111
54	2	PSC Girder G-4	CR	S	Diagonal Crack	Face of Girder Boltom of Slab @ Bay	t = 0.173 mm	Slight	One	с	112
55	2	Deck Slab	CR/FL	м	Crack /Free Lime	3	t = 0.40 mm	Moderate	One	b	113
56	2	PSC Girder G-4	CR	н	Random Cracks	Face of Girder @ Gerber Hinge	l = 0.432 mm	Heavy	Three	а	114
57	2	PSC Girder G-4	CR	н	Random Cracks	Face of Girder @ Gerber Hinge	.t = 2.286 mm	Heavy	Five	а	115
58	2	PSC Girder G-4	SER	S	Spall	Face of Girder @	A = 0.09 m ²	Slight	One	с	116
59	2	PSC Girder G-4	SER	S	Exposed Rebar	Gerber Hinge Face of Girder	A = 0.01 m ²	Slight	One	c	117
60	2					Face of Girder @					
_		PSC Girder G-4	CR	S	Diagonal Cracks	Gerber Hinge Bottom of Cantilever	l = 0.076 mm	Slight	Two	c	118
61	2	Cantilever Slab	CR	S	Crack	Slab	t = 0.102 mm	Slight	One	Ċ	119
62	2	PSC Girder (G-1)	SER	S	Spall	Top of Rocker	A = 0.04 m ²	Slight	One	с	120
63	2	Bearing Shoe	L/M	s	Loose Nut & Rusted	Bearing @ Girder One	1 Nut	Slight	One	с	120
64	2	PSC Girder G-1	CR	н	Random Cracks	Face of Girder @	l = 0.584 mm	Heavy	Many	а	138
65	2	PSC Girder G-1	CR	н	Random Cracks	Gerber Hinge Face of Girder @	t = 1.092 mm				
						Gerber Hinge Face of Girder @		Heavy	Many	а	139
66	2	PSC Girder G-1	SER	S	Exposed Rebar	Gerber Hinge	A = 0.02 m ²	Slight	One	c	140
67	2	Cantilever Slab	SER	S	Spall	Bottom of Slab @ Gerber Hinge	A = 0.03 m ²	Slight	One	с	141
68	2	PSC Girder G-1	CR	н	Random Cracks	Face of Girder @ Gerber Hinge	l = 2.280 mm	Heavy	Seven	а	142
69	2	Pier 1 (Wall)	CR	м	Random Cracks	Face of Pier	t = 0.483 mm	Moderate	Малу	b	143
70	2	Pier 1 (Wall)	CR	н	Vertical Crack	Face of Pier	t = 1.169 mm	Heavy	Two	а	144
71	2	Pier 1 (Wall)	SER	s	Exposed Rebar	Face of Pier	$A = 0.12 m^2$				
•• 1	~			3	-vhosen vepsi.	Bollom of Girder @	M - U.12 M	Slight	Two	C	145

Appendix 7.1.1-1 (10/34)

Damage	Span		Type of	Rank of			cription of Damag	e	····	Remarks	Photo
No.	No.	Name of Member	Damage	Damage	Nature	Location/ Pattern	Scale	Severity	No. of Damages	(a, b, c)	No.
73	2	PSC Girder G-2	SER	м	Exposed Rebar	Bottom of Girder @. Gerber Hinge	A = 0.16 m ²	Moderate	One	b	148
74	2	PSC Girder G-2	SER	S	Exposed Rebar	Bottom of Girder	A = 0.02 m ²	Slight	Two	с	149
75	2	PSC Girder G-2	SER	S	Exposed Rebar	Bottom of Girder	A = 0.02 m ²	Slight	One	с	150
76	2	End Diaphragm	SER	S	Exposed Rebar	Bottom of Girder @ Gerber Hinge	A = 0.06 m ²	Slight	One	с	151
77	2	PSC Girder G-3	SER	м	Exposed Rebar	Bottom of Girder @ Gerber Hinge	A = 0.25 m ²	Moderate	Two	b	152
78	2	PSC Girder G-3	SER	S	Exposed Rebar	Bottom of Girder	A = 0.02 m ²	Slight	One	с	153
79	2	PSC Girder G-3	HC	м	Honey Comb	Bottom of Girder @ Gerber Hinge	A = 0.21 m ²	Moderate	One	b	154
80	2	End Diaphragm	SER	S	Exposed Rebar	Face of Diaphragm	A = 0.01 m ²	Slight	Two	с	155
81	2	PSC Girder G-4 & End Diaphgram	HC	м	Honey Comb	Bottom of Diaphragm & Girder @ Gerber	A = 0.12 m ²	Moderate	One	b .	156
82	2	PSC Girder G-4	SER	м	Exposed Rebar	Bottom of Girder	A = 0.12 m ²	Moderate	One	b	157
83	2	PSC Girder G-4	CR	м	Diagonal Crack	Face of Girder & Gerber	t = 0.229 mm	Moderate	One	b	158
84	2	PSC Girder G-4	SER	S	Exposed Rebar	Face of Girder	A = 0.06 m ²	Slight	Three	с	159
85	2	PSC Girder G-4	CR	н	Random Cracks	Face of Girder @ Gerber Hinge	t = 0.610 mm	Heavy	Two	а	160
86	2	PSC Girder G-4	CR	S	Vertical and Horizontal Cracks	Face of Girder	t = 0.173 mm	Slight	• Two	c ·	161
87	2	PSC Girder G-4	CR	S	Diagonal Cracks	Face of Girder	t = 0.127 mm	Slight	Two	c	162
88	2	PSC Girder G-4	SER	S	Exposed Rebar	Face of Girder	A = 0.01 m ²	Slight	One	с	163
89	2	Cantilever Slab	SER	м	Exposed Rebar	Side Face of Slab	A = 0.10 m ²	Moderate	Two	b	164
90	2	Fender	FR	м	Fracture	At Bottom Above Footing		Moderate	One	b	165
91	3	Railing	SER	S	Exposed Rebar	3.0 m from Abut. B	A = 0.01 m ²	Slight	Two	с	4
92	3	Steel Angle Expansion Joint	FR	М	Fracture	2.5 m from Median Curb	L ≠ 0.82 m	Moderate	One	b	6
93	3	Pier 2, shear Block & Coping	CR	s	Crack	Construction Joint of Shear Block and	t = 0.229 mm	Slight	Two	с	
94	3	Pier 2 Coping	SER	S	Spall	Face of Coping	A = 0.04 m ²	Slight	One	c	
95	3	PSC Girder G-2	HC	S	Honeycomb	Bottom of Girder	A = 0.03 m ²	Slight	One	с	
96	3	Abutment B Shear Block	CR	S	Vertical Cracks	Face of Shear Block	t = 0.173 mm	Slight	Six	с	
97	3	Abutment B Extended	CR	s	Random Cracks	Face of Coping	l = 0.102 mm	Slight	Many	с	
98	3	Pier 2 (Wall)	SER	S	Exposed Rebar	1.5 m from Bottom of Girder DS	A = 0.06 m ²	Slight	Оле	c	36
99	3	PSC Girder G-4	CR	s	Vertical Crack	Face of Girder	t = 0.076 mm	Slight	One	с	37
100	3	Pier 2 (Wall)	CR	н	Random Vertical Cracks	Front Face	t = 0.610 mm	Heavy	Two	а	72
101	3	Pier 2 (Wall)	CR	м	Vertical Crack	Front Face	t = 0.432 mm	Moderate	One	b	73
102	3	Fender	SER	S	Exposed Rebar	Face of Fender	A = 0.01 m ²	Slight	Many	с	74
103	3	Pier 2 (Wall)	HC	s	Honeycomb	Downstream Face of Pier	A = 0.02 m ²	Slight	Three	с	75
104	3	Fender	SER	s	Exposed Rebar	Bottom Face of Fender	A = 0.18 m ²	Slight	One	c	76
105		Rail Post	SER	S	Exposed Rebar	Along approach road @ D/S	A = 0.10 m ²	Slight	One	с	5

Name of Bridge : Pa9.2 GUADALUPE BRIDGE (UPSTREAM)

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 Date of Inspect
 12/3,4,9&10, 2002

 Inspector
 R. Abad / J. Abadam

 Checker
 J. Agnes

Damage	Span		Type of	Rank of		Des	cription of Damage			Remarks	Photo
No.	No.	Name of Member	Damage	Damage	Nature	Location/ Pattern	Scale	Severity	No. of Damages	(a, b, c)	No.
1	1	Cantilever Slab	SER	м	Exposed Rebar	Side Face of Slab	A = 0.12 m ²	Moderate	One	b	187
2	1	Intermediate Diaphragm	SER	S	Exposed Rebar	Bottom of Diaphragm @ Bay 2	A = 0.10 m ²	Slight	One	с	213
3	2	PSC Girder G-1	SER	s	Exposed Reabr	Face of Girder	A = 0.03 m ²	Slight	One	c	38
4	2	Gerber Hinge	SER	м	Spall	Bottom of Girder and End Diaphragm	A = 0.177 m ²	Moderate	Three	b	41
5	2	Pier 2 (Wall)	SER	м	Exposed Reabr	Face of Column	A = 1.14 m ²	Moderate	One	b	60
6	2	Pier 2 (Wall)	SER	S.	Exposed Reabr	Center of Pier Wall	A = 0.09 m ²	Slight	One	с	61
7	2	Pier 2 (Wall)	CR	S	Random Map Cracking	Face of Pier 2	t = 0.076 mm	Slight	Many	с	62
8	2	Pier 2 (Wall)	CR	S	Vertical and Horizontal Cracks	Face of Pier 2	t = 0.152 mm	Slight	Two	c ·	63
9	2	Pier 2 (Footing)	SER	S	Exposed Reabr	Footing @ Upstream	A = 1.00 m ²	Slight	Many	c	64
10	2	Intermediate Diaphragm @ Bay 3	SER	S	Exposed Reabr	Bottom of Diaphragm	A = 0.01 m ²	Slight	One	с	87
11	2	Intermediate Diaphragm	CR	S	Vertical and Longitudinal Cracks	Bottom of Diaphragm	t = 0.173 mm	Slight	Two	с	88
12	2	PSC Girder G-1	CR	S	Longitudinal Crack	Top Flange of Girder	(= 0.102 mm	Slight	Two	с	89
13	2	PSC Girder G-1	CR	s	Vertical Crack	Face of Girder	t = 0.102 mm	Slight	Оле	c	90
14	2	Utilities Hanger	со	н	Corrosion	Bottom of Hanger	Remarkable Corrosion Spread Over Member	Heavy	Two	а	166
15	2	Fender Beam	FR	н	Frectured	At End Near Pier		Heavy	Two	а	169
16	2	Pier 1 (Wall)	CR	S	Random Cracks	Face of Pier	t = 0.254 mm	Slight	Three	с	170
.17	2	PSC Girder G-4	нс	н	Honeycomb	Bollom of Girder	A = 0.96 m ²	Heavy	One	а	171
18	2	End Diaphragm	HC	М	Honeycomb	Bottom of Diaphragm	A = 0.18 m ²	Moderate	One	С	172
19	2	End Diaphragm	CR	S	Crack	Bollom of Diaphragm @ Bay 3	1 = 0.102 mm	Slight	One	с	173
20	2	End Diaphragm	нс	S	Honeycomb	Top Face of Diaphragm @	A = 0.09 m ²	Slight	One	с	174
21	2	End Diaphragm	нс	S	Honeycomb	Bollom of Diaphragm @ Bay 3	A = 0.06 m ²	Slight	One	с	175
22	2	PSC Girder G-3	SER	S	Exposed Rebar	Bottom of Girder	A = 0.25 m ²	Slight	One	с	176
23	2	PSC Girder G-3	SER	S	Exposed Rebar	Boltom of Girder	A = 0.60 m ²	Slight	Two	c	177
24	2	End Diaphragm	SER	S	Exposed Rebar	Bottom of Diaphragm @ Bay 2	A = 0.05 m ²	Slight	One	с	178
25	2	End Diaphragm	SER	н	Exposed Rebar	Bottom of Diaphragm	A = 0.35 m ²	Heavy	One	a	179

Appendix 7.1.1-1 (11/34)

Damage	Span	1	Type of	Rank of		Des	cription of Damag	e		Remarks	Photo
No.	No.	Name of Member	Damage	Damage	Nature	Location/ Pattern	Scale	Severity	No. of Damages	(a, b, c)	No.
26	2	End Diaphragm	SER	S	Spall	Bottom of Diaphragm @ Bay 1	A = 0.18 m ²	Slight	One	b	180
27	2	End Diaphragm	SER	s	Exposed Rebar	Top Face of Diaphragm @	A = 0.05 m ²	Slight	One	c	181
28	2	End Diaphragm	SER	S	Exposed Rebar	Bottom of Diaphragm	A = 0.09 m ²	Slight	One	c	182
29	2	PSC Girder G-1	CR	s	Random Cracks	Face of Girder @ Gerber Hinge	t = 0.127 mm	Slight	Four	с	183
30	2	PSC Girder G-1	CR	S	Random Cracks	Face of Girder @ Gerber	t = 0.173 mm	Slight	many	с	184
31	2	PSC Girder G-1	CR	S	Horizontal Cracks	Face of Girder	l = 0.152 mm	Slight	Two	c	185
32	2	Pier 1 (Wall)	SER	S	Exposed Rebar	Face of Pier	A = 0.30 m ²	Slight	One	c	186
33	2	Fender	FR	м	Fractured			Heavy	One	b	188
34	2	PSC Girder G-4	SER	S	Exposed Rebar	Face of Girder	A = 0.02 m ²	Slight	One	c	189
35	2	PSC Girder G-4	SER	м	Exposed Rebar	Bottom of Girder	A = 0.20 m ²	Moderate	One	b	190
36	2	PSC Girder G-4	SER	s	Exposed Rebar	Bottom of Girder @ Bay 3	A = 0.05 m ²	Slight	One	с	191
37	2	PSC Girder G-3	SER	s	Exposed Rebar	Bottom of Girder	A = 0.05 m ²	Slight	One	с	192
38	2	PSC Girder G-2	CR	s	Crack	Bottom of Girder	t = 0.173 mm	Slight	One	с	193
39	2	PSC Girder G-2	SER	s	Spalt	Botlom of Girder	A = 0.06 m ²	Slight	One	с	194
40	2	End Diaphragm	нс	М	Honeycomb	Bollom of Girder @ Bay 1	A = 0.12 m ²	Moderate	One	b	195
41	2	End Diaphragm	SER	S	Exposed Rebar	Bottom of Diaphragm	A = 0.08 m ²	Slight	One	с	196
42	2	PSC Girder G-1	SER	М	Exposed Rebar	Bottom of Girder	A = 0.12 m ²	Moderate	One	b	197
43	2	PSC Girder G-1	SER	S	Exposed Rebar	Bollom of Girder	A = 0.01 m ²	Slight	Two	с	198
44	2	PSC Girder G-1	CR	Н	Random Cracks	Face of Girder @ Gerber	t = 1.194 mm	Heavy	Many	а	199
45	2	PSC Girder G-1	CR	М	Random Cracks	Face of Girder @ Gerber	t = 0.203 mm	Moderate	Many	b	200
46	2	PSC Girder G-1	CR	s	Random Crack	Face of Girder @ Gerber	t = 0.173 mm	Slight	Many	c	201
47	2	PSC Girder G-1	CR	м	Crack	Face of Girder @ Gerber	t = 0.203 mm	Moderate	One	b	202
48	2	PSC Girder G-1	CR	S	Crack	Face of Girder @ Center Span	t = 0.173 mm	Slight	One	с	203
49	3	PSC Girder G-1	SER	s	Spall	Top of Rocker	A = 0.06 m ²	Slight	One	с	39
50	3	Cantilever Slab	SER	S	Exposed Rebar	Bottom of Cantilever Stab,	A = 0.09 m ²	Slight	One	с	40
51	3	Pier 2 (Wall)	SER	s	Exposed Rebar	Pier Well @ Bay 3	A = 0.06 m ²	Slight	One	с	42
52	3	PSC Girder G-1	нс	н	Honeycomb	Bottom of Girder	A = 0.66 m ²	Heavy	One	а	43
53	3	PSC Girder G-1	нс	S	Honeycomb	Face of Girder	A = 0.01 m ²	Slight	One	с	44
54	3	Extended Coping	CR	S	Random Cracks	Front Face Below Girder G-2	t = 0.229 mm	Slight	Three	с	45
55	3	End Diaphragm @ Bay 2	CR	S	Random Cracks	Front Face	l = 0.102 mm	Slight	Two	с	46
56	3	Extended Coping	CR	s	Random Cracks	Front Face Below Girder G-3	t = 0.229 mm	Slight	Three	с	47
57	3	End Diaphragm @ Bay 3	CR	S	Vertical Crack	Front Face	t = 0.173 mm	Slight	One	c	48
58	3	PSC Girder G-4	SER	S	Spall	Top of Rocker	A = 0.06 m ²	Stight	One	c	49
59	3	Utilities	M	s	Missing Portion	D/S of Girder G-4	L = 0.80 m	Heavy	Many	c	51
60	. 3	PSC Girder G-4	SER	s	Exposed Rebar	Top Flange of Girder	A = 0.01 m ²	Slight	Three	c	54
61	3	Cantilever Slab	SER	s	Spall	Bottom of Cantilever	A = 0.03 m ²	Slight	Many	c	55
62	3	PSC Girder G-1	HC	н	Honeycomb	Slab Bottom of Girder	A = 0.4 m ²	Heavy	One	a	56
63	3	PSC Girder G-1	SER	s	Exposed Rebar	Face of Girder	A = 0.04 m ²	Slight	Three	c	57
64	3	Cantilever Slab	нс	S	Honeycomb	Haunch of Cantilever	A = 0.06 m ²	Slight	One	c	58
65	3	Fender	FR	м	Damaged	Slab Pier 2 Upstream		Heavy	Öne	b ·	59

Name of Bridge : Pa10 C - 5 BRIDGE

 Date of Inspe
 Nov. 22/Dec. 10-13,2002

 Inspector
 E. Pagaragap/R. Quiwa

 Checker
 J. B. Agnes

Damage	Span	··· ··· ·	Type of	Rank of		Dese	cription of Damage)		Remarks	Photo
No.	No.	Name of Member	Damage	Damage	Nature	Location/ Pattern	Scale	Severity	No. of Damages	(a, b, c)	No.
1	1	Abutment A Wing Wall	CR	S	Random Cracks	Outer Face	t = 0.076 mm	Slight	Several	c	58a/61
2	1	Abutment A	CR	м	Horizontal Crack	Backwall	l = 0.356 mm	Moderate	One	b	60
3	1	Abutment A Back Wall	нс	s	Honeycomb	Face of Backwall		Slight	One	c	
4	1	Abutment A	CR ·	S	Diagonal Crack	Below end of Risers	t = 0.076 mm	. Slight	Two	с	59
5	1	Pier 1 Coping	CR	S	Vertical Crack	Bay 2 near Girder G-2	t = 0.30 mm	Slight	One	с	62
6	1	Pavement	CRPL	н	Pothole	Above P1	0.3 m ø	Heavy	One	а	365
7	2	Railing	FR ·	s	Fractured Railing	2nd Panel From P1	A = 0.06 m ²	Slight	One	C ·	63
8	2	Railing/Rail Post	SER	S	Spell	Lower rail & Post 8 m from P2 DS	A = 0.02 m ²	Slight	One	c	64
9	2	Railing	SER	S	Exposed Rebar	3 m. from P1	A = 0.03 m ²	Slight	One	с	356
10	2	Pavement	CRPL	н	Pothole	1.2 m. from P2	0.7 m dia.	Heavy	One	a [.]	357
11	3	Railing/Rail Post	SER	s	Spall Railing & Rail Post	P3 Downstream Side	A = 0.03 m ²	Slight	One	с	65
12	3	Pavement	CRPL	S	Concrete Spill		•	Slight	One	с	66/67
13	3	Railing / Ralpost	SER	s	Exposed Rebar	3 m. from P2	A = 0.01 m ²	Slight	One	с	358
14	3	Railing	SER	S	Exposed Rebar	10.3 m from P3	A = 0.05 m ²	Slight	Several	с	359
15	3	Curb	SER	S	Spall	Above P3	A = 0.08 m ²	Slight	One	с	360
16	3	Sidewalk	SER	S	Spall	Above P3	A = 0.015 m ²	Slight	One	c	366
17	4	Pier 3 Coping	CR	S	Crack	End of Riser at Bay 5	t = 0.072 mm / t = 0.152 mm	Slight	One	c	13/14
18	4	Pier 3 Coping	CR	S	Crack	End of Riser at Bay 4	t = 0.173 mm	Slight	One	C	15
19	4	Pier 3 Coping	CR	S	Crack	Bay 3	l = 0.102 mm	Slight	One	с	16

Appendix 7.1.1-1 (12/34)

Damage No.	Span No.	Name of Member	Type of Damage	Rank of Damage	Nature	Des Location/ Pattern	scription of Damag Scale	e Severity	No. of Damages	Remarks (a, b, c)	Phot No.
20	4	Pier 3 Coping	CR	S	Vertical Crack	Bay 1 & 2	l = 0.102 mm	Slight	One	с	17/1
21	4	PSC Girder G-12	SER	s	Spalling	End of Girder	A = 0.01 m ²	Slight	One	c	1
22	4	Pier 3 Coping	CR	м	Vertical Crack	Down Stream Side	l = 0.356 mm	Moderate	One	b	2
23	4	Restraining Bar @ Pier 3	со	н	Corrosion	Pier 3	Reduction in Cross	Heavy	All	a	3
24	4	End Diaphragm @ P3	SER	s	Spall	End Diaphgram	A = 0.04 m ²	Slight	One	c	4
25	4	Pier 3 Coping	CR	s	Vertical Crack	End of Riser	t = 0,156 mm	Slight	One	c	5
26	4	Pier 3 Coping	CR	s	Vertical Crack	End of Riser	l = 0.173 mm	Slight	One	c	6
27	4	End Diaphragm at Pier 3	CR	s	Vertical Crack	Bay 6	t = 0.152 mm	Slight	One	c	7
28	4	Deck Slab	нс	м	Honey Comb	Underside of Deck	A = 1.80 m ²	Heavy	One		8
29	4	Pier 3 Coping	CR	s	Vertical Crack	Slab at P3 at Bay6 Bay6	t = 0.102 mm		One	a	
30	4	Pier 3 Coping	нс	s	Honeycomb			Slight		c	9
31	4	PSC Girder G-6	SER	s	· · · · · · · · · · · · · · · · · · ·	Bay 6	A = 0.02 m ²	Slight	One	с	10
32					Spall	Bollom Side of Girder	A = 0.05 m ²	Slight	One	c	11
	.4	Pier 3 Coping	CR	S	Crack	Face of Coping	t = 0.076 mm	Slight	One	c	12
33	4	Railing	CR	S	Horizontal Crack	1.5 m. from P4	t = 0.09 mm	Slight	One	c	361
34	5	Sidewalk Slab	HC	н	Honeycomb	Bottom of Slab	A = 0.12 m ²	Moderate	One	b	251
35	5	Sidewalk Slab	CR	S	Random Cracks	Bottom of Slab	t = 0.127 mm	Slight	Several	c	252
36	5	End of Diaphragm @ Bay 1 / P5	нс	S	Honeycomb	Haunch at Top	A = 0.01 m ²	Slight	Several	c	253
37	5	Restraining Bar	м	н	Missing	Bay 1	One Bay	Heavy	Two	а	254
38	5	PSC Girder G-2	SER	s	Spall	Bottom Flange @ Bay 1 Side	A = 0.02 m ²	Slight	One	c	255
39	5	PSC Girder G-3	SER	S	Spall	Bottom Flange @ Bay 2 Side	A = 0.10 m ²	Slight	Ône	c	256
40	5	Deck Siab @ Bay 3/P5	CR	s	Random Cracks	Bottom of Slab	t = 0.102 mm	Slight	One	c	257
41	5	PSC Girder G-4	SER	s	Spall	Bottom Flange @ Bay	A = 0.09 m ²	Slight	One	c	258
42	5	End Diaphragm @ Bay 4/P5	SER	 	Spall	4 Side Top of Diaphgram	A = 0.01 m ²	Slight	Several	с с	258
43	5	PSC Girder G-6	HC	s	Honeycomb	Bottom Flange @ Bay	A = 0.05 m ²				
44	5					4 Side		Slight	One	c	260
		End Diaphragm @ Bay 5/P5	HC	S	Honeycomb	Top of Diaphgram Top Frange @ Bay	A = 0.03 m ²	Slight	Several	C	261/2
45	5	PSC GIRDER G-7	SER	S	Spall	6/P5	A = 0.05 m ²	Slight	One	c	263
46	5	End Diapragm @ Bay 6/P5	SER	S	Exposed Rebar	Top of Diaphgram	A = 0.01 m ²	Slight	One	С	264
47	5	End Diapragm @ Bay 7/P5	CR	S	Vertical Crack	@ Bay 7/P5	t = 0.127 mm	Slight	One	С	267
48	5.	End Diapragm @ Bay 7/P5	SER	S	Exposed Rebar	Near Top	A = 0.01 m ²	Slight	One	с	266
49	5	PSC Girder G-9	SER	S	Spall	Bottom Flange @ Bay 7 P5	A = 0.02 m ²	Slight	One	с	265
50	5	PSC Girder G-9	SER	S	Spall	Bottom Flange @ Bay 8 P5	A = 0.06 m ²	Slight	One	c	270
51	5	End Diapragm @ Bay 9/P5	CR	s	Vertical Crack	Back Face NR G-8/P5	t = 0.173 mm	Slight	One	c	271
52	5	Railing	CR	н	Horizontal Crack	6 m from P4	t = 1.00 mm	Heavy	One	a	367
53	5	Coping	CR	M	Crack	Top of Shear Block @	l = 0.60 mm	Moderate	One	b	275
54	5	End Diaphragm @ Pier 4	нс	M	Honeycomb	Pier 5	A = 0.20 m ²				·
55	6					Front Face @ Bay 11 Bottom Flange		Moderate	One	b	348
		PSC Girder G-10	SER	<u>s</u>	Spall	@ Bay 10 Restraining Bar @	A = 0.07 m ²	Slight	Two	c	170
56	6	Restraining Bar	M	н	Missing	Bay 10 Bottom Flange	One Bay	Heavy	One	а	171
57	6	PSC Girder G-1'	SER	S	Spall	@ Bay 10	A = 0.06 m ²	Slight	One	c	172
58	6	End Diaphragm @ Pier 6	SER	М	Exposed Rebar	Bottom @ Bay 10	A = 0.12 m ²	Moderate	One	b	173
59	6	PSC Girder G-11	SER	S	Spell	Bottom Flange @ Bay 11	A = 0.02 m ²	Slight	One	с	174
60	6	Restraining Bar	м	н	Missing	Retraining Bars @ Bav 11	One Bay	Heavy	Two	а	175
61	6	Deck Slab	SER	S	Spall	Bottom of Slab	A = 0.10 m ²	Slight	One	с	176
62	6	PSC Girder G-12	SER	s	Spali	Bottom Flange @ Bay 11	A = 0.03 m ²	Slight	One	с	177
63	6	Pler 6 Coping	CR	н	Crack	End downstream Side	t = 2.134 mm	Heavy	One	а	178
64	6	PSC Girder G-12	SER	s	Spall	Spalling @ end	A = 0.03 m ²	Slight	One	c	179
65	6	Sidewalk slab	CR	н	Crack	Crack @ Bottom	l = 2.261 mm	Heavy	One	a	180
66	6	Pier 6 Coping	SER	м	Exposed Rebars	Down Stream Side	A = 0.15 m ²				
67	6	Deck Slab	HC			Bottom of Slab @ Bay		Moderate	One	b	181/18
		·· · · · · · · · · · · · · · · · · · ·		н	Honey Comb	7 Vertical Crack	A = 1.60 m ²	Heavy	One	a	158
68	6	Pier 6 Coping	CR	<u> </u>	Vertical Crack	@ Bay 7 Bottom Flange	l = 0.173 mm	Slight	One	c	159
69	6	PSC Girder G-7	SER	S	Spall	@ Bay 8	A = 0.08 m ²	Slight	One	c	160
70	6	Restraining Bar	м	н	Missing	Restraining Bar @ Bay 8	One Bay	Heavy	Two	а	161
71	6	Deck Slab	HC	н	Honeycomb	Bottom of Slab @ Bay 8	A = 0.45 m ²	Heavy	One	а	162
72	6	End Diaphragm @ Pier 6	нс	S	Honeycomb	Bottom Corner @ Bav 8	A = 0.03 m ²	Slight	One	с	163
73	6	Pier 6 Coping	CR	S	Vertical Crack	Vertical Crack @ Bay 8	t = 0.254 mm	Slight	One	с	164
74	6	Pier 6 Coping	CR	s	Vertical Crack	Below Girder G-9	l = 0.173 mm	Slight	One	с	165
75	6	PSC Girder G-9	SER	s	Spall	Bottom Flange	$A = 0.02 m^2$	Slight	One	c	166
76	6	Restraining Bar	M	н	Missing	@ Bay 9 Restraining Bar @	One Bay	Heavy	Two		167
77	6	End Diaphragm	HC	M	Honeycomb	Bay 9 Top and Haunch of				a	
78						Slab	$A = 0.12 m^2$	Moderate	One	b	168
	6	PSC Girder G-10	SER	S	Spall	Bottom Flange	A = 0.04 m ²	Slight	One	c	169
79	6	End Diaphragm	HC	S	Honeycomb	Bollom @ Bay 5	A = 0.10 m ²	Slight	One	с	147
80	6	Restraining Bar	м	н	Missing	Restraining Bar at Bay 5	One Bay	Heavy	Two	а	148
81	6	End Diaphragm @ Pier 6	нс	м	Honeycomb	Top and Bottom	A = 0.15 m ²	Moderate	Two	b	149
82	6	PSC Girder G-5	SER	S	Spall	TopFlange @ Bay 5	A = 0.04 m ²	Slight	Several	с	150

Appendix 7.1.1-1 (13/34)

Damage No.	Span No.	Name of Member	Type of Damage	Rank of Damage	Nature	De: Location/ Pattern	scription of Damag	e Severity	No. of	Remarks (a, b, c)	Photo No.
83	6	PSC Girder G-6	SER	S	Spall	Bottom Flange	A = 0.03 m ²	Slight	Damages Several	c	151
84	6	PSC Girder G-8	SER	м	Spall	@ Bay 5 Bottom Flange	A = 0.28 m ²	Moderate	Several	b	152
85	6	Restraining Bar	м	н	Missing	@ Bay 8 Restraining Bar @	One Bay	Heavy	Two	a	153
86	6	Deck Slab	SER	s	Spall	Bay 6 Bottom of Slab	A = 0.09 m ²	Slight	One	с с	154
87	6	End Diaphragm at Pier 6	НС	s	Spall	Тор	A = 0.06 m ²	Slight	One	c c	155
88	. 6	PSC Girder G-7	SER	s	Spall	Bottom Flange	$A = 0.02 \text{ m}^2$	Slight	One	c	155
89	6	PSC Girder G-7	SER	s	Spall	@ Bay 6 Boltom Flange	A = 0.06 m ²	Slight	One	c	156
90	6	Restraining Bar	M	н	Missing	@ Bay 7 Restraining Bar @	One Bay	Heavy	Two	a	157
91	6	PSC Girder G-3	SER	s	Spall	Bay 7 Top Flange	A = 0.08 m ²	Slight	One	c a	137
92	6	Pier 6 Coping	CR	м	Vertical Crack	Vertical Cracks @ Bay	1	Moderate	One		
93	6	Pier 6 Coping	CR	s	Random Cracks	2 Below Girder G-3	I = 0.229 mm			b	136
94	6	PSC Girder G-3	SER	S S		Bottom Flange near		Slight	One	c	137
95	6				Spall	end of Girder Restraining Bar @	A = 0.08 m ²	Slight	One	c	138
		Restraining Bar @ Pier 6	M	н	Missing	Bay 3	One Bay	Heavy	One	а	139
96	6	PSC Girder G-4	SER	M	Spall	Bottom Flange	A = 0.125 m ²	Moderate	One	b	140
97	6	End Diaphragm @ Pier 6	HC	S	Honeycomb	Bottom @ Bay 4	A = 0.02 m ²	Slight	One	с	141
98	6	End Diaphragm @ Pier 6	HC	S	Honeycomb	Top @ Bay 4 Bottom Flange	A = 0.016 m ²	Slight	One	с	142
99	6	PSC Girder G-5	SER	м	Spall	@ Bay 4	A = 0.15 m ²	 Moderate 	One	b	143
100	6	Restraining Bar	М	н	Missing	Restraining Bars @ Bav 4	One Bay	Heavy	One	а	144
101	6	PSC Girder G-5	SER	S	Spall	Spalling @ Bottom	A = 0.01 m ²	Stight	One	c	146
102	6	End Diaphragm	HC	S	Honeycomb	Upstream Side	A = 0.03 m ²	Slight	One	c	123
103	6	Sidewalk slab	HC	S	Honeycomb	Bottom	A = 0.070 m ²	Slight	One	c	124
104	6	Pier 6 Coping	SER .	s	Spall	Edge Upstream Side	A = 0.02 m ²	Slight	One	с.	125
105	6	PSC Girder G-1	SER	s	Spall	Bottom Flange near end of Girder	A = 0.02 m ²	Slight	One	с	126
106	6	PSC Girder G-1	SER	S	Spall	Botom Flange	A = 0.05 m ²	Slight	One	с	127
107	6	PSC Girder G-1	SER	S	Spall	Bollom of Girder	A = 0.04 m ²	Slight	One	c	128
108	6	PSC Girder G-1	SER	н	Spall	Top flange @ Bay 1	A = 0.40 m ²	Heavy	Оле	а	129
109	6	Pier 6 Coping	CR	s	Vertical & Horizontal Crack	Face of Coping @ U/S	t = 0.076 mm	Slight	One	с	130
110	6	PSC Girder G-2	SER	S	Spall	Bottom Flange @ Bay 1	A = 0.05 m ²	Slight	One	с	131
111	6	Restraining Bar @ Pier 6	·M.	н	Missing	Restraining Bar @ Bay 10	One Bay	Heavy	One	а	132
112	6	PSC Girder G-2	SER	s	Spall	Bottom Flange @ Bay 2	A = 0.08 m ²	Slight	One	с	133
113	6	Restraining Bar @ Pier 6	м	н	Missing	Restraining Bar @ Bay 2	One Bay	Heavy	Öne	а	134
114	6	Sidewalk slab	HC	м	Honeycomb	Bottom of Siab	A = 0.16 m ²	Moderate	One	b	279
115	6	Restraining Bar @ Pier 5	м	н	Missing	Restraining Bar @	One Bay	Heavy	Two	а	281
116	6	PSC Girder G-1	SER	M	Spall	Bay 10 Top flange @ Bay 1	$A = 0.175 m^2$	Moderate	One	b	283
117	6	Coping	CR	м	Random Cracks	Fracture @ Bay 4	l = 0.33 mm	Moderate	Several	b	297
118	6	PSC Girder G-6	SER	M	Spall	Bottom Flange	A = 0.15m ²	Moderate	One	b	300
119	6	PSC Girder G-8	SER	M	Spall	@ Bav 5 End Block @ Bay 8	A = 0.15m ²	Moderate	One	b	305
120	6	PSC Girder G-9	SER	M	Spall	Bottom Flange	A = 0.10m ²	Moderate	One		307
121	6	PSC Girder G-12	CR	н	Random Cracks	@ Bay 8 Ext. Face of End	t = 0.405 mm	Heavy	Several	D	316
122	6	Pier 5 Coping	CR	н	Random Cracks	Block Front Face Near End	l = 0.610 mm	Heavy	Several	a	
123	6	Pier 5 Coping	CR	н	Vertical Cracks	End of Coping @		· · · · · · · · · · · · · · · · · · ·		a	318
123	7					Downstream	t = 1.194 mm	Heavy	One	a	319
124	7	End Diaphragm @ Bay 5	HC	S	Honeycomb	Boltom of Diaphragm	$A = 0.020 \text{ m}^2$	Slight	One	c	203
125	7	Deck Slab @ Bay 5	CR	S	Random Cracks	Bottom of Slab	l = 0.149 mm	Slight	Several	c	204
		Pier 7 Coping	SER	S	Exposed Rebar	Back Face @ Bay 5	A = 0.020 m ²	Slight	One	с	205
127	7	Restraining Bar @ Bay 4	M	н	Missing	End Diaphgram	One Bay	Heavy	Two	а	206
128	7	Deck Slab @ Bay 4	CR	s	Random Cracks	Bottom of Slab	l = 0.102 mm	Slight	Several	c	207
129	7	End Diaphragm @ Bay 4	HC	s	Honeycomb	At Joint w/ Girder	A = 0.06 m ²	Slight	One	c	208
130	7	PSC Girder G-4	SER	S	Spall	Bottom Flange	A = 0.01 m ²	Slight	One	c	209
131	7	End Diaphragm @ Pier 7	HC	м	Honeycomb	Bottom Flange	A = 0.11 m ²	Moderate	One	b	210
132	7	Deck slab @ Bay 3	CR	S	Random Cracks	Bottom of Slab	t = 0.076 mm	Slight	Several	c	211
133	7	End Diaphragm @ Bay 2	НС	M	Honeycomb	At Joint w/ Girder	A = 0.12 m ²	Moderate	One	b	212
134	7	Deck Slab @ Bay 2	SER	м	Exposed Rebar	Construction Joint	A = 0.14 m ²	Moderate	Two	b	213
135	7	Deck slab @ Bay 1	CR	s	Random Cracks	Bottom of Slab	t = 0.129 mm	Slight	Several	c	214
136	7	End Diaphragm @ Bay 1	SER	S	Spall	Top pf Diaphgram @ Corner	A = 0.02 m ²	Slight	One	с	215
137	7	End Diaphragm @ Bay 1	CR	S	Horizontal Crack	Back Face	t = 0.03 mm	Slight	One	c ·	216
138	7	End Diaphragm @ Bay 1	нс	м	Honeycomb	Back Face	A = 0.26 m ²	Moderate	One	b	217
139	7	Deck Slab @ Bay 1	SER	м	Exposed Rebar	Bottom of Slab	A = 0.12 m ²	Moderate	One	b	218
140	7	PSC Girder G-1	SER	s	Spall	Bottom Flange	A = 0.03 m ²	Slight	One	с	219
141	7	Sidewalk	SER	s	Exposed Rebar	Above P 7	A = 0.002 m ²	Slight	One	с	368
142	8	Pier 7 Coping	CR	S	Horizontal Crack	End of Coping Downstream	t = 0.173 mm	Slight	One	c	183
143	8	Pier 7 Coping	CR	s	Vertical Creck	Front Face Bay 11	t = 0.203 mm	Slight	One	c	184
	8	Pier 7 Coping	CR	s	Random Cracks	Front Face Bay 11	t = 0.207 mm	Slight	Three	c ·	185
144		rici i doping i	011								

Appendix 7.1.1-1 (14/34)

