APPENDIX 9.4-1

Bridges – Plaridel Bypass (Initial & Ultimate Stage)

BRIDGE NO. 1 (PLARIDEL BYPASS-INITIAL AND ULTIMATE STAGE)

1. RIVER CONDITION

- The riverbed is composed of clay with gravel & sand with its riverbank in stable condition.
- The upstream & downstream conditions of riverbed & river banks are stable.
- No evidence of drifts/debris during flooding
- The river discharge at 50 year return period is 125.40 cum/sec
- The river velocity at 50 years return period is 2.699 m/sec.
- The top river width at design flood level is 29.0 m.

2. SOIL / GROUND CONDITION

- The site is covered by 1.00m to 3.00m grayish silty clay, followed by a very thick layer of very dense sand and gravel.
- N-value at the upper 1.00mm to 7.50m depth ranges from 4 to 40 and after which N-values > 50 is consistent towards the end of borehole.

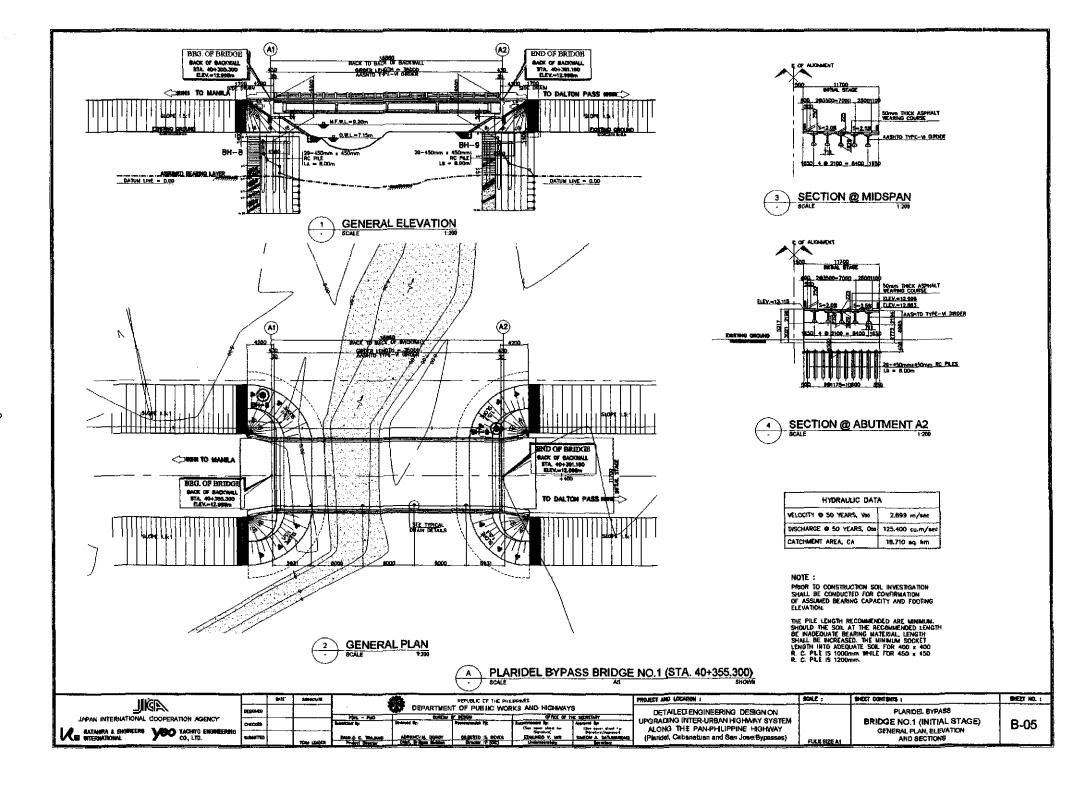
3. DESCRIPTION OF BRIDGE

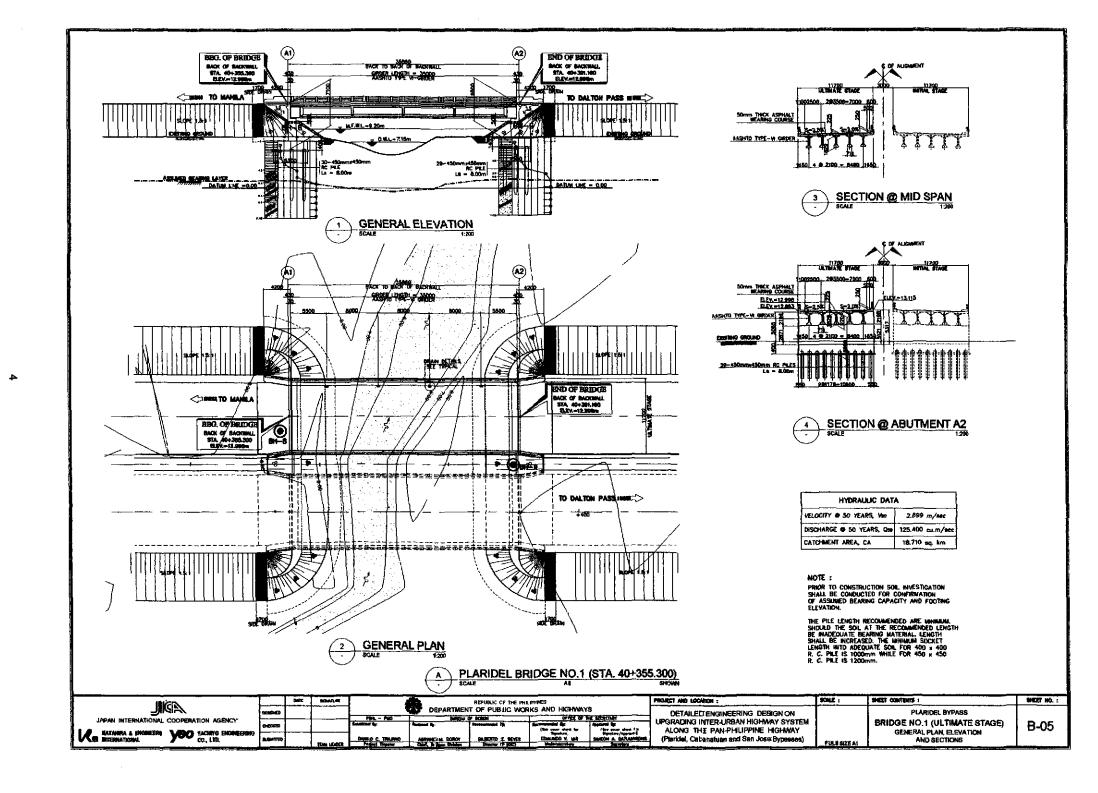
(1) SELECTION OF SPAN COMPOSITION AND BRIDGE TYPE

	INITIAL STAGE	ULTIMATE STAGE
NO. OF BRIDGE	1	1
BRIDGE LENGTH	35.86 m	35.86 m
CARRIAGEWAY WIDTH	2 x 3.50m	2 x 3.50m
SHOULDER WIDTH	2.50 m	2.50 m
SIDEWALK WIDTH	1.10m + 0.60m	1.10m + 0.60m
SUPERSTRUCTURE TYPE	1-span, PCDG Type VI (35m)	1-span, PCDG Type VI (35m)
SUBSTRUCTURE TYPE	Closed-type Abutment	Closed-type Abutment
FOUNDATION TYPE	RC Piles (450 x 450mm)	RC Piles (450 x 450mm)

- The top width of river during the design flood is 29.0 m. To minimize river encroachment considering abutment slope protection, the bridge length becomes 35.86m for a single-span bridge.
- A single Span bridge is proposed due to :
 - Two-span bridge is not recommended since one pier will have to be located at the center of the river and may cause problems on river hydraulics.
 - A three span bridge tends to be more expensive because of additional substructure cost.
- Pile foundation is proposed due to presence of soft soil layer.

• Grouted Riprap Riverbank Protection with Loose Boulder Apron provided at toe.





PROPOSED BRI		Survey Date				Bridge No./Statio		
Married Williams of the Control of t			∃ypass N	ame	PLARIDEL		Prepared by	ACP / ENS
Location : PLARIDEL	River Name : BRIU DSED ALIGNMEAT SEE STEER	River Condition River Width (m): 8.0 Stream Velocity (m/s): 0.30 During Flood: 0.30 Chennel Slope (%):	s. Plan			Rice flow	field	Proposed Centerline
	Liands.	Upstream : Downstream :	b. Section / Elev	ation		Rice	field	
	BR: NO. 1	Site Access During Construction: Possible Difficult	2.00 m.	*******		8.00		
Photograph		Comments:	2.50 m.		***	Ş UNI		
<u> </u>		Meander Situation	Downstream	Existing	None	: 		·····
		Riverbed Material	Large Gravel	☐ Gravel	Send	♥ \$oll	Olhers	
114	The second		Upstream	Stable - (೧೯ □	icouring/Eroding	- L R With	Protection/Revetment - L R
		Riverside/Bank Condition	Downstream	- ektasiz 🗹				Protection/Revetment - L R
	100 100 100 100 100 100 100 100 100 100	Tendency to River Course and Riverbed Change	Stable Scour	Ing As	zgradetion	Degradation	Sedment Transport	Olhers
March 1995		Flood signs	₩ None	lng -				
Land Use	Commercial Industrial Residential Plagricultural Others	Evidence of Orift & Debris	None ☑ Exis	ling	Driftwoo Others	xds, Φ=	0.5 - 0.7 Length	± 3.0·3.5
Geographical Features / Terrain	Mountenous area Aluviel Plain Cthers	Maximum Flood Water Level ☑ Interview ☐ Flood Data ☐ Flood Marks	Height above bank (m) Frequency (yrs) Cause	0.30		Comments:	NONE	
Stream / River Type	Stratght Braided Meandaring	Recommendation for Bridge	Verify maximum flo	ood water leve	l. Provide 1.	50m clearand	e from MFWL to sof	It of superstructure.
Nearby Structures	Upstream Single span 12.0m long bridge at 450m from site. Downstream Gridge at 4.6km from site.	Recommendation for River Protection Works	Provide rubble co	ncrete slope	protection	on road emb	ankment.	
Environmental Condition	☐ Fish & Fish Habitat							
Water Level (During survey)	0.50m 0.50m 1.0m 1.0m 2.5m	Comments / Others						

Table 3.4-6 Bridge No. 1 Site Condition

BRIDGE NO. 2 (PLARIDEL BYPASS-INITIAL AND ULTIMATE STAGE)

1. RIVER CONDITION

- The riverbed is composed of clayey silt underlain by the granular deposits consisting of gravel and sand.
- The upstream and downstream conditions of riverbed & river banks are stable.
- No evidence of drifts/debris during flooding.
- Meandering river course.
- The river discharge at 50 year return period is 124.90 cum/sec.
- The river velocity at 50 year return period is 2.798 m/sec.
- The top river width at design flood level is 25.0 m.
- The bridge is skewed at 15°.

2. SOIL / GROUND CONDITION

- The riverbed is composed of silty clay underlain by the granular deposits consisting of gravel and sand.
- The upper clay deposits (6 to 9m) in depth have N-values range of 4 to 10m.

3. DESCRIPTION OF BRIDGE

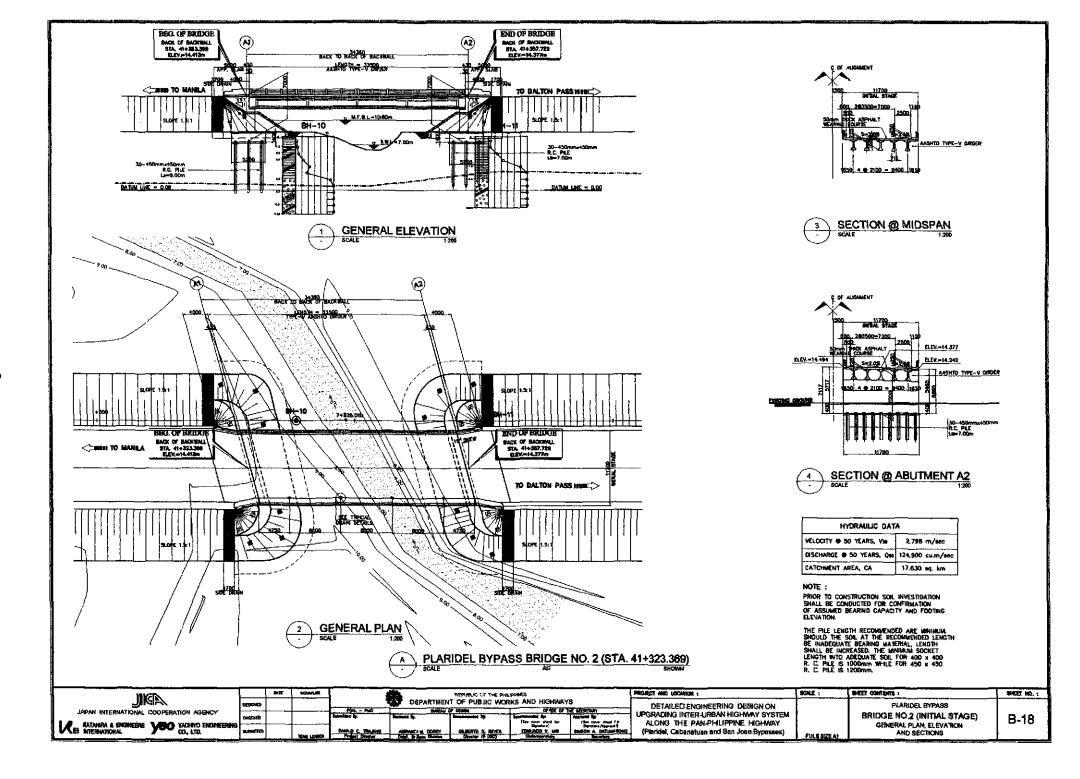
(1) SELECTION OF SPAN COMPOSITION AND BRIDGE TYPE

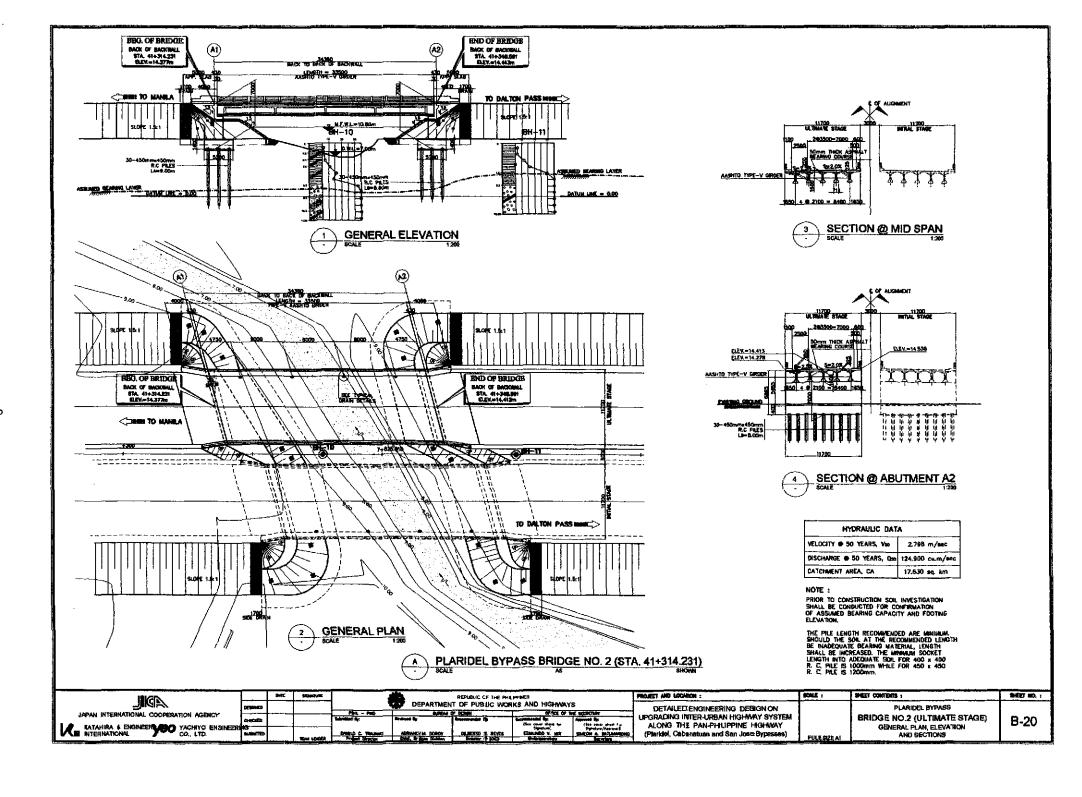
	INITIAL STAGE	ULTIMATE STAGE
NO. OF BRIDGE	11	1
BRIDGE LENGTH	34.36 m	34.36 m
CARRIAGEWAY WIDTH	2 x 3.50m	2 x 3.50m
SHOULDER WIDTH	2.50 m	2.50 m
SIDEWALK WIDTH	1.10m + 0.60m	1.10m + 0.60m
SUPERSTRUCTURE TYPE	1-span, PCDG Type V (33.5m)	1-span, PCDG Type V (33.5m)
SUBSTRUCTURE TYPE	Closed-type Abutment	Closed-type Abutment
FOUNDATION TYPE	RC Piles (450 x 450mm)	RC Piles (450 x 450mm)

- The top width of river during the design flood is 25.0 m. To minimize river encroachment considering abutment slope protection, the bridge length becomes 34.36m for a single-span bridge.
- A single Span bridge is proposed due to :
 - Two-span bridge is not recommended since one pier will have to be located at the center of the river and may cause problems on river hydraulics.

