

TYPICAL CROSS-SECTION SCALE

16748

168.42

4250

cu. m.

55.44 11.18

53.19

13.39

APPROACH SLAB

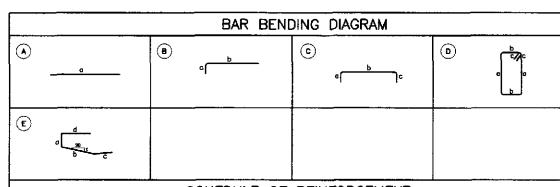
DECK SLAB

DIAPHRAGN GIRDER

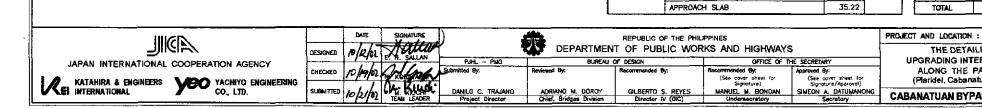
STRUCTURAL CONCRETE

SIDEWALK, RAILING, & POST

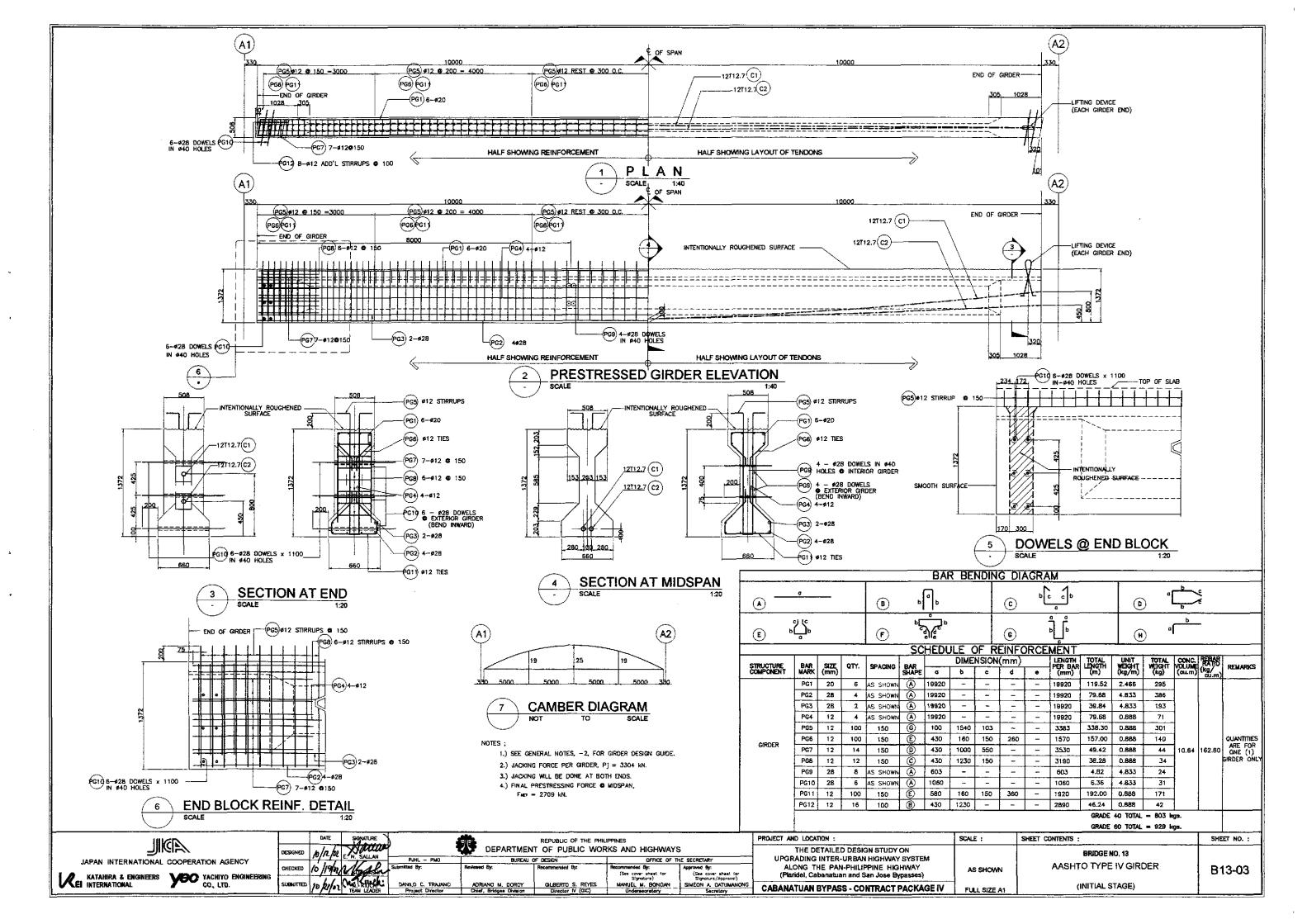
405(1)

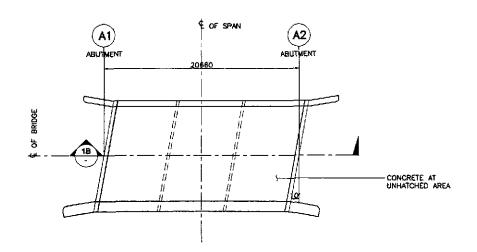


LOCATION	CONCRETE VOLUME	8AR	BAR SIZE	QTY.	SD4OMO	BAR	DIMENS	IONS (m	m) 0UT	TO OUT	LENGTH EACH BAR	TOTAL LENGTH	UNIT WT.	WEIGHT	REBAR
LOCATION	(nr)	MARK	SIZE	WIT.	SPACING	SHAPE	a	Ь	c	ď	(mm)	(m)	(kg/m)	IN (leg)	REBAR RATIO (kg/m ³
		Gt	16	10	AS SHOWN	(8)	19900	-	-	-	19900	199.00	1.579	315	
		S1	16	61	300	©	145	11600	145	-	11890	725.29	1.579	1146]
		Sta	16	14	300	©	145	6590	145	-	6880	96.32	1.579	153	
		S2	16	122	300	B	145	2000		-	2145	261.69	1.579	414	
		S2a	16	183	300	(A)	1700	-		_	1700	311.10	1.579	492]
		S2b	16	244	300	(A)	1950	_	-	-	1950	475.80	1.579	752]
	55.44	53	16	61	300	(A)	11600	-	-	-	11600	707.60	1.579	1118	1
		S3a	16	14	300	(A)	5590	-	-	-	6590	92.26	1.579	146	1
DECK SLAB		S4	16	48	150	(A)	19900	-	-	-	19900	955.20	1.579	1509	164.70
		S5	16	48	150	(A)	19900	-	_	_	19900	955.20	1.579	1509	1
		\$6	16	12	AS SHOWN	(A)	19900	_	-	-	19900	238.80	1.579	378	1
		57	16	12	AS SHOWN	(8)	19900	-	_	-	19900	238.80	1.579	378	1
		58	16	20	AS SHOWN	(E)	11780	-	_	-	11780	235.60	1.579	373	1
İ	1	\$9	16	28	AS SHOWN	(8)	6590	-		-	6590	184.52	1.579	292	1
		S10	12	90	450	(E)	145	90D	600	300	1945	175.05	0.888	156]
TOTAL	55.44			•							GF	ADE 40	TOTAL =	9,131	KOR.



SHEET CONTENTS : SCALE : SHEET NO. : THE DETAILED DESIGN STUDY ON BRIDGE NO. 13 UPGRADING INTER-URBAN HIGHWAY SYSTEM DECK FRAMING PLAN ALONG THE PAN-PHILIPPINE HIGHWAY B13-02 AS SHOWN (Plaridel, Cabanatuan and San Jose Bypasses) AND SECTIONS (INITIAL STAGE) CABANATUAN BYPASS - CONTRACT PACKAGE IV FULL SIZE A1



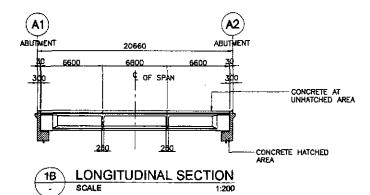


PLAN

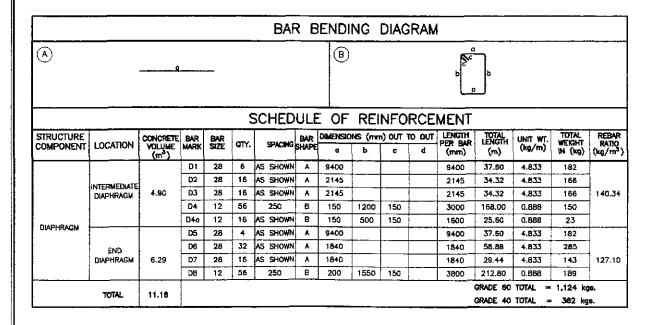
SCALE 1:200

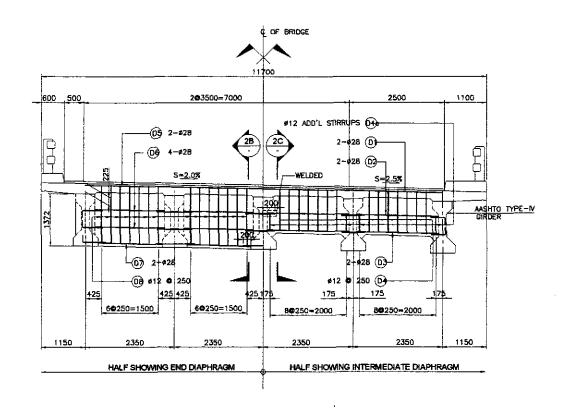
MOTES

- CONCRETE AT HATCHED AREAS SHALL BE PLACED AT LEAST TWENTY ONE (21) DAYS AHEAD OF CONCRETE AT UNHATCHED AREAS.
- 2. SEE GIRDER DETAILS FOR SPACING OF #28 DOWELS.

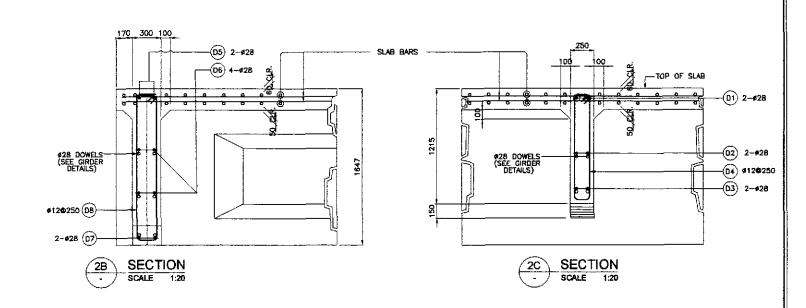


1 CONCRETE POURING SEQUENCE 1:200

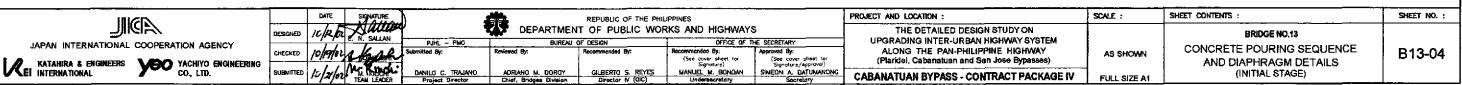


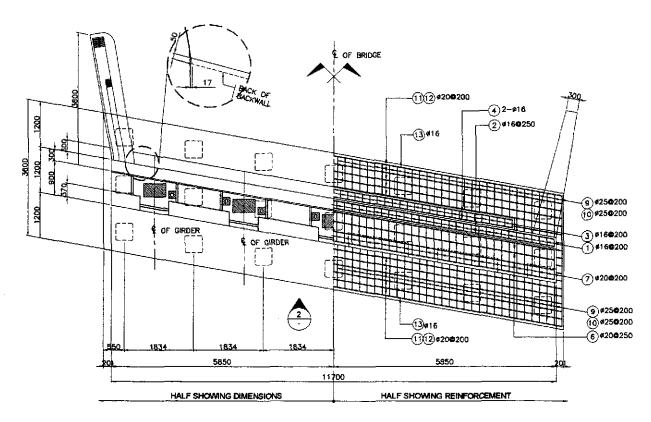


ELEVATION
SCALE 1:50

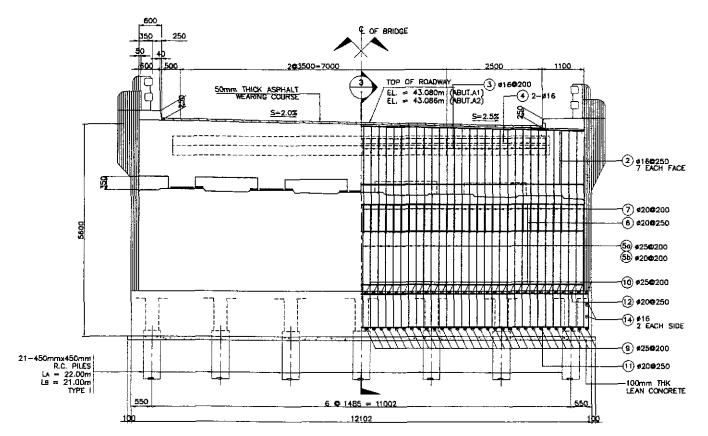


2 DETAIL OF END & INTERMEDIATE DIAPHRAGM
- SCALE AS SHOWN

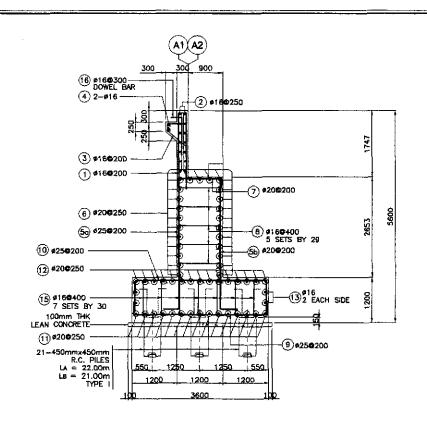








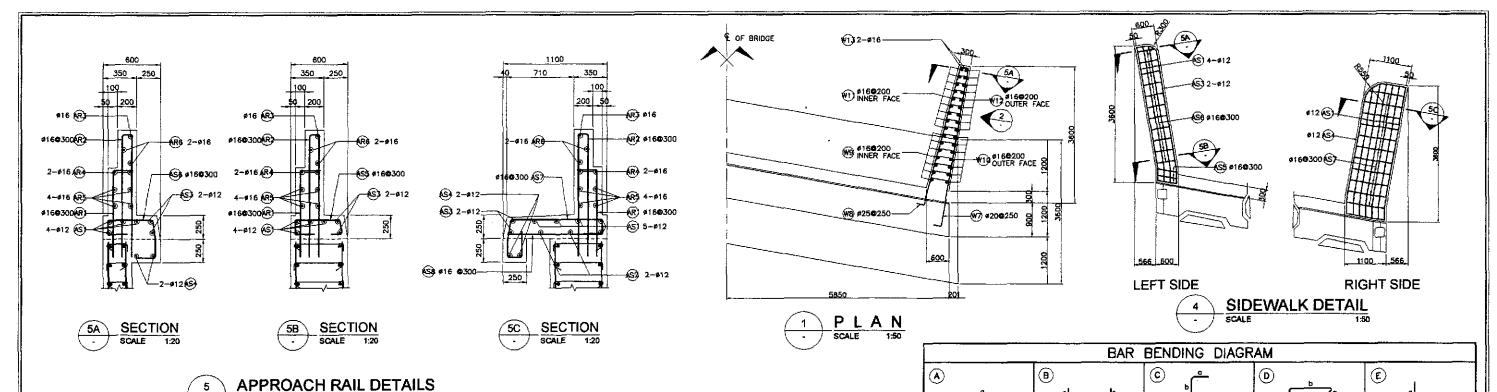




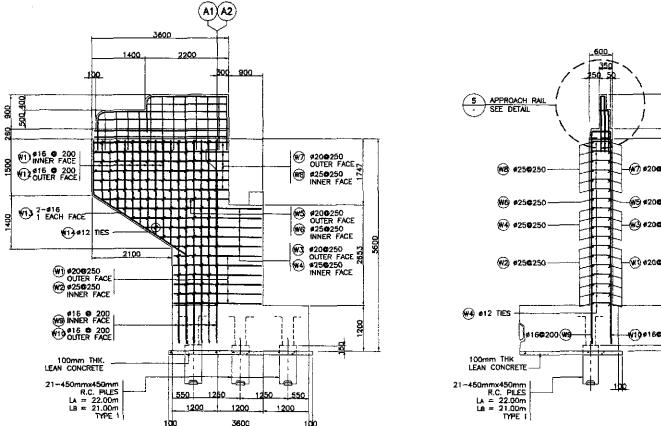
3 SECTION
- SCALE 1:50

						BAR	BE	INDI	NG [DIAG	RAM						
(A)	<u>a</u>		B) ∘(ь	Jc		©	٥	-		Ð	- ا	<u>ь</u>		Ē)	ь	_
F bC	_ و		e هر	<u> </u>			(1)	م 200	ە ئې		0	اله	_ a		<u>J</u>	ا ر	
		S	CHE	DUL	<u>E 01</u>	- R	EIN	FOR	CEME	NT	PER	AB	UTME	NT			
LOCATION	CONCRETE VOLUME (m ³)	BAR MARK	BAR SIZE	QTY.	SPACING	BAR SHAPE	g D	IMENSIO b	NS (mm	d d	TO OUT	f	LENGTH EA BAR (mm)	TOTAL LENGTH (m)	UNIT WT. (kg/m)	WEIGHT (kg)	REBAR RATEO (kg/m)
_		1	16	59	200	(B)	2000	200	200G		-	-	4200	247.80	1.579	392	1.000
BACKWALL	7.25	2	16	14	250	(A)	11800			-	-	-	11800	165.20	1.579	261	111.07
DACKMALL	,,,20	3	16	51	200	©	600	150	750		~	-	1500	76. 5 0	1.579	121	171.07
		④	16	2	as shown	(A)	10050	-	-	_		-	10050	20.10	1.579	32	
		59	25	59	200	<u>(E)</u>	400	3600	_	_			+000	236.00	3.854	910	
		€	20	59	200	<u>(E)</u>	400	3500	-				4000	236.00	2.466	582	
MAINWALL	37.25	6	20	25	250	<u>(A)</u>	11800		~	_			11800	295.00	2.466	728	75 .71
		0	20	59	200	<u> </u>	250	1100	250	_			1600	94.40	2.466	233	
		<u>®</u>	16	145	400	<u> </u>	250	1100	250		-		1600	232.00	1.579	367	
		9	25	- 61	200	<u>B</u>	700	3450	700				4850	295.85	3.854	1141	
		100	25	61	200	<u> </u>	700	3450	700				4850	295.85	3.854	1141	
	52.28	12	20	15	250 250	<u>B</u>	700	12200	700		<u>-</u>		13600	204.00	2.466	504	74.67
FOOTING	32.20	(3)	16	4	AS SHOWN	(A)	12200		700				12200	48.80	1.579	78	74.07
		(A)	16	4	AS SHOWN	Ã	3450				- -	_	3450	13.80	1.579	22	
		(15)	16	210	400	<u> </u>	250	1050	250				1550	325.50	1.579	514	
DOWEL		(iii)	16	34	300	Ē	650	500	-		-	_	1150	39.10	1.579	62	
TOTAL	96.79					<u> </u>								40 TOTAL 60 TOTAL		49 kgs. 13 kgs.	

