## Appendix B

Meteo-hydorology

# Appendix B <br> Meteo-hydorology 

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## Appendix B Meteo-hydorology

## 1 BANG GIANG AND KY CUNG RIVER BASIN

### 1.1 Basin Characteristics

### 1.1.1 River Basin

Bang Giang and Ky Cung River Basin lies in the north east of Vietnam and it is situated between $21^{\circ} 30^{\prime}$ to $23^{\circ} 00^{\prime}$ north latitude and $105^{\circ} 45^{\prime}$ to $107^{\circ} 20^{\prime}$ east longitude.

The Bang Giang and the Ky Cung are both part of the upper limit of the Tay Giang basin that debouches into the East Sea (South China Sea) near Hong Kong. The two tributaries have their confluence at Long Zhou in China, some 20 km east of the border inside China. The catchment area of the respective sub-basins at the boundary with China as follows:

| Ky Cung : | $6,790 \mathrm{~km}^{2}$ |
| :--- | ---: |
| Bang Giang : | $4,460 \mathrm{~km}^{2}$ |
| Total : | $11,250 \mathrm{~km}^{2}$ |

For the location map of the basin reference is made to Figure B.1.

### 1.1.2 Climatological Features

Mean annual temperature of this basin varies from $20^{\circ} \mathrm{C}$ to $22^{\circ} \mathrm{C}$. Evaporation varies from 700 mm to 800 mm . Mean annual rainfall varies from 1200 mm to 1600 mm . Rainy season lasts from May to September.

### 1.2 Natural Runoff

For the assessment of the availability of water in the Vietnamese part of the Bang Giang and Ky Cung basin, an estimate has been made of the natural runoff at the locations where the two tributaries of this basin leave the Vietnamese territory. The catchment area of the respective sub-basins at the boundary with China is as follows:

| Ky Cung | $6,790 \mathrm{~km}^{2}$ |
| :--- | :--- |
| Bang Giang | $4,460 \mathrm{~km}^{2}$ |

The measurement intensity in the basin is very low and especially the availability of historic discharge data is limited.

In the Ky Cung sub-basin two discharge measurement stations were identified. In the Bang Giang sub-basin no historic discharge date could be made available. However, discharge data are available of a station in the Quay Son sub-catchment adjacent to the Bang Giang sub-basin. The runoff observations used in the present analysis refer to:

| Station | Longitude | Latitude | Catchment | Tributary | Observation Period |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Lang Son | 106.45 | 21.50 | $1,560 \mathrm{~km}^{2}$ | Ky Cung | $1958-2000$ |
| Van Miich | 106.22 | 22.06 | $2,360 \mathrm{~km}^{2}$ | BacGiang | $1960-1974$ |
| Ban Gioc | 106.42 | 22.51 | $2,123 \mathrm{~km}^{2}$ | Quay Son | $1960-1976$ |

In addition to these daily discharge data, the following records of daily precipitation have been made available:

| Station | Longitude | Latitude | Province | Observation Period |
| :---: | :---: | :---: | :---: | :---: |
| Dinh Lap | 107.06 | 21.32 | Lang Son | $1976-2000$ |
| Lang Son | 106.46 | 21.50 | Lang Son | $1976-2000$ |
| Ngan Son | 105.59 | 22.26 | Cao Bang | $1976-2000$ |
| LocBinh | 106.55 | 21.45 | Cao Bang | 1976-1978 .1980-1989. 1991-2000 |
| Cao Bang | 106.14 | 22.39 | Cao Bang | $1976-2000$ |

It is noted that the observation periods of the runoff series in the respective sub-basins have no overlap with the rainfall series. Only for the Ky Cung sub-basin upstream of Long Sang, the overlap corresponds with the full length of the rainfall series.

For the preparation of discharge series for both sub-basins, the following alternative approaches have been considered:
a. to use the simultaneous Lang Son discharge and rainfall series for the assessment of a rainfall runoff relation for the catchment $\mathrm{u} / \mathrm{s}$ of Lang Son, and apply the relation of this upper catchment for both sub-basins using the 25 year historical rainfall data;
b. to prepare the runoff series for the 15 year period for which most direct discharge information is available.

The second approach has been selected for the following reasons:
a. The Lang Son catchment represents only $23 \%$ of the Ky Cung sub-basin and $14 \%$ of the combined Ky Cung and Bang Giang sub-basins.
b. The Lang Son catchment is located in the southern edge of the combined catchment, that has significantly different topographical features as compared to the northern part of the Bang Giang sub-basin. Moreover, the geological features of the Lang Son upper catchment are essentially different from the rest of the sub-basins (reference is made to the Satellite Image Analysis carried out in the framework of the present study).
c. Although the area rainfall in both sub-basins is quite similar (a difference between both sub-basins of some $6 \%$ was found), it is anticipated that the assumption of the
rainfall runoff relation of the upper Ky Cung to be valid for the combined basins may create a substantial error in the runoff estimate.
d. For the period 1960-1974 runoff data are available at Lang Son and Van Mich, together representing $58 \%$ of the Ky Cung sub-basin. The transformation of the combined Lang Son - Van My series into Ky Cung sub-basin series is anticipated to give more reliable results than by making use of the above mentioned rainfall runoff relation.
e. No discharge data are available of the Bang Giang sub-basin. Examination of the topographical and geological features of the Bang Giang sub-basin (reference is made to the Satellite Image Analysis) reveals that the southern part of this sub-basin has basically different characteristics as compared to the northern part. The southern part bears clear resemblance with the Bac Giang catchment $\mathrm{u} / \mathrm{s}$ Van Mich, while the characteristics of the northern part are similar to those of the Quay Son sub-catchment u/s Ban Gioc.
f. It is anticipated that the runoff of the southern part of the Bang Giang basin shows the same characteristics as the runoff at Van Mich, while the northern runoff is similar to the Ban Gioc discharge. Moreover, the size of the southern and northern part are similar to the size of the catchment of Van Mich and Ban Gioc respectively.
g. Based on above observations it is taken that the combined use of the historical monthly runoff series of Van Mich and Ban Gioc provides a fairly accurate estimate of the runoff of the Bang Giang sub-basin.

Based on above analysis, the 15 years natural runoff series for both the Ky Cung and the Bang Giang sub-basins have been compiled with the help of the historical runoff series of Lang Son, Van Mich and Ban Gioc for the period 1960-1974.

The Ky Cung runoff is estimated assuming that the combined Lang Son / Van Mich runoff per $\mathrm{km}^{2}$ is representative for the entire sub-basin within the Vietnamese territory.

The Bang Giang runoff is estimated under the assumption that the average of the runoff per $\mathrm{km}^{2}$ of the Van Mich and the Ban Gioc catchments is representative for the Bang Giang sub-basin within the Vietnamese territory.

The generated 15 years monthly runoff series of both sub-basins are presented in the Tables B.1.

On the basis of the generated monthly runoff series, the monthly natural runoff for different levels of dependability has been assessed as shown in the following table. It is noted that the dependable monthly runoff volumes have been calculated for each month separately. As a consequence, the succession of, for instance, $10 \%$ dry months should not
be considered as a $10 \%$ dry year.
Dependable Monthly Natural Runoff at the boundary with China ( $\mathrm{A}=\mathbf{1 1 , 2 5 0} \mathbf{k m}^{\mathbf{2}}$ )

|  |  |  |  |  |  |  |  | unit million $\mathrm{m}^{3}$ |  |  |  |  |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| $50 \%$ | 116 | 93 | 109 | 183 | 315 | 831 | 1,238 | 1,502 | 917 | 429 | 225 | 145 |
| $75 \%$ | 96 | 85 | 87 | 117 | 208 | 550 | 837 | 1,031 | 650 | 314 | 181 | 118 |
| $90 \%$ | 81 | 79 | 71 | 79 | 143 | 379 | 588 | 735 | 476 | 237 | 149 | 98 |

Flood Runoff
To estimate the flood runoff in the Bang Giang and Ky Cung basin, an analysis has been made of the observed peak flows in the basin and of the rainfall intensities.

Data on monthly instantaneous peak flows are available for the Lang Son station in the Ky Cung catchment for the period 1958 - 2000. From this series the following probable discharges have been calculated assuming a Log-normal and Pearson-3 probability distribution respectively. The frequency distributions are shown in Figure B.2.

Probable yearly peak discharges at Lang Son in the Ky Cung sub-basin in m $\mathbf{m}^{\mathbf{3}} \mathbf{s}$

| Return Period in years | 2 | 3 | 5 | 10 | 20 | 50 | 100 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Log- Normal Function | 1,153 | 1,473 | 1,862 | 2,394 | 2,945 | 3,718 | 4,343 |
| Pearson 3 Function | 1,185 | 1,551 | 1,960 | 2,471 | 2,952 | 3,555 | 3,998 |
| Round average | 1,200 | 1,550 | 1,950 | 2,450 | 2,950 | 3,650 | 4,200 |

The catchment area upstream of the Lang Son station amounts to $1,560 \mathrm{~km}^{2}$. The specific peak discharge at that location is, consequently, calculated at some $770 \mathrm{l} / \mathrm{s} / \mathrm{km}^{2}$ at the once in two year flood, increasing to some $2,700 \mathrm{l} / \mathrm{s} / \mathrm{km}^{2}$ during the once in 100 years flood.

The concentration time of the basin is estimated of the order of 1-2 days. It is, therefore, anticipated that intensities corresponding with one to two day rainfall are indicative for the peak floods.

For both the Ky Cung sub-basin and the Bang Giang sub-basin, the maximum one-day and two day rainfall has been analysed. The following daily rainfall data have been used:

$$
\begin{array}{ll}
\text { Cao Bang } & 1976-2000 \text { (with minor gaps) } \\
\text { Ngan Son } & 1976-2000 \text { (with minor gaps) } \\
\text { Loc Binh } & 1976-1978,1980-1989,1990-2000 \text { (with minor gaps) } \\
\text { Lang Son } & 1976-2000 \text { (with minor gap) } \\
\text { Dinh Lap } & 1976-2000 \text { (complete) }
\end{array}
$$

The minor gaps in the Cao Bang and Ngan Son series have been mutually filled. The Lang Son series have been completed with the help of Dinh Lap data. Loc Binh series
have been completed with the help of the completed Lang Son series.
The area rainfall on the Ky Cung sub-catchment and the Bang Giang sub-catchment has been calculated on the basis of the following weights of the respective stations:

| Sub-catchment | Rainfall stations |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
|  | Cao Bang | Ngan Son | Loc Binh | Lang Son |
| Ky Cung | - | $25 \%$ | $15 \%$ | $60 \%$ |
| Bang Giang | $85 \%$ | $15 \%$ | - | - |

Twenty-five years daily area rainfall series have been compiled and the following one and two-day probable rainfall intensities have been calculated assuming a log-normal distribution of the yearly maximum values.

Maximum yearly area rainfall on the Ky Cung basin

| Return period in years | 10 | 20 | 50 | 100 |
| :---: | :---: | :---: | :---: | :---: |
| One-day | 114 | 128 | 148 | 162 |
| Two-day | 148 | 165 | 188 | 204 |

## Maximum yearly area rainfall on the Bang Giang basin

| Return period in years | 10 | 20 | 50 | 100 |
| :---: | :---: | :---: | :---: | :---: |
| One-day | 131 | 142 | 157 | 167 |
| Two-day | 157 | 168 | 182 | 192 |

From these rainfall intensities it can be estimated that the following flood volumes may pass, assuming a runoff of some $60 \%$ of the rainfall volume, at the mentioned locations.

Estimated flood volumes ( $\mathbf{M m}^{\mathbf{3}}$ )

| Return period in years | 10 | 20 | 50 | 100 |
| :---: | :---: | :---: | :---: | :---: |
| Ban Lai Dam-site | 45 | 50 | 60 | 70 |
| Lang Son | 100 | 120 | 140 | 150 |
| Ky Cung at border crossing | 500 | 600 | 700 | 750 |
| Bang Giang at border crossing | 350 |  |  | 500 |

A first approximation of peak discharges corresponding with these flood volumes is as follows:

Approximate peak discharges ( $\mathrm{m}^{3} / \mathbf{s}$ )

| Return period in years | 10 | 20 | 50 | 100 |
| :---: | :---: | :---: | :---: | :---: |
| Ban Lai Dam-site | 2,000 | 2,300 | 2,800 | 3,200 |
| Lang Son | 2,450 | 2,950 | 3,650 | 4,200 |
| Ky Cung at border crossing | 6,500 | 8,000 | 9,000 | 10,000 |

Rainfall intensities in the period January - August, have been calculated as follows:

Maximum January - August area rainfall on the Ky Cung basin

| Return period in years | 2 | 3 | 5 | 10 | 20 |
| :---: | :---: | :---: | :---: | :---: | :---: |
| One-day | 72 | 84 | 97 | 114 | 131 |
| Two-day | 98 | 112 | 128 | 147 | 165 |

Maximum January - August area rainfall on the Bang Giang basin

| Return period in years | 2 | 3 | 5 | 10 | 20 |
| :---: | :---: | :---: | :---: | :---: | :---: |
| One-day | 96 | 106 | 118 | 132 | 144 |
| Two-day | 121 | 132 | 144 | 159 | 170 |

These intensities correspond essentially with the yearly maximum values, since the rainy season is almost completely included in the period ending the end of August.

From these rainfall intensities it can be estimated that the following flood volumes may pass, assuming a runoff of some $60 \%$ of the rainfall volume, at the mentioned locations.

Estimated flood volumes ( $\mathbf{M m}^{\mathbf{3}}$ )

| Return period in years | 2 | 3 | 5 | 10 | 20 |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Ban Lai Dam-site | 30 | 35 | 40 | 45 | 50 |
| Lang Son | 70 | 80 | 90 | 100 | 120 |
| Ky Cung at border crossing | 350 | 400 | 450 | 500 | 600 |
| Bang Giang at border crossing | 250 | 280 | 300 | 350 | 400 |

A first approximation of peak discharges corresponding with these flood volumes is as follows:

Approximate peak discharges ( $\mathrm{m}^{3} / \mathrm{s}$ )

| Return period in years | 2 | 3 | 5 | 10 | 20 |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Ban Lai Dam-site | 1,000 | 1,200 | 1,500 | 2,000 | 2,300 |
| Lang Son | 1,200 | 1,550 | 1,950 | 2,450 | 2,950 |
| Ky Cung at border crossing | 4,500 | 5,000 | 6,000 | 6,500 | 8,000 |

A typical flood at Lang Son is shown Figure B.3, presenting the July 1980 flood, with peak discharge of $2,800 \mathrm{~m}^{3} / \mathrm{s}$ following a two day storm with total area rainfall of some 170 mm .

## 2 RED RIVER / THAI BINH RIVER BASIN

### 2.1 Basin Characteristics

### 2.1.1 River Basin

The Red River flows from north-west in the south-easterly direction towards the Gulf of Tonkin. The main tributaries of the Red River basin, i.e. the Da and Thao rivers, originate in China, between $25^{\circ}$ and $26^{\circ}$ north latitude. The Lo river also has its origin in China at a north latitude between $23^{\circ}$ and $24^{\circ}$. These three tributaries enter Vietnamese territory at north latitude between $22^{\circ}$ and $23^{\circ}$ and enter the Gulf of Tonkin between latitude $20^{\circ}$ and $21^{\circ}$ and eastern longitude $106^{\circ}-107^{\circ}$.

The catchment area of the Red River is $169,000 \mathrm{~km}^{2}$. Nearly $50 \%$ of this area lies within China. This area includes the Thai Binh basin that joins with the Red river in their common delta area.

The Thai Binh is located on the eastern edge of the basin. Its upper catchment, drained by the Cau River, contributes for about $25 \%$ to the discharge of the Thai Binh River, while this river receives most of its water ( $75 \%$ ) from the Red River through the Duong River that bifurcates from the Red River just upstream of Hanoi. The main tributaries of the Red River are listed below:

| Red and Thai Binh River (Total) | $169,040 \mathrm{~km}^{2}$ |
| :--- | :--- |
| Major Sub-basins |  |
| Thao : | $51,800 \mathrm{~km}^{2}$ |
| Da : | $52,900 \mathrm{~km}^{2}$ |
| Lo : | $39,000 \mathrm{~km}^{2}$ |
| Cau \& Thuong \& LucNam : | $12,700 \mathrm{~km}^{2}$ |
| Red \& ThaiBinh Delta : | $12,640 \mathrm{~km}^{2}$ |

The location plan of the Red River and Thai Binh basin is presented in Figure B.4.

### 2.1.2 Climatological Features

Mean annual rainfall varies from 1200 mm to 4800 mm . This variation is much less significant in the Red River Delta, which has average rainfall of $1,740 \mathrm{~mm}$. However, seasonal variations are significant everywhere. From November to March is the dry season in the basin. Rainy season lasts from May to September. The mean temperature in this basin varies greater within the year.

### 2.2 Natural Runoff

Runoff of the Red River and Thai Binh basin has extensively been addressed in the framework of the Red River Delta Master Plan, 1994, the National Hydropower Plan Study, 1999 and the Red River Basin Water Resources Management project, 2001. The latter presents information about water resources data, rather then analysing the data.

The present study does not allow a more detailed hydrological study of the Red River basin than the ones that have been carried out previously. Therefore, for the estimate of runoff and water availability at the base point of the Red River and Thai Binh basin, use is made, with due care, of the results of these previous studies, rather then making a full analysis on the basis of basic rainfall and runoff data as has been done for the other basins.

The base point of the Red River and Thai Binh basin is difficult to define. Since the runoff analysis is made in view of the assessment of the water resources potential of the basin, it is considered appropriate to establish runoff and availability just upstream of the area where the major part of the water use occurs, i.e. the delta area. For that reason, the runoff is analysed at Son Thay (Red River basin runoff). The run-off of the Thai Binh sub-basin at Pha Lai, where the Thai Binh meets the Red River Delta, is considered of minor importance at the entire Red River - Thai Binh basin level, since this runoff, estimated at some $4,000 \mathrm{Mm}^{3}$ per year, corresponds with some $3 \%$ of the basin runoff only.

The runoff at Son Tay is to a considerable extent effected by the main upstream reservoir at Hoa Binh on the Da River and, to a less extent by Thac Ba on the Chay River. The influence of the Thac Ba reservoir, that came into operation in 1974, is limited since its capacity is rather small. However, the observed discharges at Son Tay cannot be considered as the natural basin run-off as from 1986, the year in which the filling of the Hoa Binh reservoir was commenced. Therefore, for the assessment of the natural basin runoff, either the Son Tay discharges are corrected as from 1986, or only the observations prior to this year are used. The latter option has been chosen, that is to say that for the analysis of natural runoff only the existing discharge series 1957-1986 have been used.

From these series, the following monthly average runoffs have been derived for the main sub-catchments:

| Month | Monthly mean natural runoff ( $\mathrm{Mm}^{3}$ ) |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
|  | Da sub-basin at Hoa Binh | Thao sub-basin at Yen Bai | Lo sub-basin at Vu Quang | Red River basin at Son Tay |
| January | 1,482 | 883 | 1,030 | 3,453 |
| February | 1,059 | 684 | 862 | 2,632 |
| March | 933 | 638 | 864 | 2,441 |
| April | 1,007 | 725 | 1,029 | 2,823 |
| May | 1,925 | 1,078 | 1,991 | 5,060 |
| June | 5,714 | 2,290 | 3,966 | 12,023 |
| July | 11,008 | 3,500 | 5,884 | 20,699 |
| August | 12,218 | 4,828 | 6,423 | 24,043 |
| September | 7,837 | 3,817 | 4,588 | 17,137 |
| October | 4,861 | 2,867 | 2,912 | 11,112 |
| November | 3,185 | 1,808 | 1,984 | 7,269 |
| December | 2,039 | 1,190 | 1,270 | 4,591 |
| Year | 53,268 | 24,307 | 32,805 | 113,282 |

Taking into account the estimated $4,000 \mathrm{Mm}^{3}$ yearly average Thai Binh runoff, the following distribution of the tributaries to the basin runoff can be derived:

| - Da sub-basin: | $47 \%$ |
| :--- | :---: |
| - Thao sub-basin: | $21 \%$ |
| - Lo sub-basin: | $29 \%$ |
| - Thai Binh sub-basin | $3 \%$ |

The thirty years 1957 - 1986 runoff series of the basin at Son Tay and of the main tributaries is presented in the Tables B.2.

On the basis of these series the duration curve of monthly discharges has been established for each station and for each month. From these curves the discharges with different levels of dependability have been assessed, as presented in the tables. These discharges are summarised in the following tables.

## Dependable Monthly Natural Runoff at Hoa Binh (A=51,800km²)

|  |  |  |  |  |  | unit $:$ million $\mathrm{m}^{3}$ |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Jan | Feb | Mar | Apr | May | Jun | Jul | Aug | Sep | Oct | Nov | Dec |
| $50 \%$ | 1,449 | 1.041 | 916 | 970 | 1,719 | 5,336 | 10,589 | 11,809 | 7,615 | 4,768 | 3,017 | 1,965 |
| $75 \%$ | 1,219 | 894 | 785 | 772 | 1,153 | 3,904 | 8,458 | 9,640 | 6,238 | 4,068 | 2,306 | 1,579 |
| $90 \%$ | 1,098 | 815 | 715 | 674 | 907 | 3,237 | 7,391 | 8,534 | 5,535 | 3,698 | 1,962 | 1,385 |

Dependable Monthly Natural Runoff at Yen Bai ( $A=48,000 \mathrm{~km}^{2}$ )

|  |  |  |  |  |  |  |  | unit :million $\mathrm{m}^{3}$ |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Jan | Feb | Mar | Apr | May | Jun | Jul | Aug | Sep | Oct | Nov | Dec |
| $50 \%$ | 866 | 673 | 617 | 697 | 1,010 | 2,137 | 3,310 | 4,610 | 3,673 | 2,773 | 1,727 | 1,144 |
| $75 \%$ | 736 | 579 | 507 | 552 | 749 | 1,556 | 2,526 | 3,709 | 2,917 | 2,259 | 1,345 | 919 |
| $90 \%$ | 667 | 529 | 451 | 480 | 626 | 1,287 | 2,148 | 3,255 | 2,540 | 1,997 | 1,158 | 807 |

Dependable Monthly Natural Runoff at Son Tay ( $\mathrm{A}=144,000 \mathrm{~km}^{2}$ )

|  |  |  |  |  |  |  | unit :million m |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Jan | Feb | Mar | Apr | May | Jun | Jul | Aug | Sep | Oct | Nov | Dec |
| $50 \%$ | 3,384 | 2,593 | 2,380 | 2,721 | 4,659 | 11,293 | 19,879 | 23,221 | 16,595 | 10,899 | 6,925 | 4,442 |
| $75 \%$ | 2,870 | 2,245 | 2,027 | 2,173 | 3,326 | 8,371 | 15,763 | 19,107 | 13,448 | 9,183 | 5,342 | 3,595 |
| $90 \%$ | 2,601 | 2,058 | 1,840 | 1,899 | 2,717 | 6,994 | 13,715 | 16,997 | 11,855 | 8,291 | 4,572 | 3,166 |

Dependable Monthly Natural Runoff at Vu Quang (A=37,000km²)

|  |  |  |  |  |  |  |  | unit :million $\mathrm{m}^{3}$ |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Jan | Feb | Mar | Apr | May | Jun | Jul | Aug | Sep | Oct | Nov | Dec |
| $50 \%$ | 994 | 826 | 820 | 964 | 1,828 | 3,633 | 5,633 | 6,111 | 4,347 | 2,811 | 1,868 | 1,230 |
| $75 \%$ | 799 | 651 | 637 | 722 | 1,301 | 2,551 | 4,399 | 4,776 | 3,305 | 2,255 | 1,402 | 996 |
| $90 \%$ | 700 | 564 | 547 | 607 | 1,061 | 2,063 | 3,792 | 4,119 | 2,804 | 1,976 | 1,180 | 877 |

The natural runoff that was presented in the previous section corresponds with the runoff of the Red River basin without taking into account the effect of the Hoa Binh reservoir. For the assessment of the availability of water in the Red River Delta, this effect needs to be taken into account. However, this effect is subject to the operating rules that are applied in the Hoa Binh scheme.

For the current operation policy for Hoa Binh and Thac Ba simulations were carried out of the Son Tay runoff for the period 1957 - 1990 in the framework of the Red River Delta Master Plan Study. The results of these simulations show that the reservoirs contribute considerably to the low flows during the period December - May. The effects for different levels of dependability are presented in the following table:

Mean monthly water availability at Son Tay in Mm3 with different levels of dependability

|  | December | January | February | March | April | May |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Natural runoff | 3,595 | 2,870 | 2,245 | 2,027 | 2,173 | 3,326 |
| $75 \%$ | 3,166 | 2,601 | 2,058 | 1,840 | 1,899 | 2,717 |
| $90 \%$ |  |  |  |  |  |  |
| Effect reservoirs | 482 | 937 | 1,123 | 1,446 | 1,555 | 1,553 |
| $75 \%$ | 723 | 1,071 | 1,245 | 1,446 | 1,555 | 1,553 |
| $90 \%$ |  |  |  |  |  |  |
| Water availability | 4,071 | 3,803 | 3,368 | 3,482 | 3,732 | 4,875 |
| $75 \%$ (round) | 3,884 | 3,669 | 3,295 | 3,294 | 3,447 | 4,259 |
| $90 \%$ (round) |  |  |  |  |  |  |

### 2.3 Flood Runoff

Flood runoff of the Red River and Thai Binh basin has extensively been addressed in the framework of the Red River Delta Master Plan, 1994. This study recommended on the basis of an extensive analysis that it would be prudent to assume a log normal distribution of extreme values for designing the flood protection measures in the Red River Delta.

Maximum instantaneous discharges for the years 1956-1986 (so prior to the functioning of the Hoa Binh reservoir) were used for the statistical analysis of flood discharges. The data after 1986 when the Hoa Binh Dam commenced its operation were not used, since the flood discharges to downstream were ones which were already regulated by the reservoir and not proper for the use in the statistical analysis. The log normal distribution function on the basis of historic observation gives the following probable peak discharges for Son Tay and Hanoi:

| Estimated peak discharges $\left(\mathrm{m}^{3} / \mathrm{s}\right)$ |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Return period | 10 years | 20 years | 50 years | 100 years | 500 years | 1000 years |  |
| Son Tay | 23,500 | 27,000 | 32,050 | 35,730 | 46,390 | 51,580 |  |
| Hanoi | 16,150 | 18,020 | 20,540 | 22,230 | 26,860 | 28,970 |  |

In connection with the probable discharges at Hanoi(the Red River), the following is noted:

The Red River bifurcates to the Red River and the Duong River in the downstream of Son Tay. Although the discharges at Hanoi in the Red River after the bifurcation are as shown in the above table on the basis of the statistic analysis with historical observations, the discharges in the Duong River are not always the difference between the discharge at Son Tay and the discharge at Hanoi due to the possible right bank overflow between Son Tay and Hanoi in the occurrence of large floods. The discharges of the Duong River are not shown due to lack of data for the said right bank overflow in the historical events.

The maximum peak discharges observed at these locations refer to the 1971 flood with following hydrograph.


It is observed that the discharges in Son Tay were still rising while in downstream located Hanoi the peak already had passed. This phenomena could be explained by a sudden diversion of flood waters between Son Tay and Hanoi, possibly caused by a dike breach.

The observed maximum discharges in 1971 and corresponding estimated return period are as follows:

|  | Son Tay | Hanoi |
| :---: | :---: | :---: |
| Peak discharge $\left(\mathrm{m}^{3} / \mathrm{s}\right)^{1)}$ | 34,200 | 22,200 |
| Estimated return period $($ years $)$ | 72 | 90 |

1) these discharges were distracted from the data base presented in the Background Report 1 on Hydrology and Meteorology, Volume 3, of the Red River Delta Master Plan, Binnie \& Partners et al, 1994. In the Master Plan report also an other reference is made to the 1971 peak discharge in Son Tay being $38400 \mathrm{~m}^{3} / \mathrm{s}$. Such discharge would have a Return Period of 141 years, applying the log normal distribution function.

The historic flood events of $1968\left(\mathrm{Q}_{\text {max }}\right.$ at Son Tay: $\left.24,000 \mathrm{~m}^{3} / \mathrm{s}\right), 1969\left(\mathrm{Q}_{\text {max }}\right.$ at Son Tay: $28,300 \mathrm{~m}^{3} / \mathrm{s}$ ) and $1945\left(\mathrm{Q}_{\text {max }}\right.$ at Son Tay: $\left.33,500 \mathrm{~m}^{3} / \mathrm{s}\right)$, correspond approximately with floods with an estimated return period of 11,25 and 64 years respectively. The corresponding flood volumes at Son Tay over 8 days flood duration are as follows (source Red River Master Plan):

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| year | Qmax $\left(\mathrm{m}^{3} / \mathrm{s}\right)$ | Estimated Return Period | 8- day Flood Volume $\left(\mathrm{Mm}^{3}\right)$ |
| :---: | :---: | :---: | :---: |
| 1968 | 24,000 | 11 years | 12,960 |
| 1969 | 28,300 | 25 years | 16,500 |
| 1945 | 33,500 | 64 years | 18,800 |
| $1971^{1)}$ | 38,400 | 141 years | 19,600 |

1) see above note on 1971 Son Tay peak discharges

For the same historic floods the contribution from the different sub-basins to the peak discharge was estimated as follows (source Red River Master Plan):

| year | Qmax $\left(\mathrm{m}^{3} / \mathrm{s}\right.$ ) | Hoa Binh <br> (Da River) | Yen Bai <br> (Thao River) | Phu Ninh <br> (Lo River) |
| :---: | :---: | :---: | :---: | :---: |
| 1968 | 24000 | $39 \%$ | $28 \%$ | $25 \%$ |
| 1969 | 28300 | $54 \%$ | $17 \%$ | $31 \%$ |
| 1945 | 33500 | $54 \%$ | $20 \%$ | $25 \%$ |
| $1971^{1)}$ | 38400 | $38 \%$ | $25 \%$ | $39 \%$ |

1) see above note on 1971 Son Tay peak discharges

## 3 MA RIVER BASIN

### 3.1 Basin Characteristics

### 3.1.1 River Basin

The Ma basin is located in the north-central Vietnam and borders with Laos. The river basin are situated between $19^{\circ} 30^{\prime}$ to $21^{\circ} 30^{\prime}$ north latitude and between $103^{\circ}$ to $106^{\circ}$ east longitude.

Ma river basin covers a total catchment area of $31,060 \mathrm{~km}^{2}$, which included Yen River Basin and some small river Basins located coastal Area. The Yen river basin borders the Chu basin, in the lower part there is no clear divide between these two basins. About 35\% of catchment area is located in Laos.

| Ma River Basin (Total) | $31,060 \mathrm{~km}^{2}$ |
| :--- | ---: |
| Major Sub-basins |  |
| Chu : | $7,460 \mathrm{~km}^{2}$ |
| Yen : | $720 \mathrm{~km}^{2}$ |
| Lower Area (Delta): | $3,060 \mathrm{~km}^{2}$ |

For the location map of the Ma basin reference is made to Figure B.5.

### 3.1.2 Climatological Features

Mean annual temperature is $23.4^{\circ} \mathrm{C}$. Mean annual humidity is $85 \%$ and annual evaporation varies from 640 mm to 860 mm .

Mean annual rainfall distribution is different from regions, $1,200-1,600 \mathrm{~mm}$ in the upstream and $1,600-2,200 \mathrm{~mm}$ in the down stream. The rainy season lasts from May to October.

### 3.2 Natural Runoff

For the selection of the data series to be used for the assessment of the water resources potential of the Ma Basin, it has been recognised that for such assessment, and the eventual planning of the use of this potential, it is inevitable to consider the two main catchments, the Chu sub-basin and the Ma Basin upstream of the confluence with the Chu river. The Yen river basin borders the Chu basin, in the lower part there is no clear divide between these two basins. In the present study, the Yen river is considered as a sub-basin of the Ma river.

The Ma river bifurcates in its lower stretch into a number of branches, forming a delta area. The base point of the basin is not clearly defined. For the assessment of the basin runoff a distinction has been made between the upper and middle part of the basin on one side and the lower area. The transition between the two areas coincides approximately
with the confluence of the Ma and Buoi river and the intake in the Chu river at Bai Thoung.

The discharge measurement stations that cover largest part of the respective sub-basins are located at Cua Dat in the Chu river and Cam Thuy in the Ma river. In addition to these stations, the Xuan Khanh station with a six year record was selected, since this station covers almost the entire Chu catchment and allows, consequently the assessment of the downstream Cua Dat runoff of the Chu basin. The catchments covered by Xuan Khanh and Cam Thuy correspond with $82 \%$ of the total Ma basin catchment area, including the delta area.

The series of observations in Cam Thuy show a substantial gap during the period 1989 1994. To enable the completion of the Cam Thuy series, it was found convenient to collect the complete series of observations at Xa La as well. Moreover, this series allows also for the assessment of the runoff of the upper Ma basin before it enters Lao territory.

The full set of discharge data used in the present analysis refers to:

| Station | Longitude | Latitude | Catchment | Tributary | Observation Period |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Cua Dat | 105.17 | 19.52 | $6,170 \mathrm{~km}^{2}$ | Chu | $1978-2000$ |
| Xuan Khanh | 105.34 | 19.55 | $7,460 \mathrm{~km}^{2}$ | Chu | $1976-1978$ |
| Cam Thuy | 105.28 | 20.12 | $17,500 \mathrm{~km}^{2}$ | Ma | $1976-1988.1995-2000$ |
| Xa La | 103.55 | 20.56 | $6,430 \mathrm{~km}^{2}$ | Ma | $1976-2000$ |

In addition to the discharge observations, the following set of rainfall data have been processed and introduced in the data base.

| Station | Longitude | Latitud | Province | Observation Period |
| :---: | :---: | :---: | :---: | :---: |
| Thuong Xuan (Cua Dat) | 105.14 | 19.49 | Thanh Hoa | $1976.1978-1990$ |
| Bat Mot (BatBot) | 105.03 | 20.01 | Thanh Hoa | $1976-2000$ |
| Lang Chanh | 105.14 | 20.09 | Thanh Hoa | $1976-2000$ |
| Ba Thuoc (CanhNong) | 105.13 | 20.21 | Thanh Hoa | $1976-1979.1981-1990$ |
| Song Ma | 103.44 | 21.04 | Son La | $1976-2000$ |
| Tuan Giao (LaiChau) | 103.25 | 21.35 | Lai Chau | $1976-2000$ |
| Thanh Hoa | 105.46 | 19.49 | Thanh Hoa | $1976-1985,1987-2000$ |

For the preparation of the monthly natural runoff series of the Ma basin just downstream of the confluence of the Buoi and Ma rivers, use has been made of the observed discharges at Cam Thuy. These discharges are considered natural, the impact of minor storage facilities upstream of Cam Thuy on the river discharges at this station is considered negligible as are the withdrawals upstream of Cam Thuy for water supply and
irrigation. Since the discharges at Cam Thuy represent almost $90 \%$ of the Ma catchment at the Buoi confluence, it is deemed acceptable to apply a simple area proportionality factor for the estimate of the natural runoff at this confluence.

For the completion of the Cam Thuy series of observations, different relations have been investigated between Cam Thuy discharges and monthly catchment rainfall and also between Cam Thuy discharges and the discharges in the upstream Xa La and the neighbouring Chu basin at Cua Dat. It was found that correlation between Xa La discharges and the Cam Thuy discharges is rather poor. It is concluded that the runoff characteristics of the upper catchment are substantially different from the runoff characteristics of the middle part of the catchment. Also the correlation between Cua Dat discharges and Cam Thuy is weak. Finally it was found that the best correlation is found when a relation is sought between the Cam Thuy discharges and the combination of Xa La and Cua Dat discharges. Apparently, the runoff of the Ma sub-basin shows the combined characteristics of the upper Ma catchment and the Chu basin. The relation that has been used for the completion of the Cam Thuy series of the average monthly discharge in $\mathrm{m}^{3} / \mathrm{s}$ reads as follows:

$$
\mathrm{Q}_{\text {CamThuy }}=0.6775\left(\mathrm{Q}_{\mathrm{XaLa}}+\mathrm{Q}_{\text {CuaDat }}\right)+13.9
$$

The corresponding correlation coefficient amounts to $\mathrm{R}^{2}=0.933$
For the preparation of the monthly runoff series of the Chu sub-basin just downstream the confluence with the Am river (total catchment area: 7,460 $\mathrm{km}^{2}$ ), use has been made of the observed discharges at Cua Dat. The discharges observed at Cua Dat represent $83 \%$ of the natural runoff of the sub-basin at the Am confluence. For that reason, it is also for this sub-basin deemed adequate to apply an area proportionality factor for the calculation of natural runoff.

Finally, the series of the monthly runoff of the entire Ma basin at its base point has been prepared by summation of runoff of both sub-basins and the runoff of the lower basin, which has been estimated on the basis of the net rainfall on this lower area as explained in the previous section.

The generated series of monthly runoff of both sub-basins and of the entire basin are presented in the Tables B.3. The dependable basin runoff has been estimated as follows:

Dependable Monthly Natural Runoff in the whole basin ( $\mathrm{A}=\mathbf{3 1 , 0 6 0} \mathrm{km}^{2}$ )

|  | Jan | Feb | Mar | Apr | May | Jun | Jul | Aug | Sep | Oct | Nov | Dec |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| $50 \%$ | 538 | 407 | 410 | 415 | 683 | 1,151 | 1,828 | 2,501 | 2,541 | 1,832 | 972 | 671 |
| $75 \%$ | 451 | 342 | 345 | 342 | 556 | 851 | 1,382 | 1,824 | 1,880 | 1,306 | 739 | 561 |
| $90 \%$ | 384 | 292 | 295 | 288 | 462 | 648 | 1,074 | 1,373 | 1,433 | 963 | 577 | 478 |

### 3.3 Flood Runoff

### 3.3.1 Flood Runoff Chu Sub-basin

The flood runoff of the Chu basin has been estimated at Cua Dat, just upstream of the flood prone area of the Chu basin. Instantaneous yearly peak discharges at Cua Dat stations have been collected and processed for the period 1976 - 2000. It is noted that the availability of only twenty five observed maximum year discharges provides a rather weak basis for the frequency analysis of peak discharges.

The frequency analysis has been carried out using a number of probability distribution functions. The goodness-of-fit tests and the confidence intervals indicate that the normal distribution describes best the probability of occurrence of the peak discharges. Also the Log Pearson, Gumbel and Goodrich give above average results. The results of the frequency analysis are summarised below for these distribution functions and some are presented in Figure B.6.

