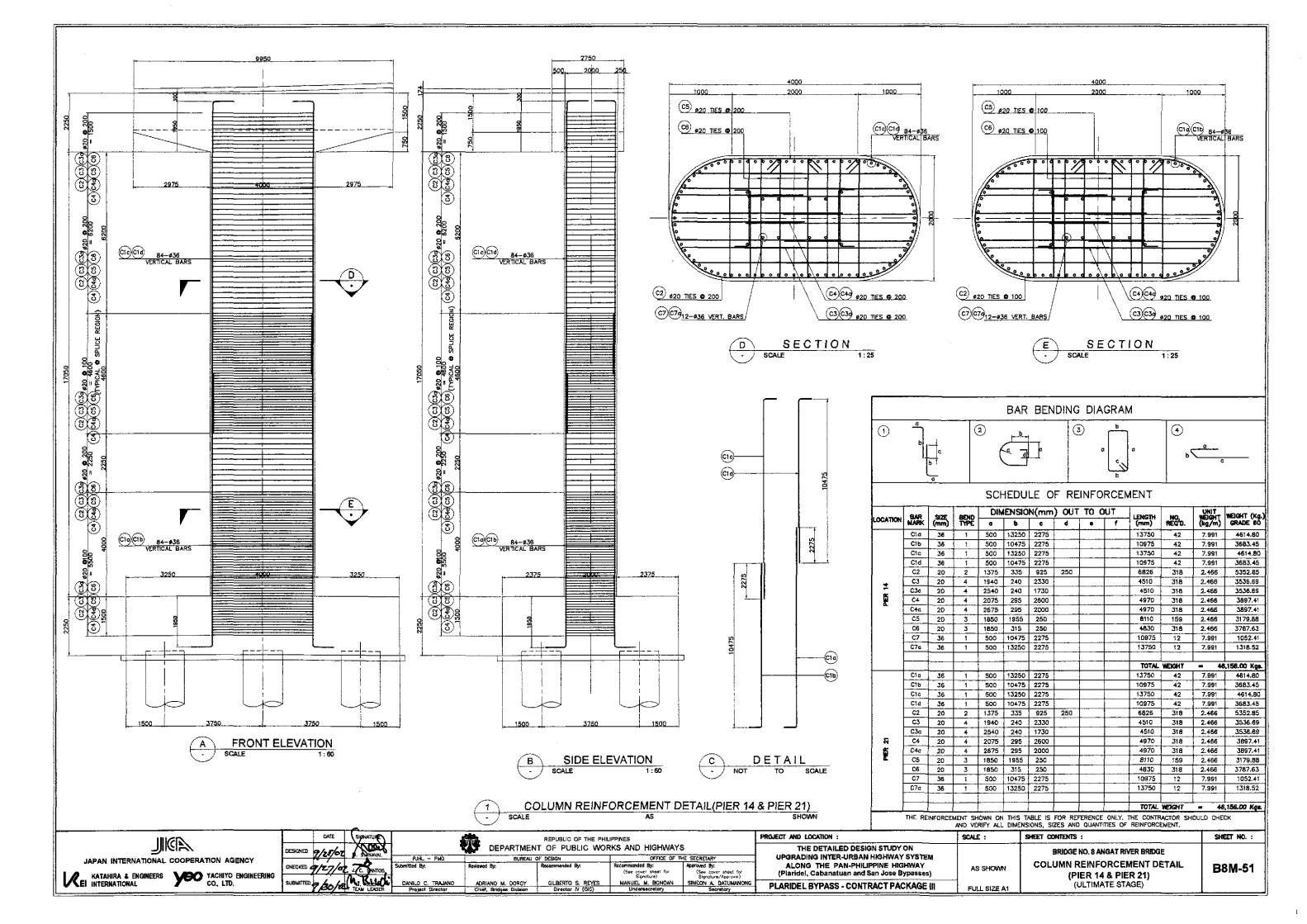
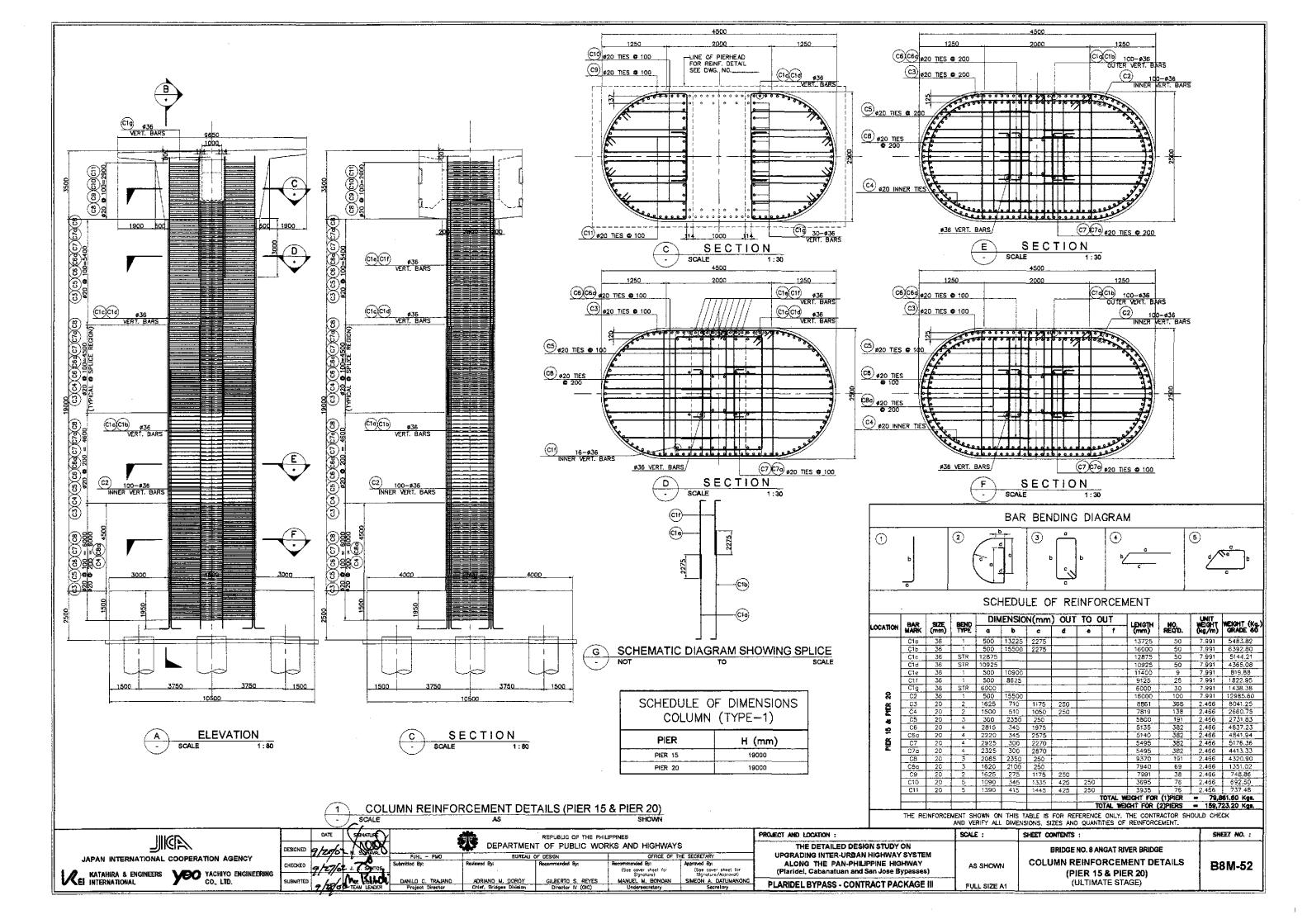
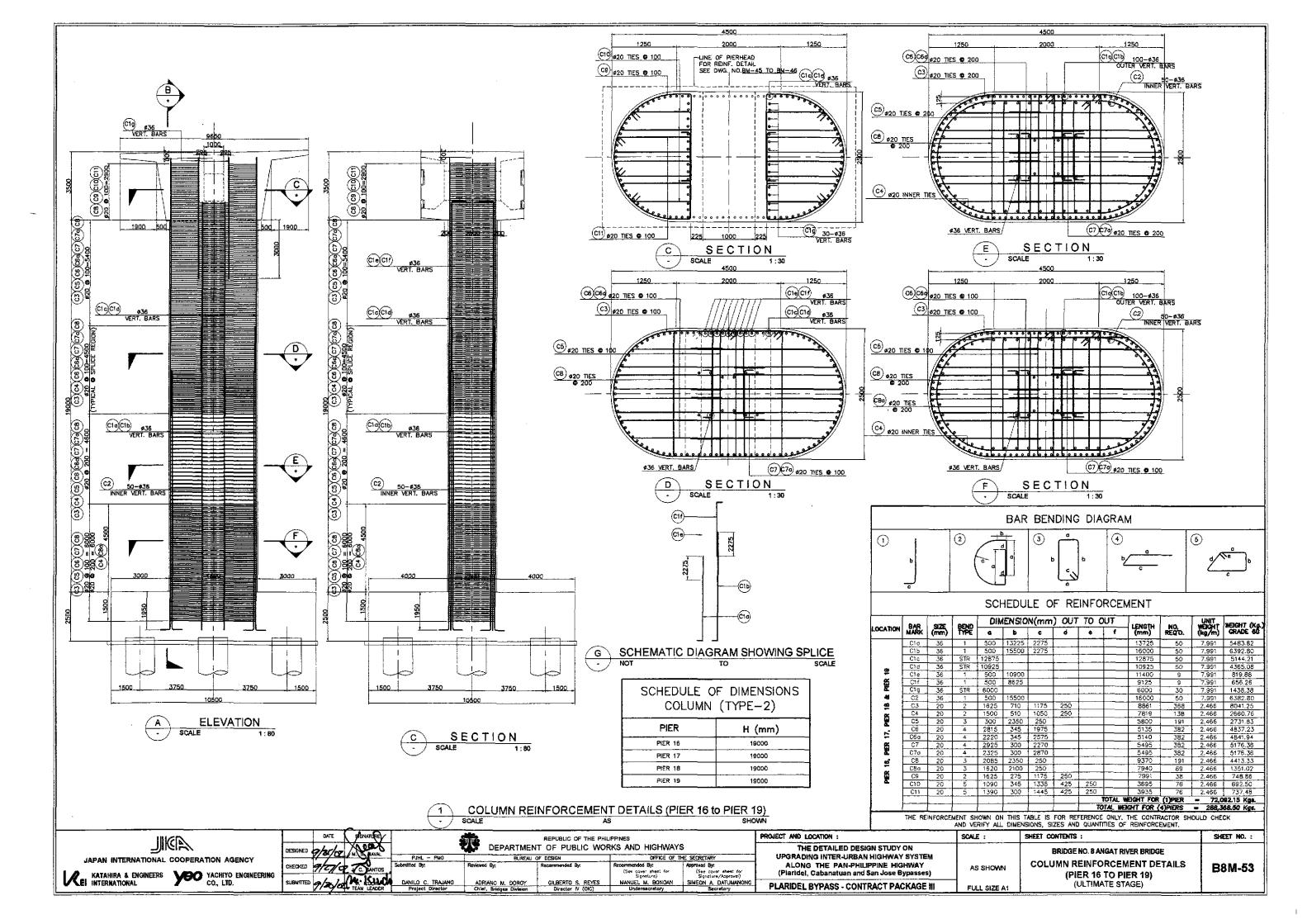
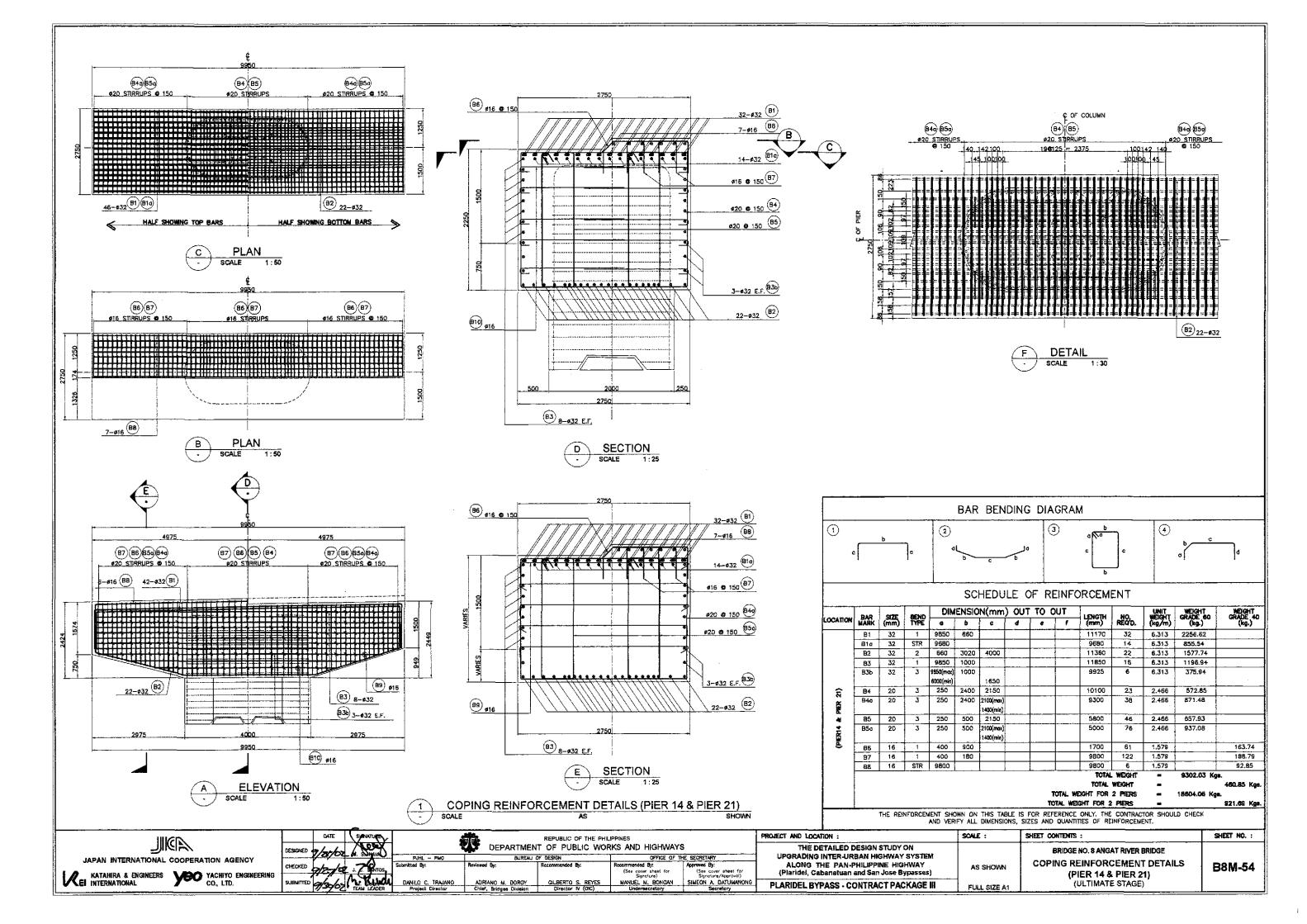
SUBSTRUCTURE REINFORCING BARS