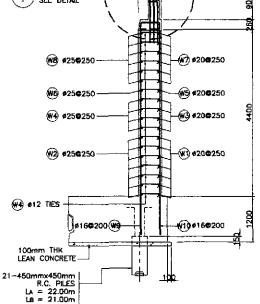
III/GFD		DATE	SIGNATURE			REPUBLIC OF THE PHA	LIPPINES		PROJECT AND LOCATION :	SCALE :	SHEET CONTENTS :	SHEET NO. :
JAPAN INTERNATIONAL COOPERATION AGENCY	DESIGNED	10/2/11	GONZALES	Puhl — PMO	BUREAU	OF DESIGN	411114	HE SECRETARY	THE DETAILED DESIGN STUDY ON UPGRADING INTER-URBAN HIGHWAY SYSTEM		BRIDGE NO. 13 ABUTMENT A1 & A2	
KATAHIRA & ENGINEERS YACHIYO ENGINEERING CO., LTD.	CHECKED	10 19 21 g	A SUL	Submitted By:	ADRIANO N. COROY	Recommended By:	Recommended By: (See cover sheet for Signature) MANUEL M. BONDAN	Approved By: (See cover sheel for Signoture/Approval) SIMEON A. DATLIMANONG	ALONG THE PAN-PHILIPPINE HIGHWAY (Plaridel, Cabanatusan and San Jose Bypasses)	1:50	MAINWALL REINFORCEMENT DETAILS	B13-05
CO., LTD.	SUBMITTED	ojzijoi "	EAN LEADER	Project Director	Chiet, Bridges Division	Director IV (OIC)	Undersecretary	Secretory	CABANATUAN BYPASS - CONTRACT PACKAGE IV	FULL SIZE A1	(INITIAL STAGE)	



APPROACH RAIL DETAILS SCALE



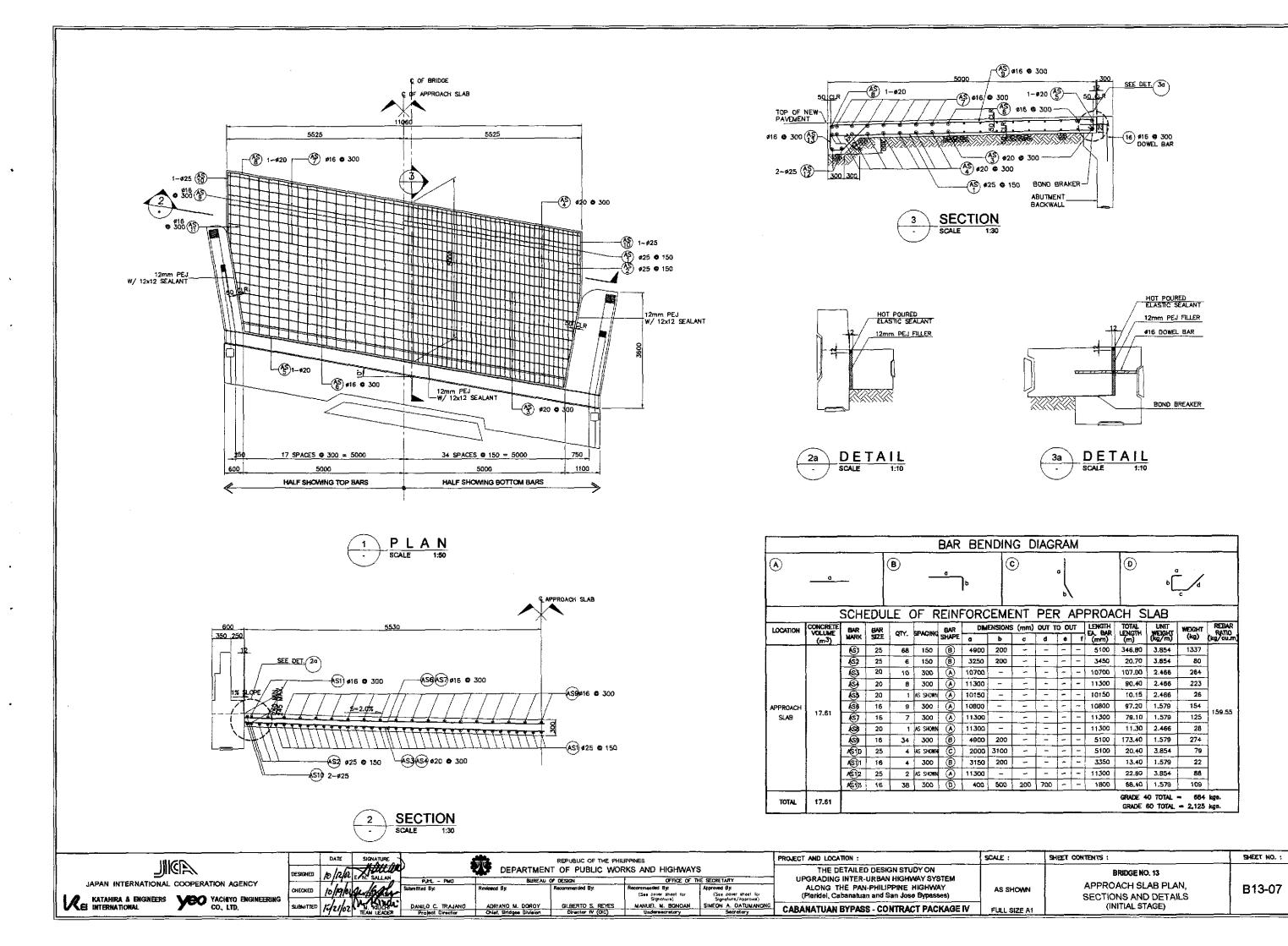
WINGWALL ELEVATION

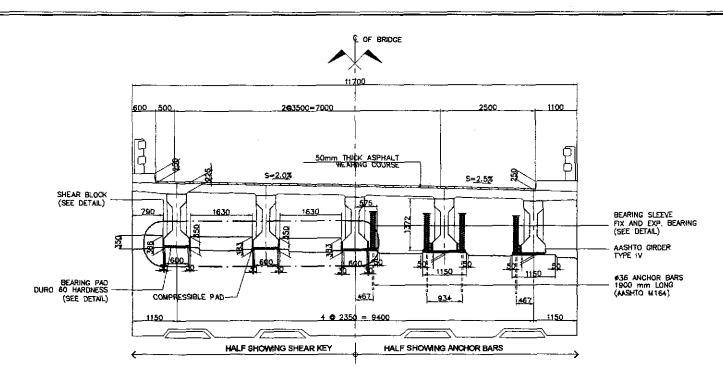


SECTION

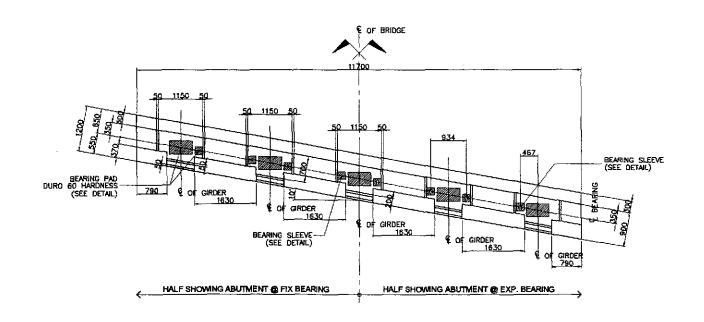
F L	d	(G _{b(}	a 6	{q}•		H)	200 f	ا ا		D	- °		J (Ū	٠,٠	
	5	CHE	DUL	E C	FR	EINF	ORG	EME	ENT_	PEF	₹ A	Вι	JTMEN	VT.			
LOCATION	CONCRETE VOLUME (m3)	BAR	BAR SIZE	QTY.	SPACING	BAR SHAPE	a Di	MENSION	kS (mm	OUT d	TO CA	Л f	LENGTH EA BAR (mm)	TOTAL LENGTH (m)	UNIT WT. (kg/m)	WEIGHT (kg)	REBAR RATIO (kg/m ³
	, (III-)	((W))	20	12	250	(B)	400	2300	150	_	_	-	2850	34.20	2.466	85	(19/111-
		<u>@</u>	25	12	250	(B)	400	2300	150		-	-	2850	34.20	3.854	132	1
		(v3)	20	10	250	₿	400	3100	150	-	-	-	3650	36.50	2.466	Ė	1
]		₩ →	25	10	250	<u>B</u>	400	3100	150	-	-	-	3650	36.50	3.854	141	
		45	20	2	250	₿	400	3350	150	_	-	-	3900	7.80	2.466	20]
		₩	25	2	250	B	400	3350	150	-	-	-	3900	7.80	3.854	31]
		(20	12	250	(8)	400	3500	150	-	- :	-	4050	48.60	2.468	120]
INGWALL	8.89	₩8	25	12	250	₿	400	3500	150	_	-	-	4050	48.60	3.854	188	150.94
		. ₩9	16	12	250	Œ	250	5350	-]	_	_	-	5600	67.20	1.579	107]
		(2)	16	12	250	(E)	250	5350			-	_	5600	67.20	1,579	107	
		(2)	15	20	250	E	250	2100			-	_	2350	47.00	1.579	75]
		٩	16	20	250	E)	250	2100		_	-	-	2350	47.00	1.579	75]
	I	*	16	4	as shown	©	250	1500	3000	-	-	-	4750	19.00	1.579	31	j
į		WB.	12	198	as shown	(D)	170	450	170	-	-	-	790	156.42	0.888	139	<u> L</u>
i															O TOTAL O TOTAL	= 808 = 534	kgs. kgs.
		(S)	12	9	AS SHOWN	(A)	3500	-			-	-	3500	31.50	0.888	28	Г
		(S)₂	12	2	AS SHOWN	Ā	3500	-	_	_	-	-	3500	7.00	0.888	7	1
		Š	12	4	AS SHOWN	Ā	3500	_		_	-	-	3500	14.00	0.888	13	1
		(S)	12	4	AS SHOWN	(A)	3500			_	-	-	3500	14.00	0.888	13	1
		© ©	16	3	300	Ē	200	170	08#	200	200	-	1250	3.75	1.579	6	1
		Š	16	11	300	Õ	200	170	480	200	170	200	1420	15.62	1.579	25	1
		(S)	16	14	300	Θ	200	170	980	200	170	200	2120	29.68	1.579	47	
		(3)	16	14	300	E	200	1020	-	-	-	-	1220	17.08	1.579	27	98.34
PPROACH RAILING	7.57	æ	16	8	300	E	200	900	-	-	-	-	1100	8.80	1.579	14	90.34
AND SIDEWALK	3.53	₽.	16	16	300	(1300	120	1300	-	-	-	2720	43.52	1.579	69	
		₽3	16	2	AS SHOWN	0	2100	236	1300		-	ţ	3636	7.27	1.579	12	
		(R)	16	4	as shown	\odot	3400	236	900		_	<u> </u>	4536	18.14	1.579	29	_
		₽	16	8	as shown	③	3400				-	-	3400	27.20	1.579	43]
		₽86	16	4	as shown	<u> </u>	2100				-	-	2100	8.40	1.579	14	<u> </u>
i			1											GRADE 4	O TOTAL	= 347	kgs.

IIIGD	DATE SIGNATURE		REPUBLIC OF THE PHILIPPINES		PROJECT AND LOCATION :	SCALE :	SHEET CONTENTS :	SHEET NO. :
JAPAN INTERNATIONAL COOPERATION AGENCY	DESIGNED 10 R R P. GONZA	ES PJHL - PWO BUREAU	NT OF PUBLIC WORKS AND HIGHWA'	THE SECRETARY	THE DETAILED DESIGN STUDY ON UPGRADING INTER-URBAN HIGHWAY SYSTEM		BRIDGE NO. 13	
	CHECKED 10/19/02	Submitted By: Reviewed By:	Recommended By: (See cover sheet for Signature)	Approved By: (See cover sheel for Signature/Approval)	ALONG THE PAN-PHILIPPINE HIGHWAY (Ptaridel, Cabanatuan and San Jose Bypasses)	AS SHOWN	ABUTMENT A1 & A2 WINGWALL REINFORCEMENT DETAILS	B13-06
CO, LTD.	SUBMITTED 10/21/01 M. RICCHT	DANILO C. TRAJANO ADRIANG M. BORDY Project Director Chief, Bridges Division	GLBERTO S. REYES MANUEL M. BONDAN Director N (CIC) Undersecretory	SIMEON A. DATUMANONG Secretory	CABANATUAN BYPASS - CONTRACT PACKAGE IV	FULL SIZE A1	(INITIAL STAGE)	

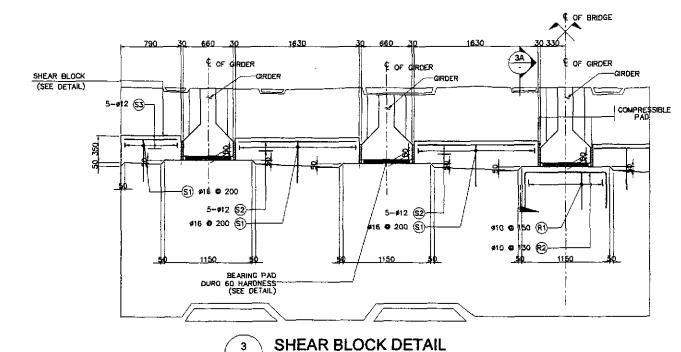


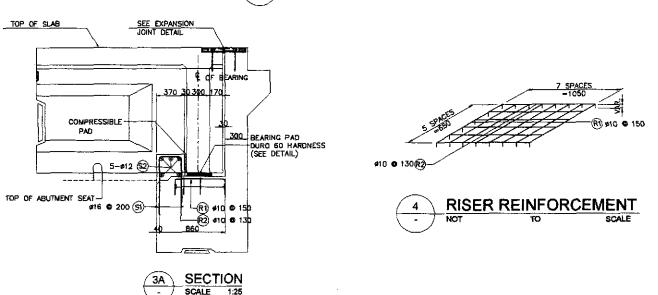


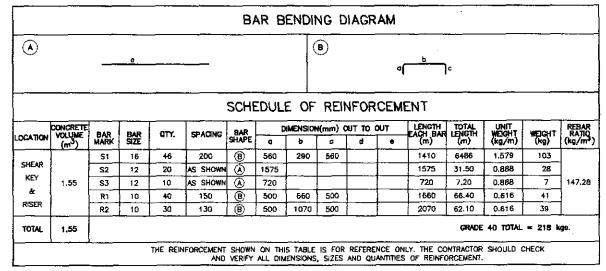
SECTION AT ABUTMENT SEAT 1:50

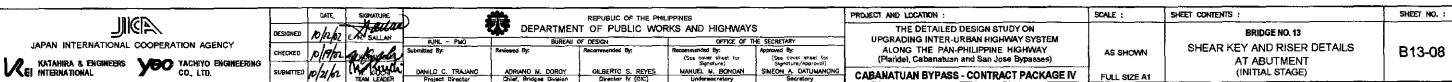


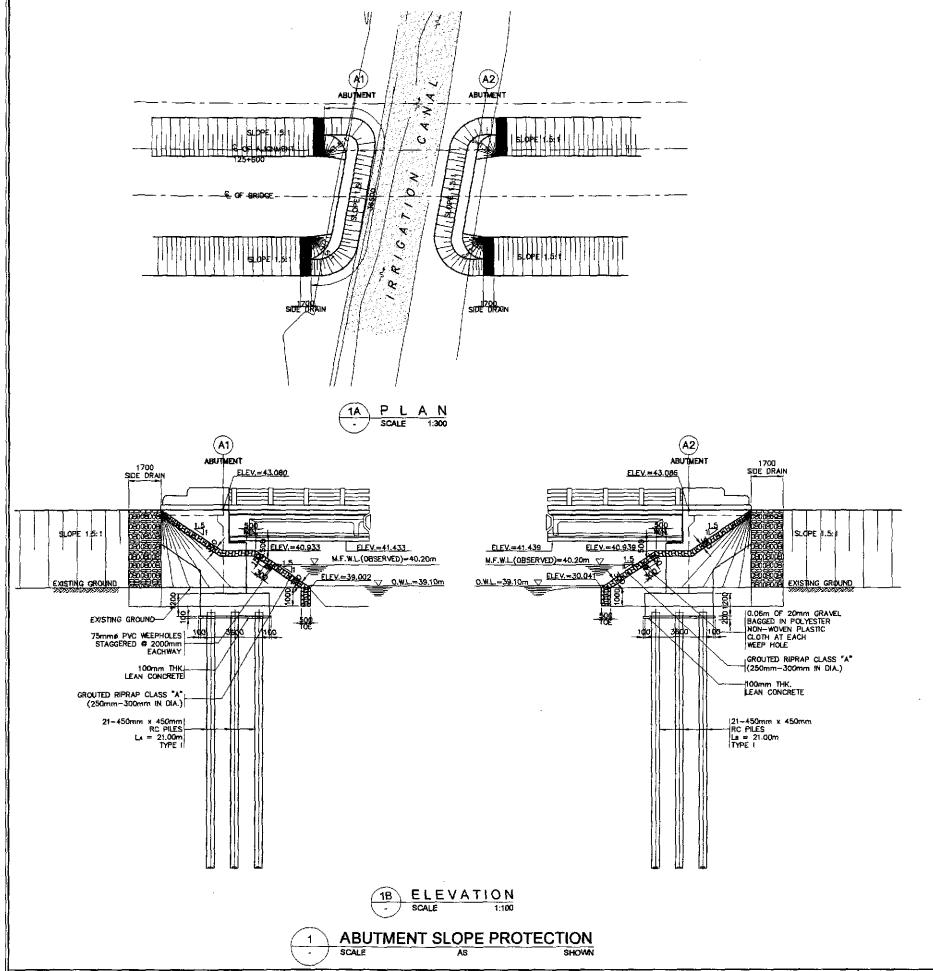
PLAN AT ABUTMENT SEAT
SCALE 1:50











GENERAL NOTES:

- 1. GROUTED RIPRAP (250mm-300mmDIA.) SHALL BE USED FOR THE FACING AND SHALL BE CAREFULLY HANDLAID WITH THE LONGEST DIMENSIONS PERPENDICULAR TO THE SLOPE AND FIRMLY BEDDED INTO THE SLOPE AND ADJACENT TO THE ADJOINING BOULDERS SPACED BETWEEN THE BOULDERS. THE SPACE BETWEEN THE BOULDERS SHALL BE COMPLETELY FILED WITH MORTAR. THE OUTSIDE SURFACE OF THE BOULDERS SHALL BE LEFT EXPOSED AND THE SURFACE OF THE MORTAR SHALL BE LIFET INTO A CITED SPOOM.
- 2. GEOTEXTILE
- THE FOLLOWING SPECIFICATIONS ARE REQUIRED:

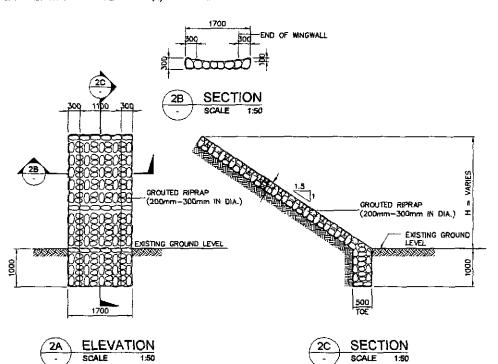
 1. POLYESTER OR POLYPROPELINE 100%
- 2. MECHANICALLY BONDED/HEAT BONDED 3. NON-WOVEN
- 6. WEIGHT 200g/sq. m. (MIN.)
 7. CBR PLINCTURE STRENGTH 400N (MIN.)
- 8. MULTI-DIRECTIONAL TENSILE STRENGTH 13KN/m

5. THICKNESS UNDER PRESSURE - 0.80mm (MIN.)

- GRAVEL FILTER SHALL BE COARSE AGGREGATES MATERIALS WHICH SATISFY THE REQUIREMENTS FOR ITEM 405, STRUCTURAL CONCRETE, GRADING B OF TABLE 405.1 AS REVISED.
- 4. NO CONCRETING UNDER WATER SHALL BE PERMITTED.

4. EFFECTIVE OPENING SIZE - 110 MICRONS (MAX.)

5. PROVIDE 1.0 m BERM WHEN HEIGHT (H) IS > 4.0 m.



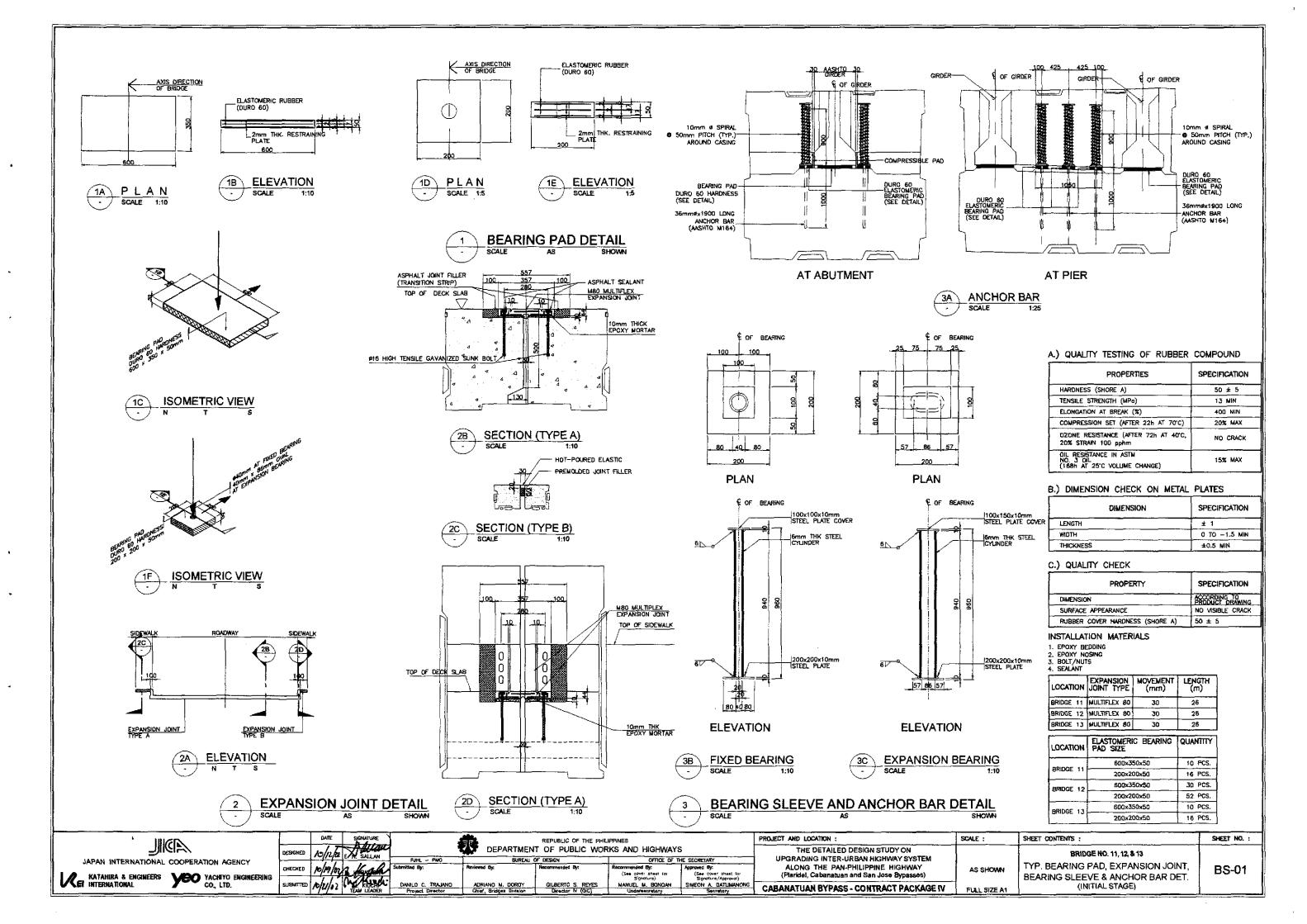


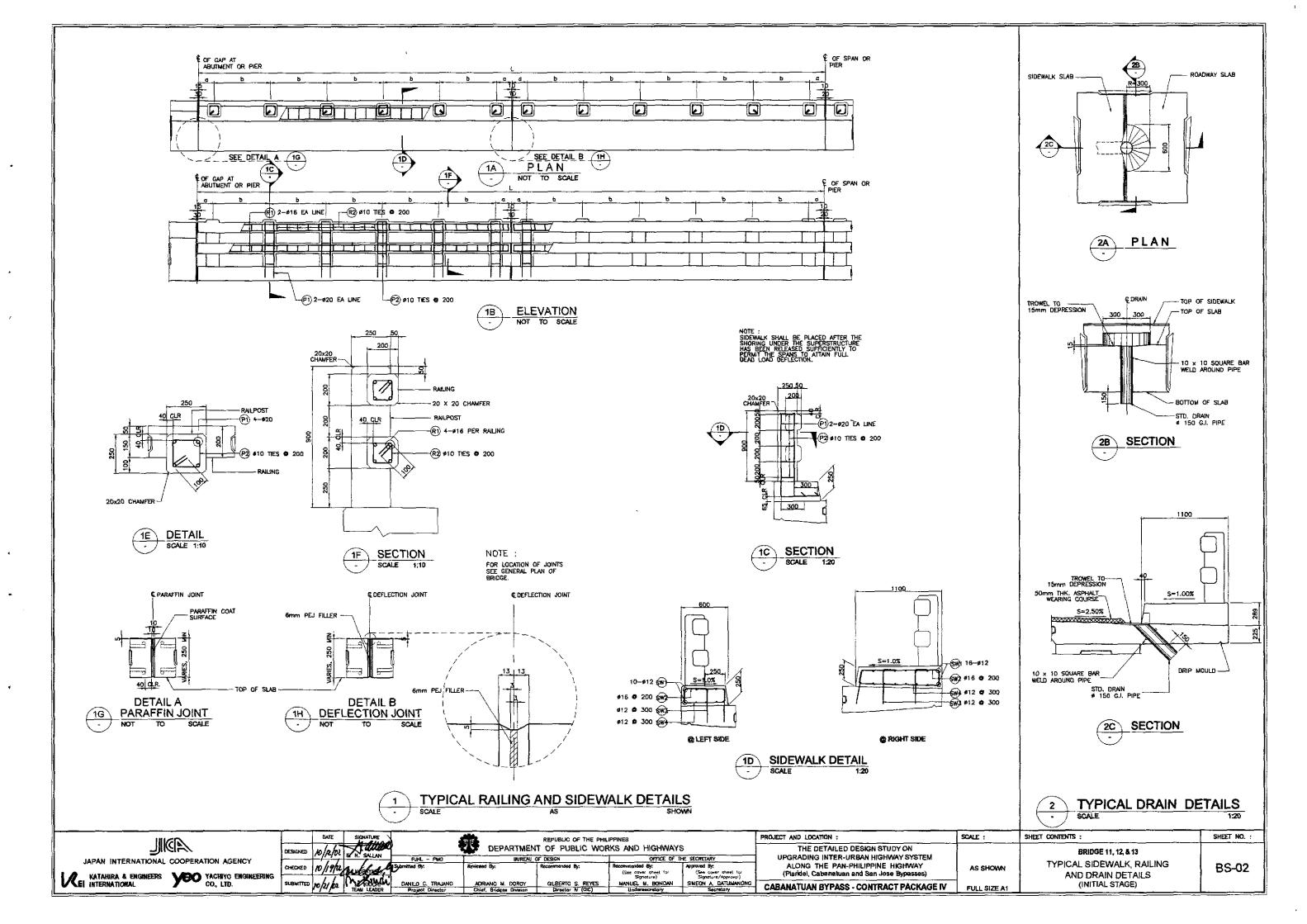
AETOCIL A	ROCK SIZE (n	nm)
(m/sec)	VERY TURBULENT FLOW	SMOOTH FLOW
1.00	40	-
1.50	135	-
2.00	170	-
2.50	255	137
3.00	370	197
3.50	515	270
4.00	690	350
4.50	825	425
5.00	>900	590

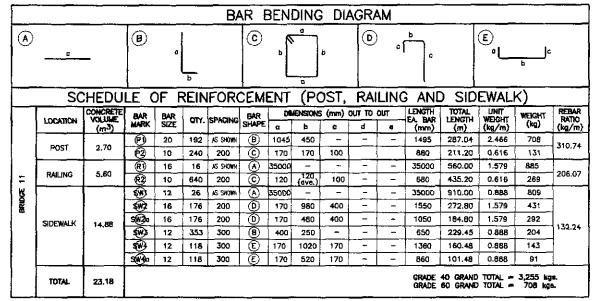
PER ABUTMENT

LOCATION	SIZES	QUA	YTITA
	3723	ABUT. A1	ABUT. A2
SIDE DRAIN	200mm-300mm IN DIA.	9.06 cu. m.	9,06 cu. m.
GROUTED RIPRAP	250mm-300mm IN DIA.	50.29 cu, m.	50.28 cu. m

	SCALE AS SHOWN		
IIIGN	DATE SIGNATURE REPUBLIC OF THE PHILIPPINES PROJECT AND LOCATION :	SCALE :	SHEET CONTENTS : SHEET NO. :
	DEPARTMENT OF PUBLIC WORKS AND HIGHWAYS THE DETAILED DESIGN STUDY ON UPGRADING INTER-URBAN HIGHWAY SYST.	EM	. BRIDGE NO. 13
JAPAN INTERNATIONAL COOPERATION AGENCY KATAHIRA & ENGINEERS VSC YACHIYO ENGINEERING	E Print Submitted By: Recommended By: Recommended By: Recommended By: Recommended By: Approved By: Approved By: ALONG THE PAN-PHILIPPINE HIGHWAY (See cover sheet for Signifure) Signifure/Approval) (Planidel, Cabanatuan and San Jose Bypasses		ABUTMENT PROTECTION B13-09
KATAHIRA & ENGINEERIS YACHIYO ENGINEERING CO, LTD.	DATES TEAM LEADER Project Director Chief, hydroxides Unidion (DIC) Director N (CIC) Undersecretory Secretary CABANATUAN BYPASS - CONTRACT PACK	AGE IV FULL SIZE A1	(INITIAL STAGE)







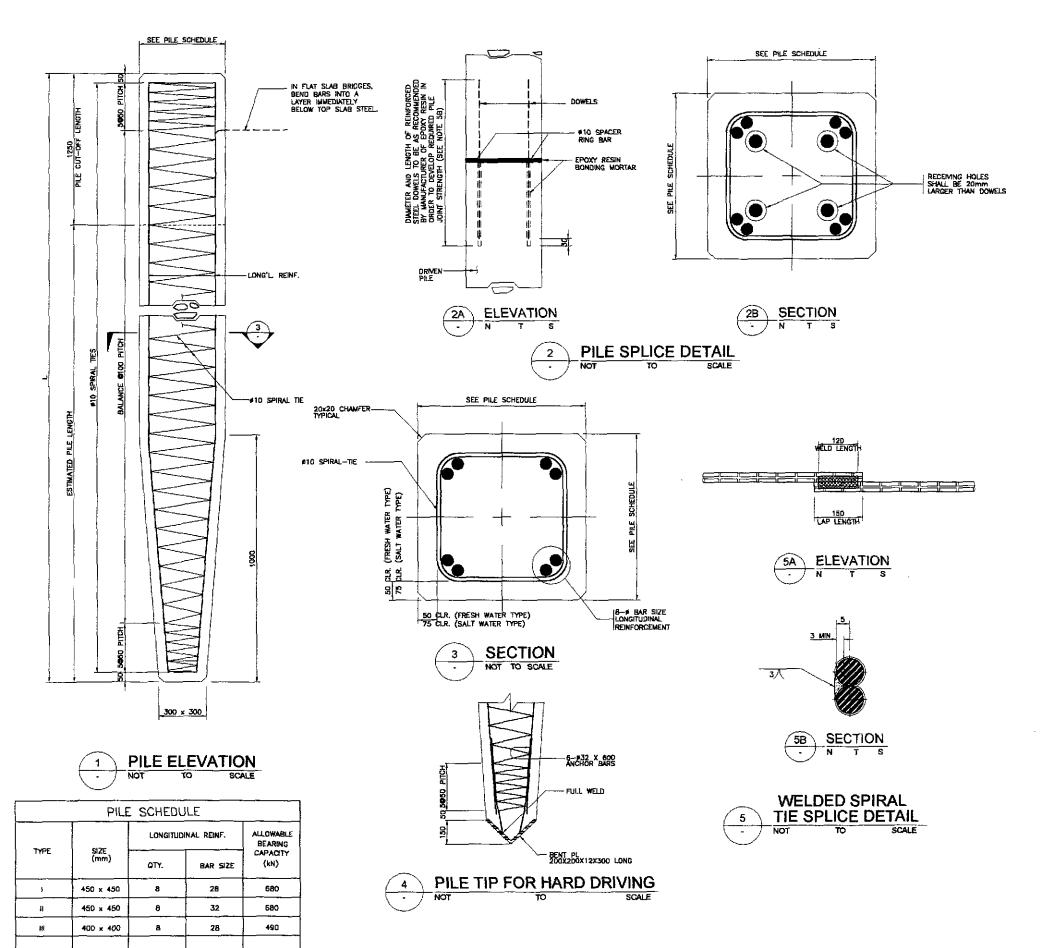
	TOTAL.	23.18											GRADE	60 GRAND	TOTAL -	708 kg	B.
						B/	AR E	BEN	DING	DIA	\GR/	\M					
•	0	_	В	a	Ь		©	١		b	0	a	c c		Ē.	- b	
	SC	HEDU	LE (OF F	REIN	FOR	CEM	ENT	(PC	ST,	RAI	LINC	ANE	SID	EWAL	K)	
	LOCATION	CONCRETE VOLUME (m ³)	BAR MARK	BAR SIZE	QTY.	SPACING	BAR SHAPE		(ENSIONS	(mm)	0T TU	OUT	LENGTH EA BAR (mm)	TOTAL LENGTH (m)	UNIT WEIGHT (kg/m)	WEIGHT (kg)	REBAR RATIO (kg/m)
	_		(9)	20	120	AS SHOWN	(8)	1045	450		-	-	1495	179.40	2.466	443	311.11
	POST	1.69	(2)	10	150	200	©	170	170	100			880	132.00	0.616	82	
	RAILING	3.20	(R)	16	16	AS SHOWN		20000	-	-			20000	320.00	1.579	506	205.94
12			€2	10	364	200	<u>©</u>	120	120	100			680	247.52	0.616	153	
BRIDGE			₩)	12	26	AS SHOWN	<u>(A)</u>	20000					20000	520.00	0.888	462	4
&			₩	16	101	200	0	170	980	400			1550	156.55	1.579	248	4
	SIDEWALK	8,50	5(W)3	16	203	300	0	170	480	400			1050 650	106.05	1.579 0.888	168	133.05
l			\$ ₩	12	20,3	300	(B)	400 170	250 1020	170	<u> </u>		1360	92.48	0.888	63	1
			SW40	12	68	300	É	170	520	170			860	58.48	0.888	52	1