Unit: $\mathrm{m}^{3 / \mathrm{s}}$

| Estimated Peak Discharges at Cua Dat with corresponding Return Period |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| Return Period | 10 years | 20 years | 50 years | 100 years |
| Normal | 3,565 | 3,912 | 4,302 | 4,563 |
| Log-Pearson | 3,771 | 4,745 | 5,328 | 6,031 |
| Gumbel | 3,587 | 4,123 | 4,816 | 5,336 |
| Goodrich | 3,577 | 3,934 | 4,360 | 4,634 |
| Average round values | 3,650 | 4,200 | 4,700 | 5,150 |

For the estimate of volumes of floods that are generated by 1,2 and 3-day peak rainfall on the Chu basin, an analysis has been made of the daily rainfall data from the rainfall stations upstream of Bai Thuong, the downstream location of the upper and middle Chu catchment. These stations are:

| Thuong Xuan (Cua Dat) | $1976,1978-1990$ |
| :---: | :---: |
| Lang Chanh | $1976-2000$ (with minor gaps) |
| Bat Mot | $1976-2000$ (with minor gaps) |
| Song Ma | $1976-2000$ (complete) |

The Thoung Xuan data series has been combined with the Lang Chang series to obtain a complete series that represents the rainfall in the lower $10 \%$ of the catchment area under
consideration. The minor gaps in the Bat Mot series were filled with Lang Chang data, to obtain a complete series that represent the rainfall on the central $75 \%$ of the area. Song Ma data series represent the rainfall on the upper $15 \%$ of the Chu catchment. 25 year of area daily rainfall series have been compiled using the above mentioned series and corresponding weights. Also series were compiled of two and three consecutive days area rainfall. From all these series the yearly maximum values were selected, which were put through a frequency analysis. The results are given in the table below using the log-normal and Gumbel probability distribution functions. These functions gave the best fit.

Maximum yearly one-day Chu area rainfall in mm

| Return period | 10 years | 20 years | 50 years | 100 years |
| :---: | :---: | :---: | :---: | :---: |
| Log-normal | 179 | 202 | 229 | 250 |
| Gumbel | 179 | 202 | 231 | 254 |

## Maximum yearly two-day Chu area rainfall in mm

| Return period | 10 years | 20 years | 50 years | 100 years |
| :---: | :---: | :---: | :---: | :---: |
| Log-normal | 267 | 305 | 346 | 379 |
| Gumbel | 267 | 302 | 348 | 383 |

## Maximum yearly three-day Chu rainfall in mm

| Return period | 10 years | 20 years | 50 years | 100 years |
| :---: | :---: | :---: | :---: | :---: |
| Log-normal | 315 | 356 | 414 | 456 |
| Gumbel | 314 | 358 | 415 | 457 |

These rainfall intensities correspond with the following $n$-day precipitation volumes in the Chu basin upstream of Bai Thuong (7,460 km ${ }^{2}$ )

Maximum yearly n-day rainfall volume on Chu basin u/s Bai Thuong in $\mathbf{M m}^{\mathbf{3}}$

| Return period | 1 day | 2 days | 3 days |
| :---: | :---: | :---: | :---: |
| 10 years | 1,335 | 1,992 | 2,346 |
| 20 years | 1,507 | 2,264 | 2,663 |
| 50 years | 1,716 | 2,589 | 3,092 |
| 100 years | 1,880 | 2,842 | 3,405 |

Maximum yearly n-day rainfall volume on Chu basin u/s Cua Dat ( $6170 \mathbf{~ k m}^{2}$ ) in $\mathbf{M m}^{3}$

| Return period | 1 day | 2 days | 3 days |
| :---: | :---: | :---: | :---: |
| 10 years | 1,104 | 1,648 | 1,940 |
| 20 years | 1,246 | 1,873 | 2,203 |
| 50 years | 1,419 | 2,141 | 2,557 |
| 100 years | 1,555 | 2,351 | 2,816 |

Some typical floods and corresponding rainfall are presented in Figure B.7.

### 3.3.2 Flood Runoff Ma Basin

Observed peak discharges at Cam Thuy are extremely scanty. From the available discharge data between 1975 and 2000, only 8 maximum year discharges could be derived and 20 maximum daily discharges. The observed instantaneous peak discharges are insufficient for a frequency analysis, while the daily data can deviate substantially from the real peak discharges as a consequence of the flashy character of the floods.

For the estimate of probable peak discharges, use could be made of the estimated peak discharges at Xa La and Cua Dat, assuming Creager type of relation between the respective flood discharges. This relation assumes that the peak discharge is proportional to A to the power $\mathrm{A}^{-0.05}$, with A is the catchment area. The respective catchment areas are as follows:

| Xa La | $6,430 \mathrm{~km}^{2}$ |
| :--- | :--- |
| Cua Dat | $6,170 \mathrm{~km}^{2}$ |
| Cam Thuy | $17,500 \mathrm{~km}^{2}$ |
| Ma - Chu confluence | $28,000 \mathrm{~km}^{2}$ |

Based on these areas, the following proportionality factors are calculated:

| Cam Thuy / Xa La | 1.40 |
| :---: | :--- |
| Cam Thuy / Cua Dat | 1.42 |
| Ma - Chu / Cam Thuy | 1.15 |

Comparison of the estimated peak discharges at Xa La and Cua Dat, indicate however that the probable floods at Cua Dat are $50-60 \%$ higher than the Xa La floods, in spite of the fact that the Xa La area is slightly bigger than the Cua Dat area. From this comparison it is concluded that, if the observed and estimated peak discharges are merely correct, then there is a significant difference between the runoff of both areas.

For the monthly discharges at the respective stations, a fairly good correlation was found between the Cam Thuy discharge and the sum of the Xa La and Cua Dat discharges. The correlation between Cam Thuy and each of the two stations separately appeared to be poor. It is, therefore, concluded that the runoff at Cam Thuy shows characteristics of both the runoff of the upper Ma catchment (at Xa La) and the Chu tributary (at Cua Dat).

Therefore, for a first estimate of peak discharges at Cam Thuy, use is made of the average of the probable discharges at $\mathrm{Xa} \mathrm{La} \mathrm{and} \mathrm{Cua} \mathrm{Dat}$, proportionality factors. The result is as follows:

Estimated Peak discharges in $\mathrm{m}^{3} / \mathbf{s}$ at Cam Thuy with corresponding Return Period

| Return Period | 10 years | 20 years | 50 years | 100 years |
| :---: | :---: | :---: | :---: | :---: |
| Peak Discharge | 4100 | 4800 | 5500 | 6100 |

Similarly, the peak discharges of the Ma river downstream of the Chu confluence can be estimated, applying the area related Creager proportionality to the Cam Thuy discharges.

## Estimated Peak discharges in $\mathrm{m}^{3} / \mathrm{s}$ at Ma - Chu confluence with corresponding Return Period

| Return Period | 10 years | 20 years | 50 years | 100 years |
| :---: | :---: | :---: | :---: | :---: |
| Peak Discharge | 4700 | 5500 | 6300 | 7000 |

From above analysis it became apparent that an adequate rainfall - runoff modelling is essential for a more accurate estimate of the probable floods in the lower part of the basin.

A typical historical flood and corresponding area rainfall is shown in Figure B.8.
For the estimate of volumes of floods that are generated by 1,2 and 3-day peak rainfall on the Ma basin upstream of the flood prone area, an analysis has been made of the daily rainfall upstream of Ma - Buoi confluence. The following daily rainfall data have been used:

| Thanh Hoa | $1976-1985,1987-2000$ (with minor gaps) |
| :--- | :--- |
| Lang Chanh | $1976-2000$ |
| Bat Mot | $1976-2000$ (completed) |
| Song Ma | $1976-2000$ (complete) |
| Tuan Giao | $1976-2000$ (complete) |

The Thanh Hoa data series has been completed with Lang Chang series to obtain a complete series that represents the rainfall in the lower $5 \%$ of the catchment area under consideration. The minor gaps in the Bat Mot series were filled with Lang Chang data, to obtain a complete series that represent the rainfall on the central $60 \%$ of the area. Song Ma and Tuan Giao data series represent the rainfal on respectively the upper $20 \%$ and $15 \%$ of the Ma catchment. 25 year of areal daily rainfall series have been compiled using the above mentioned series and corresponding weights. Also series were compiled of two and three consecutive days area rainfall. From all these series the yearly maximum values were selected, which were put through a frequency analysis. The results are given in the table below using the log-normal and Gumbel probability distribution functions. These functions gave the best fit.

Maximum yearly one-day Ma area (excluding Chu basin) rainfall in mm

| Return Period | 10 years | 20 years | 50 years | 100 years |
| :---: | :---: | :---: | :---: | :---: |
| Log-normal | 141 | 159 | 179 | 194 |
| Gumbel | 141 | 158 | 181 | 198 |

## Maximum yearly two-day Ma area (excluding Chu basin) rainfall in mm

| Return Period | 10 years | 20 years | 50 years | 100 years |
| :---: | :---: | :---: | :---: | :---: |
| Log-normal | 213 | 238 | 275 | 301 |
| Gumbel | 212 | 240 | 277 | 304 |

## Maximum yearly three-day Ma area (excluding Chu basin) rainfall in mm

| Return Period | 10 years | 20 years | 50 years | 100 years |
| :---: | :---: | :---: | :---: | :---: |
| Log-normal | 253 | 291 | 333 | 367 |
| Gumbel | 253 | 288 | 333 | 368 |

These rainfall intensities correspond with the following n-day precipitation volumes on the Ma basin upstream of the Ma - Buoi confluence $\left(19,820 \mathrm{~km}^{2}\right)$

## Maximum yearly n-day rainfall volume on Ma basin u/s Ma - Buoi confluence in $\mathbf{M m}^{3}$

| Return period | 10 years | 20 years | 50 years | 100 years |
| :---: | :---: | :---: | :---: | :---: |
| 1 day | 2,800 | 3,100 | 3,600 | 3,900 |
| 2 days | 4,200 | 4,700 | 5,500 | 6,000 |
| 3 days | 5,000 | 5,700 | 6,600 | 7,300 |

## 4 CA RIVER BASIN

### 4.1 Basin Characteristics

### 4.1.1 River Basin

The Ca basin is located in the north-central Vietnam and borders with Laos. The river basin are situated between $18^{\circ} 00^{\prime}$ to $20^{\circ} 30^{\prime}$ north latitude and between $103^{\circ} 30^{\prime}$ to $106^{\circ} 00^{\prime}$ east longitude.

The catchment area of Ca river basin covers about $30,000 \mathrm{~km}^{2}$, which included some small basins located coastal Area. About some 30\% of catchment area are located in Laos. Major tributaries in the Ca system are:

| Ca River Basin (Total) | $29,850 \mathrm{~km}^{2}$ |
| :--- | ---: |
| Major Sub-basins |  |
| Hieu : | $5,290 \mathrm{~km}^{2}$ |
| Giang : | $1,090 \mathrm{~km}^{2}$ |
| Ngan Pho : | $1,120 \mathrm{~km}^{2}$ |
| Ngan Sau : | $2,090 \mathrm{~km}^{2}$ |
| Lower Area (Delta) | $4,420 \mathrm{~km}^{2}$ |

For the location map of the basin reference is made to Figure B.9.

### 4.1.2 Climatological Features

Mean annual temperature varies from $20^{\circ} \mathrm{C}$ to $23^{\circ} \mathrm{C}$ with the minimum temperature of $17^{\circ} \mathrm{C}$ and the maximum temperature of $29^{\circ} \mathrm{C}$. Mean annual humidity varies from some $75 \%$ on the higher elevated areas of the upper basin to $85 \%$ close to sea.

Mean annual rainfall varies from 1200 mm to 2400 mm . Heavy rains concentrate from August to October. There are two rainfall peaks, i.e. small in May and main rainfall in August to November.

### 4.2 Natural Runoff

Availability of water in the Ca basin is most important in the lower section of this basin downstream of Do Luong. At this location Ca waters are diverted to the coastal area north of the Ca basin. The natural runoff at Do Luong is therefore an important factor in the assessment of the water resources potential of Ca basin.

Downstream of Do Luong several small sub-catchment areas contribute to the availability of water in the lower Ca basin i.e. the Giang river and the Ngan Pho and Ngan Sau. The contribution of the Giang river is measured at the Yen Thuong station in the Ca river, downstream of the confluence with the Giang river. However, this contribution can only be estimated in an indirect way, since the discharge in the Ca river just upstream of this confluence is not known (as a consequence of the unknown volumes that are diverted at

## Do Luong).

The contribution of the Ngan Sau and the Ngan Pho are observed directly in respectively the Hoa Duyet station and the Son Diem station.

For the analysis of the basin runoff, the following discharge data have been collected and stored in the data base.

| Station | Longitude | Latitude | Catchment | Tributary | Observation Period |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Yen Thuong | 105.38 | 18.34 | $23,000 \mathrm{~km}^{2}$ | Ca | $1976-2000$ |
| Dua | 105.02 | 18.59 | $20,800 \mathrm{~km}^{2}$ | Ca | $1976-2000$ |
| Hoa Duyet | 105.35 | 18.22 | $1,880 \mathrm{~km}^{2}$ | Ngan Sau | 1976-1981. 1997-2000 |
| Son Diem | 105.21 | 18.30 | $790 \mathrm{~km}^{2}$ | Ngan Pho | 1976-1981.1997-2000 |

In addition to these discharge data, the following rainfall data have been collected:

| Station | Longitude | Latitude | Province | Observation Period |
| :---: | :---: | :---: | :---: | :---: |
| Hoa Duyet | 105.35 | 18.22 | Ha Tinh | $1976-2000$ |
| Son Diem | 105.20 | 18.30 | Ha Tinh | $1976-2000$ |
| Do Luong | 105.18 | 18.54 | Nghe An | $1976-2000$ |
| Mon Son | 104.55 | 18.53 | Nghe An | $1976-1990$ |
| Con Cuong | 104.53 | 19.02 | Nghe An | $1976-1989.1991-2000$ |
| Nghia Khanh | 105.20 | 19.26 | Nghe An | $1977-2000$ |
| Quy Chau | 105.06 | 19.34 | Nghe An | $1976-2000$ |
| Muong Xen | 104.08 | 19.24 | Nghe An | $1976-2000$ |
| Vinh | 105.40 | 18.40 | Nghe An | $1976-2000$ |

The runoff that is observed at Dua, corresponds with the runoff of more than $98 \%$ of the catchment area at Do Luong. It is therefore considered justified to derive the natural runoff at Do Luong directly from the discharge series of Dua station. Storage facilities upstream of this station are insignificant and the impact on the runoff is neglected.

For the contribution of the Giang river to the basin runoff, it is assumed that the runoff of this sub-basin per $\mathrm{km}^{2}$ is the same as for the rest of the middle and upper catchment of the Ma basin, including Ngan Pho and Ngan Sau.

The contribution of the latter two sub-basins has been derived from the observed discharges in the respective rivers. However, discharge series of these two stations were incomplete, making it necessary to find a relation to complete these series. The best relation was found by comparing the combined monthly runoff at Ngan Pho and Ngan Sau with the runoff at Dua. This relation reads as follows:

$$
\mathrm{Q}_{\text {HoaDuyet }}+\mathrm{Q}_{\text {SonDiem }}=0.103 \mathrm{Q}_{\text {Dua }}+54\left(\mathrm{in}^{3} / \mathrm{s}\right)
$$

The runoff of the lower basin has been has been estimated on the basis of the net rainfall on this area, as explained previously.

The series of monthly basin runoff for the period $1976-2000$ is presented in Table B.4. The corresponding dependable discharges are as follows:

$$
\text { Dependable Monthly Natural Runoff in the whole basin }\left(A=29,850 \mathrm{~km}^{2}\right)
$$

|  |  |  |  |  |  |  |  |  |  |  |  |  |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
|  | Jan | Feb | Mar | Apr | May | Jun | Jul | Aug | Sep | Oct | Nov | Dec |
| $50 \%$ | 714 | 564 | 546 | 547 | 897 | 1,219 | 1,573 | 2,623 | 4,315 | 4,444 | 1,839 | 952 |
| $75 \%$ | 605 | 482 | 464 | 472 | 694 | 895 | 1,187 | 1,924 | 2,916 | 2,914 | 1,258 | 775 |
| $90 \%$ | 521 | 419 | 400 | 413 | 551 | 677 | 922 | 1,457 | 2,049 | 1,994 | 894 | 645 |

### 4.3 Flood Runoff

The flood prone area of the Ca basin is situated downstream of Do Luong. At this location Ca waters are diverted to the coastal area north of the Ca basin. The peak discharges at Do Luong are therefore an important factor in the formulation of flood mitigation measures in the Ca basin.

Discharges measured at Dua station correspond to more than $98 \%$ of the catchment area at Do Luong. As a consequence, peak discharges at Do Luong are expected to deviate from the Dua peak discharges by less than $1 \%$. It is, therefore, considered justified to derive the peak discharges at Do Luong directly from the discharge series of Dua station. From the 1976-2000 discharge series at Dua, the yearly maximum daily discharges were collected and a frequency analysis has been carried out.

Several probability distribution functions have been assumed for the estimate of the peak discharges with corresponding return periods. The results of the most likely distribution functions are presented below and some are shown in Figure B.10.

Estimated Peak discharges at Dua with corresponding Return Period,Unit: $\mathbf{m}^{\mathbf{3} / \mathbf{s}}$

| Return Period (years) | 2 | 5 | 10 | 20 | 50 | 100 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Log Normal | 3,329 | 5,246 | 6,656 | 8,090 | 10,106 | 11,712 |
| Pearson-3 | 3,426 | 5,507 | 6,862 | 8,118 | 9,716 | 10,874 |
| Log Pearson | 3,218 | 5,351 | 7,066 |  | 11,756 | 14,162 |
| Gumbel | 3,484 | 5,468 | 6,781 | 8,041 | 9,672 | 10,894 |
| Average round values | 3,400 | 5,400 | 6,800 | 8,200 | 10,300 | 11,900 |

In addition to the above analysis, also historical annual maximum discharges at Yen Thuong have been analysed for the period 1976-2000. This station is located downstream of Do Luong. This analysis has given the following results:

Estimated Peak discharges in $\mathrm{m}^{3} / \mathbf{s}$ at Yen Thuong with corresponding Return Period, Unit:

| Return Period (years) | 2 | 5 | 10 | 20 | 50 | 100 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Log Normal | 3,621 | 5,119 | 6,136 | 7,070 | 8,432 | 9,433 |
| Pearson-3 | 3,701 | 5,250 | 6,206 | 7,075 | 8,139 | 8,902 |
| Gumbel | 3,663 | 5,159 | 6,149 | 7,080 | 8,329 | 9,250 |
| Average round values | 3,700 | 5,200 | 6,200 | 7,100 | 8,300 | 9,200 |

These discharges are some $10 \%$ (at the ten years peak) to over $20 \%$ (at the 100 years peak), lower than the peak flows at Dua, indicating that part of the peak discharges is diverted at Do Luong and/or stored in the flooded area between Do Luong and Yen Thuong.

For the estimate of volumes of floods that are generated by 1,2 and 3-day peak rainfall on the Ca basin upstream of the flood prone area, an analysis has been made of the daily rainfall upstream of Do Luong. The following daily rainfall data have been used:

| Do Luong | $1976-2000$ (with minor gaps) |
| :--- | :--- |
| Con Cuong | $1976-1989 ; 1990-2000$ (with minor gaps) |
| Quy Chau | $1976-2000$ (with minor gaps) |
| Muong Xen | $1976-2000$ (complete) |
| Mon Son | for gap filling |
| Nghia Khanh | for gap filling |

The Do Luong data series has been completed with Mon Son series to obtain a complete series that represents the rainfall in the lower $5 \%$ of the catchment area under consideration. The minor gaps in the Con Cuong series were filled with Mon Son data, to obtain a complete series that represent the rainfall on $15 \%$ of the central part of the area. Quy Chau series have been completed with Nghia Khanh data to create a complete series representing $20 \%$ of the central part of the catchment. Muong Xen series represent the rainfal on the upper $60 \%$ of the Ca catchment. 25 year of area daily rainfall series have been compiled using the above mentioned series and corresponding weights. Also series were compiled of two and three consecutive days area rainfall. From all these series the yearly maximum values were selected, which were put through a frequency analysis. The results are given in the table below using the log-normal and Gumbel probability distribution functions. These functions gave the best fit.

Maximum yearly one-day Ca area upstream of Do Luong rainfall in mm

| Return period in years | 2 | 5 | 10 | 20 | 50 | 100 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Log-normal | 87 | 119 | 140 | 162 | 188 | 209 |
| Gumbel | 87 | 119 | 141 | 161 | 188 | 208 |

Maximum yearly two-day Ca area rainfall upstream Do Luong in mm

| Return period in years | 2 | 5 | 10 | 20 | 50 | 100 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Log-normal | 122 | 169 | 200 | 229 | 268 | 297 |
| Gumbel | 123 | 169 | 199 | 229 | 267 | 295 |

Maximum yearly three-day Ca area rainfall upstream Do Luong in mm

| Return period in years | 2 | 5 | 10 | 20 | 50 | 100 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Log-normal | 136 | 188 | 223 | 258 | 301 | 334 |
| Gumbel | 137 | 189 | 223 | 256 | 299 | 331 |

These rainfall intensities correspond with the following n-day precipitation volumes on the Ca basin upstream of Do Luong (21,130 $\mathrm{km}^{2}$ ).

Maximum yearly n-day rainfall volume on Ca basin u/s Do Luong in Mm ${ }^{3}$

| Return period | 1 day | 2 days | 3 days |
| :---: | :---: | :---: | :---: |
| 2 years | 1,800 | 2,600 | 2,900 |
| 5 years | 2,500 | 3,600 | 4,000 |
| 10 years | 3,000 | 4,200 | 4,700 |
| 20 years | 3,400 | 4,800 | 5,400 |
| 50 years | 4,000 | 5,700 | 6,300 |
| 100 years | 4,400 | 6,300 | 7,000 |

A typical historical flood in the Ca basin with corresponding rainfall is shown in Figure B. 11 .

## 5 THACH HAN RIVER

### 5.1 Basin Characteristics

### 5.1.1 River Basin

Thach Han basin is situated in the middle central of Vietnam between $16^{\circ} 20^{\prime}$ to $16^{\circ} 55^{\prime}$ north latitude and $106^{\circ} 35^{\prime}$ to $107^{\circ} 15^{\prime}$ east longitude within Quang Tri province.

Thac Han river basin covers a total catchment area of only $2,550 \mathrm{~km}^{2}$, completely located in Vietnam.

The main stream of Thac Han River (Quang Tri River) originates from the east slopes of the Truong Son Mountain ranges which border with Lao. In this area, the river flows in the north-west direction. From the confluence with Rao Quan River, Thac Han River flows in the north-east direction and has its outlet to the East Sea at Dong Ha. Major tributaries in Thach Han system are:

| Thach Han Basin (Total) | $2,550 \mathrm{~km}^{2}$ |
| :--- | :--- |
| Major Sub-basins |  |
| Quang Tri u/s Thach Han weir : | $1,390 \mathrm{~km}^{2}$ |
| Cam Lo \& Vinh Phuoc : | $790 \mathrm{~km}^{2}$ |
| Lower Area | $370 \mathrm{~km}^{2}$ |

For the location map of the basin reference is made to Figure B.12.

### 5.1.2 Climatological Features

Mean annual temperature varies from $22.8^{\circ} \mathrm{C}$ to $24.9^{\circ} \mathrm{C}$ with the minimum temperature of $6.8^{\circ} \mathrm{C}$ and the maximum of $40^{\circ} \mathrm{C}$. Mean Annual humidity is $85 \%$. Evaporation varies from 800 mm in hilly areas and $1,000 \mathrm{~mm}$ to $1,500 \mathrm{~mm}$ in plains.

Mean annual rainfall varies from $2,300 \mathrm{~mm}$ to $2,800 \mathrm{~mm}$. There are 2 rains peaks year, i.e., small rainfall and heavy rainfall. The heavy rainfall concentrates in September to November. In May and June, rains may cause small floods. The dry season starts in January and end in August.

### 5.2 Natural Runoff

The only record of discharge observations in the Thach Han basin refer to Rao Quang measurement station where during a period of three years (1983-1985) the discharge was observed of a catchment of $159 \mathrm{~km}^{2}$, or $7 \%$ of the total Thach Han basin. This record is not considered adequate for the analysis of the basin runoff.

Rainfall data, however, are available to a greater extent, allowing for the estimate of the basin runoff on the basis of a rainfall - runoff relation. The following rainfall data have been collected for this purpose:

| Station | Longitude | Latitude | Province | Observation Period |
| :---: | :---: | :---: | :---: | :---: |
| Khe Sanh | 106.50 | 16.38 | Quang Tri | $1976-2000$ |
| Thach Han | 107.14 | 16.45 | Quang Tri | $1976-2000$ |
| Dong Ha | 107.05 | 16.50 | Quang Tri | $1976-2000$ |

The absence of discharge data complicates the assessment of a rainfall - runoff relation for the Thach Han basin. Coincidentally, however, the present study includes the preparation of a rainfall - runoff model for the neighbouring basin of the Huong river, which has similar characteristics as the Thach Han basin. Size and topographic features are comparable. It is therefor anticipated that model parameters that have been derived from the calibration of the Huong rainfall - runoff model can also be applied in a similar modelling of the Thach Han basin.

Based on this assumption, the runoff of the Thach Han basin has been generated with the help of the Sacramento rainfall - runoff concept and using the series of area rainfall on the Thach Han basin. The model was set up for the Thach Han sub-basin upstream of the Thach Han weir. The area of this sub-basin amounts to $1,390 \mathrm{~km}^{2}$, or $62 \%$ of the entire catchment area. The area rainfall on this sub-basin was assessed on the basis of the following rainfall stations and the corresponding weights:

| Sub-basin | Rainfall station |  |  |
| :---: | :---: | :---: | :---: |
|  | Thach Han | Aluoi | Khe Sanh |
| ThachHan-QuangTri | $20 \%$ | $25 \%$ | $55 \%$ |

Series of daily rainfall have been used for the period 1977 - 2000. Gaps in the series of rainfall data have been filled as follows:

- Thach Han series completed with Dong Ha data;
- Khe San series completed with A Luoi data.

The runoff of the entire Thach Han basin is composed of the runoff at the Thach Han weir, increased by the contribution of the Cam Lo river and the Vinh Phuoc river and the runoff of the lower area. This latter runoff has been estimated on the basis of the net rainfall on this area of $370 \mathrm{~km}^{2}$. The contribution of the Cam Lo and Vinh Phuoc, with a joint area of $790 \mathrm{~km}^{2}$, has been estimated on the assumption that the runoff of these sub-basins per $\mathrm{km}^{2}$ is the same as the runoff per $\mathrm{km}^{2}$ of the Thach Han sub-basin. The resulting generated basin runoff is presented in the Table B.5. The corresponding dependable monthly runoff is calculated as follows:

Dependable Monthly Natural Runoff in the whole basin ( $\mathrm{A}=\mathbf{2 , 5 5 0} \mathrm{km}^{2}$ )

|  | Jan | Feb | Mar | Apr | May | Jun | Jul | Aug | Sep | Oct | Nov | Dec |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| $50 \%$ | 243 | 135 | 94 | 63 | 75 | 78 | 71 | 113 | 320 | 850 | 836 | 513 |
| $75 \%$ | 191 | 107 | 75 | 47 | 50 | 48 | 43 | 68 | 189 | 612 | 587 | 384 |
| $90 \%$ | 154 | 87 | 61 | 36 | 35 | 31 | 27 | 43 | 117 | 456 | 427 | 296 |

### 5.3 Flood Runoff

As mentioned above, long series of historic data of flood events in the Thach Han basin are absent. The estimate of probable floods on the basis of a frequency analysis of historic data is, therefore, not feasible. On the other hand, long records of daily rainfall in the Thach Han basin exist, allowing for a frequency analysis of storm events in the basin. However, no discharge measurements during storm events are available and, therefore, calibration of rainfall - runoff modelling is not possible for the Thach Han basin. For the neighbouring Huong basin, such modelling was done and calibrated. The calibration results, in the form of parameters for the Sacramento model, have been used for the Thach Han basin as well, in combination with the area rainfall data of this basin.

For the present study the following relevant rainfall data were available:

| Available daily rainfall data Thach Han basin |  |
| :--- | :--- |
| Thach Han | August 1976 - 2000 (with 1 month gap) |
| Dong Ha | $1976-2000$ (with one month gap) |
| Khe San | July 1976-2000 (with 4 months gaps ) |
| A Luoi | $1976-2000$ (complete) |

The first two stations are located in the lower basin area. Khe San is located in the upper part of the basin, however at a relatively low location in the river valley, showing substantial lower rainfall than the higher area, that are represented better by the A Luoi station.

The gaps in the Thach Han and Dong Ha series have been filled mutually. The minor gaps in the Khe San series have been filled with A Luoi data.

For the estimate of the area rainfall, the following weights have been used:

| Sub-basin | Rainfall station |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
|  | Thach Han | Dong Ha | Khe San | A Luoi |
| Thach Han - Quang Tri | $20 \%$ | - | $55 \%$ | $25 \%$ |
| Lower area | - | $100 \%$ | - | - |

Frequency analyses were carried out using both the Gumbel and the Log-Normal probability distribution functions. The analyses were carried out for the yearly maximum area rainfall values and for the "early flood" rainfall values (maximum rainfall in period January - August). The Figures B.13. show the frequency curves for both types of storms.

The results (averaged and rounded Gumbel and Log Normal values) are given in the tables below:

One day maximum year rainfall in $\mathbf{m m}$

| Return Period (years) | 2 | 10 | 20 | 50 | 100 |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Upper and middle Thach Han basin | 175 | 275 | 315 | 370 | 410 |

One day maximum "early flood" rainfall in mm

| Return Period (years) | 2 | 10 | 20 | 50 | 100 |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Upper and middle Thach Han basin | 95 | 170 | 200 | 245 | 275 |

These values are substantially lower than the estimated peak rainfall in the Huong basin. During the main flood season, the Huong rainfall intensities are over $50 \%$ higher than in the Thach Han basin. During the early flood season this difference is of the order of $30 \%$.

For the simulation of the flood runoff, a hourly distribution of the above daily rainfall values is assumed similar to the distribution in the Huong basin. Maximum hourly intensities have been estimated between 35 mm ( 10 years storm) and 60 mm ( 100 years storm) during the main flood season and between 25 mm (10 years storm) and 40 mm (100 years storm) during the early flood season.

The response of the Quang Tri sub-basin upstream of the Thach Han weir to storms is supposed to be similar to the response of the Bo sub-basin to similar storms. However, the shape of the Quang Tri sub-basin and the length of the river branches justify the assumption that the Quang Tri floods are subject to more attenuation on their way down to the flood prone area then the Huong flood waves are. This phenomenon has been reflected by applying a Muskingum parameter K value of 7 (hours) as compared to 5 in the Bo basin. The estimated peak discharges and corresponding flood volumes resulting from a one day storm are as follows.

## Main flood at Thach Han weir

| Return Period $(\mathrm{years})$ | 2 | 10 | 20 | 50 | 100 |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Peak discharge $\left(\mathrm{m}^{3} / \mathrm{s}\right)$ | 1830 | 3,500 | 4,300 | 5,200 | 6,100 |
| Flood volume $\left(\mathrm{Mm}^{3}\right)$ | 95 | 200 | 250 | 310 | 360 |
| Runoff Coefficient | 0.39 | 0.54 | 0.57 | 0.61 | 0.63 |

Early flood at Thach Han weir

| Return Period $($ years $)$ | 2 | 10 | 20 | 50 | 100 |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Peak discharge $\left(\mathrm{m}^{3} / \mathrm{s}\right)$ | 870 | 1,800 | 2,300 | 3,000 | 3,500 |
| Flood volume $\left(\mathrm{Mm}^{3}\right)$ | 35 | 90 | 120 | 170 | 200 |
| Runoff Coefficient | 0.26 | 0.39 | 0.44 | 0.50 | 0.53 |

The above flood simulation has been made for the floods at the Thach Han weir. The confluences of the Vinh Phuoc and the Cam Lo river with the Thach Han river are situated in the flood prone area. If flood control measures are taken that prevent waters from both the Thach Han, the Vinh Phuoc and the Cam Lo to spill or divert from the respective river channels, than peak discharges at the Cam Lo confluence could be estimated by applying a Creager factor to the Thach Han weir peak discharges. This implies that the peak discharges at the Cam Lo confluence would be some $20 \%$ higher than the peak discharges with corresponding return period at Thach han weir.

## 6 HUONG RIVER BASIN

### 6.1 Basin Characteristics

### 6.1.1 River Basin

The Huong basin is located in the north-central Vietnam. The river basin are situated between $16^{\circ} 00^{\prime}$ to $16^{\circ} 40$ ' north latitude and between $107^{\circ} 15^{\prime}$ to $108^{\circ}$ east longitude within Thua Thien Hue province.

The Huong river basin covers a total catchment area about $3,300 \mathrm{~km}^{2}$, which included Truoi River basin and around lagoon area. The Huong River originates from the east slopes of the Truong Son Mountain ranges which border with Lao. The Huong River flows in the north and has its outlet to the Tam Giang Lagoon at Hue.

| Huong River Basin (Taotal) | $3,300 \mathrm{~km}^{2}$ |
| :--- | :--- |
| Major Sub-basins |  |
| Ta Trach (at Tuan) : | $790 \mathrm{~km}^{2}$ |
| Huu Trach (at Tuan) : | $670 \mathrm{~km}^{2}$ |
| Bo (at CoBi): | $720 \mathrm{~km}^{2}$ |
| Lower Huong (Delta) : | $460 \mathrm{~km}^{2}$ |
| Lagoon and Truoi Basin | $660 \mathrm{~km}^{2}$ |

For the location map of the basin reference is made to Figure B.14.

### 6.1.2 Climatological Features

Mean annual temperature varies from $21^{\circ} \mathrm{C}$ to $25^{\circ} \mathrm{C}$ with the minimum temperature of $16^{\circ} \mathrm{C}$ and the maximum temperature of $29^{\circ} \mathrm{C}$. Mean Annual humidity is $82 \%$.

Mean annual rainfall varies from $2,800 \mathrm{~mm}$ to $3,500 \mathrm{~mm}$. In the mountain area, i.e. Nam Dong and A Luoi areas have biggest rainfall. Heavy rains concentrate from September to beginning of December. Small rains occur in May and June. The dry season starts in end of December and lasts until July or beginning of August.

### 6.2 Natural Runoff

The Huong basin is considered a priority basin for which a comprehensive water resources management master plan is to be formulated in the framework of the present study.

In accordance with the Terms of Reference, special treatment has been given to the generation of the series of monthly runoff in this basin. This special approach is not only related to the importance that has been given to this basin, but also a consequence of the fact that in this basin no long records of discharge observations are available. The following discharge data could be collected:

| Station | Longitude | Latitude | Catchment | Tributary | Observation Period |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Thuong Nhat | 107.41 | 16.07 | $208 \mathrm{~km}^{2}$ | Ta Track | $1981-2000$ |
| Truoi | 107.46 | 16.15 | $70 \mathrm{~km}^{2}$ | Truoi | $1992-1996$ |
| Binh Dien | 107.30 | 16.20 | $570 \mathrm{~km}^{2}$ | Huu Track | $1979-1985$ |
| Co Bi | 107.25 | 16.28 | $720 \mathrm{~km}^{2}$ | Bo | $1979-1985$ |
| Duong Hoa | 107.38 | 16.18 | $717 \mathrm{~km}^{2}$ | Ta Track | $1986-1987$ |

For the generation of series of runoff in the respective sub-basins of the Huong, use has been made of rainfall-runoff modelling. The runoff series have been generated with the help of area rainfall series of the period 1977 - 2000 that have been compiled on the basis of the following set of collected daily rainfall data:

| Station | Longitude | Latitude | Province | Observation Period |
| :---: | :---: | :---: | :---: | :---: |
| Nam Dong | 107.43 | 16.09 | Thua Thien Hue | $1977-2000$ |
| Phu Loc | 107.53 | 16.15 | Thua Thien Hue | $1978-1990$ |
| Hue | 107.34 | 16.25 | Thua Thien Hue | $1976-2000$ |
| Phuoc | 107.28 | 16.32 | Thua Thien Hue | $1977.1980-2000$ |
| A Luoi | 107.13 | 16.15 | Thua Thien Hue | $1976-2000$ |

With the help of the rainfall - runoff modelling series have been generated for several key locations in the basin. In the Tables B.6, these series are presented for the Bo river at Co Bi, the Huu Trach - Ta Trach basin at the confluence at Tuan and for the entire basin. The latter series has been composed by adding up the runoff volumes of the Bo and the Ta Trach - Huu Trach and the runoff of the lower area that has been estimated with the help of the net rainfall on that area.

The dependable monthly runoff volumes at Co Bi , Tuan and for the entire basin are presented below:

## $\mathrm{CoBi}\left(\mathrm{A}=720 \mathrm{~km}^{2}\right)$

unit :million $\mathrm{m}^{3}$

|  | Jan | Feb | Mar | Apr | May | Jun | Jul | Aug | Sep | Oct | Nov | Dec |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| $50 \%$ | 95 | 48 | 33 | 32 | 45 | 41 | 32 | 32 | 85 | 298 | 363 | 251 |
| $75 \%$ | 58 | 30 | 22 | 21 | 32 | 27 | 21 | 22 | 52 | 205 | 254 | 181 |
| $90 \%$ | 38 | 20 | 16 | 16 | 23 | 18 | 15 | 15 | 33 | 147 | 185 | 135 |

Tuan ( $\mathrm{A}=1,460 \mathrm{~km}^{2}$ )

|  |  |  |  |  |  |  |  |  |  |  |  |  |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
|  | Jan | Feb | Mar | Apr | May | Jun | Jul | Aug | Sep | Oct | Nov | Dec |
| $50 \%$ | 210 | 109 | 71 | 52 | 68 | 69 | 53 | 57 | 148 | 589 | 726 | 498 |
| $75 \%$ | 144 | 79 | 53 | 35 | 40 | 40 | 32 | 36 | 84 | 370 | 497 | 354 |
| $90 \%$ | 102 | 59 | 40 | 24 | 26 | 24 | 20 | 23 | 51 | 243 | 353 | 260 |

## Entire Basin (A=3,300 $\mathbf{k m}^{\mathbf{2}}$ )

|  |  |  |  |  |  |  |  |  |  |  |  |  |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
|  | Jan | Feb | Mar | Apr | May | Jun | Jul | Aug | Sep | Oct | Nov | Dec |
| $50 \%$ | 342 | 166 | 112 | 86 | 120 | 122 | 90 | 95 | 431 | 1,497 | 1,623 | 989 |
| $75 \%$ | 220 | 118 | 81 | 56 | 76 | 71 | 55 | 61 | 243 | 976 | 1,153 | 691 |
| $90 \%$ | 148 | 87 | 61 | 38 | 51 | 44 | 36 | 41 | 144 | 663 | 847 | 500 |

### 6.3 Flood Runoff

Long series of historic data of flood events in the Huong basin are absent. In the major discharge measurement stations at Co Bi (Bo River), Binh Dien (Huu Track) and Duong Hoa (Ta Track) observations were made in the period 1979-1985 in Co Bi and Binh Dien, and during only two years (1986-1987) in Duong Hoa. The estimate of probable floods on the basis of a frequency analysis of historic data is, therefore, not feasible. On the other hand, long records of rainfall in the Huong basin exist, allowing for a frequency analysis of storm events in the basin. Moreover a number of flood events and corresponding rainfall were registered on an hourly basis, allowing the calibration and verification of rainfall runoff modelling under flood conditions.

