Standard Well Design (type-1 and type-2)

Appendix 1



Standard Well Design (type-1 and type-2)

Spread Sheet on Plan and Design for Rural Water Supply

CT				PREAD S	m	EET O	NI	PLAN AN	۹D	DESIGN	FC	DR RUR	AL	. WA	ΓE	ER SUPPLY					_		T
GE	Description	NF	JRMATIC	JN						Dals Lals			-				_						-
1	Province Name of	Con	mune/Ter	wn	-				:	Dak Lak	an (тпш	w	VV	T	VII VIII VI	п	D			-		+
2	System N		hinune/10	wii	+				•	D 1	an (1, 11, 111,	1 .	, v, v	1,	vii, viii, vi	11	1)			-		┝
4	Water Re	SOI	rce		+				•	Deen We	-11	(present	nu	mher	of	wells	1)			-		┢
-	water re	500	itt		_			1	·	Deep ine		present	. 114	anoer	01	wens)					-
W	ATER SUP	PPL	Y PLAN		Т								1								Τ		Г
Α	Plan Para	me	ter (1)																				t
1	Water res	sou	·ce							Deep we	11												T
1)	Altitude o	of J	ICA deep	well		El m			:	714													T
2)	Latitude					Ν			:	1432676						Longitude	Е	213371					Γ
3)	Permissib	ole y	ield /well			m3/da	ıy (l/sec)	:	346	(4)	(s	see not	te)	_							Γ
4)	Static wa	ter	surface lev	vel (S.W.L)	m			:	12													Γ
5)	Dynamic	wa	ter level (I	D.W.L)		m			:	28													
6)	Well dian	nete	r			mm			:	150/110													
2	Water de	ma	nd							2001			2	2010				2020					
1)	Number o	of h	ousehold			Numb	bers		:	1998													Ļ
2)	Populatio	n				Numb	ers		:	10989			12	2903				15423					Ļ
3)	Per capita	a co	nsumption	1	_	1/s/d			:	30				60			_	60			_		+
4)	Maximun	1 02	iny water o	uemanu u domond	+	m3/da	ly		:	129			-	02/			_	1444			_		+
2	Dogning	n 110	nhor of m		+	nc	Jui		l	11	-	+	+	200	_			120	\vdash		-	Total	┝
R	Plan Para	nu.	ter (2)	-11	+	110.			ŀ			+	+	2	-		-	2	H		+	TOTAL	┝
1	Water int	ane ab	(4)	-	+					Deen W	-11		+-						+		+		⊢
2	Raw wate	ant r fi	ansmissio	n	+			-	•	GI Pines		+ +	+-				+		+		+		⊢
3	Reservoir				+			1	:	Ground F	Resi	ervoir an	d el	levate	d t	tower	+		+		+		t
4	Water tre	atn	ient		$^{+}$			1	:	Chlorina	tion	l,		uit	- 1				H		+		t
5	Water dis	tril	oution		t			1	:	PVC, PE	, Pı	iblic taps	s, ho	ouse c	on	nnection	1		Ħ		+		t
6	Power su	pply	/		T			1	:	Public N	et		T	Ī			1		T		1		Γ
W.	ATER SUP	PPL	Y FACILI	ITY DESI	GI	N															-		F
A	Source		Deen Wells	s	_																		┢
	504100		Numbers no	eeded in ad	ldi	ition to	exi	sting Jica	we	-11			1										┢
			2002-2010	2	2		•	ling view															t
			2010-2020	2	2								+										t
			2010 2020	-																			t
В	Pump	1	Submersib	le pump									B	ooste	r p	oumps							t
			Yield: 4.	.0 1/s			Lif	55	m						-	Yield				Lift			F
			2002-2010	3	s r	105							20	02-20	10	86		m3/hr		15	m		F
			2010-2020	2	2 r	los							20	10-20	20	34		m3/hr		15	m		
С	Power		Public net																				-
					+								_				_				_		+
n	Reservoir	.	Ground ro	servoir	+				-	<u> </u>		+	F	lovot	h	Tower	+		\vdash		+		┝
	ACSCI VUII	-	2002-2010	301 1011) r	n3			-				20	02_20	10	10wei 43 1	m	3	+		+		⊢
-			2010-2020	122	2 r	m3		1	\vdash			+ +	20	10-20	20	17	m	3	+		+		⊢
1		-	Fotal	422	2 r	n3		1		1			T	otal		60	m	3	H		+		t
		+			1	-		1	F	1	-		1				T		\vdash		+		t
Е	Treatmen	t i	Chlorinati	on	t			1		1			1				+		Ħ		+		t
					T								1				:		Ħ				T
