

Appendix A-7

Structural Calculations for Water Treatment Plant

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1. GENERAL

1. ОБЩЕЕ ОПИСАНИЕ

1. GENERAL

1.1 Reference Standards

- BS 8110 : Structural use of Concrete
 BS 8007 : Code of Practice for Design of Concrete Structures for Retaining Aqueous Liquids
 BS CP3 : Code of Basic Data for Design of Buildings: Chapter V: Part 2: Loading Uniform Building By-laws, 1984

1.2 Design Materials

This calculation is based on the following design materials.

- | | | | |
|-------------------|---------|-----------------------------------|----------------------------|
| • Concrete | BS 8110 | Grade 30 or Equivalent : | $f_{cu}=30 \text{ N/mm}^2$ |
| | BS 8007 | Grade 30A or Equivalent : | $f_{cu}=30 \text{ N/mm}^2$ |
| • Reinforcing Bar | BS 4449 | High Yield Deformed Bar Grade 365 | $f_y=365 \text{ N/mm}^2$ |
| | | or Equivalent: | |
- (f_y : high yield having a minimum characteristic strength)

1.3 Design Ground Level and Ground Water Level

Design Ground Water Level (GWL)
 GWL =

1.4 Minimum Concrete Cover

Minimum concrete cover on the outer reinforcement shall be as follows.

- | | |
|---|------|
| - Cast against and permanently exposed to earth | 75mm |
| - Exposed to earth, weather or water | 50mm |
| - Not exposed to weather or contact with the ground:
slabs, walls, beams, girders, columns | 40mm |

1.5 Crack width control

Crack width control shall satisfy the requirements of the BS 8007:1987.

- | | |
|---------------------------|-----------------------------------|
| 1) Allowable crack width: | 0.2 mm (Section 3.2.2 of BS 8007) |
| 2) Applicable Structure: | Pit and Basin |

1.6 Design Loadings

1) DEAD LOADS : (DL)

Unit Weight of Materials

Water	: 10 kN/m ³
Soil	: 18 kN/m ³
Plain concrete	: 23 kN/m ³
Reinforced concrete	: 24 kN/m ³

2) LIVE LOADS (Imposed Loads) : (LL)

3) EARTH PRESSURE / WATER PRESSURE : (EP/ WP)

1.7 LOAD COMBINATIONS

1) Load combinations and Load factors for Pit/Basin based on BS 5950, BS 8110 and UBC are applied.

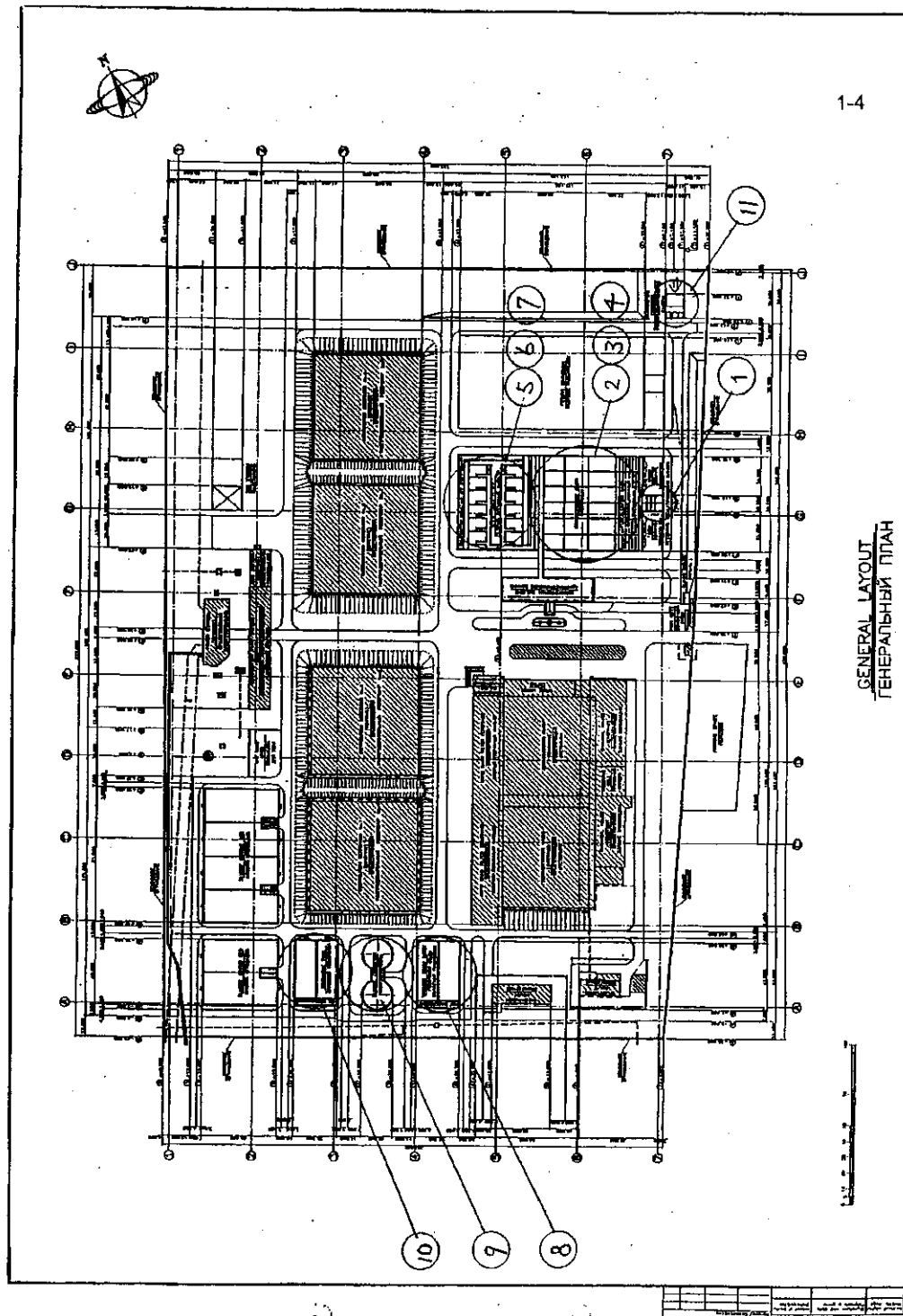
Pit & Basin

1.4 (DL) + 1.6 (LL) + 1.4 (EP)

1.4 (DL) + 1.6 (LL) + 1.4 (EP + WP)

Remarks : The stability of the structure shall be checked for non-factored load combinations.

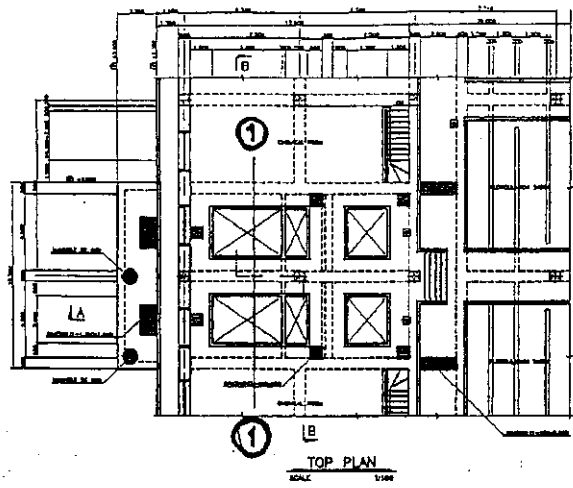
A-7-3



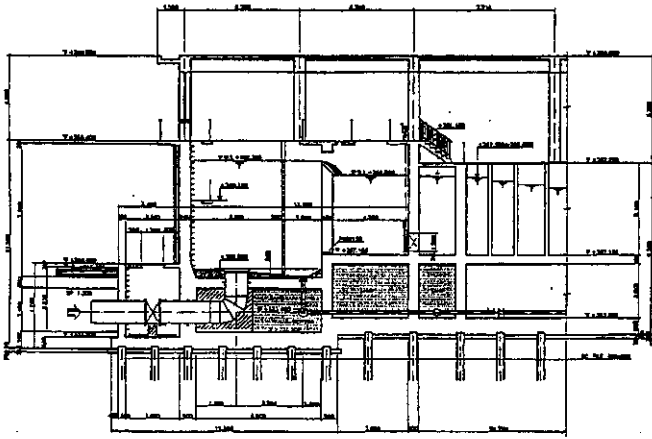
GENERAL LAYOUT
ГЕНЕРАЛЬНЫЙ ПЛАН

2. RECEIVING WELL

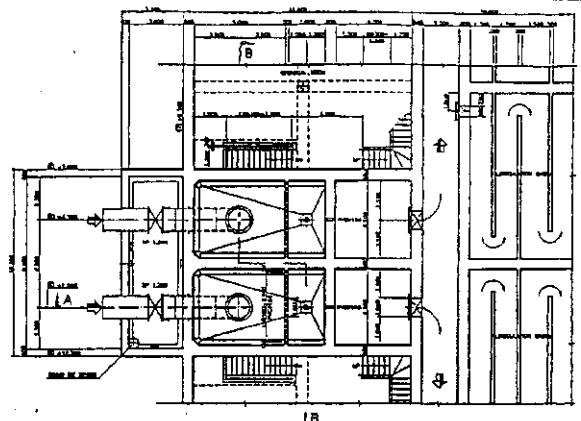
2. ПРИЕМНАЯ КАМЕРА



TOP PLAN
SCALE 1/100

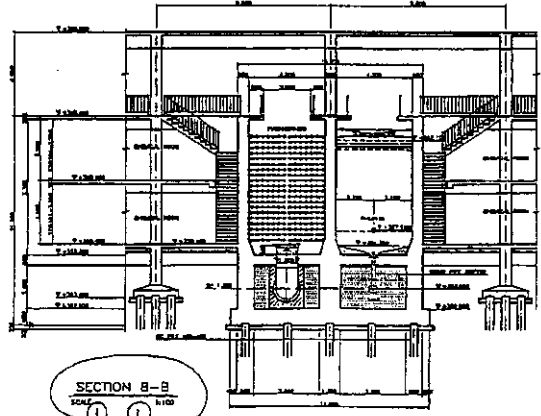


SECTION A-A
SCALE 1/100



BOTTOM PLAN
SCALE 1/100

香水と空



SECTION B-B
SCALE 1/100

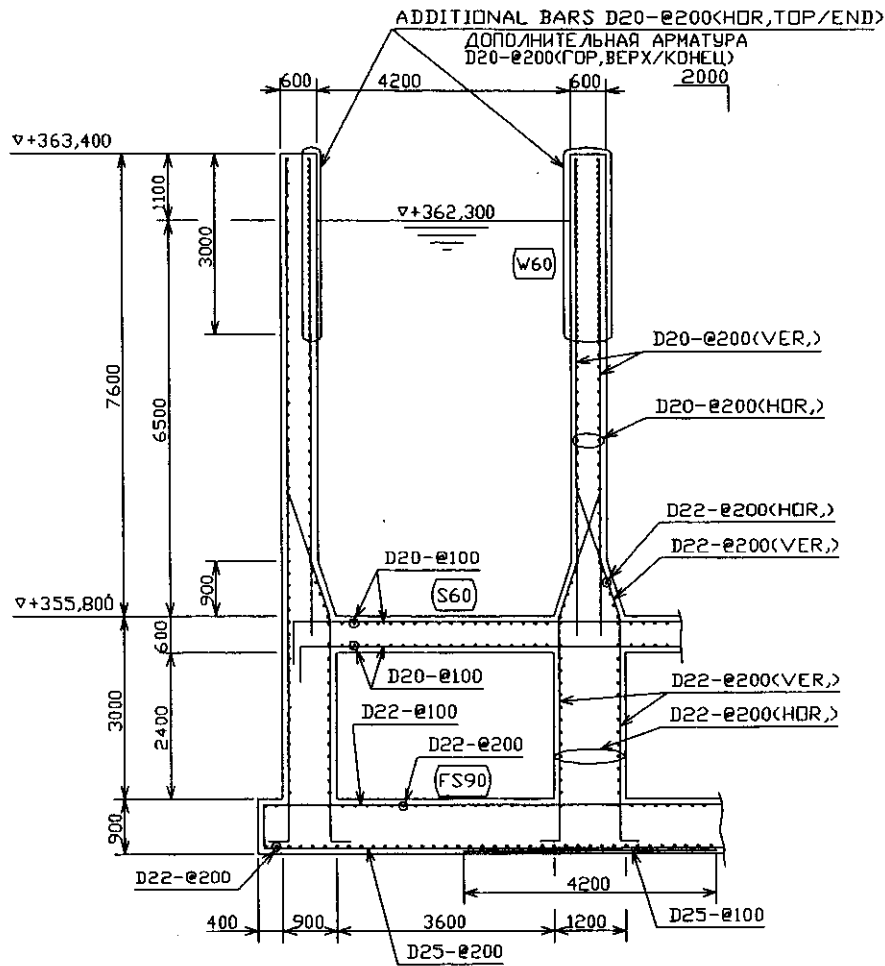
1 - 1

2-1 Plan and Section
2-1 План и разрез

2-2

2-1

2.2 DESIGN OF WALL & SLAB ① - ①
 2.2 ПРОЕКТИРОВАНИЕ СТЕНЫ И ПЛИТЫ



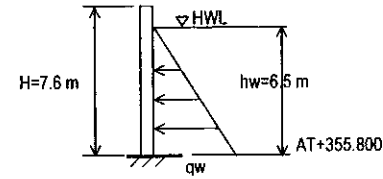
① - ①
 S=1/80

A.7-5

2.2.1 CALCULATION OF WALL (W60)
 (AT+355.800)

①-①

1) DESIGN LOAD (Water Pressure)

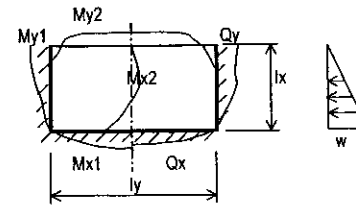


$$hw = 6.50 \text{ m}$$

$$\gamma_w = 10.0 \text{ KN/m}^3$$

$$qw = \gamma_w \cdot hw = 65.0 \text{ KN/m}^2/\text{m}$$

2) FACTORED STRESS (W60)



$$lx = 6.50 \text{ m}$$

$$ly = 7.20 \text{ m}$$

$$\lambda = 1.11$$

$$w = qw = 65.0 \text{ KN/m}^2/\text{m}$$

$$\text{Factored Load } w' = 1.4 \cdot w = 91.00 \text{ KN/m}^2/\text{m}$$

$$w' \cdot lx^2 = 3844.8$$

$$w' \cdot lx = 591.5$$

SEE Fig.2

$$Mx1 = 0.040 \times 3844.8 = 153.8 \text{ (KN} \cdot \text{m/m)}$$

$$Mx2 = 0.010 \times 3844.8 = 38.4$$

$$My1 = 0.033 \times 3844.8 = 126.9$$

$$My2 = 0.012 \times 3844.8 = 46.1$$

$$Qx = 0.35 \times 591.5 = 207.0 \text{ (KN/m)}$$

$$Qy = 0.25 \times 591.5 = 147.9$$

3) DESIGN OF SECTION

a) VERTICAL : BOT (AT+355.800)

$$\begin{aligned} \mu_u &= 153.8 \text{ KN}\cdot\text{m} \\ \nu_u &= 207.0 \text{ KN/m} \end{aligned}$$

$$\begin{aligned} b &= 1000 \text{ mm} & f_y &= 365 \text{ N/mm}^2 \\ D &= 900 \text{ mm} & f_{cu} &= 30 \text{ N/mm}^2 \\ d &= 840 \text{ mm} \end{aligned}$$

[REQUIRED RE-BAR]

$$\begin{aligned} K &= \mu_u / (f_{cu} \cdot b \cdot d^2) & &= 0.007 < 0.156 \\ Z_1 &= d \cdot [0.5 + \sqrt{(0.25 - K/0.9)}] & &= 833 \\ Z_2 &= 0.95 \cdot d & &= 798 \\ Z &= & &798 \\ A_{s1} &= \mu_u / (0.95 \cdot f_y \cdot Z) & &= 556 \text{ mm}^2 \\ A_{smin} &= 0.0013 \cdot b \cdot D & &= 1170 \text{ mm}^2 \end{aligned}$$

$$\text{USE ; } D 22 \text{ @ } 200 \text{ (} A_s = 1900 \text{ mm}^2 \text{)}$$

[CHECK OF SHEAR STRESS]

$$\begin{aligned} V &= \nu_u / (b \cdot d) & &= 0.25 < 4.4 \text{ N/mm}^2 \text{ OK} \\ (100 \cdot A_s) / (b \cdot d) & & &= 0.23 < 3.0 \\ 400/d & & &= 0.5 < 1.0 \rightarrow 1.0 \end{aligned}$$

$$\begin{aligned} V_c &= 0.79 \cdot \left\{ (100 \cdot A_s) / (b \cdot d) \right\}^{1/3} \cdot (400/d)^{1/4} \cdot (1/1.25) \cdot (f_{cu}/25)^{1/3} \\ &= 0.41 \text{ N/mm}^2 > V = 0.25 \text{ N/mm}^2 \text{ OK} \end{aligned}$$

b) HORIZONTAL : TOP/END

$$\begin{aligned} \mu_u &= 126.9 \text{ KN}\cdot\text{m} \\ \nu_u &= 147.9 \text{ KN/m} \end{aligned}$$

$$\begin{aligned} b &= 1000 \text{ mm} & f_y &= 365 \text{ N/mm}^2 \\ D &= 600 \text{ mm} & f_{cu} &= 30 \text{ N/mm}^2 \\ d &= 520 \text{ mm} \end{aligned}$$

[REQUIRED RE-BAR]

$$\begin{aligned} K &= \mu_u / (f_{cu} \cdot b \cdot d^2) & &= 0.016 < 0.156 \\ Z_1 &= d \cdot [0.5 + \sqrt{(0.25 - K/0.9)}] & &= 511 \\ Z_2 &= 0.95 \cdot d & &= 494 \\ Z &= & &494 \\ A_{s1} &= \mu_u / (0.95 \cdot f_y \cdot Z) & &= 741 \text{ mm}^2 \\ A_{smin} &= 0.0013 \cdot b \cdot D & &= 780 \text{ mm}^2 \end{aligned}$$

$$\text{USE ; } D 22 \text{ @ } 200 \text{ (} A_s = 1900 \text{ mm}^2 \text{)}$$

[CHECK OF SHEAR STRESS]

$$\begin{aligned} V &= \nu_u / (b \cdot d) & &= 0.28 < 4.4 \text{ N/mm}^2 \text{ OK} \\ (100 \cdot A_s) / (b \cdot d) & & &= 0.37 < 3.0 \\ 400/d & & &= 0.8 < 1.0 \rightarrow 1.0 \end{aligned}$$

$$\begin{aligned} V_c &= 0.79 \cdot \left\{ (100 \cdot A_s) / (b \cdot d) \right\}^{1/3} \cdot (400/d)^{1/4} \cdot (1/1.25) \cdot (f_{cu}/25)^{1/3} \\ &= 0.48 \text{ N/mm}^2 > V = 0.28 \text{ N/mm}^2 \text{ OK} \end{aligned}$$

4) CHECK OF CRACKING
W60 (VERTICAL:BOT.)

①-①

a) CHECK OF MINIMUM REINFORCEMENT (BASED ON 2.6.2.3 OF BS8007)

$$\begin{array}{llll} \text{SECTION} & b = 1000 \text{ mm} & \text{SURFACE ZONE :} & d_1 = 250 \text{ mm} \\ & D = 900 \text{ mm} & & \\ & dv = 840 \text{ mm} & & \\ & dh = 820 \text{ mm} & & \end{array}$$

$$\text{Min. } \rho = 0.0035 \text{ (} 0.35 \% \text{)}$$

$$\begin{aligned} A_{s(\text{ver.})} &= \rho \cdot b \cdot d_1 = 875 \text{ mm}^2 \\ A_{s(\text{her.})} &= \rho \cdot b \cdot d_1 = 875 \text{ mm}^2 \end{aligned}$$

$$\begin{array}{llll} \text{USE ; } & \text{VERTICAL} & D 22 & @ 200 & (A_s = 1900 \text{ mm}^2) \\ \text{USE ; } & \text{HORIZONTAL} & D 22 & @ 200 & (A_s = 1900 \text{ mm}^2) \end{array}$$

b) CHECK OF STEEL STRESS IN DIRECT OR FLEXURAL TENSION FOR SERVICEABILITY LIMIT STATES
(BASED ON 3.2.2 OF BS8007)

$$\begin{array}{llll} \text{Ver.} & \mu_u = 153.8 \text{ KN}\cdot\text{m} & M = 109.9 \text{ KN}\cdot\text{m} & b = 1000 \text{ mm} \\ \text{Hor.} & \mu_u = \text{KN}\cdot\text{m} & M = 0.0 \text{ KN}\cdot\text{m} & D = 900 \text{ mm} \\ & & & dv = 840 \text{ mm} \\ & & & dh = 820 \text{ mm} \\ & & & f_y = 130 \text{ N/mm}^2 \\ & & & f_{cu} = 30 \text{ N/mm}^2 \end{array}$$

VERTICAL

$$\begin{aligned} K &= M / (f_{cu} \cdot b \cdot dv^2) & &= 0.0052 \\ Z_1 &= dv \cdot [0.5 + \sqrt{(0.25 - K/0.9)}] & &= 835 \\ Z_2 &= 0.95 \cdot dv & &= 798 \\ Z &= & &798 \\ A_s &= M / f_y \cdot Z & &= 1059.4 \text{ mm}^2 \end{aligned}$$

$$\text{USE ; } D 22 \text{ @ } 200 \text{ (} A_s = 1900 \text{ mm}^2 \text{)}$$

HORIZONTAL

$$\begin{aligned} K &= M / (f_{cu} \cdot b \cdot dh^2) & &= 0 \\ Z_1 &= dh \cdot [0.5 + \sqrt{(0.25 - K/0.9)}] & &= 0 \\ Z_2 &= 0.95 \cdot dh & &= 0 \\ Z &= & &0 \\ A_s &= M / f_y \cdot Z & &= 0 \text{ mm}^2 \end{aligned}$$

$$\text{USE ; } D 22 \text{ @ } 200 \text{ (} A_s = 1900 \text{ mm}^2 \text{)}$$

c) CHECK OF CRACK WIDTH (BASE ON APPENDIX A OF BS8007)

1) CRACK SPACING

$$\begin{aligned} S_{\text{max}}(V) &= (f_{ct}/f_b) \cdot (\psi/2\rho(V)) & &= 969.74 \text{ mm} \\ S_{\text{max}}(H) &= (f_{ct}/f_b) \cdot (\psi/2\rho(H)) & &= 969.7 \text{ mm} \end{aligned}$$

$$\begin{array}{ll} \text{Where ; } f_{ct}/f_b = 0.67 & \rho(V) = A_s/b \cdot d_1 = 0.0076 \\ & \rho(H) = A_s/b \cdot d_1 = 0.0076 \end{array}$$

2) CRACK WIDTHS ARISING FROM RESTRAINED SHRINKAGE AND HEAT OF HYDRATION MOVEMENT

$$\begin{aligned} W_{\text{max}}(V) &= S_{\text{max}}(V) \cdot (\alpha/2) \cdot T_1 & &= 0.19 < 0.2 \text{ mm} \\ W_{\text{max}}(H) &= S_{\text{max}}(H) \cdot (\alpha/2) \cdot T_1 & &= 0.19 < 0.2 \text{ mm} \end{aligned}$$

Where ; α : COEFFICIENT OF THERMAL EXPANSION OF MATURE CONCRETE

$$\begin{aligned} \alpha &= 1E-05 \\ T_1 &= 40 \text{ from TABLE 4.2} \end{aligned}$$

a) CHECK OF MINIMUM REINFORCEMENT (BASED ON 2.6.2.3 OF BS8007)

SECTION b = 1000 mm SURFACE ZONE : d1 = 250 mm
 D = 600 mm
 dv = 540 mm
 dh = 520 mm

Min. ρ = 0.0035 (0.35 %)

As (ver.) = ρ · b · d1 = 875 mm²
 As (her.) = ρ · b · d1 = 875 mm²

USE ;	VERTICAL	D 20	@ 200	(As = 1570 mm ²)
USE ;	HORIZONTAL	D 20	@ 200	(As = 1570 mm ²)

b) CHECK OF STEEL STRESS IN DIRECT OR FLEXURAL TENSION FOR SERVICEABILITY LIMIT STATES (BASED ON 3.2.2 OF BS8007)

Ver. Mu = KN·m M = 0.0 KN·m b = 1000 mm
 Hor. Mu = 126.9 KN·m M = 90.6 KN·m D = 600 mm
 dv = 540 mm
 dh = 520 mm
 fy = 130 N/mm²
 fcu = 30 N/mm²

VERTICAL

$K = M / (f_{cu} \cdot b \cdot d_v^2) = 0$
 $Z1 = d_v \cdot [0.5 + \sqrt{ (0.25 - K/0.9) }] = 0$
 $Z2 = 0.95 \cdot d_v = 0$
 $Z = 0$
 $A_s = M / f_y \cdot Z = 0 \text{ mm}^2$

USE ;	D 20	@ 200	(As = 1570 mm ²)
-------	------	-------	------------------------------

HORIZONTAL

$K = M / (f_{cu} \cdot b \cdot d_h^2) = 0.0112$
 $Z1 = d_h \cdot [0.5 + \sqrt{ (0.25 - K/0.9) }] = 513$
 $Z2 = 0.95 \cdot d_h = 494$
 $Z = 494$
 $A_s = M / f_y \cdot Z = 1410.8 \text{ mm}^2$

USE ;	D 20	@ 200	(As = 1570 mm ²)
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c) CHECK OF CRACK WIDTH (BASE ON APPENDIX A OF BS8007)

1) CRACK SPACING

$S_{max(V)} = (f_{ct}/f_b) \cdot (\sigma/2\rho(V)) = 1063.5 \text{ mm}$
 $S_{max(H)} = (f_{ct}/f_b) \cdot (\sigma/2\rho(H)) = 1063.5 \text{ mm}$

Where ; $f_{ct}/f_b = 0.67$ $\rho(V) = A_s/b \cdot d1 = 0.0063$
 $\rho(H) = A_s/b \cdot d1 = 0.0063$

2) CRACK WIDTHS ARISING FORM RESTRAINED SHRINKAGE AND HEAT OF HYDRATION MOVEMENT

$W_{max(V)} = S_{max(V)} \cdot (a/2) \cdot T1 = 0.19 < 0.2 \text{ mm}$
 $W_{max(H)} = S_{max(H)} \cdot (a/2) \cdot T1 = 0.19 < 0.2 \text{ mm}$

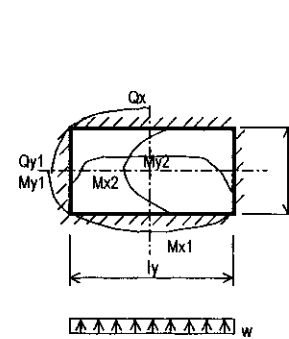
Where ; a : COEFFICIENT OF THERMAL EXPANSION OF MATURE CONCRETE

$a = 1E-05$
 $T1 = 35$ from TABLE 4.2

A.7-7

2.2.2 CALCULATION OF SLAB (S60) (AT+355.800)

1) DESIGN LOAD & FACTORED STRESS



hw = 6.50 m
 t = 0.60 m
 γw = 10.0 KN/m³
 γc = 24.0 KN/m⁴
 $w = \gamma_w \cdot hw + \gamma_c \cdot t = 79.4 \text{ KN/m}^2/\text{m}$

lx = 3.60 m
 ly = 6.90 m
 λ = 1.92
 w = 79.4 KN/m²/m
 $w' = 1.4 \cdot w = 111.16 \text{ KN/m}^2/\text{m}$
 $w' \cdot l_x^2 = 1440.6$
 $w' \cdot l_x = 400.2$

SEE FIG.2

Mx1	=	0.083 x 1440.6	=	119.6 (KN · m/m)
Mx2	=	0.040 x 1440.6	=	57.6
My1	=	0.057 x 1440.6	=	82.1
My2	=	0.010 x 1440.6	=	14.4
Qx	=	0.52 x 400.2	=	208.1 (KN/m)
Qy	=	0.46 x 400.2	=	184.1

2) DESIGN OF SECTION

Mu = 119.6 KN·m
 Vu = 208.1 KN/m

b = 1000 mm fy = 365 N/mm²
 D = 600 mm fcu = 30 N/mm²
 d = 540 mm

[REQUIRED RE-BAR]

$K = M_u / (f_{cu} \cdot b \cdot d^2) = 0.014 < 0.156$
 $Z1 = d \cdot [0.5 + \sqrt{ (0.25 - K/0.9) }] = 532$
 $Z2 = 0.95 \cdot d = 513$
 $Z = 513$
 $A_s1 = M_u / 0.95 \cdot f_y \cdot Z = 672 \text{ mm}^2$
 $A_{smin} = 0.0013 \cdot b \cdot D = 780 \text{ mm}^2$

USE ;	D 20	@ 200	(As = 1570 mm ²)
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[CHECK OF SHEAR STRESS]

$V = V_u / (b \cdot d) = 0.39 < 4.4 \text{ N/mm}^2 \text{ OK}$
 $(100 \cdot A_s) / (b \cdot d) = 0.29 < 3.0$
 $400/d = 0.7 < 1.0 \rightarrow 1.0$

$V_c = 0.79 \cdot \{ (100 \cdot A_s) / (b \cdot d) \}^{1/3} \cdot (400/d)^{1/4} \cdot (1/1.25) \cdot (f_{cu}/25)^{1/3}$
 $= 0.45 \text{ N/mm}^2 > V = 0.39 \text{ N/mm}^2 \text{ OK}$

3) CHECK OF CRACKING
SLAB (S60)

①-①

2-9

2-10

a) CHECK OF MINIMUM REINFORCEMENT (BASED ON 2.6.2.3 OF BS8007)

SECTION b = 1000 mm SURFACE ZONE : d(T) = 250 mm
D = 600 mm d(B) = 100 mm
d = 540 mm

Min. $\rho = 0.0035$ (0.35 %)

As (top) = $\rho \cdot b \cdot d(T) = 875 \text{ mm}^2$
As (bot) = $\rho \cdot b \cdot d(B) = 350 \text{ mm}^2$

USE ; TOP D 20 @ 200 (As = 1570 mm²)
USE ; BOTTOM D 20 @ 200 (As = 1570 mm²)

b) CHECK OF STEEL STRESS IN DIRECT OR FLEXURAL TENSION FOR SERVICEABILITY LIMIT STATES
(BASED ON 3.2.2 OF BS8007)

Top Mu = 119.6 KN·m M = 85.4 KN·m b = 1000 mm
D = 600 mm
Bot. Mu = 82.1 KN·m M = 58.6 KN·m d = 540 mm
fy = 130 N/mm²
fcu = 30 N/mm²

TOP
 $K = M / (f_{cu} \cdot b \cdot d^2)$ = 0.0098
 $Z1 = d \cdot [0.5 + \sqrt{(0.25 - K/0.9)}]$ = 534
 $Z2 = 0.95 \cdot d$ = 513
Z = 513
As = M / fy · Z = 1280.6 mm²

USE ; D 20 @ 200 (As = 1570 mm²)

BOTTOM
 $K = M / (f_{cu} \cdot b \cdot d^2)$ = 0.0067
 $Z1 = d \cdot [0.5 + \sqrt{(0.25 - K/0.9)}]$ = 536
 $Z2 = 0.95 \cdot d$ = 513
Z = 513
As = M / fy · Z = 878.69 mm²

USE ; D 20 @ 200 (As = 1570 mm²)

c) CHECK OF CRACK WIDTH (BASE ON APPENDIX A OF BS8007)

1) CRACK SPACING

Smax(T) = (fct/fb) · (σ/2ρ(T)) = 1063.5 mm
Smax(B) = (fct/fb) · (σ/2ρ(B)) = 426.8 mm

Where ; fct/fb = 0.67 ρ(T) = As/b · d(T) = 0.0063
ρ(B) = As/b · d(B) = 0.0157

2) CRACK WIDTHS ARISING FORM RESTRAINED SHRINKAGE AND HEAT OF HYDRATION MOVEMENT

Wmax(T) = Smax(T) · (α/2) · T1 = 0.13 < 0.2 mm
Wmax(B) = Smax(B) · (α/2) · T1 = 0.05 < 0.2 mm

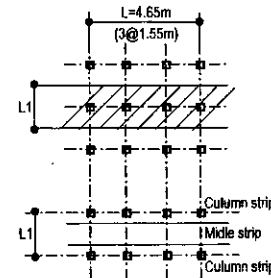
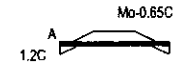
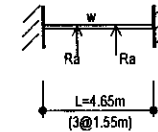
Where ; α : COEFFICIENT OF THERMAL EXPANSION OF MATURE CONCRETE

α = 1E-05
T1 = 25 from TABLE 4.2

2.2.3 CALCULATION OF FOOTING SLAB (FS90)
(AT+352.800)

①-①

1) DESIGN LOAD & FACTORED STRESS



• Design load
Sand : Ds = 2.4 m
F.Slab : Df = 0.9 m
• Factored Load w' = 1.4 · (w1+w2) = 145.9 KN/m
• Pile Reaction Ra = 525 KN/pile
Ra' = 1.4 · Ra = 735 KN/pile

• Factored stress
L = 4.65 m
C = (2/9 · Ra' · L) - (1/12 · w' · L²) = 496.6 KN · m
Mo = (1/3 · Ra' · L) - (1/8 · w' · L²) = 744.9 KN · m
MA = 1.3 · C = 645.6 KN · m
Mo-0.65C = 422.1 KN · m
Vu = Ra' - 1/2 · w' · L = 395.8 KN

• Column strip
L1 = 1.67 m
BOT: Mu1 = 0.375 · (1.3 · C) / (1/4 · L1) = 579.9 KN · m
TOP: Mu2 = 0.375 · (Mo-0.65 · C) / (1/4 · L1) = 379.2 KN · m

A.7-8

2) DESIGN OF SECTION

①-①

Revised all sheet

a) HORIZONTAL : BOT/END

$$\begin{aligned} M_u &= 579.9 \text{ KN}\cdot\text{m} \\ V_u &= 395.8 \text{ KN/m} \end{aligned}$$

$$\begin{aligned} b &= 1000 \text{ mm} & f_y &= 365 \text{ N/mm}^2 \\ D &= 900 \text{ mm} & f_{cu} &= 30 \text{ N/mm}^2 \\ d &= 840 \text{ mm} \end{aligned}$$

[REQUIRED RE-BAR]

$$\begin{aligned} K &= M_u / (f_{cu} \cdot b \cdot d^2) &= 0.027 < 0.156 \\ Z_1 &= d \cdot [0.5 + \sqrt{(0.25 - K/0.9)}] &= 814 \\ Z_2 &= 0.95 \cdot d &= 798 \\ Z &= 798 \\ A_s 1 &= M_u / (0.95 \cdot f_y \cdot Z) &= 2096 \text{ mm}^2 \\ A_{smin} &= 0.0013 \cdot b \cdot D &= 1170 \text{ mm}^2 \end{aligned}$$

$$\text{USE ; } D 25 \text{ @ } 100 \text{ (} A_s = 4910 \text{ mm}^2 \text{)}$$

[CHECK OF SHEAR STRESS]

$$\begin{aligned} v &= V_u / (b \cdot d) &= 0.47 < 4.4 \text{ N/mm}^2 \text{ OK} \\ (100 \cdot A_s) / (b \cdot d) &= 0.58 < 3.0 \\ 400/d &= 0.5 < 1.0 \rightarrow 1.0 \end{aligned}$$

$$\begin{aligned} v_c &= 0.79 \cdot \{ (100 \cdot A_s) / (b \cdot d) \}^{1/3} \cdot (400/d)^{1/4} \cdot (1/1.25) \cdot (f_{cu}/25)^{1/3} \\ &= 0.56 \text{ N/mm}^2 > v = 0.47 \text{ N/mm}^2 \text{ OK} \end{aligned}$$

b) HORIZONTAL : TOP/CENT

$$\begin{aligned} M_u &= 379.2 \text{ KN}\cdot\text{m} \\ V_u &= 395.8 \text{ KN/m} \end{aligned}$$

$$\begin{aligned} b &= 1000 \text{ mm} & f_y &= 365 \text{ N/mm}^2 \\ D &= 900 \text{ mm} & f_{cu} &= 30 \text{ N/mm}^2 \\ d &= 820 \text{ mm} \end{aligned}$$