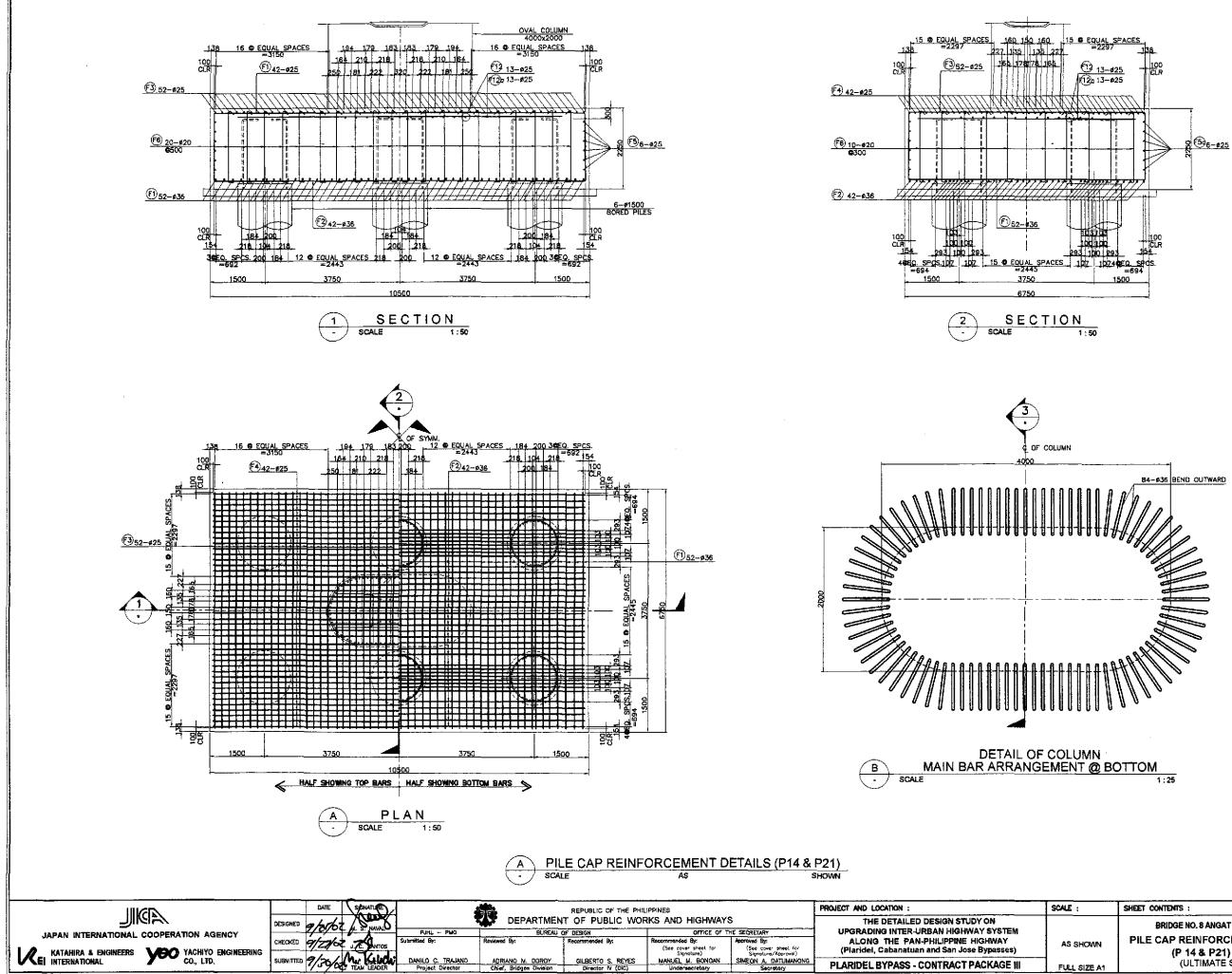




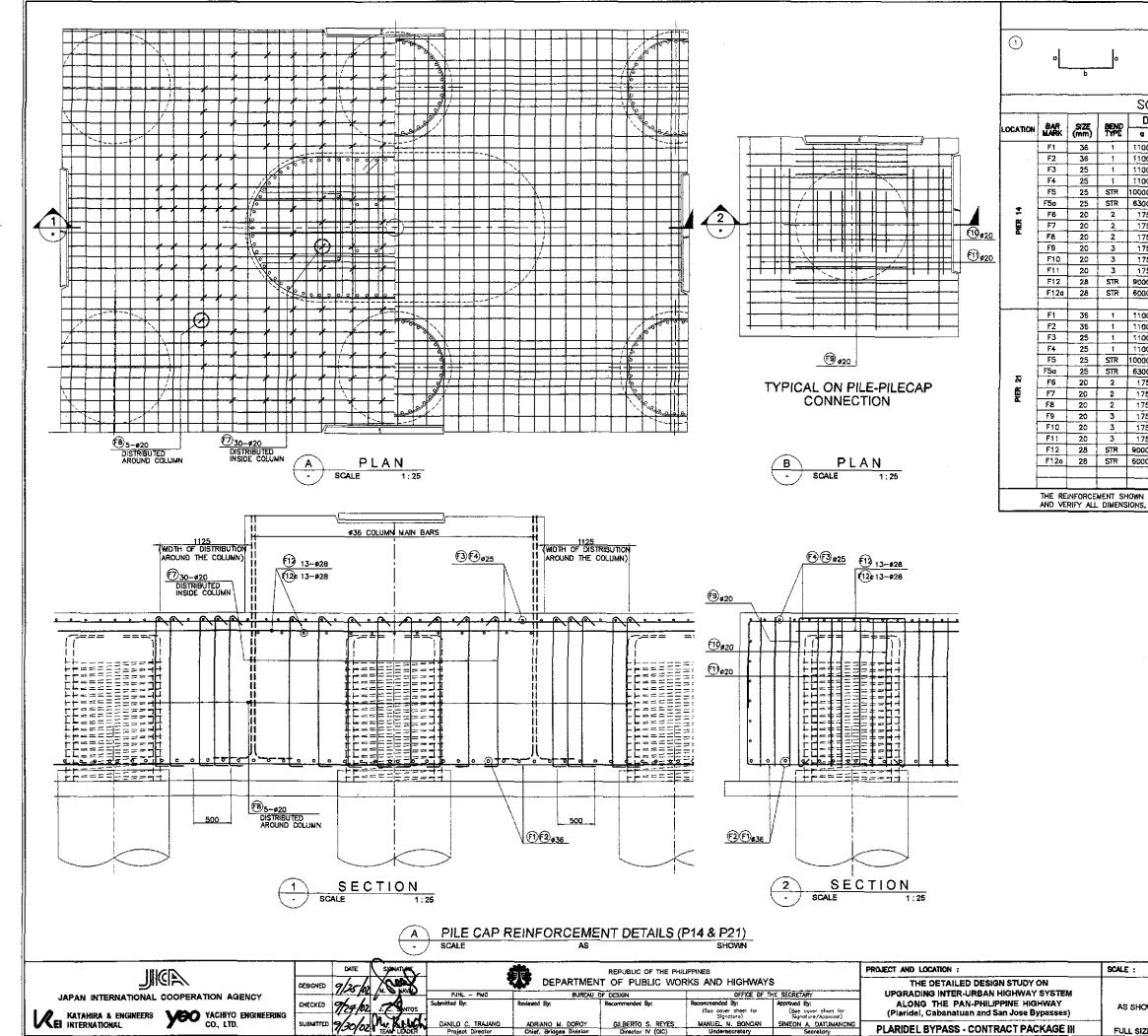






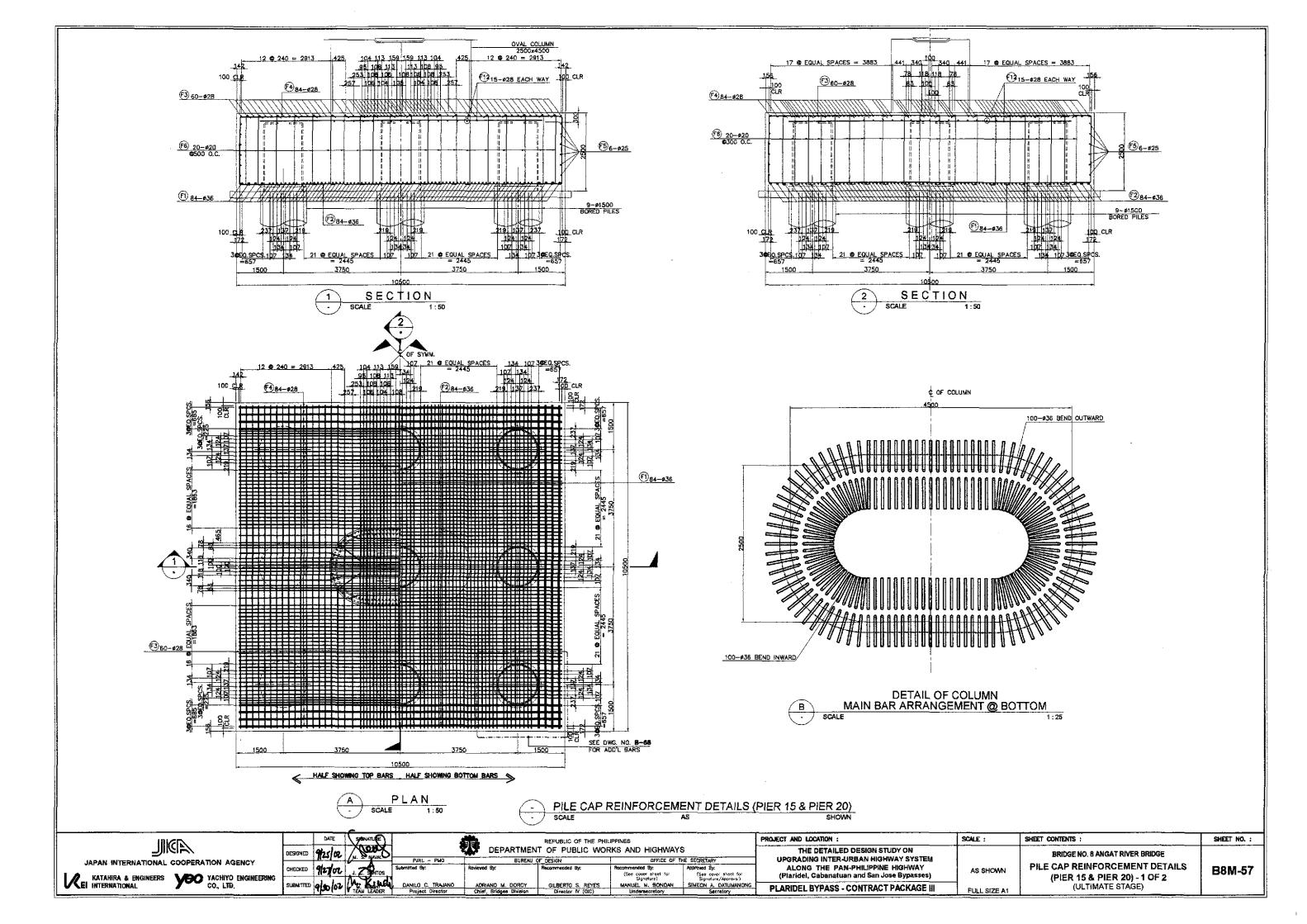


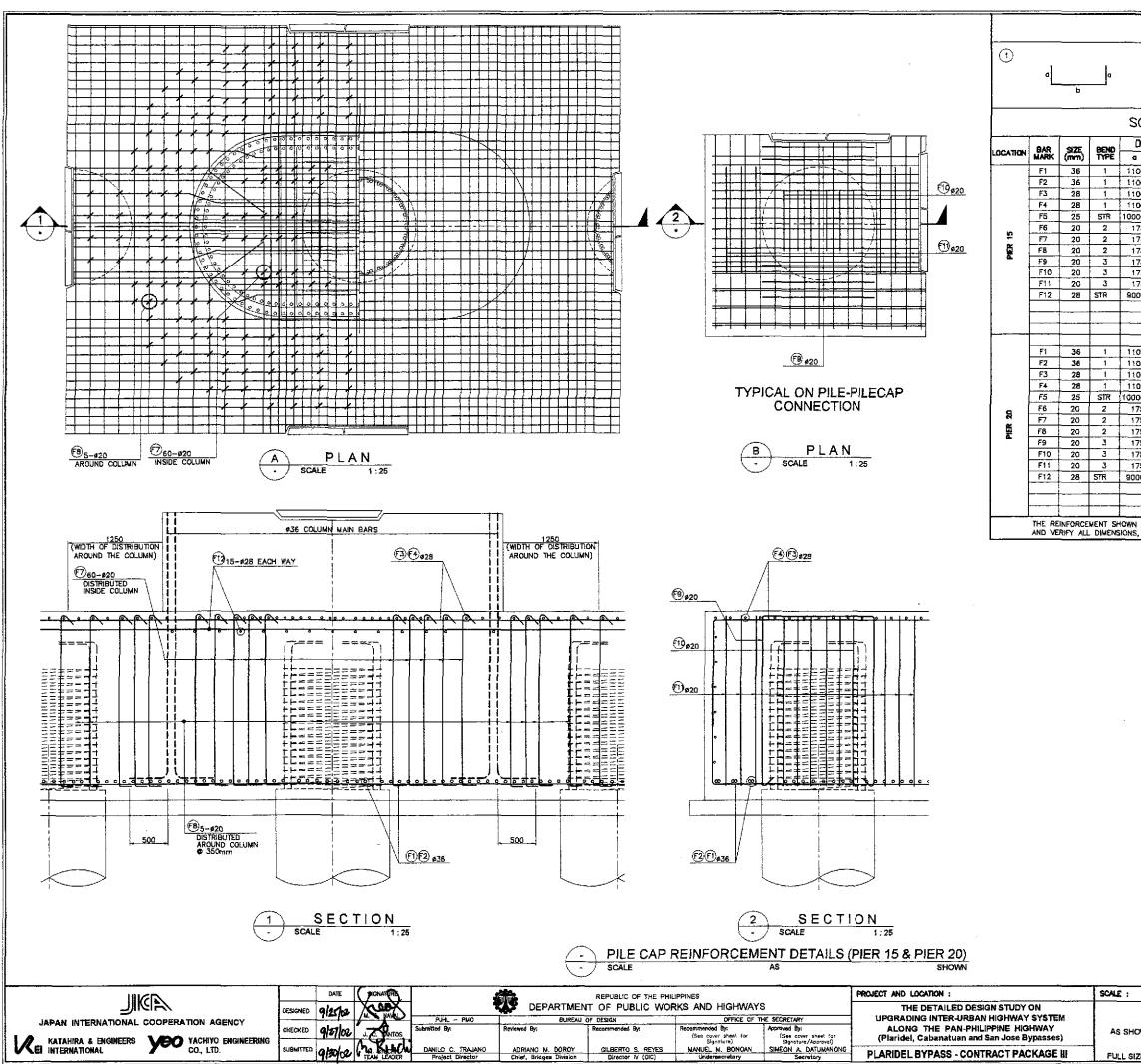
	SHEET CONTENTS :	SHEET NO. :
HOWN SIZE A1	BRIDGE NO. 8 ANGAT RIVER BRIDGE PILE CAP REINFORCEMENT DETAILS (P 14 & P21) - 1 OF 2 (ULTIMATE STAGE)	B8M-55



b b b b b SCHEDULE OF REINFORCEMENT DIMENSION(mm) OUT TO OUT LENCTH HO, MEIGHT WEIGHT WEIGHT <th></th> <th colspan="14">BAR BENDING DIAGRAM</th>		BAR BENDING DIAGRAM													
C C C SCHEDULE OF REINFORCEMENT DIMENSION(mm) OUT COUT Length NO, (Kg/m) Length Recut. WEIGHT (Kg/m) 100 6550 6 7.991 3635.91 100 6550 8750 52 7.991 3635.91 100 6550 8750 52 3.854 1753.35 100 10300 12500 42 3.854 2023.35 00 10300 12300 42 3.854 2023.35 00 10300 123.854 2023.35 2023.35 2023.35 00 10300 123.854 291.36 176.195.0 125.01 42 3.854 191.36 175 1950 175 2300 30 2.466 1008.92 175 12300 36 2.466 204.18 175 1950 175 2300 36 2.466 204.18 175 1950 175 <t< td=""><td></td><td colspan="12"></td></t<>															
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100 10300 12500 42 3.854 2023.35 000 10000 12 3.854 462.48 300 5300 12 3.854 462.48 300 175 2.300 188 2.465 1066.30 175 1950 175 2.300 192 2.465 1088.99 175 1950 175 2.300 36 2.466 204.18 175 1950 175 2.300 36 2.466 204.18 175 1950 175 2.300 36 2.466 204.18 175 1950 175 2.300 36 2.466 204.18 175 1950 175 2.300 36 2.466 204.18 000 0 9000 13 4.833 565.46 000 10300 12500 42 7.991 4185.28 100 10300 12500 42 3.854 462.4	100	10300					12500	42	7.991	4195.28					