- o A three span bridge tends to be more expensive because of additional substructure cost.
- Pile foundation is proposed due to presence of soft soil layer.

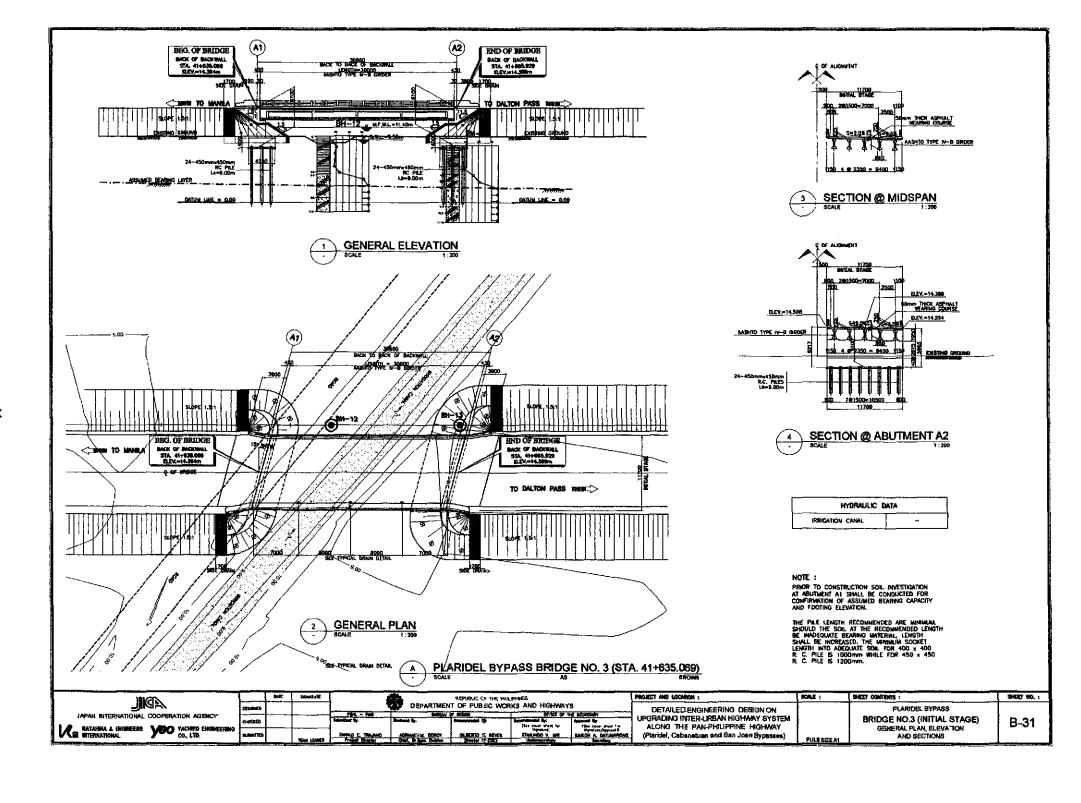
- Since river velocity during worst flooding is relatively slow (2.798 m/sec), the bridge utilizes Grouted Riprap Riverbank Protection.
- Grouted Riprap will be protected by Loose Boulder Apron at toe.

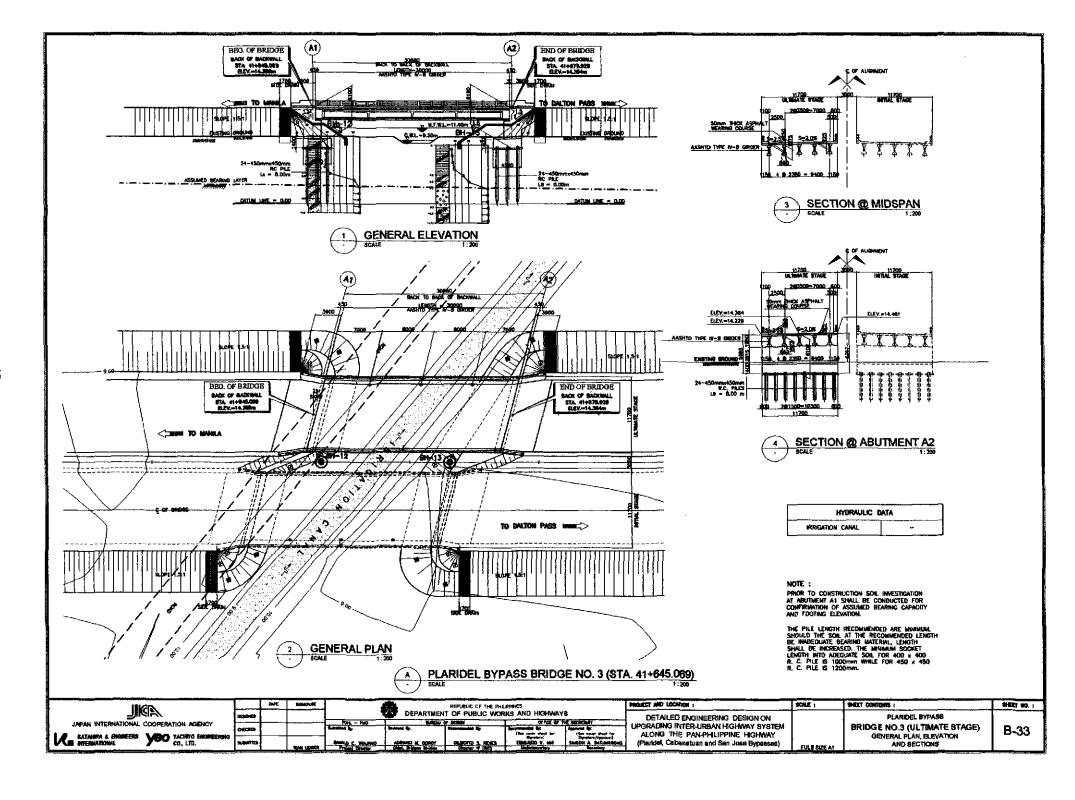




PROPOSED BRIDGE SITE SURVEY				te	19-Jun-01		Bridge No./Station	Bridge No. 2 / STA 7+808.800
			Bypass Na	ame	PLARIE	DEL	Prepared by	ACP/ENS
Location : PLARIDEL	River Name:	River Width (m): 8.0	a. Plan		Rice F	ield		
(And	OSED ALIGNMENT	Stream Velocity (m/s): Normal 0.6 During Flood :				flow	Proposed	1 Centerline
and the second	(Bin State) (Bin State) (Bin State)	Channel Slope (%): Upstream: Downstream:			Fish P	ond		
		Site Access During Construction: Possible Difficult	b. Section / Eleva			8.0 m. → MFWL		-
Photograph		Comments:	4.00 m.		***/	Ş. OWL	\(\sigma_{200} \)	%
			3.0 m.	· · · · · · · · · · · · · · · · · · ·		10000		
ERS!		Meander Situation	Upstream Downstream	☑ Existing	None None		and the second	
		Riverbed Material	Large Gravel	Gravel		₩ 80#	Others	
			Upstream				***************************************	ection/Revetment - L R
T.Care		Riverside/Bank Condition	Downstream		00 🗆 ::			ection/Revetment - L R
75.5	¥ ****	Tendency to River Course and Riverbed Change	Stable Scoun	ing A	ogradation [] (Degradation [Sediment Transport	Others
	· 	Flood signs	Mone Exist	ing -				
Land Use	Commercial Industrial Residential Agricultural Others	Evidence of Drift & Debris	None Exist	ing	Driftwood Others:	я, Ф=	Length =	
Geographical Features / Terrain	Mountemous area Alluvial Plain Others	Maximum Flood Water Level Maximum Flood Water Level Flood Data Flood Marks	Height above bank (m) Frequency (yrs) Cause	1.20	C	Comments :		
Stream / River Type	Straight Braided Meandering	Recommendation for Bridge	 	od water leve	el. Provide 1.50	Om clearance	from MFWL to soffit of	superstructure.
Nearby Structures	Upstream Irrigation canal 500m from alte.	Recommendation for River	 					
Nearby Structures	Downstream Single span 12.0m long bridge at 470m from site.	Protection Works						
Environmental Condition	▼ Trees/v egetation		1					
Water Level (During survey)	0 - 0.60m	30 Comments / Others						

Table 3.4-7 Bridge No. 2 Site Condition





PROPOSED BRIDGE SITE SURVEY			Survey Date 19-Jun-01			Bridge No		Bridge No. 3 / STA 8+126.000	
				Bypass N	ame	PLARIDEL	Prepar	ed by	GTR / ENS
Location : PLARIDEL	River Name:	IRRIGATION CANAL	River Condition River Width (m): 7.2	a. Plan		Rice Field			
San-		Canerda Charges 1	Stream Velocity (m/s): Normal 0.30 During Flood:				low 1	Propose	d Centerline
		NO. 5)	Channel Slopa (%): Upstream : Downstream :		-	Acces Roa	d I		
PROPOS	EDYLIGHT		Site Access During Construction: V Possible T Difficult	b. Section / Elev	ation		7.2 m.	NIA RO	AD 2000
Photograph			Comments:	1.5 m.					
April Light	en en		10 Meander Situation	Upstream Downstream		✓ None			
2 2000			11 Riverbed Material	Large Gravei			Soil 🔽 Others		GONCRETÉ
		- 13 - 13	12 Riverside/Bank Condition	Upstream Downstream	Stable · L		g/Eroding - L R		ection/Revetment - LOFO
		era ^{era}	Tendency to River Course and Riverbed Change	Stable Scoul	ing Aggr	adation Degra	adation Sediment Tr	ensport	Othera
			14 Flood signs	None Exist	ing -				
^	Compared Cindustrial C 5	lesidential Agricultural Others	15 Evidence of Drift & Debris	None Exis	ing	Others :	D =	Length =	
3 Land Use Geographical Features / Terrain	Mounterious area Alluvia		Maximum Flood Water Level 16 Interview Flood Data Flood Marks	Height above bank (m) Frequency (yrs) Cause		Сони	nerks:		
5 Stream / River Type	Straight Braided	Meandering	17 Recommendation for Bridge	Verify maximum flo	ood water level.	Provide 1.0m c	learance from MFWI	and/or from	n bank.
6 Nearby Structures	Upstream Imigation canal num Downstream Single span 12.0m	ing parallet. long bridge at 900m from site.	Recommendation for River Protection Works						
7 Environmental Condition	Trees/v egetation	Fish & Fish Habitat			 -				
8 Water Level (During survey)	☐ 0 - 0.50m ☐ 0.50m - 1.0m	7 1.0m - 1.5m Others	19 Comments / Others						

BRIDGE NO. 3 (PLARIDEL BYPASS-INITIAL AND ULTIMATE STAGE)

1. RIVER CONDITION (IRRIGATION CANAL)

- The riverbed is composed of clayey silt underlain by the granular deposits consisting of gravel and sand.
- The upstream and downstream conditions of riverbed & river banks are stable.
- No evidence of drifts/debris during flooding.
- Meandering river course.
- The top river width at design flood level is 25.0 m.
- The bridge is skewed at 15°.

2. SOIL CONDITION

- The riverbed is composed of silty clay underlain by the granular deposits consisting of gravel and sand.
- Gravel and sand deposit is overlain uppermost plastic silt or clay with thickness ranging from 6.5 to 8.5 meters and after which N-values > 50 is consistent towards the end of borehole.

3. DESCRIPTION OF BRIDGE

(1) SELECTION OF SPAN COMPOSITION AND BRIDGE TYPE

	INITIAL STAGE	ULTIMATE STAGE
NO. OF BRIDGE	1	1
BRIDGE LENGTH	30.86 m	30.86 m
CARRIAGEWAY WIDTH	2 x 3.50m	2 x 3.50m
SHOULDER WIDTH	2.50 m	2.50 m
SIDEWALK WIDTH	1.10m + 0.60m	1.10m + 0.60m
SUPERSTRUCTURE TYPE	1-span, PCDG Type IV-B (30m)	1-span, PCDG Type IV-B (30m)
SUBSTRUCTURE TYPE	Closed-type Abutment	Closed-type Abutment
FOUNDATION TYPE	RC Piles (450 x 450mm)	RC Piles (450 x 450mm)

- The top width of river during the design flood is 25.0 m. To minimize river encroachment considering abutment slope protection, the bridge length becomes 30.86m for a single-span bridge.
- A single Span bridge is proposed due to :
 - Two-span bridge is not recommended since one pier will have to be located at the center of the river and may cause problems on river hydraulics.
 - A three span bridge tends to be more expensive because of additional substructure cost.

• Pile foundation is proposed due to presence of soft soil layer.

(3) PROTECTION AGAINST SCOURING

• Grouted riprap will be provided for abutment protection.

BRIDGE NO. 4 (PLARIDEL BYPASS-INITIAL AND ULTIMATE STAGE)

1. RIVER CONDITION

- The riverbed is composed of clayey silt underlain by the granular deposits consisting of gravel and sand.
- The upstream and downstream conditions of riverbed & river banks are stable.
- No evidence of drifts/debris during flooding.
- Meandering river course with riverbank heavily vegetated.
- The river discharge at 50 year return period is 109.20 cum/sec.
- The river velocity during 50 year flooding is 3.624 m/sec.
- The top river width at design flood level is 18.0 m.
- The bridge is skewed at 10°.

2. SOIL CONDITION

- The riverbed is composed of silty clay underlain by the granular deposits consisting of gravel and sand.
- Gravel and sand deposit is overlain uppermost plastic silt or clay with thickness ranging from 7.5 to 9.5 meters and after which N-values > 50 is consistent towards the end of borehole.