			Chlorinatio	n via conci	ret	e mixii	ng t	ank									Ţ						Γ
					Ι		_																Γ
																							L
F	Pipeline,	raw	water																Ц				L
			Diameter	100) r	nm			_				_						Ц				Ļ
1			2002-2010	4.5) k	cm			_				-				_		Ц				4
			2010-2020	3	k	cm			-				-						\square		_		╀
	1	ior	nines		+				-				+						H		+		+
C	Dietribut	01	Diameter	-	+	2002	201	0	\vdash	2010 202	20	+	+								+		┢
G	Distributi		25-65	nm	+	2002-	km		-	1 0	kn		+						H		+		┢
G	Distributi		75 125	 1m	+	12 5	km	1	\vdash	1.9	km	·	+				+		+		+		t
G	Distributi	-	(.)-1/3 Im		+	3.5	km		\vdash	1.1	km	1	+				+		+		+		t
G	Distributi	-	50-200 m	nm				1	1	1			+				+		+		+		t
G	Distributi		150-200 m	ım													- 1				- L		1
G	Distributi		150-200 m	ım																	+		┢
G	Distributi Public tar	DS 1	150-200 m	1m 1	l			Number		10													
G	Distributi	DS 1	Гуре :	1111 1	l			Number	:	10											_		ļ
G	Distributi	DS 1	Гуре :	nm 1	1			Number	:	10													
G	Distributi Public tap tes	DS	Гуре :	nm 1	1			Number	:	10													
G	Public tap	os ld c	(3-125 m 150-200 m Fype : f 346 m3/c	IM I I I I I I I I I I I I I I I I I I	fo	or plann	ing	Number		10													

					SP	READ SH	IF	EET O	NI	PLAN AN	ND	DESIGN	F	OR RUR	RA	L WA	TF	ER SUPPLY	ľ						
I	GE	NERAL I	NF	ORMAT	IO	N																			
	1	Province									:	Dak Lak													
	2	Name of O	Coi	mmune/T	lowi	n					:	Ea Leo (K1	I, K2, K3	3, I	K4, K5	, K	C6, K7, K8, 1	K9	9, K10, K	11	, Blech, L	e D	a)	
	3	System N Water Re	um	iber			-				:	D-2 Deen We	-11	(presen	t r	umber	of	fwells	1						
L	-				1							Deep ne		presen		luinoe			-						
Π	WA	ATER SUP	PPI	LY PLAN	I																				
	A	Plan Para	m	eter (1)								D													
	1	Water res	sou f	rce	n w	ما	_	El m				Deep we			_										
	2)	Latitude		JICA uce	ър w	en		N			:	1461593						Longitude	E	196617			_		
	3)	Permissib	le	yield /we	11			m3/da	iy (1	/sec)	:	225	((2.6) ((See no	ote)	-	190017					
	4)	Static wa	ter	surface	leve	el (S.W.L)		m			:	24													
	5)	Dynamic	wa	ater level	(D.	W.L)		m			:	46													
	6)	Well dian	net	er				mm			:	150/110				2010				2020					
	2	Number of	ma of b	na nousehold	1			Numł	pers			2001			_	2010				2020					
	2)	Populatio	n	lousenone				Numb	bers		:	15180				18464				22953					
	3)	Per capita	a co	onsumpti	on			l/s/d			:	30				60				60					
	4)	Maximun	n d	aily wate	r de	emand		m3/da	iy		:	178				1469				2148					
	5)	Maximun	n h	ourly wa	ter	demand		m3/ho	our		<u> </u>	15	<u> </u>			123				179				F-4-1	10
-	3 B	Required	nu	mber of	well	l		no.			:		-		+	6			_	3	-		-[ı otal	10
-	ט 1	Water inf	uil(ak	e (2)						+	:	Deen We	11	+ +	+				-		$\left \right $		+		
-	2	Raw wate	er t	- ransmiss	ion						:	GI Pipes			┥				-		\vdash		+		
	3	Reservoir									:	Ground I	Res	ervoir			_								
L	4	Water tre	ati	nent						<u> </u>	:	Chlorina	tio	n,							[
<u> </u>	5	Water dis	tri	bution							:	PVC, PE	, P	ublic tap	s, i	house	cor	nnection					_		
-	0	rower su	hbi	у	-						ŀ	r uonc N	εt	+ +	+				-		┢		+		
	I	1 1			I	1		1	L	1	<u> </u>	L	L	1				I			I	1			I
III	WA	ATER SUP	PPI	LY FACI	LIT	Y DESIG	N	1					L				_								
				_					-			[1		_								