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000 13 4.833 376.97 100 6550 707AL WEXHT = 16242.37 Kg 100 6550 8750 52 7.991 3635.91 100 10300 12500 42 7.991 4185.22 100 6550 6750 52 3.854 1753.57 100 10300 12500 42 3.854 2023.35 000 10300 12500 42 3.854 2023.35 000 10000 12 3.854 2023.35 000 10000 12 3.854 201.36 000 10000 12 3.854 201.36 175 1950 175 2300 30 2.466 1068.30 175 1950 175 2300 36 2.466 1088.99 175 1950 175 2300 36 2.466 204.18 175 1950 175 2300 <td< td=""><td>000</td><td></td><td></td><td></td><td></td><td>1</td><td>9000</td><td>13</td><td></td><td></td></td<>	000					1	9000	13							
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TOTAL WEIGHT - 16242.37 Kg TOTAL WEIGHT FOR (2) PIERS - 32484.74 Kg															
TOTAL WEIGHT FOR (2) PIERS = 32484.74 Kg				· · ·		1									
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NS, SIZES AND QUANTITIES OF REINFORCEMENT.	IN ON 15, 51	i This Th Izes and	ABLE IS QUANTI	for ref Ties of	ERENC	E ONLY.	THE CONTR								

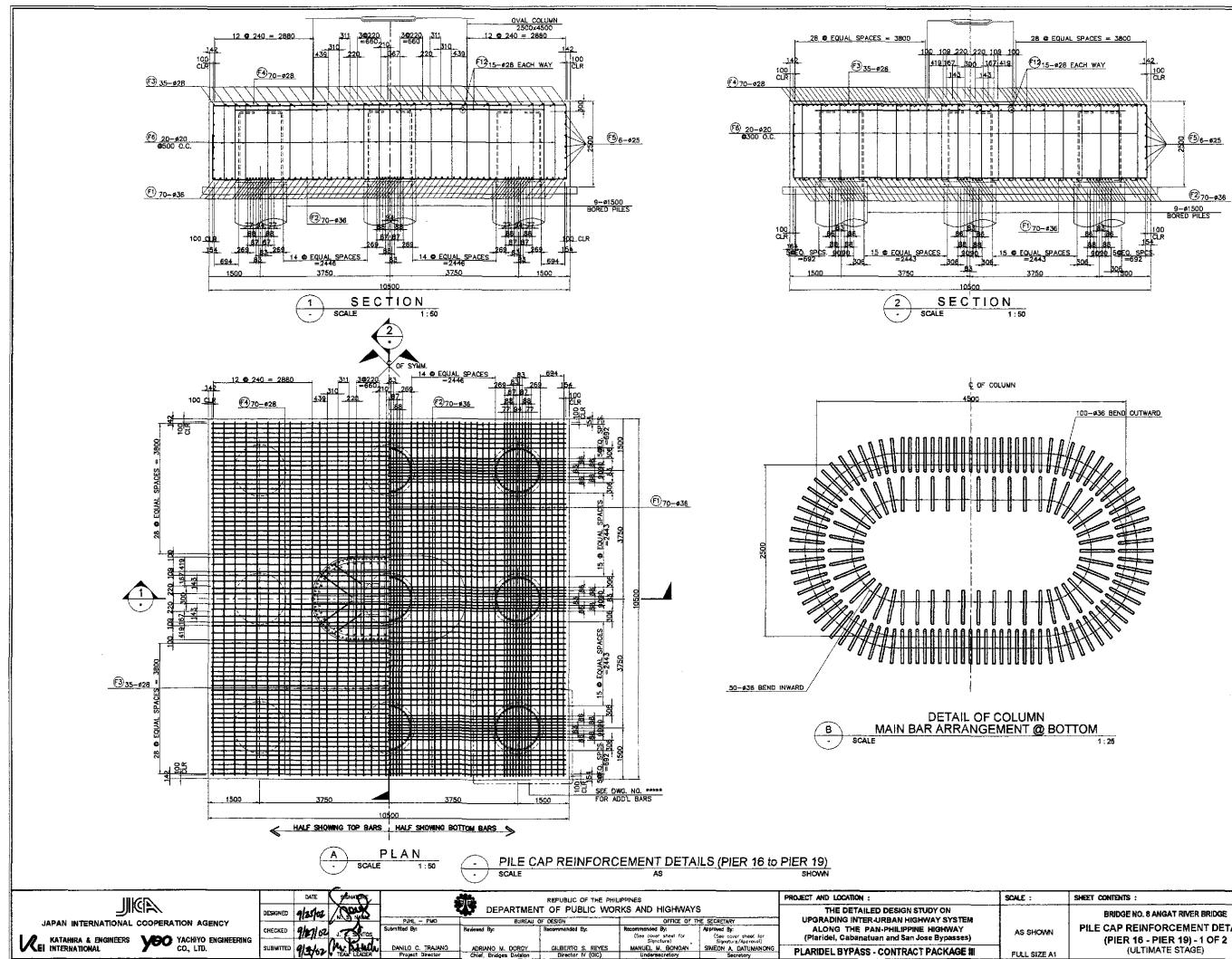
	SHEET CONTENTS :	SHEET NO. :
	BRIDGE NO. 8 ANGAT RIVER BRIDGE	
IOWN	PILE CAP REINFORCEMENT DETAILS (P 14 & P21) - 2 OF 2	B8M-56
IZE A1	(ULTIMATE STAGE)	



