Exploratory Adit (Alignment and Geological Sketch)

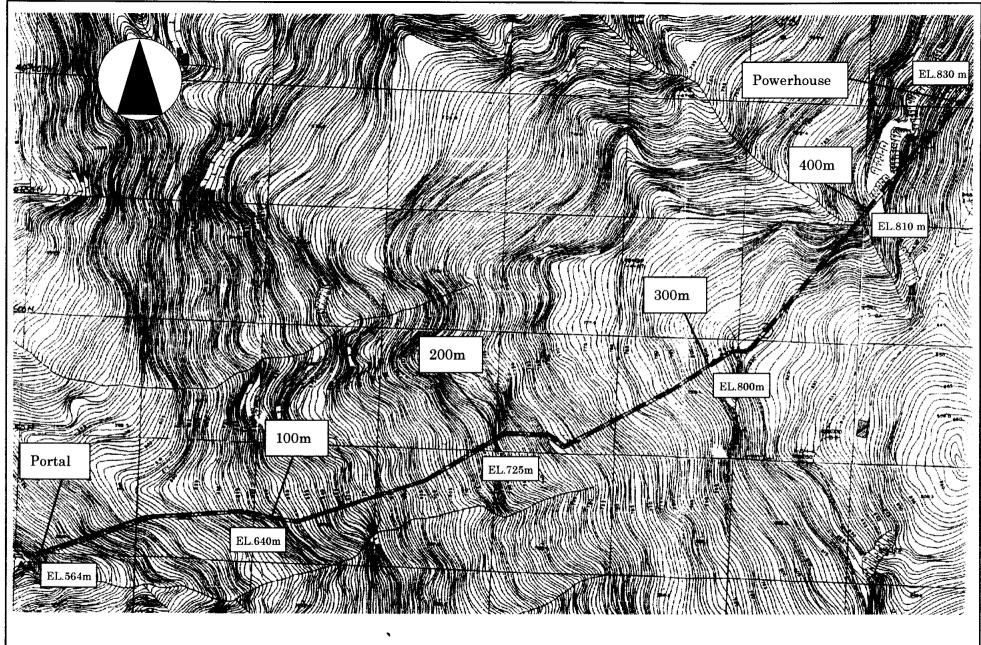
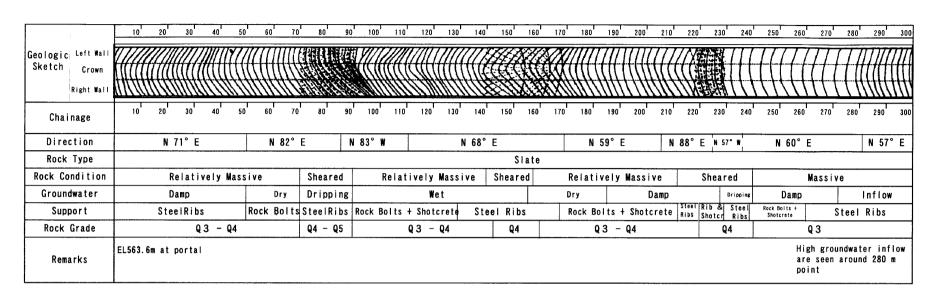


Figure B3.9.1 Alignment of Exploratory Adit



	310 320	330	340	350	360	370	380	390	400 ¹	410	420	430	440	450	460
Geologic Left Wall Sketch Crown								(((加			W	
Chainage	310 320	330	340	350	360	370	380	390	400	410	420	430	440	450	460
Direction	N 60° E N 8	;° ¢		N 37° E											
Rock Type			Slat	е							Dolom	ıi te			
Rock Condition	Mass	ive	Sheared					Mas	sive						
Groundwater	Inflow	Damp	Dripping				D	amp /	/ Dry						
Support	No Supp	ort		Rib & Shotor	Shoter			No S	uppor	t					
Tunnel Condition	Q3			Q4	- Q5	بىر			Q	2 - Q	3				
Remarks				Rock faces change from dolomitic slate to scilicous dolomite at the lithological											

Rock Grade	Modulus of Deformation (MPa)	Shear Strength (MPa)	Friction Degree (degree)	Q value by Q system	CRIEPI classification system	
Q1	> 3,000	> 2.5	> 50	> 40	В	
Q2	3,000	2.5	50	10 to 40	CH	
Q3	1,000	1.2	45	4 to 10	СМ	
Q4	500	0.6	40	1 to 4	CL	
Q5	250	0.1	35	1 >	D	

Figure B3.9.2 Geological Condition of Exploratory Adit

RESULT OF IN-SITU TESTING

Plate Loading Test (See ①, ②, ③)