3. DESCRIPTION OF BRIDGE

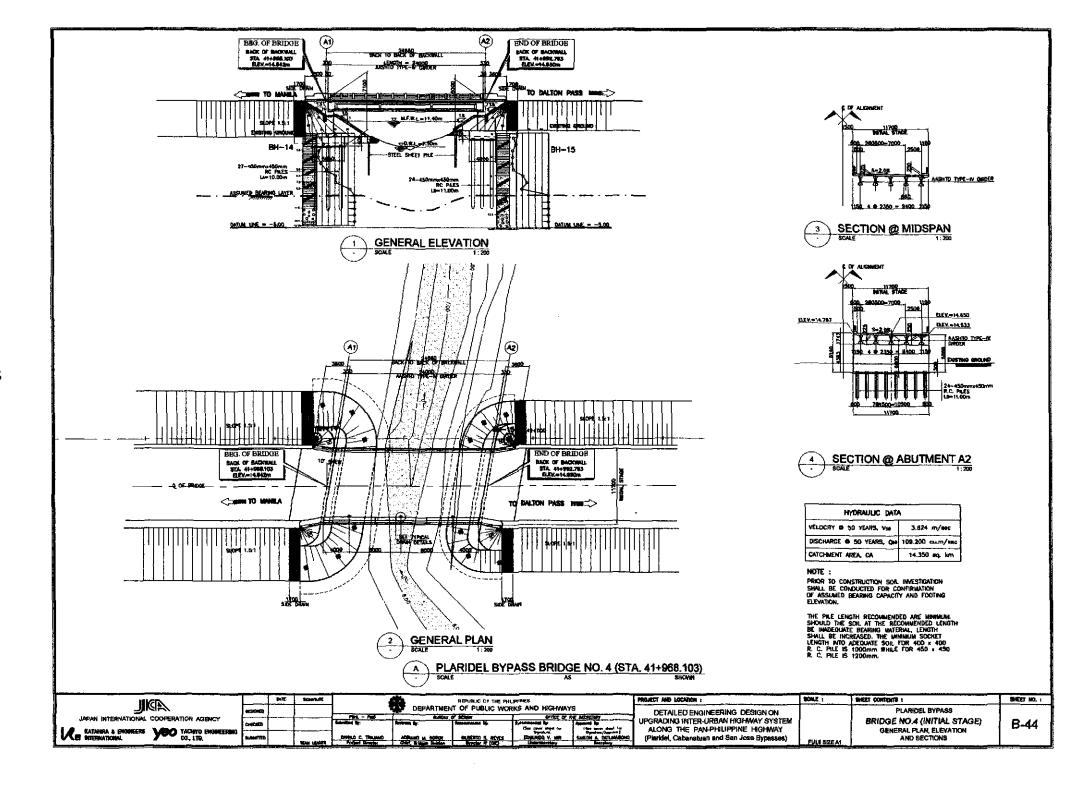
(1) SELECTION OF SPAN COMPOSITION AND BRIDGE TYPE

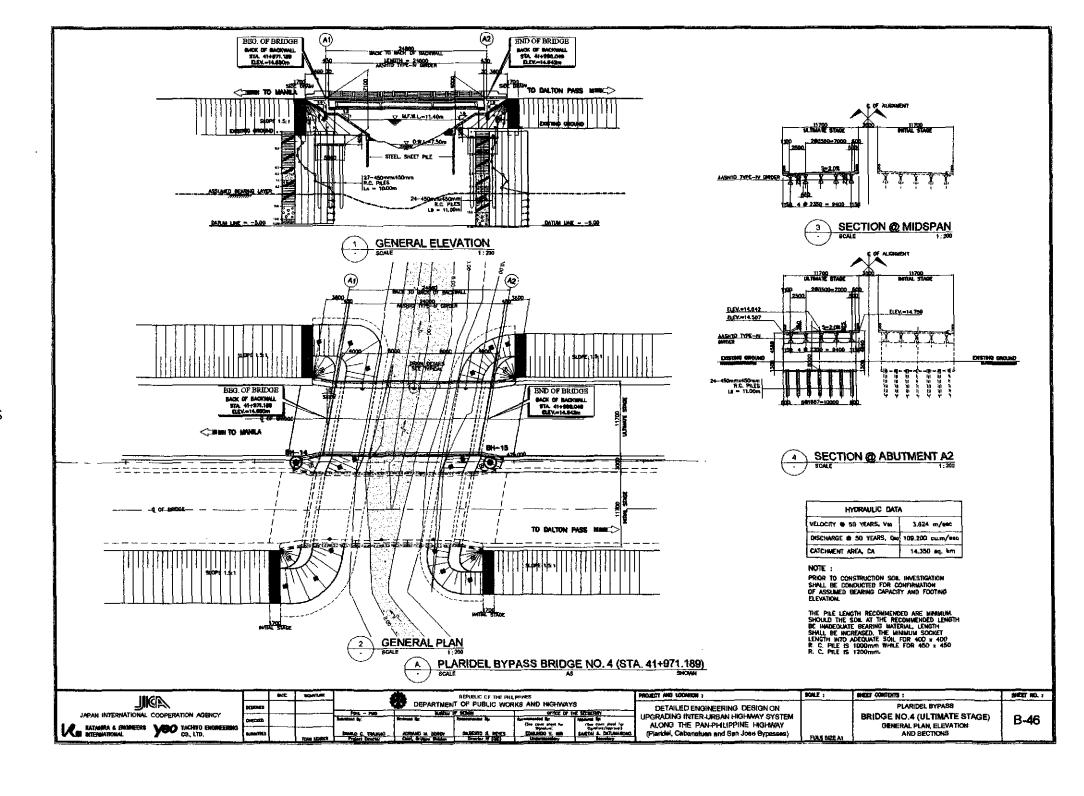
	INITIAL STAGE	ULTIMATE STAGE
NO. OF BRIDGE	1	1
BRIDGE LENGTH	24.86 m	24.86 m
CARRIAGEWAY WIDTH	2 x 3.50m	2 x 3.50m
SHOULDER WIDTH	2.50 m	2.50 m
SIDEWALK WIDTH	1.10m + 0.60m	1.10m + 0.60m
SUPERSTRUCTURE TYPE	1-span, PCDG Type IV (24m)	1-span, PCDG Type IV (24m)
SUBSTRUCTURE TYPE	Closed-type Abutment	Closed-type Abutment
FOUNDATION TYPE	RC Piles (450 x 450mm)	RC Piles (450 x 450mm)

- The top width of river during the design flood is 18.0 m. To minimize river encroachment considering abutment slope protection, the bridge length becomes 24.86m for a single-span bridge.
- A single Span bridge is proposed due to :
 - Two-span bridge is not recommended since one pier will have to be located at the center of the river and may cause problems on river hydraulics.

- o A three span bridge tends to be more expensive because of additional substructure cost.
- Pile foundation is proposed due to presence of soft soil layer.

- Since water velocity is relatively fast (3.624 m/sec), DPWH criteria requires Rubble Concrete River Bank Protection.
- River Bank Protection will be supported by steel sheet piles penetrating thru 5 meters of silty clay.





PROPOSED BRI					Bridge No. 4 / STA 8+452.800			
			Bypass N	ame	PLARIO	EL.	Prepared by	ACP/ENS
Location : PLARIDEL An an annex to be a series of the ser	River Name: BR. NO. 4 Market State	River Width (m): 150 Stream Velocity (m/s): Normal : 0.40 During Flood :	b. Section / Eleve		Rice Fi	······································	tiony	d Centerline
E DE SE		Meander Situation	Upstream Downstream Large Graval	Existing Existing Gravel	None Sand	▼ so#	Others	
			Upstream				*	scilon/Revelment - L R
		Riverside/Bank Condition	L					ection/Revelment - L R
		Tendency to River Course and Riverbed Change	Stable Scouri	ng 🗆 A	ggradellon 🔲 D	Degradation [Sediment Transport	Others
		L Flood signs	None Existi	ng -				
Land Use	Commercial Industrial Residential Agricultural Chera	Evidence of Drift & Debris	☑ None ☐ Existi	ing	Oriftwoods Others :	в, Ф=	Length =	
	The second of th	Maximum Flood Water Level	Height above bank (m)	0.50	ļ Ca	omments :	····	
Geographical Features / Terrain	Mountérious area Alluviel Plain Citrers	✓ Interview ☐ Flood Data ☐ Flood Marks	Frequency (yrs)					
Stream / River Type	Straigni Braidoo V Meandering	Recommendation for Bridge	Verify maximum flo	od water leve	t. Provide 1,50	Im clearance	from MFWL to soffit of	superstructure.
Nearby Structures	Upstream Three-celled 12.0m box culvert at 5.0km from site.	Recommendation for River	 		 			
	Downstream (Irrigation canal running parallel.	Protection Works						
Environmental Condition	Trees/vegatation Fish & Fish Habitat		į.					
Water Level (During survey)	0 - 0,50m 0.60m - 1.0m 1.0m - 1.5m C Others 2	Comments / Others						

Table 3.4-9 Bridge No. 4 Site Condition

BRIDGE NO. 5 (PLARIDEL BYPASS-INITIAL AND ULTIMATE STAGE)

1. RIVER CONDITION

- The riverbed is composed of clayey silt underlain by the granular deposits consisting of gravel and sand.
- The upstream and downstream conditions of riverbed & river banks are stable.
- No evidence of drifts/debris during flooding.
- Meandering river course with riverbank heavily vegetated.
- The river discharge at 50 year return period is 85.60 cum/sec.
- The river velocity during 50 year flooding is 1.298 m/sec.
- The top river width at design flood level is 26.0 m.
- The bridge is skewed at 15°.

2. SOIL CONDITION

- Bridge site is underlain by highly weathered bedrock.
- Bedrock layer is covered by thick, stiff residual clay and clayey silt, which probably derived from underlying parent rock.
- N-value at the upper 12 m thick is generally not more than 20 blows while the subsequent depths ranged between 30 to 50 blows.

3. DESCRIPTION OF BRIDGE

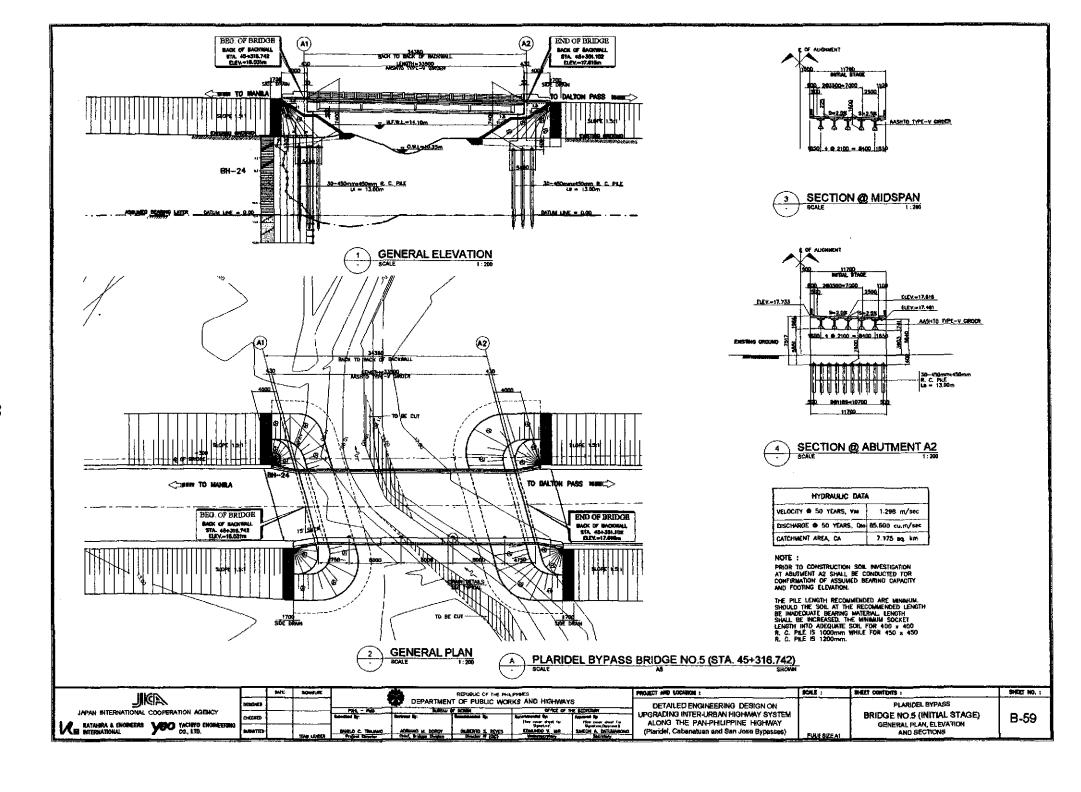
(1) SELECTION OF SPAN COMPOSITION AND BRIDGE TYPE

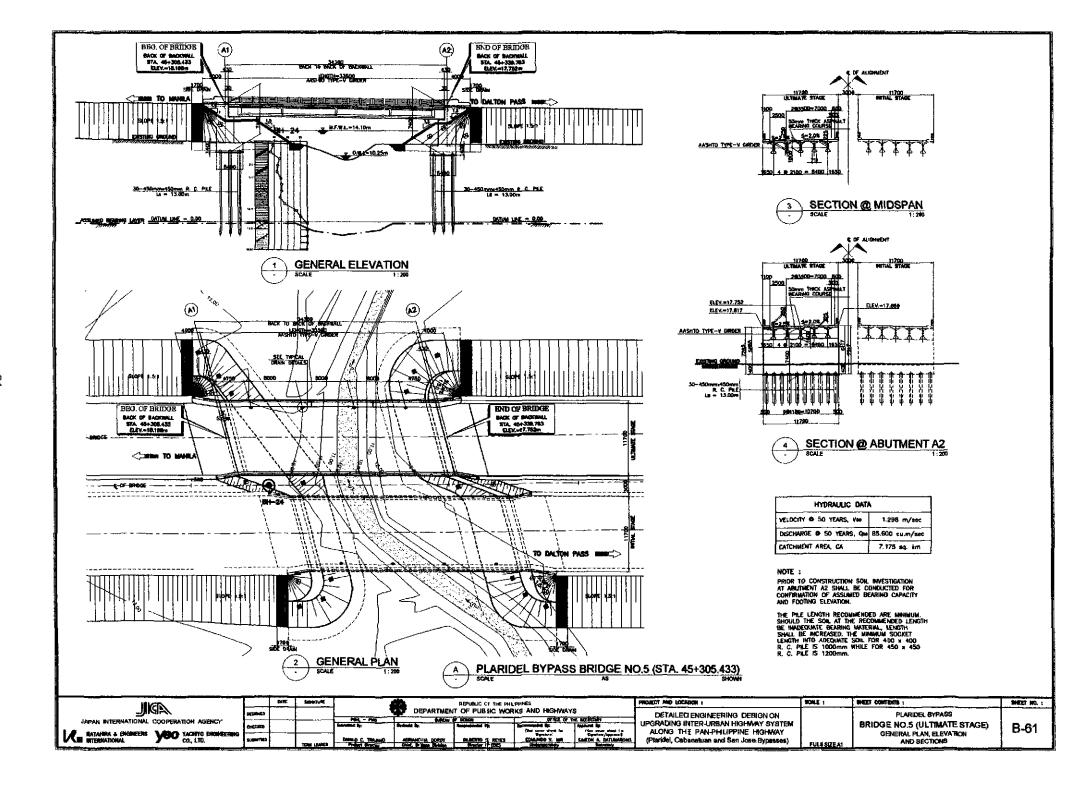
	INITIAL STAGE	ULTIMATE STAGE
NO. OF BRIDGE	1	1
BRIDGE LENGTH	24.86 m	24.86 m
CARRIAGEWAY WIDTH	2 x 3.50m	2 x 3.50m
SHOULDER WIDTH	2.50 m	2.50 m
SIDEWALK WIDTH	1.10m + 0.60m	1.10m + 0.60m
SUPERSTRUCTURE TYPE	1-span, PCDG Type V (33.5m)	1-span, PCDG Type V (33.5m)
SUBSTRUCTURE TYPE	Closed-type Abutment	Closed-type Abutment
FOUNDATION TYPE	RC Piles (450 x 450mm)	RC Piles (450 x 450mm)

- The top width of river during the design flood is 26.0 m. To minimize river encroachment considering abutment slope protection, the bridge length becomes 24.86m for a single-span bridge.
- A single Span bridge is proposed due to :
 - Two-span bridge is not recommended since one pier will have to be located at the center of the river and may cause problems on river hydraulics.

- o A three span bridge tends to be more expensive because of additional substructure cost.
- Pile foundation is proposed due to presence of soft soil layer.

- Since river velocity during worst flooding is relatively slow (1.298m/sec), the bridge utilizes grouted riprap river bank protection.
- Grouted riprap protected by hand laid rocks at the toe.





PROPOSED BRIDGE SITE SURVEY				Survey Date		19-Jun-01		Bridge No./Station	Bridge No. 5 / STA 11+800.200
Location : BUSTOS	River Name :		Phor Condition	Bypass N	ame	PLARIDE	L	Prepared by	ACP/ENS
	River Name :		River Condition River Width (m): 10.0 Stream Velocity (m/s): Normal: 0.02 During Flood:	b. Section / Elev	ation	Pr	Rice Fi	flaw_ Rice	Field
The Linkscone	Bis. No. 6	Materialy	Site Access During Construction: Possible Difficult	_		10,0	0 m.	****	
Photograph	j.		Comments:	3,0 m.			OWL		
3.75	ين. م		Meander Situation	Upstream Downstream		☐ None	<u></u>		
	Action 1889		Riverbed Material	Large Gravel	<u> </u>		₹ Soll	Others	z organyfilling gandan
			Miver Dear Indiental	Upstream	Stable -			7	ection/Revelment - L R
146_1			Riverside/Bank Condition	L	Stable -				action/Revelment - L R
7.5			Tendency to River Course and Riverbed Change	Stable Scoun	ring Aggr	radallon De	egradation [Sediment Transport	Olhers
1445			Flood signs	▼ None	Ing -				
Land Use	Commercial Industrial Residenti	at 🗸 Agricultural 🗖 Others	Evidence of Drift & Debris	☑ None ☐ Exist	ling	Driftwoods, Others:		Length =	
			Maximum Flood Water Level	Height above bank (m)	0.50	Cor	mments:		
Geographical Features / Terrain	Mountehous area Allovial	Plain Others	☐ Flood Marks	Frequency (yrs) Cause					
Stream / River Type	Straight Braided Mean	dering	Recommendation for Bridge		ood water level.	Provide 1.50r	m clearance	from MFWL to soffit or	superstructure.
Nearby Structures	Upstream Abandoned timber bridge at Downstream Irrigation canal running para		Recommendation for River Protection Works	Provide river ban	k slope protect	ion.			
Environmental Condition	Trees/vegetation	Frish & Fish Habitet		-					
Water Level (During survey)	0 - 0.50m 0.50m - 1.0m 2 1.0	m - 1.5m Olhers	Comments / Others						

Table 3.4-10 Bridge No. 5 Site Condition

BRIDGE NO. 6 (PLARIDEL BYPASS-INITIAL AND ULTIMATE STAGE)

1. RIVER CONDITION (IRRIGATION CANAL)

- The riverbed is composed of silty clay underlain by the granular deposits consisting of gravel and sand.
- The upstream and downstream conditions of riverbed & river banks are stable.
- Evidence of drifts/debris during flooding.
- Meandering river course with heavily vegetated riverbank.
- The top river width at design flood level is 35.5 m.
- The bridge is skewed at 15°.

2. SOIL CONDITION

- Bridge site is covered by 1.0 to 9.0 m of silty clay underlain by sand and gravel.
- Bedrock layer is covered by thick, stiff residual clay and clayey silt, which
 probably derived from underlying parent rock.
- N-value at the upper 12 m thick is generally not more than 20 blows while the subsequent depths ranged between 30 to 50 blows.