Flood modelling with the help of rainfall - runoff modelling has been carried out with the help of the Sacramento model for the land-phase of the runoff generation, while the outflow of this land-phase was routed through the tributaries using the standard Muskingum method.

In addition to the Sacramento modelling, the modelling of the floods in the Huong basin has also been done with the help of the Mike 11 rainfall-runoff simulation module NAM. The auto-calibration option of this modelling package presented quite promising calibration results. Further model running with the parameters found through auto-calibration gave results that are substantially higher than the Sacramento modelling results. For the present study the following rainfall data were available:

## Available daily rainfall data Huong basin

| Hue | $1976-2000$ (with minor gaps) |
| :--- | :--- |
| Phu Oc | $1977,1980-2000$ (for gap filling) |
| Phu Loc | $1978-1990$ (for gap filling) |
| Nam Dong | $1977-2000$ (with minor gaps) |
| A Luoi | $1976-2000$ (complete) |

On the basis of the length of records and the spatial distribution, it was decided to use the data of Hue, Nam Dong and A Luoi for the analysis of the area rainfall in the respective sub-basins (Bo, Huu Trach, Ta Trach and the lower Huong basin). With Nam Dong series
starting at 1977, it was decided to complete the three series for the 24 years period 1977 2000.

The Hue data series has been completed with Phu Oc and Phu Loc series. Nam Dong series have been completed using A Luoi data.

For the assessment of the area rainfall on the respective sub-catchments the following area weights have been applied:

| Sub-catchment | Rainfall station |  |  |
| :---: | :---: | :---: | :---: |
|  | Hue | Nam Dong | A Luoi |
| Bo u/s Co Bi | $14 \%$ | $0 \%$ | $86 \%$ |
| Huu Trach u/s Binh Dien | $15 \%$ | $55 \%$ | $30 \%$ |
| Huu Trach u/s Tuan | $28 \%$ | $47 \%$ | $25 \%$ |
| Ta Trach u/s Duong Hoa | $0 \%$ | $100 \%$ | $0 \%$ |
| Ta Trach u/s Tuan | $10 \%$ | $90 \%$ | $0 \%$ |
| Lower Huong basin | $100 \%$ | $0 \%$ | $0 \%$ |

Frequency analyses were carried out using both the Gumbel and the Log-Normal probability distribution functions. The analyses were carried out for the yearly maximum area rainfall values and for the "early flood" rainfall values (maximum rainfall in period January - August). The respective probability distributions are shown in the Figures B. 15 - B. 18 .

The results (averaged and rounded Gumbel and Log Normal values) are given in the tables below:

One day maximum year rainfall in mm

| Return Period (years) | 2 | 5 | 10 | 20 | 50 | 100 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Bo u/s Co Bi | 260 | 365 | 440 | 505 | 600 | 665 |
| Huu Trach u/s Binh Dien | 285 | 355 | 400 | 450 | 500 | 550 |
| Ta Trach u/s Duong Hoa | 340 | 425 | 475 | 525 | 590 | 635 |
| Lower Huong basin | 240 | 345 | 425 | 495 | 585 | 655 |

One day maximum "early flood" rainfall in mm

| Return Period (years) | 2 | 5 | 10 | 20 | 50 | 100 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Bo u/s Co Bi | 85 | 130 | 165 | 195 | 235 | 270 |
| Huu Trach u/s Binh Dien | 85 | 155 | 210 | 260 | 335 | 400 |
| Ta Trach u/s Duong Hoa | 100 | 185 | 260 | 330 | 420 | 495 |
| Lower Huong basin | 95 | 160 | 210 | 255 | 320 | 365 |

For the simulation of the flood runoff in the respective sub-basins, a certain hourly distribution of the above daily rainfall values is assumed. From the available hourly rainfall data during flood events it was learned that peak intensities are in the order of 30
$-70 \mathrm{~mm} /$ hour. From these records it is moreover learned that during excessive rainfall events the high rainfall intensities last some $8-10$ hours. In the present analysis it is assumed that the daily rainfall is concentrated in 12 hours and that the highest intensities are $30 \mathrm{~mm} /$ hour for the moderate storms, increasing to values of $70 \mathrm{~mm} / \mathrm{hour}$ for the most extreme events.

For the estimate of flood runoff volumes related to storms with longer duration then one day, also the 2, 3, 4 and 5 day probable yearly rainfall has been calculated for the full year and the early season. These rainfalls have been calculated for the areas upstream of the potential dam sites. From the rainfall runoff modelling it is learned that the flood runoff volume can be approximated by using the following runoff factors (is part of the storm volume that is converted directly into runoff):

| Sub-basin | Main Flood | Early Flood |
| :---: | :---: | :---: |
| Bo sub-basin | 0.75 | 0,50 |
| Huu Trach - Ta Trach sub-basin | 0.80 | 0.55 |

Two-day maximum year rainfall in mm

| Return Period (years) | 2 | 5 | 10 | 20 | 50 | 100 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Bo u/s Co Bi | 390 | 565 | 690 | 805 | 965 | 1085 |
| Huu Trach u/s Binh Dien | 400 | 545 | 640 | 725 | 845 | 935 |
| Ta Trach u/s Duong Hoa | 470 | 635 | 745 | 850 | 980 | 1080 |

Two-day maximum "early flood" rainfall in mm

| Return Period (years) | 2 | 5 | 10 | 20 | 50 | 100 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Bo u/s Co Bi | 115 | 180 | 225 | 275 | 345 | 400 |
| Huu Trach u/s Binh Dien | 115 | 210 | 290 | 375 | 510 | 625 |
| Ta Trach u/s Duong Hoa | 135 | 260 | 360 | 475 | 650 | 805 |

Three-day maximum year rainfall in $\mathbf{m m}$

| Return Period (years) | 2 | 5 | 10 | 20 | 50 | 100 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Bo u/s Co Bi | 450 | 690 | 855 | 1020 | 1240 | 1405 |
| Huu Trach u/s Binh Dien | 475 | 675 | 810 | 935 | 1110 | 1240 |
| Ta Trach u/s Duong Hoa | 550 | 770 | 915 | 1055 | 1240 | 1380 |

Three-day maximum "early flood" rainfall in mm

| Return Period (years) | 2 | 5 | 10 | 20 | 50 | 100 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Bo u/s Co Bi | 130 | 200 | 250 | 295 | 365 | 420 |
| Huu Trach u/s Binh Dien | 130 | 225 | 310 | 400 | 525 | 630 |
| Ta Trach u/s Duong Hoa | 150 | 275 | 380 | 495 | 670 | 815 |

Four-day maximum year rainfall in mm

| Return Period (years) | 2 | 5 | 10 | 20 | 50 | 100 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Bo u/s Co Bi | 510 | 800 | 1000 | 1205 | 1470 | 1680 |
| Huu Trach u/s Binh Dien | 545 | 800 | 970 | 1140 | 1360 | 1530 |
| Ta Trach u/s Duong Hoa | 630 | 905 | 1090 | 1270 | 1510 | 1690 |

Four-day maximum "early flood" rainfall in mm

| Return Period (years) | 2 | 5 | 10 | 20 | 50 | 100 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Bo u/s Co Bi | 140 | 210 | 260 | 305 | 370 | 420 |
| Huu Trach u/s Binh Dien | 135 | 240 | 320 | 405 | 535 | 640 |
| Ta Trach u/s Duong Hoa | 160 | 290 | 400 | 515 | 685 | 825 |

Five-day maximum year rainfall in $\mathbf{m m}$

| Return Period (years) | 2 | 5 | 10 | 20 | 50 | 100 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Bo u/s Co Bi | 560 | 885 | 1110 | 1340 | 1645 | 1880 |
| Huu Trach u/s Binh Dien | 585 | 860 | 1050 | 1230 | 1475 | 1660 |
| Ta Trach u/s Duong Hoa | 670 | 970 | 1170 | 1370 | 1630 | 1825 |

Five-day maximum "early flood" rainfall in mm

| Return Period (years) | 2 | 5 | 10 | 20 | 50 | 100 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Bo u/s Co Bi | 155 | 225 | 270 | 320 | 380 | 430 |
| Huu Trach u/s Binh Dien | 145 | 250 | 330 | 415 | 540 | 645 |
| Ta Trach u/s Duong Hoa | 170 | 300 | 405 | 525 | 685 | 825 |

The estimated peak discharges and corresponding flood volumes resulting from the one day design storm are as follows:

## Main flood at $\mathbf{C o} \mathbf{B i}$

| Return Period (years) | 10 | 20 | 50 | 100 |
| :---: | :---: | :---: | :---: | :---: |
| Peak discharge $\left(\mathrm{m}^{3} / \mathrm{s}\right)$ <br> Flood volume $\left(\mathrm{Mm}^{3}\right)$ | 3,900 <br> 200 | 5,100 <br> 250 | 6,700 <br> 320 | 7,800 <br> 370 |
| Runoff Coefficient | 0.65 | 0.69 | 0.74 | 0.76 |

Early flood at Co Bi

| Return Period (years) | 10 | 20 | 50 | 100 s |
| :---: | :---: | :---: | :---: | :---: |
| Peak discharge $\left(\mathrm{m}^{3} / \mathrm{s}\right)$ | 1,000 | 1,250 | 1,600 | 2,000 |
| Flood volume $\left(\mathrm{Mm}^{3}\right)$ | 40 | 60 | 80 | 100 |
| Runoff Coefficient | 0.35 | 0.41 | 0.47 | 0.51 |

## Main flood at Tuan

| Return Period (years) | 10 | 20 | 50 | 100 |
| :---: | :---: | :---: | :---: | :---: |
| Peak discharge $\left(\mathrm{m}^{3} / \mathrm{s}\right)$ | 9,800 | 11,400 | 13,400 | 15,100 |
| Flood volume $\left(\mathrm{Mm}^{3}\right)$ | 490 | 570 | 660 | 730 |
| Runoff Coefficient | 0.77 | 0.79 | 0.81 | 0.83 |

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## Early flood at Tuan

| Return Period (years) | 10 | 20 | 50 | 100 |
| :---: | :---: | :---: | :---: | :---: |
| Peak discharge $\left(\mathrm{m}^{3} / \mathrm{s}\right)$ | 3,100 | 4,400 | 6,400 | 8,400 |
| Flood volume $\left(\mathrm{Mm}^{3}\right)$ | 140 | 200 | 300 | 390 |
| Runoff Coefficient | 0.40 | 0.47 | 0.55 | 0.61 |

## 7 VU GIA AND THU BON RIVER BASIN

### 7.1 Basin Characteristics

### 7.1.1 River Basin

Thu Bon river basin is contained entirely in Vietnam. The river basin are situated between $15^{\circ} 00^{\prime}$ to $16^{\circ} 00$ north latitude and between $107^{\circ} 00^{\prime}$ to $108^{\circ} 30^{\prime}$ east longitude. This basin covers Quang Nam province and Da Nang city.

This basin is combined with the Thu Bon and Vu Gia River. These rivers are connecting at downstream of Ai Nghia and flow into the sea at Hoi An and Da Nang.

This basin covers a total catchment area about $11,500 \mathrm{~km}^{2}$, which included TamKy River basin and some small river basin located connecting coastal area. Major tributaries and its catchement area are shown as follows:

| Vu Gia and Thu Bon Basin (Total) | $11,510 \mathrm{~km}^{2}$ |
| :--- | ---: |
| Major Sub-basins |  |
| Thu Bon (without lower area): | $3,590 \mathrm{~km}^{2}$ |
| Vu Gia (without lower area): | $5,420 \mathrm{~km}^{2}$ |
| Lower Area (Delta) : | $1,370 \mathrm{~km}^{2}$ |
| Tam Ky Basin : | $1,130 \mathrm{~km}^{2}$ |

For the location map of the basin reference is made to Figure B.19.

### 7.1.2 Climatological Features

Mean annual temperature varies from $24^{\circ} \mathrm{C}$ to $26^{\circ} \mathrm{C}$. Mean annual humidity varies from $80 \%$ to $87 \%$ and annual evaporation varies from 700 mm to 1100 mm .

Mean annual rainfall varies from $2,000 \mathrm{~mm}$ to $4,000 \mathrm{~mm}$. In the southern region of basin have biggest rainfall. There are 2 rains peaks year, i.e., small rainfall and heavy rainfall. The heavy rainfall concentrates in October to December. In May and June, rains may cause small floods.

## 7.2 <br> Natural Runoff

For the assessment of the water availability in the Thu Bon basin, an estimate has been made of the natural runoff of the basin at the confluence of the Thu Bon and Vu Gia tributaries at Ai Nghia. Upstream of this confluence $9,010 \mathrm{~km}^{2}$, or some $87 \%$, of the entire Tu Bon basin (excluding the Tam Ky basin) is situated. In the flat area downstream of this confluence the runoff conditions are essentially different from the upstream area as a consequence of topographic and land use features. The runoff of this lower part is, therefore, treated separately.

For the estimate of the natural runoff at Ai Nghia, use has been made of historical
discharge series at Nong Son in the Thu Bon sub-basin and Thanh My in the Vu Gia sub-basin. Complete simultaneous records of daily discharges in both stations are available since 1984 only. Prior to this year no records are available for Thanh My station. For the present study it is considered appropriate to use the 1984 - 2000 discharge series for the estimate of the natural runoff of the basin. The following discharge data have been collected and processed:

| Station | Longitude | Latitude | Catchment | Tributary | Observation Period |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Nong Son | 108.03 | 15.42 | $3,130 \mathrm{~km}^{2}$ | Thu Bon | $1976-2000$ |
| Thanh My | 107.50 | 15.46 | $1,850 \mathrm{~km}^{2}$ | Vu Gia | $1984-2000$ |

In addition to these discharge data, the following rainfall data have been used for the hydrological analysis:

| Station | Longitude | Latitude | Province | Observation Period |
| :---: | :---: | :---: | :---: | :---: |
| Tra My | 108.14 | 15.21 | Quang Nam | $1977-2000$ |
| Son Tan (HiepDuc) | 108.05 | 15.35 | Quang Nam | $1976-2000$ |
| Trao (Hien) | 107.38 | 15.56 | Quang Nam | $1978-2000$ |
| Thanh My | 107.50 | 15.46 | Quang Nam | $1976-2000$ |
| Ai Nghia | 108.07 | 15.53 | Quang Nam | $1976-2000$ |
| DaNang | 108.11 | 16.02 | Da Nang | $1976-2000$ |

The discharges observed at Nong Son represent the runoff of $3,130 \mathrm{~km}^{2}$, or some $87 \%$, of the $3,590 \mathrm{~km}^{2}$ Thu Bon sub-basin. It is, therefore, considered acceptable to estimate the runoff of the entire sub-basin by assuming the ratio between the entire sub-basin area and the observed area to be the same as the ratio between the respective discharges. In this way, the monthly Thu Bon sub-basin discharges have been estimated by multiplying the observed monthly discharges at Nong Son with the factor 1.147.

The discharges observed at Thanh My represent the runoff of $1,850 \mathrm{~km}^{2}$ of the $5,420 \mathrm{~km}^{2}$ Vu Gia sub-basin. This corresponds with only $34 \%$ of the total sub-basin. Before using the Thanh My discharge series for estimating the Vu Gia sub-basin runoff, the following analyses have been carried out.
a. in order to assess to what extent the observed Thu Bon sub-basin runoff can also be considered representative for the Vu Gia sub-basin, a comparison was made between the area rainfall on the Thu-Bon sub-basin and the area rainfall on the Vu Gia sub-basin.
b. With the same objective, the correlation has been examined between the monthly observed discharges at Nong Son in the Thu Bon sub-basin and the monthly observed
discharges at Thanh My in the Vu Gia sub-basin.
Area rainfall on the respective sub-basins has been estimated on the basis of historical rainfall series, assuming the following weights:

| Sub-basin | Rainfall station |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | Ai Nghia | Son Tan | Tra My | Thanh My | Trao |
| Thu Bon | $5 \%$ | $40 \%$ | $45 \%$ | $10 \%$ | - |
| Vu Gia | $8 \%$ | $12 \%$ | $8 \%$ | $36 \%$ | $36 \%$ |
| Vu Gia u/s Thanh My | $0 \%$ | $30 \%$ | $20 \%$ | $50 \%$ | - |

The results of these analyses indicate that the area rainfall on the Thu Bon sub-basin is substantially higher than the area rainfall on the Vu Gia sub-basin. From the daily rainfall data for the period $1984-2000$ it is learned that the average yearly rainfall on the Thu Bon sub-basin amounts to $3,421 \mathrm{~mm}$, as compared to $2,438 \mathrm{~mm}$ on the Vu Gia sub-basin. Assuming a similar rainfall-runoff relation for both basins and taking into account the catchment areas of respectively $3,590 \mathrm{~km}^{2}$ and $5,420 \mathrm{~km}^{2}$, it can be anticipated that the Vu Gia runoff is less than $10 \%(7.6 \%)$ higher than the Thu Bon runoff, notwithstanding the fact that the Vu Gia sub-basin is 1.5 times as big as the Thu Bon sub-basin. The assumption of a similar runoff-rainfall relation in both sub-basins may even lead to an overestimate of the Vu Gia runoff, since the runoff coefficients tend to increase with increasing rainfall intensities.

Comparing the rainfall on that part of the Vu Gia sub-basin that generates the runoff that is observed at Thanh My with the Vu Gia sub-basin rainfall, it is learned that the average yearly rainfall on the upstream Thanh My area $(2,845 \mathrm{~mm})$ is about $17 \%$ higher than the Vu Gia sub-basin rainfall. The assumption that the runoff per $\mathrm{km}^{2}$ at Thanh My is representative for the entire Vu Gia sub-basin, may, therefore, lead to an overestimate of the Vu Gia runoff.

Comparison of the monthly observed discharges at Nong Son (Thu Bon sub-basin) and Thanh My (Vu Gia sub-basin) confirms the substantially higher specific runoff in the Thu-Bon basin as compared to the Vu Gia sub-basin. With a correlation coefficient of $\mathrm{R}^{2}=0.9575$ the following relation is found for the monthly discharges (in $\mathrm{m}^{3} / \mathrm{s}$ ):

$$
\mathrm{Q}_{\text {Than My }}=0.4075 \mathrm{Q}_{\text {Nong Son }}+15
$$

This indicates that, taking into account the different catchment areas, the discharge per $\mathrm{km}^{2}$ at Thanh My is of the order of only $70 \%$ of the discharge per $\mathrm{km}^{2}$ at Nong Son. This percentage increases to some $90 \%$ when only the dry months (January -September) are taken into account. This can be explained by the fact that during the dry period the runoff is merely determined by base flow, rather than by the direct runoff from the rainfall.

From the results of the above analyses the following is concluded:

1. The use of Thu Bon sub-basin runoff rates for the entire Thu Bon - Vu Gia basin is not acceptable as a consequence of the significant variation of the rainfall between the respective sub-basins.
2. The use of the Thanh My runoff per $\mathrm{km}^{2}$ for the entire Vu Gia basin will give an overestimate of the natural runoff of this sub-basin.
3. This overestimate will be higher in the wet season than in the dry period.

On the basis of the area rainfall it was found that the year rainfall in the Vu Gia basin upstream of Thanh My is some $17 \%$ higher than for the entire Vu Gia basin. Using the Thanh My runoff rate for the entire basin may give a year around runoff overestimate in the order of $15-20 \%$. This overestimate will be reflected mainly in the wet season runoff. The overestimate during the dry season could be substantially lower, assuming that the base flow runoff is similar in the entire sub-basin, as appears to be the case when comparing the Thu Bon and Vu Gia runoff under low flow conditions. In this study, the overestimate during the dry season is assumed to be round $5 \%$.

Based on this consideration the series of monthly discharges of the Vu Gia sub-basin at the confluence with the Thu Bon has been derived by applying $95 \%$ of the runoff per $\mathrm{km}^{2}$ at Thanh My for the entire sub-basin. By this approach, the runoff during the flood season is likely to be overestimated. This, however, is not considered of relevance for the elaboration of the water balance of the Thu Bon basin.

The runoff series that has been generated in accordance with above described approach for the entire Thu Bon basin for the period 1984 - 2000 is presented in Table B.7. This runoff includes the runoff of the Thu Bon and Vu Gia sub-basins and the runoff from the $1,370 \mathrm{~km}^{2}$ lower area, the runoff of which has been estimated on the basis of the net rainfall on that area. The Tam Ky basin has not been included in the Thu Bon basin runoff. The dependable basin runoff has been calculated as follows:

Dependable Monthly Natural Runoff in the whole basin ( $\mathrm{A}=\mathbf{1 0}, 460 \mathrm{~km}^{2}$ )

|  |  |  |  |  |  |  |  |  |  |  |  |  |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
|  | Jan | Feb | Mar | Apr | May | Jun | Jul | Aug | Sep | Oct | Nov | Dec |
| $50 \%$ | 1,492 | 845 | 637 | 515 | 734 | 635 | 524 | 584 | 1,141 | 4,344 | 5,617 | 3,953 |
| $75 \%$ | 1,169 | 682 | 522 | 387 | 523 | 464 | 409 | 428 | 777 | 2,768 | 3,832 | 2,760 |
| $90 \%$ | 939 | 562 | 437 | 300 | 385 | 349 | 327 | 323 | 549 | 1,845 | 2,716 | 1,997 |

## 7.3 <br> Flood Runoff

The flood prone area of the Thu Bon basin is situated downstream of the confluence of the Thu Bon and Vu Gia river. At this confluence the sub-catchments of Thu Bon and Vu Gia have an area of $3,590 \mathrm{~km}^{2}$ and $5,420 \mathrm{~km}^{2}$ respectively. The discharges of $87 \%$ of the Thu Bon sub-catchment are observed at Nong Son. In the Vu Gia sub-catchment
observations are available of only $34 \%$ of the area, at Thanh My.
For the Nong Son station, data are available for the period 1976-2000 (25 years), while observations in Thanh My are restricted to the period 1984 - 2000 (17 years). From the 1976-2000 discharge series at Nong Son the yearly maximum daily discharges were collected and a frequency analysis has been carried out.

Several probability distribution functions have been assumed for the estimate of the peak discharges with corresponding return periods. The results of the most likely distribution functions are presented below:

Unit: $\mathrm{m}^{3} / \mathrm{s}$
Estimated Peak discharges in $\mathrm{m}^{3} / \mathbf{s}$ at Nong Son with corresponding Return Period

| Return Period (years) | 2 | 5 | 10 | 20 | 50 | 100 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Log Normal | 5,433 | 7,505 | 8,888 | 10,150 | 11,956 | 13,276 |
| Pearson-3 | 5,542 | 7,667 | 8,964 | 10,129 | 11,564 | 12,583 |
| Log Pearson | 5,324 | 7,486 | 9,045 |  | 12,844 | 14,622 |
| Gumbel | 5,466 | 7,526 | 8,890 | 10,198 | 11,892 | 13,161 |
| Average round values | 5,400 | 7,500 | 8,900 | 10,300 | 12,100 | 13,400 |

For the maximum yearly discharges at Thanh My, a similar analysis has been carried out. It is noted that the number of values (17) is not considered adequate for such analysis. However, in the absence of a proper rainfall - runoff model for the entire catchment, it is considered appropriate to make use to the maximum of the available information. The results are presented below:
Estimated Peak discharges in $\mathrm{m}^{3} / \mathrm{s}$ at Thanh My with corresponding Return Period,

| Ueturn Period (years) |  |  |  |  |  |  |  | 2 | 5 | 10 | 20 | 50 | 100 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Log Normal | 3,304 | 4,831 | 5,894 | 6,907 | 8,353 | 9,447 |  |  |  |  |  |  |  |
| Pearson-3 | 3,387 | 4,987 | 5,909 | 6,904 | 8,044 | 8,861 |  |  |  |  |  |  |  |
| Log Pearson | 3,175 | 4,914 | 6,271 | 7,449 | 9,881 | 11,697 |  |  |  |  |  |  |  |
| Gumbel | 3,373 | 4,910 | 5,928 | 6,902 | 8,168 | 9,115 |  |  |  |  |  |  |  |
| Average round values | 3,300 | 4,900 | 6,000 | 7,000 | 8,000 | 9,800 |  |  |  |  |  |  |  |

Assuming a Creager type of relation between the peak discharges and the catchment area, a first approximation of the peak flows at the confluence of the Vu Gia and the Thu Bon (total catchment area: $9,010 \mathrm{~km}^{2}$ ) is as follows:

Approximate peak discharges in $\mathrm{m}^{3} / \mathrm{s}$ at the Vu Gia- Thu Bon confluence.

| Return Period (years) | 2 | 5 | 10 | 20 | 50 | 100 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Peak Discharge | 8,000 | 10,500 | 12,500 | 14,000 | 17,000 | 19,000 |

The concentration time of the basin is estimated of the order of 1 day. It is, therefore, anticipated that intensities corresponding with one day rainfall are indicative for the peak
floods.
For both the Thu Bon sub-basin and the Vu Gia sub-basin, the maximum one-day rainfall has been analysed. The following daily rainfall data have been used:

| Trao | $1978-2000$ (complete) |
| :--- | :--- |
| Thanh My | $1978-2000$ (complete) |
| Son Tan | $1978-2000$ (with minor gap) |
| Tra My | $1978-2000$ (with minor gap) |
| Ai Nghia | $1978-2000$ (with minor gap) |

The Trao and Thanh My data series represent each the rainfall of $40 \%$ of the Vu Gia basin. The minor gaps in the Son Tan series were filled with Tra My data, to obtain a complete series that represent the rainfall on $10 \%$ of the upper Vu Gia basin and $35 \%$ of central part of the Thu Bon basin. Tra My series have been completed with Son Tan data to create a complete series representing the upper $10 \%$ of the Vu Gia basin and the upper $50 \%$ of the Thu Bon basin. The lower $15 \%$ of the Thu Bon basin is represented by Ai Nghia station, the series of which have been completed with Thanh My data.

23 years of areal daily rainfall series have been compiled for both the Vu Gia and the Tu Bon basin, using the above mentioned series and corresponding weights. The results are given in the table below using the log-normal and Gumbel probability distribution functions. These functions gave the best fit.

Maximum yearly one-day area rainfall on the Vu Gia basin

| Return period in years | 2 | 5 | 10 | 20 | 50 | 100 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Log-normal | 183 | 255 | 302 | 348 | 408 | 453 |
| Gumbel | 185 | 255 | 302 | 347 | 405 | 448 |

## Maximum yearly one-day area rainfall on the Thu Bon basin

| Return period in years | 2 | 5 | 10 | 20 | 50 | 100 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Log-normal | 215 | 284 | 328 | 370 | 423 | 462 |
| Gumbel | 215 | 283 | 327 | 370 | 426 | 468 |

Maximum yearly one-day area rainfall on combined Vu Gia - Thu Bon basin

| Return period in years | 2 | 5 | 10 | 20 | 50 | 100 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Log-normal | 189 | 246 | 282 | 314 | 359 | 391 |
| Gumbel | 188 | 244 | 281 | 317 | 363 | 398 |

The corresponding volume of one day rainfall on the middle and upper Vu Gia - Thu Bon catchment corresponds with:

Maximum yearly one-day area rainfall volume in $\mathrm{Mm}^{3}$ on the Vu Gia - Thu Bon basin

| Return period in years |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 2 | 5 | 10 | 20 | 50 | 100 |  |
| 1,700 | 2,200 | 2,500 | 2,800 | 3,300 | 3,600 |  |

Early floods may occur as a consequence of high rainfall intensities during the period prior to main wet season. Here the early floods are defined as the floods that are a result of storms in the period January - August.

For this period of the year the high flow analysis has been carried out similarly to the analysis for full years. The results of the most likely distribution functions are presented below and some are shown in Figure B.20.

Estimated Early Flood Peak discharges in $\mathrm{m}^{3} / \mathrm{s}$ at Nong Song with corresponding Return Period

| Return Period (years) | 2 | 5 | 10 | 20 | 50 | 100 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Peak Discharge | 700 | 1,400 | 2,000 | 2,600 | 3,400 | 4,000 |

Estimated Early Flood Peak discharges in $\mathrm{m}^{3} / \mathrm{s}$ at Thanh My with corresponding Return Period

| Return Period (years) | 2 | 5 | 10 | 20 | 50 | 100 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Peak Discharge | 400 | 900 | 1,400 | 1,900 | 2,500 | 3,100 |

The conversion of these peak discharges into early flood discharges at the Vu Gia - Thu Bon confluence gives the following approximation.

Approximate early flood peak discharges in $\mathrm{m}^{3} / \mathrm{s}$ at the Vu Gia- Thu Bon confluence.

| Return Period (years) | 2 | 5 | 10 | 20 | 50 | 100 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Peak Discharge | 1,000 | 2,000 | 2,800 | 3,700 | 4,800 | 5,800 |

Early storms have been analysed, and the following maximum one-day precipitation has been found for the entire area upstream of the Vu Gia - Thu Bon confluence:

Maximum yearly early one-day area rainfall on combined Vu Gia - Thu Bon basin

| Return period in years | 2 | 5 | 10 | 20 | 50 | 100 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Log-normal | 64 | 99 | 124 | 152 | 187 | 215 |
| Gumbel | 66 | 103 | 126 | 150 | 180 | 202 |

The corresponding volume of one day rainfall on the middle and upper Vu Gia - Thu Bon catchment prior to September corresponds with:

Maximum yearly early one-day area rainfall volume in $\mathbf{M m}^{3}$ on the $\mathrm{Vu} \mathbf{G i a}$ - Thu Bon basin

| Return period in years |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 2 | 5 | 10 | 20 | 50 | 100 |  |
| 600 | 9,100 | 1,100 | 1,400 | 1,700 | 1,900 |  |

## 8 TRA KHUC RIVER BASIN

### 8.1 Basin Characteristics

### 8.1.1 River Basin

The basin is situated in south central Vietnam between $14^{\circ} 30^{\prime}$ to $15^{\circ} 20^{\prime}$ north latitude and $108^{\circ} 15^{\prime}$ to $109^{\circ}$ east longitude.

Tra Khuc river basin covers a total catchment area of $5,200 \mathrm{~km}^{2}$, which included Ve River basin and some small basin located connecting coastal area.

Major tributaries and its catchement area are shown as follows:

| Tra Khuc Basin (Total) | $5,200 \mathrm{~km}^{2}$ |
| :--- | :--- |
| Major Sub-basins |  |
| Tra Khuc (without lower area) : | $3,030 \mathrm{~km}^{2}$ |
| Ve (without lower area) : | $820 \mathrm{~km}^{2}$ |
| Lower \& Coastal area : | $840 \mathrm{~km}^{2}$ |

The location map of the basin is presented in Figure B.21.

### 8.1.2 Climatological Features

Mean annual temperature varies from $20^{\circ} \mathrm{C}$ to $26^{\circ} \mathrm{C}$, annual humidity is about $85 \%$ and annual evaporation varies from 680 mm to $1,040 \mathrm{~mm}$.

Mean annual rainfall varies from 1700 mm to 2200 mm in plains; 2300 mm to 2700 mm in low mountains (AnChi, SonHa) and 3200 mm to 4000 mm in high mountains (TraBong, BaTo, GiaVuc).

The rainy season lasts in 4 month from September to December. Specially, heavy rains usually happen in October and November.

### 8.2 Natural Runoff

The Tra Khuc basin is composed of a number of sub-basins that each has its own exit to the sea. The main sub-basin is the Tra Khuc itself and the second important one is the Ve sub-basin. Other smaller sub-basins, among them the Tra Cau, enter the common coastal zone of the Tra Khuc basin.

Daily discharge data in the basin are scarce. The following data have been collected and processed:

| Station | Longitude | Latitude | Catchment | Tributary | Observation Period |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Son Giang | 108.34 | 15.02 | $2,440 \mathrm{~km}^{2}$ | Tra Khuc | $1987-2000$ |
| An Chi | 108.49 | 14.59 | $814 \mathrm{~km}^{2}$ | Ve | $1987-2000$ |

The period of 14 years only is considered short for making a proper assessment of the natural runoff and water availability in the basin. Therefore, efforts have been made to
extent the series to a longer period. The following two methods have been investigated:

1. Correlation with rainfall, for which data are available as from 1976,
2. Correlation with the neighbouring catchment area of Thu Bon.

The set of rainfall data that have been collected and processed is as follows:

| Station | Longitude | Latitude | Province | Observation Period |
| :---: | :---: | :---: | :---: | :---: |
| Tra Bong | 108.32 | 15.15 | Quang Ngai | $1976-2000$ |
| Quang Ngai | 108.47 | 15.09 | Quang Ngai | $1976-2000$ |
| Ba To | 108.45 | 14.46 | Quang Ngai | $1976-2000$ |
| Gia Vuc | 108.34 | 14.42 | Quang Ngai | $1977-2000$ |
| Son Giang (Son Ha) | 108.34 | 15.02 | Quang Ngai | $1977-2000$ |

For the catchment area upstream of the Son Giang gauging station (2,440 $\mathrm{km}^{2}$ ), monthly area rainfall series were compiled for the period 1976 (June) - 2000 (December). For this compilation the observed daily rainfall data were used of the following stations:

| Sub-basin | Rainfall station |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
|  | Gia Vuc | Tra Bon | Quang Ngai | Ba To |
| TraKhuc u/s SonGiang | $55 \%$ | $35 \%$ | $6 \%$ | $4 \%$ |

Small gaps in the Gia Vuc rainfall series were filled with data of the nearby Ba To station. For the period 1987 - 2000 a multiple regression analysis was carried out to obtain the relation between the monthly area rainfall and the monthly river discharges at Song Giang. This analysis was performed with the following values:

1. the Song Giang monthly discharge $\left(\mathrm{Q}_{\mathrm{SQm}}\right)$ as the dependant variable
2. the monthly area rainfall in the corresponding month $\left(\mathrm{P}_{\mathrm{m}}\right)$
3. the monthly area rainfall in the three preceding months $\left(\mathrm{P}_{\mathrm{m}-1}, \mathrm{P}_{\mathrm{m}-2}\right.$ and $\left.\mathrm{P}_{\mathrm{m}-3}\right)$

The results of these regression analyses are as follows:

| Regression equation | Standard Error of <br> Estimate $\left(\mathrm{m}^{3} / \mathrm{s}\right)$ | Multiple Correlation <br> Coefficient |
| :--- | :---: | :---: |
| $\mathrm{Q}_{\mathrm{SQ}}=+6+0.70 \mathrm{P}_{\mathrm{m}}$ | 177 | 0.912 |
| $\mathrm{Q}_{\mathrm{SQm}}=-26+0.60 \mathrm{P}_{\mathrm{m}}+0.21 \mathrm{P}_{\mathrm{m}-1}$ | 94 | 0.944 |
| $\mathrm{Q}_{\mathrm{SQ} \mathrm{m}}=-40+0.62 \mathrm{P}_{\mathrm{m}}+0.17 \mathrm{P}_{\mathrm{m}-1}+0.08 \mathrm{P}_{\mathrm{m}-2}$ | 91 | 0.949 |
| $\mathrm{Q}_{\mathrm{SQ} \mathrm{m}}=-49+0.62 \mathrm{P}_{\mathrm{m}}+0.17 \mathrm{P}_{\mathrm{m}-1}+0.06 \mathrm{P}_{\mathrm{m}-2}+0.04 \mathrm{P}_{\mathrm{m}-3}$ | 90 | 0.95 |

These relations indicate that on the average the monthly runoff corresponds with $67 \%$ of the monthly rainfall plus $18 \%$ of rainfall of the preceding month and some $7.5 \%$ of the rainfall of the month before the preceding month. The standard error of estimate is considered substantial and the correlation coefficient acceptable. A better correlation and reduction of the standard error can be achieved by carrying out the regression analysis per season instead of per year.

A similar regression analysis was carried out for the Ve basin upstream of An Chi (814 $\mathrm{km}^{2}$ ). The catchment rainfall was assessed based on the following data:

| ub-basin | Rainfall station |  |  |
| :---: | :---: | :---: | :---: |
|  | Gia Vuc | Quang Ngai | Ba To |
| Ve u/s AnChi | $20 \%$ | $5 \%$ | $75 \%$ |

The multiple regression analysis for the period 1987 - 2000 gave the following results for the An Chi discharges $\left(\mathrm{Q}_{\mathrm{ACm}}\right)$ :

| Regression equation | Standard Error of <br> Estimate $\left(\mathrm{m}^{3} / \mathrm{s}\right)$ | Multiple Correlation <br> Coefficient |
| :--- | :---: | :---: |
| $\mathrm{Q}_{\mathrm{ACm}}=-7+0.252 \mathrm{P}_{\mathrm{m}}$ | 42 | 0.921 |
| $\mathrm{Q}_{\mathrm{ACm}}=-19+0.216 \mathrm{P}_{\mathrm{m}}+0.074 \mathrm{P}_{\mathrm{m}-1}$ | 33 | 0.952 |
| $\mathrm{Q}_{\mathrm{ACm}}=-24+0.222 \mathrm{P}_{\mathrm{m}}+0.057 \mathrm{P}_{\mathrm{m}-1}+0.032 \mathrm{P}_{\mathrm{m}-2}$ | 31 | 0.957 |
| $\mathrm{Q}_{\mathrm{ACm}}=-27+0.225 \mathrm{P}_{\mathrm{m}}+0.057 \mathrm{P}_{\mathrm{m}-1}+0.025 \mathrm{P}_{\mathrm{m}-2}+0.012 \mathrm{P}_{\mathrm{m}-3}$ | 31 | 0.958 |

This regression analysis gives a slightly better result than the analysis of the Song Giang discharges.

From above equations it can be derived that on the average the monthly runoff of the Ve sub-basin corresponds with $71 \%$ of the monthly rainfall plus $18 \%$ of rainfall of the preceding month and some $9 \%$ of the rainfall of the month before the preceding month. The comparison with the discharges observed in the Thu Bon basin has led to the following relations:

$$
\begin{aligned}
& \mathrm{Q}_{\mathrm{SQm}}=0.705 \mathrm{Q}_{\text {NongSon }}-1\left(\text { with } \mathrm{R}^{2}=0.97\right) \text { and } \\
& \mathrm{Q}_{\mathrm{ACm}}=0.252 \mathrm{Q}_{\text {NongSon }}-6.6\left(\text { with } \mathrm{R}^{2}=0.88\right)
\end{aligned}
$$

Especially for the main Tra Khuc sub-basin, the correlation with Thu Bon basin is considered adequate for being used for the extension of the Tra Khuc runoff series. It has, therefore, been decided to use the relations with the Thu Bon discharges for the extension of the series, rather than the runoff rainfall relations derived from the multiple regression analysis. It is anticipated, however, that in further planning phases a proper rainfall -runoff modelling will generate more accurate results.

With the help of above described methodology, monthly runoff series have been generated for the entire Tra Khuc basin as presented in Table B.8.

The runoff has been composed of:

- Tra Khuc runoff by multiplying the Son Giang discharge with the area proportionality factor of 1.24;
- Ve runoff at An Chi
- Runoff from the smaller basins with total area of $510 \mathrm{~km}^{2}$, assuming that the runoff
from this area per $\mathrm{km}^{2}$ corresponds with the combined Thra Khuc - Ve runoff per $\mathrm{km}^{2}$;
- The runoff from the lower (and coastal) area estimated with the help of the net rainfall on this area.

