JAPAN INTERNATIONAL COOPERATION AGENCY (JICA) GOVERNMENT OF GUJARAT

THE RECONSTRUCTION SUPPORT FOR THE GUJARAT-EARTHQUAKE DISASTER IN THE DEVASTATED AREAS IN INDIA

APPENDICES

OCTOBER, 2002

YAMASHITA SEKKEI INC. NIHON SEKKEI, INC.

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1. GEOTECHNICAL INVESTIGATION

(1) Mundra Site

(2) Anjar Site

2. DRAWINGS

1. GEOTECHNICAL INVESTIGATION

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YAMASHITA SEKKEI INC.

TOKYO, JAPAN

TECHNICAL REPORT OF

GEOTECHNICAL INVESTIGATION

FOR

THE RECONSTRUCTION SUPPORT FOR THE GUJARAT-EARTHQUAKE DISASTER IN THE DIVASTATED AREAS IN INDIA

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SHRI BUDHIBAI MATERNITY HOME, GOVERNMENT OF GUJARAT, MUNDRA

BY:

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JULY 2001

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Abstract

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The report is presented herewith analyses based on thorough study of the geotechnical investigation results.

A complete geotechnical investigation was undertaken by us to obtain the required subsurface information to study and to indicate the nature and behavior of soil under the application of loads of community health centers at Anjar and Mundra to be reconstructed, after devastation caused by Bhuj Earthquake.

Such information were obtained form the following:

- 1. By making bore holes and collecting disturbed and undisturbed soil samples.
- 2. By conducting standard penetration tests.
- 3. By testing soil samples in laboratory for physical properties and strength characteristic of soils.
- 4. By analysis of results.

The results obtained from above were interpreted, analyzed and following conclusions are derived.

1. The study of soil investigation at the site at various depths and various locations indicate the layers as under:

Bore No.	Depth (m)	Description
BH–1	0.00 to 10.0	Clayey Sand (SC)
BH–2	0.00 to 10.0	Clayey Sand (SC)

- Isolated footings are suggested for proposed structures. The safe bearing capacities are worked out for various widths of isolated square footings at various depths and are suggested in Appendix – I. The depths of footings for which bearing capacities are calculated are considered with respect to the existing ground level.
- 3. Drainage properties of soil is fair.
- 4. Ground water table is not encountered upto the depth of Investigation.
- 5. Any variation found in stratification in any of the foundation location, shall be studied thoroughly and brought to our notice before executing the foundation work.

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GEOTECHNICAL INVESTIGATION REPORT FOR THE RECONSTRUCTION SUPPORT FOR THE GUJARAT EARTHQUAKE DISASTER IN THE DIVASTATED AREAS IN INDIA AT SHRI BUDHIBAI MATERNITY HOME, MUNDRA

1.0 Introduction

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For foundation analysis of the structure on the site, It is necessary

- 1. To determine the soil profile of the site
- 2. To know physical properties and strength characteristics of soil at various depths.

For this purpose, the geotechnical investigation was entrusted to us by Yamashita Sekkei Inc. The following points were decided.

- 1. No. Bore hole 2
- 2. Depth of bore hole 10.0
- 3. Standard penetration tests at every 1.0 m interval.
- 4. Undisturbed samples at every 3.0 m interval.
- 5. Collection of disturbed samples.
- 6. To find physical properties and strength characteristics of undisturbed samples.
- 7. To find physical properties of disturbed samples.
- 8. To locate ground water table, if any.
- 9. Interpretation of results, Analysis.
- 10. Recommendations.

Based on the above points the detailed Geotechnical Investigation Program included the following :

(A) Field Investigation

- 1. Drilling of bore hole.
- 2. Collection of soil samples (Disturbed and Undisturbed)
- 3. Conducting Standard Penetration Test.

(B) Laboratory Investigation

- 1. Bulk Density and moisture content
- 2. Grain size analysis
- 3. Index properties
- 4. Shear tests (Direct shear test)
- 5. Consolidation test
- 6. Unconfined Compression Test

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(c) Recommendations

Based on above investigations, the results are to be obtained. The findings would be based on interpretation of Results, Analysis and computations as per relevant Indian standards.

2.0 Field Investigation

2.1 Boring

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The exploratory boreholes of 150 mm diameter were drilled by Rotary Drilling. The depth of the test bore at the proposed location is as under:

Bore Hole No.	Location	Reduced Level of Bore Hole (m)	Depth Investigated (m)
BH – 1	N 1053.4 E 1085.8	99.3	10.0
BH – 2	N 1085.3 E 1074.5	99.6	10.0

2.2 Sampling

2.2.1 Disturbed samples

Disturbed samples were collected during boring and from the split spoon sampler. The samples recovered were logged, labeled and placed in polyethylene bags and sent to laboratory for testing.

2.2.2 Undisturbed samples

Undisturbed soil samples were collected in thin walled Shelby tubes, as per IS 2132. The samples were sealed with wax, labeled and transported to our laboratory at Paldi, Ahmedabad for testing.

2.2.3 Standard penetration test

The standard penetration tests were conduct in accordance with IS:2131 in test bore at every 1.0 m. The test gives N - Value, the blow counts of last 30 cm penetration of split spoon sampler with 65 kg hammer falling from 75 cm height. These values are given in table no. 1.

3.0 Laboratory investigation

The following laboratory tests were conducted on undisturbed and disturbed soil samples collected form various depths to find physical properties and strength characteristics. For measurements of soil properties in the laboratory the following table lists various laboratory tests, which were conducted in the laboratory.



Tests	Recommended procedure	Type Samples			
1. Sample Penetration	IS 2720 Pt I	DS / UDS			
2. Moisture Content	IS 2720 Pt II	DS / UDS			
3. Dry Unit Weight	LAMBE	UDS			
4. Specific Gravity	IS 2720 Pt III	DS			
5. Liquid Limit	IS 2720	DS			
6. Plastic Limit	IS 2720 Pt V	DS			
7. Grain Size Analysis	IS 2720 Pt IV	DS			
8. Soil Classification	IS 1498	DS / UDS			
9. Consolidation	IS 2720 Pt XV	UDS			
10. U.Comp. Strength	IS 2720 Pt X	UDS			
11. Direct Shear Test	IS 2720 Pt X	UDS			

4.0 Results

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1) Locations of boreholes are shown in fig no. 1

2) Bore log details are shown in fig No. 2 & 3

3) SPT N values are appended in table no. 1

4) Results of laboratory permeability tests are given in table no. 2

5) Laboratory test results of Bore holes are appended in table nos. 3 & 4

6) Results of the grain size analysis of are given in fig nos. 4 & 5

7) Results of the shear test are given in fig nos. 6 & 7

5.0 General stratification

Stratifications observed in bore holes are as described below :-

Bore No.	Depth (m)	Description
BH-1	0.00 to 10.0	Clayey Sand (SC)
BH-2	0.00 to 10.0	Clayey Sand (SC)

6.0 Computation of Safe Bearing Capacity

The detailed calculations of the safe Bearing capacity for various widths of isolated square footings, at various depths are given in Appendix- I.



7.0 Conclusions

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- 1. The study of soil investigation at the site at various depths and various locations indicate the layers as shown in general stratification.
- Isolated footings are suggested for proposed structures. The safe bearing capacities are worked out for various widths of isolated square footings at various depths and are suggested in Appendix – I. The depths of footings for which bearing capacities are calculated are considered with respect to the existing ground level.
- 3. Drainage properties of soil is fair.
- 4. Ground water table is not encountered upto the depth of Investigation.
- 5. Any variation found in stratification in any of the foundation location, shall be studied thoroughly and brought to our notice before executing the foundation work.

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APPENDIX - I

Calculation of Safe Bearing Capacity Based on Shear Parameters C - ϕ

qu = 1 / FS [2 / 3 C Nc dc Sc ic + γ d (Nq - 1) Sq dq iq Wq + 0.5 γ B N γ S γ d γ i γ W γ]

Project: Government of Gujarat Hospital, Shri Budhibai Maternity Home - Mundra

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		Wate	Соп	Мq	8	<u>}</u>	1.00	1 20	3	1.00		1.00	200	3	1.00		1.00	1.00		1.00	1 00	2	1.00
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		dγ			1.13		1.12	1.10		1.14		1.13	1 1 1		1.16	4	0[·]	1.13		1.18	1.16		4
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		Nγ			2.41		2.41	2.41		2.41	14 0		2.41		2,41	1.0	-+ - 	2.41	1	×.4 I	2.41	17	12.5
		0.5γ			0.82	6	7870	0.82		U.82	0 8.0		0.82		0.82	0 8.0	-+-	0.82	50.0		0.82	680	-
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	0)	Ka/cm ²	0.00	24.5	0.29	0.29		0.29		87'N	0.29		0.29		0.29	0.29		N.23	0.29		0.29	
	Depth	of Footing		1.80		1.80	1.80		2.00		3.4	2.00		2.30		Z.3U	2.30	02 0	z.90	2.50		DQ:2	
	Width	of Footing o	٤	1.80		2.00	2.30		1.80	00 6	20.4	2.30		1.80	000		2.30	1 80	00.1	2.00		Z.3U	
	Length	of Footing o	٤	1.80	200	z.uu	2.30	ŝ	1.80	2.00		2.30		1.80	600	3	2.30	1.80		2.00	0000	- 30 I	
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Nayak, N.V.

Kaniraj S.R.

Alam Singh

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TABLE NO. - 1

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S.P.T. " N " Values

Sr.	Bore Hole	Depth	S.P.T. N Value											
No. No. (m)		N1	N2	N3	N									
			(0 - 15 cm)	(15 - 30cm)	(30 - 45 cm)	45								
1	BH - 1	1.0	4	6	9	15								
2	BH - 1	3.0	5	7	11	18								
3	BH - 1	4.0	6	10	10	20								
4	BH - 1	6.0	7	10	13	23								
5	BH - 1	7.0	9	11	15	26								
6	BH - 1	9.0	11	14	17	31								

Sr.	Bore Hole	Depth	S.P.T. N Value											
No. No. (m)		(m)	N1 (0 - 15 cm)	N2 (15 - 30cm)	N3 (30 - 45 cm)	N								
1.	BH – 2	1.0	4	6	8	14								
2	BH – 2	2.0	5	7	10	17								
3	BH 2	4.0	8	7	12	19								
4	BH 2	5.0	10	9	16	25								
5	BH 2	7.0	10	12	17	29								
6	BH – 2	8.0	12	15	. 15	30								
7	BH – 2	10.0	14	15	19	34								

TABLE NO. - 2 Results of laboratory permeability

Sr. No.	Bore Hole No.	Depth (m)	Laboratory permeability (mm / s)
1	BH – 1	1.00	0.62 X 10 ⁻³
2	BH – 1	2.00	0.65 X 10 ⁻³
3	BH – 2	1.00	0.56 X 10 ⁻³
4	BH – 2	3.00	0.60 X 10 ⁻³

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RESULTS OF LABORATORY TEST

Project :- Government of Gujarat Hospital, Shri Budhibal Maternity Home - Mundra

Co ordinates :- N1085.3 E1074.5 BH No. :- 2

Porosity 36.2 35.8 * 35.0 . . , ÷ CON STATES . 1 . Volds Ratio 0.57 0.56 0.54 1 1 1 Rob æ ı. r . . ÷ • , ł Value (on) z r 7 5 r 10 25 ī 28 8 34 . Comp index ပိ ۲ , ī 4 4 . . 1 , . . Kg/cm² ucs ï ł. ı. . . ı 'n , ï • Degrae Shear Parameter ٠ 1 24 ÷. ı. • , 8 . 1 8 r Kg/cm² 0.32 o 0.36 1 ı. r. 0.87 r 1 £ . 1 fication Classi Soil SS S S S S ŝ S S ပ္တ SS ပ္တ ŝ * r , ı. ı ł , , , . ı, Kg/cm² 5 . . ı . ī . ١ t . , 4 ร . * , . , 1 r , Consistancy limits ā ,**3**4 S ω 00 0 8 ω ω ∞ ð 80 ø Ľ, * 17 13 5 16 7 7 8 38 9 9 ₽ Н * 26 Ы 8 25 23 8 28 26 2 27 27 o * ო ო 4 4 2 4 4 2 2 ო e Grain Size Analysis Σ 26 ę 9 6 ÷ 얻 Ŧ 4 33 우 6 έ ø * 84 87 8 85 86 83 84 89 85 84 87 œ × 0 0 0 0 0 o 0 0 0 0 0 ŝ 2.65 ٩, 5 2.85 . a. 2.66 Ø . ī ł . NMC 7.84 7.87 7.93 7.95 * 8.02 8.07 8.09 8.27 8.30 8.34 8.21 £ gm / cc Density Field 1.69 Reduced Level :- 99.60 1.70 1.73 . r . . , ı. . . Sample Type For KCT Consultancy Services, ъ SOD SPT SPT ដ SQN SPT SPT ŝ SPT SPT SPT Depth 0.00 1.00 10.00 2.8 3.0 4.00 6.00 E 5.00 2.00 9.00 8.00 š Ŷ ÷ ~ N 3 4 ŝ 9 ~ 9 ø 6

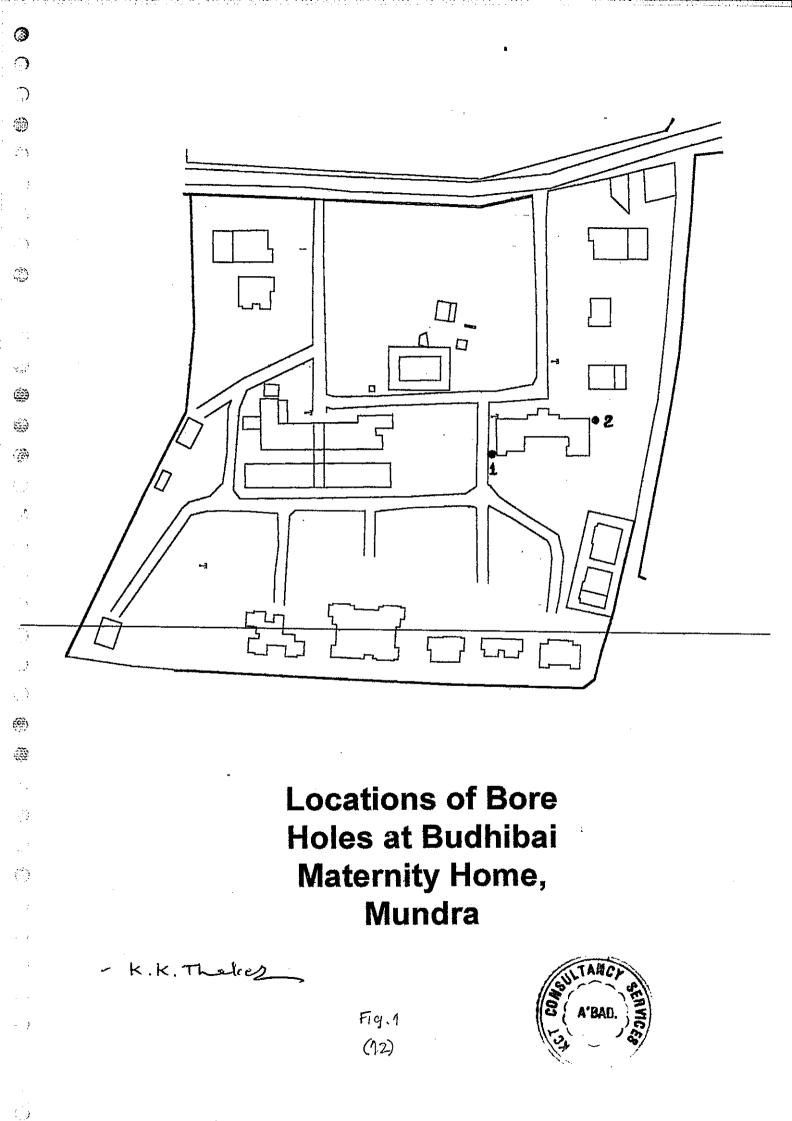
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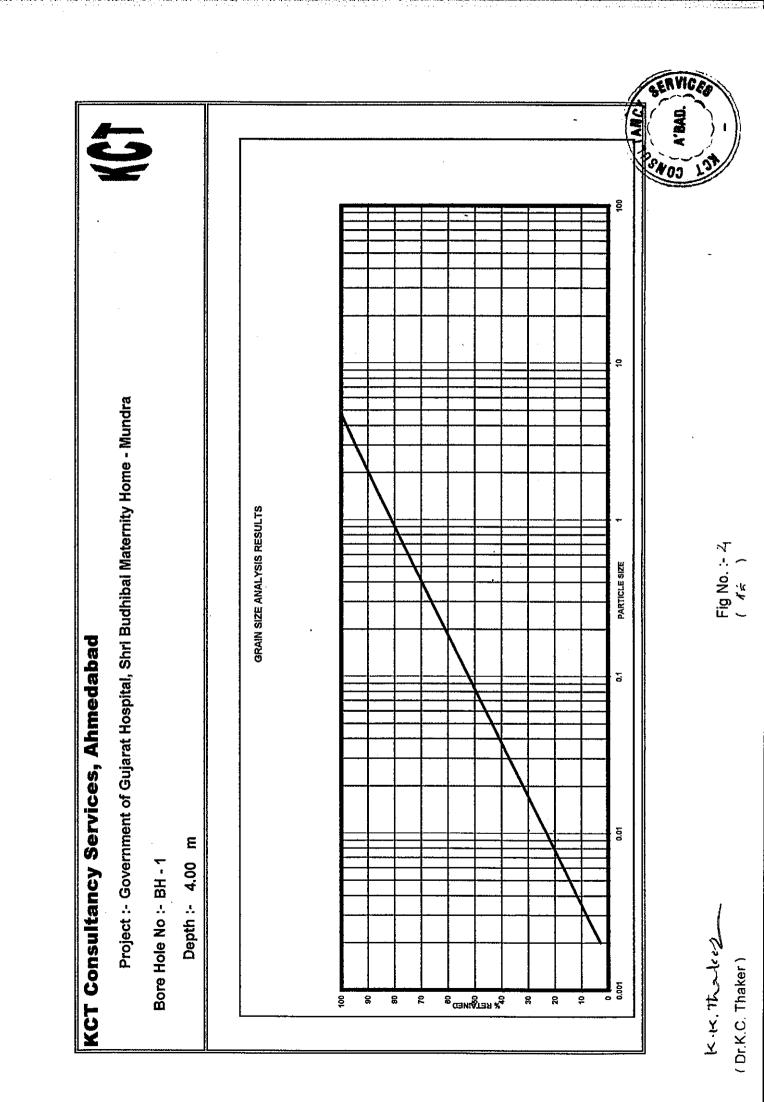
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		Nos.	- 4	Sąmple/S.P.T. R.Q.D. Z Z Z Z Z Z Z Z Z		0.00	1.00	••••••••	3.00	4.00	·····	6.00	7.00		9.00		SERVICES UNALA 1081
		Sample	SQ NDS	Dndisturbed Sample Disturbed		0		2.00	ന 	4	5.00		1.1	8.00		10.00	
		Nos.	8	to .oN .T.9.2 Swold			4+6+9		5+7+11	6 + 10+ 10		7 + 10 + 13	9 + 11 + 15		11 + 14 + 17		
		Field Test	1dS		80 90 100											_	- - - -
LOG DATA SHEET	ndra			S.P.T. "N" Value	101201301401501601701		2		18	20		SZ	56		31		Hg No.2 (13)
BORE	Government of Gujarat Hospital, Shri Budhibai Maternity Home - Mundra N 1053 4 F 1085 B			Soil Description							Clayey Sand (SC)						
KCT CONSULTANCY SERVICES, AHMEDABAD	ernment of Gujarat Hospital, 53 4 F 1085 8	E	τ Ε	Ceptur (m) Lhickness of Soc Soc Soc Soc Soc Soc Soc Soc Soc Soc		0.00	1.00	2.00	3.00	6.50 4.00	6.00 10.0 m	6.00	7.00	8.00	8.50 9.00	9.50 4111111	(
ONSULTANC	Project : Gove Cordinates: N 10		Bore Hole No. : BH ~ 1 Termination Depth : 10.0 m	ეფsing 3ore Dia. (mm Depth (m)	3										;		K, K. Thurle & (KK.Thaker)
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		Nos.	- 0	ציסים.		ł										X	100
		Sample	SO	Disturbed Sample/S.P.T.		0.00	1.00	2.00		4.00	5.00	<u> </u>	7.00	8.00		10.00	
		Sal		Undisturbed Sample	l				3.00			6.00			9.00		
		Nos.	7	fo .oN .T.q.S blows			4+6+8	5 + 7 + 10		8 + 7 + 12	10 + 9 + 16		10 + 12 + 17	12 + 15 + 15		14 + 15 + 19	
		Fleid Test	LdS		901 100 1												-
				Value	1 70 80												
ET				S.P.T. "N" Value	40 50 60						25		59	30		34	
LOG DATA SHEET				,	2013014					19							-
DAT	Mundra				10												
BORE LOG	Government of Gujarat Hospital, Shri Budhibai Maternity Home - Mundra N 1085.3 E 1074.5			Soil Description							Clayey Sand (SC)						
	of Gujarat Hospital, 074.5			Layer (m) Layer (m)							10.0 m						
	ment o			Thickness of		0.9						20	00		88	018	
	Government of Guj N 1085.3 E 1074.5		BH - 2 10.0 m	Depth (m)			0.00 1-00	1.50	3.00	3.50	5.00	0.00 0.00	6.50	8.0	8.50 9.00	<u>-0</u>	
	Project : Cordinates:		te No. : Depth :	mm) .siG əroB		-	.			6						≻	
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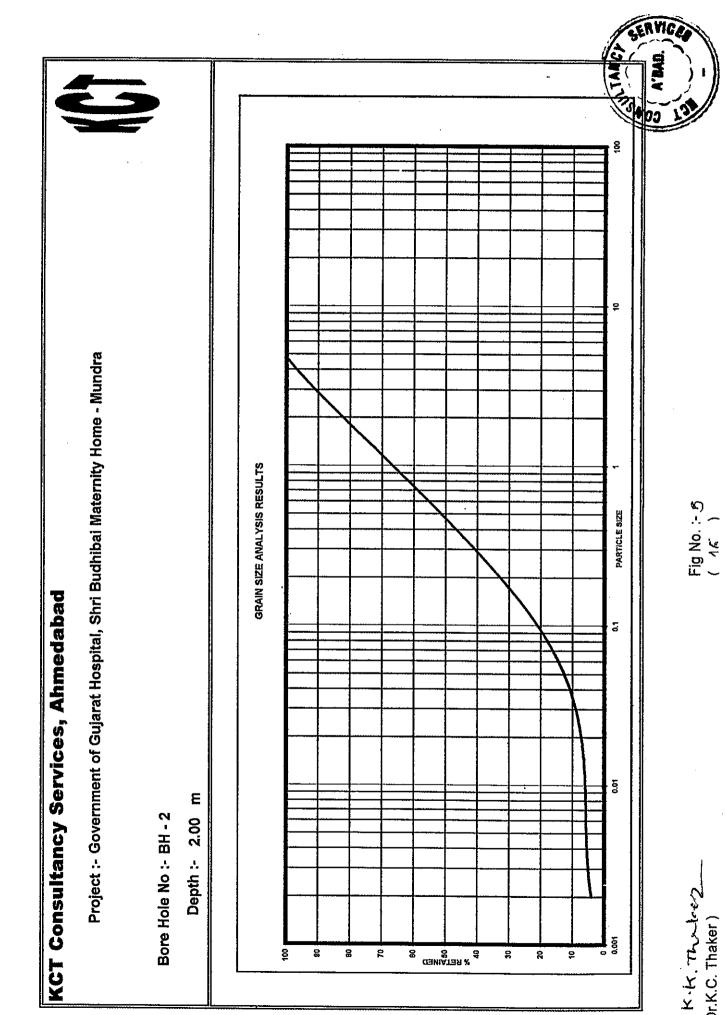
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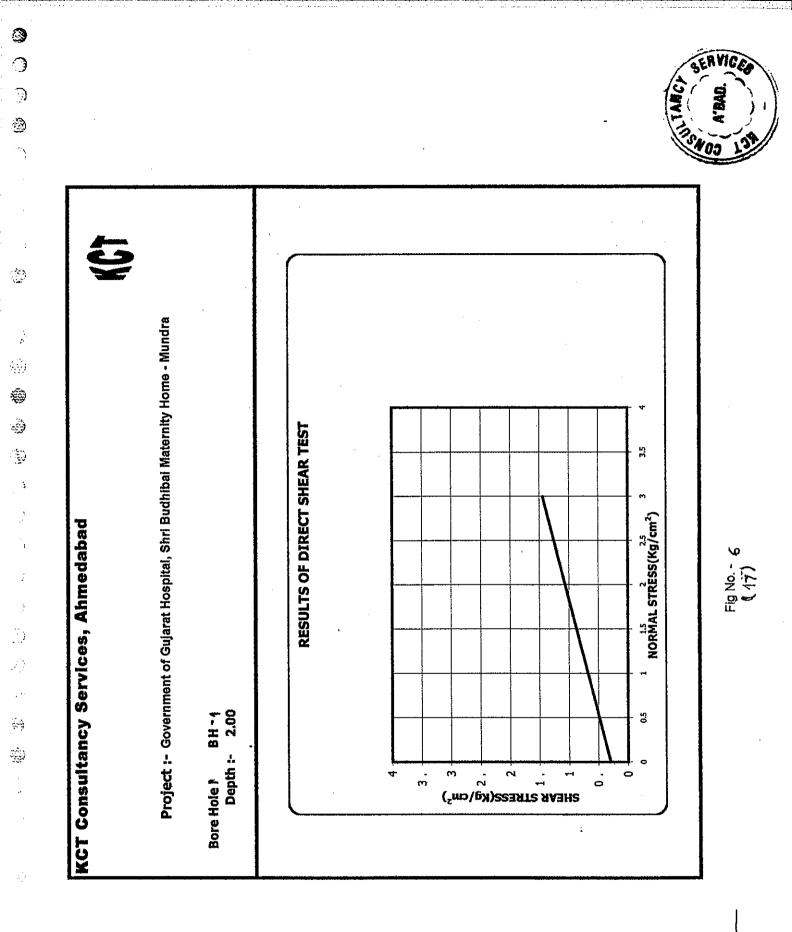
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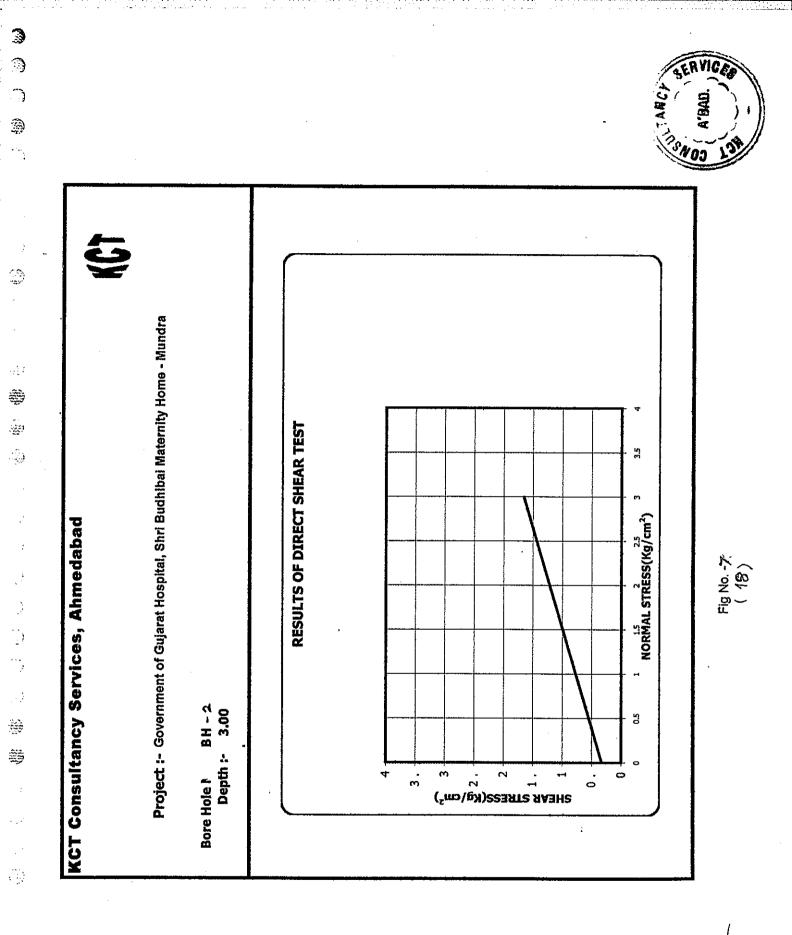
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TECHNICAL REPORT OF

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FOR

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AT

RAFFRAL HOSPITAL, ANJAR

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The report is presented herewith analyses based on thorough study of the geotechnical investigation results.