3. DESCRIPTION OF BRIDGE

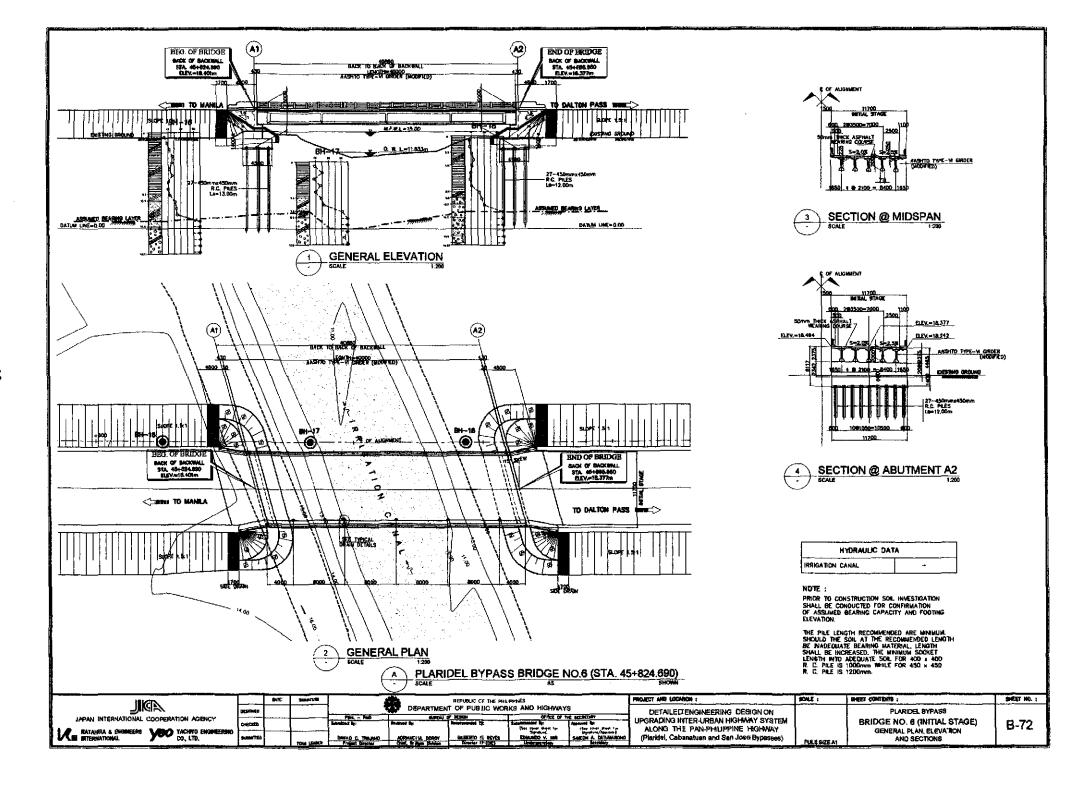
(1) SELECTION OF SPAN COMPOSITION AND BRIDGE TYPE

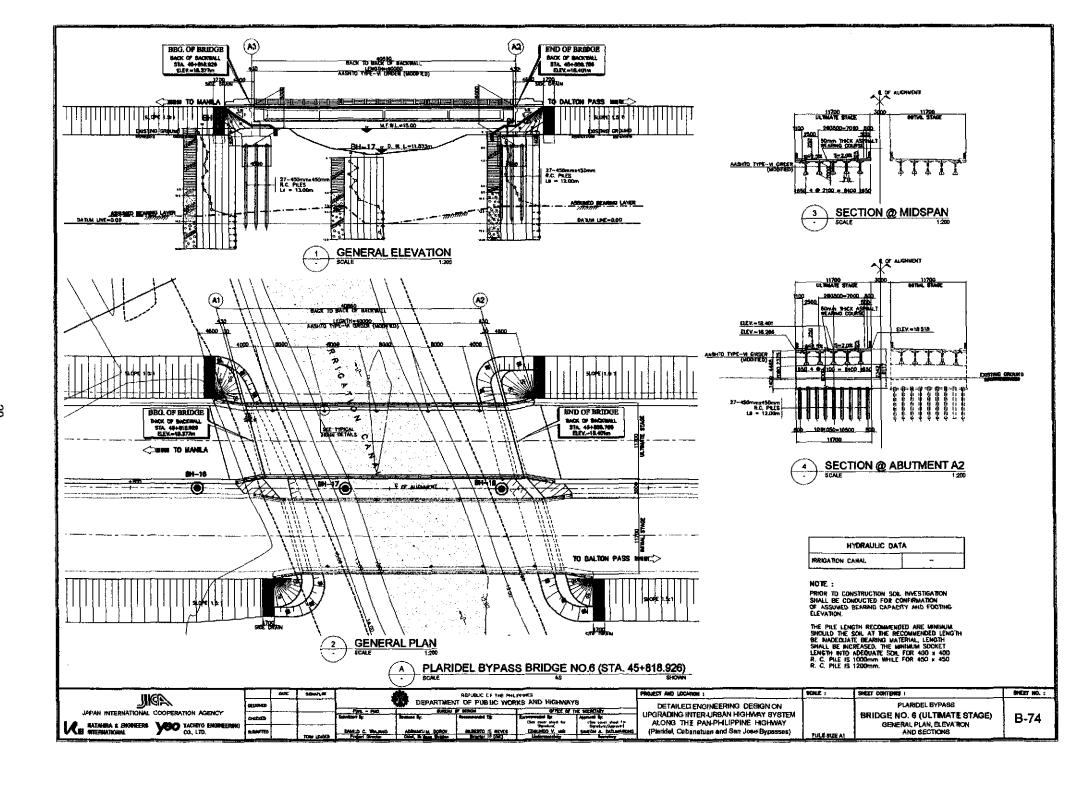
	INITIAL STAGE	ULTIMATE STAGE
NO. OF BRIDGE	1	1
BRIDGE LENGTH	40.86 m	40.86 m
CARRIAGEWAY WIDTH	2 x 3.50m	2 x 3.50m
SHOULDER WIDTH	2.50 m	2.50 m
SIDEWALK WIDTH	1.10m + 0.60m	1.10m + 0.60m
SUPERSTRUCTURE TYPE	1-span, PCDG Type VI-Modified (40m)	1-span, PCDG Type Vi-Modified (40m)
SUBSTRUCTURE TYPE	Closed-type Abutment	Closed-type Abutment
FOUNDATION TYPE	RC Piles (450 x 450mm)	RC Piles (450 x 450mm)

- The top width of river during the design flood is 35.5 m. To minimize river encroachment considering abutment slope protection, the bridge length becomes 40.86m for a single-span bridge.
- A single Span bridge is proposed due to :
 - Two-span bridge is not recommended since one pier will have to be located at the center of the river and may cause problems on river hydraulics.

- o A three span bridge tends to be more expensive because of additional substructure cost.
- Pile foundation is proposed due to presence of soft soil layer.

• Grouted riprap will be provided for abutment protection.





PROPOSED BRIDGE SITE SURVEY						19-Jun-01 Bridge No./Station		Bridge No. 6 / STA. 12+310 500	
				Вуравь №	ame	PLARI	DEL	Prepared by	GTR/ENS
Location : BUSTOS PAOPE Padro Guitaturia Photograph	River Name : IRRI Lanawan Usaga Manta BR NO. 6 Tagany D	GATION CANAL	River Condition River Width (m): 29.3 Stream Velocity (m/s): 0.15 During Flood: 0.15 Channel Slope (%): Upstream: 0.15 Downstream: 0.15 Site Access During Construction: Pessible 0fficuit Comments:	a. Plan b. Section / Eleve 0.25 m. 2.00 m.		Rice F	flow	3 m.	od Centerline 5.3 m. HA ROAD
		3	Meander Situation Riverbed Material	Upstream Downstream Large Gravel	Existing Existing Gravel	✓ None ✓ None	☑ 501	Olhers	
			Riverside/Bank Condition	Upstream Downstream	Stable - (rection/Revelment - LORO
	h e		Tendency to River Course and Riverbed Change	Stable 5cour	Ing Ag	gradalion	Oegradation	Sediment Transport	Others
			Flood signs	☑ None 🔲 Existi	ing •				
	Commercial Industrial Resident		Evidence of Drift & Debris	☐ None 🗹 Exist	Ing	Driftwood Others: \		Length =	
Land Use	C commercial C unnarrue. C vesicibili	IN EL Witchman Custo	Maximum Flood Water Level	Height above bank (m)			Comments :		<u> </u>
Geographical Features / Terrain	☐ Mountenous area ☐ Alluvial ☑	Plain [] Others		Frequency (yrs)	ļ				
Stream / River Type	Straight Braided W Mean	ndering	Recommendation for Bridge	Verify maximum fix	ood water leve	ł. Provide 1.0	m clearance	from MFWL and/or fro	m top of bank.
Nearby Structures	Upstream 26.0m long bridge at 450m Downstream Single span 22m long bridge		Recommendation for River		- · - · - · · · · ·				
Environmental Condition	Downstream Single span 22m long bridg	ge at 1.70km from site.	Protection Works	 		· · · · · · · · · · · · · · · · · · ·			
Water Level (During survey)	0 - 0.50m 0.50m - 1.0m 1.0	<u></u>	Comments / Others						

Table 3.4-11 Bridge No. 6 Site Condition

BRIDGE NO. 7 (PLARIDEL BYPASS-INITIAL AND ULTIMATE STAGE)

1. RIVER CONDITION

- The riverbed is composed of silty clay underlain by the granular deposits consisting of gravel and sand.
- The upstream and downstream conditions of riverbed & river banks are stable.
- Evidence of drifts/debris during flooding.
- River bank is heavily vegetated.
- Located nearby is a three-celled 24.5m long bridge structure with sluice gate.
- The river discharge at 50 year return period is 14.70 cum/sec.
- The river velocity during 50 year flooding is 1.16 m/sec.
- The top river width at design flood level is 37.5 m.
- The bridge is skewed at 15°.

2. SOIL CONDITION

- Bridge site is covered by 1.0 to 8.0 m of silty clay underlain by sand and gravel.
- Bedrock layer is covered by thick, stiff residual clay and clayey silt, which
 probably derived from underlying parent rock.
- N-value at the upper 12 m thick is generally not more than 20 blows while the subsequent depths ranged between 30 to 50 blows.

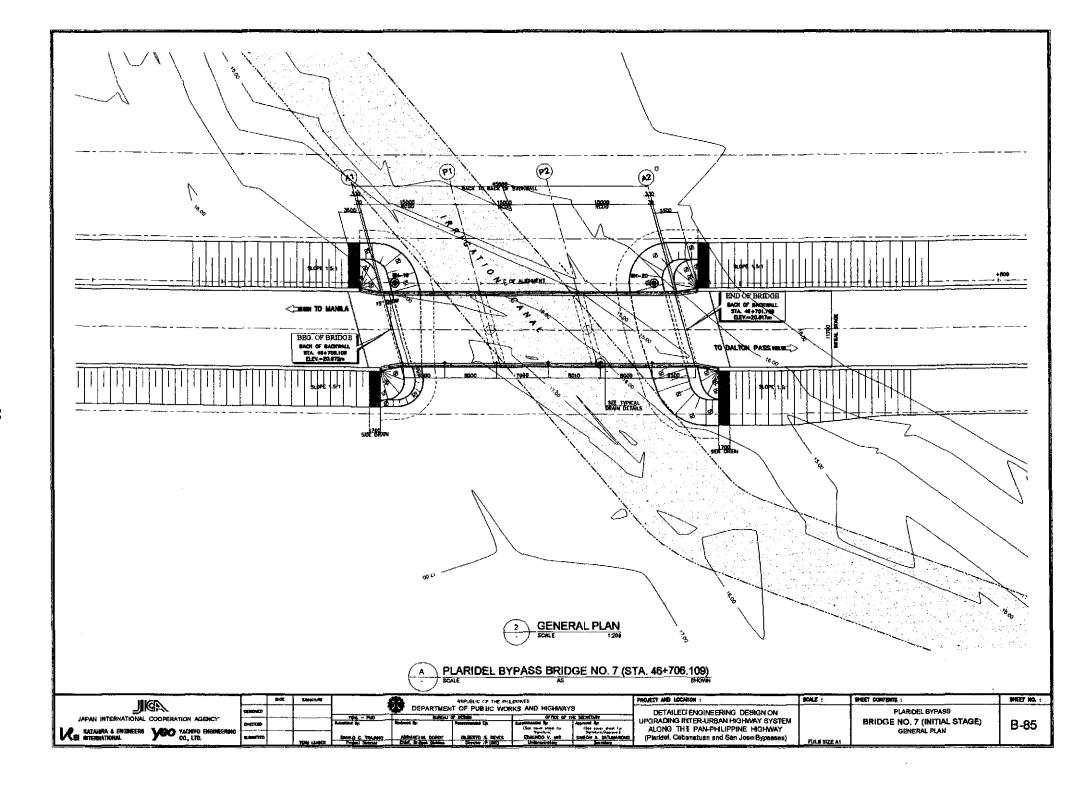
3. DESCRIPTION OF BRIDGE

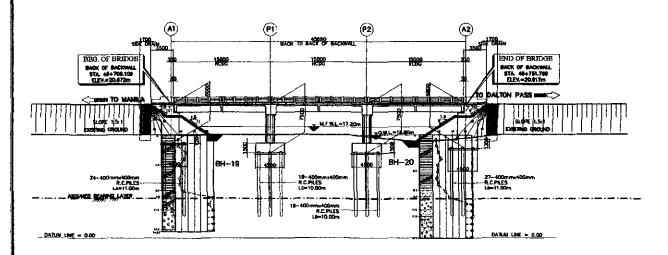
(1) SELECTION OF SPAN COMPOSITION AND BRIDGE TYPE

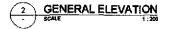
	INITIAL STAGE	ULTIMATE STAGE			
NO. OF BRIDGE	1	11			
BRIDGE LENGTH	45.66 m	45.66 m			
CARRIAGEWAY WIDTH	2 x 3.50m	2 x 3.50m			
SHOULDER WIDTH	2.50 m	2.50 m			
SIDEWALK WIDTH	1.10m + 0.60m	1.10m + 0.60m			
SUPERSTRUCTURE TYPE	3-span, RCDG (15.00+15.00+15.00m = 45.0m)	3-span, RCDG (15.00+15.00+15.00m = 45.0m)			
SUBSTRUCTURE TYPE	Closed-type Abutment / Single Column Pier	Closed-type Abutment / Single Column Pier			
FOUNDATION TYPE	RC Piles (400 x 400mm)	RC Piles (400 x 400mm)			

- A three span bridge was proposed being more economical than a 4-span bridge and more hydraulically advantageous than a 2-span bridge.
- Although more economical, a 2-span bridge with its pier located at center
 of river will have problems in case of debris collision to pier column and
 local scour at pier foundation.

- Since river velocity during worst flooding is relatively slow (1.924m/sec), the bridge utilizes Grouted Riprap Riverbank Protection.
- Grouted Riprap protected by hand laid rocks at the toe.







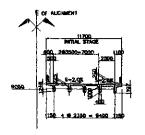
HYDRAULIC DATA							
VELOCITY • 60 YEARS, Van	1.16 m/sec						
DISCHARGE # 50 YEARS, Diss	14.70 cu.m/sec						
CATCHMENT AREA, CA	1.350 mg km						

NOTE :

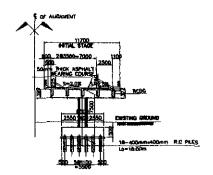
PRIOR 10 CONSTRUCTION SOM INVESTIGATION SHALL BE CONDUCTED FOR CONFIRMATION OF ASSUMED BEARING CAPACITY AND FOOTING ELEVATION.

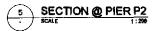
THE PILE LENGTH RECOMMENDED ARE MINIMUM. SHOULD THE 50H. AT THE RECOMMENDED LENGTH BE MADEQUATE BEARING MATERIAL LENGTH SHALL SE, INCREASED, THE MINIMUM SOCKET LENGTH MOTO ADEQUATE SOIL FOR 40D x 40D R. C. PILE IS 1000mm WHILE FOR 450 x 430 R. C. PILE IS 1000mm WHILE FOR 450 x 450 R. C. PILE IS 1200mm.