Damage No.	Span No.	Name of Member	Type of Damage	Rank of Damage	Nature	Des Location/	cription of Damag		No. of	Remarks	Phot
				-		Pattern Top pf Diaphgram @		Severity	Damages	(a, b, c)	No.
146	8	End Diaphragm @ Pier 7	CR	S	Horizontal Crack	Sav 11	t = 0.127 mm Reduction in Cross	Slight	One	c	183
147	8	Restraining Bar @ Bay 11	co	Н	Corrosion	Pipe Sleeve Construction Joint	Section	Heavy	Two	а	187
148	8	End Diaphragn @ Bay 11	HC	S	Honeycomb	@ Top	A = 0.10 m ²	Slight	One	c	188
149	8	Pier 7 Coping	CR	S,	Vertical Crack	Front Face @ Bay 10	t = 0.127 mm	Slight	Two	c	189
150	8	Pier 7 Coping	CR	S	Random Cracks	Front Face Below Girder G-10	t = 0.127 mm	Slight	Many	c	191
151	8	Restraining Bar @ Bay 10	B/R	Н	Fracture	Pipe Sleeve	Borken due to Corrosion	Heavy	Two	а	192
152	8	End Diaphragm @ Pier 10	нс	S	Honeycomb	Construction Joint @ Top	A = 0.06 m ²	Slight	One	c	193
153	8	PSC Girder 6 - 11	CR	s	Diagonal Crack		t = 0.127 mm	Slight	One	c	
154	8	Deck slab @ Bay 10	CR	S	Random Cracks	Bottom of Slab	l = 0.108 mm	Slight	Several	с	
155	8	Pier 7 Coping	CR	s	Vertical Crack	Front Face Bay 9	t = 0.108 mm	Slight	One	C ·	194
156	8	End Diaphragm @ Bay 9	м	М	No Holes for Restraining	@ Pier 8		Moderate	One	b	195
157	- 8	Deck slab @ Bay 9	CR	s	Random Cracks	Bottom of Slab	l ≃ 0.108 mm	Slight	Several	с	196
· 158 ·	8	End Diaphragm @ Bay 9	CR	S	Random Cracks	Hunch/Fillet	t = 0.108 mm	Slight	Several	с	
159	8	Restraining Bar @ Bay 8	CO	н	Corrosion	End	Reduction in Cross Section	Heavy	Two	а	197
160	8	PSC Girden G-8	CR	S	Diagonal Crack	End Block	t = 0.102 mm	Slight	Several	с	
161	8	Deck Slab @ Bay 8	CR	s	Random Cracks	Bottom of Slab	t = 0.102 mm	Slight	Several	с	
162	8	Pier 7 Coping	CR	S	Vertical Crack	Front Face @ Bay 7	1 = 0.152 mm	Slight	One	с	198
163	8	Restraining Bar @ Bay 7	м	н	Missing Restraining Bars	End Diaphragm	One Bay	Heavy	Two	а	199
164	8	Deck Slab @ Bay 7	CR	s	Random Cracks	Bottom of Slab	t = 0.127 mm	Stight	Several	c	200
165	8	Pier 7 Coping	нс	м	Honeycomb	Face of Soffit	A = 0.12 m ²	Moderate	One	b	201
166	8	Pier 8 Coping	CR	s	Cracks	Below Girder G-11	t = 0.127 mm	Slight	Two	c	93
167	8	Pier 8 Coping	CR	s	Cracks	Bay 11	t = 0.107 mm	Slight	One	c	94
168	8	PSC Girder	нс	н	Honeycomb	Bottom	A = 0.47 m ²	Heavy	One		95
169	8	Sidewalk Slab	CR	S	Crack	Boltom	t = 0.254 mm	Slight	One	c	96
170	8	Pier 8 Column	CR	s	Vertical at Column	One Upstream Face	t = 0.229 mm	Slight	One	с с	97
171	8	Pier 8 Coping	SER	н	Insufficient Conc.	Boltom	A = 0.23 m ²	Heavy	One	а	98
172	8	Pier 8 Column - 2	CR	s	Horizontal Crack	Bottom of Coping	1 = 0.203 mm	Slight	One		90
173	8	Pier 8 Coping	CR	s	Vertical Crack	Bay 4	t = 0.076 mm			c	
174	8	End Diaphragm at Pier 8	HC	 M	Honeycomb	Top near Bottom of		Slight	One	c	81
175	8	End Diaphragm at Pier 8	SER	S		Slab	A = 0.15 m ²	Moderate	One	b	82
176	8			·	Exposed Rebar Missing Restraining	Bottom	A = 0.06 m ²	Slight	One	с	83
177	8	Restraining Bar	M	H	Bar	near Girder G-5	One Bay	Heavy	One	а	84
		Deck Slab	HC	Н	Honeycomb	Bottom of Bay 4	A = 0.54 m ²	Heavy	One	a	85
178	8	Pier 8 Coping	CR	S	Vertical Crack	Bay 5 Restraining Bar &	t = 0.173 mm	Slight	One	c	86
179	8	Restraining Bar	M -	н	Missing	Missing nut at Bay 5	One Bay	Heavy	One	a -	87
180	8	End Diaphragm	FR	S	Fraclured	Bay 6 Bottom Flange near	A = 0.08 m ²	Slight	One	c	88
181	8	PSC Girder G-9	SER	S	Spall	end of Girder	A = 0.01 m ²	Slight	One	с	89
182	8	Pier 8 Coping End Diaphragm/Slab @ Pier	CR	M.	Crack	Bay 10	l = 0.559 mm	Moderate	One	b	90
183		8 End Diaphragm/Slab @ Pier	CR	S	Horizontal Crack	Haunch @ Bay 10	. t = 0.254 mm	Slight	One	c	91
184	8		CR	S	Horizontal Crack	Haunch @ Bay 9	t = 0.076 mm	Slight	One	C	92
185	8	Pier 8 Coping	CR	S	Crack	End Upstream Side	t = 0.076 mm	Slight	One	. c	70
186	8	End Diaphragm/Slab @ Pier	НС	н	Damaged Portion	Honey Comb	A = 1.09 m ²	Heavy	One	а	71
187	8	Deck Slab Bay 1	CR	S	Random Cracks	Bottom	t = 0.203 mm	Slight	Several	с	72
188	8	Pier 8 Coping	CR	S	Crack	Back Face at Bay 1	l = 0.102 mm	Slight	One	с	73
189	8	End Diaphragm/Slab @ Pier 8	CR	н	Horizontal Crack	Top of Daphgram @ Bay 1	l = 0.635 mm	Heavy	One	а	
190	8	Pier 8 Coping	CR	S ·	Vertical Crack	Bay 1	t = 0.254 mm	Slight	One	c	74
191	8	Pier 8 Coping	CR	s	Cracks	Back Face below Girder G-2	t = 0.072 mm	Slight	Two	с	75
192	8	Pier 8 Coping	CR	S	Crcks	Back Face at Bay 2	l = 0.254 mm	Slight	One	с	76
193	8	Restraining Bar	FR	н	Fractured Restraining Bar	Near Girder G-3	Borken due to Corrosion	Heavy	One	а	77
194	8	Deck Slab	CR	S	Random Cracks	Bottom at Bay 2	t = 0.046 mm	Slight	Several	c	78
195	8	Pier 8 coping	CR	S.	Vertical Crack	8ay 3	t = 0.102 mm	Slight	One	c	79
196	8	End Diaphragm	SER	S	Spall	Near Girder G-3	A = 0.06 m ²	Slight	One	с	80
197	9	PSC Girder G-4	CR	м	Crack	End Block Upstream Side	t = 0.229 mm	Moderate	Two	b	54
198	9	Pier 9 Coping	CR	s	Crack	Bay 3	1 = 0.076 mm	Slight	One	с	55
199	9	Drain Pipe at Pier 9	м	S	Missing	End Cap Missing		Slight	One	c	56
200	9	Pier 9 Coping	CR	S	Horizontal Crack	Joint of End Block &	t = 0.076 mm	Slight	One	c	57
201	9	Pier 9 Coping	SER	н	Thin Concrete	Coping Bottom	A = 3.0 m ²	Moderate	One	a	58
202	. 9	PSC Girder G-12	CR	M	Cover Horizontal Crack	End Block	t = 0.331 mm	Moderate			
202	9	Pier 9 Coping	CR	S	Vertical Crack	Below Girder G-12			Several	b	42
203	9	Pier 9 Coping					t = 0.064 mm	Slight	One	C	43
204			CR	M	Crack Cut-off Restraining	Front Face at Bay 11	t = 0.33 mm	Moderate	One	b	44
	9	Restraining Bar	B/R		Bar	Bay 11	One Bay	Heavy	One	а	45
	ا ^ن م					word Kone of Rev 10	t = 0.072 mm		One	-	46
206	9	Pier 9 Coping Pier 9 Coping	CR CR	S S	Vertical Crack Vertical Crack	Front Face at Bay 10 Bay 10	t = 0.152 mm	Slight Slight	One ·	c c	40

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Appendix 7.1.1-1 (15/34)

Damage No.	Span No.	Name of Member	Type of Damage	Rank of Damage	Nature	Location/	scription of Damag	e Severity	No. of	Remarks (a, b, c)	Photo No.
209	9	Pier 9 Coping	CR	S	Crack	Pattern	<u> </u>		Damages	<u> </u>	
209	9	Pier 9 Coping	CR	s		Bay 9	t = 0.064 mm	Slight	One	c	49
210	9	Pier 9 Coping	CR	s	Cracks	Bay 7	l = 0.102 mm	Slight	One	С	50
212	9	PSC 9 Girder G-4				Bay 5	t = 0.102 mm	Slight	Two	c	51/52
212	9		CR	S	Cracks	Near end Block	t = 0.102 mm	Slight	One	с	52a
		Pier 9 Coping	CR	S	Cracks	Bay 4	t = 0.152 mm	Slight	Two	c	53
214	9	Pier 8 Coping	SER	S	Spall	Bay 5	A = 0.009 m ²	Slight	One	С	30
215	9	Pier 8 Coping	CR	S	Vertical Crack	Front Face @ Bay 4	t = 0.064 mm	Slight	One	с	31
216	9	Pier 8 Coping	CR	S	Vertical Crack	Front Face @ Bay 3	t = 0.102 mm	Slight	One	c	32
217	9	Pier 8 Coping	CR	S	Vertical Crack	Front Face @ Bay 3	t = 0.127 mm	Slight	One	c	33
218	9	Pier 8 Coping	CR ·	S	Vertical Crack	Front Face @ Bay 2	. t = 0.102 mm	Slight	One	c	34
219	9	Pier 8 Coping	CR	S	Vertical Crack	Front Face @ Bay 2	t = 0.127 mm	Slight	One	с	35
220	9	Pier 8 Coping	CR	s	Vertical Crack	Front Face @ Bay 2	t = 0.173 mm	Slight	One	с	36
221	9	Pier 8 Coping	CR	S	Vertical Creck	Front Face @ Bay 1	t = 0.229 mm	Slight	One	c	37
222	9	Pier 8 Coping	CR	S	Horizontal Crack	End Block & Coping	t = 0.102 mm	Slight	One	С	38
223	9	End Diaphragm at Pier 8	FL	S	Presence of Lime	Bay 1 3-11	A = 0.01 m ²	Slight	One	с	39
224	9	Deck Slab	нс	s	Honeycomb	Bottom of Stab @ Bay 2	A = 0.02 m ²	Slight	One	с	40
225	9	Pier 8 Coping	SER	s	Exposed Rebar	Bottom of Coping @ Bay 6	A = 0.01 m ²	Slight	One	с	41
226	9	Pier 8 Coping	SER	s	Spail	End of Coping	A = 0.05 m ²	Slight	One	c	19
227	9	Pier 8 Coping	CR	s	Vertical Crack	Front Face	t = 0.102 mm	Slight	One	c	20
228	9	Pier 8 Coping	CR	s	Vertical Crack	Front Face @ Bay 11	t = 0.102 mm	Slight	One	c	20
229	9	Pier 8 Coping	CR	s	Vertical Crack	Front Face @ Bay 11	t = 0.076 mm	· · · · · · · · · · · · · · · · · · ·			
230	9	Pier 8 Coping	CR	S	Cracks			Slight	One	C	22
						Front Face @ Bay 10	t = 0.127 mm	Slight	Séveral	c	23/24
231	9	Pier 8 Coping	CR	S	Cracks	Front Face @ Bay 9	t = 0.173 mm	Slight	Several	c	25/26
232	9	Pier 8 Coping	CR	S	Cracks	Front Face @ Bay 7	t = 0.076 mm	Slight	Several	c	27/28
233	9	Pier 8 Coping	CR	S	Cracks	Front Face @ Bay 6	t = 0.076 mm	Slight	Several	C	29
234	10	Pier 9 Coping	CR	S	Vertical Crack	Front Face @ Bay 11	t = 0.229 mm	Slight	One	c	220
235	10	Pier 9 Coping	CR	S	Vertical Crack	Front Face @ Bay 11	t = 0.203 mm	Slight	Two	c	221
236	10	Restraining Bar	М	H.	Missing	End Down @ Bay 11	One Bay	Heavy	Two	а	222
237	10	End Diaphragm	SER	s	Exposed Rebar	Front Face NR Bottom	A = 0.01 m ²	Slight	One	с	223
238	10	Deck Slab @ Bay 11	CR	S	Random Cracks	Bollom of Slab	t = 0.173 mm	Slight	Several	с	224
239	10	Pier 9 Coping	CR	s	Vertical Crack	Front Face @ Bay 11	t = 0,102 mm	Slight	Two	с	225
240	10	End Diaphragm @ Bay 7	НС	s	Honeycomb	Front Face @ Bay 7	A = 0.09 m ²	Slight	Several	c	226
241	10	Deck Slab @ Bay 7	CR	s	Diagonal Creck	Bottom of Slab	t = 0.152 mm	Slight	One	c	227
242	10	Drain Pipe	SER	s	Missing End Cap	NR Girder G-7	One Portion	Slight	One	C C	228
243	10	Pier 9 Coping	CR	s	Diagonal Crack	Corner of Riser @ Bay	t = 0.102 mm				
244	10	Pier 9 Coping	CR	s		6 Front Face @ Bay 5		Slight	One	c	229
245	10				Vertical Crack		l = 0.254 mm	Stight	One	c	230
		Deck Slab @ Bay 5	CR	S	Random Cracks	Bottom of Slab	t = 0.203 mm	Slight	Several	C ·	231
246	10	End Diaphragm @ Bay 5	CR	S	Horizontal Crack	Front Face @ Bay 5	t = 0.257mm	Slight	One	c	
247	10	PSC Girder G-5	SER	S	Spall	Bottom Flange	A = 0.01 m ²	Slight	Óne	c	232
248	10	Pier 9 Coping	CR	S	Vertical Crack	Front Face @ Bay 4	t = 0.103 mm	Slight	One	с	233
249	10	Deck Slab @ Bay 3	HC	S _	Honeycomb	Bottom of Slab @ Bay 3	A = 0.04 m ²	Slight	One	c	234
250	10	Deck Slab @ Bay 3	CR	S	Random Cracks	Bottom of Stab	l = 0.103 mm	Slight	One	с	235
251	10	Deck Slab @ Bay 2	CR	s	Random Cracks	Bottom of Slab	t = 0.103 mm	Slight	Several	с	236
252	10	Pier 9 Coping	CR	S	Vertical Crack	Front Face @ Bay 2	t = 0.203 mm	Slight	Several	c	237
253	10	Abutment B Wall	CR	S	Vertical Crack	Bay 5	t = 0.162 mm	Slight	One	c	111
254	10	Abutment B	CR	S	Vertical Crack	Bay 4	t = 0.187 mm	Slight	One	с	112
255	10	Abutment B	CR	s	Vertical Crack	Bay 3	t = 0.173 mm	Slight	One	с	113
256	10	Abutment B	CR	s	Vertical Crack	Bay 3	t = 0.203 mm	Slight	One	c	115
257	10	End of Diaphragm @ Abut.	нс	M	Honeycomb	Bottom @ Bay 3	Å = 0.18 m ²	Moderate	One	b	114
258	10	Abutment B	CR	s	Vertical Crack	Bay 2 near	t = 0.132 mm	Slight	One	c	116
259	10	Abutment B	CR	s	Vertical Crack	Girder G-2 Bay 2 near	t = 0.152 mm	Slight	One	c	
260	10	Deck Slab	CR	S		Girder G-2					117
					Crack	Bottom @ Bay 2	l = 0.173 mm	Slight	One	c	118
261	10	Abutment B	CR	S	Vertical Crack	Bay 1	t = 0.254 mm	Slight	One	c	119
262	10	Abutment B	CR	S	Vertical Crack Fractured	Bay 1 Bay 1 near	t = 0.173 mm	Slight	One	c	120
263	10	Restraining Bar	B/R	н	Restraining Bar	Gifder G-1	One Bay	Heavy	One	а	121
264	10	Abutment B	CR	S	Horizontal Crack	Upstream Side	t = 0.076 mm	Slight	One	c	122
265	10	Abutment B	CR	н	Vertical Crack	Bay 4	l = 0.61 mm	Heavy	One	а	100
266	10	End Diaphragm @ Abut. B	HC	s	Honeycomb	Near Girder G-12	A = 0.06 m ²	Slight	One	с	101
267	10	End Diaphragm @ Abut. B	нс	S	Honeycomb	Near Girder G-9	A = 0.02 m ²	Slight	One	С	102
268	10	Abutment B Wall	CR	н	Vertical Crack	Bay 9	t = 0.61 mm	Heavy	Оле	а	103
269	10	End Diaphragm @ Abut. B	НС	S	Honeycomb	Bottom @ Bay 9	A = 0.04 m ²	Slight	One	с	104
270		End Diaphragm @ Abut. B	нс	S	Honeycomb	Bay 8	A = 0.12 m ²	Slight	One	c	105
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Appendix 7.1.1-1 (16/34)

Damage	Span		Type of	Rank of			cription of Damag	e		Remarks	Photo
No.	No.	Name of Member	Damage	Damage	Nature	Location/ Pattern	Scale	Severity	No. of Damages	(a, b, c)	No.
272	10	Restraining Bar	B/R	н	Fractured Restraining Bar	Bay 6	One Bay	Heavy	One	а	107
273	10	End Diaphragm	HC	S	Cavity	Bay 6	A = 0.001 m ²	Slight	One	с.	108
274	10	Abutment B	CR	м	Crack	Bay 6	t = 0.457 mm	Moderate	Several	b	109
275	10	Deck Slab	CR	S	Crack	Bollom @ Bay 5	t = 0.203 mm	Slight	One	с	110
276	10	Abutment B	CR	S	Vertical Crack	Bay 5	l = 0.152 mm	Slight	One	с	111
277	10	Deck Slab @ Bay 2	SER	м	Exposed Rebar	Bottom Slab Corner	A = 0.16 m ²	Moderate	One	ъ	238
278	10	Pier 9 Coping	CR	S	Vertical Crack	Front Face @ Bay 2	t = 0.173 mm	Slight	One	с	239
279	10	Pier 9 Coping	CR	s	Vertical Crack	Front Face @ Bay 1	t = 0.102 mm	Slight	One	с	240
280	10	Deck Slab @ Bay 1	CR	s	Random Cracks	Bottom of Slab	t = 0.103 mm	Slight	Several	с	241
281	10	End Diaphragm	CR	М	Vertical Crack	Front Face @ Bay 1	t = 0.483 mm	Moderate	One	b	242
282	10	End Diaphragm	CR	M	Random Cracks	Front Face @ Bay 1	t = 0.381 mm	Moderate	Several	ь.	243
283	10	Pier 9 Coping	CR	S	Vertical Crack	Front Face @ Bay 1	t = 0.203 mm	Slight	Two	с	244
284	10	Drain Pipe	Mi	S	Missing End Cap	Drain Pipe @ Bay 1	One Portion	Slight	One	с	245
285	10	Pier 9 Coping	CR	S	Horizontal Crack	Joint with Shear Block @ End	t = 0.152 mm	Slight	One	с	246
286	10	Sidewalk Slab	HC	S	Honeycomb	Bottom	A = 0.09 m ²	Slight	One	c	247
287	10	Sidewalk Slab	CR	S	Diagonal Crack	Near Const. JE	t = 0.173 mm	Slight	One	с	248
288	10	Shear Block @ End of Coning	SER	S	Spall	Inner Face @ Mid Length	A = 0.06 m ²	Slight	One	с	249
289	10	Pier 9 Coping	CR	S	Vertical Crack	Front Face @ Bay 4	t = 0.152 mm	Slight	One	С	250
290	App. A	Sidewalk Downstream Side	F/C	s	Faulting on Sidewalk Slab	Settlement of Approach	t = 5 mm	Slight	One	с	68
291	App. A	Sidewalk	F/C	S	Faulting	Settlement of Sidewalk	1 = 8 mm	Slight	One	с	69
292	App. A	Pavement	CR	S	Transverse Crack	8 m. from Abut. "A"	t = 1.2 mm	Slight	Two	с	352
293	App. A	Pavement	CRPL	. н	Pothole	8 m. from Abut. "A"	36 mm ø	Heavy	Several	а	353
294	App. A	Sidewalk	CR	S	Transverse Crack	4 m. from Abut. "A"	t = 2.00 mm	Slight	One	с	354
295	App. A	Pavement	CR	S	Transverse Crack	4 m. from Abut. "A"	t = 3.00 mm	Slight	One	c	363
296	App. A	Sidewalk	CR	н	Transverse Crack	0.7 m from Abut. "A"	l = 3.00 mm	Slight	One	с	364
297	App. B.	Sidewalk	F/C	м	Faulting	Abut. "B"	d = 30 mm	Moderate	One	b	362
298	App. B.	Sidewalk/PCCP	F/C	S	Settlement	2 m. from Abut. "B"	d = 15 mm	Slight	One	с	369
299	App. B.	Railing	SER	S	Exposed Rebar	2 m. from Abut, "B"	A = 0.001 m ²	Slight	One	с	370

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Name of Bridge : Pa11 BAMBANG BRIDGE

 Date of Inspe
 Nov. 27 / Dec. 19-20, 2002

 Inspector
 R. Quiwa/J. Abadam

 Checker
 J. B. Agnes

Damage	Span		Type of	Rank of		Des	cription of Damage)		Remarks	Photo
No.	No.	Name of Member	Damage	Damage	Narure	Location/ Pattern	Scale	Severity	No. of Damages	(a, b, c)	No.
1	1	Abut. A / Coping	CR	м	Horizontal Crack	Abut, End U/S near G-1	t = 0.450 mm	Moderate	Several	b	1
2	1	PSC Girder G-1 (Channel Beam)	CR	S	Diegonal Crack Right	Web near end diaph. @ Bay 1	t = 0.20 mm	Stight	One	c	2
3	1.	End Diaphragm @ Abut. A	SER	S	Exposed Rebar	Bottom @ Bay 1	A = 0.003 m ²	Slight	One	с	3
4	1	End Diaphragm @ Abut. A	CR	н	Diegonal Crack	Under G-2	t = 1.80 mm	Heavy	One	а	4
5	1	End Diaphragm @ Abut. A	CR	н	Diagonal Crack	Under G-3	t = 1.0 mm	Heavy	One	а	5
6	1	PSC Girder G-4	SER	S	Spall	Top Flange @ Bay 3	A = 0.01 m ²	Slight	One	с.	6
7	· 1	End Diaphragm @ Abut. A	SER	S	Exposed Rebar	@ Bay 4	A = 0.03 m ²	Slight	One	с	7
8	1	End Diaphragm @ Abut. A	SER	S	Exposed Rebar	Under G-7	A = 0.12 m ²	Slight	One	с	8
9	1	Abut. A / Coping	CR	н	Random Cracks	Coping Face near G-7	t = 1.50 mm	Heavy	One	а	9
10	1	End Diaphragm @ Abut. A	SER	s	Exposed Rebar	@ Bay 7	A = 0.001 m ²	Slight	Several	с	10
11	1	End Diaphragm @ Abut. A	CR	н	Diagonal Crack	Under G-8	t = 0.90 mm	Heavy	One	а	11
12	1 -	Abut. A / Coping	CR	S	Random Cracks	Coping Face D/S near G-8	t = 0.30 mm	Slight	One	c	12
13	1	Back Wall @ Abut. A	CR	М	Vertical Crack	Near G-8	t = 0.50 mm	Moderate	Several	b	13
14	1	PSC Girder	SER	М	Exposed Rebar	Top Flange @ D/S	$A = 0.15 m^2$	Moderate	One	b	14
15	1	End Diaphragm @ Pier 1	SER	S	Random Crack & Exposed Rebar	@ Bay 2	A = 0.02 m ²	Slight	One	с	15
16	1	Coping @ Pier 1	CR	S	Vertical Crack	Coping Face under G-3	l = 0.20 mm	Slight	One	c ·	16
17	1	End Diaphragm @ Pier 1	SER	S	Exposed Rebar	@ Bay 4	A = 0.09 m ²	Slight	One	с	17
18	1	End Diaphragm @ Pier 1	SER	S	Exposed Rebar	@ Bay 5	A = 0.03 m ²	Slight	Two	с	18
19	1.	End Diaphragm @ Pier 1	SER	М	Exposed Rebar	@ Bay 6	A = 0.102 m ²	Moderate	One	b	19
20	1	End Diaphragm @ Pier 1	SER	S	Exposed Rebar	Bottom @ Bay 7	A = 0.009 m ²	Slight	Öne	с	20
21	· 1	Coping @ P1	HC	S	Honeycomb	Bottom near G-8	A = 0.09 m ²	Slight	One	c	21
22	1	Underside of Sidewalk @ P1 Expansion Joint @	CR	S	Transverse Crack	Bottom from Exp. Joint	t = 0.20 mm	Slight	Several	c	22
23	1	Expansion Joint @ Abutment A	SER	м	Spall, No Gap	End of Slab & Approach Slab	A = 0.28 m ²	Moderate	Whole	b	94/98
24	1	Expansion Joint @ Pier 1	SER	м	Spall, No Gap	End of Slab	A = 0.25 m ²	Moderate	Whole	b	95
25	1	Deck Slab	CR	s	Random Cracks	Top of Slab of Span 1	l ≃ 0.300 mm	Slight	Many	с	96
26	1	Deck Slab	CR	н	Transverse Cracks	Top of Slab	t = 1.000 mm	Heavy	Three	а	99
27	1	Deck Slab	CRPL	м	Rutting	Top of Slab	L = 32 mm	Moderate	One	ъ	100
28	1	Sidewalk Slab	CRPL	н	Pothole	Slab	428 mm ø	Heavy	Two	a	101
29	_1	Deck Slab	CR	S	Random Cracks	Top of Slab	t = 0.076 mm	Slight	Several	с	124
30	1	Sidewalk	CR	S	Transverse Crack w/ spall	Top of Slab	t = 0.015 mm	Slight	One	c	125
31	2	@ Pier 1 PSC Girder G-1 (Channel)	CR	s	Vertical Crack	Front Face @ Bay 1	t = 0.20 mm	Slight	One	с	23

Appendix 7.1.1-1 (17/34)

Damage No.	Span No.	Name of Member	Type of Damage	Rank of Damage	Nature	De: Location/ Pattern	scription of Damag	ge Severity	No. of Damages	Remarks (a, b, c)	Photo No.
32	2	Restraining Bar @ Pier 1	L/M	S	Loose	Diaph. @ Bay 1	One Portion	Slight	One	с	. 24
33	2	End Diaphragm @ Pier 1	SER	s	Exposed Rebar	Front Face @ Bay 4	A = 0.01 m ²	Slight	One	c	25
34	2	End Diaphragm @ Pier 1	CR	S	Random Cracks	Front Face @ Bay 5	t = 0.25 mm	Slight	Several	c	26
35	2	End Diaphragm @ Pier 1	SER	S	Exposed Rebar	Front Face @ Bay 6	A = 0.06 m ²	Slight	One	c	27
36	2	End Diaphragm @ Pier 1	CR	s	Random Cracks	Front Face @ Bay 7	t - 0.25 mm	Slight	Several	c	
37	2	End Diaphragm @ Pier 1	SER	s	Exposed Rebar	Front Face @ Bay 5	A = 0.09 m ²	Slight	One	c	28
38	2	End Diaphragm @ Pier 1	CR	s	Vertical Crack	Front Face @ G-7	t = 0.25 mm	Slight	One	с	29
39	2	End Diaphragm @ Pier 1	CR	S	Random Cracks	Front Face @ G-8	t = 0.25 mm	Slight	Several	с	30
40	2	PSC Girder G-8	CR	н	Diagonal Crack	Inner Face of Outer Web	t = 0.45 mm	Heavy	One	a	31
41	2	PSC Girder G-5	CR	s	Longitudinal Crack	Top Flange	t = 0.20 mm	Slight	One	с	32
42	2	End Diaphragm @ Pier 2	CR	M	Random Cracks	Diaph. Under G-5	t = 0.40 mm	Moderate	Several	b	33
43	2	End Diaphragm @ Pier 2	CR	н	Horizontal Cracks	Diaph. under G-6	l = 1.50 mm	Heavy	One	a	34
44	2	End Diaphragm @ Pier 2	CR	S	Random Cracks	Diaph, under G-7	t = 0.30 mm	Slight	Several	c	
45	2	End Diaphragm @ Pier 2	CR	М	Random Cracks	Diaph. under G-8	l ≈ 0.60 mm	Moderate	One	b	35
46	2	Interm, Diaph.	CR	S .	Crack	Bottom	t = 0.25 mm	Slight	Several	с	
47	- 2	Expansion Joint @ Pier 2	CRPL	м	Fracture, No Gap	End of Slab	A = 0.03 m2	Moderate	Whole	b	97
48	2	Deck Slab	CR	S	Random Cracks	Top of Slab	t = 0.300 mm	Slight	Many	с	102
49	3	PSC Girder G-1 @ Pier 3	SER	S	Spall	Bottom Flange	A = 0.05 m2	Slight	Two	с	102a
50	3	Deck Slab @ Pier 3	SER	н	Exposed Rebar	Bottom	A = 0.35 m2	Heavy	One	а	103a
51	3	End Diaphragm @ Pier 3	HC	s	Honeycomb	Bottom	A = 0.10 m2	Slight	One	с	104a
52	3	End Diaphragm @ Pier 3	CR	М	Diagonal Crack	Back Face @ Bay 1	t = 0.550 mm	Moderate	Two	b	104a
53	3	End Diaphragm @ Pier 3	CR	М	Diagonal Crack	Back Face @ Bay 2	t = 0.450 mm	Moderate	One	b	
54	3	End Diaphragm @ Pier 3	CR	м	Diagonal & Vertical Crack	Back Face @ Bay 3	i = 0.500 mm	Moderate	Two	b	105a
55	3	PSC Girder G-1,G-2 & G-3 @ Pier 3	CR	н	Horizontal Crack	End Block @ Both Sides	t = 0.550 mm	Heavy	One	а	106a
56	3	Cantilever Slab	CR	S	Transverse Crack	Near Pier 4	t = 0.076 mm	Slight	One	c	107a
57	3	PSC Girder G-4 @ Pier 3	CR	S	Diagonal Crack	End Block @ Both Sides	t = 0.150 mm	Slight	One (Each Side)	с	108a
58	3	Sidewalk	SER	s	Exposed Rebar	Near Curb	A = 0.05 m2	Slight	One	с	126
59	3 -	Deck Slab	CR	s	Random Cracks	Top of Slab	t = 0.300 mm	Slight	Many	с	103
60	3	Deck Slab	SER	н	Spall w/ Pothole	Const. Joint	A = 5.0 m2	Heavy	One	а	127
61	4	PSC Girder G-1 @ Pier 4	CR	s	Diagonal Crack	End Block @ P4	t = 0.150 mm	Slight	One (Each Side)	c	90a
62	4	Coping @ Pier 4	CR	S	Vertical Crack	Back Face under G-1	t = 0.200 mm	Slight	One	с	91a
63	4	End Diaphragm @ Pier 4	CR	M	Diagonal Crack	Back Face @ Bay 1	t = 4.000 mm	Moderate	One	b	92a
64	4	End Diaphragm @ Pier 4	нс	S	Honeycomb	Back Face @ Bay 1	A = 0.04 m2	Slight	One	с	93a
65	4	PSC Girder G-2 @ Pier 4	SER	м	Spall	Bottom Flange	A = 0.20 m2	Moderate	. One	b.	94a
66	4	PSC Girder G-2 @ Pier 4	CR	н	Diagonal Crack	End Block	t = 0.500 mm	Heavy	One (Each Side)	a	95a
67	4	End Diaphragm @ Pier 4	нс	S	Honeycomb	Bollom	A = 0.01 m2	Slight	One	c	96a
68	4	PSC Girder G-3 @ Pier 4	CR	н	Diagonal Crack	End Block	t = 0.600 mm	Heavy	One (Each Side)	а	97a
69	4	End Diaphragm @ Pier 4	CR	м	Crack	Back Face @ Bay 3	1 = 0.600 mm	Moderate/Slight	One	b	98a
70	4	PSC Girder G-4 @ Pier 4	CR	S	Horizontal Crack	End Block @ P4	t = 0.200 mm	Slight	One (Each Side)	с	99a
71	4	Underside of Sidewalk @ Pier 4	CR	s	Transverse Crack	Bottom	t = 0.300 mm	Slight	Several	с	100a
72	4	PSC Girder G-4 @ Pier 4	SER	S	Spall	Bottom Flange @ P4	A = 0.04 m2	Slight	One	с	101a
73	4	End Diaphragm @ Pier 3	HC	M	Honeycomb	@ Bay 3	A = 0.23 m2	Moderate	One	b	155
74	4	Coping @ Pier 3	SER	м	Exposed Rebar	End of Coping	A = 0.15 m2	Moderate	One	b	153
75	4	PSC Girder G-4 @ Pier 3	CR	s	Horizontal Crack	Web	t = 0.150 mm	Slight	Two	c	154
76	4	End Diaphragm @ Pier 3	SER	s	Spall	@ Bay 1	A = 0.05 m2	Slight	One	c	156
77	4	PSC Girder G-1 @ Pier 3	SER	s	Spall	Bottom Flange	A = 0.06 m2	Slight	One	c	157
78	4	Under Side of Sidewalk	CR	s	Transverse Cracks	@ Cablilever Slab	t = 0.200 mm	Slight	Several	с	158
79	4	Deck Slab	CRPL	н	Patched w/ Asphalt Scaling	Downstream Side	1 m ø	Heavy	Several	а	128
80	5	Rail Post	SER	S	Spall	Inner Face	A = 0.003 m2	Slight	One	с	130
81	5	PSC Girder G-1 @ Pier 5	SER	S	Spall	Bottom Flange	A = 0.10 m2	Slight	One	c	139
82	5	End Diaphragm @ Pier 5	CR	s	Random Cracks	@ Bay 1	t = 0.300 mm	Slight	Several	c	140
83	5	Coping @ Pier 5	нс	s	Honeycomb	End of Coping U.S.	A = 0.07 m2	Slight	One	c	141
84	5	PSC Girder G-1 @ Pier 5	CR	н	Horizontal Crack	End Block	t = 0.800 mm	Heavy	Two	а	142
85	5	PSC Girder G-1 @ Pier 5	SER	s	Spall	Bottom Flange @ Bay 1	A = 0.045 m2	Slight	One	c	143
86	5	End Diaphragm @ Pier 5	CR	м	Random Crack	@ Bay 1 @ Bay 2	t = 0.600 mm	Moderate	Several	b	144
87	5	PSC Girder G-3 @ Pier 5	CR	s	Horizontal Crack	End Block @ Bolh	t = 0.200 mm	Slight	One	c	145
88	5	PSC Girder G-4 @ Pier 5	CR	н	Diagonal &	Sides End Block @ Both	1 = 0.65 mm	Heavy	Several	a	145
89	5	Coping @ Pier 5	CR	S	Horizontal Cracks	Sides Coping Face @ Bay 1	l = 0.300 mm	Slight	One	c	140
90	5	Expansion Jt.	F/C	s	Uneven Elev.	Above Pier 5	d = 10 mm	Slight	One	с с	129
91	6	PSC Girder G-2 @ Pier 5	CR	н	Horizontal Crack	End Block	t = 0.50 mm	Heavy	One (Each Side)		
[6	PSC Girder G-3 @ Pier 5	CR	н	Horizontal Crack	End Block	t = 0.450 mm	Heavy	One (Each Side)	a	134
92 I			(, ieavy	One	a	135
92	6	Coping @ Pier 5	CR	м	Vertical Crack	Coping Face	t = 0.400 mm	Moderate	One	b	136

Appendix 7.1.1-1 (18/34)

_	1.			·····	· · · · · · · · · · · · · · · · · · ·	Da	contration of Demos	*			.
Damage No.	Span No.	Name of Member	Type of Damage	Rank of Damage	Nature	Location/	scription of Damag Scale	Severity	No. of	Remarks (a, b, c)	Photo No.
95	6	Lindernide of Sidourolly				Pattern			Damages		
95	6	Underside of Sidewalk	CR	S	Transverse Crack	@ Cantilever slab End Block @ @ Both	t = 0.300 mm	Slight	Several	с	138
		PSC Girder G-1 @ Pier 6	CR	M	Horizontal Crack	Sides	t = 0.400 mm	Moderate	Two	b	148
97	6	End Diaphragm @ Pier 6	CR	н	Random Crack	@ Bay 1 End Block @ Both	t = 0.900 mm	Heavy	Several	а	149
98	6	PSC Girder G-3 @ Pier 6	CR	н	Horizontal Crack	Sides End Block @ Both	t = 0.600 mm	Heavy	One	а	150
99	6	PSC Girder G-2 @ Pier 6	CR	н	Horizontal Crack	Sides Bottom Face on All	t = 0.600 mm	Heavy	One	a	151
100	6	Deck Slab	CR	S	Random Cracks Damaged Asphalt	Bays	l = 0.300 mm	Slight	Many	c	152
101	7	Expansion Joint @ Pier 7	CRPL	м	Overlay	Above Pier 7	t = 10 mm	Moderate	One	b	104
102	8	Expansion Joint @ Pier 8	SER	м	Spall	End of Slab	A = 0.21 m2	Moderate	One	b	131
103	8	Sidewalk Slab	SER	S	Spall	Top of Curb	A = 0.015 m2	Slight	One	c	132
104	8	P8	SER	s	Exposed Rebar	Bottom	A = 0.08 m2	Slight	One	c	36
105	8	End Diaphragm @ Pier 8	SER	s	Exposed Rebar	@ Bay 1	A = 0.04 m2	Slight	One	с	52
106	- 8	Coping @ Pier 8	CR	M	Vertical Crack	Under G-1 & G-3	t = 0.40 mm	Moderate	Two	b	
107	8	End Diaphragm @ Pier 8	CR	M .	Diagonal Crack	Under G-2	t = 0.40 mm	Moderate	One	b	53
108	8	End Diaphragm @ Pier 8	CR	м	Diagonal Crack	@ Bay 2 under G-2	t = 0.45 mm	Moderate	One	b	54
109	8	Coping @ Pier 8	CR	M	Vertical Crack	@ Coping Face under G-2	t = 0,45 mm	Moderate	One	b.	55
110	8	End Diaphragm @ Pier 8	SER	s	Exposed Rebar	Under G-6	A = 0.05 m2	Slight	One	с	56
111	8	Intermediate Diaphragm	CR	s	Vertical and Map Cracks	Under G-7	t = 0.25 mm	Slight	Several	с	57
112	8	End Diaphragm @ Pier 8	CR	н	Vertical Crack	Under G-8	t = 1.60 mm	Heavy	One	а	58
113	8	Coping @ Pier 8	CR	м	Diagonal Crack	@ Coping Face	t = 0.45 mm	Moderate	One	b	59
114	8	Under Side of Sidewalk	SER	S	Exposed Rebar	@ Cantilever D.S.	A = 0.06 m2	Slight	One	c	60
115	8	Coping @ Pier 7	HC	s	Honeycomb	Coping Face @ Bay 4	A = 0.08 m2	Slight	One		
116	8	End Diaphragm @ Pier 7	CR	н	Crack					c	61
117	8	PSC Girder G-7 @ Pier 7	CR	н		@ Bay 5	l = 2.00 mm	Heavy	One	а	62
	<u> </u>				Diagonal Crack	Inner Face of Web	t = 3.00 mm	Heavy	One	а	63
118	8	Pier 7 Coping	CR	н	Diagonal Crack	Coping Face @ Bay 7	t = 0.90 mm	Heavy	One	a	64
119	8	End Diaphragm @ Pier 7	SER	S	Exposed Rebar	@ Bay 7	A = 0.04 m2	Slight	One	C .	65
120	8	End Diaphragm @ Pier 7	SER	S	Spall	Bottom @ Bay 8	A = 0.01 m2	Slight	One	c	66
121	8	Coping @ Pier 7	CR	н	Diagonal Crack	Top of Coping @ Baγ 8	l = 2.50 mm	Heavy	Several	а	67
122	9	Backwall @ Abut. "B"	FR	н	Fracture	End of Back Wall @ Upstream Side	t = 10.00 mm	Heavy	One	а	68
123	9	End Diaphragm @ Abut. "B"	SER	S	Exposed Rebar	Near G-1	A = 0.03 m2	Slight	One	с	69
124	9	Abutment "B"	CR	M	Horizontal Crack	Abut. Face under G-2	t = 0.500 mm	Moderate	One	b	70
125	9	Abutment "B"	CR	М	Vertical and Horizontal Crack	Abut. Face @ underG- 3	t = 0.500 mm	Moderate	Several	b	71
126	9	Abutment "B"	SER	s	Spall	Abut. Face under G-4	A = 0.075 m2	Slight	One	с	72
127	9	Abutment "B"	CR	Н	Vertical Crack	Abut. Face under G-5	l = 1.500 mm	Heavy	One	а	73
128	9	Abutment "B"	CR	н	Diagonal Crack	Abut, Face under G-3	t = 1.000 mm	Heavy	One	а	74
129	. 9	Bearing Plates @ Abut. "B"	со	M	Corrosion	to G-7 Under G-5 & G-7	Reduction of Cross	Moderate	One	b	75
130	9	End Diaphragm @ Abut. "B"	SER	S	Exposed Rebar	Bottom under G-8	A = 0.01 m2	Slight	One	c	76
131	9	Back Wall @	FR	н	Fracture	End of Backwall	t = 12 mm				
132	9	Abut "B" Deck Slab	SER	s		Downstream Slab		Heavy	One	a	77
133	9				Spall Exposed Rebar		A = 0.01 m2	Slight	One	c	105
		Rail Post	SER	<u>s</u>		Split Post	A = 0.07 m2	Slight	One	c	106
134	9	Deck Slab Expansion Joint @	CR	S	Random Cracks	Top of Slab End of Slab &	t = 0.300 mm	Slight	Many	c	107
135	9	Ahutment B Underside of Sidewalk @	SER	S	Spall	Approach Slab	A = 0.02 m2	Slight	Several	c	108
136	9	P8	HC	S	Honeycomb	Bottom	A = 0.1 m2	Slight	One	c	37
137	9	End Diaphragm @ Pier 8	CR	м	Random Cracks	Under G-1 (Common Under G-2)	t = 0.50 mm	Moderate	Several	b	38
138	9	End Diaphragm @ Pier 8	CR	м	Random Cracks	@ Bay 2	t = 0.55 mm	Moderate	Several	b	39
139	9	Coping @ Pier 8	CR	S	Vertical Crack	Under G-3	l = 0.30 mm	Slight	One	c	40
140	9	End Diaphragm @ Pier 8	CR	S	Horizontal Crack	Under G-4	t = 0.30 mm	Slight	One	c	41
141	9	PSC Girder G-5 (Channel Beam)	CR	s	Longiludinal Crack	Top Flanges @ Bay 4	i = 0.20 mm	Slight	One	с	42
142	9	1st Interm. Diaph.	CR	н	Vertical Crack	@ Bay 4	t = 1.00 mm	Heavy	One	a	43
143	9	End Diaphragm @ Pier 8	SER	s	Spall	Under G-5	A = 0.005 m2	Slight	One	c	44
144	9	PSC Girder G-6 @ Pier 8	CR	S	Diagonal & Horizontal Crack	Web & Top Flange	t = 0.15 mm	Slight	Two	с	45
145	9	End Diaphragm @ Pier 8	CR	s	Diegonal Crack	Under G-6	l = 0.20 mm	Slight	One	c	45
146	9	End Diaphragm @ Pier 8	SER	S	Exposed Rebar	@ Corner under G-6	A = 0.03 m2	Slight	One	c	46
147	9	PSC Girder G-6 @ Pier 8	CR	s	Horizontal Crack	Web & Top Flange	t = 0.20 mm	Slight	One	c	40
148	9	End Diaphragm @ Pier 8	CR	s	Random Cracks	Under G-7	t = 0.30 mm	Slight	Several		
149	9.	1st Interm. Diaph.	CR	н	Horizontal Cracks					c	48
145	9					@ Bay 7	t = 1.00 mm	Heavy	One	a	49
		End Diaphragm @ Pier 8	SER	S	Spall	Under G-8 @ Cantilever under	A = 0.01 m2	Slight	One	с	50
151	9	Coping @ Pier 8 Approach Slab @ Abutment	CR	н		End of Shear Blk D.S.	t = 1.00 mm	Heavy	Several	a	51
152		Approach Slab @ Abutment Approach Slab @ Abutment	CRPL	н	Pothole	Pavement	1.2 m ø	Heavy	Several	а	89 90
153		Λ Ι	CR	S	Longitudinal Cracks	Pavement	t = 0.300 mm	Slight	Several	с	91
154		Approach Slab @ Abutment	CR	s	Transverse Cracks	Pavement	t = 0.700 mm	Slight	Two	c	92 93
155		Approach Slab @ Abutment B	CR	S	Random Cracks	Top of Slab	t = 0.300 mm	Slight	Many	с	109
156		Approach Slab	CR	м	Pavement Crack	Top of Slab	t = 7.000 mm	Moderate	Three	b	111 112
		Approach Slab /	CRPL	S	Excess Asphalt @	1		Slight	One	с	110