LOCATION	MODULUS OF DEFORMATION (MPa)
PL-1	3,183.9
PL-2	9,366.9
PL-3	1,869.5

Block Shear Test (See ④) $\tau = 0.3 \text{ to } 0.5 \text{ MPa}, \quad \phi = 40 \text{ to } 50^{\circ}$

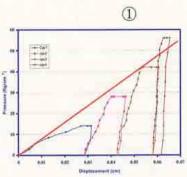


Plate Load Test (PL-1)

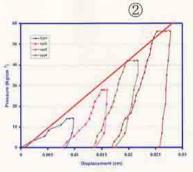
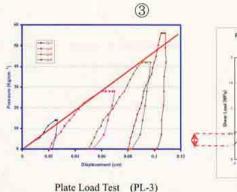


Plate Load Test (PL-2)



Relationship between Shear stress and Vertical stress.

Vertical stress.

Vertical Lase (MPs)

Block Shear Test

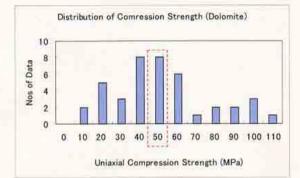
(4)

RESULTS OF LABORATORY TEST

Modulus of Deformation (See ⑤) 2,000 to 3,500 MPa (when gu = 50MPa)

Shear Strength (See ⑥) 2 to 3 MPa (when qu = 50MPa)

[qu: uniaxial compression strength]



A Plate Loading

20

A Plate Loading

SOMPa

10

200

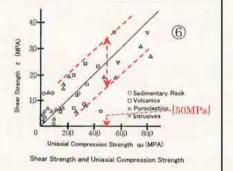
400

600

800

Unissial Compression Strength; up UMPA)

Deformation Modulus and Unitaxial Compression Strength



[Ref: Rock Classification by Uniaxial Compression Strength etc. (K.Kuwabara; Japan Society of Engineering Geology, 1984)]

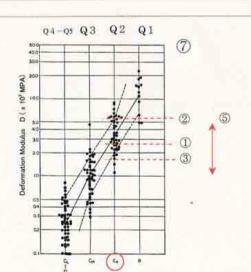
CHARACTERISTICS OF ROCKS

(at location of In-situ Rock Testing spot in Branch Adit)

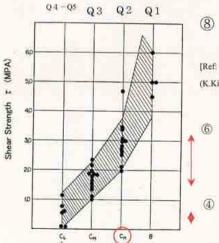
Modulus of Deformation (See ⑦) 3,000 to 5,000 MPa

Shear Strength (See ®) 2 to 3 MPa

(Friction angle 45 to 50 degrees)



Rock Grade and Deformation Modulus



Rock Grade and Shear Strength

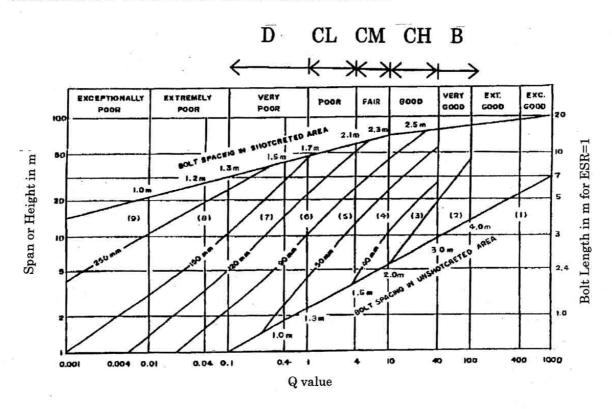
[Ref: Geotechnical Integrated Evaluation on the stability of Dam Foundation Rocks (K.Kikuchi; Japan Society of Engineering Geology, 1984)]

Figure B4.1.1 .Discussion on Properties of Rocks

ROCK CLASSIFICATION AND REINFORCEMENT CATERGORIES BY CRIEPI ROCK CLASSIFICATION(1991)

Rock	Support	Shoto	reting		Rock Bolt	Steel Support		
Grade	Pattern	Thickness (cm)	Area	Length (m)	Nos of bolts	Pitch (m)	Materilals	Pitch (m)
В	Α	5	Arch	2	0 to 5	-	_	<u> </u>
CH	В	5	Arch · Wall	2	7	1.5	-	7
СМ	CI	10	Arch · Wall	2	10	1.5	_	:
CIVI	CII	15	Arch · Wall	2	12	1.2	1	_
OI.	DI	15	Arch · Wall	3	12	1.2	-	\ <u>-</u>
CL	DII	15	Arch · Wall	3	12	1.2	(100H)	1.0 to 1.2
D	Е	15 to 20	Arch · Wall	3	14	1.0 to 1.2	(125H)	1.0 to 1.2

