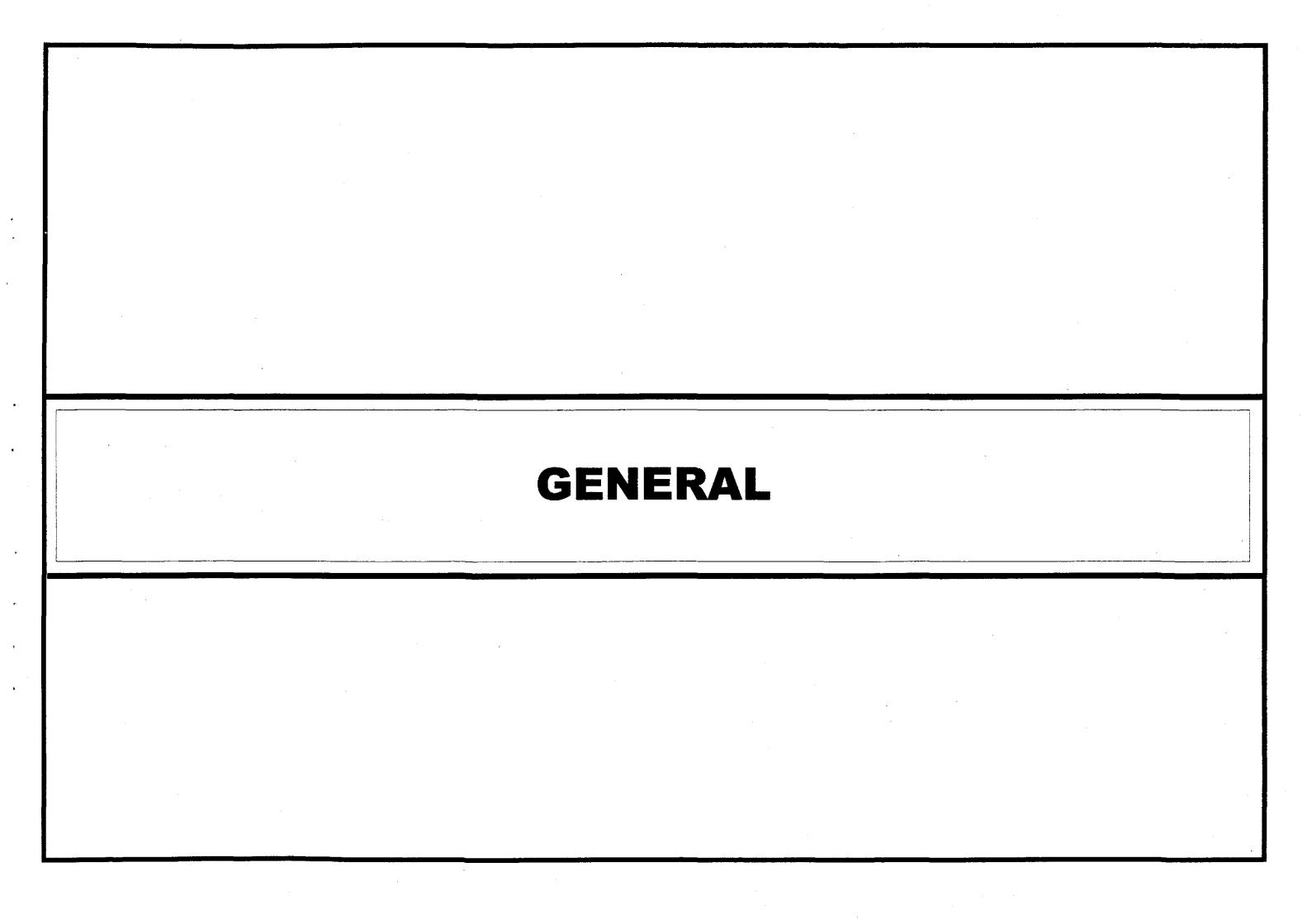


CABANATUAN BYPASS (ULTIMATE STAGE)

BRIDGE NO. 10



GENERAL NOTES FOR BRIDGES - 1

A. DESIGN CRITERIA

- 1. DESIGN SPECIFICATION
 - A. DPWH DESIGN GLIDELINES CRITERIA AND STANDARDS FOR PUBLIC WORKS AND HIGHWAYS, VOL.II
 - B. NATIONAL STRUCTURAL CODE OF THE PHILIPPINES, VOL. II, 2nd ED. 1997
 - C. THE AMERICAN ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICIALS (AASHTO) STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES 16TH EDITION, 1996.
 - D. JAPAN ROAD ASSOCIATION SPECIFICATIONS FOR HIGHWAY BRIDGES
- 2. DESIGN METHODOLOGY

ALLOWABLE STRESS DESIGN (ASD) & LOAD FACTOR DESIGN (ULTIMATE STRENGTH DESIGN)

3.1 DEAD LOADS

WEIGHT

A. CONCRETE B. STEEL

24.50 kN/m 77.00 kN/m³ 19.00 kN/m³

C. EARTH D. WEARING SURFACE (50mm THK.)

3.2 LIVE LOADS

A. AASHTO MS18 (HS20) TRUCK AND EQUIVALENT LANE LOADING.

B. SIDEWALK LOAD

 $SPAN \le 30.5m ; 4.07 kN/m^2$

107 kN 107 kN

SPAN > 30.5m; $\left(\frac{1.437 + \frac{43.798}{15.24}}{15.24}\right) \frac{(16.76-W)}{15.24} \frac{kN/m^2}{15.24}$

L : LOADED LENGTH

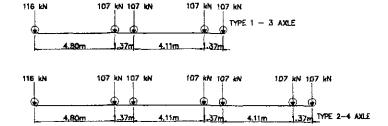
W : SIDEWALK WIDTH

C. ALTERNATE MILITARY LOADING.

107 kN 107 kN 1.22m

116 KN

D. PERMIT DESIGN LOAD (SPECIAL PERMIT REQUIRED BEFORE PASSING BRIDGE)



- 3.3 IMPACT
 - IN ACCORDANCE WITH DIVISION 1 OF AASHTO STANDARD SPECIFICATIONS, 1996.
- 3.4 SEISMIC LOAD

IN ACCORDANCE WITH DIVISION 1A OF THE 1996 AASHTO STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES USING ACCELERATIONS COEFFICIENT OF 0.40 AND SEISMIC PERFORMANCE CATEGORY D.

3.5 HYDRAULIC DESIGN DATA

50-year design discharge, $q_{50}=6,990$ m³/sec. design flow velocity, $v_{50}=1.88$ m/sec. design flood water level, dfwl = el + 32.3 m catchment area, ca = 2,508.6 km²

3.6 TEMPERATURE RANGES

ASSUMED BASE TEMPERATURE : +280° MINIMUM AMBIENT AIR TEMPERATURE : +180° MAXIMUM AMBIENT AIR TEMPERATURE : +38C* TEMPERATURE DIFFERENCE BETWEEN TOP OF SLAB AND OTHER PARTS OF STRUCTURE: +100"

3.7 CONSTRUCTION LOADS

CONSTRUCTION LOADS SHALL BE AS STIPULATED IN THE AASHTO GUIDE SPECIFICATIONS. THE CONTRACTOR IS RESPONSIBLE FOR ENSURING THAT THESE LOADS ARE NOT EXCEEDED AND THAT THE MEMBER STRESSES ARE WITHIN ALLOWABLE DURING CONSTRUCTION.

3.B OTHER LOADS

IN ACCORDANCE WITH AASHTO STANDARD SPECIFICATIONS, 1996.

- 3.9 LOAD COMBINATION (LOAD FACTOR DESIGN)
- A. GROUP 1 = 1.3 [1.0 D + 1.67(L+1)n + 1.0 SF]
- GROUP 1B = 1.3 [1.0 D + 1.0(L+1)p + 1.0 SF
- GROUP VII = 1.3 [1.0 D + 1.0 SF + EQ
- D. OTHER LOAD COMBINATIONS SHALL BE IN ACCORDANCE WITH AASHTO GUIDE SPECIFICATIONS.

B. MATERIALS

1. CONCRETE

UNLESS INDICATED OTHERWISE ON PLANS, THE CONCRETE CLASS AND STRENGTH SHALL BE AS FOLLOWS:

STRUCTURAL MEMBER	CLASS	28 - DAY STREN	CYLINDER IGTH	MAX. SIZE OF COARSE	REMARKS
STRUCTURAL MEMBER	ULAGG	MPo	PSI	AGGREGATE mm (in.)	KEWAKKS
CAST — IN PLACE GIRDERS, SLABS, DIAPHRAGMS, WINGWALLS BACKWALLS, ABUTMENT COPINGS, COLUMNS, SLABS, SHEAR KEYS	AA2	28	4060	20	
FOOTINGS, PILE CAP. BORED PILES, APPROACH SLAB	AA1	28	4060	25	*SEE NOTE BELOW
THIN REINFORCED SECTIONS, PARAPET, RAILINGS & RAILPOST CURB AND SIDEWALK	С	21	3000	12	
PRESTRESSED CONCRETE MEMBERS: AASHTO GRDERS, PRECAST DECK SLAB PANELS,	₽₽	35 41	5075 5946	20 20	• Transfer
C.I.P. POST-TENSIONED SLAB		21		38	- SERVICE
STEEL SHEET PILE CAP	A	Z1	3000	Ja	
RUBBLE CONC./CONC. BLOCKS FOR SLOPE PROTECTION	В	16.5	2400	50	
LEAN CONCRETE		17	1450	38	

* NOTE:
THE CEMENT CONTENT OF THE DESIGN MIX SHALL BE ADJUSTED IN
ACCORDANCE WITH THE AASHTO PROVISIONS WHEN CONCRETING UNDER
WATER TO COMPENSATE FOR THE LOSS OF STRENGTH DUE TO WATER
INFILTRATION.

2. REINFORCING STEEL

(a) REINFORCING STEEL SHALL CONFORM TO AASHTO M31 (ASTM A615), GRADES 40 & 60 DEFORMED WITH MINIMUM YIELD STRENGTH AS DESCRIBED BELOW.

REBAR GRADE	YIELD STRENGTH fy (MPa)	SIZE (mm)
40	276 (40 ksi)	16mm# & BELOW, UNLESS OTHERWISE NOTED
60	415 (60 ksi)	20mmø & ABOVE

- (b) REINFORCING STEEL SHALL BE FREE OF MILL SCALES, OIL OR ANY SUBSTANCES WHICH WILL WEAKEN THE BOND WITH CONCRETE.
- (c) REINFORCING STEEL SHALL BE WELDABLE TYPE. WELDING REINFORCING STEEL SHALL CONFORM TO ANSI/AWS D1.4.

3. PRESTRESSING STEEL

PRESTRESSING STEEL SHALL BE SEVEN-WIRE UNCOATED STRESS-RELIEVED STRANDS AND SHALL CONFORM TO AASHTO M203 (ASTM A416) WITH MINIMUM ULTIMATE STRENGTH OF Fy = 1860 MPa (270,000psi).

4. STRUCTURAL STEEL, BOLTS AND WELDS

MATERIALS	YIELD STRENGTH fy (MPa)	REFERENCE SPECIFICATIONS
STRUCTURAL STEEL FOR MAIN GIRDERS	345 (SMA 490W / GRADE 50W)	JIS SMA 490W (ATMOSPHERIC CORROSION RESISTANT); AASHTO/ASTM M270 (A709)
STRUCTURAL STEEL OTHER THAN MAIN GIRDER	250- (SMA 400W / GRADE 36W)	JIS SMA 400W (ATMOSPHERIC CORROSION RESISTANT); AASHTO/ASTM M270 (A709)
SHEAR STUD CONNECTORS FOR MAIN BRIDGE	345	ASTM A108 (CORROSION RESISTANT)
HIGH STRENGTH BOLTS FOR MAIN BRIDGE		AASHTO M253, ASTM 490M (CORROSION RESISTANT)
WELDS		LATEST ANSI /AASHTO/AWS D1.5 BRIDGE WELDING CODE FOR CORROSION RESISTANT STEEL

5. ELASTOMERIC BEARING PADS

ELASTOMERIC BEARING PADS SHALL BE 100% VIRGIN CHLOROPRENE (NEOPRENE) PADS WITH DUROMETER HARDNESS 60 AND SHALL BE LAMINATED WITH NON-CORROSIVE MILD STEEL SHEETS (ASTM A570). ELASTOMERIC PADS SHALL CONFORM TO THE REQUIREMENTS AS PRESCRIBED IN OPWH D.O. NO. 25 SERIES OF 1997 "REVISED OPWH STANDARD SPECIFICATION FOR ELASTOMERIC BEARING PAD."