A complete geotechnical investigation was undertaken by us to obtain the required subsurface information to study and to indicate the nature and behavior of soil under the application of loads of community health centers at Anjar and Mundra to be reconstructed, after devastation caused by Bhuj Earthquake.

Such information were obtained form the following:

- 1. By making bore holes and collecting disturbed and undisturbed soil/rock samples.
- 2. By conducting standard penetration tests.
- 3. By testing soil/rock samples in laboratory for physical properties and strength characteristic of soils.
- 4. By analysis of results.

The results obtained from above were interpreted, analyzed and following conclusions are derived.

1.	The	study	of	soil	investi	gation	at	the	site	at	various	depths	and
	vario	ous loc	atio	ons ii	ndicate	the lay	ers	ası	under	r:			

Bore No.	Depth (m)	Description
	0.00 to 1.40	Clayey Sand (SC)
BH-1	1.40 to 10.0	Soft Weathered Rock
	0.00 to 1.30	Clayey Sand (SC)
BH–2	1.30 to 10.0	Soft Weathered Rock
	0.00 to 0.60	Clayey Sand (SC)
BH-3	0.60 to 10.0	Soft Weathered Rock
	0.00 to 0.80	Clayey Sand (SC)
BH4	0.80 to 10.0	Soft Weathered Rock
	0.00 to 1.00	Clayey Sand (SC)
BH–5	1.00 to 10.0	Soft Weathered Rock
	0.00 to 0.80	Clayey Sand (SC)
BH–6	0.80 to 10.0	Soft Weathered Rock



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- Isolated footings are suggested for proposed structures. The safe bearing capacities are worked out for various widths of isolated square footings at various depths and are suggested in Appendix – I. The depths of footings for which bearing capacities are calculated are considered with respect to the existing ground level.
- 3. Drainage properties of soil is fair in top layer of clayey soils and poor in case of weathered rock.
- 4. Ground water table is not encountered upto the depth of Investigation.
- 5. Any variation found in stratification in any of the foundation location, shall be studied thoroughly and brought to our notice before executing the foundation work.

(Dr. K.C. Thaker)

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GEOTECHNICAL INVESTIGATION REPORT FOR THE RECONSTRUCTION SUPPORT FOR THE GUJARAT EARTHQUAKE DISASTER IN THE DIVASTATED AREAS IN INDIA

1.0 Introduction

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For foundation analysis of the structure on the site, It is necessary

- 1. To determine the soil profile of the site
- 2. To know physical properties and strength characteristics of soil at various depths.

For this purpose, the geotechnical investigation was entrusted to us by Yamashita Sekkei Inc. The following points were decided.

- 1. No. Bore hole 6
- 2. Depth of bore hole 10.0
- 3. Standard penetration tests at every 1.0 m interval.
- 4. Undisturbed samples at every 3.0 m interval.
- 5. Collection of disturbed samples.
- 6. To find physical properties and strength characteristics of undisturbed samples.
- 7. To find physical properties of disturbed samples.
- 8. To locate ground water table, if any.
- 9. Interpretation of results, Analysis.
- 10. Recommendations.

Based on the above points the detailed Geotechnical Investigation Program included the following :

(A) Field Investigation

- 1. Drilling of bore hole.
- 2. Collection of soil / rock samples (Disturbed and Undisturbed)
- 3. Conducting Standard Penetration Test.

(B) Laboratory Investigation

- 1. Bulk Density and moisture content
- 2. Grain size analysis
- 3. Index properties
- 4. Shear tests (Direct shear test)
- 5. Consolidation test
- 6. Unconfined Compression Test

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(c) Recommendations

Based on above investigations, the results are to be obtained. The findings would be based on interpretation of Results, Analysis and computations as per relevant Indian standards.

2.0 Field Investigation

2.1 Boring

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The exploratory boreholes of 150 mm diameter were drilled by Rotary Drilling. The depth of the test bore at the proposed location is as under:

Bore Hole No.	Location	Reduced Level of Bore Hole (m)	Depth Investigated (m)
BH - 1	N 1004.5 E 1012.6	100.84	10.0
BH – 2	N 969.71 E 983.99	100.87	10.0
BH – 3	N 949.06 E 997.84	100.24	10.0
BH - 4	N 940.39 E 966.79	100.30	10.0
BH – 5	N 941.89 E 925.79	100.44	10.0
BH – 6	N 934.94 E 881.35	099.77 ·	10.0

2.2 Sampling

2.2.1 Disturbed samples

Disturbed samples were collected during boring and from the split spoon sampler. The samples recovered were logged, labeled and placed in polyethylene bags and sent to laboratory for testing.

2.2.2 Undisturbed samples

Undisturbed soil samples were collected in thin walled Shelby tubes where it was possible, however at all other depths intact core samples were taken using double tube core barrel, as per IS 2132. The samples were sealed with wax, labeled and transported to our laboratory at Paldi, Ahmedabad for testing.

2.2.3 Standard penetration test

The standard penetration tests were conduct in accordance with IS:2131 in test bore at every 1.0 m. The test gives N – Value, the blow counts of last 30 cm penetration of split spoon sampler with 65 kg hammer falling from 75 cm height. These values are given in table no. 1.

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3.0 Laboratory investigation

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The following laboratory tests were conducted on undisturbed and disturbed soil / rock samples collected form various depths to find physical properties and strength characteristics. Measurements of soil / rock properties in the laboratory the following table lists various laboratory tests, which were conducted in the laboratory.

Tests	Recommended procedure	Type Samples
1. Sample Penetration	IS 2720 Pt I	DS / UDS
2. Moisture Content	IS 2720 Pt II	DS / UDS
3. Dry Unit Weight	LAMBE	UDS
4. Specific Gravity	IS 2720 Pt III	DS
5. Liquid Limit	IS 2720	DS
6. Plastic Limit	IS 2720 Pt V	DS
7. Grain Size Analysis	IS 2720 Pt IV	DS
8. Soil Classification	IS 1498	DS / UDS
9. Consolidation	IS 2720 Pt XV	UDS
10. U.Comp. Strength	IS 2720 Pt X	UDS
11. Direct Shear Test	IS 2720 Pt X	UDS

4.0 Results

1) Locations of boreholes are shown in fig no. 1

2) Bore log details are shown in fig No. 2 to 7

3) SPT N values are appended in table no. 1

4) Results of laboratory permeability tests are given in table no. 2

5) Laboratory test results of Bore holes are appended in table nos. 3 to 8

6) Results of the grain size analysis of are given in fig nos. 8 to 13

7) Results of the shear test are given in fig nos. 14 to 19.

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5.0 General str Stratifications obs	atification served in bore holes are	as described below :-
Bore No.	Depth (m)	Description
BH-1	0.00 to 1.40	Clayey Sand (SC)
Dri 1	1.40 to 10.0	Soft Weathered Rock
BH-2	0.00 to 1.30	Clayey Sand (SC)
B11-2	1.30 to 10.0	Soft Weathered Rock
BH-3	0.00 to 0.60	Clayey Sand (SC)
D11-0	0.60 to 10.0	Soft Weathered Rock
BH-4	0.00 to 0.80	Clayey Sand (SC)
011-4	0.80 to 10.0	Soft Weathered Rock
BH5	0.00 to 1.00	Clayey Sand (SC)
01-0	1.00 to 10.0	Soft Weathered Rock
BH-6	0.00 to 0.80	Clayey Sand (SC)
0-110	0.80 to 10.0	Soft Weathered Rock

6.0 Computation of Safe Bearing Capacity

The detailed calculations of the safe Bearing capacity for various widths of isolated square footings, at various depths are given in Appendix- I.

7.0 Conclusions

- 1. The study of soil investigation at the site at various depths and various locations indicate the layers as shown in general stratification.
- Isolated footings are suggested for proposed structures. The safe bearing capacities are worked out for various widths of isolated square footings at various depths and are suggested in Appendix – I. The depths of footings for which bearing capacities are calculated are considered with respect to the existing ground level.
- 3. Drainage properties of soil are fair in top layer of clayey soils and poor in case of weathered rock.
- 4. Ground water table is not encountered upto the depth of Investigation.
- 5. Any variation found in stratification in any of the foundation location, shall be studied thoroughly and brought to our notice before executing the foundation work.

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APPENDIX - I

Calculation of Safe Bearing Capacity Based on Shear Parameters C - ϕ

qu = 1 / FS [2 / 3 C Nc dc Sc ic + γ d (Nq - 1) Sq dq iq Wq + 0.5 γ B N γ S γ d γ i γ W γ]

Project : Raffral Hospital at Mundra Road, Anjar

		SBC		t/m ²	24	25	25	29	29	30	32	32	32	36	36	37	
		Water Table	Correction	W۲	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
		Wate	Соп	ЪХ	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
		بد ب			1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	6	1.00	1.00	1.00	
		ġ			1.12	1.11	1.10	1.15	1.13	1.12	1.16	1.15	1.13	1.19	1.17	1.15	
		Sγ			0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.80	0.80	
		ž			9.79	9.79	9.79	9.79	9.79	9.79	9.79	9.79	9.79	9.79	9.79	9.79	
		0.5 7			0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	
		ĕ			1.00	1.00	1.00	1.00	1.00	- 00. 100	1.00	1.00	1.00	1.00	1.00	1.00	
		р			1.12	1.11	1.10	1.15	1.13	1.12	1.16	1.15	1.13	1.19	1.17	1.15	
	oting	Sq			1.20	1.20	1.20	1.20	1.20	1.20	1.20	1.20	1.20	1.20	1.20	1.20	
	For Isolated Square Footing	Nq - 1			12.13	12.13	12.13	12.13	12.13	12.13	12.13	12.13	12.13	12.13	12.13	12.13	
7	ted Sq	٢		gm/cc	1.88	1.88	1.88	1.88	1.88	1.88	1.88	1.88	1.88	1.88	1.88	1.88	
	r Isola	<u>.</u> 2			1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
1 N A A	ŝ	qc			1.25	1.22	1.19	1.30	1.27	1.23	1.33	1.30	1.26	1.38	1.34	1.30	
inini a		Sc			1.30	1.30	1.30	1.30	1.30	1.30	1.30	1.30	1.30	1.30	1.30	1.30	
		Nc			21.26	21.26	21.26	21.26	21.26	21.26	21.26	21.26	21.26	21.26	21.26	21.26	
		ቀ		degree	31	31	31	31	31	31	31	31	31	31	31	31	
		с		Kg/cm ²	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
		Depth	of Footing	ε	1.50	1.50	1.50	1.80	1.80	1.80	2.00	2.00	2.00	2.30	2.30	2.30	
•		Width	of Footing of Footing	ε	1.80	2.00	2.30	1.80	2.00	2.30	1.80	2.00	2.30	1.80	2.00	2.30	
		Length	of Footing	ε	1.80	2.00	2.30	1.80	2.00	2.30	1.80	2.00	2.30	1.80	2.00	2.30	
		Sr.		No.		2	ო	4	ى	9	7	8	ი	10	11	12	



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Notations

C	Cohesion Angle of internal friction of soil Disturbed Sample Undisturbed Sample Natural Moisture Content Non Plastic Soils Specific Gravity Gravel Content Sill Content Sand Content Liquid Limit Plastic Limit Plastic Limit Plastic Limit Plastic Limit Plastic Limit Plastic Limit Plastic Jindex Swelling Pressure Free Swell Index Rock Quality Designation Compression Index Coefficient of Permeability Unconfined Compression SPT Value Bore Hole The Number of Bore Holes Bearing Capacity Factor Shape Factors Density of Soil Depth of foundation Factor of Safety Coefficient of consolidation Unconsolidated undrained triaxial test Consolidated undrained triaxial test Consolidated drained triaxial test Consolidated drained triaxial test Consolidated drained triaxial test Clayey Gravels Poorely Graded Gravels Well Graded Gravels
Cc	Compression Index
	•
Sc,Sq,Sy	
Ŷ	•
	Coefficient of consolidation
	Poorely Graded Gravels
GW	Well Graded Gravels
SC SM	Clayey Sand Silty Sand
SW	Wel Graded Sand
SP	Poorly Graded Sand
СН	Clays of High Plasticity
CI CL	Clays of Intermediate Plasticity Clays of Low Plasticity
МН	Silts of High Plasticity
MI	Silts of Intermediate Plasticity
ML	Silts of Low Plasticity
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Śr.	Bore Hole	Depth		S.P.T. N	Value	
No.	No.	(m)	N1	N2	N3	N
			(0 - 15 cm)	(15 - 30cm)	(30 - 45 cm)	
1	BH - 1	1.0	11	22	30	52
2	BH - 1	2.0	12	22	33	55
3	BH - 1	4.0	14	25	33	58
4	BH - 1	5.0	15	26	33	59
5	BH - 1	7.0	20	26	35	61
6	BH - 1	8.0	22	30	33	63
7	BH - 1	10.0	25	31	34	65
	<u> </u>	I		<u> </u>		
Sr.	Bore Hole	Depth		S.P.T. N	Value	
NO.	No.	(m)	N1	N2	N3	N
			(0 - 15 cm)	(15 - 30cm)	(30 - 45 cm)	
1	BH – 2	1.0	13	21	32	53
2	BH – 2	3.0	15	25	31	56
3	BH – 2	4.0	18	24	32	56
4	BH – 2	6.0	21	25	35	60
5	BH – 2	7.0	24	28	. 35	63
6	BH – 2	9.0	26	30	35	65
	· · · · ·	<u> </u>				
Sr.	Bore Hole	Depth		S.P.T. N	Value	
No.	No.	(m)	N1	N2	N3	N
			(0 - 15 cm)	(15 - 30cm)	(30 - 45 cm)	
1	BH – 3	1.0	16	25	25	50
2	BH – 3	2.0	18	25	27	52
3	BH – 3	4.0	18	27	27	54 .
4	BH – 3	5.0	22	25	31	56
5	BH – 3	7.0	25	25	33	58
6	BH – 3	8.0	25	27	• 33	• 60
7	BH – 3	10.0	28	31	35	66

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1944 A	2	BH - 4	3.0
٢	3	BH – 4	4.0
	4	BH - 4	6.0
	5	BH - 4	7.0
	6	BH – 4	9.0
Sec.			
	Sr.	Bore Hole	Depth
0	No.	No.	(m)
ોંગ્રે			
8.5.	1	BH – 5	1.0
	2	BH – 5	2.0
	3	BH 5	4.0
	4	BH – 5	5.0
	5	BH – 5	7.0
			7.0

5

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Sr.

No.

Bore Hole

No.

BH – 4

Depth

(m)

1.0

7.0

9.0

N1

(0 - 15 cm)

17

17

19

23

25

25

N1

(0 - 15 cm)

16

18

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26

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4	BH – 5	5.0	20	30	32	62
5	BH – 5	7.0	26	28	36	65
6	BH – 5	8.0	28	28	49	69
7	BH – 5	10.0	27	31	38	73
				· · · · · · · · · · · · · · · · · · ·		
Sr.	Bore Hole	I Depth		SPT N	Value	
Sr. No	Bore Hole	Depth	NI	S.P.T. N		. M
Sr. No.	Bore Hole No.	Depth (m)	N1	N2	Value N3	N
	i	· ·	N1 (0 - 15 cm)			N
	i	· ·	•	N2	N3	N 51
No.	No.	(m)	(0 - 15 cm)	N2 (15 - 30cm)	N3 (30 - 45 cm)	
No. 1	No. BH 6	(m) 1.0	(0 - 15 cm) 21	N2 (15 - 30cm) 25	N3 (30 - 45 cm) 26	51
No. 1 2	No. BH 6 BH 6	(m) 1.0 3.0	(0 - 15 cm) 21 24	N2 (15 - 30cm) 25 25	N3 (30 - 45 cm) 26 30	51 55

S.P.T. N Value

S.P.T. N Value

N3

(30 - 45 cm)

33

32

36

35

35

35

N3

(30 - 45 cm)

32

30

30

33

40

N

53

54

60

63

65

67

N

54

58

59

64

70

N2

(15 - 30cm)

20

22

24

28

30

32

N2

(15 - 30cm)

22

28

29

31

30

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BH – 6

BH – 6

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YAMASHITA SEKKEI INC. TOKYO, JAPAN THE RECONSTRUCTION SUPPORT FOR THE GUJARAT - EARTHQUAKE **DISASTER IN THE DIVASTATED AREAS IN INDIA** RAFFRAL HOSPITAL, ANJAR

TABLE NO. - 2

Results of laboratory permeability

Sr. No.	Bore Hole No.	Depth (m)	Laboratory permeability (mm / s)
1	BH – 1	1.00	0.65x10 ⁻³
2	BH – 2	1.00	0.68 x 10 ⁻³
3	BH 3	0.50	0.56 x 10 ⁻³
4	BH 4	0.50	0.59 x 10 ⁻³
5	BH – 5	0.50	0.61 x 10 ⁻³
6	BH – 6	0.50	0.68 x 10 ⁻³

(Dr. K.C.Thaker)

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Porosity 29.3 28.4 31.4 ī • ı, 1 . . . * 0.40 0.46 0.42 • Volds Ratio , , go . . , . . . • . . . × 88 Value 80 (oN) . 2 58 • 88 . 8 23 • z Comp kepul • . . • . . , . , ÷ ÷ ទ័ Kglem² 47.00 UCS i. . . ı. , . . . 1 , Degree . • , న Shear Parameter . • 4 . . . 9 + Kg/cm² . . ł, . . ï 4 . . , o • W.ROCK **RESULTS OF LABORATORY TEST** fication Class Soll SC , ï ÷ . • . . . r ŝ * . Kg/cm² 1 . . • ī ī . ı. ÷ • , 5 , . ï • r . ł . . i. . * ដ Consistancy limits Å Ł £ e. đ ŝ å å å c) å ö. * đ đ ₽ å đ å đ å Å 20 å ď ۲ đ å đ £ đ £ å £ £ £ 23 Ľ ~ , • • • • ٠ • • . 1 . KCT Cousultancy Services, Ahmedabad e ۰ ο Grain Size Analysis 1 . • 5 • . ≳ 3 , , • . • . . , Project :- Raffrai Hospital at Mundra Road, Anjar \$. , ŝ * ι • r ı. 0 ï G ~ (, 2.78 2.76 2.74 . , . ī . . ġ ę o 1.13 1.18 1.17 1.16 1.25 1.21 1.40 1.33 1.27 1.27 9.26 Co ordinates :- N949.06 E997.84 NMC > ε gm / cc 1.95 1.99 Density 1,88 . r Field r ¢ Reduced Level :- 100.24 . ł • . BH No. -- 3 For KCT Consultancy Services, SPT Sau Sample ŝ Sau SPT SPT SPT SPT SPT Type SPT ß ъ 10.00 8.00 9.<u>0</u> 3.00 4.0 5.00 8.00 7.00 2.00 Depth 0.0 8.1 E 7 <u>ç</u> 9 60 8 5 ഗ ო 4 2 ŝ š

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Table No. - じ (ろ)

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Porosity 31.9 25.9 28.4 23.7 . . ٠ . ¥ • Voids 0.31 Ratio 0.47 0.40 0.35 , . • • , Rab . × ÷ . . . Value (°N) 2 8 ន ß 67 z . ន . E . • Сощр xepuj ဗီ . , . . , . , . . . Kg/cm² 39.00 42.00 ncs ī . ï ı . 1 , . Deprae Shear Parameter g 8 . • . , . . 4 ٠ Kg/cm² ï o , ı. 1 . , **RESULTS OF LABORATORY TEST** W.ROCK fication Clarssi Soll S . . ŝ × 1 4 Kg/cm² . ŝ . . . ı. e. . ÷ . ı × . . . S , . . Consistancy limits × å đ å ₽ ₽ å £ å å å ۵. 6 å ď å đ å å d, £ 뤞 đ 19 ፈ -₽ đ đ £ ď đ ÷ £ £ ŝ Ч 2 28 KCT Cousultancy Services, Ahmedabad ı. • o * ო • • . , • . • ı. Grain Size Analysis 5 • . Σ * • . ī . 1 . ı. . Project :- Raffral Hospital at Mundra Road, Anjar 84 , * • • , s , ٠ ÷ , , ٠ . ı. e. ı • . ī r ø * 0 . ı. ; ; 2.75 2.78 2.79 2.73 å õ • . σ . . Co ordinates :- N940.39 E966.79 1.08 NMC 1.23 1.19 1.14 1.08 1.09 8.28 1.37 1.32 1.27 1.21 ۶. ε gm/cc Density 1.86 2.06 2.13 Field 1.97 Reduced Level :- 100.30 ı. ŧ. ÷ . ٠ . . BH No. -- 4 For KCT Consultancy Services, Sample Type San San Sau 11 10.00 UDS SPT SPT SPT SPT SPT SPT ដ ъ Depth 8.00 0.0 <u>6</u> 2.00 3.00 4.00 5.00 6.0 7.00 9.00 Ę <u>₽</u> -8 š 3 ო 4 ø 80 ŝ ß ~



Table No. - 6 (16)

K.K. Thate to (Dr. K.C. Thaker)

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Porosity 30.0 28.4 23.7 . i, ×. 4 . • ÷ Volda Ratio 0.43 0.0 0.31 . . , . , • . • RQD × . . . , . . ÷ . Value (oN) . 2 88 59 2 z , ន 85 8 . • xepuj Comp ő • ī , . , Kg/cm^{*} 44.08 ucs ī Degree Shear Parameter . 9 . ı, ŝ • . . . , ٠ Kg/cm² o . , 4 . ī . , . . **RESULTS OF LABORATORY TEST** W.ROCK W.ROCK W.ROCK W.ROCK W.ROCK W.ROCK W.ROCK W.ROCK fication W.ROCK W.ROCK Classi Soli S . 1 . ŝ × , • • . , t . . ī Kg/cm² • ł. • . . . ŝ . . ī . . ដ * ı. 1 4 1 4 . . Consistancy limits å å ď đ ₽ ŝ Å ŝ £ ā, . œ ż 45 å đ £ ₽ å å å đ đ ₽ ۲ × ٩ å đ å £ <u>d</u> ₽ å ₽ ŝ ۲ 23 * KCT Cousultancy Services, Ahmedabad • ٠, o * 2 r 4 • • • t ı . Grain Size Analysis 2 ı. , . • ٤ **3**¢ ı. . ī . 1 . Project :- Raffral Hospital at Mundra Road, Anjar 88 . . , ı. r > . , , . , S . ÷ . σ * 0 r 1 ī. . ÷ ŧ ı • 2.78 2.79 2.77 å ę. ø , ł . , , . 1 Co ordinates :- N941.89 E925.79 1.25 1.16 1.11 9.29 1.38 1.34 1.32 1.28 1.21 1.21 1.20 NMC * ε gm/cc Density Reduced Level :- 100.44 Field 1.94 1.89 2.13 r . ŧ ÷ . . 4 BH No. :- 5 Sample Type ŝ Sgo San SPT SPT SPT SPT SPT SPT SPT ъ മ 10.00 Depth 9.00 0,00 3.0 4.00 5.00 2.00 8 2.00 8.8 8 8.00 ε Ξ 9 Ŷ æ 9 ያ ~ 2 ო ß ø 4 ~



Table No. - 7 (17)

Kik. Theres For KCT Consultancy Services, (Dr. K.C. Thaker)

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KCT Cousultancy Services, Ahmedabad	Project :- Raffral Hospital at Mundra Road, Anjar Co ordinates :- N934.94 E886.35 BH No. :- 6 Bodurod Loval -: 60 77	- IAAAI '-	Type	đ	Sample	SC	SPT	san	SPT	SPT	Sau	SPT	SPT
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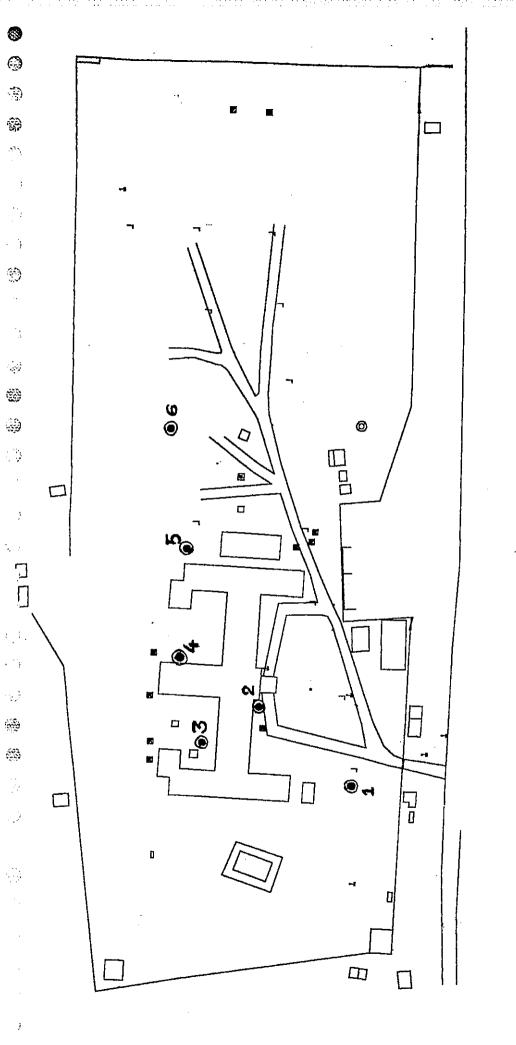
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Table No. - 8 (18)