						B/	\R E	3ENI	DING	DI/	AGR/	<u>M</u>					
(A)	a	_	B	a	d		©	b		b	0	a (c c		E L	с	
	SC	HEDU	LE (OF F	REIN	FORG	CEM	ENT	(PC	DST,	RAI	LINC	ANI	SID	EWAL	K)	
	LOCATION	CONCRETE VOLUME (m ³)	BAR MARK	BAR SIZE	QTY.	SPACING	BAR SHAPE	DNA a	ENSIONS b	(mm)	OT TUO	our	LENGTH EA. BAR (mm)	TOTAL LENGTH (m)	UNIT WEIGHT (kg/m)	WEIGHT (kg)	REBAR RATIO (kg/m
			ම	20	576	AS SHOWN	(B)	1045	450	-	-	-	1495	861.12	2.466	2124	310.52
ļ	POST	8.10	@	10	720	200	©	170	170	100			880	633.60	0.616	391	310.52
- 1	RAILING	16.00	®	16	16	AS SHOWN	(A)	10180				_	101800	1628.80	1.579	2572	210.31
2			®	10	1892	200	(၂)	120	120	100		-	680	1286.56	0.616	793	210.5
岌				12	26	AS SHOWN		10180	<u> </u>			_	101800	2646.80	0.888	2351	
BRIDGE		!	_\$₩ 2	16	510	200	0	170	980	400	-		1550	790.50	1.579	1249	
_	SIDEWALK	43.27	S(¥2)o	16	510	200	0	170	480	400			1050	535,50	1.579	846	Ţ
	SIDEMNO] ''	\$ ₩)	12	1020	300	₿	400	250			-	650	663.10	0.888	589	131.B8
			*	12	340	300	(E)	170	1020	170		-	1360	462.40	0.888	411	
Į			SW40	12	340	300	E	170	520	170	_	_	860	292_40	0.888	260	

RAILING FOR BRIDGES

8RIDGE NO.	SPAN LENGTH (m)	NO. OF EXP. JT. INSIDE SPAN	NO. OF POST W/IN EXP. JT.	NO. OF RAIL POST PER SPAN	(mm)	a (mm)	b (mm)
BR. 11	35.00	3	6	48	17515	250	1652
	25.00	2	6	36	12665	250	1589
BR. 12	25.00	2	6	36	25300	250	1587
BR. 13	20.00	2	5	30	10015	250	1545

- 11													
I	IIIGD	}	DATE	SIGNATURE			REPUBLIC OF THE PH	ILIPPINES		PROJECT AND LOCATION :	SCALE :	SHEET CONTENTS :	SHEET NO. :
		DESIGNED	-106	Sanan		DEPARTMEN	IT OF PUBLIC WO	RKS AND HIGHWAY	S	THE DETAILED DESIGN STUDY ON		BRIDGE 11, 12, & 13	
	JAPAN INTERNATIONAL COOPERATION AGENCY		7,90	E. R. SALIAN	PJHL PMO	BUREAU	T		HE SECRETARY	UPGRADING INTER-URBAN HIGHWAY SYSTEM	ļ	SCHEDULE OF REINFORCEMENT	
- {	I A CATALOGN & THOMPSON & CO. LANGUAGE TO SHIP THE SHIP TO SHIP THE SHIP TH	CHECKED	0/19/01/	andrada	Submitted By:	Reviewed By.	Recommended By:	(See cover sheet for	Approved By: (See cover sheet for	ALONG THE PAN-PHILIPPINE HIGHWAY (Plaridel, Cabanatuan and San Jose Bypasses)	AS SHOWN	(POST, RAILING AND SIDEWALK)	BS-02a
	KATAHIRA & ENGINEERS YACHIYO ENGINEERING CO, LTD.	SUBMITTED A	المكيك	Marking the	DANILO C. TRAJANO	ACIRIANO N. DOROY	GILBERTO S. REYES	Signature) MANUEL M. BONDAN	SIGNATURE/Approvel) SIMEON A. DATHMANONG		-	(INITIAL STAGE)	
- il	The state of the s	1	<i>UKIJOL</i>	TEAN LEADER	Project Director	Chief, Bridges Division	Director IV (OIC)	Undersecretory	Secretory	CABANATUAN BYPASS - CONTRACT PACKAGE IV	FULL SIZE A1	(MITTAL OTAGE)	



NOTES

1. CONCRETE :

CONCRETE SHALL CONFORM TO THE REQUIREMENTS OF CLASS AA CONCRETE. WITH 28 MPG CYUNDER STRENGTH AND 19.0mm MAXIMUM AGGREGATE SIZE.

2. REINFORCENMENT:

- A. ALL REINFORCING STEEL SHALL BE DEFORMED BARS COMFORMING TO ASSHTO M31 (ASTM A615) GRADE 40 AND 60.
- B. SPLICES OF ADJACENT LONGTUDINAL STEEL SHALL BE STAGGERRED 100 BAR DIAMETERS APART. LENGTH OF SPLICES SHALL BE 1000mm FOR #25 AND 1300mm FOR #28 AND 1700mm FOR #32. C. SPIRAL-TIES SHALL BE WELDED AT SPLICES.

3. DRIVING :

- A PILE HEADS SHALL BE PROTECTED FROM DIRECT IMPACT OF THE HAMMER BY CUSHION BLOCKS CONSISTING OF SEVERAL BLOCKS OF WOOD OR OF OTHER APPROVED MATERIALS.

 9. PILES SHALL BE ORIVEN TO A DEPTH THAT WILL PRODUCE THE REQUIRED ALLOWABLE BEARING CAPACITY.

4. PILE FOUNDATION DESIGN:

- A. IN PILE-BENT PIERS, PILE LENGTHS SHALL BE DETERMINED BY THE ENGINEER/
 CONSULTANT BASED ON THE ALLOWABLE PILE BEARING CAPACITY SPECIFIED BELOW,
 B. IN COLUMN-BENT PIERS, THE NUMBER, LOCATION AND LENGTH OF PILES SHALL BE
 DETERMINED BY THE ENGINEER/CONSULTANT BASED ON THE LOADING INFORMATION
 GIVEN IN THE PIER DETAILS.
- 5. PILE SPLICE :
- A. PILES MAY BE SPLICED ONLY IF STRICTLY NECESSARY AND APPROVED BY THE ENGINEER/CONSULTANT, PILE SPLICES SHALL BE LOCATED AT LEAST 10m BELOW THE EXISTING GROUND LEVEL B. PILE SPLICE SHALL DEVELOP 100% AXIAL, AND 50% BENDING OF THE CAPACITY OF THE PILE SECTION WHERE THE SPLICE IS LOCATED.
- 6. ALLOWABLE PILE BEARING CAPACITY : (SEE PILE SCHEDULE)
- 7. MINIMUM HAMMER ENERGY RATING = 55 kN-m
- 8. BASIS FOR COMPUTING ALLOWABLE PILE BEARING CAPACITY:

Pol)=
$$\left(\frac{167 \text{ eh Eh}}{S + 2.54}\right) \left(\frac{\text{Wr} + 0.16 \text{ Wp}}{\text{Wr} + \text{Wp}}\right)$$

WHERE:

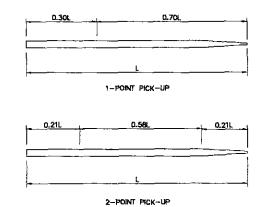
- Poil = ALLOWABLE PILE BEARING CAPACITY (kN)

 sh = HAMMER EFFICENCY
 En = HAMMER ENERGY RATING (kN-m)
 WF = WEIGHT OF RAM (kN)
 WD = WEIGHT OF PILE NNS OTHER DRIVEN WEIGHTS (kN)
 S = ALERAGE PENETRATION PER BLOW FOR THE LAST
 150mm OF DRIVING (mm)

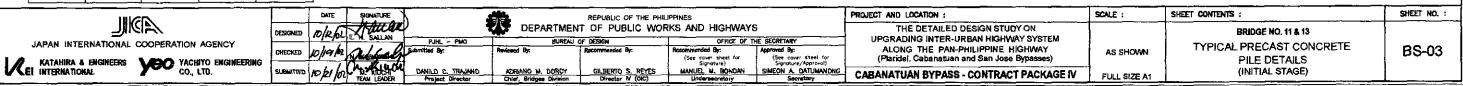
TEST PILES SHALL BE DRIVEN WITH THE SAME HAMMER USED FOR DRIMING REGULAR PILES AND MAY BE PART OF FOUNDATION IF APPROVED BY THE ENGINEER/CONSULTANT.

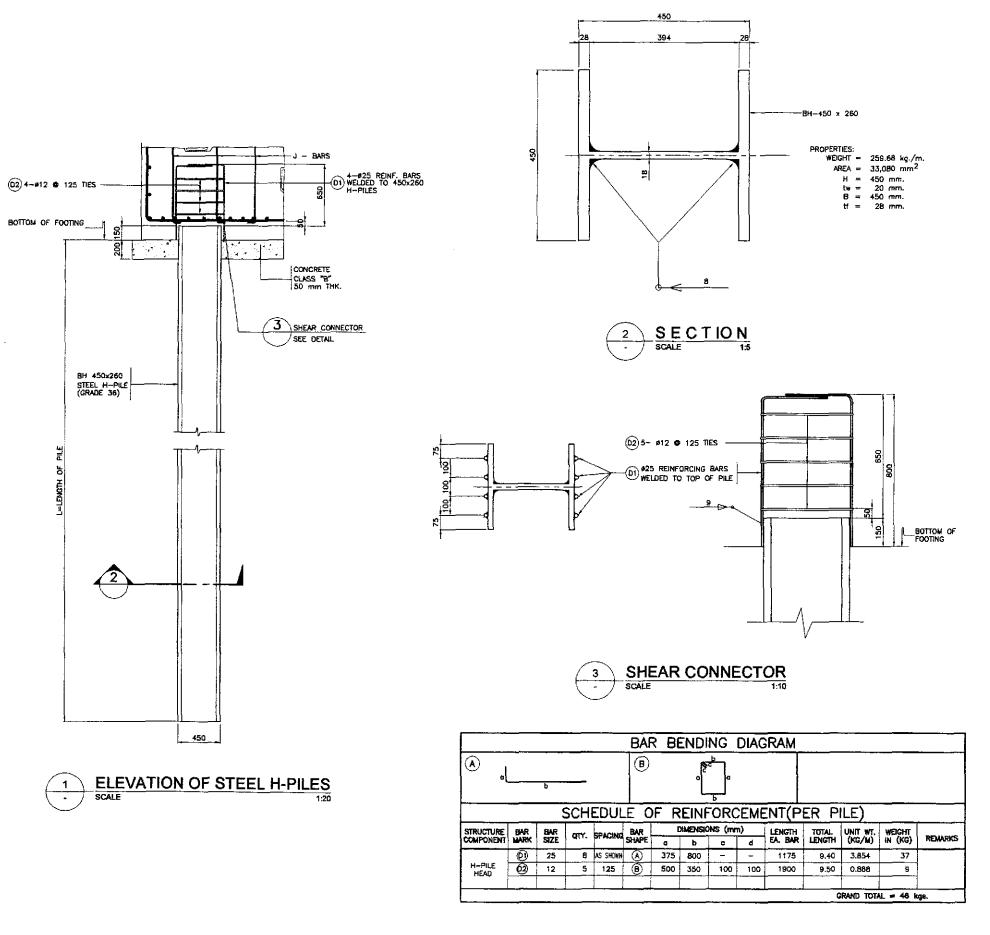
10. PICK-UP POINTS :

PICK-UP POINTS SHALL BE MARKED ON ALL PILES AND ALL LIFTING SHALL BE DONE AT THESE POINTS.



THE USE OF SPECIAL EMBEDDED OR ATTACHED LIFTING DEVICES SHALL BE SUBJECT TO THE APPROVAL OF THE ENGINEER/CONSULTANT.





NOTES

- 1 GENERAL
- 1.1 FOR GENERAL NOTES REFER TO SHEET B-02.
- 2 STRUCTURAL STEEL
- 2.1 STRUCTURAL STEEL SHALL CONFORM TO AASHTO M183 (ASTM A36).
- 2.2 WELDS SHALL CONFORM TO AWS D1.1 183, E70 XX SERIES.
- 2.3 DESIGNATION OF SECTION IN ACCORDANCE WITH ASEP STEEL HANDBOOK 1994.
- 3 PILE BEARING
- 3.1 ALLOWABLE PILE BEARING CAPACITY SHALL BE 600kM.
- 3.2 PILES SHALL BE DRIVEN TO A DEPTH OF NOT LESS THAN 1800mm PENETRATION OR TO REFUSAL, INTO THE BEARING STRATUM. THE BEARING STRATUM SHALL BE ROCK, SAND AND GRAVELS OR EQUIVALENT COHESIONLESS MATERIAL.

4 PILE DRIVING

- 4.1 MINIMUM HAMMER ENERGY RATING SHALL BE 55kN-m.
- 4.2 THE PILE BEARING CAPACITY SHALL BE DERIVED AS FOLLOWS:

Pail =
$$\begin{bmatrix} 167 \text{ eh Eh} \\ S + 2.54 \end{bmatrix} \begin{bmatrix} Wr + 0.16 \text{ Wp} \\ Wr + \text{Wp} \end{bmatrix}$$

WHERE:

Pall = ALLOWABLE PILE BEARING CAPACITY (KN) en = HAMMER EFFICIENCY

en - HAMMER EFFICIENCY

Eh = HAMMER ENERGY RATING (kN-m)

W = WEIGHT OF PILE AND OTHER DRIVEN WEIGHTS (kN)
S = AVERAGE PENETRATION PER BLOW FOR THE LAST

- 4.3 PILE HEADS SHALL BE PROTECTED FROM DIRECT IMPACT OF THE HAMMER BY CUSHION BLOCKS CONSISTING OF SEVERAL BLOCKS OF WOOD OR OTHER
- 4.4 PILES SHALL BE DRIVEN TO A DEPTH THAT WILL PRODUCE THE REQUIRED ALLOWABLE BEARING CAPACITY.

5 PILE SPLICE

- 5.1 PILES SHALL BE SPLICED ONLY IF STRICTLY NECESSARY AND WHEN APPROVED BY THE ENGINEER/CONSULTANT PILE SPLICES SHALL BE LOCATED AT LEAST 10m BELOW THE EXISTING GROUND LEVEL
- 5.2 PILE SPLICES SHALL DEVELOP AT LEAST 100% OF THE AXIAL CAPACITY AND 50% OF THE BENDING CAPACITY OF THE PILE SECTION WHERE THE SPLICE IS LOCATED.