SPECIFICATIONS							
DURO HARDNESS, SHORE A (A	STM D-2240)60±5						
TENSILE STRENGTH ASTM	0 412-175 Kg/cm ² (min)						
ULTIMATE ELONGATION %	350 % (min)						
MATERIAL	NEOPRENE						

C. CONSTRUCTION

THESE NOTES ARE PROVIDED FOR QUICK REFERENCE ONLY AND SHALL BE READ IN CONJUNCTION WITH THE TECHNICAL SPECIFICATIONS FOR THE PROJECT.

THE DESIGN OF BRIDGES IS BASED ON THE CONSTRUCTION SEQUENCE SHOWN IN THE DRAWINGS. ANY VARIATION FROM THE SEQUENCE MUST BE APPROVED BY THE ENGINEER.

CONSTRUCTION SHALL COMPLY WITH 1995 DPWH STANDARD SPECIFICATION FOR HIGHWAYS, BRIDGES AND AIRPORTS OR MODIFIED BY SPECIAL PROVISIONS.

1. DIMENSIONS

- 1.1 SECTION, DIMENSIONS AND DISTANCES SHALL NOT BE SCALED FOR CONSTRUCTION PURPOSES. THE INDICATED DIMENSION SHALL GOVERN UNLESS OTHERWISE SPECIFIED.
- 1.2 ALL DIMENSIONS SHOWN ARE IN MILLIMETERS UNLESS
- 1.3 ALL STATIONING ARE IN KILOMETER PLUS METER AND

HIGO		DATE	SIGNATURE	4		REPUBLIC OF THE PHIL			PROJECT AND LOCATION :	SCALE :	SHEET CONTENTS :	SHEET NO. :
	DESIGNED	16/14/or	Trefly low	FUHL - PMO	DEPARTMENT	T OF PUBLIC WOR		YS THE SECRETARY	THE DETAILED DESIGN STUDY ON UPGRADING INTER-URBAN HIGHWAY SYSTEM		BRIDGE NO. 10 PAMPANGA RIVER BRIDGE	
JAPAN INTERNATIONAL COOPERATION AGENCY	CHECKED	017192	I C SANTOS		Phylipsed by:	Recommended By:	Recommended By: (See cover sheet for	Approved By: (See cover sheet for	ALONG THE PAN-PHILIPPINE HIGHWAY (Plaridel, Cabanatuan and San Jose Bypasses)	AS SHOWN	GENERAL NOTES	B10G-01
KATAHIRA & ENGINEERS YACHIYO ENGINEERING CO., LTD.	SUBMITTED	digioz	A. JAN DE	DANILO C. TRAJANO Project Director	ADRIANO M. DOROY	DILBERTO S. REYES	Signature) MANUEL M. BONGAN	Signature/Approvol) SIMEON A. DATUMANONG Secretory	CABANATUAN BYPASS - CONTRACT PACKAGE III	FULSIZE A1	(1 OF 3) (ULTIMATE STAGE)	1

GENERAL NOTES FOR BRIDGES - 2

2. SETTING OUT

THE SETTING OUT AND THE ELEVATIONS OF THE DIFFERENT COMPONENTS OF THE STRUCTURE SHALL BE APPROVED BY THE ENGINEER PRIOR TO THE START OF ANY CONSTRUCTION WORK.

3. REINFORCED CONCRETE

3.1 CAST IN PLACE CONCRETE SHALL BE CLASS "AA1" OR "AA2" EXCEPT RAILINGS WHICH SHALL BE CLASS "C". UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED EDGES SHALL BE CHAMFERED 20mm EXCEPT RAILINGS AND RE-ENTRANT ANGLES WHICH SHALL BE CHAMFERED AND FILLETED 13mm RESPECTIVELY.

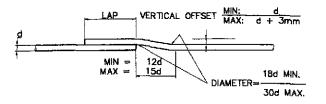
3.2 CONGRETE MIX AND PLACING

- (1) DESIGN OF CONCRETE MIX SHALL MEET THE DESIGN CONCRETE STRENGTH GIVEN UNDER ITEM 1 OF MATERIALS.
- (2) CONCRETE SHALL BE DEPOSITED, VIBRATED AND CURED IN ACCORDANCE WITH THE SPECIFICATION.
- (3) FOR CONCRETE DEPOSITED AGAINST THE GROUND, LEAN CONCRETE WITH A MINIMUM THICKNESS OF 100mm SHALL BE LAID FIRST BEFORE INSTALLING THE REINFORCEMENT. THIS LEAN CONCRETE SHALL NOT BE CONSIDERED IN MEASURING THE STRUCTURAL DEPTH OF CONCRETE SECTION.
- (4) THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER FOR APPROVAL PLACING SEQUENCES FOR ALL CONCRETING WORK.

3.3 BAR BENDING, SPLICING AND PLACING

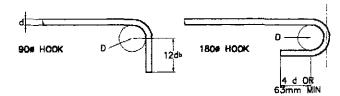
- (1) THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER, FOR APPROVAL, SHOP DRAWINGS INDICATING THE BENDING, CUTTING, SPLICING AND INSTALLATION OF ALL REINFORCING BARS.
- (2) BARS SHALL BE BENT COLD. BARS PARTIALLY EMBEDDED IN CONCRETE SHALL NOT BE FIELD BENT UNLESS PERMITTED BY
- (3) BAR SPLICING NOT INDICATED ON DRAWINGS SHALL BE SUBJECT TO THE APPROVAL OF THE ENGINEER.
- (4) WELDED SPLICES, IF APPROVED BY THE ENGINEER, SHALL DEVELOP IN TENSION AT LEAST 125% OF THE SPECIFIED YIELD STRENGTH OF THE RAPS
- (5) NOT MORE THAN 50% OF THE BARS AT ANY ONE SECTION SHALL BE SPLICED.
- (6) UNLESS OTHERWISE SHOWN ON DRAWINGS, THE CLEAR DISTANCE BETWEEN PARALLEL BARS IN A LAYER SHALL NOT BE LESS THAN 1.5 TIMES THE NOMINAL DIAMETER OF THE BAR NOR LESS THAN 1.5 TIMES THE MAXIMUM SIZE OF COARSE AGGREGATE. THE CLEAR DISTANCE BETWEEN LAYERS SHALL NOT BE LESS THAN 25mm NOR ONE BAR DIAMETER. THE BARS IN THE UPPER LAYER SHALL BE PLACED DIRECTLY ABOVE THOSE IN THE BOTTOM LAYER.

(7) CRANKED SPLICES

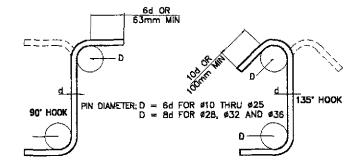


(8) HOOKS AND BENDS

DIMENSIONS OF 90-DEGREE AND 180-DEGREE HOOKS



PIN DIAMETER: D = 6d FOR \$10 THRU \$25 D = 8d FOR \$28, \$32 AND \$36 DIMENSIONS FOR STIRRUPS AND TIE HOOKS



3.4 CONCRETE COVER TO REINFORCEMENT

UNLESS OTHERWISE NOTED, ALL BAR DIMENSIONS ARE REFERRED TO THE CENTER OF BARS AND THE MINIMUM COVERING MEASURED FROM THE SURFACE OF THE CONCRETE TO THE FACE OF ANY BAR SHALL BE 40mm. FOR SUBSTRUCTURE PERMANENTLY EXPOSED TO EARTH, COVERING SHALL BE 75mm.

3.5 CONSTRUCTION JOINT

- (1) THE POSITION AND FORM OF ANY CONSTRUCTION JOINT SHALL BE AS SHOWN ON DRAWINGS OR AS AGREED WITH THE ENGINEER.
- (2) THE INTERFACE BETWEEN THE FIRST AND SECOND POUR CONCRETE SHALL BE ROUGHENED WITH AN AMPLITUDE OF FIMM MINIMINA.

3.6 FALSEWORK

ALL FALSEWORK SHALL BE DESINED BY THE CONTRACTOR SUBJECT TO THE APPROVAL BY THE ENGINEER. FALSEWORKS SHOWN IN THE DRAWINGS SHALL SERVE AS REFERENCE ONLY.

3.7 FORMWORK

FORMWORKS SHALL BE CONSTRUCTED SUCH THAT IT WILL NOT MELD UNDER THE LOAD AND SHALL BE SUCH AS TO AVOID THE FORMATION OF FINE. ALL CORNERS OF CONCRETE MEMBERS SHALL BE CHAMFERED TO 25mm UNLESS NOTED OTHERWISE ON DRAWINGS. STRIPPING OF FORMS AND SHORES SHALL BE AS DESIGNATED BY THE ENGINEER. THE FOLLOWING MAYBE USED AS A GUIDE.

								10111	4. 53141
SHORING UNDER GIRDE	RS, BE	AMS,	FRAM	IES.			1	14	DAYS
DECK SLABS							•	14	DAYS
WALLS								7	DAYS
COLUMNS				-	-			7	DAYS
SIDES OF BEAMS AND VERTICAL SURFACES						_		2	DAYS

MIN TIME

3.8 PROTECTION AND CURING OF CONCRETE

CONCRETE SURFACES SHALL BE PROTECTED FROM HARMFUL EFFECTS OF SUN, WIND AND RUNNING WATER AND SHALL BE KEPT DAMP FOR AT LEAST 7 DAYS.

4. EMBANKMENT CONSTRUCTION SEQUENCE

APPROACH EMBANKMENT SHALL BE CONSTRUCTED PRIOR TO CONSTRUCTION OF ABUTMENT PILES.

5. REINFORCED CONCRETE CAST-IN-PLACE BORED PILES

5.1 THE REQUIRED ALLOWABLE BEARING CAPACITY FOR EACH PILE DIAMETER IS AS FOLLOWS:

PILE DIA.	NORMAL	(KN)	ULTIMATE	(KN)
FILE DIA	COMPRESSION	TENSION	COMPRESSION	TENSION
ø1000	3000	1200	9000	3600
ø1200	4000	1500	12000	7000
ø1500	5500	1900	16500	9000

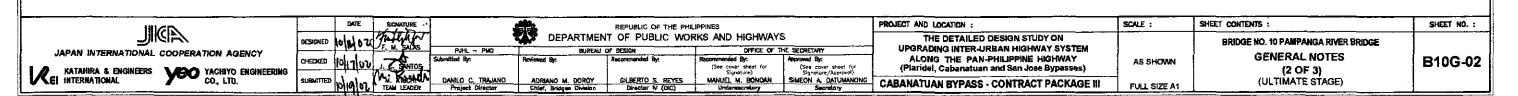
- 5,2 BOTTOM OF BORED PILES SHALL BE EMBEDDED AT LEAST TWO TIMES PILE DIAMETER (2D) INTO HARD STRATA CAPABLE OF DEVELOPING ALLOWABLE BEARING CAPACITY AS SPECIFIED. IF THE ABOVE CONDITION IS NOT MET DURING CONSTRUCTION. THE PILE SHALL BE INCREASED AND THE DESIGNER/CONSULTANT SHALL BE NOTIFIED FOR CONFIRMATION. AN ON-SITE SUBSURFACE INVESTIGATION SHALL ALSO BE UNDERTAKEN DURING CONSTRUCTION FOR CONFIRMATION/VERIFICATION OF DATA USED IN THE DESIGN.
- 5.3 PILE LENGTHS SHOWN ARE ESTIMATED LENGTHS DURING DESIGN. DETERMINATION OF REQUIRED PILE LENGTHS SHALL BE DETERMINED BY THE CONTRACTOR BASED ON THE RESULTS OF FIELD INVESTIGATIONS CARRIED OUT BY THE CONTRACTOR. SEE THE SPECIAL PROVISIONS OF THE TECHNICAL SPECIFICATIONS.
- 5.4 ULTRASONIC INTEGRITY TESTING (AS PER SPECIFICATIONS) SHALL BE CONDUCTED FOR ALL PILES TO VERIFY/CHECK THE CONCRETE HOMOGENEITY AND TO LOCATE/EVALUATE ANY POSSIBLE IRREGULARITY IN THE COMPLETED BORED PILES AS DESCRIBED IN THE SPECIAL PROVISIONS.
- 5.5 STATIC LOAD TEST AND HIGH STRAIN DYNAMIC LOAD TEST SHALL BE CONDUCTED
 AS INDICATED IN THE SCHEDULE OF PILE LOAD TEST OF THE COMPLETED BORED PILES.
 THE RESULT SHALL BE SUBMITTED FOR EVALUATION AND REFERENCE.