The dependable monthly runoff of the entire Tra Khuc basin has been calculated as follows:

$$
\text { Dependable Monthly Natural Runoff in the whole basin }\left(A=5,200 \mathrm{~km}^{2}\right)
$$

|  | Jan | Feb | Mar | Apr | May | Jun | Jul | Aug | Sep | Oct | Nov | Dec |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| $50 \%$ | 740 | 377 | 273 | 200 | 295 | 293 | 231 | 230 | 515 | 2,083 | 3,169 | 1,801 |
| $75 \%$ | 573 | 305 | 215 | 150 | 203 | 201 | 170 | 161 | 332 | 1,304 | 2,141 | 1,142 |
| $90 \%$ | 454 | 252 | 173 | 116 | 145 | 143 | 129 | 117 | 223 | 856 | 1,504 | 758 |

### 8.3 Flood Runoff

For the estimate of the probable peak discharges in the Tra Khuc river, use has been made of the historical discharge data of the Son Giang station. Recorded instantaneous maximum discharges were available for only fourteen years. This provides a weak basis for the frequency analysis. The results of the analysis are, nevertheless, summarised below for the maximum yearly floods.

Yearly maximum discharges at Son Giang with corresponding Return Period Unit: m³/s

|  | 5 years | 10 years | 20 years | 50 years | 100 years |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Log Normal | 8,257 | 9,967 | 11,701 | 13,865 | 15,578 |
| Pearson 3 | 8,489 | 10,098 | 11,563 | 13,369 | 14,664 |
| Gumbel | 8,348 | 10,001 | 11,588 | 13,641 | 15,180 |
| Goodrich | 8,606 | 9,915 | 11,058 | 12,277 | 13,042 |
| Average round values | 8,500 | 10,000 | 11,500 | 13,500 | 15,000 |

A similar analysis has been carried out with the maximum yearly early flood events (maximum discharge in the period January -August).

Yearly Early maximum discharges at Son Giang with corresponding Return Period Unit: $\mathrm{m}^{3} / \mathrm{s}$

|  | 5 years | 10 years | 20 years | 50 years | 100 years |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Log Normal | 884 | 1273 | 1729 | 2416 | 3028 |
| Log Pearson | 832 | 1241 | 1637 | 2644 | 3512 |
| Gumbel | 1064 | 1425 | 1771 | 2220 | 2555 |
| Average round values | 950 | 1,300 | 1,700 | 2,400 | 3,000 |

The frequency distributions are shown in Figure B.22.
For the estimate of flood volumes corresponding with the above mentioned peak discharges, an analysis has been made of the one-day area rainfall in the upper and
middle Tra Khuc basin. In view of the size and shape of the basin, it is anticipated that the peak discharges are a result of extreme rainfall intensities with duration less than one day. Since no long series of hourly rainfall data have been made available, use is made of daily rainfall data. For this analysis the following series of historic daily rainfall data have been used:

## Rainfall data used for area rainfall analysis in Tra Khuc basin

| Gia Vuc | $1977-2000$ |
| :---: | :---: |
| Tra Bon | $1977-2000$ |
| Quang Ngai | $1977-2000$ |
| Ba To | $1977-2000$ |

Small gaps in the Gia Vuc rainfall series were filled with data of the nearby Ba To station.
With the help of the Thiessen method the following area weights have been assessed for the area rainfall on the $2,440 \mathrm{~km}^{2}$ of the Tra Khuc basin upstream of Son Giang. upper and middle catchment of the Ba basin.

| Sub-catchment | Rainfall station |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
|  | Gia Vuc | Tra Bon | Quang Ngai | Ba To |
| Tra Khuc u/s SonGiang | $55 \%$ | $35 \%$ | $6 \%$ | $4 \%$ |

Using these weights, area rainfall series have been compiled of 24 years of daily rainfall. On these series frequency analyses were carried out using both the Gumbel and the Log-Normal probability distribution functions. The analysis were carried out for the yearly maximum area rainfall values and for the "early flood" rainfall values (maximum rainfall in period January - August).

The results (averaged and round Gumbel and Log Normal values) are given in the table below:

One day maximum area rainfall main flood season in mm

| Return Period (years) | 2 | 5 | 10 | 20 | 50 | 100 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Upper and middle Tra Khuc basin | 230 | 315 | 370 | 425 | 495 | 550 |

One day maximum "early flood" rainfall in mm

| Return Period (years) | 2 | 5 | 10 | 20 | 50 | 100 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Upper and middle Tra Khuc basin | 65 | 100 | 120 | 145 | 175 | 200 |

The corresponding rainfall volumes on the upper and middle $3,030 \mathrm{~km} 2$ of the Tra Khuc basin are as follows:

Estimated one-day rainfall volumes in $\mathbf{M m}^{\mathbf{3}}$ on upper and middle Tra Khuc basin

| Return period | 2 years | 5 years | 10 years | 20 years | 50 years | 100 years |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Main flood season | 700 | 1,000 | 1,100 | 1,300 | 1,500 | 1,700 |
| Early flood season | 200 | 300 | 350 | 450 | 550 | 600 |

The hydrographs and corresponding area rainfall of the 1999 floods are presented in Figure B.23. The direct flood runoff as a consequence of these main storms is of the order of $70 \%$ of the storm rainfall volume. It is anticipated that during the early floods this runoff percentage will be lower and could be of the order of $50 \%$ or even less.

## 9 KONE RIVER BASIN

### 9.1 Basin Characteristics

### 9.1.1 River Basin

The Kone river basin is situated in the south central Vietnam between $13^{\circ} 30^{\prime}$ to $14^{\circ} 30^{\prime}$ north latitude and $108^{\circ} 30$ ' to $109^{\circ} 15^{\prime}$ east longitude. Almost of catchment is situated Binh Dinh province.

Kone river basin covers a total catchment area of only $3,600 \mathrm{~km}^{2}$, which included Ha Thanh river basin.

Major tributaries and its catchement area are shown as follows:

| Kone Basin (Total) | $3,640 \mathrm{~km}^{2}$ |
| :---: | :---: |
| Major Sub-basins |  |
| Kone (at Binh Thanh) : | 2,250km ${ }^{2}$ |
| Lower Area (Delta) : | $640 \mathrm{~km}^{2}$ |
| Ha Thanh : | $630 \mathrm{~km}^{2}$ |

For the location map of the Kone Basin is presented in Figure B.24.

### 9.1.2 Climatological Features

Mean annual temperature varies from $20^{\circ} \mathrm{C}$ to $27^{\circ} \mathrm{C}$. Mean monthly humidity varies from 70 to $80 \%$ and annual evaporation varies from 800 mm in mountains to $1,200 \mathrm{~mm}$ in coastal plains.

Mean annual rainfall varies from $1,800 \mathrm{~mm}$ to $2,100 \mathrm{~mm}$. There are 2 rains peaks, i.e., small rainfall and heavy rainfall. The heavy rainfall concentrates in October to November. Small rains occur in May and June. The dry season starts in January and lasts until August.

### 9.2 Natural Runoff

Only one discharge measurement station with adequate record length has been identified in the Kone River basin. This station, Cay Muong, has been used as base station for the assessment of the basin runoff.

| Station | Longitude | Latitude | Catchment | Tributary | Observation Period |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Cay Muong | 108.52 | 13.56 | $1,677 \mathrm{~km} 2$ | Kone | 1976 -1977 1979-2000 |

Based on record length and geographical position, the following rainfall stations have been selected for the assessment of the area rainfall in the Kone River basin:

| Station | Longitude | Latitude | Province | Observation Period |
| :---: | :---: | :---: | :---: | :---: |
| Binh Tuong (Cay Muong) | 108.56 | 13.55 | Binh Dinh | $1976-2000$ |
| Phu Cat | 109.04 | 14.00 | Binh Dinh | $1976-2000$ |
| Dinh Binh (Binh Quang) | 108.48 | 14.08 | Binh Dinh | $1979-2000$ |
| Vinh Kim | 108.46 | 14.14 | Binh Dinh | $1982-2000$ |

Daily discharge data of the Cay Muong station have been collected and processed for the period 1976 - 2000. These discharges do not correspond fully with the natural runoff, since the reservoir at Vinh Son has a regulating effect on the runoff. This effect relates to $97 \mathrm{~km}^{2}$, or $5.8 \%$ of the catchment that is measured at Cay Muong.

The base point of the Kone River basin is not a single geographic position, since the river bifurcates into several branches before it discharges into the Quy Nhon lagoon (Thi Nai Swamp). If, however, this lagoon is considered part of the basin, then the connection between this lagoon and the sea is the base point. Consequently, the Ha Than basin is to be considered as a sub-basin of the Kone River basin.

In the Kone basin a clear distinction can be made between the upper and middle catchment on one side and the lower catchment on the other side. This lower catchment consists essentially of the deltaic zone that has its apex at Binh Thanh just upstream of the Bay Yen weir in the Kone river. This apex is some 30 km upstream of the Kone river mouth.

The natural monthly discharge series derived for the Cay Muong station represents the runoff of the upper $1,677 \mathrm{~km}^{2}$ of the Kone River basin. This upper area corresponds with $75 \%$ of the catchment that drains through Binh Thanh and to $56 \%$ of the total catchment excluding the low-lying delta area. The area of this lower area has been measured at 640 $\mathrm{km}^{2}$.

To arrive at natural monthly discharges at Binh Thanh and from the entire catchment, the following approach has been used.

With the help of the monthly rainfall data of the selected stations the monthly catchment rainfall has been established for the entire basin. Also for the upper catchment, that is measured at Cay Muong, the monthly area rainfall has been established, using the relevant stations. A comparison between the monthly basin rainfall data and the monthly upper basin rainfall data reveals that the basin rainfall tends to be slightly higher than the upper catchment rainfall. The following relation was found:
$\mathrm{P}_{\mathrm{uc}}=0.98 \mathrm{P}_{\text {basin }}+6\left(\mathrm{R}^{2}=0.96\right)$
With:
$P_{u c} \quad=$ monthly rainfall upper catchment in mm
$P_{\text {basin }}=$ monthly rainfall basin in mm

From this relation it is concluded that no substantial error is introduced with the assumption that the basin rainfall and the upper catchment rainfall are the same. If, moreover, it is assumed that the rainfall runoff relation of the Nui Mot and Ha Thanh sub-catchments is similar to the rainfall runoff relation of the upper area, then a direct area proportionality factor can be applied to obtain the monthly runoff at Binh Thanh and the monthly runoff from the entire basin excluding the delta area. This approach has been used for this stage of the planning process. The above assumptions will be verified with the help of rainfall runoff modelling in case a comprehensive basin plan is to be elaborated for the Kone basin.

The area factors that have been used to convert the Cay Muong measured discharges to the Binh Thanh and the total middle and basin runoff are respectively 1.34 and 1.79.

The resulting generated series of monthly runoff for the period 1976-2000 (with exception of 1978) is presented in the Table B.9. This runoff includes that runoff from the lower area, that have been estimated with the help of the net rainfall on this area. The length of the series is considered sufficient for the purpose of the study, reason why no effort has been made to fill the 1978 gap in the series. The dependable basin runoff has been calculated as follows:

## Dependable Monthly Natural Runoff in the whole basin ( $\mathrm{A}=\mathbf{3 , 6 4 0} \mathrm{km}^{\mathbf{2}}$ ) <br> (including HaThanh Basin)

unit :million $\mathrm{m}^{3}$

|  | Jan | Feb | Mar | Apr | May | Jun | Jul | Aug | Sep | Oct | Nov | Dec |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| $50 \%$ | 218 | 135 | 108 | 87 | 109 | 122 | 98 | 100 | 162 | 819 | 1,107 | 514 |
| $75 \%$ | 165 | 106 | 85 | 69 | 83 | 86 | 76 | 72 | 104 | 463 | 659 | 288 |
| $90 \%$ | 128 | 84 | 69 | 56 | 65 | 62 | 61 | 53 | 70 | 278 | 413 | 171 |

## 9.3

Flood Runoff

For the estimate of the probable peak discharges in the Kone river, use has been made of the historical discharge data of the Cay Muong station.

Frequency distribution analyses were carried out with the above series of maximum discharges. Figures B. 25 show the distribution for different probability functions. It is noted that the availability of only twenty five observed maximum year discharges
provides a rather weak basis for the frequency analysis of peak discharges.
From the goodness-of- fit tests it appears that the Log-normal, Pearson 3, Gumbel and Goodrich functions, describes best the frequency distribution. The results are summarised below for the yearly maximum peak discharges at Cay Muong:

| ${\text { Unit: } \mathrm{m}^{3} / \mathrm{s}}_{\mid \text {Return period (years) }}^{2}$ |  |  |  |  |  |  |  | 2 | 5 | 10 | 20 | 50 | 100 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Log Normal | 2,137 | 3,443 | 4,401 | 5,135 | 6,205 | 7,005 |  |  |  |  |  |  |  |
| Pearson 3 | 2,201 | 3,653 | 4,469 | 5,141 | 5,977 | 6,577 |  |  |  |  |  |  |  |
| Gumbel | 2,256 | 3,621 | 4,424 | 5,145 | 6,078 | 6,778 |  |  |  |  |  |  |  |
| Goodrich | 2,125 | 3,779 | 4,506 | 5,072 | 5,760 | 6,211 |  |  |  |  |  |  |  |

Peak discharges at Binh Thanh, the location where the Kone river enters the flood prone lower Kone basin have been estimated as follows.

A Creager type area relation is assumed for the peak discharges. According to this relation, the peak discharge is proportional to the A to the power $\mathrm{A}^{-0.05}$, with A is the catchment area. Following this relation, the peak discharge at Binh Thanh (catchment area $=2,350$ $\mathrm{km}^{2}$ ) is 1.157 times the peak discharge at Cay Muong (catchment area $=1,667 \mathrm{~km}^{2}$ ). For the Cay Muong peak discharges the average value of the above estimated probable discharges has been taken. Using this approach, the following probable round peak discharge values have been calculated for Binh Thanh.

| Peak discharges in $\mathbf{~ m}^{\mathbf{3} / \mathbf{s}}$ at Binh Thanh with corresponding Return Period (years) |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 2 | 5 | 10 | 20 | 50 | 100 |
| 2,500 | 4,200 | 5,100 | 5,900 | 6,900 | 7,700 |

A similar analysis has been carried out with the maximum yearly early flood events. The log-normal function gave by far the best fit, and the following results:

| Early flood peak discharges in $\mathbf{m}^{\mathbf{3}} / \mathbf{s}$ at Binh Thanh with corresponding Return <br> Period |  |  |  |
| :---: | :---: | :---: | :---: |
| 2 | 5 | 10 | 20 |
| 160 | 310 | 430 | 560 |

For the estimate of volumes of floods that are generated by 1-day peak rainfall on the Kone basin, an analysis has been made of the daily rainfall data from the rainfall stations upstream of the apex of the Kone delta at Binh Thanh. These stations are:

| Binh Tuong (Cay Muong) | $1976-2000$ |
| :--- | :--- |
| Binh Quang (Dinh Binh) | $1979-2000$ |
| Vinh Kim | $1982-2000$ |

The Binh Quang and Vinh Kim series show some small gaps that have been filled with Binh Tuong data. Catchment rainfall was calculated for the period 1982 - 2000, assuming
equal weight for each station. With the help of these area rainfall series a frequency analysis was carried out of yearly maximum day rainfall. Comparison of the results of this analysis with the maximum rainfall of Binh Tuong (Cay Muong) station over the same period reveals that the Binh Tuong maximum values are essentially the same as the basin values. Since this station has the longest record, it has been decided to use the 1976 - 2000 Binh Tuong data for the estimate of the peak n-day rainfall in the entire basin. The result is as shown below:

Maximum yearly one-day rainfall at Binh Tuong in mm

| Return period in years | 2 | 5 | 10 | 20 | 50 | 100 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Log-normal | 169 | 224 | 259 | 291 | 336 | 368 |
| Gumbel | 169 | 223 | 259 | 293 | 338 | 371 |

Maximum yearly two-day rainfall at Binh Tuong in mm

| Return period in years | 2 | 5 | 10 | 20 | 50 | 100 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Log-normal | 238 | 318 | 370 | 415 | 483 | 530 |
| Gumbel | 238 | 317 | 370 | 420 | 485 | 534 |

Maximum yearly three-day rainfall at Binh Tuong in mm

| Return period in years | 2 | 5 | 10 | 20 | 50 | 100 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Log-normal | 270 | 355 | 409 | 458 | 526 | 574 |
| Gumbel | 269 | 353 | 408 | 458 | 530 | 582 |

These rainfall intensities correspond with the following n-day precipitation volumes in the basin upstream of Binh Thanh ( $2250 \mathrm{~km}^{2}$ ):

Maximum yearly n-day rainfall volume on Kone basin u/s Binh Thanh in $\mathbf{M m}^{\mathbf{3}}$

| Return period | 2 years | 5 years | 10 years | 20 years | 50 years | 100 years |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 day | 380 | 503 | 583 | 657 | 758 | 831 |
| 2 days | 535 | 714 | 833 | 939 | 1,089 | 1,197 |
| 3 days | 606 | 797 | 919 | 1,031 | 1,188 | 1,301 |

It is anticipated that the peak discharges that occur at Binh Thanh correspond with one day rainfall intensities, rather than with more day intensities. A typical historic hydrograph and corresponding rainfall is shown in Figure B.26.

Early floods are flood events that occur prior to the start of the rainy season. Here the early floods are defined as the flood events taking place in the period from 1 January to 1 September. One-day rainfall intensities have been assessed for this season in a similar way as for the maximum year values. The results are as follows:

Maximum early (January - September) one-day rainfall at Binh Tuong in mm

| Return period in years | 2 | 5 | 10 | 20 | 50 | 100 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Log-normal | 73 | 105 | 128 | 150 | 178 | 200 |
| Gumbel | 75 | 106 | 128 | 149 | 175 | 195 |

The corresponding early storm volume in $\mathrm{Mm}^{3}$ on the Kone basin upstream of Binh Thanh is estimated as follows:

| Return period in years |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 2 | 5 | 10 | 20 | 50 | 100 |  |
| 170 | 240 | 300 | 350 | 400 | 450 |  |

## 10 BA RIVER BASIN

### 10.1 Basin Characteristics

### 10.1.1 River Basin

The Ba river basin is situated in the south of central Vietnam between $12^{\circ} 30^{\prime}$ to $14^{\circ} 30^{\prime}$ north latitude and $108^{\circ}$ to $109^{\circ} 20^{\prime}$ east longitude. The Ba river basin covers 3 provinces: Gia lai and Dak Lak (Central Highlands) and Phu Yen in south central coast.

Ba river basin covers a total catchment area of $14,000 \mathrm{~km}^{2}$, completely located in Vietnam.
Major tributaries in Ba river basin are:

| Ba River basin (Total) | $14,030 \mathrm{~km}^{2}$ |
| :--- | ---: |
| Major Sub-basins |  |
| Ia Ayun : | $2,830 \mathrm{~km}^{2}$ |
| Krong H Nang : | $1,740 \mathrm{~km}^{2}$ |
| Hinh : | $970 \mathrm{~km}^{2}$ |
| Lower Area (Delta) : | $440 \mathrm{~km}^{2}$ |

The location map of the basin is presented in Figure B.27.
10.1.2 Climatological Features

Mean annual temperature ranges from 21 to $24^{\circ} \mathrm{C}$, from 25 to $26^{\circ} \mathrm{C}$ and from 26 to $27^{\circ} \mathrm{C}$ in the upper, middle and lower, respectively. Mean Annual humidity is about $80 \%$ and evaporation varies from $1,300 \mathrm{~mm}$ to $1,500 \mathrm{~mm}$.

Mean annual rainfall in the whole basin reaches 1700 mm . In the western upper basin (central highlands), the rainy season lasts from May to October, while the dry season lasts from November to April. In the middle and lower basin the rainy season lasts from September to December and dry season lasts from January to August.

### 10.2 Natural Runoff

For the assessment of the natural runoff at basin level, the measured discharges at Cung Son furnish an excellent starting point. At this location passes the runoff of $88 \%$ of the entire Ba catchment and more than $90 \%$ of the upper and middle catchment. The following historic information on daily discharge observations has been collected and processed:

| Station | Longitude | Latitude | Catchment | Tributary | Observation Period |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Cung Son | 108.59 | 13.02 | $12,800 \mathrm{~km}^{2}$ | Ba | $1977-2000$ |
| An Khe | 108.39 | 13.57 | $1,440 \mathrm{~km}^{2}$ | Ba | $1978-2000$ |

For the assessment of the area rainfall in the Ba basin the following data have been included in the database:

| Station | Longitude | Latitude | Province | Observation Period |
| :---: | :---: | :---: | :---: | :---: |
| M' Drak | 108.46 | 12.45 | Dak Lak | $1977-2000$ |
| Son Hoa (Cung Son) | 108.59 | 13.03 | Phu Yen | $1976-2000$ |
| Tuy Hoa | 109.17 | 13.05 | Phu Yen | $1977-2000$ |
| Cheo Reo (A Yun Pa) | 108.27 | 13.24 | Gia Lai | $1977-2000$ |
| Chu Prong | 107.52 | 13.44 | Gia Lai | $1978-1995.1997-2000$ |
| An Khe | 108.39 | 13.57 | Gia Lai | $1977-2000$ |

The discharge record of Cung Son ranges over the period from 1977 onwards. For the conversion of measured discharges at Cung Son into natural runoff, the following upstream developments need to be taken into account:

- The Ia Yun Ha reservoir came into operation in 1990, and regulates the runoff of 1600 $\mathrm{km}^{2}$, or almost $13 \%$ of the catchment $\mathrm{u} / \mathrm{s}$ Cung Son. Moreover, the water from this reservoir that is used to irrigate 4,500 ha is to be considered as been withdrawn partly from the basins runoff. Corrections of the natural runoff could be estimated on the basis of reservoir operation data and irrigation demands.
- The Song Hinh hydro-electric plant came recently into operation in 2001. The Song Hinh reservoir regulates the runoff of $722 \mathrm{~km}^{2}$, or about $6 \%$ of the catchment $\mathrm{u} / \mathrm{s}$ Cung Son.

The Dong Cam irrigation scheme withdraws water from the river downstream of the Cung Son measurement station and has, therefore, no impact on the measured discharges.

Since the details on the Ia Yun Ha reservoir operation were not available in time for the present analysis, it has been decided to select the runoff series prior to the completion of the reservoir for being used in the water balance analysis. This series of monthly runoff, covering the period 1977 - 1989, is presented in Table B.10. These series have been derived directly from the Cung Son discharge data, using an area multiplication factor of 1.096 to arrive at the natural runoff of the entire middle and upper basin, and adding the runoff of the lower 440 km 2 basin area by applying the net rainfall approach. The dependable basin runoff has been calculated as follows:

Dependable Monthly Natural Runoff in the whole basin $\left(A=14,030 \mathrm{~km}^{2}\right)$

|  |  |  |  |  |  |  |  |  |  |  |  |  |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
|  | Jan | Feb | Mar | Apr | May | Jun | Jul | Aug | Sep | Oct | Nov | Dec |
| $50 \%$ | 397 | 212 | 145 | 111 | 210 | 329 | 355 | 609 | 948 | 1,640 | 2,246 | 860 |
| $75 \%$ | 302 | 168 | 113 | 79 | 140 | 200 | 244 | 388 | 709 | 956 | 1,454 | 532 |
| $90 \%$ | 236 | 136 | 91 | 58 | 97 | 127 | 173 | 259 | 546 | 588 | 984 | 345 |

### 10.3 Flood Runoff

For the estimate of the probable peak discharges in the Ba river, use has been made of the
historical discharge data of the Cung Son station. For this analysis only 13 years of records of monthly instantaneous peak discharges were available in this study. This provides an extremely weak basis for the frequency analysis. The results of the analysis are, nevertheless, summarised below. The frequency distributions are shown Figure B.28.

| Unit: $^{3} / \mathrm{s}$ |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Log Normal | 2 y | 5 y | 10 y | 20 y | 50 y | 100 y |  |
| Pearson 3 | 5,590 | 9,665 | 13,029 | 16,793 | 22,006 | 26,477 |  |
| Gumbel | 6,010 | 10,480 | 13,876 | 17,143 | 21,350 | 24,468 |  |
| Goodrich | 5,945 | 10,585 | 13,731 | 16,681 | 20,499 | 23,360 |  |
| Averaged round values | 5,800 | 10,400 | 13,525 | 16,500 | 17,000 | 19,441 |  |

${ }^{1)}$ Reportedly a $24000 \mathrm{~m}^{3} / \mathrm{s}$ flood was observed in 1938 , while in 1964 a flood of $21,850 \mathrm{~m}^{3} / \mathrm{s}$ was reported.

A similar analysis has been carried out with the maximum yearly early flood events.

|  | Unit: $\mathrm{m}^{3} / \mathrm{s}$ |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Log Normal | 7 y | 5 y | 10 y | 20 y | 50 y | 100 y |
| Gumbel | 753 | 1,136 | 1,443 | 1,752 | 2,195 | 2,545 |
| Averaged round values | 750 | 1,185 | 1,470 | 1,744 | 2,099 | 2,365 |

For the estimate of flood volumes corresponding with the above mentioned peak discharges, an analysis has been made of the n-day area rainfall in the upper and middle Ba basin. For this analysis the following series of historic daily rainfall data have been used:

## Rainfall data used for n-day area rainfall analysis in Ba basin

An Khe
Chu Prong
Cheo Reo
Ma Drak Son Hoa

1978-2000 (complete)
1978-1995, 1997-2000 (4 months gap)
1978-2000 (2monthas gap)
1978 - 2000 ( 1 moth gap)
1978-2000 (complete)

The gaps in the Chu Prong and Cheo Reo series have been filled with An Khe rainfall data. The Ma Drak series has been completed with Son Hoa data.

With the help of the Thiessen method the following area weights have been assessed for the area rainfall on the $12,990 \mathrm{~km}^{2}$ upper and middle catchment of the Ba basin.

| Sub-catchment | Rainfall station |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | An Khe | Chu Prong | Cheo Reo | Ma Dark | Son Hoa |
| Upper and middle Ba basin | $30 \%$ | $9 \%$ | $32 \%$ | $16 \%$ | $13 \%$ |

Using these weights, area rainfall series have been compiled of 23 years of daily, 2-days and 3-days rainfall. On these series frequency analyses were carried out using both the Gumbel and the Log-Normal probability distribution functions. The analysis were carried out for the yearly maximum area rainfall values and for the "early flood" rainfall values (maximum rainfall in period January - August)
.The results (averaged and round Gumbel and Log Normal values) are given in the tables below:

One day maximum area rainfall main flood season in mm

| Return Period (years) | 2 | 5 | 10 | 20 | 50 | 100 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Upper and middle Ba basin | 110 | 155 | 180 | 210 | 250 | 275 |

One day maximum "early flood" rainfall in mm

| Return Period (years) | 2 | 5 | 10 | 20 | 50 | 100 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Upper and middle Ba basin | 45 | 60 | 75 | 85 | 100 | 110 |

Two day maximum area rainfall main flood season in $\mathbf{m m}$

| Return Period (years) | 2 | 5 | 10 | 20 | 50 | 100 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Upper and middle Ba basin | 150 | 220 | 265 | 310 | 370 | 420 |

Two day maximum "early flood" rainfall in mm

| Return Period (years) | 2 | 5 | 10 | 20 | 50 | 100 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Upper and middle Ba basin | 60 | 80 | 95 | 105 | 125 | 135 |

Three day maximum area rainfall main flood season in mm

| Return Period (years) | 2 | 5 | 10 | 20 | 50 | 100 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Upper and middle Ba basin | 165 | 245 | 290 | 340 | 405 | 455 |

Three day maximum "early flood" rainfall in mm

| Return Period (years) | 2 | 5 | 10 | 20 | 50 | 100 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Upper and middle Ba basin | 70 | 100 | 115 | 130 | 150 | 165 |

The corresponding rainfall volumes on the upper and middle basin are as follows

Estimated yearly n-day rainfall volumes in Mm3 on upper and middle Ba basin

| Storm | 2 years | 5 years | 10 years | 20 years | 50 years | 100 years |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| One-day storm | 1,400 | 2,000 | 2,300 | 2,700 | 3,200 | 3,600 |
| Two-day storm | 1,900 | 2,800 | 3,400 | 4,000 | 4,800 | 5,500 |
| Three -day storm | 2,100 | 3,200 | 3,800 | 4,400 | 5,300 | 5,900 |

Estimated early n-day rainfall volumes in Mm3 on upper and middle Ba basin

| Return period | 2 years | 5 years | 10 years | 20 years | 50 years | 100 years |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| One-day storm | 600 | 800 | 1,000 | 1,100 | 1,300 | 1,400 |
| Two-day storm | 800 | 1,000 | 1,200 | 1,400 | 1,600 | 1,800 |
| Three -day storm | 900 | 1,300 | 1,500 | 1,700 | 1,900 | 2,100 |

It is anticipated that the peak discharges at Cung Son are produced by two-day storms. A typical storm and corresponding flood hydrograph is presented in Figure B.29.

## 11 SESAN RIVER BASIN

### 11.1 Basin Characteristics

### 11.1.1 Location

The Sesan River, which originates in the southern highlands of Vietnam and flows into Cambodia, is one of the main tributaries of the Mekong River. The total area of Sesan river basin is $18,570 \mathrm{~km} 2$. Nearly $40 \%$ of the Sesan basin is located in Cambodia. The River basin is situated in the south of central Vietnam between $13^{\circ} 40^{\prime}$ to $15^{\circ} 20^{\prime}$ north latitude and $107^{\circ} 20^{\prime}$ to $108^{\circ} 40^{\prime}$ east longitude. North part of basin lies in the Kon Tum province, and south part of it lies in the Gia Lai Province. Major tributaries in Sesan basin are:

| Sesan Basin (Total) | $11,530 \mathrm{~km}^{2}$ (within Vietnam) |
| :---: | :---: |
| Major Sub-basins |  |
| Krong Po Ko : | $3,450 \mathrm{~km}^{2}$ |
| Dak Bla : | $3,410 \mathrm{~km}^{2}$ |

Location map of the Vietnamese part of the Sesan basin is presented in the Figure B. 30.

### 11.1.2. Climatological Features

Mean annual temperature varies from $18^{\circ} \mathrm{C}$ to $24^{\circ} \mathrm{C}$. Mean monthly humidity may increase from some 70\% for March to more than $90 \%$ for August.

Mean annual rainfall varies within the range 1300 mm to more than 2800 mm . The rainy season lasts from May to October. The dry season lasts from December to April.

### 11.2 Natural Runoff

The Se San basin has been studied extensively in view of the development of the hydropower potential in this basin. Recent studies of, especially, the Sesan 3 project (Feasibility Study by SWECO, 1999) and Sesan 4 (SWECO, 1997 and Halcrow, 1998) present long series of monthly runoff at both sites, covering respectively $68 \%$ and $81.5 \%$ of the Vietnamese part of the Se San basin. The discharge series have reportedly been prepared by PECC1. The hydrological analyses made in the framework of the power studies are mainly based on daily discharge observations at the following stations:

| Station | Longitude | Latitude | Catchment | Tributary | Observation period |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Kontum | 108.01 | 14.20 | 3,056 | Dak Bla | $1967-$ date (with gaps) |
| Trung Nghia | 107.52 | 14.25 | 3,320 | KrongPoKo | $1990-$ date |
| Sa Binh | 107.51 | 14.19 | 6,732 | Sesan | $1982-1990$ |

Details on the methodology that is used in the preparation of the monthly natural runoff series have not been obtained. For the assessment of the basin rainfall, the following

Study on Nationwide Water Resources Development and Management in the Socialist Republic of Vietnam
rainfall information has been collected and processed:

| Station | Longitude | Latitude | Province | Observation Period |
| :---: | :---: | :---: | :---: | :---: |
| Dac Glei | 107.45 | 15.04 | Kon Tum | 1977-78.1980-1984. 1986-1995.1997-2000 |
| Dak To | 107.50 | 14.40 | Kon Tum | $1977-2000$ |
| Kon Tum | 108.00 | 14.21 | Kon Tum | $1976-2000$ |
| Pleiku | 108.00 | 13.59 | Gia Lai | $1976-2000$ |

From the monthly runoff series presented in the above mentioned study, the 1976-1997 series at the Se San 4 site have been selected for the estimate of the basin runoff at the base point of the basin, i.e. the location where the Sesan river leaves the Vietnamese territory.

The generated monthly runoff series at the base point assumes that the specific discharge of the entire Vietnamese part of the basin corresponds with the runoff per $\mathrm{km}^{2}$ obtained from the Se San 4 series. The Se San 4 series have, consequently been multiplied by a factor 1.236 . The dependable runoff at the base point has been calculated as follows:

$$
\text { Dependable Monthly Natural Runoff ( } \mathrm{A}=11,530 \mathrm{~km}^{2} \text { ) }
$$

|  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Jan | Feb | Mar | Apr | May | Jun | Jul | Aug | Sep | Oct | Nov | Dec |
| $50 \%$ | 572 | 406 | 375 | 356 | 514 | 799 | 1,188 | 2,017 | 2,054 | 1,839 | 1,333 | 872 |
| $75 \%$ | 489 | 343 | 320 | 295 | 404 | 589 | 939 | 1,632 | 1,621 | 1,504 | 1,033 | 710 |
| $90 \%$ | 425 | 294 | 277 | 250 | 325 | 447 | 760 | 1,348 | 1,310 | 1,254 | 821 | 590 |

### 11.3 Flood Runoff

To estimate the flood runoff in the Sesan basin, an analysis has been made of the observed peak flows in the basin and of the rainfall intensities. Data on monthly instantaneous peak flows are available for the Kon Tum station in the Dak Bla sub-catchment for the period 1978 - 2000. From this series the following probable discharges have been calculated assuming a Log-normal, Pearson-3 and Gumbel probability distribution respectively:

Probable yearly peak discharges at Kon Tum in the Dak Bla sub-basin in $\mathbf{m}^{\mathbf{3} / \mathbf{s}}$

| Return Period years | 2 | 5 | 10 | 20 | 50 | 100 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Log-nor. | 1,465 | 2,071 | 2,482 | 2,877 | 3,411 | 3,815 |
| Pears. 3 | 1,497 | 2,124 | 2,510 | 2,861 | 3,292 | 3,600 |
| Gumbel | 1,482 | 2,087 | 2,487 | 2,872 | 3,369 | 3,742 |
| Round average | 1,500 | 2,100 | 2,500 | 2,900 | 3,400 | 3,750 |

The catchment area upstream of the Kon Tum station amounts to $3,056 \mathrm{~km}^{2}$. The specific peak discharge at that location, is consequently calculated at some $490 \mathrm{l} / \mathrm{s} / \mathrm{km}^{2}$ at the
once in two year flood, increasing to some $1,200 \mathrm{l} / \mathrm{s} / \mathrm{km}^{2}$ during the once in 100 years flood.

A similar analysis has been carried out for the floods that occur in the period January -August. Using the same probability distribution, the following results have been obtained:

Probable peak discharges in the period January - August at Kon Tum in the Dak Bla sub-basin in m3/s

| Return Period years | 2 | 3 | 5 | 10 | 20 |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Log-nor. | 485 | 615 | 774 | 988 | 1,214 |
| Pears.3 | 499 | 649 | 815 | 1,022 | 1,216 |
| Gumbel | 510 | 652 | 811 | 1,010 | 1,200 |
| Round average | 500 | 650 | 800 | 1,000 | 1,210 |

The concentration time of the basin is estimated of the order of 1 day. It is, therefore, anticipated that intensities corresponding with one day rainfall are indicative for the peak floods. For the Dak Bla sub-basin, the maximum one-day rainfall has been analysed. The following daily rainfall data have been used:

| Kon Tum | $1976-2000$ (complete) |
| :--- | :--- |
| Pleiku | $1976-2000$ (with one month gap) |

The minor gap in the Pleiku series has been filled with Kon Tum data. The area rainfall on the Dak Bla sub-catchment has been calculated on the basis of the following weights of the respective stations:

| Sub-basin | Rainfall station |  |
| :--- | :--- | :--- |
|  | Kon Tum | Pleiku |
| Dak Bla | $75 \%$ | $25 \%$ |

Twenty five years daily area rainfall series have been compiled and the following one-day probable rainfall intensities have been calculated assuming a log-normal distribution of the yearly maximum values.

Maximum one day area rainfall on the Dak Bla sub-basin (mm)

| Return period in years | 2 | 3 | 5 | 10 | 20 |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Full year | 87 | 100 | 113 | 130 | 145 |
| January - August | 74 | 87 | 101 | 119 | 137 |

Figure B. 31 shows a typical storm and corresponding flood runoff at Kon Tum. From the hydrograph it can be derived that the direct flood runoff volume corresponds with some $30 \%$ of the rainfall volume, assuming that the calculated area rainfall occurs over the full sub-basin area.

It is anticipated that the flood runoff of the Krong Po Ko sub-basin (3,450 km2) has similar characteristics as the flood runoff from the Dak Bla sub-basin (3,410 km2).

## 12 SREPOK RIVER

### 12.1 Basin Characteristics

### 12.1.1 River Basin

Srepok river, which originates in the southern highland of Vietnam and flows into Cambodia, is one of the main tributaries of Mekong river. Vietnamese part of the basin is situated between $12^{\circ}$ to $13^{\circ}$ north latitude and $107^{\circ} 30^{\prime}$ to $108^{\circ} 45^{\prime}$ east longitude. North part of basin lies in the Dac Lac province, and south part of it lies in the Lam Dong Province.

The catchment area at the boundary with Cambodia is $12,000 \mathrm{~km}^{2}$. Major tributaries in the basin are:

| Srepok Basin (total) | $12,030 \mathrm{~km}^{2}$ (within Vietnam) |
| :---: | :---: |
| Major Sub-basins |  |
| Ea Krong Ana : | $3,860 \mathrm{~km}^{2}$ |
| Ea Krong Kno : | $3,910 \mathrm{~km}^{2}$ |

The location map of the Vietnamese part of Srepok basin is presented in Figure B.32.

### 12.1.2 Climatological Features

Mean annual temperature varies from $22^{\circ} \mathrm{C}$ to $24^{\circ} \mathrm{C}$ and evaporation varies from $1,000 \mathrm{~mm}$ to $1,600 \mathrm{~mm}$.

Mean annual rainfall varies from 1,400 to $2,000 \mathrm{~mm}$. The rainy season lasts from May to October. The dry season starts from December and lasts until May or April.

### 12.2 Natural Runoff

Most of the runoff of the $12,030 \mathrm{~km}^{2}$ Vietnamese catchment area of the Srepok basin is observed at the Ban Don station, that covers $10,700 \mathrm{~km}^{2}$ of the basin. All over the basin a number of water management infrastructures are in place like reservoirs, weirs and intake works for irrigation schemes. The impact of this infrastructure on the runoff at Ban Don is difficult to assess. In the present study, it is assumed that this impact is minor and that the observed runoff at Ban Don corresponds broadly with the natural basin runoff.