- $6.1\,$ one test pile shall be driven at each bridge foundation. The location of the test pile shall be as directed by the engineer/consultant.
- 6.2 TEST PILES SHALL BE DRIVEN WITH THE SAME HAMMER USED FOR DRIVING REGULAR PILES AND MAY BECOME PART OF THE FOUNDATION IF APPROVED
- 6.3 TEST PILES SHALL BE LOADED TO 150% OF THE ALLOWABLE BEARING CAPACITY OF THE PILE. THE TEST PILE ACCEPTANCE CRITERIA SHALL BE:

MAXIMUM SETTLEMENT

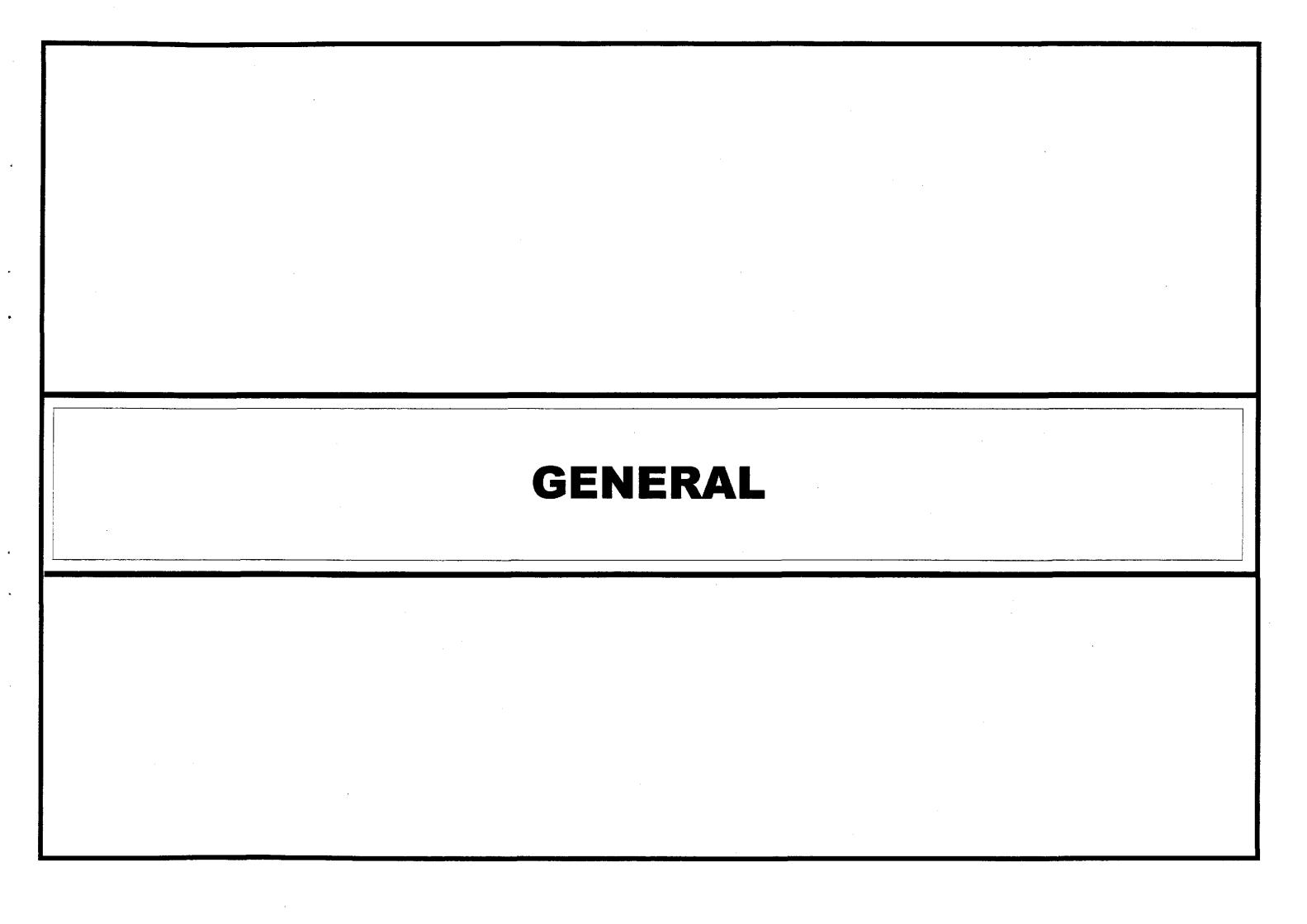
A. 100% OF ALLOWABLE PILE BEARING CAPACITY B. 150% OF ALLOWABLE PILE BEARING CAPACITY

C. ON UNLOADING

10mm (RESIDUAL)

PROJECT AND LOCATION : SCALE : SHEET CONTENTS : SHEET NO. : REPUBLIC OF THE PHILIPPINES REPUBLIC OF THE PHILIPPINES

DEPARTMENT OF PUBLIC WORKS AND HIGHWAYS THE DETAILED DESIGN STUDY ON BRIDGE NO. 12 UPGRADING INTER-URBAN HIGHWAY SYSTEM OFFICE OF THE SECRETARY JAPAN INTERNATIONAL COOPERATION AGENCY TYPICAL STEEL H-PILES ALONG THE PAN-PHILIPPINE HIGHWAY (Plaridel, Cabanatuan and San Jose Bypasses) AS SHOWN BS-04 KATAHIRA & ENGINEERS YOU YACHYO ENGINEERING CO, LTD. DETAILS Signeture)
MANUEL M. BONDAN SINEON A. DATLIMANONG (INITIAL STAGE) CABANATUAN BYPASS - CONTRACT PACKAGE IV



GENERAL NOTES FOR BRIDGES - 1

A. DESIGN CRITERIA

- 1. DESIGN SPECIFICATION
 - A. DPWH DESIGN GUIDELINES CRITERIA AND STANDARDS FOR PUBLIC WORKS AND HIGHWAYS, VOL.11.
 - B. NATIONAL STRUCTURAL CODE OF THE PHILIPPINES, VOL. II, 2nd ED. 1997
 - C. THE AMERICAN ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICIALS (AASHTO) STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES 16TH EDITION, 1996.
 - D. JAPAN ROAD ASSOCIATION SPECIFICATIONS FOR HIGHWAY BRIDGES
- 2. DESIGN METHODOLOGY

ALLOWABLE STRESS DESIGN (ASD) & LOAD FACTOR DESIGN (ULTIMATE STRENGTH DESIGN)

3.1 DEAD LOADS

WEIGHT

A. CONCRETE 24.50 kN/m 77.00 kN/m³ B. STEEL

C. EARTH D. WEARING SURFACE (50mm THK.) 1.10 kN/m²

19.00 kN/m³

3.2 LIVE LOADS

A. AASHTO MS18 (HS20) TRUCK AND EQUIVALENT LANE LOADING. B. SIDEWALK LOAD

 $SPAN \leq 30.5m$; 4.07 kN/mSPAN > 30.5m : $(1.437 + \frac{43.798}{L})(\frac{16.76-W}{15.24}) \text{ kN/m}^2 < 2.874 \text{ kN/m}^2$

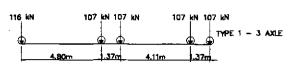
L : LOADED LENGTH

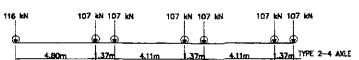
W : SIDEWALK WOTH

C. ALTERNATE MILITARY LOADING.

107 kN 107 kN 1.22m

D. PERMIT DESIGN LOAD (SPECIAL PERMIT REQUIRED BEFORE PASSING BRIDGE)





3.3 IMPACT

IN ACCORDANCE WITH DIVISION 1 OF AASHTO STANDARD SPECIFICATIONS, 1996.

3.4 SFISMIC LOAD

IN ACCORDANCE WITH DIVISION 1A OF THE 1996 AASHTO STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES USING ACCELERATIONS COEFFICIENT OF 0.40 AND SEISMIC PERFORMANCE CATEGORY D.

3.5 HYDRAULIC DESIGN DATA

50—YEAR DESIGN DISCHARGE, $Q_{50}=1,570$ m//sec. DESIGN FLOW VELOCITY, $V_{50}=2.08$ m//sec. DESIGN FLOOD WATER LEVEL, DFWL = EL + 43.25 m CATCHMENT AREA, CA = 463 km²

3.6 TEMPERATURE RANGES

ASSUMED BASE TEMPERATURE : +28C* MINIMUM AMBIENT AIR TEMPERATURE: +180° MAXIMUM AMBIENT AIR TEMPERATURE - +380* TEMPERATURE DIFFERENCE BETWEEN TOP OF SLAB AND OTHER PARTS OF STRUCTURE: +10C*

3.7 CONSTRUCTION LOADS

CONSTRUCTION LOADS SHALL BE AS STIPULATED IN THE AASHTO GUIDE SPECIFICATIONS. THE CONTRACTOR IS RESPONSIBLE FOR ENSURING THAT THESE LOADS ARE NOT EXCEEDED AND THAT THE MEMBER STRESSES ARE WITHIN ALLOWABLE DURING CONSTRUCTION.

3.8 OTHER LOADS

IN ACCORDANCE WITH AASHTO STANDARD SPECIFICATIONS, 1996.

- 3.9 LOAD COMBINATION (LOAD FACTOR DESIGN)
 - A. GROUP 1 = 1.3 [1.0 0 + 1.67(L+1)n + 1.0 SF]
 - GROUP 1B = 1.3 [1.0 D + 1.0(L+1)p + 1.0 SF]
 - GROUP VI = 1.3 [1.0 D + 1.0 SF + EQ OTHER LOAD COMBINATIONS SHALL BE IN ACCORDANCE WITH AASHTO GUIDE SPECIFICATIONS.

B. MATERIALS

1. CONCRETE

UNLESS INDICATED OTHERWISE ON PLANS, THE CONCRETE CLASS AND STRENGTH SHALL BE AS FOLLOWS:

STRUCTURAL MEMBER	CLASS	28 - DAY STREE	CYLINDER	MAX. SIZE OF COARSE AGGREGATE	REMARKS
		MPc	PSI	mm (in.)	
CAST — IN PLACE GIRDERS, SLABS, DIAPHRAGMS, WINGWALLS BACKWALLS, ABUTMENT COPINGS, COLUMNS, SLABS, SHEAR KEYS	AA2	28	4060	20	
FOOTINGS, PILE CAP, BORED PILES, APPROACH SLAB	AA1	28	4060	25	*SEE NOTE BELOW
THIN REINFORCED SECTIONS, PARAPET, RAILINGS & RAILPOST CURB AND SIDEWALK	u	21	3000	12	
PRESTRESSED CONCRETE		35	5075	20	O TRANSFER
MEMBERS : AASHTO GIRDERS	Ьb	41	5946	20	SERVICE
STEEL SHEET PILE CAP	Α	21	3000	38	
RUBBLE CONC./CONC. BLOCKS FOR SLOPE PROTECTION	В	16.5	2400	50	
LEAN CONCRETE		17	1450	38	

* NOTE :

THE CEMENT CONTENT OF THE DESIGN MIX SHALL BE ADJUSTED IN ACCORDANCE WITH THE AASHTO PROVISIONS WHEN CONCRETING UNDER WATER TO COMPENSATE FOR THE LOSS OF STRENGTH DUE TO WATER INFILTRATION.

2. REINFORCING STEEL

(a) REINFORCING STEEL SHALL CONFORM TO AASHTO M31 (ASTM A615), GRADES 40 & 60 DEFORMED WITH MINIMUM YIELD STRENGTH AS

REBAR GRADE	YIELD STRENGTH fy (MPa)	SIZE (mm)
40	276 (40 ksi)	16mm# & BELOW, UNLESS OTHERWISE NOTED
60	415 (60 ksi)	20mmø & ABGVE

- (b) REINFORCING STEEL SHALL BE FREE OF MILL SCALES, OIL OR ANY SUBSTANCES WHICH WILL WEAKEN THE BOND WITH CONCRETE.
- (c) REINFORCING STEEL SHALL BE WELDABLE TYPE.
- WELDING REINFORCING STEEL SHALL CONFORM TO ANSI/AWS D1.4.

3. PRESTRESSING STEEL

PRESTRESSING STEEL SHALL BE SEVEN-WIRE UNCOATED STRESS-RELIEVED STRANDS AND SHALL CONFORM TO AASHTO M203 (ASTM A416) WITH MINIMUM ULTIMATE STRENGTH OF Fy = 1860 MPa (270,000psi).

PC STRESS BARS SHALL BE HIGH TENSILE COLD WORKED STRESS BAR CONFORMING TO ASTM-A722/1SO 6934 (SBPR 930/1180) WITH NOMINAL TENSILE STRENGTH OF 1176 MPa.

4. STRUCTURAL STEEL BOLTS AND WELDS

MATERIALS	YIELD STRENGTH fy (MPa)	REFERENCE SPECIFICATIONS
STRUCTURAL STEEL	250 (GRADE 36)	AASHTO M270, (ASTM A709)
HIGH STRENGTH BOLTS		AASHTO M253, ASTM A490M
WELDS		LATEST ANSI /AASHTO/AWS D1.5 BRIDGE WELDING CODE

5. ELASTOMERIC BEARING PADS

ELASTOMERIC BEARING PADS SHALL BE 100% VIRGIN CHLOROPRENE (NEOPRENE) PADS WITH DUROMETER HARDNESS 50 AND SHALL BE LAMINATED WITH NON-CORROSIVE MILD STEEL SHEETS (ASTM A570). ELASTOMERIC PADS SHALL CONFORM TO THE REQUIREMENTS AS PRESCRIBED IN DPWH D.O. NO. 25 SERIES OF 1997 "REVISED DPWH STANDARD SPECIFICATION FOR ELASTOMERIC BEARING PAD.

SPECI	FICATIONS
DURO HARDNESS, SHORE A (A	ASTM D-2240)60±5
TENSILE STRENGTH ASTM	D 412-175 Kg/cm ² (min
ULTIMATE ELONGATION %	350 % (min)
MATERIAL	NEOPRENE

C. CONSTRUCTION

THESE NOTES ARE PROVIDED FOR QUICK REFERENCE ONLY AND SHALL BE READ IN CONJUNCTION WITH THE TECHNICAL SPECIFICATIONS FOR THE PROJECT.

THE DESIGN OF BRIDGES IS BASED ON THE CONSTRUCTION SEQUENCE SHOWN IN THE DRAWINGS. ANY VARIATION FROM THE SEQUENCE MUST BE APPROVED BY THE ENGINEER.

CONSTRUCTION SHALL COMPLY WITH 1995 DPWH STANDARD SPECIFICATION FOR HIGHWAYS, BRIDGES AND AIRPORTS OR MODIFIED BY SPECIAL PROVISIONS.

1. DIMENSIONS

- SECTION, DIMENSIONS AND DISTANCES SHALL NOT BE SCALED FOR CONSTRUCTION PURPOSES. THE INDICATED DIMENSION SHALL GOVERN UNLESS OTHERWISE SPECIFIED.
- 1.2 ALL DIMENSIONS SHOWN ARE IN MILLIMETERS UNLESS OTHERWISE NOTED.
- 1.3 ALL STATIONING ARE IN KILOMETER PLUS METER AND

SCALE : SHEET CONTENTS : SHEET NO. : PROJECT AND LOCATION : DATE SIGNATURE REPUBLIC OF THE PHILIPPINES DEPARTMENT OF PUBLIC WORKS AND HIGHWAYS THE DETAILED DESIGN STUDY ON BRIDGE NO.14 TALAVERA RIVER BRIDGE UPGRADING INTER-URBAN HIGHWAY SYSTEM OFFICE OF THE SECRETARY JAPAN INTERNATIONAL COOPERATION AGENCY **GENERAL NOTES 1 OF 3** ALONG THE PAN-PHILIPPINE HIGHWAY (Plaridel, Cabanatuan and San Jose Bypasses) B14G-01 AS SHOWN V90 YACHIYO ENGINEERING 10/21/02 (mi tout) KATAHIRA & ENGINEERS LA HITERNATIONAL CO. LTD. MANUEL M. BONOAN (INITIAL STAGE) CABANATUAN BYPASS - CONTRACT PACKAGE IV

GENERAL NOTES FOR BRIDGES - 2

2. SETTING OUT

THE SETTING OUT AND THE ELEVATIONS OF THE DIFFERENT COMPONENTS OF THE STRUCTURE SHALL BE APPROVED BY THE ENGINEER PRIOR TO THE START OF ANY CONSTRUCTION WORK.

3. REINFORCED CONCRETE

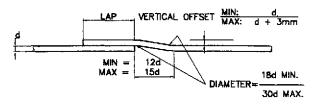
3.1 CAST IN PLACE CONCRETE SHALL BE CLASS "AA1" OR "AA2" EXCEPT RAILINGS WHICH SHALL BE CLASS "C". UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED EDGES SHALL BE CHAMFERED 25mm EXCEPT RAILINGS AND RE-ENTRANT ANGLES WHICH SHALL BE CHAMFERED AND FILLETED 13mm RESPECTIVELY.

3.2 CONCRETE MIX AND PLACING

- (1) DESIGN OF CONCRETE MIX SHALL MEET THE DESIGN CONCRETE STRENGTH GIVEN UNDER ITEM 1 OF MATERIALS.
- (2) CONCRETE SHALL BE DEPOSITED, VIBRATED AND CURED IN ACCORDANCE WITH THE SPECIFICATION.
- (3) FOR CONCRETE DEPOSITED AGAINST THE GROUND, LEAN CONCRETE WITH A MINIMUM THICKNESS OF 100mm SHALL BE LAID FIRST BEFORE INSTALLING THE REINFORCEMENT. THIS LEAN CONCRETE SHALL NOT BE CONSIDERED IN MEASURING THE STRUCTURAL DEPTH OF CONCRETE SECTION.
- (4) THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER FOR APPROVAL PLACING SEQUENCES FOR ALL CONCRETING WORK.

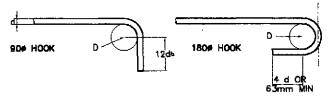
3.3 BAR BENDING, SPLICING AND PLACING

- (1) THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER, FOR APPROVAL, SHOP DRAWINGS INDICATING THE BENDING, CUTTING, SPLICING AND INSTALLATION OF ALL REINFORCING BARS.
- (2) BARS SHALL BE BENT COLD. BARS PARTIALLY EMBEDDED IN CONCRETE SHALL NOT BE FIELD BENT UNLESS PERMITTED BY THE FINGINFER
- (3) BAR SPLICING NOT INDICATED ON DRAWINGS SHALL BE SUBJECT TO THE APPROVAL OF THE ENGINEER.
- (4) WELDED SPLICES, IF APPROVED BY THE ENGINEER, SHALL DEVELOP IN TENSION AT LEAST 125% OF THE SPECIFIED YIELD STRENGTH OF THE BARS.
- (5) NOT MORE THAN 50% OF THE BARS AT ANY ONE SECTION SHALL BE SPLICED.
- (6) UNLESS OTHERWISE SHOWN ON DRAWINGS, THE CLEAR DISTANCE
 BETWEEN PARALLEL BARS IN A LAYER SHALL NOT BE LESS THAN
 1.5 TIMES THE NOMINAL DIAMETER OF THE BAR NOR LESS THAN
 1.5 TIMES THE MAXIMUM SIZE OF COARSE AGGREGATE. THE CLEAR
 DISTANCE BETWEEN LAYERS SHALL NOT BE LESS THAN 25mm NOR
 ONE BAR DIAMETER. THE BARS IN THE UPPER LAYER SHALL BE
 PLACED DIRECTLY ABOVE THOSE IN THE BOTTOM LAYER.
- (7) CRANKED SPLICES



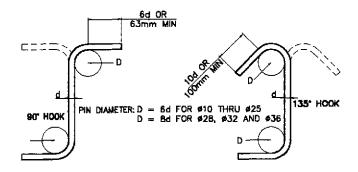
(8) HOOKS AND BENDS

DIMENSIONS OF 90-DEGREE AND 180-DEGREE HOOKS



PIN DIAMETER: D = 6d FOR \$10 THRU \$25 D = 8d FOR \$28, \$32 AND \$36

DIMENSIONS FOR STIRRUPS AND TIE HOOKS



3.4 CONCRETE COVER TO REINFORCEMENT

UNLESS OTHERWISE NOTED, ALL BAR DIMENSIONS ARE REFERRED TO THE CENTER OF BARS AND THE MINIMUM COVERING MEASURED FROM THE SURFACE OF THE CONCRETE TO THE FACE OF ANY BAR SHALL BE 40mm. FOR SUBSTRUCTURE PERMANENTLY EXPOSED TO EARTH, COVERING SHALL BE 75mm.