6. ADDITIONAL SOIL INVESTIGATION

ADDITIONAL SUBSURFACE INVESTIGATION (BORE HOLES) SHALL BE CONDUCTED FOR EACH PIER OF MAIN BRIDGE AND ABUTMENT LOCATION AND HALF THE NUMBER OF PIERS FOR THE APPROACH SPANS TO CONFIRM/VERIFY THE DESIGN SCIL PROFILE AND CAPACITIES. IF THE RESULTS OF THE SOIL INVESTIGATION DIFFERS FROM THE SCIL DATA USED IN DESIGN, THE CONTRACTOR SHALL NOTIFY THE ENGINEER/CONSULTANT TO MAKE THE NECESSARY ADJUSTMENTS IN THE FOUNDATION.

CAMBER

- 7.1 STEEL AND CONCRETE GIRDERS SHALL BE CONSTRUCTED WITH CAMBER INDICATED IN THE DRAWINGS.
- 7.2 AFTER ERECTION IS COMPLETE, THE FLANGE ELEVATION OF THE GIRDERS SHALL BE SURVEYED. BASED ON THIS INFORMATION, THE CONTRACTOR SHALL DETERMINE THE HAUNCH HEIGHTS REQUIRED ALONG THE STRUCTURE IN ORDER THAT THE FINISHED GRADE SHOWN IN THE DRAWINGS WILL BE ACHIEVED, TAKING DUE ACCOUNT OF FURTHER DEFLECTIONS TO BE INCURRED WHEN THE DECK AND SIDEWALKS ARE ADDED AND THE ORDER IN ERECTION OF DECK PANEL IS TO TAKE PLACE.
- 7.3 THE CONTRACTOR SHOULD PREPARE & SUBMIT A GEOMETRY CONTROL REPORT TO THE ENGINEER INDICATING THE ASSUMPTIONS AND CALCULATION PROCEDURES THAT HAVE BEEN FOLLOWED IN DETERMINING HAUNCH HEIGHTS. THE CONTRACTOR SHOULD MONITOR AND UPDATE THIS REPORT AS NECESSARY AS ERECTION PROCEEDS.



GENERAL NOTES FOR BRIDGES - 3

8. STRUCTURAL STEEL

THE CONTRACTOR SHALL PREPARE AND SUBMIT SHOP DRAWINGS FOR ALL STRUCTURAL STEEL WORK, THESE SHOP DRAWINGS SHALL BE APPROVED BY THE ENGINEER BEFORE ANY FABRICATION COMMENCES. STRUCTURAL STEEL WORKS SHALL COMPLY WITH THE LATEST ANSI/AASHTO/AWS D1.5 BRIDGE WELDING CODE AND FABRICATION REQUIREMENTS.

9. SHORING

- 9.1 CAMBER FOR REINFOCED CONCRETE SUPERSTRUCTURES WERE DETERMINED BASED ON THE USE OF SHORINGS DURING CONSTRUCTION.
- 9.2 CAMBER FOR COMPOSITE SUPERSTRUCTURES WITH PRECAST PRESTRESSED GIRDERS WERE DETERMINED BASED ON UNSHORED CONDITIONS.

10. EXCAVATION

EXCAVATION FOR STRUCTURES SHALL BE TO THE NEAT LINES OF FOOTING OR AS SPECIFIED IN THE STANDARD SPECIFICATIONS.

11. WATER ELEVATION

WATER ELEVATIONS SHOWN ON PLANS ARE APPROXIMATE ONLY ANY VARIATION FOUND DURING CONSTRUCTION SHALL NOT BE CONSIDERED AS A BASIS FOR EXTRA COMPENSATION.

12. DETOUI

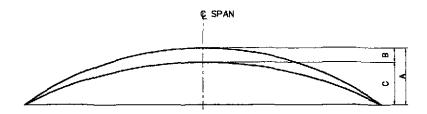
THE CONTRACTOR SHALL CONSTRUCT AND MAINTAIN DETOUR BRIDGES, AND/OR ROADS DURING CONSTRUCTION TO ALLOW CONTINUOUS FLOW OF TRAFFIC. THEY SHALL BE CONSTRUCTED ON LOCATION AS SHOWN ON PLANS OR AS DIRECTED BY THE ENGINEER, NO ADDITIONAL COST SHALL BE ALLOWED FOR ANY RELOCATION OF DETOUR.

13. PRESTRESSED CONCRETE

GIRDER DESIGN GUIDE

- 13.1 POST—TENSIONING; THE PROPOSED TYPE OF TENDONS WHICH WILL
 BE USED IN THE POST—TENSIONED DESIGNS AND ALL NECESSARY
 ADDITIONAL DETAILS INCLUDING THOSE FOR END ANCHORAGES, METHODS
 TO BE EMPLOYED AND PROCEDURES TO BE FOLLOWED, SHALL BE AS
 APPROVED BY THE ENGINEER. PORTION OF THE TENDONS SHALL BE
 DRAPED LONGITUDINAL IN PARABOLIC PORTIONS, ALL TENDONS SHALL BE PLACED
 SO THAT THEIR CENTER OF GRANTY WILL BE AT THE POSITION SHOWN ON PLANS.
 THE TOTAL POST—TENSION FORCE AFTER LOSSES REQUIRED AT MIDSPAN SHALL
 BE PROVIDED AS CALLED FOR IN THE VARIOUS DESIGNS. THE REQUIRED FORCES
 AFTER LOSSES SHALL BE OBTAINED BY APPLYING INITIAL TENSILE FORCES OF
 SUFFICIENT MAGNITUDE TO ALLOW FOR ALL SUBSEQUENT LOSSES, INCLUDING
 THOSE FOR ELASTIC SHORTENING, SHRINKAGE, CREEP, RELAXATION, FRICTION,
 AND EFFICIENCY OF END ANCHORAGES. AFTER SECURING THE END
 ANCHORAGES ALL TENDONS SHALL BE PRESSURE GROUTED IN THEIR
 CONDUITS IN ACCORDANCE WITH THE "SPECIFICATIONS".
- 13.2 CONCRETE FOR GIRDERS SHALL BE A MINIMUM STRENGTH OF 41 N/mm 2 (5,945 PSI) AT THE AGE OF 28 DAYS.
- 13.3 CONCRETE FOR CAST-IN-PLACE SLAB HAVE A MINIMUM STRENGTH OF 28 N/mm2 (4,060 PSI) AT THE AGE OF 28 DAYS.
- 13.4 THE CONTRACTOR MAY PROPOSE ANY ALTERNATIVE TENDON SIZE
 AND LAYOUT WHICH SHALL MEET THE APPROVAL OF THE ENGINEER.
- 13.5 THE REQUIRED STRENGTH OF CONCRETE AT TIME OF TENSIONING SHALL BE 35 MPd (5,075 PSI). A GRID CONSISTING OF #12 BARS AT 100 CENTERS IN BOTH DIRECTIONS SHALL BE PLACED NEAR EACH ANCHORAGE OF THE POST—TENSIONING SYSTEM.

- 13.6 HANDLING PRESTRESSED CONCRETE BEAMS: THE BEAMS SHALL BE MAINTAINED IN AN UPRIGHT POSITION AND SHALL BE LIFTED BY SUITABLE DEVICES PROVIDED AT THE ENDS OF THE BEAMS, ATTENTION IS DIRECTED TO THE INCREASED DIFFICULTY OF LIFTING BEAMS WITHOUT END BLOCKS. THE CONTRACTOR'S PROPOSED LIFTING DETAILS SHOULD BE GIVEN CAREFUL CONSIDERATION BEFORE BEING SUBMITTED ON SHOP DRAWING FOR APPROVAL. THE USE OF HOLES FOR LIFTING PURPOSES WILL NOT BE PERMITTED.
- 13.7 CONTRACTOR SHALL SUBMIT FOR APPROVAL BY THE ENGINEER THE CALCULATED ELONGATION OF THE PRESTRESSING TENDONS CORRESPONDING TO THE REQUIRED JACKING FORCES.



DEAD LOAD CAMBER DIAGRAM

- A = INITIAL CAMBER ESTIMATED PRESTRESS CAMBER LESS DEFLECTION DUE TO GIRDER DEAD LOAD
- B = DEFLECTION DUE TO SLAB, DIAPHRAGM, SIDEWALKS, RAILING AND RAILPOST

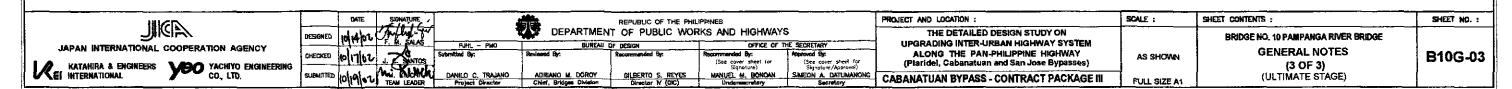
C = FINAL CAMBER

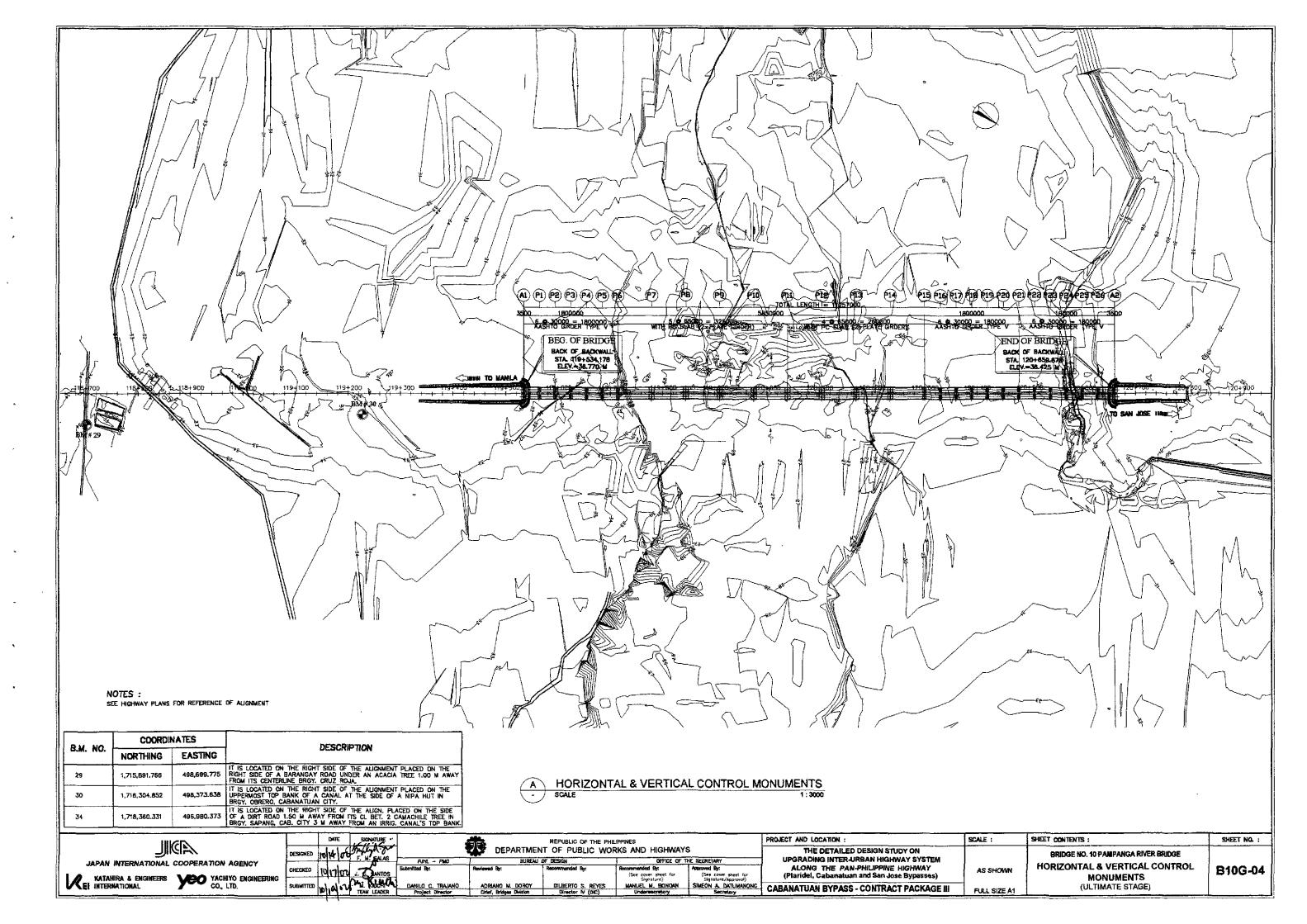
- NOTE; A AND 8 ARE THEORETICAL VALUES AND MAY VARY WITH ACTUAL (AGE) CONCRETE STRENGTH, VARIOUS PRESTRESSING CONDITIONS, CREEP FACTOR, AND PRESTRESS LOSSES.