[REQUIRED RE-BAR]

$$\begin{aligned} K &= M_u / (f_{cu} \cdot b \cdot d^2) &= 0.019 < 0.156 \\ Z_1 &= d \cdot [0.5 + \sqrt{(0.25 - K/0.9)}] &= 803 \\ Z_2 &= 0.95 \cdot d &= 779 \\ Z &= 779 \\ A_s 1 &= M_u / (0.95 \cdot f_y \cdot Z) &= 1404 \text{ mm}^2 \\ A_{smin} &= 0.0013 \cdot b \cdot D &= 1170 \text{ mm}^2 \end{aligned}$$

$$\text{USE ; } D 22 \text{ @ } 100 \text{ (} A_s = 3800 \text{ mm}^2 \text{)}$$

[CHECK OF SHEAR STRESS]

$$\begin{aligned} v &= V_u / (b \cdot d) &= 0.48 < 4.4 \text{ N/mm}^2 \text{ OK} \\ (100 \cdot A_s) / (b \cdot d) &= 0.46 < 3.0 \\ 400/d &= 0.5 < 1.0 \rightarrow 1.0 \end{aligned}$$

$$\begin{aligned} v_c &= 0.79 \cdot \{ (100 \cdot A_s) / (b \cdot d) \}^{1/3} \cdot (400/d)^{1/4} \cdot (1/1.25) \cdot (f_{cu}/25)^{1/3} \\ &= 0.52 \text{ N/mm}^2 > v = 0.48 \text{ N/mm}^2 \text{ OK} \end{aligned}$$

3) CHECK OF CRACKING
FOOTING SLAB (FS90)

①-①

a) CHECK OF MINIMUM REINFORCEMENT (BASED ON 2.6.2.3 OF BS8007)

SECTION	b = 1000 mm	SURFACE ZONE :	d(T) = 250 mm
	D = 900 mm		d(B) = 100 mm
	d = 740 mm		

$$\text{Min. } \rho = 0.0035 \text{ (} 0.35 \% \text{)}$$

$$\begin{aligned} A_s (\text{top}) &= \rho \cdot b \cdot d(T) = 875 \text{ mm}^2 \\ A_s (\text{bot}) &= \rho \cdot b \cdot d(B) = 350 \text{ mm}^2 \end{aligned}$$

$$\begin{aligned} \text{USE ; } & \text{TOP } D 22 \text{ @ } 100 \text{ (} A_s = 3800 \text{ mm}^2 \text{)} \\ \text{USE ; } & \text{BOTTOM } D 25 \text{ @ } 100 \text{ (} A_s = 4910 \text{ mm}^2 \text{)} \end{aligned}$$

b) CHECK OF STEEL STRESS IN DIRECT OR FLEXURAL TENSION FOR SERVICEABILITY LIMIT STATES
(BASED ON 3.2.2 OF BS8007)

Top	$M_u = 379.2 \text{ KN}\cdot\text{m}$	$M = 270.9 \text{ KN}\cdot\text{m}$	b = 1000 mm
Bot.	$M_u = 579.9 \text{ KN}\cdot\text{m}$	$M = 414.2 \text{ KN}\cdot\text{m}$	D = 900 mm
			d = 840 mm
			$f_y = 130 \text{ N/mm}^2$
			$f_{cu} = 30 \text{ N/mm}^2$

TOP

$$\begin{aligned} K &= M / (f_{cu} \cdot b \cdot d^2) &= 0.0128 \\ Z_1 &= d \cdot [0.5 + \sqrt{(0.25 - K/0.9)}] &= 828 \\ Z_2 &= 0.95 \cdot d &= 798 \\ Z &= 798 \\ A_s &= M / (f_y \cdot Z) &= 2611.3 \text{ mm}^2 \end{aligned}$$

$$\text{USE ; } D 22 \text{ @ } 100 \text{ (} A_s = 3800 \text{ mm}^2 \text{)}$$

BOTTOM

$$\begin{aligned} K &= M / (f_{cu} \cdot b \cdot d^2) &= 0.0196 \\ Z_1 &= d \cdot [0.5 + \sqrt{(0.25 - K/0.9)}] &= 821 \\ Z_2 &= 0.95 \cdot d &= 798 \\ Z &= 798 \\ A_s &= M / (f_y \cdot Z) &= 3992.7 \text{ mm}^2 \end{aligned}$$

$$\text{USE ; } D 25 \text{ @ } 100 \text{ (} A_s = 4910 \text{ mm}^2 \text{)}$$

c) CHECK OF CRACK WIDTH (BASE ON APPENDIX A OF BS8007)

1) CRACK SPACING

$$\begin{aligned} s_{max}(T) &= (f_{ct}/f_b) \cdot (\phi/2\rho(T)) &= 484.87 \text{ mm} \\ s_{max}(B) &= (f_{ct}/f_b) \cdot (\phi/2\rho(B)) &= 170.6 \text{ mm} \end{aligned}$$

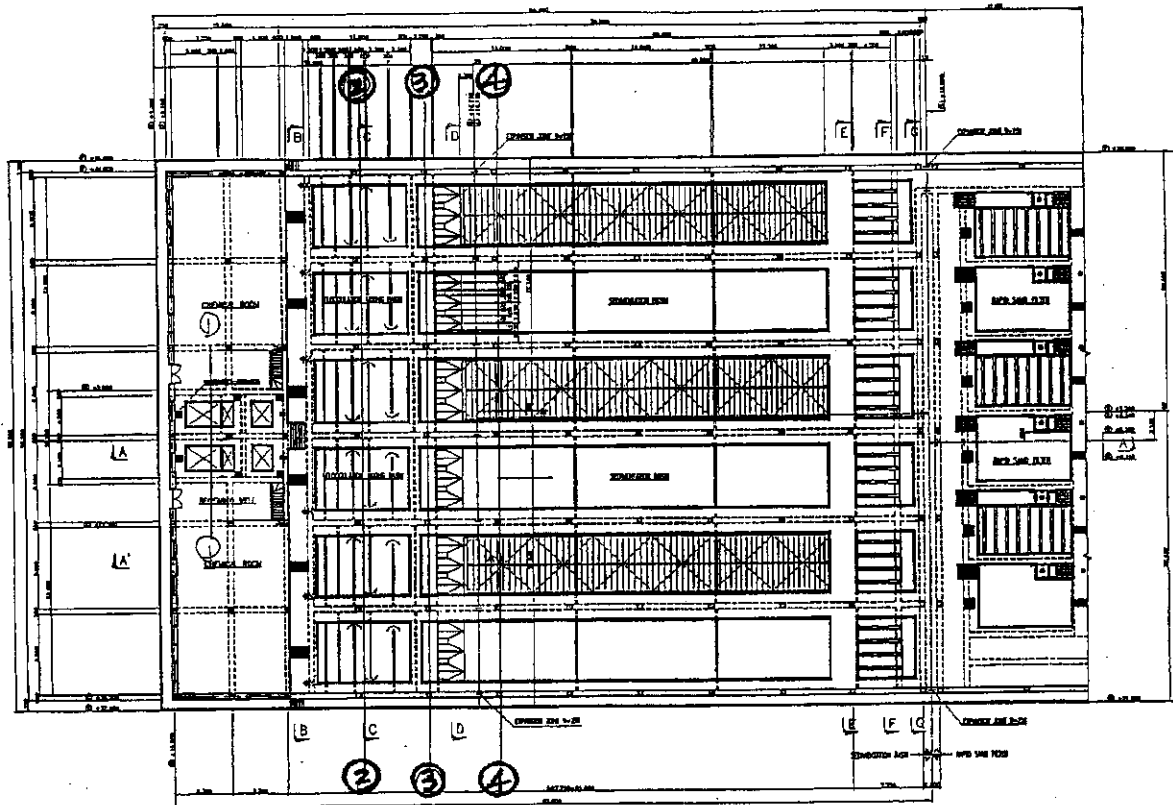
$$\begin{aligned} \text{Where ; } f_{ct}/f_b &= 0.67 & \rho(T) &= A_s/b \cdot d(T) = 0.0152 \\ & & \rho(B) &= A_s/b \cdot d(B) = 0.0491 \end{aligned}$$

2) CRACK WIDTHS ARISING FROM RESTRAINED SHRINKAGE AND HEAT OF HYDRATION MOVEMENT

$$\begin{aligned} w_{max}(T) &= s_{max}(T) \cdot (\alpha/2) \cdot T_1 &= 0.06 < 0.2 \text{ mm} \\ w_{max}(B) &= s_{max}(B) \cdot (\alpha/2) \cdot T_1 &= 0.02 < 0.2 \text{ mm} \end{aligned}$$

Where ; α : COEFFICIENT OF THERMAL EXPANSION OF MATURE CONCRETE

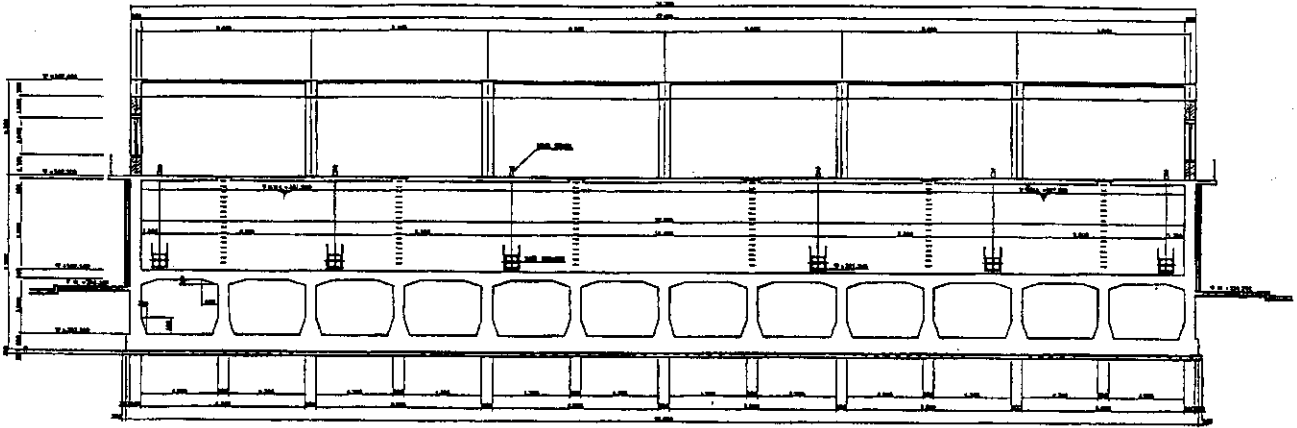
$$\begin{aligned} \alpha &= 1E-05 \\ T_1 &= 25 \text{ from TABLE 4.2} \end{aligned}$$



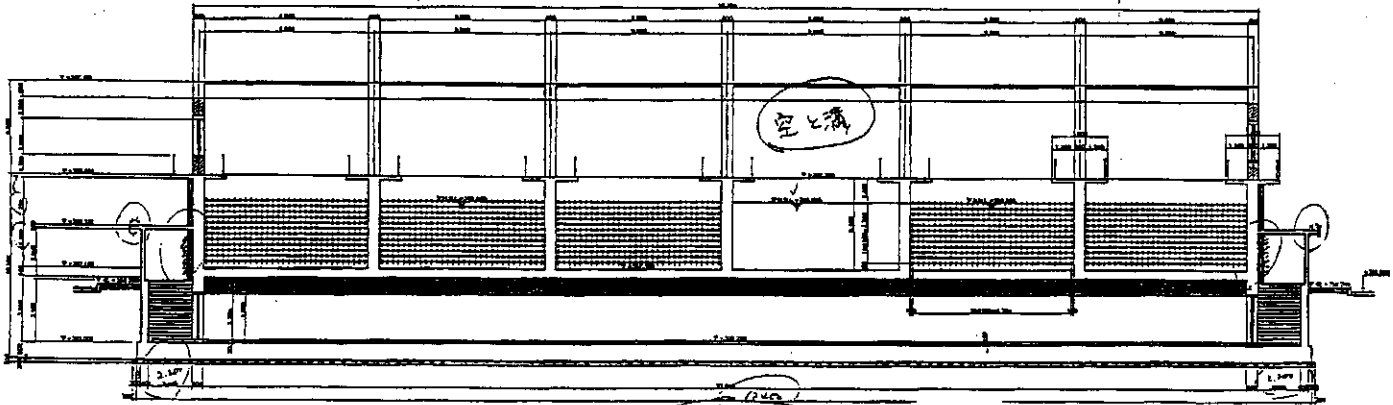
TOP PLAN
SCALE 1:200

3. SEDIMENTATION BASIN

3. ОТСТОЙНИК



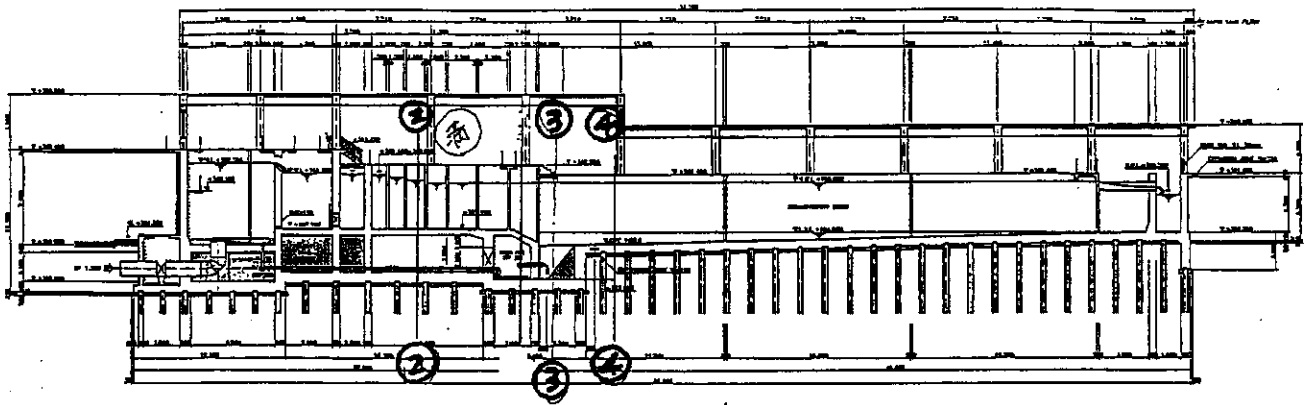
SECTION B-B



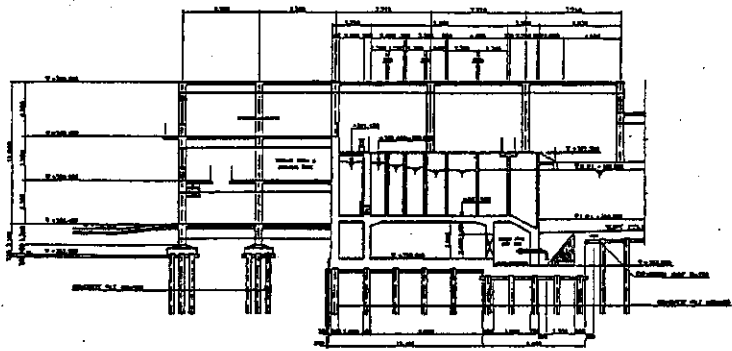
SECTION C-C

② - ②

3-4



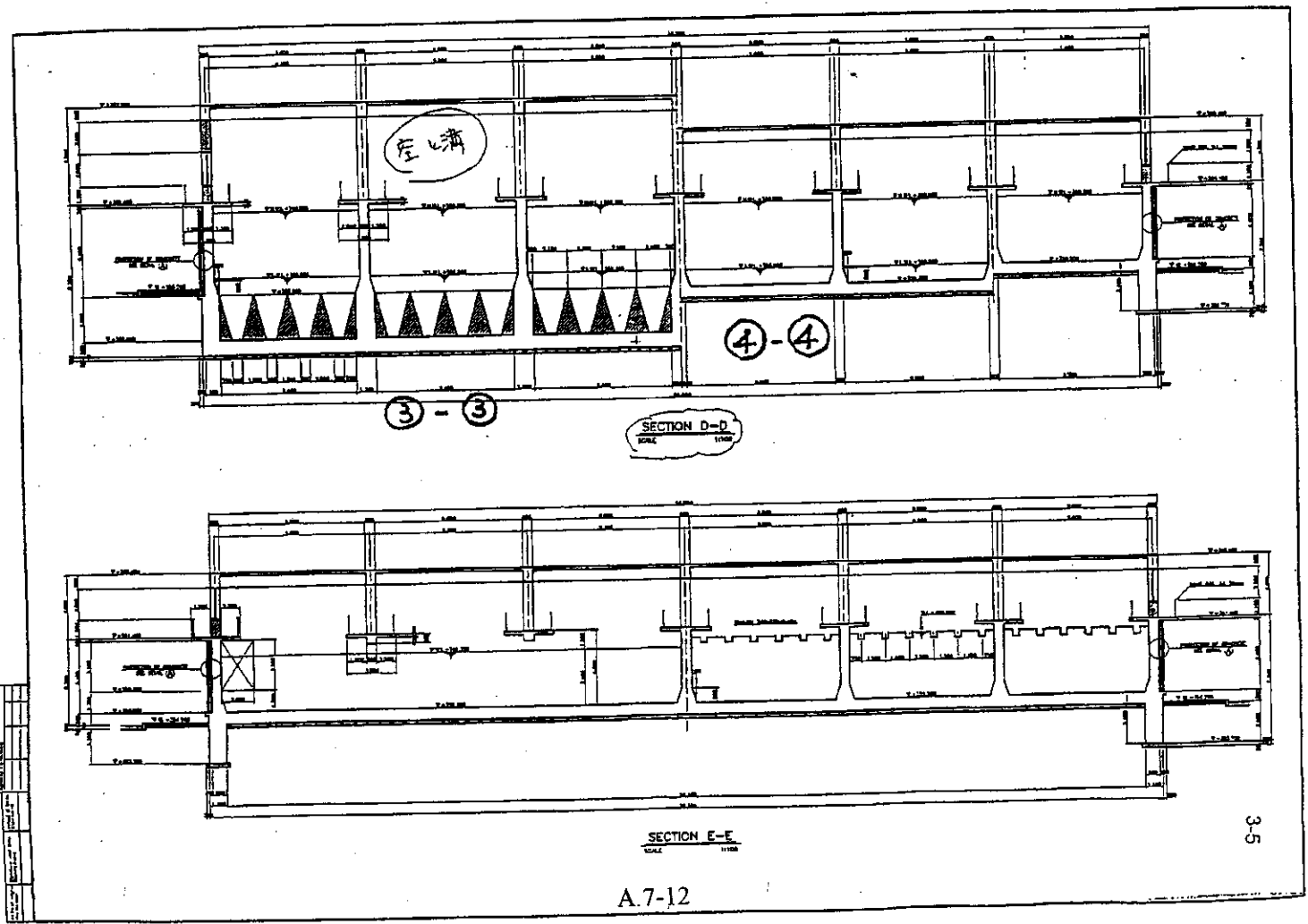
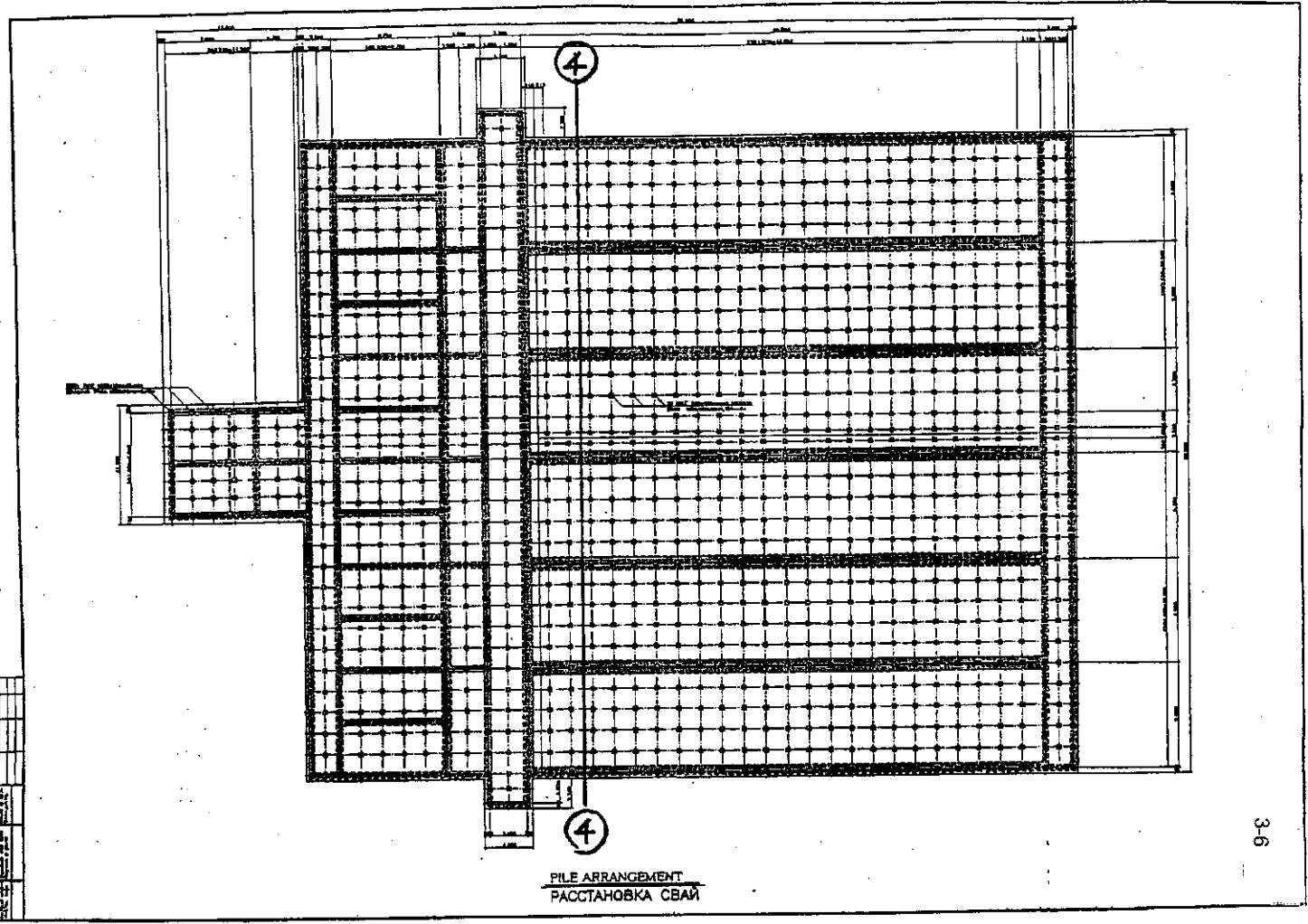
SECTION A-A



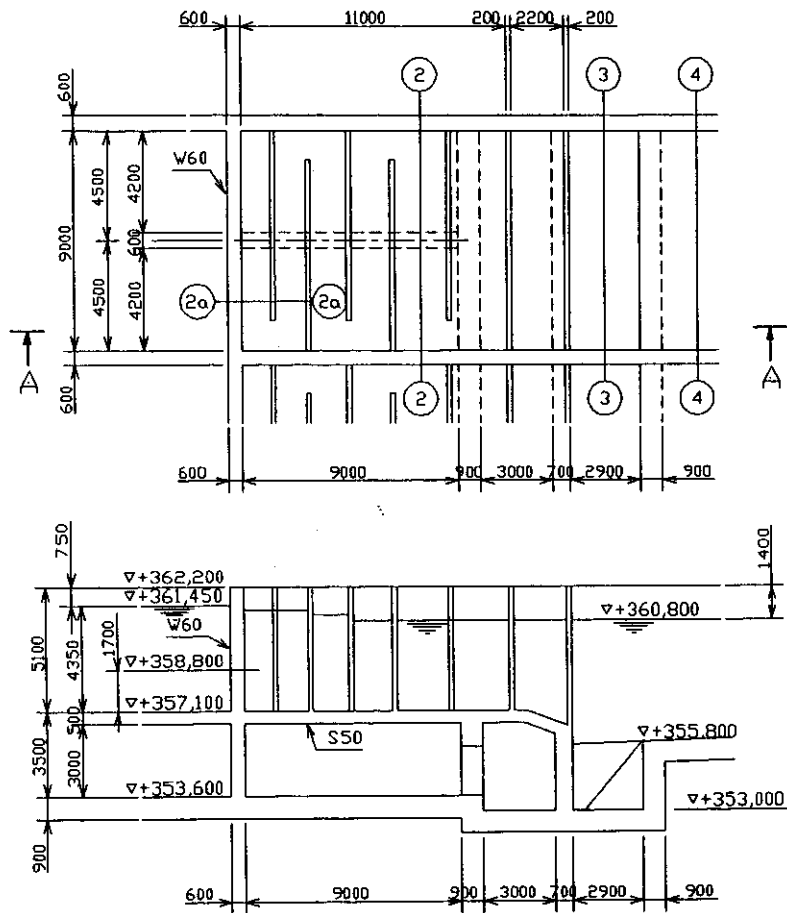
SECTION A'-A'

A7-11

3-3



3.2 DESIGN OF WALL & SLAB ② - ②
 3.2 ПРОЕКТИРОВАНИЕ СТЕНЫ И ПЛИТЫ

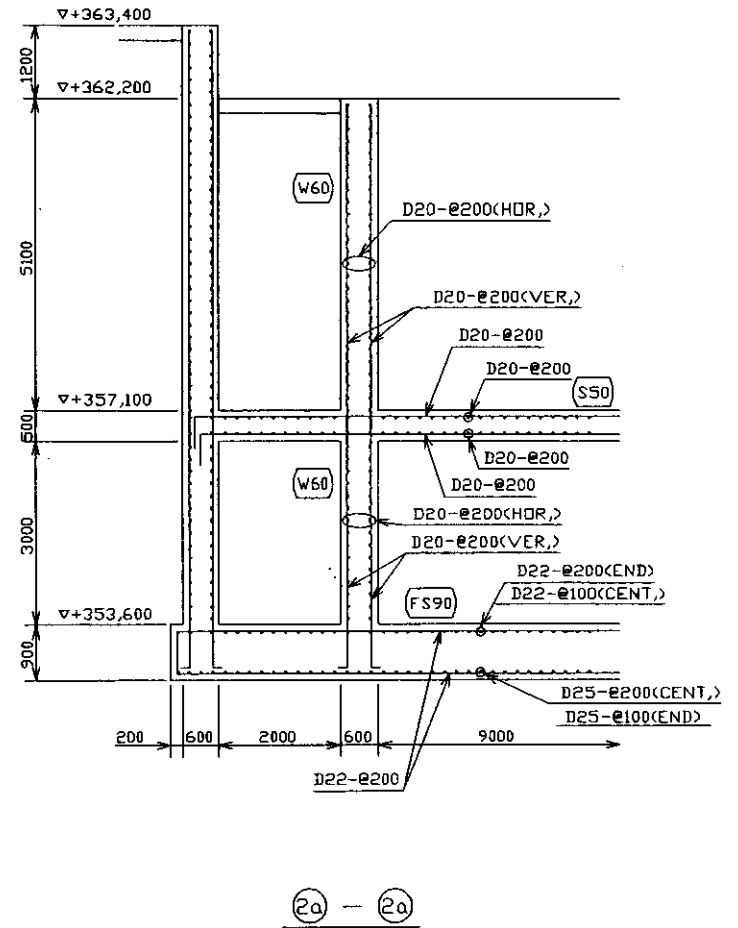


SECTION A-A/РАЗРЕЗ A-A

S=1/200

③ - ③ SAME AS ④ - ④
 ③ - ③ ТАК ЖЕ КАК ④ - ④

A.7-13

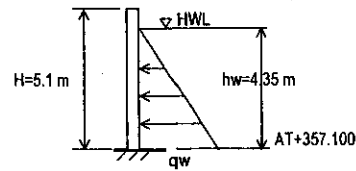


CALCULATION OF (FS90) SAME AS ①-① SECTION REFER TO PAGE 2-3
 РАСЧЕТ (FS90) ТАК ЖЕ, КАК РАЗРЕЗ ①-① СМ. СТР. 2-3 S=1/80

3.2.1 CALCULATION OF WALL (W60)
(AT+357.100)

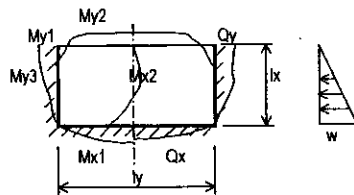
②-②

1) DESIGN LOAD (Water Pressure)



$$\begin{aligned} hw &= 4.35 \text{ m} \\ yw &= 10.0 \text{ KN/m}^3 \\ qw &= yw \cdot hw = 43.5 \text{ KN/m}^2/\text{m} \end{aligned}$$

2) FACTORED STRESS (W60)



$$\begin{aligned} lx &= 4.35 \text{ m} \\ ly &= 9.00 \text{ m} \\ \lambda &= 2.07 \\ w &= qw = 43.5 \text{ KN/m}^2/\text{m} \\ \text{Factored Load} \\ w' &= 1.4 \cdot w = 60.90 \text{ KN/m}^2/\text{m} \\ w' \cdot lx^2 &= 1152.4 \\ w' \cdot lx &= 264.9 \end{aligned}$$

SEE Fig.1

$$\begin{aligned} Mx1 &= 0.088 \times 1152.4 = 101.4 \text{ (KN} \cdot \text{m/m)} \\ Mx2 &= 0.010 \times 1152.4 = 11.5 \\ My1 &= 0.068 \times 1152.4 = 78.4 \\ My2 &= 0.027 \times 1152.4 = 31.1 \\ My3 &= 0.068 \times 1152.4 = 78.4 \\ Qx &= 0.47 \times 264.9 = 124.5 \text{ (KN/m)} \\ Qy &= 0.26 \times 264.9 = 68.9 \end{aligned}$$

A.7-14

3) DESIGN OF SECTION

a) VERTICAL : BOT (AT+357.100)

$$\begin{aligned} Mu &= 101.4 \text{ KN} \cdot \text{m} \\ Vu &= 124.5 \text{ KN/m} \end{aligned}$$

$$\begin{aligned} b &= 1000 \text{ mm} & fy &= 365 \text{ N/mm}^2 \\ D &= 600 \text{ mm} & fcu &= 30 \text{ N/mm}^2 \\ d &= 540 \text{ mm} \end{aligned}$$

[REQUIRED RE-BAR]

$$\begin{aligned} K &= Mu / (fcu \cdot b \cdot d^2) & &= 0.012 < 0.156 \\ Z1 &= d \cdot [0.5 + \sqrt{(0.25 - K/0.9)}] & &= 533 \\ Z2 &= 0.95 \cdot d & &= 513 \\ Z & & &= 513 \\ As1 &= Mu / (0.95 \cdot fy \cdot Z) & &= 570 \text{ mm}^2 \\ Asmin &= 0.0013 \cdot b \cdot D & &= 780 \text{ mm}^2 \end{aligned}$$

USE ; D 20 @ 200 (As = 1570 mm²)

[CHECK OF SHEAR STRESS]

$$\begin{aligned} V &= Vu / (b \cdot d) & &= 0.23 < 4.4 \text{ N/mm}^2 \text{ OK} \\ (100 \cdot As) / (b \cdot d) & & &= 0.29 < 3.0 \\ 400/d & & &= 0.7 < 1.0 \rightarrow 1.0 \end{aligned}$$

$$\begin{aligned} Vc &= 0.79 \cdot \{ (100 \cdot As) / (b \cdot d) \}^{1/3} \cdot (400/d)^{1/4} \cdot (1/1.25) \cdot (fcu/25)^{1/3} \\ &= 0.45 \text{ N/mm}^2 > V = 0.23 \text{ N/mm}^2 \text{ OK} \end{aligned}$$

b) HORIZONTAL : TOP/END

$$\begin{aligned} Mu &= 78.4 \text{ KN} \cdot \text{m} \\ Vu &= 68.9 \text{ KN/m} \end{aligned}$$

$$\begin{aligned} b &= 1000 \text{ mm} & fy &= 365 \text{ N/mm}^2 \\ D &= 600 \text{ mm} & fcu &= 30 \text{ N/mm}^2 \\ d &= 520 \text{ mm} \end{aligned}$$

[REQUIRED RE-BAR]

$$\begin{aligned} K &= Mu / (fcu \cdot b \cdot d^2) & &= 0.010 < 0.156 \\ Z1 &= d \cdot [0.5 + \sqrt{(0.25 - K/0.9)}] & &= 514 \\ Z2 &= 0.95 \cdot d & &= 494 \\ Z & & &= 494 \\ As1 &= Mu / (0.95 \cdot fy \cdot Z) & &= 457 \text{ mm}^2 \\ Asmin &= 0.0013 \cdot b \cdot D & &= 780 \text{ mm}^2 \end{aligned}$$

USE ; D 20 @ 200 (As = 1570 mm²)

[CHECK OF SHEAR STRESS]

$$\begin{aligned} V &= Vu / (b \cdot d) & &= 0.13 < 4.4 \text{ N/mm}^2 \text{ OK} \\ (100 \cdot As) / (b \cdot d) & & &= 0.30 < 3.0 \\ 400/d & & &= 0.8 < 1.0 \rightarrow 1.0 \end{aligned}$$

$$\begin{aligned} Vc &= 0.79 \cdot \{ (100 \cdot As) / (b \cdot d) \}^{1/3} \cdot (400/d)^{1/4} \cdot (1/1.25) \cdot (fcu/25)^{1/3} \\ &= 0.45 \text{ N/mm}^2 > V = 0.13 \text{ N/mm}^2 \text{ OK} \end{aligned}$$

3) CHECK OF CRACKING
SLAB (S50)

②-②

3-14

a) CHECK OF MINIMUM REINFORCEMENT (BASED ON 2.6.2.3 OF BS8007)

SECTION	b = 1000 mm	SURFACE ZONE:	d(T) = 250 mm
	D = 500 mm		d(B) = 100 mm
	d = 440 mm		

Min. $\rho = 0.0035$ (0.35 %)

As (top) = $\rho \cdot b \cdot d(T) = 875 \text{ mm}^2$
As (bot) = $\rho \cdot b \cdot d(B) = 350 \text{ mm}^2$

USE:	TOP	D 20	@ 200	(As = 1570 mm ²)
USE:	BOTTOM	D 20	@ 200	(As = 1570 mm ²)

b) CHECK OF STEEL STRESS IN DIRECT OR FLEXURAL TENSION FOR SERVICEABILITY LIMIT STATES
(BASED ON 3.2.2 OF BS8007)

Top	Mu = 113.8 KN·m	M = 81.3 KN·m	b = 1000 mm
Bot.	Mu = 78.1 KN·m	M = 55.8 KN·m	D = 500 mm
			d = 440 mm
			fy = 130 N/mm ²
			fcu = 30 N/mm ²