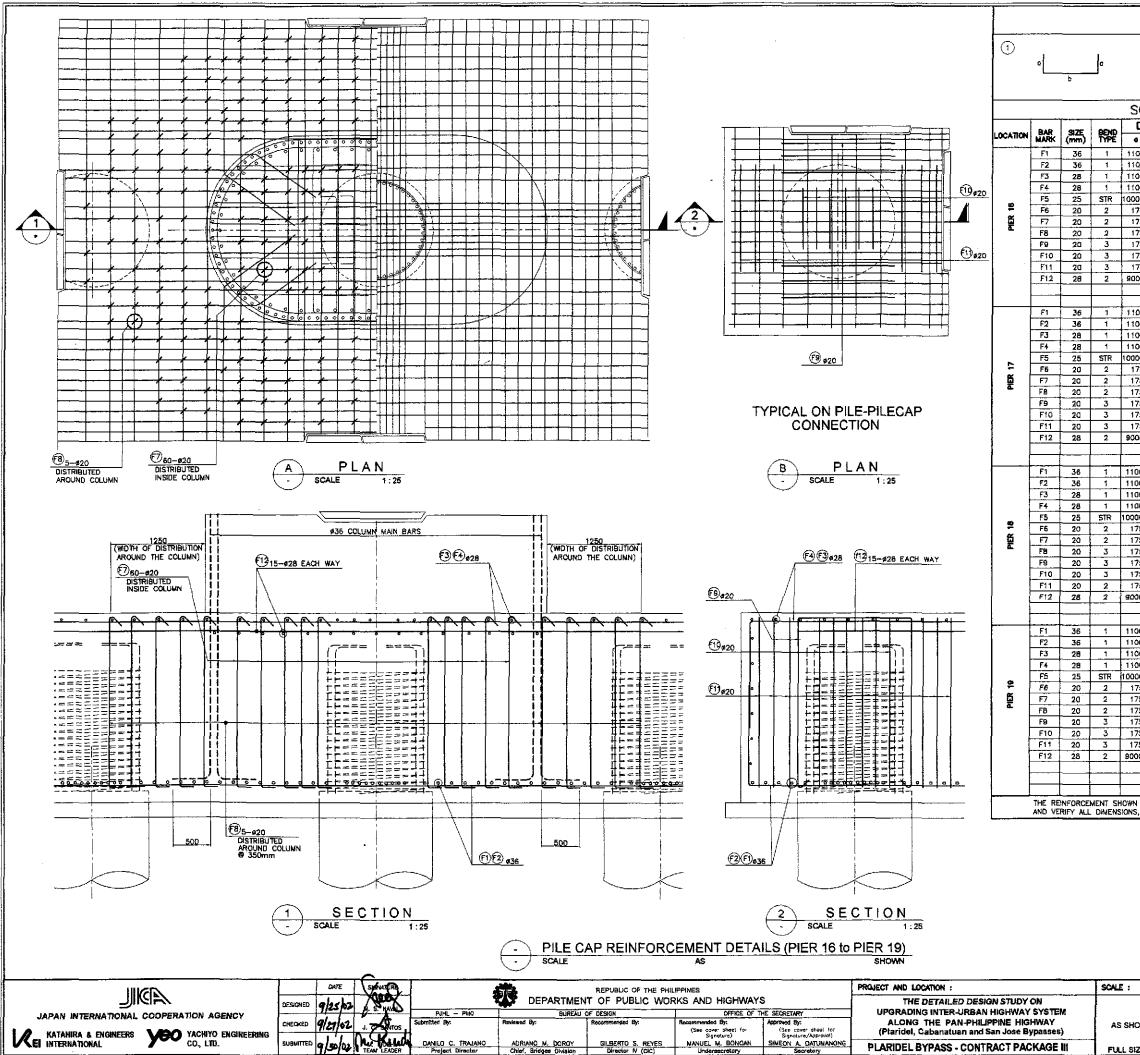


	BAR	BEN	IDING	DIA	GRAM	A			
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			V				Ļ	~ \	
CH	HEDUI	LE O	F RE	INFO	RCEN	IENT			
Σί₩	ENSIO	N(mm) OUT	TOC	NT	цемотн	NO.	UNIT WEIGHT	WEIGHT (Ka
	Ь	C	d	•	f	(mm)	NO, REG'D.	(kg/m)	WEIGHT (Kg. GRADE 80
20	10300					12500	84	7.991	8390.55
00	10300				L	12500	84	7.991	8390.55
00	10300				ļ	12500	84	4.833	5074.65
x	10300					12500	84	4,833	5074.65
00						10000	24	3,854	924.96
75	2200	175				2550	340	2.465	2138.02
75	2200	175			_	2550	60	2.466	377.30
75	2200	175			_	2550	266	2.468	1672.69
75	2200	175			 	2550	48	2.466	301.84
75	2200	175				2550	48	2.466	301.84
75	2200	175			<u> </u>	2550	48	2.465	301.84
00	ļ]	9000	30	4.833	1304.91
						÷			
					ļ			ł <u>.</u>	
•					<u> </u>	TOTAL	WEIGHT	- 3	4253.80 Kga
00	10300		<u> </u>		+	12500	84	7.991	8390.55
00	10300	· · ·			1	12500	84	7.991	8390.55
00	10300				1	12500	84	4.833	5074.65
30	10300				+	12500	84	4.833	5074.65
10					1	10000	24	J.854	924.96
75	2200	175	<u> </u>		<u> </u>	2550	340	2.466	2138.02
75	2200	175				2550	60	2.466	377.30
75	2200	175			1	2550	265	2.466	1672.69
75	2200	175				2550	48	2.466	301.84
75	2200	175				2550	48	2.466	301.84
75	2200	175			<u>†</u>	2550	48	2.466	301.84
00					ł	9000	30	4.833	1304,91
							† <u></u>		
_						-			
	4 I	1			1	I TOTAL	WEIGHT	- 3	1253.80 Kga

	SHEET CONTENTS :	SHEET NO. :
	BRIDGE NO. 8 ANGAT RIVER BRIDGE	
IOWN	PILE CAP REINFORCEMENT DETAILS (PIER 15 & PIER 20) - 2 OF 2	B8M-58
SIZE A1	(ULTIMATE STAGE)	

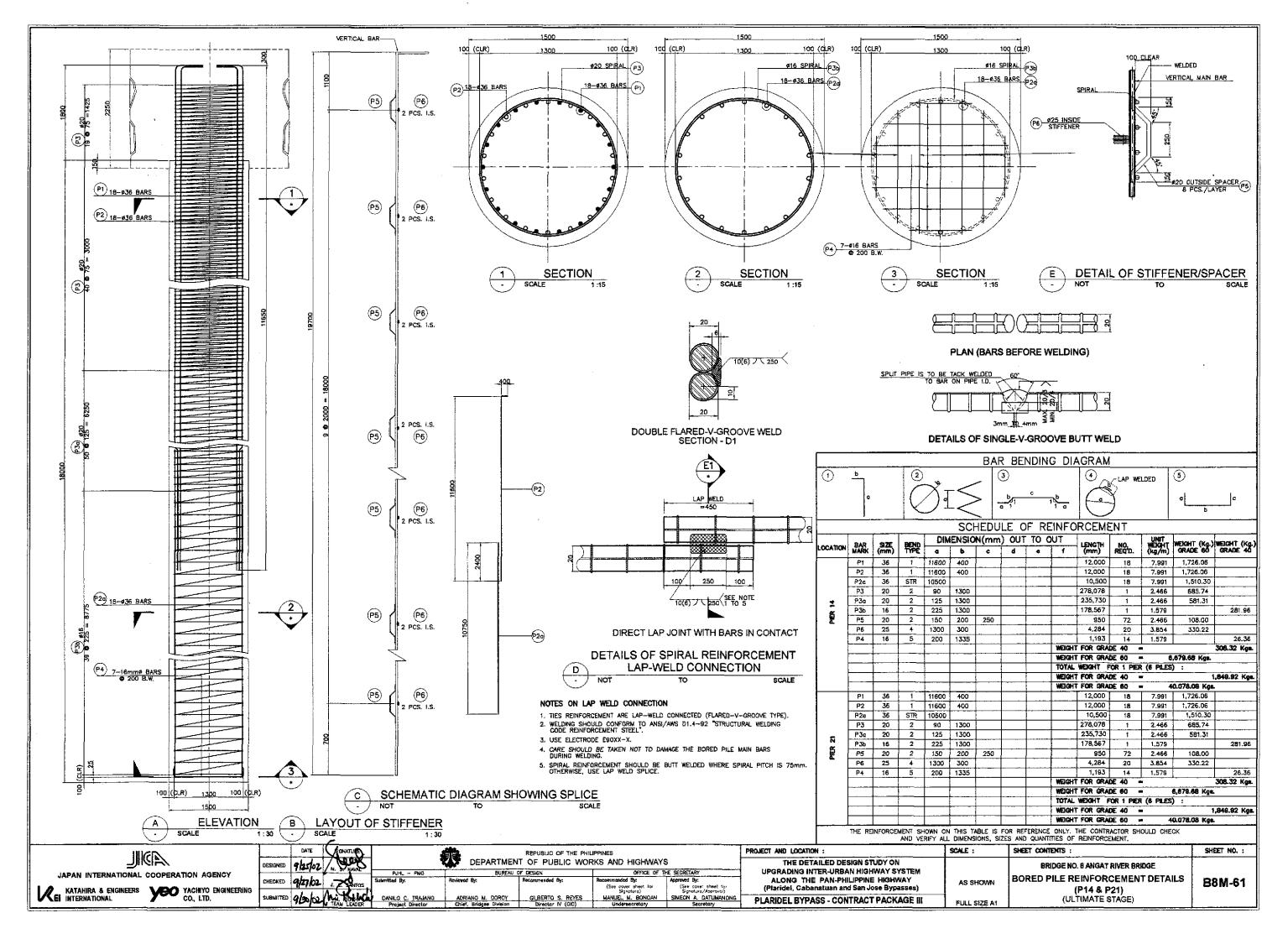


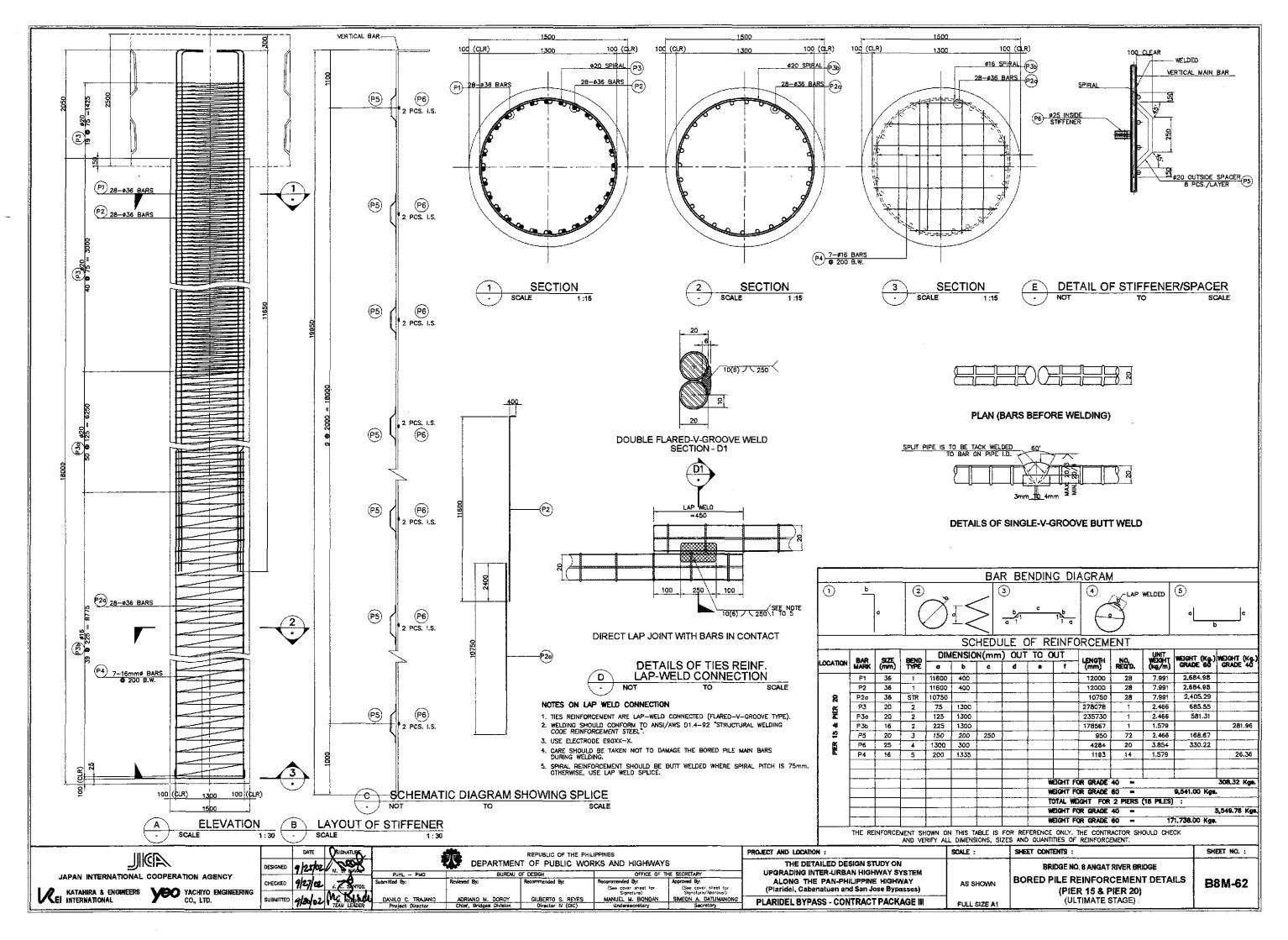
	SHEET NO. :
IOWN PILE CAP REINFORCEMENT DETAILS (PIER 16 - PIER 19) - 1 OF 2 (ULTIMATE STAGE)	B8M-59