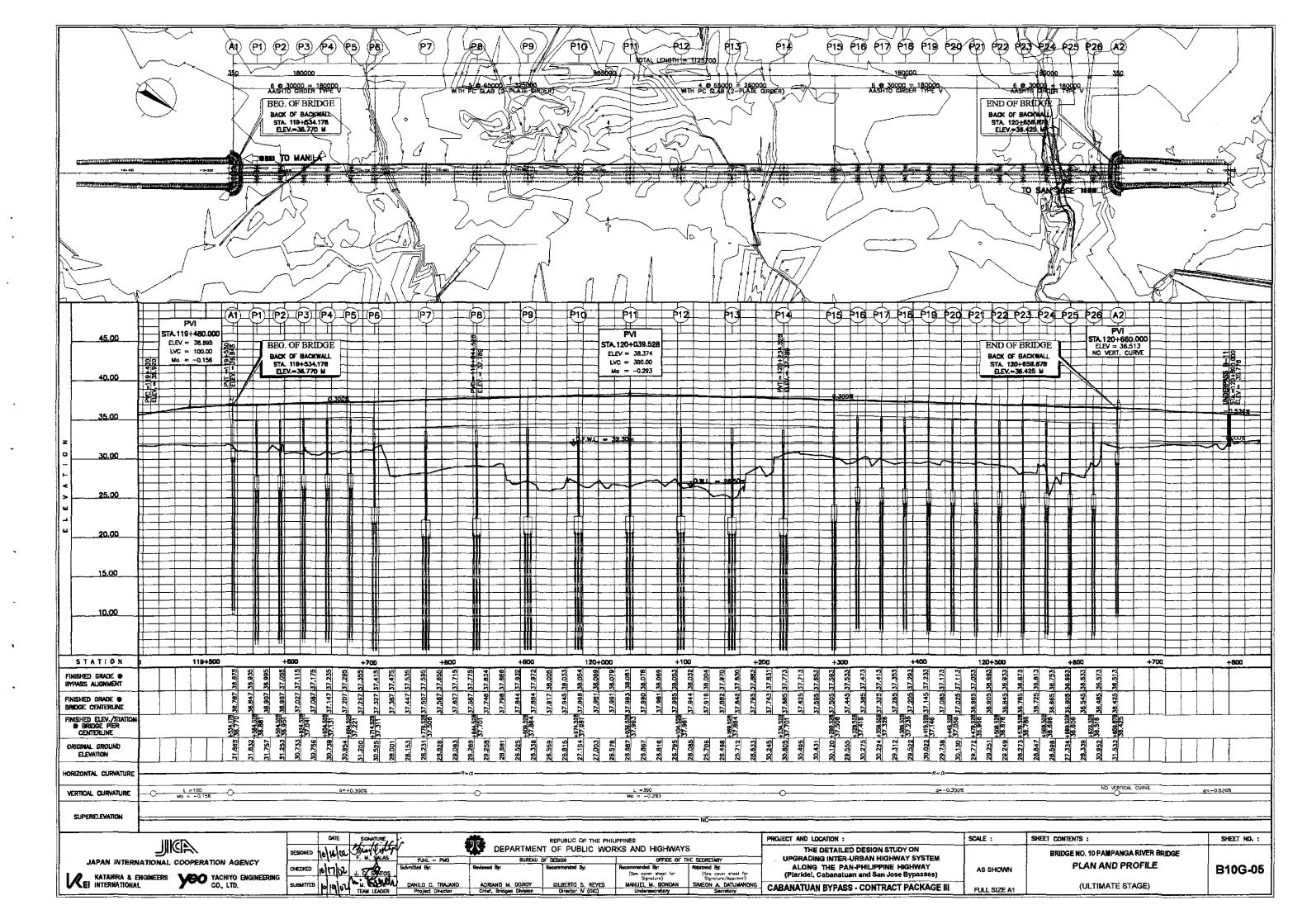
 CONTRACTOR SHALL SURVEY TOP OF GIRDERS TO OBTAIN ACTUAL VALUE OF A AND ADJUST PROFILE ACCORDINGLY.
- 13.8 PRECAST GIRDERS AND DECK PANELS SHALL MEET THE TOLERANCES SPECIFIED IN THE AASHTO GUIDE SPECIFICATIONS FOR DESIGN AND CONSTRUCTION OF SEGMENTAL CONCRETE BRIDGES.
- 13.9 TRANSVERSE DEFLECTION OF PRECAST GIRDERS SHALL NOT EXCEED 1/500 th of the girder length. Where deflection exceed this value, procedures for correction shall be submitted to the engineer for review, if correction by approved procedure is not possible, the girder shall be rejected.

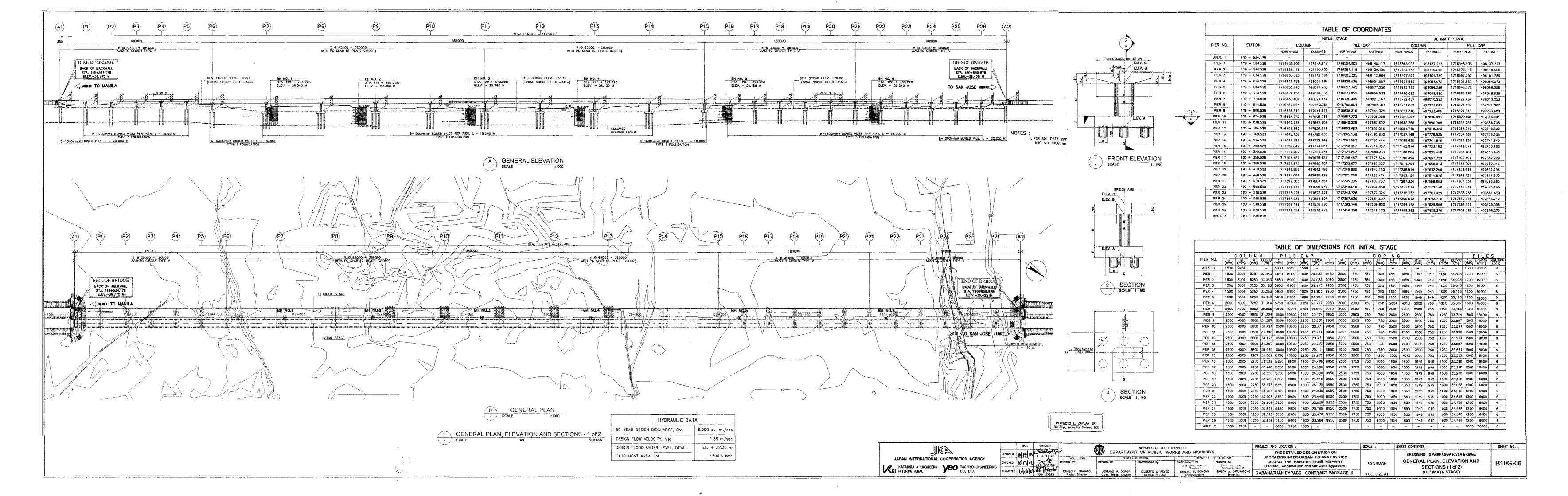
14. PRECAST CONCRETE DECK PANELS

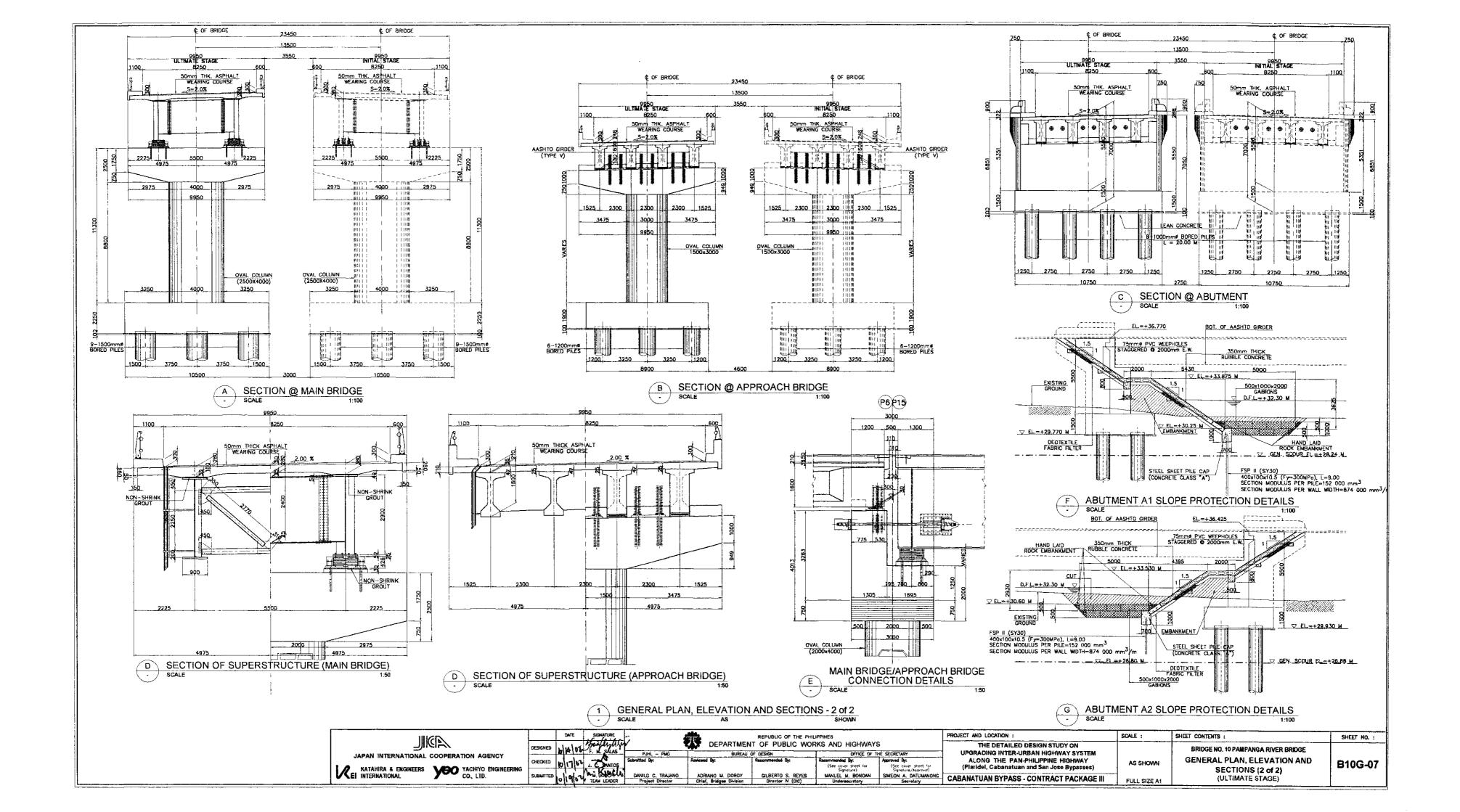
- 14.1 PRIOR TO START OF DECK PANEL FABRICATION, A REPORT SHALL BE PREPARED AND SUBMITTED TO THE ENGINEER FOR REVIEW OUTLINING THE COMPLETE FABRICATION AND ERECTION PROCESS FOR THE DECK PANEL. ALL ASPECTS AND STAGES OF THE CONSTRUCTION PROCESS SHALL BE FULLY DESCRIBED INCLUDING BUT NOT LIMITED TO CONCRETE PRODUCTION, PLACEMENT, CURING, TRANSVERSE PRESTRESSING, HANDLING, STORAGE, TRANSPORT, ERECTION, GEOMETRY CONTROL, INFILL CONCRETE SECTIONS, INFILL OF SHEAR STUD BLOCKOUTS, LEVELING GROUTS, ETC.
- 14.2 DECK PANELS SHALL BE CAST HORIZONTALLY,
- 14.3 FORMED LENGTH SHALL BE AS INDICATED IN THE DRAWINGS.
- 14.4 THE RUBBED FINISH APPLIED TO THE PRECAST CONCRETE SLAB SHALL PRODUCE A UNIFORM AND HOMOGENOUS APPEARANCE ACCROSS ADJACENT PANELS.