TOP

$$K = M / (f_{cu} \cdot b \cdot d^2) = 0.014$$

$$Z_1 = d \cdot [0.5 + \sqrt{ (0.25 - K/0.9) }] = 433$$

$$Z_2 = 0.95 \cdot d = 418$$

$$Z = 418$$

$$A_s = M / f_y \cdot Z = 1496.1 \text{ mm}^2$$

USE:	D 20	@ 200	(As = 1570 mm ²)
------	------	-------	------------------------------

BOTTOM

$$K = M / (f_{cu} \cdot b \cdot d^2) = 0.0096$$

$$Z_1 = d \cdot [0.5 + \sqrt{ (0.25 - K/0.9) }] = 435$$

$$Z_2 = 0.95 \cdot d = 418$$

$$Z = 418$$

$$A_s = M / f_y \cdot Z = 1026.9 \text{ mm}^2$$

USE:	D 20	@ 200	(As = 1570 mm ²)
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c) CHECK OF CRACK WIDTH (BASE ON APPENDIX A OF BS8007)

1) CRACK SPACING

S_{max(T)} = (f_{ct}/f_b) · (σ /2 ρ (T)) = 1063.5 mm
S_{max(B)} = (f_{ct}/f_b) · (σ /2 ρ (B)) = 426.8 mm

Where ; f_{ct}/f_b = 0.67 ρ (T) = A_s/b · d(T) = 0.0063
 ρ (B) = A_s/b · d(B) = 0.0157

2) CRACK WIDTHS ARISING FORM RESTRAINED SHRINKAGE AND HEAT OF HYDRATION MOVEMENT

W_{max(T)} = S_{max(T)} · (α /2) · T₁ = 0.13 < 0.2 mm
W_{max(B)} = S_{max(B)} · (α /2) · T₁ = 0.05 < 0.2 mm

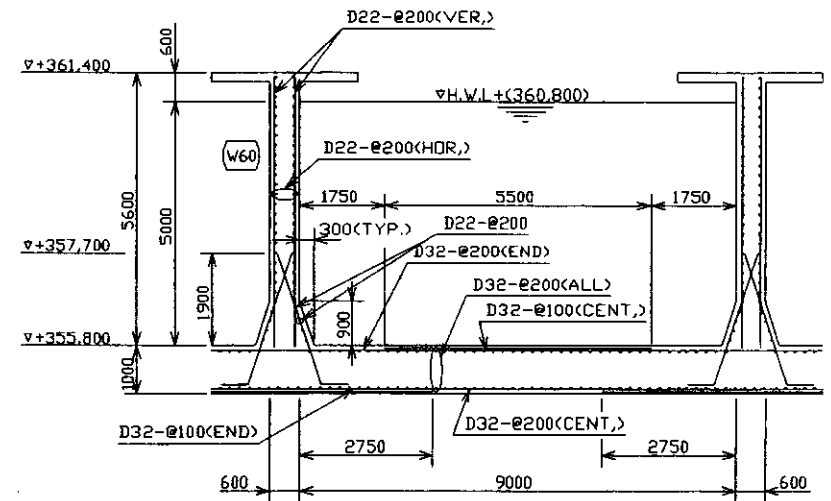
Where ; α : COEFFICIENT OF THERMAL EXPANSION OF MATURE CONCRETE

α = 1E-05
T₁ = 25 from TABLE 4.2

A.7-16

3-15

3.3 DESIGN OF WALL & SLAB ④ - ④
3.3 ПРОЕКТИРОВАНИЕ СТЕНЫ И ПЛИТЫ

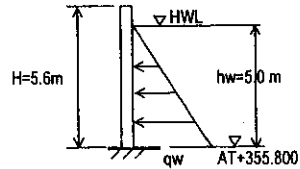


④ - ④
S=1/100

3.3.1 CALCULATION OF WALL (W60) (AT+355.800)

④-④

1) DESIGN LOAD & FACTORED STRESS



$$\begin{aligned} H &= 5.6 \text{ m} \\ h_w &= 5.00 \text{ m} \\ y_w &= 10.0 \text{ KN/m}^3 \end{aligned}$$

$$\text{• Design load} \\ q_w = y_w \cdot h_w = 10.0 \times 5.00 = 50.0 \text{ (KN/m}^2\text{)}$$

$$\text{• Factored Load} \\ w' = 1.4 \cdot q_w = 1.4 \times 50.0 = 70.00 \text{ (KN/m}^2\text{)}$$

$$\begin{cases} M_u = 1/6 \cdot w' \cdot h_w^2 & = 291.7 \text{ (KN} \cdot \text{m/m)} \\ V_u = 1/2 \cdot w' \cdot h_w & = 175.0 \text{ (KN/m)} \end{cases}$$

2) DESIGN OF SECTION

$$\begin{aligned} M_u &= 291.7 \text{ KN} \cdot \text{m} \\ V_u &= 175.0 \text{ KN/m} \end{aligned}$$

$$\begin{aligned} b &= 1000 \text{ mm} & f_y &= 365 \text{ N/mm}^2 \\ D &= 1200 \text{ mm} & f_{cu} &= 30 \text{ N/mm}^2 \\ d &= 1140 \text{ mm} \end{aligned}$$

[REQUIRED RE-BAR]

$$\begin{aligned} K &= M_u / (f_{cu} \cdot b \cdot d^2) & = 0.007 < 0.156 \\ Z_1 &= d \cdot [0.5 + \sqrt{(0.25 - K/0.9)}] & = 1130 \\ Z_2 &= 0.95 \cdot d & = 1083 \\ Z &= & 1083 \\ A_s 1 &= M_u / (0.95 \cdot f_y \cdot Z) & = 777 \text{ mm}^2 \\ A_{smin} &= 0.0013 \cdot b \cdot D & = 1560 \text{ mm}^2 \end{aligned}$$

$$\text{USE ; } D 22 \text{ @ } 200 \text{ (} A_s = 1900 \text{ mm}^2\text{)}$$

[CHECK OF SHEAR STRESS]

$$\begin{aligned} V &= V_u / (b \cdot d) & = 0.15 < 4.4 \text{ N/mm}^2 \text{ OK} \\ (100 \cdot A_s) / (b \cdot d) & & = 0.17 < 3.0 \\ 400/d & & = 0.4 < 1.0 \rightarrow 1.0 \end{aligned}$$

$$\begin{aligned} V_c &= 0.79 \cdot \{ (100 \cdot A_s) / (b \cdot d) \}^{1/3} \cdot (400/d)^{1/4} \cdot (1/1.25) \cdot (f_{cu}/25)^{1/3} \\ &= 0.37 \text{ N/mm}^2 > V = 0.15 \text{ N/mm}^2 \text{ OK} \end{aligned}$$

3) CHECK OF CRACKING W60 (VERTICAL:BOT.)

④-④

a) CHECK OF MINIMUM REINFORCEMENT (BASED ON 2.6.2.3 OF BS8007)

$$\begin{aligned} \text{SECTION} \quad b &= 1000 \text{ mm} & \text{SURFACE ZONE:} \quad d_1 &= 250 \text{ mm} \\ D &= 1200 \text{ mm} \\ d_v &= 1140 \text{ mm} \\ d_h &= 1120 \text{ mm} \end{aligned}$$

$$\text{Min. } \rho = 0.0035 \text{ (} 0.35 \% \text{)}$$

$$\begin{aligned} A_s \text{ (ver.)} &= \rho \cdot b \cdot d_1 = 875 \text{ mm}^2 \\ A_s \text{ (hor.)} &= \rho \cdot b \cdot d_1 = 875 \text{ mm}^2 \end{aligned}$$

$$\begin{aligned} \text{USE ; VERTICAL} & \quad D 22 \quad \text{@ } 200 \quad (A_s = 1900 \text{ mm}^2) \\ \text{USE ; HORIZONTAL} & \quad D 22 \quad \text{@ } 200 \quad (A_s = 1900 \text{ mm}^2) \end{aligned}$$

b) CHECK OF STEEL STRESS IN DIRECT OR FLEXURAL TENSION FOR SERVICEABILITY LIMIT STATES (BASED ON 3.2.2 OF BS8007)

$$\begin{aligned} \text{Ver.} \quad M_u &= 291.7 \text{ KN} \cdot \text{m} & M &= 208.4 \text{ KN} \cdot \text{m} & b &= 1000 \text{ mm} \\ \text{Hor.} \quad M_u &= 0.0 \text{ KN} \cdot \text{m} & M &= 0.0 \text{ KN} \cdot \text{m} & D &= 1200 \text{ mm} \\ & & & & d_v &= 1140 \text{ mm} \\ & & & & d_h &= 1120 \text{ mm} \\ & & & & f_y &= 365 \text{ N/mm}^2 \\ & & & & f_{cu} &= 30 \text{ N/mm}^2 \end{aligned}$$

VERTICAL

$$\begin{aligned} K &= M / (f_{cu} \cdot b \cdot d_v^2) & = 0.0053 \\ Z_1 &= d_v \cdot [0.5 + \sqrt{(0.25 - K/0.9)}] & = 1133 \\ Z_2 &= 0.95 \cdot d_v & = 1083 \\ Z &= & 1083 \\ A_s &= M / f_y \cdot Z & = 1480.2 \text{ mm}^2 \end{aligned}$$

$$\text{USE ; } D 22 \text{ @ } 200 \text{ (} A_s = 1900 \text{ mm}^2\text{)}$$

HORIZONTAL

$$\begin{aligned} K &= M / (f_{cu} \cdot b \cdot d_h^2) & = 0 \\ Z_1 &= d_h \cdot [0.5 + \sqrt{(0.25 - K/0.9)}] & = 0 \\ Z_2 &= 0.95 \cdot d_h & = 0 \\ Z &= & 0 \\ A_s &= M / f_y \cdot Z & = 0 \text{ mm}^2 \end{aligned}$$

$$\text{USE ; } D 22 \text{ @ } 200 \text{ (} A_s = 1900 \text{ mm}^2\text{)}$$

c) CHECK OF CRACK WIDTH (BASE ON APPENDIX A OF BS8007)

1) CRACK SPACING

$$\begin{aligned} S_{max}(V) &= (f_{ct}/f_b) \cdot (\sigma/V) & = 969.74 \text{ mm} \\ S_{max}(H) &= (f_{ct}/f_b) \cdot (\sigma/H) & = 969.7 \text{ mm} \end{aligned}$$

$$\begin{aligned} \text{Where ; } f_{ct}/f_b &= 0.67 & \rho(V) &= A_s/b \cdot d_1 = 0.0076 \\ & & \rho(H) &= A_s/b \cdot d_1 = 0.0076 \end{aligned}$$

2) CRACK WIDTHS ARISING FROM RESTRAINED SHRINKAGE AND HEAT OF HYDRATION MOVEMENT

$$\begin{aligned} W_{max}(V) &= S_{max}(V) \cdot (\alpha/2) \cdot T_1 & = 0.20 < 0.2 \text{ mm} \\ W_{max}(H) &= S_{max}(H) \cdot (\alpha/2) \cdot T_1 & = 0.20 < 0.2 \text{ mm} \end{aligned}$$

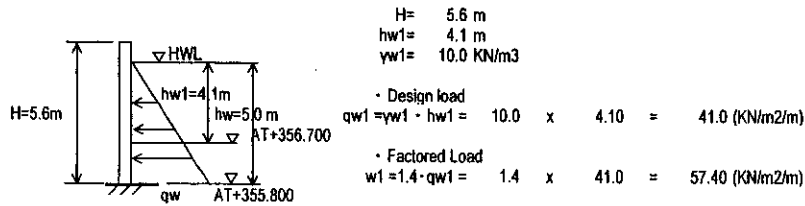
Where ; α : COEFFICIENT OF THERMAL EXPANSION OF MATURE CONCRETE

$$\begin{aligned} \alpha &= 1E-05 \\ T_1 &= 42 \text{ from TABLE 4.2} \end{aligned}$$

3.3.2 CALCULATION OF WALL (W60) (AT+356.700)

④-④

1) DESIGN LOAD & FACTORED STRESS



$$\begin{aligned} H &= 5.6 \text{ m} \\ hw1 &= 4.1 \text{ m} \\ qw1 &= 10.0 \text{ KN/m}^3 \end{aligned}$$

$$\begin{aligned} \text{• Design load} \\ qw1 &= qw1 \cdot hw1 = 10.0 \times 4.1 = 41.0 \text{ (KN/m}^2\text{/m)} \end{aligned}$$

$$\begin{aligned} \text{• Factored Load} \\ w1 &= 1.4 \cdot qw1 = 1.4 \times 41.0 = 57.40 \text{ (KN/m}^2\text{/m)} \end{aligned}$$

$$\begin{aligned} \left[\begin{aligned} Mu &= 1/6 \cdot w1 \cdot hw1^2 = 160.8 \text{ (KN} \cdot \text{m/m)} \\ Vu &= 1/2 \cdot w1 \cdot hw1 = 117.7 \text{ (KN/m)} \end{aligned} \right. \end{aligned}$$

2) DESIGN OF SECTION

$$\begin{aligned} Mu &= 160.8 \text{ KN} \cdot \text{m} \\ Vu &= 117.7 \text{ KN/m} \end{aligned}$$

$$\begin{aligned} b &= 1000 \text{ mm} & fy &= 365 \text{ N/mm}^2 \\ D &= 600 \text{ mm} & fcu &= 30 \text{ N/mm}^2 \\ d &= 540 \text{ mm} \end{aligned}$$

[REQUIRED RE-BAR]

$$\begin{aligned} K &= Mu / (fcu \cdot b \cdot d^2) = 0.018 < 0.156 \\ Z1 &= d \cdot (0.5 + \sqrt{(0.25 - K/0.9)}) = 529 \\ Z2 &= 0.95 \cdot d = 513 \\ Z &= 513 \\ As1 &= Mu / (0.95 \cdot fy \cdot Z) = 904 \text{ mm}^2 \\ Asmin &= 0.0013 \cdot b \cdot D = 780 \text{ mm}^2 \end{aligned}$$

$$\text{USE ; D 22 @ 200 (As = 1900 mm}^2\text{)}$$

[CHECK OF SHEAR STRESS]

$$\begin{aligned} V &= Vu / (b \cdot d) = 0.22 < 4.4 \text{ N/mm}^2 \text{ OK} \\ (100 \cdot As) / (b \cdot d) &= 0.35 < 3.0 \\ 400/d &= 0.7 < 1.0 \rightarrow 1.0 \end{aligned}$$

$$\begin{aligned} Vc &= 0.79 \cdot ((100 \cdot As) / (b \cdot d))^{1/3} \cdot (400/d)^{1/4} \cdot (1/1.25) \cdot (fcu/25)^{1/3} \\ &= 0.47 \text{ N/mm}^2 > V = 0.22 \text{ N/mm}^2 \text{ OK} \end{aligned}$$

3) CHECK OF CRACKING W60 (VERTICAL TOP)

④-④

a) CHECK OF MINIMUM REINFORCEMENT (BASED ON 2.6.2.3 OF BS8007)

$$\begin{aligned} \text{SECTION} \quad b &= 1000 \text{ mm} & \text{SURFACE ZONE:} & & d1 &= 250 \text{ mm} \\ D &= 600 \text{ mm} \\ dv &= 540 \text{ mm} \\ dh &= 520 \text{ mm} \end{aligned}$$

$$\text{Min. } \rho = 0.0035 \text{ (0.35 \%)}$$

$$\begin{aligned} As \text{ (ver.)} &= \rho \cdot b \cdot d1 = 875 \text{ mm}^2 \\ As \text{ (her.)} &= \rho \cdot b \cdot d1 = 875 \text{ mm}^2 \end{aligned}$$

$$\begin{aligned} \text{USE ; VERTICAL} & \text{ D 22 @ 200 (As = 1900 mm}^2\text{)} \\ \text{USE ; HORIZONTAL} & \text{ D 22 @ 200 (As = 1900 mm}^2\text{)} \end{aligned}$$

b) CHECK OF STEEL STRESS IN DIRECT OR FLEXURAL TENSION FOR SERVICEABILITY LIMIT STATES (BASED ON 3.2.2 OF BS8007)

$$\begin{aligned} \text{Ver.} \quad Mu &= 160.8 \text{ KN} \cdot \text{m} & M &= 114.9 \text{ KN} \cdot \text{m} & b &= 1000 \text{ mm} \\ \text{Hor.} \quad Mu &= 0.0 \text{ KN} \cdot \text{m} & M &= 0.0 \text{ KN} \cdot \text{m} & D &= 600 \text{ mm} \\ & & & & dv &= 540 \text{ mm} \\ & & & & dh &= 520 \text{ mm} \\ & & & & fy &= 130 \text{ N/mm}^2 \\ & & & & fcu &= 30 \text{ N/mm}^2 \end{aligned}$$

VERTICAL

$$\begin{aligned} K &= M / (fcu \cdot b \cdot dv^2) = 0.0131 \\ Z1 &= dv \cdot (0.5 + \sqrt{(0.25 - K/0.9)}) = 532 \\ Z2 &= 0.95 \cdot dv = 513 \\ Z &= 513 \\ As &= M / fy \cdot Z = 1722.9 \text{ mm}^2 \end{aligned}$$

$$\text{USE ; D 22 @ 200 (As = 1900 mm}^2\text{)}$$

HORIZONTAL

$$\begin{aligned} K &= M / (fcu \cdot b \cdot dh^2) = 0 \\ Z1 &= dh \cdot (0.5 + \sqrt{(0.25 - K/0.9)}) = 0 \\ Z2 &= 0.95 \cdot dh = 0 \\ Z &= 0 \\ As &= M / fy \cdot Z = 0 \text{ mm}^2 \end{aligned}$$

$$\text{USE ; D 22 @ 200 (As = 1900 mm}^2\text{)}$$

c) CHECK OF CRACK WIDTH (BASE ON APPENDIX A OF BS8007)

1) CRACK SPACING

$$\begin{aligned} Smax(V) &= (fct/fb) \cdot (\sigma/2\rho(V)) = 969.74 \text{ mm} \\ Smax(H) &= (fct/fb) \cdot (\sigma/2\rho(H)) = 969.7 \text{ mm} \end{aligned}$$

$$\begin{aligned} \text{Where ; } fct/fb &= 0.67 & \rho(V) &= As/b \cdot d1 = 0.0076 \\ \rho(H) &= As/b \cdot d1 = 0.0076 \end{aligned}$$

2) CRACK WIDTHS ARISING FROM RESTRAINED SHRINKAGE AND HEAT OF HYDRATION MOVEMENT

$$\begin{aligned} Wmax(V) &= Smax(V) \cdot (\alpha/2) \cdot T1 = 0.17 < 0.2 \text{ mm} \\ Wmax(H) &= Smax(H) \cdot (\alpha/2) \cdot T1 = 0.17 < 0.2 \text{ mm} \end{aligned}$$

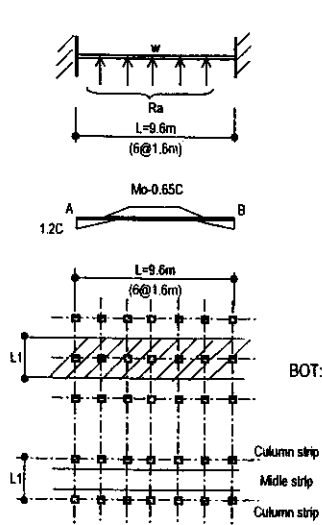
Where ; α : COEFFICIENT OF THERMAL EXPANSION OF MATURE CONCRETE

$$\begin{aligned} \alpha &= 1E-05 \\ T1 &= 35 \text{ from TABLE 4.2} \end{aligned}$$

3.3.3. CALCULATION OF FOOTING SLAB (FS100)
(AT+355.800)

④-④

1) DESIGN LOAD & FACTORED STRESS



• Design load $D = 1.0 \text{ m}$

$w = 24.0 \cdot D \cdot L1 = 41.5 \text{ KN/m}$

• Factored Load $w' = 1.4 \cdot w = 58.1 \text{ KN/m}$

• Pile Reaction $Ra = 275 \text{ KN/pile}$

$Ra' = 1.4 \cdot Ra = 385 \text{ KN/pile}$

• Factored stress

$L = 9.6 \text{ m}$

$C = (2/5 \cdot Ra' \cdot L) - (1/12 \cdot w' \cdot L^2) = 1032.0 \text{ KN} \cdot \text{m}$

$Mo = (3/5 \cdot Ra' \cdot L) - (1/8 \cdot w' \cdot L^2) = 1548.0 \text{ KN} \cdot \text{m}$

$MA = 1.2 \cdot C = 1238.4 \text{ KN} \cdot \text{m}$

$Mo-0.65C = 877.2 \text{ KN} \cdot \text{m}$

$Vu = (4/2 \cdot Ra') - (1/2 \cdot w' \cdot L) = 491.0 \text{ KN}$

• Column strip $L1 = 1.73 \text{ m}$

$Mu1 = 0.375 \cdot (1.2 \cdot C) / (1/4 \cdot L1) = 1073.8 \text{ KN} \cdot \text{m}$

$Mu2 = 0.375 \cdot (Mo - 0.65 \cdot C) / (1/4 \cdot L1) = 760.5 \text{ KN} \cdot \text{m}$

A-7-19

2) DESIGN OF SECTION

a) HORIZONTAL : BOT/END

$Mu = 1073.8 \text{ KN} \cdot \text{m}$

$Vu = 491.0 \text{ KN/m}$

$b = 1000 \text{ mm}$

$D = 1000 \text{ mm}$

$d = 940 \text{ mm}$

$fy = 365 \text{ N/mm}^2$

$fcu = 30 \text{ N/mm}^2$

[REQUIRED RE-BAR]

$K = Mu / (fcu \cdot b \cdot d^2) = 0.041 < 0.156$

$Z1 = d \cdot [0.5 + \sqrt{(0.25 - K/0.9)}] = 896$

$Z2 = 0.95 \cdot d = 893$

$Z = 893$

$As1 = Mu / (0.95 \cdot fy \cdot Z) = 3468 \text{ mm}^2$

$Asmin = 0.0013 \cdot b \cdot D = 1300 \text{ mm}^2$

USE ; D 32 @ 100 (As = 8040 mm²)

[CHECK OF SHEAR STRESS]

$V = Vu / (b \cdot d) = 0.52 < 4.4 \text{ N/mm}^2 \text{ OK}$

$(100 \cdot As) / (b \cdot d) = 0.86 < 3.0$

$400/d = 0.4 < 1.0 \rightarrow 1.0$

$Vc = 0.79 \cdot \{ (100 \cdot As) / (b \cdot d) \}^{1/3} \cdot (400/d)^{1/4} \cdot (1/1.25) \cdot (fcu/25)^{1/3}$

$= 0.64 \text{ N/mm}^2 > V = 0.52 \text{ N/mm}^2 \text{ OK}$

b) HORIZONTAL : TOP/CENT

$Mu = 760.5 \text{ KN} \cdot \text{m}$

$Vu = \text{KN/m}$

$b = 1000 \text{ mm}$

$D = 600 \text{ mm}$

$d = 520 \text{ mm}$

$fy = 365 \text{ N/mm}^2$

$fcu = 30 \text{ N/mm}^2$

[REQUIRED RE-BAR]

$K = Mu / (fcu \cdot b \cdot d^2) = 0.094 < 0.156$

$Z1 = d \cdot [0.5 + \sqrt{(0.25 - K/0.9)}] = 459$

$Z2 = 0.95 \cdot d = 494$

$Z = 459$

$As1 = Mu / (0.95 \cdot fy \cdot Z) = 4783 \text{ mm}^2$

$Asmin = 0.0013 \cdot b \cdot D = 780 \text{ mm}^2$

USE ; D 32 @ 100 (As = 8040 mm²)

[CHECK OF SHEAR STRESS]

$V = Vu / (b \cdot d) = 0.00 < 4.4 \text{ N/mm}^2 \text{ OK}$

$(100 \cdot As) / (b \cdot d) = 1.55 < 3.0$

$400/d = 0.8 < 1.0 \rightarrow 1.0$

$Vc = 0.79 \cdot \{ (100 \cdot As) / (b \cdot d) \}^{1/3} \cdot (400/d)^{1/4} \cdot (1/1.25) \cdot (fcu/25)^{1/3}$

$= 0.78 \text{ N/mm}^2 > V = 0.00 \text{ N/mm}^2 \text{ OK}$

3) CHECK OF CRACKING
FOOTING SLAB (FS)

④-④

3-21

4-1

a) CHECK OF MINIMUM REINFORCEMENT (BASED ON 2.6.2.3 OF BS8007)

SECTION	b = 1000 mm	SURFACE ZONE :	d(T) = 250 mm
	D = 1000 mm		d(B) = 100 mm
	d = 840 mm		

Min. $\rho = 0.0035$ (0.35 %)

As (top) = $\rho \cdot b \cdot d(T) = 875 \text{ mm}^2$
As (bot) = $\rho \cdot b \cdot d(B) = 350 \text{ mm}^2$

USE :	TOP	D 32	@	100	(As = 8040 mm ²)
USE :	BOTTOM	D 32	@	100	(As = 8040 mm ²)

b) CHECK OF STEEL STRESS IN DIRECT OR FLEXURAL TENSION FOR SERVICEABILITY LIMIT STATES
(BASED ON 3.2.2 OF BS8007)

Top	Mu = 760.6 KN·m	M = 543.3 KN·m	b = 1000 mm
Bott.	Mu = 1073.7 KN·m	M = 766.9 KN·m	D = 1000 mm
			d = 840 mm
			fy = 130 N/mm ²
			fcu = 30 N/mm ²

TOP
 $K = M / (fcu \cdot b \cdot d^2) = 0.0257$
 $Z1 = d \cdot [0.5 + \sqrt{ (0.25 - K/0.9) }] = 815$
 $Z2 = 0.95 \cdot d = 798$
 $Z = 798$
 $As = M / fy \cdot Z = 5237.1 \text{ mm}^2$

USE :	D 32	@	100	(As = 8040 mm ²)
-------	------	---	-----	------------------------------

BOTTOM
 $K = M / (fcu \cdot b \cdot d^2) = 0.0362$
 $Z1 = d \cdot [0.5 + \sqrt{ (0.25 - K/0.9) }] = 805$
 $Z2 = 0.95 \cdot d = 798$
 $Z = 798$
 $As = M / fy \cdot Z = 7392.5 \text{ mm}^2$

USE :	D 32	@	100	(As = 8040 mm ²)
-------	------	---	-----	------------------------------

c) CHECK OF CRACK WIDTH (BASE ON APPENDIX A OF BS8007)

1) CRACK SPACING

Smax(T) = $(fct / fb) \cdot (\phi / 2 \rho(T)) = 332.92 \text{ mm}$
 Smax(B) = $(fct / fb) \cdot (\phi / 2 \rho(B)) = 133.3 \text{ mm}$

Where ; $fct / fb = 0.67$ $\rho(T) = As / b \cdot d(T) = 0.0322$
 $\rho(B) = As / b \cdot d(B) = 0.0804$

2) CRACK WIDTHS ARISING FROM RESTRAINED SHRINKAGE AND HEAT OF HYDRATION MOVEMENT

Wmax(T) = $Smax(T) \cdot (\alpha / 2) \cdot T1 = 0.04 < 0.2 \text{ mm}$
 Wmax(B) = $Smax(B) \cdot (\alpha / 2) \cdot T1 = 0.02 < 0.2 \text{ mm}$

Where ; α : COEFFICIENT OF THERMAL EXPANSION OF MATURE CONCRETE

$\alpha = 1E-05$
 $T1 = 25$ from TABLE 4.2

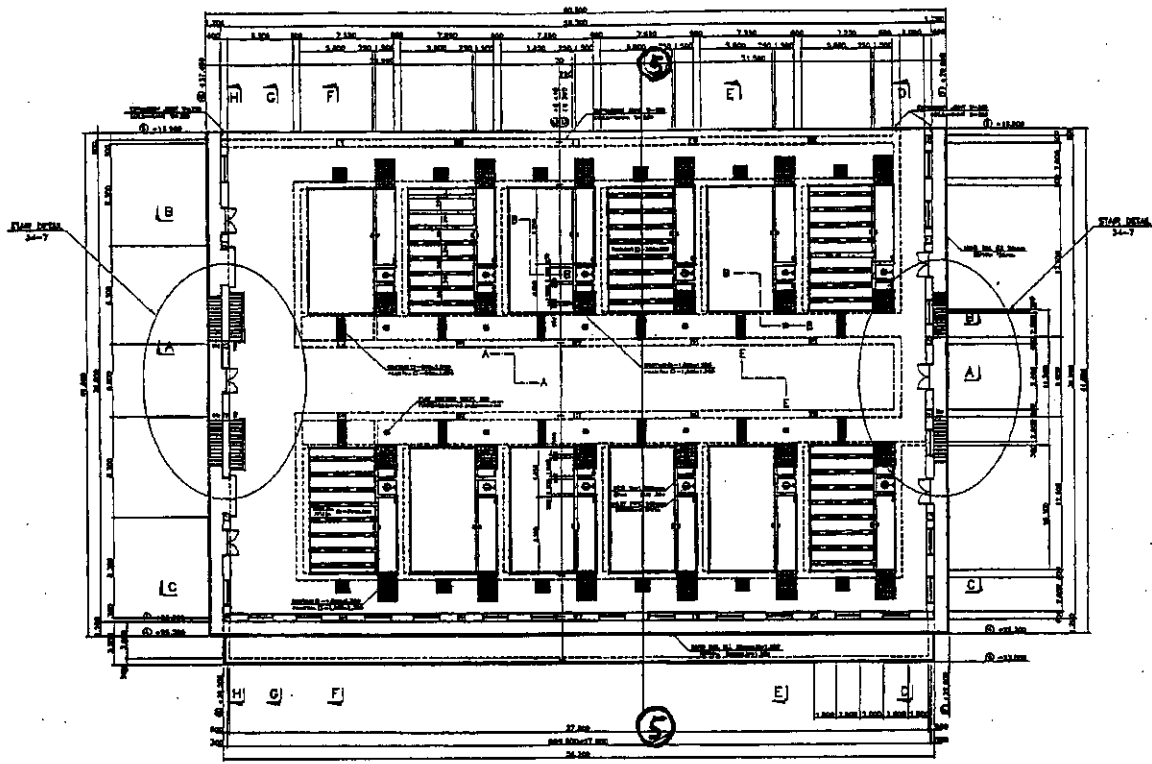
4. RAPID SAND FILTER

4. СКОРЫЙ ПЕСЧАНЫЙ ФИЛЬТР

Sections 6 and 7 are the same as Section 5
 Разделы 6 и 7 являются такими же, как и Раздел 5

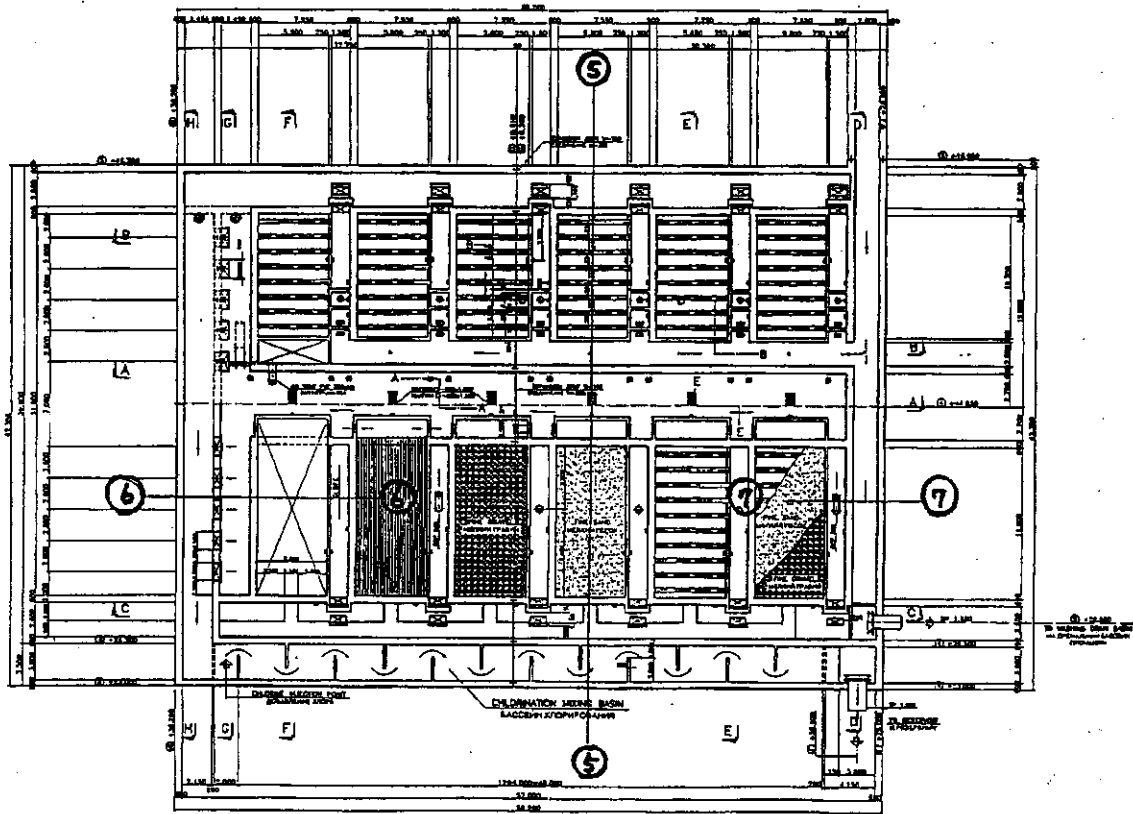
A.7-20

4-1 Plan and Section
4-1 План и разрез



TOP PLAN
ПЛАН НАДЗЕМНОЙ ЧАСТИ

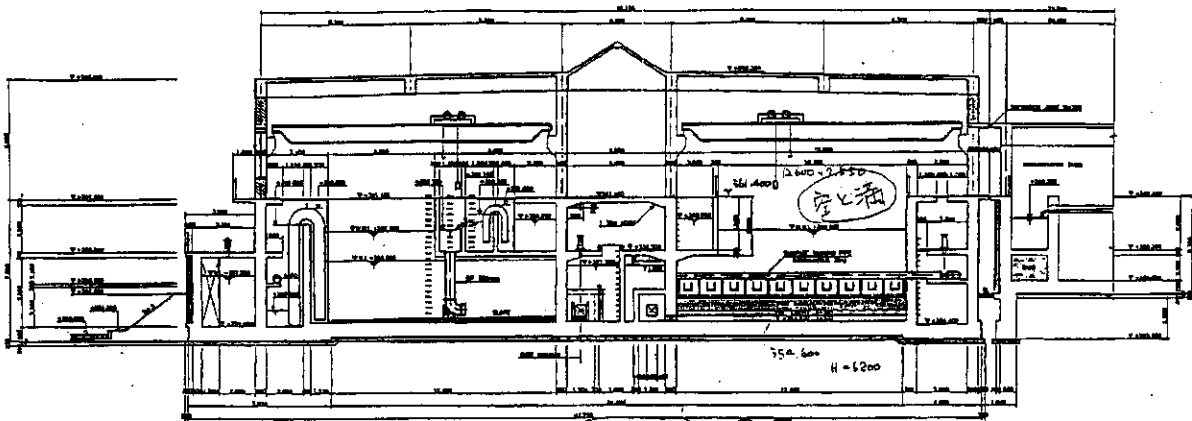
4-2



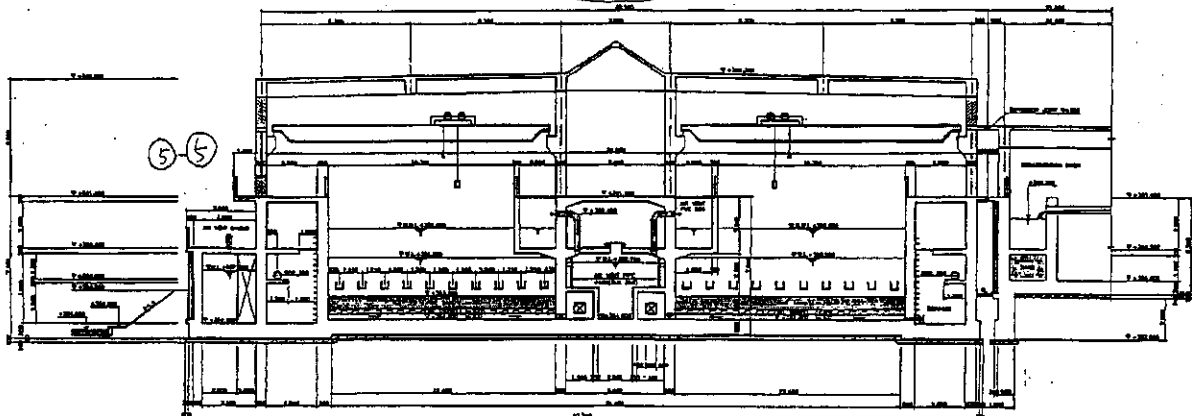
BOTTOM PLAN
ПЛАН ПОДЗЕМНОЙ ЧАСТИ

4-3

A.7-21

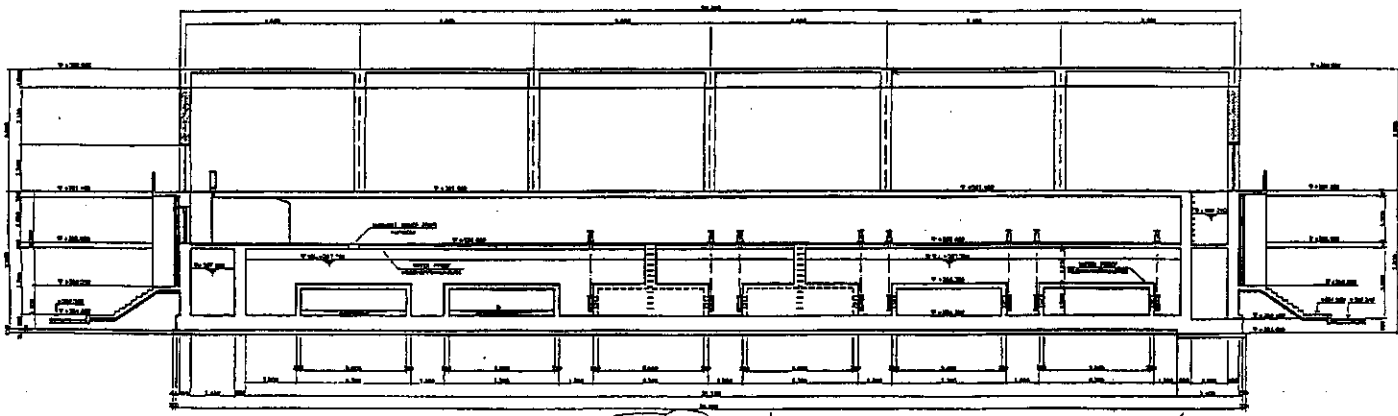


SECTION E-E
SCALE 1/100 (5-5)