I	Α	Source		Deep We	ells	4.4: *						.11			_										
<u> </u>			_	Numbers	nee	eded in add	11t	tion to	exi	sting Jica	we		-		+				_		-		-		
⊢			_	2010-202	20	3					-		-		+				-		\vdash		+		
-				_010 202		5					ŀ				┥				-		\vdash		+		
	B	Pump		Submers	sible	e pump															L				
				Yield:	2.6	1/s			Lif	80	m				ļ		_				-				
-				2002-201	0	7	n	os			-		<u> </u>		+				-		-		_		
-			_	2010-202		3	n	05		+	-	1	-	+ +	+				-		$\left \right $		+		
	С	Power		Public n	et																				
	D	Reservoir		Ground	rese	ervoir		2																	
				2002-201	20	490	m	13							_										
-			_	Total		717	m	13			╞		\vdash		+				-		\vdash		+		
	Е	Treatmen	ıt	Chlorina	tior	n , 1 ,		-			1				╈				-		\vdash		1		
																			:						
_				Chlorinat	tion	via concre	ete	e mixii	ng t	ank	1			+	1										
			_								-		-		+				_		-		-		
-	F	Pipeline	rav	v water	-					 	-		\vdash	+ +	+				-		\vdash		+		
	ſ			Diameter	ł	100	m	nm			1				╉				-		\vdash		1		
				2002-201	0	10.5	k	m																	
				2010-202	20	4.5	k	m							ļ		_								
_	C	Distribut	0~	ninca							-		-		+				-		-		+		
-	G	Distributi	on	Diameter	-			2002-	201	0	-	2010-202	20		+				-				+		
			_	25-65	mm	1		21.5	km		-	8.5	kr	n	┥				╞		ŀ		+		
	L			75-125	mn	1		12	km		L	3.5	kr	n	t		_		L		L				
				150-200	mn	1		3.5	km			0.5	kn	n											
													_												
	т	Dublia 4		Tunc						Number		17	<u> </u>	1	+				-				_		
	1	r unic tar	15	туре	:		-			number		16	-		+				-		$\left \right $		+		
	I	II			I	I		1		I	1	I	I	1				I	L		L	<u> </u>	1		I
IV	No	tes	_				_				_		_		_		_		_		_		_		
		The well c	lril	led by the	e stu	dy team or	nl	y yield	led	0,5 l/s. Fo	or c	alculation	ı pı	irposes a	yi	ield of	21	6 m3/day ha	s l	been used	fc	or future w	ell	5	
L																									

					SPI	READ SH	I	EET O	N F	PLAN AN	D	DESIGN	F	OR RUR	AL WA	TI	ER SUPPLY	ľ						
I	GE	NERAL I	NF	ORMAT	IO	N														Ι				
_	1	Province			1						:	Dak Lak												
	2	Name of	Co	mmune/T	lowi	n					:	Krung Pu	ık	(Nos. 4, 9	, MB,	Cho	o, Krung Bul	κ,	Krai A, K	Cla	l)			
	3	System N	un	ıber							:	D 3-1												
	4	Water Re	so	urce							:	Deep We	211	(present	numbe	r of	fwells	1						
п	W/	TED SH	DD		J						1					1				T		1		
	A	Plan Para	m	eter (1)												-								
	1	Water res	sou	irce								Deep we	11											
	1)	Altitude o	of .	JICA dee	ep w	ell		El m			:	484												
	2)	Latitude						N			:	1412609					Longitude	E	217070					
	3)	Permissib	ole	yield /we	11			m3/da	ıy (1	/sec)	:	415	1	(4.8)										
	4)	Static wa	itei	r surface	leve	1 (S.W.L)		m			:	9												
	5)	Dynamic Well dian	W:	ater level	(D.	w.L)		m			:	30								_				
	2	Water de	ma	nd							•	2001			2010				2020	-				
	- 1)	Number of	of I	iousehold	ł			Numb	bers		:	1192			2010				2020					
	2)	Populatio	n					Numb	bers		:	6791			8556				11060					
	3)	Per capita	a c	onsumpti	ion			l/s/d			:	30			60				60					
	4)	Maximun	n d	aily wate	er de	emand		m3/da	ıy		:	79			681				1035					
	5)	Maximun	n h	ourly wa	ter	demand		m3/ho	our			7	-	+	57	-			86	1			Tote1	2
_	э В	Required	11U	otor (2)	well	4		110.			•		-	+		\vdash			1	╞		_	1 Otal	3