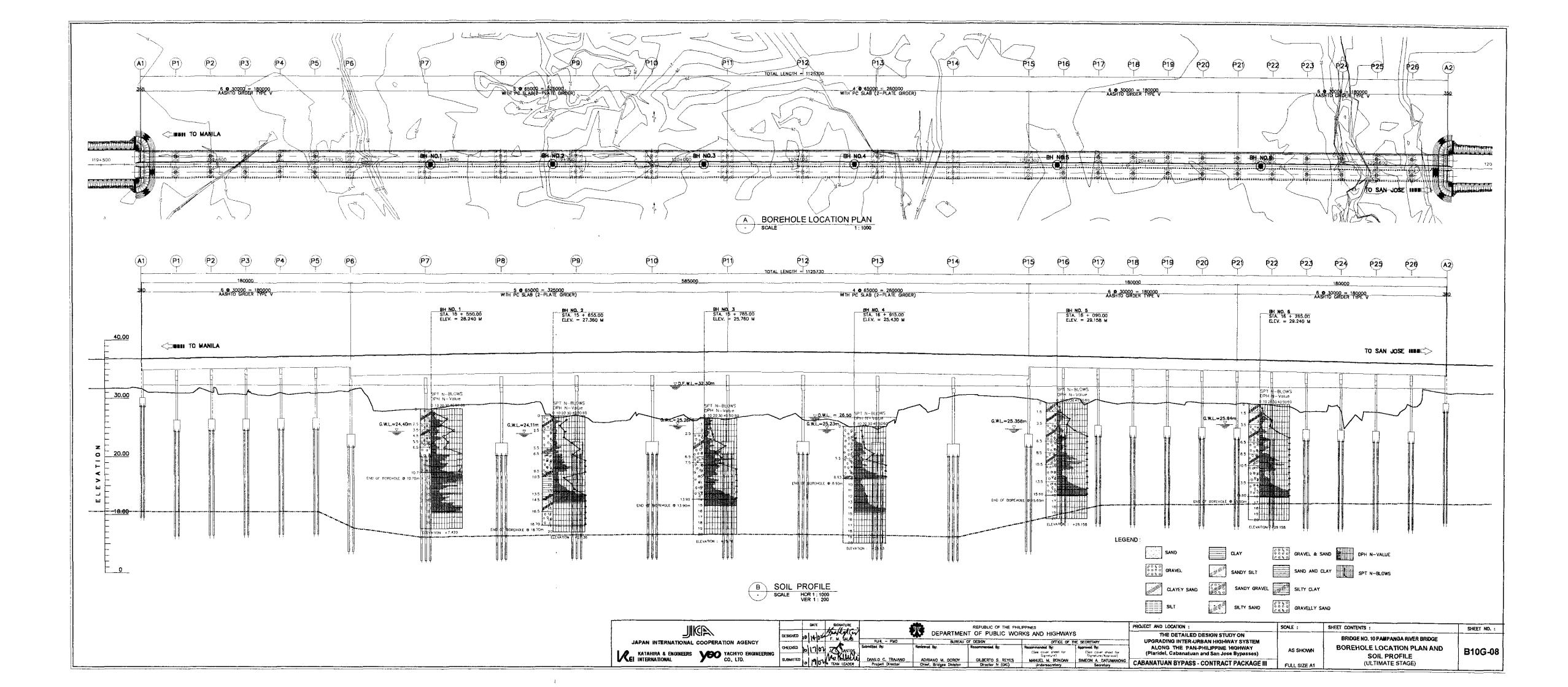


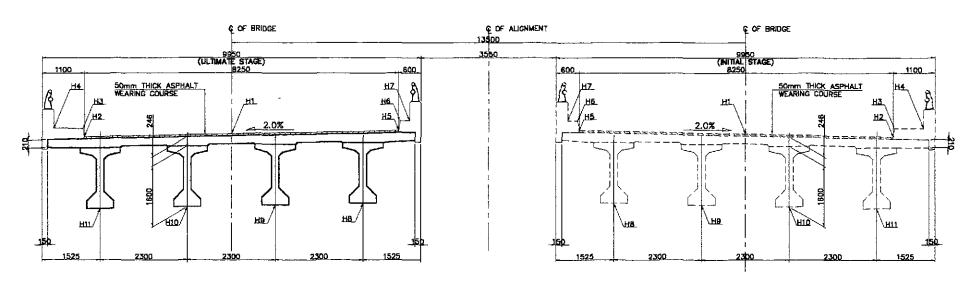








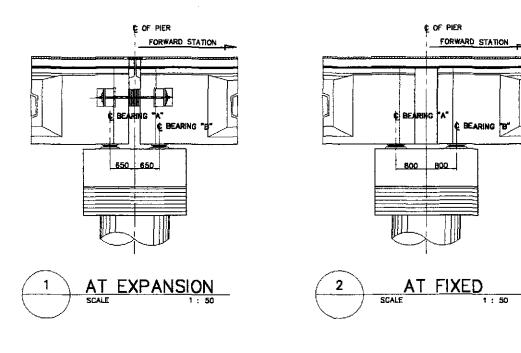




TYPICAL SECTION @ APPROACH BRIDGE 1:50

TABLE OF ELEVATIONS											
STATION	Ht	H2	нз	H4	H5	H6	H7				
119+540.000	36.787	36.709	36.959	36.894	36.874	37.124	37.180				
119+560.000	36.847	36.769	37.019	36.954	36.934	37.184	37.240				
119+580.000	36.907	36.829	37,079	37.014	36.994	37.244	37.300				
119+600.000	36.967	36.889	37.139	37.074	37.054	37,304	37.360				
119+620.000	37.027	36.949	37.199	37.134	37.114	37.364	37.420				
119+640,000	37.087	37.009	37.259	37.194	37.174	37.424	37.480				
119+660.000	37.147	37.069	37.319	37.254	37.234	37.484	37.540				
119+68D.000	37.207	37,129	37.379	37.314	37.294	37.544	37.600				
119+700.000	37.267	37.189	37.439	37.374	37.354	37.604	37.660				
120+300.000	37.505	37.427	37.677	37.612	37.592	37.842	37.898				
120+320,000	37.445	37.367	37.617	37.552	37.532	37.782	37,838				
120+340.000	37.385	37,307	37,557	37.492	37.472	37.722	37.778				
120+380,000	37.325	37.247	37,497	37.432	37,412	37.662	37.718				
120+380.000	37.265	37.187	37.437	37.372	37.352	37.602	37.658				
120+400,000	37.205	37.127	37.377	37.312	37.292	37.542	37.598				
120+420,000	37.145	37.067	37.317	37.252	37.232	37,482	37.538				
120+440.000	37.0B5	37,007	37,257	37.192	37,172	37.422	37.478				
120+460.000	37.025	35.947	37.197	37.132	37.112	37.362	37.418				
120+480.000	36.965	36.887	37.137	37.072	37.052	37.302	37.358				
120+500.000	39.905	39.827	40.077	40.012	39.992	40.242	40,298				
120+520.000	36.845	36.767	37.017	36.952	36.932	37,182	37.238				
120+540.000	36.785	36.707	36,957	36.892	36.872	37.122	37.178				
120+560.000	36.725	36,647	36.897	36.832	35.812	37.062	37.118				
120+580.000	36.665	36.587	36.837	36.772	36,752	37.002	37.05B				
120+600.000	36.605	36.527	36.777	36.712	36,692	36,942	36.998				
120+620.000	36.545	36.467	36.717	36.652	36.632	36,882	36.938				
120+640.000	36.485	36.407	36.657	35.592	36.572	36.822	36.878				

LOCATION	BEARING SIDE	STATION	H8	H9	H10	H11
ABUT. A1	A	119+534.698	34.945	34.899	34.853	34.807
PIER 1	В	119+563.778	35.032	34.986	34.94D	34.894
, , ,	A	119+565,278	35.036	34.990	34.944	34.886
PIER 2	В	119+593,778	35.122	35.076	35.030	34.984
I ICIN E	A	119+595.278	35.212	35.166	35.120	35.07
PIER 3	B	119+623,778	35.214	35.168	35.122	35.070
	A	119+625.278	35.216	35.170	35.124	35.07
PIER 4	Ð	119+653.778	35.302	35,256	35.210	35.16
, ILI 4	A	119+655,278	35.306	35.260	35.214	35.16
PIER 5	В	119+683.778	35.392	35.346	35.300	35.25
TIES E	A	119+685.278	35.396	35.350	35.304	35.25
PIER 6	8					
PIER 15	A					
DICD 46	В	120+328.760	35.591	35.545	35.499	35.45
PIER 16	A	120+330.260	35.587	35.541	35.495	35.44
	B	120+358.780	35.503	35.457	35.411	35.36
PIER 17	Α	120+360.280	35.497	35.451	35.405	35.35
0.00	В	120+388.780	35.411	35.365	35.319	35.27
PIER 18	A	120+390.280	35.407	35.361	35.315	35.26
DIE 46	В	120+418.780	35,321	35.275	35.229	35,18
PIER 19	A	120+420,280	35.317	35.271	35.225	35.17
	8	120+448.780	35.231	35.185	35.139	35.09
PIER 20	A	120+450.280	35.227	35.181	35.135	35.08
54FD 04	В	120+478.805	35.141	35.095	35.049	35.00
PIER 21	A	120+480.155	35.137	35.091	35.045	34.99
	8	120+508.780	35.051	35.DO5	34.959	34.91
PIER 22	A	120+510.280	35,047	35.001	34.955	34.90
	В	120+538.780	34.961	34.915	34.869	34.82
PIER 23	٨	120+540.280	34.957	34.911	34.865	34.81
DIES 64	В	120+568.780	34.871	34.825	34.779	34.73
PIER 24	A	120+570.280	34.867	34.821	34.775	34.72
	В					
PIER 25	A					
	В	120+628.780	34.691	34.645	34.599	34.55
PIER 26	A	120+630,280	34.687	34,641	34,595	34.54



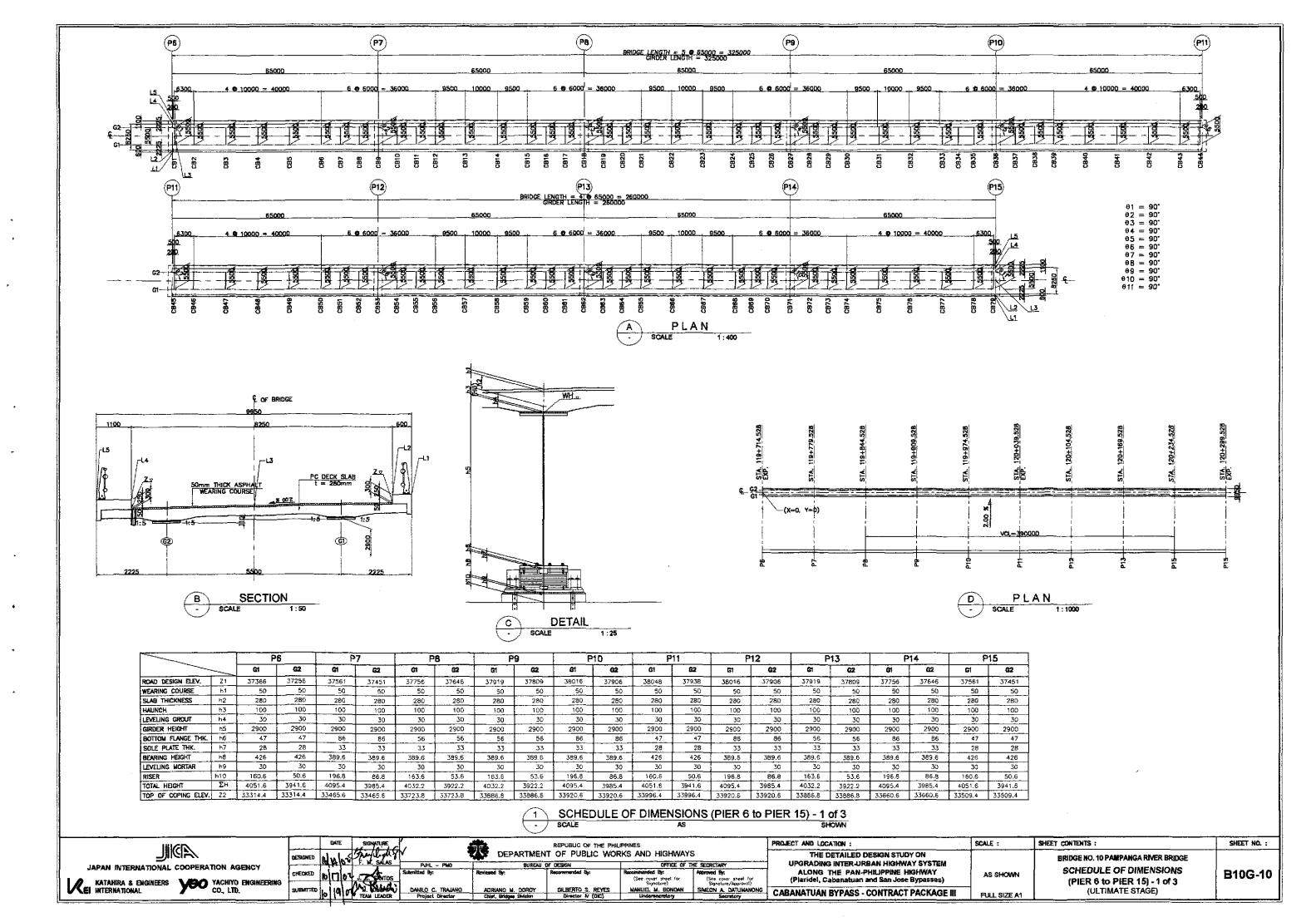
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JAPAN INTERNATIONAL	COOPERATION AGENCY
KATAHIRA & ENGINEERS INTERNATIONAL	YACHIYO ENGINEE