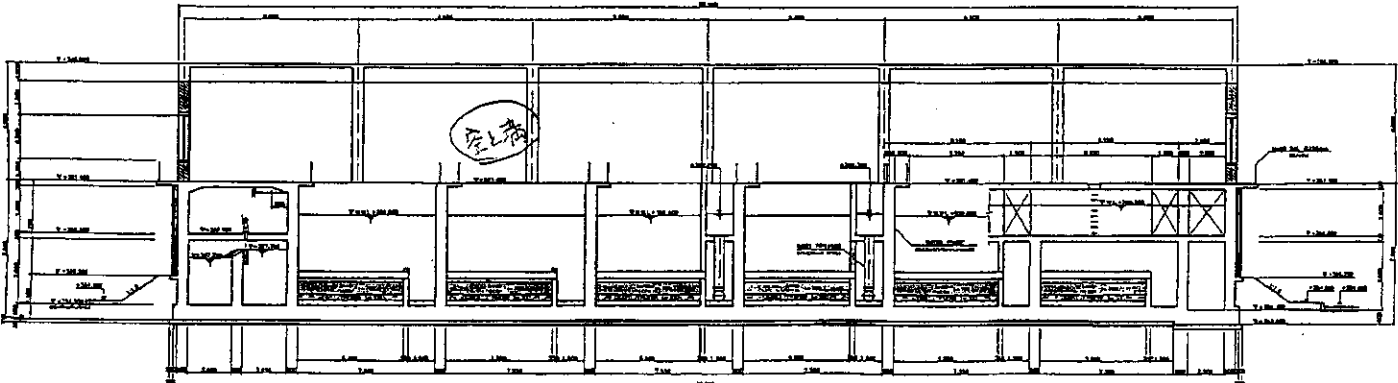


SECTION F-F
SCALE 1/100 (5-5)

4-4



SECTION A-A
PA3PE3 A-A (7-7)



(6-6)

SECTION B-B
PA3PE3 B-B

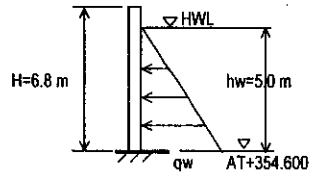
4-5

A.7-22

4.2.1 CALCULATION OF WALL (W60)
(AT+354.600)

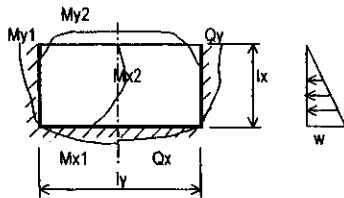
⑤-⑤

1) DESIGN LOAD (Water Pressure)



$$\begin{aligned} hw &= 5.00 \text{ m} \\ \gamma_w &= 10.0 \text{ KN/m}^3 \\ q_w &= \gamma_w \cdot hw = 50.0 \text{ KN/m}^2 \end{aligned}$$

2) FACTORED STRESS (W60)



$$\begin{aligned} l_x &= 5.00 \text{ m} \\ l_y &= 12.60 \text{ m} \\ \lambda &= 2.52 \\ w &= q_w = 50.0 \text{ KN/m}^2 \\ \text{Factored Load } w &= 1.4 \cdot w = 70.0 \text{ KN/m}^2 \\ w \cdot l_x^2 &= 1750.0 \\ w \cdot l_x &= 350.0 \end{aligned}$$

SEE FIG.

$$\begin{aligned} M_{x1} &= 0.105 \times 1750.0 = 183.8 \text{ (KN} \cdot \text{m/m)} \\ M_{x2} &= 0.013 \times 1750.0 = 22.8 \\ M_{y1} &= 0.082 \times 1750.0 = 143.5 \\ M_{y2} &= 0.026 \times 1750.0 = 45.5 \\ Q_x &= 0.50 \times 350.0 = 175.0 \text{ (KN/m)} \\ Q_y &= 0.28 \times 350.0 = 98.0 \end{aligned}$$

A.7-24

3) DESIGN OF SECTION

a) VERTICAL : BOT (AT+354.600)

$$\begin{aligned} \mu_u &= 183.8 \text{ KN} \cdot \text{m} \\ \nu_u &= 175 \text{ KN/m} \\ b &= 1000 \text{ mm} & f_y &= 365 \text{ N/mm}^2 \\ D &= 600 \text{ mm} & f_{cu} &= 30 \text{ N/mm}^2 \\ d &= 540 \text{ mm} \end{aligned}$$

[REQUIRED RE-BAR]

$$\begin{aligned} K &= \mu_u / (f_{cu} \cdot b \cdot d^2) &= 0.021 < 0.156 \\ Z_1 &= d \cdot [0.5 + \sqrt{(0.25 - K/0.9)}] &= 527 \\ Z_2 &= 0.95 \cdot d &= 513 \\ Z &= 513 \\ A_s 1 &= \mu_u / (0.95 \cdot f_y \cdot Z) &= 1033 \text{ mm}^2 \\ A_{smin} &= 0.0013 \cdot b \cdot D &= 780 \text{ mm}^2 \end{aligned}$$

USE ; D	20 @ 100	(As = 3140 mm ²)	Bottom
D	20 @ 200	(As = 1570 mm ²)	Top

[CHECK OF SHEAR STRESS]

$$\begin{aligned} V &= \nu_u / (b \cdot d) &= 0.32 < 4.4 \text{ N/mm}^2 \text{ OK} \\ (100 \cdot A_s) / (b \cdot d) &= 0.58 < 3.0 \\ 400/d &= 0.74 < 1.0 \rightarrow 1.0 \end{aligned}$$

$$\begin{aligned} V_c &= 0.79 \cdot \{ (100 \cdot A_s) / (b \cdot d) \}^{1/3} \cdot (400/d)^{1/4} \cdot (1/1.25) \cdot (f_{cu}/25)^{1/3} \\ &= 0.56 \text{ N/mm}^2 > V = 0.32 \text{ N/mm}^2 \text{ OK} \end{aligned}$$

b) HORIZONTAL : TOP/END

$$\begin{aligned} \mu_u &= 143.5 \text{ KN} \cdot \text{m} \\ \nu_u &= 98 \text{ KN/m} \\ b &= 1000 \text{ mm} & f_y &= 365 \text{ N/mm}^2 \\ D &= 600 \text{ mm} & f_{cu} &= 30 \text{ N/mm}^2 \\ d &= 520 \text{ mm} \end{aligned}$$

[REQUIRED RE-BAR]

$$\begin{aligned} K &= \mu_u / (f_{cu} \cdot b \cdot d^2) &= 0.018 < 0.156 \\ Z_1 &= d \cdot [0.5 + \sqrt{(0.25 - K/0.9)}] &= 510 \\ Z_2 &= 0.95 \cdot d &= 494 \\ Z &= 494 \\ A_s 1 &= \mu_u / (0.95 \cdot f_y \cdot Z) &= 838 \text{ mm}^2 \\ A_{smin} &= 0.0013 \cdot b \cdot D &= 780 \text{ mm}^2 \end{aligned}$$

USE ; D	20 @ 100	(As = 3140 mm ²)	Top/End
D	20 @ 200	(As = 1570 mm ²)	Center, Bot/End

[CHECK OF SHEAR STRESS]

$$\begin{aligned} V &= \nu_u / (b \cdot d) &= 0.19 < 4.4 \text{ N/mm}^2 \text{ OK} \\ (100 \cdot A_s) / (b \cdot d) &= 0.60 < 3.0 \\ 400/d &= 0.77 < 1.0 \rightarrow 1.0 \end{aligned}$$

$$\begin{aligned} V_c &= 0.79 \cdot \{ (100 \cdot A_s) / (b \cdot d) \}^{1/3} \cdot (400/d)^{1/4} \cdot (1/1.25) \cdot (f_{cu}/25)^{1/3} \\ &= 0.57 \text{ N/mm}^2 > V = 0.19 \text{ N/mm}^2 \text{ OK} \end{aligned}$$

a) CHECK OF MINIMUM REINFORCEMENT (BASED ON 2.6.2.3 OF BS8007)

SECTION	b = 1000 mm	SURFACE ZONE:	d1 = 250 mm
	D = 600 mm		
	dv = 540 mm		
	dh = 520 mm		

Min. ρ = 0.0035 (0.35 %)

As (ver.) = ρ · b · d1 = 875 mm²
As (hor.) = ρ · b · d1 = 875 mm²

USE;	VERTICAL	D 20	@ 100	(As = 3140 mm ²)
USE;	HORIZONTAL	D 20	@ 100	(As = 3140 mm ²)

b) CHECK OF STEEL STRESS IN DIRECT OR FLEXURAL TENSION FOR SERVICEABILITY LIMIT STATES (BASED ON 3.2.2 OF BS8007)

Ver.	Mu = 183.8 KN·m	M = 131.3 KN·m	b = 1000 mm
Hor.	Mu = 143.5 KN·m	M = 102.5 KN·m	D = 600 mm
			dv = 540 mm
			dh = 520 mm
			fy = 130 N/mm ²
			fcu = 30 N/mm ²

VERTICAL

$K = M / (f_{cu} \cdot b \cdot d_v^2) = 0.015$
 $Z1 = d_v \cdot [0.5 + \sqrt{0.25 - K/0.9}] = 531$
 $Z2 = 0.95 \cdot d_v = 513$
 $Z = 513$
 $As = M / f_y \cdot Z = 1968.8 \text{ mm}^2$

USE;	D 20	@ 100	(As = 3140 mm ²)
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HORIZONTAL

$K = M / (f_{cu} \cdot b \cdot d_h^2) = 0.0126$
 $Z1 = d_h \cdot [0.5 + \sqrt{0.25 - K/0.9}] = 513$
 $Z2 = 0.95 \cdot d_h = 494$
 $Z = 494$
 $As = M / f_y \cdot Z = 1596.1 \text{ mm}^2$

USE;	D 20	@ 100	(As = 3140 mm ²)
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c) CHECK OF CRACK WIDTH (BASE ON APPENDIX A OF BS8007)

1) CRACK SPACING

$S_{max(V)} = (f_{ct}/f_b) \cdot (\sigma/2\rho(V)) = 531.75 \text{ mm}$
 $S_{max(H)} = (f_{ct}/f_b) \cdot (\sigma/2\rho(H)) = 531.7 \text{ mm}$

Where ; $f_{ct}/f_b = 0.67$
 $\rho(V) = As/b \cdot d1 = 0.0126$
 $\rho(H) = As/b \cdot d1 = 0.0126$

2) CRACK WIDTHS ARISING FORM RESTRAINED SHRINKAGE AND HEAT OF HYDRATION MOVEMENT

$W_{max(V)} = S_{max(V)} \cdot (\alpha/2) \cdot T1 = 0.09 < 0.2 \text{ mm}$
 $W_{max(H)} = S_{max(H)} \cdot (\alpha/2) \cdot T1 = 0.09 < 0.2 \text{ mm}$

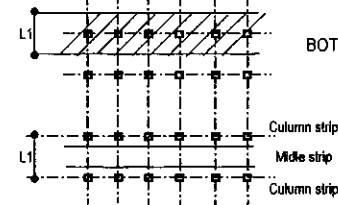
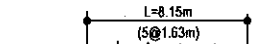
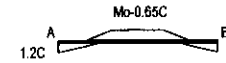
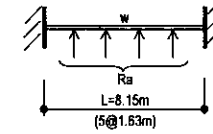
Where ; α : COEFFICIENT OF THERMAL EXPANSION OF MATURE CONCRETE

α = 1E-05
 T1 = 35 from TABLE 4.2

A.7-25

4.2.3 CALCULATION OF SLAB (S80) (5)-(5)
(AT+354.600)

1) DESIGN LOAD & FACTORED STRESS



• Design load $D = 1.0 \text{ m}$
 $w = 24.0 \cdot D \cdot L1 = 39.6 \text{ KN/m}$
 • Factored Load $w' = 1.4 \cdot w = 55.4 \text{ KN/m}$
 • Pile Reaction $Ra = 262 \text{ KN/pile}$
 $Ra' = 1.4 \cdot Ra = 367 \text{ KN/pile}$

• Factored stress $L = 8.15 \text{ m}$
 $C = (2/5 \cdot Ra' \cdot L) - (1/12 \cdot w' \cdot L^2) = 888.9 \text{ KN} \cdot \text{m}$
 $Mo = (3/5 \cdot Ra' \cdot L) - (1/8 \cdot w' \cdot L^2) = 1333.3 \text{ KN} \cdot \text{m}$
 $MA = 1.2 \cdot C = 1066.7 \text{ KN} \cdot \text{m}$
 $Mo - 0.68C = 755.6 \text{ KN} \cdot \text{m}$
 $Vu = (4/2 \cdot Ra') - (1/2 \cdot w' \cdot L) = 507.7 \text{ KN}$

• Column strip $L1 = 1.65 \text{ m}$
 BOT: $Mu1 = 0.375 \cdot (1.2 \cdot C) / (1/4 \cdot L1) = 969.7 \text{ KN} \cdot \text{m}$
 TOP: $Mu2 = 0.375 \cdot (Mo - 0.65 \cdot C) / (1/4 \cdot L1) = 686.9 \text{ KN} \cdot \text{m}$

2) DESIGN OF SECTION (X-Direction)

a) HORIZONTAL : BOT/END

Mu = 969.7 KN·m
Vu = 507.7 KN/m

b = 1000 mm fy = 365 N/mm2
D = 1000 mm fcu = 30 N/mm2
d = 940 mm

[REQUIRED RE-BAR]

$K = Mu / (fcu \cdot b \cdot d^2)$ = 0.037 < 0.156
 $Z1 = d \cdot [0.5 + \sqrt{(0.25 - K/0.9)}]$ = 900
 $Z2 = 0.95 \cdot d$ = 893
Z = 893
As1 = Mu / (0.95 · fy · Z) = 3132 mm2
Asmin = 0.0013 · b · D = 1300 mm2

USE ; D 32 @ 75 (As = 10720 mm2)
D 25 @ 200 (Y-Direction)

[CHECK OF SHEAR STRESS]

V = Vu / (b · d) = 0.54 < 4.4 N/mm2 OK
(100 · As) / (b · d) = 1.14 < 3.0
400/d = 0.4 < 1.0 → 1.0

Vc = 0.79 · ((100 · As) / (b · d))^{1/3} · (400/d)^{1/4} · (1/1.25) · (fcu/25)^{1/3}
= 0.70 N/mm2 > V = 0.59 N/mm2 OK

b) HORIZONTAL : TOP/CENT

Mu = 686.9 KN·m
Vu = KN/m

b = 1000 mm fy = 365 N/mm2
D = 1000 mm fcu = 30 N/mm2
d = 920 mm

[REQUIRED RE-BAR]

$K = Mu / (fcu \cdot b \cdot d^2)$ = 0.027 < 0.156
 $Z1 = d \cdot [0.5 + \sqrt{(0.25 - K/0.9)}]$ = 891
 $Z2 = 0.95 \cdot d$ = 874
Z = 874
As1 = Mu / (0.95 · fy · Z) = 2266 mm2
Asmin = 0.0013 · b · D = 1300 mm2

USE ; D 32 @ 100 (As = 8040 mm2)
D 25 @ 200 (Y-Direction)

[CHECK OF SHEAR STRESS]

V = Vu / (b · d) = 0.00 < 4.4 N/mm2 OK
(100 · As) / (b · d) = 0.87 < 3.0
400/d = 0.4 < 1.0 → 1.0

Vc = 0.79 · ((100 · As) / (b · d))^{1/3} · (400/d)^{1/4} · (1/1.25) · (fcu/25)^{1/3}
= 0.64 N/mm2 > V = 0.00 N/mm2 OK

A.7-26

3) CHECK OF CRACKING FOOTING SLAB (FS80)

a) CHECK OF MINIMUM REINFORCEMENT (BASED ON 2.6.2.3 OF BS8007)

SECTION b = 1000 mm SURFACE ZONE : d(T) = 250 mm
D = 1000 mm d(B) = 100 mm
d = 640 mm

Min. ρ = 0.0035 (0.35 %)

As (top) = ρ · b · d(T) = 875 mm2
As (bot) = ρ · b · d(B) = 350 mm2

USE ; TOP D 32 @ 100 (As = 8040 mm2)
USE ; BOTTOM D 32 @ 75 (As = 10720 mm2)

b) CHECK OF STEEL STRESS IN DIRECT OR FLEXURAL TENSION FOR SERVICEABILITY LIMIT STATES (BASED ON 3.2.2 OF BS8007)

Top Mu = 686.9 KN·m M = 490.6 KN·m b = 1000 mm
Bot Mu = 969.7 KN·m M = 692.6 KN·m D = 1000 mm
d = 640 mm
fy = 130 N/mm2
fcu = 30 N/mm2

TOP
 $K = M / (fcu \cdot b \cdot d^2)$ = 0.04
 $Z1 = d \cdot [0.5 + \sqrt{(0.25 - K/0.9)}]$ = 610
 $Z2 = 0.95 \cdot d$ = 608
Z = 608
As = M / fy · Z = 6207 mm2

USE ; D 32 @ 100 (As = 8040 mm2)

BOTTOM
 $K = M / (fcu \cdot b \cdot d^2)$ = 0.056
 $Z1 = d \cdot [0.5 + \sqrt{(0.25 - K/0.9)}]$ = 597
 $Z2 = 0.95 \cdot d$ = 608
Z = 597
As = M / fy · Z = 8924 mm2

USE ; D 32 @ 75 (As = 10720 mm2)

c) CHECK OF CRACK WIDTH (BASE ON APPENDIX A OF BS8007)

1) CRACK SPACING

Smax(T) = (fct/fb) · (σ/2ρ(T)) = 332.92 mm
Smax(B) = (fct/fb) · (σ/2ρ(B)) = 100.0 mm

Where ; fct/fb = 0.67 ρ(T) = As/b · d(T) = 0.0322
ρ(B) = As/b · d(B) = 0.1072

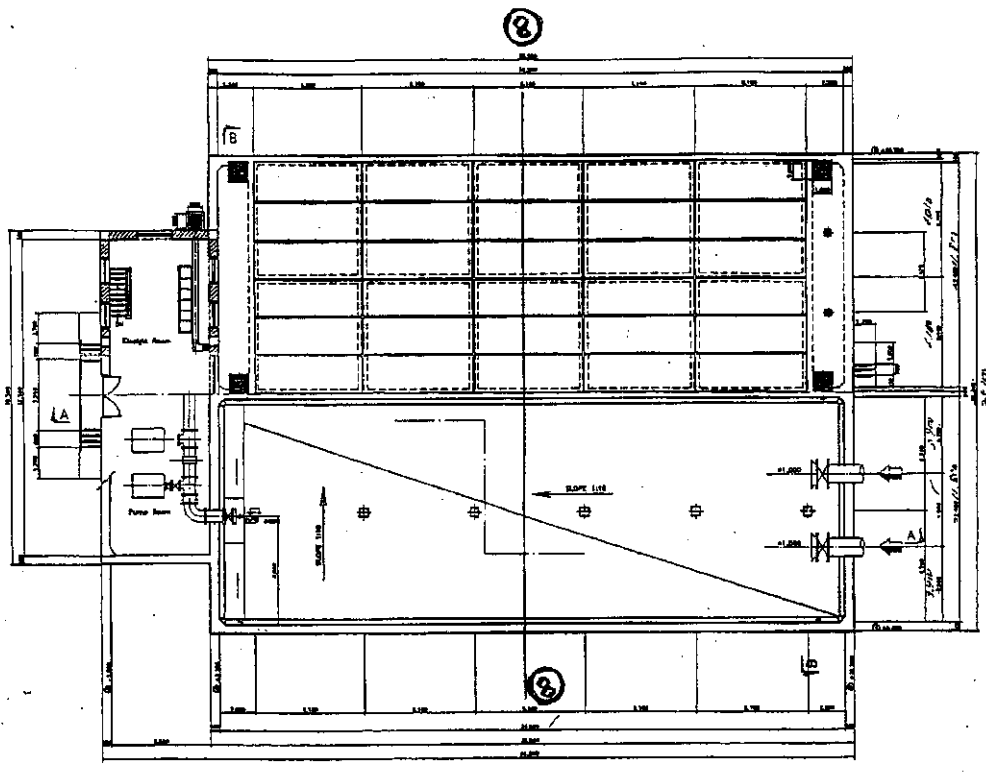
2) CRACK WIDTHS ARISING FROM RESTRAINED SHRINKAGE AND HEAT OF HYDRATION MOVEMENT

Wmax(T) = Smax(T) · (α/2) · T1 = 0.04 < 0.2 mm
Wmax(B) = Smax(B) · (α/2) · T1 = 0.01 < 0.2 mm

Where ; α : COEFFICIENT OF THERMAL EXPANSION OF MATURE CONCRETE

α = 1E-05
T1 = 25 from TABLE 4.2

5-1 Plan and Section
5-1 План и разрез

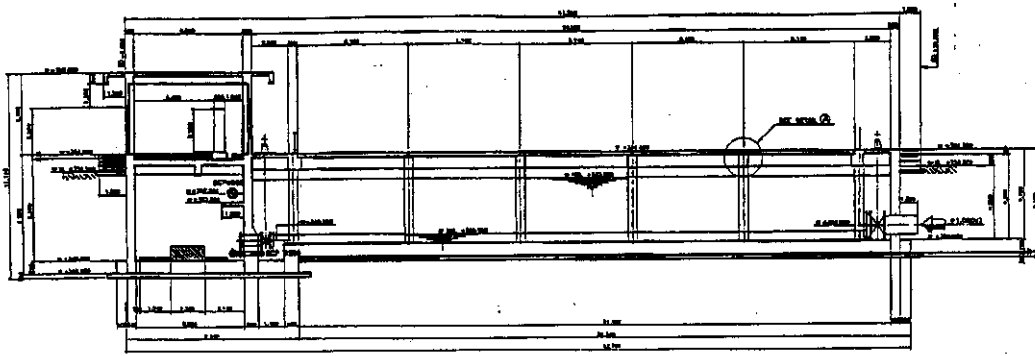


TOP PLAN AND BOTTOM PLAN

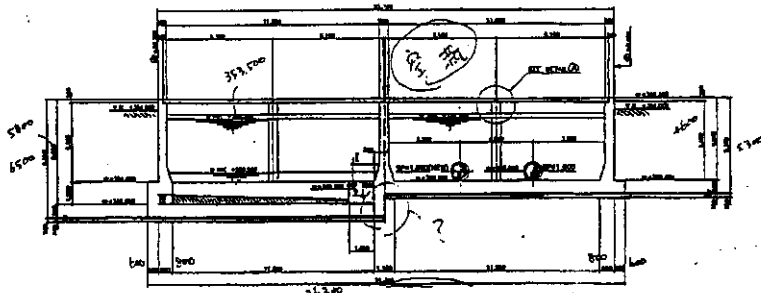
5-2

5. BACKWASH DRAINAGE BASIN

5. ПРОМЫВНОЙ ДРЕНАЖНЫЙ БАССЕЙН

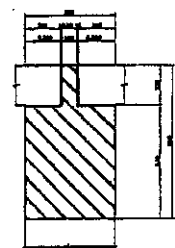


SECTION A-A



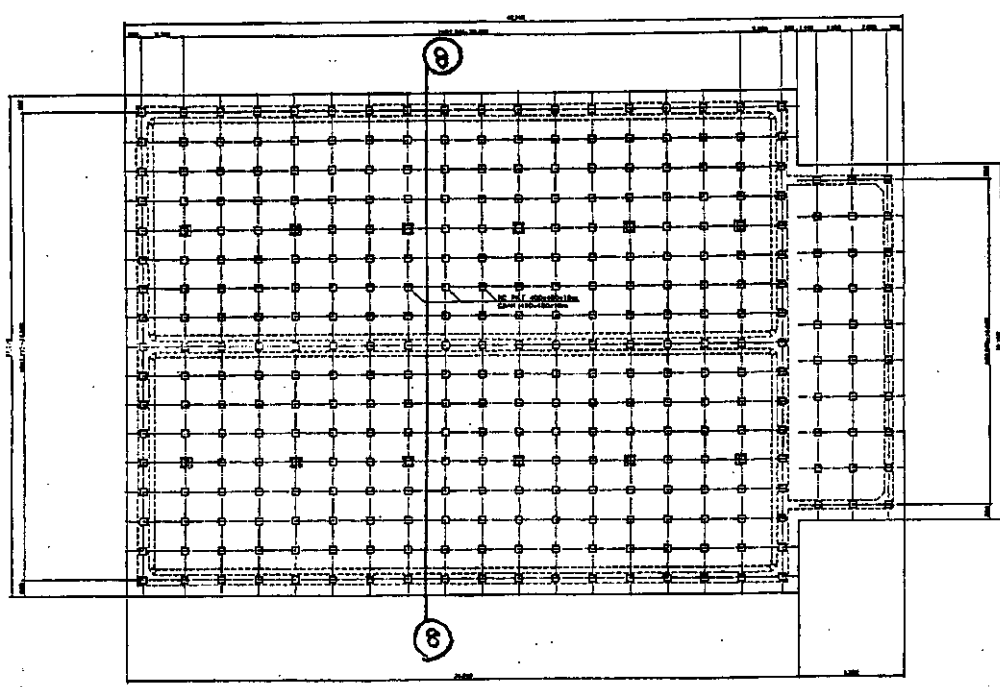
SECTION B-B

8 8



DETAIL (A)

5-3

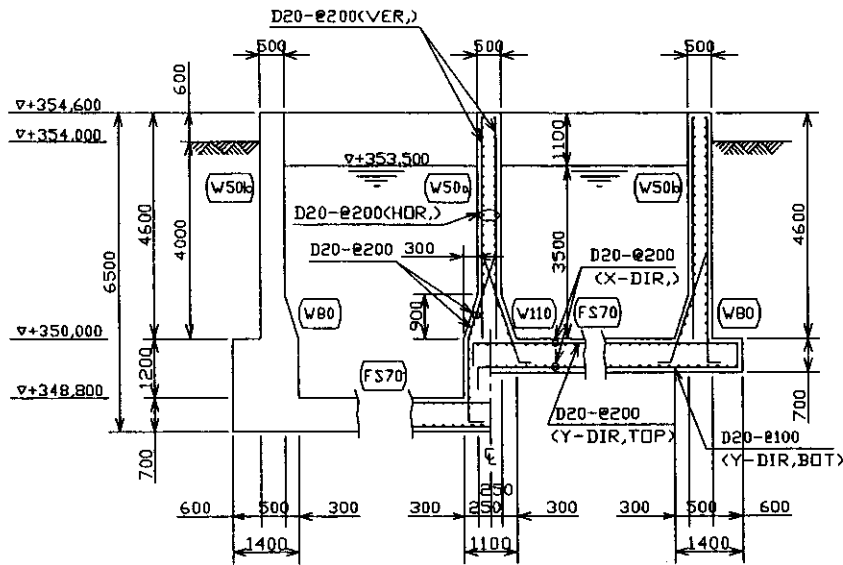


PILE ARRANGEMENT
РАССТАНОВКА СВАИ

5-4

5.2 DESIGN OF WALL & FOOTING
5.2 ПРОЕКТИРОВАНИЕ СТЕНЫ И ОСНОВАНИЯ

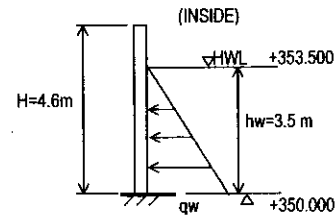
A.7-29



Ⓢ - Ⓢ
S=1/100

5.2.1 CALCULATION OF WALL (W80) Ⓢ-Ⓢ
(AT+350.000)

1) DESIGN LOAD & FACTORED STRESS (Water Pressure)



H= 4.6 m
hw= 3.50 m
yw= 10.0 KN/m³

• Design load
 $q_w = y_w \cdot h_w = 10.0 \times 3.50 = 35.0 \text{ (KN/m}^2\text{/m)}$

• Factored Load
 $w = 1.4 \cdot q_w = 1.4 \times 35.0 = 49.0 \text{ (KN/m}^2\text{/m)}$

• Factored stress at +350.000

$$\begin{cases} M_u = 1/6 \cdot w \cdot h_w^2 = 100.0 \text{ (KN} \cdot \text{m/m)} \\ V_u = 1/2 \cdot w \cdot h_w = 85.8 \text{ (KN/m)} \end{cases}$$

2) DESIGN OF SECTION (INSIDE/VERTICAL BOT.)