	1	Water inf	ak	e (2)							:	Deen We	11	+ +		\vdash				+		_		
	2	Raw wate	er f	- ransmiss	ion						:	GI Pipes		+ +		\vdash				$\left \right $	-			-
	3	Reservoir	•							l	:	Ground I	Res	servoir wit	h eleva	ted	tower.			T				l
	4	Water tre	at	ment							:	Chlorina	tio	n,				_						
	5	Water dis	stri	bution				<u> </u>			:	PVC, PE	, P	ublic taps	, house	coi	nnection							
	6	rower su	pp	ly							:	Public N	et	+ +		\vdash				\vdash				
				1	<u> </u>	1		I		1	I	1	<u> </u>	1	I	1	1		1	<u> </u>	1			I
ш	WA	ATER SUF	PPI	LY FACI	LIT	Y DESIG	N	Ň												1				
					1											t				t				
	A	Source		Deep We	ells																			
				Numbers	nee	ded in add	lit	tion to	exi	sting Jica	we	el												
				2002-201	10	1		<u> </u>						+ +										
				2010-202	20	1								+ +		\vdash				\vdash				
	в	Pumn		Submers	sible	numn		-							Boost	er 1	oumps			+				
	~	P	-	Yield:	4.8	l/s			Lif	55	m				20000	1	Yield			1	Lift			
				2002-201	10	2	n	os							2002-2	010	57	_	m3/hr		15	m		
				2010-202	20	1	n	os							2010-2	020	29	_	m3/hr		15	m		
	6	_		D 1 "	Ļ																			
	C	rower		Public n	et								-	+ +	<u> </u>	\vdash				-	-	_		
					-			-								\vdash				+				
	D	Reservoir		Ground	rese	ervoir		<u> </u>					-	+	Eleva	ted	Tower			+				
	-			2002-201	10	199	n	n3							2002-2	010	28	m	13					
				2010-202	20	103	n	n3							2010-2	020	15	m	13	L				
				Total		302	n	n3							Total		43	m	13					
	F	an i	Ļ		Ļ			<u> </u>			-			+	L	_								
	E	1 reatmen	It	Chlorina	atior	1		<u> </u>				-		+ $+$	<u> </u>	\vdash		:		-	-			
				Chloring	tion	via concre	et/	e mixir	1g te	ank						\vdash				+				
			-	Chiorma		, in concil			- <u></u> - с							+				+				
	F	Pipeline,	rav	w water	L		_									L				t				
				Diameter	r 🗌	100	n	nm	-															
				2002-201	10	3	k	m																
				2010-202	20	1.5	k	m						+ +										
	C	Distributi	ior	nines	-									+ +		-				$\left \right $				
	J	Distribut	UI	Diameter	- -			2002-	201	0		2010-202	20	+ +		\vdash				•		_		
			-	25-65	mn	1		10	km	-		1.5	kr	n		+				ŀ				
				75-125	mn	1		3.5	km			0.5	kr	n		T				T				
				150-200	mm	ı		1	km				kr	n										
								<u> </u>																
	T	D-LP /		T	<u> </u>	-		<u> </u>		N 1				+ +										
	1	Public tap	DS	Type	<u> :</u>	1				Numbers	ŀ-	9	-	+ +		-								
					1			I		1		1			I	<u> </u>			1	1	1			
IV	No	tes																						
- 1																								
													_					_		_		_		
					-										-	-								

				SP	READ SH	H	EET O	N I	PLAN AN	ND	DESIGN	F	OR RUR	AL WA	TI	ER SUPPLY	7						
I	GE	NERAL IN	FORMAT	101	N																		<u> </u>
_	1	Province								:	Dak Lak												
	2	Name of Co	ommune/7	low	n					:	Krung Pu	ık	(Nos. 8, 1	0,11, 1	2, 1	17)							
	3	System Nu	nber							:	D-3-2												
	4	Water Reso	ource							:	Deep We	ell	(present	numbe	r of	fwells	0)					
п	w	TER SUPP	UV PLAN	J	1													1	Г				
	A	Plan Paran	neter (1)																				
	1	Water reso	urce								Deep we	11											
	1)	Altitude of	JICA dee	ep w	ell		El m			:													
	2)	Latitude					N			:						Longitude	E						