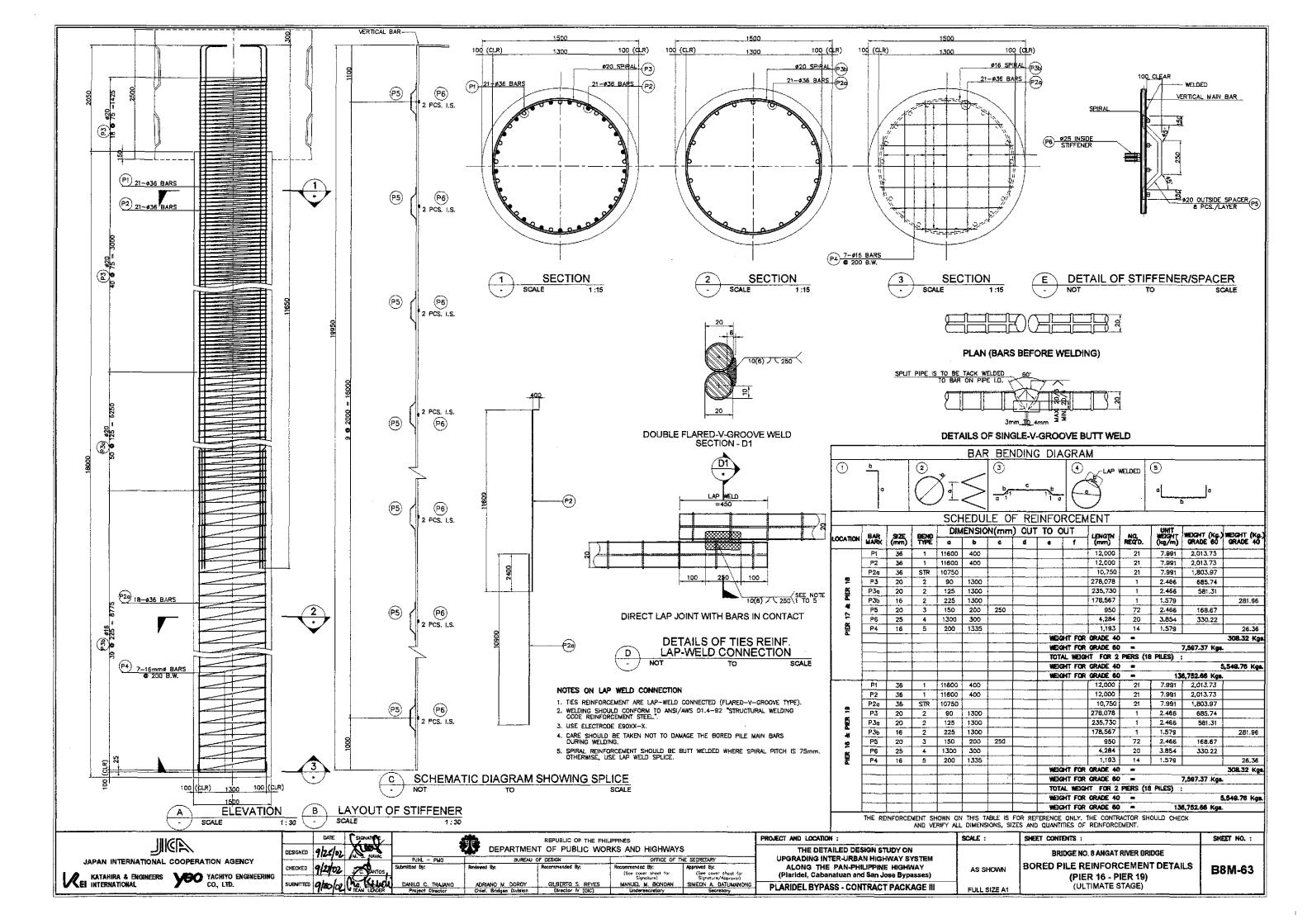


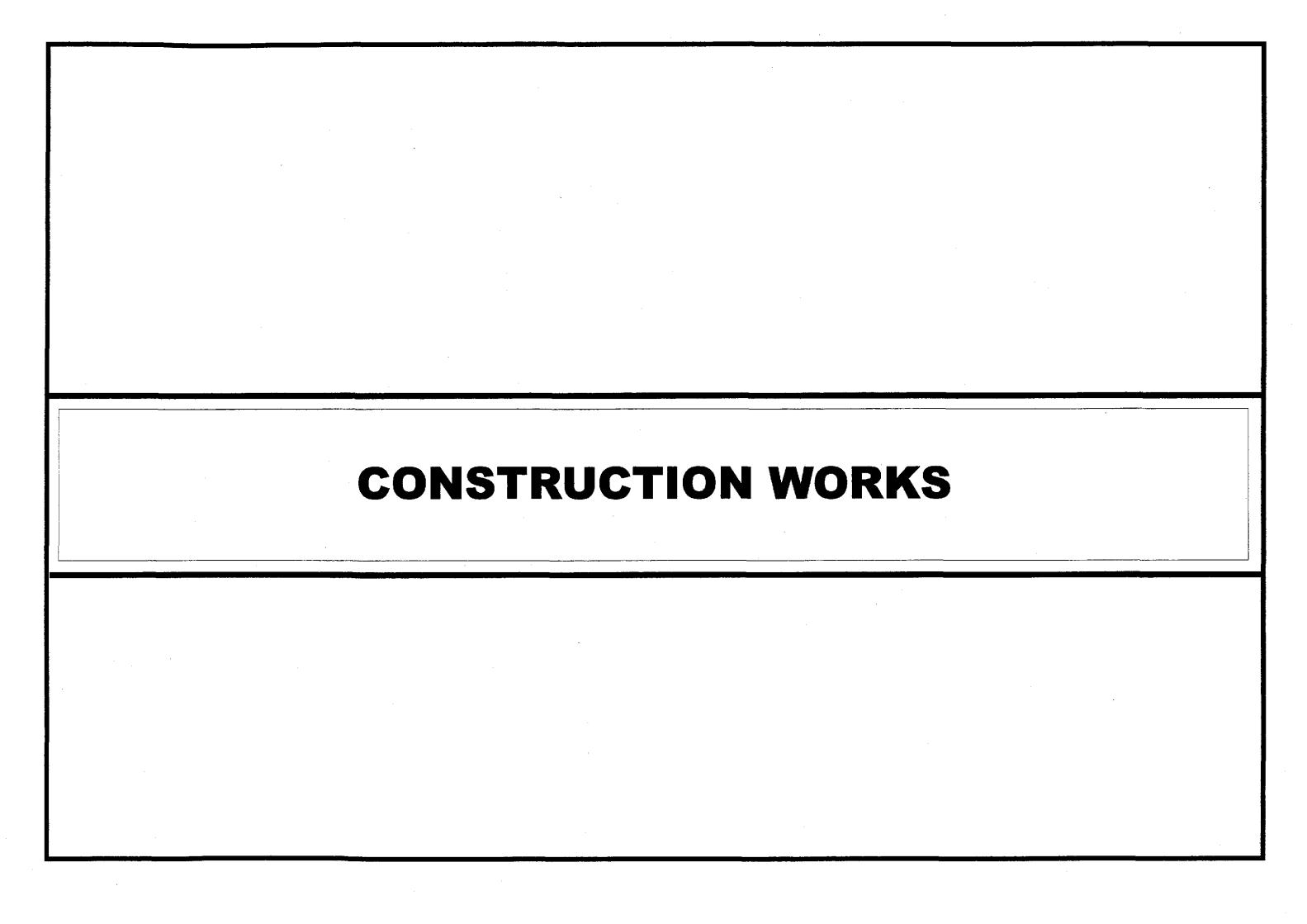
CHEDULE OF REINFORCEMENT LINET (MENSION(mm)) OUT TO OUT (Mg/m) LINET (Mg/m) MCION (Mg/m)		2		b b c			3	ь	~ ~	b
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2200 175 2550 27C 2.466 1697.84 2200 175 2550 48 2.466 301.84 2200 175 2550 48 2.466 301.84 2200 175 2550 48 2.466 301.84 2200 175 2550 48 2.466 301.84 2200 175 2550 48 2.466 301.84 2200 175 2550 48 2.466 301.84 2200 9000 30 4.833 1304.91 TOTAL WEIGHT FOR (4) PIERS 110704.43 Kga. TOTAL WEIGHT FOR (4) PIERS 110704.43 Kga. ON THIS TABLE IS FOR REFERENCE ONLY. THE CONTRACTOR SHOULD CHECK)	((,		
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2200 175 2550 48 2.466 301.84 2200 9000 30 4.833 1304.91 TOTAL WEIGHT FOR (4) PIERS = 27676.11 Kgs. TOTAL WEIGHT FOR (4) PIERS = 110704.43 Kgs. ON THIS TABLE IS FOR REFERENCE ONLY. THE CONTRACTOR SHOULD CHECK) 5	2200	175				2550	48	2.466	301.84
2200 9000 30 4.833 1304.91 TOTAL WEIGHT - 27676.11 Kga. TOTAL WEIGHT FOR (4) PIERS - 110704.43 Kga. ON THIS TABLE IS FOR REFERENCE ONLY. THE CONTRACTOR SHOULD CHECK) ;	2200								
TOTAL WEIGHT - 27676.11 Kga. TOTAL WEIGHT FOR (4) PIERS - 110704.43 Kga. ON THIS TABLE IS FOR REFERENCE ONLY. THE CONTRACTOR SHOULD CHECK) ; ;	2200 2200								
ON THIS TABLE IS FOR REFERENCE ONLY. THE CONTRACTOR SHOULD CHECK) ; ;	2200 2200 2200	175				9000	30	4.833	1304.91
ON THIS TABLE IS FOR REFERENCE ONLY. THE CONTRACTOR SHOULD CHECK) ; ;	2200 2200 2200	175					L	ł	
ON THIS TABLE IS FOR REFERENCE ONLY. THE CONTRACTOR SHOULD CHECK SIZES AND QUANTITIES OF REINFORCEMENT.)))	2200 2200 2200					TOTAL	WEIGHT	- 2	7876,11 Kan
SIZES AND QUANTITIES OF REINFORCEMENT.)))	2200 2200 2200	175		TOTA	L WEIGHT				
		2200 2200 2200 2200		FOR REF	ERENCE	ONLY.	THE CONTR	PIERS	= 110	704.43 Kgs.