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For KCT Consultancy Services,



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Locations of Bore Holes at Raffral Hospital, Anjar

K.H. Thules

Fig. N/0.1 (19)

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Remarks Nos. ო R.O.D. 10.00 T.9.2 lalqme2 7.00 2.00 4.00 £.00 8.00 0.00 1.00 Sample Disturbed SQ alqme2 9.00 3.00 6.00 DednutsibnU 20 + 26 + 35 22 + 30 + 33 25 + 31 + 34 11 + 22 + 30 12 + 22 + 33 14 + 25 + 33 15 + 26 + 33 SMOIO Nos. 10 .0N T.9.2 50 Field Test SPT 06 8 65 80 S.P.T. "N" Value 59 ß 58 70 હે 60 22 50 BORE LOG DATA SHEET 40 90 20 10 Soft Weathered Rock Clayey Sand (SC) Soil Description KCT CONSULTANCY SERVICES, AHMEDABAD Project : Raffral Hospital at Mundra Road, Anjar Notation 1.40 m 8.60 m rayer (m) Cordinates: N1004.5 E 1012.6 Thickness of 4.00 5.00 6.50 6.50 6.50 7.00 7.50 9.50 9.50 9.50 9.50 0.00 0.50 1.00 2.00 3.00 3.50 Reduce Level: 100.84 m Bore Hole No. : BH - 1 Termination Depth: 10.0 m Depth (m) 6 E mm) .siū ero8 Not Used **Baise**O Boring Rotary Drilling Method Nethod of

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> F1g.Nc-2 (20)

> > K. K. Thaker)

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(K.K.Thaker)

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FIG. No 5 (23)

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KCT CONSULTANCY SERVICES, AHMEDABAD

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oad, Anjar			Soil Description	. 10	Clayey Sand (SC)				· ·	Soft Weathered Rock					
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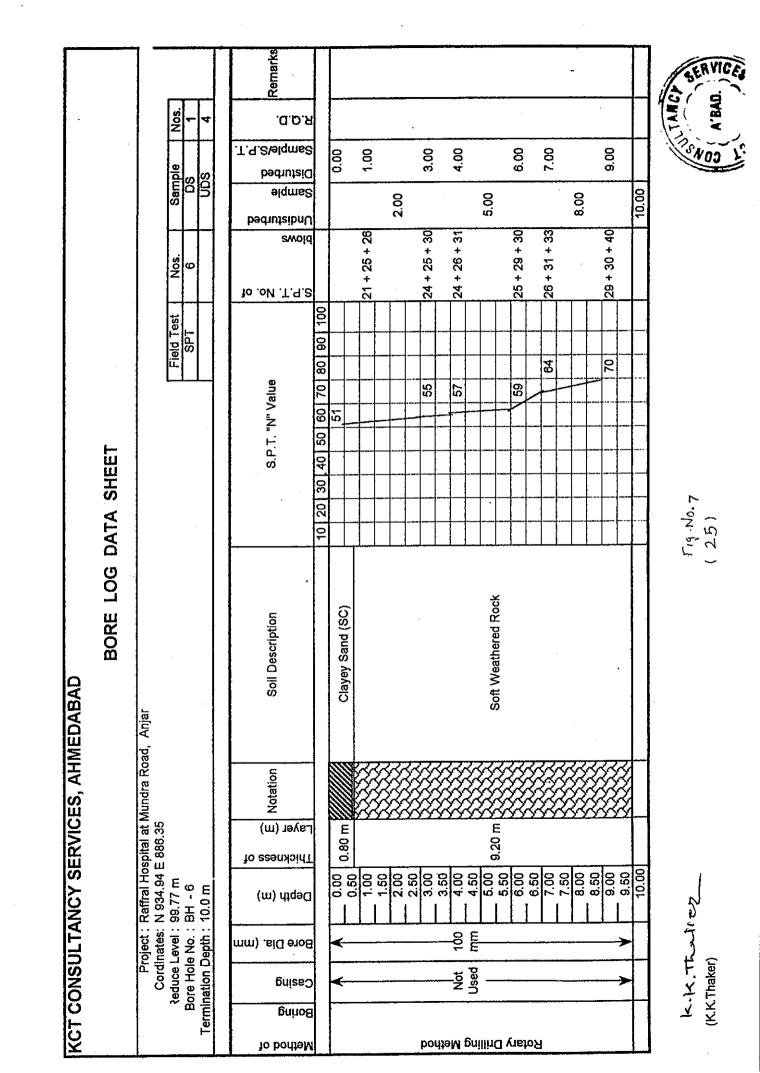
Fig No. 6 (24)

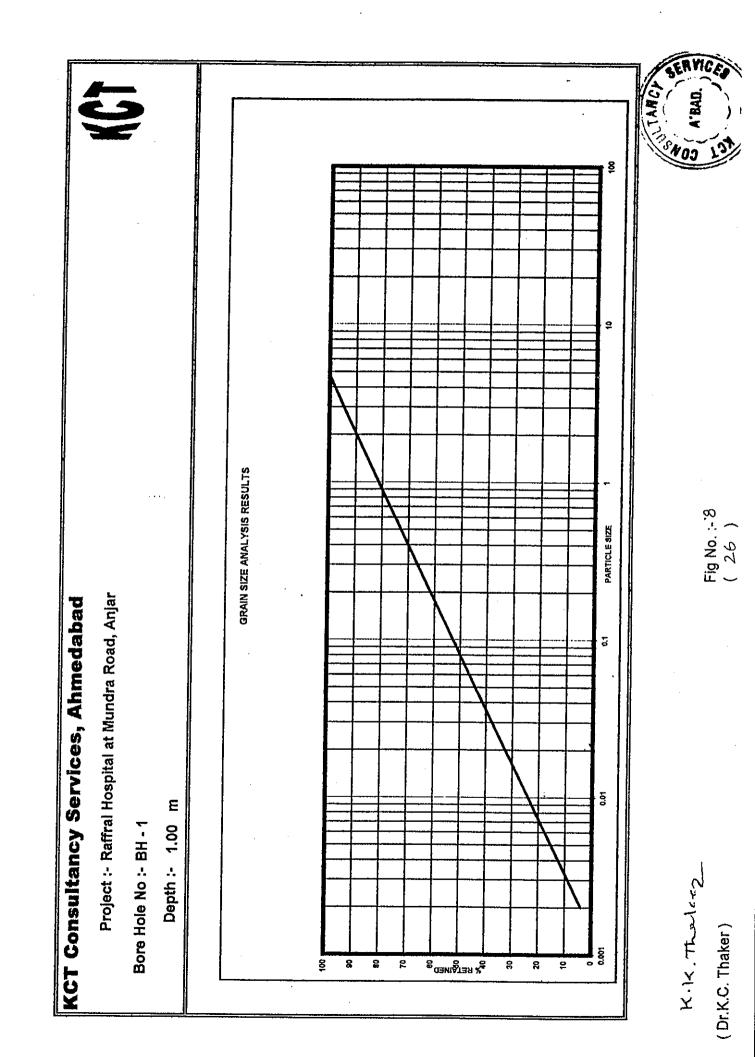
K.K.Thulie2 (K.K.Thaker)

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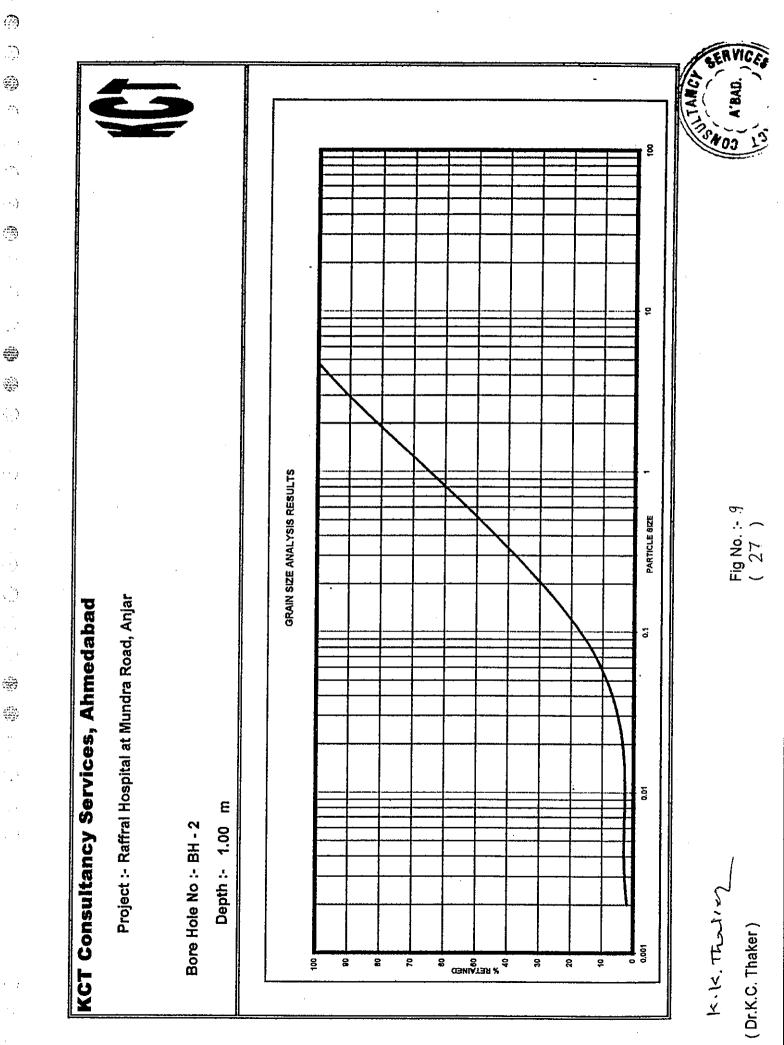
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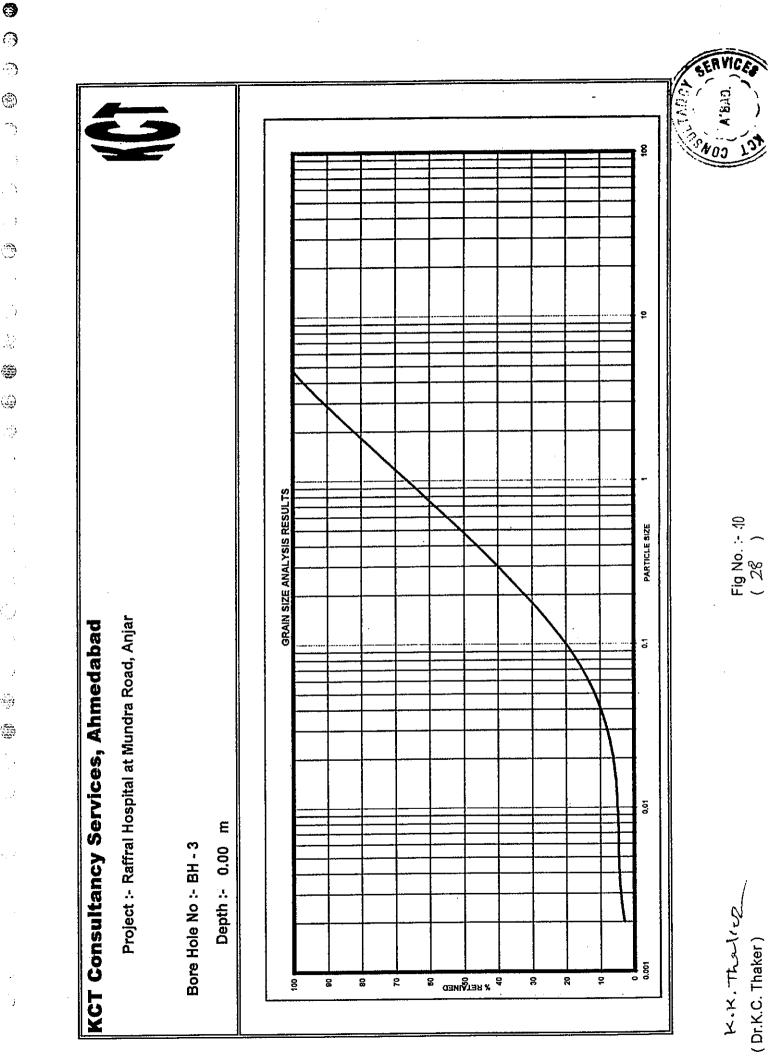
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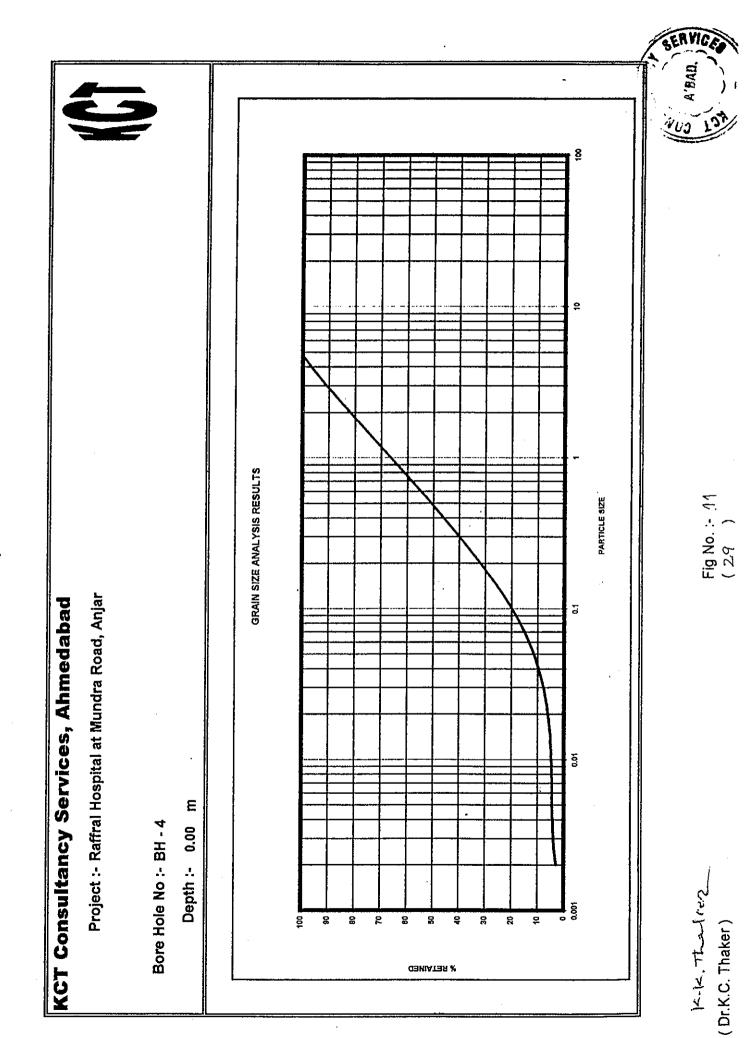


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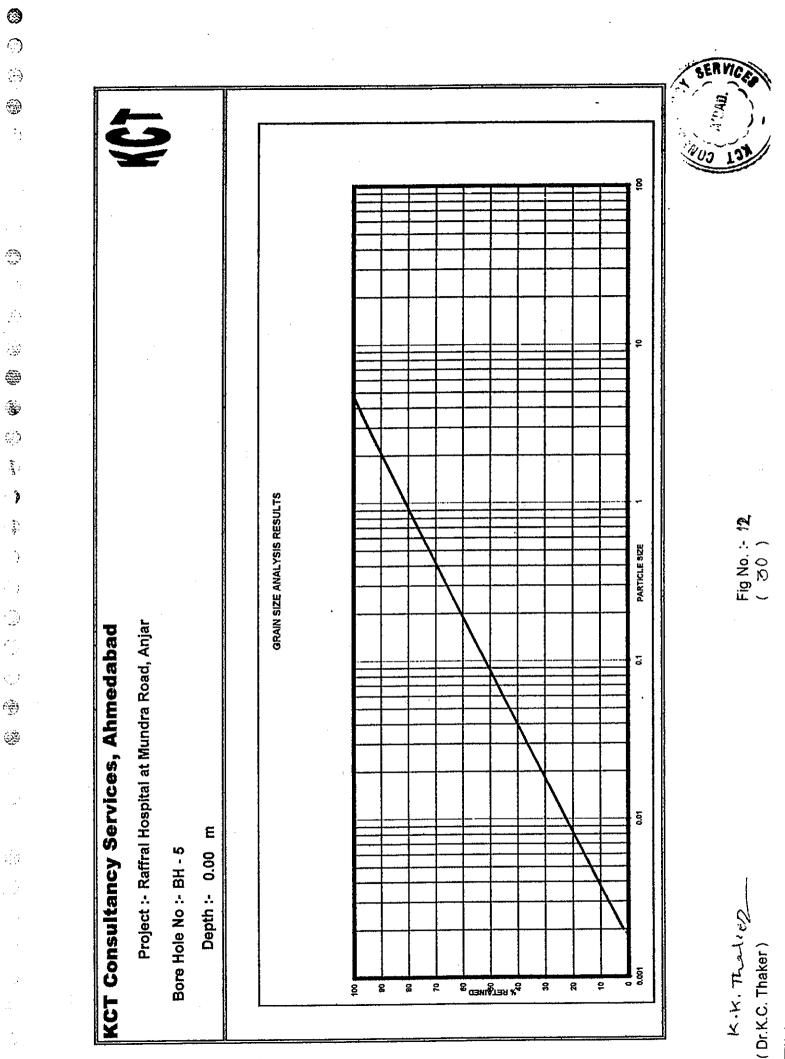
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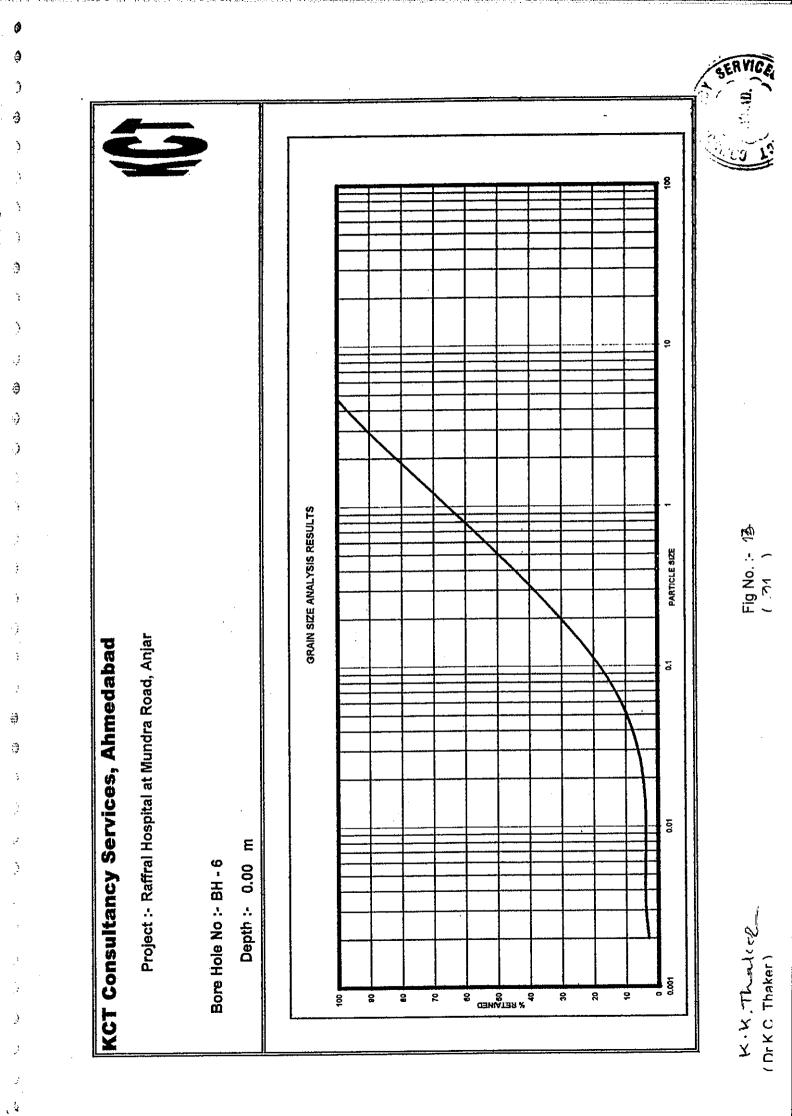
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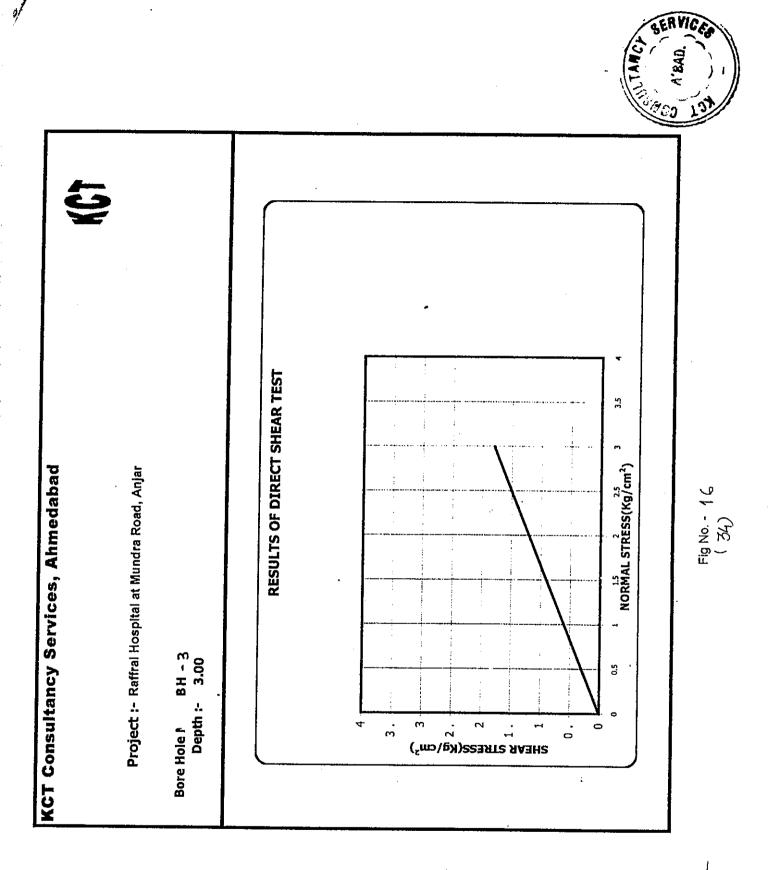
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KCT Consultancy Services, Ahmedabad Project :- Raffral Hospital at Mundra Road, Anjar Bore Hole 1 8H - 2. Depth :- 3.00	RESULTS OF DIRECT SHEAR TEST	Crux/gX/SZE3KT2 AA3H2 O O O <td< td=""><td>Fig No15 (33</td></td<>	Fig No15 (33

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KCT Consultancy Services, Ahmedabad	RESULTS OF DIRECT SHEAR TEST	(^{fund}) ^{gala}	Fig No 18 (36)

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Project :- Raffral Hospital at Mundra Road, Anjar Bore Hole 1 BH - 6 Depth :- 5.00	RESULTS OF DIRECT SHEAR TEST	SHEAR STRESS(Kg/cm ²)

K.K. Thaker)



Fig No. - 19 (37) . .