	3)	Permissible	yield /we	<u>11</u>			m3/da	ay (l/sec)	:	415		(4.8)										
	4)	Static wate	er surface	leve	<u>el (S.W.L)</u>		m			:													
	5)	Dynamic w	ater level	(D.	W.L)		m			:													
	2	Water dem	and				111111			•	2001			2010				2020					
	1)	Number of	household	1			Numł	bers		:	640			2010				2020					
	2)	Population					Numl	bers		:	3543			4463				5770	ŀ				
	3)	Per capita	consumpti	ion			l/s/d			:	30			60				60	L				
	4)	Maximum	daily wate	er de	emand		m3/da	ay		:	41			355				540					
	5)	Maximum	hourly wa	ter	demand		m3/h	our		<u> </u>	4			30				45				T. 1	-
	3 D	Required n	umber of	well			no.		-	1:	1	-	+ +	0	-			1	-			ı otal	2
	Di 1	r Iall Parall Water into	ieter (2)	-			-	-			Deen We	11	+ +		-				-				
	2	Raw water	transmiss	ion		-	-	-		:	GI Pines		+ +		-				+				
	3	Reservoir		1.54	1	ŀ		-	1	:	Ground I	Res	servoir and	i elevat	ed	tower		1	┢				
	4	Water trea	tment	1						:	Chlorina	tio	n,						T				
	5	Water distr	ibution							:	PVC, PE	, P	ublic taps	, house	coi	nnection	_					-	
	6	Power supp	oly	-			<u> </u>			:	Public N	et a	and/or gen	erator									┣──
			<u> </u>	<u> </u>	L	L	<u> </u>			<u> </u>	1	L						L		1			L
ш	w	TER SUPP	LV FACI	LIT	V DESU	2N	v												1				
	•• F	LIER SUFF	LIPACI		. DESIC	· 1 و	Ì			-		-		<u> </u>					┢				
	A	Source	Deep We	ells		-		۱ <u> </u>	1	ı	L			<u> </u>					t				-
			Numbers	nee	ded in add	dit	tion to	exi	sting Jica	we	el								T				
			2002-201	10	1																		
			2010-202	20	1																		
	n	D	C1				<u> </u>						+ +	D-									<u> </u>
	в	rump	Vield	4 9	2 pump	-	-	1.44	70	m			+ +	BOOST	r I	viald				I i A			
			2002-201	10	1 1	p	os		,0				+ +	2002-2	010	30		m3/hr	1	15	m		<u> </u>
			2010-202	20	1	n	os							2010-2	020	15		m3/hr	1	15	m		
																			L				
	С	Power	Public n	et, a	lternativ	ely	y																
			Diesel ge	ener	ator																		
	n	Decemuein	Crownd											Flore	ad	Towar							
	D	Reservoir	2002 201	rese	104	17	13							Eleval	ea	1ower 15	1111	3	-				
			2010-202	20	54	n	n3							2010-2	020	8	m	13					
			Total	-	158	n	n3							Total		23	m	13	1				
																	_		L				
	E	Treatment	Chlorina	atio	1														L				
			Ch1-	 41 -									+ +				:		L				├
			Chlorina	tion	via concre	ete	e mixi	ng t	anĸ	-		-	+ +		-				-				
	F	Pineline ra	w water			┢		-	-	\vdash		\vdash	+ +		-				╞				
	-	- ipenne, la	Diameter	r	100	n	nm												┢				
			2002-201	10	1.5	k	m							<u> </u>					t				
			2010-202	20	1.5	k	m			L		L							L				
			1	1			L																
	G	Distributio	n pipes				2002	201		<u> </u>	2010 202		+	L									
			Diameter				2002-	201	U	-	2010-202	2U 1		<u> </u>	-				:				
			23-03	mn	1 1	╞	15	кm km		-	2	Kľ kr	n	-	-				╞				
			150-200	mn	1	┢	2	km	1	-	2	kr	n –	<u> </u>	-				╞	-			<u> </u>
			200																1				
																	_		L				
	I	Public taps	Туре	:	1				Number	:	7						_		1				
				<u> </u>						L		L											
w	Nat	0.0																					
11	101	No well was	construct	ed in	n the area	d	uring +	he f	irst nhace	of	the study												
		110 well was	sconstruct	cu II	n ine area	al	unig t	ne l	n st pliase	01	are study.												