		DATE	Ξ	SIGNATURE
	DESIGNED	K/H/	O.F	F. M. BALAS
_	CHECKED	٦١١٥	ิงเ	J. C. SANTOS
3	SURMITTED	6 9	ี่ดเ	MI HOLHCA

		DATE	E	SIGNATURE			REPUBLIC OF THE PHILL	PPINES	
EZIGNED	H	u.	lo K	grally M.C.	•	DEPARTMENT	T OF PUBLIC WOR	KS AND HIGHWAYS	3
	100	12	-	F. M. BALAS	PUHL - PMC	BUREAU C	OF DESIGN	OFFICE OF TH	E SECRETARY
HECKED	ы		la.	~6	Submitted By:	Reviewed By:	Recommended By:	Recommended By:	Approved By:
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JENNTTED	k	a.	m	(MI- REHOW	DANILO C. TRAJANO	ADRIANO M. DORCY	GLBERTO S. REYES	Signature) MANUEL M. BONDAN	Signature/Approval) SIMEON A DATUMANON
	P	<u> 114</u>	U	TEAM LEADER	Project Director	Chief, Bridges Division	Oirector IV (OIC)	Linderencratory	Secretary
		+ +	4						

ROJECT AND LOCATION :	SCALE :	SHEET CONT
THE DETAILED DESIGN STUDY ON UPGRADING INTER-URBAN HIGHWAY SYSTEM ALONG THE PAN-PHILIPPINE HIGHWAY (Plaridel, Cabanatuan and San Jose Bypasses)	AS SHOWN	BRID
ABANATUAN BYPASS - CONTRACT PACKAGE III	FIRE SIZE A1	-

SHEET CONTENTS :	SHEET NO. :
BRIDGE NO. 10 PAMPANGA RIVER BRIDGE	
TABLE OF ELEVATIONS	B10G-09
(APPROACH BRIDGE)	D 10G-03
(ULTIMATE STAGE)	



		P6	CB1	CB2	CB3	C84	CB5	CB6	C87	CB8	CB9(P7)	CB10	CB11	CB12	CB13	CB14	C815	C816	C817	C818(P8)	C819	CB20	CB21	CB22	CE23	CB24	CB25	CB26
	Х	0,00D	0.600	7.000	17.000	27.000	37.000	47.000	53,000	59.000	65.000	71.000	77.000	83.000	92.500	102.500	112.000	118.000	124.000	130,000	136.000	142.000	148.000	157.500	167.500	177.000	183,000	189.000
LI LI	Y	-4.975	-4.975	-4,975	~4.975	-4.975	-4.975	-4.975	-4.975	-4.975	-4.975	-4.975	-4.975	-4.975	-4.975	-4.975	-4.975	-4.975	-4.975	~4.975	-4.975	-4.975	-4.975	-4.975	-4.975	~4.975	-4.975	-4.975
L	Z	37.410	37.412	37.431	37.461	37.491	37.521	37.551	37.569	37.587	37.605	37.623	37.641	37.659	37.688	37.718	37.746	37.764	37.782	37.800	37.618	37. 83 5	37.852	37.877	37.902	37.924	37.937	37.950
	×	0.000	0.600	7.000	17.000	27,000	37.000	47.000	53.000	59.000	65.000	71.000	77.000	83.000	92.500	102.500	112.000	118.000	124.000	130,000	136.000	142.000	148.000	157,500	167.500	177.000	183.000	189.000
12	Y	-4.375	-4,375	-4,375	-4.375	-4.375	-4.375	4,375	-4.375	-4.375	-4.375	-4.375	-4.375_	-4.375	-4.375	-4.375	-4.375	-4.375	~4.375	-4.375	-4.375	-4.375	-4.375	-4.375	-4.375	-4.375	-4.375	-4.375
	Z	37.398	37.400	37.419	37.449	37,479	37.509	37,539	37.557	37.575	37.593	37,611	37.52 9	37.647	37.676	37.706	37.734	37.752	37.770	37.788	37.806	37.823	37.840	37.865	37.690	37.912	37.925	37.938
ļ	×	0.000	0.600	7.000	17.000	27.000	37.000	47.000	53.000	59.000	65.000	71.000	77.000	83.000	92.500	102.500	112,000	118.000	124.000	130,000	136.000	142.000	148.000	157.500	167.500	177.000	183,000	189.000
	Y	-2.750	-2.750	-2,750	-2.750	-2.750	-2.750	-2.750	-2.750	~2.750	-2.750	~2.750	-2.750	~2.750	-2.750	-2.750	-2.750	-2.750	-2.750	~2.750	-2.750	-2.750	-2.750	-2.750	-2.750	-2.750	-2.750	-2.750
G	Z	37.366	37,368	37.387	37.417	37.447	37,477	37.507	37.525	37.543	37.561	37.579	37.597	37.615	37.644	37.674	37.702	37.720	37.738	37.756	37.774	37. 7 91	37.808	37.833	37.858	37.880	37.893	37.906
1	HU	0.130	0.130	0.130	0.130	0.130	0.130	0.130	0.130	0.130	0.130	0.130	0.130	0.130	0.130	0.130	0.130	0.130	0.130	0.130	0.130	0.130	0.130	0.130	0.130	0.130	0.130	0.130
	WH	36.906	36,908	36.927	36,957	35.987	37.017	37,047	37.065	37.083	37.101	37.119	37.137	37.155	37.184	37.214	37.242	37.260	37.278	37.296	37.314	37.33t	37.348	37.373	37,398	37.420	37.433	37.446
	X	0.000	0.600	7.000	17.000	27.000	37.000	47,000	53.000	59.000	65.000	71.000	77.000	83.000	92.500	102.500	112.000	118.000	124.000	130.000	136.000	142.000	148,000	157.500	167.500	177.000	183.000	189.000
L3	Y	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
	Z	37.311	37.313	37.332	37.362	37.392	37.422	37.452	37.470	37.488	37.506	37.524	37.542	37.560	37.589	37,619	37.647	37.665	37.683	37,701	37.719	37.736	37.753	37.778	37.803	37.8 2 5	37.838	37.851
}	X	0.000	0.600	7.000	17.000	27.000	37.000	47.00D	53.000	59.000	65.000	71,000	77.000	83,000	92.500	102.500	112.000	118.000	124,000	130,000	136,000	142,000	148.000	157.500	167.500	177.000	183.000	189.000
	Y	2.750	2.750	2.750	2.750	2.750	2.750	2.750	2.750	2.750	2.750	2.750	2.750	2.750	2.750	2.750	2.750	2.750	2.750	2.750	2.750	2.750	2.750	2.750	2.750	2.750	2.750	2.750
62	Z	37.256	37.258	37.277	37.307	37.337	37,387	37.397	37.415	37.433	37.451	37,469	37.487	37.505	37.534	37.564	37.592	37.610	37.628	37,646	37.664	37.681	37. 6 98	37.723	37,748	37.770	37.783	37.795
	HU	0.130	0.130	0.130	0.130	0.130	0.130	0.130	0.130	0.135	0.130	0.130	0.130	0.130	0.130	0.130	0.130	0.130	0.130	0.130	0.130	0.130	0.130	0.130	0.130	0.130	0.130	0.130
	WH	36.796	36.798	36.817	36.847	36.877	36.907	36.937	36.955	36.973	36.991	37,009	37.027	37.045	37.074	37.104	37.132	37.150	37.168	37.1B6	37.204	37.221	37.238	37.263	37,288	37.310	37.323	37.336
	×	0.000	0.600	7.000	17.000	27.000	37,000	47.000	53,000	59.000	65.000	71.000	77.000	83.000	92.500	102.500	112.000	118.000	124.000	130.000	136.000	142-000	148.000	157.500	167.500	177.000	183.000	189.000
L4	Y	3.875	3.875	3.675	3.875	3.875	3.875	3.875	3.875	3.875	3.875	3.875	3.875	3.875	3.875	3.875	3.875	3.875	3.875	3.875	3.875	3.875	3.875	3.875	3.875	3.875	3.875	3.875
	Z	37.233	37.235	37.254	37.284	37.314	37,344	37.374	37.392	37.410	37.428	37.446	37.464	37.482	37.511	37.541	37.569	37.587	37.605	37.623	37.641	37.658	37.675	37.700	37.725	37.747	37.760	37.773
	X	0.000	0.600	7.000	17.000	27.000	37.000	47.000	53.000	59.000	65.000	71.000	77.000	83,000	92.500	102.500	112.000	118.000	124,000	130.000	136.000	142.000	148.000	157.500	167.500	177.000	183,000	189.00C
1.5	Y	4.975	4.975	4.975	4.975	4.975	4.975	4.975	4.975	4.975	4.975	4.975	4.975	4.975	4.975	4.975	4.975	4.975	4.975	4.975	4.975	4,975	4.975	4.975	4.975	4.975	4.975	4.975
<u> </u>] Z	37.211	37.213	37.232	37.262	37.292	37.322	37.352	37,370	37.388	37.406	37.424	37,442	37.460	37.489	37.519	37.547	37,565	37.583	37.601	37.619	37.636	37.653	37.678	37.703	37.725	37.738	37.751

		C927(P9)	CB28	CB:26	CB30	CB31	CB32	CB33	CB34	CB35	CB36(P10)	C937	C938	CB356	C840	CB41	CB42	C843	CB44	P11	SPAN 1	SPAN 2	SPAN 3	SPAN 4	SPAN 5	TOTAL
	x	195,000	201.000	207,000	213.000	222.500	232.500	242.0DG	248.000	254.000	260.000	256.000	272.000	278.000	288.000	298.000	308.000	318.000	324.400							
L1	Y	-4.975	-4.975	-4.975	-4.975	-4.975	-4.975	-4,975	-4.975	-4.975	-4.975	-4.975	-4.975	-4.975	-4.975	-4.975	-4.975	-4.975	-4.975		64.400	65.000	65.000	65.000	64.400	323,800
	Z	37.963	37.974	37.985	37.996	38.012	38.027	38.040	38.047	38.054	38.060	38.066	38.071	38.076	38.082	38.087	38,090	38.093	38.093		l					
	X	195.000	201.000	207.000	213.000	222.500	232.500	242.000	248.000	254.000	260.000	266.DOQ	272.000	278.000	288.000	298.000	308.000	318.000	324,400							
L2	Y	-4,375	-4.375	-4.375	-4.375	-4.375	-4.375	-4.375	-4.375	-4.375	-4.375	-4.375	-4.375	-4.375	-4.375	-4.375	-4.375	-4.375	-4.375		64.400	65,000	65.000	65.000	64.400	323.800
	Z	37.951	37.962	37.973	37.984	38.000	38.015	38.028	38.035	38.042	38,048	38.054	38.059	38.064	38.070	38.075	38.078	38.081	38,081		<u> </u>			<u>.</u> l		
i	×	195.000	201.000	207.000	213,000	222.500	232.500	242.000	248.000	254.000	260.000	266.000	272,000	278.000	288.000	298.000	308.000	318.000	324.400							
	Y	-2.750	-2.750	-2.750	-2.750	-2.750	~2.750	-2.750	-2.750	-2.750	-2.750	-2.750	-2.750	-2.750	-2.750	-2.750	-2.750	-2.750	-2.750							
G1	Z	37.919	37. 93 0	37.941	37.952	37.968	37.983	37,996	38.003	38.010	38.015	38.022	38.027	36.032	38.038	38.043	38.045	38.049	38.049		64,400	65.000	65.000	65.000	64.400	323.800
Į	HU	0.130	0.130	0.130	0.130	0.130	0.130	0.130	0.130	0.130	0.130	0.130	0.130	0.130	0.130	0.130	0.130	0.130	0.130]					
	WH	37.459	37.470	37,481	37.492	37.508	37.523	37.536	37.543	37.550	37.556	37.562	37.567	37.572	37.578	37.583	37,586	37.589	37.589		<u> </u>					
	X	195.000	201.000	207.000	213.000	222.500	232.500	242.000	248.000	254.000	260.000	266.000	272.000	278.000	288.000	298.000	308.000	318.000	324.400							
ᅜ	Y	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000		64,400	65.000	65.000	65.000	64.400	323.800
	Z	37.864	37.875	37.886	37.897	37.913	37.928	37.941	37.948	37.955	37.961	37.967	37.972	37.977	37.983	37.988	37.991	37.994	37.994		<u> </u>					
	X	195,000	201.000	207,000	213.000	222.500	232.500	242.000	248.000	254.000	260.000	266.000	272.000	278.000	288.000	298.000	308.000	318.000	324.400	_						
	Y	2.750	2.750	2.750	2.750	2.750	2.750	2.750	2.750	2,750	2.750	2.750	2.750	2.750	2.750	2.750	2,750	2.750	2,750	_	ĺ					
G2	Z	37.809	37.820	37,831	37.842	37.858	37.873	37.686	37.893	37.900	37.906	37.912	37.917	37.922	37.928	37.933	37,936	37.939	37.939		54,400	65.000	65.000	65.000	64.400	323.800
i	HU	0.130	0.130	0.130	0.130	0.130	0.130	0.130	0.130	0.130	0.130	0.130	0.130	0.130	0.130	0.130	0.130	0.130	0.130		i i		i	' i		l
	WH -	37.349	37.360	37.371	37.382	37.398	37.413	37.426	37.433	37. 44 0	37.446	37.452	37.457	37.462	37.468	37.473	37.476	37.479	37,479							
	×	195.000	201.000	207.000	213.000	222.500	232.500	242.000	248.000	254.000	260.000	266.000	272.000	278.000	288.000	296.000	308,000	318.000	324.400		·					
L4	Y	3.875	3.875	3.875	3.875	3.875	3.875	3.875	3.875	3.875	3.875	3.875	3.875_	3.875	3.B75	3,875	3.875	3.875	3.875		64.400	65.000	65.000	65.000	64.400	323.800
	Z	37.786	37.797	37.808	37.819	37.635	37.850	37.863	37.870	37.877	37.883	37.889	37.894	37.899	37.905	37.910	37,913	37.916	37.916							
	X	195,000	201.000	207.000	213.000	222,500	232.500	242.000	248.000	254.000	260.000	266.000	272.000	278.000	288.000	298.000	308.000	318.000	324.400			į				
L5	Y	4,975	4.975	4.975	4.975	4.975	4.975	4.975	4.975	4.975	4.975	4.975	4.975	4.975	4.975	4.975	4.975	4.975	4.975		64.400	85.000	65.000	65.000	64.400	323.800
	Z	37.764	37.775	37. 78 6	37.797	37,813	37.528	37.841	37.848	37.855	37.861	37.867	37.872	37.877	37.BB3	37.688	37.891	37.894	37.894				ĺ	i	<u> </u>	l