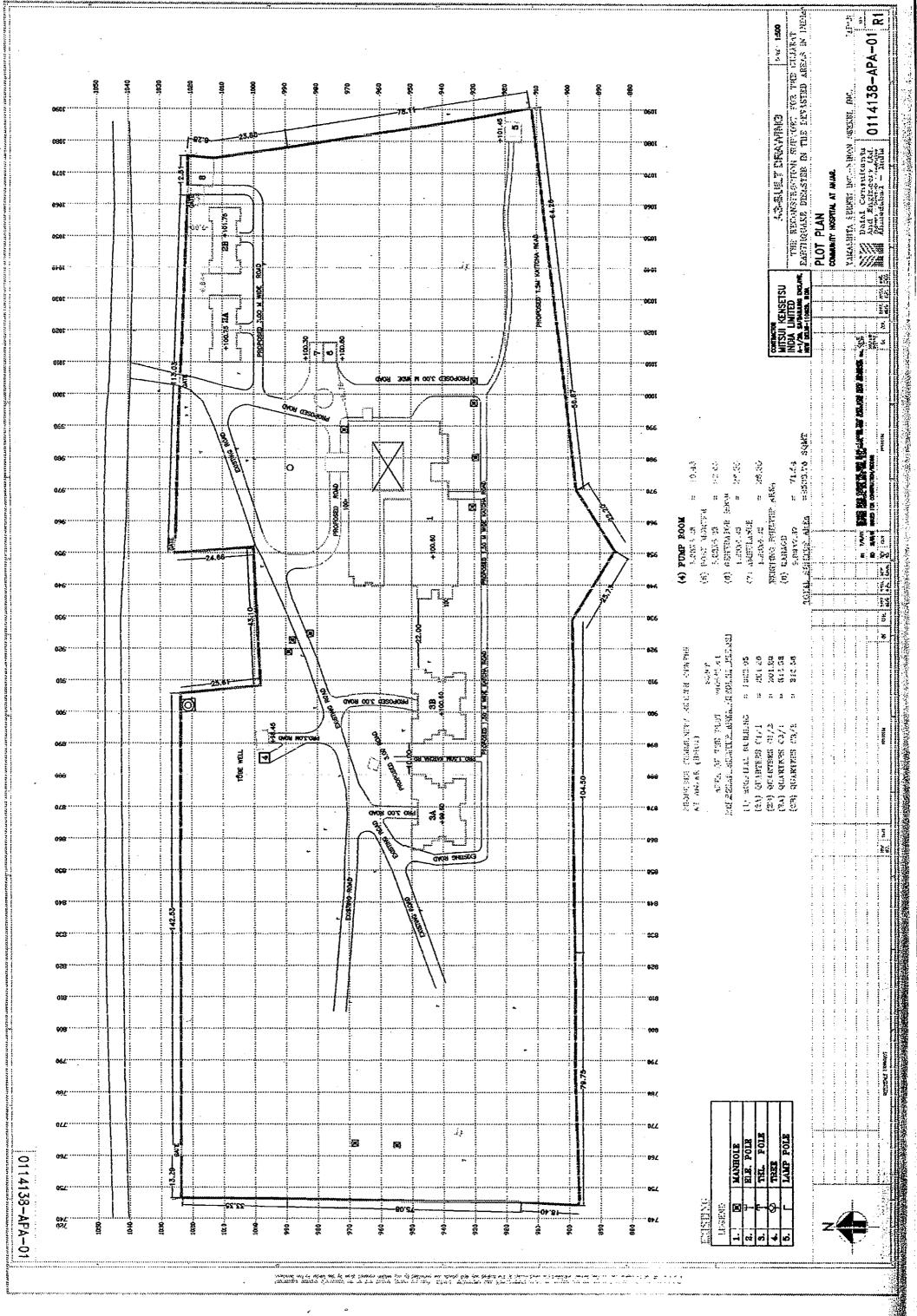
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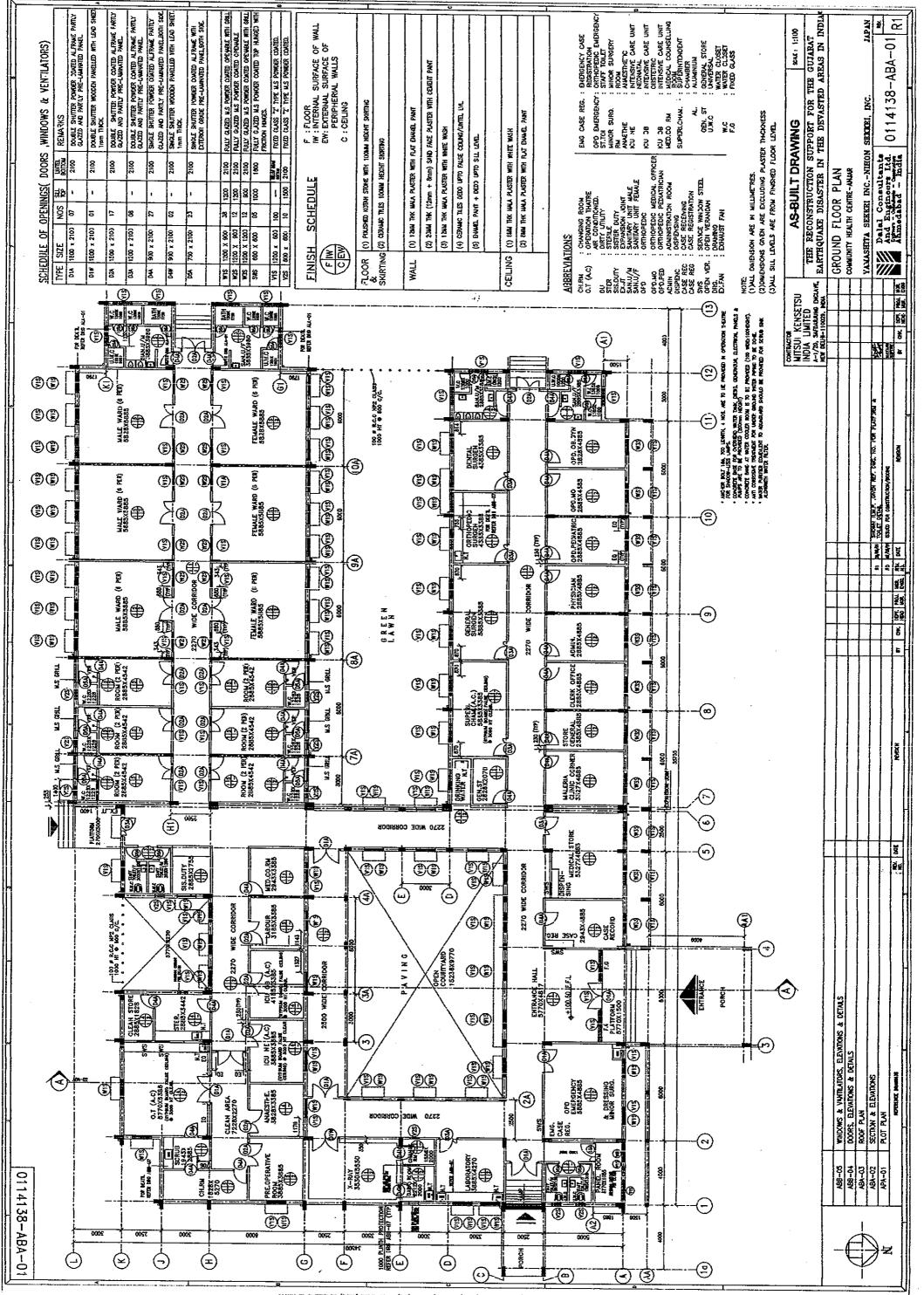
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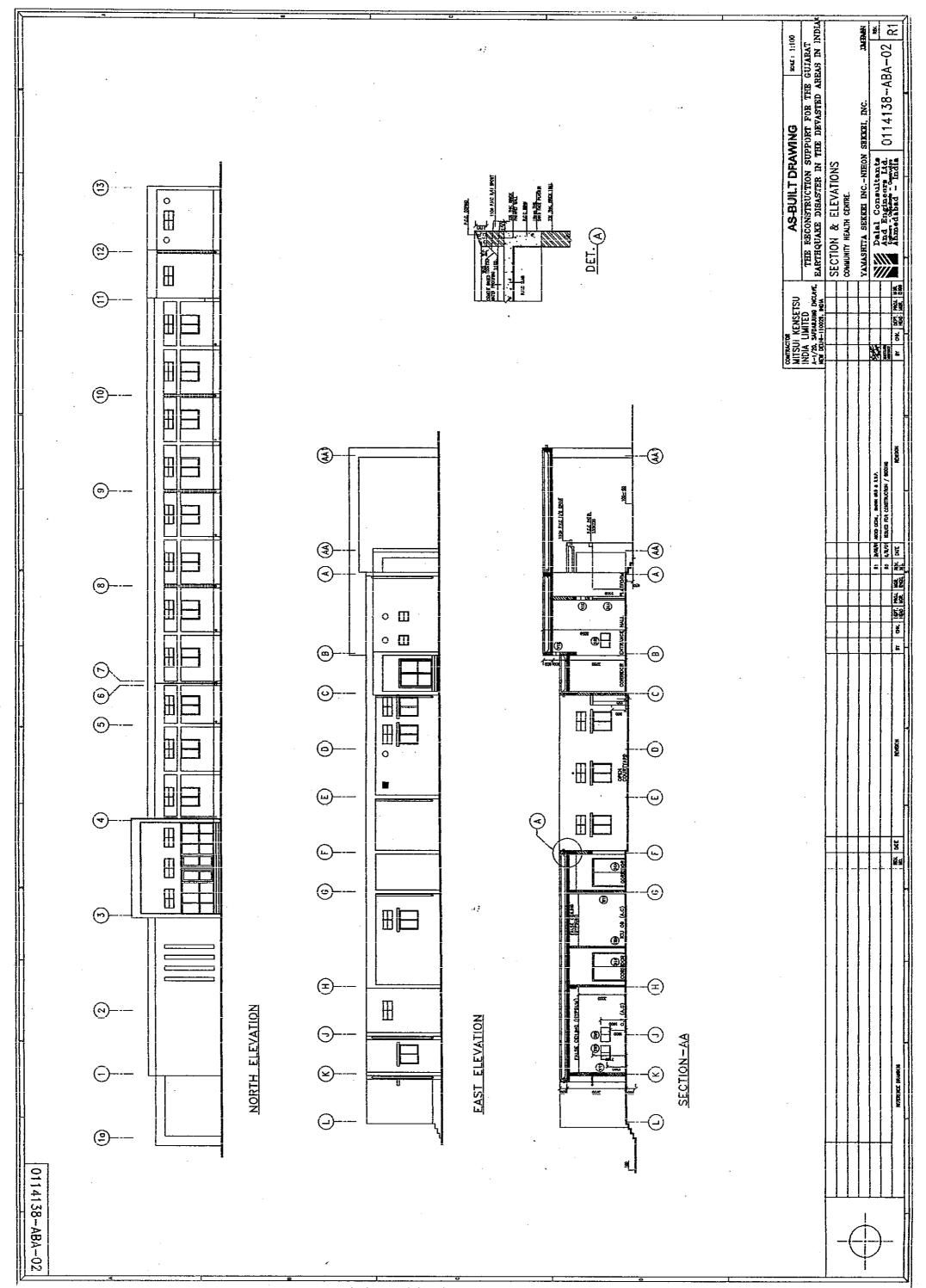
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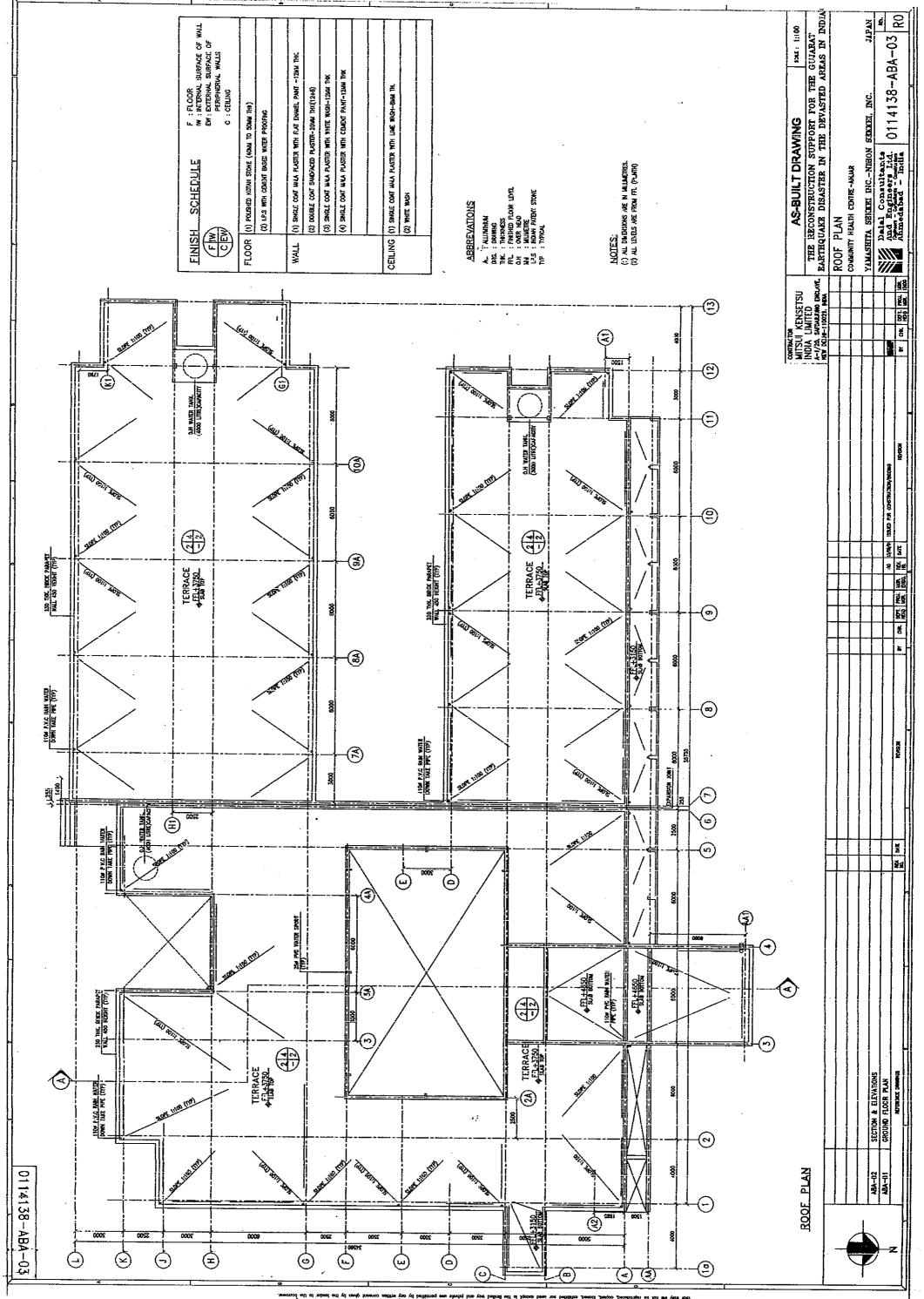


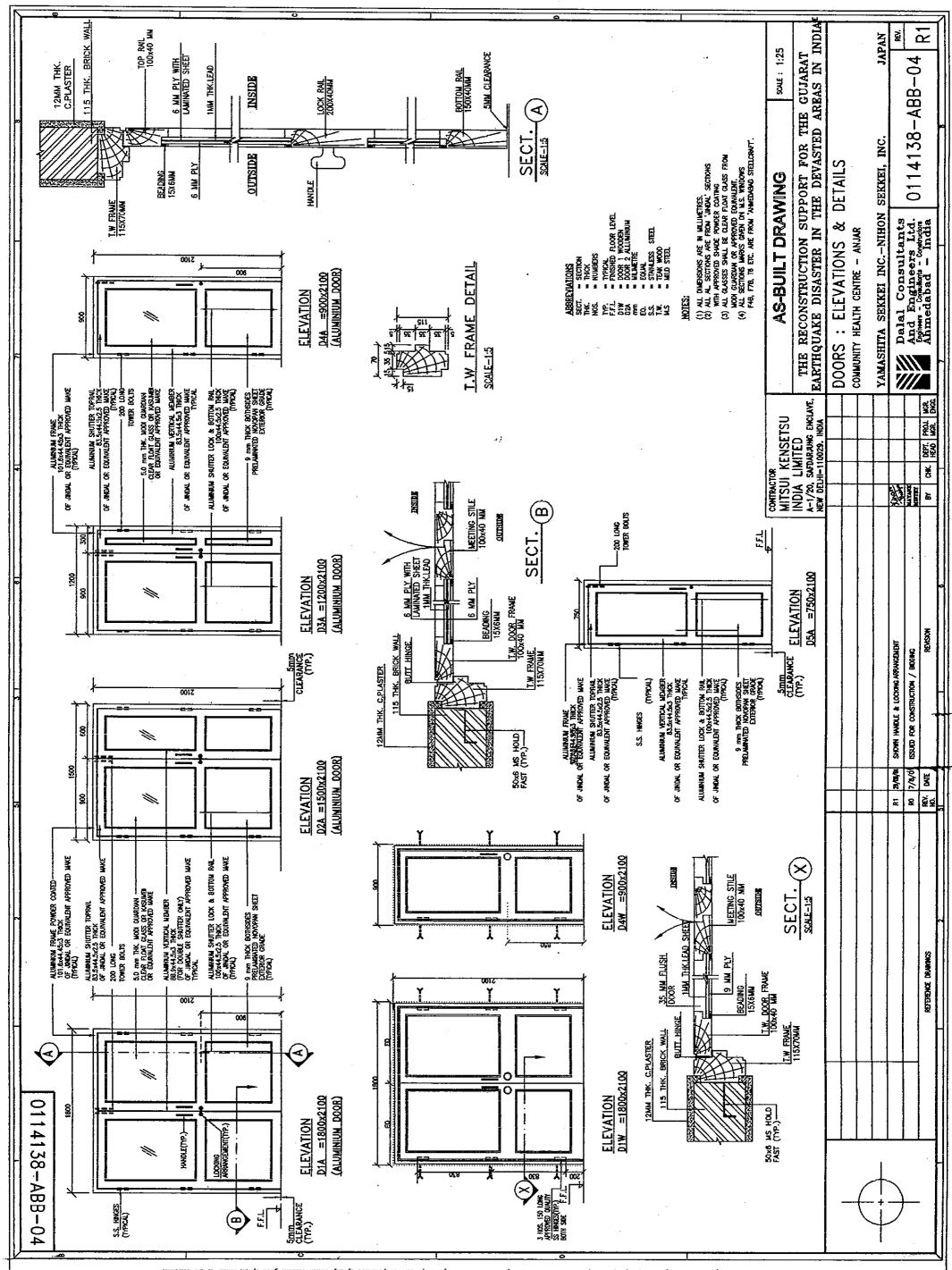


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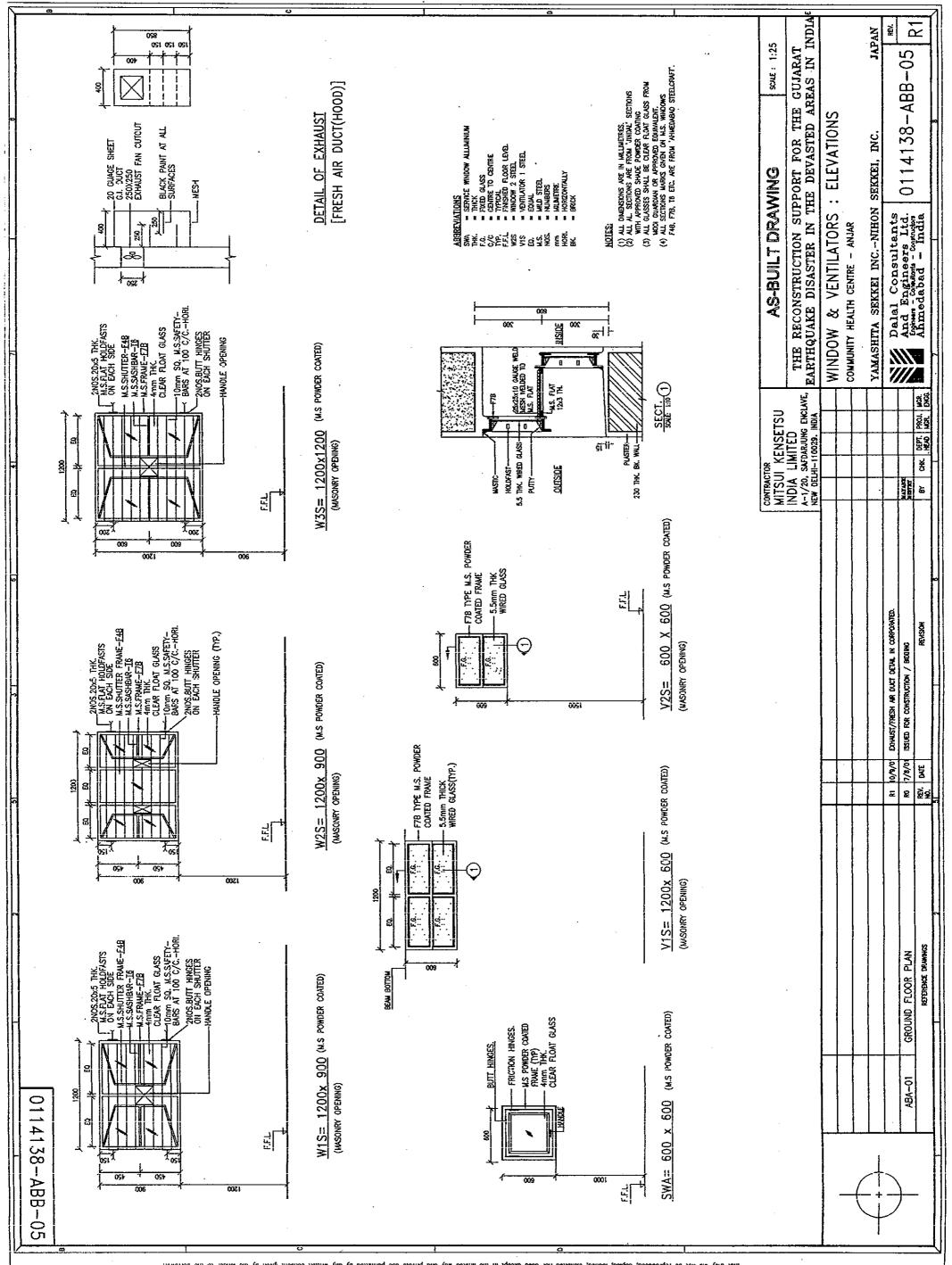


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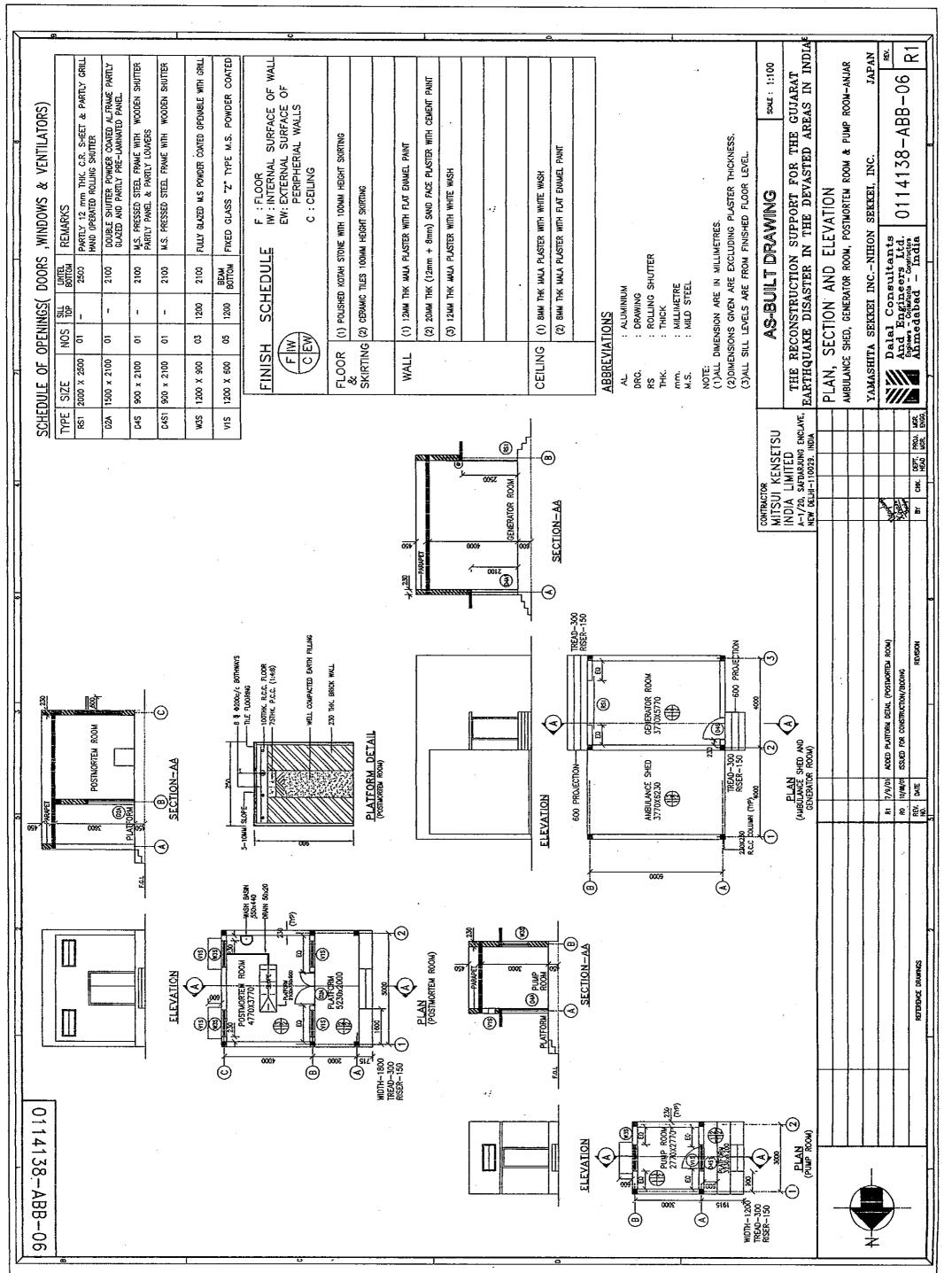


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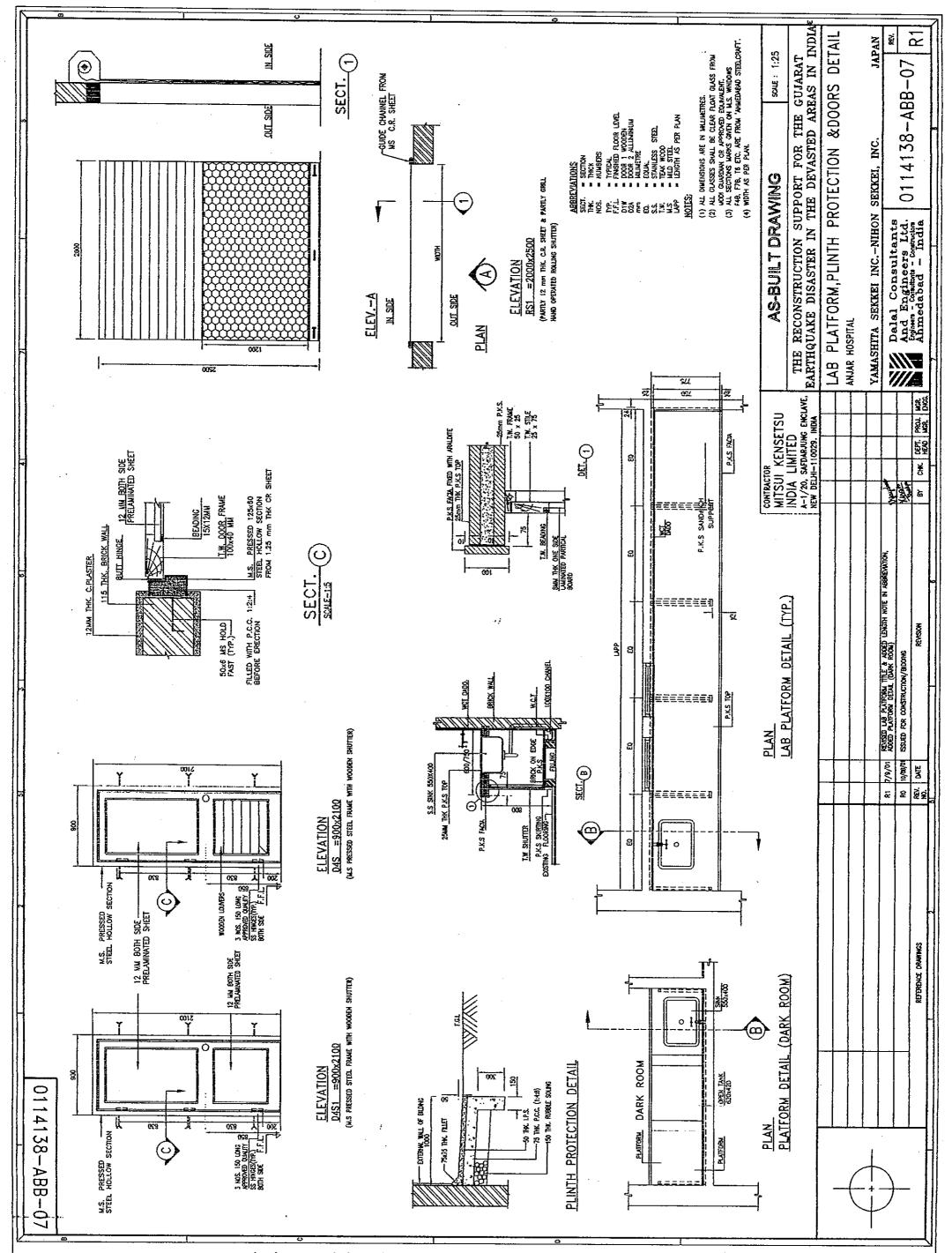