SYSTEM AND ASSUMED REINFORCEMENT CATEGORIES BASED ON Q CORRELATION WITH CRIEPI ROCK CLASSIFICATION



REINFORCEMENT CATERGORIES

(1) Unsupported

- (6) Fibre reinforced shotcrete, 90 -120 mm and shotcrete

(2) Spot bolting

- (7) Fibre reinforced shotcrete, 120 150 mm and shotcrete
- (8) Fibre reinforced shotcrete, >150 mm with reinforced ribs of shotcrete
- (4) Systematic bolting with

(3) Systematic bolting

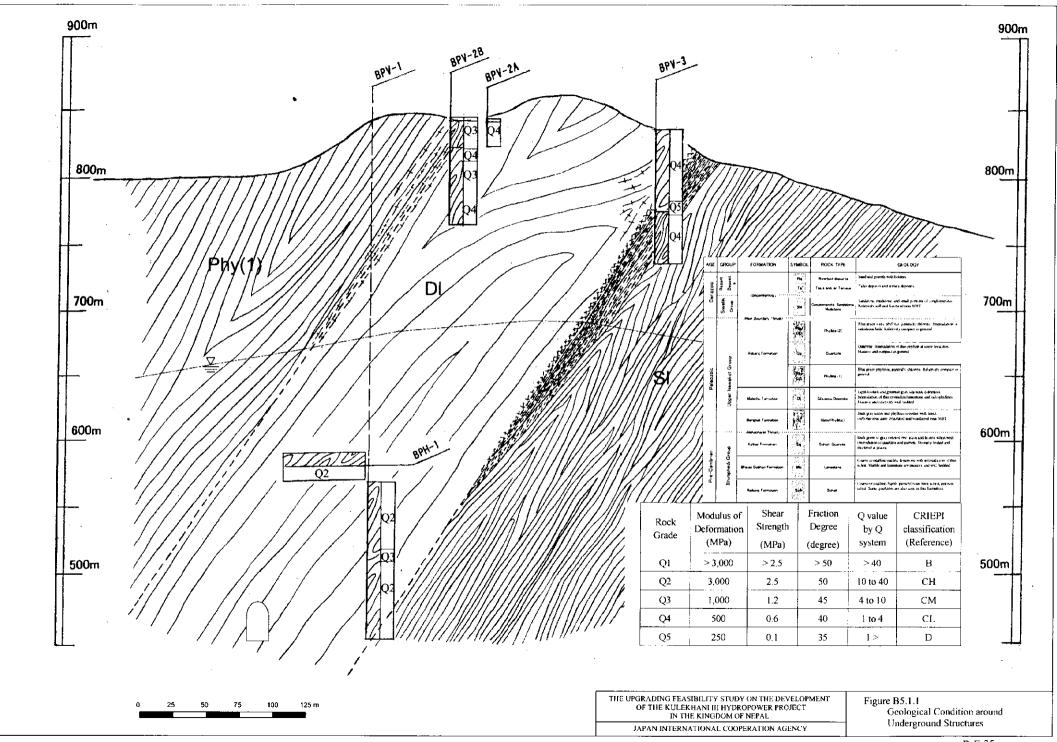
- and bolting.
- 40 100 mm unreinforced shotcrete
- (9) Cast concrete lining
- (5) Fibre reinforced shotcrete, 50 90 mm