$M_u = 100.0 \text{ KN} \cdot \text{m}$
 $V_u = 85.8 \text{ KN/m}$

$b = 1000 \text{ mm}$ $f_y = 365 \text{ N/mm}^2$
 $D = 800 \text{ mm}$ $f_{cu} = 30 \text{ N/mm}^2$
 $d = 740 \text{ mm}$

[REQUIRED RE-BAR]

$$\begin{aligned} K &= M_u / (f_{cu} \cdot b \cdot d^2) &&= 0.006 < 0.156 \\ Z_1 &= d \cdot [0.5 + \sqrt{(0.25 - K/0.9)}] &&= 735 \\ Z_2 &= 0.95 \cdot d &&= 703 \\ Z &= &&703 \\ A_{s1} &= M_u / (0.95 \cdot f_y \cdot Z) &&= 410 \text{ mm}^2 \\ A_{smin} &= 0.0013 \cdot b \cdot D &&= 1040 \text{ mm}^2 \end{aligned}$$

USE ; D 20 @ 200 ($A_s = 1570 \text{ mm}^2$)

[CHECK OF SHEAR STRESS]

$$\begin{aligned} V &= V_u / (b \cdot d) &&= 0.12 < 4.4 \text{ N/mm}^2 \text{ OK} \\ (100 \cdot A_s) / (b \cdot d) & &&= 0.21 < 3.0 \\ 400/d & &&= 0.5 < 1.0 \rightarrow 1.0 \end{aligned}$$

$$\begin{aligned} V_c &= 0.79 \cdot \left(\frac{100 \cdot A_s}{b \cdot d} \right)^{1/3} \cdot (400/d)^{1/4} \cdot (1/1.25) \cdot (f_{cu}/25)^{1/3} \\ &= 0.40 \text{ N/mm}^2 > V = 0.12 \text{ N/mm}^2 \text{ OK} \end{aligned}$$

3) CHECK OF CRACKING

(B)-(8)

5-9

5-10

W50a (INSIDE/VERTICAL TOP)

(AT+350.900)

a) CHECK OF MINIMUM REINFORCEMENT (BASED ON 2.6.2.3 OF BS8007)

SECTION b = 1000 mm SURFACE ZONE : d1 = 250 mm
 D = 500 mm
 dv = 440 mm
 dh = 420 mm

Min. ρ = 0.0035 (0.35 %)

As (ver.) = ρ · b · d1 = 875 mm²
 As (her.) = ρ · b · d1 = 875 mm²

USE ; VERTICAL D 20 @ 200 (As = 1570 mm²)
 USE ; HORIZONTAL D 20 @ 200 (As = 1570 mm²)

b) CHECK OF STEEL STRESS IN DIRECT OR FLEXURAL TENSION FOR SERVICEABILITY LIMIT STATES (BASED ON 3.2.2 OF BS8007)

Ver. Mu = 41.0 KN·m M = 29.3 KN·m b = 1000 mm
 Hor. Mu = 0.0 KN·m M = 0.0 KN·m D = 500 mm
 dv = 440 mm
 dh = 420 mm
 fy = 130 N/mm²
 fcu = 30 N/mm²

VERTICAL

$K = M / (fcu \cdot b \cdot dv^2) = 0.005$
 $Z1 = dv \cdot [0.5 + \sqrt{ (0.25 - K/0.9) }] = 438$
 $Z2 = 0.95 \cdot dv = 418$
 $Z = 418$
 $As = M / fy \cdot Z = 539.07 \text{ mm}^2$

USE ; D 20 @ 200 (As = 1570 mm²)

HORIZONTAL

$K = M / (fcu \cdot b \cdot dh^2) = 0$
 $Z1 = dh \cdot [0.5 + \sqrt{ (0.25 - K/0.9) }] = 0$
 $Z2 = 0.95 \cdot dh = 0$
 $Z = 0$
 $As = M / fy \cdot Z = 0 \text{ mm}^2$

USE ; D 20 @ 200 (As = 1570 mm²)

c) CHECK OF CRACK WIDTH (BASE ON APPENDIX A OF BS8007)

1) CRACK SPACING

$Smax(V) = (fct/fb) \cdot (\phi/2 \rho(V)) = 1066.9 \text{ mm}$
 $Smax(H) = (fct/fb) \cdot (\phi/2 \rho(H)) = 1066.9 \text{ mm}$

Where ; fct/fb = 0.67 ρ(V) = As/b · d1 = 0.0063
 ρ(H) = As/b · d1 = 0.0063

2) CRACK WIDTHS ARISING FORM RESTRAINED SHRINKAGE AND HEAT OF HYDRATION MOVEMENT

$Wmax(V) = Smax(V) \cdot (\alpha/2) \cdot T1 = 0.17 < 0.2 \text{ mm}$
 $Wmax(H) = Smax(H) \cdot (\alpha/2) \cdot T1 = 0.17 < 0.2 \text{ mm}$

Where ; α : COEFFICIENT OF THERMAL EXPANSION OF MATURE CONCRETE

α = 1E-05
 T1 = 32 from TABLE 4.2

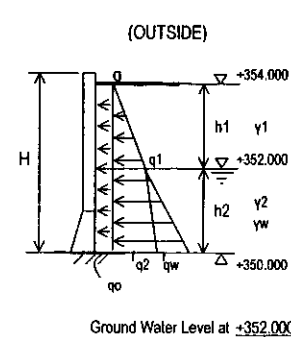
A.7-31

5.2.3 CALCULATION OF WALL (W80)

(B)-(8)

(AT+350.000)

1) DESIGN LOAD & FACTORED STRESS(Soil Pressure)



q : Live Load
 Ko: Coefficient of soil pressure at rest
 γ1 = 18.0 KN/m³ q = 5.0 (KN/m²/m)
 γ2 = 8.0 KN/m³ Ko = 0.5
 γw = 10.0 KN/m³

• Design load
 $qo = Ko \cdot q = 2.5 \text{ (KN/m}^2\text{/m)}$
 $q1 = Ko \cdot \gamma1 \cdot h1 = 18.0$
 $q2 = q1 + Ko \cdot \gamma2 \cdot h2 = 26.0$
 $qw = \gammaw \cdot h2 = 20.0$
 $\Sigma q = q2 + qw = 46.0$

• Factored Load
 $w1 = 1.4 \cdot qo = 3.5 \text{ (KN/m}^2\text{/m)}$
 $w2 = 1.4 \cdot \Sigma q = 64.4$

• Factored stress at +350.000
 $Mu1 = 1/2 \cdot w1 \cdot h^2 = 28.00 \text{ (KN} \cdot \text{m/m)}$
 $Vu1 = w1 \cdot h = 14.00 \text{ (KN/m)}$
 $Mu2 = 1/6 \cdot w2 \cdot h^2 = 171.73 \text{ (KN} \cdot \text{m/m)}$
 $Vu2 = 1/2 \cdot w2 \cdot h = 128.80 \text{ (KN/m)}$
 $Mu = Mu1 + Mu2 = 199.7 \text{ (KN} \cdot \text{m/m)}$
 $Vu = Vu1 + Vu2 = 142.8 \text{ (KN/m)}$

2) DESIGN OF SECTION (OUTSIDE/VERTICAL TOP)

Mu = 199.7 KN·m
 Vu = 142.8 KN/m

b = 1000 mm fy = 365 N/mm²
 D = 800 mm fcu = 30 N/mm²
 d = 740 mm

[REQUIRED RE-BAR]

$K = Mu / (fcu \cdot b \cdot d^2) = 0.012 < 0.156$
 $Z1 = d \cdot [0.5 + \sqrt{ (0.25 - K/0.9) }] = 730$
 $Z2 = 0.95 \cdot d = 703$
 $Z = 703$
 $As1 = Mu / (0.95 \cdot fy \cdot Z) = 819 \text{ mm}^2$
 $Asmin = 0.0013 \cdot b \cdot D = 1040 \text{ mm}^2$

USE ; D 20 @ 200 (As = 1570 mm²)

[CHECK OF SHEAR STRESS]

$V = Vu / (b \cdot d) = 0.19 < 4.4 \text{ N/mm}^2 \text{ OK}$
 $(100 \cdot As) / (b \cdot d) = 0.21 < 3.0$
 $400/d = 0.5 < 1.0 \rightarrow 1.0$

$Vc = 0.79 \cdot \{ (100 \cdot As) / (b \cdot d) \}^{1/3} \cdot (400/d)^{1/4} \cdot (1/1.25) \cdot (fcu/25)^{1/3}$
 $= 0.40 \text{ N/mm}^2 > V = 0.19 \text{ N/mm}^2 \text{ OK}$

3) CHECK OF CRACKING

(B)-(B)

5-11

W80 (OUTSIDE/VERTICAL:BOT)

(AT+350.000)

a) CHECK OF MINIMUM REINFORCEMENT (BASED ON 2.6.2.3 OF BS8007)

SECTION	b = 1000 mm	SURFACE ZONE :	d1 = 250 mm
	D = 800 mm		
	dv = 740 mm		
	dh = 720 mm		

Min. $\rho = 0.0035$ (0.35 %)

As (ver.) = $\rho \cdot b \cdot d1 = 875$ mm²
 As (her.) = $\rho \cdot b \cdot d1 = 875$ mm²

USE ;	VERTICAL	D 20	@ 200	(As = 1570 mm ²)
USE ;	HORIZONTAL	D 20	@ 200	(As = 1570 mm ²)

b) CHECK OF STEEL STRESS IN DIRECT OR FLEXURAL TENSION FOR SERVICEABILITY LIMIT STATES (BASED ON 3.2.2 OF BS8007)

Ver.	Mu = 199.7 KN·m	M = 142.7 KN·m	b = 1000 mm
Hor.	Mu = 0.0 KN·m	M = 0.0 KN·m	D = 800 mm
			dv = 740 mm
			dh = 720 mm
			fy = 130 N/mm ²
			fcu = 30 N/mm ²

VERTICAL

$K = M / (fcu \cdot b \cdot dv^2)$ = 0.0087
 $Z1 = dv \cdot [0.5 + \sqrt{(0.25 - K/0.9)}]$ = 733
 $Z2 = 0.95 \cdot dv$ = 703
 $Z = 703$
 $As = M / fy \cdot Z$ = 1561.1 mm²

USE ;	D 20	@ 200	(As = 1570 mm ²)
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HORIZONTAL

$K = M / (fcu \cdot b \cdot dh^2)$ = 0
 $Z1 = dh \cdot [0.5 + \sqrt{(0.25 - K/0.9)}]$ = 0
 $Z2 = 0.95 \cdot dh$ = 0
 $Z = 0$
 $As = M / fy \cdot Z$ = 0 mm²

USE ;	D 20	@ 200	(As = 1570 mm ²)
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c) CHECK OF CRACK WIDTH (BASE ON APPENDIX A OF BS8007)

1) CRACK SPACING

$S_{max}(V) = (fct/fb) \cdot (\sigma/2\rho(V))$ = 1066.9 mm
 $S_{max}(H) = (fct/fb) \cdot (\sigma/2\rho(H))$ = 1066.9 mm

Where : $fct/fb = 0.67$
 $\rho(V) = As/b \cdot d1 = 0.0063$
 $\rho(H) = As/b \cdot d1 = 0.0063$

2) CRACK WIDTHS ARISING FROM RESTRAINED SHRINKAGE AND HEAT OF HYDRATION MOVEMENT

$W_{max}(V) = S_{max}(V) \cdot (\alpha/2) \cdot T1$ = 0.17 < 0.2 mm
 $W_{max}(H) = S_{max}(H) \cdot (\alpha/2) \cdot T1$ = 0.17 < 0.2 mm

Where : α : COEFFICIENT OF THERMAL EXPANSION OF MATURE CONCRETE

$\alpha = 1E-05$
 $T1 = 32$ from TABLE 4.2

A.7-32

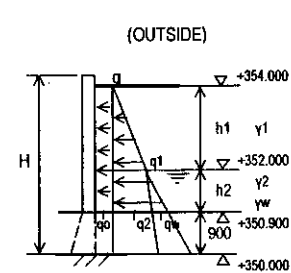
5-12

5.2.4 CALCULATION OF WALL (W50b)

(B)-(B)

(AT+350.900)

1) DESIGN LOAD & FACTORED STRESS (Soil Pressure)



q : Live Load

$\gamma1 = 18.0$ KN/m ³	$q = 5.0$ (KN/m ² /m)
$\gamma2 = 8.0$ KN/m ³	$Ko = 0.5$
$\gammaw = 10.0$ KN/m ³	

• Design load

$qo = Ko \cdot q$	= 2.5 (KN/m ² /m)
$q1 = Ko \cdot \gamma1 \cdot h1$	= 18.0
$q2 = q1 + Ko \cdot \gamma2 \cdot h2$	= 22.4
$qw = \gammaw \cdot h2$	= 11.0
$\Sigma q = q2 + qw$	= 33.4

• Factored Load

$w1 = 1.4 \cdot qo$	= 3.5 (KN/m ² /m)
$w2 = 1.4 \cdot \Sigma q$	= 46.8

• Factored stress at +350.900

$Mu1 = 1/2 \cdot w1 \cdot h^2$	= 16.82 (KN·m/m)
$Vu1 = w1 \cdot h$	= 10.85 (KN/m)
$Mu2 = 1/6 \cdot w2 \cdot h^2$	= 74.89 (KN·m/m)
$Vu2 = 1/2 \cdot w2 \cdot h$	= 72.48 (KN/m)
$Mu = Mu1 + Mu2$	= 91.7 (KN·m/m)
$Vu = Vu1 + Vu2$	= 83.3 (KN/m)

2) DESIGN OF SECTION (OUTSIDE/VERTICAL TOP)

$Mu = 91.7$ KN·m
 $Vu = 83.3$ KN/m

$b = 1000$ mm $fy = 365$ N/mm²
 $D = 500$ mm $fcu = 30$ N/mm²
 $d = 440$ mm

[REQUIRED RE-BAR]

$K = Mu / (fcu \cdot b \cdot d^2)$ = 0.016 < 0.156
 $Z1 = d \cdot [0.5 + \sqrt{(0.25 - K/0.9)}]$ = 432
 $Z2 = 0.95 \cdot d$ = 418
 $Z = 418$
 $As1 = Mu / 0.95 \cdot fy \cdot Z$ = 633 mm²
 $Asmin = 0.0013 \cdot b \cdot D$ = 650 mm²

USE ;	D 20	@ 200	(As = 1570 mm ²)
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[CHECK OF SHEAR STRESS]

$V = Vu / (b \cdot d)$ = 0.19 < 4.4 N/mm² OK
 $(100 \cdot As) / (b \cdot d)$ = 0.36 < 3.0
 $400/d$ = 0.9 < 1.0 → 1.0

$Vc = 0.79 \cdot \{ (100 \cdot As) / (b \cdot d) \}^{1/3} \cdot (400/d)^{1/4} \cdot (1/1.25) \cdot (fcu/25)^{1/3}$
 $= 0.48$ N/mm² > $V = 0.19$ N/mm² OK

3) CHECK OF CRACKING

(8)-(8)

5-13

W50b (OUTSIDE/VERTICAL:TOP)

(AT+350.900)

a) CHECK OF MINIMUM REINFORCEMENT (BASED ON 2.6.2.3 OF BS8007)

SECTION	b =	1000	mm	SURFACE ZONE :	d1 =	250	mm
	D =	500	mm				
	dv =	440	mm				
	dh =	420	mm				

Min. ρ = 0.0035 (0.35 %)

As (ver.) = ρ · b · d1 = 875 mm²
 As (her.) = ρ · b · d1 = 875 mm²

USE :	VERTICAL	D 20	@ 200	(As = 1570 mm ²)
USE :	HORIZONTAL	D 20	@ 200	(As = 1570 mm ²)

b) CHECK OF STEEL STRESS IN DIRECT OR FLEXURAL TENSION FOR SERVICEABILITY LIMIT STATES (BASED ON 3.2.2 OF BS8007)

Ver.	Mu =	91.7	KN·m	M =	65.5	KN·m	b =	1000	mm
Hor.	Mu =	0.0	KN·m	M =	0.0	KN·m	D =	500	mm
							dv =	440	mm
							dh =	420	mm
							fy =	130	N/mm ²
							fcu =	30	N/mm ²

VERTICAL

$K = M / (f_{cu} \cdot b \cdot d_v^2) = 0.0113$
 $Z1 = d_v \cdot [0.5 + \sqrt{ (0.25 - K/0.9) }] = 434$
 $Z2 = 0.95 \cdot d_v = 418$
 $Z = 418$
 $As = M / f_y \cdot Z = 1205.5 \text{ mm}^2$

USE : D 20 @ 200 (As = 1570 mm²)

HORIZONTAL

$K = M / (f_{cu} \cdot b \cdot d_h^2) = 0$
 $Z1 = d_h \cdot [0.5 + \sqrt{ (0.25 - K/0.9) }] = 0$
 $Z2 = 0.95 \cdot d_h = 0$
 $Z = 0$
 $As = M / f_y \cdot Z = 0 \text{ mm}^2$

USE : D 20 @ 200 (As = 1570 mm²)

c) CHECK OF CRACK WIDTH (BASE ON APPENDIX A OF BS8007)

1) CRACK SPACING

$S_{max(V)} = (f_{ct}/f_b) \cdot (q/2\rho(V)) = 1066.9 \text{ mm}$
 $S_{max(H)} = (f_{ct}/f_b) \cdot (q/2\rho(H)) = 1066.9 \text{ mm}$

Where ; $f_{ct}/f_b = 0.67$
 $\rho(V) = As/b \cdot d1 = 0.0063$
 $\rho(H) = As/b \cdot d1 = 0.0063$

2) CRACK WIDTHS ARISING FORM RESTRAINED SHRINKAGE AND HEAT OF HYDRATION MOVEMENT

$W_{max(V)} = S_{max(V)} \cdot (\alpha/2) \cdot T1 = 0.17 < 0.2 \text{ mm}$
 $W_{max(H)} = S_{max(H)} \cdot (\alpha/2) \cdot T1 = 0.17 < 0.2 \text{ mm}$

Where ; α : COEFFICIENT OF THERMAL EXPANSION OF MATURE CONCRETE

$\alpha = 1E-05$
 $T1 = 32 \text{ from TABLE 4.2}$

A.7-33

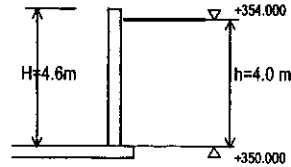
5-14

5.2.6 CALCULATION OF FOOTING SLAB (FS70)

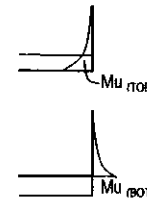
(8)-(8)

(AT+350.000)

1) FACTORED STRESS



*Factored stress at +350.000



from Moment of Inside of Wall (refer to para.7.5.2.1)

$Mu_{top} = 100.0 \text{ (KN·m/m)}$

from Moment of Outside of Wall (refer to para.7.2.3)

$Mu_{bot} = 199.7 \text{ (KN·m/m)}$

2) DESIGN OF SECTION

	BOT	TOP
Mu =	199.7 KN·m	(100.0 KN·m)
Vu =	KN/m	

b =	1000	mm	fy =	365	N/mm ²
D =	700	mm	fcu =	30	N/mm ²
d =	640	mm			

{ REQUIRED RE-BAR }

$K = Mu / (f_{cu} \cdot b \cdot d^2) = 0.016 < 0.156$
 $Z1 = d \cdot [0.5 + \sqrt{ (0.25 - K/0.9) }] = 628$
 $Z2 = 0.95 \cdot d = 608$
 $Z = 608$
 $As1 = Mu / 0.95 \cdot fy \cdot Z = 947 \text{ mm}^2$
 $Asmin = 0.0013 \cdot b \cdot D = 910 \text{ mm}^2$

USE : D 20 @ 100 (As = 3140 mm²) BOT
 (D 20 @ 200 As = 1570 mm²) TOP

{ CHECK OF SHEAR STRESS }

$V = Vu / (b \cdot d) = 0.00 < 4.4 \text{ N/mm}^2 \text{ OK}$
 $(100 \cdot As) / (b \cdot d) = 0.49 < 3.0$
 $400/d = 0.6 < 1.0 \rightarrow 1.0$

$V_c = 0.79 \cdot \{ (100 \cdot As) / (b \cdot d) \}^{1/3} \cdot (400/d)^{1/4} \cdot (1/1.25) \cdot (f_{cu}/25)^{1/3}$
 $= 0.53 \text{ N/mm}^2 > V = 0.00 \text{ N/mm}^2 \text{ OK}$

3) CHECK OF CRACKING

(B)-(B)

5-15

6-1

FOOTING SLAB (FS70)

a) CHECK OF MINIMUM REINFORCEMENT (BASED ON 2.6.2.3 OF BS8007)

SECTION	b = 1000 mm	SURFACE ZONE :	d(T) = 250 mm
	D = 700 mm		d(B) = 100 mm
	d = 640 mm		

$$\text{Min. } \rho = 0.0035 \text{ (0.35 \%)}$$

$$\begin{aligned} \text{As (top)} &= \rho \cdot b \cdot d(T) = 875 \text{ mm}^2 \\ \text{As (bot)} &= \rho \cdot b \cdot d(B) = 350 \text{ mm}^2 \end{aligned}$$

USE ;	TOP	D 20	@ 200	(As = 1570 mm ²)
USE ;	BOTTOM	D 20	@ 100	(As = 3140 mm ²)

6. SLUDGE THICKENER6. ИЛОУПЛОТНИТЕЛЬ

b) CHECK OF STEEL STRESS IN DIRECT OR FLEXURAL TENSION FOR SERVICEABILITY LIMIT STATES (BASED ON 3.2.2 OF BS8007)

Top	Mu = 100.0 KN·m	M = 71.5 KN·m	b = 1000 mm
Bott.	Mu = 199.7 KN·m	M = 142.7 KN·m	D = 700 mm
			d = 640 mm
			fy = 130 N/mm ²
			fcu = 30 N/mm ²

TOP

$$\begin{aligned} K &= M / (f_{cu} \cdot b \cdot d^2) &= 0.0058 \\ Z_1 &= d \cdot [0.5 + \sqrt{ (0.25 - K/0.9) }] &= 636 \\ Z_2 &= 0.95 \cdot d &= 608 \\ Z &= &= 608 \\ \text{As} &= M / f_y \cdot Z &= 904.08 \text{ mm}^2 \end{aligned}$$

USE ;	D 20	@ 200	(As = 1570 mm ²)
-------	------	-------	------------------------------

BOTTOM

$$\begin{aligned} K &= M / (f_{cu} \cdot b \cdot d^2) &= 0.0116 \\ Z_1 &= d \cdot [0.5 + \sqrt{ (0.25 - K/0.9) }] &= 632 \\ Z_2 &= 0.95 \cdot d &= 608 \\ Z &= &= 608 \\ \text{As} &= M / f_y \cdot Z &= 1805 \text{ mm}^2 \end{aligned}$$

USE ;	D 20	@ 100	(As = 3140 mm ²)
-------	------	-------	------------------------------

c) CHECK OF CRACK WIDTH (BASE ON APPENDIX A OF BS8007)

1) CRACK SPACING

$$\begin{aligned} S_{\text{max}}(T) &= (f_{ct}/f_b) \cdot (\rho/2p(T)) &= 1066.9 \text{ mm} \\ S_{\text{max}}(B) &= (f_{ct}/f_b) \cdot (\rho/2p(B)) &= 213.4 \text{ mm} \end{aligned}$$

$$\begin{aligned} \text{Where ; } f_{ct}/f_b &= 0.67 & \rho(T) &= \text{As}/b \cdot d(T) = 0.0063 \\ & & \rho(B) &= \text{As}/b \cdot d(B) = 0.0314 \end{aligned}$$

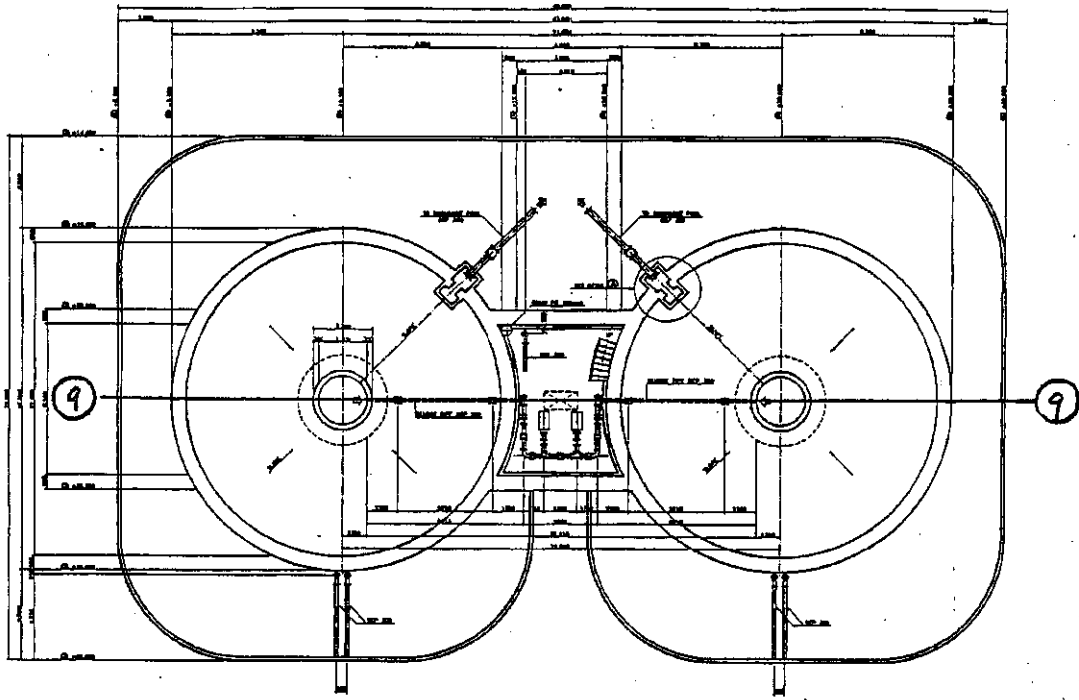
2) CRACK WIDTHS ARISING FROM RESTRAINED SHRINKAGE AND HEAT OF HYDRATION MOVEMENT

$$\begin{aligned} W_{\text{max}}(T) &= S_{\text{max}}(T) \cdot (d/2) \cdot T_1 &= 0.13 < 0.2 \text{ mm} \\ W_{\text{max}}(B) &= S_{\text{max}}(B) \cdot (d/2) \cdot T_1 &= 0.03 < 0.2 \text{ mm} \end{aligned}$$

Where ; α : COEFFICIENT OF THERMAL EXPANSION OF MATURE CONCRETE

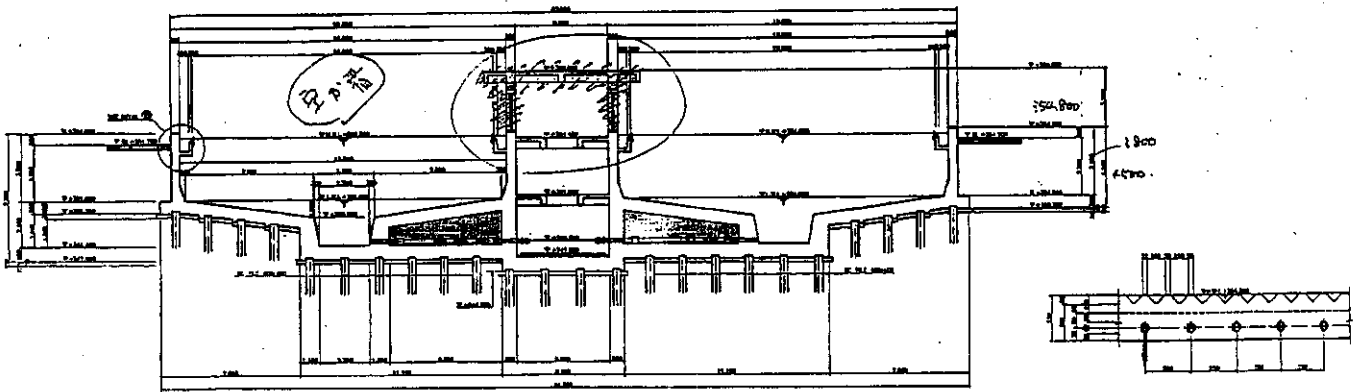
$$\begin{aligned} \alpha &= 1E-05 \\ T_1 &= 25 \text{ from TABLE 4.2} \end{aligned}$$

6-1 Plan and Section
6-1 План и разрез

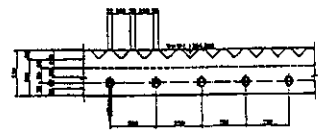


BOTTOM PLAN
SCALE 1/16

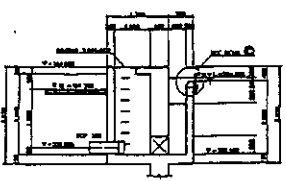
6-2



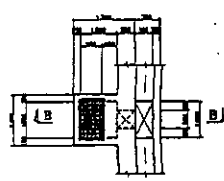
9-9 SECTION A-A
SCALE 1/16



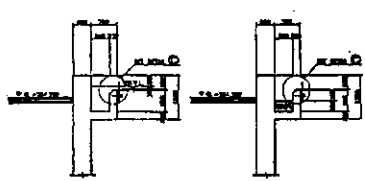
SECTION C-C
SCALE 1/16



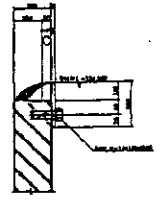
SECTION B-B
SCALE 1/16



MANHOLE
SCALE 1/16



DETAIL (B)
SCALE 1/16



DETAIL (C)
SCALE 1/16

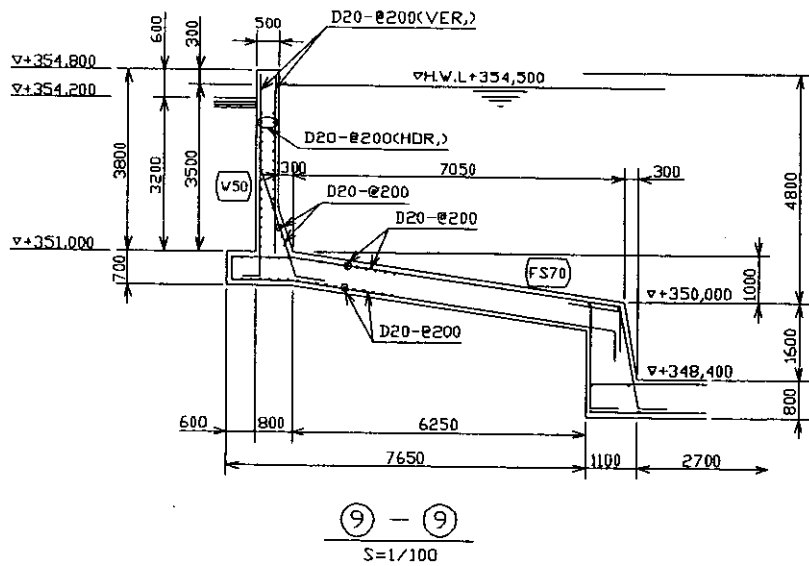
DETAIL (A)
SCALE 1/16

A.7-35

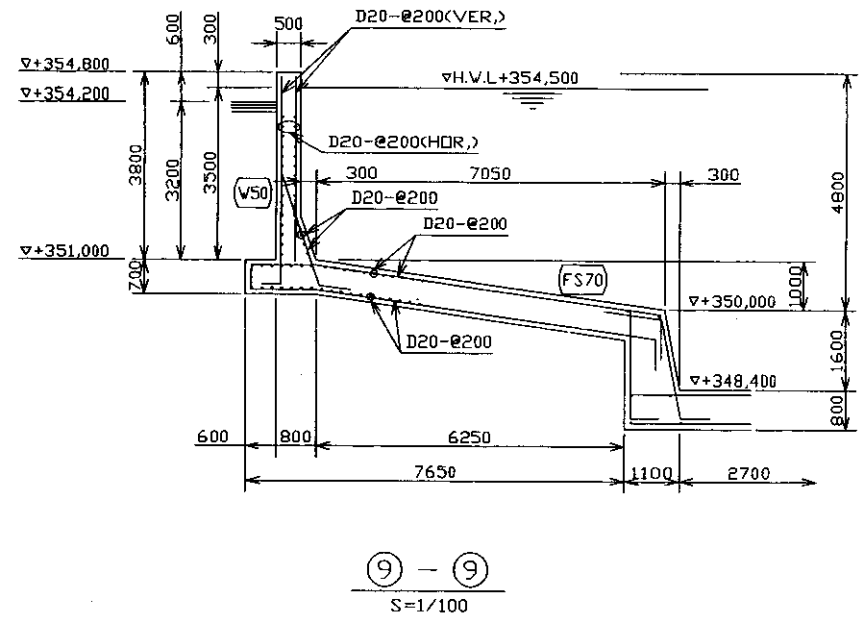
6-3

6.2 DESIGN OF WALL & FOOTING SLAB
 6.2 ПРОЕКТИРОВАНИЕ СТЕНЫ И ПЛИТЫ ОСНОВАНИЯ

A.7-36



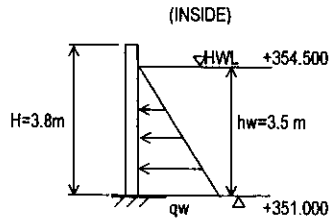
6.2 DESIGN OF WALL & FOOTING SLAB
 6.2 ПРОЕКТИРОВАНИЕ СТЕНЫ И ПЛИТЫ ОСНОВАНИЯ



6.2.1 CALCULATION OF WALL (W50) (9)-(9)

(AT+351.000)

1) DESIGN LOAD & FACTORED STRESS (Water Pressure)



$$\begin{aligned} H &= 3.8 \text{ m} \\ h_w &= 3.50 \text{ m} \\ \gamma_w &= 10.0 \text{ KN/m}^3 \end{aligned}$$

$$\begin{aligned} \text{Design load} \\ q_w = \gamma_w \cdot h_w &= 10.0 \times 3.50 = 35.0 \text{ (KN/m}^2\text{/m)} \end{aligned}$$

$$\begin{aligned} \text{Factored Load} \\ w' = 1.4 \cdot q_w &= 1.4 \times 35.0 = 49.00 \text{ (KN/m}^2\text{/m)} \end{aligned}$$

Factored stress at +351.000

$$\begin{cases} M_u = 1/6 \cdot w' \cdot h_w^2 = 100.0 \text{ (KN} \cdot \text{m/m)} \\ V_u = 1/2 \cdot w' \cdot h_w = 85.8 \text{ (KN/m)} \end{cases}$$

2) DESIGN OF SECTION (INSIDE/VERTICAL BOT.)

$$\begin{aligned} M_u &= 100.0 \text{ KN} \cdot \text{m} \\ V_u &= 85.8 \text{ KN/m} \end{aligned}$$

$$\begin{aligned} b &= 1000 \text{ mm} & f_y &= 365 \text{ N/mm}^2 \\ D &= 500 \text{ mm} & f_{cu} &= 30 \text{ N/mm}^2 \\ d &= 440 \text{ mm} \end{aligned}$$

[REQUIRED RE-BAR]

$$\begin{aligned} K &= M_u / (f_{cu} \cdot b \cdot d^2) = 0.017 < 0.156 \\ Z_1 &= d \cdot [0.5 + \sqrt{(0.25 - K/0.9)}] = 431 \\ Z_2 &= 0.95 \cdot d = 418 \\ Z &= 418 \\ A_{s1} &= M_u / (0.95 \cdot f_y \cdot Z) = 690 \text{ mm}^2 \\ A_{smin} &= 0.0013 \cdot b \cdot D = 650 \text{ mm}^2 \end{aligned}$$

USE ; D 20 @ 200 (As = 1570 mm²)

[CHECK OF SHEAR STRESS]

$$\begin{aligned} V &= V_u / (b \cdot d) = 0.19 < 4.4 \text{ N/mm}^2 \text{ OK} \\ (100 \cdot A_s) / (b \cdot d) &= 0.36 < 3.0 \\ 400/d &= 0.9 < 1.0 \rightarrow 1.0 \end{aligned}$$

$$\begin{aligned} V_c &= 0.79 \cdot \{ (100 \cdot A_s) / (b \cdot d) \}^{1/3} \cdot \{ 400/d \}^{1/4} \cdot (1/1.25) - (f_{cu}/25)^{1/3} \\ &= 0.48 \text{ N/mm}^2 > V = 0.19 \text{ N/mm}^2 \text{ OK} \end{aligned}$$

A.7-37

3) CHECK OF CRACKING (9)-(9)

W50 (INSIDE/VERTICAL BOT)

a) CHECK OF MINIMUM REINFORCEMENT (BASED ON 2.6.2.3 OF BS8007)

SECTION	b = 1000 mm	SURFACE ZONE :	d1 = 250 mm
	D = 500 mm		
	dv = 440 mm		
	dh = 420 mm		

$$\text{Min. } \rho = 0.0035 \text{ (0.35 \%)}$$

$$\begin{aligned} A_s \text{ (ver.)} &= \rho \cdot b \cdot d1 = 875 \text{ mm}^2 \\ A_s \text{ (her.)} &= \rho \cdot b \cdot d1 = 875 \text{ mm}^2 \end{aligned}$$

USE ; VERTICAL D 20 @ 200 (As = 1570 mm²)
USE ; HORIZONTAL D 20 @ 200 (As = 1570 mm²)

b) CHECK OF STEEL STRESS IN DIRECT OR FLEXURAL TENSION FOR SERVICEABILITY LIMIT STATES (BASED ON 3.2.2 OF BS8007)

Ver.	Mu = 100.0 KN·m	M = 71.5 KN·m	b = 1000 mm
Hor.	Mu = 0.0 KN·m	M = 0.0 KN·m	D = 500 mm
			dv = 440 mm
			dh = 420 mm
			fy = 130 N/mm ²
			fcu = 30 N/mm ²

VERTICAL

$$\begin{aligned} K &= M / (f_{cu} \cdot b \cdot dv^2) = 0.012 \\ Z_1 &= dv \cdot [0.5 + \sqrt{(0.25 - K/0.9)}] = 434 \\ Z_2 &= 0.95 \cdot dv = 418 \\ Z &= 418 \\ A_s &= M / (fy \cdot Z) = 1315 \text{ mm}^2 \end{aligned}$$

USE ; D 20 @ 200 (As = 1570 mm²)

HORIZONTAL

$$\begin{aligned} K &= M / (f_{cu} \cdot b \cdot dh^2) = \wedge \\ Z_1 &= dh \cdot [0.5 + \sqrt{(0.25 - K/0.9)}] = \wedge \\ Z_2 &= 0.95 \cdot dh = \wedge \\ Z &= \wedge \\ A_s &= M / (fy \cdot Z) = \wedge \text{ mm}^2 \end{aligned}$$

USE ; D 20 @ 200 (As = 1570 mm²)

c) CHECK OF CRACK WIDTH (BASE ON APPENDIX A OF BS8007)

1) CRACK SPACING

$$\begin{aligned} S_{max}(V) &= (f_{ct}/f_b) \cdot (\phi/2\rho(V)) = 1066.9 \text{ mm} \\ S_{max}(H) &= (f_{ct}/f_b) \cdot (\phi/2\rho(H)) = 1066.9 \text{ mm} \end{aligned}$$

$$\begin{aligned} \text{Where ; } f_{ct}/f_b &= 0.67 & \rho(V) &= A_s/b \cdot d1 = 0.0063 \\ & & \rho(H) &= A_s/b \cdot d1 = 0.0063 \end{aligned}$$

2) CRACK WIDTHS ARISING FROM RESTRAINED SHRINKAGE AND HEAT OF HYDRATION MOVEMENT

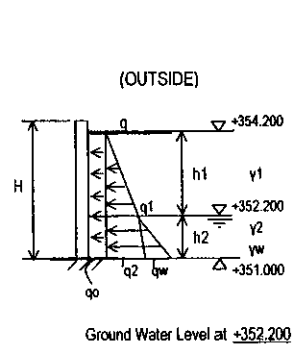
$$\begin{aligned} W_{max}(V) &= S_{max}(V) \cdot (\alpha/2) \cdot T_1 = 0.17 < 0.2 \text{ mm} \\ W_{max}(H) &= S_{max}(H) \cdot (\alpha/2) \cdot T_1 = 0.17 < 0.2 \text{ mm} \end{aligned}$$

Where ; α : COEFFICIENT OF THERMAL EXPANSION OF MATURE CONCRETE

$$\begin{aligned} \alpha &= 1E-05 \\ T_1 &= 32 \text{ from TABLE 4.2} \end{aligned}$$

6.2.2 CALCULATION OF WALL (W50) (9)-(9)
(AT+351.000)

1) DESIGN LOAD & FACTORED STRESS(Soil Pressure)



q : Live Load
Ko: Coefficient of soil pressure at rest
y1= 18.0 KN/m³ q = 5.0 (KN/m²/m)
y2= 8.0 KN/m³ Ko = 0.5
yw= 10.0 KN/m³

-Design load
qo = Ko · q = 2.5 (KN/m²/m)
q1 = Ko · y1 · h1 = 18.0
q2 = q1 + Ko · y2 · h2 = 22.8
qw = yw · h2 = 12.0
Σq = q2 + qw = 34.8

-Factored Load
w1 = 1.4 · qo = 3.5 (KN/m²/m)
w2 = 1.4 · Σq = 48.7

-Factored stress at +351.000
Mu1 = 1/2 · w1 · h² = 17.92 (KN · m/m)
Vu1 = w1 · h = 11.20 (KN/m)
Mu2 = 1/6 · w2 · h² = 83.15 (KN · m/m)
Vu2 = 1/2 · w2 · h = 77.95 (KN/m)
Mu = Mu1 + Mu2 = 101.1 (KN · m/m)
Vu = Vu1 + Vu2 = 89.2 (KN/m)

H= 3.8 m
h1= 2.00 m
h2= 1.20 m
h=h1+h2= 3.20 m

A.7-38

2) DESIGN OF SECTION (OUTSIDE/VERTICAL BOT.)