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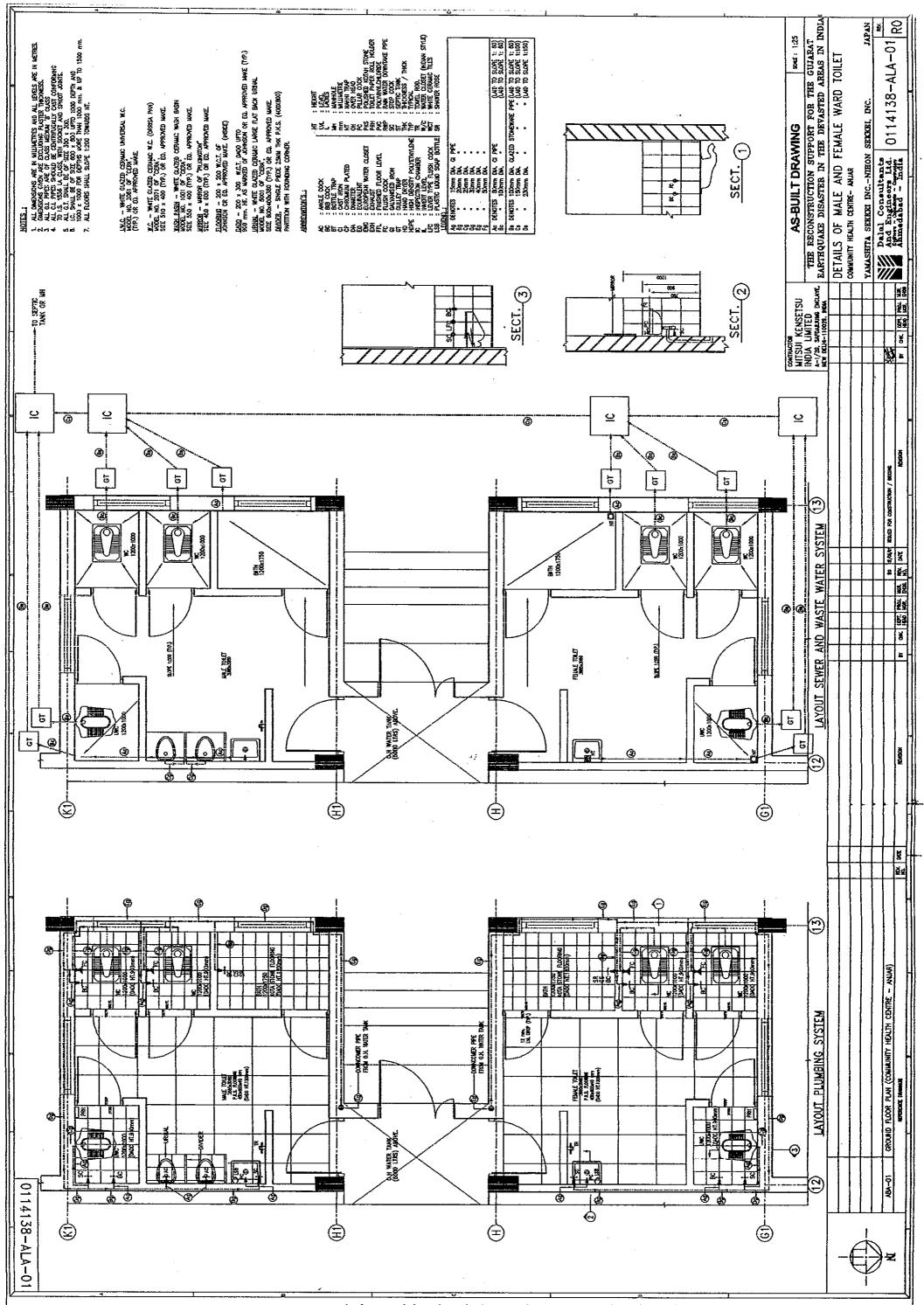
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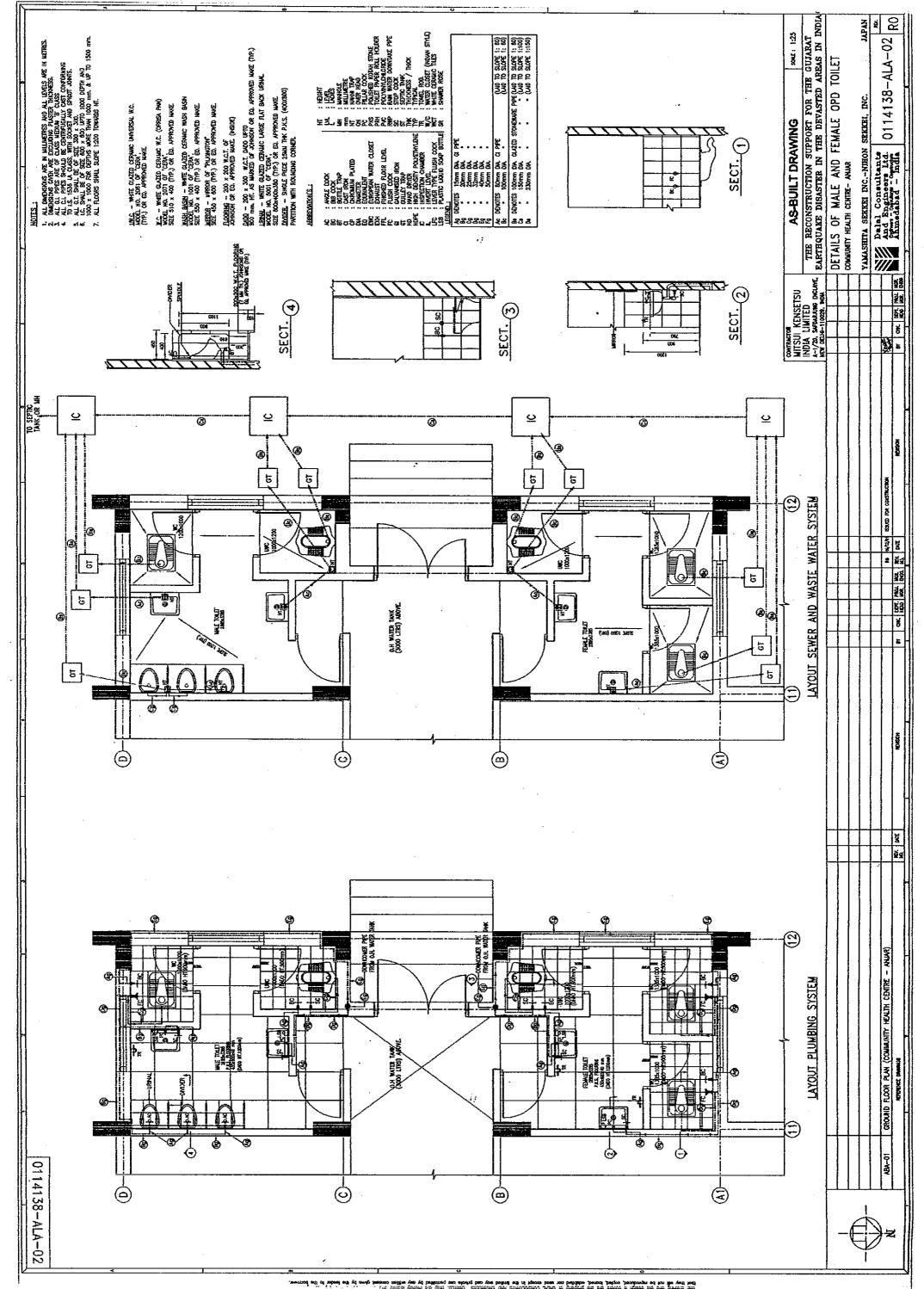
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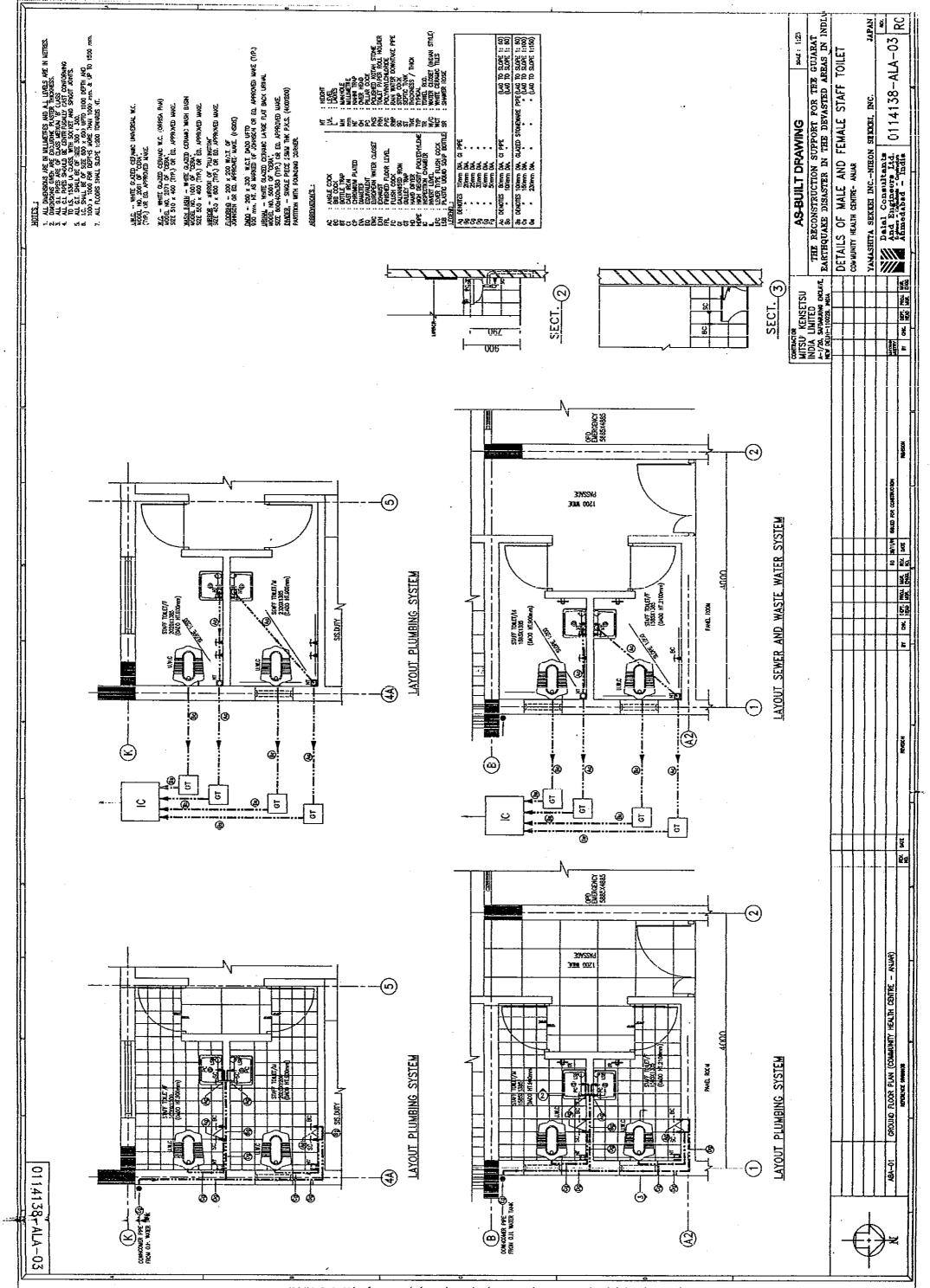


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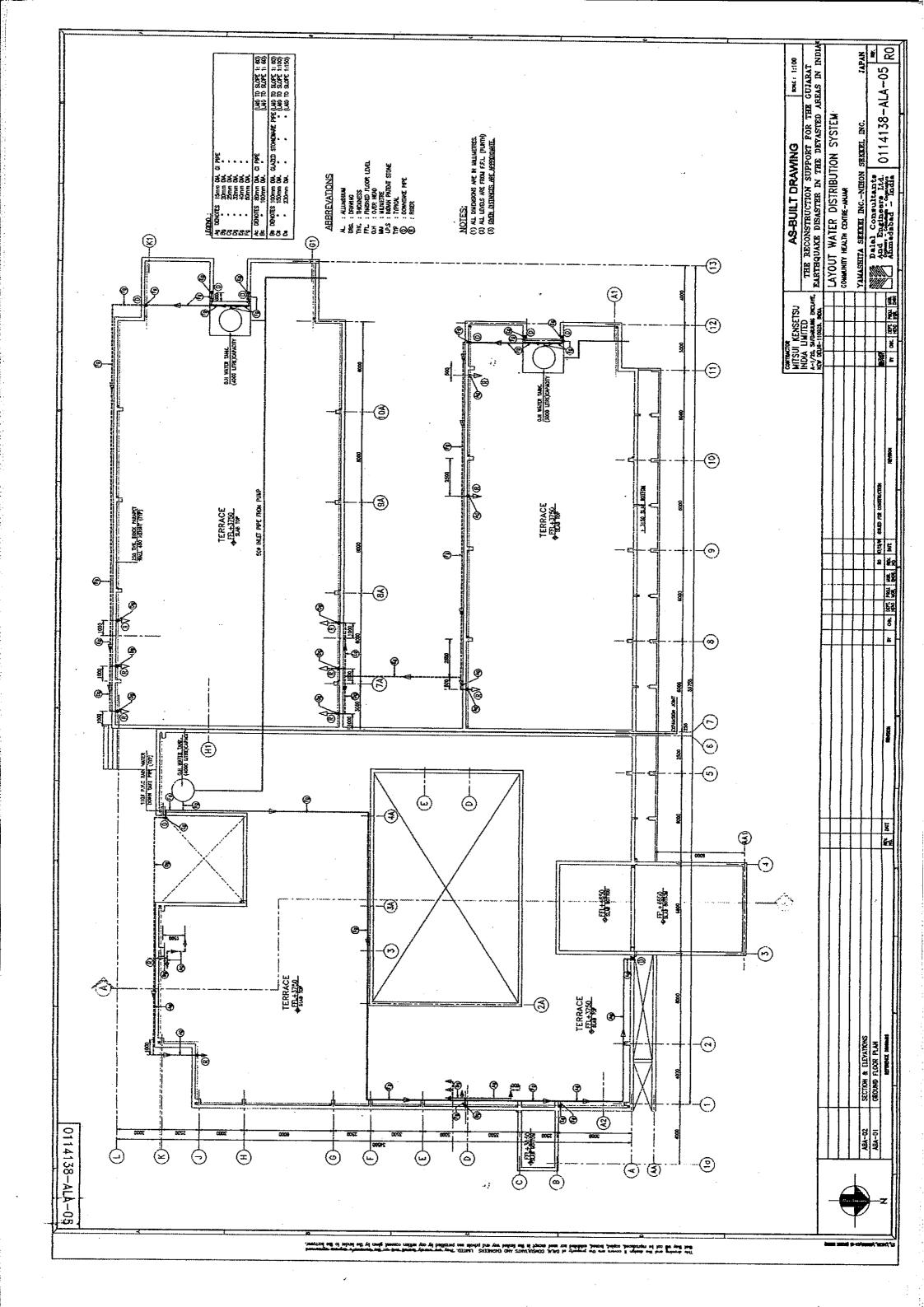


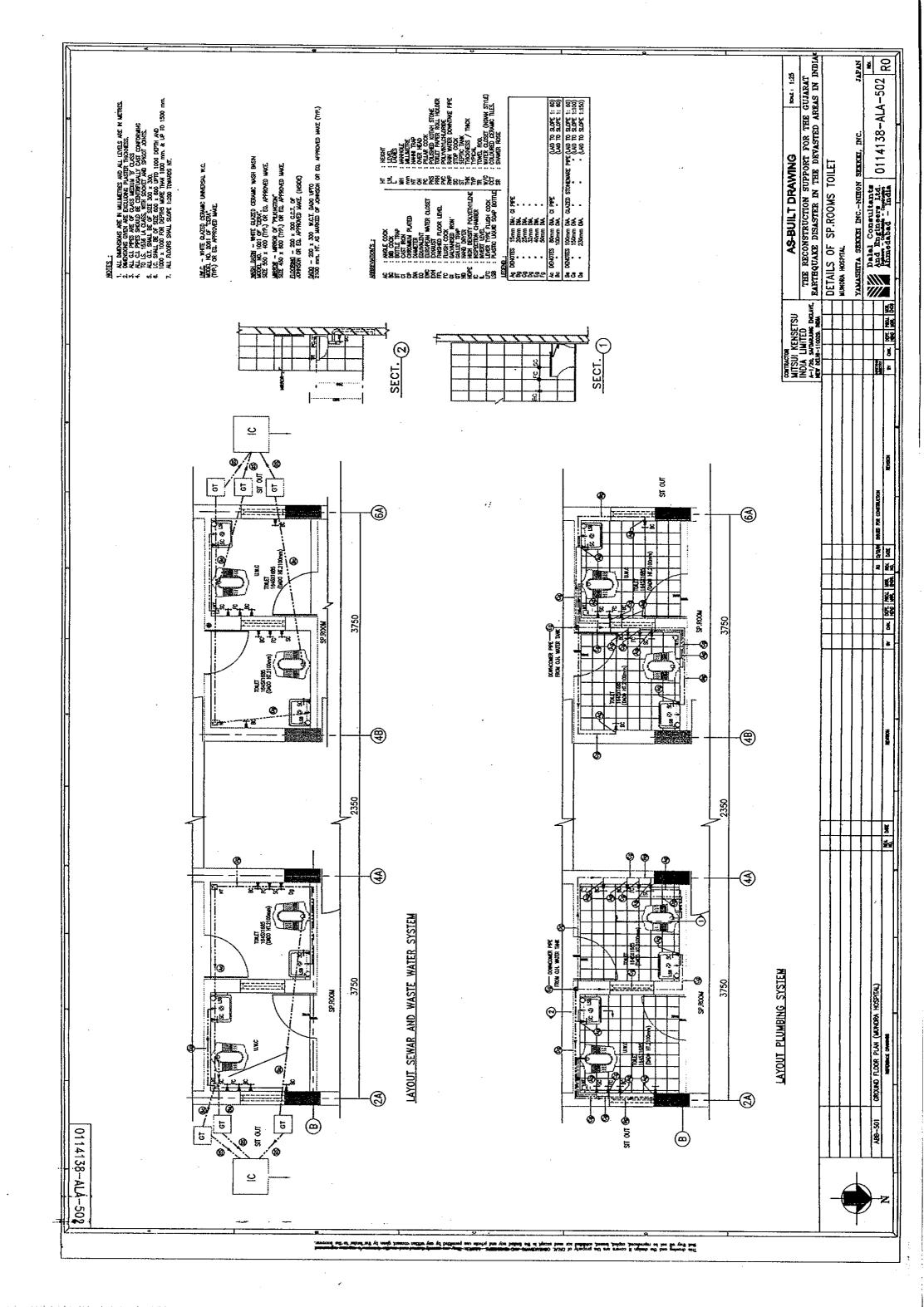
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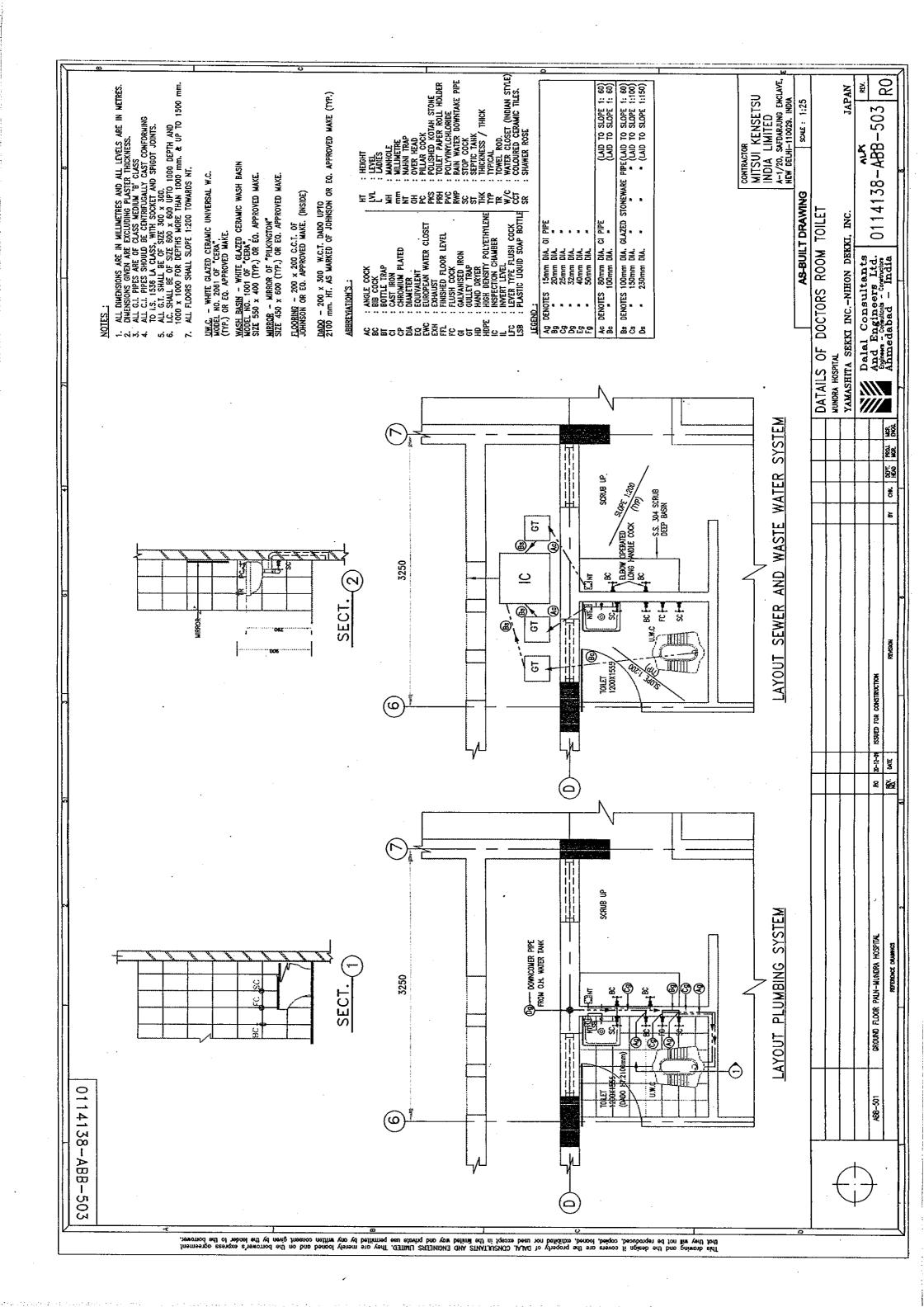