3.5 CONSTRUCTION JOINT

- (1) THE POSITION AND FORM OF ANY CONSTRUCTION JOINT SHALL BE AS SHOWN ON DRAWINGS OR AS AGREED WITH THE FINGINEER.
- (2) THE INTERFACE BETWEEN THE FIRST AND SECOND POUR CONCRETE SHALL BE ROUGHENED WITH AN AMPLITUDE OF SAME MINIMUM.

3.6 FALSEWORK

ALL FALSEWORK SHALL BE DESINED BY THE CONTRACTOR SUBJECT TO THE APPROVAL BY THE ENGINEER. FALSEWORKS SHOWN IN THE DRAWINGS SHALL SERVE AS REFFERENCE ONLY.

3.7 FORMWORK

FORMWORKS SHALL BE CONSTRUCTED SUCH THAT IT WILL NOT YIELD UNDER THE LOAD AND SHALL BE SUCH AS TO AVOID THE FORMATION OF FINE. ALL CORNERS OF CONCRETE MEMBERS SHALL BE CHAMFERED TO 25mm UNLESS NOTED OTHERWISE ON DRAWINGS. STRIPPING OF FORMS AND SHORES SHALL BE AS DESIGNATED BY THE ENGINEER. THE FOLLOWING MAYBE USED AS A GUIDE.

	MIN. TIME
SHORING UNDER GIRDERS, BEAMS, FRAMES	14 DAYS
DECK SLABS	14 DAYS
WALLS	7 DAYS
COLUMNS	. 7 DAYS
SIDES OF BEAMS AND ALL OTHER VERTICAL SURFACES	. 2 DAYS

3.8 PROTECTION AND CURING OF CONCRETE

CONCRETE SURFACES SHALL BE PROTECTED FROM HARMFUL EFFECTS OF SUN, WIND AND RUNNING WATER AND SHALL BE KEPT DAMP FOR AT LEAST 7 DAYS.

4. EMBANKMENT CONSTRUCTION SEQUENCE

APPROACH EMBANKMENT SHALL BE CONSTRUCTED PRIOR TO CONSTRUCTION OF ABUTMENT PILES.

5. REINFORCED CONCRETE CAST-IN-PLACE BORED PILES

5.1 THE REQUIRED ALLOWABLE BEARING CAPACITY FOR EACH PRIF DIAMETER IS AS FOLLOWS:

PILE DIA.	NORMAL	(KN)	ULTIMATE (KN)				
	COMPRESSION	TENSION	COMPRESSION	TENSION			
Ø1000	3000	1200	9000	7200			
ø 1500	6000	2100	18000	13000			

- 5.2 BOTTOM OF BORED PILES SHALL BE EMBEDDED AT LEAST TWO TIMES PILE DIAMETER (2D) INTO HARD STRATA CAPABLE OF DEVELOPING ALLOWABLE BEARING CAPACITY AS SPECIFIED. IF THE ABOVE CONDITION IS NOT MET DURING CONSTRUCTION. THE PILE SHALL BE INCREASED AND THE DESIGNER/CONSULTANT SHALL BE NOTIFIED FOR CONFIRMATION. AN ON-SITE SUBSURFACE INVESTIGATION SHALL ALSO BE UNDERTAKEN DURING CONSTRUCTION FOR CONFIRMATION/VERIFICATION OF DATA USED IN THE DESIGN.
- 5.3 PILE LENGTHS SHOWN ARE ESTIMATED LENGTHS DURING DESIGN. DETERMINATION OF REQUIRED PILE LENGTHS SHALL BE DETERMINED BY THE CONTRACTOR BASED ON THE RESULTS OF FIELD INVESTIGATIONS CARRIED OUT BY THE CONTRACTOR. SEE THE SPECIAL PROVISIONS OF THE TECHNICAL SPECIFICATIONS.
- 5.4 ULTRASONIC INTEGRITY TESTING (AS PER SPECIFICATIONS) SHALL BE CONDUCTED FOR ALL PILES TO VERIFY/CHECK THE CONCRETE HOMOGENEITY AND TO LOCATE/EVALUATE ANY POSSIBLE IRREGULARITY IN THE COMPLETED BORED PILES AS DESCRIBED IN THE SPECIAL PROVISIONS.
- 5.5 STATIC LOAD TEST AND HIGH STRAIN DYNAMIC LOAD TEST SHALL BE CONDUCTED AS INDICATED IN THE SCHEDULE OF PILE LOAD TEST OF THE COMPLETED BORED PILES. THE RESULT SHALL BE SUBMITTED FOR EVALUATION AND REFERENCE.

6. ADDITIONAL SOIL INVESTIGATION

ADDITIONAL SUBSURFACE INVESTIGATION (BORE HOLES) SHALL BE CONDUCTED FOR EACH PIER AND ABUTMENT LOCATION TO CONFIRM/VERIFY THE DESIGN SCIL PROFILE AND CAPACITIES. IF THE RESULTS OF THE SOIL INVESTIGATION DIFFERS FROM THE SOIL DATA USED IN DESIGN, THE CONTRACTOR SHALL NOTIFY THE ENGINEER/CONSULTANT TO MAKE THE NECESSARY ADJUSTMENTS IN THE FOUNDATION.

7. CAMBER

- 7.1 PRECAST PRESTRESSED CONCRETE GIRDERS SHALL BE CONSTRUCTED WITH CAMBER INDICATED IN THE DRAWINGS.
- 7.2 AFTER ERECTION IS COMPLETE, THE FLANGE ELEVATION OF THE GIRDERS SHALL BE SURVEYED. BASED ON THIS INFORMATION, THE CONTRACTOR SHALL DETERMINE THE HAUNCH HEIGHTS REQUIRED ALONG THE STRUCTURE IN ORDER THAT THE FINISHED GRADE SHOWN IN THE DRAWINGS WILL BE ACHIEVED, TAKING DUE ACCOUNT OF FURTHER DEFLECTIONS TO BE INCURRED WHEN THE DECK AND SIDEWALKS ARE ADDED AND THE ORDER IN ERECTION OF DECK PANEL IS TO TAKE PLACE.
- 7.3 THE CONTRACTOR SHOULD PREPARE & SUBMIT A GEOMETRY CONTROL REPORT TO THE ENGINEER INDICATING THE ASSUMPTIONS AND CALCULATION PROCEDURES THAT HAVE BEEN FOLLOWED IN DETERMINING HAUNCH HEIGHTS. THE CONTRACTOR SHOULD MONITOR AND UPDATE THIS REPORT AS NECESSARY AS ERECTION PROCEEDS.

INCID	DATE SIGNATURE	411	REPUBLIC OF THE	HILIPPINES		PROJECT AND LOCATION ;	SCALE :	SHEET CONTENTS:	SHEET NO. :
	DESIGNED 10/12/07 F. P. DE DESUS	***	DEPARTMENT OF PUBLIC V			THE DETAILED DESIGN STUDY ON		BRIDGE NO.14 TALAVERA RIVER BRIDGE	
JAPAN INTERNATIONAL COOPERATION AGENCY	1.1-/- 30	FUHL - PMO Submitted By: Revie	BUREAU OF DESIGN Becommended for	Recommended Br:	THE SECRETARY ADDRESS BY:	UPGRADING INTER-URBAN HIGHWAY SYSTEM ALONG THE PAN-PHILIPPINE HIGHWAY		GENERAL NOTES 2 OF 3	D440 00
ATAHIRA & ENGINEERS VACHIYO ENGINEERING	CHECKED /0/19/02 1 2 30/10S]	esse by.	(See cover sheet for	(See cover sheet for	(Planidel, Cabanatuan and San Jose Bypasses)	AS SHOWN	1	B14G-02
KATAHIRA & ENGINEERS YOU YACHIYO ENGINEERING CO., LTD.	SUBMITTED 15/5/ (Mar. Knikdach	DANIEC C. TRAJANO A	ADRIANO M. DOROY GILBERTO S. REYE	MANUEL M. BONCAN	Signeture/Approval) SIMEON A. DATUMANONG	CABANATUAN BYPASS - CONTRACT PACKAGE IV	CI II 0.75 44	(INITIAL STAGE)	l li
	TEAM LEADER	Project Director C	Her, Bridges Division Director N (OIC)	Undersecretory	Secretary	CHEMINIONI DIFMOS - CONTRACT FACIONOLIV	FULL SIZE A1	<u> </u>	

GENERAL NOTES FOR BRIDGES - 3

8. STRUCTURAL STEEL

THE CONTRACTOR SHALL PREPARE AND SUBMIT SHOP DRAWINGS FOR ALL STRUCTURAL STEEL WORK. THESE SHOP DRAWINGS SHALL BE APPROVED BY THE ENGINEER BEFORE ANY FABRICATION COMMENCES.

9. SHORING

- 9.1 CAMBER FOR REINFOCED CONCRETE SUPERSTRUCTURES WERE DETERMINED BASED ON THE USE OF SHORINGS DURING CONSTRUCTION
- 9.2 CAMBER FOR COMPOSITE SUPERSTRUCTURES WITH PRECAST PRESTRESSED GIRDERS WERE DETERMINED BASED ON UNSHORED CONDITIONS.

10. EXCAVATION

EXCAVATION FOR STRUCTURES SHALL BE TO THE NEAT LINES OF FOOTING OR AS SPECIFIED IN THE STANDARD SPECIFICATIONS.

11. WATER ELEVATION

WATER ELEVATIONS SHOWN ON PLANS ARE APPROXIMATE ONLY ANY VARIATION FOUND DURING CONSTRUCTION SHALL NOT BE CONSIDERED AS A BASIS FOR EXTRA COMPENSATION.

12. DETOUR

THE CONTRACTOR SHALL CONSTRUCT AND MAINTAIN DETOUR BRIDGES, AND/OR ROADS DURING CONSTRUCTION TO ALLOW CONTINUOUS FLOW OF TRAFFIC. THEY SHALL BE CONSTRUCTED ON LOCATION AS SHOWN ON PLANS OR AS DIRECTED BY THE ENGINEER. NO ADDITIONAL COST SHALL BE ALLOWED FOR ANY RELOCATION OF DETOUR.

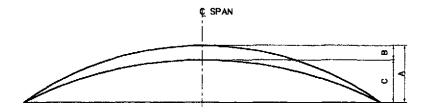
13. PRESTRESSED CONCRETE

GIRDER DESIGN GUIDE

- 13.1 POST--TENSIONING; THE PROPOSED TYPE OF TENDONS WHICH WILL
 BE USED IN THE POST--TENSIONED DESIGNS AND ALL NECESSARY
 ADDITIONAL DETAILS INCLUDING THOSE FOR END ANCHORAGES, METHODS
 TO BE EMPLOYED AND PROCEDURES TO BE FOLLOWED, SHALL BE AS
 APPROVED BY THE ENGINEER, PORTION OF THE TENDONS SHALL BE
 DRAPED LONGITUDINAL IN PARABOLIC PORTIONS. ALL TENDONS SHALL BE PLACED
 SO THAT THEIR CENTER OF GRAVITY WILL BE AT THE POSITION SHOWN ON PLANS.
 THE TOTAL POST--TENSION FORCE AFTER LOSSES REQUIRED AT MIDSPAN SHALL
 BE PROVIDED AS CALLED FOR IN THE VARIOUS DESIGNS. THE REQUIRED FORCES
 AFTER LOSSES SHALL BE OBTAINED BY APPLYING INITIAL TENSILE FORCES OF
 SUFFICIENT MAGNITUDE TO ALLOW FOR ALL SUBSEQUENT LOSSES, INCLUDING
 THOSE FOR ELASTIC SHORTENING, SHRINKAGE, CREEP, RELAXATION, FRICTION,
 AND EFFICIENCY OF END ANCHORAGES. AFTER SECURING THE END
 ANCHORAGES ALL TENDONS SHALL BE PRESSURE GROUTED IN THEIR
 CONDUITS IN ACCORDANCE WITH THE "SPECIFICATIONS".
- 13.2 CONCRETE FOR GIRDERS SHALL BE A MINIMUM STRENGTH OF 41 N/mm² (5,945 PSI) AT THE AGE OF 28 DAYS.
- 13.3 CONCRETE FOR CAST-IN-PLACE SLAB HAVE A MINIMUM STRENGTH OF 28 N/mm2 (4,060 PSI) AT THE AGE OF 28 DAYS.
- 13.4 THE CONTRACTOR MAY PROPOSE ANY ALTERNATIVE TENDON SIZE
 AND LAYOUT WHICH SHALL MEET THE APPROVAL OF THE ENGINEER.
- 13.5 THE REQUIRED STRENGTH OF CONCRETE AT TIME OF TENSIONING SHALL BE 35 MPG (5,075 PSI). A GRID CONSISTING OF #12 BARS AT 100 CENTERS IN BOTH DIRECTIONS SHALL BE PLACED NEAR EACH ANCHORAGE OF THE POST-TENSIONING SYSTEM.

13.6 HANDLING PRESTRESSED CONCRETE BEAMS: THE BEAMS SHALL BE MAINTAINED IN AN UPRIGHT POSITION AND SHALL BE LIFTED BY SUITABLE DEVICES PROVIDED AT THE ENDS OF THE BEAMS. ATTENTION IS DIRECTED TO THE INCREASED DIFFICULTY OF LIFTING BEAMS WITHOUT END BLOCKS. THE CONTRACTOR'S PROPOSED LIFTING DETAILS SHOULD BE GIVEN CAREFUL CONSIDERATION BEFORE BEING SUBMITTED ON SHOP DRAWING FOR APPROVAL. THE USE OF HOLES FOR LIFTING PURPOSES WILL NOT BE PERMITTED.

13.7 CONTRACTOR SHALL SUBMIT FOR APPROVAL BY THE ENGINEER THE CALCULATED ELONGATION OF THE PRESTRESSING TENDONS CORRESPONDING TO THE REQUIRED JACKING FORCES.