Mu = 101.1 KN · m
Vu = 89.2 KN/m
b = 1000 mm fy = 365 N/mm²
D = 500 mm fcu = 30 N/mm²
d = 440 mm

[REQUIRED RE-BAR]

K = Mu / (fcu · b · d²) = 0.017 < 0.156
Z1 = d · [0.5 + √(0.25 - K/0.9)] = 431
Z2 = 0.95 · d = 418
Z = 418
As1 = Mu / (fy · Z) = 897 mm²
Asmin = 0.0013 · b · D = 650 mm²

USE ; D 20 @ 100 (As = 3140 mm²)

[CHECK OF SHEAR STRESS]

V = Vu / (b · d) = 0.20 < 4.4 N/mm² OK
(100 · As) / (b · d) = 0.71 < 3.0
400/d = 0.9 < 1.0 → 1.0

Vc = 0.79 · ((100 · As) / (b · d))^{1/3} · (400/d)^{1/4} · (1/1.25) · (fcu/25)^{1/3}
= 0.60 N/mm² > V = 0.20 N/mm² OK

W50 (OUTSIDE/VERTICAL BOT)

a) CHECK OF MINIMUM REINFORCEMENT (BASED ON 2.6.2.3 OF BS8007)

SECTION b = 1000 mm SURFACE ZONE : d1 = 250 mm
D = 500 mm
dv = 440 mm
dh = 420 mm

Min. ρ = 0.0035 (0.35 %)

As (ver.) = ρ · b · d1 = 875 mm²
As (her.) = ρ · b · d1 = 875 mm²

USE ; VERTICAL D 20 @ 200 (As = 1570 mm²)
USE ; HORIZONTAL D 20 @ 200 (As = 1570 mm²)

b) CHECK OF STEEL STRESS IN DIRECT OR FLEXURAL TENSION FOR SERVICEABILITY LIMIT STATES (BASED ON 3.2.2 OF BS8007)

Ver. Mu = 101.1 KN · m M = 72.2 KN · m b = 1000 mm
Hor. Mu = 0.0 KN · m M = 0.0 KN · m D = 500 mm
dv = 440 mm
dh = 420 mm
fy = 130 N/mm²
fcu = 30 N/mm²

VERTICAL

K = M / (fcu · b · dv²) = 0.0124
Z1 = dv · [0.5 + √(0.25 - K/0.9)] = 434
Z2 = 0.95 · dv = 418
Z = 418
As = M / fy · Z = 1328.5 mm²

USE ; D 20 @ 200 (As = 1570 mm²)

HORIZONTAL

K = M / (fcu · b · dh²) = 0
Z1 = dh · [0.5 + √(0.25 - K/0.9)] = 0
Z2 = 0.95 · dh = 0
Z = 0
As = M / fy · Z = 0 mm²

USE ; D 20 @ 200 (As = 1570 mm²)

c) CHECK OF CRACK WIDTH (BASE ON APPENDIX A OF BS8007)

1) CRACK SPACING

Smax(V) = (fct/fb) · (φ/2ρ(V)) = 1066.9 mm
Smax(H) = (fct/fb) · (φ/2ρ(H)) = 1066.9 mm

Where ; fct/fb = 0.67 p(V) = As/b · d1 = 0.0063
p(H) = As/b · d1 = 0.0063

2) CRACK WIDTHS ARISING FORM RESTRAINED SHRINKAGE AND HEAT OF HYDRATION MOVEMENT

Wmax(V) = Smax(V) · (α/2) · T1 = 0.17 < 0.2 mm
Wmax(H) = Smax(H) · (α/2) · T1 = 0.17 < 0.2 mm

Where ; α : COEFFICIENT OF THERMAL EXPANSION OF MATURE CONCRETE

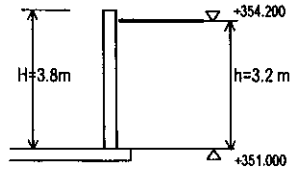
α = 1E-05
T1 = 32 from TABLE 4.2

6.2.3 CALCULATION OF FOOTING SLAB (FS70)

(AT+351.000)

⑨-⑨

1) FACTORED STRESS



Factored stress at +351.000



from Moment of Inside of Wall (refer to para.5.2.1)
 $Mu_{(TOP)} = 100.0 \text{ (KN} \cdot \text{m/m)}$
 from Moment of Outside of Wall (refer to para.5.2.2)
 $Mu_{(BOT)} = 101.1 \text{ (KN} \cdot \text{m/m)}$

2) DESIGN OF SECTION

BOT TOP
 $Mu = 101.1 \text{ KN} \cdot \text{m}$ (100.0 KN·m)
 $Vu = \text{KN/m}$

$b = 1000 \text{ mm}$ $fy = 365 \text{ N/mm}^2$
 $D = 700 \text{ mm}$ $fcu = 30 \text{ N/mm}^2$
 $d = 640 \text{ mm}$

[REQUIRED RE-BAR]

$K = Mu / (fcu \cdot b \cdot d^2)$ = 0.008 < 0.156
 $Z1 = d \cdot [0.5 + \sqrt{(0.25 - K/0.9)}]$ = 634
 $Z2 = 0.95 \cdot d$ = 608
 $Z = 608$
 $As1 = Mu / 0.95 \cdot fy \cdot Z$ = 479 mm²
 $Asmin = 0.0013 \cdot b \cdot D$ = 910 mm²

USE ; D 20 @ 200 (As = 1570 mm²) BOT
 (D 20 @ 200 As = 1570 mm²) TOP

[CHECK OF SHEAR STRESS]

$V = Vu / (b \cdot d)$ = 0.00 < 4.4 N/mm² OK
 $(100 \cdot As) / (b \cdot d)$ = 0.25 < 3.0
 $400/d$ = 0.6 < 1.0 → 1.0

$Vc = 0.79 \cdot \{(100 \cdot As) / (b \cdot d)\}^{1/3} \cdot (400/d)^{1/4} \cdot (1/1.25) \cdot (fcu/25)^{1/3}$
 $= 0.42 \text{ N/mm}^2 > V = 0.00 \text{ N/mm}^2 \text{ OK}$

A.7-39

FOOTING SLAB (FS70)

a) CHECK OF MINIMUM REINFORCEMENT (BASED ON 2.6.2.3 OF BS8007)

SECTION $b = 1000 \text{ mm}$ SURFACE ZONE : $d(T) = 250 \text{ mm}$
 $D = 700 \text{ mm}$ $d(B) = 100 \text{ mm}$
 $d = 640 \text{ mm}$

Min. $\rho = 0.0035$ (0.35 %)

$As \text{ (top)} = \rho \cdot b \cdot d(T) = 875 \text{ mm}^2$
 $As \text{ (bot)} = \rho \cdot b \cdot d(B) = 350 \text{ mm}^2$

USE ; TOP D 20 @ 200 (As = 1570 mm²)
 USE ; BOTTOM D 20 @ 200 (As = 1570 mm²)

b) CHECK OF STEEL STRESS IN DIRECT OR FLEXURAL TENSION FOR SERVICEABILITY LIMIT STATES (BASED ON 3.2.2 OF BS8007)

Top $Mu = 100.0 \text{ KN} \cdot \text{m}$ $M = 71.5 \text{ KN} \cdot \text{m}$ $b = 1000 \text{ mm}$
 Bott. $Mu = 101.1 \text{ KN} \cdot \text{m}$ $M = 72.2 \text{ KN} \cdot \text{m}$ $D = 700 \text{ mm}$
 $d = 640 \text{ mm}$
 $fy = 130 \text{ N/mm}^2$
 $fcu = 30 \text{ N/mm}^2$

TOP
 $K = M / (fcu \cdot b \cdot d^2)$ = 0.0058
 $Z1 = d \cdot [0.5 + \sqrt{(0.25 - K/0.9)}]$ = 636
 $Z2 = 0.95 \cdot d$ = 608
 $Z = 608$
 $As = M / fy \cdot Z$ = 904.08 mm²

USE ; D 20 @ 200 (As = 1570 mm²)

BOTTOM
 $K = M / (fcu \cdot b \cdot d^2)$ = 0.0059
 $Z1 = d \cdot [0.5 + \sqrt{(0.25 - K/0.9)}]$ = 636
 $Z2 = 0.95 \cdot d$ = 608
 $Z = 608$
 $As = M / fy \cdot Z$ = 913.36 mm²

USE ; D 20 @ 200 (As = 1570 mm²)

c) CHECK OF CRACK WIDTH (BASE ON APPENDIX A OF BS8007)

1) CRACK SPACING

$Smax(T) = (fct/fb) \cdot (\phi/2\rho(T))$ = 1066.9 mm
 $Smax(B) = (fct/fb) \cdot (\phi/2\rho(B))$ = 426.8 mm

Where ; $fct/fb = 0.67$ $\rho(T) = As/b \cdot d(T) = 0.0063$
 $\rho(B) = As/b \cdot d(B) = 0.0157$

2) CRACK WIDTHS ARISING FROM RESTRAINED SHRINKAGE AND HEAT OF HYDRATION MOVEMENT

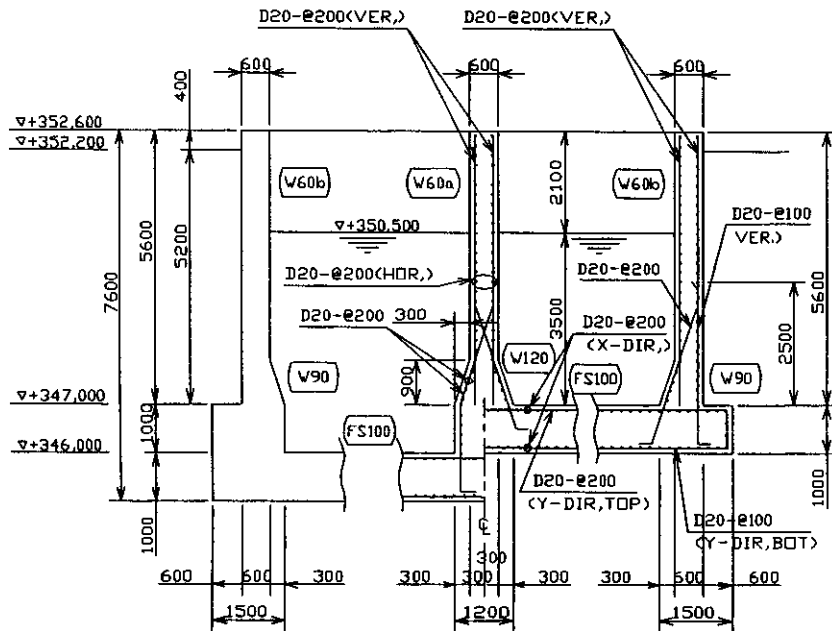
$Wmax(T) = Smax(T) \cdot (\alpha/2) \cdot T1$ = 0.13 < 0.2 mm
 $Wmax(B) = Smax(B) \cdot (\alpha/2) \cdot T1$ = 0.05 < 0.2 mm

Where ; α : COEFFICIENT OF THERMAL EXPANSION OF MATURE CONCRETE

$\alpha = 1E-05$
 $T1 = 25$ from TABLE 4.2

7.2 DESIGN OF WALL & FOOTING
7.2 ПРОЕКТИРОВАНИЕ СТЕНЫ И ОСНОВАНИЯ

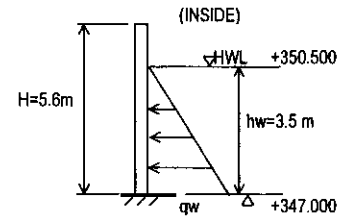
A.7.42



⑩ - ⑩
S=1/100

7.2.1 CALCULATION OF WALL (W90)
(AT+347.000) ⑩-⑩

1) DESIGN LOAD & FACTORED STRESS (Water Pressure)



H= 5.60 m
hw= 3.50 m
yw= 10.0 KN/m³

• Design load
 $q_w = y_w \cdot h_w = 10.0 \times 3.50 = 35.0 \text{ (KN/m}^2\text{/m)}$

• Factored Load
 $w = 1.4 \cdot q_w = 1.4 \times 35.0 = 49.0 \text{ (KN/m}^2\text{/m)}$

• Factored stress at +347.000

$\left\{ \begin{aligned} M_u &= 1/6 \cdot w \cdot h_w^2 = 100.0 \text{ (KN}\cdot\text{m/m)} \\ V_u &= 1/2 \cdot w \cdot h_w = 85.8 \text{ (KN/m)} \end{aligned} \right.$

2) DESIGN OF SECTION (INSIDE/VERTICAL BOT.)

$M_u = 100.0 \text{ KN}\cdot\text{m}$
 $V_u = 85.8 \text{ KN/m}$

$b = 1000 \text{ mm}$ $f_y = 365 \text{ N/mm}^2$
 $D = 900 \text{ mm}$ $f_{cu} = 30 \text{ N/mm}^2$
 $d = 840 \text{ mm}$

[REQUIRED RE-BAR]

$K = M_u / (f_{cu} \cdot b \cdot d^2) = 0.005 < 0.156$
 $Z1 = d \cdot [0.5 + \sqrt{(0.25 - K/0.9)}] = 836$
 $Z2 = 0.95 \cdot d = 798$
 $Z = 798$
 $A_{s1} = M_u / (0.95 \cdot f_y \cdot Z) = 362 \text{ mm}^2$
 $A_{smin} = 0.0013 \cdot b \cdot D = 1170 \text{ mm}^2$

USE ; D 20 @ 200 (As = 1570 mm²)

[CHECK OF SHEAR STRESS]

$V = V_u / (b \cdot d) = 0.10 < 4.4 \text{ N/mm}^2 \text{ OK}$
 $(100 \cdot A_s) / (b \cdot d) = 0.19 < 3.0$
 $400/d = 0.5 < 1.0 \rightarrow 1.0$

$V_c = 0.79 \cdot \{ (100 \cdot A_s) / (b \cdot d) \}^{1/3} \cdot (400/d)^{1/4} \cdot (1/1.25) \cdot (f_{cu}/25)^{1/3}$
 $= 0.38 \text{ N/mm}^2 > V = 0.10 \text{ N/mm}^2 \text{ OK}$

3) CHECK OF CRACKING

(10-10)

7-7

W90 (INSIDE/VERTICAL BOT)

(AT+347.000)

a) CHECK OF MINIMUM REINFORCEMENT (BASED ON 2.6.2.3 OF BS8007)

SECTION b = 1000 mm SURFACE ZONE : d1 = 250 mm
 D = 900 mm
 dv = 840 mm
 dh = 820 mm

Min. p = 0.0035 (0.35 %)

As (ver.) = p · b · d1 = 875 mm²
 As (her.) = p · b · d1 = 875 mm²

USE ; VERTICAL D 20 @ 200 (As = 1570 mm²)
 USE ; HORIZONTAL D 20 @ 200 (As = 1570 mm²)

b) CHECK OF STEEL STRESS IN DIRECT OR FLEXURAL TENSION FOR SERVICEABILITY LIMIT STATES (BASED ON 3.2.2 OF BS8007)

Ver. Mu = 100.0 KN·m M = 71.4 KN·m b = 1000 mm
 Hor. Mu = 0.0 KN·m M = 0.0 KN·m D = 900 mm
 dv = 840 mm
 dh = 820 mm
 fy = 130 N/mm²
 fcu = 30 N/mm²

VERTICAL

$K = M / (fcu \cdot b \cdot dv^2) = 0.0034$
 $Z1 = dv \cdot [0.5 + \sqrt{ (0.25 - K/0.9) }] = 837$
 $Z2 = 0.95 \cdot dv = 798$
 $Z = 798$
 $As = M / fy \cdot Z = 688.26 \text{ mm}^2$

USE ; D 20 @ 200 (As = 1570 mm²)

HORIZONTAL

$K = M / (fcu \cdot b \cdot dh^2) = 0$
 $Z1 = dh \cdot [0.5 + \sqrt{ (0.25 - K/0.9) }] = 0$
 $Z2 = 0.95 \cdot dh = 0$
 $Z = 0$
 $As = M / fy \cdot Z = 0 \text{ mm}^2$

USE ; D 20 @ 200 (As = 1570 mm²)

c) CHECK OF CRACK WIDTH (BASE ON APPENDIX A OF BS8007)

1) CRACK SPACING

$S_{max}(V) = (fct/fb) \cdot (\phi/2\rho(V)) = 1066.9 \text{ mm}$
 $S_{max}(H) = (fct/fb) \cdot (\phi/2\rho(H)) = 1066.9 \text{ mm}$

Where ; fct/fb = 0.67 $\rho(V) = As/b \cdot d1 = 0.0063$
 $\rho(H) = As/b \cdot d1 = 0.0063$

2) CRACK WIDTHS ARISING FORM RESTRAINED SHRINKAGE AND HEAT OF HYDRATION MOVEMENT

$W_{max}(V) = S_{max}(V) \cdot (\alpha/2) \cdot T1 = 0.17 < 0.2 \text{ mm}$
 $W_{max}(H) = S_{max}(H) \cdot (\alpha/2) \cdot T1 = 0.17 < 0.2 \text{ mm}$

Where ; α : COEFFICIENT OF THERMAL EXPANSION OF MATURE CONCRETE

$\alpha = 1E-05$
 $T1 = 32$ from TABLE 4.2

A.7-43

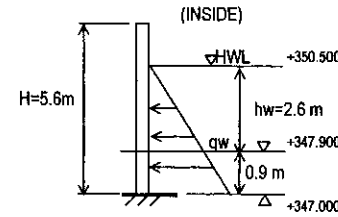
7-8

7.2.2 CALCULATION OF WALL (W60a)

(10-10)

(AT+347.900)

1) DESIGN LOAD & FACTORED STRESS(Water Pressure)



H= 5.60 m
 hw= 2.60 m
 $\gamma_w = 10.0 \text{ KN/m}^3$

• Design load
 $q_w = \gamma_w \cdot h_w = 10.0 \times 2.60 = 26.0 \text{ (KN/m}^2\text{/m)}$

• Factored Load
 $w' = 1.4 \cdot q_w = 1.4 \times 26.0 = 36.40 \text{ (KN/m}^2\text{/m)}$

• Factored stress at +347.900

$M_u = 1/6 \cdot w' \cdot h_w^2 = 41.0 \text{ (KN} \cdot \text{m/m)}$
 $V_u = 1/2 \cdot w' \cdot h_w = 47.3 \text{ (KN/m)}$

2) DESIGN OF SECTION (INSIDE/VERTICAL TOP)

$M_u = 41.0 \text{ KN} \cdot \text{m}$
 $V_u = 47.3 \text{ KN/m}$

b = 1000 mm $f_y = 365 \text{ N/mm}^2$
 D = 600 mm $f_{cu} = 30 \text{ N/mm}^2$
 d = 540 mm

[REQUIRED RE-BAR]

$K = M_u / (fcu \cdot b \cdot d^2) = 0.005 < 0.156$
 $Z1 = d \cdot [0.5 + \sqrt{ (0.25 - K/0.9) }] = 537$
 $Z2 = 0.95 \cdot d = 513$
 $Z = 513$
 $As1 = M_u / 0.95 \cdot f_y \cdot Z = 231 \text{ mm}^2$
 $As_{min} = 0.0013 \cdot b \cdot D = 780 \text{ mm}^2$

USE ; D 20 @ 200 (As = 1570 mm²)

[CHECK OF SHEAR STRESS]

$V = V_u / (b \cdot d) = 0.09 < 4.4 \text{ N/mm}^2 \text{ OK}$
 $(100 \cdot As) / (b \cdot d) = 0.29 < 3.0$
 $400/d = 0.7 < 1.0 \rightarrow 1.0$

$V_c = 0.79 \cdot \{ (100 \cdot As) / (b \cdot d) \}^{1/3} \cdot (400/d)^{1/4} \cdot (1/1.25) \cdot (fcu/25)^{1/3}$
 $= 0.45 \text{ N/mm}^2 > V = 0.09 \text{ N/mm}^2 \text{ OK}$

3) CHECK OF CRACKING

(10-10)

W60a (INSIDE/VERTICAL TOP)

(AT+347.900)

a) CHECK OF MINIMUM REINFORCEMENT (BASED ON 2.6.2.3 OF BS8007)

SECTION	b =	1000	mm	SURFACE ZONE :	d1 =	250	mm
	D =	600	mm				
	dv =	540	mm				
	dh =	520	mm				

Min. $\rho = 0.0035$ (0.35 %)

As (ver.) = $\rho \cdot b \cdot d1 = 875$ mm²
 As (her.) = $\rho \cdot b \cdot d1 = 875$ mm²

USE :	VERTICAL	D 20	@ 200	(As = 1570 mm ²)
USE :	HORIZONTAL	D 20	@ 200	(As = 1570 mm ²)

b) CHECK OF STEEL STRESS IN DIRECT OR FLEXURAL TENSION FOR SERVICEABILITY LIMIT STATES (BASED ON 3.2.2 OF BS8007)

Ver.	Mu =	41.0	KN·m	M =	29.3	KN·m	b =	1000	mm
Hor.	Mu =	0.0	KN·m	M =	0.0	KN·m	D =	600	mm
							dv =	540	mm
							dh =	520	mm
							fy =	130	N/mm ²
							fcu =	30	N/mm ²

VERTICAL

$K = M / (fcu \cdot b \cdot dv^2) = 0.0033$
 $Z1 = dv \cdot [0.5 + \sqrt{(0.25 - K/0.9)}] = 538$
 $Z2 = 0.95 \cdot dv = 513$
 $Z = 513$
 $As = M / fy \cdot Z = 439.35$ mm²

USE : D 20 @ 200 (As = 1570 mm²)

HORIZONTAL

$K = M / (fcu \cdot b \cdot dh^2) = 0$
 $Z1 = dh \cdot [0.5 + \sqrt{(0.25 - K/0.9)}] = 0$
 $Z2 = 0.95 \cdot dh = 0$
 $Z = 0$
 $As = M / fy \cdot Z = 0$ mm²

USE : D 20 @ 200 (As = 1570 mm²)

c) CHECK OF CRACK WIDTH (BASE ON APPENDIX A OF BS8007)

1) CRACK SPACING

$Smax(V) = (fct/fb) \cdot (\sigma/2\rho(V)) = 1066.9$ mm
 $Smax(H) = (fct/fb) \cdot (\sigma/2\rho(H)) = 1066.9$ mm

Where : $fct/fb = 0.67$
 $\rho(V) = As/b \cdot d1 = 0.0063$
 $\rho(H) = As/b \cdot d1 = 0.0063$

2) CRACK WIDTHS ARISING FORM RESTRAINED SHRINKAGE AND HEAT OF HYDRATION MOVEMENT

$Wmax(V) = Smax(V) \cdot (\alpha/2) \cdot T1 = 0.17 < 0.2$ mm
 $Wmax(H) = Smax(H) \cdot (\alpha/2) \cdot T1 = 0.17 < 0.2$ mm

Where ; α : COEFFICIENT OF THERMAL EXPANSION OF MATURE CONCRETE

$\alpha = 1E-05$
 $T1 = 32$ from TABLE 4.2

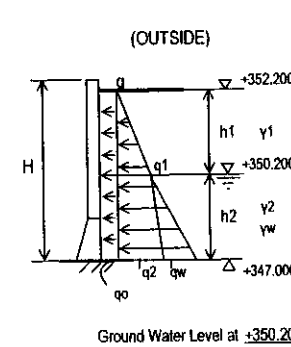
A.7-44

7.2.3 CALCULATION OF WALL (W90)

(10-10)

(AT+347.000)

1) DESIGN LOAD & FACTORED STRESS(Soil Pressure)



q : Live Load
 Ko: Coefficient of soil pressure at rest

y1 =	18.0	KN/m ³	q =	5.0	(KN/m ² /m)
y2 =	8.0	KN/m ³	Ko =	0.5	
yw =	10.0	KN/m ³			

• Design load

qo =	Ko · q	=	2.5	(KN/m ² /m)
q1 =	Ko · y1 · h1	=	18.0	
q2 =	q1 + Ko · y2 · h2	=	30.8	
qw =	yw · h2	=	32.0	
Σq =	q2 + qw	=	62.8	

• Factored Load

w1 =	1.4 · qo	=	3.5	(KN/m ² /m)
w2 =	1.4 · Σq	=	87.9	

• Factored stress at +347.000

Mu1 =	1/2 · w1 · h ²	=	47.32	(KN·m/m)
Vu1 =	w1 · h	=	18.20	(KN/m)
Mu2 =	1/6 · w2 · h ²	=	396.23	(KN·m/m)
Vu2 =	1/2 · w2 · h	=	228.59	(KN/m)

Mu =	Mu1 + Mu2	=	443.5	(KN·m/m)
Vu =	Vu1 + Vu2	=	246.8	(KN/m)

2) DESIGN OF SECTION (OUTIDE/VERTICAL BOT)

Mu =	443.5	KN·m	fy =	365	N/mm ²
Vu =	246.8	KN/m	fcu =	30	N/mm ²
b =	1000	mm			
D =	900	mm			
d =	840	mm			

[REQUIRED RE-BAR]

$K = Mu / (fcu \cdot b \cdot d^2) = 0.021 < 0.156$
 $Z1 = d \cdot [0.5 + \sqrt{(0.25 - K/0.9)}] = 820$
 $Z2 = 0.95 \cdot d = 798$
 $Z = 798$
 $As1 = Mu / 0.95 \cdot fy \cdot Z = 1603$ mm²
 $Asmin = 0.0013 \cdot b \cdot D = 1170$ mm²

USE : D 20 @ 100 (As = 3140 mm²)

[CHECK OF SHEAR STRESS]

$V = Vu / (b \cdot d) = 0.29 < 4.4$ N/mm² OK
 $(100 \cdot As) / (b \cdot d) = 0.37 < 3.0$
 $400/d = 0.5 < 1.0 \rightarrow 1.0$

$Vc = 0.79 \cdot \{ (100 \cdot As) / (b \cdot d) \}^{1/3} \cdot \{ 400/d \}^{1/4} \cdot (1/1.25) \cdot (fcu/25)^{1/3}$
 $= 0.48$ N/mm² > $V = 0.29$ N/mm² OK

3) CHECK OF CRACKING

(10-10)

7-11

W90 (OUTSIDE/VERTICAL BOT)

(AT+347.000)

a) CHECK OF MINIMUM REINFORCEMENT (BASED ON 2.6.2.3 OF BS8007)

SECTION b = 1000 mm SURFACE ZONE : d1 = 250 mm
 D = 900 mm
 dv = 840 mm
 dh = 820 mm

Min. ρ = 0.0035 (0.35 %)

As (ver.) = ρ · b · d1 = 875 mm²
 As (hor.) = ρ · b · d1 = 875 mm²

USE ; VERTICAL D 20 @ 100 (As = 3140 mm²)
 USE ; HORIZONTAL D 20 @ 200 (As = 1570 mm²)

b) CHECK OF STEEL STRESS IN DIRECT OR FLEXURAL TENSION FOR SERVICEABILITY LIMIT STATES (BASED ON 3.2.2 OF BS8007)

Ver. Mu = 443.5 KN·m M = 316.8 KN·m b = 1000 mm
 Hor. Mu = 0.0 KN·m M = 0.0 KN·m D = 900 mm
 dv = 840 mm
 dh = 820 mm
 fy = 130 N/mm²
 fcu = 30 N/mm²

VERTICAL

$K = M / (f_{cu} \cdot b \cdot d_v^2)$ = 0.015
 $Z1 = d_v \cdot [0.5 + \sqrt{(0.25 - K/0.9)}]$ = 826
 $Z2 = 0.95 \cdot d_v$ = 798
 $Z = 798$
 $As = M / f_y \cdot Z$ = 3053.8 mm²

USE ; D 20 @ 100 (As = 3140 mm²)

HORIZONTAL

$K = M / (f_{cu} \cdot b \cdot d_h^2)$ = 0
 $Z1 = d_h \cdot [0.5 + \sqrt{(0.25 - K/0.9)}]$ = 0
 $Z2 = 0.95 \cdot d_h$ = 0
 $Z = 0$
 $As = M / f_y \cdot Z$ = 0 mm²

USE ; D 20 @ 200 (As = 1570 mm²)

c) CHECK OF CRACK WIDTH (BASE ON APPENDIX A OF BS8007)

1) CRACK SPACING

Smax(V) = (fct/fb) · (σ/2ρ(V)) = 533.4 mm
 Smax(H) = (fct/fb) · (σ/2ρ(H)) = 1066.9 mm

Where ; fct/fb = 0.67 ρ(V) = As/b · d1 = 0.0126
 ρ(H) = As/b · d1 = 0.0063

2) CRACK WIDTHS ARISING FORM RESTRAINED SHRINKAGE AND HEAT OF HYDRATION MOVEMENT

Wmax(V) = Smax(V) · (α/2) · T1 = 0.09 < 0.2 mm
 Wmax(H) = Smax(H) · (α/2) · T1 = 0.17 < 0.2 mm

Where ; α : COEFFICIENT OF THERMAL EXPANSION OF MATURE CONCRETE

α = 1E-05
 T1 = 32 from TABLE 4.2

A.7-45

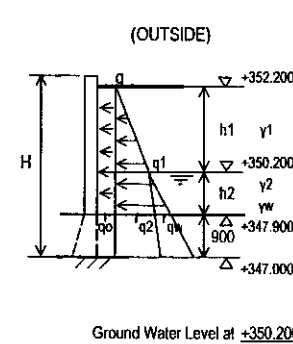
7-12

7.2.4 CALCULATION OF WALL (W60b)

(10-10)

(AT+347.900)

1) DESIGN LOAD & FACTORED STRESS(Soil Pressure)



q : Live Load

Ko: Coefficient of soil pressure at rest
 q = 5.0 (KN/m²/m)
 Ko = 0.5

γ1 = 18.0 KN/m³
 γ2 = 8.0 KN/m³
 γw = 10.0 KN/m³

• Design load

qo = Ko · q = 2.5 (KN/m²/m)
 q1 = Ko · γ1 · h1 = 18.0
 q2 = q1 + Ko · γ2 · h2 = 27.2
 qw = γw · h2 = 23.0
 Σq = q2 + qw = 50.2

• Factored Load

w1 = 1.4 · qo = 3.5 (KN/m²/m)
 w2 = 1.4 · Σq = 70.3

• Factored stress at +347.900

Mu1 = 1/2 · w1 · h² = 32.36 (KN·m/m)
 Vu1 = w1 · h = 15.05 (KN/m)
 Mu2 = 1/6 · w2 · h² = 216.58 (KN·m/m)
 Vu2 = 1/2 · w2 · h = 151.10 (KN/m)
 Mu = Mu1 + Mu2 = 248.9 (KN·m/m)
 Vu = Vu1 + Vu2 = 166.2 (KN/m)

2) DESIGN OF SECTION (OUTSIDE/VERTICAL BOT)

Mu = 248.9 KN·m
 Vu = 166.2 KN/m

b = 1000 mm fy = 365 N/mm²
 D = 600 mm fcu = 30 N/mm²
 d = 540 mm

[REQUIRED RE-BAR]

$K = Mu / (f_{cu} \cdot b \cdot d^2)$ = 0.028 < 0.156
 $Z1 = d \cdot [0.5 + \sqrt{(0.25 - K/0.9)}]$ = 522
 $Z2 = 0.95 \cdot d$ = 513
 $Z = 513$
 $As1 = Mu / 0.95 \cdot f_y \cdot Z$ = 1399 mm²
 $Asmin = 0.0013 \cdot b \cdot D$ = 780 mm²

USE ; D 20 @ 100 (As = 3140 mm²)

[CHECK OF SHEAR STRESS]

V = Vu / (b · d) = 0.31 < 4.4 N/mm² OK
 (100 · As) / (b · d) = 0.58 < 3.0
 400/d = 0.7 < 1.0 → 1.0

Vc = 0.79 · ((100 · As) / (b · d))^{1/3} · (400/d)^{1/4} · (1/1.25) · (fcu/25)^{1/3}
 = 0.56 N/mm² > V = 0.31 N/mm² OK

3) CHECK OF CRACKING

(10-10)

7-13

W60b (OUTSIDE/VERTICAL:BOT)

(AT+347.900)

a) CHECK OF MINIMUM REINFORCEMENT (BASED ON 2.6.2.3 OF BS8007)

SECTION b = 1000 mm SURFACE ZONE: d1 = 250 mm
 D = 800 mm
 dv = 540 mm
 dh = 520 mm

Min. ρ = 0.0035 (0.35 %)

As (ver.) = ρ · b · d1 = 875 mm²
 As (her.) = ρ · b · d1 = 875 mm²

USE ; VERTICAL D 20 @ 100 (As = 3140 mm²)
 USE ; HORIZONTAL D 20 @ 200 (As = 1570 mm²)

b) CHECK OF STEEL STRESS IN DIRECT OR FLEXURAL TENSION FOR SERVICEABILITY LIMIT STATES (BASED ON 3.2.2 OF BS8007)

Ver. Mu = 248.9 KN·m M = 177.8 KN·m b = 1000 mm
 Hor. Mu = 0.0 KN·m M = 0.0 KN·m D = 600 mm
 dv = 540 mm
 dh = 520 mm
 fy = 130 N/mm²
 fcu = 30 N/mm²

VERTICAL

K = M / (fcu · b · dv²) = 0.0203
 Z1 = dv · [0.5 + √(0.25 - K/0.9)] = 528
 Z2 = 0.95 · dv = 513
 Z = 513
 As = M / fy · Z = 2666.1 mm²

USE ; D 20 @ 100 (As = 3140 mm²)