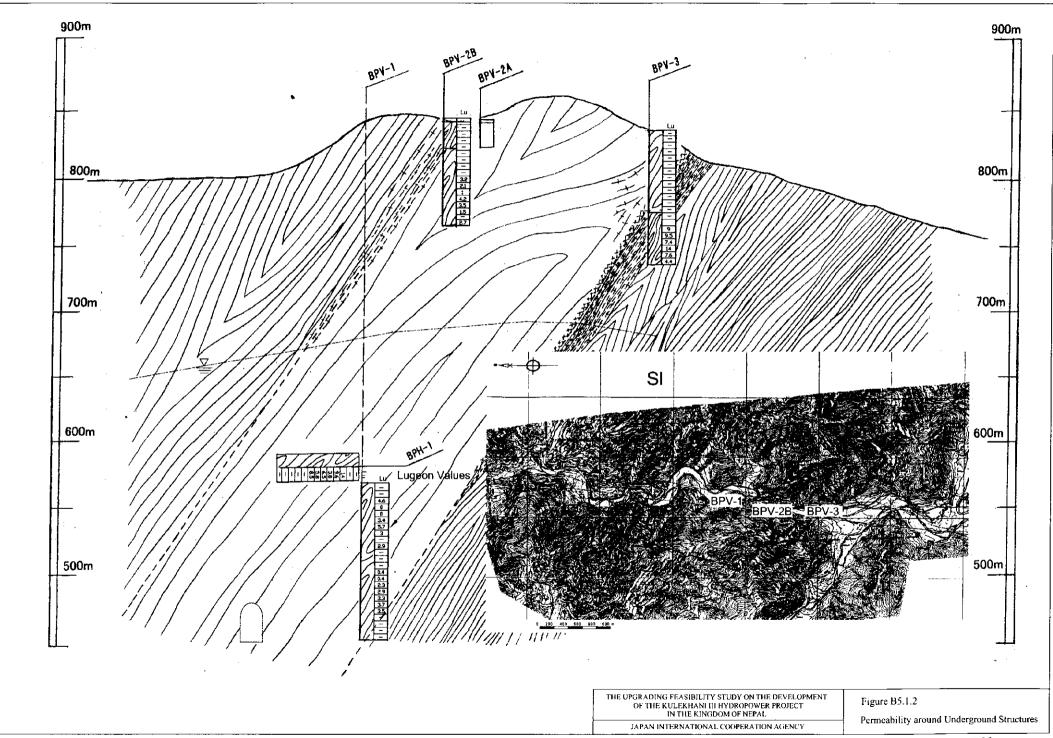
and shotcrete

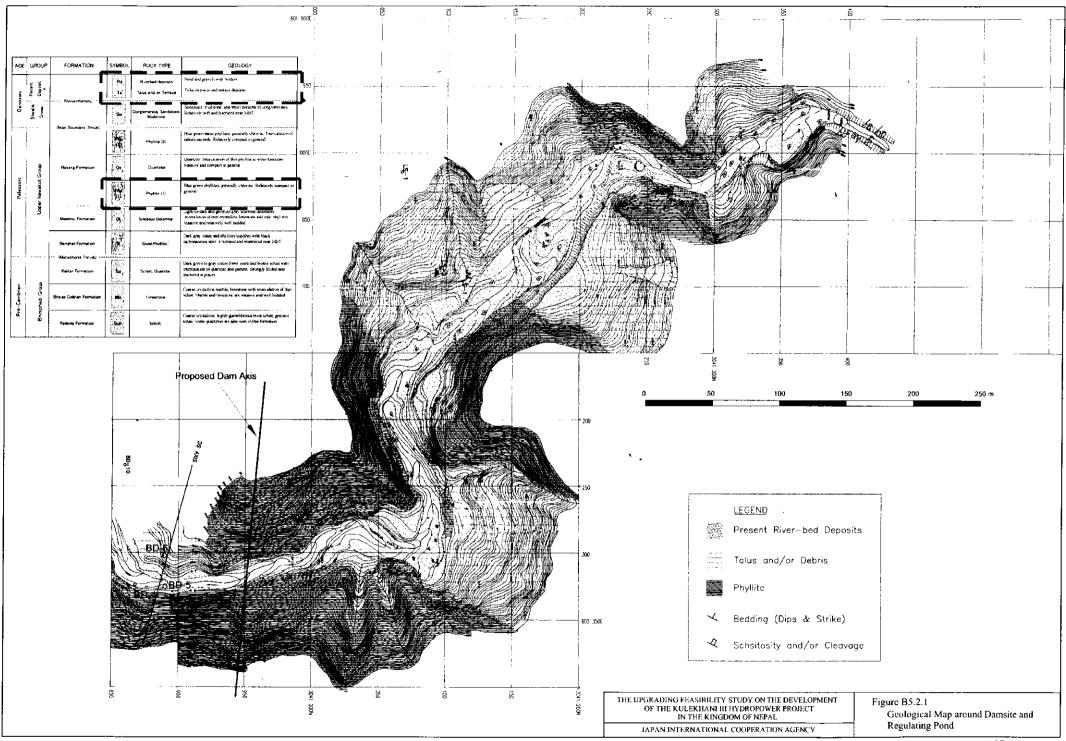
THE UPGRADING FEASIBILITY STUDY ON THE DEVELOPMENT OF THE KULEKHANI III HYDROPOWER PROJECT IN THE KINGDOM OF NEPAL