For the hydrological analysis of the Srepok basin, the following discharge data have been collected and processed:

| Station | Longitude | Latitude | Catchment | Tributary | Observation Period |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Giang Son | 108.11 | 12.30 | $3,180 \mathrm{~km}^{2}$ | Krong Ana | $1977-2000$ |
| Ban Don | 107.47 | 12.53 | $10,700 \mathrm{~km}^{2}$ | Srepok | $1977-2000$ |
| Duc Xuyen | 108.59 | 12.18 | $3,080 \mathrm{~km}^{2}$ | Krong Kno | $1978-2000$ |

For the assessment of the area rainfall in the Srepok basin the following data have been included in the database:

| Station | Longitude | Latitude | Province | Observation Period |
| :---: | :---: | :---: | :---: | :---: |
| Buon Ho | 108.16 | 12.55 | Dak Lak | $1977-2000$ |
| Cau42 (Krong Buk) | 108.22 | 12.46 | Dak Lak | $1976-2000$ |
| Krong Bong | 108.27 | 12.33 | Dak Lak | $1977-1990.1992-1995$ |
| Giang Son | 108.11 | 12.30 | Dak Lak | $1976-2000$ |
| Buon Ma Thuot | 108.03 | 12.40 | Dak Lak | $1977-2000$ |
| Cau14 | 107.56 | 12.37 | Dak Lak | $1977-2000$ |
| Duc Xuyen | 108.59 | 12.18 | Dak Lak | $1978-2000$ |
| Dak Mil | 107.37 | 12.27 | Dak Lak | $1977-1993.1998-2000$ |

The series of observed daily discharges at the Ban Don station has readily been used for the generation of the natural runoff series of the entire Vietnamese part of the Srepok basin for the period 1977 - 2000. It has been assumed that the specific runoff of the catchment area between Ban Don and the Cambodian border corresponds with the specific discharge of the area upstream of this station.

The generated series of monthly natural runoff at the Cambodian border is presented in Table B.12. The dependable runoff at the base point has been calculated as follows:

$$
\text { Dependable Monthly Natural Runoff ( } \mathrm{A}=12,030 \mathrm{~km}^{2} \text { ) }
$$

| unit :million $\mathrm{m}^{3}$ |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Jan | Feb | Mar | Apr | May | Jun | Jul | Aug | Sep | Oct | Nov | Dec |
| 50\% | 410 | 233 | 187 | 186 | 337 | 538 | 747 | 1,032 | 1,358 | 1,718 | 1,199 | 844 |
| 75\% | 306 | 185 | 149 | 139 | 242 | 369 | 591 | 794 | 1,103 | 1,360 | 849 | 564 |
| 90\% | 235 | 150 | 122 | 106 | 179 | 263 | 479 | 627 | 915 | 1,102 | 622 | 392 |

### 12.3 Flood Runoff

To estimate the flood runoff in the Srepok basin, an analysis has been made of the observed peak flows in the basin and of the rainfall intensities. Data on monthly instantaneous peak flows are available for the Ban Don for the period 1977 - 2000. From this series the following probable discharges have been calculated assuming a Log-normal, Pearson-3 and Gumbel probability distribution respectively:

Probable yearly peak discharges at Ban Don in the Srepok basin m3/s

| Return Period years | 2 | 3 | 5 | 10 | 20 | 50 | 100 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Log-nor. | 1,403 | 1,709 | 2,049 | 2,498 | 2,952 | 3,538 | 4,632 |
| Pears.3 | 1,438 | 1,764 | 2,114 | 2,538 | 2,929 | 3,407 | 3,753 |
| Gumbel | 1,431 | 1,739 | 2,082 | 2,513 | 2,926 | 3,460 | 3,861 |
| Round average | 1,450 | 1,750 | 2,100 | 2,550 | 2,950 | 3,500 | 4,100 |

The catchment area upstream of the Ban Don station amounts to $10,700 \mathrm{~km}^{2}$. The specific peak discharge at that location, is consequently calculated at some $135 \mathrm{l} / \mathrm{s} / \mathrm{km}^{2}$ at the once in two year flood, increasing to some $400 \mathrm{l} / \mathrm{s} / \mathrm{km}^{2}$ during the once in 100 years flood.

Observed flood hydrographs (see Figure B.33) show peak discharges as a result of oneand two-day rain storms. These storms have been calculated for the Srepok basin for different return periods. The area rainfall has been estimated on the basis of the average day rainfall in the following stations:

- Buon Ma Thuot;
- Cau 42
- Giang Son and
- Duc Xuyen.

The following probable area rainfall has been calculated, assuming a log-normal probability distribution.

Maximum yearly area rainfall on the Srepok basin (mm)

| Return period in years | 2 | 3 | 5 | 10 | 20 | 50 | 100 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| One-day | 89 | 106 | 125 | 150 | 175 | 205 | 229 |
| Two-day | 117 | 146 | 178 | 222 | 268 | 326 | 373 |

## 13 DONG NAI RIVER

### 13.1 Basin Characteristics

### 13.1.1 Location

Dong Nai river basin is situated in the south east of Vietnam between $11^{\circ} 20^{\prime}$ to $12^{\circ} 20^{\prime}$ north latitude and $105^{\circ} 50^{\prime}$ to $108^{\circ} 40^{\prime}$ east longitude.

The river basin covers a total catchment area about $40,000 \mathrm{~km}^{2}$, which included Saigon and Van Co River Basins. In addition, $15 \%$ of catchments located in Cambodia.

The Dong Nai River basin is the largest river in the southern Vietnam. The Dong Nai has its outlet to the South China Sea close to the Ho Chi Minh City. Major tributaries of the Dong Nai River are:

| DongNai Basin (Total) | $39,580 \mathrm{~km}^{2}$ |
| :--- | ---: |
| Major Sub-basins |  |
| DongNai (include LaNga) : | $19,070 \mathrm{~km}^{2}$ |
| Be : | $7,430 \mathrm{~km}^{2}$ |
| Saigon | $4,720 \mathrm{~km}^{2}$ |
| Van Co | $8,360 \mathrm{~km}^{2}$ |

The location map of this basin is presented in Figure B. 34 .

### 13.1.2 Climatological Features

Mean annual temperature varies from: $17^{\circ} \mathrm{C}$ to $28^{\circ} \mathrm{C}$ and mean annual humidity varies from $77 \%$ to $85 \%$.

Mean annual rainfall varies from $1,500 \mathrm{~mm}$ to $2,800 \mathrm{~mm}$. The rainy season lasts from April to November or December. The Dry season lasts from December until March.

### 13.2 Natural Runoff

In the present study, the Dong Nai basin has been defined as the catchment that is drained through the Dong Nai river and the Saigon river, and that has its base point at the confluence of both rivers at Ho Chi Minh City. The total basin area at that confluence amounts to $29,120 \mathrm{~km}^{2}$.

In 1996 a Master Plan Study on the Dong Nai River and surrounding basins was completed by Nippon Koei under JICA assignment. In the framework of this study, the series of monthly runoff have been generated for a number of potential dam sites in the basin. The runoff was generated with the help of the Tank model, using 29 years of historical rain fall series (1964-1992). The model was calibrated with the help of historical series of daily discharge observations at gauging stations in the different sub-basins.

The two most downstream locations for which the runoff series was generated refer to:

- Hoa An in the Dong Nai river, at this location the entire runoff passes of the 22,594 $\mathrm{km}^{2}$ middle and upper catchment of the Dong Nai river basin.
- Dau Tieng, in the Saigon river, where the runoff passes of $2,700 \mathrm{~km}^{2}$ of the upper and middle Saigon sub-basin. For this station runoff series were prepared for the period prior to 1985.

The runoff series that were generated for these locations have been used as basis for the estimate of the entire basin runoff at the base point. The total area covered by these two locations corresponds with $87 \%$ of the entire basin.

The runoff of the remaining area has been estimated as follows:

- downstream of Dau Tieng Saigon river is joined by the Thi Thin tributary with a catchment area of $950 \mathrm{~km}^{2}$, the specific runoff of this area is assumed to be the same as the specific runoff of the Saigon catchment upstream of Dau Tieng;
- the right bank (looking in downstream direction) of the Saigon river downstream of Dau Tieng is considered as lower area, for which the runoff has been estimated on the basis of the net rainfall on that area; the same approach has been used for the basin area downstream of respectively Hoa An and the confluence of Thi Tinh and Saigon river.

The resulting generated series of monthly natural runoff at Ho Chi Minh City is presented in Table B. 13 for the period 1964 - 1984. The dependable runoff at the base point has been calculated as follows:

Dependable Monthly Natural Runoff at the confluence of Saigon River ( $\mathbf{A}=\mathbf{2 9 , 1 2 0} \mathbf{k m}^{\mathbf{2}}$ )

|  |  |  |  |  |  | ${\text { unit :million } \mathrm{m}^{3}}^{\prime}$ |  |  |  | Jan | Feb | Mar |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Apr | May | Jun | Jul | Aug | Sep | Oct | Nov | Dec |  |  |  |  |
| $50 \%$ | 777 | 349 | 266 | 319 | 751 | 1,982 | 3,542 | 4,649 | 5,472 | 5,979 | 3,465 | 1,617 |
| $75 \%$ | 700 | 318 | 245 | 256 | 501 | 1,576 | 3,022 | 4,036 | 4,747 | 5,254 | 3,026 | 1,410 |
| $90 \%$ | 638 | 292 | 228 | 210 | 348 | 1,282 | 2,619 | 3,554 | 4,176 | 4,676 | 2,679 | 1,247 |

### 13.3 Flood Runoff

Flood runoff in the Dong Nai basin has been addressed by Nippon Koei (1996) in the Master Plan Study on Dong Nai River and Surrounding Basins Water Resources Development. In the National Hydropower Plan Study by SWECO et al, reference is made to design flood estimates prepared by PECC2 for the different hydropower projects, no further analysis on basin flood runoff is presented in the 1999 Phase-1 report of that study. The VWRAP study on Dam Safety Issues, carried out by HASKONING in 2001 and which included the Dau Tieng reservoir in the Saigon basin, concentrates on design discharges for dams, rather than on flood damage mitigation issues.

In the present study, reference is made to the 1996 Nippon Koei Master Plan study. It was therein concluded that insufficient historic data on peak discharges are available for making a frequency analysis of flood events. As a result, the approach of Flood Runoff Modelling was followed, using the Storage Function simulation method. The flood runoff analysis was carried out for several locations, mainly existing or potential dam sites. The most downstream locations that were analysed are:

- Tri An in the Dong Nai river (catchment area: $14,025 \mathrm{~km}^{2}$ )
- Dau Tieng in the Saigon river (catchment area: 2,700 $\mathrm{km}^{2}$ ).

The flood runoff analysis was carried out for the 20 and 100 year return period only. For the three locations mentioned above, the following flood discharges were calculated:

| Return Period | 20 years | 100 years |
| :---: | :---: | :---: |
| Tri An | $6,459 \mathrm{~m}^{3} / \mathrm{s}$ | $8,265 \mathrm{~m}^{3} / \mathrm{s}$ |
| Dau Tieng | $2,351 \mathrm{~m}^{3} / \mathrm{s}$ | $3,197 \mathrm{~m}^{3} / \mathrm{s}$ |

Peak discharges at the locations where the Dong Nai and the Saigon enter the lower area at respectively Hoa An and the Saigon- Thi Tinh confluence are not presented. If a Creager type of relation is assumed between the peak discharges, than it could be estimated from these figures that peak flows at Hoa An (catchment area: 22,594 $\mathrm{km}^{2}$ ) and the Saigon - Thi Tinh confluence could approximately be as follows:

| Return Period | 20 years | 100 years |
| :---: | :---: | :---: |
| Hoa An | $8,000 \mathrm{~m}^{3} / \mathrm{s}$ | $10,000 \mathrm{~m}^{3} / \mathrm{s}$ |
| Saigon Thi Tinh confluence | $3,000 \mathrm{~m}^{3} / \mathrm{s}$ | $4,000 \mathrm{~m}^{3} / \mathrm{s}$ |

## 14 CUU LONG RIVER DELTA

### 14.1 Basin Characteristics

### 14.1.1 River basin

The lower reaches of the Mekong are known as Cuu Long. Vietnamese part of the Mekong Delta corresponds with the most southern part of the Vietnamese mainland. It spreads between $8^{\circ} 40^{\prime}$ to $11^{\circ}$ north latitude and $104^{\circ} 30^{\prime}$ to $106^{\circ} 40^{\prime}$ east longitude.

The catchment of the Mekong River starts in Tibet and totals $795,000 \mathrm{~km}^{2}$. The Cuu Long delta has its apex at Kratie in Cambodia and covers $49,500 \mathrm{~km}^{2}, 79 \%$ of this area, or some $39,000 \mathrm{~km}^{2}$ is located in the Vietnamese territory. The northern border of the catchment corresponds with the international boundary with Cambodia. The north eastern boundary of the delta basin is not clearly defined. The divide between the Delta area and the Van Co basin is difficult to assess because of manmade canals that connect both systems. In the present study, the west Van Co basin is considered part of the Mekong delta area. The western and south-eastern edge of the delta is formed by respectively the Gulf of Thailand and the South China Sea.

When entering the Vietnamese territory, the delta is formed by two branches, the Mekong and the Bassac. In the Vietnamese part of the delta a redistribution of flow takes place from the Mekong to the Bassac river through the Vam Nao. The Bassac maintains its single channel flow throughout to its mouth, while the Mekong river branches out into three estuaries before entering the South China Sea.

Location map of the Vietnamese part of the Cuu Long river basin is presented in the Figure B. 35 .
14.1.2 Climatological Features

Mean annual temperature is about $27^{\circ} \mathrm{C}$ and annual humidity varies from $70 \%$ to some 80\%.

Mean annual rainfall varies from 1400 mm to 2400 mm . The rainy season lasts from May to November. The Dry season lasts from December to March.

### 14.2 Natural Runoff

The generation of series of natural runoff of the Mekong basin is impracticable. Numerous discharge regulating infrastructure is in place in the basin. The implementation of a rainfall-runoff model for the entire basin would lack possibilities of calibration if the operation of existing infrastructure would not be taken into account. Such exercise would go far beyond the scope of the present study.

Even the assessment of the actual, regulated, runoff at the base point of the Mekong basin
is extremely complicated. Discharge measurements in the Cuu Long main branches, Mekong and Bassac, are not available. Measurement of these discharges is seriously hampered by the tidal fluctuation. The most downstream station outside of the tidal influence is located at Kratie in Cambodia. Daily discharges at this location are available only until 1969. Moreover, the Tongle Sap system, located downstream of Kratie, has a strong regulating effect on the discharges before the Mekong runoff enters the Cuu Long basin.

As a consequence of these complications, only a first estimate of the monthly runoff could be elaborated in the present study. For this estimate, the following methodology has been used:

- monthly discharge data at Kratie have been obtained from the Mekong Delta Master Plan studies, (1990 - 1993, NEDECO);
- the regulating effect of Tonle Sap has been derived from the monthly series of flow to and from Tonle Sap at Prek Kdam, prepared for the period 1960 - 1971 by the Netherlands Delta Team in 1974;
- these two series allow the preparation of the series of monthly runoff at the Vietnam Cambodia border, corresponding with the inflow into the Cuu Long Delta, for the period 1960 - 1971;
- extension of this series to 25 years ( 1960 - 1984) has been achieved by averaging the regulating effect of Tonle Sap, assuming that during the years 1972-1984 the monthly discharges at Prek Dam correspond with the average of the monthly discharges derived from the 1960-1971 values;
- the contribution of the Cuu Long Delta itself to the Mekong basin runoff has been estimated on the basis of the monthly net rainfall on the Cuu Long Delta. For this estimate the series of daily rainfall (1978-2000) at Can Tho have been used.

Based on this approach a series of monthly runoff of the Mekong basin at its base point has been estimated for the period $1960-1984$, as presented in Table B.14. Based on this series the following dependable runoff has been calculated.

|  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  | nit :mil | ion $\mathrm{m}^{3}$ |
|  | Jan | Feb | Mar | Apr | May | Jun | Jul | Aug | Sep | Oct | Nov | Dec |
| 50\% | 18,046 | 9,381 | 7,288 | 6,296 | 9,063 | 27,269 | 47,577 | 80,752 | 87,292 | 73,335 | 51,421 | 32,289 |
| 75\% | 15,895 | 8,233 | 6,500 | 5,636 | 7,304 | 20,746 | 37,718 | 67,990 | 76,089 | 64,509 | 47,561 | 29,721 |
| 90\% | 14,179 | 7,319 | 5,864 | 5,102 | 6,015 | 16,220 | 30,603 | 58,237 | 67,241 | 57,477 | 44,336 | 27,585 |

### 14.3 Flood Runoff

The flood runoff of the Mekong basin enters the Cuu Long Delta along various ways. Prior to entering Vietnam, already a substantial volume of the flood runoff is diverted from the river system to flooded areas in Cambodia. This volume has been estimated at 5 $-10 \%$ of the river discharge. Moreover, a considerable part of the flood discharge is diverted to the Great Lake through the Tonle Sap.

Efforts have been made in past and are still being made to estimate the peak discharges and flood volumes that eventually enter the Cuu Long Delta, either through the two branches of the river system, or via overland flow. An analysis made of the 1978 flood flows indicate that some $20 \%$ of the flood waters enter Vietnam overland.

The order of magnitude of the flood flows can be derived from the peak discharges at Kratie. Series of maximum day discharges are available from this station for the period 1924 - 1969. From these series, the following discharges have been estimated with the help of a number probability functions. The Pearson3 distribution is shown as an example in Figure B. 36 .

Estimated peak discharges in $\mathrm{m}^{3} / \mathrm{s}$ Kratie, based on discharge series 1924-1969

| Return period |  |  |  |
| :---: | :---: | :---: | :---: |
| 10 years | 20 years | 50 years | 100 years |
| 61,500 | 64,500 | 68,000 | 70,000 |

It is noted that the Coefficient of Variation of the yearly peak discharges is remarkably low. With a mean value of peak discharges of $52,657 \mathrm{~m}^{3} / \mathrm{s}$, and a standard deviation of $6,637 \mathrm{~m}^{3} / \mathrm{s}$, the $\mathrm{C}_{\mathrm{v}}$ amounts to 0.126 only. It is anticipated that further peak attenuation between Kratie and the Vietnamese border will even reduce this value.

Assuming a diversion of the Mekong flood runoff towards the Great Lake in the order of $15 \%$ and a $5-10 \%$ attenuation due to flooding upstream of the Vietnamese border, it can be estimated that flood flows entering Vietnam will be some $20 \%$ lower than the Kratie discharges.

Table B. 1 Generated Monthly Discharge Series Bang Giang and Ky Cung River Basin Bang Giang River at International Boundry ( $\mathrm{A}=4,460 \mathrm{~km} 2$ )

|  |  |  |  |  |  |  |  |  |  |  | (million m3) |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Jan | Feb | Mar | Apr | May | Jun | Jul | Aug | Sep | Oct | Nov | Dec | total |
| 1960 | 45 | 33 | 60 | 28 | 131 | 215 | 856 | 1,045 | 542 | 247 | 111 | 100 | 3,413 |
| 1961 | 53 | 42 | 92 | 127 | 59 | 540 | 323 | 697 | 551 | 205 | 232 | 133 | 3,054 |
| 1962 | 119 | 53 | 47 | 49 | 85 | 564 | 547 | 525 | 166 | 131 | 53 | 36 | 2,377 |
| 1963 | 28 | 27 | 30 | 27 | 36 | 53 | 364 | 423 | 209 | 116 | 172 | 87 | 1,572 |
| 1964 | 59 | 48 | 56 | 92 | 131 | 426 | 508 | 579 | 337 | 375 | 113 | 66 | 2,792 |
| 1965 | 42 | 32 | 38 | 205 | 113 | 859 | 535 | 347 | 119 | 108 | 142 | 59 | 2,599 |
| 1966 | 44 | 32 | 26 | 53 | 58 | 748 | 1,064 | 332 | 200 | 123 | 82 | 35 | 2,798 |
| 1967 | 32 | 31 | 30 | 41 | 179 | 242 | 209 | 707 | 402 | 92 | 57 | 40 | 2,062 |
| 1968 | 34 | 35 | 44 | 88 | 274 | 720 | 737 | 1,712 | 626 | 287 | 144 | 72 | 4,774 |
| 1969 | 43 | 31 | 31 | 59 | 180 | 457 | 390 | 1,137 | 285 | 112 | 88 | 40 | 2,854 |
| 1970 | 37 | 30 | 29 | 92 | 247 | 453 | 748 | 751 | 486 | 175 | 74 | 63 | 3,185 |
| 1971 | 40 | 33 | 35 | 33 | 171 | 386 | 960 | 1,370 | 479 | 299 | 85 | 54 | 3,945 |
| 1972 | 39 | 31 | 30 | 33 | 144 | 242 | 243 | 688 | 380 | 206 | 93 | 68 | 2,197 |
| 1973 | 49 | 37 | 128 | 135 | 677 | 668 | 946 | 697 | 792 | 215 | 77 | 51 | 4,471 |
| 1974 | 43 | 32 | 33 | 52 | 53 | 523 | 658 | 244 | 424 | 372 | 87 | 61 | 2,582 |
| $\overline{\text { Ave(mil m3) }}$ | 47 | 35 | 47 | 74 | 169 | 473 | 606 | 750 | 400 | 204 | 107 | 64 | 2,978 |

Ky Cung River at International Boundry ( $\mathrm{A}=\mathbf{6 , 7 9 0} \mathrm{km} 2$ )


BangGiang + KyCung at International Boundary ( $\mathrm{A}=\mathbf{1 1 , 2 5 0 \mathrm { km } 2 \text { ) } ) ~}$

|  | Jan | Feb | Mar | Apr | May | Jun | Jul | Aug | Sep | Oct | Nov | Dec | total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1960 | 143 | 108 | 186 | 85 | 277 | 408 | 2,035 | 2,475 | 1,534 | 676 | 294 | 242 | 8,463 |
| 1961 | 125 | 107 | 200 | 469 | 185 | 1,009 | 565 | 1,140 | 1,327 | 593 | 455 | 282 | 6,458 |
| 1962 | 246 | 121 | 113 | 125 | 231 | 1,376 | 1,416 | 1,597 | 484 | 292 | 137 | 98 | 6,235 |
| 1963 | 75 | 75 | 84 | 83 | 111 | 187 | 1,262 | 1,217 | 768 | 340 | 362 | 186 | 4,750 |
| 1964 | 134 | 113 | 131 | 300 | 354 | 861 | 1,345 | 1,193 | 816 | 942 | 245 | 164 | 6,599 |
| 1965 | 106 | 83 | 108 | 600 | 327 | 2,001 | 1,663 | 648 | 357 | 227 | 256 | 135 | 6,511 |
| 1966 | 112 | 84 | 72 | 149 | 127 | 1,724 | 2,039 | 795 | 412 | 254 | 156 | 90 | 6,015 |
| 1967 | 81 | 99 | 82 | 126 | 347 | 456 | 388 | 1,224 | 1,065 | 196 | 145 | 110 | 4,318 |
| 1968 | 87 | 98 | 130 | 315 | 518 | 1,160 | 1,449 | 4,403 | 1,480 | 493 | 269 | 149 | 10,551 |
| 1969 | 156 | 87 | 90 | 178 | 285 | 687 | 702 | 2,138 | 600 | 282 | 233 | 102 | 5,539 |
| 1970 | 99 | 82 | 81 | 303 | 695 | 1,187 | 1,518 | 1,619 | 1,097 | 419 | 179 | 152 | 7,431 |
| 1971 | 104 | 89 | 94 | 79 | 394 | 699 | 2,938 | 3,535 | 939 | 585 | 196 | 138 | 9,791 |
| 1972 | 100 | 83 | 78 | 91 | 547 | 462 | 461 | 1,903 | 1,026 | 485 | 238 | 160 | 5,634 |
| 1973 | 126 | 96 | 202 | 380 | 1,155 | 1,276 | 2,282 | 1,884 | 2,454 | 533 | 202 | 140 | 10,731 |
| 1974 | 122 | 90 | 87 | 130 | 170 | 1,122 | 1,426 | 562 | 1,221 | 826 | 189 | 135 | 6,079 |
| Ave(mil m3) | 121 | 94 | 116 | 228 | 382 | 974 | 1,433 | 1,756 | 1,039 | 476 | 237 | 152 | 7,007 |

Table B. 2 Generated Monthly Discharge Series Red River and Thai Binh River
Monthly Discharge at Son Tay ( $\mathrm{A}=144,000 \mathrm{~km} 2$ )

|  | Jan | Feb | Mar | Apr | May | Jun | Jul | Aug | Sep | Oct | Nov | Dec | to |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1957 | 2,461 | 1,744 | 1,993 | 3,084 | 3,053 | 15,889 | 26,409 | 15,883 | 10,368 | 9,267 | 4,199 | 2,946 | 97,298 |
| 1958 | 2,518 | 2,637 | 1,899 | 1,612 | 2,258 | 8,865 | 21,400 | 26,730 | 16,563 | 8,598 | 4,406 | 2,919 | 100,406 |
| 1959 | 2,453 | 1,880 | 3,000 | 3,318 | 6,750 | 14,282 | 17,677 | 25,204 | 15,733 | 10,231 | 5,676 | 3,616 | 109,821 |
| 1960 | 3,375 | 2,831 | 2,376 | 1,713 | 2,598 | 9,046 | 20,088 | 25,043 | 17,159 | 9,508 | 5,417 | 4,794 | 103,949 |
| 1961 | 3,241 | 2,806 | 3,187 | 3,732 | 3,669 | 13,504 | 12,963 | 30,052 | 17,004 | 14,169 | 9,124 | 5,812 | 119,264 |
| 1962 | 5,303 | 3,169 | 2,630 | 2,773 | 4,634 | 17,081 | 21,133 | 20,142 | 11,379 | 8,464 | 4,303 | 2,919 | 103,930 |
| 1963 | 2,285 | 1,860 | 2,073 | 1,768 | 2,186 | 5,599 | 17,115 | 20,624 | 11,483 | 9,964 | 12,623 | 5,384 | 92,962 |
| 1964 | 3,321 | 2,531 | 2,373 | 2,748 | 4,794 | 13,323 | 27,320 | 20,945 | 16,278 | 15,053 | 7,595 | 5,169 | 121,449 |
| 1965 | 3,535 | 2,516 | 2,341 | 3,396 | 3,964 | 14,489 | 19,713 | 15,936 | 9,513 | 11,544 | 11,586 | 5,598 | 104,131 |
| 1966 | 4,018 | 2,758 | 2,116 | 2,312 | 2,919 | 17,392 | 32,676 | 24,534 | 22,213 | 13,660 | 7,776 | 4,848 | 137,223 |
| 1967 | 3,803 | 2,758 | 2,330 | 2,561 | 3,964 | 7,595 | 13,687 | 23,275 | 15,422 | 10,392 | 6,895 | 5,250 | 97,932 |
| 1968 | 4,152 | 3,282 | 3,241 | 4,355 | 6,080 | 12,156 | 27,320 | 28,123 | 20,943 | 13,847 | 9,539 | 4,821 | 137,859 |
| 1969 | 3,321 | 2,231 | 2,001 | 2,035 | 3,750 | 8,268 | 16,579 | 39,640 | 13,530 | 6,830 | 5,832 | 3,107 | 107,124 |
| 1970 | 2,518 | 2,231 | 1,701 | 2,227 | 7,366 | 11,897 | 33,480 | 24,588 | 17,677 | 8,732 | 4,743 | 6,616 | 123,774 |
| 1971 | 3,348 | 2,734 | 2,344 | 2,773 | 5,839 | 15,319 | 29,195 | 50,354 | 24,261 | 11,785 | 7,076 | 4,232 | 159,259 |
| 1972 | 3,375 | 2,473 | 2,103 | 2,525 | 4,928 | 9,590 | 19,606 | 20,972 | 17,418 | 13,392 | 8,813 | 8,008 | 113,203 |
| 1973 | 4,366 | 3,387 | 4,473 | 3,940 | 8,062 | 15,656 | 25,070 | 28,123 | 25,505 | 11,812 | 8,165 | 5,196 | 143,754 |
| 1974 | 3,884 | 2,855 | 2,419 | 2,592 | 4,526 | 11,820 | 19,552 | 19,017 | 19,777 | 12,508 | 6,065 | 3,964 | 108,978 |
| 1975 | 3,669 | 2,279 | 2,025 | 3,473 | 6,830 | 18,377 | 16,713 | 14,544 | 15,733 | 9,428 | 6,480 | 4,071 | 103,623 |
| 1976 | 3,107 | 3,332 | 2,338 | 2,799 | 7,687 | 11,457 | 15,588 | 25,472 | 14,100 | 8,732 | 8,243 | 4,339 | 107,194 |
| 1977 | 3,455 | 2,685 | 2,443 | 3,084 | 4,071 | 5,236 | 24,186 | 21,856 | 10,212 | 9,428 | 5,858 | 3,750 | 96,264 |
| 1978 | 3,723 | 2,354 | 2,392 | 2,170 | 8,330 | 17,574 | 19,606 | 21,883 | 21,721 | 13,740 | 5,988 | 3,937 | 123,416 |
| 1979 | 3,455 | 2,951 | 2,451 | 2,385 | 4,152 | 11,664 | 16,017 | 23,168 | 25,065 | 8,866 | 4,692 | 3,268 | 108,132 |
| 1980 | 2,839 | 2,556 | 1,950 | 1,903 | 3,669 | 5,210 | 20,275 | 22,579 | 17,470 | 8,357 | 4,432 | 3,294 | 94,534 |
| 1981 | 3,080 | 2,364 | 2,164 | 4,069 | 9,053 | 15,111 | 21,240 | 26,248 | 17,937 | 12,428 | 10,316 | 5,464 | 129,474 |
| 1982 | 4,232 | 3,338 | 2,705 | 3,551 | 2,812 | 7,258 | 13,740 | 26,248 | 17,340 | 13,338 | 6,998 | 5,330 | 106,892 |
| 1983 | 4,178 | 3,242 | 3,991 | 2,696 | 3,910 | 6,299 | 9,107 | 21,722 | 18,118 | 13,499 | 11,223 | 5,437 | 103,422 |
| 1984 | 4,446 | 2,957 | 2,215 | 2,260 | 6,616 | 14,334 | 20,061 | 15,642 | 13,738 | 12,187 | 5,910 | 3,268 | 103,632 |
| 1985 | 2,547 | 1,928 | 1,995 | 2,366 | 3,910 | 11,094 | 16,124 | 18,668 | 22,395 | 9,187 | 10,679 | 5,839 | 106,734 |
| 1986 | 3,589 | 2,298 | 1,958 | 4,458 | 9,428 | 15,319 | 27,320 | 24,079 | 18,040 | 14,410 | 7,413 | 4,526 | 132,838 |
| $\overline{\text { Ave(mil m3) }}$ | 3,453 | 2,632 | 2,441 | 2,823 | 5,060 | 12,023 | 20,699 | 24,043 | 17,137 | 11,112 | 7,269 | 4,591 | 113,282 |

Monthly Discharge at Hoa Binh ( $\mathrm{A}=51,800 \mathrm{~km} 2$ )

|  | Jan | Feb | Mar | Apr | May | Jun | Jul | Aug | Sep | Oct | Nov | Dec | total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1957 | 1,053 | 728 | 868 | 1,306 | 964 | 7,880 | 12,749 | 7,500 | 4,329 | 4,446 | 1,799 | 1,200 | 44,821 |
| 1958 | 1,010 | 958 | 680 | 575 | 702 | 3,940 | 11,464 | 14,945 | 6,817 | 4,018 | 2,206 | 1,548 | 48,863 |
| 1959 | 1,205 | 806 | 1,221 | 1,213 | 2,678 | 7,854 | 9,133 | 15,213 | 7,724 | 5,437 | 2,825 | 1,845 | 57,156 |
| 1960 | 1,489 | 1,165 | 868 | 645 | 1,085 | 5,495 | 10,714 | 12,347 | 8,476 | 3,723 | 2,136 | 2,129 | 50,272 |
| 1961 | 1,412 | 1,205 | 1,061 | 1,120 | 1,221 | 6,687 | 7,633 | 17,463 | 7,957 | 5,866 | 3,603 | 2,569 | 57,797 |
| 1962 | 2,303 | 1,466 | 1,146 | 1,042 | 2,140 | 9,850 | 12,803 | 12,026 | 5,832 | 3,964 | 2,175 | 1,444 | 56,191 |
| 1963 | 1,104 | 810 | 865 | 671 | 873 | 2,955 | 8,785 | 10,231 | 5,702 | 4,553 | 5,106 | 2,239 | 43,896 |
| 1964 | 1,505 | 1,042 | 937 | 897 | 2,030 | 5,806 | 16,713 | 11,089 | 8,372 | 5,785 | 3,188 | 2,231 | 59,597 |
| 1965 | 1,519 | 1,055 | 892 | 995 | 1,219 | 6,636 | 10,821 | 9,053 | 4,614 | 5,866 | 5,495 | 2,786 | 50,949 |
| 1966 | 1,969 | 1,231 | 972 | 874 | 1,077 | 8,683 | 16,740 | 13,258 | 10,316 | 6,187 | 3,214 | 2,041 | 66,562 |
| 1967 | 1,503 | 1,040 | 854 | 941 | 1,253 | 3,084 | 8,223 | 13,499 | 6,661 | 4,071 | 3,136 | 2,092 | 46,359 |
| 1968 | 1,722 | 1,263 | 1,074 | 1,555 | 2,325 | 4,899 | 13,553 | 11,946 | 8,942 | 6,241 | 4,147 | 2,046 | 59,713 |
| 1969 | 1,409 | 943 | 790 | 723 | 1,508 | 4,277 | 9,133 | 20,275 | 5,625 | 3,134 | 2,281 | 1,320 | 51,419 |
| 1970 | 1,093 | 803 | 651 | 741 | 3,321 | 5,314 | 18,133 | 11,383 | 7,595 | 3,830 | 2,159 | 3,375 | 58,398 |
| 1971 | 1,599 | 1,243 | 1,010 | 1,265 | 2,319 | 7,413 | 12,669 | 22,579 | 9,668 | 4,419 | 2,722 | 1,690 | 68,597 |
| 1972 | 1,337 | 955 | 812 | 977 | 1,637 | 4,562 | 11,919 | 10,312 | 7,232 | 6,000 | 3,992 | 3,616 | 53,348 |
| 1973 | 1,738 | 1,306 | 1,446 | 1,586 | 3,509 | 7,569 | 11,490 | 13,071 | 9,279 | 4,419 | 3,447 | 1,931 | 60,793 |
| 1974 | 1,436 | 960 | 879 | 972 | 1,661 | 5,573 | 11,089 | 10,821 | 11,249 | 5,571 | 2,696 | 1,792 | 54,697 |
| 1975 | 1,543 | 936 | 766 | 1,239 | 2,443 | 7,750 | 8,571 | 7,419 | 7,724 | 4,312 | 3,110 | 1,958 | 47,772 |
| 1976 | 1,495 | 1,501 | 1,053 | 1,081 | 3,080 | 5,780 | 9,562 | 13,713 | 7,154 | 4,285 | 3,629 | 2,170 | 54,502 |
| 1977 | 1,570 | 1,110 | 956 | 1,203 | 1,446 | 3,344 | 13,553 | 10,767 | 5,132 | 4,580 | 2,877 | 1,867 | 48,405 |
| 1978 | 1,813 | 1,098 | 919 | 744 | 3,053 | 7,309 | 8,571 | 9,830 | 7,620 | 3,937 | 1,957 | 1,355 | 48,208 |
| 1979 | 1,101 | 878 | 729 | 666 | 983 | 3,577 | 7,339 | 11,597 | 12,079 | 4,232 | 1,910 | 1,382 | 46,473 |
| 1980 | 1,020 | 784 | 659 | 604 | 1,015 | 2,076 | 8,785 | 10,714 | 7,050 | 3,910 | 1,869 | 1,414 | 39,901 |
| 1981 | 1,181 | 839 | 790 | 925 | 3,455 | 6,998 | 10,794 | 13,312 | 9,020 | 5,544 | 5,288 | 2,670 | 60,818 |
| 1982 | 1,647 | 1,207 | 1,002 | 1,301 | 1,085 | 3,836 | 8,410 | 13,151 | 7,724 | 5,785 | 3,136 | 2,210 | 50,495 |
| 1983 | 1,666 | 1,176 | 1,374 | 892 | 1,178 | 2,618 | 4,553 | 11,919 | 10,290 | 5,651 | 5,314 | 2,598 | 49,229 |
| 1984 | 2,028 | 1,268 | 1,018 | 980 | 3,214 | 7,750 | 13,205 | 7,446 | 7,465 | 6,696 | 2,696 | 1,623 | 55,387 |
| 1985 | 1,213 | 922 | 820 | 1,001 | 1,773 | 5,962 | 9,214 | 10,151 | 10,135 | 3,910 | 4,614 | 2,303 | 52,017 |
| 1986 | 1,781 | 1,055 | 879 | 1,467 | 3,509 | 5,936 | 13,928 | 9,508 | 7,335 | 5,464 | 2,825 | 1,722 | 55,409 |
| $\overline{\text { Ave(mil m3) }}$ | 1,482 | 1,059 | 933 | 1,007 | 1,925 | 5,714 | 11,008 | 12,218 | 7,837 | 4,861 | 3,185 | 2,039 | 53,268 |