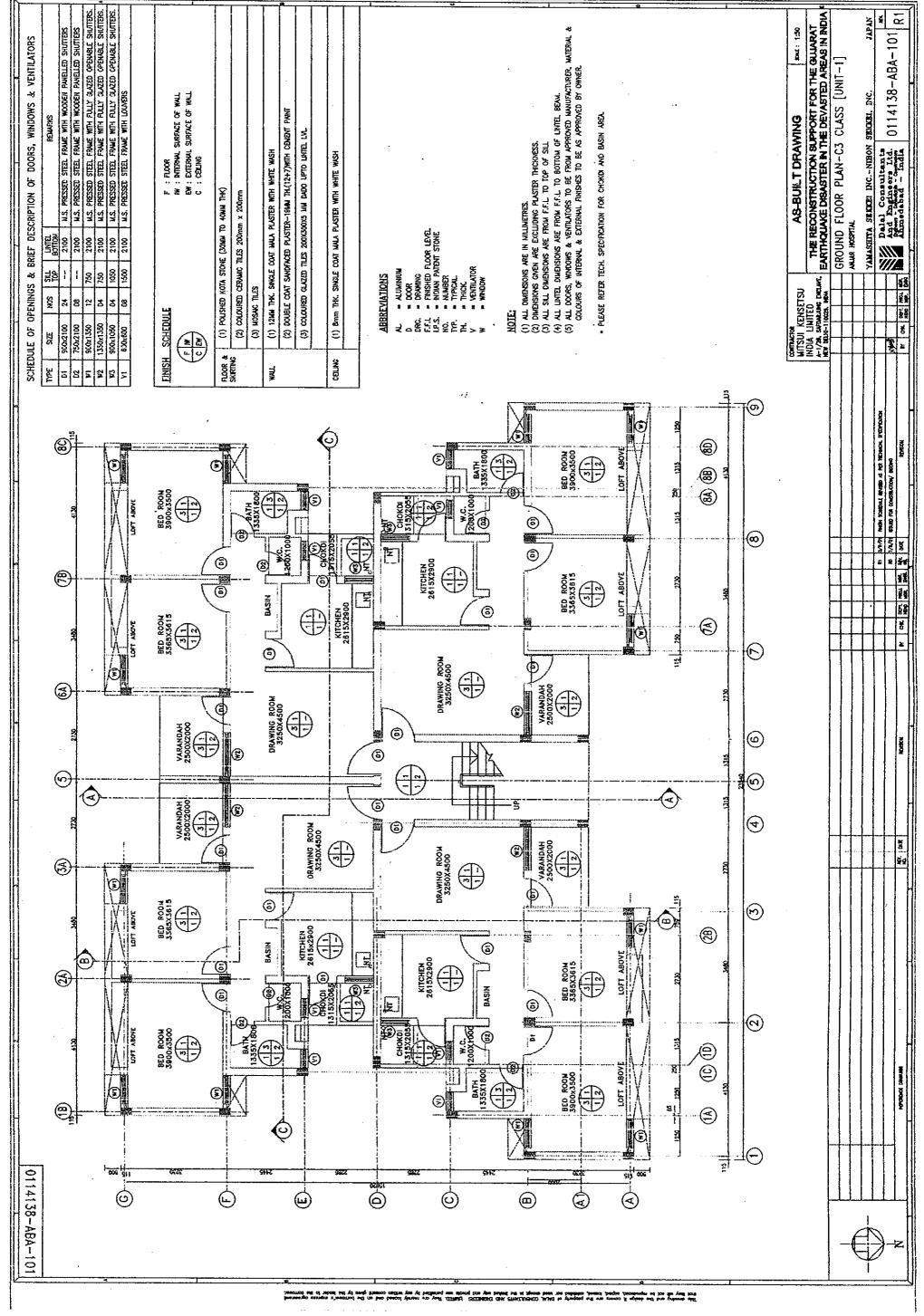


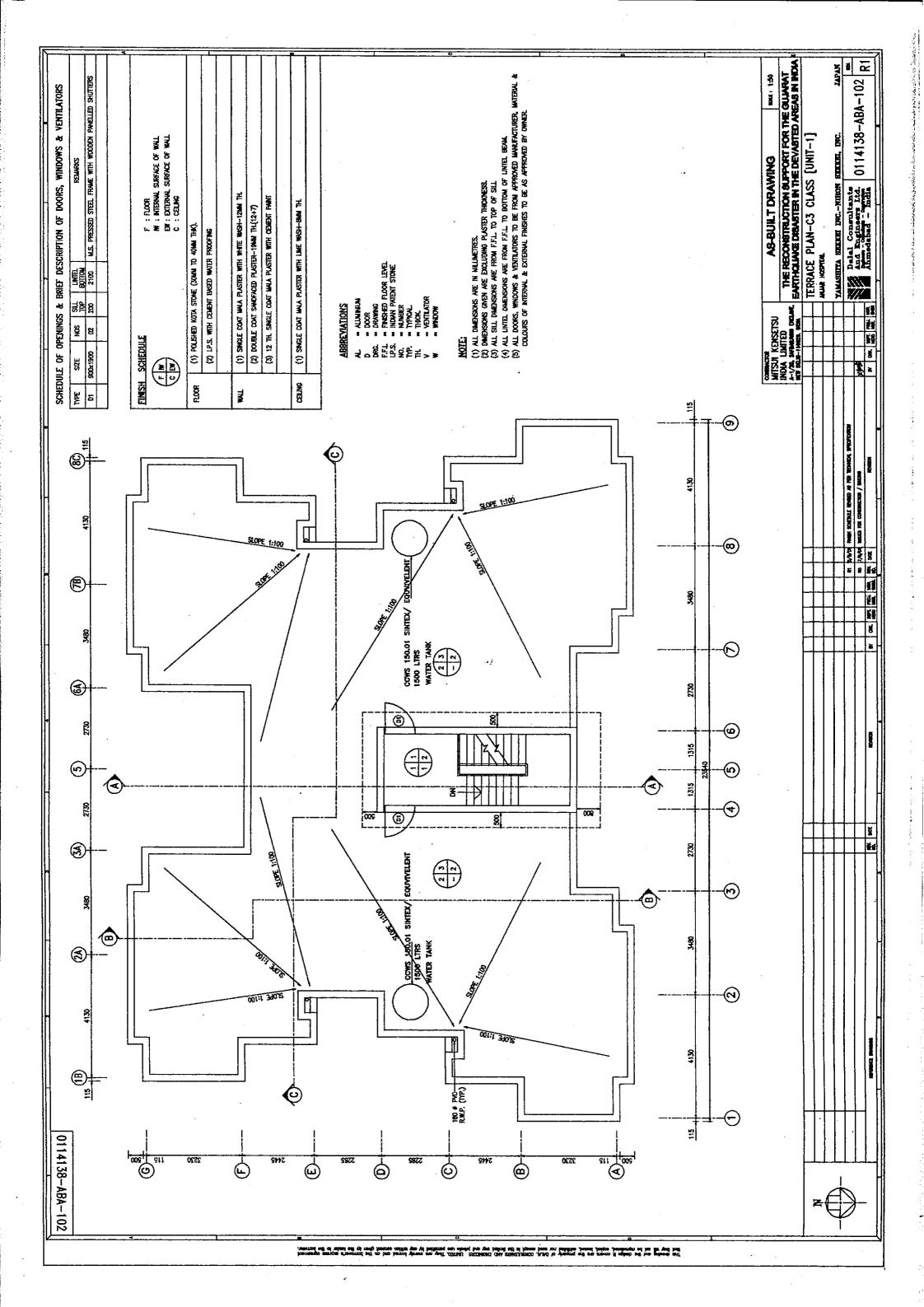
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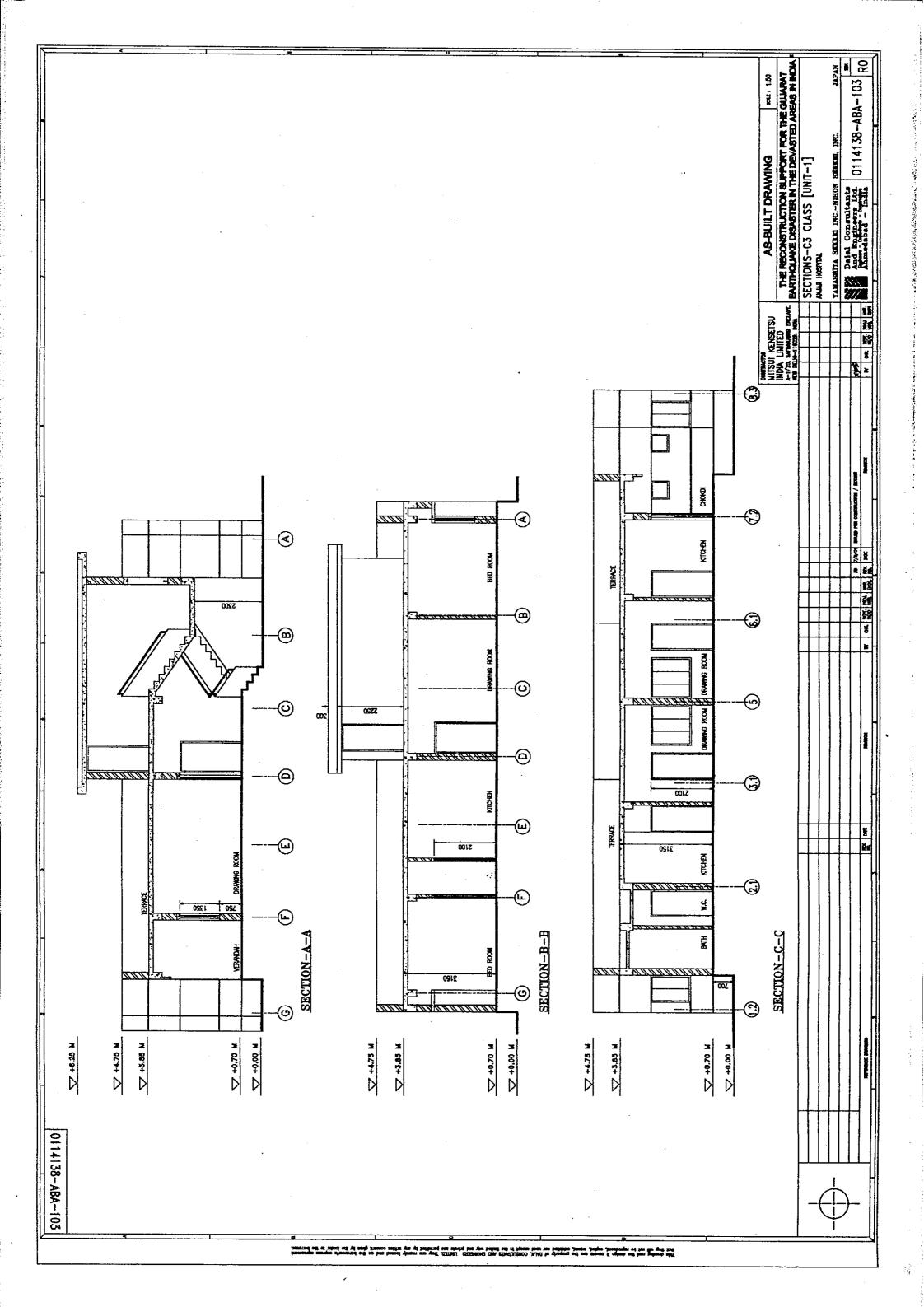




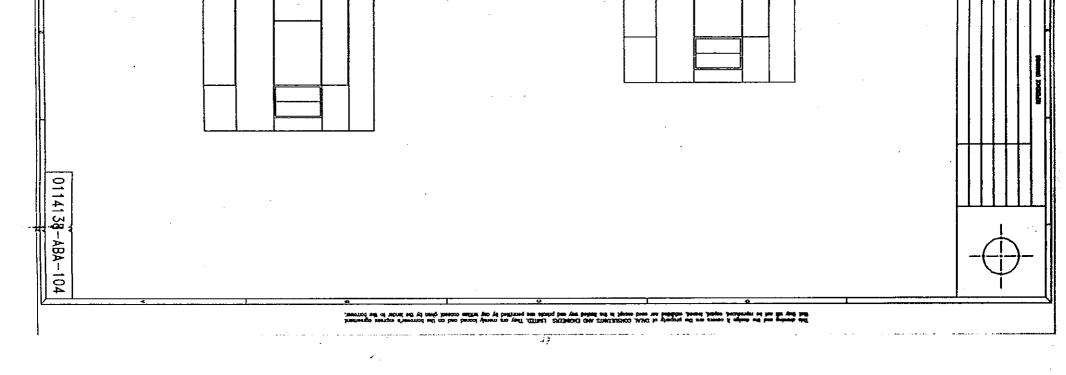


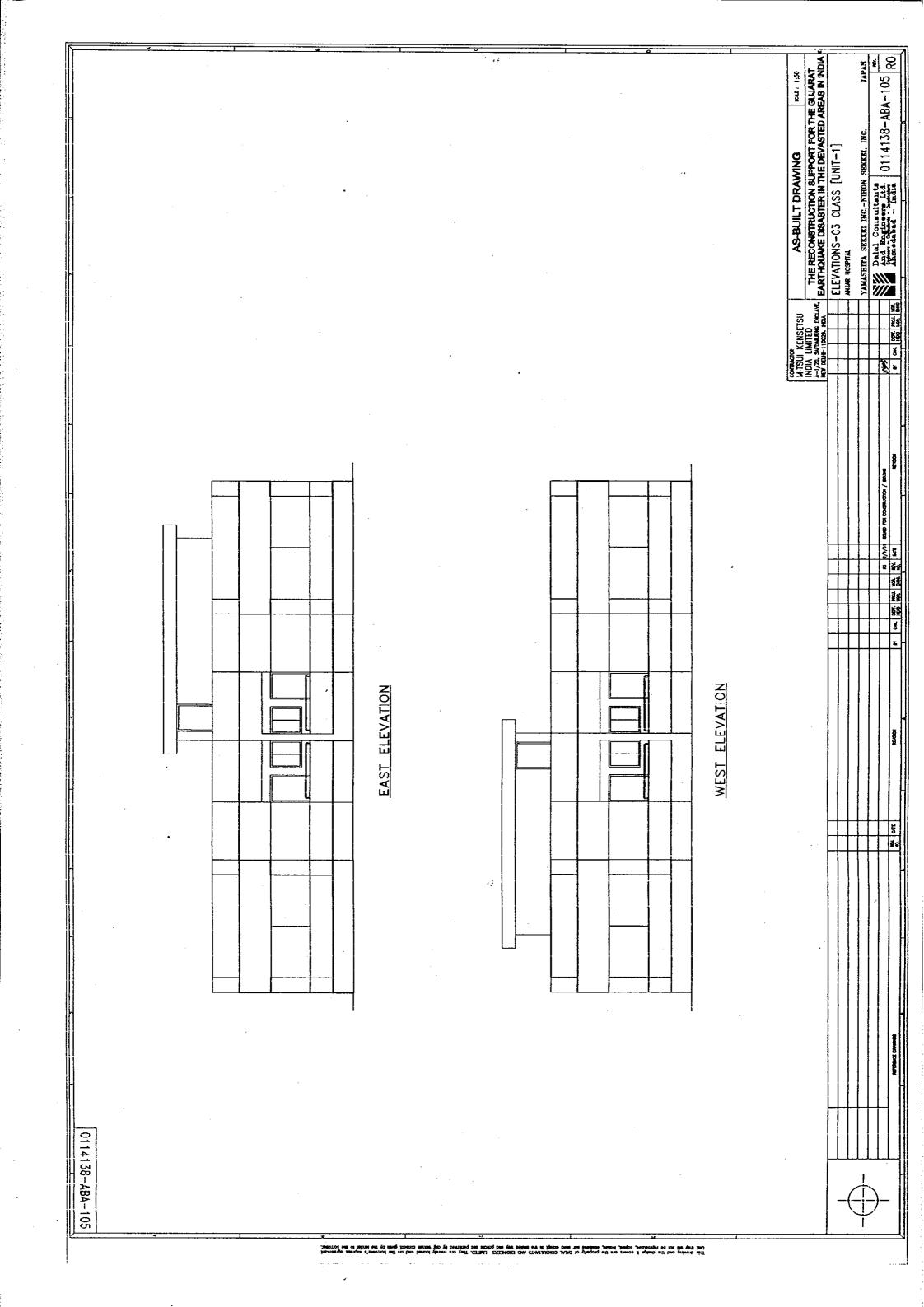


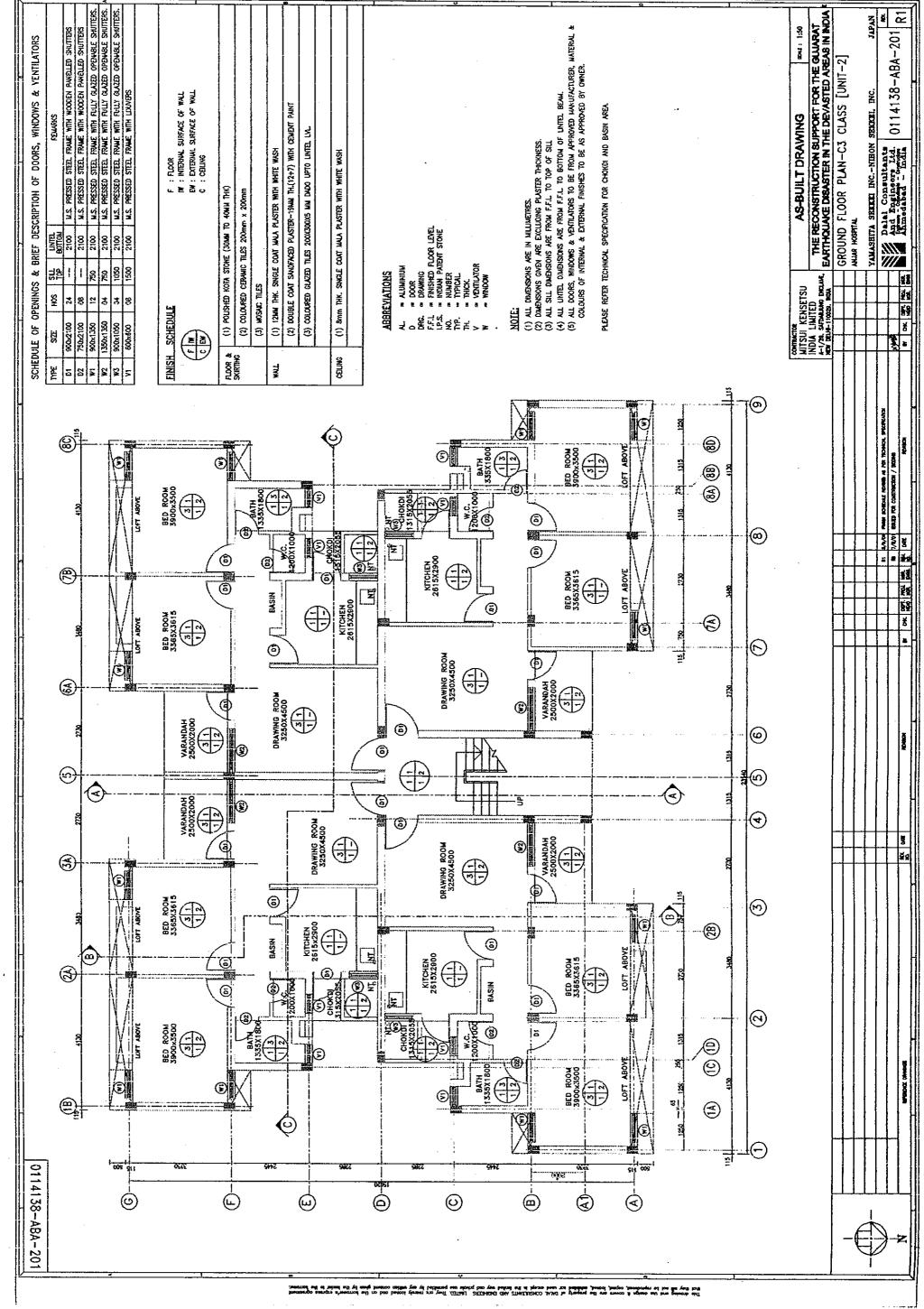


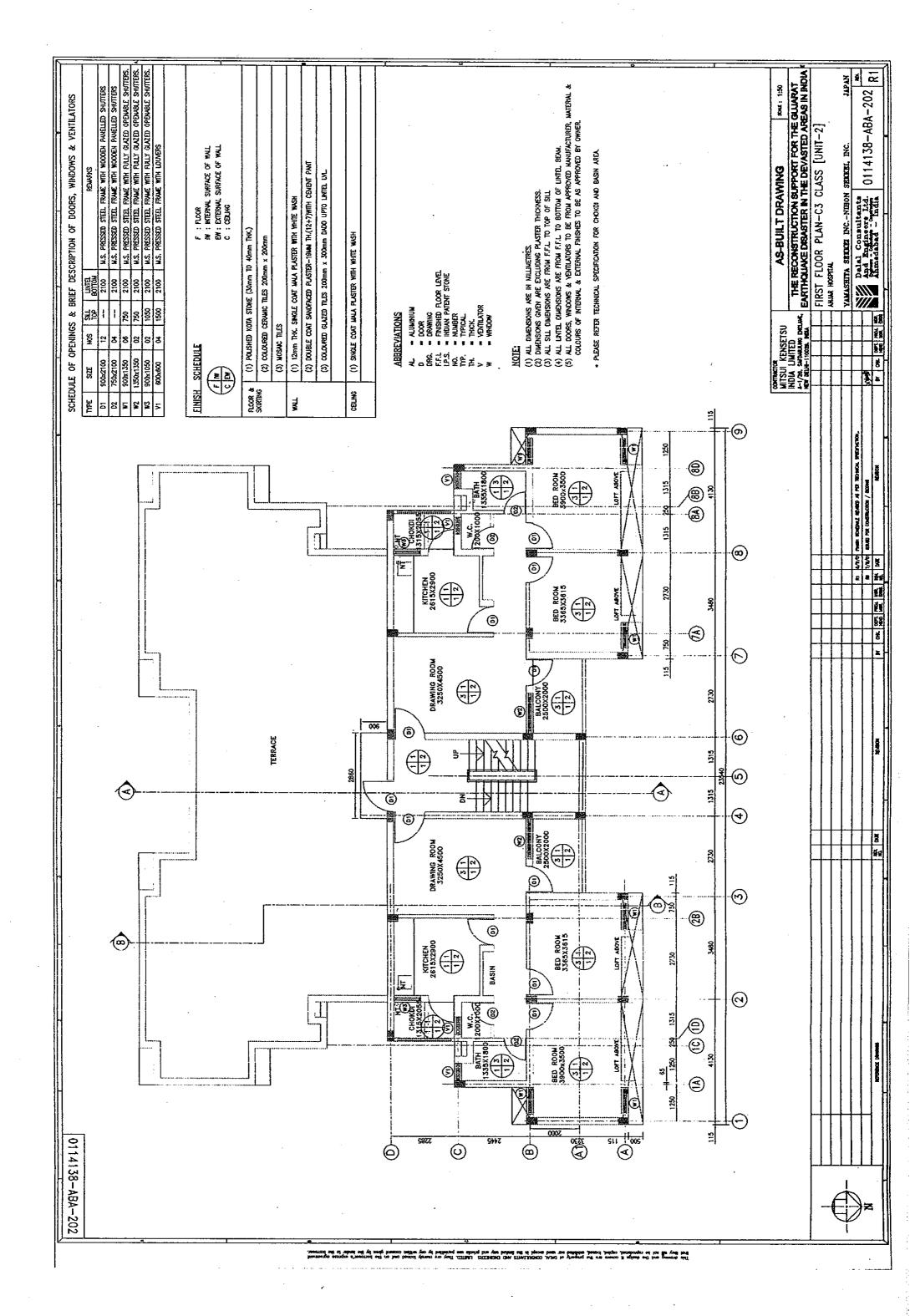


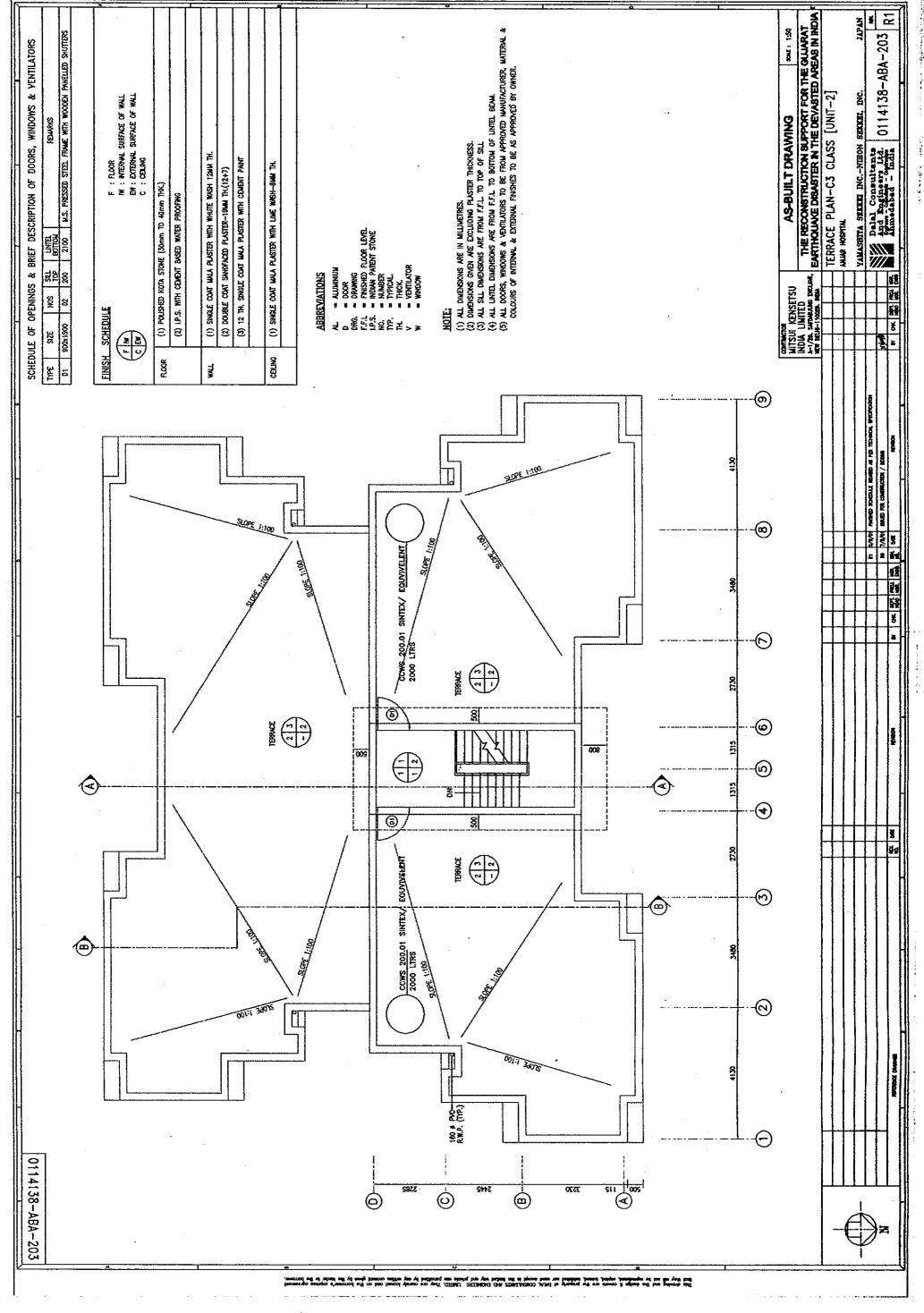
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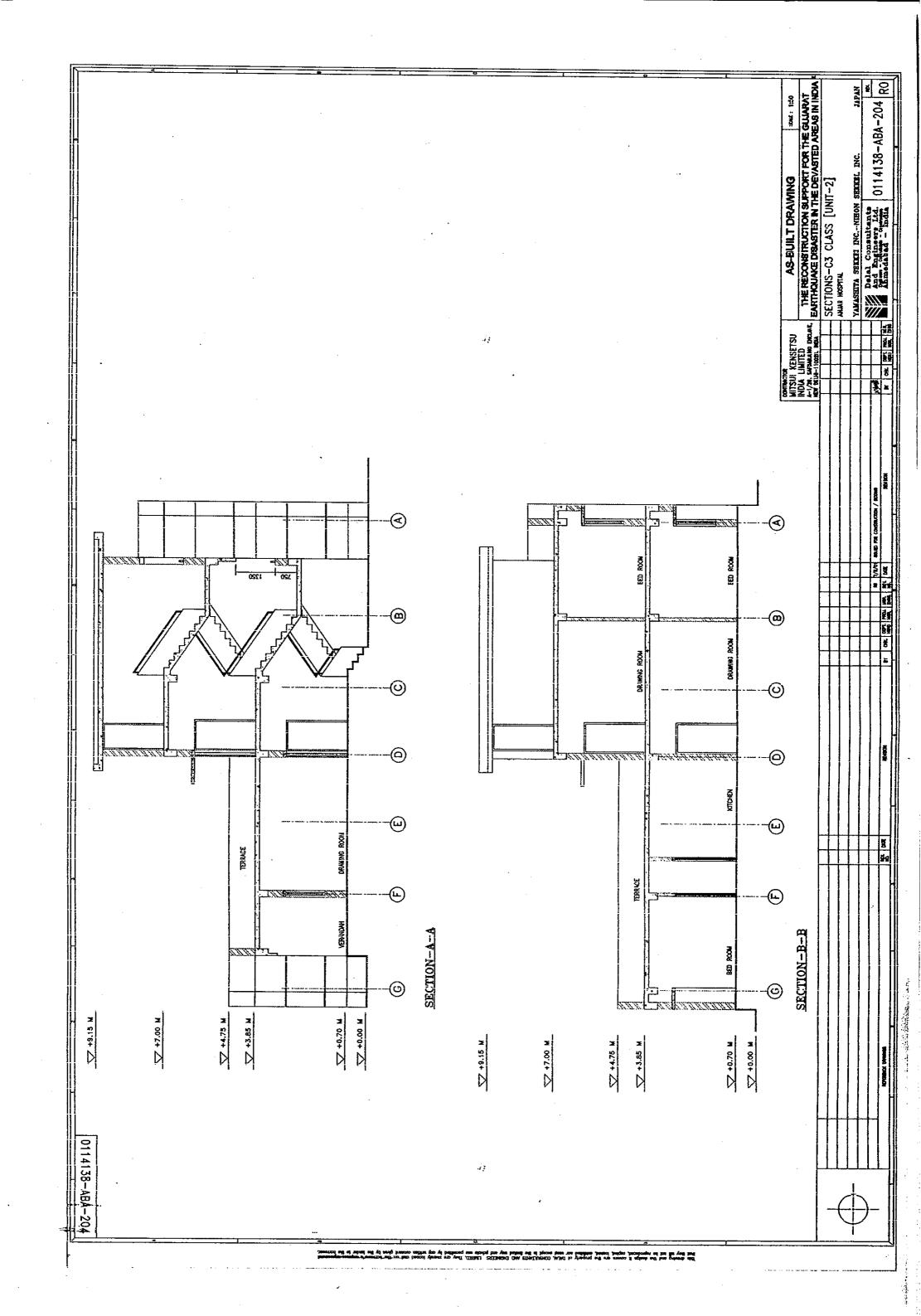