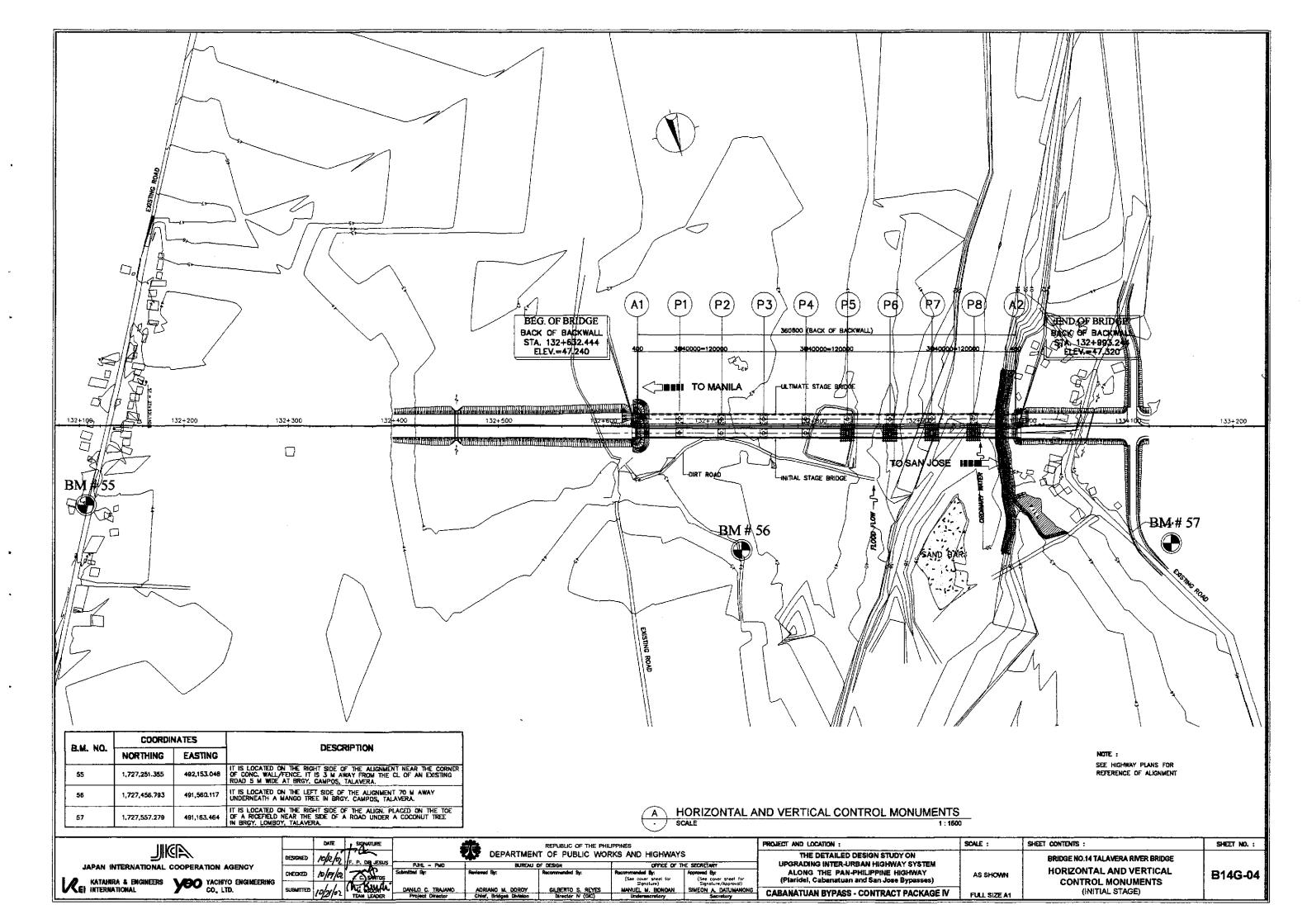


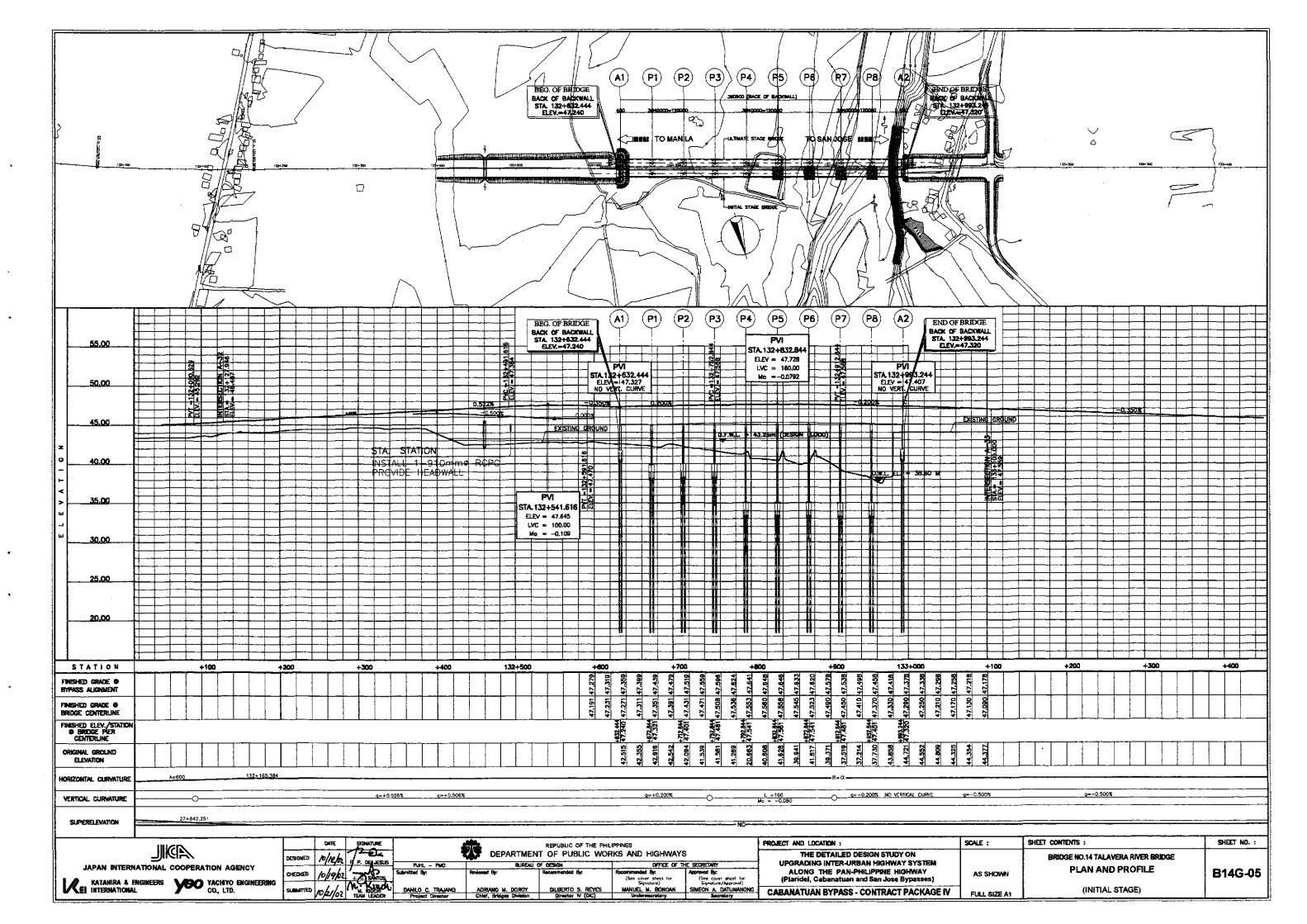
DEAD LOAD CAMBER DIAGRAM

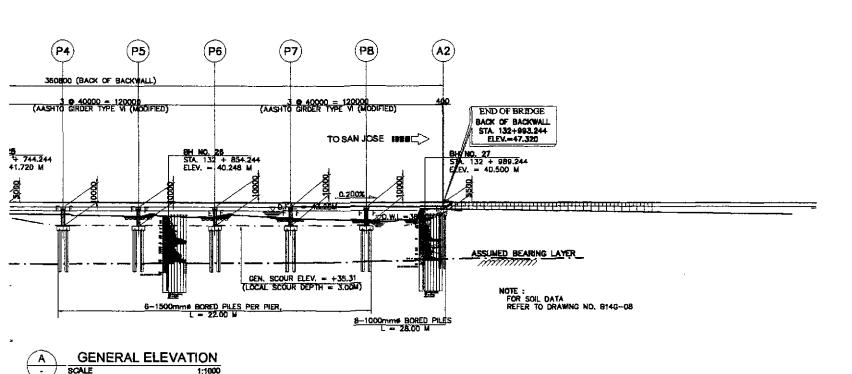
- A = INITIAL CAMBER ESTIMATED PRESTRESS CAMBER LESS DEFLECTION DUE TO GIRDER DEAD LOAD
- B = DEFLECTION DUE TO SLAB, DIAPHRAGM, SIDEWALKS, RAILING AND RAILPOST
- C = FINAL CAMBER
- NOTE: A AND B ARE THEORETICAL VALUES AND MAY VARY WITH ACTUAL (AGE) CONCRETE STRENGTH, VARIOUS PRESTRESSING CONDITIONS, CREEP FACTOR, AND PRESTRESS LOSSES.

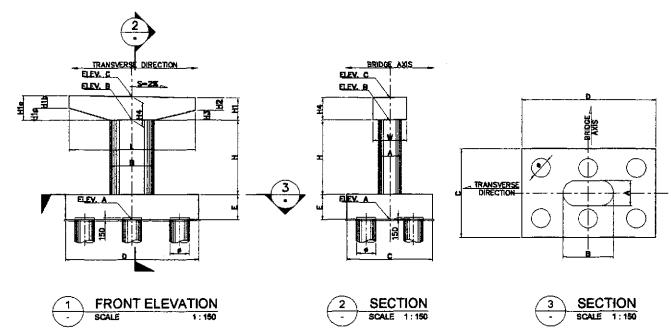
 CONTRACTOR SHALL SURVEY TOP OF GIRDERS TO OBTAIN ACTUAL VALUE OF A AND ADJUST PROFILE ACCORDINGLY.
- 13.8 PRECAST GIRDERS AND DECK PANELS SHALL MEET THE TOLERANCES SPECIFIED IN THE AASHTO GUIDE SPECIFICATIONS FOR DESIGN AND CONSTRUCTION OF SEGMENTAL CONCRETE BRIDGES.
- 13.9 TRANSVERSE DEFLECTION OF PRECAST GIRDERS SHALL NOT EXCEED 1/500_{TH}. OF THE GIRDER LENGTH, WHERE DEFLECTION EXCEED THIS VALUE, PROCEDURES FOR CORRECTION SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW, IF CORRECTION BY APPROVED PROCEDURE IS NOT POSSIBLE, THE GIRDER SHALL BE REJECTED.

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SILVED.		DATE SISSATURE		REPUBLIC OF TH	ie philippines		PROJECT AND LOCATION :	SCALE :	SHEET CONTENTS :	SHEET NO. :
	DESIGNED	10/2/01 F. P. DE JES	JS	DEPARTMENT OF PUBLIC	WORKS AND HIGHWAY	5	THE DETAILED DESIGN STUDY ON		BRIDGE NO.14 TALAVERA RIVER BRIDGE	
JAPAN INTERNATIONAL COOPERATION AGENCY		1/1/	PJHL - PMC	BUREAU OF DESIGN		HE SECRETARY	UPGRADING INTER-URBAN HIGHWAY SYSTEM		GENERAL NOTES 3 OF 3	1
I A MATARIDA & PROMETTO & ADAM MANTE PRINCIPALITY	CHECKED	10/19/02 -7-3410	; Submitted By:	Reviewed By: Recommended By:	Recommended By: (See cover sheet for	Approved By: (See cover sheet for	ALONG THE PAN-PHILIPPINE HIGHWAY (Platidel, Cabanatuan and San Jose Bypasses)	AS SHOWN	GENERAL NOTES SOFS	B14G-03
KATAKIRA & ENGINEERS YOU YACHYO ENGINEERING CO, LTD.	SUBMITTED		- T LIMILLI L. INCLINU	ADRIANO M. DOROY GLBERTO S. RE Chief, Bridges Division Director N (Ot		Signature/Approval) SIMEON A DATUMANONG		FULL SIZE A1	(INITIAL STAGE)	
		TEAM LEADE	Project Director	Chief, Bridges Division Director M (Ot	Undersearetary	i Secretary		I OUL SIZE AT	<u> </u>	









SCALE	1:1000	
) P4 350800 (BACK OF B.		PP PB
(AASHTO GROER TYPE		(AASHTO GIRDER / TYPE VI (MODIFIED) END OF BRIDGE BACK OF BACKWALL STA. 132+993.244 ELEV47.320
- Section -	BH NO.36	1322-101-100
		TO SAN JOSE BARRETTO STATE OF THE STATE OF T
		OGENIAMOR. T
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			TAB	LE OF CO	ORDINATES	5				
			INITIAL	L STAGE		ULTIMA	TE STAGE			
PIER NO.	STATION	COL	UMN	PILE	CAP	COL	LIMN	PILE CAP		
		NORTHINGS	EASTINGS	NORTHINGS	EASTINGS	NORTHINGS	EASTINGS	NORTHINGS	EASTINGS	
ABUT. 1	132 + 632,444	1727322.526	491627.227	1727322.526	491627.227	1727309,498	491623,589	1727309.498	491623.68	
PIER 1	132 + 672.844	1727333.114	491588.239	1727333.114	491588.239	1727320.086	491584.701	1727320.086	491584.70	
PIER 2	132 + 712.844	1727343.598	491549.637	1727343,598	491549.637	1727330.570	491546.099	1727330.570	491546,099	
PIER 3	132 + 752.844	1727354.081	491511.036	1727354.081	491511.036	1727341.053	491507.497	1727341.053	491507.497	
PIER 4	132 + 792.844	1727364.564	491472.434	1727364.564	491472.434	1727351.536	491468.896	1727351.536	491468.896	
PIER 5	132 + 832.844	1727375.047	491433.832	1727375.047	491433.832	1727362.019	491430,294	1727362,019	491430.294	
PIER 6	132 + 872.844	1727385.531	491395.230	1727385.531	491395.230	1727372,502	491391.692	1727372.502	491391.692	
PIER 7	132 + 912.844	1727396.014	491356.628	1727395.014	491356.628	1727382,986	491353.090	1727382.986	491353,090	
PIER 8	132 + 952.844	1727408.497	491318.026	1727406.497	491318-026	1727393.469	491314.488	1727393.469	491314.48	
ABUT. 2	132 + 993,244	1727417.085	491279.038	1727417.085	491279.038	1727404.057	491275.500	1727404.057	491275.50	

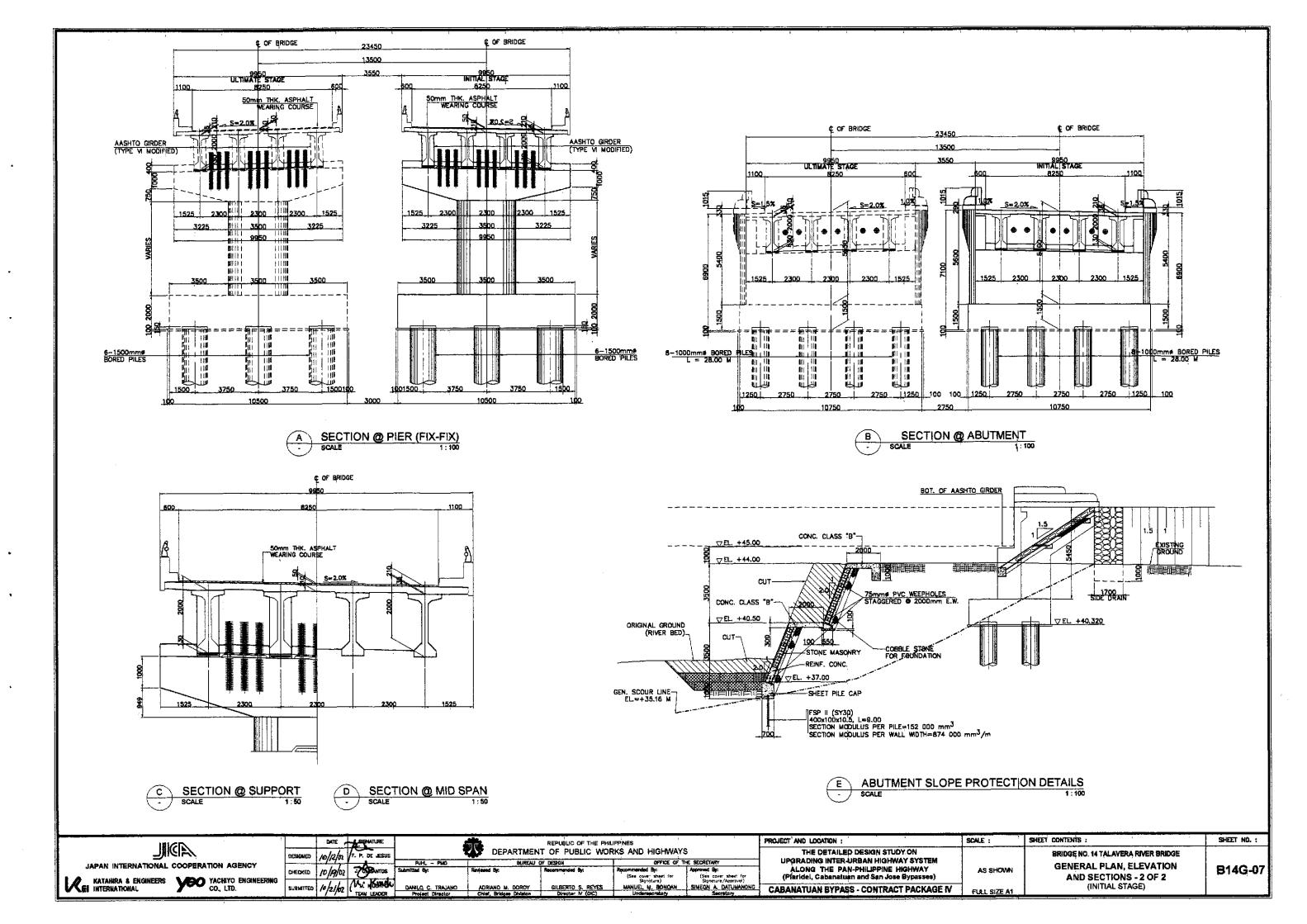
NOTE: * ABUTWENT COORDINATES ARE BASED ON COORDINATES OF BACK OF BACKWALL AT BRIDGE CENTERLINE.

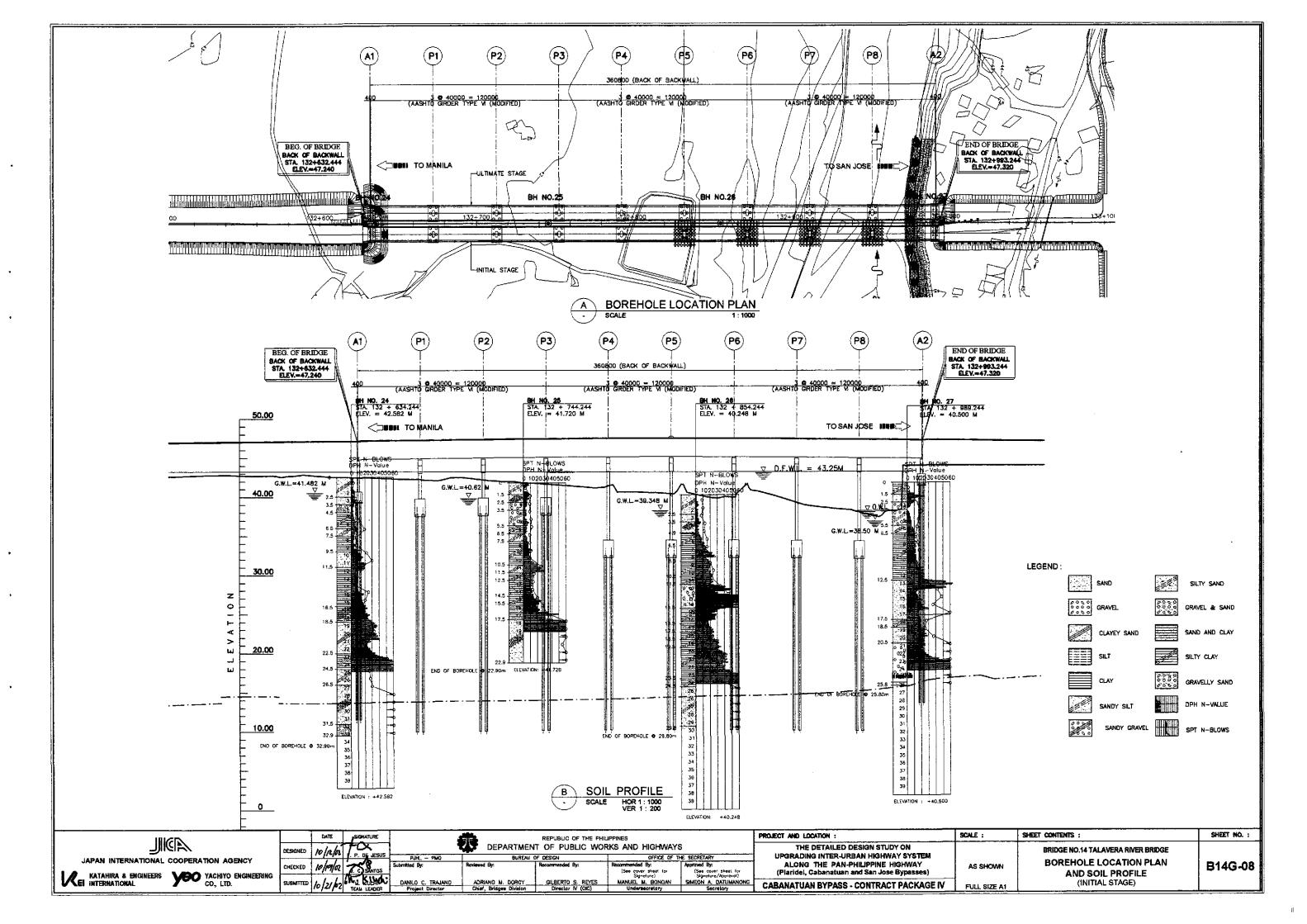
	L (COL			PILE CAP			L .	COPING									PILES			
PIER NO.	(mm)	(mm)	(mm)	35	(mm)	(mm)	(mm)	尘	(mm)	(mm) W	(mm)	(mm)	H3 (mm)	H4 (mm)	Į.) 35	HEE C) DE(LENGTH (mm)	NUMB
ABUT. 1	_	-	-	_	5000	10750	1500	40.24		-	<u>-</u> .		-	-	-	1	1	44.814	1000	28000	8
PIER 1	1800	3500	3250	43.045	6750	10500	2000	37.795	9950	2700	1750	1000	750	1850	1949	1000	949	44.695	1500	27500	8
PIER 2	1800	3500	3250	43.125	6750	10500	2000	37.875	9950	2700	1750	1000	750	1850	1949	1000	949	44.975	1500	27500	6
PIER 3	1800	3500	3250	43.205	6750	10500	2000	37.955	9950	2700	1750	1000	750	1850	1949	1000	949	45.055	1500	27500	6
PIER 4	1800	3500	8250	43.265	6750	10500	2000	33.015	9950	2700	1750	1000	750	1850	1949	1000	949	45.115	1500	22000	6
PIER 5	1800	3500	8250	43.285	6750	10500	2000	33.035	9950	2700	1750	1000	750	1850	1949	1000	949	45.135	1500	22000	6
PIER 6	1800	3500	8250	43_265	6750	10500	2000	33.015	9950	2700	1750	1000	750	1850	1949	1000	949	45.115	1550	22000	6
PIER 7	1800	3500	8250	43.205	6750	10500	2000	32.055	9950	2700	1750	1000	750	1850	1849	1000	849	45.055	1500	22000	6
PIER B	1800	3500	B250	43.125	6750	10500	2000	32.875	9950	2700	1750	1000	750	1850	1949	1000	949	44.975	1500	22000	6
ABUT, 2	1 -	_	_	-	5000	10750	1500	40.32	_	_		_	_	-	_	-	-	44.894	1000	28000	8