Appendix 7.1.1-1 (19/34)

Damage			Type of	Rank of			ription of Damage	3		Remarks	Photo
No.	No.	Name of Member	Damage	Damage	Nature	Location/ Pattern	Scale	Severity	No. of Damages	(a, b, c)	No.
158		Nosing & Asphalt Pvmt, @ Approach B	SER	S	Spall	@ end of Nosing & Damage Aspl. Pav.	A = 0.02 m2	Slight	One	с	113
159		Approach Sidewalk @ Abutment B	CR	s	Random Cracks	Top of Slab	t = 0.300 mm	Slight	Several	с	133
160		Rail Post	SER	S	Exposed Rebar	@ Outer Face 52 m from Abut. "A"	A = 0.05 m2	Slight	One	с	115a
161		Approach Slab /	SER	S	Spall	@ Base 60 m from Abut, "A"	A = 0.08 m2	Slight	One	с	117
162		Approach Slab /	SER	S	Exposed Rebar	@ Outer Face 60 m from Abut, "A"	A = 0.08 m2	Slight	One	c	118
163		Approach Slab / Ann A Approach Slab /	SER	М	Spall	@ Outer Face 72 m from Abut, "A"	A = 0.175 m2	Moderate	One	b	119
164		Ann A	CRPL	н	Pot Hole Patched w/ Asphall	@ 75 m from Abut. "A" on Whole Span	1.38 m ø	Heavy	One	а	120
165		Approach Slab /	CR	М	Random Cracks		t = 6 mm	Moderate	Several	b	121
166		Approach Slab /	SER	S	Spall	Near Sidewalk	A = 1.5 m2	Slight	One	с	122
167		Approach Slab / App.A	CR	S	Transverse Crack	PCCP from Sidewalk to Sidewalk	t = 1.20 mm	Slight	One	с	123

Name of Bridge : Ma1.1 VARGAS BRIDGE "A" (Upstream)

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Date of Inspec <u>Nov. 26, 2002</u> Inspector <u>R. Millo/E. Pagaragan</u> Checker J. B. Agnes

Damage	Span		Type of	Rank of		Des	cription of Damag	e		Remarks	Phot
No.	No.	Name of Member	Damage	Damage	Nature	Location/ Pattern	Scale	Severity	No. of Damages	(a, b, c)	No.
1	1	Approach Slab	CR	м	Trans., Diag. & Longitudinal Cracks	PCCP @ Top of Approach	t = 8.00 mm	Moderate	Three	b	29
2	1	Approach Slab	CR	м	Trans., Diagonal & Longitudinal Crack	Top of Slob	t = 8.00 mm	Moderate	Three	b	29
3	• 1	Curb	FR	н	Fracture	Top of Curb near Approach S	l = 0.65 mm	Heavy	One	а	33
4	່ 1	Deck Slab	SER	м	Spall	For Repair Works @ Bay 1, 2 & 3 @ P1	A = 0.30 m ²	Moderate	Three	b	18
5	1	Extended Coping	CR	м	Crack	Face of Coping @ P-1 Downstream	t = 0.600 mm	Moderate	One	b	20
6	1	Pavement of Approach Slab	CRPL	м	Tranverse Crack	PCCP Carriageway near Approach Slab	t = 8.00 mm	Moderate	One	ь	32
7	1	Pier 2 Column	CR	s	Map Crack	Pier Body @ Upper	t = 0.076 mm	Slight	Many	c	69
8	1	Pier 2 Column	CR	S	Vertical Cracks	Part near Coping Face of Column @ Bare Side	l = 0.102 mm	Slight	Two	c	71
9	1	Pier 2 Footing	SER	S	Exposed Rebar	Top of Faating	A = 0.09 m ²	Slight	One	C .	70
10	1	Pier Post	CR	н	Vertical Crack	@ Upstream Top Pier Post	t = 0.70 mm	Heavy	One	a	21
11	1	Pier Wall	CR	M	Vertical Crack	Downstream @ P1 Near P-1	t = 0.350 mm	Moderate	Two	b	22
12	1	PSC Girder (G-3)	CR	н	Vertical Crack	Top Flange OF Girder	t = 1.00 mm	Heavy	One	a	19
13	1	Rail Post	FR	н	Fractured	@ P-1 Top Face of Post	t = 1.2 mm	Heavy	One	<u>+</u>	
14	1	Railing Post	SER	s	Spall	@ 10.0 m from End Top of Rail Post				a	72
15	1	Sidewalk	CR	M	Crack	Top of Sidewalk @	A = 0.05 m ²	Slight	One	c	34
16	1	Sidewalk	SER	S		14.0m from End Post	1 = 0.500 mm	Moderate	One	b	73
17	1				Exposed Rebar	Top of Sidewalk	A = 0.02 m ²	Slight	One	C	74
		Sidewalk	CR	н	Tranverse Crack	Top of Sidewalk	t = 3,00 mm	Heavy	Four	a	38
18	1	Sidewalk	FR	н	Fracture	end of Approach Bottom of Coping @	d = 150 mm	Heavy	Several	а	28
19	2	Coping / P2	HC	S	Honeycomb	Pier 2 Pier Body @	A = 0.8 m ²	Slight	One	c	66
20	2	Pier 2 Column	CR	м	Horizontal Crack	Downstream	t = 0.400 mm	Moderate	One	b	65
21	2	Pier 2 Column	CR	S	Vertical Cracks	Pier Body @ Front Side	t = 0.173 mm	Slight	Two	c	67
22	2	Pier 2 Column	CR	S	Map Crack	Pier Body @ Downstream	i = 0.076 mm	Slight	Many	c	68
23	3	Girder G-4 / Cantilever Slab	SER	м	Spall	Bottom of Cantilever Stab Flange of Girder	A = 0.15 m ²	Moderate	Two	b	53
24	3	Girder 1	CR	м	Vertical Crack	End of Cantilever	t = 0.350 mm	Moderate	Two	b	44
25	3	Cantilever Slab	SER	s	Exposed Rebar	Bottom of Sidewalk Upstream Side @ P2	A = 0.06 m ²	Slight	One	с	1
26	3	Cantilever Slab	SER	S	Exposed Rebar	Bottom Face of Sidewalk	A = 0.008 m ²	Slight	One	с	9
27	3	Cantilever Slab	CR	н	/Crack	Bottom of Cantilever Slab (Sidewalk)	t = 3.00 mm	Heavy	Two	а	45
28	3	Cantilever Slab	HC	н	Honey Comb	Bollom of Cantilever Slab (Sidewalk)	A = 1.44 m ²	Heavy	One	а	46
29	3	Deck Slab	SER	S	Spall	Bottom of Deck Slab @ Bay 1	A = 0.04 m ²	Slight	Öne	c	49
30	3	Deck Slab	SER	S	Spall	Boltom of Deck Slab @ Bay 2	A = 0.03 m ²	Slight	One	с	50
31	3	Extended Coping	SER	S	Exposed Rebar	Face of Coping @ P2	A = 0.02 m ²	Slight	One	с	10
32	3	Extended Coping	CR	s	Vertical Crack	Face of Coping @ P2	t = 0.300 mm	Slight	Two	с	11
33	3	Extended Coping, Pier 3	CR	м	Vertical Crack	Face of Coping	l = 0.450 mm	Moderate	Random	b	42
34	3	Girder 1	SER	S	Spall	Bollom of Girder 1 mid Span-S3	A = 0.09 m ²	Slight	One	с	43
35	3	Girder 1	SER	S	Exposed Rebar	Face of Girder 1	A = 0.015 m ²	Slight	Two	- c	48
36	3	Girder 4	SER	s	Spall	Face to Bottom of	A = 0.04 m ²	Slight	One	c	52
37	3	Girder G-4	CR	м	Diagonal Crack	Girder Face of Girder	t = 0.300 mm	Moderate	Two	b	54
38	3	Intermediate Diaphragm	нс	M	Honeycomb	Bottom of Intermediate	A = 0.105 m ²	Moderate	One	b	3
39	3	Intermediate Diaphragm	НС	н		Diaphragm @ Bay 1, Bottom of Diaphragm	A = 1.30 m ²	Heavy	One		47
40	3	Intermediate Diaphragm	SER	s		Face of Diaphragm @	A = 0.025 m ²	Slight	One	a c	51
41	3	Pier Post (P3)	CR	м	Vertical Crack	Bav 3 Near Footing of Pier	t = 0.350 mm	Moderate	Random		
42	3	PSC Girder (G-1)	SER	s	Exposed Rébar	Bottom of Girder 1	A = 0.02 m ²			b	
43	3	PSC Girder (G-2)	SER	s		@ P2 Top Flange of Girder 2		Slight	One	с 	2
43	3	PSC Girder (G-2)			Spall	@ P2 Bottom of Girder 3	A = 0.015 m ²	Slight	One	c	5
45	3		SER	S	Exposed Rebar	145@ P2 Bottom of Girder 3	A = 0.08 m ²	Slight	One	c	4
		PSC Girder (G-3)	SER	s	Exposed Rebar	@ P2 Bottom of Girder 3	A = 0.04 m ²	Slight	One	с	6
46 47	3	PSC Girder (G-3)	SER	S	Exposed Rebar	@ P2 Top Flange of Girder 3	A = 0.012 m ²	Slight	One	c	7
	3	PSC Girder (G-3)	SER	s	Exposed Rebar	@ P2	A = 0.015 m ²	Slight	One	с	8

Appendix 7.1.1-1 (20/34)

Damage	Span		Type of	Rank of		Desc	ription of Damage			Remarks	Photo
No.	No.	Name of Member	Damage	Damage	Nature	Location/ Pattern	Scale	Severity	No. of Damages	(a, b, c)	No.
49	3	Sidewalk	CR	н	Tranverse Crack	Top of Sidewalk near End of Approach Slab	t = 1.500 mm	Heavy	Two	а	75
50	4	Sidewalk	SER	м	Spall	Top of Sidewalk	A = 0.22 m ²	Moderate	One	b	31
51	2&3	Sidewalk	SER	М	Spall	Top of Sidewalk Between Span 2 & 3	A = 0.02 m ²	Moderate	One	b	30

Name of Bridge : Ma1.2 VARGAS BRIDGE "B" (Downstream)

Date of Inspectiv Nov. 26, 2002 Inspector E. Pagaragan/R. Abad Checker J. B. Agnes

Damage	Span		Type of	Rank of		Des	cription of Damage	8		Bornalis	Db-t-
No.	No.	Name of Member	Damage	Damage	Nature	Location/ Pattern	Scale	Severity	No. of Damages	Remarks (a, c, c)	Photo No.
1	1	Sidewalk @ Abut. A"	CR	н	Transverse Crack	Downstream Side	t = 1.50 mm	• Heavy	One	а	16
2	1	Approach Slab	CR	M	Transverse Crack	Whole Width of Road	t = 5 mm	Moderate	One	b	17
3	1	Approach Slab	CR	s	Random Cracks	Outer Lane of Road	t = 1 mm	Slight	Many	С	18
4	1	Abut. "A" Curb	SER	S	Spall	Along Expansion Join	A = 0.03 m ²	Slight	One	c	19
5	_ 2 _	Deck Slab	SER	S	Exposed Rebar	Around Restreining Boltom of Slab @	A = 0.01 m ²	Slight	Опе	c	-
6	2	Cantilever Slab	CR	S	Crack	Linstream 150 mm Diameter	t = 0.254 mm Remarkable Reduction	Slight	One	c	11
7	2	Drainage Pipe	co	н	Corrosion	GI Pine @ End Bottom of Stab	of Section	Heavy	One	a	14
8	2	Deck Slab	SER	S	Exposed Rebar	Bollom of Slab	A = 0.09 m ²	Slight	One	c ·	12
9	2	Deck Slab	SER	S	Exposed Rebar	- @ Bay 2	A = 0.02 m ²	Slight	One	C	13
10	3	Railing	SER	S	Exposed Rebar	Bottom of Railing Bottom Face of Slab	A = 0.04 m ²	Slight	One	c	20
12	3	Cantilever Slab Cantilever Slab	HC CR	H M	Honeycomb	Bottom Face of Slab	$A = 1 m^2$	Heavy	One	a	1
13	3	Cantilever Slab	HC	H	Cracks Honeycomb	Boltom Face of Slab	t = 0.450 mm	Moderate	Many	b	1
14	3	Bearing Plate	CO	 	Carrosion	Below Girder 1	A = 1 m ² Remarkable Reduction	Heavy	One	a	2
15	3	Coping Pier 2	CR	M	Vertical Crack	Pier 2 Face of Coping @	t = 0.600 mm	Moderate Moderate	One	b	3
16	3	Coping Pier 2	CR	M	Vertical Cracks	- Bay 1 corper G1 Face of Coping	t = 0.350 mm	Moderate	Two	b	4
17	3	Coping Pier 2	SER	S	Spall		A = 0.03 m ²	Slight	One	-	5
18	3	Coping Pier 2	CR	s	Vertical Crack	A Bay 1 Face of Coping	l = 0.300 mm	Slight	One	c c	5
19	3	Coping Pier 2	CR	s	Vertical Cracks	Face of Coping	t = 0.300 mm	Slight	Three	· c	
20	3	Coping Pier 2	CR	M	Vertical Crack	Face of Coping	t = 0.550 mm	Moderate	One	b	6
21	3	Cantilever Slab	CR	S	Map Crack	Below Girder 3 Boltom of Slab	t = 0.200 mm	Slight	Many	c	7
22	3	Girder 3	co	S	Corrosion	Boliom Flange	Surface Rust	Slight	One	c	8
23	3	Deck Slab	SER	M	Exposed Rebar	of Girder Bottom Face of Stab	A = 0.12 m ²	Moderate	One	b	9
24	3	Deck Slab	SER	S	Exposed Rebar	Bottom Face of Stab	A = 0.10 m ²	Slight	One	c	10
25	3	Railing	SER	М	Spall	Top of Railing	A = 0.13 m ²	Moderate	One	b	21
- 26	3	Deck Slab	нс	S	Honeycomb	Around Protruding Threaded Bolt @ Bav	A = 0.005 m ²	Slight	One	с	28
27	3	Coping @ Pier 3	CR	S	Vertical Crack	Coping Face @ Bay 1	t = 0.250 mm	Slight	One	с	29
28	3	Coping @ Pier 3	CR	S	Diagonal Crack	Coping Face under Girder 2	1 = 0.300 mm	Slight	One	c	30
29	3	Coping @ Pier 3	SER	S	Spall	Coping Face under Girder 2	A = 0.06 m ²	Slight	One	C	30
30	3	Coping @ Pier 3	CR	М	Random Cracks	Coping Face @ Bay 2	t = 0.550 mm	Moderate	Seven	b	-
31	3	Deck Slab	SER	S	Spall	Bollom of Slab @ Bay 2	A = 0.03 m ²	Slight	Four	с	31
32	3	Deck Slab	CR	H ·	Crack	Bottom of Slab @ Bay 2	t = 1.500 mm	Heavy	One	а	32
33	3	Interior Support @ Pier 3	со	S	Corrosion	Top of Bracing	Surface Rust	Slight	One	c	33
34	3	Interior Support @ Pier 3	со	S	Corrosion	Top of Bracing	Surface Rust	Slight	. One	с	34
35	3	Deck Slab	CR	м	Crack	Bottom of Slab @ Bay 2	t = 0.400 mm	Moderate	One	b	35
36	3	Deck Slab Steel Bracing	SER	S	Exposed Rebar	Bottom of Slab @ Bay 2	A = 0.01 m ²	Slight	One	c	35
37	3	@ Pier 3	со	н	Corrosion	Expansion Joint	Remarkable Reduction of Section	Heavy	One	а	36
38	3	Deck Slab	SER	S	Exposed Rebar	Bottom of Slab	A = 0.02 m ²	Slight	One	с	37
39	3	Deck Slab	CR	S	Crack	Bottom of Slab	t = 0.102 mm	Slight	One	c	37
40	4	Sidewalk	CR	s	Transverse Crack	Abutment B	t = 0.173 mm	Slight	One	с	15
41 42	4	Curb Sidewalk	SER CR	<u>s</u> н	Spall Transverse Crack	Top of Curb Near Abutment	A = 0.03 m ² l = 1 mm	Slight Heavy	One	c b	22 23
43	4	Sidewalk	CR	М	Longitudinal Crack	Top of Sidewalk	t = 0.361 mm	Moderate	One	b	23 23A
44	4	Sidewalk	SER	S	Spall	Top of Sidewalk	A = 0.02 m ²	Slight	One	D	23A
45	4	Railing	CR	н	Crack	Face of Railing	t = 3.00 mm	Heavy	One	a	24
46	4	Sidewalk	CR	Н	Crack	Top of Sidewalk	l = 1.00 mm	Heavy	One	а	25
47	4	Railing	CR	М	Crack	Top of Railing	t = 0.550 mm	Moderate	One	b	26
48	4	Curb	SER	S	Spall	Top of Curb 3.0 m from Approach	A = 0.08 m ²	Slight	One	c	27

Name	of	Bridge	:
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Date of Inspection <u>'December 12,13 &16 2002</u> Inspector <u>N.M.Castro/ R.A. Abad</u> Checker J.B. Agnes

Name of Br	idge :	Ma2_ROSARIO BRIDGE						Inspector Checker		R.A. Abad	
Damage	Span		Type of	Rank of		Desc	ription of Damage	9		Deservice	Direte
No.	No.	Name of Member	Damage	Damage	Nature	Location/ Pattern	Scale	Severity	No. of Damages	Remarks (a, b, c)	Photo No.
1	1	Pier 1 Coping	CR	S	Transverse Crack	End of Coping Downstream	t = 0.129 mm	Slight	Two	с	1
2	1.	Cantilever Slab	SER	S	Exposed Rebar	Bottom Face Downstream	A = 0.05 m ²	Slight	One	с	2
.3	1	Cantilever Slab	CR	н	Transverse Crack	Boltom Face Downstream	. t = 0.61 mm	Heavy	One	а	3
4	1	Cantilever Slab	SER	s	Exposed Rebar	Bottom of Slab & Haunch	A = 0.02 m ²	Slight	One	c	4
5	1	PSC Girder G-1	CR	s	Vertical Crack	Top of Diaphgram @ Bay 1	t - 0.102 mm	Slight	One	c	5
6	1	Cantilever Slab	SER	S	Exposed Rebar	Pipe Sleeve	A = 0.046 m ²	Slight	One	c	6
7	1	PSC Girder G-1	CR	s	Random Cracks	Construction Joint @ Top	l = 0.076 mm	Slight	Several	с	7
8	1	End Diaphgram @ Pier 1	SER	s	Spall	Front Face @ Downstream	A = 0.01 m ²	Slight	One	с	8
9	[·] 1	Cantilever Slab U/S	CR	м	Transverse Crack	Bottom @ 0.5 m from Abut A	t = 0.35 mm	Moderate	One	b	170
10	1	Cantilever Slab U/S	SER	s	Spall	Bottom @ Mid-Span	A = 0.05 m ²	Slight	One	с	171
11	1	PSC Girder G-1	SER	S .	Spall	Bottom Flange @ Mid- Span	A = 0.06 m ²	Slight	One	с	172

Appendix 7.1.1-1 (21/34)