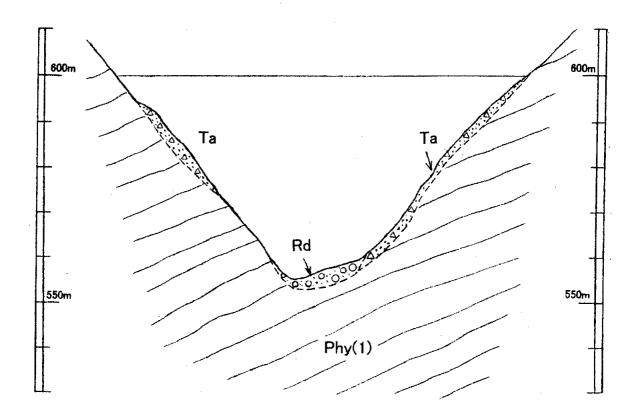
JAPAN INTERNATIONAL COOPERATION AGENCY

Figure B4.1.2 Correlation of Q System and CRIEPI Rock Classification









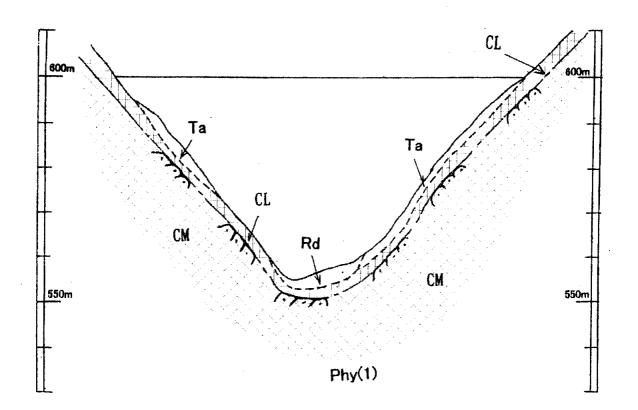
Legend

AGE	GROUP	FORMATION	ROCKS	GEOLOGY
ozoic		Recent Deposits	O Riverbed deposits	Sand, gravels and bolders of phyllites, limestone and schist are mainly distributed. Bolders and gravels and coarse materials are mainly distributed in the river due to high gradients of river-bed.
Cenozo		·	Δ Talus deposits Δ Δ	Sand, gravels and bolders of phyllites are generally distributed in the area.
Paleozoic	Upper Nawakot Group	(Uncomformity)—— Robang Formation	Phy Phyllite (1)	Blue green phyllites, generally chloritic. Relatively compact in general.