SCHEDULE OF DIMENSIONS (PIER 6 to PIER 15) - 2 of 3
- SCALE AS SHOWN

JAPAN INTERNATIONAL COOPERATION AGENCY

KATAHIRA & ENGINEERS

YEO YACHIYO ENGINEERING
CO., LTD.

DESIGNED TO 14 BY F. W. SMAS

CHECKED TO 170 2 J. E. SMTOS

SUBMITTED TO 19 DY TEAM LEADER

REPUBLIC OF THE PHILIPPINES

DEPARTMENT OF PUBLIC WORKS AND HIGHWAYS

ALAS

PUBLIC PMO

BUREAU OF DESIGN

OFFICE OF THE SECRETARY

Recommended by:

Recommended by:

Recommended by:

Recommended by:

Recommended by:

Recommended by:

Suprolited By:

Supro

PROJECT AND LOCATION: SCALE:

THE DETAILED DESIGN STUDY ON

UPGRADING INTER-URBAN HIGHWAY SYSTEM

ALONG THE PAN-PHILIPPINE HIGHWAY

(Plaridel, Cabanatuan and San Jose Bypasses)

CABANATUAN BYPASS - CONTRACT PACKAGE (III FULL SIZE A1)

SHEET CONTENTS:

BRIDGE NO. 10 PAMPANGA RIVER BRIDGE

SCHEDULE OF DIMENSIONS

(PIER 6 to PIER 15) - 2 of 2

(ULTIMATE STAGE)

B10G-11

SHEET NO. :

		Pt1	CB45	CB46	CB47	CB48	CB49	CB50	CB51	CB52	CB53(P12)	CB54	CB55	CB56	CB57	CB58	C959	CB50	CB61	CS82(P13)	CB63	CB64
	X		325,602	332.002	342.002	352.002	362,002	372.002	378.002	384.002	390.002	396.002	402.002	408.002	417.502	427,502	437,002	443.002	449.002	455.002	461.002	467.002
Ħ	Y		-4.975	-4.975	-4.975	-4.975	-4.975	-4.975	~4.975	-4.975	-4,975	-4.975	~4.975	-4.975	~4.975	-4.975	~4.975	-4.975	-4.975	-4.975	-4.975	-4.975
1	Z		38.093	38.092	38.090	38.087	38.082	38.076	38.071	38.066	38.060	38.054	38.047	38.040	JB.027	3B.012	37.996	37.985	37.974	37.963	37.950	37.937
	X		325.502	332.002	342.002	352.002	362.002	372.002	378.002	384.002	390.002	396.002	402.00	408.002	417.502	427.502	437,002	443.002	449.002	455.002	461.002	467.002
1.2	Y		-4.375	-4.375	-4.375	-4,375	-4.375	-4.375	-4.375	-4.375	-4.375	-4.375	-4.375	-4,375	-4.375	-4.375	-4.375	-4.375	-4.375	-4.375	-4.375	-4.375
	Z		38.081	38.080	38.078	36.075	38.070	38.064	38.059	35.054	38.048	38.042	38.035	38,028	38.015	38.000	37.984	37.973	37.962	37.951	37.938	37.925
	X		325.602	332.002	342.002	352.002	362.002	372.002	378.002	384.002	390.002	396.002	402.002	408.002	417.502	427.502	437.002	443.002	449.002	455.002	451.002	467.002
j	Ÿ		-2.750	-2.750	-2.750	-2.750	-2,750	-2.750	-2.750	-2.750	-2.750	-2.750	-2.750	2.750	-2.750	-2.750	-2,750	-2.750	-2.750	-2.750	-2.750	-2.750
Gf	Z		38.049	38.048	38.046	38.043	38.038	38.032	38.027	38.022	38.016	38.010	38,003	37.996	37.983	37.968	37.952	37.941	37.930	37.919	37,906	37.893
	HU		0.130	0.130	0.130	0.130	0.130	0.130	0.130	0.130	0.130	0,130	0,130	0.130	0.130	0.130	0.130	0.130	0.130	0.130	0.130	0.130
L	WH		37.589	37.588	37.586	37.583	37.578	37.572	37.567	37,562	37.556	37.550	37.545	37.536	37.523	37,508	37.492	37.481	37.470	37.459	37.446	37.433
	X		325,502	332.002	342.002	352.002	362.002	372.002	378.002	384.002	390.002	396.002	402.002	408.002	417.502	427.502	437.002	443.002	449.002	455.002	461.002	467.002
L3	Y		5,000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
_	7	_	37.994	37.993	37.991	37.98B	37.983	37.977	37.972	37.967	37.961	37.955	37.948	37,941	37,928	37.913	37.897	37.886	37.875	37.884	37.851	37.838
	Х		325,602	332.002	342,002	352.002	362.002	372.002	378,002	384.002	390.002	396.002	402.002	408.D02	417.502	427,502	437.002	443.002	449.002	455.002	461.002	467.002
	Υ		2.750	2.750	2.750	2.750	2.750	2.750	2.750	2.750	2.750	2.750	2.750	2.750	2.750	2.750	2.750	2.750	2.750	2.750	2.750	2.750
E2	Z		37,939	37.938	37.936	37.933	37.928	37.922	37.917	37.912	37.906	37.900	37.893	37.886	37.873	37.858	37.842	37.831	37.820	37.809	37.796	37.783
	HU		D.130	0.130	0.130	0.130	0.130	0.130	0.130	0.130	D. 130	0.130	0.150	0.130	0.130	0.130	0.130	0.130	0.130	0.130	0.130	0.130
	WH		37.479	37.478	37.476	37.473	37.468	37,462	37.457	37.452	37.446	37,440	37.435	37.426	37.413	37.398	37.382	37.371	37.360	37.349	37.336	37.323
	×		325.602	332.002	342.002	352.002	362,002	372.002	378.002	384.002	390.002	396.002	402.002	408.002	417.502	427,502	437,002	443.002	449.002	455.002	461.002	467.002
L4	Υ	·	3,875	3.875	3.875	3.875	3.875	3.875	3.875	3.875	3.875	3.875	3.875	3.875	3.875	3,875	3.875	3.875	3,875	3.875	3.875	3.875
	Z		37.916	37.915	37.913	37,910	37.905	37.899	37.B94	37.889	37.883	37.877	37.870	37.863	37.850	37.B35	37.819	37.808	37.797	37.786	37.773	37.760
	X		325.602	332.002	342.002	352.002	362.002	372.002	378.002	384.002	390.002	396.002	402,002	408.D02	417.502	427.502	437.002	443.002	449.002	455.002	461.002	467.002
LS	Y		4,975	4.975	4.975	4.975	4.975	4.975	4.975	4.975	4.975	4.975	4.975	4.975	4.975	4.975	4.975	4.975	4.975	4.975	4.975	4.975
	Z		37.894	37.893	37.891	37.888	37.883	37.877	37.872	37.867	37.861	37.855	37.848	37.841	37.828	37.813	37.797	37.786	37.775	37.764	37.751	37.738

		CB85	C956	C967	C868	CBBB	C870	CB71(P14)	C972	C873	CB74	CB75	C976	C877	C978	C979	P15	SPAN 1	SPAN 2	SPAN 3	SPAN 4	TOTAL
	X.	473.002	482.502	492.502	502.002	508.002	514.002	520,002	526.D02	532.002	538,002	548.002	558.002	568.002	578.002	584,402	585.002					
L1	Y	-4,975	-4.975	-4.975	-4.975	-4.975	-4.975	-4.975	-4.B75	-4.975	-4,975	-4.975	-4.975	-4.975	-4.975	-4.975	-4.975	64.400	65.000	65.000	64.400	258.800
	Z	37.924	37.902	37.877	37.852	37.835	37.618	37.800	37.782	37.764	37.746	37.716	37.686	37.656	37.626	37.607	37.605	i	i			
	X	473.002	482.502	492.502	502.002	508.002	514.002	520.002	526.002	532.002	538,002	548.002	558.002	568.002	578.002	584.402	585.002		1			
L2	Υ	4.375	-4.375	-4.375	-4.375	-4.375	-4.375	-4.375	~4.375	-4.375	-4.375	-4.375	-4.375	-4.375	-4.375	-4.375	-4.375	54,400	55.000	65.000	64.400	258.800
	Z	37.912	37.890	37.865	37.B40	37.823	37.806	37.788	37.770	37,752	37.734	37.704	37.674	37.644	37.614	37.595	37.593	ì	1 .			
	X	473.002	482.502	492.502	502.002	508.002	514.002	520.002	526.002	532.002	538,002	548.002	558.002	568.002	578.002	584.402	585.002					
	Y	-2.750	-2.750	-2.750	-2.750	-2.750	-2.750	-2.750	-2.750	-2.750	-2,750	-2.750	-2.750	-2.750	-2.750	-2,750	-2.750					
Gt	Z	37.880	37.858	37.833	37.808	37.791	37.774	37.756	37.738	37.720	37.702	37.672	37.642	37.612	37.582	37.563	37.561	64.400	65.000	65.000	64.400	258.800
	HU	0.130	0.130	0.130	0.130	0.130	0.130	0.130	0.130	0.130	0.130	0.130	0.130	0.130	0.130	0.130	0.130					
	WH	37.420	37.398	37.373	37.348	37.331	37.314	37.296	37.278	37.260	37.242	37.212	37.182	37.152	37.122	37,103	37.101					
	Х	473.002	482.502	492.502	502.002	508.002	514.002	520.002	526.002	532.002	538.002	548.002	558.002	568.002	578.002	584.402	565.0D2					
L3	Y	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	64.400	65.000	65.000	54.400	258.800
	Z	37.825	37.803	37.778	37.753	37.736	37.719	37.701	37.683	37.665	37.647	37.617	37.587	37.557	37.527	37.508	37.506	•				
	X	473.002	482,502	492.502	502.002	508.002	514.002	520.002	526.002	532.002	538,002	548.002	558.002	568.002	\$78.002	584,402	585.002				_	
	Y	2.750	2.750	2,750	2.750	2.750	2.750	2.750	2.750	2.750	2.750	2.750	2.750	2.750	2.750	2,750	2.750	i	{	ĺ		
G2	Z	37.770	37.748	37.723	37.698	37.681	37.664	37.646	37.628	37.610	37.592	37.562	37.532	37.502	37.472	37.453	37,451	64,400	65.000	65.000	64.400	258.800
	HU	۵.130	0.130	D. 130	0,130	0.130	0.130	0.130	0.130	0,130	0.130	0.130	0.130	0.130	0.130	0.130	0.130	}	1	,	i	
	₩H	37.310	37.288	37.263	37.238	37.221	37.204	37.186	37.16B	37.150	37.132	37.102	37.072	37.042	37.012	36.693	36.991					
	X	473.002	482.502	492.502	502.002	508.002	514.002	520.002	526.002	532.002	538.002	548.002	558.002	568.002	578.002	584,402	585.002					
L4	Υ	3.875	3.875	3.875	3.875	3.875	3.875	3.875	3.875	3.875	3.875	3.875	3.875	3.875	3.875	3,875	3,875	64.400	65.000	65,000	64.400	258.800
	Z	37.747	37.725	37.700	37.675	37.658	37.641	37.623	37.605	37.587	37.569	37.539	37.509	37,479	37.449	37.430	37.428					
	Х	473.002	482:502	492.502	502.002	508.002	514.002	520.002	526.002	532.002	538.002	548.002	558.002	568.002	578.002	584,402	585.002			· ·		
LS	Υ	4.975	4.975	4.975	4.975	4.975	4,975	4.975	4.975	4.975	4.975	4.975	4.975	4.975	4.975	4.975	4.975	64.400	65.000	65.000	64.400	258.800
	Z	37.725	37.703	37.678	37.653	37.636	37.619	37.601	37.583	37.565	37.547	37.517	37.487	37.457	37.427	37.408	37.406					