Int Number Number <th>Damage</th> <th>Span</th> <th>1</th> <th>Type of</th> <th>Rank of</th> <th><u> </u></th> <th>Des</th> <th>scription of Damag</th> <th>e</th> <th></th> <th>Demostre</th> <th>Dhata</th>	Damage	Span	1	Type of	Rank of	<u> </u>	Des	scription of Damag	e		Demostre	Dhata
10. 1. PRESCRAP SRN 3. Journal of any array of a second of any array array of any array of			Name of Member			Nature		Scale	Severity		Remarks (a, b, c)	Photo No.
11 1 Degliging Bay2 CR 9 Processors Processor	12	1		SER	s	Exposed Rebar		$A = 0.005 m^2$	Slight		c	173
14 Integrit, Dayle, Dayle, Sympositic Sympositic A - 20.2 rd Bypositic Byp	13	1				·/	Bottom @ 2.5 m from					1
11 Deck Sing Physics CDI 1 Ansame 1 = 1.5 mm How Act 00.5 mm How How How How	14	1	Interm. Diaph.		· · · · · · · · · · · · · · · · · · ·							
11 1 PB2 (1996) SEB 5 Parameter Ten Targe A 1400/A Base Cont Cont< Cont Cont Cont Cont Cont Cont Cont Cont Cont <td>i</td> <td></td> <td></td> <td>1</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td>	i			1								
11 Hole Same S		-						+				
1 1 6 6 100	⊢ −−−								Slight	Several	c	177
13 Hiers Doph 10. 8 Longents Longents <thlongents< th=""> <thlongents< th=""> <thlongents< th=""></thlongents<></thlongents<></thlongents<>		+	@ Bay 4		· · · ·			A = 0.06 m ²	Slight	One	<u>с</u>	178
10 1	18	1		SER	S	Exposed Rebar	Top Flange @ Bay 6	A = 0.04 m ²	Slight	One	C.	179
21 1 Description CPU 1 Perspective 1 New Lease Lease <thlease< th=""> <thlease< th=""> Lease<td>19</td><td>1</td><td></td><td>HC</td><td>S</td><td>Honeycomb</td><td>Тор</td><td>A = 0.009 m²</td><td>Slight</td><td>One</td><td>с</td><td>180</td></thlease<></thlease<>	19	1		HC	S	Honeycomb	Тор	A = 0.009 m ²	Slight	One	с	180
1 1	20	1	PSC Girder G-6	SER	S	Spall	Bottom Flange	A = 0.03 m ²	Slight	One	c	181 /182
1 358mail CR H notace Type of a feat o	21	1	Deck Slab	CRPL	н	Random Cracks	Above Abut. "A"	t = 1.5 mm	Heavy	Several	а	193
24 1 Abghalt Parented FPC 1.4 Constant Insu Fr 6 a - 2 m. Moore and Oos 30 200 25 1 Raining OEF M Balaned 1 ma Fr 6 a - 2 m. Moore and Oos 0 20 20 25 1 Approade Status CRFL Sicurati Male Parenter 1 - 100 mm Moore and Oos 0 10 27 1 Approade Status CRFL Sicurati	22	1	Sidewalk	CR	н	Random Cracks	2 m. from Abut. "A"	t = 1.0 mm	Heavy	Many	a	194
24 1 Append Programm 570 M Computer dot 21 0 + 20 M Dot 20 25 1 Relating DEF M Dot mode In the D Relation 20 Main 20 Com D 20 1 Approx 50 Com Com D 100 D D 100 27 1 Approx 50 CRL M Grand Append Factors Handback 20/cms Number 20 D	23	1	Sidewalk	CR	н	Random Cracks	Typical @ all Spans	t = 0.921 mm	Heavy	Many	а	206
1 Selling OEF M Dubment In turn P Resautb Dataton Justame Ora Data 28 1 Appool Sale CRPL N Ones Appa Amount It 200ms Bayes One C 191 28 1 Appool Sale CRPL M Ones Appa Amount It 200ms Bayes One C 191 28 1 Approal Sale CRPL M Ones Appa Amount Homode An a 33 d Bayes One D 201 33 1 Reling DM M Method Sale Part Amount Hemataba Datato Amount D 200 33 2 PSC Carder G-1 CR Nutations Reling Amount Amount Hemataba Datato Amount D	24	1	Asphalt Pavement	F/C	м	Corrugation		d = 25 mm			<u> </u>	
1 Approach Sub CSRP 9 Cuaca Apper Amment I = 20 mm Signt Cua 0 100 27 1 Approach Sub CRP N Generation Construction Notations Nom Dots 10 Streads Streads <td>25</td> <td>1</td> <td></td>	25	1										
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29 1 Approach Sizb CMP 8 Coulds Apple Pacing Bar Col Col 30 1 Raling DEF M Othermel Bird Appendix Diracits Networks Ore Bo 202 31 1 Raling DM M Defmer Bird Appendix Rescalable Electronic Ore Bo 202 32 2 PSC Clefer G-1 CR S Westar Cass Prest Device Feature		-				Crocodile Cracks	Asphalt Pavement	t = 10.00 mm	Moderate		b	192
30 1 Ruling DEF M Polement But if Appaul Remarked buffetts Votes 7 Core De 223 31 1 Raling DM M Polement 2m in Anjung None Acesses Out Do 233 2 PSC Grade C-1 CR S Yutata Case Form Failury Form Failury Form Failury Signt Social C B 34 2 PSC Grade C-1 SER S Social Pole Crade C-1 SER Social Pole Crade C-1 SER Social Pole Crade C-1 SER Social Pole Crade C-1 SSC Pole Crade C-1 Pol							Edge	A = 0.315 m ²	Slight	One	C	200
1 1 Rulling DM M Decomporting Uterung 2 mit management and multicity Management and multicity Management and multicity Management and multicity Management and multicity Management 	L	1	Approach Slab	CRPL	S	Cracks	Asphalt Pavement	t = 3.00 mm	Slight	Several	c	201
1 1	30	1	Railing	DEF	м		Start of Approach	Remarkable Deflection	Moderate	Two	b	202
1 Imain of the set of the	31	1	Railing	D/M	М		23 m from Approach	Remarkable Deflection	Moderate	One	b	203
Job L Job Cardine Srift CA S Number Mark Desker Job Hold Mark Basker Job Solar C Basker Job Solar Constraint Walk Lob Cardine's Walk Basker Job C Basker Job Disc Solar Job Disc Job Disc Job Disc Job	32	1	Railing	D/M	М		0.5 m from Abut. "A"	Remarkable Missing	Moderate	One	b	204
34 2 Contrace-III SER S Factor Mark	33	2	PSC Girder G-1	CR	s	Vertical Crack		t = 0.102 mm	Slight	Several		
35 2 PSC Differ C-1 SER S Spat Description WS A = 0.1 m ² Bight Ore G 10 36 2 Sett Start Of Control CO M Correlation WS A = 0.1 m ² Bight Ore A D 11 37 2 Piert Coping SER S Expect Netw Ore Note A = 0.0 m ² Bight Served C 113 38 2 Deck Slab SER M Expect Rest Field Tot A = 0.0 m ² Bight Served C 113 39 2 Piert Coping SER M Expect Rest Field Tot A = 0.0 m ² Moderst Ores b) 15 41 2 Deck Slab SER S Expect Rest	34	2	Cantilever Slab									
36 2 Steel Bearing m. Bearl CO M Contrain Ite Reductor of the main of the 1-to Reductor of the Reductor of the steps Moderate (1) 0.24 Moderate (1) Moderate	35	2	PSC Girder G-1				Construction Joint @					
37 2 Disbler A Piel (Coping) SER S begeer Relative (Second) Batter (Second) Device (Second) Device (Second) <thdevice (Second) <thdevice (Second) <th< td=""><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></th<></thdevice </thdevice 												
30 2 Deck Slab CR S Rendem Create Prior Face (gibre) 1 = 0.24 mm Signt Security Encode Constraints 39 2 Pier 1 Coping Coping SER S Exponed Rear. Field Coping A = 0.24 m ² Signt Security Cons C 14 40 2 Deck Slab SER M Signt Security A = 0.0 m ² Moderate A = 0.0 m ² One C 17 41 2 Deck Slab SER S Exponed Rear. End A = 0.10 m ² Signt One C 17 43 2 PSC Girder C-4 SER S Exponed Rear. End A = 0.12 m ² Signt One C 18 45 2 PSC Girder C-4 SER S Exponed Rear. Fore Girder S Signt One C 19 46 2 PSC Girder C-4 SER S Exponed Rear. Fore Girder S Signt One C 21 47 2 <td></td> <td></td> <td></td> <td>· · · · ·</td> <td></td> <td></td> <td></td> <td>Section</td> <td></td> <td></td> <td> </td> <td></td>				· · · · ·				Section				
39 2 Pier I Coping SER S Expont Return Face of Coping A = 0.05 m² Sign Dow D 15 410 2 Deck Slab SER M Spatt Broken Teste One D 15 41 2 Deck Slab SER M Spatt Broken Teste A = 0.10 m² Sight One D 16 42 2 Deck Slab SER S Expound Return End A = 0.10 m² Sight One C 17 43 2 PSC Girder G-5 SER S Expound Return End Rot A = 0.10 m² Sight One C 19 45 2 PSC Girder G-5 SER S Expound Return To Ringe @ Sigst A = 0.02 m² Sight One C 21 46 2 Deck Sible @ Bay 7 CR H Readom Cracks Bolom of Bab i = 0.01 m² Hespet Severat a 22								A = 0.06 m ²	Slight	One	c	12
40 2 Deck Slab SER M Seed Bottom of See A = 0.25 m² Moderate One De 15 41 2 Deck Slab SER M Seed Hundrifiet A = 0.25 m² Moderate One D 16 42 2 Deck Slab SER SE Expeed Rear End A = 0.10 m² Sight One C 17 44 2 PSC Girder G-4 SER S Expeed Rear Feldeck A = 0.25 m² Sight One C 18 45 2 PSC Girder G-6 SER S Exposed Rear Proling @ Bay 6 A - 0.25 m² Sight One C 21 46 2 Deck Sibb @ Bay 6 CR H Redom Crass Belom of Bab i = 0.61 m² Sight One C 21 47 2 Deck Sibb @ Bay 6 CR H Redom Crass Belom of Bab i = 0.15 m² Sight Seerant <t< td=""><td></td><td></td><td>Deck Slab</td><td>CR</td><td>S</td><td>Random Cracks</td><td>Front Face @ Bay 9</td><td>t = 0.254 mm</td><td>Slight</td><td>Several</td><td>c</td><td>13</td></t<>			Deck Slab	CR	S	Random Cracks	Front Face @ Bay 9	t = 0.254 mm	Slight	Several	c	13
41 2 Deck Slab SER M Beel HumonField A = 0.17 ar Moderate Data b 16 42 2 Deck Slab SER S Exposed Rear End A = 0.10 m ² Bight One C 17 43 2 PSC Girder G-1 SER S Exposed Rear End Bloch A = 0.02 m ² Bight One C 17 44 2 PSC Girder G-5 SER S Exposed Rear Deck Slab (Q Bay C C 18 45 2 PSC Girder G-6 SER S Exposed Rear Top Prange (B Bay C A = 0.02 m ² Sight One C 20 47 2 Deck Slab (Q Bay C CR H Readom Oracia Bettern of Bab 1 = 0.01 mm Heary Several a 22 49 2 Deck Slab (Q Bay 7 CR H Readom Creats Bottern of Bab 1 = 0.01 mm Heary Several a 22	39	2	Pier 1 Coping	SER	S	Exposed Repar	Face of Coping	A = 0.04 m ²	Slight	One	c	14
42 2 Deck Stab SER S Exposed Relaw Fred Mail Are 0.10 m² Bight One C 17 43 2 PSC Girdler G-4 SER S Exposed Relaw End Book A = 0.10 m² Bight One C 17 44 2 RSC Girdler G-5 SER S Exposed Relaw Fast Grade Bight One C 18 44 2 PSC Girdler G-5 SER S Exposed Relaw Fast Grade A = 0.02 m² Bight One C 10 44 2 PSC Girdler G-6 SER S Exposed Relaw Tap Fange @ Bayt A = 0.015 m² Bight One C 21 48 2 Deck Stab @ Bayt CR H Readom Cracka Battom of Bab 1 = 0.015 m² Bight Several a 22 50 2 Deck Stab @ Bayt CR S Rendom Cracka Bottom of Bab 1 = 0.17 m² Bight Several<	40	2	Deck Slab	SER	м	Spall	Bottom of Slab	A = 0.255 m ²	Moderate	One	b	15
43 2 PSC Girder G-4 SER S Exposed Reture End Block A = 0.0 m ² Style One C 117 44 2 PSC Girder SER S Exposed Reture Ballon of Sile A = 0.0 m ² Style One C 18 45 2 PSC Girder G-5 SER S Exposed Reture Pron Face (g Bary 7 A = 0.0 m ² Style One C 19 47 2 PSC Girder G-6 SER S Exposed Reture Top Farlong (g Bary 7 A = 0.0 m ² Style One C 20 49 2 Deck Slab (g Bary 7 CR H Rendom Crads Balton of Slab I = 0.01 m Heary Beered Hear A = 0.12 m ² Moderate One D 2 25 50 2 Deck Slab (g Bary 6 CR S Paradom Crads Balton of Slab I = 0.127 m Skylt Skylt Skylt Skylt Skylt Skylt Skylt Skylt Skylt	41	2	Deck Slab	SER	М	Spell	Hunch/Fillet	A = 0.174 m ²	Moderate	One	b	16
43 2 PSC Girdor G-4 SER S Exposed Retur End Block. A = 0.0 m ² Stiph One c 11 44 2 PSC Girdor G-5 SER S Exposed Retur Bolton of Site A = 0.0 m ² Sight One c 18 45 2 PSC Girdor G-6 SER S Exposed Retur Top Fance (B 28) A = 0.0 m ² Sight One c 19 44 2 DecK Slab (D 24) G CR H Rendom Creds Bolton of Site 4 = 0.0 m ² Sight One C 21 48 2 DecK Slab (D 24) T CR H Rendom Creds Bolton of Site 1 = 0.61 mm Heavy Several a 22 49 2 DecK Slab (D 24) T CR N Rendom Creds Bolton of Site A = 0.17 am Moderate One D 22 51 2 DecK Slab (D 24) S CR S Rendom Creds Bolton of Site A = 0.17 am	42	2	Deck Slab	SER	s	Exposed Rebar	End	A = 0.10 m ²	Slight	One	с	17
44 2 PSC Girder Cut & Cut SER S Exposed Reser flottom of Sile A = 0.02 m ² Sight One C 18 45 2 PSC Girder G-5 SER S Exposed Reser Pert Fare @ Bay7 A = 0.02 m ² Sight One C 20 46 2 PSC Girder G-5 SER S Exposed Reser Top Parage @ Bay6 A = 0.02 m ² Sight One C 20 47 2 Deck Slab @ Bay6 CR H Rendom Oracks Belond fills i = 0.61 nm Heavy Secard a 22 49 2 Deck Slab @ Bay7 CR H Rendom Crasks Belond fills i = 0.61 nm Heavy Secard a 23 50 2 Deck Slab @ Bay8 CR S Rendom Crasks Belond fills i = 0.127 nm Sight Secard c 28 51 2 Deck Slab @ Bay8 SER M Exposed Rebser Const.Jotst	43	2	PSC Girder G-4	SER	S	Exposed Rebar	End Block	$A = 0.10 \text{ m}^2$	Stight	One	c	
List A ten List A ten List A ten List A ten Com Com <thc< td=""><td>44</td><td>2</td><td></td><td>SER</td><td></td><td></td><td></td><td></td><td>-</td><td></td><td></td><td></td></thc<>	44	2		SER					-			
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47 2 PSC Girder C-6 SER S Exposed Retar Top Flange @ Bay A = 0.0 am ² Sight One C 2 48 2 Deck Slab @ Bay 6 CR H Random Cracks Boltom of Slab 1= 0.01 mm Heavy Several a 23 50 2 Deck Slab @ Bay 7 CR H Random Cracks Boltom of Slab 1= 0.01 mm Heavy Several a 23 50 2 Deck Slab @ Bay 7 CR H Random Cracks Boltom of Slab 1= 0.11 mm Moderate One D 24 51 2 Deck Slab @ Bay 8 CR S Random Cracks Boltom of Slab 1= 0.127 mm Slipht Several C 25 53 2 Deck Slab @ Bay 8 SER M Exposed Rebe Const. Joint A = 0.17 mm Slipht Several C 28 55 2 Deck Slab @ Bay 8 SER S Exposed Rebe Boltom of Slab												
48 2 Deck Slab @ Bay 6 CR H Random Create Bettom of Slab 1=0.81 nm Heavy Several a 2.2 49 2 Deck Slab @ Bay 7 CR H Random Create Extinuation Joint A = 0.174 m² Moderate One b 2.3 50 2 Deck Slab @ Bay 7 SER M Exposed Reber Construction Joint A = 0.174 m² Moderate One b 2.4 51 2 Deck Slab @ Bay 8 CR S Random Create File 1.027 nm Stylt Several C 2.8 53 2 Deck Slab @ Bay 8 CR S Random Create Bottom of Slab 1=0.127 nm Stylt Several C 2.8 54 2 Deck Slab @ Bay 9 CR S Random Create Bottom of Slab 1=0.127 nm Stylt Several C 2.8 55 2 Deck Slab @ Bay 9 SER M Exposee Reber Const. Joint A							Top Flange @ Bay 5	A = 0.015 m ²	Slight	One	c	20
49 2 Deck Slab @ Bay 7 CR H Random Cracke Bottom of Slab 1 = 0.51 mm Heave Several a 2 50 2 Deck Slab @ Bay 7 SER M Exposed Reber Construction Joint A = 0.174 m² Moderate One b 24 51 2 Deck Slab @ Bay 8 CR S Random Cracke Bottom of Slab 1 = 0.127 mm Slight Several C 25 52 2 Deck Slab @ Bay 8 CR S Random Cracke Bottom of Slab 1 = 0.127 mm Slight Several C 25 53 2 Deck Slab @ Bay 9 CR S Random Cracke Bottom of Slab 1 = 0.127 mm Slight Several C 28 54 2 Deck Slab @ Bay 9 CR S Random Cracke Bottom of Slab 1 = 0.127 mm Slight One C 31 55 2 Deck Slab @ Bay 9 CR S Random Cracke Botto			PSC Girder G-6	SER	S	Exposed Rebar	Top Flange @ Bay 6	A = 0.03 m ²	Slight	One	c	21
50 2 Deck Slab @ Bay,7 SER M Exposed Relar Construction Joint A = 0.174 m ² Moderate One b 24 51 2 Pier 1 Coping CR S Random Crack Front Face i= 0.127 mm Sight Several C 25 52 2 Deck Slab @ Bay 8 CR S Random Crack Boltom of Slab i = 0.127 mm Sight Several C 25 53 2 Deck Slab @ Bay 8 SER M Exposed Rebat Const. Joint A = 0.170 m ² Moderate One b 27 54 2 Deck Slab @ Bay 9 SER M Exposed Rebat Const. Joint A = 0.170 m ² Moderate One b 28 56 2 Dieck Slab @ Bay 9 SER M Exposed Rebat Const. Joint A = 0.170 m ² Moderate One a 30 57 2 Cantilever Slab SER S Exposed Rebat Boitom of Sl	48	2	Deck Slab @ Bay 6	CR	н	Random Cracks	Bottom of Slab	t = 0.61 mm	Heavy	Several	a	22
512Pier 1 Coping Data CRCRSRandom CracksPront Face Betword 27I = 0.127 mmSightSeveralC25522Deck Slab @ Bay 8CRSRandom CracksBetword 27I = 0.127 mmSightSeveralC25532Deck Slab @ Bay 8SERMExposed RebarConst. Joint $A = 0.170 \text{ m}^3$ ModerateOnebb27542Deck Slab @ Bay 9CRSRandom CracksBoltom of Slab $I = 0.127 \text{ mm}$ SightSeveralC28552Deck Slab @ Bay 9SERMExposed RebarConst. Joint $A = 0.174 \text{ m}^3$ ModerateOne029562Pier 1 CopingCRHHortsontal CrackEnd of Coping $I = 2 \text{ rm}$ MeavyOnec31572Cantilever SlabSERSExposed RebarBoltom of Slab $A = 0.10 \text{ m}^2$ SightOnec33602Pier 1 BodySERSSpaalEnd Upstream Side $A = 0.06 \text{ m}^2$ SightOnec33612Pier 1 BodySERSSpaalEnd Upstream Side $A = 0.06 \text{ m}^2$ SightOnec33622Cantilever SlabCRHTransverse CrackBoltom Bow P2 Boltom Bow P2LeaderamHeavyThreea36632Cantilever SlabCRH <t< td=""><td>49</td><td>2</td><td>Deck Slab @ Bay 7</td><td>CR</td><td>н</td><td>Random Cracks</td><td>Bottom of Stab</td><td>t = 0.61 mm</td><td>Heavy</td><td>Several</td><td>а</td><td>23</td></t<>	49	2	Deck Slab @ Bay 7	CR	н	Random Cracks	Bottom of Stab	t = 0.61 mm	Heavy	Several	а	23
1 2 Pref Portuging CM 3 Reliable All Below G2ab 1 = 0.12 mm Stight Several C 25 52 2 Deck Slab @ Bay Ø CR S Random Cracka Below of Sab 1 = 0.12 mm Stight Several C 28 53 2 Deck Slab @ Bay Ø SER M Exposed Rebar Const. Joint A = 0.170 m² Moderate One bb 28 55 2 Deck Slab @ Bay Ø SER M Exposed Rebar Const. Joint A = 0.174 m² Moderate One bb 29 56 2 Deck Slab @ Bay Ø SER S Exposed Rebar Boltom of Slab A = 0.174 m² Moderate One c 31 57 2 Cantilever Slab SER S Exposed Rebar Boltom of Slab A = 0.076 m² Sight One c 32 59 2 Pier 1 Body SER S Verited Creck Front Face i=	50	2	Deck Slab @ Bay 7	SER	м	Exposed Rebar	Construction Joint	A = 0.174 m ²	Moderate	One	b	24
52 2Deck Slab @ Bay 8CRSRendom CrackaBottom of Stab $t= 0.127 \text{ mm}$ StightSeveralC28 53 2Deck Slab @ Bay 8SERMExposed RebarConst. Joint $A = 0.170 \text{ m}^3$ ModerateOneDe27 54 2Deck Slab @ Bay 9CRSRandom CracksBottom of Stab $1=0.127 \text{ mm}$ StightSeveralC28 55 2Deck Slab @ Bay 9SERMExposed RebarConst. Joint $A = 0.174 \text{ m}^3$ ModerateOneDe29 56 2Pier 1 CopingCRHHotzontal CrackEnd of Coping $1=2 \text{ rm}$ HeavyOnea300 57 2Cantiliever SlabSERSExposed RebarBottom of Stab $A = 0.05 \text{ m}^3$ StightOnec31 58 2Orantiliever SlabSERSExposed RebarBottom of Stab $A = 0.05 \text{ m}^3$ StightOnec32 59 2Cantiliever SlabSERSExposed RebarBottom of Stab $A = 0.05 \text{ m}^3$ StightOnec33 60 2Pier 1 BodySERSExposed RebarBottom of Stab $A = 0.05 \text{ m}^3$ StightOnec34 61 2Pier 1 BodyCRSSpallEnd Uptatern Side $A = 0.05 \text{ m}^3$ StightOnec34 61 2Pier 1 BodyCR <t< td=""><td>51</td><td>2</td><td>Pier 1 Coping</td><td>CR</td><td>s</td><td>Random Cracks</td><td></td><td>t = 0.127 mm</td><td>Slight</td><td>Several</td><td>с</td><td>25</td></t<>	51	2	Pier 1 Coping	CR	s	Random Cracks		t = 0.127 mm	Slight	Several	с	25
53 2 Deck Slab @ Bay 8 SER M Exposed Reber Const. Joint A = 0.170 m² Moderate One b 27 54 2 Deck Slab @ Bay 9 CR S Random Crecks Bottom of Slab I = 0.127 mm Sight Several C 28 55 2 Deck Slab @ Bay 9 SER M Exposed Reber Const. Joint A = 0.174 m² Moderate One A 30 56 2 Pier 1 Coping CR H Hotzontal Creak End of Coping I = 2 mm Heavy One a 30 57 2 Cantilever Slab SER S Exposed Rebar Bottom of Slab A = 0.06 m² Sight One c 31 58 2 Cantilever Slab HC S Honeycomb Bottom of Slab A = 0.06 m² Sight One c 33 60 2 Pier 1 Body CR S Varical Creck Front Face I = 0.229 mm Sight One c 35 61 2 Pier 1 Body CR H Transverse Creck Bottom above P2, Unceta Creck I = 0.69 mm Heavy Tree a 36	52	2	Deck Slab @ Bay 8	CR	S	Random Cracks		t=0.127 mm	Slight	Several	с	26
542Deck Slab @ Bay 9CRSRandom CrecksBottom of SlabL = 0.17 mNutchenAnd the severalC28552Deck Slab @ Bay 9SERMExposed RebarConst. JohnA = 0.174 m²ModeraleOneb29562Pier 1 CopingCRHHortzontal CreckEnd of Coping1 = 2 nmHeavyOnea30572Cantillever SlabSERSExposed RebarBottom of SlabA = 0.10 m²SlightOnec31582Cantillever SlabHCSHonsycombBottomA = 0.00 m²SlightOnec32592Pier 1 BodySERSSpallEnd Upstreem SideA = 0.00 m²SlightOnec33602Pier 1 ColumnSERSSpallCent CreckFront Face1 = 0.229 mmSlightOnec33612Pier 1 ColumnSERSSpallCent CreckFront Face1 = 0.08 m²SlightOnec33622Cantilever SlabCRHTransverse CreckBottom show P2, Upstreamt= 0.08 m²SlightOnec33632Cantilever SlabSERSSpallEnd of Grete @ 2A = 0.05 m²SlightOnec37642Stell Bearing @ Pier 2SERSSpallEnd of Grete @ 2A = 0.05 m²<	53	2						· · · ·				
552Deck Slab @ Bay 9SERMExposed RebarConst. Joint $A = 0.174 \text{ m}^2$ ModerateOneb29562Pier 1 CopingCRHHorizontal CreekEnd of Coping $1 = 2 \text{ nm}$ HeavyOnea30572Cantilever SlabSERSExposed RebarBottom of Sieb $A = 0.0 \text{ m}^3$ SightOnec31582Cantilever SlabHCSHoneycombBottom of Sieb $A = 0.06 \text{ m}^3$ SightOnec32592Pier 1 BodySERSSpallEnd Upstream Side $A = 0.06 \text{ m}^3$ SightOnec33602Pier 1 BodyCRSVarical CreekFront Face $I = 0.229 \text{ nm}$ SightOnec34612Pier 1 ColumnSERSSpallCence Column $A = 0.06 \text{ m}^3$ SightOnec35622Cantilever SlabCRHTransverse CreekBottom Edge $A = 0.06 \text{ m}^3$ SightOnec37632Cantilever SlabSERSSpallBottom Edge $A = 0.06 \text{ m}^3$ SightOnec37642Cantilever SlabSERSSpallBottom Edge $A = 0.07 \text{ m}^3$ SightOnec39652PSC Girder G-1SERSSpallEnd of Girder@ P2 $A = 0.07 \text{ m}^3$ Sight<												
562Pier 1 CopingCRHHorizontal CreekEnd of Coping $i = 2 \text{ rm}$ HeavyOnea30572Cantilever StabSERSExposed RebarBoltom of Stab $A = 0.00 \text{ m}^2$ SightOnec31582Cantilever StabHCSHoneycombBoltom $A = 0.06 \text{ m}^2$ SightOnec32592Pier 1 BodySERSSpallEnd Upstreem Side $A = 0.006 \text{ m}^2$ SightOnec33602Pier 1 BodyCRSVarical CreckFront Face $i = 0.29 \text{ rm}$ SightOnec33612Pier 1 ColumnSERSSpallCenter Column $A = 0.06 \text{ m}^2$ SightOnec35622Cantilever StabCRHTransverse CreckBoltom bove P2, Ustream $1 = 0.66 \text{ m}^3$ SightOnec37632Cantilever StabSERSSpallBoltom Edge $A = 0.06 \text{ m}^3$ SightOnec37642Steel Bearing M.DerzCOMCorrosionG-1 to G-10Reduction of Cross BoltomModerateAtbb38652PSC Girder G-1SERSSpallEnd of Girder (0; P2 $A = 0.07 \text{ m}^2$ SightOnec40662PSC Girder G-1CRSRandom CracksExposed ReberBack												
572Cantilever SlabSERSExposed RebarBoltom of Slab $A = 0.10 m^2$ SlightOnea300582Cantilever SlabHCSHoneycombBoltom of Slab $A = 0.06 m^2$ SlightOnec31592Pier 1 BodySERSSpallEnd Upstream Side $A = 0.06 m^2$ SlightOnec33602Pier 1 BodyCRSVarital CreackFront Face $i = 0.229 mn$ SlightOnec33612Pier 1 ColumnSERSSpallCenter Column $A = 0.06 m^2$ SlightOnec33622Cantilever SlabCRHTransverse CreackBoltom above P2 Upstream $i = 0.898 mn$ HeavyThreea36632Cantilever SlabSERSSpallBoltom Edge $A = 0.06 m^2$ SlightOnec37642Steel Bearing m P2rc Girder G-1SERSSpallEnd of Girder GP2 $A = 0.05 m^2$ SlightOnec39652PSC Girder G-1CRSRendom CracksEX Face GP2 $A = 0.05 m^2$ SlightOnec40662PSC Girder G-1CRSRendom CracksEX Face GP2 $A = 0.05 m^2$ SlightOnec40672Pier 2SERSExposed ReberBack Face $A = 0.05 m^2$ Slight </td <td>••••••</td> <td></td>	••••••											
582Cantilever SlabHCSHoneycombBottom $A = 0.06 m^2$ StightOneC32592Pier 1 BodySERSSpallEnd Upstream Side $A = 0.06 m^2$ StightOneC33602Pier 1 BodyCRSVerticel CreckFront Face $i = 0.229 mm$ StightOneC33612Pier 1 ColumnSERSSpallCenter Column $A = 0.06 m^2$ StightOneC34622Cantilever SlabCRHTransverse CreckBottom Bove P2 Upstream $i = 0.899 m^3$ StightOneC35632Cantilever SlabSERSSpallBottom Edge $A = 0.06 m^3$ StightOneC37642Cantilever SlabSERSSpallBottom Edge $A = 0.06 m^3$ StightOneC39652PSC Girder G-1SERSSpallEnd of Girde P2 Bottom Edge $A = 0.06 m^3$ StightOnec39662PSC Girder G-1CRSRandom CracksExt Face of End Bottom Edge $i = 0.167 mm$ StightOnec40672Pier 2SERSExposed ReberBack Face $A = 0.03 m^2$ StightOnec40682Deck Slab @ Bay 1HCMHoneycombBottom of Slab Top of Bottom of Slab Top of Back Face												
592Pier 1 BodySERSSpellEnd Upstream SideA = 0.006 m²SightOneC33602Pier 1 BodyCRSVarical CrackFront Face $i = 0.29 \text{ mm}$ SightOneC34612Pier 1 ColumnSERSSpallCenter Column $A = 0.096 \text{ m}^2$ SightOneC34612Pier 1 ColumnSERSSpallCenter Column $A = 0.09 \text{ m}^2$ SightOneC35622Cantilever SlabCRHTransverse CrackBottom above P2, Upstream $i = 0.869 \text{ mm}$ HeavyThreea36632Cantilever SlabSERSSpallBottom Edge $A = 0.06 \text{ m}^2$ SightOneC37642Steel Bearing On Pier 2COMCorrosionG-1 to G-10Reduction of Cross SectionsModerateAllb38652PSC Girder G-1SERSSpallEnd of Girder @ P2 $A = 0.075 \text{ m}^2$ SightOnec40662PSC Girder G-1CRSRandom CrocksEXt. Feee of End Block $i = 0.167 \text{ rm}$ SightOnec45682Deck Slab @ Bay 1HCMHoneycomb DiabraramBottom near End Diabraram $A = 0.12 \text{ m}^2$ ModerateOnea46692Deck Slab/Diaphragm @ Bat/ Diaphragm<							Bollom of Slab	A = 0.10 m ²	Slight	One	c	31
602Pier 1 BodyCRSVertical CreckFront Face1 = 0.229 mmSightOneC34612Pier 1 ColumnSERSSpallCenter ColumnA = 0.09 m²SlightOneC35622Cantilever SlabCRHTransverse CreckBoltom above P2, Upstream $t = 0.669 mm$ HeavyThreea36632Cantilever SlabSERSSpallBoltom above P2, Upstream $t = 0.669 mm$ HeavyThreea36642Steel Bearing M Eler?COMCorrosionG-1 to G-10Reduction of Cross SectionModerateAllb38652PSC Girder G-1SERSSpallEnd of Girder @P2 $A = 0.075 m^2$ SlightOnec39662PSC Girder G-1CRSRendom CracksExt Face of End Block $t = 0.167 mm$ SlightOnec40672Pier 2SERSExposed ReberBoltom near End Diabraram $A = 0.12 m^2$ ModerateOnec45682Deck Slab/Diaphragm @SERHSpallBoltom of Slab Top of Dileph. $A = 0.45 m^2$ HeavyOnea48692Deck Slab/Diaphragm @SERSExposed RebarBoltom of Slab Top of Dileph. $A = 0.45 m^2$ HeavyOnec49712Deck Slab/Diaphragm @ </td <td></td> <td></td> <td>· · · · · · · · · · · · · · · · · · ·</td> <td></td> <td></td> <td>Honeycomb</td> <td>Bottom</td> <td>A = 0.06 m²</td> <td>Slight</td> <td>One</td> <td>с</td> <td>32</td>			· · · · · · · · · · · · · · · · · · ·			Honeycomb	Bottom	A = 0.06 m ²	Slight	One	с	32
612Pier 1 ColumnSERSSpallCenter Column $A = 0.09 \text{ m}^2$ StightOneC35622Cantilever SlabCRHTransverse CreckBoltom above P2, Upstream $t = 0.869 \text{ mm}$ HeavyThreea36632Cantilever SlabSERSSpallBoltom above P2, Upstream $t = 0.869 \text{ mm}$ HeavyThreea36642Cantilever SlabSERSSpallBoltom above P2, Upstream $t = 0.869 \text{ mm}$ HeavyThreea36642Cantilever SlabSERSSpallBoltom dove P2, Upstream $t = 0.869 \text{ mm}$ HeavyOnec37642Steel Bearing (m Pier 2)COMCorrosionG-1 to G-10Reduction of Cross SectionModerateAllb38652PSC Girder G-1SERSSpallEnd of Girder @P2 $A = 0.075 \text{ m}^2$ SlightOnec40662PSC Girder G-1CRSRandom CracksEX Face of End Block $t = 0.167 \text{ mm}$ SlightOnec45682Deck Slab/@Bay 1HCMHoneycombBoltom near End Diabram $A = 0.12 \text{ m}^2$ ModerateOnec45692Deck Slab/Diaphragm @SERHSpallBoltom of Slab Top of Diabr $A = 0.12 \text{ m}^2$ ModerateOnec48 <td>59</td> <td>2</td> <td>Pier 1 Body</td> <td>SER</td> <td>S</td> <td>Spall</td> <td>End Upstream Side</td> <td>A = 0.006 m²</td> <td>Slight</td> <td>One</td> <td>с</td> <td>33</td>	59	2	Pier 1 Body	SER	S	Spall	End Upstream Side	A = 0.006 m ²	Slight	One	с	33
622Cantilever SlabCRHTransverse CreckBoltom above P2, Upstreamt = 0.869 mmHeavyThreea36632Cantilever SlabSERSSpallBottom Edge $A = 0.06 m^2$ SlightOneC37642Steel Bearing (m) Pier 2COMCorrosionG-1 to G-10Reduction of Cross SectionModerateAllb38652PSC Girder G-1SERSSpallEnd of Grider @ P2 $A = 0.075 m^2$ SlightOneC39662PSC Girder G-1CRSRandom CracksExt Face of End Block $i = 0.167 mm$ SlightSeveralC40672Pier 2SERSExposed RebarBack Face $A = 0.03 m^2$ SlightOneC45682Deck Slab/@ Bay 1HCMHoneycombBoltom near End Diabnaram $A = 0.12 m^4$ ModerateOneb46692Deck Slab/@ Bay 1HCMHoneycombBoltom of Slab Top of Dilahnaram $A = 0.48 m^2$ HeavyOnea48702Pier 2 CopingSERSExposed RebarBack Face under G-3 Dilahnaram $A = 0.04 m^2$ SlightOnec49712Deck Slab/Diaphragm @ Rav.3SERHExposed RebarBoltom of Slab Dilahnaram $A = 0.04 m^2$ SlightOnec4971 <td>60</td> <td>2</td> <td>Pier 1 Body</td> <td>CR</td> <td>S</td> <td>Vertical Crack</td> <td>Front Face</td> <td>t = 0.229 mm</td> <td>Slight</td> <td>One</td> <td>c</td> <td>34</td>	60	2	Pier 1 Body	CR	S	Vertical Crack	Front Face	t = 0.229 mm	Slight	One	c	34
622Cantilever SlabCRHTransverse CrockBoltom above P2, Upstream $t = 0.869 \text{ mm}$ HeavyThreea36632Cantilever SlabSERSSpallBoltom Edge $A = 0.06 m^2$ SlightOnec37642Steel Bearing (m) Pier 2COMCorrosionG-1 to G-10Reduction of Cross SectionModerateAllb38652PSC Girder G-1SERSSpallEnd of Grider @P2 $A = 0.05 m^2$ SlightOnec39662PSC Girder G-1CRSRandom CracksExt. Face of End Block $t = 0.167 mm$ SlightSeveralC40672Pier 2SERSExposed RebarBack Face $A = 0.03 m^2$ SlightOnec45682Deck Slab @ Bay 1HCMHoneycombBoltom near End Dleabrarm $A = 0.12 m^2$ ModerateOneb46692Deck Slab/Diaphragm @SERHSpallBoltom of Slab Top of Dleabrarm $A = 0.45 m^2$ HeavyOnec49702Pier 2 copingSERHExposed RebarBoltom of Slab $A = 0.04 m^2$ SlightOnec49712Deck Slab/Diaphragm @SERSExposed RebarBoltom of Slab $A = 0.04 m^2$ SlightOnec53722PSC GirderSER </td <td>61</td> <td>2</td> <td>Pier 1 Column</td> <td>SER</td> <td>S</td> <td>Spall</td> <td>Center Column</td> <td>A = 0.09 m²</td> <td>Slight</td> <td>One</td> <td>c</td> <td>35</td>	61	2	Pier 1 Column	SER	S	Spall	Center Column	A = 0.09 m ²	Slight	One	c	35
632Cantilever SlabSERSSpallBoltom Edge $A = 0.06 m^2$ SlightOnec37642Steel Bearing (h) Pier 2COMCorrosionG-1 to G-10Reduction of Cross SectionModerateAllb38652PSC Girder G-1SERSSpallEnd of Grider @P2 $A = 0.05 m^2$ SlightOnec39662PSC Girder G-1CRSRandom CracksExt Face of End Block $i = 0.167 mm$ SlightSeveralc40672Pier 2SERSExposed RebarBack Face $A = 0.03 m^2$ SlightOnec45682Deck Slab @ Bay 1HCMHoneycombBoltom near End Dlabhram $A = 0.12 m^2$ ModerateOneb46692Deck Slab/Diaphragm @ Rav 2SERHSpallBoltom of Slab Top of Dlabh, $A = 0.04 m^2$ SlightOnec49702Pier 2 CopingSERHExposed RebarBoltom of Slab $A = 0.04 m^2$ SlightOnec49712Deck Slab/Diaphragm @ Rav 3SERHExposed RebarBoltom of Slab $A = 0.04 m^2$ SlightOnec53722PSC GirderSERSSpallAbove Pier 2 $A = 0.04 m^2$ SlightOnec53732PSC GirderSERSSpal	62	2	Cantilever Slab	CR	н	Transverse Crack		t = 0,869 mm	Heavy	Three		
642Steel Bearing $M \operatorname{Pier 2}$ COMCorrosionG-1 to G-10Reduction of Cross SectionModerateAllb38652PSC Girder G-1SERSSpallEnd of Girder @P2 $A = 0.075 n^2$ SlightOneC40662PSC Girder G-1CRSRandom CracksExt Face of End Block $1 = 0.167 \text{ mm}$ SlightSeveralC40672Pier 2SERSExposed RebarBack Face $A = 0.03 m^2$ SlightOneC45682Deck Slab @ Bay 1HCMHoneycombBoltom near End Dilaphram $A = 0.12 m^2$ ModerateOneb46692Deck Slab/Diaphragm @ Bav 2SERHSpallBoltom of Slab Top of Dilaph, $A = 0.45 m^2$ HeavyOnea48702Pier 2 CopingSERSExposed RebarBoltom of Slab Top of Dilaph, $A = 0.04 m^2$ SlightOnec49712Deck Slab/Diaphragm @ Rav.3SERHExposed RebarBoltom of Slab $A = 0.04 m^2$ SlightOnec49722PSC GirderSERSSpallAbove Pier 2 $A = 0.04 m^2$ Slightonec53732PSC GirderSERSSpallTop Flange $A = 0.08 m^2$ SlightSeveralc54	63	2	Cantilever Slab									
652PSC Girder G-1SERSSpellEnd of Girder @ P2 $A = 0.075 m^2$ SlightOneC39662PSC Girder G-1CRSRandom CracksExt. Face of End Block $i = 0.167 mm$ SlightSeveralC40672Pier 2SERSExposed ReberBack, Face $A = 0.03 m^2$ SlightOneC45682Deck Slab @ Bay 1HCMHoneycombBoltom near End Diaphram $A = 0.12 m^2$ ModerateOneb46692Deck Slab Diaphragm @ Bav 2SERHSpellBoltom of Slab Top of Diaph. $A = 0.45 m^2$ HeavyOnea48702Pier 2 CopingSERSExposed RebarBack Face $A = 0.04 m^2$ SlightOnec49712Deck Slab/Diaphragm @ Bav 3SERHExposed RebarBack Face under G-3 $A = 0.04 m^2$ SlightOnea50712Pier 2 CopingSERSExposed RebarBack Face under G-3 $A = 0.04 m^2$ SlightOnea50722PSC GirderSERSSpellAbove Pier 2 $A = 0.08 m^2$ Slightonec53732PSC GirderSERSSpellTop Flange $A = 0.08 m^2$ SlightSeveralc54			Steel Bearing					Reduction of Cross				
66 2PSC Girder G-1CRSRandom CracksExt. Face of End Block $i = 0.167 \text{ mm}$ SlightSeveralC40 67 2Pier 2SERSExposed ReberBack Face $A = 0.03 \text{ m}^2$ SlightOneC45 68 2Deck Slab @ Bay 1HCMHoneycombBoltom near End Disphram $A = 0.12 \text{ m}^2$ ModerateOneb46 69 2Deck Slab/Diaphragm @ Bav 2SERHSpellBoltom of Slab Top of Diaphram $A = 0.45 \text{ m}^2$ HeavyOnea48 70 2Pier 2 CopingSERSExposed RebarBack Face $A = 0.45 \text{ m}^2$ SlightOnec49 71 2Deck Slab/Diaphragm @ Bav 3SERHExposed RebarBoltom of Slab Top of Diaph. $A = 0.04 \text{ m}^2$ SlightOnec49 71 2Deck Slab/Diaphragm @ Bav 3SERHExposed RebarBoltom of Slab $A = 0.38 \text{ m}^2$ HeavyOnea50 72 2PSC GirderSERSSpellAbove Pier 2 $A = 0.06 \text{ m}^2$ Slightonec53 73 2PSC GirderSERSSpellTop Flange $A = 0.08 \text{ m}^2$ SlightSeveralc54								Section				
CC Z Piec Ginder Gri CR S $Random Gaces$ $Block$ $I = 0.16 mm$ $Sight$ $Several$ C 40 67 2 Pier 2SERSExposed ReberBack, Face $A = 0.03 m^2$ SlightOne C 45 68 2 Deck Slab/Diaphragm @ Bav2SERHMoneycombBoltom near End Diaphrain $A = 0.12 m^2$ ModerateOne b 46 69 2 Deck Slab/Diaphragm @ Bav2SERHSpellBoltom of Slab Top of Diaph. $A = 0.45 m^2$ HeavyOne a 48 70 2 Pier 2 CopingSERSExposed RebarBeck Face under G-3 $A = 0.04 m^2$ SlightOne c 49 71 2 Deck Slab/Diaphragm @ Bav3SERHExposed RebarBoltom of Slab $A = 0.39 m^2$ HeavyOne a 50 72 2 PSC GirderSERSSpellAbove Pier 2 $A = 0.06 m^2$ Slightone c 53 73 2 PSC GirderSERSSpellTop Flange $A = 0.08 m^2$ SlightSeveral c 54							_					
682Deck Slab @ Bay 1HCMHoneycombBottom near End DiabhraamA = 0.12 m²ModerateOneb46692Deck Slab/Diaphragm @ Rav 2SERHSpellBottom of Slab / DiaphragmA = 0.12 m²ModerateOneb46702Pier 2 CopingSERSExposed RebarBack Face under G-3A = 0.45 m²HeavyOnea48712Deck Slab/Diaphragm @ Rav 3SERHExposed RebarBack Face under G-3A = 0.04 m²SlightOnec49712Deck Slab/Ciaphragm @ Rav 3SERHExposed RebarBottom of SlabA = 0.39 m²HeavyOnea50722PSC GirderSERSSpellAbove Pier 2A = 0.06 m²Slightonec53732PSC GirderSERSSpellTop FlangeA = 0.08 m²SlightSeveralc54							Block			Several	c	40
602Deck Slab/Diaphragm @ Bav 2SERHSpellDilophragm @ Dilophragm & Boltom of Slab Top of DilophA = 0.45 m²HeavyOnea448702Pier 2 CopingSERSExposed RebarBoltom of Slab Top of DilophA = 0.45 m²HeavyOnea48712Deck Slab/Diaphragm @ Rav.3SERHExposed RebarBoltom of SlabA = 0.04 m²SlightOnec49712Deck Slab/Diaphragm @ Rav.3SERHExposed RebarBoltom of SlabA = 0.39 m²HeavyOnea50722PSC GirderSERSSpellAbove Pier 2A = 0.06 m²Slightonec53732PSC GirderSERSSpellTop FlangeA = 0.08 m²SlightSeveralc54				SER		Exposed Rebar		A = 0.03 m ²	Slight	One	с	45
Op2Bav 2SERFISpailDiaph. $A = 0.45 \text{ m}^4$ HeavyOnea48702Pier 2 CopingSERSExposed RebarBack Face under G-3 $A = 0.04 \text{ m}^2$ StightOnec49712Deck Slab/Diaphragm @ Bav.3SERHExposed RebarBottom of Stab $A = 0.39 \text{ m}^2$ HeavyOnea50722PSC GirderSERSSpailAbove Pier 2 $A = 0.06 \text{ m}^2$ Stightonec53732PSC Girder G-5A G-6SERSSpailTop Flange $A = 0.08 \text{ m}^2$ StightSeveralc54	68			нс	м		Diaphoram	A = 0.12 m ²	Moderate	One	b	46
70 2 Pier 2 Coping SER S Exposed Rebar Back Face under G-3 $A = 0.04 \text{ m}^2$ Stight One c 49 71 2 Deck Slab/Diaphragm @ Bax 3 SER H Exposed Rebar Boltom of Slab $A = 0.04 \text{ m}^2$ Stight One c 49 72 2 PSC Girder SER S Spall Above Pier 2 $A = 0.04 \text{ m}^2$ Stight one c 53 73 2 PSC Girder SER S Spall Top Flange $A = 0.08 \text{ m}^2$ Stight Several c 54	69	2		SER	н	Spall		A = 0.45 m ²	Heavy	One	a	48
r_1 z $R_{av,3}$ SER n Expose Repr Bolicon of State $A = 0.38 \text{ m}^{-1}$ Heavy One a 50 72 2 PSC Girder SER S Spall Above Pier 2 $A = 0.08 \text{ m}^2$ Stight one C 53 73 2 PSC Girder SER S Spall Top Flange $A = 0.08 \text{ m}^2$ Stight Several C 54	70	2	Pier 2 Coping	SER	s	Exposed Rebar		A = 0.04 m ²	Slight	One	c	49
72 2 PSC Girder SER S Spall Above Pier 2 A = 0.06 m ² Slight one C 53 73 2 PSC Girder G-5A G-6 SER S Spall Top Flange A = 0.08 m ² Slight Several C 54	71	2		SER	н	Exposed Rebar	Bottom of Slab	A = 0.39 m ²	Heavy	One		
73 2 PSC Girder G-5.8 G-6 SER S Spall Top Flange A = 0.08 m ² Slight Several C 54		2									·	
	72				-	· ·	··· · -				~	~~
74 2 End Diaphragm @ Bay 7 CR H Crack Top t=0.61 mm Heavy One a 55				SER	S	Spall	Top Flange	A = 0.09 m ²	Slight	Several		64

Appendix 7.1.1-1 (22/34)

Damage No.	Span No.	Name of Member	Type of Damage	Rank of Damage	Nature	De: Location/ Pattern	scription of Damag	severity	No. of Damages	Remarks (a, b, c)	Photo No.
75	2	Deck Slab @ Bay 7	CR	м	Random Cracks	Bottom	t = 0,50 mm	Moderate	Several	b	56
76	2	Deck Slab @ Bay 8	SER	м	Spall	Bottom	A = 0.30 m ²	Moderate	Several	b	57
77	2	Deck Slab @ Bay 9	CR	н	Random Cracks	Bottom	t = 1.753 mm	Heavy	Several	а	58
78	2	End Diaphragm @ Bay 9	SER	М	Spall	Top @ Joint	A = 0.12 m ²	Moderate	One	b	59
79	2	Cantilever Slab	SER	н	Spall	Bottom	A = 0.36 m ²	Heavy	One	a	60
80	2	Pier 3 Coping	CR	н	Diagonal Crack	Boltom Face	t= 0.61 mm	Heavy	One	a	61
81	2	Cantilever Slab U/S	SER	S	Exposed Rebar	Bottom	$A = 0.023 \text{ m}^2$	Slight	One	c c	183
82	2	Deck Slab @ Bay 2	CR	s	Random Cracks	Boltom near Mid-Spar	l = 0.15 mm	Slight	Several	c	184
83	2	PSC Girder G-5	SER	s	Exposed Rebar	Top Flange @ Bay 4	A = 0.10 m ²	Slight	Several	c	185
84	2	PSC Girder G-4	SER	s	Exposed Rebar	Top Flange @ Bay 4	A = 0.06 m ²	Slight	One		
85	2	PSC Girder G-6	SER	s		Top Flange L/4				C C	186
86	2	PSC Girder G-6			Exposed Rebar	near P1 Top Flange L/4	A = 0.03 m ²	Slight	Several	c	187
	2		SER	S	Exposed Rebar	near P2	A = 0.03 m ²	Slight	Several	c	188
		Deck Slab @ Bay 6	HC	S	Honeycomb	Bollom near Mid-Spar	A = 0.08 m ²	Slight	One	C .	189
88	2	2nd Interm. Diaph. @ Bay 7	SER	S	Spall	Bottom	A = 0.075 m ²	Slight	One	c	190
89	2	Asphalt Pavement	CRPL	S	Longitudinal Crack	Near P2	t = 1.5 mm	Slight	One	с	209
90	3	Cantilever Slab	SER	S	Exposed Rebar	Bollom near Girder G-1	A = 0.10 m ²	Slight	One	с	41
91	3	Cantilever Slab	CR	м	Transverse Crack	Bottom	t = 0.458 mm	Moderate	Four	b	42
92	3	Cantilever Slab	SER	S	Exposed Rebar	Bottom Edge	A = 0.06 m ²	Slight	One	с	43
93	3	Steel Bearing	со	М	Corrosion	G-1 to G-10	Reduction of Cross Section	Moderate	All	b	44
94	3	End Diagphram @ Bay 5	SER	s	Exposed Rebar	Bottom	A = 0.025 m ²	Slight	Five	c	51
95	3	Deck Slab @ Bay 5	CR	s	Random Cracks	Bottom	t = 0.15 mm	Slight	Several	c	52
96	3	End Diaph. G-1	CR	M	Random Cracks	End of Diaph. U/S	t = 0.35 mm	Moderate	Several	b	137
97	3	PSC Girder G-1	CR	н	Vertical Crack	End Block	t = 0.50 mm	Heavy	One	a	138
98	3	Cantilever Slab U/S	SER	S	Spall	Near Support	A = 0.045 m ²	Slight	One	c	139
99	3	Cantilever Slab U/S	CR	M	Random Cracks	@ Exp. Joint Bottom	l = 0.35 mm	Moderate			
100	3	Cantilever Slab U/S	SER	S	t				Several	Ь	140
101	3	Steel Bearing			Exposed Rebar	Bottom	A = 0.10 m ² Reduction of Cross	Slight	Several	C	141
		@ Pier 3	co	M	Corrosion	G-1 to G-10	Section	Moderate	Ali	b	142
102	3	PSC Girder G-1	CR	M	Random Cracks	End Block	t = 0.30 mm	Moderate	Several	b	143
103	3	Deck Slab @ Bay 1	SER	M	Exposed Rebar	Bollom	A = 0.17 m ²	Moderate	One	b	144
104	3	Deck Slab @ Bay 2	SER	M	Exposed Rebar	Bottom	A = 0.16 m ²	Moderate	Two	b	147
105	3	Deck Slab @ Bay 3	SER	М	Exposed Rebar	Bollom	A = 0.17 m ²	Moderate	One	b	148
106	3	End Diaph. @ Bay 3	SER	М	Exposed Rebar	Diaph. Haunch	$A = 0.17 m^2$	Moderate	One	b	149
107	3	PSC Girder G-5	SER	S	Spall	Top Flange @ Bay 4	$A = 0.06 m^2$	Slight -	One	c	150
108	3	PSC Girder G-5	SER	S	Spall	Bottom Flange @ Bay 5	A = 0.03 m ²	Slight	One	с	151
109	3	Deck Slab @ Bay 5	CR	M	Transverse Crack	Bottom	t = 0.40 mm	Moderate	Two	b	152
110	3	Deck Slab @ Bay 7	SER	м	Exposed Rebar	Bottom	A = 0.261 m ²	Moderate	Two	b	155
111	3	Pier 3 Coping	SER	s	Spall	Back Face @ Bay 7	A = 0.03 m ²	Slight	One	c	157
112	3	Deck Slab @ Bay 8	CR	s	Random Cracks	Bottom	t = 0.15 mm	Slight	Several	c	159
113	3	Deck Slab @ Bay 8	SER	M	Exposed Rebar	Bottom	A = 0.261 m ²	Moderate	One	b	160
114	3	Deck Slab @ Bay 9	SER	M	Exposed Rebar	Bottom	A = 0.261 m ²	Moderate			
115	3				Transverse Crack			· · · ·	One	<u>ь</u>	162
		Deck Slab @ Bay 9	CR	M	·· · · ·	Bottom	t = 0,40 mm	Moderate	One	b	163
116	3	End Diaph. @ Pier 3	CR	H	Random Cracks	Bottom @ Bay 3	t = 1.38 mm	Heavy	Several	a	164
117	3	End Diaph. @ Pier 3	SER	S	Exposed Rebar	Al end D/S	A = 0.05 m ²	Slight	One	c	165
118	3	Cantilever Slab Steel Bearing	SER	S	Exposed Rebar	Bottom D/S	A = 0.02 m ²	Slight	Several	c	166
119	3	Pier 3	со	м	Corrosion	G1 to G10	Reduction of Cross Section	Moderate	All	b	167
120	3	Pier 3 Copping	SER	S	Exposed Rebar	Back Face near end D/S	A = 0.02 m ²	Slight	Several	с	168
121	3	Pier 3 Coping	CR	м	Vertical Crack	Back Face	t = 0.35 mm	Moderate	One	b	169
122	3	Railing	DEF	S	Deflected Outwards	14 m. from P2	Deflection of Raling	Slight	One	с	195
123	3	Railing	DEF	S	Deformed	8 m from P2	Deflection of Raling	Slight	One	с	210
124	3	Asphalt Pavement	CRPL	s	Intermetent Crack	Typical @ all Spans	l = 2 mm	Slight	Several	с	211
125	3	Railing	B/R	м	Top Rail Broken	12 m from P3	Damage	Moderate	One	b	212
126	4	Cantilever Slab	CR	Н	Transverse Crack	Bottom D/S	t = 0.61 mm	Heavy	One	а	103
127	4	Cantilever Slab	SER	S	Spall	Bottom	A = 0.05 m ²	Slight	One	c	104
128	4	Pier 4 Coping	CR	s	Vertical Crack	Back Face	t = 0.30 mm	Slight	One	c	104
129	4	Cantilever Slab U/S	нс	S	Honeycomb	Bottom	A = 0.09 m ²				
130	4	Deck Slab @ Bay 1						Slight	One	C	112
			CR	<u>M</u> .	Transverse Crack	Bottom	t = 0.60 mm	Moderate	One	b	113
131	4	Deck Slab @ Bay 1	SER	M	Exposed Rebar	Bottom	A = 0.15 m ²	Moderate	One	b	114
132	4	Deck Slab @ Bay 2	SER	M	Exposed Rebar	Bottom	A = 0.15 m ²	Moderate	One	b	115
133	4	End Diaph. @ Bay 3	CR	м	Crack	Back Face	t = 0.60 mm	Moderate	One	b	116
134	4	PSC Girder G-5	SER	S	Spall	Top Flange @ Bay 4	A = 0.08 m ²	Slight	One	с	118
135	4	Steel Bearing @ Pier 4	со	м	Corrosion	G-1 to G-10	Reduction of Cross Section	Moderate	All	b	119
136	4	PSC Girder G-5	SER	S	Spall	Top Flange @ Bay 5	A = 0.075 m ²	Slight	One	c	120
137	4	Deck Slab @ Bay 5	SER	S	Spall	Bottom	A = 0.08 m ²	Slight	One	с	121

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Appendix 7.1.1-1 (23/34)