THE UPGRADING FEASIBILITY STUDY ON THE DEVELOPMENT OF THE KULEKHANI III HYDROPOWER PROJECT IN THE KINGDOM OF NEPAL

JAPAN INTERNATIONAL COOPERATION AGENCY

Figure B5.2.2 Geological Profile of Proposed Damsite



Legend

[<u>[</u>] CM

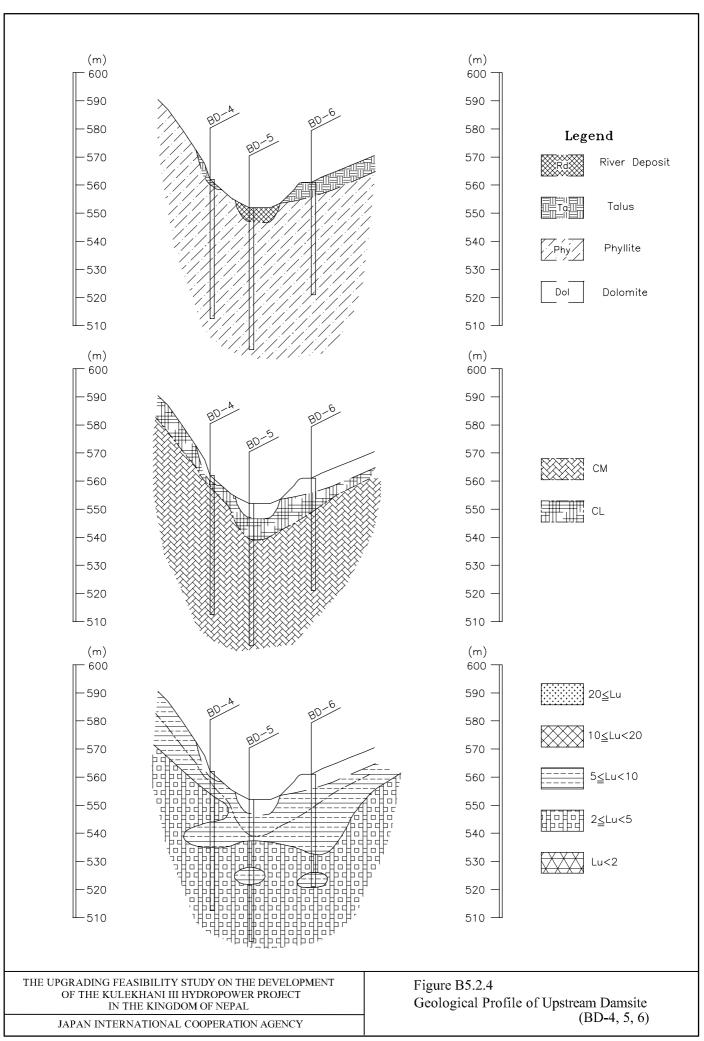
CL CL

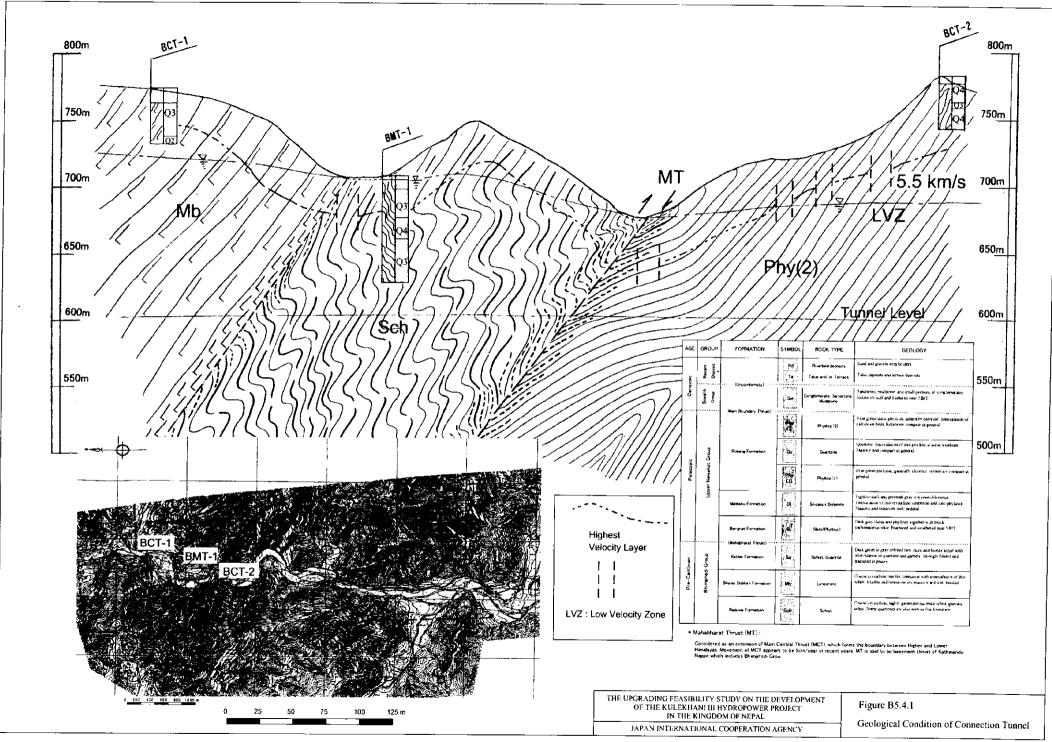
Excavation Line

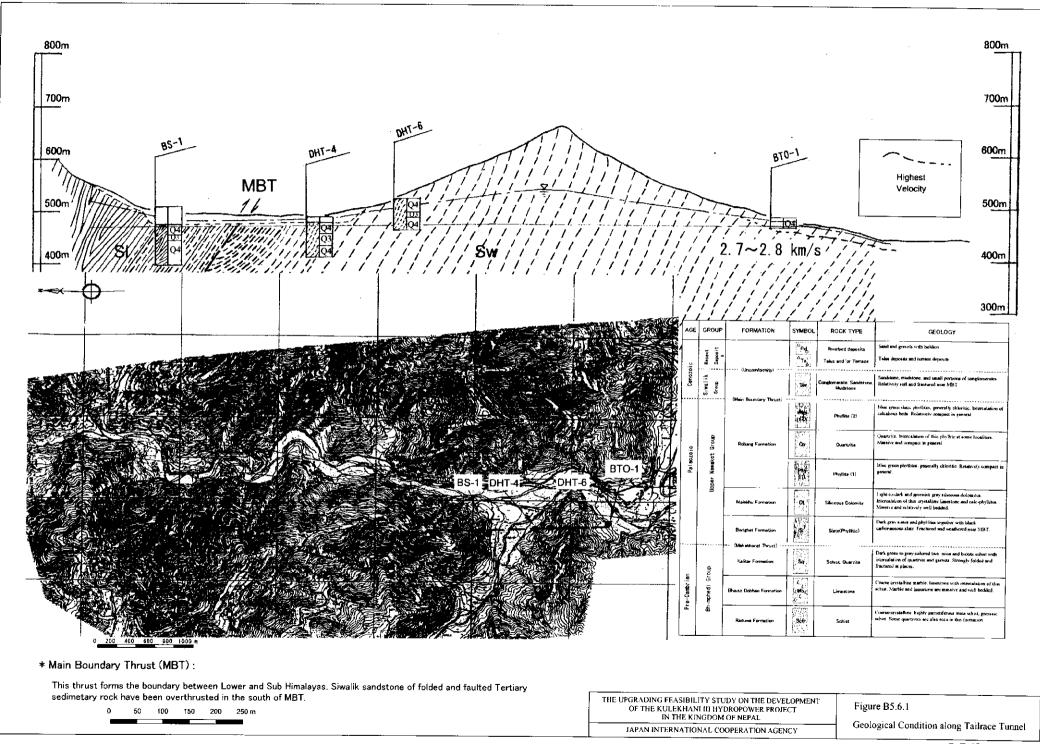
THE UPGRADING FEASIBILITY STUDY ON THE DEVELOPMENT OF THE KULEKHANI III HYDROPOWER PROJECT IN THE KINGDOM OF NEPAL