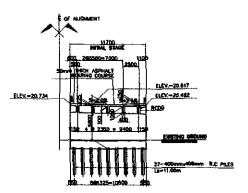


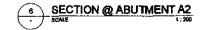


SECTION @ SUPERSTRUCTURE

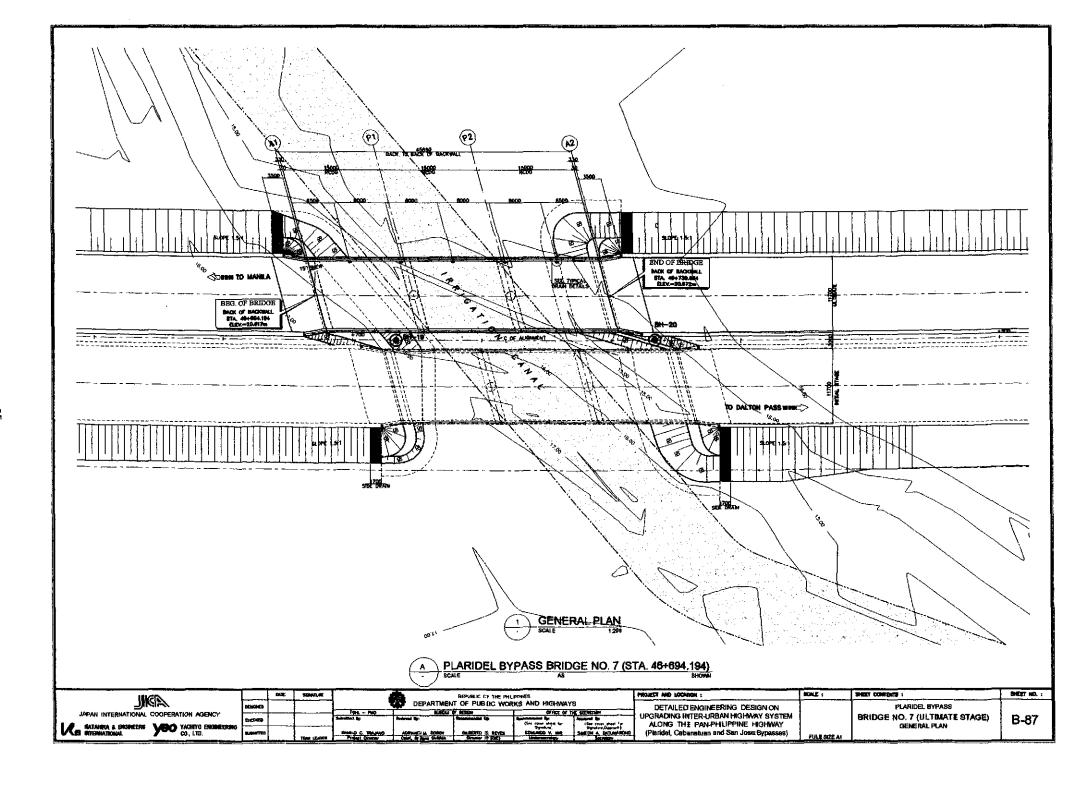


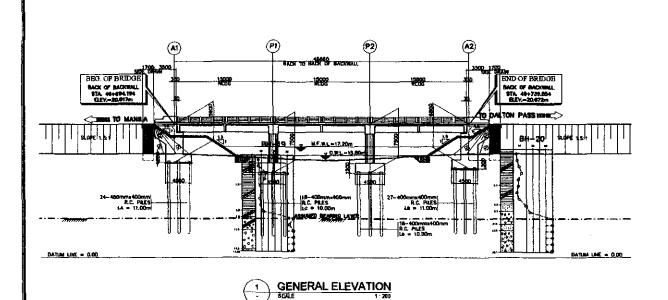


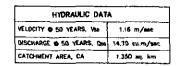




MICE		QAYE:	SOUTH					PROJECT MID LOCKRON :	SCALE :	SHEET CONTRIES :	SHIDEY NO. :	
	PERMIT					T OF PUBLIC WOR		'S	DETAILECTENGINEERING DESIGN ON		PLARIDEL BYPASS	
1949AN INTERNATIONAL COOPERATION AGENCT	54000			Seturbial Sp.	Personal ly			Appendix By:	UPGRADING INTER-URBAN HIGHWAY SYSTEM ALONG THE PAN-PHILIPPINE HIGHWAY		BRIDGE NO. 7 (INITIAL STAGE)	B-86
RATAHRA & ENGINEERIS YOU YACHIYO ENGINEERISMO CO., LED.	SUPPORTED.		Time utabilit	DANKS C. TRUNKS	ADDRAMCI M. DOPOT	MARCHO IL REVES	COMMON MA	SACON A DATUMATIONS	(Plandel, Cabanatuan and San Jose Bypasses)	FULE SIZE A1	GENERAL ELEVATION AND SECTIONS	



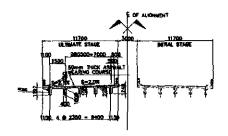




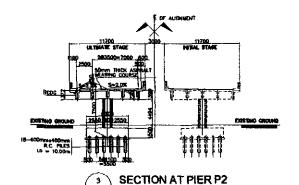
NOTE :

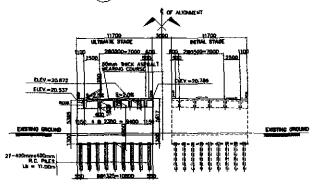
PRIOR TO CONSTRUCTION SOIL INVESTIGATION AT ABUTMENT AT AND AZ SHALL BE CONDUCTED FOR CONFRINATION OF ASSUMED BEARING CAPACITY AND FOOTING ELEVATION.

THE PRE LENGTH RECOMMENDED ARE MINIMUM. SHOULD THE 50K AT THE RECOMMENDED LENGTH BE WASCEAUSE BEARDY MATERIAL, LENGTH SHALL BE WACKASED. THE MINIMUM SOCKET LENGTH WITH A DECLARE SOUL FOR 400 × 400 R. C. PILE IS 1000mm WHALE FOR 450 N 450 R. C. PILE IS 1200mm.



2 SECTION AT SUPERSTRUCTURE
1:200





SECTION AT ABUTMENT A2
1: 200

A PLARIDEL BYPASS BRIDGE NO.7 (STA. 46+694.194) SOLE AS BROWN

IUGIS		BATE	Salvature			REPUBLIC OF THE PHIL			PROJECT AND LOCATION :	HOME :	SHEET CONFINES :	MACET NO. :
	DESMONED			· · · · · · · · · · · · · · · · · · ·	DEPARTMEN	TOF PUBLIC WOR			DETAILED ENGINEERING DESIGN ON		PLARIDEL SYPASS	
JAPAN INTERNATIONAL COOPERATION AGENCY	CHECKER			Bahastat Br.	Ambout Sp:	Recommended Ele	Accordance by:	Append by	UPGRADING INTER-URBAN HIGHWAY SYSTEM		BRIDGE NO. 7 (ULTMATE STAGE)	B-88
KATAHRA A ENGINEERS YOU YACHIYO ENGINEERING	SI BANTERS			DANS C The said	ADBANCIAL DOROY	dudom s neces	(See cover ofers for Signature) EDULANDO V. WHI	SPREDN V DVIDNOVADNOV Spanjave/patrona &	ALONG THE PAN-PHILIPPINE HIGHWAY (Planidel, Cabanatuan and San Jose Bypasses)		GENERAL ELEVATION AND SECTIONS	1 2-00
V SM HICKMANN. DJ., LVII.		أحيطا	TON LEADER	Propert Menter	54, 6 to 1	Director 19 (02)	Undergrowing	Secretary	(Invest, Commission and Con Ocea Cypnesses)	FULE SIZE A1	AND SECTIONS	<u> </u>

PROPOSED BRI	DGE SITE SURVEY		Survey Da		19-Ju		Bridge No./Station	Bridge No. 7 / STA 13+157.650
		cont.	Вуразь Na	me	PLAR	IDEL	Prepared by	ACF/ENS
Photograph	River Name : ###################################	River Condition River Width (m): 25.0 Stream Velocity (m/s). Normal 0.005 During Flood: Channel Slope (%). Upstream: Downstream: Site Access During Construction: Possible Difficult Comments:	b. Section / Eleva	,	Resid	· · · · · · ·) m.	2.8 m.
		Meander Situation Riverbed Material	Upetream Cownstream Large Gravel Upstream	Existing Existing Gravel	None None Sand	Soll	Others With Pre	tection/Revetment - L R
		Riverside/Bank Condition				couring/Eroding -		lection/Revelment - L R
		Tendency to River Course and Riverbed Change	Steble Scouri		ggradalion	Degradation [Sediment Transport	Olhers
		Flood signs	None Existin	ng -				
Land Use	Commercial industrial Residential Agricultural Chers	Evidence of Drift & Debris	None Existin	ng	Oriftwoo Others	ds, Φ≐ Water lilies, tres	Length =	
		Maximum Flood Water Level	Height above bank (m)			Comments:		
Geographical Features / Terrain	Mountengus area Alluvial Plain Others	✓ Interview ☐ Flood Data ☐ Flood Marks	Frequency (yrs) Cause					
Stream / River Type	☑ Streight Braided Meandering	Recommendation for Bridge	 -	od water leve	el. Provide 1.	50m clearance	e from MFWL to soffit of	f superstructure.
Nearby Structures	Upstream Downstream Three-ceiled 24.50m long bridge w/ siulce gate at 300m from s	Recommendation for River	Provide river bank	slope prote	ection.			
Environmental Condition	☐ Trees/v egetation ☐ Fish & Fish Habitat							
Water Level (During survey)	0 - 0.50m 0.60m - 1.0m 1.5m 7 Others 2:	Comments / Others						

Table 3.4-12 Bridge No. 7 Site Condition

BRIDGE NO. 9 (PLARIDEL BYPASS-INITIAL AND ULTIMATE STAGE)

1. RIVER CONDITION

- The riverbed is composed of silty clay underlain by the granular deposits consisting of gravel and sand.
- Evidence of drifts/debris during flooding.
- Meandering river course.
- The river discharge at 50 year return period is 119.80 cum/sec.
- The river velocity during 50 year flooding is 1.924 m/sec.
- The top river width at design flood level is 35.0 m.
- The bridge is skewed at 15°.

2. SOIL CONDITION

- Bridge site is covered by 1.0 to 12.0 m of silty clay underlying tuffaceous siltstone and tuffaceous sandstone.
- Bedrock was encountered at a depth of 12.5 m and 13.5 m respectively.
- N-value at the upper 12 m thick is generally not more than 20 blows while the subsequent depths ranged between 30 to 50 blows.

3. DESCRIPTION OF BRIDGE

(1) SELECTION OF SPAN COMPOSITION AND BRIDGE TYPE

	INITIAL STAGE	ULTIMATE STAGE
NO. OF BRIDGE	1	1
BRIDGE LENGTH	40.86 m	40.86 m
CARRIAGEWAY WIDTH	2 x 3.50m	2 x 3.50m + 1.0m Median + 3.50m FRONTAGE
SHOULDER WIDTH	1.25 m	<u> </u>
SIDEWALK WIDTH	1.50m + 0.60m	1.50m + 0.60m
SUPERSTRUCTURE TYPE	1-span, PCDG Type VI-Modified (40m)	1-span, PCDG Type VI-Modified (40m)
SUBSTRUCTURE TYPE	Closed-type Abutment	Closed-type Abutment
FOUNDATION TYPE	RC Piles (450 x 450mm)	RC Piles (450 x 450mm)

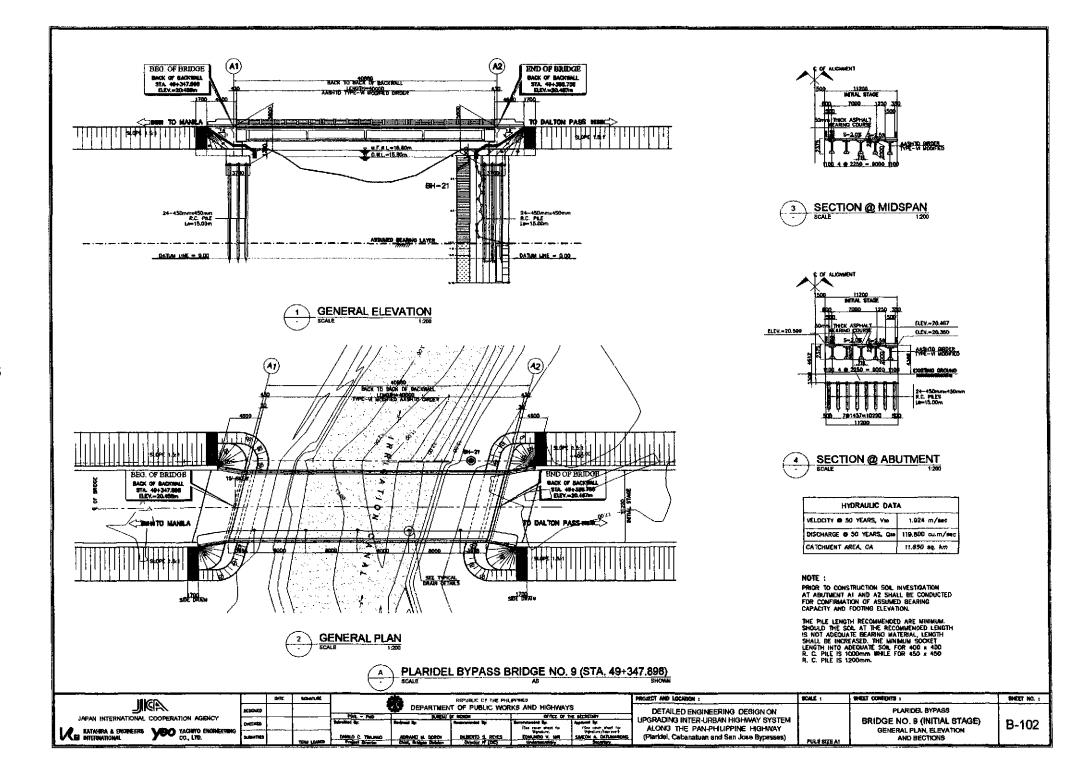
(2) DETERMINATION OF FINAL SCHEME

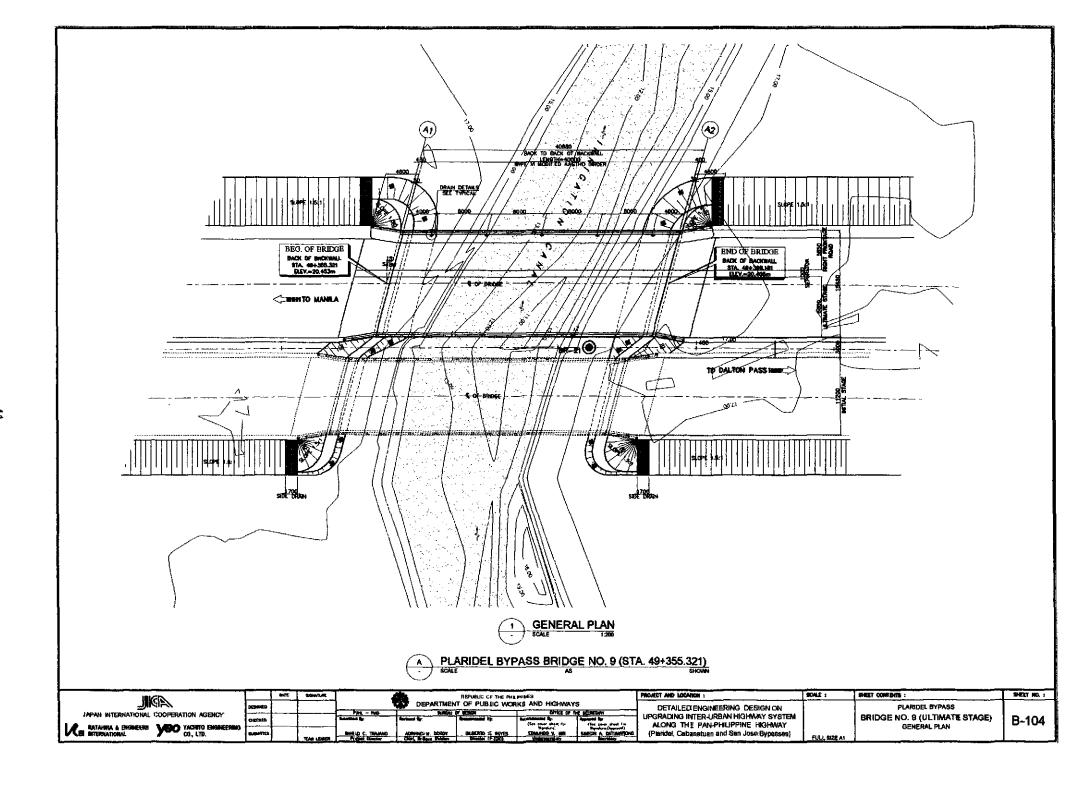
- The top width of river during the design flood is 35.0 m. To minimize river encroachment considering abutment slope protection, the bridge length becomes 40.86m for a single-span bridge.
- A single Span bridge is proposed due to :
 - Two-span bridge is not recommended since one pier will have to be located at the center of the river and may cause problems on river hydraulics.
 - A three span bridge tends to be more expensive because of additional substructure cost.

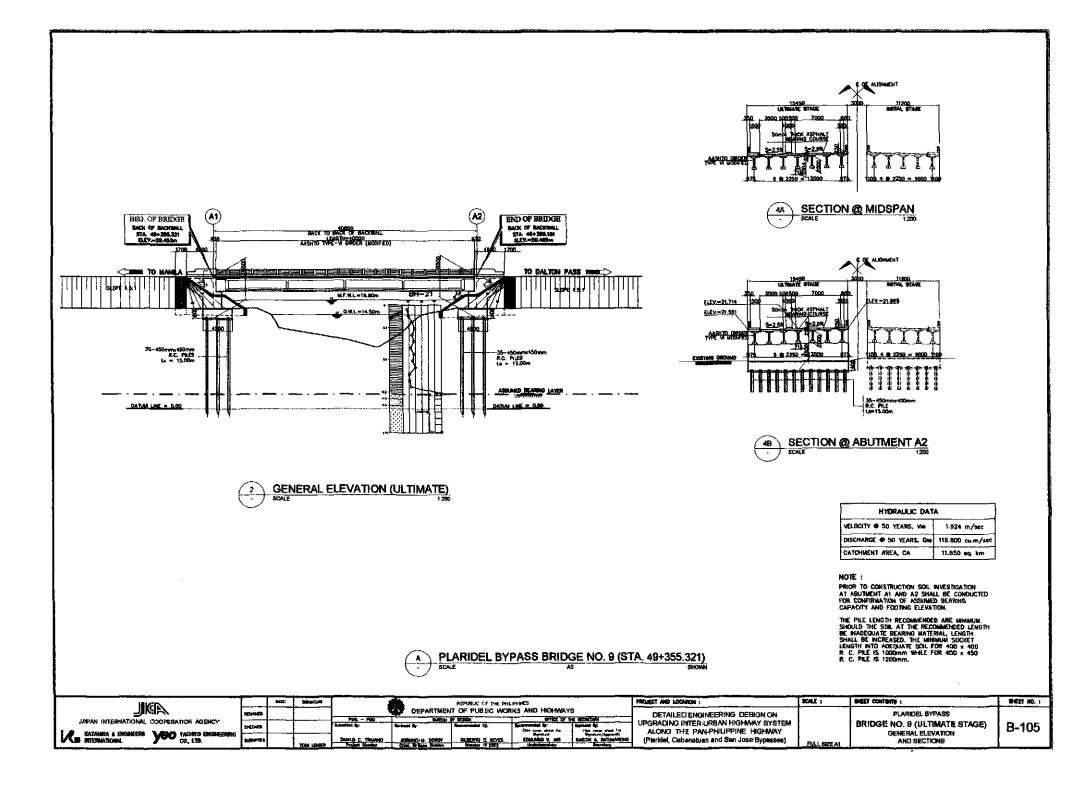
• Pile foundation is proposed due to presence of soft soil layer.