HORIZONTAL

K = M / (fcu · b · dh²) = 0
 Z1 = dh · [0.5 + √(0.25 - K/0.9)] = 0
 Z2 = 0.95 · dh = 0
 Z = 0
 As = M / fy · Z = 0 mm²

USE ; D 20 @ 200 (As = 1570 mm²)

c) CHECK OF CRACK WIDTH (BASE ON APPENDIX A OF BS8007)

1) CRACK SPACING

Smax(V) = (fct/fb) · (ρ/2ρ(V)) = 533.4 mm
 Smax(H) = (fct/fb) · (ρ/2ρ(H)) = 1066.9 mm

Where; fct/fb = 0.67 ρ(V) = As/b · d1 = 0.0126
 ρ(H) = As/b · d1 = 0.0063

2) CRACK WIDTHS ARISING FROM RESTRAINED SHRINKAGE AND HEAT OF HYDRATION MOVEMENT

Wmax(V) = Smax(V) · (α/2) · T1 = 0.09 < 0.2 mm
 Wmax(H) = Smax(H) · (α/2) · T1 = 0.17 < 0.2 mm

Where ; α : COEFFICIENT OF THERMAL EXPANSION OF MATURE CONCRETE

α = 1E-05
 T1 = 32 from TABLE 4.2

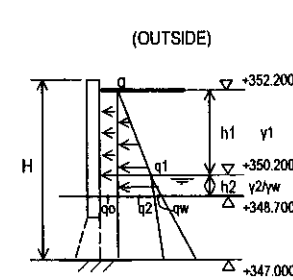
A.7-46

7-14

7.2.5 CALCULATION OF WALL (W60b) (AT+348.700)

(10-10)

1) DESIGN LOAD & FACTORED STRESS(Soil Pressure)



Ground Water Level at +350.200

H = 5.6 m
 h1 = 2.00 m
 h2 = 1.50 m
 h = h1 + h2 = 3.50 m

q : Live Load

Ko: Coefficient of soil pressure at rest

γ1 = 18.0 KN/m³ q = 5.0 (KN/m²/m)
 γ2 = 8.0 KN/m³ Ko = 0.5
 γw = 10.0 KN/m³

• Design load

q0 = Ko · q = 2.5 (KN/m²/m)
 q1 = Ko · γ1 · h1 = 18.0
 q2 = q1 + Ko · γ2 · h2 = 24.0
 qw = γw · h2 = 15.0
 Σq = q2 + qw = 39.0

• Factored Load

w1 = 1.4 · q0 = 3.5 (KN/m²/m)
 w2 = 1.4 · Σq = 54.6

• Factored stress at +348.700

Mu1 = 1/2 · w1 · h² = 21.44 (KN·m/m)
 Vu1 = w1 · h = 12.25 (KN/m)
 Mu2 = 1/6 · w2 · h² = 111.48 (KN·m/m)
 Vu2 = 1/2 · w2 · h = 95.55 (KN/m)
 Mu = Mu1 + Mu2 = 132.9 (KN·m/m)
 Vu = Vu1 + Vu2 = 107.8 (KN/m)

2) DESIGN OF SECTION (INSIDE/VERTICAL TOP)

Mu = 132.9 KN·m
 Vu = 107.8 KN/m

b = 1000 mm fy = 365 N/mm²
 D = 600 mm fcu = 30 N/mm²
 d = 540 mm

[REQUIRED RE-BAR]

K = Mu / (fcu · b · d²) = 0.015 < 0.156
 Z1 = d · [0.5 + √(0.25 - K/0.9)] = 531
 Z2 = 0.95 · d = 513
 Z = 513
 As1 = Mu / 0.95 · fy · Z = 747 mm²
 Asmin = 0.0013 · b · D = 780 mm²

USE ; D 20 @ 200 (As = 1570 mm²)

[CHECK OF SHEAR STRESS]

V = Vu / (b · d) = 0.20 < 4.4 N/mm² OK
 (100 · As) / (b · d) = 0.29 < 3.0
 400/d = 0.7 < 1.0 → 1.0

Vc = 0.79 · [(100 · As) / (b · d)]^{1/3} · [(400/d)]^{1/4} · (1/1.25) · (fcu/25)^{1/3}
 = 0.45 N/mm² > V = 0.20 N/mm² OK

FOOTING SLAB (FS70)

a) CHECK OF MINIMUM REINFORCEMENT (BASED ON 2.6.2.3 OF BS8007)

SECTION	b = 1000 mm	SURFACE ZONE :	d(T) = 250 mm
	D = 1000 mm		d(B) = 100 mm
	d = 940 mm		

$$\text{Min. } \rho = 0.0035 \text{ (0.35 \%)}$$

$$\begin{aligned} A_s(\text{top}) &= \rho \cdot b \cdot d(T) = 875 \text{ mm}^2 \\ A_s(\text{bot}) &= \rho \cdot b \cdot d(B) = 350 \text{ mm}^2 \end{aligned}$$

USE :	TOP	D 20	@ 200	(As = 1570 mm ²)
USE :	BOTTOM	D 20	@ 100	(As = 3140 mm ²)

b) CHECK OF STEEL STRESS IN DIRECT OR FLEXURAL TENSION FOR SERVICEABILITY LIMIT STATES (BASED ON 3.2.2 OF BS8007)

Top	Mu = 100.0 KN·m	M = 71.4 KN·m	b = 1000 mm
Bot.	Mu = 443.5 KN·m	M = 316.8 KN·m	D = 1000 mm
			d = 940 mm
			fy = 130 N/mm ²
			fcu = 30 N/mm ²

TOP

$$\begin{aligned} K &= M / (f_{cu} \cdot b \cdot d^2) &= 0.0027 \\ Z_1 &= d \cdot [0.5 + \sqrt{(0.25 - K/0.9)}] &= 937 \\ Z_2 &= 0.95 \cdot d &= 893 \\ Z &= &= 893 \\ A_s &= M / f_y \cdot Z &= 615.04 \text{ mm}^2 \end{aligned}$$

USE :	D 20	@ 200	(As = 1570 mm ²)
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BOTTOM

$$\begin{aligned} K &= M / (f_{cu} \cdot b \cdot d^2) &= 0.012 \\ Z_1 &= d \cdot [0.5 + \sqrt{(0.25 - K/0.9)}] &= 927 \\ Z_2 &= 0.95 \cdot d &= 893 \\ Z &= &= 893 \\ A_s &= M / f_y \cdot Z &= 2728.9 \text{ mm}^2 \end{aligned}$$

USE :	D 20	@ 100	(As = 3140 mm ²)
-------	------	-------	------------------------------

c) CHECK OF CRACK WIDTH (BASE ON APPENDIX A OF BS8007)

1) CRACK SPACING

$$\begin{aligned} S_{\text{max}}(T) &= (f_{ct}/f_b) \cdot (\phi/2\rho(T)) &= 1066.9 \text{ mm} \\ S_{\text{max}}(B) &= (f_{ct}/f_b) \cdot (\phi/2\rho(B)) &= 213.4 \text{ mm} \end{aligned}$$

$$\begin{aligned} \text{Where ; } f_{ct}/f_b &= 0.67 & \rho(T) &= A_s/b \cdot d(T) = 0.0063 \\ & & \rho(B) &= A_s/b \cdot d(B) = 0.0314 \end{aligned}$$

2) CRACK WIDTHS ARISING FROM RESTRAINED SHRINKAGE AND HEAT OF HYDRATION MOVEMENT

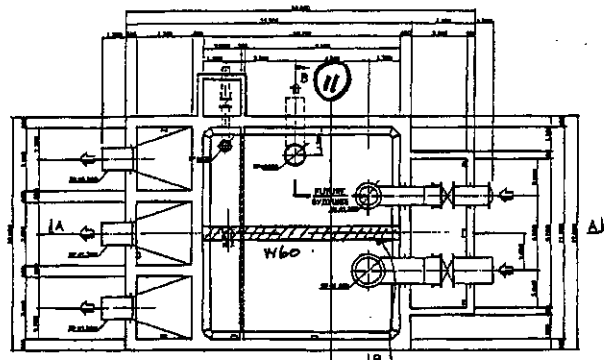
$$\begin{aligned} W_{\text{max}}(T) &= S_{\text{max}}(T) \cdot (\alpha/2) \cdot T_1 &= 0.13 < 0.2 \text{ mm} \\ W_{\text{max}}(B) &= S_{\text{max}}(B) \cdot (\alpha/2) \cdot T_1 &= 0.03 < 0.2 \text{ mm} \end{aligned}$$

Where ; α : COEFFICIENT OF THERMAL EXPANSION OF MATURE CONCRETE

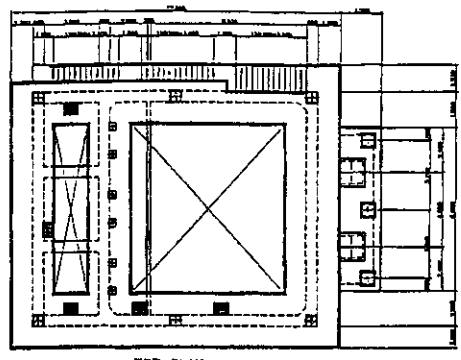
$$\begin{aligned} \alpha &= 1E-05 \\ T_1 &= 25 \text{ from TABLE 4.2} \end{aligned}$$

8. DISTRIBUTION CHAMBER

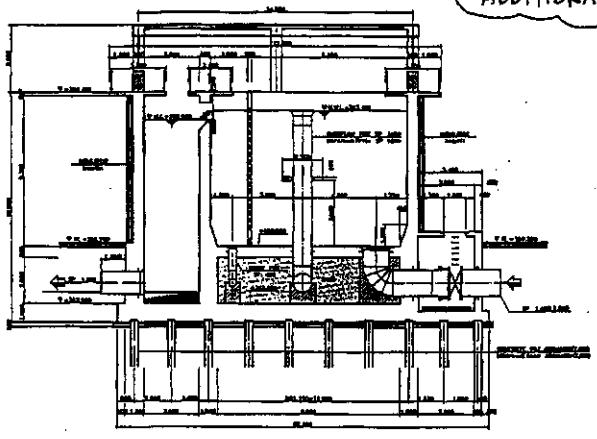
8. РАСПРЕДЕЛИТЕЛЬНАЯ КАМЕРА



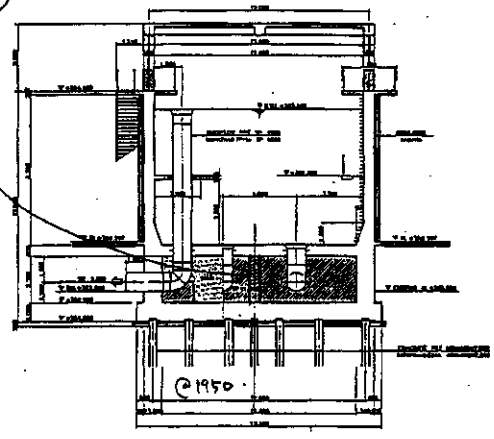
BOTTOM PLAN



TOP PLAN



SECTION A-A



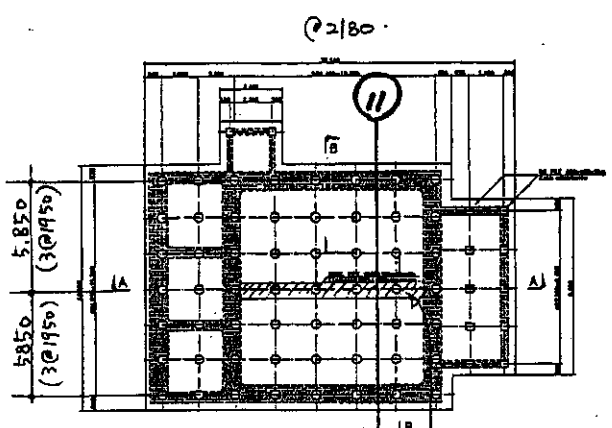
SECTION B-B

ADDITIONAL WALL

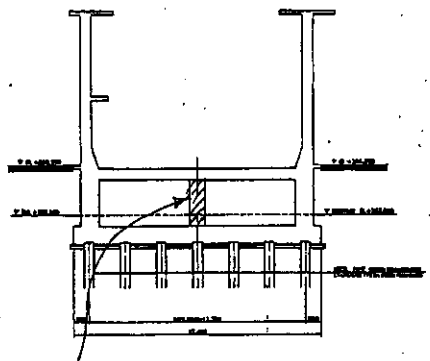
II - II

8-1 Plan and Section
8-1 План и разрез

8-2



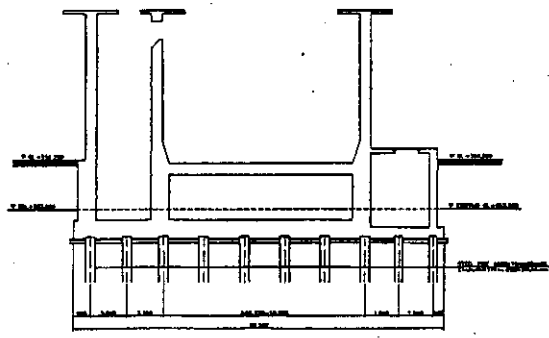
PILE ARRANGEMENT
РАСПОСЛАНКА СВАЙ



SECTION B-B
РАЗРЕЗ B-B

ADDITIONAL WALL

II - II

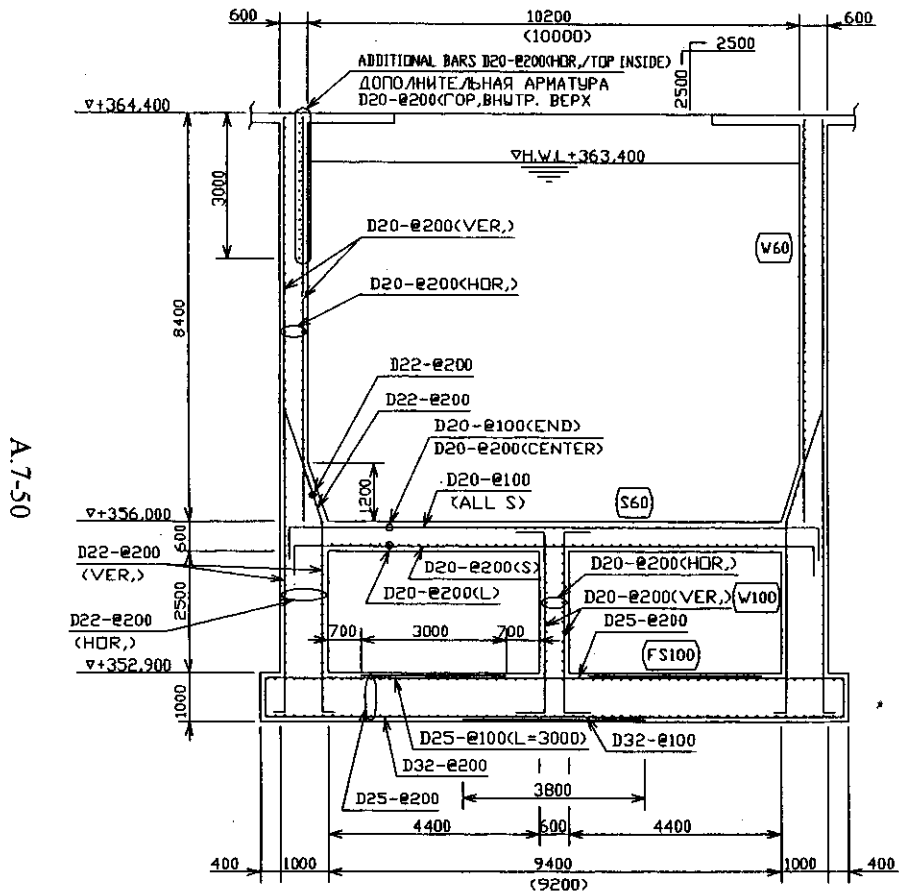


SECTION A-A
РАЗРЕЗ A-A

A.7-49

8-3

8.2 DESIGN OF WALL & FOOTING
8.2 ПРОЕКТИРОВАНИЕ СТЕНЫ И ОСНОВАНИЯ



11 - 11
S=1/100

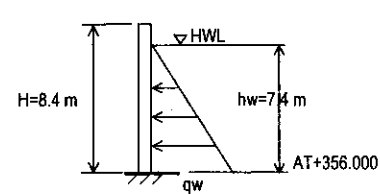
LEGEND

S : SHORT DIRECTION
L : LONG DIRECTION
S : КОРОТКОЕ НАПРАВЛЕНИЕ
L : ДЛИННОЕ НАПРАВЛЕНИЕ

8.2.1 CALCULATION OF WALL (W100~60)
(AT+356.000)

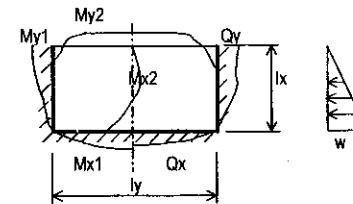
11-11

1) DESIGN LOAD (Water Pressure)



hw = 7.40 m
yw = 10.0 KN/m3
qw = yw · hw = 74.0 KN/m2/m

2) FACTORED STRESS (W60)



lx = 7.40 m
ly = 10.20 m
λ = 1.38
w = qw = 74.0 KN/m2/m
Factored Load
w' = 1.4 · w = 103.60 KN/m2/m
w' · lx^2 = 5673.1
w' · ly = 766.6

SEE FIG.1

Mx1 = 0.052 x 5673.1 = 295.0 (KN · m/m)
Mx2 = 0.012 x 5673.1 = 68.1
My1 = 0.033 x 5673.1 = 187.2
My2 = 0.018 x 5673.1 = 102.1
Qx = 0.39 x 766.6 = 299.0 (KN/m)
Qy = 0.12 x 766.6 = 92.0

2) DESIGN OF SECTION

a) VERTICAL : BOT (AT+355.800)

$$\begin{aligned} \mu_u &= 295.0 \text{ KN}\cdot\text{m} \\ \nu_u &= 299.0 \text{ KN/m} \end{aligned}$$

$$\begin{aligned} b &= 1000 \text{ mm} & f_y &= 365 \text{ N/mm}^2 \\ D &= 1000 \text{ mm} & f_{cu} &= 30 \text{ N/mm}^2 \\ d &= 940 \text{ mm} \end{aligned}$$

[REQUIRED RE-BAR]

$$\begin{aligned} K &= \mu_u / (f_{cu} \cdot b \cdot d^2) & &= 0.011 < 0.156 \\ Z_1 &= d \cdot [0.5 + \sqrt{(0.25 - K/0.9)}] & &= 928 \\ Z_2 &= 0.95 \cdot d & &= 893 \\ Z &= & &= 893 \\ A_{s1} &= \mu_u / 0.95 \cdot f_y \cdot Z & &= 953 \text{ mm}^2 \\ A_{smin} &= 0.0013 \cdot b \cdot D & &= 1300 \text{ mm}^2 \end{aligned}$$

$$\text{USE ; } \underline{D 22 @ 200} \quad (A_s = 1900 \text{ mm}^2)$$

[CHECK OF SHEAR STRESS]

$$\begin{aligned} V &= \nu_u / (b \cdot d) & &= 0.32 < 4.4 \text{ N/mm}^2 \text{ OK} \\ (100 \cdot A_s) / (b \cdot d) & & &= 0.20 < 3.0 \\ 400/d & & &= 0.4 < 1.0 \rightarrow 1.0 \end{aligned}$$

$$\begin{aligned} V_c &= 0.79 \cdot \{ (100 \cdot A_s) / (b \cdot d) \}^{1/3} \cdot (400/d)^{1/4} \cdot (1/1.25) \cdot (f_{cu}/25)^{1/3} \\ &= 0.39 \text{ N/mm}^2 > V = 0.32 \text{ N/mm}^2 \text{ OK} \end{aligned}$$

b) HORIZONTAL : TOP/END

$$\begin{aligned} \mu_u &= 187.2 \text{ KN}\cdot\text{m} \\ \nu_u &= 92.0 \text{ KN/m} \end{aligned}$$

$$\begin{aligned} b &= 1000 \text{ mm} & f_y &= 365 \text{ N/mm}^2 \\ D &= 600 \text{ mm} & f_{cu} &= 30 \text{ N/mm}^2 \\ d &= 520 \text{ mm} \end{aligned}$$

[REQUIRED RE-BAR]

$$\begin{aligned} K &= \mu_u / (f_{cu} \cdot b \cdot d^2) & &= 0.023 < 0.156 \\ Z_1 &= d \cdot [0.5 + \sqrt{(0.25 - K/0.9)}] & &= 506 \\ Z_2 &= 0.95 \cdot d & &= 494 \\ Z &= & &= 494 \\ A_{s1} &= \mu_u / 0.95 \cdot f_y \cdot Z & &= 1093 \text{ mm}^2 \\ A_{smin} &= 0.0013 \cdot b \cdot D & &= 780 \text{ mm}^2 \end{aligned}$$

$$\text{USE ; } \underline{D 20 @ 100} \quad (A_s = 3140 \text{ mm}^2)$$

(Hor./End)
(Hor./Cent.)

[CHECK OF SHEAR STRESS]

$$\begin{aligned} V &= \nu_u / (b \cdot d) & &= 0.18 < 4.4 \text{ N/mm}^2 \text{ OK} \\ (100 \cdot A_s) / (b \cdot d) & & &= 0.60 < 3.0 \\ 400/d & & &= 0.8 < 1.0 \rightarrow 1.0 \end{aligned}$$

$$\begin{aligned} V_c &= 0.79 \cdot \{ (100 \cdot A_s) / (b \cdot d) \}^{1/3} \cdot (400/d)^{1/4} \cdot (1/1.25) \cdot (f_{cu}/25)^{1/3} \\ &= 0.57 \text{ N/mm}^2 > V = 0.18 \text{ N/mm}^2 \text{ OK} \end{aligned}$$

4) CHECK OF CRACKING
W100~60 (VERTICAL:BOT.)

(1)-(1)

a) CHECK OF MINIMUM REINFORCEMENT (BASED ON 2.6.2.3 OF BS8007)

$$\begin{aligned} \text{SECTION } b &= 1000 \text{ mm} & \text{SURFACE ZONE : } d_1 &= 250 \text{ mm} \\ D &= 1000 \text{ mm} \\ d_v &= 940 \text{ mm} \\ d_h &= 920 \text{ mm} \end{aligned}$$

$$\text{Min. } \rho = 0.0035 \text{ (} 0.35 \% \text{)}$$

$$\begin{aligned} A_s(\text{ver.}) &= \rho \cdot b \cdot d_1 = 875 \text{ mm}^2 \\ A_s(\text{her.}) &= \rho \cdot b \cdot d_1 = 875 \text{ mm}^2 \end{aligned}$$

$$\begin{aligned} \text{USE ; } \underline{\text{VERTICAL } D 22 @ 200} & \quad (A_s = 1900 \text{ mm}^2) \\ \text{USE ; } \underline{\text{HORIZONTAL } D 22 @ 200} & \quad (A_s = 1900 \text{ mm}^2) \end{aligned}$$

b) CHECK OF STEEL STRESS IN DIRECT OR FLEXURAL TENSION FOR SERVICEABILITY LIMIT STATES
(BASED ON 3.2.2 OF BS8007)

$$\begin{aligned} \text{Ver. } \mu_u &= 295.0 \text{ KN}\cdot\text{m} & M &= 210.7 \text{ KN}\cdot\text{m} & b &= 1000 \text{ mm} \\ \text{Hor. } \mu_u &= & M &= & D &= 1000 \text{ mm} \\ & & & & d_v &= 940 \text{ mm} \\ & & & & d_h &= 920 \text{ mm} \\ & & & & f_y &= 130 \text{ N/mm}^2 \\ & & & & f_{cu} &= 30 \text{ N/mm}^2 \end{aligned}$$

VERTICAL

$$\begin{aligned} K &= M / (f_{cu} \cdot b \cdot d_v^2) & &= 0.0079 \\ Z_1 &= d_v \cdot [0.5 + \sqrt{(0.25 - K/0.9)}] & &= 932 \\ Z_2 &= 0.95 \cdot d_v & &= 893 \\ Z &= & &= 893 \\ A_s &= M / f_y \cdot Z & &= 1815 \text{ mm}^2 \end{aligned}$$

$$\text{USE ; } \underline{D 22 @ 200} \quad (A_s = 1900 \text{ mm}^2)$$

HORIZONTAL

$$\begin{aligned} K &= M / (f_{cu} \cdot b \cdot d_h^2) & &= 0 \\ Z_1 &= d_h \cdot [0.5 + \sqrt{(0.25 - K/0.9)}] & &= 0 \\ Z_2 &= 0.95 \cdot d_h & &= 0 \\ Z &= & &= 0 \\ A_s &= M / f_y \cdot Z & &= 0 \text{ mm}^2 \end{aligned}$$

$$\text{USE ; } \underline{D 22 @ 200} \quad (A_s = 1900 \text{ mm}^2)$$

c) CHECK OF CRACK WIDTH (BASE ON APPENDIX A OF BS8007)

1) CRACK SPACING

$$\begin{aligned} S_{\max}(V) &= (f_{ct}/f_b) \cdot (\phi/2\rho(V)) & &= 969.74 \text{ mm} \\ S_{\max}(H) &= (f_{ct}/f_b) \cdot (\phi/2\rho(H)) & &= 969.7 \text{ mm} \end{aligned}$$

$$\begin{aligned} \text{Where ; } f_{ct}/f_b &= 0.67 & \rho(V) &= A_s/b \cdot d_1 = 0.0076 \\ & & \rho(H) &= A_s/b \cdot d_1 = 0.0076 \end{aligned}$$

2) CRACK WIDTHS ARISING FROM RESTRAINED SHRINKAGE AND HEAT OF HYDRATION MOVEMENT

$$\begin{aligned} W_{\max}(V) &= S_{\max}(V) \cdot (\alpha/2) \cdot T_1 & &= 0.19 < 0.2 \text{ mm} \\ W_{\max}(H) &= S_{\max}(H) \cdot (\alpha/2) \cdot T_1 & &= 0.19 < 0.2 \text{ mm} \end{aligned}$$

Where ; α : COEFFICIENT OF THERMAL EXPANSION OF MATURE CONCRETE

$$\begin{aligned} \alpha &= 1E-05 \\ T_1 &= 40 \text{ from TABLE 4.2} \end{aligned}$$

a) CHECK OF MINIMUM REINFORCEMENT (BASED ON 2.6.2.3 OF BS8007)

SECTION b = 1000 mm SURFACE ZONE: d1 = 250 mm
 D = 600 mm
 dv = 540 mm
 dh = 520 mm

Min. ρ = 0.0035 (0.35 %)

As (ver.) = ρ · b · d1 = 875 mm²
 As (her.) = ρ · b · d1 = 875 mm²

USE;	VERTICAL	D 20	@ 200	(As = 1570 mm ²)
USE;	HORIZONTAL	D 20	@ 100	(As = 3140 mm ²)

b) CHECK OF STEEL STRESS IN DIRECT OR FLEXURAL TENSION FOR SERVICEABILITY LIMIT STATES (BASED ON 3.2.2 OF BS8007)

Ver. Mu = 68.1 KN·m M = 48.6 KN·m b = 1000 mm
 Hor. Mu = 187.2 KN·m M = 133.7 KN·m D = 600 mm
 dv = 540 mm
 dh = 520 mm
 fy = 130 N/mm²
 fcu = 30 N/mm²

VERTICAL

$K = M / (f_{cu} \cdot b \cdot d_v^2) = 0.0056$
 $Z1 = d_v \cdot [0.5 + \sqrt{ (0.25 - K/0.9) }] = 537$
 $Z2 = 0.95 \cdot d_v = 513$
 $Z = 513$
 $A_s = M / f_y \cdot Z = 728.74 \text{ mm}^2$

USE;	D 20	@ 200	(As = 1570 mm ²)
------	------	-------	------------------------------

HORIZONTAL

$K = M / (f_{cu} \cdot b \cdot d_h^2) = 0.0165$
 $Z1 = d_h \cdot [0.5 + \sqrt{ (0.25 - K/0.9) }] = 510$
 $Z2 = 0.95 \cdot d_h = 494$
 $Z = 494$
 $A_s = M / f_y \cdot Z = 2081.9 \text{ mm}^2$

USE;	D 20	@ 100	(As = 3140 mm ²)
------	------	-------	------------------------------

c) CHECK OF CRACK WIDTH (BASE ON APPENDIX A OF BS8007)

1) CRACK SPACING

Smax(V) = (fct/fb) · (φ/2ρ(V)) = 1063.5 mm
 Smax(H) = (fct/fb) · (φ/2ρ(H)) = 531.7 mm

Where ; fct/fb = 0.67 ρ(V) = As/b · d1 = 0.0063
 ρ(H) = As/b · d1 = 0.0126

2) CRACK WIDTHS ARISING FORM RESTRAINED SHRINKAGE AND HEAT OF HYDRATION MOVEMENT

Wmax(V) = Smax(V) · (α/2) · T1 = 0.19 < 0.2 mm
 Wmax(H) = Smax(H) · (α/2) · T1 = 0.09 < 0.2 mm

Where ; α : COEFFICIENT OF THERMAL EXPANSION OF MATURE CONCRETE

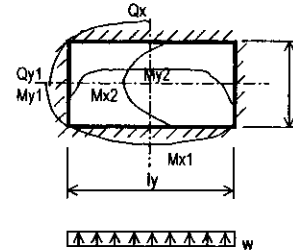
α = 1E-05
 T1 = 35 from TABLE 4.2

A.7-52

8.2.2 CALCULATION OF SLAB (S60)
 (AT+356.000)

①-①

1) DESIGN LOAD & FACTORED STRESS



hw = 7.40 m
 t = 0.60 m
 yw = 10.0 KN/m³
 yc = 24.0 KN/m⁴
 $w = yw \cdot hw + yc \cdot t = 88.4 \text{ KN/m}^2/\text{m}$

lx = 4.40 m
 ly = 9.20 m
 λ = 2.09

w = 88.4 KN/m²/m
 $w' = 1.4 \cdot w = 123.76 \text{ KN/m}^2/\text{m}$

$w' \cdot l_x^2 = 2396.0$
 $w' \cdot l_x = 544.5$

SEE FIG 2

Mx1	=	0.083	x	2396.0	=	198.9 (KN · m/m)
Mx2	=	0.040	x	2396.0	=	95.8
My1	=	0.057	x	2396.0	=	136.6
My2	=	0.028	x	2396.0	=	67.1
Qx	=	0.52	x	544.5	=	283.2 (KN/m)
Qy	=	0.46	x	544.5	=	250.5

3) DESIGN OF SECTION (AT+356.000)

a) X-DIRECTION

$$\begin{aligned} \mu_u &= 198.9 \text{ KN}\cdot\text{m} & (95.8) \\ \nu_u &= 283.2 \text{ KN/m} \end{aligned}$$

$$\begin{aligned} b &= 1000 \text{ mm} & f_y &= 365 \text{ N/mm}^2 \\ D &= 600 \text{ mm} & f_{cu} &= 30 \text{ N/mm}^2 \\ d &= 540 \text{ mm} \end{aligned}$$

[REQUIRED RE-BAR]

$$\begin{aligned} K &= \mu_u / (f_{cu} \cdot b \cdot d^2) & &= 0.023 < 0.156 \\ Z_1 &= d \cdot [0.5 + \sqrt{0.25 - K/0.9}] & &= 526 \\ Z_2 &= 0.95 \cdot d & &= 513 \\ Z &= & &= 513 \\ A_{s1} &= \mu_u / 0.95 \cdot f_y \cdot Z & &= 1118 \text{ mm}^2 \\ A_{smin} &= 0.0013 \cdot b \cdot D & &= 780 \text{ mm}^2 \end{aligned}$$

$$\begin{array}{l} \text{USE; } \underline{\underline{D 20 @ 100}} \quad (A_s = 3140 \text{ mm}^2) \quad (\text{Top}) \\ \underline{\underline{D 20 @ 200}} \quad (\text{Bottom}) \end{array}$$

[CHECK OF SHEAR STRESS]

$$\begin{aligned} V &= \nu_u / (b \cdot d) & &= 0.52 < 4.4 \text{ N/mm}^2 \text{ OK} \\ (100 \cdot A_s) / (b \cdot d) & & &= 0.58 < 3.0 \\ 400/d & & &= 0.7 < 1.0 \rightarrow 1.0 \end{aligned}$$

$$\begin{aligned} V_c &= 0.79 \cdot \{ (100 \cdot A_s) / (b \cdot d) \}^{1/3} \cdot (400/d)^{1/4} \cdot (1/1.25) \cdot (f_{cu}/25)^{1/3} \\ &= 0.56 \text{ N/mm}^2 > V = 0.52 \text{ N/mm}^2 \text{ OK} \end{aligned}$$

b) Y-DIRECTION

$$\begin{aligned} \mu_u &= 136.6 \text{ KN}\cdot\text{m} & (67.1) \\ \nu_u &= 250.5 \text{ KN/m} \end{aligned}$$

$$\begin{aligned} b &= 1000 \text{ mm} & f_y &= 365 \text{ N/mm}^2 \\ D &= 600 \text{ mm} & f_{cu} &= 30 \text{ N/mm}^2 \\ d &= 520 \text{ mm} \end{aligned}$$

[REQUIRED RE-BAR]

$$\begin{aligned} K &= \mu_u / (f_{cu} \cdot b \cdot d^2) & &= 0.017 < 0.156 \\ Z_1 &= d \cdot [0.5 + \sqrt{0.25 - K/0.9}] & &= 510 \\ Z_2 &= 0.95 \cdot d & &= 494 \\ Z &= & &= 494 \\ A_{s1} &= \mu_u / 0.95 \cdot f_y \cdot Z & &= 797 \text{ mm}^2 \\ A_{smin} &= 0.0013 \cdot b \cdot D & &= 780 \text{ mm}^2 \end{aligned}$$

$$\begin{array}{l} \text{USE; } \underline{\underline{D 20 @ 100}} \quad (A_s = 3140 \text{ mm}^2) \quad (\text{Top/End}) \\ \underline{\underline{D 20 @ 200}} \quad (\text{Bot./All}) \end{array}$$

[CHECK OF SHEAR STRESS]

$$\begin{aligned} V &= \nu_u / (b \cdot d) & &= 0.48 < 4.4 \text{ N/mm}^2 \text{ OK} \\ (100 \cdot A_s) / (b \cdot d) & & &= 0.60 < 3.0 \\ 400/d & & &= 0.8 < 1.0 \rightarrow 1.0 \end{aligned}$$

$$\begin{aligned} V_c &= 0.79 \cdot \{ (100 \cdot A_s) / (b \cdot d) \}^{1/3} \cdot (400/d)^{1/4} \cdot (1/1.25) \cdot (f_{cu}/25)^{1/3} \\ &= 0.57 \text{ N/mm}^2 > V = 0.48 \text{ N/mm}^2 \text{ OK} \end{aligned}$$

3) CHECK OF CRACKING
SLAB (S60)