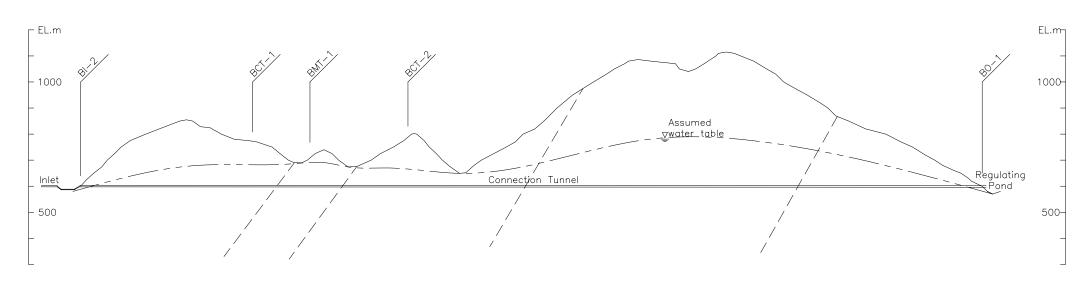
JAPAN INTERNATIONAL COOPERATION AGENCY

Figure B5.2.3 Geological Profile of Proposed Damsite (Rock Grade)



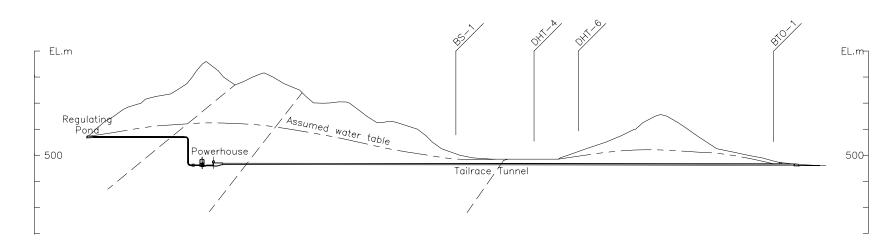






Additional Distance (m)	500	1000	1500	2000	2500 300	00
Rock Grade (interval)	Q2 (685)	Q4 Q3 Q5 (60) (205) (75)	Q3 Q4 (590) (80	Q2 (1000)	Q4 (80)	Q3 (700)
Geology	Marble	Schist	Phyllite/ Quartzitic Phyllite	Quarzite		Phyllite
Groundwater	Med	ium	Large	Large — Medium		Medium

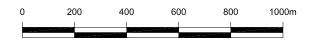
(Inlet To Regulating Pond)



Additional Distance (m)	1 1	5	00 _{Q5}	1000	1 1 1	1500	1 1	2000	2500
Rock Grade (interval)	Q3 (315)	Q4 Q2 (20) (26	11111	Q3 (425)	Q4 (320)	Q5 (280)	Q4 (100)	Q3 (820)	Q4 (50)
Geology	Phyllite	Dolom	nite (50)	Slo	Slate			Sandstone	·
Groundwater	Medium			Large				Medium	

Rock Grade	Modulus of Deformation (MPa)	Shear Strength (MPa)	Friction Degree (degree)	Q value by Q system	CRIEPI classification (Reference)
Q1	> 3,000	> 2.5	> 50	> 40	В
Q2	3,000	2.5	50	10 to 40	СН
Q3	1,000	1.2	45	4 to 10	СМ
Q4	500	0.6	40	1 to 4	CL
Q5	250	0.1	35	1 >	D

(Regulating Pond To Tailrace)