						-																	

I GENERAL INFORMATION i Dak Lak i 1 Province i Dak Lak i i 2 Name of Commune/Town : Ea Drong (T1, T2, T3, T4, T5, T6, E-Joh) i 3 System Number i D 4-1 i i 4 Water Resource : Deep Well (present number of wells 1 4 Water Resource i Deep Well (present number of wells 1 1 Water resource Deep well i i i 1 Water resource Deep well i i i 2 Latitude N : 1427255 Longitude E 209295 3 Pyramissible yield /well m3/day (U/sec) : 268 (3.1 i	I I I I
1 Province : Dak Lak	I I I I
2 Name of Commune/Lown : Ea Drong (11, 12, 13, 14, 15, 16, E-Joh) 3 System Number : D4-1	I I
3 System Number : D4-1 Image: D4-1 Image: D4-1 4 Water Resource : Deep Well (present number of wells 1 WATER SUPPLY PLAN Image: Deep Well (present number of wells 1 A Plan Parameter (1) Image: Deep Well (present number of wells 1 1 Water resource Deep well (present number of wells 1 1 Water resource Deep well (present number of wells 1 2 Latitude I N : 1427255 3 Permissible yield /well m3/day (Usec) : 268 (3.1) 4 Static water surface level (S.W.L) m : 16 5 Dynamic water level (D.W.L) m : 160 6 Well diameter mm : 150 2 Water demand 2001 2010 3 Per capita consumption 1%/day (I/sec) : 82 619 3 Per capita consumption 1%/day : 82 619 4 Maximum dualy water demand m3/day : 82 619 5 Maximum hourly water demand m3/day : 82 619 7 S2 68 3 8 Required number of well no. : 1 1 Water intake : Deep Well Image: 1 2 Raw water transmission : GI Pipes <t< th=""><th>Image: Constraint of the second sec</th></t<>	Image: Constraint of the second sec
WATER SUPPLY PLAN i begin Wein (present names of weins in the set	Total
WATER SUPPLY PLAN Image: constraint of the second constraint on the second	Total
A Plan Parameter (1) Deep well Deep well Image: strain of the str	Total
1 Water resource Deep well Deep well Deep well 1) Altitude of JICA deep well El m : 615 Image: Construct of the second secon	Total
1) Altitude of JICA deep well E1 m : 615 Longitude E 202925 2) Latitude N : 1427255 Longitude E 209295 3) Permissible yield /well m3/day (l/sec) : 268 (3.1) Image: Construction of the second o	Total
2) Latitude 1 <td< td=""><td>Total</td></td<>	Total
4) Static water surface level (S.W.L) m : 16	Total
5) Dynamic water level (D.W.L) m : 42	Total
6) Well diameter mm : 150 200 2 Water demand 2001 2010 2020 1) Number of household Numbers: : 1245	Total
2 Water demand 2001 2010 2020 1) Number of household Numbers 1245 11245 11245 2) Population Numbers 2084 7775 8760 3) Per capita consumption 1/s/d 200 60 60 60 4) Maximum daily water demand m3/day 202 619 820 60 5) Maximum hourly water demand m3/day 202 68 8 3 Required number of well no. 1 2 1 B Plan Parameter (2) 1 2 1 1 1 Water transmission 2 GI Pipes 1 1 3 Reservoir 2 Ground Reservoir with elevated tower 1 2 1 4 Water treatment 2 Aeration and Slow filtration. Chlorination, 1 2 5 Water distribution 2 PUblic Net and/or generator/solar systems 1 1 4 WATER SUPPLY FACILITY DESIGN 1 1 1 1	Total
1) Numbers 1 127 2) Population Numbers 3) Per capita consumption 1/s/d 4) Maximum daily water demand m3/day 5) Maximum hourly water demand m3/day 6884 7775 82 619 82 619 82 619 82 619 82 619 82 619 82 619 82 619 82 619 82 619 83 Required number of well 90 1 91 2 1 2 1 2 1 2 1 2 1 2 1 2 1 2 1 2 1 2 1 2 1 2 1 2 2 1 3 Reservoir 3 Reservoir 4	Total
3) Per capita consumption 1/s/d : 30 60 60 4) Maximum daily water demand m3/day : 82 619 820 5) Maximum hourly water demand m3/day : 82 619 820 5) Maximum hourly water demand m3/hour 7 52 68 3 Required number of well no. : 1 2 1 B Plan Parameter (2) 1 Water intake : Deep Well 2 Raw water transmission : GI Pipes 3 Reservoir : Ground Reservoir with elevated tower 4 Water treatment : Aeration and Slow filtration. Chlorination, 5 Water distribution : PVC, PE, Public taps, house connection 6 Power supply : Public Net and/or generator/solar systems 1 : Aeration and Slow filtration. : Public Net and/or generator/solar systems <	Total
4) Maximum daily water demand m3/day : 82 619 820 5) Maximum hourly water demand m3/hour 7 52 68 3 Required number of well no. : 1 2 1 B Plan Parameter (2) . . 1 2 1 1 Water intake : Deep Well . . . 2 Reservoir : GI Pipes 3 Reservoir : Ground Slow filtration. Chlorination, 3 Water distribution : PVC, PE, Public taps, house connection 5 Water distribution : Public Net and/or generator/solar systems 1 WATER SUPPLY FACILITY DESIGN 	Total
5) Maximum hourly water demand m3/hour 7 52 68 3 Required number of well no. : 1 2 1 B Plan Parameter (2)	Total
3 required number of weil no. : 1 2 1 B Plan Parameter (2) .	