Table B. 2 Generated Monthly Discharge Series Red River and Thai Binh River
(2/2)
Monthly Discharge at Yen Bai ( $\mathrm{A}=\mathbf{4 8 , 0 0 0 \mathrm { km } 2 \text { ) } ) ~}$

|  | Jan | Feb | Mar | Apr | May | Jun | Jul | Aug | Sep | Oct | Nov | Dec | tota |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1957 | 766 | 576 | 718 | 1,008 | 908 | 3,421 | 4,821 | 4,071 | 2,748 | 3,134 | 1,164 | 828 | 24,162 |
| 1958 | 675 | 823 | 520 | 446 | 458 | 1,680 | 3,857 | 5,276 | 3,862 | 2,298 | 1,138 | 779 | 21,811 |
| 1959 | 645 | 523 | 962 | 850 | 1,645 | 2,100 | 2,973 | 5,089 | 3,629 | 2,946 | 1,607 | 956 | 23,924 |
| 1960 | 860 | 679 | 546 | 435 | 726 | 1,776 | 4,125 | 5,384 | 3,499 | 2,341 | 1,229 | 1,178 | 22,778 |
| 1961 | 787 | 820 | 854 | 1,026 | 884 | 2,903 | 2,403 | 6,455 | 5,106 | 3,884 | 2,320 | 1,567 | 29,009 |
| 1962 | 1,355 | 888 | 688 | 780 | 1,125 | 3,473 | 3,830 | 4,044 | 2,722 | 2,309 | 1,260 | 790 | 23,265 |
| 1963 | 664 | 559 | 667 | 524 | 603 | 1,296 | 3,134 | 4,044 | 2,582 | 2,660 | 2,644 | 1,259 | 20,634 |
| 1964 | 841 | 639 | 600 | 710 | 1,125 | 2,403 | 3,777 | 3,964 | 3,370 | 3,696 | 1,794 | 1,275 | 24,193 |
| 1965 | 903 | 668 | 608 | 796 | 879 | 2,773 | 2,786 | 2,973 | 2,042 | 2,786 | 2,473 | 1,594 | 21,279 |
| 1966 | 1,039 | 629 | 496 | 555 | 916 | 3,240 | 4,821 | 5,571 | 5,521 | 4,339 | 1,897 | 1,141 | 30,165 |
| 1967 | 948 | 704 | 619 | 664 | 755 | 1,672 | 2,017 | 4,232 | 3,421 | 2,611 | 1,711 | 1,283 | 20,637 |
| 1968 | 1,034 | 854 | 771 | 1,143 | 1,074 | 1,905 | 3,884 | 6,616 | 5,780 | 3,937 | 2,338 | 1,205 | 30,542 |
| 1969 | 900 | 595 | 533 | 570 | 742 | 1,319 | 2,678 | 6,776 | 3,110 | 2,041 | 1,602 | 916 | 21,784 |
| 1970 | 779 | 668 | 530 | 715 | 1,856 | 2,903 | 6,107 | 5,357 | 4,018 | 2,389 | 1,680 | 2,595 | 29,597 |
| 1971 | 1,211 | 977 | 779 | 946 | 1,529 | 3,110 | 7,366 | 12,401 | 5,210 | 3,616 | 2,291 | 1,369 | 40,806 |
| 1972 | 1,074 | 787 | 651 | 1,006 | 868 | 1,846 | 3,080 | 4,285 | 3,525 | 2,670 | 2,193 | 1,875 | 23,859 |
| 1973 | 1,031 | 837 | 1,061 | 886 | 1,259 | 2,004 | 4,285 | 5,544 | 5,676 | 2,622 | 2,468 | 1,275 | 28,949 |
| 1974 | 865 | 578 | 538 | 591 | 876 | 2,929 | 2,973 | 3,964 | 3,551 | 2,486 | 1,467 | 962 | 21,780 |
| 1975 | 1,034 | 658 | 560 | 845 | 1,176 | 3,499 | 3,187 | 3,616 | 3,551 | 2,365 | 1,908 | 1,109 | 23,507 |
| 1976 | 822 | 900 | 664 | 861 | 1,760 | 1,962 | 2,574 | 4,553 | 2,851 | 1,966 | 1,825 | 1,055 | 21,793 |
| 1977 | 865 | 714 | 624 | 832 | 846 | 1,029 | 3,777 | 3,723 | 2,644 | 2,095 | 1,332 | 943 | 19,423 |
| 1978 | 945 | 602 | 541 | 500 | 1,637 | 3,551 | 3,910 | 5,759 | 5,728 | 3,750 | 1,467 | 970 | 29,360 |
| 1979 | 814 | 685 | 536 | 542 | 905 | 1,711 | 1,998 | 4,205 | 4,303 | 2,003 | 967 | 769 | 19,437 |
| 1980 | 568 | 474 | 378 | 399 | 763 | 1,047 | 3,509 | 3,830 | 4,303 | 2,403 | 1,128 | 865 | 19,665 |
| 1981 | 742 | 525 | 442 | 560 | 1,444 | 2,022 | 2,333 | 3,161 | 2,227 | 1,856 | 1,786 | 1,307 | 18,403 |
| 1982 | 731 | 588 | 455 | 643 | 533 | 1,312 | 2,322 | 5,009 | 3,862 | 2,866 | 1,586 | 1,104 | 21,010 |
| 1983 | 1,010 | 791 | 1,173 | 604 | 729 | 1,135 | 1,497 | 3,696 | 3,421 | 3,830 | 3,344 | 1,489 | 22,720 |
| 1984 | 1,120 | 672 | 554 | 638 | 1,733 | 3,655 | 3,964 | 3,214 | 2,903 | 2,732 | 1,159 | 790 | 23,133 |
| 1985 | 632 | 527 | 536 | 648 | 927 | 2,696 | 2,625 | 3,535 | 4,743 | 2,411 | 2,592 | 1,296 | 23,168 |
| 1986 | 841 | 583 | 530 | 1,021 | 1,655 | 2,333 | 4,393 | 4,500 | 4,588 | 4,955 | 1,864 | 1,165 | 28,428 |
| Ave(mil m3) | 883 | 684 | 638 | 725 | 1,078 | 2,290 | 3,500 | 4,828 | 3,817 | 2,867 | 1,808 | 1,190 | 24,307 |

Monthly Discharge at Vu Quang ( $\mathrm{A}=\mathbf{2 9 , 6 0 0 \mathrm { km } 2 \text { ) } ) ~}$

|  | Jan | Feb | Mar | Apr | May | Jun | Jul | Aug | Sep | Oct | Nov | Dec | total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1957 | 675 | 532 | 573 | 982 | 1,446 | 4,873 | 7,473 | 3,455 | 2,618 | 1,671 | 1,166 | 846 | 26,312 |
| 1958 | 654 | 750 | 536 | 495 | 937 | 2,491 | 5,276 | 7,017 | 4,925 | 2,314 | 1,231 | 822 | 27,449 |
| 1959 | 691 | 561 | 956 | 1,156 | 2,786 | 4,458 | 6,107 | 6,776 | 4,380 | 2,167 | 1,345 | 1,004 | 32,388 |
| 1960 | 887 | 739 | 779 | 485 | 1,079 | 2,447 | 6,321 | 5,919 | 5,184 | 2,461 | 1,457 | 1,283 | 29,041 |
| 1961 | 860 | 723 | 1,224 | 1,446 | 1,420 | 4,562 | 3,375 | 8,089 | 4,147 | 3,375 | 2,825 | 1,661 | 33,706 |
| 1962 | 1,505 | 854 | 726 | 874 | 1,334 | 4,484 | 5,973 | 5,651 | 2,722 | 1,902 | 1,122 | 817 | 27,963 |
| 1963 | 648 | 617 | 702 | 648 | 822 | 1,672 | 4,339 | 4,821 | 2,320 | 2,282 | 3,732 | 1,540 | 24,143 |
| 1964 | 862 | 694 | 704 | 933 | 1,596 | 5,262 | 6,134 | 6,107 | 3,421 | 4,259 | 2,037 | 1,358 | 33,368 |
| 1965 | 945 | 641 | 667 | 1,374 | 1,760 | 3,966 | 5,464 | 3,482 | 1,941 | 2,786 | 3,110 | 1,283 | 27,419 |
| 1966 | 1,004 | 728 | 568 | 765 | 911 | 4,795 | 9,937 | 4,768 | 3,888 | 2,346 | 1,700 | 1,098 | 32,508 |
| 1967 | 769 | 614 | 570 | 695 | 1,730 | 2,312 | 3,134 | 5,250 | 3,603 | 2,443 | 1,322 | 1,074 | 23,516 |
| 1968 | 870 | 767 | 830 | 1,283 | 1,870 | 4,458 | 8,357 | 7,419 | 5,262 | 3,134 | 2,478 | 1,331 | 38,059 |
| 1969 | 916 | 612 | 587 | 710 | 1,270 | 2,696 | 4,473 | 10,606 | 4,121 | 2,370 | 2,141 | 1,192 | 31,694 |
| 1970 | 879 | 781 | 645 | 918 | 2,255 | 3,525 | 8,491 | 6,455 | 5,028 | 2,266 | 1,257 | 1,275 | 33,775 |
| 1971 | 846 | 706 | 595 | 907 | 2,033 | 4,795 | 7,794 | 14,570 | 7,128 | 3,509 | 1,778 | 1,409 | 46,071 |
| 1972 | 892 | 749 | 640 | 687 | 1,888 | 2,538 | 3,937 | 4,821 | 4,484 | 3,161 | 1,446 | 1,082 | 26,325 |
| 1973 | 1,039 | 1,016 | 1,642 | 1,226 | 3,214 | 5,054 | 6,589 | 7,258 | 6,636 | 3,616 | 1,757 | 1,409 | 40,457 |
| 1974 | 1,347 | 1,164 | 844 | 778 | 1,821 | 3,396 | 5,973 | 3,803 | 4,847 | 3,589 | 1,470 | 1,184 | 30,215 |
| 1975 | 996 | 692 | 828 | 1,348 | 3,777 | 7,569 | 5,518 | 3,991 | 3,681 | 2,384 | 1,591 | 1,133 | 33,506 |
| 1976 | 903 | 1,017 | 887 | 874 | 3,080 | 3,214 | 3,964 | 5,464 | 3,473 | 2,095 | 1,952 | 919 | 27,840 |
| 1977 | 937 | 885 | 745 | 863 | 1,709 | 889 | 5,812 | 4,928 | 2,317 | 2,582 | 1,231 | 691 | 23,590 |
| 1978 | 737 | 464 | 680 | 687 | 3,134 | 6,506 | 5,571 | 5,276 | 5,910 | 4,446 | 2,532 | 1,567 | 37,511 |
| 1979 | 1,513 | 1,340 | 1,144 | 1,115 | 2,044 | 6,299 | 6,991 | 7,500 | 7,076 | 2,579 | 1,752 | 1,074 | 40,426 |
| 1980 | 1,465 | 1,273 | 758 | 692 | 1,607 | 2,193 | 6,241 | 7,500 | 4,743 | 1,955 | 1,190 | 967 | 30,583 |
| 1981 | 1,195 | 1,110 | 1,398 | 2,366 | 4,018 | 4,069 | 5,410 | 6,669 | 5,106 | 4,044 | 2,773 | 1,711 | 39,872 |
| 1982 | 1,524 | 1,304 | 1,211 | 1,363 | 1,173 | 3,214 | 4,500 | 7,741 | 5,262 | 3,964 | 2,773 | 1,982 | 36,011 |
| 1983 | 1,497 | 1,323 | 1,454 | 1,078 | 1,942 | 2,773 | 3,000 | 7,982 | 5,936 | 3,643 | 3,266 | 1,827 | 35,721 |
| 1984 | 1,444 | 1,123 | 817 | 982 | 2,124 | 5,651 | 7,098 | 6,241 | 4,795 | 4,232 | 2,317 | 1,377 | 38,199 |
| 1985 | 1,350 | 1,159 | 1,476 | 1,405 | 1,553 | 2,877 | 4,259 | 5,330 | 7,387 | 2,370 | 2,722 | 1,628 | 33,516 |
| 1986 | 1,063 | 919 | 750 | 1,747 | 3,402 | 5,936 | 9,026 | 7,794 | 5,288 | 3,428 | 2,045 | 1,553 | 42,952 |
| Ave(mil m3) | 1,030 | 862 | 864 | 1,029 | 1,991 | 3,966 | 5,884 | 6,423 | 4,588 | 2,912 | 1,984 | 1,270 | 32,805 |

Table B. 3 Generated Monthly Discharge Series Ma River
Ma River at confluence of Buoi River ( $\mathrm{A}=\mathbf{1 9 , 8 2 0} \mathbf{k m} 2$ )

|  | Jan | Feb | Mar | Apr | May | Jun | Jul | Aug | Sep | Oct | Nov | Dec | total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1981 | 419 | 300 | 310 | 381 | 649 | 1,261 | 1,731 | 2,330 | 1,876 | 1,790 | 844 | 578 | 12,469 |
| 1982 | 485 | 362 | 344 | 394 | 399 | 829 | 1,437 | 4,156 | 2,910 | 1,811 | 882 | 628 | 14,638 |
| 1983 | 542 | 406 | 426 | 349 | 456 | 436 | 785 | 1,578 | 1,869 | 2,101 | 824 | 565 | 10,337 |
| 1984 | 485 | 359 | 307 | 300 | 592 | 1,216 | 1,961 | 1,791 | 1,142 | 1,350 | 995 | 539 | 11,038 |
| 1985 | 482 | 379 | 339 | 470 | 542 | 912 | 1,074 | 2,135 | 3,201 | 1,436 | 904 | 608 | 12,482 |
| 1986 | 462 | 328 | 318 | 352 | 962 | 1,357 | 1,736 | 1,587 | 1,485 | 972 | 610 | 490 | 10,659 |
| 1987 | 405 | 330 | 319 | 297 | 350 | 399 | 771 | 1,976 | 1,736 | 935 | 573 | 441 | 8,534 |
| 1988 | 395 | 301 | 291 | 291 | 467 | 338 | 790 | 1,516 | 1,091 | 2,111 | 650 | 429 | 8,670 |
| 1989 | 229 | 176 | 202 | 184 | 350 | 800 | 722 | 803 | 800 | 1,442 | 499 | 323 | 6,531 |
| 1990 | 269 | 228 | 301 | 228 | 436 | 1,057 | 1,533 | 1,113 | 1,031 | 1,057 | 531 | 336 | 8,121 |
| 1991 | 284 | 214 | 216 | 221 | 258 | 889 | 1,136 | 1,250 | 639 | 428 | 319 | 293 | 6,149 |
| 1992 | 264 | 208 | 184 | 155 | 224 | 423 | 932 | 620 | 872 | 462 | 294 | 260 | 4,898 |
| 1993 | 217 | 167 | 177 | 182 | 344 | 280 | 535 | 886 | 1,047 | 490 | 307 | 259 | 4,891 |
| 1994 | 201 | 159 | 196 | 228 | 412 | 751 | 2,038 | 1,864 | 1,692 | 967 | 507 | 420 | 9,436 |
| 1995 | 403 | 288 | 268 | 224 | 298 | 1,319 | 2,101 | 3,817 | 2,746 | 960 | 602 | 424 | 13,452 |
| 1996 | 388 | 299 | 343 | 285 | 548 | 769 | 2,617 | 5,035 | 3,625 | 1,642 | 1,537 | 703 | 17,791 |
| 1997 | 531 | 390 | 457 | 562 | 449 | 671 | 2,764 | 3,297 | 3,458 | 1,638 | 746 | 558 | 15,522 |
| 1998 | 424 | 330 | 305 | 340 | 441 | 803 | 1,308 | 947 | 1,340 | 614 | 379 | 310 | 7,541 |
| 1999 | 262 | 185 | 182 | 229 | 462 | 1,656 | 1,337 | 2,251 | 2,199 | 934 | 686 | 515 | 10,899 |
| 2000 | 396 | 345 | 349 | 306 | 712 | 1,149 | 2,277 | 2,061 | 2,822 | 1,292 | 647 | 501 | 12,854 |
| Ave(mil m3) | 377 | 288 | 292 | 299 | 468 | 866 | 1,479 | 2,051 | 1,879 | 1,222 | 667 | 459 | 10,346 |

Chu River at Bai Thuong ( $\mathrm{A}=7,460 \mathrm{~km} 2$ ) including Am River

|  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
|  | Jan | Feb | Mar | Apr | May | Jun | Jul | Aug | Sep | Oct | Nov | Dec | total |
| 1981 | 189 | 141 | 146 | 183 | 210 | 388 | 736 | 604 | 602 | 1,190 | 453 | 263 | 5,105 |
| 1982 | 176 | 123 | 110 | 125 | 140 | 392 | 406 | 715 | 1,398 | 1,194 | 467 | 285 | 5,531 |
| 1983 | 240 | 161 | 155 | 111 | 140 | 148 | 204 | 374 | 441 | 983 | 312 | 183 | 3,452 |
| 1984 | 144 | 115 | 100 | 102 | 116 | 349 | 455 | 512 | 479 | 827 | 529 | 234 | 3,963 |
| 1985 | 188 | 141 | 133 | 141 | 231 | 520 | 336 | 640 | 1,289 | 926 | 474 | 304 | 5,322 |
| 1986 | 202 | 142 | 121 | 147 | 476 | 436 | 470 | 501 | 673 | 587 | 313 | 207 | 4,275 |
| 1987 | 180 | 135 | 123 | 126 | 171 | 226 | 325 | 1,070 | 598 | 473 | 289 | 195 | 3,910 |
| 1988 | 135 | 103 | 84 | 73 | 214 | 175 | 226 | 457 | 262 | 2,107 | 384 | 211 | 4,431 |
| 1989 | 160 | 117 | 129 | 99 | 303 | 692 | 720 | 604 | 729 | 1,888 | 505 | 288 | 6,234 |
| 1990 | 230 | 186 | 269 | 174 | 319 | 547 | 794 | 824 | 1,089 | 1,284 | 547 | 286 | 6,548 |
| 1991 | 231 | 164 | 158 | 157 | 167 | 438 | 564 | 1,049 | 520 | 311 | 214 | 186 | 4,158 |
| 1992 | 173 | 123 | 102 | 84 | 126 | 315 | 415 | 383 | 845 | 339 | 181 | 151 | 3,236 |
| 1993 | 116 | 84 | 81 | 100 | 306 | 180 | 333 | 636 | 867 | 334 | 177 | 141 | 3,356 |
| 1994 | 106 | 84 | 114 | 159 | 391 | 414 | 882 | 1,525 | 1,726 | 769 | 359 | 281 | 6,809 |
| 1995 | 198 | 143 | 137 | 110 | 239 | 436 | 575 | 1,108 | 1,619 | 454 | 268 | 203 | 5,491 |
| 1996 | 169 | 134 | 154 | 124 | 241 | 440 | 737 | 1,405 | 1,791 | 741 | 1,022 | 370 | 7,327 |
| 1997 | 264 | 189 | 189 | 291 | 212 | 328 | 852 | 933 | 1,157 | 918 | 374 | 267 | 5,975 |
| 1998 | 198 | 140 | 122 | 134 | 191 | 265 | 256 | 305 | 600 | 264 | 150 | 132 | 2,758 |
| 1999 | 91 | 66 | 61 | 92 | 353 | 668 | 318 | 603 | 672 | 560 | 464 | 277 | 4,227 |
| 2000 | 186 | 141 | 140 | 150 | 432 | 516 | 540 | 579 | 1,176 | 561 | 301 | 218 | 4,939 |
| Ave(mil m3) | 179 | 132 | 131 | 134 | 249 | 394 | 507 | 741 | 927 | 835 | 389 | 234 | 4,852 |

Ma River Basin ( $\mathrm{A}=\mathbf{3 1 , 0 6 0 \mathrm { km } 2 \text { ) } ) ~}$

|  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Year | Jan | Feb | Mar | Apr | May | Jun | Jul | Aug | Sep | Oct | Nov | Dec | total |
| 1981 | 608 | 441 | 456 | 572 | 859 | 1,649 | 2,466 | 3,090 | 2,612 | 4,876 | 1,297 | 841 | 19,768 |
| 1982 | 662 | 485 | 454 | 519 | 604 | 2,051 | 1,843 | 4,871 | 7,118 | 4,998 | 1,722 | 913 | 26,240 |
| 1983 | 782 | 617 | 581 | 460 | 597 | 584 | 988 | 2,218 | 2,513 | 5,228 | 1,136 | 749 | 16,454 |
| 1984 | 628 | 475 | 407 | 402 | 708 | 2,173 | 2,482 | 2,355 | 2,787 | 2,747 | 1,577 | 773 | 17,514 |
| 1985 | 670 | 520 | 472 | 611 | 772 | 1,545 | 1,663 | 3,296 | 6,347 | 2,572 | 1,614 | 911 | 20,994 |
| 1986 | 664 | 470 | 438 | 499 | 1,438 | 1,913 | 2,354 | 2,648 | 3,236 | 2,123 | 965 | 697 | 17,445 |
| 1987 | 585 | 464 | 442 | 423 | 529 | 625 | 1,097 | 3,910 | 2,983 | 1,563 | 862 | 637 | 14,120 |
| 1988 | 530 | 404 | 375 | 364 | 681 | 514 | 1,016 | 1,973 | 1,818 | 5,191 | 1,034 | 640 | 14,539 |
| 1989 | 389 | 294 | 331 | 283 | 801 | 2,884 | 2,285 | 1,736 | 1,759 | 4,531 | 1,003 | 611 | 16,907 |
| 1990 | 499 | 414 | 570 | 402 | 755 | 1,604 | 2,327 | 2,496 | 3,197 | 4,175 | 1,726 | 622 | 18,788 |
| 1991 | 515 | 378 | 374 | 379 | 426 | 1,327 | 1,700 | 2,717 | 1,159 | 739 | 533 | 479 | 10,725 |
| 1992 | 437 | 331 | 286 | 239 | 350 | 737 | 2,572 | 1,087 | 3,412 | 801 | 475 | 411 | 11,138 |
| 1993 | 334 | 251 | 258 | 282 | 676 | 461 | 868 | 1,651 | 2,747 | 825 | 484 | 400 | 9,235 |
| 1994 | 307 | 244 | 311 | 387 | 1,408 | 1,964 | 4,443 | 4,458 | 6,130 | 2,496 | 865 | 700 | 23,713 |
| 1995 | 601 | 431 | 405 | 334 | 537 | 1,991 | 2,808 | 5,919 | 4,795 | 1,414 | 870 | 627 | 20,734 |
| 1996 | 558 | 433 | 497 | 409 | 788 | 1,208 | 3,882 | 7,683 | 7,045 | 2,551 | 3,825 | 1,073 | 29,952 |
| 1997 | 796 | 579 | 646 | 1,162 | 661 | 1,271 | 3,979 | 5,428 | 4,911 | 3,209 | 1,120 | 825 | 24,589 |
| 1998 | 622 | 470 | 427 | 474 | 785 | 1,132 | 1,563 | 1,415 | 2,313 | 877 | 530 | 442 | 11,051 |
| 1999 | 353 | 251 | 243 | 386 | 1,177 | 2,324 | 1,655 | 3,073 | 3,016 | 3,815 | 1,524 | 793 | 18,612 |
| 2000 | 582 | 486 | 488 | 456 | 1,144 | 1,665 | 2,817 | 2,932 | 5,405 | 1,975 | 948 | 719 | 19,617 |
| Ave | 556 | 422 | 423 | 452 | 785 | 1,481 | 2,240 | 3,248 | 3,765 | 2,835 | 1,206 | 693 | 18,107 |

Table B. 4 Generated Monthly Discharge Series Ca River Basin
Ca River at Do Luong ( $\mathrm{A}=21,130 \mathrm{~km} 2$ )

|  | Jan | Feb | Mar | Apr | May | Jun | Jul | Aug | Sep | Oct | Nov | Dec | total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1976 | 438 | 369 | 303 | 307 | 497 | 682 | 828 | 1,756 | 958 | 907 | 1,131 | 469 | 8,644 |
| 1977 | 390 | 277 | 252 | 337 | 417 | 281 | 769 | 1,086 | 1,811 | 662 | 437 | 345 | 7,065 |
| 1978 | 292 | 216 | 211 | 308 | 405 | 930 | 1,087 | 2,890 | 6,314 | 4,703 | 1,445 | 806 | 19,608 |
| 1979 | 591 | 418 | 360 | 356 | 558 | 1,432 | 911 | 1,790 | 1,832 | 859 | 473 | 350 | 9,933 |
| 1980 | 286 | 242 | 202 | 223 | 443 | 776 | 1,495 | 1,522 | 6,316 | 2,813 | 790 | 551 | 15,659 |
| 1981 | 431 | 286 | 275 | 417 | 503 | 1,207 | 2,114 | 1,851 | 1,697 | 3,324 | 1,261 | 686 | 14,051 |
| 1982 | 543 | 406 | 337 | 433 | 478 | 1,400 | 1,211 | 2,384 | 4,760 | 4,118 | 1,538 | 929 | 18,535 |
| 1983 | 756 | 532 | 580 | 395 | 513 | 492 | 782 | 1,403 | 1,604 | 3,233 | 1,218 | 624 | 12,132 |
| 1984 | 462 | 360 | 304 | 291 | 682 | 831 | 1,115 | 1,870 | 1,195 | 2,868 | 1,493 | 673 | 12,143 |
| 1985 | 525 | 400 | 357 | 334 | 396 | 1,056 | 1,015 | 1,771 | 3,132 | 1,951 | 1,121 | 722 | 12,780 |
| 1986 | 478 | 352 | 322 | 338 | 1,587 | 1,325 | 1,292 | 1,408 | 1,918 | 1,909 | 901 | 556 | 12,388 |
| 1987 | 393 | 297 | 288 | 289 | 385 | 693 | 800 | 2,803 | 2,054 | 1,375 | 792 | 492 | 10,662 |
| 1988 | 361 | 293 | 278 | 236 | 457 | 382 | 442 | 1,053 | 815 | 5,573 | 1,154 | 714 | 11,758 |
| 1989 | 428 | 313 | 309 | 267 | 1,037 | 2,240 | 2,043 | 2,585 | 2,747 | 4,909 | 1,507 | 758 | 19,144 |
| 1990 | 558 | 423 | 604 | 349 | 539 | 1,376 | 2,503 | 2,250 | 2,762 | 3,912 | 1,676 | 824 | 17,775 |
| 1991 | 572 | 425 | 396 | 383 | 400 | 930 | 1,304 | 3,642 | 1,708 | 1,617 | 760 | 585 | 12,719 |
| 1992 | 570 | 394 | 319 | 208 | 329 | 764 | 1,386 | 1,230 | 1,628 | 1,277 | 482 | 370 | 8,956 |
| 1993 | 263 | 183 | 173 | 215 | 647 | 519 | 1,498 | 1,234 | 2,049 | 860 | 430 | 313 | 8,385 |
| 1994 | 250 | 230 | 247 | 270 | 635 | 972 | 2,075 | 4,410 | 4,201 | 1,987 | 975 | 754 | 17,006 |
| 1995 | 541 | 395 | 375 | 323 | 575 | 990 | 1,325 | 2,789 | 4,517 | 1,590 | 828 | 586 | 14,833 |
| 1996 | 421 | 345 | 355 | 334 | 520 | 648 | 1,075 | 3,276 | 5,825 | 2,475 | 2,745 | 1,041 | 19,061 |
| 1997 | 716 | 531 | 492 | 682 | 615 | 813 | 2,201 | 2,785 | 3,056 | 2,131 | 927 | 635 | 15,585 |
| 1998 | 460 | 368 | 349 | 366 | 411 | 535 | 843 | 755 | 993 | 528 | 334 | 308 | 6,250 |
| 1999 | 259 | 205 | 204 | 308 | 874 | 1,317 | 857 | 1,519 | 1,712 | 1,610 | 1,173 | 571 | 10,610 |
| 2000 | 385 | 302 | 311 | 289 | 1,002 | 1,597 | 1,615 | 1,448 | 2,815 | 1,283 | 695 | 474 | 12,217 |
| Ave(mil m3) | 455 | 342 | 328 | 330 | 596 | 967 | 1,303 | 2,060 | 2,737 | 2,339 | 1,051 | 605 | 13,116 |

At the confluence of Ngan Sau River and Ngan Pho River ( $\mathrm{A}=\mathbf{3 , 2 1 0 k m}$ )

|  | Jan | Feb | Mar | Apr | May | Jun | Jul | Aug | Sep | Oct | Nov | Dec | total |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| 1976 | 210 | 194 | 199 | 152 | 261 | 196 | 139 | 165 | 186 | 739 | 1,692 | 304 | 4,437 |
| 1977 | 285 | 201 | 175 | 229 | 197 | 85 | 67 | 169 | 448 | 535 | 572 | 265 | 3,229 |
| 1978 | 245 | 186 | 270 | 221 | 381 | 204 | 230 | 829 | 4,056 | 1,489 | 553 | 409 | 9,072 |
| 1979 | 314 | 213 | 176 | 199 | 365 | 368 | 185 | 1,077 | 2,234 | 502 | 245 | 185 | 6,063 |
| 1980 | 166 | 143 | 131 | 155 | 213 | 287 | 171 | 176 | 1,884 | 1,863 | 553 | 383 | 6,126 |
| 1981 | 276 | 182 | 162 | 130 | 474 | 259 | 351 | 194 | 1,476 | 1,363 | 1,038 | 332 | 6,238 |
| 1982 | 241 | 207 | 216 | 222 | 233 | 340 | 322 | 466 | 750 | 677 | 357 | 288 | 4,318 |
| 1983 | 267 | 223 | 245 | 217 | 237 | 229 | 270 | 346 | 365 | 569 | 318 | 251 | 3,536 |
| 1984 | 231 | 207 | 212 | 204 | 258 | 270 | 311 | 403 | 315 | 525 | 351 | 257 | 3,543 |
| 1985 | 239 | 206 | 218 | 210 | 223 | 298 | 298 | 391 | 551 | 413 | 306 | 263 | 3,615 |
| 1986 | 233 | 201 | 214 | 210 | 368 | 331 | 332 | 346 | 403 | 408 | 279 | 242 | 3,567 |
| 1987 | 222 | 194 | 210 | 204 | 222 | 253 | 272 | 517 | 420 | 342 | 266 | 235 | 3,356 |
| 1988 | 219 | 199 | 208 | 198 | 230 | 216 | 228 | 303 | 268 | 855 | 310 | 262 | 3,496 |
| 1989 | 227 | 196 | 212 | 201 | 301 | 442 | 424 | 490 | 504 | 774 | 353 | 267 | 4,392 |
| 1990 | 243 | 209 | 248 | 211 | 240 | 337 | 480 | 449 | 506 | 652 | 373 | 275 | 4,225 |
| 1991 | 244 | 209 | 223 | 216 | 223 | 282 | 334 | 619 | 377 | 372 | 262 | 246 | 3,607 |
| 1992 | 244 | 211 | 213 | 194 | 215 | 262 | 344 | 325 | 368 | 330 | 228 | 220 | 3,153 |
| 1993 | 207 | 180 | 196 | 195 | 253 | 232 | 357 | 325 | 419 | 279 | 221 | 213 | 3,078 |
| 1994 | 205 | 186 | 205 | 202 | 252 | 287 | 428 | 713 | 682 | 417 | 288 | 266 | 4,131 |
| 1995 | 241 | 206 | 220 | 208 | 245 | 290 | 336 | 515 | 721 | 369 | 270 | 246 | 3,866 |
| 1996 | 226 | 205 | 218 | 210 | 238 | 248 | 306 | 574 | 880 | 477 | 504 | 302 | 4,388 |
| 1997 | 310 | 249 | 207 | 256 | 193 | 115 | 136 | 250 | 618 | 1,020 | 261 | 170 | 3,786 |
| 1998 | 126 | 121 | 108 | 94 | 108 | 84 | 80 | 96 | 661 | 349 | 244 | 253 | 2,324 |
| 1999 | 176 | 118 | 119 | 191 | 425 | 169 | 96 | 92 | 110 | 1,054 | 830 | 243 | 3,623 |
| 2000 | 185 | 156 | 200 | 180 | 369 | 508 | 286 | 704 | 1,040 | 1,103 | 540 | 451 | 5,721 |
| Ave(mil m3) | 231 | 192 | 200 | 196 | 269 | 264 | 271 | 421 | 810 | 699 | 449 | 273 | 4,276 |

Table B. 4 Generated Monthly Discharge Series Ca River Basin

Giang River ( $\mathrm{A}=\mathbf{1 , 0 9 0} \mathbf{k m}$ 2)

|  | Jan | Feb | Mar | Apr | May | Jun | Jul | Aug | Sep | Oct | Nov | Dec | total |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| 1976 | 29 | 25 | 22 | 21 | 34 | 39 | 43 | 86 | 51 | 74 | 126 | 35 | 586 |
| 1977 | 30 | 21 | 19 | 25 | 28 | 16 | 37 | 56 | 101 | 54 | 45 | 27 | 461 |
| 1978 | 24 | 18 | 22 | 24 | 35 | 51 | 59 | 167 | 464 | 277 | 90 | 54 | 1,284 |
| 1979 | 41 | 28 | 24 | 25 | 41 | 81 | 49 | 128 | 182 | 61 | 32 | 24 | 716 |
| 1980 | 20 | 17 | 15 | 17 | 29 | 48 | 75 | 76 | 367 | 209 | 60 | 42 | 976 |
| 1981 | 32 | 21 | 20 | 24 | 44 | 66 | 110 | 92 | 142 | 210 | 103 | 46 | 909 |
| 1982 | 35 | 27 | 25 | 29 | 32 | 78 | 69 | 128 | 247 | 215 | 85 | 54 | 1,023 |
| 1983 | 46 | 34 | 37 | 27 | 34 | 32 | 47 | 78 | 88 | 170 | 69 | 39 | 702 |
| 1984 | 31 | 25 | 23 | 22 | 42 | 49 | 64 | 102 | 68 | 152 | 83 | 42 | 702 |
| 1985 | 34 | 27 | 26 | 24 | 28 | 61 | 59 | 97 | 165 | 106 | 64 | 44 | 734 |
| 1986 | 32 | 25 | 24 | 25 | 88 | 74 | 73 | 79 | 104 | 104 | 53 | 36 | 714 |
| 1987 | 28 | 22 | 22 | 22 | 27 | 42 | 48 | 149 | 111 | 77 | 47 | 33 | 628 |
| 1988 | 26 | 22 | 22 | 19 | 31 | 27 | 30 | 61 | 49 | 288 | 66 | 44 | 683 |
| 1989 | 29 | 23 | 23 | 21 | 60 | 120 | 110 | 138 | 146 | 254 | 83 | 46 | 1,054 |
| 1990 | 36 | 28 | 38 | 25 | 35 | 77 | 134 | 121 | 146 | 204 | 92 | 49 | 985 |
| 1991 | 37 | 28 | 28 | 27 | 28 | 54 | 73 | 191 | 93 | 89 | 46 | 37 | 731 |
| 1992 | 36 | 27 | 24 | 18 | 24 | 46 | 77 | 70 | 89 | 72 | 32 | 26 | 542 |
| 1993 | 21 | 16 | 17 | 18 | 40 | 34 | 83 | 70 | 111 | 51 | 29 | 24 | 513 |
| 1994 | 20 | 19 | 20 | 21 | 40 | 56 | 112 | 229 | 219 | 108 | 57 | 46 | 947 |
| 1995 | 35 | 27 | 27 | 24 | 37 | 57 | 74 | 148 | 235 | 88 | 49 | 37 | 837 |
| 1996 | 29 | 25 | 26 | 24 | 34 | 40 | 62 | 172 | 300 | 132 | 145 | 60 | 1,050 |
| 1997 | 46 | 35 | 31 | 42 | 36 | 42 | 105 | 136 | 165 | 141 | 53 | 36 | 867 |
| 1998 | 26 | 22 | 21 | 21 | 23 | 28 | 41 | 38 | 74 | 39 | 26 | 25 | 384 |
| 1999 | 19 | 14 | 14 | 22 | 58 | 67 | 43 | 72 | 82 | 119 | 90 | 36 | 637 |
| 2000 | 26 | 20 | 23 | 21 | 61 | 94 | 85 | 96 | 173 | 107 | 55 | 41 | 803 |
| Ave(mil m3) | 31 | 24 | 24 | 24 | 39 | 55 | 71 | 111 | 159 | 136 | 67 | 39 | 779 |

Ca River Basin (A=29,850km2)


Table B. 5 Generated Monthly Discharge Series Thach Han River Basin

|  | Jan | Feb | Mar | Apr | May | Jun | Jul | Aug | Sep | Oct | Nov | Dec | total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1977 | 273 | 154 | 107 | 69 | 49 | 29 | 24 | 38 | 107 | 324 | 407 | 180 | 1,760 |
| 1978 | 118 | 62 | 50 | 33 | 83 | 62 | 77 | 226 | 466 | 363 | 362 | 293 | 2,195 |
| 1979 | 137 | 76 | 52 | 32 | 29 | 101 | 97 | 208 | 294 | 287 | 240 | 162 | 1,716 |
| 1980 | 89 | 51 | 34 | 22 | 68 | 117 | 119 | 62 | 444 | 875 | 656 | 316 | 2,853 |
| 1981 | 213 | 109 | 75 | 56 | 99 | 170 | 129 | 74 | 120 | 487 | 843 | 389 | 2,763 |
| 1982 | 182 | 102 | 70 | 43 | 35 | 23 | 47 | 23 | 213 | 216 | 252 | 161 | 1,365 |
| 1983 | 85 | 48 | 33 | 21 | 16 | 41 | 69 | 37 | 32 | 319 | 670 | 214 | 1,583 |
| 1984 | 128 | 74 | 49 | 32 | 41 | 106 | 76 | 271 | 142 | 438 | 468 | 266 | 2,092 |
| 1985 | 127 | 73 | 51 | 32 | 27 | 91 | 67 | 47 | 131 | 554 | 550 | 369 | 2,119 |
| 1986 | 163 | 90 | 62 | 38 | 65 | 40 | 25 | 59 | 59 | 365 | 320 | 290 | 1,576 |
| 1987 | 144 | 82 | 55 | 41 | 42 | 28 | 25 | 107 | 271 | 171 | 268 | 179 | 1,414 |
| 1988 | 92 | 53 | 35 | 21 | 25 | 13 | 9 | 29 | 95 | 477 | 297 | 255 | 1,400 |
| 1989 | 163 | 78 | 59 | 39 | 140 | 187 | 155 | 182 | 118 | 235 | 189 | 182 | 1,728 |
| 1990 | 85 | 51 | 41 | 25 | 42 | 80 | 52 | 87 | 435 | 1,279 | 834 | 325 | 3,336 |
| 1991 | 177 | 100 | 73 | 44 | 32 | 23 | 16 | 66 | 74 | 408 | 274 | 334 | 1,620 |
| 1992 | 146 | 83 | 56 | 33 | 26 | 28 | 76 | 185 | 168 | 577 | 589 | 280 | 2,248 |
| 1993 | 164 | 87 | 63 | 39 | 34 | 26 | 16 | 14 | 75 | 378 | 363 | 864 | 2,122 |
| 1994 | 259 | 143 | 101 | 67 | 44 | 54 | 43 | 35 | 202 | 186 | 242 | 330 | 1,707 |
| 1995 | 161 | 90 | 61 | 38 | 32 | 22 | 20 | 46 | 281 | 1,077 | 852 | 389 | 3,069 |
| 1996 | 200 | 118 | 76 | 54 | 49 | 69 | 71 | 59 | 471 | 839 | 1,156 | 555 | 3,718 |
| 1997 | 223 | 123 | 86 | 114 | 69 | 42 | 28 | 84 | 208 | 368 | 261 | 234 | 1,840 |
| 1998 | 104 | 59 | 41 | 26 | 26 | 19 | 18 | 41 | 214 | 407 | 740 | 505 | 2,199 |
| 1999 | 274 | 150 | 99 | 77 | 154 | 98 | 63 | 93 | 76 | 341 | 1,422 | 543 | 3,390 |
| 2000 | 212 | 125 | 83 | 66 | 84 | 57 | 64 | 47 | 186 | 443 | 471 | 323 | 2,160 |
| Ave(mil m3) | 163 | 91 | 63 | 44 | 54 | 64 | 58 | 88 | 203 | 476 | 530 | 331 | 2,165 |