Damage No.	Span No.	Name of Member	Type of Damage	Rank of Damage	Nature	De: Location/ Pattern	scription of Damag Scale	e Severity	No. of Damages	Remarks (a, b, c)	Photo No.
138	4	End Diaph. @ Bay 5	SER	S	Spall	Bottom	A = 0.04 m ²	Slight	One	с	122
139	4	PSC Girder G-6 & G-7	SER	S	Exposed Rebar	Top Flange @ Bay 6	A = 0.045 m ²	Slight	One	с	124
140	4	PSC Girder G-7	SER	s	Spall	Top Flange @ Bay 7	A = 0.03 m ²	Slight	Оле	с	125
. 141	4	Deck Slab @ Bay 7	SER	м	Exposed Rebar	Bottom	A = 0.261 m ²	Moderate	One	b	126
142	4	Deck Slab @ Bay 7	CR	s	Transverse Crack	Bottom	t = 0.25 mm	Slight	Two	с	127
143	4	Deck Slab @ Bay 6	SER	s	Exposed Rebar	Bottom	A = 0.10 m ²	Slight	One	c	128
144	4	End Diaph. @ Bay.9	SER	s	Spall	Тор	A = 0.06 m ²	Slight	One	c	129
145	4	Cantilever Slab D/S	SER	н	Exposed Rebar	Expansion Joint @ P4		Heavy	One	a	130
146	4	Steel Railing	B/R	н	Broken	Top Rail	Remarkable Damage	Heavy	One	a	131/ 1
147	4	Cantilever Slab D/S	CR	s	Transverse Crack	Bottom	t = 0.25 mm	Slight	One	c	133
148	4	Cantilever Slab D/S	SER	s	Spall	Bottom	A = 0.025 m ²	Slight	One	c c	134
149	4	End Diaph.	CR	M	Horizontal Crack	at End D/S	t = 0.45 mm	Moderate	Two		
150	4	PSC Girder G-1	CR	S						b	135
151	4			· · · · · · · · · · · · · · · · · · ·	Vertical Crack	End Block	1 = 0.20 mm	Slight	One	c	136
		Deck Slab @ Bay 1	нс	н	Honeycomb	Bottom near P3	A = 16.01 m ²	Heavy	One	а	145
152	4	End Diaph. @ Pier 3	HC.	<u> </u>	Honeycomb	Тор	A = 0.12 m ²	Moderate	One	b	146
153	4	Deck Slab @ Bay 6	D/D	M	Deterioration	Bottom	A ≈ 1.05 m ²	Moderate	One ·	b	154
154	4	PSC Girder G-7	SER	S	Spall	Top Flange @ Bay 7	A = 0.005 m ²	Slight	One	c	158
155	4	End Diaph. @ Pier 3	CR	S	Cracks	Bottom	t = 0.2 mm	Moderate	Several	с	158
156	4	End Diaph. @ Pier 3	SER	S	Exposed Rebar	Bottom @ Bay 8	A = 0.013 m ²	Slight	Several	c	161
157	4	Sidewalk	SER	S	Spall	Above P3	A = 0.09 m ²	Slight	One	c	196
158	4	Expansion Joint	D/M	s	Missing Plate Section	Above P3	A = 0.04 m ²	Slight	One	с	213
159	3/4	Deck Slab @ Bay 5	SER	M	Exposed Rebar	Bottom	A = 0.174 m ²	Moderate	One	b	153
160	4	Sidewalk	SER	S	Spall	Typical @ all Spans	A = 0.02 m ²	Slight	Малу	с	197
161	5	PSC Girder G-1	CR	м	Random Cracks	Fece of End Black	t = 0.229 mm	Moderate	Several	b	67
162	5	Cantilever Slab	SER	s	Spall	Bottom	A = 0.08 m ²	Slight	One		
163	5	Cantilever Slab	CR	M	Random Cracks		· · · · · · · · · · · · · · · · · · ·			c	68
						Bollom	t = 0.35 mm	Moderate	Several	b	69
164	5	Pier 5 Coping	CR	M	Random Cracks	Back Face Pear G-1	t = 0.381 mm	Moderate	Several	b	71
165	5	Deck Slab @ Bay 2	CR	М	Long. Crack	Boltom	l = 0.356 mm	Moderate	One	b	72
166	5	Deck Slab @ Bay 3	SER	S	Exposed Rebar	Bottom	A = 0.08 m ²	Slight	One	C	73
167	5	End Diaph. @ Bay 4	SER	S	Spall	Тор	A = 0.10 m ²	Slight	One	с	75
168	5	PSC Girder G-4	SER	S	Exposed Rebar	Top Frange @ Bay 4	A = 0.06 m ²	Slight	One	c	76
169	5	PSC Girder G-7	SER	S	Exposed Rebar	Top Flange @ Bay 6	A = 0.075 m ²	Slight	One	с	79
170	5	Shear Block @ Bay 6	нC	S	Honeycomb	End	A = 0.09 m ²	Slight	Óne	с	80
171	5	Shear Block @ Bay 7	SER	s	Spall	. End	A = 0.09 m ²	Slight	One	с	81
172	5	Deck Slab @ Bay 9	SER	M	Exposed Rebar	Bottom	A = 0.15 m ²	Moderate	One	b	82
173	5	Pier 5 Coping	CR	н	Vertical Crack	Back Face @ end D/S	1 = 0.783 mm	Heavy	One	a	83
174	5	Pier 5 Coping	CR	s	Horizontal Crack	Back Face D/S	l = 0.152 mm	Slight	One	c	85
175	5	Cantilever Slab	SER	s	Exposed Rebar	Bottom D/S	A = 0.06 m ²	Slight	One	c	86
176	5	End. Diaph.	CR	н	Random Cracks	End of Diaph, D/S	t = 1.00 mm				
177	5	PSC Girder G-1	CR					Heavy	Several	a	106
				н	Random Cracks	Bollom	t = 0.50 mm	Heavy	Several	a	107
178	5	PSC Girder G-1	CR	S	Vertical Crack	Bottom	t = 0.20 mm	Slight	One	c	108
179	5	Cantilever Slab	SER	M .	Exposed Rebar	Bottom	A = 0.15 m ²	Moderate	One	b	109
180	5	Cantilever Slab U/S	CR	M	Transverse Crack	Bottom	t = 0.40 mm	Moderate	Several	b	110
181	5	Cantilever Slab U/S	SER	S	Exposed Rebar	Bottom	A = 0.06 m ²	Slight	One	c	111
182	5	PSC Girder G-4	SER	S	Spall	Top Flange @ Bay 3	A = 0.06 m ²	Slight	One	с	117
183	5	PSC Girder G-5 & G-6	SER	s	Spall	Top Flange @ Bay 5	A = 0.10 m ²	Slight	Cne	с	123
184	5	Sidewalk	SER	S	Spall	Above P4	A = 0.04 m ²	Slight	One	с	214
185	5	Railing	B/R	н	Top Rail Broken	6 m. from P4	Remarable Damage	Heavy	One	a	215
186	6	PSC Girder G-2	CR	н	Random Cracks	End Block	t ≃ 0.813 mm	Heavy	Several	а	62
187	6	Cantilever Slab	CR	н	Random Cracks	Bottom Upstream	t = 0.766 mm	Heavy	Several	a	63
188	6	Cantilever Slab	SER	M	Exposed Rebar	Bottom	A = 0.12 m ²	Moderate	One	b	64
189	6	Steal Bearing	со	M	Corrosion	G-1 to G-10	Reduction of Cross	Moderate	All	b	65
190	6	© Pier 5 Pier 5 Coping	CR	S	Horizontal Crack	Back Face	Section t = 0.102 mm				
191	6							Slight	One	c	66
		End Diaph. @ Bay 4	SER	s	Exposed Rebar	Face Near Support	A = 0.08 m ²	Slight	One	C .	74
192	6	PSC Girder G-5	SER	S	Exposed Rebar	Top Flange @ Bay 5	A = 0.038 m ²	Slight	One	c	77
193	6	Cantilever Slab	SER	s	Exposed Rebar	Bottom D/S	A = 0.06 m ²	Slight	Оле	с	87
194	6	Cantilever Slab	SER	S	Exposed Rebar	Bottom	A = 0.01 m ²	Slight	One	с	88
195	6	Deck Slab @ Bay 3	SER	м	Exposed Rebar	Bollom near Mid-Span	A = 0.24 m ²	Moderate	One	b	89
196	6	Deck Slab @ Bay 3	SER	м	Exposed Rebar	Bottom near P5	A = 0.24 m ²	Moderate	One	b	90
197	6	Interm Diaph. @ Bay 3	SER	м	Spall	Bollom	A = 0.25 m ²	Moderate	One	b	91
198	6	Deck Slab @ Bay 3	SER	м	Spall	Bottom near Abut. B	A = 0.16 m ²	Moderate	One	b	92
199	6	PSC Girder G-4	SER	s		Top Flange @ Bay 4	A = 0.05 m ²	Slight	One	c	92
		PSC Girder G-4		- й		Top Flange @ Bay 4	t = 1.50 mm	Heavy	One	<u> </u>	50

Appendix 7.1.1-1 (24/34)

Damage	Span	•	Type of	Rank of		Des	cription of Damage)		Remarks	Photo
No.	No.	Name of Member	Damage	Damage	Nature	Location/ Pattern	Scale	Severity	No. of Damages	(a, b, c)	No.
201	6	PSC Girder G-6	SER	S	Exposed Rebar	Top Flange @ Bay 5	A = 0.08 m ²	Slight	One	с	94
202	6	Interm Diaph. Near P5	нс	S	Honey Comb.	Diaph, Haunch	A = 0.09 m ²	Slight	One	с	95
203	6	PSC Girder G-5	SER	s	Exposed Rebar	Top Flange @ Bay 5	A = 0.08 m ²	Slight	One	c	96
204	6	PSC Girder G-5	CR	н	Long, Crack	Top Flange @ Bay 5	l = 1.50 mm	Heavy	One	а	96
205	6	PSC Girder G-5	SER	S	Spall	Top Flange @ Bay 5	A = 0.06 m ²	Slight	One	с	97
206	6	PSC Girder C-6	SER	S	Exposed Rebar	Top Flange @ Bay 6	A = 0,10 m ²	Slight	One	с	98
207	6	Interm. Diaph. @ Bay 6	нс	s	Honey Comb.	Diaph, Haunch	A = 0.02 m ²	Slight	One	с	99
208	6	Interm. Diaph. @ Bay 7	SER	S	Exposed Rebar	Bottom near P5	A ≈ 0.01 m ²	Slight	One	с	100
209	6	Deck Slab @ Bay 7	SER	S	Exposed Rebar	Bottom	A = 0.03 m ²	Slight	One	с	101
210	6	Interm. Diaph. @ Bay 7	SER	s	Exposed Rebar	Bottom near G-8	A = 0.04 m ²	Slight	One	c	102
211	6	Railing	SER	S	Spall	Above Abut. "B"	A = 0.045 m ²	Slight	One	с	216
212	6	Approach Slab	CRPL	s	Random Cracks	Asphalt Pavement	t = 3 mm	Slight	Several	с	217
213	. 6	Approach Slab	CRPL	м	Potholes	4 m. from Abut. "B"	a = 30 mm	Moderate	One	b	218
214	6	Sidewalk	MISD	м	Settlement	Near Abut. "B"	d = 100 mm	Moderate	One	b	219

Name of Bridge : Ma3 MARCOS BRIDGE

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Date of Inspection December 14-20, 2002 Inspector R.Millo / R. Buencamino

Name of Br	idge : M	a3 MARCOS BRIDGE							rJ.B. Agnes		
Damage	Span		Type of	Rank of	1	Des	cription of Damag	e		Demort-	Dhat
No.	No.	Name of Member	Damage	Damage	Nature	Location/ Pattern	Scale	Severity	No. of Damages	Remarks (a, b, c)	Photo No.
1	1	Coping	CR	н	Random Cracks	Pier 1 Coping @ Upstream	t = 1.500 mm	Heavy	Many	a	195
2	1	Coping	CR	м	Horizontal Cracks and Map Cracks	Pier 1 End of Coping @ Upstream	1 = 0.500 mm	Moderate	Many	b	196
3	1.	Coping	CR	н	Vertical and Horizontal Cracks	Pier 1 Face of Coping @ Bay 1	t = 0.610 mm	Heavy	Many	а	197
• 4	1	Coping	CR	s	Vertical and Horizontal Cracks	Pier 1 Face of Coping @ Bay 2	t = 3.000 mm	Slight	Many	с	198
5	1 .	Pier 1, Girder 3	SER	s	Spall	Top Flange of Girder @ Bay 2	A = 0.030 m ²	Slight	One	с	199
6	1	Coping	CR	S	Vertical Cracks	Pier 1 Face of Coping @ Bay 3	t = 0.200 mm	Slight	Two	с	200
7	1	Coping	CR	м	Random Cracks	Pier 1 Face of Coping @ Bay 4	t = 0.550 mm	Moderate	Many	ъ	201
8	1	Pier 1, Girder 5	SER	S	Spall	Top Flange of Girder @ Bay 5	A = 0.020 m ²	Slight	One	с	202
9	1	Coping	CR	м	Random Cracks	Pier 1 Face of Coping @ Bay 6	t = 0.356 mm	Moderate	Many	b	203
10	1	End Diaphragm	НС	s	Honeycomb	Diaphregm Haunch @ Bay 6	A = 0.035 m ²	Slight	One	c	204
11	1	Coping	CR	м	Random Cracks	Pier 1 Face of Coping @ Bay 7	t = 0.350 mm	Moderate	Three	b	205
12	1	Pier 1, Girder 8	нс	м	Honeycomb	Bottom Flange of Girder @ Bay 7	A = 0.150 m ²	Moderate	One	b	206
13	1	Coping	CR	s	Vertical Crack	Pier 1 Face of Coping	l = 0.300 mm	Slight	Three	с	207
14	1	Coping	CR	M	Vertical Crack	@ Bay 8 Pier 1 Face of Coping	t = 0.400 mm	Moderate	One	b	208
15	1	Coping	CR	M	Vertical Crack	@ Bay 9 Pier 1 Face of Coping		Moderate	Two	b	209
16	1	Deck Slab	нс	M ·	Honeycomb	@ Bay 10 Bottom Face of Slab	A = 0.210 m ²	Moderate	One	b	210
17	1	Coping	CR	M	Random Cracks	@ Bav 10 Pier 1 @ Bay 11	t = 0.400 mm	Moderate	Many	b	211
18	1	Deck Slab	нс	s	Honeycomb	Boltom Face of Slab	A = 0.010 m ²	Slight	One	c	212
19	1	Coping	CR	н	Cracks	@ Bay 11 Near end Coping	t = 10.000 mm	Heavy	Three	a	212
20	1	Railing Post	SER	s	Exposed Rebar	Downstream Face of Rail Post	A = 0.015 m ²	Slight	One		335
21	1	Asphalt Pavement	CRPL	M	Crack	Deck Slab Wearing	t = 7,000 mm	Moderate	One	C b	
22	1	Side Walk	CR	S	Random Cracks	Course Inner Lane Top of Side Walk	t = 0.254 mm		One	b	336
23	1	Asphalt Pavement	CRPL	S	Random Cracks	Deck Slab		Slight		c	361
24	1	Railing Post	SER	s	Exposed Rebar	Wearing Course 10.0 m. from	l = 4.000 mm	Slight	One	c	362
25	2	·	CR		Horizontal Cracks	Abutment "B" Pier 2 Coping	A = 0.030 m ²	Slight	One	c	363
25	2	Coping		- H	and Map Cracks	@ Upstream Pier 2 Coping	t = 2.500 mm	Heavy	Many	а	151
20		Coping	CR	H	Random Cracks	@ Upstream Bottom Flange of	t = 3.000 mm	Heavy	Many	a	152
	2	Pier 2, Girder 2	HC	M	Honeycomb	Girder @ Bay 1 Pier 2 Coping Under	A = 0.120 m ²	Moderate	One	b	153
28		Coping	CR	M	Horizontal Cracks	Girder 2 Bottom Flange of	t = 0.381 mm	Moderate	Many	b	154
29	2	Pier 2, Girder 2	HC	M	Honeycomb	Girder @ Bay 2 Pier 2 Bottom of Slab	A = 0.130 m ²	Moderate	One	b	155
30	2	Deck Slab	CR	S	Random Cracks	@ Bay 3 Pier 2 Bottom of Slab	l = 0.102 mm	Slight	Many	С	156
31	2	Deck Slab	CR	S	Random Cracks	@ Bay 4 Top Flange of Girder	t = 0.102 mm	Slight	Many	c	157
32	2	Pier 2, Girder 5	SER	S	Spall	@ Bay 4 Pier 2 Bottom of Slab	A = 0.035 m ²	Slight	One	c	158
33	2	Deck Slab	нс	м	Honeycomb	@ Bav 5	A = 0.200 m ²	Moderate	One	b	159
34	2	End Diaphragm	SER	S	Spall	Pier 2 Diaphragm Haunch @ Bay 5	A = 0.020 m ²	Slight	One	с	160
35	2	Pier 2, Girder 7	НС	м	Honeycomb	Bottom Flange of Girder @ Bay 6	A = 0.120 m ²	Moderate	One	b	161
36	2	Pier 2, Girder 7	SER	M .	Exposed Rebar	Top Flange of Girder @ Bay 7	A = 0.150 m ²	Moderate	One	b	162
37	2	Deck Slab	CR	S	Random Cracks	Pier 2 Bollom of Slab @ Bav 7	t = 0.076 mm	Slight	Many	C	163
38	2	Deck Slab	CR	S	Random Cracks	Pier 2 Bottom of Stab @ Bay 8	1 = 0.102 mm	Slight	Many	с	164
39	2	Deck Slab	CR	S	Random Cracks	Pier 2 Bottom of Slab @ Bay 9	l ≈ 0.102 mm	Slight	Малу	c	165
40	2	Pier 2, Girder 11	нс	м	Honeycomb	Bottom of Flange of Girder @ Bay 10	A = 0.130 m ²	Moderate	One	b	166
41	2	End Diaphragm	нс	s	Honeycomb	Pier 2 Face of Disphragm @ Bay 10	A = 0.020 m ²	Slight	One	с	167
42	2	Coping	CR	н	Random Cracks	Pier 2 End of Coping @ Downstream	l = 5.000 mm	Heavy	Many	а	168
43	2	Coping	CR	н	Random Cracks	Pier 1 End of Coping @ Upstream	t = 3.000 mm	Heavy	Many	а	169
44	2	Pier 1, Girder 1	SER	S	Spall	Bottom of Girder	A = 0.020 m ²	Slight	One	с	170

Appendix 7.1.1-1 (25/34)

Damage No.	Span No.	Name of Member	Type of	Rank of	i	De: Location/	scription of Damag		No. of	Remarks	Photo
			Damage	Damage	Nature	Pattern Pier 1 Bottom of Stat	Scale	Severity	Damages	(a, b, c)	No.
45	2	Deck Slab	CR	S	Random Cracks	@ Bay 1 Rier 1 Coping	1 = 0.076 mm	Slight	Малу	c	171
46	2	Coping	CR	н	Horizontal Cracks	@ Bay 1 Pier 1 Coping	l = 1.000 mm	. Heavy	One	a	172
47	2	Coping	CR	м	Random Cracks	@ Bav 2 Pier 1 Coping	t = 0.500 mm	Moderate	Many	b	173
48	2	Coping	CR	н	Random Cracks	@ Bav 3	1 = 1.000 mm	Heavy	Many	a	174
49	2	Deck Slab	CR	S	Random Map Cracks	Bottom of Stab @ Bay 3	t = 0.076 mm	Slight	Many	c	175
50	2	Pier 1, Girder 3	HC	S	Honeycomb	Boltom Face of Girde @ Bay 3	A = 0.070 m ²	Slight	One	c	176
51	2	Coping	CR	н	Random Cracks	Pier 1 Coping @ Bay 4	t ≃ 0.610 mm	Heavy	Мелу	а	177
52	2	Coping	CR	н	Random Cracks	Pier 1 Coping @ Bay 5	t = 1.000 mm	Heavy	Many	а	178
53	2	Deck Slab	CR	S	Random Cracks	Pier 1 Bollom of Slab @ Bay 5	t= 0.175 mm	Slight	Many	c	179
54	2	Pier 1, Girder 5	нс	S	Honeycomb	Face of Girder @ Bay 5	A = 0,010 m	Slight	One	c	180
55	2	Coping	CR	н	Random Cracks	Pier 1 Face of Coping (@ Bay 6	1 = 0.610 mm	Heavy	Many	а	181
56	2	Coping	CR	м	Random Cracks	Pier 1 Face of Coping @ Bay 7	t = 0.500 mm	Moderate	Many	b	182
57	2	Pier 1, Girder 8	HC	м	Honeycomb	Bottom Flange of Girder @ Bay 7	$A = 0.160 \text{ m}^2$	Moderate	One	b	183
58	2	Coping	CR	н	Vertical Cracks	Pier 1 Face of Coping @ Bay 8	t = 0.610 mm	Heavy	Three	а	184
. 59	2	Pier 1, Girder 8	HC	м	Honeycomb	Face of Girder @ Bay 8	A = 0.160 m ²	Moderate	One	b	185
60	2	Coping	CR	м	Vertical and Horizontal Cracks	Pier 1 Face of Coping @ Bay 9	t = 0.500 mm	Moderate	Three	b	186
61	2	Deck Slab	SER	S	Exposed Rebar	Pier 1 Bottom of Slab @ Bay 9	A = 0.02 m ²	Slight	One	с	187
62	2	Coping	CR	s	Random Cracks	Pier 1 Face of Coping	t = 0.229 mm	Slight	Many	с	188
63	2	Pier 1, Girder 10	НС	M	Honeycomb	@ Bay 10 Bottom Flange of	A = 0.12 m ²	Moderate	One	b	189
64	2	Coping	CR	s	Horizontal Cracks	Girder @ Bay 10 Pier 1 Face of Coping		Slight	One	c	103
65	2	Deck Slab	нс	s	Honeycomb	@ Bav 11 Pier 1 Bottom of Slab	A = 0.06 m ²	Slight	One	c c	190
66	2	Coping	CR	н	Random Cracks	@ Bay 11 Pier 1 End of Coping	t = 7.000 mm	Heavy	Many		191
67	2	Cantilever Slab	SER	s	Exposed Rebar	@ Downstream Pier 1 Bottom of Slab			· · · · · · · · · · · · · · · · · · ·	а	
68	2	Coping	CR		Random Cracks	@ Downstream Pier 1 Near End of	A = 0.06 m ²	Slight	One	С	193
69	2					Coping @ DS Deck Slab Wearing	t = 6.500 mm	Heavy	Many	a	194
70	2	Asphalt Pavement	CR	s	Crack	Course Inner Lane Deck Slab Wearing	t = 4.00 mm	Slight	One	с	334
		Asphalt Pavement	CRPL	S	Crack	Course Inner Lane Deck Slab	t = 3.00 mm	Slight	One	<u>с</u>	346
71	2	Asphalt Pavement	CRPL	S	Corrugation	Wearing Course	t = 8.000 mm	Slight	One	с	347
72	2	Curb	SER	S	Spall	Top of Curb	A = 0.020 m ²	Slight	One	C.	358
73	2	Side Walk	CR	S	Crack	Top of Side Walk	l = 0.254 mm	Slight	One	С	359
74	2	Railing	CR	М	Crack	Face of Railing	t = 0.500 mm	Moderate	One	b	360
75	3	Coping	CR	н	Horizontal Cracks	Pier 3 End of Coping @ Upstream	t = 3.500 mm	Heavy	Two	а	124
76	3	Coping	CR	H	Random Cracks	Pier 3 Coping Face Upstream	t = 4.000 mm	Heavy	Many	а	125
77	3	Cantilever Slab	SER	s	Exposed Rebar	Pier 3 Bottom of Slab @ Upstream	A = 0.060 m ²	Slight	One	с	126
78	3	Coping	CR	Н	Horizontal & Vertical Cracks	Pier 3 Under Girder 1	t = 0.864 mm	Heavy	Two	a	
79	3	Coping	CR	M	Horizontal Cracks	Pier 3 Coping Face Bay 1 to Bay 3	t = 0.584 mm	Moderate	Two	b '	127
80	3	Pier 3 Girder 2	SER	S	Exposed Rebar	Pier 3 Top Flange of Girder @ Bay 1	A = 0.050 m ²	Slight	One	с	128
81	3	Deck Slab	SER	м	Spall	Pier 3 Bottom of Slab @ Bay 3	A = 0.180 m ²	Moderate	One	ъ	129
82	3	Deck Slab and Girder 5	SER	М	Spall	Pier 3 Bottom of Slab @ Bay 4	A = 0.120 m ²	Moderate	One	b	130
83	3	Deck Slab	SER	S	Spall	Pier 3 Bottom of Slab @ Bay 8	A = 0.060 m ²	Slight	One	с	131
84	- 3	Coping	CR	M	Random Cracks	Pier 3 Coping Face	t = 0.305 mm	Moderate	Many	b	132
85	3	Deck Slab	SER	м	Spall	Bay 9 to Bay 11 Pier 3 Bottom of Slab	A = 0.300 m ²	Moderate	Оле	b	133
86	3	Coping	CR	н	Random Cracks	@ Bav 11 Pier 3 Coping Face	t = 5.000 mm	Heavy	Many		134
87	3	Cantilever Slab	SER	s	Exposed Rebar	Downstream Pier 3 Bottom of Slab	A = 0.020 m ²	Slight	Two	a	134
88	3	Coping	CR		Random Cracks	@ Downstream Pier 2 End Coping	t = 0.700 mm			c	
89	3		CR			@ Upstream Pier 2 Coping Face		Heavy	Many	a	136
		Coping		S	Random Cracks	@ Bay 2 Pier 2 Bottom of Slab	l = 0.300 mm	Slight	Many	c	137
90	3	Deck Slab	CR	S	Random Cracks	@ Bay 3 Top Flange of Girder	t = 0.173 mm	Slight	Many	c	138
91	3	Pier 2, Girder 4	SER	s	Spall	@ Bay 4 Top Flange of Girder	A = 0.025 m ²	Slight	One	c	139
92	3	Pier 2, Girder 5	SER	S	Spali	@ Bay 4	A = 0.025 m ²	Slight	One	C .	139
93	3	Pier 2, Girder 5	SER	S	Exposed Rebar	Top Flange of Girder @ Bay 5	A = 0.100 m ²	Slight	One	c	140
94	3	Pier 2, Girder 6	SER	м	Spall	Top Flange of Girder @ Bay 5	A = 0.300 m ²	Moderate	One	b	141
95	3	Pier 2, Girder 7	SER	м	Spall	Top Flange of Girder @ Bay 7	A = 0.240 m ²	Moderate	Оле	b	142
96	3	Deck Slab	CR	s	Random Cracks	Pier 2 Bollom of Slab @ Bay 8	t = 0.102 mm	Slight	Many	с	144
97	3	Deck Slab	CR	S	Random Cracks	Pier 2 Bottom of Slab @ Bay 9	t = 0.102 mm	Slight	Many	с	145
98	3	Coping	CR	н	Horizontal Cracks and Map Cracks	Pier 2 Coping Under Girder 11 @ Bay 11	ι= 1.011 mm	Heavy	Many	а	146
99	3	Pier 2, Girder 11	HC	м	Honeycomb	Boltom Flange of Girder @ Bay 11	A = 0.120 m ²	Moderate	One	b	147
	3	Coping	CR	н	HorizonIal Cracks and Map Cracks	Pier 2 Coping @ Downstream	t = 3.000 mm	Heavy	Many	a	148
100		Cantilever Slab	SER	S	Exposed Rebar	Pier 2 Bottom of Slab	A = 0.040 m ²	Slight	One	c	149
100 101	3		· .		Horizontal Cracks	@ Downstream Pier 2 Coping	l = 3.500 mm	Heavy	Many		150
	3	Copina	CR Í	н							100
101 102	3	Coping Asphalt Pavement	CR	н 	and Map Cracks	@ Downstream Deck Slab Wearing				a	220
101 102 103	3 3	Asphalt Pavement	CRPL	s	Corrugation		l = 8.000 mm	Slight	Оле	c	332
101 102 103 104	3 3 3	Asphalt Pavement Asphalt Pavement	CRPL CRPL	S M	Corrugation Crack	Deck Slab Wearing Course Inner Lane Deck Slab Wearing Course Outer Lane Deck Slab Wearing	l = 8.000 mm l = 10.000 mm	Slight Moderate	One One	c b	333
101 102 103	3 3	Asphalt Pavement	CRPL	s	Corrugation	Deck Slab Wearing Course Inner Lane Deck Slab Wearing	l = 8.000 mm	Slight	Оле	c	

Appendix 7.1.1-1 (26/34)

Damage No.	Span No.	Name of Member	Type of Damage	Rank of Damage	Nature	Location/	scription of Damag	e Severity	No. of	Remarks (a, b, c)	Pho
108	3	Railing	SER	S	Exposed Rebar	Pattern Bottom of Railing	A = 0.040 m ²	Slight	Damages	-	
100	4	Coping	CR	н	Horizontal Crack	Pier 4 Coping		·		c	35
110	4	Coping	CR			@ Upstream Pier 4 Coping	t = 9.000 mm	Heavy	One	a	96
111	4			н	Horizontal Crack	@ Upstream Bottom of Girder		Heavy	One	a	97
		Pier 4 Girder 1	SER	S	Spali	@ Upstream Pier 4 Coping @	A = 0.100 m ²	Slight	One	c	98
112	4	Coping		н	Horizontal Crack	Bay 2 & under Pier 4 Coping @	i = 1.169 mm	Heavy	One	a	99
113	4	Coping	CR	M	Horizontal Crack	Bay 3 under Girder 3	t = 0.559 mm	Moderate	One	b	99
114	4	Coping	CR	M	Vertical Crack	Pier 4 Under Girder 4 Pier 4 Coping	t = 0.305 mm	Moderate	One	b	99
115	4	Coping	CR	M	Horizontal Crack	(@ Bay 4	t = 0.432 mm	Moderate	One	b	99
116	4	Pier 4 Girder 3	SER	м	Spall	Bottom of Girder @ Bay 3	A = 0.300 m ²	Moderate	One	b	10
117	4	Pier 4 Girder 4	SER	м	Spall	Bottom of Girder @ Bay 4	A = 0.420 m ²	Moderate	Оле	b	10
118	4	Pier 4 Girder 9	SER	М	Spall	Bottom of Girder @ Bay 9	A = 0.250 m ²	Moderate	One	Ъ	10
119	4	Pier 4 Girder 9	SER	s	Exposed Rebar	Face of Girder	A = 0.020 m ²	Slight	One	c	10
120	· 4	Deck Slab	SER	Н	Thin Concrete Cover	Pier 4 Bottom of Slab @ All Bays	Whole Span	Heavy	Several	а	10
121 ·	4	End Diaphgram	CR	S	Vertical Crack	Pier 4 Face of Diaph. @ Bay 10	t ≈ 0.173 mm	Slight	One	с	10
122	4	Coping	CR	Н	Random Cracks	Pier 4 Coping Bay 10 to Bay 11	t = 0.610 mm	Heavy	Many	а	100
123	4 ·	Pier 4 Girder 11	SER	s	Spall	Boltom of Girder @ Bay 11	A = 0.030 m ²	Slight	One	c	10
124	4	Pier 4 Coping	CR	н	Horizontal Crack w	Face of Coping @	t = 6.000 mm	Heavy	Many	а	10
125	4	Pier 3 Coping	CR	н	Random Crack Random Cracks	Downstream Face of Coping	t = 3.000 mm	Heavy	Many	a	109
126	4	Pier 3 Girder 2	нс	S	Honeycomb	@ Upstream Boltom Flange of	A = 0.010 m ²	Slight	One		110
127	4	Coping	CR	н	Random Cracks	Ginder 2 Bay 1 Pier 3 Coping	t = 1.143 mm			c	
127	4	Diaphgram Haunch	HC			@ Bay 3 Pier 3 Diaph. Haunch		Heavy	Many	a	11
128	4	Deck Slab and End		s v	Honeycomb	@ Bay 6	A = 0.040 m ²	Slight	One	c .	112
		Dianhoram	SER	M	Spall Horizontal and	Pier 3 at Bay 7 Pier 3 Coping	A = 0.300 m ²	Moderale	One	ь	11:
130	4	Coping	CR	м	Vertical Cracks Vertical Cracks w/	@ Bay 8 Pier 3 Under	t = 0,559 mm	Moderate	Two	b	114
131	4	Coping	CR	M	Free Lime	Girder 10	t = 0.559 mm	Moderate	Məny	b	11
132	4	Deck Slab	SER	S	Exposed Rebar	Pier 3 Bottom of Slab (@ Bay 10	A = 0.020 m ²	Slight	Two	c	116
133	4	Pier 3 Girder 11	SER	S	Exposed Rebar	Top Flange of Girder @ Bay 10	A = 0.010 m ²	Slight	One	с	117
134	4	Deck Slab	SER	S	Exposed Rebar	Pier 3 Bottom of Slab @ Bay 11	A = 0.008 m ²	Slight	One	с	118
135	4	Deck Slab	SER	M	Spall	Pier 3 Bottom of Slab @ Bay 11	A = 0.400 m ²	Moderate	One .	b	119
136	4	Coping	CR	м	Map Cracks	Pier 3 Coping @ Downstream	t = 0.457 mm	Moderate	Many	b	120
137	4	Coping	CR	н	Horizontal Cracks	Pier 3 Coping @ Downstream	t = 3.000 mm	Heavy	Many	а	121
138	4	Cantilever Slab	SER	s	Exposed Rebar	Pier 3 Bottom of Slab	A = 0.030 m ²	Slight	One	c	122
139	4	Coping	CR	М	Random Cracks	@ Downstream Bot. Pier 3 Coping @	t = 0.457 mm	Moderate	Three	b	123
140	4	Railing Post	SER	S	Exposed Rebar	Downstream Face of Rail Post	A = 0.008 m ²	Slight	One	c	331
141	4	Railing Post	SER	- S	Exposed Rebar	Face of Post					
142	5				Vertical and	Pier 5 Coping	A = 0.030 m ²	Slight	One	c	355
		Coping	CR	н	Horizontal Cracks	@ Upstream Pier 5 Coping	t = 0.750 mm	Heavy	Many	а	71
143	5	Coping	CR	H	Random Cracks	@ Upstream Pier 5 Bottom of Slab	t = 0.750 mm	Heavy	Many	a	72
144	5	Cantilever Slab	CR	М	Crack	@ Upstream	t = 0.400 mm	Moderate	One	b	73
145	5	Coping	CR	М	Horizontal Cracks	Pier 5 Coping @ Bay 2	t = 0.508 mm	Moderate	Two	b	74
146	5	Pier 5 Girder 2	SER	S	Spall	Top Flange of Girder @ Bay 2	A = 0.010 m ²	Slight	One	с	75
147	5	Deck Slab	SER	м	Spall	Pier 5 Bottom of Slab @ Bav 2	A = 0.300 m ²	Moderate	One	b	76
148	5	Pier 5 Girder 3	SER	м	Exposed Rebar	Top Flange of Girder @ Bay 2	A = 0.120 m ²	Moderate	One	b	77
149	5	Deck Slab	CR	М	Random Cracks	Pier 5 Bottom of Slab @ Bay 3	t = 0.381 mm	Moderate	Three	b	78
150	5	Deck Slab	CR	s	Crack	Pier 5 Bottom of Slab @ Bay 4	t = 0.229 mm	Slight	Three	с	79
151	5	Pier 5 Girder 5	SER	S	Spall	Top Flange of Girder (0): Bay 5	A = 0.020 m ²	Slight	One	c	80
152	5	Deck Slab	SER	s	Exposed Rebar	Pier 5 Bottom of Slab	A = 0.015 m ²	Slight	One	c	81
153	5	Pier 5 Girder 11	SER	s	Spall	(2) Bay 7 Top Flange of Girder	A = 0.040 m ²	Slight	One	c	82
154	5	Coping	CR	н	Horizontal Cracks	@ Bay 11 Pier 5 Coping End	t = 10.000 mm	Heavy	Many	a	82A
155		Pier 5 Girder 12	SER	s	w/ Map Creck Exposed Rebar	Downstream Face of Girder	A = 0.010 m ²	Slight	Two		83
156	5	Pier 4 Coping	CR	S S	Random Cracks	@ Upsteam Pier 4 Coping	1 = 0.173 mm	· · ·		c	
157	5					End Upstream Face of Girder		Slight	Three	c	84
		Pier 4 girder 1	HC	н	Honeycomb	@ Upstream Pier 4 Bottom of Slab	A = 0.480 m ²	Heavy	One	а	85
158	5	Cantilever Slab	SER	S	Spall	@ Upstream Pier 4 Coping	A = 0.020 m ²	Slight	One	c	86
159	5	Coping	CR	н	Vertical Cracks	@ Bay 2 Pier 4 Coping	t = 1.199 mm	Heavy	Two	a	87
160	5	Coping	CR	H	Vertical Cracks	(@) Bav 3	t = 0,783 mm	Heavy	Two	а	88
161	5	Pier 4 Girder 9	SER	м	Spall	Bottom Flange of Girder 9 @ Bay 10	A = 0.150 m ²	Moderate	One	ъ	89
162	5	Coping	CR	S ·	Vertical Cracks and Horizontal Crack	Pier 4 Coping @ Bay 10 & Bay 11	t = 0.254 mm	Slight	Three	с	90
163	5	Pier 4 Girder 11	SER	S	Spall	Boltom of Girder @ Bay 11	A = 0.025 m ²	Slight	One	с	91
164	5	Coping	CR	н	Horizontal Crack @ Map Cracks	Pier 4 Coping @ Downstream	t = 4.000 mm	Heavy	Many	a	92
165	5	Coping	CR	н	Horizontal Crack w/	Pier 4 Coping	t = 2.500 mm	Heavy	Many	a	93
	5	Cantilever Slab	SER	s	Map Cracks Exposed Rebar	@ Downstream Pier 4 Bollom of Slab	A = 0.020 m ²	Slight	One	c	94
166	1					@ Downstream Pier 4 Bottom of Stab	A = 0.010 m ²	Slight	One		94
	5	Cantilever Slah	SER 1	S I	EXDOSed Rehar						
167	5	Cantilever Slab	CRPI	S	Exposed Rebar	@ Downstream Deck Stab Wearing				c	
	5 5 5	Cantilever Slab Asphalt Pavement Railing Post	SER CRPL SER	S S S	Exposed Rebar Corrugation Exposed Rebar		t = 5.000 mm A = 0.030 m ²	Slight	Two	c c	330 352

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Appendix 7.1.1-1 (27/34)

Damage No.	Span No.	Name of Member	Type of Damage	Rank of Damage	Nature	De Location/ Pattern	scription of Damag	je Severity	No. of Damages	Remarks (a, b, c)	Photo No.
171	5	Railing Post	SER	s	Exposed Rebar	Face of Post	A = 0.010 m ²	Slight	One	c	354
172	6	End Diaphragm	SER	S	Exposed Rebar	Pier 6 @ Upstream Side	A = 0.020 m ²	Slight	One	· c	25
173	6	End Diaphragm	нс	s	Haneycomb	Pier 6 @ Upstream Side	A = 0.030 m ²	Slight	One	c	25
174	6	Coping	CR	н	Horizontal Crack	Pier 6 Span 6	t = 5.000 mm	Heavy	One	а	26
175	6	Cantilever Slab	SER	s	Exposed Rebar	Pier 6 Bottom of Slat	A = 0.012 m ²	Slight	One	c	27
176	6	Coping	CR	s	Vertical Crack	@ Upstream Side Pier 6 @ Bay 5	t = 0.229 mm	Slight	One	c	28
177	6	Pier 6 Girder 7	SER	s	Exposed Rebar	Top Flange @ Bay 6		Slight	One	c	29
178	6	Pier 6 Girder 8	SER	M	Spall	Bollom @ Bay 7	A = 0.120 m ²	Moderate	One	b	30
179	6	Coping	CR	S	Vertical Crack	Pier 6 Under Girder 8		Slight	One	c c	31
180	6	Pier 6 Girder 8	SER	s	Spall	Bottom @ Bay 3	A = 0.025 m ²	Slight	One	c	32
181	6	Pier 6 Girder 9	SER	s	Spall	Face @ Bay 8	A = 0.060 m ²	Slight	One		
182	6	Pier 6 Girder 10	SER	s	Exposed Rebar	Top Flange @ Bay 9		Slight	One	c	33
183	6	Deck Slab	FL			Pier 6 Bottom			<u> </u>	с	34
184	6			Н	Free Lime Vertical Crack w/	@ Bay 8 Pier 6 Under	A = 5.500 m ²	Heavy	One	a	35
		Coping	CR	S	Free Lime Vertical Crack/Free	Girder 10	t = 0.254 mm	Slight	One	c	36
185	6	Coping	CR	н	Lime	Pier 6 @ Bay 10 Pier 6 Boltom	t = 1.200 mm	Heavy	One	а	37
186	6	Deck Slab	SER	S	Exposed Rebar	@ Bay 11 Pier 6 Bottom	A = 0.045 m ²	Slight	One	c	38
187	6	Deck Slab	SER	н	Exposed Rebar	@ Bay 11	A = 1.320 m ²	Heavy	One	а	39
188	6	Coping	CR	н	Horizontal Crack	Pier 6 @ Downstream	I	Heavy	One	а	40
189	6	Cantilever Slab	SER	S	Exposed Rebar	Pier 6 Bottom of Slab @ Downstream	A = 0.025 m ²	Slight	Two	c	41
190	6	Pier 5 Girder 1	SER	S	Exposed Rebar	Face of Girder @ Upstream	A = 0.090 m ²	Slight	One	с	42
191	6	Pier 5 Girder 1	SER	М	Spall	Top Flange of Girder @ Upstream	A = 0.140 m ²	Moderate	One	b	43
192	6	Cantilever Slab	CR	s	Crack	Pier 5 Bottrom of Cantilver Slab @ U/S	t = 0.173 mm	Slight	One	С	44
193	6	Coping	CR	н	Random Cracks	Pier 5 @ Upstream	t = 6.00 mm	Heavy	Many	а	45
194	6	Deck Slab	SER	s	Exposed Rebar	Bottom of Slab @ Bay	A = 0.06 m ²	Slight	One	с	46
195	6	Pier 5 Girder 2	SER	s	Exposed Rebar	Face of Girder @ Bay 10	A = 0.015 m ²	Slight	One	с	47
196	6	Pier 5 Girder 2	SER	м	Spall	Bottom of Girder @ Bay 1	A = 0.18 m ²	Moderate	One	b	48
197	6	Coping	CR	M	Random Cracks	Pier 5 @ Bay 1 to Bay		Moderate	Many	b	49
198	6	Pier 5 Girder 3	нс	S	Honeycomb	Top Flange of Girder	$A = 0.03 m^2$	Slight	One	с –	50
199	6	Coping	CR	M	Random Cracks	@ Bay 1 Pier 5 Coping	t = 0.356 mm	Moderate	Many	b	51
200	6	Coping	CR	M	Random Cracks	@ Bav 3 Pier 5 Coping	t = 0.381 mm	Moderate			
201	6	Pier 5 Girder 4				@ Bav 4 Top Flange of Girder			Many	b	52
201			SER	S	Exposed Rebar	@ Bay 4 Bottom Flange of	A = 0.01 m ²	Slight	One	С	53
	6	Pier 5 Girder 4	SER	S	Spall	Girder @ Bay 4 Top Flange of Girder	A = 0.04 m ²	Slight	One	С	53A
203	6	Pier 5 Girder 5	SER	S	Spall	@ Bav 4 Pier 5 Coping	A = 0.02 m ²	Slight	One	c	54
204	6	Coping	CR	. M	Vertical Cracks	@ Bay 5 Pier 5 Coping	l = 0.406 mm	Moderate	Two	b	55
205	6	Coping	CR	м	Vertical Cracks	@ Bay 6	t = 0.381 mm	Moderate	Two	b	56
206	6	Pier 5 Girder 7	SER	S	Spall	Top Flange of Girder @ Bay 6	A = 0.01 m ²	Slight	One	с	57
207	. 6	Coping	CR	M	Random Cracks	Pier 5 Coping @ Bay 7	l = 0.483 mm	Moderate	Many	b	58
208	6	Coping	CR	н	Vertical Crack	Pier 5Under Girder 8	t = 0.510 mm	Heavy	One	а	59
209	6	Pier 5 Girder 7	SER	S	Spall	Top Flange of Girder @ Bay 7	A = 0.015 m ²	Slight	One	c	60
210	6	Coping	CR	м	Vertical Crack	Pier 5 Under Girder 9 @ Bay 9	l = 0,306 mm	Moderate	One	. b	61
211	6	Pier 5 Girder 9	SER	s	Spall	Top Flange of Girder @ Bay 8	A = 0.03 m ²	Slight	One	с	62
212	6	Coping	CR	м	Vertical Crack	Pler 5 Coping @ Bay 9	t ≈ 0.381 mm	Moderate	One	b	63
213	6	End Diaphragm	нс	s	Honeycomb	Pier 5 Diaphragm @ Bay 10	A = 0.015 m ²	Slight	One	с	64
214	6	Deck Slab	нс	S	Honeycomb	Pier 5 Bottom of Slab @ Bay 10	A = 0.02 m ²	Slight	Two	c	65
215	6	Coping	CR	н	Random Cracks	Pier 5 Under Girder 11	t = 0.813 mm	Heavy	Many	a	66
216	6	Pier 5 Girder 11	SER	S	Spall	and Bay 11 Top Flange of Girder	A → 0.06 m ²	Slight	One	c	67
217	6	Pier 5 Girder 12	SER	s	Exposed Rebar	@ Bay 11 Top Flange of Girder	A = 0.01 m ²	Slight	One	c	68
218	6	Coping	CR	н Н	Vertical and	@ Bay 11 Pier 5 Coping	t = 0.80 mm	Heavy	Many		69
219	6	Cantilever Slab	SER	S	Horizontal Cracks Spall	@ Downstream Pier 5 Bottom of Slab	A = 0.03 m ²			a	
219	6	Railing	SER	M	Exposed Rebar	@ Downstream		Slight	One	C	70
220	6					Bollom of Railing Deck Slab Wearing	A = 0.126 m ²	Moderate	One	b	327
221		Asphalt Pavement	CRPL	H	Pot Hole	Course Deck Slab Wearing	DIAM. = 0.05 m	Heavy	One	a	328
	6	Asphalt Pavement	CRPL	S	Corrugation Fractured /Random	Course Pier 6 Coping @	t = 5.00 mm	Slight	One	С	329
223	7	Coping	CR		Cracks Fractured /Random	Upstream Site Pier 6 Coping @	l = 9.00 mm	Heavy	Many	a	1
224	7	Coping	CR	<u> </u>	Cracks	Front Side	t = 8.00 mm	Heavy	Many	а	2
225	7	Pier 6 Girder 1	CR	s	Map Cracks	End Block @ Upstream Face	t = 0.076 mm	Slight	Many	C I	3
226	7	Deck Slab	CR	S	Random Cracks	Pier 6 Bottom of Slab @ Bay 1	1 = 0.229 mm	Slight	Three	с	4
227	7	Coping	CR	н	Horizontal Cracks	Pier 6 Coping @ Bay 1	ι = 2.00 mm	Heavy	Two	а	5
228	7	Pier 6 Girder 2	SER	S	Spall	Top Flange of Girder @ Bay 1	A = 0.015 m ²	Slight	One	с	6
229	7	End Diaphragm	CR	s	Vertical Crack	Diaphgram @ Bay 2	t = 0.173 mm	Slight	One	с	7
230	7	Coping	CR	s	Random Cracks	Pier 6 Coping @ Bay 2	t = 0.127 mm	Slight	Many	с	8
		Carling	CR	s	Random Cracks	Pier 6 Under Girder 3	l = 0.102 mm	Slight	Many		9
231	7	Coping		9 1	Random Cracks	The o onder dirder of		Oldin I		G	
231 232	7	Pier 6 Girder 4	SER	 M	Spall	Bot. Edge Girder @ Bay 3	A = 0.16 m ²	Moderate	One	c b	9 10