⑩-⑩

a) CHECK OF MINIMUM REINFORCEMENT (BASED ON 2.6.2.3 OF BS8007)

$$\begin{array}{llll} \text{SECTION} & b = 1000 \text{ mm} & \text{SURFACE ZONE:} & d(T) = 250 \text{ mm} \\ & D = 600 \text{ mm} & & d(B) = 100 \text{ mm} \\ & d = 540 \text{ mm} & & \end{array}$$

$$\text{Min. } \rho = 0.0035 \text{ (0.35 \%)}$$

$$\begin{aligned} A_s(\text{top}) &= \rho \cdot b \cdot d(T) = 875 \text{ mm}^2 \\ A_s(\text{bot}) &= \rho \cdot b \cdot d(B) = 350 \text{ mm}^2 \end{aligned}$$

$$\begin{array}{l} \text{USE; } \underline{\underline{TOP}} \quad \underline{\underline{D 20 @ 100}} \quad (A_s = 3140 \text{ mm}^2) \\ \underline{\underline{BOTTOM}} \quad \underline{\underline{D 20 @ 200}} \quad (A_s = 1570 \text{ mm}^2) \end{array}$$

b) CHECK OF STEEL STRESS IN DIRECT OR FLEXURAL TENSION FOR SERVICEABILITY LIMIT STATES
(BASED ON 3.2.2 OF BS8007)

$$\begin{array}{llll} \text{Top} & \mu_u = 198.9 \text{ KN}\cdot\text{m} & M = 142.1 \text{ KN}\cdot\text{m} & b = 1000 \text{ mm} \\ \text{Bott.} & \mu_u = 95.8 \text{ KN}\cdot\text{m} & M = 68.4 \text{ KN}\cdot\text{m} & D = 600 \text{ mm} \\ & & & d = 540 \text{ mm} \\ & & & f_y = 130 \text{ N/mm}^2 \\ & & & f_{cu} = 30 \text{ N/mm}^2 \end{array}$$

$$\begin{array}{ll} \text{TOP} & \\ K = M / (f_{cu} \cdot b \cdot d^2) & = 0.0162 \\ Z_1 = d \cdot [0.5 + \sqrt{0.25 - K/0.9}] & = 530 \\ Z_2 = 0.95 \cdot d & = 513 \\ Z & = 513 \\ A_s = M / f_y \cdot Z & = 2130.8 \text{ mm}^2 \end{array}$$

$$\text{USE; } \underline{\underline{D 20 @ 100}} \quad (A_s = 3140 \text{ mm}^2)$$

BOTTOM

$$\begin{array}{ll} K = M / (f_{cu} \cdot b \cdot d^2) & = 0.0078 \\ Z_1 = d \cdot [0.5 + \sqrt{0.25 - K/0.9}] & = 535 \\ Z_2 = 0.95 \cdot d & = 513 \\ Z & = 513 \\ A_s = M / f_y \cdot Z & = 1025.6 \text{ mm}^2 \end{array}$$

$$\text{USE; } \underline{\underline{D 20 @ 200}} \quad (A_s = 1570 \text{ mm}^2)$$

c) CHECK OF CRACK WIDTH (BASE ON APPENDIX A OF BS8007)

1) CRACK SPACING

$$\begin{aligned} s_{max}(T) &= (f_{ct}/f_b) \cdot (\phi/2\rho(T)) & &= 531.75 \text{ mm} \\ s_{max}(B) &= (f_{ct}/f_b) \cdot (\phi/2\rho(B)) & &= 426.8 \text{ mm} \end{aligned}$$

$$\begin{array}{ll} \text{Where; } f_{ct}/f_b = 0.67 & \rho(T) = A_s/b \cdot d(T) = 0.0126 \\ & \rho(B) = A_s/b \cdot d(B) = 0.0157 \end{array}$$

2) CRACK WIDTHS ARISING FROM RESTRAINED SHRINKAGE AND HEAT OF HYDRATION MOVEMENT

$$\begin{aligned} w_{max}(T) &= s_{max}(T) \cdot (\alpha/2) \cdot T_1 & &= 0.07 < 0.2 \text{ mm} \\ w_{max}(B) &= s_{max}(B) \cdot (\alpha/2) \cdot T_1 & &= 0.05 < 0.2 \text{ mm} \end{aligned}$$

Where; α : COEFFICIENT OF THERMAL EXPANSION OF MATURE CONCRETE

$$\begin{array}{l} \alpha = 1\text{E-}05 \\ T_1 = 25 \text{ from TABLE 4.2} \end{array}$$

8.2.3 CALCULATION OF FOOTING SLAB (FS100)
(AT+354.600)

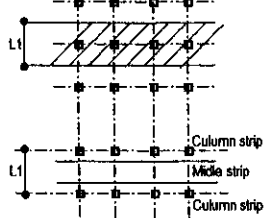
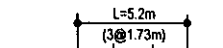
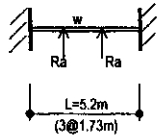
⑪-⑪

1) DESIGN LOAD & FACTORED STRESS

- Design load
Sand : $D_s = 2.5 \text{ m}$
 $w_1 = 17.0 \cdot D_s \cdot L_1 = 72.3 \text{ KN/m}$
- F.Slab : $D_f = 1.0 \text{ m}$
 $w = 24.0 \cdot D \cdot L_1 = 40.8 \text{ KN/m}$
- Factored Load $w' = 1.4 \cdot (w_1 + w_2) = 158.3 \text{ KN/m}$
- Pile Reaction $R_a = 750 \text{ KN/pile}$
 $R_a' = 1.4 \cdot R_a = 1050 \text{ KN/pile}$

- Factored stress
 $L = 5.2 \text{ m}$
 $C = (2/9 \cdot R_a' \cdot L) - (1/12 \cdot w' \cdot L^2) = 856.7 \text{ KN} \cdot \text{m}$
 $M_o = (1/3 \cdot R_a' \cdot L) - (1/8 \cdot w' \cdot L^2) = 1285.0 \text{ KN} \cdot \text{m}$
 $M_A = 1.3 \cdot C = 1113.7 \text{ KN} \cdot \text{m}$
 $M_o - 0.65C = 728.2 \text{ KN} \cdot \text{m}$
 $V_u = R_a' - 1/2 \cdot w' \cdot L = 638.5 \text{ KN}$

- Column strip
 $L_1 = 1.70 \text{ m}$
BOT: $M_{u1} = 0.375 \cdot (1.3 \cdot C) / (1/4 \cdot L_1) = 982.7 \text{ KN} \cdot \text{m}$
TOP: $M_{u2} = 0.375 \cdot (M_o - 0.65 \cdot C) / (1/4 \cdot L_1) = 642.5 \text{ KN} \cdot \text{m}$



A.7-54

2) DESIGN OF SECTION

a) HORIZONTAL : BOT/END

$M_u = 982.7 \text{ KN} \cdot \text{m}$
 $V_u = 638.5 \text{ KN/m}$

$b = 1000 \text{ mm}$ $f_y = 365 \text{ N/mm}^2$
 $D = 1000 \text{ mm}$ $f_{cu} = 30 \text{ N/mm}^2$
 $d = 940 \text{ mm}$

[REQUIRED RE-BAR]

$K = M_u / (f_{cu} \cdot b \cdot d^2) = 0.037 < 0.156$
 $Z_1 = d \cdot [0.5 + \sqrt{0.25 - K/0.9}] = 900$
 $Z_2 = 0.95 \cdot d = 893$
 $Z = 893$
 $A_{s1} = M_u / (0.95 \cdot f_y \cdot Z) = 3174 \text{ mm}^2$
 $A_{smin} = 0.0013 \cdot b \cdot D = 1300 \text{ mm}^2$

USE : D 32 @ 100 (As = 8040 mm²)

[CHECK OF SHEAR STRESS]

$V = V_u / (b \cdot d) = 0.68 < 4.4 \text{ N/mm}^2 \text{ OK}$
 $(100 \cdot A_s) / (b \cdot d) = 0.86 < 3.0$
 $400/d = 0.4 < 1.0 \rightarrow 1.0$

$V_c = 0.79 \cdot \{ (100 \cdot A_s) / (b \cdot d) \}^{1/3} \cdot (400/d)^{1/4} \cdot (1/1.25) \cdot (f_{cu}/25)^{1/3}$
 $= 0.64 \text{ N/mm}^2 < V = 0.68 \text{ N/mm}^2 \text{ NG}$

b) HORIZONTAL : TOP/CENT

$M_u = 642.5 \text{ KN} \cdot \text{m}$
 $V_u = \text{KN/m}$

$b = 1000 \text{ mm}$ $f_y = 365 \text{ N/mm}^2$
 $D = 1000 \text{ mm}$ $f_{cu} = 30 \text{ N/mm}^2$
 $d = 920 \text{ mm}$

[REQUIRED RE-BAR]

$K = M_u / (f_{cu} \cdot b \cdot d^2) = 0.025 < 0.156$
 $Z_1 = d \cdot [0.5 + \sqrt{0.25 - K/0.9}] = 893$
 $Z_2 = 0.95 \cdot d = 874$
 $Z = 874$
 $A_{s1} = M_u / (0.95 \cdot f_y \cdot Z) = 2120 \text{ mm}^2$
 $A_{smin} = 0.0013 \cdot b \cdot D = 1300 \text{ mm}^2$

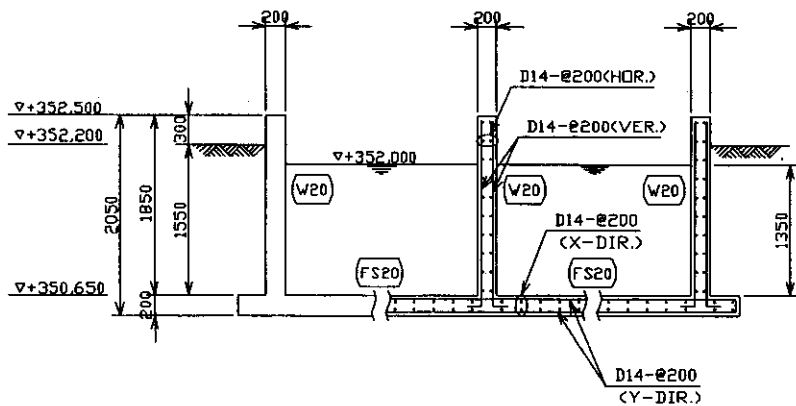
USE : D 25 @ 100 (As = 4910 mm²)

[CHECK OF SHEAR STRESS]

$V = V_u / (b \cdot d) = 0.00 < 4.4 \text{ N/mm}^2 \text{ OK}$
 $(100 \cdot A_s) / (b \cdot d) = 0.53 < 3.0$
 $400/d = 0.4 < 1.0 \rightarrow 1.0$

$V_c = 0.79 \cdot \{ (100 \cdot A_s) / (b \cdot d) \}^{1/3} \cdot (400/d)^{1/4} \cdot (1/1.25) \cdot (f_{cu}/25)^{1/3}$
 $= 0.54 \text{ N/mm}^2 > V = 0.00 \text{ N/mm}^2 \text{ OK}$

9.1 DESIGN OF WALL & FOOTING
9.1 ПРОЕКТИРОВАНИЕ СТЕНЫ И ОСНОВАНИЯ

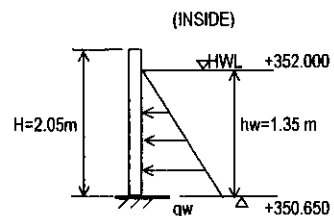


A.7-56

⑫ - ⑫
S=1/50

9.2.1 CALCULATION OF WALL (W20) (AT+350.650) ⑫-⑫

1) DESIGN LOAD & FACTORED STRESS (Water Pressure)



H= 4.6 m
hw= 1.35 m
yw= 10.0 KN/m³

• Design load
 $q_w = y_w \cdot h_w = 10.0 \times 1.35 = 13.5 \text{ (KN/m}^2\text{)}$

• Factored Load
 $w' = 1.4 \cdot q_w = 1.4 \times 13.5 = 18.90 \text{ (KN/m}^2\text{)}$

• Factored stress at +350.650

$$\begin{cases} M_u = 1/6 \cdot w' \cdot h_w^2 = 5.7 \text{ (KN}\cdot\text{m/m)} \\ V_u = 1/2 \cdot w' \cdot h_w = 12.8 \text{ (KN/m)} \end{cases}$$

2) DESIGN OF SECTION (INSIDE/VERTICAL BOT.)

$M_u = 5.7 \text{ KN}\cdot\text{m}$
 $V_u = 12.8 \text{ KN/m}$

$b = 1000 \text{ mm}$ $f_y = 365 \text{ N/mm}^2$
 $D = 200 \text{ mm}$ $f_{cu} = 30 \text{ N/mm}^2$
 $d = 140 \text{ mm}$

[REQUIRED RE-BAR]

$$\begin{aligned} K &= M_u / (f_{cu} \cdot b \cdot d^2) &&= 0.010 < 0.156 \\ Z_1 &= d \cdot (0.5 + \sqrt{0.25 - K/0.9}) &&= 138 \\ Z_2 &= 0.95 \cdot d &&= 133 \\ Z & &&= 133 \\ A_{s1} &= M_u / (0.95 \cdot f_y \cdot Z) &&= 124 \text{ mm}^2 \\ A_{smin} &= 0.0013 \cdot b \cdot D &&= 260 \text{ mm}^2 \end{aligned}$$

USE ; D 14 @ 200 (As = 770 mm²)

[CHECK OF SHEAR STRESS]

$$\begin{aligned} V &= V_u / (b \cdot d) &&= 0.09 < 4.4 \text{ N/mm}^2 \text{ OK} \\ (100 \cdot A_s) / (b \cdot d) &&&= 0.55 < 3.0 \\ 400/d &&&= 2.9 > 1.0 \rightarrow 2.9 \end{aligned}$$

$$\begin{aligned} V_c &= 0.79 \cdot \{ (100 \cdot A_s) / (b \cdot d) \}^{1/3} \cdot (400/d)^{1/4} \cdot (1/1.25) \cdot (f_{cu}/25)^{1/3} \\ &= 0.72 \text{ N/mm}^2 > V = 0.09 \text{ N/mm}^2 \text{ OK} \end{aligned}$$

3) CHECK OF CRACKING

(12-12)

9-4

W20 (INSIDE VERTICAL BOT)

a) CHECK OF MINIMUM REINFORCEMENT (BASED ON 2.6.2.3 OF BS8007)

SECTION	b = 1000 mm	SURFACE ZONE :	d1 = 100 mm
	D = 200 mm		
	dv = 140 mm		
	dh = 120 mm		

Min. ρ = 0.0035 (0.35 %)

As (ver.) = ρ · b · d1 = 350 mm²
 As (her.) = ρ · b · d1 = 350 mm²

USE ;	VERTICAL	D 14	@ 200	(As = 770 mm ²)
USE ;	HORIZONTAL	D 14	@ 200	(As = 770 mm ²)

b) CHECK OF STEEL STRESS IN DIRECT OR FLEXURAL TENSION FOR SERVICEABILITY LIMIT STATES (BASED ON 3.2.2 OF BS8007)

Ver.	Mu = 5.7 KN·m	M = 4.1 KN·m	b = 1000 mm
Hor.	Mu = 0.0 KN·m	M = 0.0 KN·m	D = 200 mm
			dv = 140 mm
			dh = 120 mm
			fy = 130 N/mm ²
			fcu = 30 N/mm ²

VERTICAL

K = M / (fcu · b · dv²) = 0.007
 Z1 = dv · [0.5 + √(0.25 - K/0.9)] = 139
 Z2 = 0.95 · dv = 133
 Z = 133
 As = M / fy · Z = 237.13 mm²

USE ;	D 14	@ 200	(As = 770 mm ²)
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HORIZONTAL

K = M / (fcu · b · dh²) = 0
 Z1 = dh · [0.5 + √(0.25 - K/0.9)] = 0
 Z2 = 0.95 · dh = 0
 Z = 0
 As = M / fy · Z = 0 mm²

USE ;	D 14	@ 200	(As = 770 mm ²)
-------	------	-------	-----------------------------

c) CHECK OF CRACK WIDTH (BASE ON APPENDIX A OF BS8007)

1) CRACK SPACING

Smax(V) = (fct/fb) · (σ/2ρ(V)) = 609.09 mm
 Smax(H) = (fct/fb) · (σ/2ρ(H)) = 609.1 mm

Where ; fct/fb = 0.67
 ρ(V) = As/b · d1 = 0.0077
 ρ(H) = As/b · d1 = 0.0077

2) CRACK WIDTHS ARISING FORM RESTRAINED SHRINKAGE AND HEAT OF HYDRATION MOVEMENT

Wmax(V) = Smax(V) · (α/2) · T1 = 0.07 < 0.2 mm
 Wmax(H) = Smax(H) · (α/2) · T1 = 0.07 < 0.2 mm

Where ; α : COEFFICIENT OF THERMAL EXPANSION OF MATURE CONCRETE

α = 1E-05
 T1 = 23 from TABLE 4.2

A.7-57

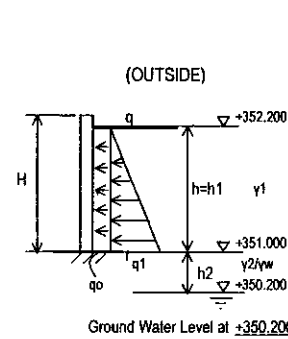
9-5

9.2.2 CALCULATION OF WALL (W20)

(AT+350.650)

(12-12)

1) DESIGN LOAD & FACTORED STRESS(Soil Pressure)



q : Live Load
 Ko : Coefficient of soil pressure at rest
 γ1 = 18.0 KN/m³
 γ2 = 8.0 KN/m³
 γw = 10.0 KN/m³
 q = 5.0 (KN/m²/m)
 Ko = 0.5

• Design load
 { qo = Ko · q = 2.5 (KN/m²/m)
 q1 = Ko · γ1 · h1 = 14.0

• Factored Load
 { w1 = 1.4 · qo = 3.5 (KN/m²/m)
 w2 = 1.4 · q1 = 19.5

• Factored stress at +351.000
 { Mu1 = 1/2 · w1 · h² = 4.20 (KN·m/m)
 Vu1 = w1 · h = 5.43 (KN/m)
 Mu2 = 1/6 · w2 · h² = 7.82 (KN·m/m)
 Vu2 = 1/2 · w2 · h = 15.14 (KN/m)

{ Mu = Mu1 + Mu2 = 12.0 (KN·m/m)
 Vu = Vu1 + Vu2 = 20.6 (KN/m)

2) DESIGN OF SECTION (OUTSIDE/VERTICAL BOT.)

Mu = 12.0 KN·m
 Vu = 20.6 KN/m

b = 1000 mm
 D = 500 mm
 d = 440 mm
 fy = 365 N/mm²
 fcu = 30 N/mm²

[REQUIRED RE-BAR]

K = Mu / (fcu · b · d²) = 0.002 < 0.156
 Z1 = d · [0.5 + √(0.25 - K/0.9)] = 439
 Z2 = 0.95 · d = 418
 Z = 418
 As1 = Mu / 0.95 · fy · Z = 83 mm²
 Asmin = 0.0013 · b · D = 650 mm²

USE ;	D 20	@ 100	(As = 3140 mm ²)
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[CHECK OF SHEAR STRESS]

V = Vu / (b · d) = 0.05 < 4.4 N/mm² OK
 (100 · As) / (b · d) = 0.71 < 3.0
 400/d = 0.9 < 1.0 → 1.0

Vc = 0.79 · { (100 · As) / (b · d) }^{1/3} · { 400/d }^{1/4} · (1/1.25) · { fcu/25 }^{1/3}
 = 0.60 N/mm² > V = 0.05 N/mm² OK

3) CHECK OF CRACKING

(12-12)

9-6

9-7

W20 (OUTSIDE VERTICAL:BOT)

a) CHECK OF MINIMUM REINFORCEMENT (BASED ON 2.6.2.3 OF BS8007)

SECTION	b =	1000	mm	SURFACE ZONE :	d1 =	100	mm
	D =	200	mm				
	dv =	140	mm				
	dh =	120	mm				

Min. ρ = 0.0035 (0.35 %)

As (ver.) = ρ · b · d1 = 350 mm²
 As (hor.) = ρ · b · d1 = 350 mm²

USE ;	VERTICAL	D 14	@ 200	(As = 770 mm ²)
USE ;	HORIZONTAL	D 14	@ 200	(As = 770 mm ²)

b) CHECK OF STEEL STRESS IN DIRECT OR FLEXURAL TENSION FOR SERVICEABILITY LIMIT STATES (BASED ON 3.2.2 OF BS8007)

Ver.	Mu =	12.0	KN·m	M =	8.6	KN·m	b =	1000	mm
Hor.	Mu =	0.0	KN·m	M =	0.0	KN·m	D =	200	mm
							dv =	140	mm
							dh =	120	mm
							fy =	130	N/mm ²
							fcu =	30	N/mm ²

VERTICAL

$K = M / (fcu \cdot b \cdot dv^2) = 0.0146$
 $Z1 = dv \cdot [0.5 + \sqrt{ (0.25 - K/0.9) }] = 138$
 $Z2 = 0.95 \cdot dv = 133$
 $Z = 133$
 $As = M / fy \cdot Z = 497.4 \text{ mm}^2$

USE ; D 14 @ 200 (As = 770 mm²)

HORIZONTAL

$K = M / (fcu \cdot b \cdot dh^2) = 0$
 $Z1 = dh \cdot [0.5 + \sqrt{ (0.25 - K/0.9) }] = 0$
 $Z2 = 0.95 \cdot dh = 0$
 $Z = 0$
 $As = M / fy \cdot Z = 0 \text{ mm}^2$

USE ; D 14 @ 200 (As = 770 mm²)

c) CHECK OF CRACK WIDTH (BASE ON APPENDIX A OF BS8007)

1) CRACK SPACING

$Smax(V) = (fct/fb) \cdot (\psi/2\rho(V)) = 609.09 \text{ mm}$
 $Smax(H) = (fct/fb) \cdot (\psi/2\rho(H)) = 609.1 \text{ mm}$

Where ; $fct/fb = 0.67$
 $\rho(V) = As/b \cdot d1 = 0.0077$
 $\rho(H) = As/b \cdot d1 = 0.0077$

2) CRACK WIDTHS ARISING FORM RESTRAINED SHRINKAGE AND HEAT OF HYDRATION MOVEMENT

$Wmax(V) = Smax(V) \cdot (\alpha/2) \cdot T1 = 0.07 < 0.2 \text{ mm}$
 $Wmax(H) = Smax(H) \cdot (\alpha/2) \cdot T1 = 0.07 < 0.2 \text{ mm}$

Where ; α : COEFFICIENT OF THERMAL EXPANSION OF MATURE CONCRETE

α = 1E-05
 T1 = 23 from TABLE 4.2

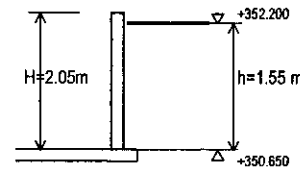
A.7-58

9.2.3 CALCULATION OF FOOTING SLAB (FS20)

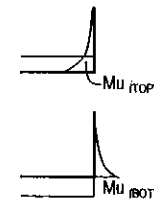
(AT+350.650)

(12-12)

1) FACTORED STRESS



Factored stress at +350.650



from Moment of Inside of Wall (refer to para.9.2.1)
 $Mu_{top} = 5.7 \text{ (KN·m/m)}$
 from Moment of Outside of Wall (refer to para.9.2.2)
 $Mu_{bot} = 12.0 \text{ (KN·m/m)}$

2) DESIGN OF SECTION (BOTTOM BAR)

$Mu = 12.0 \text{ KN·m}$
 $Vu = \text{KN/m}$

$b = 1000 \text{ mm}$
 $D = 200 \text{ mm}$
 $d = 140 \text{ mm}$
 $fy = 365 \text{ N/mm}^2$
 $fcu = 30 \text{ N/mm}^2$

[REQUIRED RE-BAR]

$K = Mu / (fcu \cdot b \cdot d^2) = 0.020 < 0.156$
 $Z1 = d \cdot [0.5 + \sqrt{ (0.25 - K/0.9) }] = 137$
 $Z2 = 0.95 \cdot d = 133$
 $Z = 133$
 $As1 = Mu / (0.95 \cdot fy \cdot Z) = 261 \text{ mm}^2$
 $Asmin = 0.0013 \cdot b \cdot D = 260 \text{ mm}^2$

USE ; D 14 @ 200 (As = 770 mm²)

[CHECK OF SHEAR STRESS]

$V = Vu / (b \cdot d) = 0.00 < 4.4 \text{ N/mm}^2 \text{ OK}$
 $(100 \cdot As) / (b \cdot d) = 0.55 < 3.0$
 $400/d = 2.9 > 1.0 \rightarrow 2.9$

$Vc = 0.79 \cdot \{ (100 \cdot As) / (b \cdot d) \}^{1/3} \cdot (400/d)^{1/4} \cdot (1/1.25) \cdot (fcu/25)^{1/3}$
 $= 0.72 \text{ N/mm}^2 > V = 0.00 \text{ N/mm}^2 \text{ OK}$

3) CHECK OF CRACKING

(12-12)

9-8

FOOTING SLAB (FS20)

a) CHECK OF MINIMUM REINFORCEMENT (BASED ON 2.6.2.3 OF BS8007)

SECTION	b =	1000	mm	SURFACE ZONE:	d(T) =	100	mm
	D =	200	mm		d(B) =	100	mm
	d =	140	mm				

$$\text{Min. } \rho = 0.0035 \text{ (0.35 \%)}$$

$$\text{As (top) = } \rho \cdot b \cdot d(T) = 350 \text{ mm}^2$$

$$\text{As (bot) = } \rho \cdot b \cdot d(B) = 350 \text{ mm}^2$$

USE;	TOP	D 14	@ 200	(As = 770 mm ²)
USE;	BOTTOM	D 14	@ 200	(As = 770 mm ²)

b) CHECK OF STEEL STRESS IN DIRECT OR FLEXURAL TENSION FOR SERVICEABILITY LIMIT STATES (BASED ON 3.2.2 OF BS8007)

Top	Mu =	KN·m	M =	0.0	KN·m	b =	1000	mm
Bott.	Mu =	12.0	KN·m	M =	8.6	KN·m	D =	200
						d =	140	mm
						fy =	130	N/mm ²
						fcu =	30	N/mm ²

TOP

$$K = M / (f_{cu} \cdot b \cdot d^2) = 0$$

$$Z1 = d \cdot [0.5 + \sqrt{ (0.25 - K/0.9) }] = 0$$

$$Z2 = 0.95 \cdot d = 0$$

$$Z = 0$$

$$\text{As} = M / f_y \cdot Z = 0 \text{ mm}^2$$

USE;	D 14	@ 200	(As = 770 mm ²)
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BOTTOM

$$K = M / (f_{cu} \cdot b \cdot d^2) = 0.0146$$

$$Z1 = d \cdot [0.5 + \sqrt{ (0.25 - K/0.9) }] = 138$$

$$Z2 = 0.95 \cdot d = 133$$

$$Z = 133$$

$$\text{As} = M / f_y \cdot Z = 497.4 \text{ mm}^2$$

USE;	D 14	@ 200	(As = 770 mm ²)
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c) CHECK OF CRACK WIDTH (BASE ON APPENDIX A OF BS8007)

1) CRACK SPACING

$$\text{Smax(T) = } (f_{ct}/f_b) \cdot (\varphi/2\rho(T)) = 609.09 \text{ mm}$$

$$\text{Smax(B) = } (f_{ct}/f_b) \cdot (\varphi/2\rho(B)) = 609.1 \text{ mm}$$

$$\text{Where ; } f_{ct}/f_b = 0.67 \quad \rho(T) = \text{As}/b \cdot d(T) = 0.0077$$

$$\rho(B) = \text{As}/b \cdot d(B) = 0.0077$$

2) CRACK WIDTHS ARISING FROM RESTRAINED SHRINKAGE AND HEAT OF HYDRATION MOVEMENT

$$W_{\text{max}}(T) = \text{Smax}(T) \cdot (\alpha/2) \cdot T1 = 0.05 < 0.2 \text{ mm}$$

$$W_{\text{max}}(B) = \text{Smax}(B) \cdot (\alpha/2) \cdot T1 = 0.05 < 0.2 \text{ mm}$$

Where ; α : COEFFICIENT OF THERMAL EXPANSION OF MATURE CONCRETE

$$\alpha = 1E-05$$

$$T1 = 15 \text{ from TABLE 4.2}$$

10. ATTACHMENT10. ПРИЛОЖЕНИЕ

Calculation of Pile Foundation

Calculation Formula

Ultimate Bearing Capacity $q_u = 30 \cdot N \cdot A_p \cdot \eta + U \cdot \sum (l_i \cdot f_i)$
 Allowable Bearing Capacity Bearing Pile Ordinary $R_a = 1/3 \cdot R_u$
 Friction Pile Ordinary $R_a = 1/4 \cdot R_u$

A_p : sectional area of a pile (m²)
 η : clogging coefficient (for steel pile)
 U : perimeter length of pile (m)
 l_i : pile length (m)
 f_i : perimeter friction (tf/m)

Facilities	Pile Length m	Bearing Strata	Embedment m	Mean N value	Pile Cross-sectional Area			Crag. Coef.	Perimeter Length			Cohesive Soil	Thickness m	Cohesion C t/m ²	Allow. Bear. Capacity (tf/pile)			Structure Area m ²	Load ton	Number of Piles			Application	
					300	400	500		300	400	500				Pile Size (mm)									
					m ²	m ²	m ²		m	m	m				300	400	500							
Distribution Chamber	3	3rd sand	2	11	0.09	0.16	0.25	1	1.2	1.6	2	① 3rd	2	0	9	17	27	190	3,700	nubr	412	218	138	
																				area	0.46	0.87	1.38	
																				intrvl	0.67	0.93	1.17	
	Receiving Well	1.5	1st sandy-silt	-	4	0.09	0.16	0.25	1	1.2	1.6	2	① 1st	1.5	4.3	6	9	14	143	2,942	nubr	491	327	211
																					area	0.29	0.44	0.68
																					intrvl	0.53	0.66	0.82
Flocculation and Sedimentation Basin		2	1st sandy-silt	-	4	0.09	0.16	0.25	1	1.2	1.6	2	① 1st	2	4.3	7	10	15	4,124	42,610	nubr	6,088	4,261	2,841
																					area	0.68	0.97	1.45
																					intrvl	0.82	0.98	1.20
	Rapid Sand Filter	4	1st sandy-silt	2	4	0.09	0.16	0.25	1	1.2	1.6	2	① 1st	2	4.3	7	10	15	2,564	26,900	nubr	3,843	2,890	1,794
																					area	0.67	0.95	1.43
																					intrvl	0.81	0.97	1.19

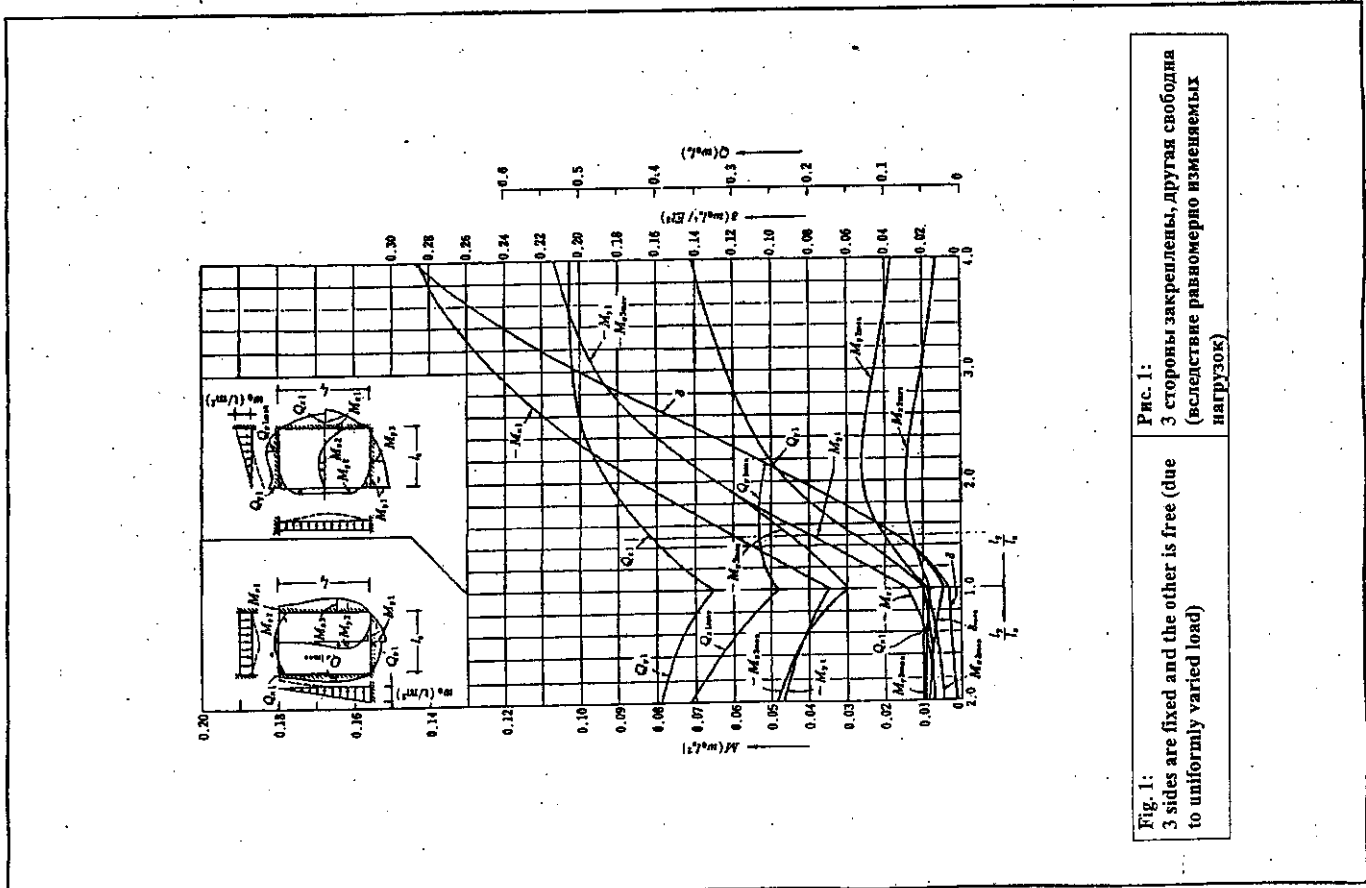


Рис. 1:
 3 стороны закреплены, другая свободна
 (вследствие равномерно изменяемых нагрузок)

Fig. 1:
 3 sides are fixed and the other is free (due to uniformly varied load)

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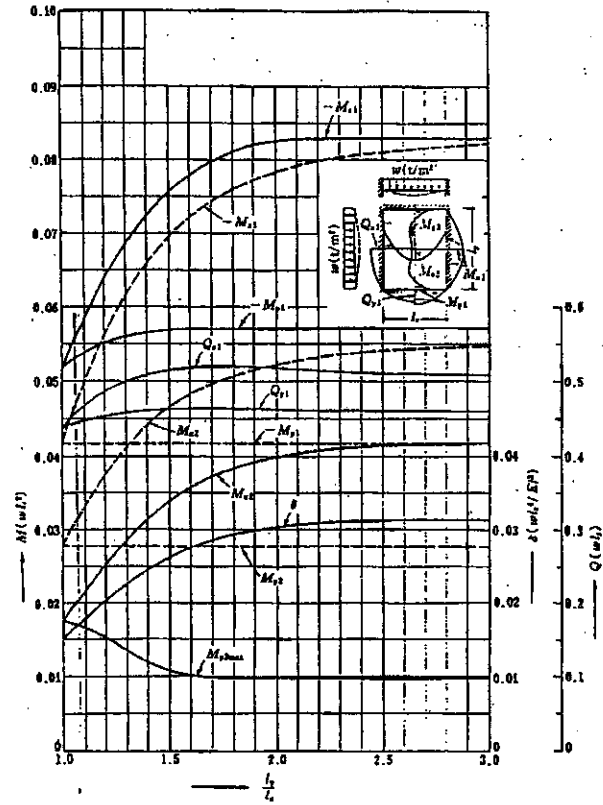


Fig. 2:
4 sides are fixed (due to uniform load)

Рис. 2:
4 стороны закреплены (вследствие
равномерных нагрузок)