(3) PROTECTION AGAINST SCOURING

 Since river velocity during worst flooding is relatively slow (1.924m/sec), the bridge utilizes Grouted Riprap Riverbank Protection.







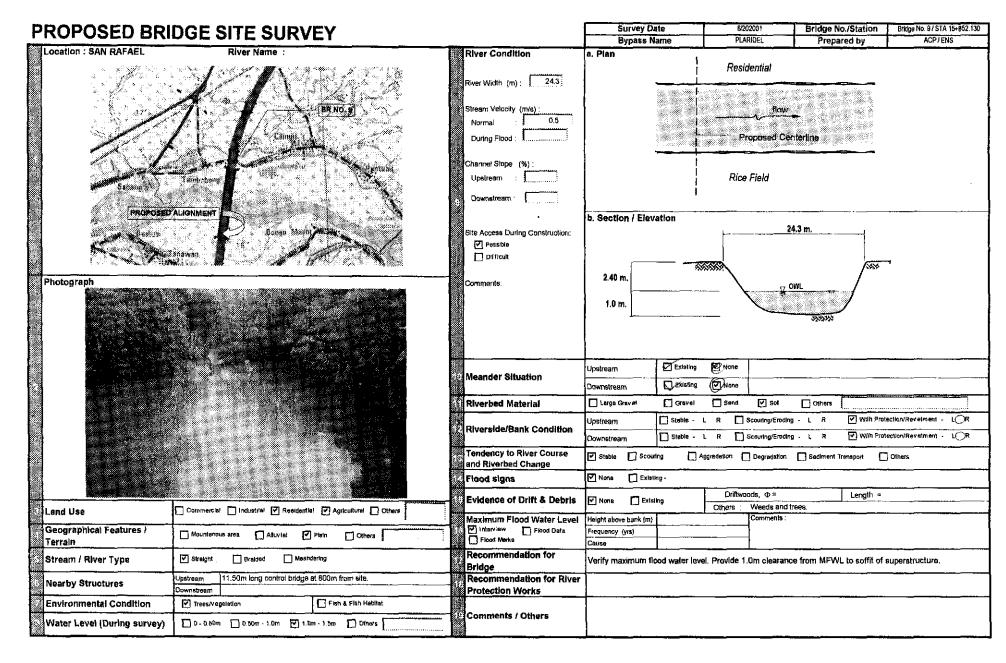


Table 3.4-14 Bridge No. 9 Site Condition

BRIDGE NO. 10 (PLARIDEL BYPASS-INITIAL AND ULTIMATE STAGE)

1. RIVER CONDITION

- The riverbed is composed of clay underlain by the granular deposits consisting of gravel and sand.
- The upstream and downstream conditions of riverbed & river banks are stable.
- No evidence of drifts/debris during flooding.
- No erosion at riverbank as it is heavily vegetated.
- Meandering river course.
- The river discharge at 50 year return period is 101.40 cum/sec.
- The river velocity during 50 year flooding is 2.133 m/sec.
- The top river width at design flood level is 29.0 m.
- The bridge is skewed at 15°.

2. SOIL CONDITION

- Bridge site is covered by 1.0 to 7.0 m of clay underlying hard siltstone.
- Bedrock was encountered below 20.0 m depth.
- N-value at the upper 4.0 m thick is generally not more than 10 blows while the subsequent depths ranged between 25 to 50 blows.

3. DESCRIPTION OF BRIDGE

(1) SELECTION OF SPAN COMPOSITION AND BRIDGE TYPE

	INITIAL STAGE	ULTIMATE STAGE
NO. OF BRIDGE	1	3
BRIDGE LENGTH	36.86 m	36.86 m
CARRIAGEWAY WIDTH	2 x 3.50m	6 x 3.50m
SHOULDER WIDTH	2.50 m	-
SIDEWALK WIDTH	0.60m	1.50m + 0.60m
SUPERSTRUCTURE TYPE	1-span, PCDG Type VI-Modified (40m)	1-span, PCDG Type VI-Modified (40m)
SUBSTRUCTURE TYPE	Closed-type Abutment	Closed-type Abutment
FOUNDATION TYPE	RC Piles (450 x 450mm)	RC Piles (450 x 450mm)

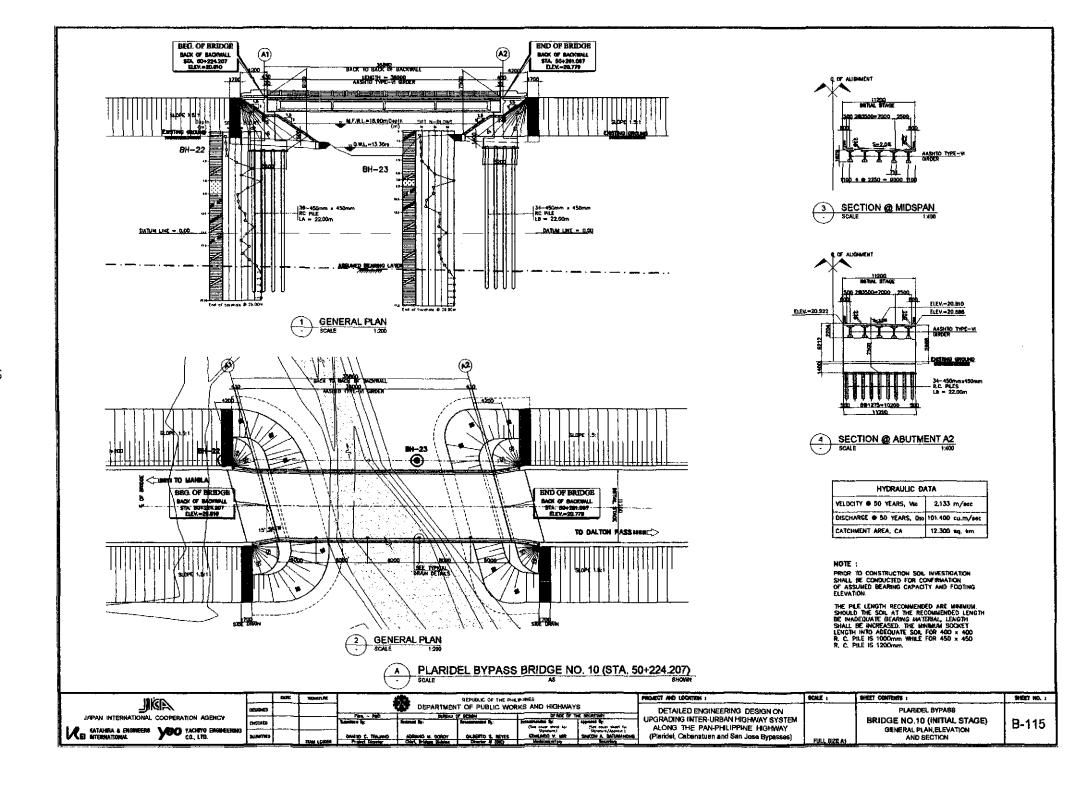
(2) DETERMINATION OF FINAL SCHEME

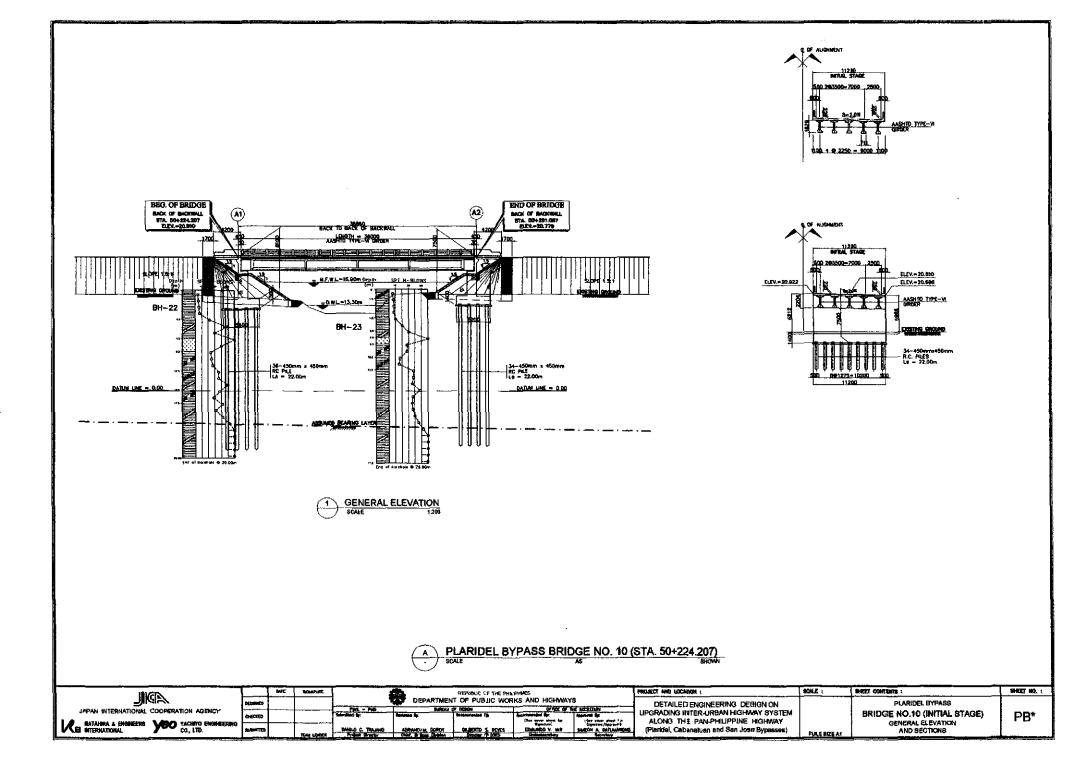
- The top width of river during the design flood is 29.0 m. To minimize river encroachment considering abutment slope protection, the bridge length becomes 36.86m for a single-span bridge.
- A single Span bridge is proposed due to:
 - Two-span bridge is not recommended since one pier will have to be located at the center of the river and may cause problems on river hydraulics.

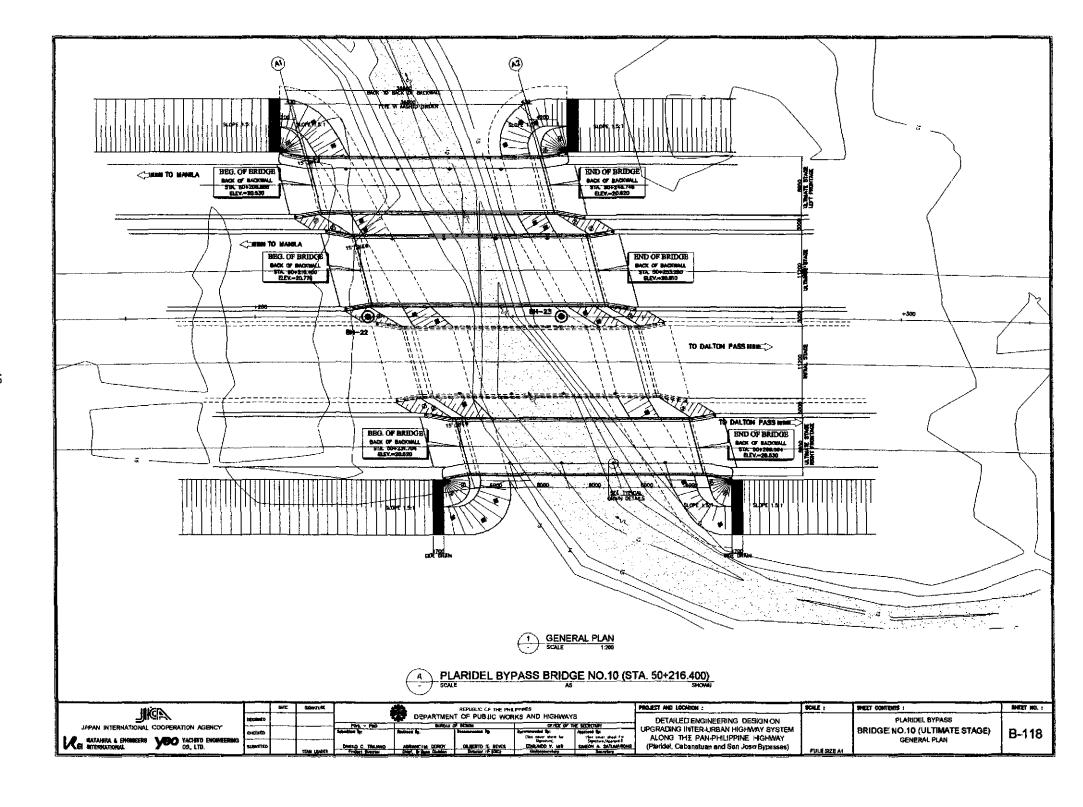
- o A three span bridge tends to be more expensive because of additional substructure cost.
- Pile foundation is proposed due to presence of soft soil layer.

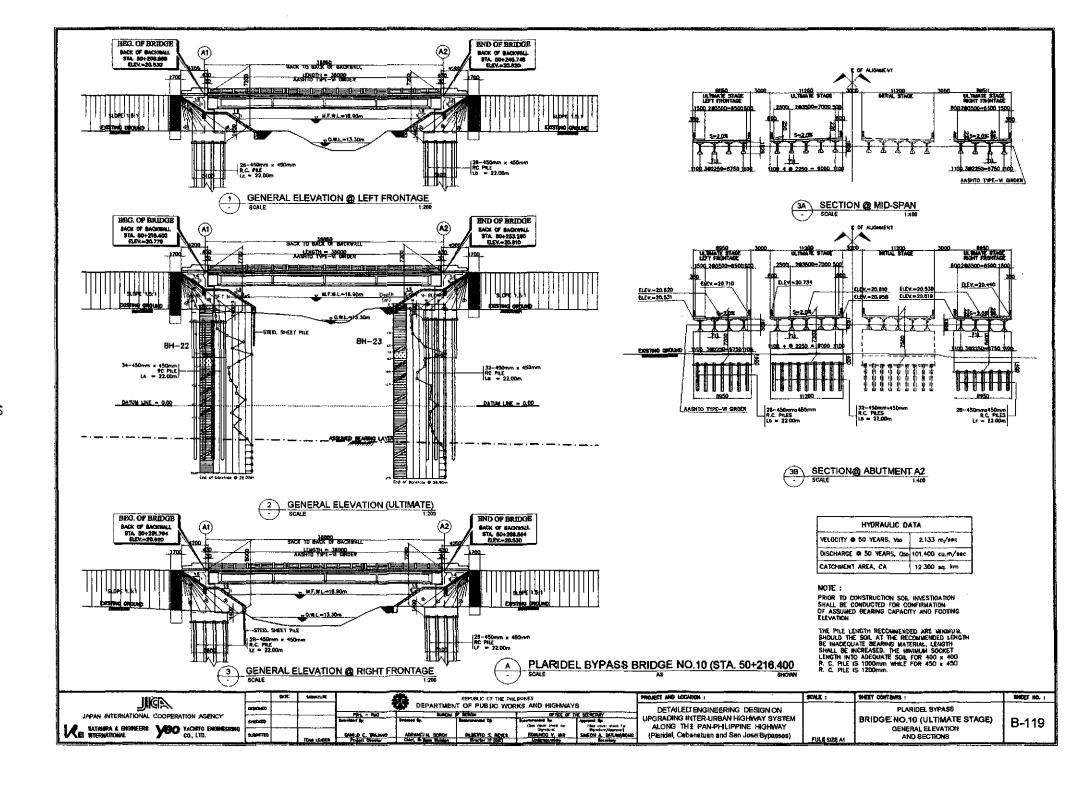
(3) PROTECTION AGAINST SCOURING

- Since river velocity during worst flooding is relatively slow (2.133 m/sec), the bridge utilizes Grouted Riprap Riverbank Protection.
- Grouted Riprap shall protected by Loose Boulder Apron at the toe.