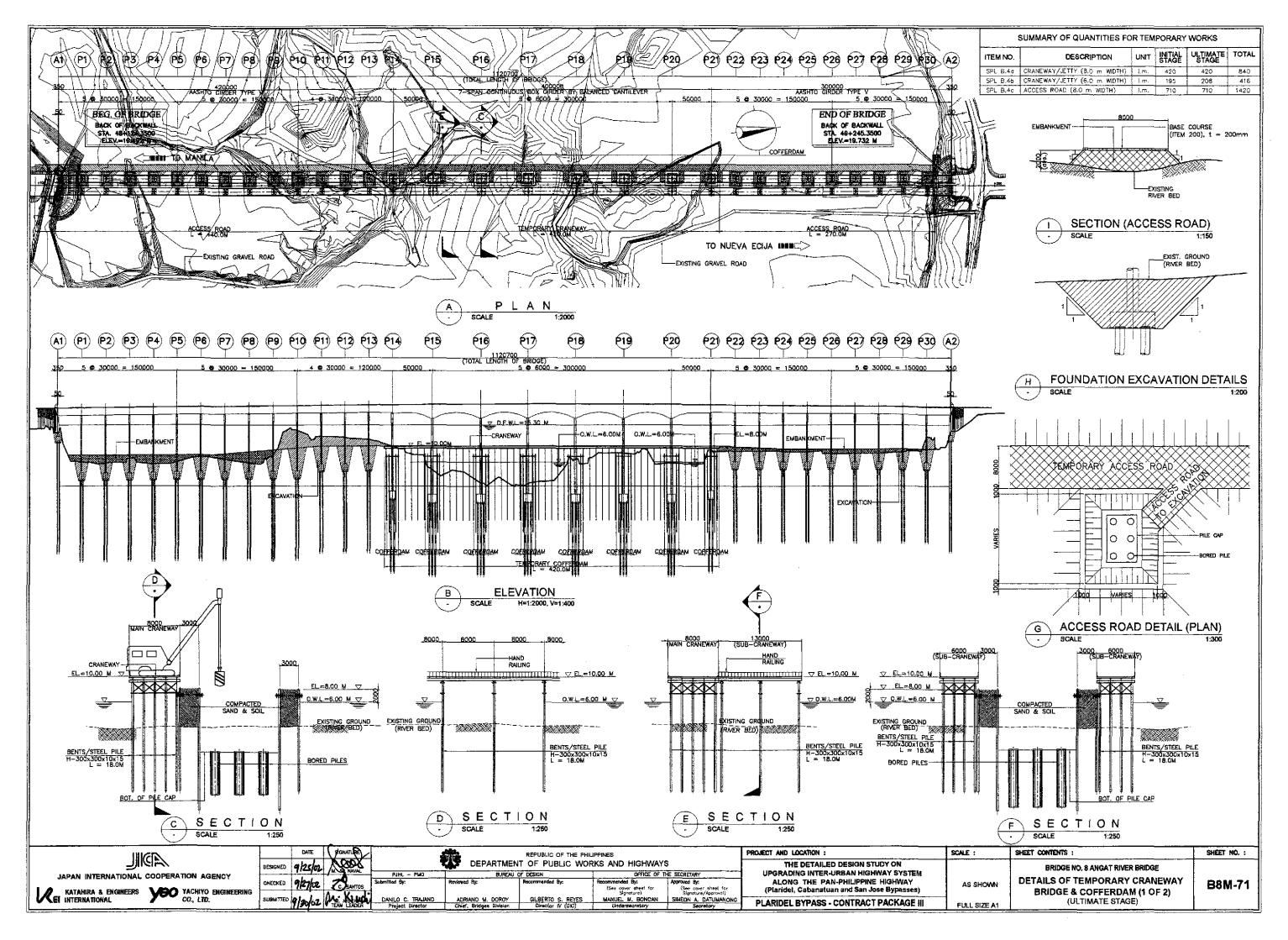
	SHEET CONTENTS :	SHEET NO. :
HOWIN BIZE A1	BRIDGE ND. 8 ANGAT RIVER BRIDGE PILE CAP REINFORCEMENT DETAILS (PIER 16 - PIER 19) - 2 OF 2 (ULTIMATE STAGE)	B8M-60