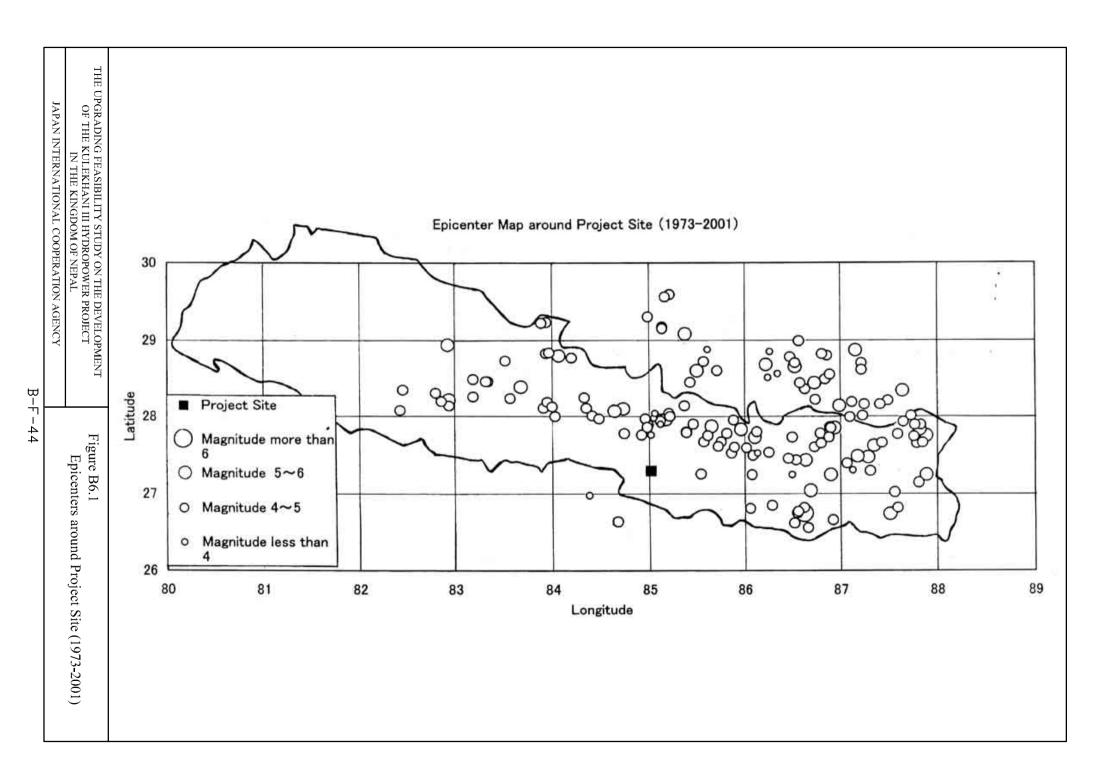


THE UPGRADING FEASIBILITY STUDY ON THE DEVELOPMENT OF THE KULEKHANI III HYDROPOWER PROJECT IN THE KINGDOM OF NEPAL

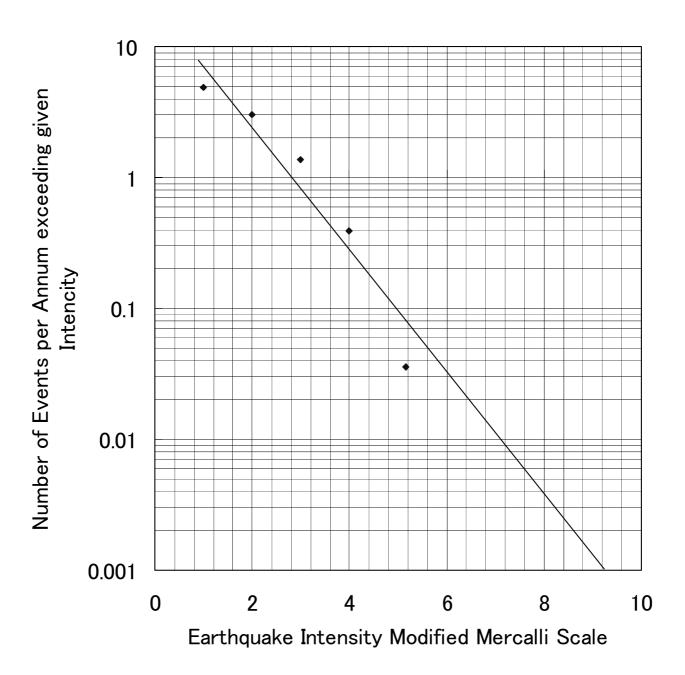
JAPAN INTERNATIONAL COOPERATION AGENCY

Figure B5.9.1

Rock Condition along Waterway



FREQUENCY-INTENSITY RELATION OF EARTHQUAKE(By Cornell)



THE UPGRADING FEASIBILITY STUDY ON THE DEVELOPMENT OF THE KULEKHANI III HYDROPOWER PROJECT IN THE KINGDOM OF NEPAL

JAPAN INTERNATIONAL COOPERATION AGENCY

Figure B6.2
Relationship between Seismic Intensity and Frequency