1 Water intake : Deep Well 2 Raw water transmission : GI Pipes 3 Reservoir : Ground Reservoir with elevated tower 4 Water treatment : Aeration and Slow filtration. Chlorination, 5 Water distribution : PVC, PE, Public taps, house connection 6 Power supply : Public Net and/or generator/solar systems 1 WATER SUPPLY FACILITY DESIGN Image: Construction of the system of th	
2 Raw water transmission : GI Pipes 3 Reservoir : Ground Reservoir with elevated tower 4 Water treatment : Aeration and Slow filtration. Chlorination, 5 Water distribution : PVC, PE, Public taps, house connection 6 Power supply : Public Net and/or generator/solar systems 1 WATER SUPPLY FACILITY DESIGN : .	
3 Reservoir : Ground Reservoir with elevated tower 4 Water treatment : Aeration and Slow filtration. Chlorination, 5 Water distribution : PVC, PE, Public taps, house connection 6 Power supply : Public Net and/or generator/solar systems 1 WATER SUPPLY FACILITY DESIGN Image: Construction of the system	
4 Water treatment : Aeration and Slow filtration. Chlorination, 5 Water distribution : PVC, PE, Public taps, house connection 6 Power supply : Public Net and/or generator/solar systems II WATER SUPPLY FACILITY DESIGN III	
5 Water distribution : PVC, PE, Public taps, house connection 6 Power supply : Public Net and/or generator/solar systems I WATER SUPPLY FACILITY DESIGN	
U WATER SUPPLY FACILITY DESIGN	
II WATER SUPPLY FACILITY DESIGN	
I WATER SUPPLY FACILITY DESIGN	
A Source Deep Wells	
Numbers needed in addition to existing Jica well	
B Pump Submersible pump B Booster pumps B	
Yield: 3.1 1/s Lift 80 m Yield	Lift
2002-2010 3 nos 2002-2010 52 m3/hr 2010-2020 1 mos 2010 2020 16 m3/hr	15 m
	15 11
C Power Public net	
Diesel generator	
Solar system	
D. Durancia Communicational de la communicación de	
D Reservoir Ground reservoir Elevated 1 ower E	
2002-2010 101 m3 2010-2021 20 m3 2010-2021 9 m3	
Total 240 m3 Total 35 m3	
E Treatment Aeration, Slow sand filtration and chlorination.	
Actration area: Reaction tank, volume: Filter area: 2002-2010 5 m2 2002-2010 26 m3 2002-2010 11 m ²	2
2010-2020 2 m2 2010-2020 9 m3 2010-2020 : 3 m2	2
Total 7 m2 Total 35 m3 Total 14 m2	2
Chlorination via concrete mixing tank	
r ripeine, raw water	
2002-2010 4.5 km	
2010-2020 1.5 km	+
G Distribution pipes	
Diameter 2002-2010 2010-2020 :	
25-05 mm 6.5 km 1 km	
150-200 mm 3 km km	
I Public taps Type : 1 Numbers: 9	
V Notes	
1000	

GE 1		51	KLAD SI	HE	EIO	N P	LAN AN	ND	DESIGN	FO	OR RUI	RA	AL WA	TF	ER SUPPLY	Y						
1	NERAL IN	FORMATIO	N	П																		
-	Province	(7		++				:	Dak Lak	(7	0.0.1		E V									
3	Name of C System Nu	ommune/1ow mber	'n					:	Ea Drong	;(/,	, 8, 9, 1	υ,	Ea Kng	5)								
4	Water Res	ource		tt				:	Deep We	11	(preser	nt	number	of	wells	0						
			1								u.						1					
W	ATER SUPI	PLY PLAN		\square																		
A	Plan Parar	neter (1)		+					Derror	1												
1)	Water reso	UCA deen v	المع	-	Flm				Deep we	1												
2)	Latitude	JICA deep v		1	N			:							Longitude	Е						
3)	Permissible	e yield /well		1	m3/da	ıy (1/	(sec)	:	268	(3.1)	(estima	ite))							
4)	Static wat	er surface lev	el (S.W.L)) 1	m			:														
5)	Dynamic v	vater level (D	.W.L)	1	m			:														
6)	Well diamo	eter		1	mm			:	150			_	2010				2020					
- 2	Water den	household		-	Numh	erc			431				2010				2020					
2)	Population	nouscholu			Numb	bers		:	1827				2034				2291					