Appendix 7.1.1-1 (28/34)

Damage	Span	Name of Member	Type of	Rank of		Des Location/	scription of Damag		No. of	Remarks	Photo
No.	No.		Damage	Damage	Nature	Pattern	Scale	Severity	Damages	(a, b, c)	No.
234	7	Pier 6 Girder 4	SER	s	Exposed Rebar	Face of Girder @ Bay 3	A = 0.004 m ²	Slight	One	c	12
235	7	End Diaphragm	CR	S	Vertical Crack	Pier 6 Diaphragm @ Bay 4	l = 0.254 mm	Slight	One	с	13
236	7	Pier 6 Girder 5	SER	S	Spall	Bottom of Girder @ Bay 5	A = 0.03 m ²	Slight	One	с	14
237	7	Diaphragm Haunch	CR	S	Vertical Crack	Pier 6 Near Girder 6 @ Bay 6 Biot 6 Disabream	l = 0.127 mm	Slight	One	с	15
238	7	End Diaphragm	CR	s	Vertical Crack	Pier 6 Diaphragm @ Bav 7 Pier 6 Diaphragm	t = 0.173 mm	Slight	One	c	16
239	7	End Diaphragm	HC	S	Honeycomb	@ Bav 7 Pier 6 Diaphragm	A = 0.02 m ²	Slight	One.	c	16
240	7	End Diaphgram	CR	н	Vertical Crack	(2) Bay 8 Top Flange of Girder	t = 0,610 mm	Heavy	One	a	17
241	7	Pier 6 Girder 9	SER	S	Spall	@ Bay 8 Top Flange of Girder	A = 0.03 m ²	Slight	One	c	18
242	7	Pier 6 Girder 10	SER	S	Exposed Rebar	@ Bay 9 Face of Girder	A = 0.01 m ²	Slight	One	c	19
243	7	Pier 6 Girder 10	SER	S	Exposed Rebar	(2) Bay 10 Pier 6 Diaphragm	A = 0.03 m ²	Slight	One	c	20
244	7	End Diaphragm	CR	н	Crack	@ Bay 10 Bottom Flange of	l = 0.610 mm	Heavy	One	а	21
245	7	Pier 6 Girder 11	SER	м	Spall	Girder @ Bay 10 Bottom of Stab @	A = 0.15 m ²	Moderate	One	b	22
246	7	Cantilever Slab	SER	S	Exposed Rebar	Downstream Pier 6 Coping @	A = 0.03 m ²	Slight	One	c	23
247	7	Coping	CR	н	Random Cracks	Downstream Bottom Face of Slab	t = 2.00 mm	Heavy	Many	a	24
248	7	Cantilever Slab	SER	s	Exposed Rebar	Upstream Outer Face of Grider	A = 0.02 m ²	Slight	One	c	213 A
249	7	Pier 7, Girder 1	SER	S	Exposed Rebar	@ Upstream Outer Face of Girder	A = 0.05 m ²	Slight	One	с	214
250	7	Pier 7, Girder 1	SER	S	Exposed Rebar	@ Upstream Bottom Flange of	A = 0.05 m ²	Slight	One	c	215
251	7	Pier 7, Girder 1	SER	S	Exposed Rebar	Girder @ Bay 1 Top Flange of Girder	A = 0.01 m ²	Slight	One	c	216
252	7	Pier 7, Girder 2	SER	S	Spall	@ Bay 2	A = 0.04 m ²	Slight	Оле	c	217
253	7	Coping	CR	M	Vertical Cracks	Pier 7 Coping @ Bay 2 Bottom Face of Slab	l = 0.350 mm	Moderate	Three	b	218
254	7	Deck Slab	CR	S	Random Cracks	@ Bay 3 @ Bay 3 Bottom Flange of	t = 0.173 mm	Slight	Many	c	219
255	7	Pier 7, Girder 4	HC	М	Honeycomb	Girder @ Bay 4 Pier 7 Coping	A = 0.12 m ²	Moderate	One	b	220
256	7	Coping	CR	М	Vertical Cracks	@ Bay 3 Bottom Flange of	t = 0.350 mm	Moderate	Three	b	221
257	7	Pier 7, Girder 6	HC	М	Honeycomb	Girder @ Bav 5 Bottom Flange of	A = 0.12 m ²	Moderate	One	b	222
258	7	Pier 7, Girder 7	HC	M	Honeycomb	Girder @ Bay 6 Top Flange of Girder	A = 0.24 m ²	Moderate	One	b	223
259	7	Pier 7, Girder 8	SER	S	Exposed Rebar	(2) Bay 7 Top Flange of Girder	A = 0.03 m ²	Slight	One	c	224
260	7	Pier 7, Girder 10	нс	S	Honeycomb	@ Bay 9 Middle Face of	A = 0.03 m ²	Slight	One	<u>с</u>	225
261	7	End Diaphragm	нс	S	Honeycomb	Diaphragm @ Bay 10 Bottom Face of	A = 0.01 m ²	Slight	One	c	226
262	7	End Diaphragm	НС	S	Honeycomb	Diaphragm @ Bay 11	A = 0.008 m ²	Slight	One	с	227
263	7	Coping	CR	S	Random Cracks	Coping Face near End Downstream	t = 0.300 mm	Slight	Two	c	228
264	7	Cantilever Slab	SER	S	Exposed Rebar	Bottom of Slab @ Downstream	A = 0.04 m ²	Slight	Тwo	с	229
265	7	Pier 7, Girder 12	SER	S	Exposed Rebar	Outer Face of Girder @ Downstream	A = 0.015 m ²	Slight	One	c	230
266		Railing	SER	S	Exposed Rebar	Bottom of Railing	A = 0.02 m ²	Slight	One	c	324
267	7	Railing	SER	м	Spall	Bottom of Railing	A = 0.26 m ²	Moderate	One -	b	325
268	7	Railing	SER	S	Spall	Bottom of Railing	A = 0.04 m ²	Slight	Two	c	326
269	7	Railing	SER	S	Exposed Rebar	Bottom of Railing	A = 0.08 m ²	Slight	One	c	350
270	7	Railing Post	SER	S	Exposed Rebar	Face of Post	A = 0.02 m ²	Slight	One	C	351
271	8	Pier 7, Girder 1	НС	н	Honeycomb	Outer Face of Girder @ Upstream	A = 0.49 m ²	Heavy	One	а	243
272	8	End Diaphragm	CR	S	Map Cracks	Face of Diaphragm @ Bay 1	t = 0.076 mm	Slight	One	c	244
273	8	Deck Slab	SER	S	Exposed Rebar	Bottom of Slab @ Bay 1	A = 0.01 m ²	Slight	One	c	245
274	8	Coping	CR	S	Verlical Cracks	Pier 7 Coping Under Girder 2	t = 0.229 mm	Slight	One	c	246
275	8	Pier 7, Girder 2	HC	M	Honeycomb	Bottom Flange of Girder @ Bay 2	A = 0.11 m ²	Moderate	One	b	247
276	8	End Diaphragm	CR	М	Vertical Cracks	Face of Diaphragm @ Bay 3 Bottom Elence of	t = 0.350 mm	Moderate	One	b	248
277	8	Pier 7, Girder 4	SER	S	Spall	Bottom Flange of Girder @ Bay 3	A = 0.04 m ²	Slight	One	с	249
278	8	Pier 7, Girder 10	HC	M	Honeycomb	Boltom Flange of Girder @ Bay 10 Bottom of Girder	A = 0.16 m ²	Moderate	One	b	250
279	8	Pier 7, Girder 10	SER	м	Exposed Rebar	@ Bay 10 Bottom of Girder	A = 0.16 m ²	Moderate	Two	b	251
280	8	Pier 7, Girder 11	нс	S	Honeycomb	@ Bay 11 Face of Diaphragm	A = 0.09 m ²	Slight	One	¢	252
281	8	End Diaphragm	CR	S	Map Cracks	@ Bay 11 Outer Face of Girder	l = 0.076 mm	Slight	One	c	253
282	8	Pier 7, Girder 12	SER	S	Exposed Rebar	Downstream Bottom of Slab @	A = 0.02 m ²	Slight	One	c	254
283	8	Cantilever Slab	SER	S	Exposed Rebar	Downstream Face of Lighting	A = 0.03 m ²	Slight	One	c	255
284	8	Lighting Post Pedestal	CR	н	Crack	Post Pedestal Bottom and Midspan	l = 3.00 mm	Heavy	Two	а	322
285	8	Railing	SER	S	Exposed Rebar	of Railing Bottom Face of Stab	A = 0.09 m ²	Slight	One	c	323
286	9	Deck Slab	SER	н	Spall	@ Bay 1 Top Flange of Girder	A = 0.72 m ²	Heavy	One	а	256
287	9	Pier 8, Girder 2	SER	S	Exposed Rebar	@ Bay 1 Bottom of Slab	A = 0.03 m ²	Slight	One	c	257
288	9	Deck Slab	SER	S	Exposed Rebar	@ Bay 2	A = 0.06 m ²	Slight	One	c	258
289	9	Pier 8, Girder 3	SER	S	Spall	Top Flange of Girder @ Bay 2 Face of Disobrecom	A = 0.02 m ²	Slight	Two	c	259
290	9	End Diaphragm	SER	S	Exposed Rebar	Face of Diaphragm @ Bay 3	A = 0.06 m ²	Slight	One	c	260
291	9	Pier 8, Girder 4	SER	s	Exposed Rebar	Face of Girder @ Bay 3	A = 0.04 m ²	Slight	One	c	261
292	9	Deck Slab	НС	s	Honeycomb	Bottom of Slab @ Bay 4	A = 0.05 m ²	Slight	One	c	262
293	9	Deck Slab	HC	s	Honeycomb	Bollom of Slab @ Bay 5	A = 0.07 m ²	Slight	One	c	263
294	9	Deck Slab	SER	S	Exposed Rebar	Boltom of Slab @ Bay 5	A = 0.03 m ²	Slight	One	С	264
295	9	Pier 8, Girder 6	SER	s	Exposed Rebar	Top Flange of Girder @ Bay 6	A = 0.04 m ²	Slight	One	с	265
296	9	Deck Slab	SER	s	Exposed Rebar	Bottom Face of Slab @ Bay 7	A ≃ 0.10 m ²	Slight	One	с	266

Appendix 7.1.1-1 (29/34)

Damage No.	Span No.	Name of Member	Type of Damage	Rank of Damage	Nature	Location/ Pattern	scription of Damag Scale	e Severity	No. of Damages	Remarks (a, b, c)	Pho No
297	9	End Diaphragm	SER	м	Spall	Pier 8 End Diaphragr @ Bay 9	A = 0.15 m	Moderate	One	b	26
298	9	Deck Slab	CR	S	Cracks	Bottom Face of Stab @ Bay 9	1 - 0.076 mm	Slight	Many	с	268
299	9	End Diaphragm	SER	S	Exposed Rebar	Pier 8 End Diaphragn @ Bay 10	n A = 0.04 m ²	Slight	One	с	269
300	9	Pier 8, Girder 12	SER	s	Spall	Top Flange of Girder @ Bay 11	A = 0.03 m ²	Slight	One	с	270
301	9	Coping	CR	м	Random Cracks	Pier 8 Coping @ Upstream	t = 0.500 mm	Moderate	Many	b	27
302	9	Coping	CR	М	Vertical Cracks	Pier 9 Coping @ Upstream	t = 0.450 mm	Moderate	One	b	298
303	9	Pier 9, Girder 2	нс	м	Honeycomb	Face of Girder	A = 0.26 m ²	Moderate	One	b	299
304	9	Pier 9, Girder 3	SER	s	Exposed Rebar	@ Bay 1 Top flange of Girder	A = 0.04 m ²	Slight	One	c	300
305	9	End Diaphragm	CR	м	Verlical Cracks	@ Bav 2 Pier 9 End Diaphragn		Moderate	Öne	b	30
306	9	End Diaphragm	CR	M	Vertical Cracks	@ Bav 3 Pier 9 End Diaphragn	1	Moderate	One	b	302
307	9	Pier 9, Girder 4	нс	м	Honeycomb	@ Bay 4 Face of Girder	A = 0.14 m ²	Moderate	One		30
308	9	Intermediate Diaphragm	SER	s	Exposed Rebar	@ Bay 3 Pier 9 End Diaphragn				b	
309	9 ·		CR	s	<u> </u>	@ Bay 5 Bottom Face of Stab	A = 0.04 m	Slight	One	с	303
		Deck Slab			Crack	@ Bay 5 Bottom Flange of	1 = 0.200 mm	Slight	One	C T	30
310	9	Pier 9, Girder 6	SER	S	Spall	Girder @ Bay 5 Bottom Flange of	A = 0.08 m ²	Slight	One	c	30
311	9	Pier 9, Girder 6	SER	S	. Spall	Girder @ Bay 6 Pier 9 End Diaphragm	A = 0.08 m ²	Slight	One	c	306
312	9	End Diaphragm	CR	S	Crack	@ Bay 8	t = 0.300 min	Slight	One	с	30
313	9	Deck Slab	CR	S	Crack	Bottom Face of Slab @ Bay 9	t = 0.102 mm	Slight	One	с	308
314	9	Pier 9, Girder 10	SER	S	. Spall	Bottom Flange of Girder @ Bay 9	A = 0.02 m ²	Slight	One	C	309
315	9	Pier 9, Girder 8	SER	s	Exposed Rebar	Bottom Flange of Girder @ Bay 8	A = 0.01 m ²	Slight	One	с	310
316	9	End Diaphragm	CR	м	Crack	Pier 9 End Diaphragm @ Bay 10	t = 0.400 mm	Moderate	One	b	31
317	9	Cantilever Slab	SER	s .	Exposed Rebar	Bottom of Slab @ Downstream	A = 0.02 m ²	Slight	One	с	312
318	9	Asphalt Pavement	CRPL	s	Longitudinal Cracks	Deck Slab Wearing	l = 5 mm	Slight	Many	· c	32
319	10	Coping	CR	S	Random Cracks	Course Pier 10 Coping @	t = 0.150 mm	Slight	Many	c	23
320	10	Deck Slab	SER	S	Exposed Rebar	Upstream, Back Side Bottom Face of Slab	A = 0.04 m ²	Slight	One	c	23
321	10	Pier 10, Girder 2	SER	S	Exposed Rebar	@ Bay 1 Top Flange @ Bay 1	A = 0.06 m ²		One	-	
322	10	Pier 10, Girder 2	SER	s		near Pier 10 Top Flange along & Span of		Slight		c	23
					Exposed Rebar	P10&1st Intr Daph @ Bay 2 Top Flange near P10		Slight	One	c	234
323	10	Pier 10, Girder 3	SER	S	Spall	@ Bay 2	A = 0.10 m ⁻	Slight	One	c	238
324	10	Pier 10, Girder 3	SER	S	Exposed Rebar	Top Flange @ Bay 3	A = 0.06 m ²	Slight	One	c	236
325	10	Pier 10, Girder 4	SER	S	Exposed Rebar	End Block @ Bay 4	A = 0.08 m ²	Slight	One	с	237
326	10	Pier 10, Girder 7	SER	S	Spall	Top Flange @ Bay 6	A = 0.06 m ²	Slight	One	c	238
327	10	Pier 10, Girder 9	SER	S	Spall	Top Flange of Girder @ Bay 9	A = 0.04 m ²	Slight	One	с	239
328	10	Deck Slab	нс	s	Honeycomb	Bottom of Slab @ Bay 10	A = 0.02 m ²	Slight	One	с	240
329	10	Cantilever Slab	SER	S	Exposed Rebar	Bottom of Slab @ Downstream	A = 0.005 m ²	Slight	Many	с	241
330	10	Pier 10, Girder 12	SER	м	Exposed Rebar	Outer Face of Girder @ Downstream	A = 0.18 m ²	Moderate	. One	b	242
331	11	Steel Bracket	DEF	н	Corrosion /	Steel Bearing of	Remarkable Reduction	Heavy	One	а	272
332	11	Breast Wall	CR	м	Deformation Vertical Cracks	Girder 1 Abut. "B" Abutment "B" near	t = 0.500 mm	Moderate	Two	b	273
333	11	End Diaphragm	SER	M	Exposed Rebar	End Upstream Abutment "B"	A = 0.21 m ²	Moderate	One	b	274
334	11	Deck Slab	CR	s	Random Cracks	@ Bay 1 Bot. Face of Siab @					
335	11		FR			Abut. "B", Bay 1 Abutment "B"	l = 0.076 mm	Slight	Many	с	275
		End Diaphragm		н	Fractured	@ Bay 2 Bottom Flange of		Heavy	Оле	а	276
336	11	Abut.B, Girder 3	HC	M	Honeycomb	Girder @ Bay 2 Abutment "B" Bearing	A = 0.14 m ²	Moderate	One	b	277
337	11	Bearing Plate	CO	H	Corrosion	Plate @ Girder 3 Abutment "B"	Remarkable Reduction	Heavy	One	a	278
338	11	Breast Wall	HC	S	Honeycomb	@ Bay 2	A = 0.04 m ²	Slight	One	c	279
339	11	End Diaphragm	FR	н	Fractured	Abutment "8" @ 8aγ 3		Heavy	One	а	280
340	11	Shear Block	CR	н	Crack	Abutment "B" @ Bay 3	t = 10.000 mm	Heavy	One	а	281
341	11	End Diaphragm	FR	н	Fractured	Abutment "B" @ Bay 4		Heavy	One	а	282
342	11	Deck Slab	CR	S	Random Cracks	Bottom Face of Slab @ Abut. "B", Bay 4	t ≈ 0.076 mm	Slight	Many	c	283
343	11	End Diaphragm	SER	М	Exposed Rebar	Abutment "B" @ Bay 5	A = 0.12 m ²	Moderate	One	b	284
344	11	Abut.B, Girder 6	SER	S	Exposed Rebar	Face of Girder @ Bay 5	A = 0.05 m ²	Slight	One	с	285
345	11	Abut.B, Girder 6	нс	S	Honeycomb	Top Flange of Girder @ Bay 6	A = 0.01 m ²	Slight	One	с	286
346	11	End Diaphragm	SER	s	Exposed Rebar	Abutment "B"	A = 0.005 m ²	Slight	Three	c	287
347	11	Abut.B, Girder 6	HC	S	Honeycomb	@ Bay 6 Bottom Flange of	A = 0.08 m ²	Slight	One	c	288
348	11	End Diaphragm	SER	s	Exposed Rebar	Girder @ Bay 7 Abutment "B"	A = 0.06 m ²	Slight	Three		289
349	11	Deck Slab	CR	S	Random Cracks	@ Bay 7 Bottom Face of Stab	l = 0.076 mm	Slight	Many		289
350	11	End Diaphragm	SER	S	Exposed Rebar	@ Abut. "B", Bay 7 Abutment "B"				c	
351						@ Bay 8 Bottom Face of Stab	A = 0.05 m ²	Slight	One	с	291
	11	Deck Slab	HC	S	Honeycomb	@ Abut. "B", Bay 9 Abutment "B"	A = 0.03 m ²	Slight	One	c	292
352	- 11	End Diaphragm	SER	S	Exposed Rebar	@ Bay 10	A ≈ 0.06 m ²	Slight	One	c	293
353	11	Deck Slab	CR	S	Transverse Crack	Bottom of Slab @ Abutment "B", Bay 10	l = 0.102 mm	Slight	One	c	294
354		Abut.B, Girder 10	SER	s	Exposed Rebar	Top Flange of Girder @ Bay 10	A = 0.06 m ²	Slight	One	c	295
355	11	End Diaphragm	SER	S	Exposed Rebar	Abulment "B" @ Bay 11	A ≈ 0.06 m ²	Slight	One	с	296
356	11	Cantilever Slab	SER	S	Exposed Rebar	Boltom Face of Slab @ Downstream	A = 0.03 m ²	Slight	One	с	297
357	11	Asphalt Pavement	CRPL	s	Corrugation	Deck Slab Wearing Course near Pier 10	t = 5 mm	Slight	One	с	320
358	11	Asphalt Pavement	CRPL	s	Corrugation	App. Road Wearing	l = 5 mm	Slight	Two	c	317
359	11	Side Walk	SER	н	Broken Sidewalk	Course @ Abut. "B" Near Abutment "B"	A = 0.48 m ²	Heavy	One	a	319

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Appendix 7.1.1-1 (30/34)

Damage	Span		Type of	Rank of		Desc	ription of Damage)		Remarks	Photo
No.	No.	Name of Member	Damage	Damage	Nature	Location/ Pattern	Scale	Severity	No. of Damages	(a, b, c)	No.
360	1	Approach Railing	HC	S	Honeycomb	Side of Approach Railing	A = 0.03 m ²	Slight	One	с	337
361	1	Asphalt Pavement	CRPL	м	Cracks	App. Slab Wearing Course @ Abut. "A"	t = 6 mm	Moderate	Two	b	338
362	1	Side Walk	SER	н	Broken Sidewalk	Near Abutment "A"	A = 1.95 m ²	Heavy	One	а	340

Name of Bridge : Ma4 MARIKINA BRIDGE

6.

Date of Inspection Dec. 18 - 19, 2002 Inspector R. Abad/N. Castro Checker J.B. Agnes

Damaga	Enon		Tuna of	Deals of		Des	scription of Damage		r_J.B. Agnes		1
Damage No.	Span No.	Name of Member	Type of Damage	Rank of Damage	Nature	Location/ Pattern	Scale	Severity	No. of Damages	Remarks (a, b, c)	Photo No.
1	1	Wingwall @ Abut A	CR	н	Vertical Crack	At Face of Abut. Wall	t = 12.5 mm	Heavy	Both Side	а	1
2	1	Trans. Restraining Bar @ Abut A End Diaph.	L/M	н	Loose	End of Girders	80% loosening	Heavy	One	a	2
3	1	End Diaph. Abut A	CR	s	Horizontal Crack	Front Face @ Bay 1	t = 0.15 mm	Slight	One	c	3
4	1	Abut A Shear Block	CR	s	Random Cracks	Front Face @ Bay 1	t = 0.15 mm	Slight	Several	с	4
5	1	Abut. A Coping	CR	s	Vertical Crack	Front Face @ Bay 1	t = 0,20 mm	Slight	One	с	5
6	1	End Diaph.	CR	s	Transverse Crack	Bottom @ Bay 2	t = 0.15 mm	Slight	Several	c	6
7	1	C Abut A End Diaph.	CR	S	Horizontal Crack	Bottom @ Bay 3	l = 0.15 mm	Slight	One		8
8	1	Chear Block	SER	s	Spall	Corner near G-4	A = 0.03 m ²	· · ·	+	c	
9	1	Deck Slab	CR	s		@ Bay 3		Slight	One	c	9
		@ Bay 4 End Diaph.			Transverse Crack	Bottom @ Bay 4	t = 0.15 mm	Slight	One	c	10
10	1	Abut A	SER	s	Spall	Bottom @ Bay 4	$A = 0.025 \text{ m}^2$	Slight	One	c	11
11	1	PSC Girder G-4	SER	S	Spall Horizontal & Vertica	End of Girders Front Face	A = 0.025 m ²	Slight	One	c	12
12	1	Abut. A Coping End Diaph.	CR	S	Crack	@ 8ay 9	t = 0.20 mm	Slight	One	с	13
13	1		SER	S	Spall	Bottom	A = 0.05 m ²	Slight	One	c	14
14	1	Deck Slab @ Bay 5	CR	S	Transverse Crack	Bottom	t = 0.15 mm	Slight	One	с	15
15	1.	End Diaph. @ Abut A	SER	S	Exposed Rebar	Bottom	A = 0.06 m ²	Slight	One	с	16
16	1	Deck Slab @ Bay 6	CR ·	s	Transverse Crack	Bottom near End Diaph,	t = 0.15 mm	Slight	One	с	17
17	1	Deck Slab @ Bay 6	CR ¹	S	Transverse Crack	Bottom @ 1.20 m from End Diaph.	l = 0.15 mm	Slight	One	c'	18
18	1	End Diaph. @ Abut A	SER	S	Exposed Rebar	Bottom @ Bay 7	A = 0.05 m ²	Slight	One	с	19
19	1	Shear Block	SER	S	Spall	Baside Girder G-8	A = 0.05 m ²	Slight	One	c	20
20	1.	On Pier Deck Slab	CR	s	Transverse Crack	Bottom @ 1.2 m	t = 0.15 mm	Slight	One	c	21
21	1	Bay 1 End Diaph.	CR	S.	Crack	from End Diaph. Bottom @ Bay 8	t= 0.15 mm	Slight	One		22
22	1	Deck Slab	CR	S	Transverse Crack	Bollom @ 1.2 m				c	
23	1	End Diaph.				from End Dieph. Bottom near G-10	t = 0.15 mm	Slight	One	c	23
		Abut A Deck Slab	CR	S	Crack	@ Bay 9 Bottom @ 1.2 m	l = 0.25 mm	Slight	One	c	24
24	1	Bay 9 End Diaph.	CR	S	Transverse Crack	from End Diaph.	t = 0.15 mm	Slight	One	с С	25
25	1	. @ Abut A	SER	S	Exposed Rebar	Bottom @ Bay 10	A = 0.10 m ²	Slight	One	c	26
26	1	PSC Girder G-11	CR	S	Diagonal Crack	Web near end Block @ Bay 11	i = 0.15 mm	Slight	One	c	27
. 27	1	Cantilever Slab	SER	S	Exposed Rebar	Bottom	A = 0.03 m ²	Slight	One	с	28
28	1	Sidewalk	MISD	М	Fracture Due to Settlement	Approach Slab	Diam. = 25 mm	Moderate	One	b	109
29	1	Deck Slab	CR	S	Random Cracks	Top of Slab	t = 0.20 mm	Slight	Several	с	110/11
30	. 1	Railing near P3	SER	S	Spall	15 m from Abul A	A = 0.019 m ²	Slight	One	с	111
31	1	Median	SER	S	Spall	Above Abut. A @	A = 0.06 m ²	Slight	Two	с	127
32	1	Expansion Joint	EJ	м	Expansion Joint	Expansion Joint @ Abut. A	Abnormal Laying gap =	Moderate	Оле	b	128
33	1	Sidewalk	CR	S	Separation Cracks	Approach Slab	6 mm l = 0.25 mm	Slight	Several	c	129
34	1	Railing	SER	s	Spall	Bottom 4.7 m	A = 0.08 m ²	Slight	One	c	130
35	2	Pier 1 Coping	CR	M	Vertical Crack	from P1 At end along U/S.	t = 0.60 mm	Moderate			150
36	2			s		Front Face near			One	b	
37	2	Pier 1 Coping	CR		Random Cracks	end along U/S.	l = 0.10 mm	Slight	Several	с	29
		Cantilever Slab Deck Slab	SER	S	Spall	Bottom along U/S. Bottom bet, End	A = 0.03 m ²	Slight	One	c	30
38	2	Bay 1 Deck Slab	SER	S	Spall	& interm. Diaph	A = 0.06 m ²	Slight	One	c	31
39	2	@ Bay 2 Deck Slab	CR	M	Crack	End corner above diaph.	t = 0.35 mm	Moderate	One	b	32
40	2	Deck Slab O Bay 2 Deck Slab	CR	М	Transverse Crack	Bottom	t = 0.35 mm	Moderate	One	b	33
41	2	Deck Slab @ Bay 3 End Diaph.	нс	М	Honeycomb	Bottom	A = 0.18 m ²	Moderate	One	b	34
42	2	End Diaph. @ Bay 3	CR	н	Horizontal Crack	Top of Diaph.	t = 1,8 mm	Heavy	One	а	35
43	2	Pier 1 Coping	CR	S	Random Cracks	Front Face All Bays	l = 0.10 mm	Slight	Several	с	36
44	2	End Diaph.	нс	s	Honeycomb	Front Face @ Bay 7	A = 0.06 m ²	Slight	One	с	37
45	2	End Diaph.	нс	S	Honeycomb	Front Face @ Bay 8	A = 0.045 m ²	Slight	One	с	38
46	2	End Diaph.	нс	s	Honeycomb	Front Face @ Bay 10	A = 0.06 m ²	Slight	One	c	39
47	2	O Pier 1 PSC Girder G-12	CR	S	Random Cracks	Web of Girder	l = 0.15 mm	Slight	Several	c	40
48	2	Pier 1 Coping	CR	s	Random Cracks	Front Face near	t = 0.30 mm	Slight			40
49	2					End D/S Front Face near			Several	C	
		Pier 1 Columns	CR	M	Random Cracks	End D/S	t = 0.40 mm	Moderate	Several	b	
50	2	Pier 1 Coping	CR	S	Random Cracks	Face of Coping	1 = 0.20 mm	Slight	Several	c	
51	2	Rail Post	SER	S	Spall	Top of Split Post	A = 0.038 m ²	Slight	One	с	113
52	2	Deck Slab	SER	S	Spall	Top of Slab 3.5 m from P1	A = 0.0875 m ²	Slight	One	c	114
53	2	Rail Post	SER	S	Spall	Top of Post 8.5 m from P1	A = 0.0625 m ²	Slight	One	с	115
54	2	Rail Post	SER	S	Spall	Split Post 12 m from P2	A = 0.075 m ²	Slight	One	с	116
	2	Deck Slab	SER	s	Exposed Rebar	Top of Slab near	A = 0.03 m ²	Slight	One	с	117
55	- 1					Sidewalk 8.6m fr. P2		+a		v 1	

Appendix 7.1.1-1 (31/34)

Damage No.	Span No.	Name of Member	Type of Damage	Rank of Damage	Nature	De Location/ Pattern	Scription of Damag	Severity	No. of Damages	Remarks (a, b, c)	Phot No.
57	2	Sidewalk	SER	s	Spall	Top of Curb 3m from P2	A = 0.015 m ²	Slight	One	с	119
58	2	Railing	SER	s	Spall	Bottom 10m from P	2 A = 0.08 m ²	Slight	One	с	131
59	3	Pier 2 Coping	CR	S	Random Cracks	At End along U/S.	t = 0.30 mm	Slight	Several	c	101
60	3	Cantilever Slab	CR	S	Random Cracks	Bottom	t = 0.25 mm	Slight .	Several	c	102
61	3	Pier 2 Coping	CR	s	Random Cracks	Coping Face	l = 0.30 mm	Slight	Several	c	103
62	3	End Diaph.	нс	S	Honeycomb	@ Cantilever Front Face along	A = 0.08 m ²	Slight	One/Diaph.	c	104
63	3	Deck Slab	нс	м	Honeycomb	Bay 1-7, 10 & 11 Bottom	A = 0.20 m ²	Moderate	One	b	105
64	3	1st Interrm. Diaph.	HC	M	Honeycomb	Back Face	A = 0.15 m ²	Moderate	One	b	100
65	3	End Diaph.	нс	s	Honeycomb	Front Face	$A = 0.08 \text{ m}^2$	Stight	One		100
66	3	Cantilever Slab	SER	м	Spall	Bollom	A = 0.00 m ²	Moderate	One	C b	
67	3	Railing	SER	S	Exposed Rebar			-		b	108
68	3	Railing	SER	s		Bot. 9m from P3	A = 0.03 m ²	Slight	One	с с	120
69	3				Spall	Bot. 12 m from P3	A = 0.0825 m ²	Slight	One	c	121
		Railing	SER	S,	Spall	Bol. 8.5 m from P3	A = 0.20 m ² Progressive	Slight	One	c	133
70	3	Deck Slab	D/D	S	Deteriation on Top	7m from P3 Bottom near	Deterioration	Slight	Several	c	134
7,1	4	Cantilever Slab D/S End Diaph.	SER	s	Exposed Rebar	Back Wall	A = 0.06 m ²	Slight	Several	с	62
72	4	<u> </u>	нс	S	Honeycomb	Back Face @ Bay 2	A = 0.06 m ²	Slight	One	c	76
73	4	@ Pier 4	нс	S	Honeycomb	Back Face @ Bay 3	A = 0.06 m ²	Slight	One	с	77
74	4	PSC Girder G-4	HC	s	Honeycomb	Top Flange @ Bay 4	A = 0.04 m ²	Slight	One	с	78
75	4	PSC Girder G-5	SER	S	Spali	Top Flange @ Bay 4	A = 0.06 m ²	Slight	One	с	79
76	4	PSC Girder G-6	SER	s	Exposed Rebar	Bollom	A = 0.03 m ²	Slight	Several	с	80
77	4	Interm. Diaph.	нс	М	Honeycomb	Front Face @ Bay 7	A = 0.15 m ²	Moderate	One	b	81
78	4	Deck Slab	нс	S	Honeycomb	Bottom	A = 0.06 m ²	Slight	One	с	82
79	4	Deck Slab	SER	М	Exposed Rebar due		A = 0.20 m ²	Moderate	One	b	83
80	4	@ Bay 9 Deck Slab	нс	S	to Thin Cover Honeycomb	Bottom	A = 0.10 m ²	Slight	One	c	84
81	4	PSC Girder G-11	SER	s	Exposed Rebar	Top Flange @ Bay 11		Slight	One		85
82	4	End Diaph.	НС	s						с	
	4	<u> Pier 4</u>			Honeycomb	Bollom @ Bay 11	A = 0.09 m ²	Slight	One	С	86
83		Cantilever Slab	SER	S	Spall	Bottom	A = 0.10 m ²	Slight	Several	c	87
84	4	Pier 3 Coping End Diaph.	CR	S	Random Crack	End of Coping U/S.	l = 0.15 mm	Slight	Several	с.,	88
85	4	@ Pier 3	нс	M	Honeycomb	End of Diaph. U/S.	A = 0.12 m ²	Moderate	One	b	89
86	4	Cantilever Slab	SER	М	Spall	Bottom	A = 0.14 m ²	Moderate	One	b	90
87	4	End Diaph.	нс	S	Honeycomb	Top Portion from G-2 @ Bay 1	A = 0.023 m ²	Slight	One	c	91
88	4	End Diaph. <u>@ Pier 3</u> End Diaph.	HC	S	Honeycomb	Front Face @ Bay 2	A = 0.0625 m ²	Slight	One	c	92
89	4	@ Pier 3	нс	S	Honeycomb	Front Face @ Bay 3	A = 0.06 m ²	Slight	One	с	93
90	4	End Diaph. @ Pier 3	нс	s	Honeycomb	Front Face @ Bay 4	A = 0.075 m ²	Slight	One	с	94
91	4	End Diaph. @ Pier 3	нс	s	Honeycomb	Front Face @ Bay 5	A = 0.09 m ²	Slight	One	c	96
92	4	Deck Slab	CR	M	Horizontal Crack	Bottom	t = 0.35 mm	Moderate	One	b	95
93	4	Deck Slab	CR	s	Random Cracks	Bottom	t = 0.30 mm	Slight	Several	с	97
94	4	Bay 3 Pier 3 Coping	CR	м	Random Cracks	Front Face	l = 0.50 mm	Moderate	Several	b	98
95	4	Deck Slab	CR	s	Crack	At Hunch near Diaph.	t = 0.30 mm	Slight	One	c	99
96	4	@ Bay 11 Railing near P3	SER	s	Spall	Bot, near C3 Panel	A = 0.225 m ²	Slight	One/Panel		
97	4	Railing	SER	s				_		С	100
					Spall	Bot. 18 m from P3	A = 0.36 m ²	Slight	One	с	122
98	4	Rail Post	SER	S	Spall	Split Post above P4	A = 0.023 m ²	Slight	One	c	123
99	4	Median	SER	s	Exposed Rebar	Top, 12 m from P3	A = 0.30 m ²	Slight	Four	c	135
100	4	Railing	SER	S	Spall	Bot., 14 m from P4	A = 0.20 m ²	Slight	One	с	
101	5	Abut. B Coping End Diaph.	CR	S	Random Cracks	Face near End	t = 0.20 mm	Slight	Several	c	41
102	5	Abut B Deck Slab	SER	S	Spail	Bottom @ Bay 1	A = 0.03 m ²	Slight	One	c	42
103	5	@ Bay 1	CR	S	Transverse Crack	Bottom near End Diaph.	t = 0.20 mm	Slight	One	с	43
104	5	PSC Girder G-2	SER	S	Exposed Rebar	Bot. near Abut. 3	A = 0.01 m ²	Slight	Two	с	44
105	5	Abut B Coping	CR	S	Horizontal Crack	0.3 m from Top	t ≐ 0.30 mm	Slight	One	с	45
106	5	Abut B Coping	CR	s	Random Cracks	Face of Coping U/S	t = 0.30 mm	Slight	Several	с	46
107	5	Deck Slab	CR	S	Transverse Crack	Bollom Near End Diaph.	t = 0.25 mm	Slight	One	· c	47
108	5	@ Bay 1 End Diaph/	SER	S	Spall	Boltom @ Bay 3	A = 0.07 m ²	Slight	One	c	48
109	5	Anchor Bar	м	м	Missing	PSC Girder G-5	Remarkable Damage	Moderate	One ·	b	49
110	5	End Diaph.	CR	s	Random Cracks	Bottom @ Bay 5	t = 0.20 mm	Slight	Several	c	 50
111	5	@ Abut B Deck Slab	CR	s	Random Cracks		1 = 0.25 mm				
		@ Bay 5		· · · · · · · · · · · · · · · · · · ·		Bottom @ Bay 5 At Anchor		Slight	Several	c	51
112	5	PSC Girder G-6 End Diaph.	SER	S	Spall	Bar Location At end @ Bay 6	A = 0.04 m ²	Slight	One	c	52
113	5	Abut B End Diaph.	CR	S	Vertical Crack	near G-6	t = 0.30 mm	Slight	One	c	53
114	5	M Abut B End Diaph.	SER	S	Spall	Bottom @ Bay 7	A = 0.0375 m ²	Slight	One	c	54
115	5	Diaph. <u> </u>	SER	м	Spall	Bottom @ Bay 8	A = 0.12 m ²	Moderate	One	b	55
116	5	Deck Slab <u> </u>	SER	S	Exposed Rebar	Bottom @ Bay 8 1.00 from End Diaph.	A = 0.03 m ²	Slight	Several	с	56
117	5	End Diaph.	SER	s	Spall	Bottom @ Bay 9	A = 0.0625 m ²	Slight	One	c	57
118	5	Anchor Bar	м	м	Missing	PSC Girder G-10 @ Abut. B	Remarkable Damage	Moderate	One	b	58
119	5	End Diaph. @ Abut_B	SER	S ·	Spall	@ Abut. B Bottom @ Bay 10	A = 0.03 m ²	Slight	One	c	59