Cam Lo + Others ( $\mathrm{A}=790 \mathrm{~km}$ )

|  | Jan | Feb | Mar | Apr | May | Jun | Jul | Aug | Sep | Oct | Nov | Dec | total |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| 1977 | 156 | 88 | 61 | 39 | 28 | 16 | 13 | 22 | 61 | 185 | 232 | 103 | 1,003 |
| 1978 | 67 | 36 | 28 | 19 | 47 | 35 | 44 | 129 | 265 | 207 | 206 | 167 | 1,251 |
| 1979 | 78 | 43 | 30 | 18 | 16 | 57 | 55 | 119 | 168 | 164 | 137 | 92 | 978 |
| 1980 | 51 | 29 | 19 | 12 | 39 | 67 | 68 | 35 | 253 | 499 | 374 | 180 | 1,626 |
| 1981 | 121 | 62 | 43 | 32 | 56 | 97 | 73 | 42 | 68 | 278 | 480 | 222 | 1,575 |
| 1982 | 104 | 58 | 40 | 24 | 20 | 13 | 27 | 13 | 121 | 123 | 143 | 92 | 778 |
| 1983 | 49 | 27 | 19 | 12 | 9 | 23 | 40 | 21 | 18 | 182 | 382 | 122 | 902 |
| 1984 | 73 | 42 | 28 | 18 | 23 | 61 | 43 | 154 | 81 | 250 | 267 | 152 | 1,192 |
| 1985 | 72 | 42 | 29 | 18 | 15 | 52 | 38 | 27 | 74 | 316 | 314 | 210 | 1,208 |
| 1986 | 93 | 52 | 35 | 22 | 37 | 23 | 14 | 34 | 34 | 208 | 182 | 165 | 898 |
| 1987 | 82 | 47 | 32 | 23 | 24 | 16 | 14 | 61 | 154 | 98 | 153 | 102 | 806 |
| 1988 | 52 | 30 | 20 | 12 | 14 | 8 | 5 | 17 | 54 | 272 | 169 | 145 | 798 |
| 1989 | 93 | 45 | 34 | 22 | 80 | 107 | 88 | 104 | 67 | 134 | 108 | 104 | 985 |
| 1990 | 48 | 29 | 23 | 14 | 24 | 46 | 29 | 49 | 248 | 729 | 475 | 185 | 1,902 |
| 1991 | 101 | 57 | 42 | 25 | 18 | 13 | 9 | 37 | 42 | 233 | 156 | 190 | 923 |
| 1992 | 83 | 47 | 32 | 19 | 15 | 16 | 43 | 106 | 96 | 329 | 336 | 160 | 1,281 |
| 1993 | 94 | 50 | 36 | 22 | 20 | 15 | 9 | 8 | 43 | 215 | 207 | 492 | 1,210 |
| 1994 | 148 | 81 | 58 | 38 | 25 | 31 | 25 | 20 | 115 | 106 | 138 | 188 | 973 |
| 1995 | 92 | 52 | 35 | 22 | 18 | 12 | 11 | 26 | 160 | 614 | 486 | 222 | 1,749 |
| 1996 | 114 | 68 | 43 | 31 | 28 | 39 | 41 | 34 | 268 | 478 | 659 | 317 | 2,119 |
| 1997 | 127 | 70 | 49 | 65 | 39 | 24 | 16 | 48 | 119 | 210 | 149 | 133 | 1,049 |
| 1998 | 59 | 33 | 23 | 15 | 15 | 11 | 10 | 24 | 122 | 232 | 422 | 288 | 1,254 |
| 1999 | 156 | 85 | 56 | 44 | 88 | 56 | 36 | 53 | 43 | 195 | 811 | 310 | 1,932 |
| 2000 | 121 | 71 | 47 | 38 | 48 | 32 | 37 | 27 | 106 | 252 | 268 | 184 | 1,231 |
| Ave(mil m 3$)$ | 93 | 52 | 36 | 25 | 31 | 36 | 33 | 50 | 116 | 271 | 302 | 188 | 1,234 |

Thach Han River Basin (A=2,550km2)

|  |  |  |  |  |  |  |  |  |  |  | (million m3) |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Jan | Feb | Mar | Apr | May | Jun | Jul | Aug | Sep | Oct | Nov | Dec | total |
| 1977 | 429 | 242 | 167 | 108 | 76 | 45 | 37 | 60 | 197 | 730 | 738 | 287 | 3,117 |
| 1978 | 186 | 98 | 78 | 52 | 130 | 97 | 121 | 411 | 962 | 796 | 620 | 503 | 4,054 |
| 1979 | 215 | 120 | 82 | 50 | 45 | 176 | 153 | 374 | 688 | 451 | 459 | 257 | 3,072 |
| 1980 | 140 | 80 | 53 | 34 | 133 | 196 | 188 | 97 | 1,175 | 1,581 | 1,108 | 599 | 5,382 |
| 1981 | 334 | 171 | 118 | 88 | 155 | 267 | 202 | 116 | 292 | 1,069 | 1,626 | 634 | 5,073 |
| 1982 | 286 | 161 | 110 | 67 | 54 | 36 | 73 | 35 | 402 | 459 | 798 | 252 | 2,734 |
| 1983 | 134 | 75 | 52 | 33 | 24 | 64 | 109 | 57 | 52 | 927 | 1,120 | 339 | 2,987 |
| 1984 | 201 | 116 | 76 | 50 | 64 | 167 | 138 | 432 | 254 | 839 | 918 | 437 | 3,692 |
| 1985 | 200 | 114 | 81 | 50 | 43 | 142 | 106 | 74 | 276 | 1,213 | 1,024 | 616 | 3,937 |
| 1986 | 256 | 142 | 97 | 60 | 101 | 62 | 39 | 93 | 93 | 812 | 603 | 534 | 2,892 |
| 1987 | 247 | 129 | 87 | 64 | 67 | 44 | 39 | 247 | 505 | 356 | 563 | 281 | 2,629 |
| 1988 | 144 | 83 | 55 | 32 | 39 | 21 | 15 | 46 | 222 | 861 | 483 | 467 | 2,468 |
| 1989 | 256 | 123 | 93 | 61 | 263 | 294 | 243 | 308 | 209 | 476 | 363 | 308 | 2,997 |
| 1990 | 134 | 83 | 64 | 40 | 66 | 126 | 81 | 203 | 809 | 2,401 | 1,398 | 521 | 5,927 |
| 1991 | 277 | 157 | 115 | 69 | 50 | 36 | 25 | 107 | 120 | 950 | 468 | 528 | 2,902 |
| 1992 | 229 | 130 | 87 | 52 | 41 | 45 | 119 | 309 | 338 | 1,403 | 949 | 513 | 4,215 |
| 1993 | 258 | 137 | 98 | 61 | 54 | 41 | 25 | 36 | 125 | 816 | 609 | 1,459 | 3,719 |
| 1994 | 407 | 224 | 159 | 105 | 68 | 85 | 68 | 55 | 504 | 377 | 482 | 594 | 3,128 |
| 1995 | 253 | 142 | 96 | 60 | 50 | 34 | 31 | 73 | 567 | 1,993 | 1,485 | 648 | 5,432 |
| 1996 | 315 | 186 | 119 | 85 | 108 | 108 | 112 | 93 | 830 | 1,545 | 1,992 | 964 | 6,456 |
| 1997 | 349 | 194 | 135 | 179 | 108 | 66 | 44 | 132 | 417 | 654 | 416 | 499 | 3,194 |
| 1998 | 163 | 92 | 64 | 40 | 44 | 30 | 28 | 65 | 540 | 720 | 1,470 | 852 | 4,108 |
| 1999 | 442 | 235 | 155 | 121 | 265 | 153 | 98 | 147 | 120 | 755 | 2,582 | 920 | 5,993 |
| 2000 | 336 | 196 | 130 | 104 | 132 | 89 | 101 | 74 | 353 | 824 | 840 | 538 | 3,717 |
| il m3) | 258 | 143 | 99 | 69 | 91 | 101 | 91 | 152 | 419 | 959 | 963 | 565 | 3,909 |

BT-7

Table B. 6 Generated Monthly Discharge Series Huong River Basin

| Huong River at Tuan (Huu Trach + Ta Trach A=1,460km2) |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
|  | Jan | Feb | Mar | Apr | May | Jun | Jul | Aug | Sep | Oct | Nov | Dec | total |
| 1977 | 32 | 41 | 51 | 21 | 12 | 6 | 6 | 27 | 157 | 419 | 510 | 198 | 1,479 |
| 1978 | 191 | 68 | 41 | 27 | 85 | 54 | 40 | 20 | 13 | 141 | 393 | 393 | 1,464 |
| 1979 | 141 | 80 | 41 | 29 | 80 | 322 | 214 | 192 | 209 | 355 | 580 | 252 | 2,494 |
| 1980 | 115 | 57 | 39 | 34 | 25 | 46 | 40 | 33 | 589 | 942 | 1,280 | 531 | 3,730 |
| 1981 | 281 | 122 | 79 | 82 | 170 | 107 | 84 | 49 | 92 | 1,049 | 1,251 | 687 | 4,053 |
| 1982 | 288 | 130 | 84 | 73 | 49 | 57 | 45 | 44 | 301 | 162 | 315 | 241 | 1,789 |
| 1983 | 168 | 68 | 42 | 27 | 19 | 174 | 154 | 103 | 78 | 958 | 1,133 | 416 | 3,338 |
| 1984 | 213 | 128 | 72 | 50 | 93 | 153 | 110 | 155 | 131 | 720 | 936 | 481 | 3,241 |
| 1985 | 229 | 122 | 86 | 70 | 68 | 119 | 69 | 42 | 179 | 576 | 946 | 748 | 3,254 |
| 1986 | 301 | 144 | 138 | 59 | 139 | 74 | 55 | 83 | 60 | 1,029 | 537 | 510 | 3,128 |
| 1987 | 178 | 88 | 63 | 39 | 31 | 38 | 25 | 141 | 296 | 197 | 632 | 408 | 2,137 |
| 1988 | 151 | 92 | 52 | 32 | 37 | 26 | 18 | 28 | 101 | 1,007 | 698 | 467 | 2,709 |
| 1989 | 260 | 117 | 79 | 57 | 310 | 204 | 148 | 121 | 93 | 196 | 201 | 174 | 1,961 |
| 1990 | 112 | 53 | 37 | 25 | 80 | 37 | 28 | 67 | 328 | 1,242 | 1,305 | 526 | 3,840 |
| 1991 | 251 | 151 | 106 | 174 | 107 | 65 | 47 | 64 | 66 | 587 | 369 | 328 | 2,314 |
| 1992 | 210 | 82 | 49 | 31 | 39 | 109 | 86 | 100 | 129 | 1,220 | 699 | 374 | 3,127 |
| 1993 | 185 | 85 | 60 | 40 | 41 | 30 | 29 | 25 | 47 | 783 | 612 | 800 | 2,736 |
| 1994 | 294 | 130 | 91 | 54 | 67 | 65 | 38 | 25 | 234 | 325 | 380 | 643 | 2,347 |
| 1995 | 218 | 152 | 73 | 41 | 37 | 30 | 32 | 31 | 172 | 1,429 | 1,157 | 704 | 4,075 |
| 1996 | 286 | 203 | 102 | 81 | 149 | 102 | 76 | 41 | 474 | 1,255 | 1,457 | 984 | 5,210 |
| 1997 | 421 | 200 | 119 | 126 | 95 | 68 | 53 | 49 | 311 | 490 | 461 | 503 | 2,897 |
| 1998 | 165 | 88 | 54 | 41 | 72 | 66 | 65 | 37 | 326 | 451 | 1,323 | 985 | 3,673 |
| 1999 | 557 | 236 | 172 | 163 | 205 | 127 | 72 | 51 | 93 | 516 | 1,934 | 1,376 | 5,500 |
| 2000 | 513 | 280 | 157 | 146 | 113 | 125 | 93 | 270 | 245 | 1,237 | 1,073 | 794 | 5,046 |
| Ave(mil m 3$)$ | 240 | 121 | 79 | 63 | 88 | 92 | 68 | 75 | 197 | 720 | 841 | 563 | 3,148 |


| Bo River at Co Bi $(\mathbf{A = 7 2 0 k m 2 )}$ |  |  |  |  |  |  |  |  |  |  |  |  |  |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
|  | Jan | Feb | Mar | Apr | May | Jun | Jul | Aug | Sep | Oct | Nov | Dec | total |
| 1977 | 6 | 5 | 5 | 11 | 22 | 10 | 7 | 14 | 64 | 224 | 322 | 118 | 808 |
| 1978 | 76 | 28 | 29 | 20 | 32 | 27 | 30 | 24 | 254 | 196 | 240 | 259 | 1,214 |
| 1979 | 90 | 41 | 26 | 19 | 43 | 116 | 54 | 57 | 107 | 175 | 229 | 135 | 1,093 |
| 1980 | 59 | 27 | 19 | 16 | 48 | 89 | 48 | 35 | 232 | 530 | 579 | 277 | 1,959 |
| 1981 | 167 | 69 | 44 | 63 | 54 | 45 | 34 | 19 | 41 | 451 | 613 | 348 | 1,948 |
| 1982 | 143 | 63 | 41 | 42 | 35 | 23 | 17 | 14 | 147 | 128 | 214 | 90 | 955 |
| 1983 | 50 | 22 | 15 | 12 | 10 | 37 | 43 | 28 | 28 | 376 | 429 | 167 | 1,215 |
| 1984 | 74 | 37 | 30 | 34 | 53 | 84 | 74 | 136 | 73 | 274 | 432 | 210 | 1,512 |
| 1985 | 85 | 43 | 38 | 32 | 48 | 70 | 43 | 22 | 67 | 277 | 480 | 326 | 1,531 |
| 1986 | 130 | 56 | 38 | 43 | 68 | 32 | 20 | 28 | 25 | 366 | 241 | 239 | 1,286 |
| 1987 | 106 | 53 | 51 | 31 | 40 | 24 | 13 | 44 | 172 | 116 | 298 | 148 | 1,094 |
| 1988 | 74 | 39 | 23 | 14 | 31 | 17 | 17 | 13 | 45 | 339 | 249 | 202 | 1,063 |
| 1989 | 116 | 43 | 35 | 22 | 114 | 94 | 95 | 61 | 61 | 115 | 174 | 126 | 1,055 |
| 1990 | 51 | 30 | 19 | 18 | 30 | 17 | 34 | 56 | 159 | 814 | 624 | 284 | 2,136 |
| 1991 | 123 | 79 | 63 | 82 | 66 | 37 | 25 | 24 | 20 | 263 | 194 | 269 | 1,246 |
| 1992 | 109 | 45 | 28 | 18 | 26 | 36 | 42 | 39 | 79 | 538 | 379 | 236 | 1,573 |
| 1993 | 100 | 47 | 32 | 21 | 45 | 28 | 23 | 19 | 54 | 254 | 210 | 382 | 1,214 |
| 1994 | 121 | 58 | 36 | 54 | 54 | 51 | 30 | 21 | 175 | 153 | 204 | 394 | 1,350 |
| 1995 | 137 | 82 | 40 | 36 | 32 | 34 | 24 | 34 | 121 | 761 | 541 | 341 | 2,184 |
| 1996 | 139 | 120 | 54 | 44 | 70 | 111 | 62 | 39 | 245 | 678 | 760 | 552 | 2,874 |
| 1997 | 218 | 101 | 69 | 104 | 59 | 38 | 29 | 21 | 134 | 293 | 211 | 214 | 1,492 |
| 1998 | 67 | 35 | 26 | 33 | 42 | 24 | 21 | 49 | 167 | 232 | 784 | 472 | 1,951 |
| 1999 | 256 | 143 | 95 | 102 | 122 | 71 | 42 | 42 | 40 | 311 | 1,252 | 599 | 3,073 |
| 2000 | 225 | 126 | 70 | 100 | 102 | 87 | 72 | 80 | 75 | 441 | 451 | 344 | 2,172 |
| Ave(mil m 3$)$ | 113 | 58 | 39 | 40 | 52 | 50 | 37 | 38 | 108 | 346 | 421 | 281 | 1,583 |

Huong River Basin ( $\mathbf{A}=\mathbf{3 , 3 0 0} \mathbf{k m}$ 2)

| Huong River Basin ( $\mathbf{A}=\mathbf{3 , 3 0 0 k m 2 \text { ) }}$ |  |  |  |  |  |  |  |  |  |  | (million m3) |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  | Jan | Feb | Mar | Apr | May | Jun | Jul | Aug | Sep | Oct | Nov | Dec | total |
| 1977 | 37 | 69 | 140 | 32 | 34 | 16 | 13 | 41 | 378 | 1,335 | 1,020 | 481 | 3,597 |
| 1978 | 406 | 95 | 70 | 47 | 117 | 81 | 69 | 44 | 1,343 | 954 | 1,187 | 942 | 5,357 |
| 1979 | 247 | 121 | 67 | 49 | 123 | 810 | 268 | 249 | 836 | 661 | 1,232 | 445 | 5,107 |
| 1980 | 174 | 85 | 57 | 50 | 73 | 135 | 88 | 68 | 1,996 | 2,437 | 2,560 | 963 | 8,686 |
| 1981 | 520 | 191 | 123 | 146 | 225 | 151 | 118 | 68 | 296 | 3,047 | 2,592 | 1,407 | 8,882 |
| 1982 | 463 | 193 | 126 | 114 | 84 | 80 | 62 | 57 | 640 | 433 | 1,226 | 417 | 3,895 |
| 1983 | 273 | 90 | 58 | 38 | 29 | 422 | 197 | 131 | 106 | 2,880 | 2,160 | 620 | 7,002 |
| 1984 | 287 | 178 | 102 | 84 | 146 | 237 | 420 | 292 | 295 | 1,607 | 2,305 | 955 | 6,909 |
| 1985 | 329 | 164 | 124 | 102 | 115 | 189 | 111 | 65 | 367 | 1,835 | 2,330 | 1,543 | 7,275 |
| 1986 | 451 | 223 | 176 | 103 | 207 | 106 | 75 | 110 | 413 | 2,199 | 1,152 | 1,119 | 6,335 |
| 1987 | 284 | 141 | 114 | 70 | 71 | 62 | 39 | 184 | 640 | 406 | 1,614 | 636 | 4,260 |
| 1988 | 225 | 131 | 76 | 46 | 68 | 43 | 35 | 42 | 290 | 1,961 | 1,292 | 824 | 5,032 |
| 1989 | 522 | 159 | 115 | 79 | 558 | 298 | 243 | 181 | 154 | 544 | 624 | 329 | 3,807 |
| 1990 | 176 | 83 | 55 | 43 | 111 | 54 | 62 | 184 | 771 | 3,122 | 2,502 | 855 | 8,016 |
| 1991 | 396 | 288 | 169 | 302 | 173 | 103 | 73 | 88 | 86 | 1,586 | 635 | 798 | 4,696 |
| 1992 | 401 | 126 | 77 | 48 | 65 | 195 | 128 | 139 | 312 | 3,024 | 1,314 | 824 | 6,653 |
| 1993 | 284 | 132 | 91 | 62 | 86 | 59 | 52 | 44 | 101 | 1,846 | 1,293 | 1,551 | 5,600 |
| 1994 | 441 | 188 | 127 | 108 | 121 | 116 | 68 | 47 | 589 | 779 | 965 | 1,596 | 5,145 |
| 1995 | 354 | 234 | 113 | 77 | 69 | 64 | 56 | 65 | 576 | 3,209 | 2,470 | 1,341 | 8,627 |
| 1996 | 425 | 357 | 156 | 125 | 219 | 213 | 137 | 80 | 1,496 | 2,676 | 3,057 | 1,875 | 10,815 |
| 1997 | 745 | 315 | 189 | 231 | 154 | 106 | 81 | 70 | 882 | 1,490 | 910 | 1,182 | 6,355 |
| 1998 | 245 | 123 | 80 | 74 | 192 | 91 | 86 | 86 | 1,144 | 897 | 3,433 | 1,913 | 8,363 |
| 1999 | 1,057 | 437 | 424 | 274 | 327 | 198 | 114 | 93 | 147 | 1,129 | 4,176 | 2,792 | 11,166 |
| 2000 | 1,032 | 406 | 226 | 246 | 215 | 212 | 165 | 532 | 501 | 2,647 | 2,053 | 1,730 | 9,967 |
| Ave(mil m3) | 407 | 189 | 127 | 106 | 149 | 168 | 115 | 123 | 598 | 1,779 | 1,838 | 1,131 | 6,731 |

Table B. 7 Generated Monthly Discharge Series Vu Gia - Thu Bon River Basin
Thu Bon River at Ai Nghia ( $\mathrm{A}=3,590 \mathrm{~km} 2$ )

|  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
|  | Jan | Feb | Mar | Apr | May | Jun | Jul | Aug | Sep | Oct | Nov | Dec | total |
| 1984 | 555 | 356 | 211 | 177 | 298 | 384 | 181 | 126 | 160 | 1,555 | 2,581 | 1,858 | 8,439 |
| 1985 | 664 | 332 | 217 | 194 | 345 | 229 | 139 | 138 | 313 | 1,122 | 3,497 | 2,473 | 9,663 |
| 1986 | 782 | 382 | 273 | 171 | 440 | 237 | 139 | 177 | 132 | 2,467 | 1,475 | 3,351 | 10,026 |
| 1987 | 764 | 426 | 453 | 233 | 185 | 282 | 146 | 167 | 697 | 261 | 2,331 | 850 | 6,796 |
| 1988 | 592 | 358 | 279 | 197 | 313 | 284 | 262 | 145 | 349 | 2,814 | 1,819 | 1,160 | 8,571 |
| 1989 | 1,161 | 443 | 351 | 196 | 596 | 399 | 331 | 368 | 483 | 595 | 1,239 | 847 | 7,009 |
| 1990 | 436 | 300 | 257 | 182 | 299 | 229 | 200 | 231 | 328 | 3,845 | 3,498 | 1,096 | 10,900 |
| 1991 | 545 | 420 | 435 | 505 | 418 | 221 | 222 | 243 | 305 | 1,957 | 1,419 | 1,880 | 8,570 |
| 1992 | 741 | 338 | 215 | 145 | 165 | 210 | 136 | 325 | 412 | 3,388 | 2,391 | 1,362 | 9,829 |
| 1993 | 605 | 289 | 235 | 188 | 203 | 192 | 187 | 104 | 192 | 1,880 | 1,719 | 3,337 | 9,131 |
| 1994 | 654 | 296 | 289 | 183 | 210 | 194 | 123 | 135 | 779 | 1,269 | 1,373 | 1,541 | 7,045 |
| 1995 | 555 | 360 | 225 | 127 | 124 | 158 | 203 | 277 | 497 | 3,443 | 3,784 | 2,349 | 12,103 |
| 1996 | 779 | 579 | 315 | 234 | 594 | 428 | 272 | 167 | 510 | 4,207 | 6,529 | 3,313 | 17,930 |
| 1997 | 748 | 352 | 234 | 229 | 339 | 271 | 274 | 144 | 1,571 | 943 | 2,117 | 876 | 8,097 |
| 1998 | 331 | 190 | 126 | 85 | 126 | 81 | 106 | 96 | 486 | 1,157 | 6,053 | 2,992 | 11,830 |
| 1999 | 1,476 | 735 | 565 | 536 | 767 | 599 | 322 | 246 | 284 | 1,668 | 5,501 | 4,784 | 17,482 |
| 2000 | 1,043 | 708 | 366 | 445 | 623 | 485 | 392 | 685 | 362 | 3,036 | 4,525 | 2,697 | 15,367 |
| Ave(mil m3) | 731 | 404 | 297 | 237 | 356 | 287 | 214 | 222 | 462 | 2,095 | 3,050 | 2,163 | 10,517 |

Vu Gia River at Ai Nghia ( $\mathrm{A}=\mathbf{5}, \mathbf{4 2 0 \mathrm { km } 2 \text { ) }}$

|  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
|  | Jan | Feb | Mar | Apr | May | Jun | Jul | Aug | Sep | Oct | Nov | Dec | total |
| 1984 | 617 | 437 | 311 | 295 | 339 | 494 | 256 | 310 | 348 | 2,020 | 2,726 | 1,534 | 9,687 |
| 1985 | 740 | 408 | 300 | 293 | 395 | 655 | 331 | 259 | 626 | 1,292 | 2,264 | 2,061 | 9,622 |
| 1986 | 803 | 407 | 314 | 238 | 448 | 281 | 254 | 304 | 220 | 2,261 | 1,452 | 2,451 | 9,434 |
| 1987 | 799 | 470 | 386 | 257 | 230 | 267 | 187 | 391 | 774 | 390 | 1,773 | 915 | 6,839 |
| 1988 | 637 | 428 | 346 | 252 | 353 | 348 | 408 | 263 | 411 | 3,301 | 2,074 | 1,390 | 10,210 |
| 1989 | 1,119 | 466 | 385 | 248 | 692 | 454 | 452 | 412 | 491 | 598 | 898 | 731 | 6,946 |
| 1990 | 375 | 265 | 243 | 185 | 432 | 269 | 237 | 340 | 640 | 5,223 | 4,166 | 1,489 | 13,866 |
| 1991 | 668 | 431 | 393 | 376 | 378 | 319 | 328 | 392 | 395 | 1,777 | 1,153 | 1,567 | 8,176 |
| 1992 | 701 | 368 | 284 | 201 | 279 | 328 | 254 | 670 | 605 | 3,651 | 2,649 | 1,310 | 11,300 |
| 1993 | 719 | 390 | 321 | 254 | 285 | 244 | 274 | 216 | 375 | 1,305 | 1,409 | 3,232 | 9,026 |
| 1994 | 830 | 400 | 358 | 321 | 334 | 284 | 329 | 345 | 1,099 | 1,074 | 1,578 | 1,560 | 8,514 |
| 1995 | 645 | 444 | 303 | 204 | 237 | 194 | 268 | 318 | 611 | 3,687 | 3,980 | 2,272 | 13,163 |
| 1996 | 976 | 666 | 398 | 334 | 593 | 610 | 447 | 382 | 1,045 | 4,582 | 6,533 | 4,342 | 20,908 |
| 1997 | 1,302 | 667 | 497 | 475 | 530 | 310 | 307 | 283 | 1,683 | 1,084 | 1,639 | 784 | 9,561 |
| 1998 | 404 | 266 | 209 | 172 | 218 | 152 | 164 | 212 | 546 | 1,239 | 6,298 | 2,972 | 12,851 |
| 1999 | 1,611 | 853 | 727 | 618 | 1,032 | 1,052 | 642 | 592 | 476 | 2,065 | 5,678 | 4,192 | 19,537 |
| 2000 | 1,277 | 919 | 488 | 656 | 887 | 740 | 791 | 1,299 | 810 | 3,281 | 4,792 | 3,246 | 19,185 |
| Ave(mil m3) | 837 | 487 | 368 | 316 | 451 | 412 | 349 | 411 | 656 | 2,284 | 3,004 | 2,121 | 11,696 |

Monthly Runoff Thach Vu Gia -Thu Bon River Basin 10,380km2

|  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
|  | Jan | Feb | Mar | Apr | May | Jun | Jul | Aug | Sep | Oct | Nov | Dec | total |
| 1984 | 1,172 | 793 | 522 | 472 | 636 | 878 | 437 | 435 | 560 | 4,509 | 6,024 | 3,550 | 19,989 |
| 1985 | 1,403 | 740 | 516 | 488 | 740 | 992 | 470 | 397 | 971 | 3,057 | 6,400 | 4,845 | 21,021 |
| 1986 | 1,585 | 789 | 587 | 409 | 995 | 519 | 393 | 558 | 352 | 5,902 | 3,075 | 6,338 | 21,501 |
| 1987 | 1,562 | 897 | 840 | 490 | 415 | 550 | 332 | 616 | 1,671 | 810 | 4,965 | 1,768 | 14,917 |
| 1988 | 1,228 | 786 | 625 | 449 | 666 | 632 | 669 | 408 | 986 | 7,117 | 3,998 | 2,725 | 20,289 |
| 1989 | 2,289 | 909 | 736 | 445 | 1,689 | 854 | 783 | 817 | 974 | 1,438 | 2,292 | 1,726 | 14,951 |
| 1990 | 811 | 565 | 500 | 366 | 732 | 497 | 444 | 597 | 1,223 | 9,763 | 8,217 | 2,585 | 26,302 |
| 1991 | 1,213 | 851 | 827 | 1,111 | 796 | 540 | 550 | 754 | 761 | 4,603 | 2,667 | 3,513 | 18,186 |
| 1992 | 1,453 | 706 | 500 | 346 | 444 | 589 | 390 | 994 | 1,067 | 8,748 | 5,215 | 2,738 | 23,189 |
| 1993 | 1,323 | 679 | 556 | 442 | 489 | 437 | 461 | 320 | 604 | 4,295 | 3,284 | 6,853 | 19,743 |
| 1994 | 1,485 | 696 | 647 | 504 | 545 | 484 | 452 | 480 | 2,407 | 2,965 | 3,211 | 3,434 | 17,307 |
| 1995 | 1,201 | 804 | 528 | 331 | 360 | 352 | 471 | 595 | 1,671 | 8,433 | 8,814 | 4,657 | 28,217 |
| 1996 | 1,756 | 1,245 | 713 | 568 | 1,187 | 1,038 | 720 | 550 | 2,067 | 9,623 | 14,013 | 7,771 | 41,251 |
| 1997 | 2,058 | 1,019 | 731 | 704 | 870 | 581 | 581 | 427 | 3,903 | 2,674 | 4,163 | 1,807 | 19,517 |
| 1998 | 779 | 455 | 335 | 257 | 344 | 233 | 270 | 308 | 1,639 | 2,794 | 13,080 | 6,162 | 26,656 |
| 1999 | 3,301 | 1,599 | 1,292 | 1,154 | 1,798 | 1,651 | 963 | 838 | 793 | 4,618 | 12,921 | 10,127 | 41,053 |
| 2000 | 2,517 | 1,627 | 854 | 1,102 | 1,509 | 1,225 | 1,182 | 2,203 | 1,207 | 7,379 | 9,762 | 6,489 | 37,056 |
| Ave(mil m3) | 1,596 | 892 | 665 | 567 | 836 | 709 | 563 | 665 | 1,344 | 5,219 | 6,594 | 4,535 | 24,185 |

Table B. 8 Generated Monthly Discharge Series Tra Khuc River Basin
TraKhuc River at the apex of delta ( $\mathrm{A}=3,030 \mathrm{~km} 2$ )

| (million m3) |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Jan | Feb | Mar | Apr | May | Jun | Jul | Aug | Sep | Oct | Nov | Dec | total |
| 1976 | 463 | 267 | 168 | 111 | 145 | 113 | 127 | 93 | 124 | 1,057 | 2,088 | 618 | 5,374 |
| 1977 | 493 | 297 | 270 | 146 | 114 | 68 | 77 | 119 | 249 | 624 | 2,169 | 537 | 5,162 |
| 1978 | 798 | 245 | 235 | 151 | 420 | 212 | 284 | 187 | 687 | 833 | 1,620 | 1,355 | 7,026 |
| 1979 | 424 | 209 | 139 | 106 | 232 | 660 | 249 | 199 | 159 | 888 | 1,504 | 803 | 5,572 |
| 1980 | 358 | 228 | 146 | 119 | 199 | 319 | 258 | 258 | 698 | 1,840 | 2,937 | 661 | 8,022 |
| 1981 | 440 | 283 | 182 | 145 | 281 | 305 | 228 | 141 | 219 | 2,902 | 3,601 | 1,957 | 10,682 |
| 1982 | 493 | 239 | 153 | 160 | 107 | 188 | 127 | 107 | 493 | 329 | 572 | 286 | 3,253 |
| 1983 | 244 | 128 | 97 | 62 | 89 | 147 | 125 | 245 | 199 | 1,539 | 2,560 | 731 | 6,166 |
| 1984 | 420 | 269 | 158 | 132 | 224 | 290 | 136 | 93 | 119 | 1,184 | 1,966 | 1,415 | 6,406 |
| 1985 | 503 | 251 | 162 | 146 | 261 | 172 | 104 | 102 | 236 | 853 | 2,665 | 1,884 | 7,340 |
| 1986 | 594 | 289 | 206 | 128 | 333 | 178 | 103 | 132 | 98 | 1,879 | 1,122 | 2,554 | 7,617 |
| 1987 | 498 | 268 | 253 | 157 | 138 | 201 | 134 | 138 | 371 | 189 | 2,079 | 729 | 5,153 |
| 1988 | 504 | 301 | 224 | 178 | 255 | 257 | 329 | 167 | 304 | 2,096 | 1,624 | 876 | 7,116 |
| 1989 | 700 | 293 | 289 | 177 | 311 | 314 | 318 | 313 | 452 | 386 | 718 | 520 | 4,792 |
| 1990 | 322 | 206 | 171 | 120 | 192 | 200 | 204 | 186 | 215 | 2,540 | 2,198 | 817 | 7,371 |
| 1991 | 420 | 297 | 262 | 269 | 242 | 205 | 168 | 226 | 276 | 1,323 | 999 | 1,256 | 5,943 |
| 1992 | 553 | 284 | 183 | 118 | 134 | 222 | 119 | 294 | 351 | 2,892 | 1,716 | 807 | 7,673 |
| 1993 | 427 | 239 | 206 | 148 | 171 | 180 | 170 | 95 | 212 | 1,363 | 1,503 | 2,642 | 7,355 |
| 1994 | 556 | 247 | 216 | 165 | 189 | 163 | 123 | 113 | 654 | 855 | 861 | 876 | 5,017 |
| 1995 | 371 | 279 | 168 | 105 | 99 | 97 | 121 | 157 | 342 | 2,537 | 2,912 | 1,590 | 8,776 |
| 1996 | 635 | 412 | 231 | 164 | 385 | 352 | 267 | 168 | 354 | 2,482 | 5,767 | 2,876 | 14,094 |
| 1997 | 572 | 261 | 177 | 153 | 263 | 205 | 209 | 134 | 966 | 568 | 1,477 | 679 | 5,664 |
| 1998 | 284 | 167 | 125 | 89 | 100 | 85 | 115 | 120 | 212 | 1,199 | 4,820 | 2,631 | 9,944 |
| 1999 | 1,160 | 550 | 427 | 348 | 662 | 490 | 299 | 250 | 335 | 1,891 | 3,616 | 4,187 | 14,213 |
| 2000 | 778 | 523 | 270 | 300 | 415 | 471 | 369 | 607 | 402 | 1,695 | 3,431 | 2,115 | 11,376 |
| Ave(mil m3) | 520 | 281 | 205 | 156 | 238 | 244 | 190 | 186 | 349 | 1,438 | 2,261 | 1,416 | 7,484 |

Monthly Discharge Ve River at the apex of delta ( $A=820 \mathrm{~km} 2$ )

|  | Jan | Feb | Mar | Apr | May | Jun | Jul | Aug | Sep | Oct | Nov | Dec | total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1976 | 118 | 62 | 32 | 16 | 25 | 17 | 20 | 10 | 20 | 290 | 590 | 163 | 1,362 |
| 1977 | 126 | 71 | 62 | 26 | 16 | 3 | 5 | 18 | 56 | 164 | 614 | 139 | 1,301 |
| 1978 | 215 | 56 | 51 | 27 | 105 | 45 | 66 | 37 | 183 | 225 | 454 | 377 | 1,843 |
| 1979 | 106 | 46 | 23 | 14 | 51 | 176 | 55 | 41 | 30 | 241 | 421 | 216 | 1,420 |
| 1980 | 87 | 50 | 26 | 18 | 41 | 76 | 58 | 58 | 187 | 518 | 837 | 175 | 2,132 |
| 1981 | 111 | 67 | 36 | 26 | 65 | 72 | 49 | 24 | 47 | 826 | 1,030 | 552 | 2,905 |
| 1982 | 126 | 54 | 28 | 30 | 14 | 38 | 20 | 14 | 127 | 79 | 150 | 66 | 747 |
| 1983 | 54 | 22 | 11 | 2 | 9 | 26 | 19 | 54 | 41 | 430 | 728 | 196 | 1,593 |
| 1984 | 105 | 62 | 29 | 22 | 48 | 68 | 23 | 10 | 18 | 327 | 555 | 394 | 1,662 |
| 1985 | 129 | 58 | 30 | 26 | 59 | 34 | 13 | 13 | 52 | 231 | 758 | 530 | 1,934 |
| 1986 | 156 | 69 | 43 | 21 | 80 | 35 | 13 | 21 | 12 | 529 | 310 | 725 | 2,014 |
| 1987 | 103 | 65 | 77 | 32 | 19 | 44 | 26 | 14 | 76 | 36 | 956 | 241 | 1,689 |
| 1988 | 133 | 62 | 41 | 34 | 30 | 36 | 21 | 8 | 51 | 402 | 309 | 128 | 1,256 |
| 1989 | 200 | 59 | 110 | 52 | 57 | 48 | 57 | 67 | 138 | 102 | 204 | 117 | 1,211 |
| 1990 | 72 | 58 | 33 | 23 | 52 | 58 | 27 | 27 | 51 | 674 | 519 | 219 | 1,812 |
| 1991 | 100 | 64 | 80 | 68 | 51 | 36 | 26 | 26 | 31 | 261 | 229 | 341 | 1,314 |
| 1992 | 136 | 69 | 55 | 35 | 17 | 39 | 18 | 71 | 56 | 857 | 506 | 215 | 2,072 |
| 1993 | 102 | 54 | 33 | 13 | 18 | 26 | 11 | 7 | 25 | 447 | 536 | 791 | 2,063 |
| 1994 | 123 | 63 | 103 | 68 | 21 | 26 | 20 | 16 | 130 | 182 | 227 | 227 | 1,204 |
| 1995 | 93 | 42 | 30 | 13 | 32 | 15 | 12 | 13 | 86 | 798 | 791 | 478 | 2,404 |
| 1996 | 154 | 115 | 50 | 34 | 75 | 46 | 36 | 25 | 73 | 671 | 1,563 | 932 | 3,773 |
| 1997 | 157 | 68 | 40 | 44 | 61 | 51 | 74 | 46 | 209 | 132 | 519 | 278 | 1,678 |
| 1998 | 94 | 43 | 24 | 23 | 37 | 24 | 28 | 25 | 61 | 465 | 1,250 | 1,008 | 3,081 |
| 1999 | 384 | 161 | 115 | 87 | 104 | 79 | 56 | 60 | 76 | 494 | 873 | 1,728 | 4,217 |
| 2000 | 398 | 157 | 60 | 47 | 67 | 72 | 43 | 89 | 54 | 299 | 837 | 600 | 2,723 |
| Ave(mil m3) | 143 | 68 | 49 | 32 | 46 | 48 | 32 | 32 | 76 | 387 | 631 | 433 | 1,976 |

Table B. 8 Generated Monthly Discharge Series Tra Khuc River Basin
Monthly Discharge Other small basins ( $\mathrm{A}=\mathbf{5 1 0 \mathrm { km } 2 \text { ) } ) ~}$


Monthly Runoff Tra Khuc Basin ( $\mathrm{A}=\mathbf{5 , 2 0 0 \mathrm { km } 2 \text { ) } ) ~}$