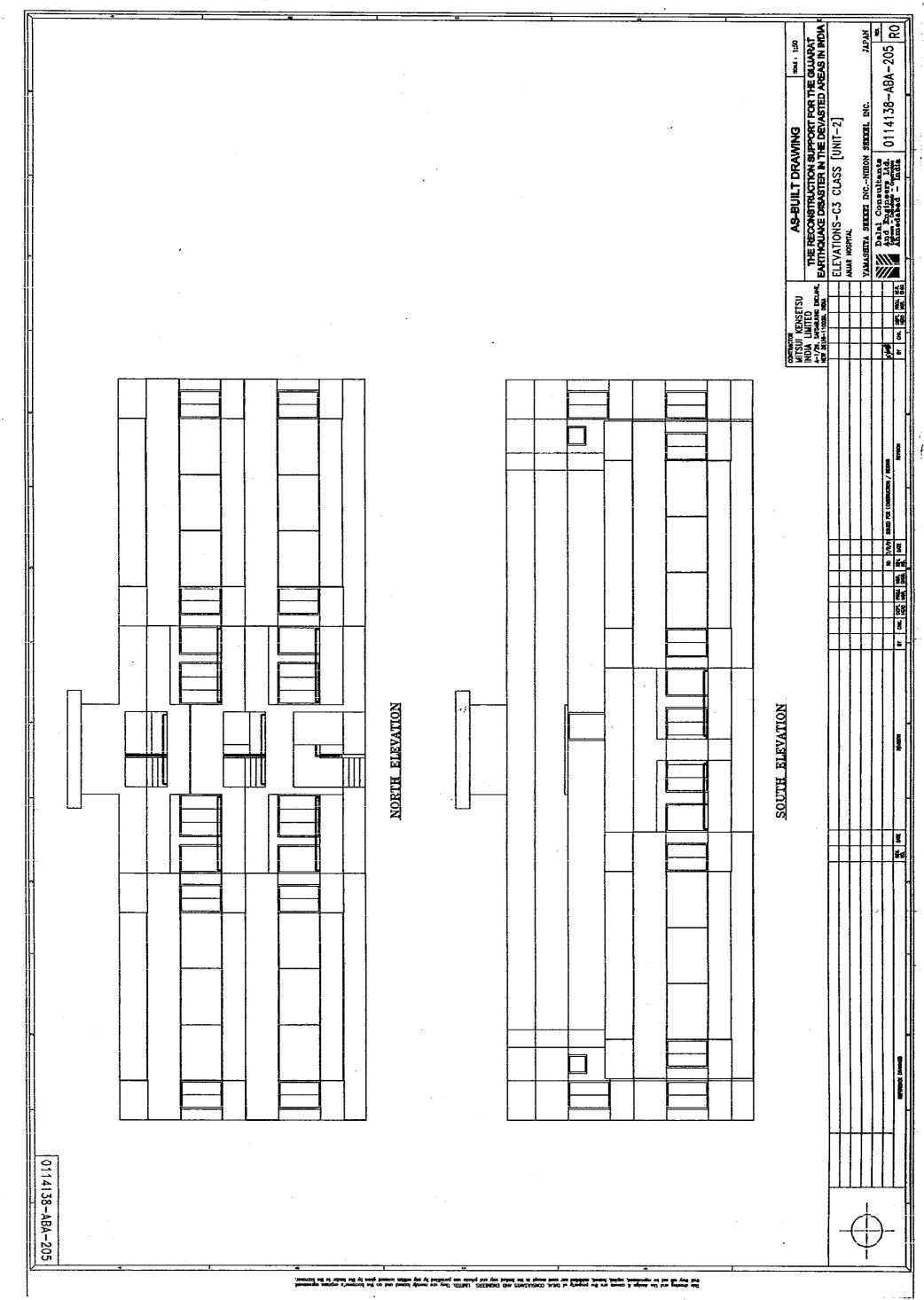


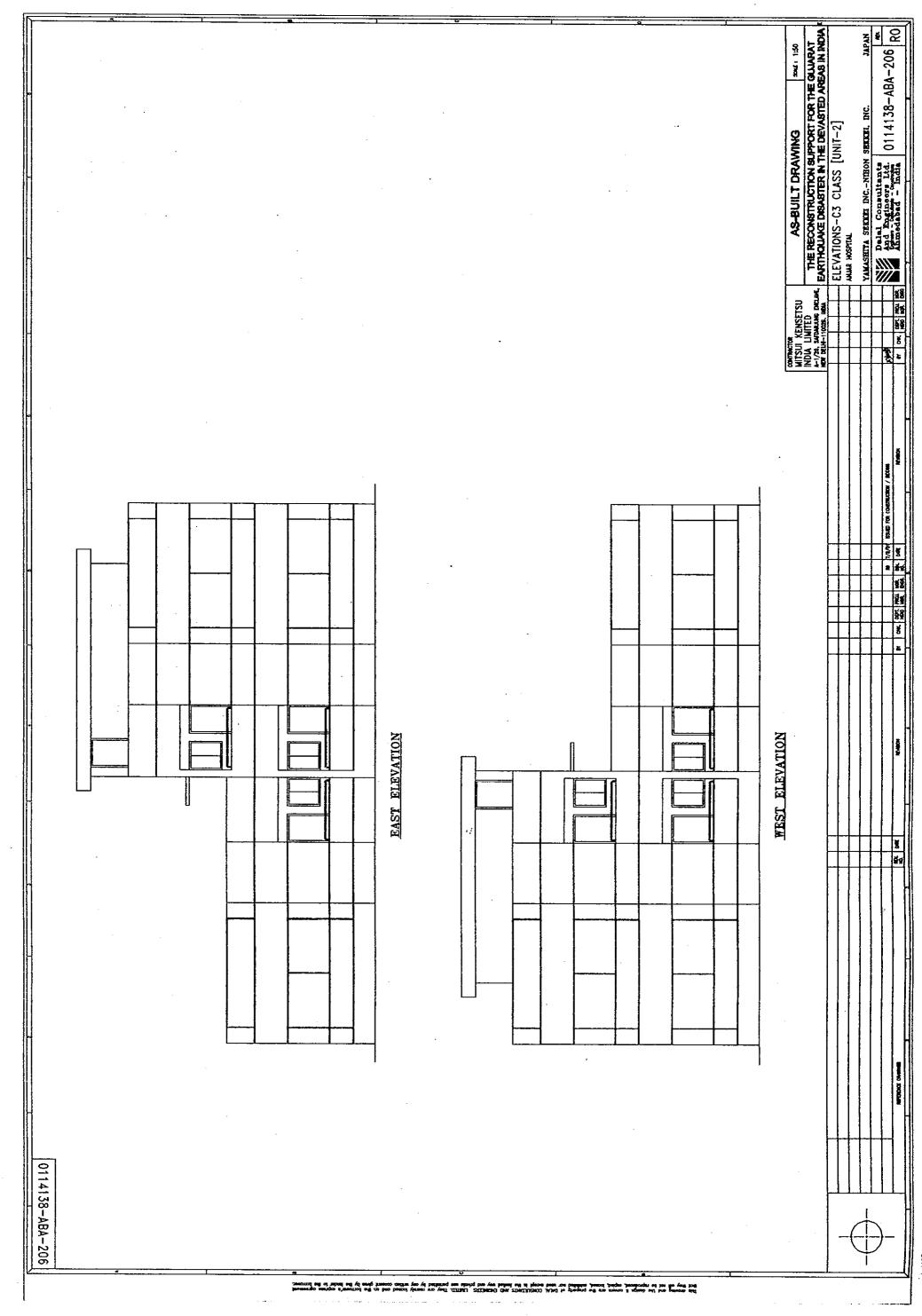




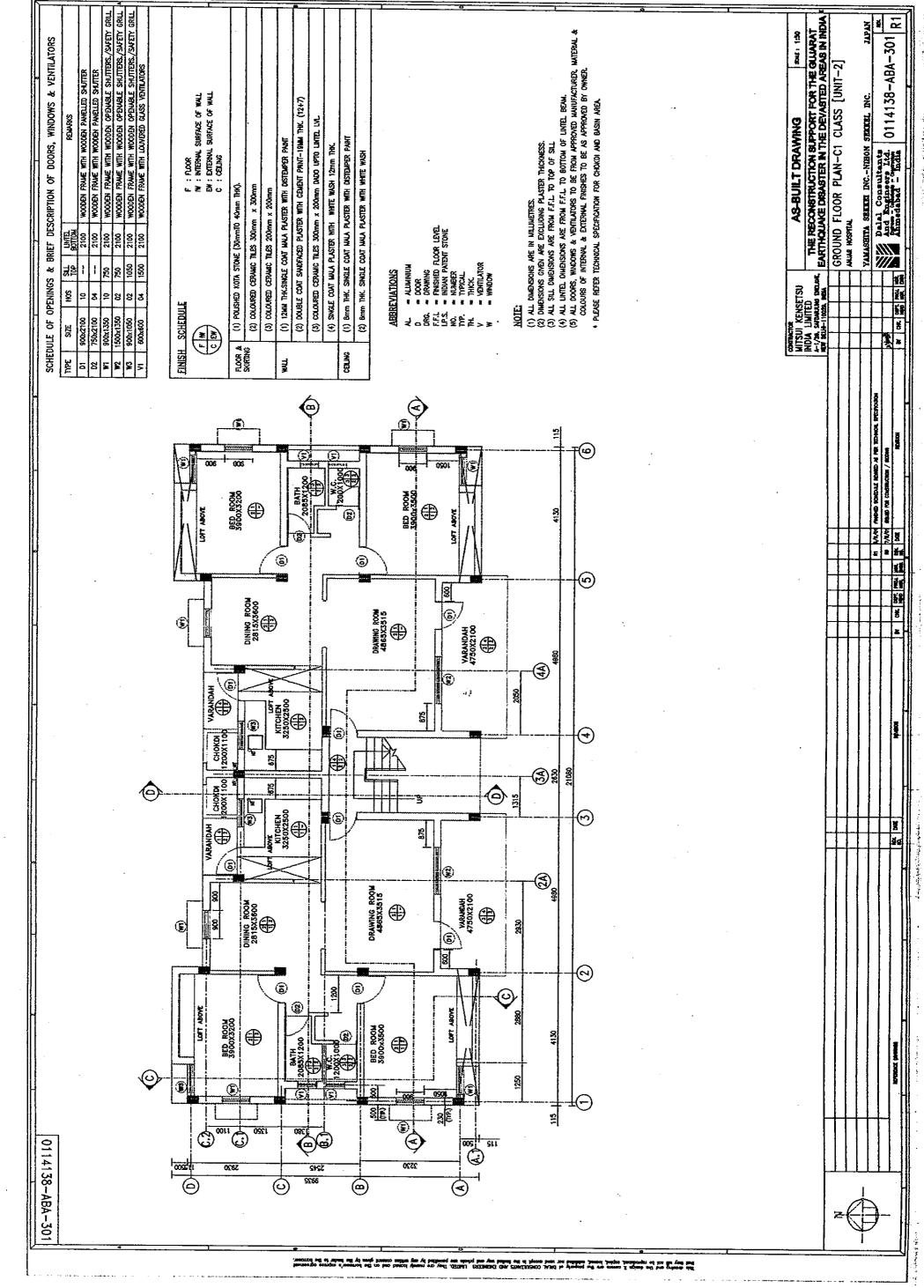


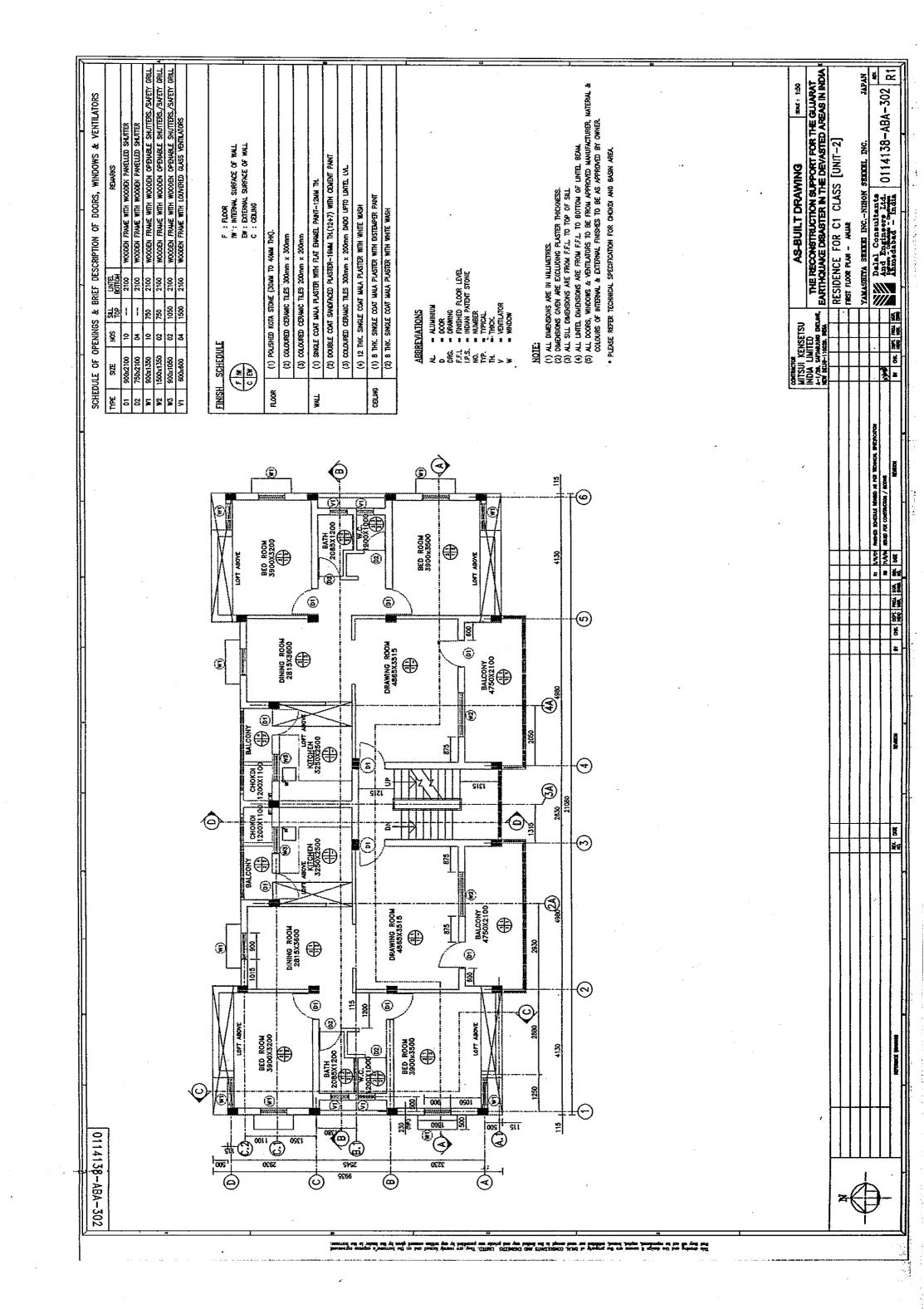


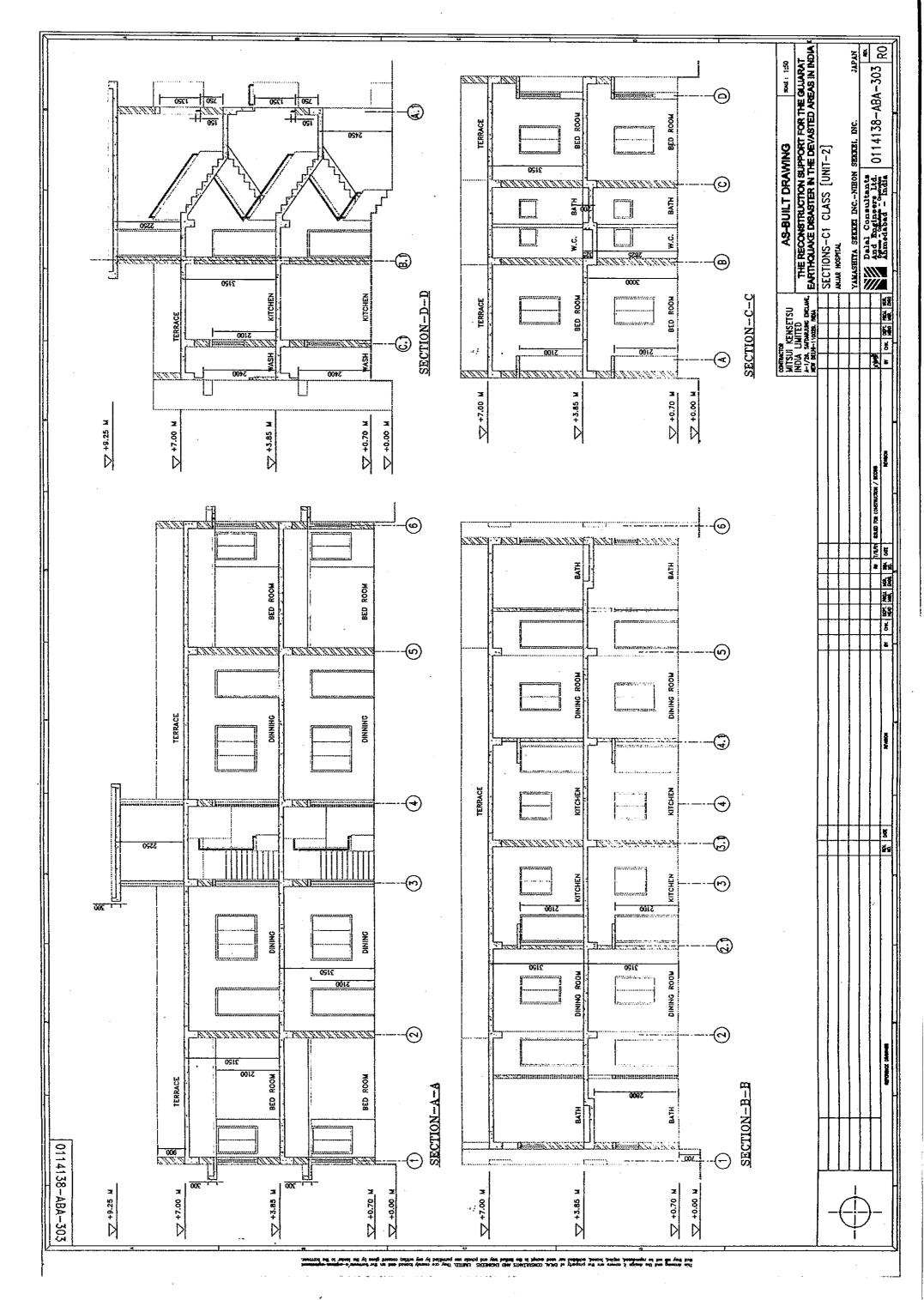


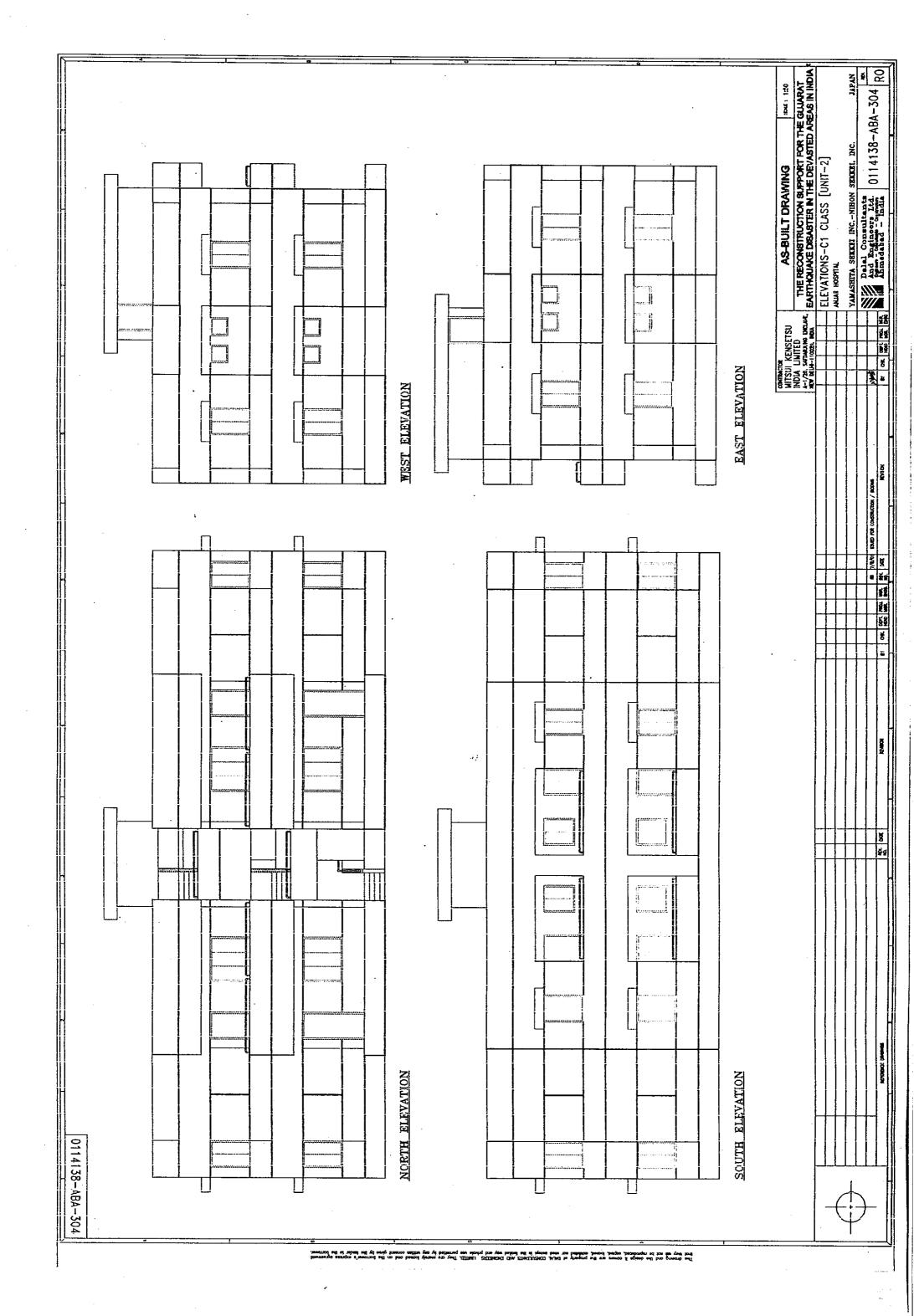


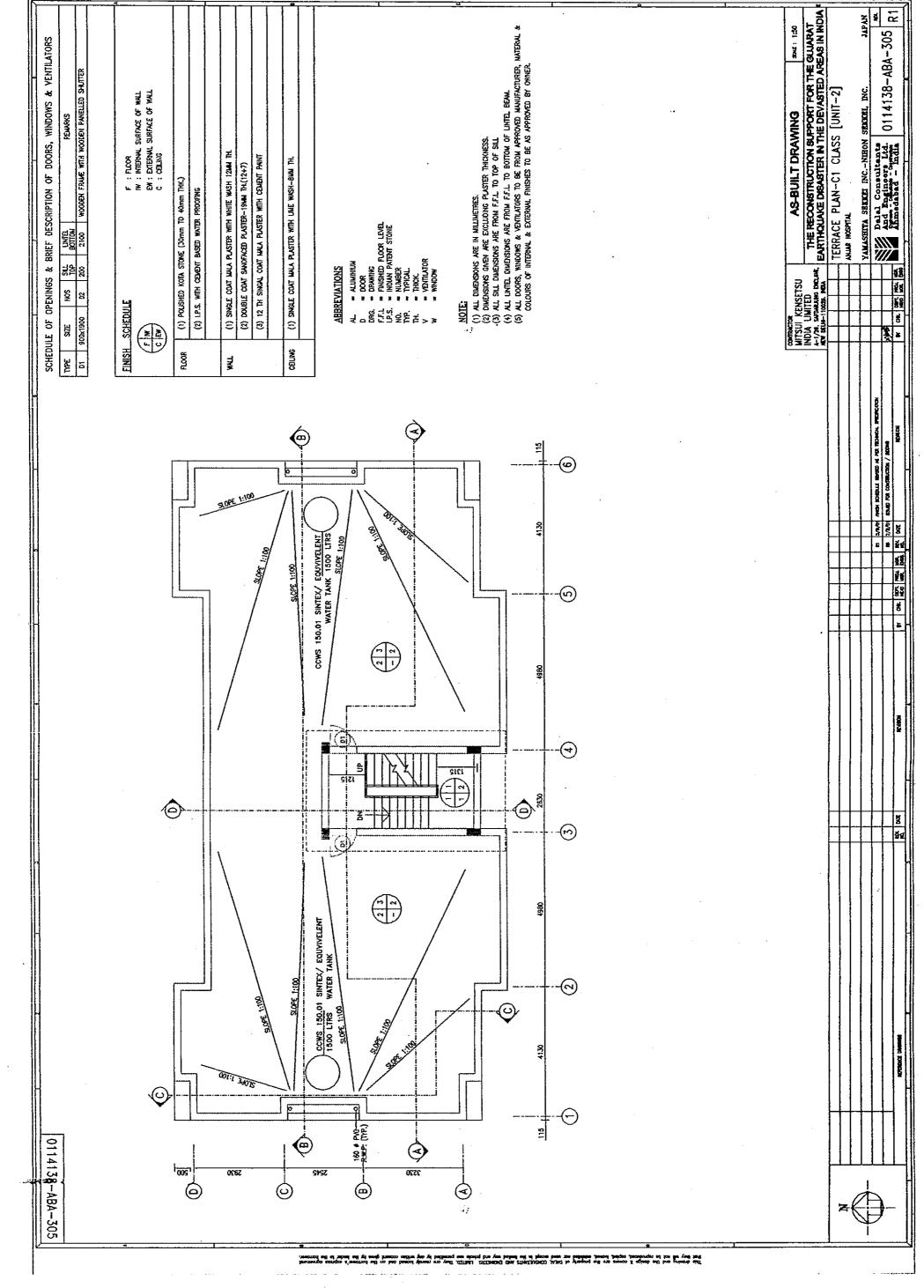
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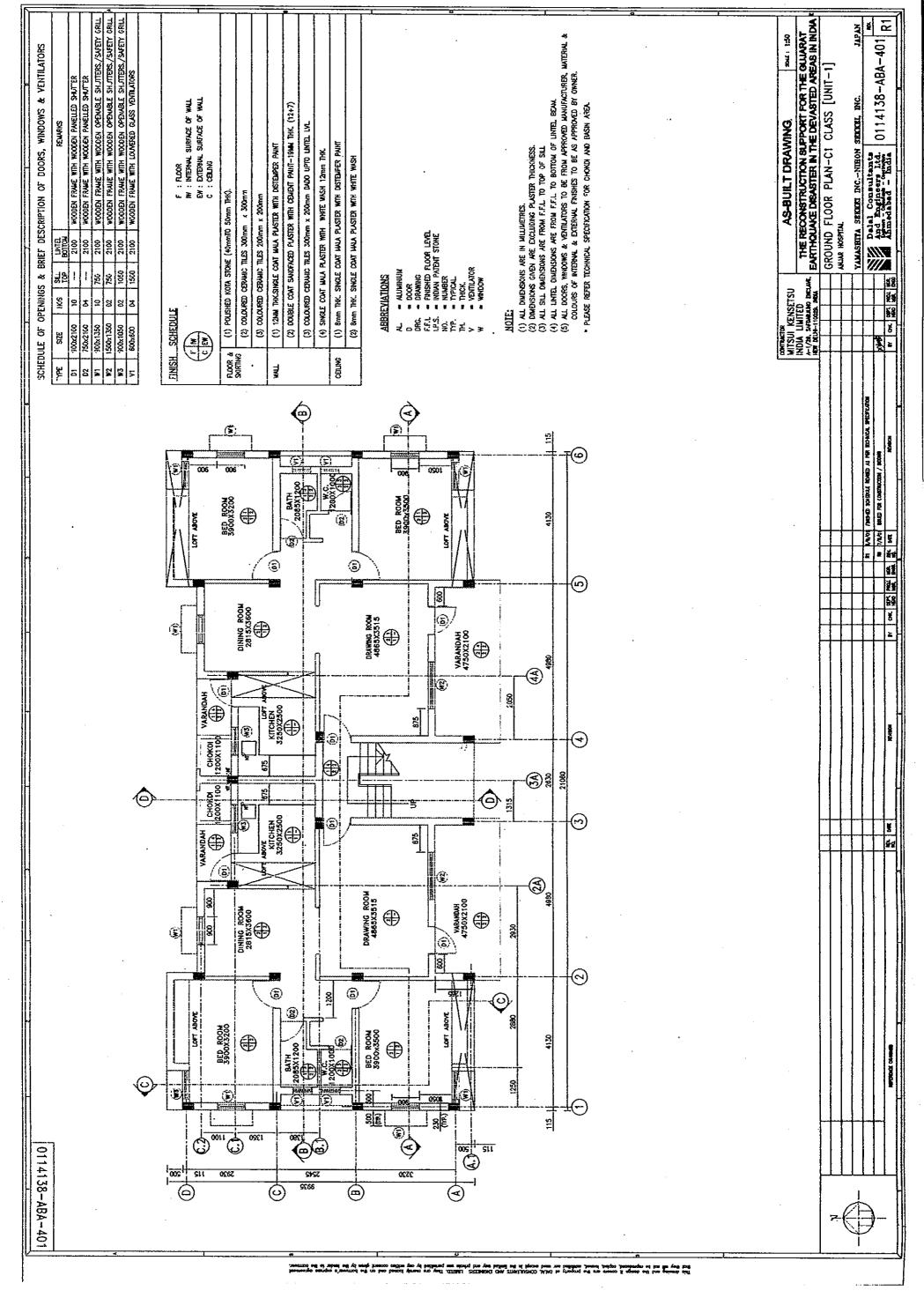