PROPOSED BRI	DGE SITE SURVEY		Survey Da		6/202001	Bridge No./Station	Bridge No. 10 / STA 16+701.650
		E88 51 0 - 4141	Bypass Na	ime	PLARIDEL	Prepared by	GTR / ENS
Location : SAN RAFAEL Saliab RROPDSBO	River Name : (BR NO. 10) (Jacobia)	Site Access During Construction:	b. Section / Eleva	F	Rice Proposed Cente	Field	
Photograph		☐ Possible ☑ Difficutt Comments:	2.00 m. 2.5 m.		X-517.	8.5 m.	,
	44.2	Meander Situation	Upstream	Existing Nor			
	2.2		Downstream	Existing No		يحشر ومساحد المساحد ال	
		Riverbed Material	Lerge Gravel	☐ Gravel ☐ Sar		Others	
	4.4	Riverside/Bank Condition			Scouring/Eroding		ection/Revetment - L R
	****	S 3 40 ABI STEP CONTINUED	Downstream	☑ Stable - ○ F○	Scouring/Eroding	i - L R With Prot	ection/Revetment - L R
		Tendency to River Course and Riverbed Change	Slable Scouri	ng Aggradation	Degradation	Sediment Transport	Others
		Flood signs	▼ None	ng -			
Land Han	Commercial Industrial Residential Agricultural Cthers	Evidence of Orift & Debris	☑ Nome ☐ Existin	ng Dr Other	iffwoods, Φ = s : Weeds and b	Length =	
Land Use	The Property Character Carlo Carrest Carlos	Maximum Flood Water Level	Height above bank (m)	0.30	Comments:		
Geographical Features / Terrain	Mountenous stea Altuvial Plain Others	☐ Interview ☐ Flood Data	Frequency (yrs)				
Stream / River Type	Straight Braided V Meandering	Recommendation for Bridge	·	od water level. Provid	de 1.50m clearan	ce from MFWL to soffit of	superstructure.
Nearby Structures	Upstream Downstream BRIDGE / IRRIGATION CANAL GATE	Recommendation for River Protection Works		· · · · · · · · · · · · · · · · · · ·			
Environmental Condition	☑ Trees/vegetation ☐ Fish & Fish Habitat					<u></u>	
Water Level (During survey)	0 - 0.50m 0.60m - 1.0m 1.0m - 1.5m Others 25	Gomments / Others					

Table 3.4-15 Bridge No. 10 Site Condition

APPENDIX 10.2-1

Hydrological Characteristics of Watersheds and Related Discharges (Cabanatuan Bypass)

Catchment	_		Catchment Area	Diff. in	Length	Tc	RAINF	ALL INTE	ENSITY	Coefficient	DIS	CHARG	E, Q	PROPOSE	D DRAINAGE STR	UCTURES	REMARKS
Area No.	Station	Limits	(km²)	Elev., ∆h (m)	(m)	(min)	2 yrs	10 yrs	25 yrs	C	2 yrs	10 yrs	25 yrs	STATION	RCPC/RCBC	CAPACITY ems	(General Recommendation)
1	100+794	101+519	1.440	8.0	2683.0	76.59	44.15	78.06	95.15	0.50	8.83	15.62	19.04	100+806	2-2.40 m x 2.40 m	22.40	Proposed Box Culvert
[į	İ			· 					 			101+074	1-910 mm ¢	1.05	Irrigation Structure
														101+334	1–910 mm ф	1.05	Irrigation Structure
]									ļ					101+514	l−1520 mm ф	4.20	Irrigation Structure
												:				28.70	Total
2	101+519	102+114	0.853	10.0	1534.0	37.0	67.08	115.78	140.32	0.50	7.95	13.72	16.63	101+524	2–1220 mm ф	4.80	Irrigation Structure
ŀ														101+774	2–1220 mm ф	4.80	Irrigation Structure
								;						101+994	2–1220 mm ф	4.80	Irrigation Structure
<u> </u>							<u> </u>				Ì					14.40	Total
3	102+114	102+654	0.304	3.0	990.0	35.5	68.36	117.83	142.76	0.50	2.89	4.99	6.04	102+184	1−1070 mm ф	1.55	Irrigation Structure
]									ļ	}				102+384	1910 mm ø	1.05	Irrigation Structure
														102+484	1–910 mm ф	1.05	Irrigation Structure
1														102+649	1-1220 mm ø	2.40	Irrigation Structure
																6.05	Total
4	102+654	103+554	26.130				See H	ydrologica	Report					102+954		92.70	Proposed Bridge No 1
														103+194	1–910 mm ф	1.05	Irrigation Structure
														103+304	1-910 mm ¢	1.05	Irrigation Structure
														103+685	1-910 mm ф	1.05	Irrigation Structure
																95.85	Total

Catchment	, <u>, , , , , , , , , , , , , , , , , , </u>		Catchment Area	Diff. in	Length	Te	RAINF	ALL INTE	NSITY	Coefficient	DIS	CHARGI	Е, Q	PROPOSE	D DRAINAGE STR	UCTURES	REMARKS
Area No.	Station	Limits	(km²)	Elev., ∆h (m)	(m)	(min)	2 yrs	10 yrs	25 yrs	C	2 yrs	10 yrs	25 yrs	STATION	RCPC/RCBC	CAPACITY cms	(General Recommendation)
5	103+554	104+044	0.739	6.0	1957.0	59.53	50.99	90.94	111.01	0.50	5.24	9.34	11.40	103+550	1–910 mm ф	1.05	Irrigation Structure
										,				103+644	1–910 mm ф	1.55	Irrigation Structure
														103+804	1–910 mm ф	1.55	Irrigation Structure
İ	· 													103+904	1-3.0 m x 2.40 m	14.20	Proposed Box Culvert
														104+039	1–910 mm ф	1.05	Irrigation Structure
	· <u> </u>								i							19.40	Total
6	104+044	104+804	0.819	8.0	1336.0	34.35	69.37	119.45	144.69	0.50	7.90	13.60	16.48	104+049	1–1220 mm ф	2.40	Irrigation Structure
														104+194	2-1220 mm ф	4.80	Irrigation Structure
												ĺ		104+294	2–1220 mm ф	4.80	Irrigation Structure
											1		:	104+334	2–1220 mm ф	4.80	Irrigation Structure
																16.80	Total
7	104+804	105+319	107.760				See H	ydrological	Report					104+809	I−1220 mm ф	2.70	Irrigation Structure
			:											105+034		512.80	Proposed Bridge No 2
														105+314	1-910 mm ф	1.05	
																516.55	Total
8	105+724	105+724	0.038	2.0	331.0	11.77	106.99	183.00	221.54	0.50	0.57	0.97	1.18	105+324	1910 mm ф	1.05	
														105+654	1–910 mm ф	1.05	Irrigation Structure
											l .					2.10	Total
9	104+724	106+204	0.034	2.0	191.0	6.26	137.04	238.08	288.75	0.50	0.65	1.12	1.36	105+724	1–910 mm ф	1.05	Irrigation Structure
									 					105+954	1–910 mm ф	1.05	Irrigation Structure
									:					106+199	1–910 mm ф	1.05	
																3.15	Total

Catchment			Catchment Area	Diff. in	Length	Te	RAINF.	ALL INTI	ENSITY	Coefficient	DIS	CHARGI	E, Q	PROPOSE	D DRAINAGE STR	UCTURES	REMARKS
Area No.	Station	Limits	(km²)	Elev., ∆h (m)	(m)	(min)	2 yrs	10 yrs	25 yrs	C	2 yrs	10 yrs	25 yrs	STATION	RCPC/RCBC	CAPACITY cms	(General Recommendation)
10	106+204	10659*4	0.126	5.0	705.0	19.74	86.35	146.20	176.71	0.50	1.51	2.55	3.09	106+209	1–1070 mm ф	1.55	Irrigation Structure
														106+364	L-910 mm φ	1.05	Irrigation Structure
ļ	ı													106+509	1–910 mm ф	1.05	Irrigation Structure
					:									106+589	1–910 mm ф	1.05	Irrigation Structure
																4.70	Total
11	106+594	107+1 5 9	0.221	6.0	897.0	24.27	80.42	136.83	165.47	0.50	2.47	4.21	5.09	106+599	1−1220 mm φ	2.40	Irrigation Structure
														106+834	2–1070 mm ф	3.10	
														107+154	1–1070 mm ф	1.55	Irrigation Structure
																7.05	Total
12	107+159	107+714	1.026	6.0	1794.0	53.86	54.51	96.19	117.17	0.50	7.77	13.72	16.71	107+234	1–910 mm ф	1.05	
							,							107+494	1–1070 mm ф	1.05	Irrigation Structure
' [107+614	1-3.0m x 2.40 m	17.57	
														107+619	1–1070 mm ф	1.55	Irrigation Structure
													[107+704	1-910 mm ¢	1.05	Irrigation Structure
																22.27	Total
13	107+714	108+144	0.081	1.0	233.0	10.27	111.91	191.64	232.10	0.50	1.25	2.15	2.60	107+804	1–910 mm ф	1.05	Irrigation Structure
			<u> </u>		ļ									108+139	1–1220 mm ф	2.70	Irrigation Structure
																3.75	Total
14	108+144	108+589	0.237	9.0	1336.0	32.83	70.71	121.60	147.25	0.50	2.33	4.01	4.86	108+154	1–910 mm ф	1.05	
														108+294	l−1070 mm ¢	1.55	
}									<u> </u>		 		 	108+494	l−1070 mm ф	1.55	
						i					 			108+584	1–910 mm ф	1.05	Irrigation Structure
																5.20	Total

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Catchment			Catchment Area	Diff. in	Length	Тс	RAINF.	ALL INTE	NSITY	Coefficient	Dis	CHARG	Ε, Q	PROPOSE	D DRAINAGE STR	RUCTURES	REMARKS
Area No.	Station	Limits	(km²)	Elev., ∆h (m)	(m)	(min)	2 yrs	10 yrs	25 yrs	C	2 yrs	10 yrs	25 yrs	STATION	RCPC/RCBC	CAPACITY cms	(General Recommendation)
15	108+589	108+854	3.945	11.0	3762.0	99.95	34.41	65.53	79.63	0.50	18.87	35.93	43.66	108+594	1–910 mm ф	1.05	Irrigation Structure
						'								108+704	3-3.00 x 2.70	46.80	Proposed Box Culvert
																47.85	Total
16	108+854	109+289	0.028	1.0	97.0	3.75	145.20	253.20	307.20	0.50	0.57	0.99	1.20	108+974	1–910 mm ф	1.05	
										}				109+174	1–910 mm ф	1.05	
				<u> </u>										109+284	1-1220 mm ф	2.40	hrrigation Structure
						_										4.50	Total
17	109+289	109+554	0.206	2.0	438.0	16.25	93.75	159.60	192.98	0.50	2.68	4.56	5.52	109+294	1–910 mm ¢	1.05	
!														109+534	2–1220 mm ф	4.80	
	L.		·							1						5.85	Total
18	109+554	109+954	0.045	1.0	149.0	6.14	137.81	239.52	290.51	0.50	0.86	1.50	1.81	109+574	2–1220 mm ф	4.80	
`										 		!		109+909	1–910 mm ф	1.05	Irrigation Structure
															- •··	5.85	Total
19	109+954	110+294	0.758	6.0	1845.0	55.63	53.41	94.55	115.25	0.50	5.63	9.96	12.15	109+994	2–1070 mm ф	1.55	
									!			<u> </u> 		110+059	1–1070 mm ф	1.55	
														110+064	2–1220 mm ф	4.80	
														110+194	1–910 mm ф	1.05	
	<u> </u>													110+284	1–910 mm ф	1.05	
																10.00	Total
20	110+294	110+604	0.092	2.0	408.0	14.97	96.50	164.57	199.01	0.50	1.23	2.10	2.54	110+374	1910 mm ф	1.05	
	l 											1		110+599	1–1070 mm ф	1.55	Irrigation Structure
									<u> </u>							2.60	Total
21	110+604	110+834	11.175				See H	lydrologica	l Report					110+694		40.60	Proposed Bridge No 3

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Catchment			Catchment Area	Diff. in	Length	Tc	RAINF.	ALL INTE	ENSITY	Coefficient	DIS	CHARG	E, Q	PROPOSE	D DRAINAGE STE	RUCTURES	REMARKS
Area No.	Station	Limits	(km²)	Elev., ∆h (m)	(m)	(min)	2 yrs	10 yrs	25 yrs	C	2 yrs	10 yrs	25 yrs	STATION	RCPC/RCBC	CAPACITY	(General Recommendation)
22	110+834	111+204	0.033	1.0	186.0	7.92	126.28	218.16	264.45	0.50	0.57	0.99	1.20	110+934	1–910 mm ø	1.05	
												,		111+084	1–1070 mm ф	1.55	Irrigation Structure
											į			111+199	1–910 mm ф	1.05	
																3.65	Total
23	111+204	111+974	0.103	1.0	276.0	12.47	164.70	178.97	216.61	0.50	2.37	2.57	3.11	111+209	1–910 mm ф	1.05	Irrigation Structure
														111+304	1–910 mm ф	1.05	Irrigation Structure
i				İ		!					 		İ	111+534	1–910 mm ø	1.05	Irrigation Structure
														111+919	1–910 mm ф	1.05	Irrigation Structure
														111+929	1–910 mm ф	1.05	
																5.25	Total
24	111+974	112+199	0.017	1.0	159.0	6.62	134.70	233.76	283.48	0.50	0.32	0.56	0.68	111+979	l–1070 mm ф	1.55	Irrigation Structure
														112+194	1–910 mm ф	1.05	Irrigation Structure
																2.60	Total
25	112+199	112+739	0.079	1.0	328.0	15.21	96.0	163.59	197.82	0.50	1.05	1.79	2.17	112+204	1–1220 mm ф	1.05	Irrigation Structure
														112+254	1–910 mm ø	1.05	Irrigation Structure
														112+734	1-910 mm ф	1.05	Irrigation Structure
																3.15	Total
26	112+739	112+864	0.120	3.0	875.0	30.80	72.50	124.47	150.66	0.50	1.21	2.07	2.51	112+754	1-910 mm ф	1.05	
									:					112+854	1–910 mm ф	1.05	
														112+859	I–1220 mm φ	2.70	Irrigation Structure
				<u>.</u>												4.80	Total
27	112+864	112+974	0.012	1.0	833.0	44.43	60.50	105.21	127.76	0.50	0.10	0.17	0.21	112+869	1–910 mm ф	1.05	
28	112+974	113+334	22.225				See H	ydrologica	Report					113+204		99.70	Proposed Bridge No 4

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			Catchment Area	Diff. in	Length	Te		ALL INTE		Coefficient		CHARGI			D DRAINAGE STR		REMARKS
Catchment Area No.	Station	Limits	(km²)	Elev., ∆h (m)	(m)	(min)	2 yrs	10 yrs	25 yrs	Cocincient	2 yrs	10 yrs	25 yrs	STATION	RCPC/RCBC	CAPACITY ems	(General Recommendation)
29	113+334	113+454	0.146	3.0	755.00	26.00	78.24	133.44	161.40	0.50	1.59	2.71	3.27	113+304	1–910 mm ф	1.05	Irrigation Structure
}	 	ĺ]			113+354	1–910 mm ф	1.05	Irrigation Structure
•														113+449	1–910 mm ф	1.05	Irrigation Structure
											l 				<u></u>	3.15	Total
30	113+454	113+994	0.188	2.0	557.00	21.42	84.01	142.42	172.16	0.50	2.19	3.71	4.49	113+464	1–910 mm ф	1.05	
		1						!		 				113+674	1–1070 mm ф	1.55	
														113+894	1-1070 mm ø	1.55	
L																4.15	Total
31	113+994	114+234	17.650				Sce H	ydrological	Report	<u></u>				114+084		63.40	Proposed Bridge No 5
32	114+234	114+614	0.093	2.00	390.00	14.22	98.96	168.89	204.29	0.50	1.28	2.19	2.65	114+364	1-910 mm ф	1.05	
														114+609	1–1070 mm ф	1.55	
·			_	ļ												2.60	
33	114+614	115+274	0.074	2.00	404.00	14.81	97.02	165.49	200.14	0.50	0.99	1.69	2.05	114+619	1–910 mm ф	1.05	
				ļ										114+660	1–910 mm ф	1.05	Irrigation Structure
														115+269	1–910 mm ф	1.05	
																3.15	Total
34	115+274	115+474	10.425				See H	lydrologica	l Report					115+324		69.00	Proposed Bridge No 6
35	115+474	115+794	0.113	2.0	454.00	16.93	90.07	153.28	185.21	0.50	1.41	2.40	2.90	115+494	1–910 mm ф	1.05	Irrigation Structure
														115+634	1–910 mm ф	1.05	
			}					}						115+794	1–910 mm ø	1.05	
									<u> </u>			<u> </u>				3.15	Total

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Catchment	· · · · · ·		Catchment Area	Diff. in	Length	Te	RAINF	ALL INTI	ENSITY	Coefficient	DIS	CHARGI	Ε, Q	PROPOSE	D DRAINAGE STR	UCTURES	REMARKS
Area No.	Station	Limits	(km²)	Elev., ∆h (m)	(m)	(min)	2 yrs	10 yrs	25 yrs	C	2 yrs	10 yrs	25 yrs	STATION	RCPC/RCBC	CAPACITY ems	(General Recommendation)
36	115+794	115+979	0.047				See H	ydrologica	l Report		_			115+804			Proposed Bridge No 7
														115+828	2–910 mm ф	2.10	
										٠				115+974	1–910 mm ф	1.05	
[_				_			_		<u> </u>		110	3.15	Total
37	115+979	116+414	0.074	2.0	331.00	11.77	108.96	186.46	225.76	0.50	1.13	1.93	2.33	115+989	1–910 mm ø	1.05	
														116+034	1–910 mm ф	1.05	
						!								116+334	1–910 mm ф	1.05	
																3.15	Total
38	116+414	116+574					See H	ydrologica	l Report					116+464	·	32.30	Proposed Bridge No 8
39	116+574	116+829	0.019	1.0	149.00	6.14	137.81	239.52	290.51	0.50	0.37	0.64	0.77	116+574	1–910 mm ф	1.05	Irrigation Structure
														116+754	1–910 mm ф	1.05	
				 	}						_		<u> </u>			2.10	Total
40	116+829	117+094	0.416	7.0	1439.00	39.39	64.94	112.33	136.22	0.50	3.75	6.49	7.87	116+834	1–1070 mm ф	1.55	Irrigation Structure
													:	117+094	2–1 <i>5</i> 20 mm ф	8.00	Irrigation Structure
	,								<u> </u>							9.55	Total
41	117+094	117+454	0.234	5.0	1004.00	29.64	73.65	126.33	152.85	0.50	2.39	4.10	4.96	117+204	1–1220 mm ф	2.70	Irrigation Structure
					ļ		1	ļ	ļ				ļ	117+454	1–1220 mm ф	2.70	Irrigation Structure
					_		_									5.40	Total
42	117+454	118+079	0.181	4.0	717.00	21.93	83.37	141.42	170.96	0.50	2.10	3.56	4.30	117+714	1–910 mm ф	1,05	
								<u> </u>				}		117+958	2–910 mm ф	2.10	Irrigation Structure
														118+074	1–910 mm ф	1.05	
								<u> </u>							·	4.20	Total