Table B. 9 Generated Monthly Discharge Series Kone River Basin
Kone River without Lower Area (Delta) ( $\mathrm{A}=\mathbf{3 , 0 0 0 \mathrm { km } 2 \text { ) }}$

|  | Jan | Feb | Mar | Apr | May | Jun | Jul | Aug | Sep | Oct | Nov | Dec | total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1976 | 163 | 126 | 86 | 64 | 84 | 70 | 90 | 83 | 186 | 528 | 837 | 251 | 2,568 |
| 1977 | 165 | 112 | 103 | 50 | 37 | 25 | 29 | 43 | 454 | 271 | 1,526 | 181 | 2,995 |
| 1978 |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1979 | 168 | 81 | 59 | 47 | 65 | 206 | 79 | 43 | 83 | 600 | 785 | 330 | 2,546 |
| 1980 | 147 | 105 | 78 | 61 | 134 | 173 | 117 | 204 | 426 | 1,451 | 2,533 | 424 | 5,853 |
| 1981 | 208 | 116 | 65 | 58 | 106 | 156 | 121 | 71 | 63 | 1,853 | 2,344 | 1,111 | 6,271 |
| 1982 | 310 | 187 | 143 | 139 | 93 | 136 | 75 | 54 | 112 | 122 | 141 | 78 | 1,590 |
| 1983 | 57 | 30 | 23 | 13 | 30 | 42 | 41 | 196 | 124 | 1,030 | 1,380 | 367 | 3,334 |
| 1984 | 225 | 148 | 91 | 84 | 117 | 202 | 71 | 41 | 49 | 585 | 1,615 | 939 | 4,167 |
| 1985 | 261 | 141 | 88 | 71 | 99 | 66 | 49 | 27 | 85 | 486 | 1,140 | 803 | 3,315 |
| 1986 | 230 | 147 | 112 | 64 | 132 | 63 | 50 | 104 | 68 | 706 | 444 | 1,413 | 3,534 |
| 1987 | 255 | 117 | 102 | 56 | 48 | 67 | 33 | 45 | 127 | 61 | 1,756 | 368 | 3,034 |
| 1988 | 192 | 109 | 80 | 54 | 50 | 50 | 84 | 35 | 99 | 1,232 | 821 | 300 | 3,107 |
| 1989 | 211 | 107 | 110 | 71 | 101 | 109 | 177 | 158 | 454 | 267 | 293 | 195 | 2,252 |
| 1990 | 102 | 59 | 45 | 37 | 133 | 280 | 118 | 108 | 96 | 1,698 | 1,372 | 367 | 4,416 |
| 1991 | 180 | 122 | 102 | 95 | 70 | 67 | 74 | 53 | 95 | 771 | 407 | 406 | 2,442 |
| 1992 | 198 | 102 | 71 | 55 | 50 | 92 | 53 | 88 | 91 | 1,790 | 791 | 266 | 3,647 |
| 1993 | 160 | 79 | 62 | 44 | 57 | 51 | 46 | 30 | 81 | 704 | 689 | 1,317 | 3,318 |
| 1994 | 269 | 116 | 99 | 68 | 95 | 96 | 68 | 74 | 294 | 596 | 274 | 349 | 2,400 |
| 1995 | 172 | 132 | 85 | 60 | 72 | 96 | 108 | 107 | 236 | 1,153 | 1,354 | 682 | 4,255 |
| 1996 | 325 | 197 | 121 | 96 | 192 | 154 | 114 | 68 | 163 | 683 | 2,856 | 1,890 | 6,859 |
| 1997 | 361 | 170 | 128 | 111 | 149 | 113 | 86 | 50 | 124 | 159 | 670 | 231 | 2,351 |
| 1998 | 113 | 64 | 46 | 60 | 72 | 57 | 60 | 76 | 147 | 895 | 2,747 | 1,726 | 6,063 |
| 1999 | 574 | 268 | 222 | 163 | 243 | 254 | 202 | 120 | 134 | 1,015 | 1,897 | 2,578 | 7,671 |
| 2000 | 420 | 272 | 153 | 141 | 156 | 207 | 165 | 252 | 161 | 701 | 1,458 | 872 | 4,959 |
| Ave(mil m3) | 228 | 129 | 95 | 73 | 99 | 118 | 88 | 89 | 165 | 807 | 1,255 | 727 | 3,873 |

Kone Basin (A=3,640km2)


Table B. 10 Generated Monthly Discharge Series Ba River Basin
Ba River without Lower Area (Delta) $(\mathbf{A}=\mathbf{1 3 , 5 9 0} \mathbf{k m} 2)$

|  | Jan | Feb | Mar | Apr | May | Jun | Jul | Aug | Sep | Oct | Nov | Dec |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| 1977 | 321 | 179 | 146 | 83 | 94 | 72 | 146 | 185 | 2,048 | 841 | 2,189 | 395 |
| 1978 | 299 | 156 | 113 | 96 | 195 | 142 | 537 | 780 | 1,238 | 1,271 | 2,041 | 673 |
| 1979 | 328 | 182 | 132 | 114 | 175 | 1,044 | 983 | 1,474 | 599 | 2,195 | 1,883 | 739 |
| 1980 | 310 | 197 | 120 | 70 | 759 | 765 | 522 | 477 | 1,339 | 1,842 | 4,075 | 815 |
| 1981 | 395 | 229 | 119 | 145 | 215 | 719 | 365 | 893 | 734 | 4,178 | 6,074 | 2,466 |
| 1,292 |  |  |  |  |  |  |  |  |  |  |  |  |
| 1982 | 591 | 310 | 220 | 218 | 152 | 418 | 530 | 297 | 815 | 468 | 414 | 200 |
| 1983 | 146 | 81 | 53 | 30 | 110 | 306 | 160 | 662 | 530 | 3,883 | 2,256 | 828 |
| 9,045 |  |  |  |  |  |  |  |  |  |  |  |  |
| 1984 | 393 | 268 | 166 | 162 | 323 | 511 | 343 | 1,315 | 1,080 | 2,094 | 3,173 | 1,401 |
| 1985 | 636 | 255 | 180 | 231 | 221 | 365 | 502 | 516 | 962 | 1,265 | 1,804 | 1,242 |
| 1986 | 391 | 220 | 162 | 88 | 281 | 129 | 202 | 1,423 | 1,186 | 1,405 | 1,131 | 3,313 |
| 1987 | 625 | 297 | 229 | 142 | 124 | 197 | 186 | 468 | 503 | 243 | 2,933 | 833 |
| 1988 | 415 | 232 | 155 | 98 | 124 | 361 | 308 | 205 | 581 | 4,171 | 3,342 | 680 |
| 1989 | 709 | 297 | 192 | 144 | 581 | 413 | 580 | 951 | 1,251 | 1,605 | 990 | 503 |
| 10,673 |  |  |  |  |  |  |  |  |  |  |  |  |
| 1,216 |  |  |  |  |  |  |  |  |  |  |  |  |
| Ave(mil m3) | 428 | 223 | 153 | 125 | 258 | 419 | 413 | 742 | 990 | 1,959 | 2,485 | 1,084 |

Ba Basin $(A=14,030 \mathrm{~km} 2)$


Table B. 11 Generated Monthly Discharge Series Sesan River Basin
Sesan River at the International Boundry ( $\mathrm{A}=\mathbf{1 1 , 5 3 0 \mathrm { km } 2 \text { ) } ) ~ ( 1 )}$

| (million m3) |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Jan | Feb | Mar | Apr | May | Jun | Jul | Aug | Sep | Oct | Nov | Dec | total |
| 1976 | 440 | 281 | 252 | 227 | 376 | 621 | 1,065 | 1,932 | 2,113 | 1,350 | 858 | 699 | 10,215 |
| 1977 | 408 | 301 | 282 | 267 | 274 | 302 | 576 | 971 | 1,751 | 964 | 1,000 | 463 | 7,558 |
| 1978 | 340 | 198 | 238 | 242 | 319 | 529 | 912 | 2,412 | 2,587 | 1,708 | 1,109 | 783 | 11,378 |
| 1979 | 538 | 352 | 289 | 282 | 601 | 1,613 | 2,169 | 3,810 | 2,143 | 2,178 | 1,315 | 847 | 16,137 |
| 1980 | 614 | 421 | 345 | 314 | 740 | 1,005 | 1,477 | 1,332 | 2,335 | 2,391 | 2,539 | 1,124 | 14,638 |
| 1981 | 701 | 452 | 442 | 437 | 558 | 2,051 | 1,444 | 2,534 | 1,317 | 2,544 | 2,401 | 1,355 | 16,236 |
| 1982 | 710 | 494 | 443 | 436 | 424 | 1,080 | 1,872 | 1,614 | 2,661 | 1,311 | 876 | 619 | 12,541 |
| 1983 | 499 | 382 | 331 | 249 | 376 | 587 | 845 | 1,679 | 1,227 | 2,521 | 1,653 | 837 | 11,187 |
| 1984 | 659 | 465 | 437 | 532 | 584 | 1,413 | 954 | 2,991 | 2,725 | 2,002 | 1,725 | 1,000 | 15,486 |
| 1985 | 693 | 478 | 454 | 471 | 515 | 1,507 | 1,346 | 2,606 | 2,027 | 1,487 | 1,047 | 766 | 13,398 |
| 1986 | 662 | 511 | 513 | 476 | 1,192 | 835 | 1,477 | 2,199 | 2,471 | 2,013 | 1,278 | 1,195 | 14,822 |
| 1987 | 656 | 478 | 413 | 372 | 427 | 623 | 1,116 | 1,784 | 1,727 | 912 | 906 | 596 | 10,012 |
| 1988 | 489 | 386 | 373 | 332 | 610 | 1,003 | 922 | 1,406 | 847 | 2,408 | 1,137 | 781 | 10,694 |
| 1989 | 619 | 460 | 486 | 466 | 968 | 972 | 1,528 | 2,665 | 2,428 | 1,592 | 923 | 716 | 13,824 |
| 1990 | 450 | 374 | 330 | 309 | 564 | 976 | 949 | 1,404 | 1,776 | 2,692 | 1,691 | 885 | 12,402 |
| 1991 | 660 | 450 | 431 | 333 | 366 | 679 | 1,236 | 2,643 | 2,469 | 2,527 | 1,149 | 862 | 13,804 |
| 1992 | 627 | 465 | 449 | 419 | 541 | 723 | 911 | 2,121 | 2,017 | 2,073 | 1,439 | 829 | 12,614 |
| 1993 | 544 | 402 | 389 | 375 | 463 | 426 | 741 | 1,868 | 1,664 | 1,624 | 1,031 | 1,127 | 10,656 |
| 1994 | 450 | 310 | 272 | 303 | 416 | 677 | 2,540 | 2,337 | 4,217 | 1,797 | 1,015 | 922 | 15,255 |
| 1995 | 528 | 372 | 320 | 246 | 338 | 451 | 907 | 1,397 | 1,377 | 1,574 | 1,657 | 893 | 10,059 |
| 1996 | 632 | 479 | 403 | 429 | 615 | 686 | 1,433 | 2,276 | 3,590 | 2,340 | 3,877 | 2,114 | 18,874 |
| 1997 | 1,018 | 700 | 600 | 634 | 812 | 753 | 1,385 | 2,563 | 2,466 | 2,151 | 1,226 | 768 | 15,077 |
| Ave(mil m3) | 588 | 419 | 386 | 371 | 549 | 887 | 1,264 | 2,116 | 2,179 | 1,916 | 1,448 | 917 | 13,039 |

Table B. 12 Generated Monthly Discharge Series Srepok River Basin
Srepok River at the International Boundry ( $\mathrm{A}=\mathbf{1 2 , 0 3 0 \mathrm { km } 2 \text { ) }}$

|  | Jan | Feb | Mar | Apr | May | Jun | Jul | Aug | Sep | Oct | Nov | Dec | total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1977 | 283 | 180 | 146 | 105 | 120 | 138 | 432 | 420 | 1,499 | 1,158 | 654 | 337 | 5,470 |
| 1978 | 229 | 146 | 135 | 125 | 302 | 324 | 768 | 1,135 | 1,709 | 1,960 | 1,341 | 621 | 8,795 |
| 1979 | 341 | 198 | 171 | 164 | 270 | 665 | 1,088 | 1,395 | 777 | 1,394 | 1,027 | 798 | 8,287 |
| 1980 | 364 | 243 | 189 | 164 | 364 | 576 | 600 | 761 | 1,893 | 1,878 | 1,930 | 848 | 9,810 |
| 1981 | 432 | 282 | 210 | 196 | 372 | 890 | 695 | 1,795 | 1,046 | 2,617 | 2,723 | 1,550 | 12,807 |
| 1982 | 693 | 387 | 328 | 323 | 365 | 733 | 829 | 696 | 1,895 | 1,107 | 638 | 407 | 8,401 |
| 1983 | 273 | 175 | 139 | 101 | 184 | 307 | 344 | 851 | 794 | 2,147 | 1,227 | 570 | 7,112 |
| 1984 | 350 | 227 | 158 | 181 | 397 | 644 | 695 | 1,532 | 1,601 | 1,978 | 1,011 | 791 | 9,565 |
| 1985 | 394 | 237 | 202 | 299 | 441 | 490 | 723 | 782 | 1,139 | 1,156 | 634 | 813 | 7,308 |
| 1986 | 356 | 217 | 179 | 138 | 226 | 274 | 540 | 1,472 | 1,167 | 1,273 | 733 | 1,093 | 7,669 |
| 1987 | 437 | 240 | 207 | 149 | 194 | 465 | 1,012 | 828 | 1,552 | 990 | 781 | 701 | 7,556 |
| 1988 | 367 | 232 | 166 | 169 | 397 | 502 | 593 | 722 | 1,002 | 2,225 | 1,767 | 784 | 8,925 |
| 1989 | 418 | 230 | 229 | 236 | 550 | 762 | 950 | 1,350 | 1,676 | 1,533 | 718 | 436 | 9,088 |
| 1990 | 269 | 156 | 131 | 148 | 233 | 1,382 | 1,032 | 1,636 | 2,251 | 1,615 | 1,870 | 985 | 11,708 |
| 1991 | 458 | 252 | 242 | 204 | 276 | 352 | 503 | 736 | 1,156 | 1,370 | 554 | 341 | 6,446 |
| 1992 | 226 | 124 | 100 | 148 | 292 | 894 | 964 | 1,870 | 1,809 | 2,121 | 1,895 | 686 | 11,130 |
| 1993 | 367 | 224 | 208 | 173 | 324 | 424 | 710 | 839 | 1,068 | 3,262 | 981 | 2,204 | 10,785 |
| 1994 | 757 | 366 | 267 | 279 | 426 | 698 | 1,100 | 955 | 1,385 | 1,286 | 804 | 533 | 8,857 |
| 1995 | 312 | 187 | 146 | 124 | 203 | 310 | 647 | 641 | 1,311 | 1,746 | 956 | 749 | 7,331 |
| 1996 | 567 | 273 | 168 | 207 | 754 | 994 | 1,115 | 1,240 | 1,681 | 2,470 | 2,710 | 2,130 | 14,309 |
| 1997 | 1,054 | 426 | 291 | 401 | 519 | 499 | 1,042 | 1,609 | 1,732 | 1,535 | 1,011 | 454 | 10,574 |
| 1998 | 223 | 133 | 96 | 80 | 218 | 252 | 398 | 670 | 690 | 1,488 | 3,166 | 2,775 | 10,190 |
| 1999 | 1,012 | 424 | 330 | 406 | 1,224 | 1,475 | 979 | 1,472 | 1,353 | 1,473 | 2,038 | 2,064 | 14,249 |
| 2000 | 710 | 389 | 313 | 381 | 579 | 894 | 1,196 | 1,213 | 1,892 | 4,192 | 1,715 | 1,798 | 15,273 |
| Ave(mil m3) | 454 | 248 | 198 | 204 | 385 | 623 | 790 | 1,109 | 1,420 | 1,832 | 1,370 | 1,019 | 9,652 |

Table B. 13 Generated Monthly Discharge Series Dongnai River Basin
Dong Nai River at Hoa An ( $\mathrm{A}=\mathbf{2 2 , 5 9 4} \mathrm{km} 2$ )

|  | Jan | Feb | Mar | Apr | May | Jun | Jul | Aug | Sep | Oct | Nov | Dec | total |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| 1964 | 660 | 285 | 156 | 220 | 1,582 | 2,020 | 2,908 | 4,199 | 4,347 | 4,763 | 3,452 | 1,549 | 26,141 |
| 1965 | 687 | 254 | 169 | 236 | 2,217 | 1,928 | 2,365 | 2,447 | 3,454 | 3,747 | 2,507 | 1,350 | 21,360 |
| 1966 | 575 | 230 | 253 | 341 | 455 | 1,528 | 2,317 | 4,518 | 3,674 | 4,063 | 3,460 | 2,164 | 23,578 |
| 1967 | 873 | 346 | 223 | 328 | 1,359 | 2,128 | 4,782 | 5,075 | 5,513 | 7,116 | 4,555 | 1,723 | 34,022 |
| 1968 | 742 | 275 | 195 | 241 | 366 | 1,393 | 3,778 | 4,228 | 4,160 | 5,435 | 3,308 | 1,574 | 25,695 |
| 1969 | 743 | 326 | 207 | 166 | 495 | 717 | 2,149 | 3,691 | 5,054 | 5,492 | 3,032 | 1,117 | 23,189 |
| 1970 | 519 | 221 | 203 | 187 | 428 | 1,981 | 3,834 | 3,131 | 3,190 | 5,710 | 4,396 | 1,960 | 25,761 |
| 1971 | 866 | 356 | 249 | 327 | 1,146 | 1,830 | 2,660 | 3,478 | 4,794 | 5,501 | 3,009 | 1,536 | 25,753 |
| 1972 | 666 | 272 | 202 | 215 | 366 | 1,264 | 1,933 | 3,075 | 5,254 | 6,293 | 3,450 | 1,867 | 24,856 |
| 1973 | 753 | 315 | 221 | 212 | 507 | 1,192 | 1,466 | 3,174 | 4,777 | 4,320 | 2,207 | 1,072 | 20,216 |
| 1974 | 526 | 244 | 210 | 195 | 1,347 | 1,529 | 2,500 | 3,587 | 3,693 | 6,023 | 3,750 | 1,310 | 24,916 |
| 1975 | 631 | 282 | 236 | 511 | 368 | 1,113 | 3,343 | 4,174 | 6,058 | 3,827 | 2,247 | 1,246 | 24,034 |
| 1976 | 597 | 294 | 228 | 207 | 315 | 1,786 | 3,700 | 4,190 | 5,626 | 4,665 | 2,604 | 1,102 | 25,314 |
| 1977 | 527 | 239 | 212 | 196 | 198 | 430 | 3,332 | 4,879 | 3,268 | 2,892 | 1,731 | 818 | 18,721 |
| 1978 | 398 | 201 | 194 | 235 | 421 | 1,283 | 2,813 | 5,048 | 7,398 | 6,576 | 3,462 | 1,457 | 29,485 |
| 1979 | 647 | 274 | 235 | 373 | 1,175 | 2,200 | 4,199 | 5,141 | 3,226 | 4,839 | 2,841 | 1,342 | 26,492 |
| 1980 | 658 | 308 | 231 | 210 | 586 | 1,915 | 2,383 | 2,177 | 5,669 | 4,973 | 2,582 | 1,319 | 23,012 |
| 1981 | 677 | 422 | 240 | 284 | 736 | 3,490 | 3,242 | 4,587 | 3,953 | 4,568 | 2,296 | 1,017 | 25,513 |
| 1982 | 491 | 250 | 266 | 573 | 565 | 698 | 2,342 | 4,657 | 5,299 | 4,975 | 2,986 | 1,342 | 24,443 |
| 1983 | 537 | 248 | 246 | 588 | 250 | 905 | 2,897 | 3,289 | 5,542 | 6,213 | 3,366 | 1,388 | 25,470 |
| 1984 | 632 | 284 | 230 | 353 | 1,049 | 1,708 | 2,793 | 5,553 | 4,722 | 4,836 | 1,837 | 897 | 24,894 |
| Ave(mil m3) | 638 | 282 | 219 | 295 | 759 | 1,573 | 2,940 | 4,014 | 4,699 | 5,087 | 3,004 | 1,388 | 24,898 |

Saigon River u/s of DauTieng +ThiThinh basin ( $\mathrm{A}=3,650 \mathrm{~km} 2$ )

|  | Jan | Feb | Mar | Apr | May | Jun | Jul | Aug | Sep | Oct | Nov | Dec | total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1964 | 120 | 51 | 35 | 31 | 32 | 58 | 413 | 505 | 394 | 544 | 528 | 261 | 2,971 |
| 1965 | 143 | 59 | 37 | 33 | 35 | 53 | 94 | 146 | 286 | 260 | 465 | 270 | 1,881 |
| 1966 | 152 | 70 | 41 | 35 | 218 | 426 | 268 | 188 | 288 | 433 | 315 | 185 | 2,618 |
| 1967 | 122 | 59 | 40 | 38 | 38 | 90 | 316 | 465 | 365 | 529 | 444 | 292 | 2,798 |
| 1968 | 162 | 81 | 47 | 37 | 103 | 108 | 440 | 453 | 529 | 714 | 387 | 213 | 3,273 |
| 1969 | 119 | 52 | 41 | 38 | 40 | 44 | 162 | 376 | 428 | 774 | 645 | 286 | 3,006 |
| 1970 | 155 | 67 | 45 | 40 | 42 | 41 | 84 | 252 | 380 | 447 | 356 | 191 | 2,101 |
| 1971 | 113 | 53 | 43 | 40 | 42 | 135 | 408 | 428 | 322 | 609 | 677 | 414 | 3,285 |
| 1972 | 218 | 114 | 60 | 43 | 51 | 161 | 402 | 447 | 513 | 633 | 490 | 260 | 3,392 |
| 1973 | 160 | 82 | 51 | 43 | 45 | 55 | 116 | 129 | 251 | 801 | 614 | 426 | 2,773 |
| 1974 | 208 | 99 | 57 | 44 | 45 | 45 | 62 | 151 | 360 | 456 | 322 | 209 | 2,058 |
| 1975 | 137 | 69 | 49 | 44 | 125 | 235 | 231 | 349 | 265 | 608 | 678 | 340 | 3,131 |
| 1976 | 182 | 85 | 53 | 46 | 47 | 48 | 249 | 394 | 668 | 427 | 191 | 133 | 2,522 |
| 1977 | 81 | 47 | 47 | 46 | 47 | 52 | 83 | 220 | 589 | 674 | 401 | 213 | 2,500 |
| 1978 | 115 | 55 | 49 | 46 | 47 | 46 | 156 | 233 | 467 | 528 | 526 | 273 | 2,539 |
| 1979 | 156 | 73 | 52 | 48 | 85 | 108 | 214 | 349 | 594 | 704 | 405 | 229 | 3,016 |
| 1980 | 138 | 69 | 52 | 48 | 51 | 106 | 342 | 387 | 376 | 503 | 289 | 187 | 2,548 |
| 1981 | 117 | 58 | 51 | 49 | 59 | 243 | 565 | 541 | 573 | 625 | 633 | 324 | 3,838 |
| 1982 | 183 | 88 | 58 | 50 | 61 | 95 | 199 | 366 | 345 | 522 | 385 | 241 | 2,593 |
| 1983 | 152 | 76 | 56 | 51 | 53 | 85 | 175 | 291 | 646 | 777 | 541 | 283 | 3,186 |
| 1984 | 160 | 78 | 56 | 51 | 53 | 54 | 196 | 395 | 490 | 587 | 476 | 267 | 2,862 |
| Ave(mil m3) | 147 | 71 | 49 | 43 | 63 | 109 | 246 | 336 | 435 | 579 | 465 | 262 | 2,804 |

Dong Nai Basin ( $\mathrm{A}=\mathbf{2 9 , 1 2 0 \mathrm { km } 2 \text { ) } ) ~}$

|  | Jan | Feb | Mar | Apr | May | Jun | Jul | Aug | Sep | Oct | Nov | Dec | total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1964 | 780 | 336 | 191 | 251 | 1,614 | 2,403 | 4,030 | 5,380 | 4,898 | 5,406 | 4,249 | 1,810 | 31,349 |
| 1965 | 830 | 312 | 207 | 269 | 2,340 | 2,347 | 2,639 | 3,117 | 4,310 | 4,289 | 2,972 | 1,620 | 25,253 |
| 1966 | 727 | 299 | 294 | 376 | 985 | 2,706 | 3,053 | 5,681 | 4,421 | 4,958 | 3,933 | 2,350 | 29,783 |
| 1967 | 996 | 405 | 263 | 366 | 1,840 | 2,288 | 5,844 | 5,585 | 6,365 | 8,419 | 5,002 | 2,015 | 39,387 |
| 1968 | 905 | 356 | 242 | 278 | 734 | 1,931 | 4,307 | 4,819 | 6,003 | 6,592 | 3,694 | 1,786 | 31,646 |
| 1969 | 862 | 378 | 247 | 204 | 536 | 1,453 | 2,757 | 4,751 | 6,073 | 6,734 | 3,678 | 1,403 | 29,075 |
| 1970 | 674 | 288 | 247 | 228 | 469 | 2,589 | 3,993 | 3,830 | 4,044 | 6,820 | 4,752 | 2,152 | 30,085 |
| 1971 | 979 | 409 | 292 | 367 | 1,369 | 2,308 | 3,477 | 4,393 | 5,265 | 6,467 | 3,687 | 1,950 | 30,964 |
| 1972 | 884 | 386 | 262 | 257 | 417 | 2,069 | 2,567 | 3,522 | 6,449 | 7,011 | 3,959 | 2,127 | 29,909 |
| 1973 | 913 | 397 | 273 | 256 | 552 | 1,721 | 2,297 | 3,543 | 5,441 | 5,526 | 2,821 | 1,498 | 25,238 |
| 1974 | 734 | 343 | 267 | 240 | 1,392 | 1,574 | 2,702 | 3,752 | 4,214 | 7,082 | 4,073 | 1,519 | 27,893 |
| 1975 | 768 | 350 | 286 | 556 | 493 | 1,839 | 3,995 | 4,908 | 6,825 | 4,822 | 2,925 | 1,587 | 29,353 |
| 1976 | 778 | 379 | 281 | 253 | 363 | 2,324 | 4,371 | 4,969 | 6,797 | 5,480 | 2,795 | 1,235 | 30,024 |
| 1977 | 608 | 286 | 259 | 242 | 245 | 779 | 4,140 | 5,899 | 4,470 | 3,815 | 2,578 | 1,031 | 24,351 |
| 1978 | 512 | 255 | 244 | 280 | 587 | 1,917 | 3,281 | 5,662 | 8,716 | 8,111 | 4,076 | 1,730 | 35,371 |
| 1979 | 804 | 347 | 287 | 420 | 1,260 | 2,494 | 4,673 | 5,556 | 3,835 | 5,961 | 3,245 | 1,571 | 30,452 |
| 1980 | 796 | 377 | 284 | 258 | 813 | 2,714 | 3,103 | 3,032 | 6,397 | 5,667 | 2,952 | 1,506 | 27,898 |
| 1981 | 794 | 480 | 292 | 333 | 796 | 4,057 | 4,813 | 5,383 | 4,547 | 5,508 | 2,929 | 1,341 | 31,273 |
| 1982 | 673 | 337 | 324 | 623 | 626 | 1,379 | 3,013 | 5,403 | 6,196 | 5,923 | 3,585 | 1,583 | 29,665 |
| 1983 | 688 | 324 | 303 | 639 | 303 | 1,174 | 3,737 | 4,496 | 6,567 | 7,194 | 4,023 | 1,671 | 31,120 |
| 1984 | 792 | 361 | 286 | 404 | 1,102 | 1,872 | 3,682 | 5,998 | 5,673 | 6,046 | 2,313 | 1,164 | 29,694 |
| Ave(mil m3) | 786 | 353 | 268 | 338 | 897 | 2,092 | 3,642 | 4,747 | 5,596 | 6,087 | 3,535 | 1,650 | 29,990 |

Table B. 14 Generated Monthly Discharge Series Cuu Long River Delta
Cuu Long (Mekong) at the boundary with Cambodia

|  | Jan | Feb | Mar | Apr | May | Jun | Jul | Aug | Sep | Oct | Nov | Dec | total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1960 | 14,404 | 7,292 | 5,308 | 3,719 | 3,380 | 9,955 | 32,126 | 96,788 | 79,403 | 83,952 | 52,228 | 34,896 | 423,450 |
| 1961 | 21,129 | 10,876 | 7,597 | 5,112 | 9,101 | 33,917 | 59,890 | 106,069 | 112,851 | 113,483 | 63,589 | 41,304 | 584,919 |
| 1962 | 26,098 | 14,822 | 9,828 | 5,816 | 8,288 | 32,690 | 53,135 | 79,962 | 84,176 | 74,707 | 53,311 | 33,293 | 476,126 |
| 1963 | 19,291 | 9,650 | 7,652 | 5,765 | 4,947 | 22,154 | 49,123 | 95,201 | 90,241 | 76,259 | 55,212 | 35,977 | 471,473 |
| 1964 | 19,907 | 10,040 | 7,688 | 6,474 | 12,073 | 21,395 | 42,273 | 60,315 | 80,614 | 85,164 | 60,726 | 39,136 | 445,804 |
| 1965 | 22,310 | 11,502 | 8,222 | 6,386 | 8,071 | 39,950 | 55,802 | 60,541 | 77,078 | 61,656 | 48,578 | 30,846 | 430,943 |
| 1966 | 18,984 | 10,623 | 8,916 | 7,681 | 16,234 | 29,784 | 63,355 | 85,759 | 125,105 | 77,934 | 54,444 | 38,159 | 536,977 |
| 1967 | 24,001 | 12,528 | 9,591 | 7,318 | 10,681 | 21,301 | 35,604 | 60,466 | 69,629 | 62,963 | 42,795 | 30,262 | 387,138 |
| 1968 | 17,414 | 9,658 | 6,772 | 5,214 | 9,653 | 16,138 | 33,236 | 65,290 | 88,643 | 64,276 | 43,808 | 26,279 | 386,382 |
| 1969 | 10,759 | 6,404 | 5,864 | 5,979 | 5,758 | 30,992 | 60,103 | 84,270 | 54,906 | 57,451 | 45,968 | 26,549 | 395,004 |
| 1970 | 14,342 | 6,757 | 5,129 | 5,377 | 10,051 | 29,529 | 66,072 | 108,598 | 102,139 | 71,330 | 44,486 | 34,517 | 498,325 |
| 1971 | 20,776 | 10,080 | 6,140 | 6,911 | 8,696 | 26,053 | 82,5 | 92, | 91,501 | 73,357 | 50,465 | 33,099 | 502,408 |
| 1972 | 20,498 | 11,017 | 8,722 | 7,990 | 9,081 | 25,942 | 40,802 | 105,250 | 74,741 | 72,460 | 53,200 | 39,680 | 469,383 |
| 1973 | 23,858 | 12,318 | 9,709 | 7,056 | 12,850 | 28,261 | 40,699 | 58,401 | 97,823 | 60,081 | 45,922 | 32,171 | 429,150 |
| 1974 | 16,683 | 8,564 | 6,626 | 7,010 | 8,925 | 25,126 | 23,662 | 83,624 | 85,135 | 47,428 | 44,302 | 29,476 | 386,561 |
| 1975 | 16,742 | 8,699 | 6,414 | 5,708 | 7,840 | 29,359 | 43,616 | 77,825 | 94,842 | 65,614 | 52,320 | 29,364 | 438,344 |
| 1976 | 15,960 | 9,010 | 7,156 | 6,818 | 10,320 | 18,895 | 29,147 | 80,619 | 46,841 | 61,838 | 55,013 | 33,269 | 374,885 |
| 1977 | 15,687 | 7,432 | 7,052 | 7,334 | 7,219 | 4,714 | 20,432 | 41,533 | 66,566 | 44,335 | 43,045 | 27,936 | 293,283 |
| 1978 | 17,269 | 8,448 | 6,816 | 6,012 | 9,407 | 29,799 | 56,389 | 132,572 | 97,551 | 87,355 | 47,838 | 28,804 | 528,260 |
| 1979 | 16,525 | 8,765 | 7,156 | 6,499 | 12,910 | 32,552 | 48,357 | 70,545 | 65,830 | 52,771 | 37,903 | 28,022 | 387,836 |
| 1980 | 15,204 | 7,705 | 6,291 | 5,882 | 7,891 | 19,595 | 40,922 | 64,757 | 106,081 | 67,894 | 53,012 | 30,700 | 425,934 |
| 1981 | 16,991 | 8,893 | 7,657 | 7,103 | 11,812 | 54,890 | 82,405 | 99,116 | 67,424 | 57,512 | 45,013 | 34,683 | 493,498 |
| 1982 | 18,705 | 9,957 | 7,874 | 7,671 | 8,111 | 14,354 | 28,312 | 62,692 | 74,943 | 79,764 | 48,216 | 30,556 | 391,155 |
| 1983 | 16,986 | 8,634 | 7,855 | 6,927 | 7,181 | 11,894 | 18,163 | 50,026 | 68,064 | 64,141 | 58,058 | 35,045 | 352,974 |
| 1984 | 18,510 | 9,346 | 6,795 | 5,638 | 10,090 | 21,689 | 54,761 | 95,326 | 80,278 | 54,435 | 48,499 | 29,385 | 434,751 |
| Ave(mil m3) | 18,361 | 9,561 | 7,393 | 6,376 | 9,223 | 25,237 | 46,437 | 80,733 | 83,296 | 68,726 | 49,918 | 32,536 | 437,799 |

Monthly Runoff Cuu Long Delta

|  | Jan | Feb | Mar | Apr | May | Jun | Jul | Aug | Sep | Oct | Nov | Dec | total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1960 | 14,404 | 7,292 | 5,308 | 3,719 | 3,494 | 13,401 | 36,027 | 98,833 | 85,349 | 90,201 | 54,197 | 34,896 | 447,119 |
| 1961 | 21,129 | 10,876 | 7,597 | 5,112 | 9,215 | 37,363 | 63,790 | 108,114 | 118,797 | 119,731 | 65,558 | 41,304 | 608,588 |
| 1962 | 26,098 | 14,822 | 9,828 | 5,816 | 8,402 | 36,136 | 57,036 | 82,007 | 90,122 | 80,955 | 55,281 | 33,293 | 499,795 |
| 1963 | 19,291 | 9,650 | 7,652 | 5,765 | 5,061 | 25,600 | 53,024 | 97,246 | 96,187 | 82,507 | 57,181 | 35,977 | 495,141 |
| 1964 | 19,907 | 10,040 | 7,688 | 6,474 | 12,186 | 24,841 | 46,174 | 62,360 | 86,559 | 91,413 | 62,695 | 39,136 | 469,473 |
| 1965 | 22,310 | 11,502 | 8,222 | 6,386 | 8,184 | 43,397 | 59,703 | 62,586 | 83,024 | 67,904 | 50,548 | 30,846 | 454,612 |
| 1966 | 18,984 | 10,623 | 8,916 | 7,681 | 16,348 | 33,230 | 67,256 | 87,804 | 131,050 | 84,182 | 56,413 | 38,159 | 560,646 |
| 1967 | 24,001 | 12,528 | 9,591 | 7,318 | 10,795 | 24,747 | 39,504 | 62,511 | 75,574 | 69,212 | 44,764 | 30,262 | 410,807 |
| 1968 | 17,414 | 9,658 | 6,772 | 5,214 | 9,767 | 19,584 | 37,137 | 67,335 | 94,589 | 70,525 | 45,777 | 26,279 | 410,051 |
| 1969 | 10,759 | 6,404 | 5,864 | 5,979 | 5,871 | 34,438 | 64,004 | 86,315 | 60,852 | 63,699 | 47,937 | 26,549 | 418,673 |
| 1970 | 14,342 | 6,757 | 5,129 | 5,377 | 10,165 | 32,975 | 69,972 | 110,643 | 108,084 | 77,578 | 46,456 | 34,517 | 521,994 |
| 1971 | 20,776 | 10,080 | 6,140 | 6,911 | 8,810 | 29,499 | 86,448 | 94,827 | 97,447 | 79,606 | 52,435 | 33,099 | 526,076 |
| 1972 | 20,498 | 11,017 | 8,722 | 7,990 | 9,195 | 29,388 | 44,703 | 107,295 | 80,687 | 78,709 | 55,169 | 39,680 | 493,052 |
| 1973 | 23,858 | 12,318 | 9,709 | 7,056 | 12,963 | 31,707 | 44,600 | 60,446 | 103,769 | 66,329 | 47,892 | 32,171 | 452,819 |
| 1974 | 16,683 | 8,564 | 6,626 | 7,010 | 9,039 | 28,572 | 27,563 | 85,669 | 91,081 | 53,677 | 46,272 | 29,476 | 410,230 |
| 1975 | 16,742 | 8,699 | 6,414 | 5,708 | 7,954 | 32,805 | 47,517 | 79,870 | 100,788 | 71,863 | 54,289 | 29,364 | 462,013 |
| 1976 | 15,960 | 9,010 | 7,156 | 6,818 | 10,434 | 22,341 | 33,048 | 82,664 | 52,786 | 68,086 | 56,982 | 33,269 | 398,554 |
| 1977 | 15,687 | 7,432 | 7,052 | 7,334 | 7,333 | 8,160 | 24,332 | 43,578 | 72,512 | 50,583 | 45,015 | 27,936 | 316,952 |
| 1978 | 17,269 | 8,448 | 6,816 | 6,012 | 9,612 | 38,218 | 61,540 | 135,711 | 106,234 | 88,484 | 50,856 | 28,804 | 558,003 |
| 1979 | 16,525 | 8,765 | 7,156 | 6,499 | 12,910 | 32,775 | 56,575 | 74,260 | 69,194 | 54,233 | 39,864 | 28,022 | 406,780 |
| 1980 | 15,204 | 7,705 | 6,291 | 5,882 | 10,391 | 29,097 | 40,922 | 66,454 | 108,558 | 76,335 | 55,106 | 30,700 | 452,644 |
| 1981 | 16,991 | 8,893 | 7,657 | 7,103 | 13,895 | 61,839 | 82,405 | 101,426 | 72,215 | 64,647 | 47,067 | 34,683 | 518,820 |
| 1982 | 18,705 | 9,957 | 7,874 | 7,671 | 8,111 | 15,690 | 37,306 | 70,058 | 80,662 | 87,285 | 48,216 | 30,556 | 422,091 |
| 1983 | 16,986 | 8,634 | 7,855 | 6,927 | 7,181 | 16,715 | 23,359 | 56,979 | 76,081 | 71,768 | 59,694 | 35,045 | 387,224 |
| 1984 | 18,510 | 9,346 | 6,795 | 5,638 | 10,090 | 30,744 | 54,761 | 98,946 | 84,777 | 57,658 | 48,499 | 29,385 | 455,148 |
| Ave(mil m3) | 18,361 | 9,561 | 7,393 | 6,376 | 9,496 | 29,331 | 50,348 | 83,357 | 89,079 | 74,687 | 51,767 | 32,536 | 462,292 |

(

Log-normal Distribution (Yearly maximum discharges)


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Figure B. 2
Frequency Distributions
Ky Cung River at Lang Son

1980 Flood at Lang Son (instantaneous peak discharge: 2,800 m3/s)

Time Series

-BG05 QH

Ky Cung Basin Area Rainfall (upstream Lang Son)
Time Series

-AKYPA

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Figure B. 3
Typical Flood
Ky Cung River at Lang Son

BF-3



Gumbel Distribution (Yearly maximum discharges)


Goodrich Distributions (Yearly maximum discharges)


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Figure B. 6
Frequency Distributions
Chu River at Cua Dat