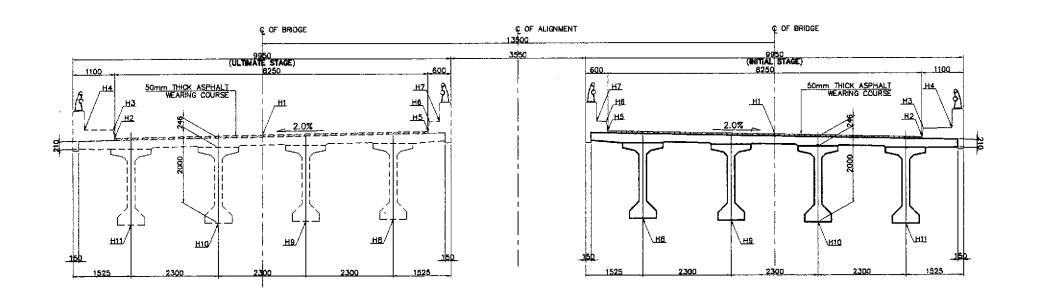
B GENERAL PLAN SCALE 1:1000

GENERAL PLAN, ELEVATION AND SECTIONS (TALAVERA RIVER BRIDGE CROSSING)
SCALE AS SHOWN

IBLICED DATE SIGNATURE						REPUBLIC OF THE PH	LIPPINES		PROJECT AND LOCATION :	SCALE :	SHEET CONTENTS :	SHEET NO. :
	DESIBNED	10/12/01	F. P. DEI JESUS		DEPARTMENT OF PUBLIC WORKS AND HIGHWAYS 1 PMO BUREAU OF DESIGN OFFICE OF THE SECRETARY				THE DETAILED DESIGN STUDY ON UPGRADING INTER-URBAN HIGHWAY SYSTEM		BRIDGE NO. 14 TALAVERA RIVER BRIDGE	
JAPAN INTERNATIONAL COOPERATION AGENCY	CHECKED	io ligilo	200	PJHL - PMO Submitted By:	Reviewed By:	OF DESIGN Recommended By:	Recommended By:	HE SECRETARY Approved By:	ALONG THE PAN-PHILIPPINE HIGHWAY	AS SHOWN	GENERAL PLAN, ELEVATION	B14G-06
KATAHIRA & ENGINEERS YOU YACHIYO ENGINEERING CO., LTD.	e soutten	to feel	Mic Kindu	DANKO C TRAJANO	ADRIANO M DORNY	GILBERTO S. REYES	(See cover sheel for Signature) MANUFL M. HONDAN	(See cover sheet for Signoture/Approval) SIMEON A. DATUMANONG	(Piaridel, Cabanatuan and San Jose Bypesses)		AND SECTIONS - 1 OF 2 (INITIAL STAGE)	
 W AEI INTERNATIONAL & CO., CID.	3/23/8/11/21/	ושוצושו	TEAM LEADER	Project Director	Chief, Bridges Division	Director W (DIC)	Undersecratory	Secretary	CABANATUAN BYPASS - CONTRACT PACKAGE IV	FULL SIZE A1	(MATERIA GIFAGE)	<u></u>



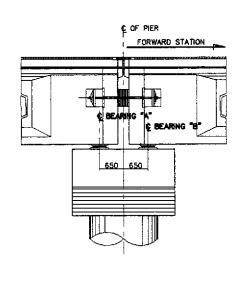




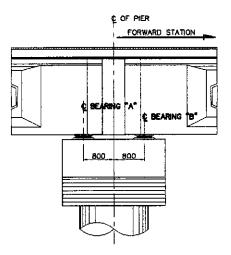


STATION	H1	H2	Н3	H4	H5	H6	H7
132+640.000	47.934	47.852	48.102	48.110	48,017	48.267	48.270
132+560.000	47.311	47.229	47.479	47.487	47.394	47.644	47.647
132+680.000	47.351	47.269	47.519	47.527	47.434	47.684	47.687
132+700.000	47.391	47.309	47.559	47.567	47.474	47.724	47.727
132+720.000	47.431	47,349	47.599	47,607	47,514	47.764	47.767
132+740.000	47.471	47.389	47.639	47.647	47,554	47.804	47.807
132+760.000	47.508	47.426	47.676	47.684	47.591	47.841	47.844
132+780.000	47.536	47.454	47.704	47.712	47.619	47.869	47.872
132+800.000	47.553	47.471	47.721	47.729	47.636	47.886	47.889
132+820.000	47.560	47.478	47.728	47.736	47.643	47.893	47.896
132+840.000	47.558	47,476	47.726	47.734	47.641	47.891	47.894
132+860.000	47.545	47.463	47.713	47.721	47.628	47.878	47.881
132+880.000	47.523	47.441	47.691	47.699	47.606	47.856	47.859
132+900.000	47,490	47.408	47.658	47.666	47.573	47.823	47.826
132+920.000	47,450	47.368	47.618	47.626	47.533	47.783	47.786
132+940.000	47,410	47.328	47.578	47.586	47.493	47.743	47.746
132+960.000	47.370	47.28B	47.538	47.546	47.453	47.703	47.706
132+980.000	47.330	47,24B	47.498	47.506	47,413	47.663	47.666

LOCATION	BEARING SIDE	STATION	H8	H9	H10	H11	
ABUT, A1	B	132+633.494	45.015	44.970	44.924		
DIFO 4	A	132+672.044	45.092	45.047	45.001	44.955	
PIER 1	8	132+673.644	45.096	45.050	45.004	44.958	
	А	132+712.044	45.172	45.127	45.081	45.035	
PIER 2	B	132+713.644	45.176	45.130	45.084	14.924 44.878 45.001 44.958 15.004 44.958 15.084 45.033 15.084 45.033 15.164 45.118 15.164 45.118 15.221 45.178 15.224 45.178 15.242 45.198 15.224 45.178 15.224 45.198 15.224 45.178 15.224 45.178 15.221 45.178	
PIER 3	Α	132+752.199	45.253	45.207	45.161	45.115	
PIER J	В	132+753.489	45.255	45.210	45,164	45.161 45.115 45.164 45.118 45.221 45.175 5.224 45.178	
PIER 4	A	132+792.044	45.312	45.267	45.221	45,175	
FIER 4	8	132+793.644	45,316	45.270	45.224	.001 44,955 .004 44,955 .081 45,035 .084 45,035 .165 45,115 .164 45,115 .221 45,175 .224 45,196 .224 45,196 .221 45,175 .221 45,196 .221 45,175 .221 45,175 .221 45,175	
PIER 5	A	132+832.044	45,334	45.288	45.242	45.196	
rigit j	Ð	132+633.644	45.334	45.288	45.242	45.196	
PIER 6	A	132+872.199	45.315	45.270	45.224	45.178	
FIER G	B	132+873.489	45.313	45.267	45.221	4 44.878 71 44.955 72 44.955 73 44.955 74 44.955 75 45.036 75 45.115 75 45.178 75 45.178 75 45.178 75 45.178 75 45.178 75 45.178 75 45.178 75 75 75 75 75 75 75 75 75 75 75 75 75 7	
PIER 7	A	132+912.044	45.256	45.210	45.164	45.118	
FILES 7	₽	132+913.644	45.252	45.207	45.161	45.118	
PIER 8	A	132+952.044	45,176	45.130	45.0B4	45.038	
CILL O	В	132+953.644	45.172	45.127	45.081	44.958 45.035 45.038 45.115 45.175 45.178 45.196 45.196 45.178 45.175 45.118 45.1038 45.038	
ABUT, A2	Α	132+992.194	45.095	45.050	45.004	44.958	









AUCD.		DATE	SIGNATURE	REPUBLIC OF THE PHILIPPINES			PROJECT AND LOCATION :	SCALE :	SHEET CONTENTS :	SHEET NO. :		
	DESIGNED	10/12/00	P. DE JESUS	1	DEPARTMENT OF PUBLIC WORKS AND HIGHWAYS				THE DETAILED DESIGN STUDY ON UPGRADING INTER-URBAN HIGHWAY SYSTEM		BRIDGE NO.14 TALAVERA RIVER BRIDGE	i
JAPAN INTERNATIONAL COOPERATION AGENCY	CHECKED	10/19/0	J. C. SANTOS	Submitted By:	Reviewed By:	OF DESIGN Recommended By:	Recommended By: (See cover sheet for	HE SECRETARY Approved By: (See cover sheet for	ALONG THE PAN-PHILIPPINE HIGHWAY (Plaridel, Cabanatuan and San Jose Bypasses)	AS SHOWN	TABLE OF ELEVATIONS	B14G-09
KATAHIRA & ENGINEERS YOU YACHIYO ENGINEERING CO, LTD.	SUBMITTED	10/21/01	M. KING	DANILO C. TRAJANO Project Director	ADRIANO M. DOROY Chief, Bridges Division	GILBERTO S. REYES Director IV (OIC)	Signature) MANUEL M. BONOAN Undersecretary	Signature/Approve() SIMEON A DATUMANONG Secretary	CABANATUAN BYPASS - CONTRACT PACKAGE IV	FULL SIZE A1	(INITIAL STAGE)	

SUMMARY OF QUANTITIES TALAVERA RIVER BRIDGE CROSSING (BRIDGE NO. 14)

ITEM NO.	DESCRIPTION	UNIT	QUANTITIES	REMARKS
PART F	BRIDGE CONSTRUCTION			
1	SUPERSTRUCTURE			
310(2)	Asphaft Mixture Wearing Course (t=50mm) Incl. Took Coat	m ²	2,970.00	
401(2)a	Steel Rolling Type A for (Angot, Tolovera and approach of Pompongo Bridge)	m	720.00	
SPL 401(3)c	Bridge Name Plate, 1000 x 600 for Talavera Bridge	e C.	2	
404(1)	Reinforcing Steel (Grade 40)	kg.	212,183,00	
404(2)	Reinfarcing Steel (Grade 60)	kg.	91,653.00	-
405(1)f	Structural Concrete Class AA2 (fc=28 Mpa, max. aggregate 20mm) for Long Bridge Superstructures	m3	979.69	
405(3)	Structural Concrete Class C (fc=21 MPa, max. aggregate 12mm) for Thin Reinforced Members	m3	315.36	
406(1)	Precest Prestressed Structural Concrete Member (AASHTO Girder Type VI Modified L=39.40m)	60.	12	
406(1)m	Precest Prestressed Structural Concrete Member (AASHTO Girder Type VI Modified L=39.55m)	60.	24	
407(1)e	Elestemenic Bearing Pad (600x400x50mm)	en.	72	
407(2)b	Expansion Joint, Multiflex M100 (Bostomeric) ±50mm movement	m	40.80	
SPL 407(3)o	Restroining Bor Ø32mm x 1495mm	ė0.	12	
SPL 407(3)b	Restroining Bor Ø32mm x 1900mm	60.	12	
407(4)	G.I Drain Pipe 2150mm for Bridge Ordinage	m	154.98	
n	SUBSTRUCTURE	_		
103(2)o	Bridge Excavation above CWL (Common Soil)	m3	2,126.43	
103(2)c	Bridge Exervation below OW. (Common Soli)	_{m3}	2,507.46	
104(4)	Embankment from Borrow (Selected Granular Material) for Bridge	m3	681.40	
200(1)	Aggregote Subbase Course	Em	30.34	
400(16)0	Cast-in-place Concrete Bared Piles Ø 1000mm		448.00	
400(16)c	Cast-in-place Concrete Bored Piles Ø 1500mm	m	1,155.00	
400(21)	Static Pile Lood Test for @1500mm Bored Piles	ec.	2	
404(1)	Reinforcing Steel (Grade 40)	kg.	18,334.00	
404(2)	Reinforcing Steel (Grade 60)	kg.	489,578.00	
405(1)e	Structural Concrete Class AA1 (fc=28 Mpd, max. aggregate 20mm) for Long Bridge Substructures	_m 3	2,160.28	· · · · · · · · · · · · · · · · · · ·
405(6)	Lean Concrete (fc=17 MPa max. aggregate 38mm)	m3	70.87	
SPL 311(2)	PCC Pavernent (Reinforced) t=300mm Approach Slab	m²	91.24	
SPL 400(23)a	High Strain Dynamic Pile Test for Ø1000mm Bored Piles	● Q.	1	
SPL 400(24)	Pila Integrity Test for Bared Piles of Various Diameter	60.	22	
SPL 900(3)	Provisional Sum for Geotechnical investigation	1.3.	1	
W .	REVETMENT (RIVERBANK PROTECTION)			
103(1)	Structure Excovation	m ³	3,489.00	
104(3)	Embankment from borrow pit	m3	682.64	
405(1)a	Structural Concrete Class A (fc=21 Mpa, max. aggregate J&mm) for Heavily Reinf. Structures	_m 3	149.73	
405(2)	Structural Concrete Class 8 (1'c=17 MPa, max. aggregate 50mm) for Ploin or Lightly Reinf. Structures	m ³	173.19	
504(5)	Grouted Riprop Class A	m ³	14.05	
505(1)	Stone Masonry	m ³	464.39	
506(1)	Hand Laid Rock Apron (Loose Boulder Apron)	m ³	138.00	
507(2)b	Steel Sheet Piles (400mmx85mm), furnish & driven	m	5,040.00	
509(1)	Gabions, (2.0 x 1.0 x 0.50)	m ³	1,213.50	
510(1)	Rubble Concrete Slope Protection	m ³	124.42	
SPL 407(5)c	Pier Protection Concrete Blacks for Talavera Bridge	m ²	896.00	
N	TEMPORARY WORKS			
SPL 420(4)c	Temporary Craneway for Talavera Bridge Construction	m	80.00	
SPL 420(5)c	Temporary Access Road (Causeway) for Tolovera Bridge Construction	m	300.00	
SPL 420(6)d	Temporary Cofferdam for Pier Construction (Tolavera Bridge)	80.	3	
V	ELECTRICAL WORKS			
SPL 620(4)c	Bridge Lighting Poles (Single Lamp)	en.	12	•
SPL 620(4)d	Street Lighting Service Pole with Panel	en.	1	

IIIGD	REPUBLIC OF THE PHILIPPINES		PROJECT AND LOCATION :	SCALE :	SHEET CONTENTS:	SHEET NO. :						
	DESIGNED	10/2/02	M. S. NAVAL	1			RKS AND HIGHWAY		THE DETAILED DESIGN STUDY ON		8RIDGE NO.14 TALAVERA RIVER BRIDGE	
JAPAN INTERNATIONAL COOPERATION AGENCY	CHECKED	10/19/m	J/C SANTOS	FUHL — PMO Submitted By:	Reviewed By:	Recommended By:	Recommended By:	THE SECRETARY Approved By:	UPGRADING INTER-URBAN HIGHWAY SYSTEM ALONG THE PAN-PHILIPPINE HIGHWAY	AS SHOWN	SUMMARY OF QUANTITIES	B14G-10
KATAHIRA & ENGINEERS YOU YACHIYO ENGINEERING CO., LTD.		10/1	Mi Kruste	GANILO C. TRAJANO	ADRIANO M. DOROY	CHIRCRITA E DEVES	(See cover sheet for Signature) MANUEL M. BONDAN	(See cover sheet for Signature/Approval) SIMEON A DATUMANONG	(Plaridel, Cabanatuan and San Jose Bypasses)		(INSTIAL CTACE)	D 140-10
CU, CID.	SUBMITTED	10/21/02	TEAN LEADER	Project Director	Chief, Bridges Division	Director N (OIC)	Lindersscratery	Secretary	CABANATUAN BYPASS - CONTRACT PACKAGE IV	FULL SIZE A1	(INITIAL STAGE)	