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Appendix 7.1.1-1 (32/34)

Damage	Span		Type of	Rank of			cription of Damag	e		Remarks	Photo
No.	No.	Name of Member	Damage	Damage	Nature	Location/ Pattern	Scale	Severity	No. of Damages	(a, b, c)	No.
120	5	End Diaph.	SER	s	Spall	Bottom @ Bay 11	A = 0.06 m ²	Slight	One	c	60
121	5	Deck Slab @ Bay 11	CR	S	Random Cracks	Bottom near end Diaph. @ Abut. A	t = 0.25 mm	Slight	Several	с	61
122	5	Pier 4 Coping	CR	м	Random Cracks	Back Face	t ≈ 0.60 mm	Moderate	Several	b	63
123	5	Pier 4 Coping	CR	S	Random Cracks	Front Face @ Cantilever	t = 0.25 mm	Slight	Several	с	64
124	5	PSC Girder G-2	CR	S	Random Cracks	End Block @ Pier 4	1 = 0.15 mm	Slight	Several	с	66
125	5	End Diaph. @ Pier 4	НС	S	Honeycomb	End along U/S,	A = 0.06 m ²	Slight	One	с	65
126	5	Deck Slab @ Bay 1	CR	s	Random Cracks	Bollom	t = 0.30 mm	Slight	Several	с	67
127	5	Deck Slab @ Bay 3	SER	S	Spall	Bottom	A = 0.06 m ²	Slight	One	с	68
128	5	Deckl Slab @ Bay 4	HC	S	Honeycomb	Bottom	$A = 0.10 \text{ m}^2$	Slight	One	с	69
129	5	Deck Slab @ Bay 4	HC	S	Honeycomb	Front Face near. Jt, of G-5	A = 0.06 m ²	Slight	One	с	70
130	5	Pier 5 Coping	D/D	S	Deterioration of Plaster	Front Face	A = 0.075 m ²	Slight	Several	с	71
131	5	Deck Slab @ Bay 6	нс	S	Honeycomb	Bottom	A = 0.06 m ²	Slight	One	c	72
132	5	End Diaph.	НС	S	Honeycomb	Front Face @ Bay 9	A = 0.09 m ²	Slight	One	c	73
133	5	Cantilever Slab	SER	· M	Spall	Bollom near P4	A = 0.20 m ²	Moderate	One	b	74
134	5	Pier 4 Coping	CR	м	Random Cracks	Front Face	t = 0.60 mm	Moderate	Several	b	
135	5	Cantilever Slab	SER	S	Exposed Rebar	Bollom	A = 0.02 m ²	Slight	Several	с	75
136	5	Railing	SER	S	Spall	Boliom near P4	A = 0.36 m ²	Slight	One	c	124
137	5	Railing	SER	S	Spall	Bot. 3.4 m from P4	A = 0.20 m ²	Slight	One	c	125
138	5	Railing	SER	s	Spall	Top 17 m from Abut, B	A = 0.05 m ²	Slight	One	с	126
139	5	Railing	SER	S	Spall	Top, 2 m from P4	A = 0.20 m ²	Slight	One	c	137
140	5	Railing	SER	s	Spall	Bottom, 9 m from Abut, B	A = 0.03 m ²	Slight	One	с	138
141	5	Railing	SER	S	Spall	Bottom, 2 m from Abut, B	A = 0.12 m ²	.Slight	One	C ·	139
142	5	Sidewalk	SER	н	Spall	Тор	A = 0.80 m ²	Heavy	One	а	140
143	5	Deck Slab	CR	S	Random Cracks	Top of Slab Typ. @ all Spans	t = 0.25 mm	Slight	Several	c	132

Name of Bridge : Ma5 SAN JOSE BRIDGE

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 Date of Inspection
 : Nov.21 / Dec. 587, 2002

 Inspector
 : R. Abed/E. Pegeregen

 Checker
 : J. B. Agnes

D	0				1	Dog	cription of Damage			J. B. Agnes	
Damage No.	Span No.	Name of Member	Type of Damage	Rank of Damage	Nature	Location/	1	· · · · ·	No. of	Remarks (a, b, c)	Photo No.
						Pattern End of Railing @	Scale	Severity	Damages	(a, b, c)	NO.
1	1	Railing	SER	S	Exposed Rebar	Abut. "A", D/S	A = 0.03 m ²	Slight	Оле	c	40
2	1	End Diaphragm	SER	s	Spali	Upstream Side of Abut, "A"	A = 0.06 m ²	Slight	One	c	8
3	1	Curb	SER	S	Exposed Rebar	Top of abut. "A" @ Exp. Joint	A = 0.03 m ²	Slight	One	c	39
4	1	PSC Girder, G-1	CR	S	Vertical Crack	End Face of Girder; 0.30 m. from Abut, "A"	t = 0.178 mm	Slight	One	с	1
5	1	PSC Girder, G-1	CR	s	Longitudinal Creck	Top Flange of Girder	t = 0.102 mm	Slight	One	С	2
6	1	PSC Girder, G-1	CR	s	Vertical Crack	End Face of Girder ; 0.30 m. from Abut. "A"	. t = 0.178 mm	Slight	One	c	4
7	1	Abutment "A", End Diaphragm (Cross Beam)	CR	. S	Longitudinal Crack	Face of Diaphragm near G-6 @ Bay 7	t = 0.102 mm	Slight	One	с	6
8	1	Steel Bearing (Rocker)	со	м	Corrosion	Steel Bearing of G-1 @ Abut. "A"	Reduction of Cross Section	Moderate	One	b	3
9	1	Steel Bearing (Rocker)	со	M	Corrosion	Steel Bearing of G-5 @ Abut. "A"	Reduction of Cross Section	Moderate	One	ь	5
10	. 1 .	Abulment "A", Backwall	CR	S	Vertical Crack	Downstream Side of Backwall	t = 0.10 mm	Slight	One	с	7
11	1	Pier 1, Expansion Joint	CRPL	S	Pothole	Wearing Course of Pavement	D = 10 mm	Slight	Оле	с	9
12	1	Pier 1, Diaphragm Wall	CR	м	Vertical Crack	D/S of Well;0.90 m.fr. Centr Column @ B.S.	l = 0.500 mm	Moderate	One	b	26
13	1	Pier 1, Diaphragm Wall	CR	н	Vertical Crack	D/S of Wall;0.90 m.fr. Centr Column @ B.S.	t = 0.635 mm	Heavy	One	а	30
14	1	Pier 1, Diaphragm Wall	CR	M	Vertical Crack	U/S of Well;1.00 m.fr. Centr Column @ F.S.	t = 0.410 mm	Moderate	One	b	27
15	1	Pier 1, Diaphragm Wall	CR	s	Random Cracks	Face of Diaphragm Wall @ Front Side	l = 0.076 mm	Slight	Many	с	28
16	1	Pier 1, Column	нс	S	Honeycomb	Face of Exterior Column @ DS	A = 0.04 m ²	Slight	One	c	32
17	1	Pier 1, Foundation	нс	S	Honeycomb	Side of Fooling @ Downstream	A = 0.12 m ²	Slight	One	с	38
18	2	Rail Post	SER	s	Exposed Rebar	Face of Split Post	A = 0.02 m ²	Slight	One	с	10
19	2	Deck Slab	HC	S	Honeycomb	Bot. of Slab @ Bay 4 3.50m from Pier 2	A = 0.002 m ²	Slight	One	с	29
20	2	Pier 2, Diaphragm Wall	CR	н	Vertical Crack	D/S : 1.00 m. from Center Column	t = 1.20 mm	Heavy	One	а	23
21	2	Pier 2, Diaphragm Wall	CR	н	Vertical Crack	U/S : 1.90 m. from Center Column	t = 0.610 mm	Heavy	One	а	24
22	2	Pier 2, Center Column	CR	s	Horizontal Crack	2.40 m. from Top of Footing	l = 0.173 mm	Slight	One	· c	25
23	3	Railing	SER	S S	Exposed Rebar	Bottom Face of Railing	A = 0.01 m ²	Slight	One	с.	11
24	3	Railing	SER	s	Exposed Rebar	Bottom Face of Railing @ Upstream	: A = 0.01 m ²	Slight	One	с	15
25	3	Railing	SER	s	Spall	Face of Railing @ Split Post, DS	A = 0.02 m ²	Slight	One	b	42
26	3	Railing and Post	SER	s	Spall	Face of Railing and Split Post	A = 0.03 m ²	Slight	Two	с	13
27 ·	3	Rail Post	SER	s	Spall	Face of Split Post @ Downstream	A = 0.02 m ²	Slight	One	с	41
28	3	Light Pole	м	м	Missing	Upstream Side	Remarkable Missing	Moderate	One	с	12
29	् 3	Sidewalk	SER	s	Exposed Rebar	Top of Sidewalk along Exp. Joint @ US	A = 0.028 m ²	Slight	One	с	14
30	3	Deck Slab	SER	S	Exposed Rebar	Bottom of Slab, 5.00 m. from Pier 3	A = 0.01 m ²	Slight	One	с	77
31	3	PSC Girder, G-1	CR	s	Vertical Crack	End Face of Girder @ Pier 3	l = 0.178 mm	Slight	One	с	71
32	3	PSC Girder, G-3	CR	s	Vertical Crack	End Face of Girder @ Pier 3	t = 0.15 mm	Slight	One	с	73
33	3	End Diaphragm	CR	s	Vertical Crack	Face of Diaphragm @ Bay 1	t = 0.10 mm	Slight	One	с	78
34	3	Steel Bearing	со	м	Corrosion	Steel Bearing of G-3 @ Pier 3	Reduction of Cross Section	Moderate	One	b .	74
35	3	Steel Bearing	со	M	Corrosion	Steel Bearing of G-4 @ Pier 3	Reduction of Cross Section	Moderate	One	b	76

Appendix 7.1.1-1 (33/34)

Damage No.	Span No.	Name of Member	Type of Damage	Rank of Damage	Nature	Location/ Pattern	scription of Damag	Severity	No. of Damages	Remarks (a, b, c)	Pho No
36	3	Steel Bearing	со	м	Corrosion	Steel Bearing of G-8		Moderate	One	ь	81
37	3	Pier 3, Coping	CR	s	Random Cracks	@ Pier 3 Face of Coping Below	W I = 0.10 mm	Slight	Many	c	72
38	3	Pier 3, Coping	CR	s	Vertical Crack	Girder G-2 Face of Coping Below		Slight	One	c	75
39	3	Pier 3, Coping	SER	s	Exposed Rebar	Girder G-3 Face of Coping Below		Slight	Three	c c	80
40	3	Pier 3, Diaphragm Wall	CR	н	Vertical Crack	Girder G-3 0.90 m. from Center	t = 2.00 mm	Heavy	One	a	34
41	3	Pier 3, Diaphragm Wall	CR	н	Horizontal Crack	Column @ Upstream 1.20 m. from top of	1	Heavy	One		35
42	3	Pier 3, Diaphragm Wall	CR	н	Vertical Crack	Footing 1.60 m. from Center				a	
43	3	Pier 3, Foundation	нс	s	Honeycomb	Column @ DS Under Side of Footing		Heavy	One	a	36
44	3	Pier 3, Foundation	SER	s	Exposed Rebar	Top of Footing @		Slight	One	с	33
	4				· · · · · · · · · · · · · · · · · · ·	Downstream Bot.Face of Railing,	A = 0.01 m ²	Slight	One	c	37
45		Railing	SER	s	Exposed Rebar	8.00m.fr.Pier 3 @ D/S Face of Railing @		Slight	One	c	43
46	4	Railing	SER	S	Exposed Rebar	Split Post, DS Bottom Face of	A = 0.06 m ²	Slight	One	с	44
47	4	Railing	SER	S	Spall	Railing @ DS	A = 0.005 m ²	Slight	One	c	45
48	4	PSC Girder, G-1	CR	S	Vertical Crack	End Face of Girder @ Pier 3	1=0.15 mm	Slight	One	с	79
49	4	PSC Girder, G-1	CR	S ·	Vertical Crack	End Face of Girder @ Pier 4	1-0.15 mm	Slight	One	c	99
50	4	PSC Girder, G-1	CR	м	Longitudinal Crack	Outer Top Flange of Girder @ Upstream	(= 0.35 mm	Moderate	. One	b	100
51	4	PSC Girder, G-1	CR	м	Random Cracks	Outer Face of Girder @ Upstream	t = 0.35 mm	Moderate	One	b	100
52	4	Steel Bearing	L/M	S	Loose Nut	Steel Bearing of G-6 @ Pier 3	One Portion	Slight	One	с	82
53	.4	Steel Bearing	L/M	S	Loose Nut	Steel Bearing of G-7 @ Pier 3	One Portion	Slight	One	с	83
54	4	Pier 4, Diaphragm Wall	CR	н	Vertical Crack	Face of Wall @ Upstream	l = 0.864 mm	Heavy	One	а	87
55	4	Pier 4, Diaphragm Wall	CR	s	Vertical Crack	Face of Column @	t = 0.127 mm	Slight	One	c	89
56	4	Pier 4, Diaphragm Wall	CR	н	Horizontal and	Downstream Face of Wall @	t = 0.610 mm	Heavy	Two	a	90
57	4	Pier 4, Diaphragm Wall	CR	s	Vertical Crack Vertical Crack	Downstream Face of Wall @	l = 0.076 mm	Slight	One		90
58	4	Pier 4, Diaphragm Wall				Downstream Face of Wall @				c	
			CR	н	Vertical Crack	Downstream Face of Column @	t = 1.194 mm	Heavy	One	а	93
59	4	Pier 4, Diaphragm Wall	HC	S	Honeycomb	Upstream Face of Column @	A = 0.36 m ²	Slight	One	С	94
60	4	Pier 4, Center Column	нс	S	Honeycomb	Upstream	A = 0.05 m ²	Slight	One	c	88
61	4	Pier 4, Center Column	HC	s	Honeycomb	Face of Column @ Downstream	A = 0.15 m ²	Slight	One	с	92
62	4	Pier 4, Column	нс	S	Honeycomb	Face of External Column @ Upstream	A = 0.33 m ²	Slight	One	с	85
63	4	Pier 4, Column	SER	S	Exposed Rebar	Face of External Column @ Upstream	A = 0.08 m ²	Slight	One	c	86
64	4	Pier 4, Column	HC	S	Honeycomb	Face of Column @ Upstream	A = 0.32 m ²	Slight	One	с	95
65	4	Pier 4, Column	CR	М	Vertical Crack	Face of Column @ Upstream	t = 0.305 mm	Moderate	One	b	96
66	4	Pier 4, Column	CR	s	Vertical Crack	Face of Column@US, 5.00 m.fr.Top of Fig.	t = 0.229 mm	Slight	One	c	97
67	4	Pier 4, Foundation	нс	s	Honeycomb	Bottom Face of	A = 0.35 m ²	Slight	One	c	101
68	4	Pier 4, PSC Pile	м	S	Missing	Footing @ DS Row 1 near End of	One Pile	Slight	One	c	98
69	5	Railing	SER	S	Spall	Footing Face of Railing along	A = 0.02 m ²	Slight	One		16
70	5	Rail Post	SER	s	Exposed Rebar	Split Post @ US Bot.Face of Post,	$A = 0.02 \text{ m}^2$			c	
71	5					16.00m. fr.Pier4@D/S		Slight	One	c	47
		Light Pole	M	M	Missing	Downstream Side Bot. of Stab @ Bay 4	Remarkable Missing	Moderate	One	b	46
72	5	Deck Slab	HC	S	Honeycomb	near Int. Diaphragm	A = 0.003 m ²	Slight	One	c	112
73	5	PSC Girder, G-1	CR	S	Random Cracks	Outer Face of Girder	l = 0.173 mm	Slight	Many	c	113
74	5 .	PSC Girder, G-1	SER	S	Exposed Rebar	End Face of Girder	A = 0.03 m ²	Slight	One	с	115
75	5	PSC Girder, G-8	CR	S	Longitudinal Crack	Top Flange of Girder near Pier 4	t = 0.102 mm	Slight	One	с	110
76	5	PSC Girder, G-8	CR	м	Longitudinal Crack	Bot. Flange of Girder, 3.00 m. from Pier 4	t = 0.30 mm	Moderate	One	b	111
77	5	PSC Girder, G-8	CR	s	Random Cracks	Outer Face of Girder	t = 0.173 mm	Slight	Many	c	113
78	5	Steel Bearing	DEF	м	Deformed	Steel Bearing of G-1 @ Pier 4	Remarkable Deflection	Moderate	One	b	102
79	5	Steel Bearing (Rocker)	DEF	M	Deformed	@ Pier 4 @ Pier 4	Remarkable Deflection	Moderate	One	b	103
80	5	Steel Bearing (Rocker)	DEF	м	Deformed	Steel Bearing of G-3	Remarkable Deflection	Moderate	One	b	104
81	5	Steel Bearing (Rocker)	DEF	м	Deformed	@ Pier 4 Steel Bearing of G-4	Remarkable Deflection	Moderate	One	b	105
82	5	Steel Bearing (Rocker)	DEF	M	Deformed	@ Pier 4 Steel Bearing of G-5	Remarkable Deflection	Moderate	One		105
83	5	Steel Bearing (Rocker)	DEF	M	Deformed	@ Pier 4 Steel Bearing of G-6	Remarkable Deflection			b	
84	5					@ Pier 4 Steel Bearing of G-7		Moderate	One	b	107
		Steel Bearing (Rocker)	DEF	M	Deformed	@ Pier 4 Steel Bearing of G-8	Remarkable Deflection	Moderate	One	b	108
85	5	Steel Bearing (Rocker)	DEF	M	Deformed	@ Pier 4 Steel Bearing of G-8	Remarkable Deflection	Moderate	One	b	109
86	5	Steel Bearing (Rocker)	L/M	S	Loose Nut	@ Pier 4	One Portion	Slight	Оле	c	109
87	5	Steel Bearing (Rocker)	со	н	Corrosion	Steel Bearing of G-1 @ Pier 5	Remarkable Reduction	Heavy	One	а	116
88	5	Steel Bearing (Rocker)	со	н	Corrosion	Steel Bearing of G-2 @ Pier 5	Remarkable Reduction	Heavy	One	а	117
89	5	Steel Bearing (Rocker)	со	н	Corrosion	Steel Bearing of G-3 @ Pier 5	Remarkable Reduction	Heavy	One	а	118
90	5	Steel Bearing (Rocker)	со	н	Corrosion	Steel Bearing of G-4 @ Pier 5	Remarkable Reduction	Heavy	One	а	119
91	5	Steel Bearing (Rocker)	со	н	Corrosion	Steel Bearing of G-5 @ Pier 5	Remarkable Reduction	Heavy	One	a	120
92	5	Steel Bearing (Rocker)	со	н	Corrosion	(a) Pier 5 Steel Bearing of G-6 (a) Pier 5	Remarkable Reduction	Heavy	One	a	121
93	5	Steel Bearing (Rocker)	со	н	Corrosion	Steel Bearing of G-7	Remarkable Reduction	Heavy	One	a	122
94	5	Steel Bearing (Rocker)	co	н	Corrosion	@ Pier 5 Steel Bearing of G-8	Remarkable Reduction	Heavy	One		122
95	5	Pier 4, Coping	CR	- Н		@ Pier 5 Face of Coping				a	
				· · · ·	Vertical Crack	Below Girder 3	l = 1.63 mm	Heavy	One	a	114
96 97	5	Pier 5, Coping	CR	s	Random Cracks	Face of Coping Face of Coping	t = 0.102 mm	Slight	Many	с	124
	5	Pier 5, Coping	CR	M	Vertical Crack	Below Girder 1	t = 0.40 mm	Moderate	One	ъ	126

Appendix 7.1.1-1 (34/34)

Dament	0.	· · · · · · · · · · · · · · · · · · ·	T	Des 1	1	Dee	cription of Damage	•	· · · ·		
Damage No.	Span No.	Name of Member	Type of Damage	Rank of Damage	Nature	Location/ Pattern	Scale	Severity	No. of Damages	Remarks (a, b, c)	Photo No.
99	5	Pier 5, Diaphragm Wall	CR	м	Vertical Crack	1.65 m. from Center Column @ Upstream	t = 0.508 mm	Moderate	Two	b	66
100	5	Pier 5, Diaphragm Wall	CR	M	Vertical Crack	1.00 m. from Center Column @ DS	t = 0.308 mm	Moderate	One	b	67
101	5	Pier 5, Foundation	нс	s	Honeycomb	Bottom Face of Footing @ Front Side	A = 0.01 m ²	Slight	Two	с	65
102	5	Pier 5, Foundation	SER	s	Spall	Top of Pile	A = 0.01 m ²	Slight	Two	с	68
103	5	Pier 5, Foundation	SER	s	Spall	Top of Pile	A = 0.15 m ²	Slight	Three	с	69
104	6	Rail Post	SER	S	Exposed Rebar	Face of Post @ Downstream	A = 0.01 m ²	Slight	One	c	49
105	6	Asphalt Pavement	CRPL	s	Pothole	Wearing Course of	19 mm ø	Slight	One	c	48
106	6	Asphalt Pavement	CRPL	s	Pothole	Pavement Wearing Course @	19 mm ø	Slight	One	c	50
107	6	Asphalt Pavement	CRPL	м	Pothole	Exp. Joint Pier 6 Wearing Course @	40 mm ø	Moderate	One	b	51
108	6	End Diaphragm	CR	s	Diagonal Crack	Exp. Joint Pier 6 Face of Diaphragm @	t = 0.173 mm	Slight	One	c	129
109	6	Steel Bearing	со	м	Corrosion	Bav 1 Steel Bearing of G-1	Reduction of Cross	Moderate	One	b	132
110	6	Steel Bearing	со	M	Corrosion	@ Pier 6 Steel Bearing of G-2	Section Reduction of Cross	Moderate	One	b	133
111	6	Steel Bearing	co	M	Corrosion	@ Pier 6 Steel Bearing of G-3	Section Reduction of Cross	Moderate	One	ь b	134
112	6	Steel Bearing	co	M	Corrosion	@ Pier 6 Steel Bearing of G-4	Section Reduction of Cross	Moderate	One	b	135
113	6	Steel Bearing	co	M	Corrosion	@ Pier 6 Steel Bearing of G-5	Section Reduction of Cross	Moderate			
114	· 6	Steel Bearing	00 00	M	Corrosion	@ Pier 6 Steel Bearing of G-6	Section Reduction of Cross	Moderate	One	b	136
-						@ Pier 6 Steel Bearing of G-7	Section Reduction of Cross	Moderate	One	b	137
115	6	Steel Bearing	 	M	Corrosion	@ Pier 6 Steel Bearing of G-8	Section Reduction of Cross		One	b	138
116	6	Steel Bearing	CO	м	Corrosion	@ Pier 6 2.70 m. from Outer	Section	Moderate	One	b	139
117	6	Pier 6, Diaphragm Wall	CR	M	Vertical Crack	Column @ Upstream 0.30 m. from Center	t = 0,387 mm	Moderate	One	b	61
118	6	Pier 6, Diaphragm Wall	CR	S	Vertical Crack	Column @ Upstream Face of Wall @	t = 0.203 mm	Slight	One	c	62
119	6	Pier 6, Diaphragm Wall	CR	H	Vertical Crack	Downstream Side Face of Column @	t ≃ 0.737 mm	Heavy	One	а	63
120	6	Pier 6, Outer Column	SER	S	Spall	Downstream Side Face of Column @	A = 0.45 m ²	Slight	One	c	64
121	6	Pier 6, Outer Column	CR	н	Vertical Crack	Downstream Side	t = 0.94 mm	Heavy	One	а	70
122	7	Railing	CR	н	Crack	Upstream Side Railing	t = 0.81 mm	Heavy	Three	a	18
123	7	Railing	SER	S	Exposed Rebar	Face of Railing @ Downstream	A = 0.03 m ²	Slight	Ône	c	54
124	7	Rail Post	SER	м	Exposed Rebar	Face of Post @ Upstream	A = 0.12 m ²	Moderate	Two	b	18
125	7	Rail Post	SER	S	Spall	Face of Post @ Downstream	A = 0.01 m ²	Slight	One	c	52
126	7	Rail Post	SER	S	Spall	Face of Split Post @ Downstream	A = 0.05 m ²	Slight	One	с	53
127	7	Cantilever Slab	SER	S	Exposed Rebar	Bottom Edge of Sidewalk	A = 0.04 m ²	Slight	Оле	с	130
128	7	Cantilever Slab	HC	S	Honeycomb	Bottom of Sidewalk	A = 0.05 m ²	Slight	One	с	131
129	7	Corbel for Light Pole	SER	s	Exposed Rebar	Face of Corbel @ Downstream	A ≈ 0.01 m ²	Slight	One	c	127
130	7	Asphalt Pavement	CRPL	S	Pothole	Wearing Course near Exp. Joint @ Pier 6	19 mm ø	Slight	One	с	17
131	7	PSC Girder; G-1	SER	S	Spall	Bottom Face of Girder	A = 0.01 m ²	Slight	One	с	140
132	7	PSC Girder, G-1	SER	s	Exposed Rebar	Outer Face of Girder @ Midspan	A = 0.01 m ²	Slight	One	c	128
133	8	Approach Slab	CR	н	Random Cracks	Wearing Course of Pav. near Abut. "B"	i = 12 mm	Heavy	Малу	а	21
134	8	Railing	SER	S	Spall	Bottom Face of Railing @ DS	A = 0.096 m ²	Slight	One	с	56
135	8	Railing	SER	s	Spall	Bottom Face of Railing @ DS	A = 0.095 m ²	Slight	One	с	58
136	8	Railing and Post	SER	S	Spall	Face of Post and Railing @ DS	A = 0.04 m ²	Slight	Two	с	22
137	8	Rail Post	SER	S	Spall	Face of Post @ Upstream	A = 0.09 m ²	Slight	One	с	19
138	8	Rail Post	SER	s	Exposed Rebar	Face of Split Post @	A = 0.03 m ²	Slight	One	c	55
139	8	Rail Post	SER	s	Exposed Rebar	Downstream Face of Split Post @	$A = 0.05 m^2$	Slight	One	c	57
140	8	Curb	SER	s	Spall	Downstream Top pf Abut. "8" @	A = 0.03 m ²	Slight	One	c	20
141	8	Abutment "B", End	CR	s	Crack	Exp. Joint Face of Diaphragm	t = 0.127 mm	Slight	One	c	59
	-	Diaphranm (Cross Beam) Abutment "B", End				near G-1 @ Bay 1 Face of Diaphragm			0.10	~	

VISUAL INSPECTION REPORT (1/5) BRIDGE INVENTORY

Nam	e of Bridge	. <u>Pa1.2 D</u>	elpan Brido	e (Downstream)								Reference Inventory Inventory	Date	Pa1.2 Dec. 13, 14 KEI - DCCI	, 19, 20, 2002 D	
Brid	де Туре	AASHTC	& PC Ger	ber Box Girders (5	-span)	Bridge Len	ngth	202	.90 m			Span Leng	gth	26.65/46.00/	57.60/46.00/26.65	
Nam	e of Road	Bonifacio	Drive		Location	Port Area, I	Manila			Chaina	age					
	Road Wid	lh (m)									Туре	Wall Type				
peo	Lane		No.	4	Width	17.32	(m)				Wall	Height	(4.80)	(m), Width	1.38 (0.46+0.92)	(m
5	Sidewalk		Туре	Concrete	Width	1.61/1.60	(m)		Abut	lment	Footing	Length	19.28	(m), Width	(5.77)	(m
Approach Road	Median		Туре	-	Width		(m)				Foundation	Туре	(PSC Pile)	Length	A1= (28.0) A2 = (15.5)	(m
Ą	Pavement		Туре	Concrete	Thickness		(cm)] ·			Туре	Wali Type				
	Traffic Vol	ume			Bo	oth Direction	/day]	1		Coping	Height		(m), Width		(m
	Alignment		Skew	30°	Curve	None					Column (Wall)	Height	(11.055)	(m), size	P1/P4 (1.8-2.30) P2/P3 (2.0-3.0)	(m)
			Туре	PC Gerber Box (Girder/AASH	TO T-IV]	Pier		Footing	Length	(19.63)	(m), Width	P1/P4 (8.0) P2/P3 (10.0)	(m)
	Main Girde	ər	Height	3.67 m (at Pier)/	1.640 m (at r	nid-span)/1.	39 m				Foundation	Туре	(PSC Pile & Bored Pile)	Length	P1/P2 (23.0) m P3 (20.0) m P4 (10.5) m	
			Number	(1) PC Gerber B	ox Girder/(7)	AASHTO T	-IV	1		Specifi	ication	(AASHTO	1973 & 1977 6	ditions)		
g			Space	2.75 m for AASH	TO T-IV		(m)	1	5	Live Lo	bad	(MS 18)				
rich.	Cross Bea		Туре	Concrete Diaphr	agm (AASH1	TO T-IV)		1	Design	Seismi	c Coefficient	0.10 (DL +	+ 1/2 LL)		1.72	
Superstructure			No.	4						Design	Date	(1982)				
Sup	Stringer		Туре	-				1		Concre	ete	fc' (21 MP	a, Structural Co	oncrete)	••••••	
	Sunger		No.	•				1	io	Reinfo	rcing Bar	fy (Grade	40, 275 MPa)			
	Pavement		Туре	AC	Thickness	5.0	(cm)	1	truct	P.C. M	aterial	f"c" (35 MP	a), f'su (1862 M	1Pa/1750 MP	a for PSC Piles)	
	Slab		Туре	Concrete	Thickness	20/28	(cm)]	Construction	Steel N	laterial					
	Shoe		Туре	Steel Bearing/ Elastomeric Pad	Reaction	387	(t)			Comple	etion Date	((1988))				
	Expansion	Joint	Туре	Steel						Po	marks	() Design	n Dimension			<u> </u>
	Handrail		Туре	Concrete						Rei	Indias	(()) Assu	mption Data			

Nam	e of Bridge ;	<u>Pa2 Jor</u>	nes Bridge		•							Reference Inventory Inventory	Date	Pa2 Dec. 2, 3, 4 KEI - DCCC	, 10 & 11, 2002	
Bridg	де Туре	Steel Pla	te Girder			Bridge Length		114.	41 m			Span Leng	gth	35.51/43.40	/35.50	
Nam	ne of Road				Location					Chaina	ige					
	Road Width	h (m)	2	1.15/21.22							Туре	Wall Type				
oad	Lane		No.	4	Width	7.60 to 7.75	(m)	1			Wall	Height	((5.80))	(m), Width	1.45	(m)
Approach Road	Sidewalk		Туре	Concrete	Width	2.35	(m)	1	Abut	ment	Footing	Length	21.80	(m), Width	((5.50))	(m)
road	Median		Туре	Concrete Curb	Width	1.22 (ave.)	(m)	1			Foundation	Туре	((Spread))	Length	21.80	(m)
App	Pavement		Туре	AC	Thickness		(cm)	1			Туре	Wall Type				
	Traffic Volu	me			B	oth Direction/da	у	1	1		Coping	Height		(m), Width		(m)
	Alignment		Skew	Normal	Curve	None		1	Pier	1/2	Column(wall)	Height	(9.00)	(m), size	1.26 / 1.82 / 4.12	(m)
			Туре	Steel Plate				1			Footing	Length	27.59	(m), Width	(8.70 TOP/10.20 BOT)) (m)
	Main Girde		Height	1.835 (exterior);	1.320 (interi	or)		1			Foundation	Туре	(Caisson)	Length	(31.80)	(m)
	Wibin Gilder		Number	8		~				Specifi	cation	Unknown	((AASHO))			
			Space	2.9			(m)	1	5	Live Lo	bad	((MS 18))				
ure	Cross Bear	_	Туре	None					Design	Seismi	c Coefficient	((No Spec	ific Provisio	n))		
Superstructure	Cross Bear	n	No.	None						Design	Date	Unknown				
pers	Stringer		Туре	None						Concre	ele	fc' ((21 MF	Pa)), SLAB /	((17 MPa)),	Substructure	
ŝ	aunger		No.	None					l ig	Reinfo	rcing Bar	fy ((275 M	Pa)), SLAB	/ ((228 MPa)), Substructure	
	Pavement		Туре	AC	Thickness	5.00	(cm)		2 2	P.C. M	aterial					
	Slab		Туре	Reinf. Conc.	Thickness	20.00	(cm)		Construction	Steel N	laterial	fy ((228 M	Pa))			
	Shoe		Туре	Steel	Reaction	160	(t)			Comple	etion Date	((1948))				
	Expansion .	Joint	Туре	Steel						0		() As Bu	uilt Data			
	Handrail		Туре	Concrete Precas	t					Ker	narks	(()) Assu	mption Data		•	

Nam	e of Bridge : <u>Pa3</u>	McArthur Bri	dge								Reference Inventory I Inventory C	Date	Pa3 Dec. 10 - 13 KEI - DCCD		
Brid	ge Type Conti	nuous Steel P	late Girder		Bridge Le	ngth	114.0	60 m.			Span Lengt	h	37.30/40.30	/37.00	
Nam	e of Road Rizal	St.		Location	Manila				Chaina	age					
	Road Width (m)	17.60 n	1							Туре	Wall Type				
peo	Lane	No.	4	Width	3.35	(m)	1			Wall	Height	((7.5))	(m), Width	1.75	(m)
е К	Sidewalk	Туре	Concreite	Width	1.80	(m)		ADU	ment	Footing	Length	((17.00))	(m), Width	((7.00))	(m)
Approach Road	Median	Туре	Concrete Curb	Width	0.60	(m)				Foundation	Туре	((Timber Pile))	Length	((15.00 min.))	(m)
App	Pavement	Туре	AC	Thickness		(cm)				. Туре	Wall Type				
	Traffic Volume			. В	oth Directio	n/day		1		Coping	Height	-	(m), Width	-	(m)
	Alignment	Skew	(90°) Normal	Curve	None		1	Pier	1/2	Column	Height	3.35/2.26/((2.89))	(m), size	1.22/1.83/4.83	(m)
		Туре	Steel Plate				1			Footing	Length	(25.00))	(m), Width	((8.00))	(m)
	Main Girder	Height	1.72 m.				1			Foundation	Туре	((Timber Pile))	Length	((15.00 min.))	(m)
	Wall Gilder	Number	7				1		Specif	ication	Unknown ((AASHO))			
		Space	2.50 m			(m)		ß	Live Lo	bad	((MS 18))				
ure	Cross Beam	Type	None					Design	Seismi	ic Coefficient	((No Specil	ic Provision))			
Superstructure	Ciuss Bealli	No.	None						Design	n Date	Unknown				
Siecs	Stringer	Туре	None						Солсте	ele	fc' ((21 MPa	a, Slab/17 MPa, Sub	ostructure))		
Suj	Singer	No.	None					Construction	Reinfo	rcing Bar	fy ((275 MP	a, Slab/228 MPa, S	ubstructure))		
	Pavement	Type	AC Overlay	Thickness	5.0	(cm)		Inc	P.C. M	laterial					
	Slab	Туре	RC Concrete	Thickness	25.0	(cm)		Sons	Steel A	Aaterial	fy ((228 MP	a, A33))			
	Shoe	Туре	Steel	Reaction	146	(t)		0	Compl	etion Date	((1948))				
	Expansion Joint	Туре	Steel						Dee		() As Bui	lt Data			
	Handrail	Туре	Steel Pipe						Ken	narks	(()) Assur	nption Data			

Appendix 7.1.2-1 (2/7)

VISUAL INSPECTION REPORT (1/5) BRIDGE INVENTORY

Pa4

Reference No

Pa5

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Reference No. Inventory Date Pa4 Nov. 27, 28, Dec. 2-5, Dec. 12-14, 16, 18, 2002 KEI - DCCD Name of Bridge : Pa4 Quezon Bridge Inventory Office Bridge Type Single Steel Type Arch Bridge Bridge Length 102.40 m Span Length 102.40 m Name of Road Location Chainage Road Width (m) 20.60 m Type Wall Type Approach Road Lane Width 4.00 Wall No. 4 (m) Height ((9.00)) (m), Width ((3.50)) (m) Abutment Sidewalk Concrete Туре Width 1.80 (m) Footing Length ((27.00)) (m), Width ((13.00)) (m) Median Туре Curb Barrier Width 1.00 (m) Foundatio Туре ((Timber Pile)) Length ((15.00)) (m) Pavement Туре AC Overlay Thickness (cm) Туре Traffic Volume Both Direction/day Coping Height (m), Width (m) Skew Alignment Normal Curve Pier Column None Height (m), size (m) Steel Arch Type Footing Length (m), Width (m) 15.04 m Height Foundation Туре Length Main Girder Number 3 Specification ((AASHO)) Space 9 (m) Design Live Load ((MS18)) I-Section ((No Specific Provision)) Туре Seismic Coefficient Cross Beam 17 pieces Unknown No. Design Date Superst Туре I-Section Concrete fc' ((21.0 MPa, slab/17.0 MPa, Substructure)) Stringer No. 12 pieces Construction Reinforcing Bar fy (275 MPa, slab)/((228 MPa, Substructure)) Pavement Туре AC Overlay Thickness 2.50 P.C. Material (cm) Slab RC Concrete fy ((228 MPa)) Туре Thickness 18.00 (cm) Steel Material Shoe Steel ((1946)) Reaction 760 Туре Completion Date (t) Expansion Joint () As-built Data Туре Steel Remarks Handrail Steel Railings Туре (()) Assumption Data

Nam	e of Bridge :	Pa5 Na	gtahan Bridg	je ·								Inventory Inventory		Dec. 10-12, KEI - DCCD		
Brid	де Туре	3-Span C	Continuous :	Steel Truss		Bridge Ler	ngth	148.93	3 m			Span Leng	th	45.60/57.73	/45.60	
Nam	e of Road	Nagtaha	n Street		Location	Sta. Mesa,	Manila			Chaina	age					
	Road Widt	h (m)	24	.62 / 29.94							Туре	Wali Type				
load	Lane		No.	6	Width	3.50	(m)	A	Abuti	ment	Wall	Height	((15.18))	(m), Width	1.20	(m)
сh	Sidewalk		Туре	Concrete	Width	1.00	(m)	F	Pier	1/4	Footing	Length	((26.0))	(m), Width	((5.0))	(m)
Approach Road	Median		Туре	Concrete Curb	Width	0.62/0.94	(m)				Foundation	Туре	((Spread))	Length	((26.0))	(m)
App	Pavement		Туре	AC	Thickness	_	(cm)				Туре	Wall Type				
	Traffic Volu	ume			B	oth Direction	n/day				Coping	Height	1.31	(m), Width	2.36	(m)
	Alignment	ad Width (m) e ewalk dian erement fric Volume n Girder ss Beam	Skew	11°	Curve	None		1	Pier	2/3	Column (Wall)	Height	2.475/((9.49))	(m), size	1.20/2.10) (m)
		e walk ian	Туре	Steel Truss							Footing	Length	((30.00))	(m), Width	((8.00))	(m)
	Main Girde		Height	4.17 at Pier/2.40) m at 🤤						Foundation	Туре	((Timber Pile))	Length	((10.00))	(m)
	Wall Glue		Number	10						Specifi	cation	Unknown	((AASHO))			
			Space	2.52			(m)		esign	Live Lo	ad	((MS18))				
fure	Cross Base	~	Туре	1-Beam					8 8	Seismi	c Coefficient	0.06 DL				
Superstructure	CIUSS Deal		No.	2 (Both Abutme	nt)	· · ·				Design	Date	Unknown				
Siecs	Stringer		Туре	- 1						Concre	te	fc' ((21 MF	a)), Slab/((17 M	Pa)), Substru	icture	
Sul	Juniger	rement	No.	-					ction	Reinfo	rcing Bar	fy ((275 M	Pa)), Slab/((228	MPa)), Subs	ructure	
	Pavement	ss Beam	Туре	AC Overlay	Thickness	((5.0))	(cm)			P.C. M	aterial					
	Slab	ger r ment 7	Туре	RC Concrete	Thickness	((20.0))	(cm)		Constru	Steel N	laterial	fy ((228 M	⊃a)) .			
	Shoe		Туре	Steel Bearing	Reaction	190	(t)		~ ľ	Compl	etion Date	((1966))			
	Expansion	Joint	Туре	Steel						De	marka	() As B	uilt Data			
	Handrail		Туре	Steel						RE	marks	(()) Assu	mption Data			

Nam	e of Bridge :	<u>Pa6 Pa</u>	ndacan Bri	idge								Referenc Inventory Inventory	Date	Pa6 Dec. 6 - 9, 2 KEI - DCCD		
Brid	ge Type 🛛 F	PC AASH	TO Girder	Bridge (5-span)		Bridge Le	ngth	147.4	40 m			Span Leng	gth	23.80/25.00/4	6.00/25.10/27.50	<u>,</u>
Nam	ne of Road				Location	Pandacan	, Manila			Chaina	age					
	Road Width	(m)		16.70 m							Туре	Column Be	ent with Diaphra	ym Wali		
pec	Lane		No.	4	Width	3.25	(m)	1			Wall (Column)	Height	((5.5/7.5))	(m), Width	1.60	(m)
ЧÄ	Sidewalk	Road Vidth (m) ine dewalk sedlan sement affic Volume growenent sent set sement set sement set sement set sement set sement set sement set set set set set set set set set se	Туре		Width	1.20	(m)		Abut	ment	Footing	Length	((17.50))	(m), Width	((7.00/8.00))	(m
Approach Road	Median		Туре	Concrete Curb	Width	1.10	(m)				Foundation	Туре	((PSC Pile/ Spread))	Length	((10.0/17.50))	(m
Αp	Pavement		Туре	AC	Thickness		(cm)				Туре	Column Be	ent			
	Traffic Volun	ne			Bi	oth Directio	n/day				Coping	Height	1.71	(m), Width	2.60	(m
	Alignment		Skew	Normal	Curve	None			Pier		Column	Height	((15.00))	(m), size	1.60	(n
			Туре	AASHTO Girder							Footing	Length	-	(m), Width	-	(m
	Main Girder		Height	2.30 m, Span 3/	1.35 m, othe	r Span					Foundation	Туре	Bored Pile	Length	((15.00))	n)
	INGIT OTOD		Number	6] [Specif	ication	((AASHTO	2))			
	· .		Space	2.60			(m)		ъ	Live Lo	bad	((MS 18))				
g	Crean Dear		Туре	Concrete Diaphr	agm				Design	Seismi	ic Coefficient	0.10 (DL -	+ 1/2 LL)			
Superstructure	Cross Beam		No.	12 Intermediate/	6 End Diaph	ragm				Design	n Date	Unknown				
ersti	Chain and		Туре	-		4		1		Concre	ete	fc' ((21 MP	a, Slab & Subst	ucture/28 MPa	a Piles))	
Sup	Stringer		No.	-					ion	Reinfo	rcing Bar	fy ((414 Mp	oa, Slab & Subst	ructure))		-
	Pavement		Туре	AC	Thickness	5.0	(cm)		Construction	P.C. M	laterial	fc' ((35 MP	a)), f'su (1862 M	Pa)		_
	Slab	b	Туре	Reinf. Conc.	Thickness	18.0	(cm)		oust	Steel N	daterial			· · ·		_
	Slab Shoe	Туре	Elastomeric Bearing Pad	Reaction	200	(t)		0	Compl	etion Date	1997				_	
	Main Girder II Cross Beam 1 Stringer N Pavement 1 Slab 1 Shoe 1 Expansion Joint 1	Туре	Steel						Po	marks	()AsB	luilt Data				
	Handrail		Туре	Concrete						Ļ.	nidi Na	'(()) Ass	umption Data			

VISUAL INSPECTION REPORT (1/5) BRIDGE INVENTORY

							SKIDU			NIO	XI	Referen	ce No.	Pa7		
√am	e of Bridge :	Pa7 Lam	bingan Bri	dge								Inventor Inventor		Dec. 4 - 5, KEI - DCCI		
Brid	де Туре	PC Gerbe	r I Girder I	Bridge (3-Span)		Bridge Le	ngth	98.1	0 m			Span Ler	ngth	18.50/61.1	0/18.50	<u> </u>
Nan	ne of Road	New Pana	aderos St.		Location	Manila				Chaina	age			.t		
	Road Width	n (m)		23.75 m							Туре	Wall Type	•			
Approach Road	Lane		No.	4 lanes	Width	5.00	(m)		A	ment	Wall	Height	((8.20))	(m), Width	1.27	(m)
5	Sidewalk		Туре	Concrete	Width	1.30	(m)		AUUL	ment	Footing	Length	((24.0))	(m), Width	((6.0))	(m)
road	Median		Туре	Concrete Curb	Width	1.00	(m)				Foundation	Туре	((Steel Pile))	Length	i ((15.00))	(m)
App	Pavement		Туре	AC.	Thickness		(cm)				Туре	Wall Type)			
	Traffic Volu	me			B	oth Directio	n/day				Coping	Height	-	(m), Width	•	(m)
	Alignment		Skew	Normal	Curve '	None			Pier	1/2	Column (Wali)	Height	4.90/((6.30))	(m), size	1.40/4.60	(m)
			Туре	PC Gerber I Gire	der						Footing	Length	((28.0))	(m), Width	((8.0))	(m)
	Main Girde		Height	1.829 m							Foundation	Туре	((Steel Pile))	Length	((15.0))	(m)
			Number	12					_	Specifi	cation	((AASHT	⁻ O))			
			Space	2.00			(m)		Design	Live Lo	ad	((MS 18))			
nre	Cross Bear	n	Туре	Concrete Diaphi	ragm				Des	Seismi	c Coefficient	((EQ=0.10	0 (DL + 0.5LL) m	in.		
Superstructure	01033 0081	"	No.	8 Interior, 1 each	h Abut. & Pie	r				Desigr	Date	Unknow	n			
erst	Stringer		Туре	-						Concre	ite	fc' ((21 M	Pa, Structural Co	ncrele))		
Sup Sup	Sunger		No.	-					tion	Reinfo	rcing Bar	fy ((275 M	Pa Structural Co	ncrete))		
	Pavement		Туре	AC Overlay	Thickness	4.0	(cm)		Construction	P.C. M	aterial	fc' ((35 M	Pa)), f'su ((1862	MPa)		
	Slab		Туре	Concrete	Thickness	20.0	(cm)		suo	Steel N	laterial	fy ((248 M	(Pa, A36))			
	Shoe		Туре	Elastomeric Bearing Pad	Reaction	300	(t)		0	Compl	etion Date	1979				
	Expansion .	Joint	Туре	-						Re	marks	() As E	Built Data			
	Handrail		Туре	Steel						,110	mano	(()) Ass	umption Data			

Reference No.