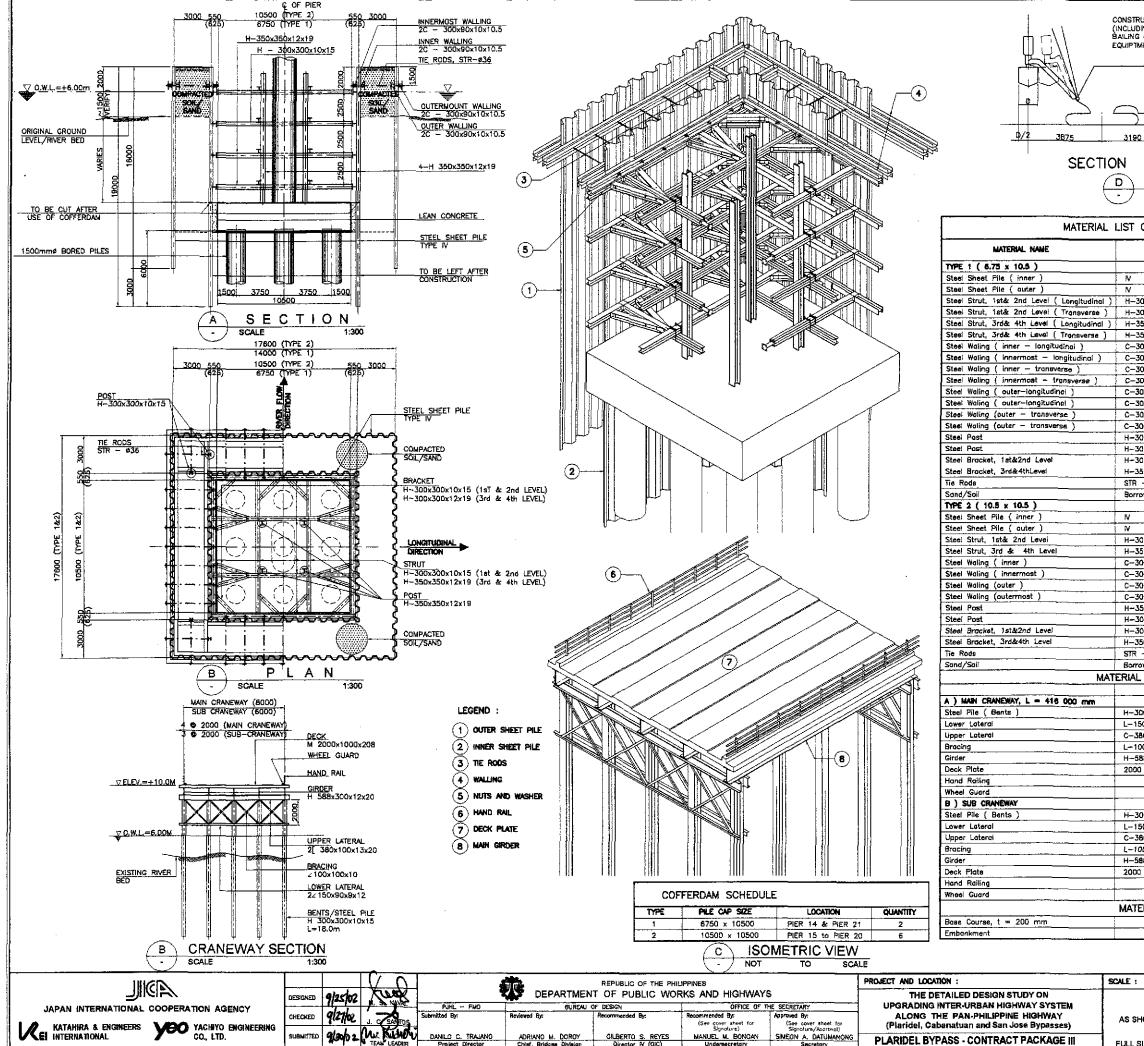




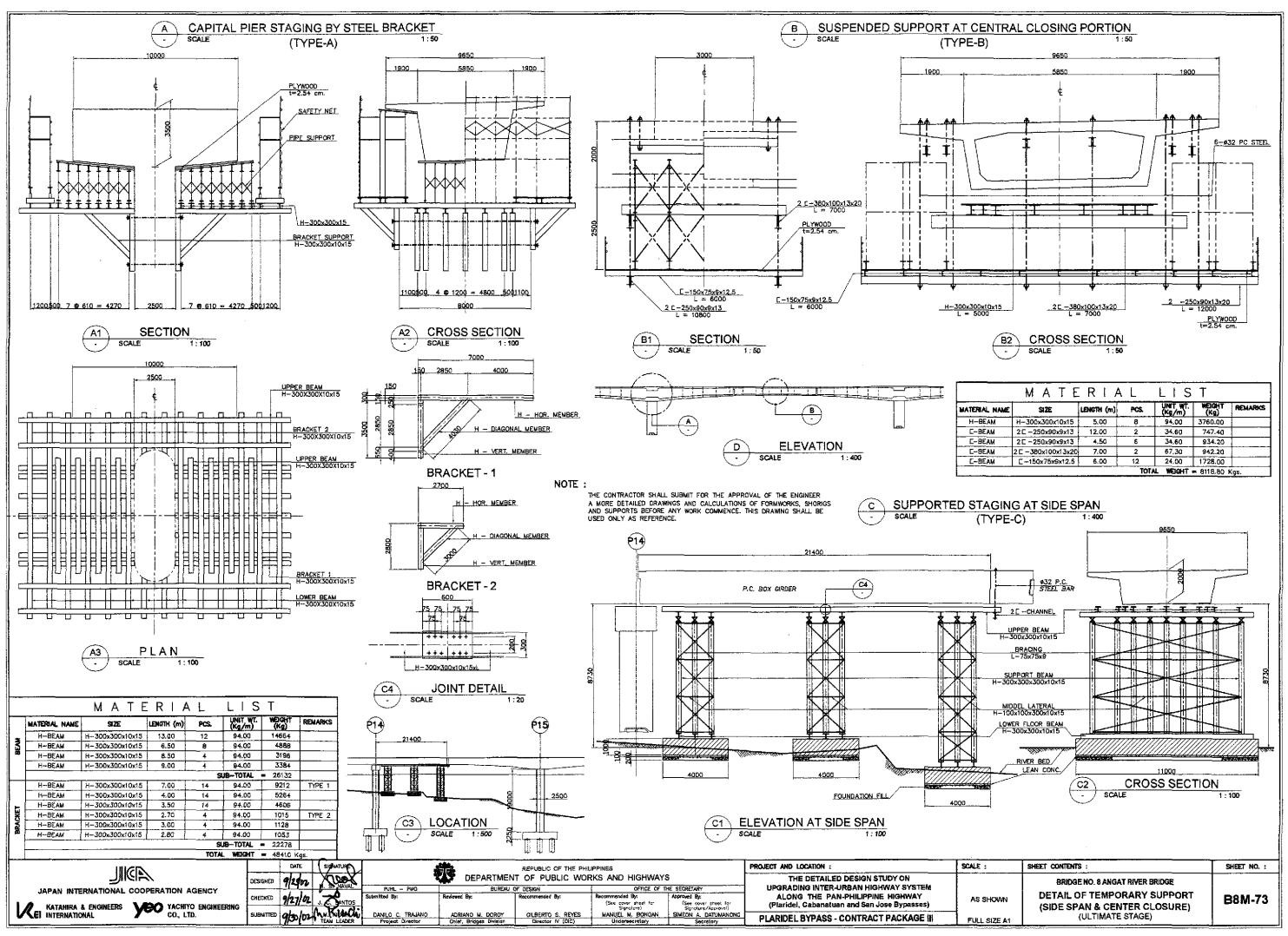


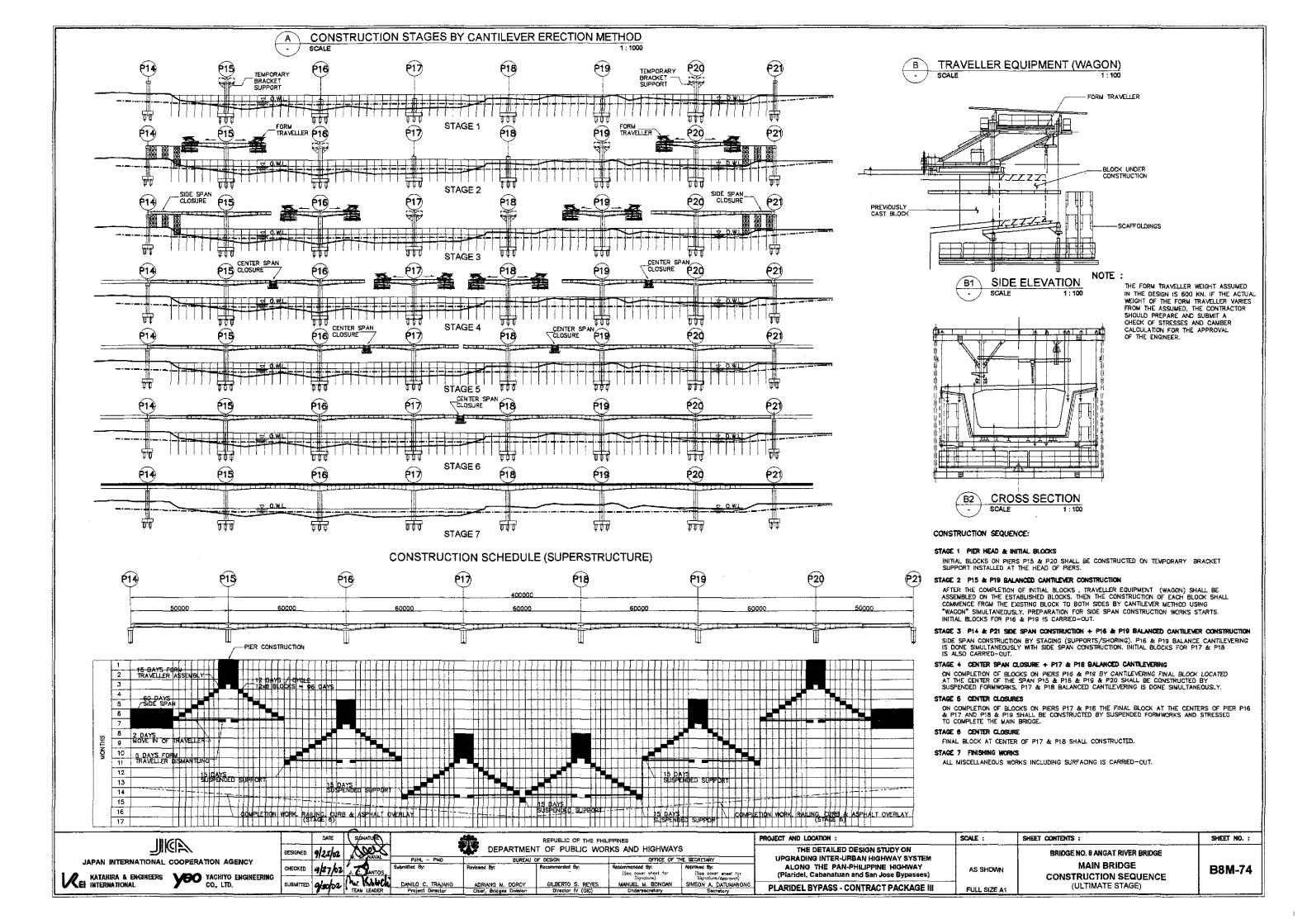


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BUCKETS, FINAL CLEA ENT, ETC.)	NING .			/	Х							
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OF TEMPORARY	COFFER	nau /	DED									
			FER	· · ·								
\$ 1 Z E	LENGTH (m)	NO. (pcs)	UNIT	UNIT WI (kg/m)	(kg.)	REMARKS						
······												
	19.00	98 158	kģs kģs	76,10	141,699.00							
0x300x10x15	6.85	4	kgs	94.00	2,576.00							
0x300x10x15	10.60	4	kçs	94.00	3,986.00							
0x350x12x19	6.85	4	kgs	137.00	3,754.00							
0x350x12x19	10.60	4	kgs	137,00 43,80	5,809.00							
0x90x10x10.5	13.45	4	kga kgs	43.80	2.357.00							
0x90x10x10.5	17.20	4	kgs	43.80	3,014.00							
0x90x10x10.5	11.20	4	kgs	43.80	1,963.00							
0x90x10x10.5	13.45	4	kģs	43.80	2,357.00							
0x90x10x10.5 0x90x10x10.5	14.25	4	kgs kgs	43.80	2,497.00 3,014.00							
0x90x10x10.5	18.00	-4	kgs kgs	43.80	3,154.00							
0x300x10x15	16.00	8	kga	94.00	12,032.00							
0x300x12x19	16.00	4	kgs	137.00	8,768.00							
0x300x10x15	2.00	24	kgs	94.00	4,512.00							
Dx350x12x19 - Ø36	2.00	24	kga	137.00	6,576.00							
w Materials		32	kgs m ³	7.99	895.00 610.80	Selected Materials						
	19.00	116	kgs	76.10	167,725.00	······						
0.200.10.10	16.00	176	kgs	76.10	214,298.00							
0x300x10x15 0x350x12x19	10.60	16 16	kgs kgs	94.00	15,943.00 23,236.00							
0x90x10x10.5	17.20	8	kgs kgs	43,80	6,027.00							
0x90x10x10.5	11.20	8	kgs	43.80	3,925.00							
0x90x10x10.5	17.20	8	kgs	43.60	6,027.00							
0x90x10x10.5	18.00	8	kgs	43.80	6,308.00							
0x350x12x19 0x300x10x15	16.00	4	kgs	137.00 94.00	8,768.00							
0x300x10x15	2.00	32	kça kçs	94.00	12,032.00							
0x350x12x19	2.00	32	kgs	137.00	8,768.00	· · · · · · · · · · · · · · · · · · ·						
- Ø36	3.50	36	kgs	7.99	1,007.00							
Moterials		DANT	m ³	()	700.80	Selected Materials						
LIST OF TEMPO	INARY (RANE	MAY	ر ر	··							
	+		 .	 								
0 x 300 x 10 x 15	18.00	265	kgs	94.00	448,380.00							
0 x 90 x 9 x 12	9,30	106	kgs	16.40	16,168.00							
1 x 100 x 13 x 20	9.30	106	kgs	67.30	66,345.00							
3 x 100 x 10 8 x 300 x 12 x 20	2.90	424 260	kgs	14.90 147.00	18,322.00							
x 1000 x 208	0.00	~ 00	kgs 2	147.00	305,760.00							
			77		840.00							
			m		840.00							
			h	0.00	00/ 050							
0 x 300 x 10 x 15 0 x 90 x 9 x 12	18.00	168 84	kgs kgs	94.00 16.40	284,256.00							
0 x 100 x 13 x 20	7.30	84	kga	67.30	41,269.00							
0 x 100 x 10	2.90	84	kos	14.90	3,630.00							
8 x 300 x 12 x 20	13.00	56	k çs	147.00	107,016.00							
x 1000 x 208	+		m 2 m		1,092.00	<u> </u>						
····			m	<u> </u>	364.00							
RIAL LIST OF A	CCESS	ROAD		,	00 1100							
	710.00		m3		1165.00							
······································	710.00		m3	<u> </u>	12330.00							
SHEET COM	• 21/GTI					SHEET NO. :						
SALE UUT						ancel WUL I						
				-								
	RIDGE NO			-								
	RIDGE NO .S OF TH DGE & (RARY	CRAN	EWAY	B8M-72						





SPECIAL NOTES FOR CONSTRUCTION OF MAIN BRIDGE AND RELATED TEMPORARY WORKS

AT LEAST ONE (1) MONTH BEFORE COMMENCEMENT OF ANY STRUCTURE WORKS. THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER FOR APPROVAL COMPLETE DETAILS OF THE CONSTRUCTION NETHOD AND SCHEDULE WHICH SHOULD INCLUDE, BUT NOT LIMITED TO THE FOLLOWING :

* DETAILED CONSTRUCTION SEQUENCE AND SCHEDULE BASED ON EQUIPMENT CAPACITY MOBILIZED, CONCRETE TRIAL MX RESULTS (DEVELOPMENT OF CONCRETE STRENGTH). TYPE OF CRANE AND ALL OTHER RELEVANT INFORMATION. THE CONSTRUCTION SEQUENCE AND CONSTRUCTION SCHEDULE OF SUPERSTRUCTURE SHOWN ABOVE SHALL BE CONSIDERED AS REFERENCE ONLY TO GUIDE THE CONTRACTOR.

PRESTRESSING METHOD, COMPLETE DETAILS OF THE PROPOSED METHOD. MATERIALS, AND EQUIPMENT TO BE USED IN THE PRESTRESSING OPERATIONS, SUCH DETAILS SHALL INCLUDE THE COMPLETE SPECIFICATIONS AND DETAILS OF THE PRESTRESSING STEEL AND ANCHORING DEVICES, TYPE OF ENCLOSURES AND ALL OTHER DATA RELATED TO PRESTRESSING OPERATION. THE CONTRACTOR SHALL ALSO SUBMIT TO THE CONSULTANT FOR APPROVAL THE PROPOSED PRESTRESSING CONTROL AND ASSURANCE METHOD.

. CAMBER CALCULATION DURING CONSTRUCTION AND AFTER COMPLETION OF THE MAIN BRIDGE BASED ON ACTUAL WEIGHT MOBILIZED, ANTICIPATED SURCHARGE AND RESPECTIVE PHYSICAL CONCRETE PROPERTIES UNDER THE DIFFERENT AGE TO BE CONSIDERED AND OTHER LOADS AFFECTING CAMBER. THE CAMBER SHALL BE CALCULATED AT LEAST BASED ON THE EFFECT OF THE FOLLOWING :

* DEFLECTION DUE TO CONSTRUCTION LIVE LOAD.

* DEFLECTION DUE TO WEIGHT OF FRESH CONCRETE.

* DEFLECTION DUE TO PRESTRESSING FORCE.

* DEFLECTION DUE TO CREEP AND SHRINKAGE OF CONCRETE, AND

 PRESTRESSING CALCULATION SHOWING THE ESTIMATED PRESTRESSING FORCE AND THE DETAILED ARRANGEMENT OF INDIVIDUAL P.C. CABLES AND PRESTRESSING SEQUENCE OF EACH CABLE, ETC. TO BE PREPARED AND SUBMITTED BY THE CONTRACTOR BASED ON THE CALCULATIONS MADE IN THE DETAILED DESIGN (WHICH WILL BE PROVIDED TO THE CONTRACTOR). ANTICIPATED ELASTICITY MODULUS OF CONCRETE RESULTING FROM THE CONCRETE TRIAL MIX AND OTHER RELATED DATA NECESSARY IN THE PRESTRESSING CALCULATION.

* STRESSES OF MEMBERS AT DIFFERENT STAGES OF CONSTRUCTION SHOULD BE CHECKED AND VERIFIED BY THE CONTRACTOR ESPECIALLY DURING STRESSING OPERATIONS.

2. CRANEWAY CONSISTING OF TEMPORARY BRIDGE AND APPROACH ROAD SHOWN IN DWG. NO. BM-71

- PRIOR TO COMMENCEMENT OF CRANEWAY CONSTRUCTION, THE CONTRACTOR SHALL SUBMIT A DETAILED CONSTRUCTION DRAWING OF THE CRANEWAY WITH STRUCTURAL ANALYSIS BASED ON THE LATEST TOPOGRAPHIC SURVEY (S=1/500) TAKEN IN PRE-CONSTRUCTION SURVEY STAGE AND IN ACCORDANCE WITH DWG. NO, CS2A-182 (WHICH SHALL BE CONSIDERED AS REFERENCE ONLY) APPLICABLE DESIGN LIVE LOAD FOR CRANEWAY DESIGN SHALL BE MS-18 IN AASHTO OR GROSS WEIGHT OF 80 TON TRAILER. WHICHEVER DOONLICES THE SEVERE SEEFCT FOR A NEWSER FOR DE DESIDATION SURVEY OF 80 TON TRAILER. WHICHEVER PRODUCES THE SEVERE EFFECT FOR A MEMBER TO BE DESIGNED.
- * IT WILL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR TO MAINTAIN AND REPAIR THE CRANEWAY AT HIS OWN EXPENSE DURING THE ENTIRE CONSTRUCTION PERIOD.
- * DAMAGES INFLICTED ON THE CRANEWAY AS A RESULT OF TYPHOON AND FLOODS SHALL NOT BE A BASIS FOR CLAIM OF ANY MANNER AGAINST DOWH AND IT IS THE OBLIGATION OF THE CONTRACTOR TO RESTORE AND REPAIR THE DAMAGES AT HIS EXPENSE.
- AFTER COMPLETION OF THE MAIN BRIDGE, THE CONTRACTOR IS RESPONSIBLE (AT HIS OWN EXPENSE) FOR THE RENOVAL OF THE CRANEWAY AND THE RESTORATION OF THE RIVER BANK TO ITS ORIGINAL CONDITION.
- 3. TEMPORARY SUPPORTS / SHORINGS FOR PIER P14 AND P21 IN DWG, NO. BN-73 AND SUSPENDED FORMWORKS FOR CENTER SPAN CLOSURE.
- * THE CONTRACTOR SHALL SUBMIT FOR THE ENGINEER'S APPROVAL WORKING DRAWINGS OF THE TEMPORARY SUPPORTS, SHORINGS & SUSPENDED FORMWORKS FOR CENTER CLOSURE INCLUDING DESIGN CALCULATIONS AS REFERRED TO DWG. NO. BM-73
- DAMAGES INFLICTED ON THE TEMPORARY SUPPORTS AS THE RESULT OF TYPHOONS AND FLOODS SHALL NOT BE A BASIS FOR CLAIM OF ANY MANNER AGAINST DPWH AND IT IS OBLIGATION OF THE CONTRACTORS TO RESTORE AND REPAIR THE DAMAGES AT HIS EXPENSE.
- 4. FORM TRAVELLER (WAGON)