Catchment			Catchment Area	Diff. in	Length	Te	RAINF.	ALL INTE	ENSITY		DIS	CHARG	E, Q	PROPOSE	D DRAINAGE STE	RUCTURES	REMARKS
Area No.	Station	Limits	(km²)	Elev., ∆h (m)	(m)	(min)	2 yrs	10 yrs	25 yrs	Coefficient C	2 yrs	10 yrs	25 yrs	STATION	RCPC/RCBC	CAPACITY cms	(General Recommendation)
43	118+079	118+454	0.102	4.0	726.00	22.25	82.97	140.79	170.21	0.50	1.18	2.00	2.42	118+084	1–910 mm ф	1.05	Irrigation Structure
														118+204	1–1070 mm ф	1.55	Irrigation Structure
																2.60	Total
44	118+454	118+704	11.950				See H	ydrological	Report	 				118+614		65.00	Proposed Bridge No 9
}	•									}				118+664	1-910 mm ф	1.05	Irrigation Structure
														118+696	1–910 mm ф	1.05	
																67.10	Total
45	118+704	118+824	0.007	1.0	143.00	5.86	139,63	242.88	294.61	0.50	0.14	0.24	0.29	118+709	1–910 mm ф	1.05	
		1	<u>'</u>						'					118+814	1910 mm ф	1.05	
																2.10	Total
46	118+824	119+554	0.407	2.0	737.00	29.56	73.75	126.46	153.03	0.50	4.18	7.16	8.67	118+894	1–910 mm ф	1.05	Irrigation Structure
														119+210	1–910 mm ф	1.05	Irrigation Structure
														119+044	1–910 mm ф	1.05	Irrigation Structure
												!		119+204	1–910 mm ф	1.05	Irrigation Structure
						!						:		119+240	1–1 <i>5</i> 20 mm ф	4.00	
		ı												119+414	1–910 mm ф	1.05	Irrigation Structure
[]									!	}				119+549	1–910 mm ф	1.05	Irrigation Structure
									!					119+614	1–910 mm ф	1.05	Irrigation Structure
																11.35	Total
47	119+554	121+364					See H	ydrological	Report					120+574		6,990.00	Proposed Bridge No 10
														121+134	1–910 mm ф	1.05	Irrigation Structure
														121+284	1–910 mm ф	1.05	
								_								6,992.10	Total

Catchment			Catchment Area	Diff. in	Length	Тс	RAINF	ALL INTE	ENSITY	Coefficient	DIS	CHARG	E, Q	PROPOSE	D DRAINAGE STR	UCTURES	REMARKS
Area No.	Station	Limits	(km²)	Elev., ∆h (m)	(m)	(min)	2 yrs	10 yrs	25 угз	C	2 yrs	10 yrs	25 yrs	STATION	RCPC/RCBC	CAPACITY cms	(General Recommendation)
48	121+364	121+914	0.055	1.0	139.00	5.67	140.86	245.15	297.39	0.50	1.08	1.87	2.27	121+374	1–910 mm ф	1.05	
		ļ								1			į	121+406	1–1070 mm ф	1.05	Irrigation Structure
						:								121+914	1–910 mm ф	1.05	Irrigation Structure
			 		<u> </u>				<u> </u>			<u></u>				3.15	Total
49	121+914	122+374					See H	ydrologica	l Report					122+061	1–1070 mm ф	1.55	Irrigation Structure
	ı	l												122+350	1–910 mm ф	1.05	
														122+374			Proposed Bridge No 11
																2.60	Total
50	122+374	122+459	0.007	1.0	147.00	6.04	138.46	240.72	291.97	0.50	0.13	0.23	0.28	122+460	1−1 <i>5</i> 20 mm φ	4.00	Irrigation Structure
51	122+459	123+459					See H	ydrologica	l Report					122+466	1–910 mm ф	1.05	
			129.925	}										122+594		735.50	Proposed Bridge No 12
														123+260	1–910 mm ф	1.05	Irrigation Structure
														123+364	1–910 mm ф	1.05	Irrigation Structure
														123+454	1–910 mm ф	1.05	
<u>. </u>													 			739.70	Total
52	123+459	123+569	0.010	1.0	219.00	9.56	115.65	198.48	240.44	0.50	0.16	0.28	0.34	123+464	1–910 mm ф	1.05	
				[1	 				123+559	1–910 mm ф	1.05	
- -														123+564	1-1220 mm ø	1.55	Irrigation Structure
<u> </u>	Lu			_			,						<u> </u>			3.65	Total
53	123+569	124+054	0.102	1.0	348.00	16.28	93.69	159.48	192.84	0.50	1.33	2.27	2.74	123+574	I−910 mm ∳	1.05	
				ļ										123+654	1–910 mm ø	1.05	Irrigation Structure
				1			1							123+934	1–910 mm ø	1.05	
												_				3.15	Total

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Appendix 10.2-1 HYDROLOGICAL CHARACTERISTICS OF WATERSHEDS AND RELATED DISCHARGES (CABANATUAN BYPASS)

					- CLOCK		-		- OF	ALEKSII	1200711	D KED	AILDI	JOCHARO	ES (CABANATU	TITABB	,
Catchment			Catchment Area	Diff. In	Length	Te	RAINF	ALL INTE	ENSITY	Coefficient	DIS	CHARG	E, Q	PROPOSE	ED DRAINAGE STR	UCTURES	REMARKS
Area No.	Station	Limits	(km²)	Elev., ∆h (m)	(m)	(min)	2 yrs	10 yrs	25 yrs	C	2 yrs	10 yrs	25 yrs	STATION	RCPC/RCBC	CAPACITY cms	(General Recommendation)
54	124+054	124+359	0.127	2.0	501.00	18.96	88.00	149.19	180.35	0.50	1.55	2.63	3.18	124+134	2–910 mm ø	2.10	
						'			'					124+359	1−910 mm ф	1.05	Irrigation Structure
						<u>.</u>				_						3.15	Total
55	124+359	124+794	0.231	2.0	896.00	37.00	67.04	115.71	140.24	0.50	2.15	3.71	4.49	124+514	2–910 mm ф	2.10	Irrigation Structure
											ĺ	ļ		124+654	2–910 mm ф	2.10	
L																4.20	Total
56	124+794	124+994	1.572	4.0	1499.00	51.21	56.15	98.65	120.05	0.50	12.27	21.56	26.23	124+924	2-2.40 m x 2.40 m	28.10	Proposed Box Culvert
57	124+994	125+294	0.046	1.0	150.00	6.19	137.49	238.92	289.78	0.50	88.0	1.53	1.85	125+014	1–910 mm ф	1.05	
														125+134	1–910 mm ф	1.05	
						<u> </u>				·						2.10	Total
58	125+294	125+639	0.040				See H	ydrologica	Report					125+374	1–910 mm ф	1.05	
\$														125+614	1–910 mm ф	1.05	
														125+624			Proposed Bridge No 13
<u> </u>					·					 	<u>,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,</u>					2.10	Total
59	125+639	125+884	0.692	4.0	1779.00	62.36	49.77	88.73	108.32	0.50	4.79	8.53	10.42	125+644	2–1220 mm ф	4.80	
														125+874	2–1220 mm ф	4.80	
1]	<u> </u>												9.60	Total
60	125+884	126+214	0.178	2.0	593.00	23.02	81.99	139.28	168.40	0.50	2.03	3.45	4.17	125+889	1–910 mm ф	1.05	
				<u> </u>])		·		<u> </u>	126+214	2–1070 mm ф	3.10	Irrigation Structure
																4.15	Total

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Appendix 10.2-1 HYDROLOGICAL CHARACTERISTICS OF WATERSHEDS AND RELATED DISCHARGES (CABANATUAN BYPASS)

Catchment			Catchment Area	Diff. in	Length	Te	RAINE	ALL INTE	ENSITY	Coefficient	DIS	CHARG	E, Q	PROPOSE	D DRAINAGE STR	UCTURES	REMARKS
Area No.	Station	Limits	(km²)	Elev., ∆h (m)	(m)	(min)	2 yrs	10 yrs	25 yrs	Coefficient	2 yrs	10 yrs	25 yrs	STATION	RCPC/RCBC	CAPACITY ems	(General Recommendation)
61	126+214	126+754	4.739	8.0	3963.00	119.95	33.01	57.52	69.82	0.50	21.74	37.89	45.99	126+434	1–910 mm ø	1.05	
]						į								126+624	3-3.0 m x 3.0 m	73.62	Proposed Box Culvert
														126+679	1–910 mm ф	1.05	
										L						75.72	Total
62	126+754	127+004	0.017	1.0	77.00	2.87	145.20	253.20	307.20	0.50	0.34	0.59	0.72	126+874	1–910 mm ф	1.05	
														126+994	1–1220 mm ф	1.55	
l						_										2.60	Total
63	127+004	127+434	0.035	1.0	91.00	3.48	145.20	253.20	307.20	0.50	0.70	1.23	1.49	127+012	l−1070 mm ф	1.05	Irrigation Structure
]														127+034	1–910 mm ф	1.05	
											i			127+334	1–910 mm ф	1.05	Irrigation Structure
																2.10	Total
64	127+434	127+844	1.062	5.0	2152.00	71.23	145.20	253.20	307.20	0.50	21.43	37.37	45.34	127+480	3-3.0 m x2.70 m	46.80	Proposed Box Culvert
				[t 	127+839	I–910 mm ф	1.05	Irrigation Structure
												_				47.85	Total
65	127+844	128+174	0.053	1.0	244.00	10.83	110.08	188.42	228.16	0.50	0.81	1.38	1.68	127+849	1–1070 mm ф	1.55	Irrigation Structure
														128+014	1–910 mm ф	1.05	
	1							<u> </u>	<u> </u>		1					2.60	Total
66	128+174	128+774	3.040	7.0	3314.00	102.80	36.78	64.38	78.23	0.50	15.54	27.20	33.06	128+300	2-3.0 m x 3.0 m	49.08	Proposed Box Culvert
					ļ	i							ļ ,	128+394	1–910 mm ф	1.05	Irrigation Structure
														128+559	1–910 mm ф	1.05	Irrigation Structure
į														128+768	1–910 mm ф	1.05	Irrigation Structure
																52.23	Total

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	Catchment				1		-31110111		*********		- REE	TI EE E	isemarkoi	E (CADAITATO	N DITAGO	·	
Catchment	64-4*	T >=====	Area	Diff. in	Length	Te	RAINF.	ALL INTE	ENSITY	Coefficient	DIS	CHARG	E, Q	PROPOSE	D DRAINAGE STR	UCTURES	REMARKS
Area No.	Station	Limits	(km²)	Elev., Ah (m)	(m)	(min)	2 yrs	10 yrs	25 yrs	C	2 yrs	10 yrs	25 yrs	STATION	RCPC/RCBC	CAPACITY ems	(General Recommendation)
67	128+774	129+054	0.023	1.0	167.00	7.00	132.24	229.25	277.92	0.50	0.42	0.74	0.89	128+784	1–1220 mm ф	2.70	Irrigation Structure
							i							128+814	1-910 mm φ	1.05	
														128+994	1–910 mm ф	1.05	
		·														4.80	Total
68	129+054	129+424	4.006	9.0	4498.00	132.60	31.03	53.85	65.31	0.50	17.28	29.98	36.36	129+112	3-3.0 m x 2.40 m	42.60	Proposed Box Culvert
														129+366	1–910 mm ф	1.05	Irrigation Structure
																43.65	Total
69	129+424	129+879	0.068	1.0	300.00	13.73	100.57	171.72	207.74	0.50	0.95	1.62	1.96	129+416	1–910 mm ф	1.05	Irrigation Structure
														129+436	1–910 mm ø	1.05	
														129+789	1–910 mm ф	1.05	Irrigation Structure
	i i				:		:							129+862	1–1220 mm ф	2.70	Irrigation Structure
											:			129+874	1–910 mm ф	1.05	
																6.90	Total
70	129+879	130+359	0.060	1.0	354.00	16.61	92.99	158.22	191.30	0.50	0.77	1.32	1.59	129+884	1–910 mm ф	1.05	
		ı												130+129	1–910 mm ф	1.05	Irrigation Structure
						!		i						130+359	1–910 mm ¢	1.05	Irrigation Structure
																3.15	Total
71	130+359	130+654	0.098	1.0	376.00	17.80	90.46	153.65	185.75	0.50	1.23	2.10	2.53	130+509	1–1070 mm ф	1.55	Irrigation Structure
												İ		130+654	1–1070 mm ф	1.55	Irrigation Structure
		<u>.</u>														3.10	Total

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Appendix 10.2-1 HYDROLOGICAL CHARACTERISTICS OF WATERSHEDS AND RELATED DISCHARGES (CABANATUAN BYPASS)

Catchment			Catchment Area	Diff. in	Length	Te	RAINF	ALL INTI	ENSITY	Coefficient	DIS	CHARG	E, Q	PROPOSE	D DRAINAGE STR	UCTURES	REMARKS
Area No.	Station	Limits	(km²)	Elev., ∆h (m)	(m)	(min)	2 yrs	10 yrs	25 yrs	C	2 yrs	10 yrs	25 yrs	STATION	RCPC/RCBC	CAPACITY ems	(General Recommendation)
72	130+654	131+144	0.606	2.0	731.00	29.28	74.11	127.01	153.69	0.50	6,24	10.70	12.94	130+934	2–1220 mm ø	4.80	
														131+088	2–1220 mm ø	4.80	
'			j.											131+144	1–1220 mm ф	2.40	Irrigation Structure
,,,					-											12.00	Total
73	131+144	131+639	0.079	1.0	326.00	15.11	96.17	163.98	198.29	0.50	1.06	1.80	2.18	131+334	1–910 mm ф	1.05	
														131+594	t−910 mm ф	1.05	Irrigation Structure
										<u> </u>						2.10	Total
74	131+639	131+904	5.267	10.0	4781.00	136.59	30.40	52.69	63.88	0.50	22.26	38.57	46.77	131+644	1-3.0 m x 2.40 m	17.57	Irrigation Canal
														131+734	2-3.0 m x 3.0 m	49.08	Proposed Box Culvert
															,	66.65	Total
75	131+904	132+124	0.040	1.0	318.00	14.68	97.45	166.24	201.05	0.50	0.55	0.93	1.13	131+904	1–910 mm ф	1.05	Irrigation Structure
														132+119	1–910 mm ф	1.05	
																2.10	Total
76	132+124	133+054					See H	lydrologica	l Report					132+274	3-3.0 m x 3.0 m	73.62	Proposed Box Culvert
														132+599	1910 mm ф	1.05	
														132+964		1,570.00	Proposed Bridge No 14
						_										1,644.67	Total
77	133+054	133+249	0.039	1.0	226.00	9.91	113.38	194.28	235.32	0.50	0.62	1.06	1.28	133+064	1–910 mm ф	1.05	Irrigation Structure
														133+244	1–910 mm ф	1.05	Irrigation Structure
								<u> </u>							,_ -	2.10	Total

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	Catchment		Catchment Area	Diff. in	Length	Te	RAINF	ALL INTE	NSITY		DIS	CHARGI	E, Q	PROPOSE	D DRAINAGE STR	UCTURES	REMARKS
Catchment Area No.	Station	Limits	(km²)	Elev., ∆h (m)	(m)	(min)	2 yrs	10 yrs	25 yrs	Coefficient C	2 yrs	10 yrs	25 yrs	STATION	RCPC/RCBC	CAPACITY ems	(General Recommendation)
78	133+249	133+849	0.075	1.0	314.00	14.47	98.14	167.45	202.53	0.50	1.02	1.74	2.10	133+254	1–910 mm ф	1.05	
	l.													133+574	1–910 mm ф	1.05	
										•		,		133+840	1–910 mm ø	1.05	
						_										3.15	Total
. 79	133+849	134+224	0.038	1.0	161.00	6.71	134,12	232.67	282.17	0,50	0.70	1.22	1.47	133+860	1-910 mm ø	1.05	
	1								•					133+920	1–910 mm ф	1.05	Irrigation Structure
														133+929	l−910 mm ¢	1.05	Irrigation Structure
	: 1				:									134+129	1-910 mm ¢	1.05	
				1		ļ								134+224	1–1220 mm ф	2.40	Irrigation Structure
														_		6.60	Total
80	134+224	134+644	0.015	1.0	137.00	5.57	141.51	246.36	298.86	0.50	0.29	0.50	0.60	134+229	t-910 mm ø	1.05	
	1						1	l			ļ	l		134+479	L-910 mm ф	1.05	
														134+634	1-910 mm ф	1.05	
																3.15	Total