Pa8

Ç

Nam	e of Bridge :	Pa8_Mak	ati-Mandal	uyong Bridge								Inventor		Nov. 26, 28 KEI - DCCI	3, Dec. 2, 3, 2 D	2002
Brid	де Туре	PC Gerbe	er I Girder E	Bridge (3-Span)	•	Bridge Lengt	th	110	.00 m			Span Ler	ngth	30/50/30	 I	
Nam	e of Road	Burgos S	t. Ext'n.		Location	Makati City				Chaina	age					
· ·	Road Widt	th (m)	16.4	5 m / 23.71 m					1		Туре	Column B	ent with Diaphra	ıgm Wali		
oad	Lane		No.	4	Width	3.85 to 5.00	(m)		Abutrr Pier N		Wall, Column	Height	(4.50)	(m), Width	(1.8 x 2.0)	(m)
۲ ۳	Sidewalk		Туре	Concrete	Width	0.25	(m)	1	1 & 4	J.	Footing	Length	(17.0)	(m), Width	(6.0/5.8)	(m)
Approach Road	Median		Туре	Concrete Curb	Width	1.00	(m)				Foundation	Туре	(Spread/Pile)	Length	(17.0/28.6)	(m)
App	Pavement		Туре	AC	Thickness		(cm)	I			Туре	Column B	ent			
	Traffic Vol	ume			В	oth Direction/d	ay	1			Coping	Height	(2.00)	(m), Width	(2.00)	(m)
	Alignment		Skew	Normal	Curve	None			Pier M 2 & 3	о.	Column	Height	(27.0)	(m), size	(2.0 Ø)	(m)
			Туре	PC Box Girder /	AASHTO Ty	rpe V			203		Footing	Length	•	(m), Width	-	(m)
	Main Girder	>r	Height	1.92 m / 1.60 m							Foundation	Туре	(Bored Pile)	Length	(27.0)	(m)
	intent ende		Number	1/9						Specifi	cation	(AASHT	D)			
			Space	9 @ 2.06, AASH	ITO Girder		(m)		uĝ	Live Lo	bad	(MS 18)		-		
ture	Cross Bea		Туре	Concrete Diaphi	agm				Design	Seismi	c Coefficient	((EQ = 0.1	i0 (DL + 0.5 LL) n	nin.		
Superstructure	Cross bea	m	No.	6						Design	Date	Unknowr	ı ·			·
pers	Stringer		Туре	None						Concre	ete .	fc' (21 MF	a, Structural Con	crete)		
Su	Stringer		No.	None					tion	Reinfo	rcing Bar	fy (420 M	Pa)			
	Pavement		Туре	None	Thickness	None	(cm)		struc	P.C. M	aterial	fc' ((35 N	IPa), l'su ((1862	MPa)		
	Slab		Туре	Reinf. Conc	Thickness	22.0	(cm)		Construction	Steel N	laterial					
	Shoe		Туре	Bearing Shoe	Reaction	225	(t)		Ĭ	Compl	etion Date	1986				
	Shoe Expansion Joint	Joint	Туре	Steel Finger Typ	e					Dies	marks	() As	Built Data			
	Handrail		Туре	Steel						Rei	naiks	(())As	sumption Data			

Nam	e of Bridge : <u>10,1 (</u>	<u>Guadalupe i</u>	<u> Bridge (Central)</u>	_							Reference No. Inventory Date Inventory Office	Pa9.1 December 13 KEI - PROCO		
Brid	ge Type 3-Span	Continuous	Truss Bridge		Bridge Length		144	.44 m)		Span Length			
Nan	e of Road EDSA			Location	Makati City &	Manda	luyon	g Çity	Chain	age		35.70 / 42.80	/ 35.94	
	Road Width (m)	32	2.70 / 36.33							Туре	Wall Type			
oad	Lane	No.	5	Width		(m)	1	1.		Wall	Height (3.91) (3.927)	(m), Width	(1.927), (2.05 ave.)	(m)
5	Sidewalk	Туре	Concrete	Width	1.28 / 3.50, 3.20	(m)	1	Abu	tment	Footing	Length 26.60	(m), Width	(5.037), (4.56)	(m)
Approach Road	Median	Туре	Concrete	Width	-/6.95	(m)	1			Foundation	Type (Spread, Timber Pile)	Length	((10))	(m)
App	Pavement	Type	Concrete	Thickness	((23))	(cm)	1			Туре	Wall Type			
	Traffic Volume			В	oth Direction/da	y	1			Coping	Height ((1.25))	(m), Width	(1.134)	(m
	Alignment	Skew	90°	Curve			1	Pie	r	Column	Height (9.197)	(m), size	(1.517) ave.	(m)
		Туре	Steel Truss				1			Footing	Length ((29.00))	(m), Width	(6.30)	(m)
	Main Girder	Height	2.19 m				1			Foundation	Type (Spread, Timber Pile)	Length	(10)	(m)
	Wall Groen	Number	10 per span				1.		Specif	ication	(AASHO 1944)			
		Space	2.637			(m)	1	5	Live Lo	bad	(H 20 - 44)			
Пē	Cross Beam	Туре	Steel				1	Design	Seism	ic Coefficient	0.06 DL			
Superstructure	Cross Beam	No.	15				1	-	Design	Date	(1947)			
Sers	Stringer	Туре	-				1		Concre	ete	f'c (21 MPa)			
S,	Sunger	No.	-	-				tion,	Reinfo	rcing Bar	fy (275 Mpa, 0	Grade 40)		
	Pavement	Туре	Asphalt	Thickness	((5))	(cm)	ĺ	Construction	P.C. N	laterial	f'c -			
	Slab	Type	Concrete	Thickness	((18))	(cm)	1	ő	Steel N	/laterial	fy (248MPa)			
	Shoe	Туре	Steel	Reaction	108.65 (max.)	(t)	1	10	Constr	uction Date	((1962))			
	Expansion Joint	Туре	Steel	· · · ·			1				() as-built data			
	Handrail	Туре					1		Rei	marks	(()) assumption data			

Appendix 7.1.2-1 (4/7)

VISUAL INSPECTION REPORT (1/5) BRIDGE INVENTORY

B.4.

										Referen		Pa9.2		
	a of Deidoo - 10.0			a						Invento		December		
vam	e of Bridge : 10.2	Suadatupe E	noge (Both Si	de, Opstream)						invento	ry Office	KEI - PRO	CONSULT	
Brid	ge Type PC Ge	rber Girder B	ridge (3-Span)		Bridge Lengt	h	144	.44 m		Span Le	ingth			
Nam	ne of Road EDSA			Location	Makati City 8	Manda	luyon	g City Cl	ainage			35.70 / 42.	80 / 35.94	
	Road Width (m)	1	6.50 / 17.63					1	Туре		Wall Type			
beo	Lane	No.	5	Width		(m)	1	Abutme	Wall	Height	(4.75)/(4.60)	(m), Width	(1.22)	n)
Approach Road	Sidewalk	Туре	Concrete	Width	~/ 3.20	(m)	1	Abume	Footing	Length	9,59	(m), Width	(6.00) / (5.00)	(n
ga	Median	Туре	Concrete	Width	-/6.95	(m)	1		Foundation	Туре	(PSC Pile)	Length	(20.20)	(m
App	Pavement	Туре	Concrete	Thickness	((23))	(cm)	1		Туре		Wall Type			
	Traffic Volume			Bo	oth Direction/d	ay	1		Coping	Height	(1.20)	(m), Width	1.15	n)
	Alignment	Skew	90°	Curve			1	Pier	Column	Height	10.80	(m), size	(1.49) ave.	(n
		Type	Prestressed	Concrete Girder]		Footing	Length	(11.247)	(m), Width	(7.00) max.	(п
	Main Girder	Height	1.68 m				1		Foundation	Туре	(PSC Pile)	Length	(16)/(21)	(л
	Wall Onder	Number	4 per span					Sp	ecification	(AASHT	O 1973 / DPW	/H 1972)		
		Space	2.40			(m)		Design Fiv	e Load	(HS 20 -	44)			
ure	Cross Beam	Туре	Concrete					a Se	ismic Coefficient	0.10(DL-	+1/2 LL)			
Superstructure	CIUSS Dealli	No.	6 Intermediat	le / Span				De	sign Date	(1979)				
Ders	Stringer	Туре	•				1		ncrete	f'c	(21 MPa)			
Sul	Stillger	No.	-				1	Construction	inforcing Bar	fy	(275 Mpa, 0	Grade 40)		
	Pavement	Туре	Asphait	Thickness	((5))	(cm)	1	2 P.0	C. Material	fc	(35 Mpa),	ſsu	(1862 Mpa, Grade 27	0)
	Slab	Туре	Concrete	Thickness	(20)	(cm)		Š Sta	el Material	fy	(248MPa)			
	Shoe	Туре	Steel & Elast	omeric Reactic	in 150 (max.)	(t)			nstruction Date	((1979))			
	Expansion Joint	Туре	Steel						Remarks	() as-bu	it data			
	Handrail	Туре	Concrete						incinaina	(()) assu	mption data			

Nam	e of Bridge :	<u>10.2 Gua</u>	dalupe Bri	idge (Both Side,	, Downstrear	<u>n)</u>						Referen Inventor Inventor	y Date		r 13, 2002 DCONSULT	
Brid	зе Туре	PC Gerb	er Girder Bi	ridge (3-Span)		Bridge Length	n	144.4	4 m			Span Lei	ngth			
Nam	e of Road	EDSA			Location	Makati City &	Manda	luyong	City	Chaina	ige		-	35.70/42	2.80 / 35.94	
	Road Widt	h (m)	16	.20 / 18.70							Туре		Wall Type			
Approach Road	Lane		No.	5	Width		(m)	1	A		Wall	Height	(4.75)/(4.60)	(m), Widt	h (1.22)	(m)
5	Sidewalk		Туре	Concrete	Width	1.28/3.50	(m)	1 ľ	Abuti	neni	Footing	Length	8.90	(m), Widt	h (6.00) / (5.00)	(m)
road	Median		Туре	Concrete	Width	- / 6.95	(m)	1			Foundation	Туре	(PSC Pile)	Leng	th (20.20)	(m)
App	Pavement		Туре	Concrete	Thickness	((23))	(cm)	1 [Туре		Wall Type			
	Traffic Vol	ume			В	oth Direction/da	ay				Coping	Height	(1.20)	(m), Widtl	h 1.15	(m)
	Alignment		Skew	90°	Curve			1	Pier		Column	Height	10.80	(m), size	(1.49) ave.	(m)
	Aignmeni		Туре	Prestressed Co	oncrete Girde	•		1			Footing	Length	(11.247)	(m), Widtl	n (7.00) max.	(m)
	Main Girde		Height	1.68 m							Foundation	Туре	(PSC Pile)	Lengt	n (16)/(21)	(m)
		21	Number	4 per span				1 [Specifi	cation	(AASHTO	0 1973 / DPW	'H 1972)		
			Space	2.40			(m)	1	<u>p</u>	Live Lo	ad	(HS 20 -	44)			
ure	Cross Bea		Туре	Concrete					Design	Seismi	c Coefficient	0.10(DL+	·1/2 LL)			
Superstructure	Cross Bea	m	No.	6 Intermediate	/ Span			1 [_	Design	Date	(1979)				
siac	Stringer		Туре	-				1		Concre	te	fc	(21 MPa)			
ľ.	Sunger		No.	-				1	lion I	Reinfo	cing Bar	fy	(275 Mpa, C	Grade 40)		
	Pavement		Туре	Asphalt	Thickness	((5))	(cm)	1	ž į	P.C. M	aterial	f'c	(35 Mpa),	f'su	(1862 Mpa, Grade 270	<i>"</i>
	Slab		Туре	Concrete	Thickness	(20)	(cm)	1	Construction	Steel N	laterial	fy	(248MPa)			
	Shoe		Туре	Steel & Elaston	neric Reactio	on 150 (max.)	(t)	1		Constr	uction Date	((1	1979))			
	Expansion Join Handrail	Joint	Туре	Steel				1 1		Dee		() as-buil	t data			
			Туре	Concrete				1		Ren	narks	(()) assu	mption data			

Nam	e of Bridge :		11.0 C-5 E	BRIDGE	<u>. </u>								nce No. ry Date ry Office	Pa10 Nov. 27 - [KEI - PRO	Dec. 12, 2002 CONSULT	
Bridg	ge Type	PCI Gird	er Bridge (3	-Span)		Bridge Length	I	272.	96 m			Span Le	ingth	24.85/24.	95 / 25.12 / 25.00 / 24.8	
Nam	ne of Road	C-5			Location	Pa	sig City			Chaina	ige			45.88 / 22.	21/26.95/26.70/26.4	15
	Road Widtl	h (m)	22	.55 / 14.50							Туре		Wall Type			
beo	Lane		No.	3	Width	3 - 3.75	(m)	1	A h		Wali	Height	A (4.157). B (6.55)	(m), Width	A (1.10), B (1.20)	(m)
Approach Road	Sidewalk		Туре	Concrete	Width	5.00,3.00	(m)	1	Abul	ment	Footing	Length	(28.70)	(m), Width	A (5.00), B(6.75)	(m)
road	Median		Type	Concrete	Width	3.00 / 1.24	(m)	1			Foundation	Туре	(Spread. Bored Pile)	Length	n (28.70)/(28.70)	(m)
App	Pavement		Туре	Asphalt	Thickness	((5))	(cm)				Туре		(Pier Pile Be	nt)		
	Traffic Volu	ime			Bo	oth Direction/da	у				Coping	Height	2.04 / 2.60	(m), Width	1.72 / 2.60	(m)
	Alignment		Skew	90°	Curve				Pier		Column	Height	(5.26 / 6.482)	(m), size	1.50 / 2.50	(m)
			Туре	AASHTO GIRE	ER TYPE IV	& VI (Modified)				ĺ	Footing	Length	None	(m), Width	None	(m)
	Main Girde	•	Height	1.372 m / 2.10	m						Foundation	Туре	(Bored Pile)	Length	(12.74 - 41.018)	(m)
	Wall Gude	•	Number	12 per span	÷			1		Specifi	cation ·	(AASHT	O 1992)			
			Space	2.30			(m)		Ē	Live Lo	ad	(HS 20 -	44)			
nre	Cross Bear		Туре	Concrete					Design	Seismi	c Coefficient	0.10(DL-	+1/2 LL)			
Superstructure	CIUSS Deal		No.	209						Design	Date	(1996)				
Sie	Chairman		Туре	-						Concre	le	f'c	(21 MPa)			
Suj	Stringer		No.	-					clion	Reinfo	cing Bar	fy	(275 Mpa, C	Grade 40)		
	Pavement		Туре	Asphalt	Thickness	((5))	(cm)			P.C. M	aterial	ſc	(35 Mpa),	f'su	(1862 Mpa, Grade 270	2)
	Slab		Туре	Concrete	Thickness	(25)	(cm)		Construc	-Steel M	laterial	fy	((248 Mpa))			-
	Shoe		Туре	Neoprene	Reaction	114.41 (max.)	(1)			Constr	uction Date	(1998)			
	Expansion .	Joint	Туре	Steel							narks	() as-bu	ilt data			
	Handrail		Туре	Concrete						Ren		(()) assu	mption data			

A.7 - 38

VISUAL INSPECTION REPORT (1/5) BRIDGE INVENTORY

						PUING		A V EI	NIOR	T					
											Referen Inventor		Pa11 November		
Nam	e of Bridge :	12.0 Bamba	ing Bridge	_							Inventor	y Office	KEI - PRO	CONSULT	
Brid		der Bridge (3	3-Span)		Bridge Leng	lh	163.	.32 m			Span Le	ngih	12.00 / 11.	65 / 11.70 / 25.90 / 40.19	97
Nan	ne of Road A. Luna			Location	F	asig City			Chaina	age			25.93 / 12.	15 / 11.95 / 11.85	
-	Road Width (m)		0.05/9.75							Туре	Wa	alf Type			
Road	Lane	No.	3	Width		(m)		Abut	meni	Wall	Height	1.00	(m), Width	0.80	(m)
÷	Sidewalk	Туре	Concrete	Width	1.50 / 3.40	(m)		1	IIIÇI K	Footing	Length	None	(m), Width	None	(m)
Approach	Median	Type	-	Width	•	(m)				Foundation	Туре	(PSC Pile)	Lengt	h (25)	(m)
Ap	Pavement	Type .	Asphalt	Thickness	((5))	(cm)				Туре	Colu	тл Туре			
	Traffic Volume			Bo	oth Direction/d	ay				Coping	Height	1.00 / 1.95	(m), Width	1.00 / 1.70	(m)
	Alignment	Skew	90°	Curve				Pier		Column	Height	((4.40)) / 2.98	(m), size	1.00 / 1.60	(m)
		Туре	Channel Beam	n / PCI Girder 1	lype IV & VI			1		Footing	Length	((7.00)) / 9.50	(m), Width	((2.50)) / 5.10	(m)
	Main Girder	Height	0.50 m / 1.37 r	m / 1.82 m				1		Foundation	Туре	(PSC Pile)	Length	(20) / (22)	(m)
		Number	48/8/4						Specifi	cation	(AASHTO	D 1988)			
		Space	1.31/2.45/2	45		(m)		ē	Live Lo	ad	(MS - 18))			
an	Cross Beam	Туре	Concrete					Design	Seismi	c Coefficient	0.10(DL+	1/2 LL)			
Superstructure	Closs Deall	No.	7					·	Design	Date	(1991)				
Sers	Stringer	Type	-					-	Concre	te	f'c	(21 MPa)			
Sri	Sunger	No.	-					tion	Reinfo	cing Bar	fy	(275 Mpa, G	Grade 40)		
	Pavement	Type	-	Thickness	-	(cm)		Construction	P.C. M	aterial	fc	(42 Mpa),	ſsu	(1862 Mpa, Grade 270)	ł
	Slab	Туре	Concrete	Thickness	(27)	(cm)	i	ğ	Steel N	1aterial	fy	((248 MPa))			
	Shoe	Туре	Neoprene	Reaction	100 (max.)	(t)			Constr	uction Date	(1	992)			
	Expansion Joint	Туре	Steel						Roc	narks	() as-buil	t data			
	Handrail	Туре	Concrete ·						Rei	Idina	(()) assu	mption data			

Name	e of Bridge :	13.1 V	argas Brid	ge (Upstream)	-	•						Referer Inventor Inventor	y Date	Ma1.1 November KEI - PROC		
Bridg	де Туре	PC Gerbe	er Girder Br	idge (4-Span)		Bridge Length	1	122.	.44 m)		Span Le	ngth	10.00 / 00 /	50 / 50 60 / 22 04	
Nam	e of Road	Pasig Bo	ulevard Ext	ension	Location	Bagong I	og, Pa	sig Cit	y	Chaina	age			19.307 30.5	507 50.607 22.04	
_	Road Width	1 (m)	7	.35 / 8.58							Туре	W	all Type			
Approach Road	Lane		No.	2	Width		(m)		Abu	tment	Wall	Height	(5.468) / (5.698)	(m), Width	(1.25) ave.	(m)
£	Sidewalk		Туре	Concrete	Width	2.66 / 0.42	(m)		100	unen	Footing	Length	(10.00)	(m), Width	(6.00)	(m)
00	Median		Туре	-	Width	-	(m)				Foundation	Туре	(Steel Pipe Pile)	Length	(22)	(m)
Apr	Pavement		Туре	Asphalt	Thickness	((5))	(cm)	I			Туре	Colu	imn Type			
	Traffic Volu	me			Bo	th Direction/da	iy				Coping	Height	(1.20)	(m), Width	1.60	(m)
	Alignment		Skew	90°	Curve				Pier	r	Column	Height	4,30	(m), size	1.70 ave.	(m)
			Туре	AASHTO Girder	Type V & VI						Footing	Length		(m), Width	5.60	(m)
	Main Girder	-	Height	1.60 m / 1.86 m							Foundation	Туре	(Steel Pipe Pile)	Length	(22)	(m)
			Number	4 per span						Specif	ication	(AASHT	O 1977)			
			Space	2.17			(m)		E,	Live Lo	bad	(MS - 18)			
nre	Cross Beam		Туре	Concrete					Design	Seism	ic Coefficient	0.10(DL+	-1/2 LL)			
Superstructure	CIUSS Deall		No.	12						Desigr	Date	(1991)				
Siece	Stringer		Туре	-						Concre	ete	fc	(20.68 MPa	ı)		
Sug	Sunger		No.	-					ţġ	Reinfo	rcing Bar	fy	(275 Mpa, 0	Grade 40)		
	Pavement		Туре	-	Thickness	-	(cm)		struc	P.C. M	aterial .	fc	(39 Mpa),	f'su	(1862 Mpa, Grade 27	'0)
	Slab		Туре	Concrete	Thickness	((20))	(cm)		Construction	Steel N	/laterial	fy	(248 MPa)			
	Shoe		Туре	Neoprene	Reaction	273.11 (max.)	(t)		۲ ⁰	Constr	uction Date	((1992))			
	Expansion J	loint	Туре	Steel								() as-built o	lata. (()) assump	tion data, Inspe	ction of Span 4 was not po	ssible due
	Handrail	Expansion Joint	Туре	Concrete						Ref	narks	to presence				

Nam	e of Bridge :	_13.2 Var	gas Bridg	e (Downstream)								Referen Inventor Inventor	y Date	Ma1.2 November 26 KEI - PROCO		
Brid	де Туре	Steel Plat	e Girder B	idge (4-Span)		Bridge Lengt	h	142.8	80 m			Span Le	ngth			
Nam	e of Road	Pasig Bou	levard Ext	ension	Location	Bagong I	log, Pas	ig City	у	Chaina	ge	1	-	-30.62730.83	/ 50.70 / 30.65	
	Road Widt	h (m)	10	0.93 / 8.22							Туре	Wa	all Type	.I		
Approach Road	Lane		No.	2	Width		(m)				Wall	Height	(6.50)	(m), Width	(1.40) ave.	(m)
8	Sidewalk		Туре	Concrete	Width	1.21/0.77	(m)		Abut	ment	Footing	Length	(10.00)	(m), Width	(5.50)	(m)
road	Median		Туре	-	Width	-	(m)				Foundation	Туре	((PSC Pile))	Length	((23.50))	(m)
App	Pavement		Туре	Asphalt	Thickness	((5))	(cm)				Туре	Colu	mn Type			
	Traffic Volu	Ime			Bo	oth Direction/da	эу				Coping	Height	1.20	(m), Width	1.67	(m)
	Alignment		Skew	90°	Curve				Pier		Column	Height	(9.00)	(m), size	(1.42) ave.	(m)
			Туре	Steel Plate Gird	er						Footing	Length	(6.80)	(m), Width	(4.00) / (5.20)	(m)
	Main Girde		Height	2.30 m							Foundation	Туре	((PSC Pile))	Length	((20.00 / 18.00))	(m)
	Wein Gilder		Number	3 per span						Specifi	cation	(AASHT	D 1965)			
			Space	3.23			(m)		Б,	Live Lo	ad	(HS 20 -	44)			
ure	Cross Bear		Туре	Steel					Design	Seismi	c Coefficient	0.10(DL+	·1/2 LL)			
Superstructure	Cioss bear		No.	((70))						Design	Date	Unknown	·			
Sers	Stringer		Туре	-				ĺ		Concre	te	f'c	((21 MPa))			
S	Sunnger		No.	-					Į Į	Reinfo	rcing Bar	fy	((275 Mpa,	Grade 40))		
	Pavement		Туре	-	Thickness	-'	(cm)		Construction	P.C. M	aterial	fc	• *			
	Slab		Туре	Concrete	Thickness	20	(cm)		ğ	Steel N	laterial	fy	((345 MPa))).		
1	Shoe		Туре	Steel	Reaction	103.65 (max.)	(1)			Constr	uction Date	((*	1973))			
	Expansion .	Joint	Туре	Steel								() as-built o	lata, (()) assump	tion data, Inspectio	on of Span 1 was not post	sible due
	Handrail		Туре	Concrete						Ren	narks	to presence				

Appendix 7.1.2-1 (6/7)

VISUAL INSPECTION REPORT (1/5) BRIDGE INVENTORY

Nam	e of Bridge :	14.0 Rosa	io Bridge	_	5				NICK		Referen Inventor Inventor	y Date	Ma2 December KEI - PRO	12 - 16, 2002 CONSULT	
Brid	ge Type PCI G	irder Bridge (i	S-Span)		Bridge Lengt	h	175.	.35 m			Span Le	ngth	25.50 / 31.3	20 / 31.19 / 30.98 / 31.0	71
Narr	ne of Road Ortiga	is Ave. Extens	ion	Location	Brgy. Ros	ario, Pa	sig Ci	ity	Chaina	sge			25.41		
	Road Width (m)	16,1	5, 10.85, 14.40				1			Туре	Colu	mn Type	•		
Approach Road	Lane	No.	4,2,2	Width	2.20, 5.42, 7.20	(m)	1	Abut		Wall	Height	(3.127)	(m), Width	(1.60)	(m)
8	Sidewalk	Туре	Concrete	Width	1.80, 0.97, 2.20	(m)	1	14001	ment	Footing	Length	21.76	(m), Width	(4.00)	(m)
load	Median	Туре	-	Width	-	(m)	1			Foundation	Туре		ls, PSC Pile, ubular Pile)	Length ((11))	(m)
App	Pavement	Type	Asphalt	Thickness	((5))	(cm)	1			Туре	Wa	all Type			
	Traffic Volume			В	oth Direction/da	ay]	i i		Coping	Height	1.55 / 1.33	(m), Width	2.30	(m)
	Alignment	Skew	90°	Curve			1	Pier		Column	Height	(4.77)/4.22	(m), size	1.50	(m)
		Туре	AASHTO GIRD	ER TYPE IV	& IV (Modified)]		;	Footing	Length	24.80	(m), Width	(3.90),(3.30)/(4.40),4.00	(m)
	Main Girder	Height	1.40 m / 1.87 m]·	·		Foundation	Туре		ile, PSC Pile, ibular Pile))	Length ((11))/ ((12))	(m)
		Number	10 per span						Specifi	cation	(AASHO	1957 / BPR 1			
		Space	1.98 m (min.) / 2	2.24 m (max.)	(m)		Design	Live Lo	ad	(HS 15 -	44)			
ne	Cross Beam	Type	Concrete					Be	Seismi	c Coefficient	0.06 DL /	0.10(DL+1/2	LL)		
Superstructure	Closs Dealth	No.	216					i I	Design	Date	(1965)				
suac	Stringer	Туре	-						Concre	le	f'c	3,000 psi (2	0.68 MPa)		
Sug	Stringer	No.						tion	Reinfo	cing Bar	fy	12,000 psi ((82.73 MPa)		
	Pavement	Туре	Asphalt	Thickness	((5))	(cm)		tr tr	P.C. M	aterial	fc	5,100 psi ((35 Mpa)),	f'su 18,000 psi (124.10) MPa)
	Slab	Туре	Concrete	Thickness	((18))	(cm)		Construction	Steel N	laterial	fy	36,000 psi (248 MPa)		
	Shoe	Туре	Steel	Reaction	54.52 (max.)	(t)			Constru	uction Date		((1952 / 1978	3))		
	Expansion Joint	Type	Steel						Per	narks	() as-built d	ata, (()) assump	tion data, Inspe	iction of Span 4 was not poss	ible due
	Handrail	Type	Flex Beam Type)					Ren	Idi Ka	to presence	of squatters			

Nam	e of Bridge :	1	5.0 Marco	s Bridge								Referen Inventor Inventor	y Date	Ma3 December KEI - PRO		
Brid	ge Type P	CI Girde	r Bridge (1	1-Span)		Bridge Length	ı	311.	.68 m			Span Lei	ngth	22,03 / 30,	00 / 27.50 / 30.15 / 30.00	0/
Narr	ne of Road M	larcos H	ighway		Location	Mari	kina Ci	ly		Chaina	age		-	30.00/30.0	0 / 30.00 / 30.00 / 30.00	/ 22.0
	Road Width ((m)	16	.41 / 25.20					Γ		Туре	Wa	all Type			
Road	Lane		No.	2 .	Width		(m)	1			Wall	Height	((1.30))	(m), Width	0.90	(m)
5	Sidewalk		Туре	Concrete	Width	1.65 / 3.50	(m)	1	Abut	ment	Footing	Length	None	(m), Width	None	(m)
Approach	Median		Туре	Concrete	Width	3.86 / 1.00	(m)	I			Foundation	Туре	((RC Pile))	Length	((15.00))	(m)
App	Pavement		Туре	Concrete	Thickness	((23))	(cm)	1			Туре	Colu	mn Type			
	Traffic Volum	ne			Bo	oth Direction/da	iy	1	1		Coping	Height	1.32	(m), Width	2.58	(m)
	Alignment		Skew	26° 30	Curve			1	Pier		Column	Height	((7.80))	(m), size	a 1.50	(m)
			Туре	AASHTO Girde	r Type IV-A			1			Footing	Length	((22.50))	(m), Width	((4.00))	(m)
	Main Girder		Height	1.49 m				ſ			Foundation	Туре	((RC Pile))	Length	((10.00))	(m)
	Wall Gilder		Number	12 per span						Specifi	ication	(AASHTO	D 1977)			
			Space	1.70			(m)		Б	Live Lo	bad	(HS 20 -	44)			
ure	Cross Beam		Туре	Concrete					Design	Seismi	ic Coefficient	0.10(DL+	1/2 LL)			
Superstructure	Cross Beam		No.	20						Desigr	n Date	(1978)				
Bersl	Stringer		Туре	-						Concre	ete	fc	(21 MPa)			
Sup	Stringer		No.	-					l <u>i</u> g	Reinfo	rcing Bar	fy	(275 Mpa, G	Grade 40)		
	Pavement		Туре	Asphalt	Thickness	((5))	(cm)		Ę	P.C. M	laterial	f'c	(35 Mpa),	f'su	(1862 Mpa, Grade 270)	}
	Slab		Туре	Concrete	Thickness	((20))	(cm)		Construction	Steel A	/iaterial	fy	((248 Mpa))		·	· · · · · · · · · · · · · · · · · · ·
	Shoe		Туре	Neoprene	Reaction	41.42 (max.)	(t)			Constr	uction Date	(*	978)			
	Expansion Joi	int	Туре	Steel							marks	() as-bui	t data			
	Handrail		Туре	Concrete						Rer		(()) assu	mption data			

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Nam	e of Bridge :	1	6.0 Mariki	na Bridge	-							Referen Inventor Inventor		Ma4 November KEI - PRO		
Brid	де Туре	PCI Girde	er Bridge (5	-Span)		Bridge Length	٦	138.2	20 m			Span Le	ngth	24 20 (20)	00 / 30.00 / 30.00 / 24.00	
Nam	ne of Road	A. Bonifa	cio Ave./E.	Rodriques Ave.	Location	Mari	ikina Ci	y		Chaina	ige			24.207 30.	007 30.007 30.007 24.00	,
_	Road Widt	ih (m)	16	6.67 / 13.10							Туре	W	all Type			
toad	Lane		No.	4	Width	3 - 8.30	(m)		Abut	ment	Wall	Height	((1.545))	(m), Width	1.30	(m)
Approach Road	Sidewalk		Туре	Concrete	Width	1.80 / 1.20	(m)	ľ		ment	Footing	Length	None	(m), Width	None	(m)
g	Median		Туре	-	Width	-	(m)				Foundation	Туре	((RC Pile))	Length	((12))	(m)
Ag	Pavement		Туре	Asphalt	Thickness	((5))	(cm)] [Туре	Colu	imn Type			
	Traffic Vol	ume			Bo	oth Direction/da	зу				Coping	Height		(m), Width	2.45	(m)
	Alignment		Skew	90°	Curve				Pier		Column	Height	5.38/9.807	(m), size	a 1.80	(m)
			Туре	AASHTO GIRDI	ER TYPE III	(Modified)					Footing	Length	22.50	(m), Width	5.00	(m)
	Main Girde	ar.	Height	1.42 m							Foundation	Туре	((RC Pile))	Length	((16 & 17))	(m)
	initiality Olitice		Number	12 per span						Specifi	cation	(AASHT	O 1977)			
			Space	1.70			(m)		ē	Live Lo	əd	(HS 20 -	44)			
lure	Cross Bea	~	Туре	Concrete					Design	Seismi	c Coefficient	0.10(DL+	-1/2 LL)			
Superstructure	Cross Bea	m	No.	128						Design	Date	(1979)				
Ders	Stringer		Туре							Concre	te	f'c	(21 MPa)			
Sup	Sunger		No.	-					Гi	Reinfo	rcing Bar	fy	(275 Mpa, G	Grade 40)		
	Pavement		Туре	Asphalt	Thickness	((5))	(cm)		struction	P.C. M	aterial	fc	(35 Mpa).	f'su	(1862 Mpa, Grade 270)	i
	Slab		Туре	Concrete	Thickness	((23))	(cm)		Suc	Steel N	laterial	fy	(248 MPa)			
	Shoe		Туре	Steel	Reaction	44.03 (max.)	(I)		Ŭ	Constr	uction Date	((1980))		•	
	Expansion	Joint	Туре	Steel (Fingers)						Dee		() as-bui	it data			
	Handrail		Туре	Concrete						Ren	narks	(()) assu	mption, other	data		

VISUAL INSPECTION REPORT (1/5) BRIDGE INVENTORY

Nam	e of Bridge :	17.0	San Jo	se Bridge	-						()	Reference No. Inventory Date Inventory Office		Ma5 December KE1 - PRO			
Bridge Type PCI Girder Bridge (8-Span)						Bridge Lengt	199.67 m			Span Length		24.90/24	97 / 24.95 / 24.9	97/25.00	1		
Name of Road Rodrigues Highway					Location	Montalban, F		izal		Chainage				24.97 / 24.96 / 24.95			
Approach Road	Road Width (m) 15.80			.80 / 15.74							Туре	w	all Type				
	Lane	N	ю.	4	Width	2 - 7,90	90 (m)				Wall	Height	((1.20))	(m), Width	1.20	5	(m
	Sidewalk		ype	Concrete	Width	1.70 / 1.80	(m)	1	Abutment		Footing	Length	None	(m), Width	None	e	(m
	Median		ype	Concrete	Width	1.40 / 1.00	(m)]	1		Foundation	Туре	((RC Pile))	Length	n ((10)	»	(m
	Pavement		ype	Concrete	Thickness	((23))	(cm)]			Туре	Col	umn Type				
	Traffic Volume				B	oth Direction/d	ву				Coping	Height	1.00	(m), Width	1.62	2	(m
	Alignment	s	ikew	90°	Curve				Pier		Column	Height	6.00 / 6.95	(m), size	1.00	<u>,</u>	(m
	Main Girder		Type AASHTO PSC Girder Type I			V (Modified)					Footing	Length	18.12	(m), Width	2.80	, ,	(m
			leight	1.40 m					ļ		Foundation	Туре	RC Pile	Length	((10)))	(m)
			lumber	8 per span				1			ication	(AASHT	O 1977)			·	-
			pace	2.30			(m)	l .			bad	(MS - 18)				
Superstructure	Cross Beam	Т	ype	Concrete					B Live Lo		ic Coefficient	0.10(DL	+1/2 LL)				
	CIUSS DEAM		ю.	14 end, 14 Interme	diate / Span					Design	n Date	Unknow	1				-
	Stringer		уре	-						Concre	ete	fc	(21 MPa)				
			0.	-					lion	Reinforcing Bar		fy	(275 Mpa, G	Grade 40)			
	Pavement		уре	Asphalt	Thickness	((5))	(cm)		ŝ	P.C. M	laterial	f'c	(35 Mpa),	f'su	(1862 Mpa, Gr	ade 270)	
	Slab		уре	Concrete	Thickness	((20))	(cm)		Construction	Steel A	Steel Material		(248 Mpa)				_
	Shoe		ype	Steel	Reaction	50 (max.)	(t)			Construction Date		(1980)				_
	Expansion Joint		ype	Steel (Angular)				<u> </u>			() as-bu	It data					
	Handrail		ype Concrete						Remarks		(()) assu	motion data					

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