SCHEDULE OF DIMENSIONS (PIER 6 to PIER 15) - 3 of 3
- scale as shown

JAPAN INTERNATIONAL COOPERATION AGENCY

KATAHRA & ENGINEERS

YACHIYO ENGINEERING
CO., LTD.

DESIGNED TO HOL F. W. SALAS

CHECKED DIT OU J. E SANTOS SI
SUBMITTED DI 19 00 TEAN LEADER

6150		DEPARTMEN	REPUBLIC OF THE PHIL T OF PUBLIC WOR		3
. KAK	PJHL - PMO	BUREAU	OF DESIGN	OFFICE OF T	HE SECRETARY
-ZP	Submitted By:	Reviewed By:	Recommended By:	Recommended By:	Approved By:
SANTOS				(See cover sheet for Signature)	(See cover sheet for Signature/Approval)
MAN MAN	DANILO C. TRAJANO	ADRIANO M. DOROY	GILBERTO S. REYES	MANUEL M. SONOAN_	SMECH A DATUMAN
LEADER	Project Director	Chief, Bridges Division	Olrector_IV (OIC)	Undersecretary	Secretory
·					

PROJECT AND LOCATION :	SCALE :
THE DETAILED DESIGN STUDY ON UPGRADING INTER-URBAN HIGHWAY SYSTEM ALONG THE PAN-PHILIPPINE HIGHWAY (Plaridel, Cabanatuan and San Jose Bypasses)	AS SHOWN
CABANATUAN BYPASS - CONTRACT PACKAGE III	FULL SIZE A1

SHEET CONTENTS:

BRIDGE NO. 10 PAMPANGA RIVER BREDGE
SCHEDULE OF DIMENSIONS
(PIER 6 to PIER 15) 3 of 3
(ULTIMATE STAGE)

B10G-12

SUMMARY OF QUANTITIES PAMPANGA RIVER BRIDGE CROSSING (BRIDGE NO. 10)

ITEM NO.	DESCRIPTION	UNIT	QU	ANTITIE	s	
ITEM NO.			APPROACH	MAIN	TOTAL.	REMARKS
PART F	BRIDGE CONSTRUCTION					
	SUPERSTRUCTURE					
310(2)	Asphalt Mixture Weering Course (t=50mm) Incl. Tack Coot	m ²	4,455.00	4,826.25	9,281.25	
401(2)a	Steel Railing Type A for (Angat, Talavera and approach of Pampanga Bridge)	m	1,080.00		1,080.00	
401(2)b SPL 401(3)b	Steel Railing Type B (far Pampanga Main Bridge) Bridge Name Plate, 1000 x 600 for Pampanga Bridge	m ea.	- 2	1,170.00	1,170.00	
403(3)	Structural Steel, furnished and fabricated	kq.		1,707,861.88	1,707,861.88	
403(5)	Structural Steel, erected	kg.		1,707,861.88	1,707,861.88	
403(8)a	Bearing Shee for Steel Plate Girder Type 1 (max. R=250t)	60,	-	8	В	
403(8)b	Bearing Shae for Steel Plate Girder Type 2 (max. R=650t)	eq.	-	8	8	
403(8)c 404(1)	Bearing Shoe for Steel Plate Girder Type 3 (max. R=650t) Reinforcing Steel (Grade 40)	ea. kg.	308,907.64	37,003.00	\$ 345,910.64	
404(2)	Reinforcing Steel (Grade 60)		114,227.82	17,003.00	114,227.82	
405(1)f	Structural Concrete Class AA2 (fc=28 Mpa, max. aggregate 20mm) for Long Bridge Superstructures	kg, m ³	1,535.15	B.20	1,543.35	
405(3)	Structural Concrete Class C (fc=21 MPa, max. aggregate 12mm) for Thin Reinforced Members	m ³	473.04	349.25	622.29	
406(1)g	Precost Prestreamed Structural Concrete Member (AASHTO Girder Type V, L=29.4m)	eq.	48		48	
406(1)h	Precest Prestressed Structural Concrete Member (AASHTO Girder Type V, L=29.55m)	ed.	24	564773	5,643.J2	
406(1)p 407(1)b	Precest Prestressed Structural Concrete (PC Deck slab, 280 x 2000 x 9650) Elastomeric Bearing Pad (600x300x50mm)	ea,	136	5,643.32	5,643.32	
407(2)b	Expansion Joint, Multiflex M100 (Elastemeric) ±50mm movement	m	20.00		20.00	
407(2)¢	Expansion Joint, Multiflex M140 (Eigstomeric) ±70mm movement	m	10.00		10.0D	
407(2)f	Expansion Joint Multiflex M330 (Ekostomeric) ±165mm movement	m	-	30.00	30.00	
407(4)	G.i Drain Pipe Ø150mm for Bridge Drainage	m2	191.25	361.35	552.60	
SPL 310(3) SPL 407(3)a	Woterproofing Layer for Pampanga Dack Sidb Restraining Bar #32mm x 1485mm		- 12	4,826.25	4,826.25	
SPL 407(3)6	Restraining Bar Ø32mm x 1900mm	#0.	6		6	
SPL 407(3)c	Restraining Cable 265mm x 4121mm (PC 7- 215)	set	-	8	8	
SPL 407(3)d	Restraining Cable Ø55mm x 4224mm (PC 7- Ø15)	eet	-	4	4	
H	SUBSTRUCTURE					
103(2)a	Bridge Excovation above OWL (Common Soll)	m ³	3,864.42	1,658.29	5,522.71	
103(2)a 104(4)	Bridge Excavation below OWL (Cammon Soli) Embankment from Borrow (Selected Granular Material) for Bridge	m ³ .	2,334.40 597.34	7,511.5B	9,845,98 597.34	
200(1)	Aggregate Subbase Course	m ³	25.76		25.76	
400(15)a	Cast-in-place Concrete Bored Files Ø 1000mm	m	320.00		320.00	
400(16)b	Cast-in-place Concrete Bored Piles Ø 1200mm	m	1,626.00		1,626.00	
400(16)c	Cost-In-place Concrete Bored Plies & 1500mm	m	-	1,512.00	1,512.00	
400(21) 404(1)	Static Pile Load Test for Ø1500mm Bored Piles Reinforcing Steel (Grade 40)	ea. kg.	37,907.77	12,890.48	50,798,25	
404(2)	Reinforcing Steel (Grade 60)	kg.	596,072.81	667,774.14	1,263,846.95	
405(1)e	Structural Concrete Class AA1 (fc=28 Mpa, max. aggregate 20mm) for Long Bridge Substructures	m ³	2,899.21	3,741.56	6,640.77	
405(6)	Lean Concrete (fic=17 MPa max. aggregate 38mm)	m ³	96.56	105.47	203.03	
SPL 311(2)	PCC Powement (Reinforced) t=300mm Approach Slab	m ²	91.04		91.04	
SPL 400(23)a SPL 400(23)b	High Strain Dynamic Pile Test for 61000mm Bored Piles	ea.	2		1	
SPL 400(24)	High Strain Dynamic Plie Test for Ø1200mm Bored Piles Pile Integrity Test for Bored Piles of Various Diameter	eq	66		<u>2</u> 56	
SPL 900(3)	Provisional Sum for Geotechnical Investigation	1.8.				
B1	REVETMENT (RIVERBANK PROTECTION)					
101(7)	Removal of Existing Slope Protection	m3	97.27		97.27	
101(8)	Removal of Existing Slope Protection (Hand-Loid Rock)	m ³	42.65		42.65 189.00	
101(9)	Removal of Existing Gabian Structure Excavation		189.00 787.50		787.50	
104(3)	Embankment from borrow pit	m3	508.62		508.62	
405(1)a	Structural Concrete Class A (fc=21 Mpa, max. aggregate 38mm) for Heavily Reinf. Structures	m ³	42.88		42.88	
405(2)	Structural Concrete Class B (f'c=17 MPa, max. aggregate 50mm) for Plain or Lightly Reinf. Structures	m ³	12.00		12.00	
504(5)	Grouted Riprop Closs A	m ₃	14,16		14.16	
506(1) 507(2)b	Hand Loid Rock Apron (Loose Boulder Apron) Steel Shest Piles (400mmx85mm), furnish & driven	m m	30.15 1,296.00		30.15 1,296.00	
509(1)	Gobiona, (2.0 x 1.0 x 0.50)	m ³	301.50		301.50	
510(1)	Rubble Concrete Slope Protection	m ³	171.63		171.63	
SPL 407(5)b	Pler Protection Concrete Blacks for Pampanga Bridge	m ²	-	840.00	840.00	
īv	TEMPORARY WORKS					
SPL 420(2)	Realignment of River/Stream	l.s.	1		720.00	
SPL 420(4)b SPL 420(5)b	Temporary Craneway for Pampanga Bridge Construction Temporary Access Road (Causeway) for Pampanga Bridge Construction		880.00	320.00	320.00 880.00	
5PL 420(6)c	Temporary Access Road (Couseway) for Pampanga Bridge Construction Temporary Cofferdom for Pier Construction (Pampanga Bridge)	ec.		5	5	
V	ELECTRICAL WORKS				_	
SPL 620(4)c	Bridge Lighting Poles (Single Lamp)	eq.	18	19	37	
SPL 620(4)d	Street Lighting Service Pole with Ponel	ea.		_	2	

INICID		DATE	SIGNATURE			REPUBLIC OF THE PHIL	LIPPINES		PROJECT AND LOCATION :	SCALE :	SHEET CONTENTS:	SHEET NOL:
ADML.	DESIGNED	dulor	X Shark	•		NT OF PUBLIC WO			THE DETAILED DESIGN STUDY ON UPGRADING INTER-URBAN HIGHWAY SYSTEM		BRIDGE NO. 10 PANIPANGA RIVER BRIDGE	
JAPAN INTERNATIONAL COOPERATION AGENCY	CHECKED	ومامر أما	- Autos	Publ PMO Submitted By:	Reviewed By:	OF DESIGN Recommended By:	Recommended By:	HE SECRETARY Approved By:	ALONG THE PAN-PHILIPPINE HIGHWAY	AS SHOWN	SUMMARY OF QUANTITIES	B10G-13
KATAHIRA & ENGINEERS VOO YACHIYO ENGINEERING		MIII.	/A. Kuni				(See cover sheet for Signature)	(See cover sheet for Signature/Approval)	(Plaridel, Cabanatuan and San Jose Bypasses)	70 01 01 101		D 100-10
CO., LTD.	SUBMITTED	10110	TEAM LEADER	Project Director	ADRIANO M. DOROY Chief, Bridges Division	GILBERTO S. REYES Director IV (OIC)	MANUEL M. BIONGAN Underwecretery	SIMEON A DATUMANONG Sacretory	CABANATUAN BYPASS - CONTRACT PACKAGE III	FULL SIZE A1	(ULTIMATE STAGE)	