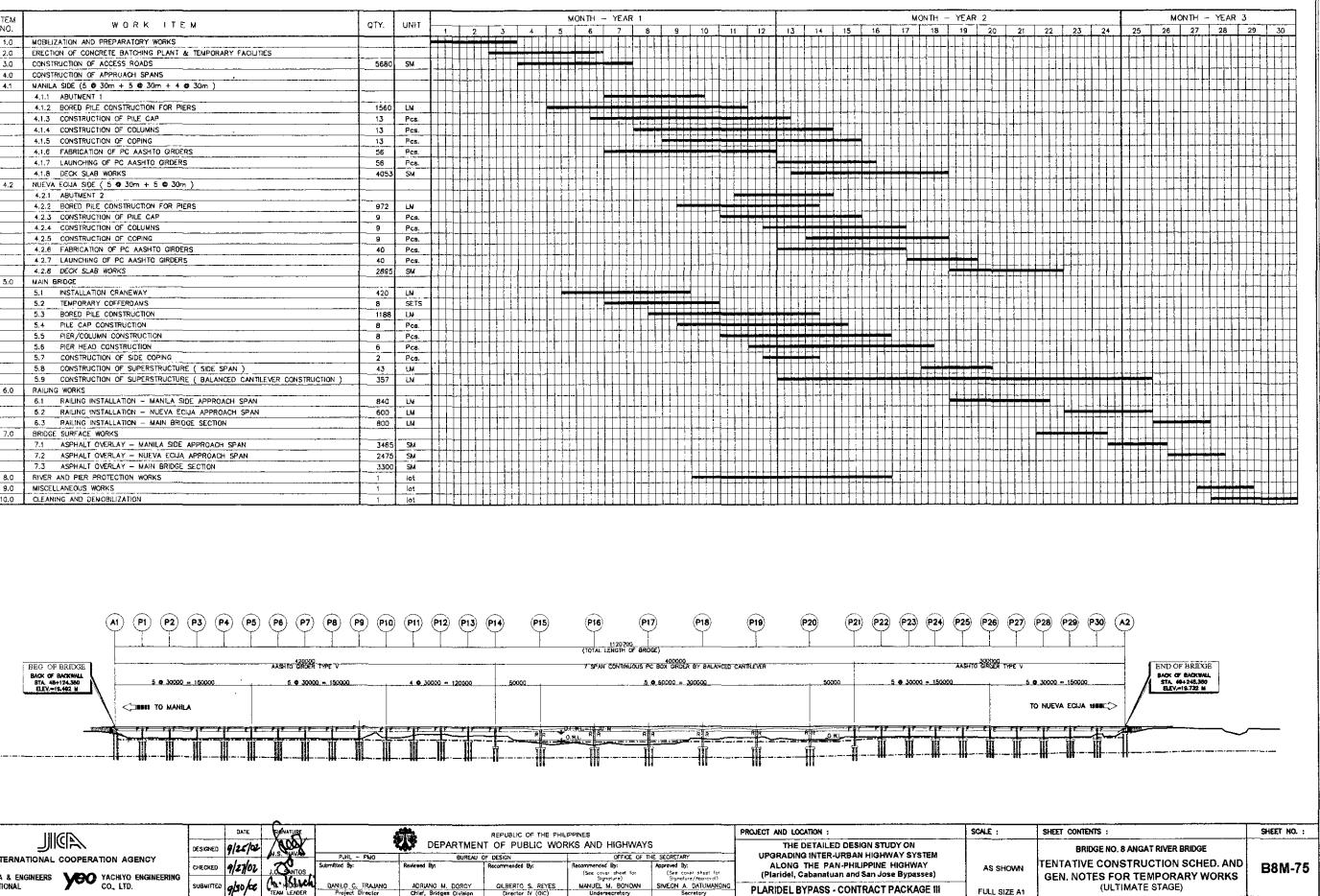
* THE CONTRACTOR SHALL SUBMIT FOR THE APPROVAL OF THE ENGINEER, THE WORKING DRAWINGS FOR FORM TRAVELLER INCLUDING DESIGN CALCULATIONS, CAPACITY AND WEIGHT OF THE TRAVELLER.

IF THE FORM TRAVELLER'S WEIGHT DIFFER FROM THE ASSUMED WEIGHT AS USED IN THE DETAILED DESIGN, THE CONTRACTOR SHALL PROVIDE A CHECK OF STRESSES FOR ALL CONSTRUCTION STAGE & AFTER COMPLETION OF THE SUPERSTRUCTURE (INCLUDING LONG TERN EFFECTS), CANBER SHALL ALSO BE INVESTIGATED.

* ASSUMED WEIGHT OF FORM TRAVELLER = 600 KN

- 5. TENTATIVE CONSTRUCTION SCHEDULE SHOWN
- THE TENTATIVE CONSTRUCTION SCHEDULE SHOWN SHALL BE CONSIDERED AS REFERENCE ONLY TO GUIDE THE CONTRACTOR IN THE PREPARATION OF CONSTRUCTION SCHEDULE BY PERT-CPM.
- ACTUAL COMMENCEMENT DATE OF THE PROJECT, WHICH WILL BE DIFFERENT FROM THAT INDICATED IN THE TENTATIVE CONSTRUCTION SCHEDULE SHALL NOT BE CONSIDERED AS BASIS FOR CLAIM OF TIME EXTENSION.

ITEM NO.	WORK ITEM	QTY.	UNIT		_	_						
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1.0	MOBILIZATION AND PREPARATORY WORKS			Ħ	Ţ	Ħ		T	11	T		-
2.0	ERECTION OF CONCRETE BATCHING PLANT & TEMPORARY FACILITIES				+	Н				—		-
3.0	CONSTRUCTION OF ACCESS ROADS	5680	SM	4	-						4	_
4.0	CONSTRUCTION OF APPRUACH SPANS				_	1		_			\square	
4.1	MANILA SIDE (5 @ 30m + 5 @ 30m + 4 @ 30m)				_	1						_
	4.1.1 ABUTMENT 1											
	4.1.2 BORED PILE CONSTRUCTION FOR PIERS	1560	LM								\bot	
	4.1.3 CONSTRUCTION OF PILE CAP	13	Pcs.									
	4.1.4 CONSTRUCTION OF COLUMNS	13	Pcs.	H	T				П	Π	TΠ	
	4.1.5 CONSTRUCTION OF COPING	13	Pcs.	П	Τ						17	
	4.1.6 FABRICATION OF PC AASHTO GIRDERS	56	Pcs.	П	Τ	T		Ŧ	T	Π		
	4.1.7 LAUNCHING OF PC AASHTO GIRDERS	56	Pcs.	П	1			-	T	ht		
	4.1.8 DECK SLAB WORKS	4053	SM	1-1	1	Ħ		+		H	T	
4.2	NUEVA ECIJA SIDE (5 @ 30m + 5 @ 30m)		· ·		T	+						Γ
	4.2.1 ABUTMENT 2	+	• •	1†	+	++			$\uparrow \uparrow$	1-+	T	
	4.2.2 BORED PILE CONSTRUCTION FOR PIERS	972	LN		t		H	+	1-	†- †•	1	Γ
	4.2.3 CONSTRUCTION OF PILE CAP		Pca,		t	+-		-	-1-1	╏╼┼	+	
	4.2.4 CONSTRUCTION OF COLUMNS		Pcs.		+			-	+-1		-	
	4.2.5 CONSTRUCTION OF COPING	9	Pcs.	1+	ϯ	+	┝─┼	-†-	+-	╉	-	
	4.2.6 FABRICATION OF PC AASHTO GIRDERS	40	Pcs.	11	+	+	┢┤			+	+	
	4.2.7 LAUNCHING OF PC AASHTO GIRDERS	40	Pcs.	\mathbf{H}	+-		┨╼┥		+-1	- +	+	
	4.2.8 DECK SLAB WORKS	2895	SM	H	-†-	+	} -†	-+-	+	t-t	+	
5.0				11	+	H	H	+	+	H	+	
0.0	5.1 INSTALLATION CRANEWAY	420	LM	11	+	H	H	+	+	\vdash	++	-
	5.2 TEMPORARY COFFERDAMS	8	SETS	1+	+	+	H	+	+	+	-	
	5.3 BORED PILE CONSTRUCTION	1188	LN		+	+	H	+	+	H		-
	5.4 PILE CAP CONSTRUCTION	8	Pcs.		+	+	H	+	+	H		-
	5.5 PIER/COLUMN CONSTRUCTION	8	Pcs.	╉┼	+	+		+	+	H		
	5.6 PIER HEAD CONSTRUCTION	6	Pcs.	╉┽	+	+				┢┼╡	+-	-
	5.7 CONSTRUCTION OF SIDE COPING	2	Pcs.	╉┼	+	+	$\left \right $	+	+	╞┼		þ
	5.8 CONSTRUCTION OF SUPERSTRUCTURE (SIDE SPAN)	43	LM	ł-ł	╉	+	┞╌┤	+	┿┥	┼╍┼	+-	-
	5.9 CONSTRUCTION OF SUPERSTRUCTURE (BALANCED CANTILEVER CONSTRUCTION)	357	LN	┢┤	+	+	H	+	+	╀	+-	<u> </u> -
6.0	RAILING WORKS	- 33/	CM	┢┈┼	+	+	+	-	+-	╆╋		⊢
0.0	6.1 RAILING INSTALLATION - MANILA SIDE APPROACH SPAN	840	LN				$\left \cdot \right $	-+-	+	┠┼	+	H
	6.2 RAILING INSTALLATION - NUEVA ECIJA APPROACH SPAN	600	LM	+	+	+	$\left \cdot \right $	-+-	-+	H	-+-	h
	6.3 RAILING INSTALLATION - MAIN BRIDGE SECTION	800	LM	╉	+	++	\mathbb{H}	-+·	- +	₽ŀŀ		┝
7.0	BRIDGE SURFACE WORKS	auu	LN	₽	+	+	$\left \right $	+	+1	┢╋		
	7.1 ASPHALT OVERLAY - MANILA SIDE APPROACH SPAN			\mathbb{H}	+	+	\square	+	+	₽	-	┝
<u>├</u> · · ·	7.1 ASPHALT OVERLAT - MANILA SIDE APPROACH SPAN 7.2 ASPHALT OVERLAY - NUEVA ECIJA APPROACH SPAN	3465	SM	Ηİ	+	-+	$\left - \right $	+	+-	+	+	\vdash
· · ·		2475	SM	+	+			+		┿┦	+	L
	7.3 ASPHALT OVERLAY - MAIN BRIDGE SECTION	3300	SM	H	+	+		+	+	++		1
8.0	RIVER AND PIER PROTECTION WORKS	1	lot	₽	+	+	\square	+	+	∔	-	-
9.0	MISCELLANEOUS WORKS	+	lot	┨┥	+	-+-	₋	+	╇	╞┥	+	-
10.0	CLEANING AND DEMOBILIZATION	<u></u>	lot	1	⊥	1	1		لل	Ц	느.	i.



	IIIER		DATE	SUMNATURE			REPUBLIC O
		DESIGNED	9/25/02	A SUCK		DEPARTMEN	T OF PUB
	JAPAN INTERNATIONAL COOPERATION AGENCY			1	PJHL ~ PMO	BUREAU	OF DESIGN
		CHECKED	9/27/02	~	Submitted By:	Reviewed By:	Recommended B
			7/4/100	J.O. SANTOS	-		1
	KATAHIRA & ENGINEERS YOO YACHIYO ENGINEERING CO_ LTD.	SUBAITTED	ahoke	(h. Kitch		ADRIANO M. DOROY	GILBERTO S
		i i	7//	TEAM LEADER	Project Director	Chief, Bridges Division	Director N
<u> </u>							

NOTE :

THIS CONSTRUCTION SCHEDULE IS LISED FOR REFERENCE ONLY THE CONTRACTOR SHOULD PREPARE A MORE DETAILED CONSTRUCTION SCHEDULE CONSIDERING PLANT CAPACITIES, EQUIPMENT CAPACITY/ AVAILABILITY, LABOR CONDITIONS, CLIMATE, WORKING DAYS, ETC. FOR THE APPROVAL OF THE ENGINEER.

TENTATIVE OVERALL CONSTRUCTION SCHEDULE OF ANGAT RIVER BRIDGE CROSSING