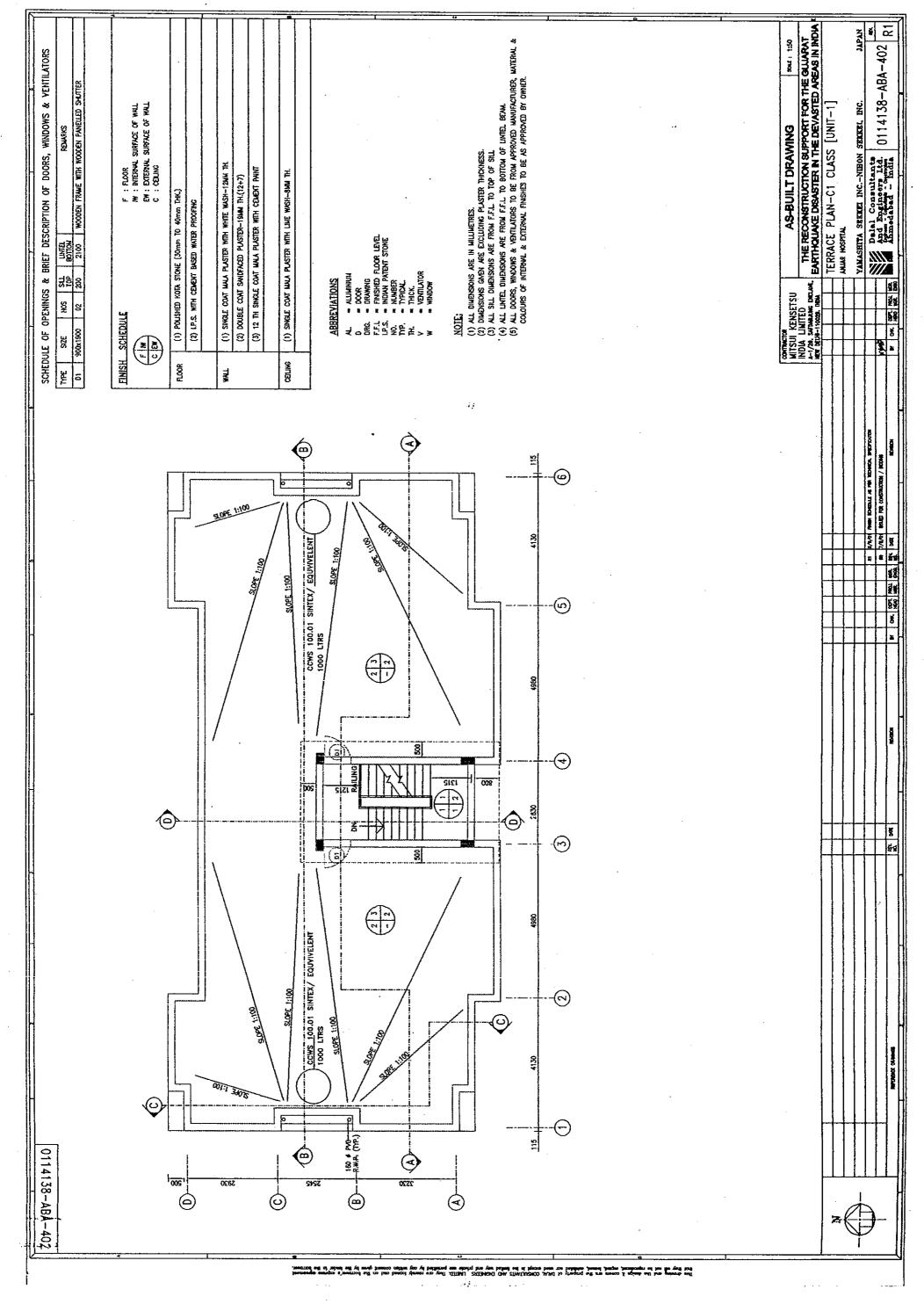




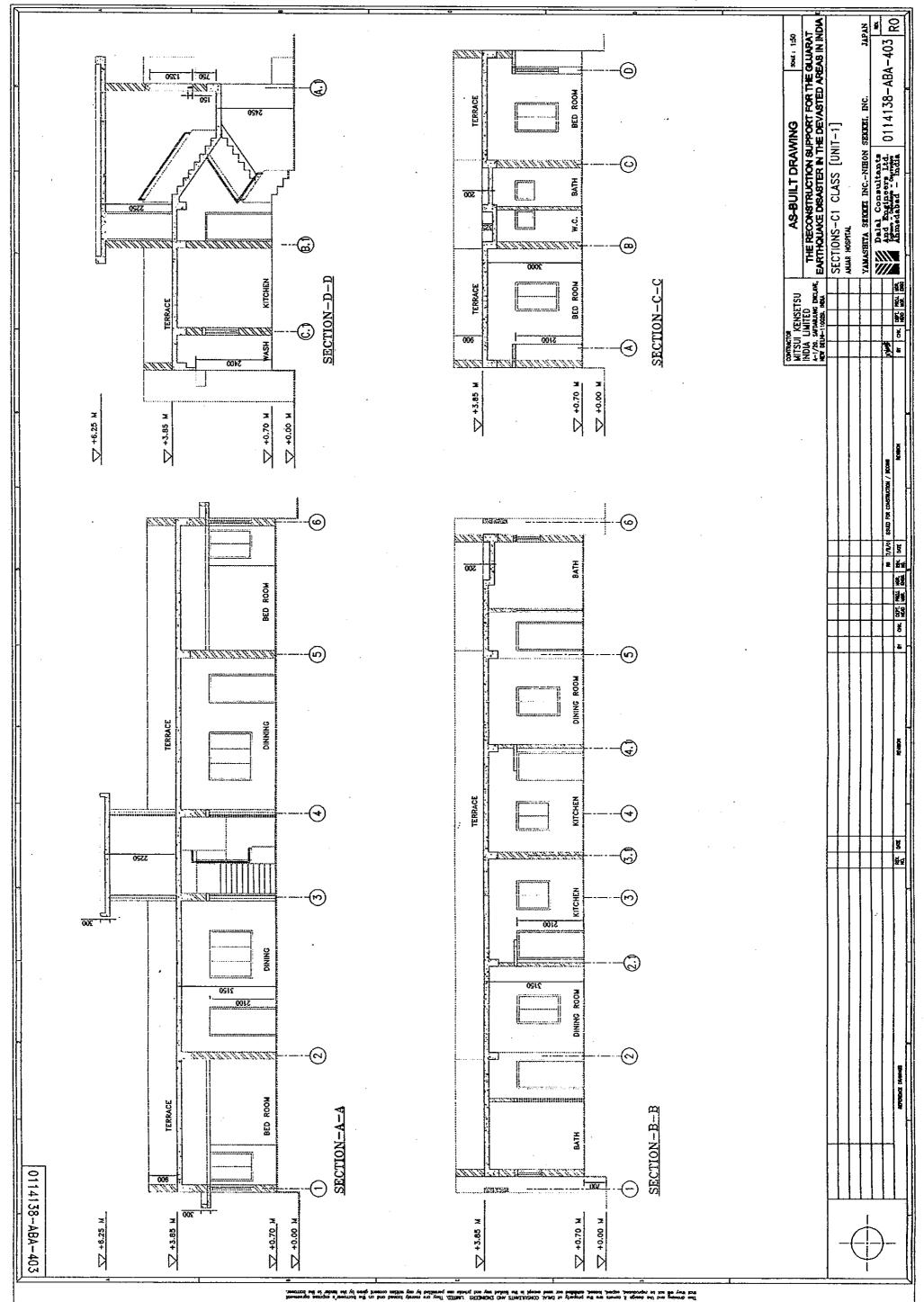


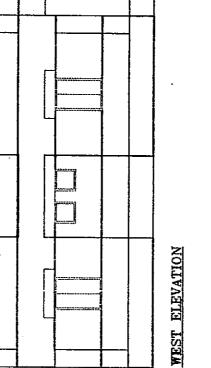




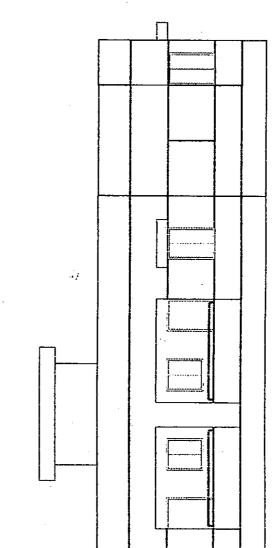


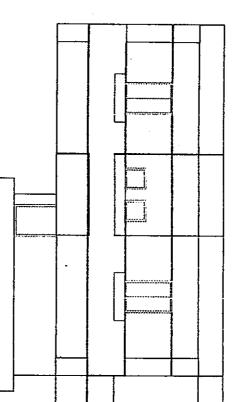
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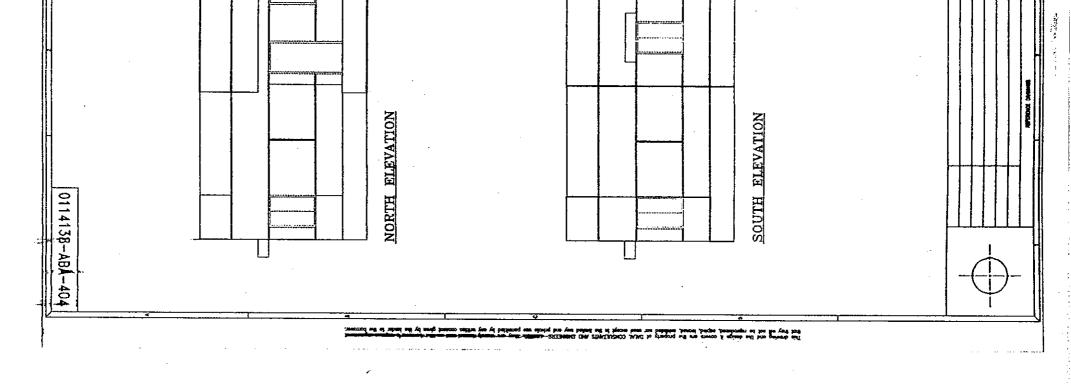
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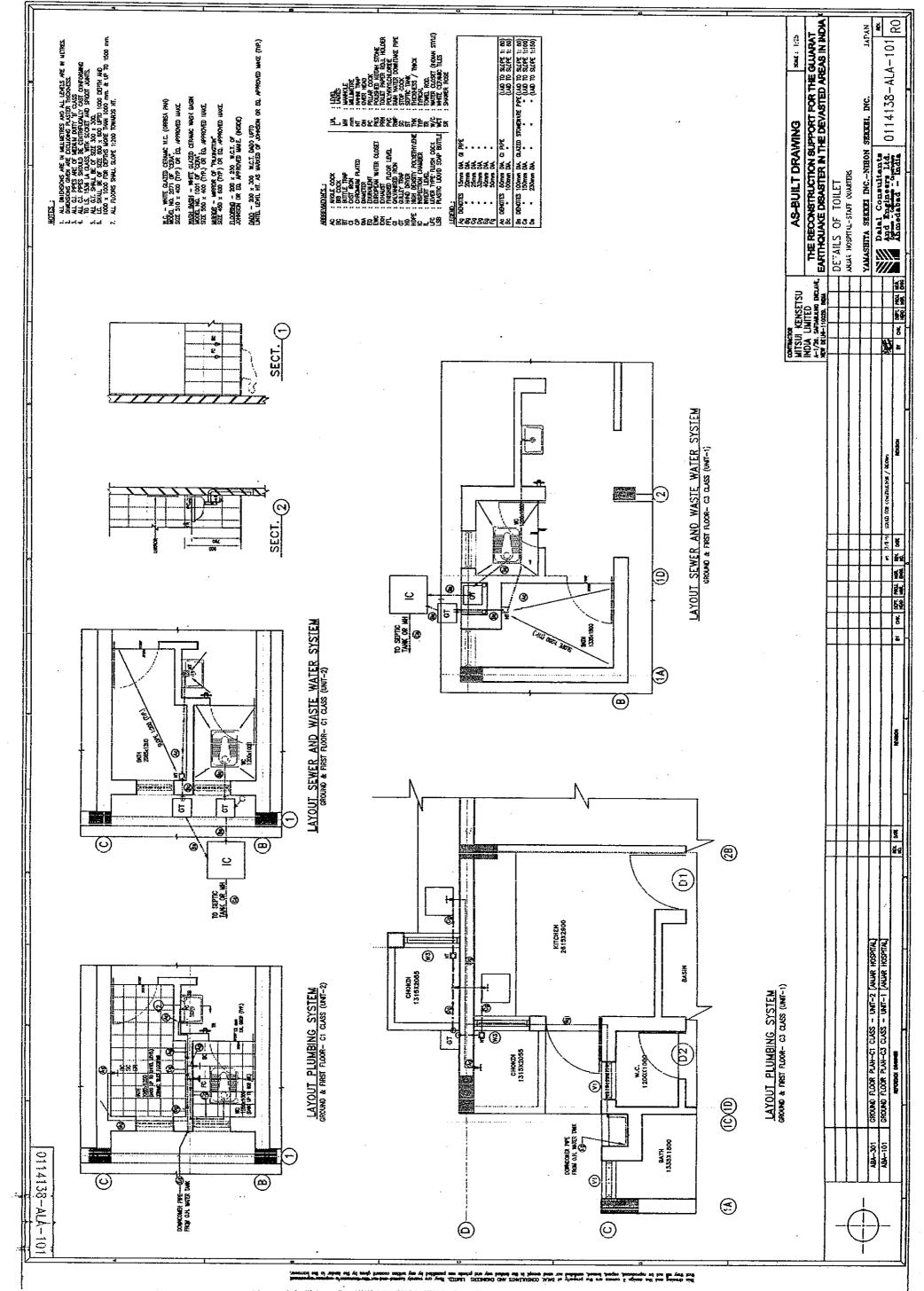
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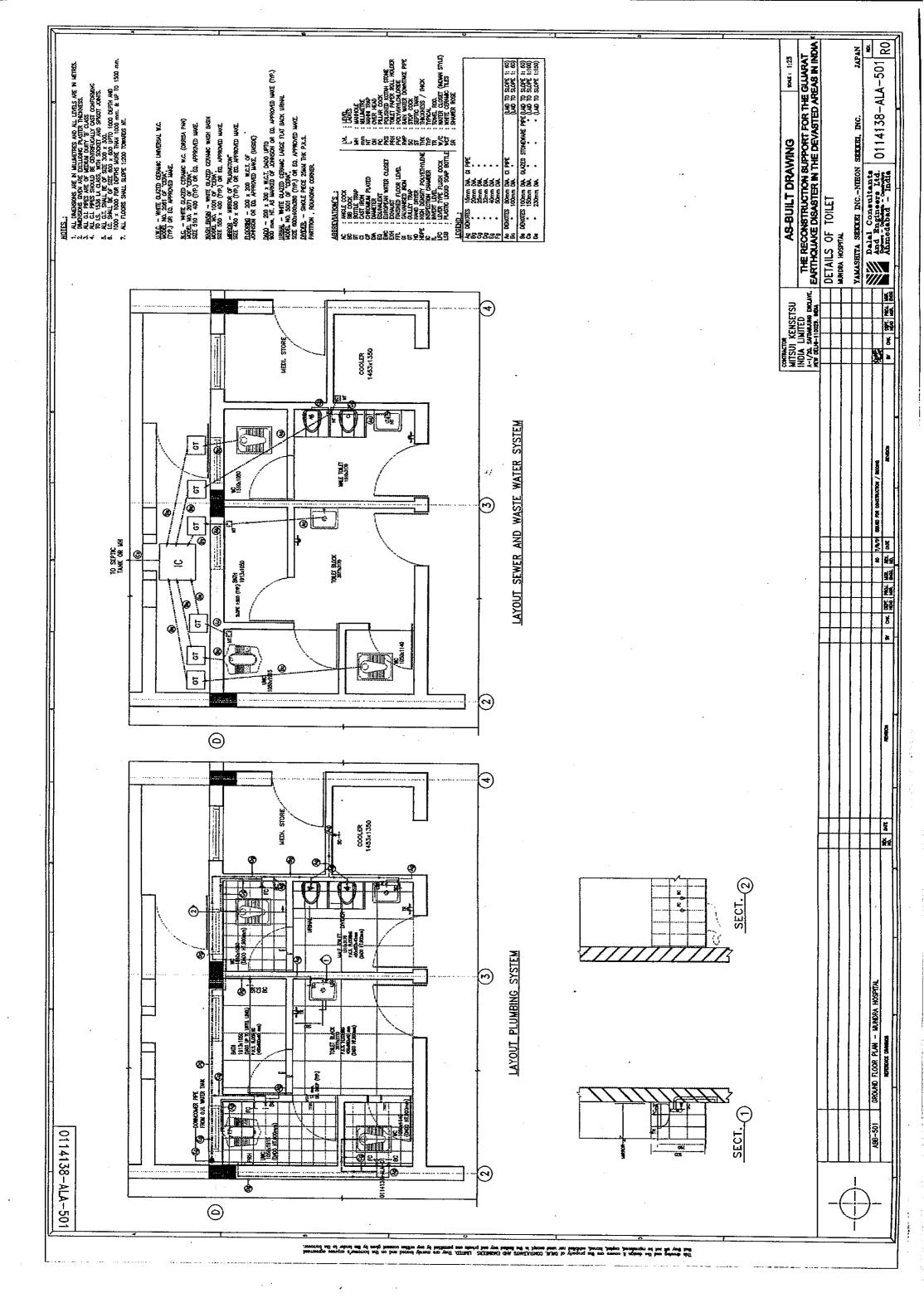
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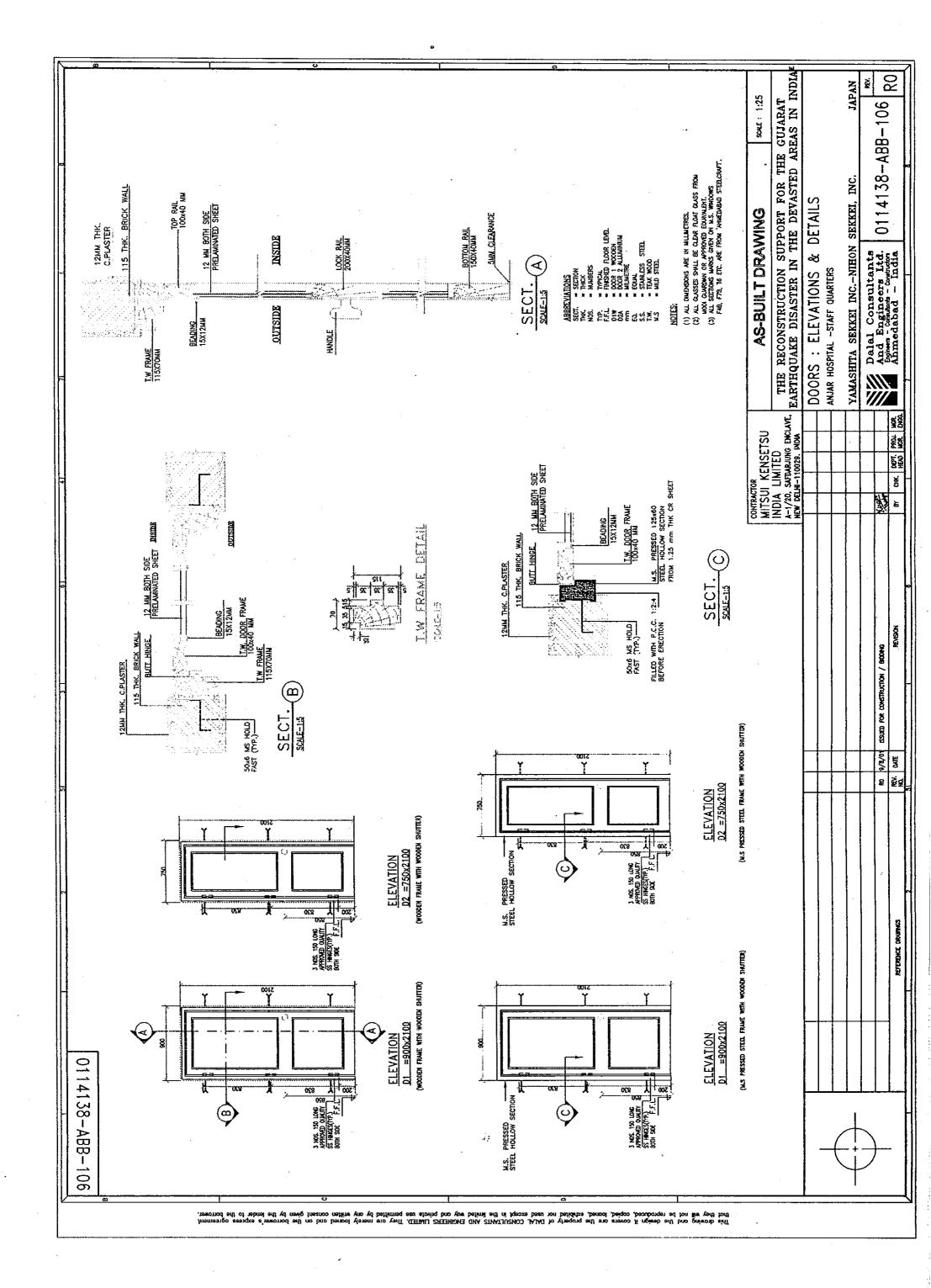
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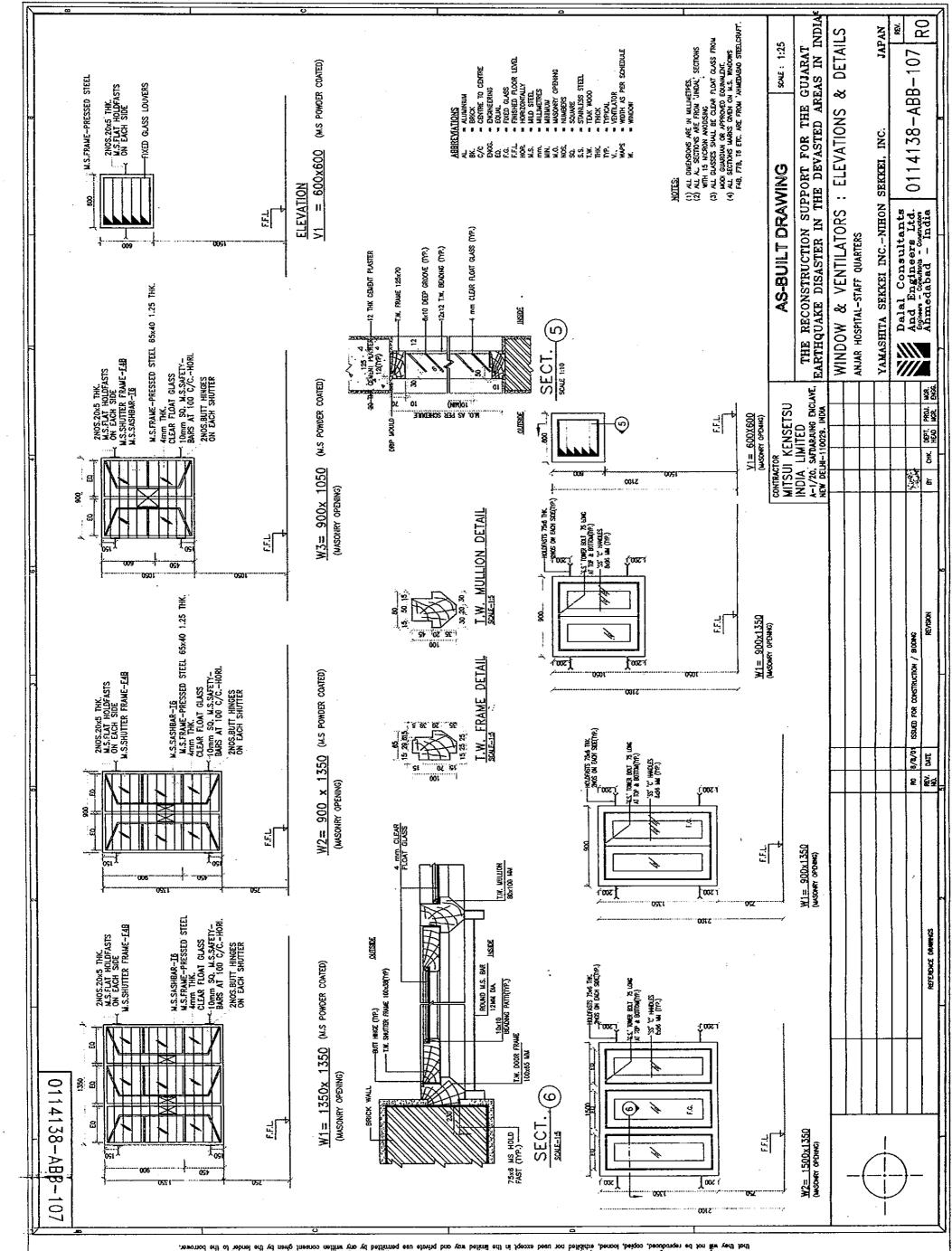






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