3)	Per capita	consumption		1	1/s/d			:	30				60				60					
4)	Maximum	daily water d	emand	1	m3/da	ıy		:	21				162				214					
5)	Maximum	hourly water	demand		m3/ho	our			2				14				18	1	<u> </u>		m · *	
3 P	Required r	umber of we	u		no.			:	1				0				0	-	+	-	rotal	-
1 1	r ian Parar Water into	ke		+					Deen We	11		Η				-		-		+		-
2	Raw water	 transmission		$^{+}$:	GI Pipes			H						-	1	+		-
3	Reservoir			Ħ				:	Ground F	lese	ervoir w	/itl	h eleva	ted	tower			l		1		1
4	Water trea	tment		Д			-	:	Aeration	and	Slow f	ĩlt	tration.	Ch	lorination,						-	
5	Water dist	ribution		$\downarrow\downarrow$	\rightarrow			:	PVC, PE	, Pu	blic tap	os,	house	cor	nnection					1		
0	rower sup	рту		++				ŀ	r uone N	ει ai	nu/or ge	;n	crator/s	oia	u system	-		╞				-
				11				<u> </u>										_		<u> </u>		
I W	ATER SUPI	PLY FACILI	TY DESIG	GN																1		1
				П	-																	
Α	Source	Deep Wells																				
_		Numbers ne	eded in ad	diti	on to	ex1s	ting Jica	we	11			_										
		2002-2010	1																			
-		2010-2020	0	++																		
В	Pump	Submersibl	e pump	1	-								Booste	r p	oumps					1		
		Yield: 3.1	1 1/s			Lift	80	m							Yield				Lif	t		
_		2002-2010	1	no)S								2002-20	$\frac{10}{20}$	14		m3/hr		15	m		
-		2010-2020	0	no)S								2010-20)20	4		m3/hr		15	m		
С	Power	Public net.	alternativ	elv																		
		Diesel gener	rator	ŤŤ																		
		Solar system	ns																			
_																						
D	Reservoir	Ground res	ervoir		-								Elevat	ed	Tower		2					
		2002-2010	48	m.	3								2002-20	$\frac{110}{120}$	2	m	3					
-		Total	63	m	3								Total	120	9	m	3					
1				Π				1										F		1		
Е	Treatment	Aeration, S	low sand f	filtı	ration	and	d chlorin	iati	on.													
+		Aeration ar	ea:	μ,	,			Re	eaction ta	nk,	volum	e:			Filter area	:	-	6		-		_
+		2002-2010	1	m	2			20	10-2020	2	m3				2002-2010	:	2	m	12	-		-
+		Total	2	m	2			∠0 Te	tal	2 9	m3	H			Total	•	4	m	12	1		-
+		Chlorination	via concr	ete	mixir	ng ta	nk	1.0		ŕ								1	i.	1		
				\square	[L				
F	Pipeline, ra	w water		Щ		[
		Diameter	100	m	m			-				Ц						-		-		-
1		2002-2010	1.5	kn kn	n			-										╞		-		-
+		2010-2020						-				Η				-		┢	1	+		1
_		n nines		Ħ				L										L		L		
G	Distributio	ii pipes			2002-2	2010)		2010-202	0								:				
G	Distributio	Diameter	1	1 1	10	km		-	3	km 1c	L								<u> </u>	-		-
G	Distributio	Diameter 25-65 mi	m	++	~			1	05	кm						1	1	1		1		1
G	Distributio	Diameter 25-65 mi 75-125 mi	m n n	+	5	km			0.5	km												
G	Distributio	Diameter 25-65 mi 75-125 mi 150-200 mi	n n n	+	5	km km			0.5	km												
G	Distributio	Diameter 25-65 mi 75-125 mi 150-200 mi	n m m n		5	km km			0.0	km												
G	Distributio	Diameter 25-65 mi 75-125 mi 150-200 mi	m m m m 1		5	km km	Number	:	6	km												
G	Distributio	Diameter 25-65 75-125 150-200 Type	m m m 1		5	km km	Number		6	km												
G	Distributio	Diameter 25-65 75-125 150-200 Type	m m m 1		5	km km	Number	٤:	6	km												
G G I I	Distributio Public taps tes No deep we	Diameter 25-65 mm 75-125 mm 150-200 mm Type :	m m n 1		5	km km	Number		6	km												

				SPI	READ SH	IF	ET O	N P	'LAN AF	ND.	DESIGN	FU	OR RUR	AL W	AT1	ER SUPPLY	l						
I	GE	NERAL IN	FORMAT	TON	N																		
	1	Province								:	Dak Lak												
	2	Name of C	ommune/T	lowi	1					:	Ea Wer (Nos	s. 3, 4, 5,	6, 7, 8	, A,	B, Ha Bac,	Ce	enter Dist	ric	:t)			
_	3	System Nu	mber							:	D 5-1 Deen We	.11	(freella	1						
	4	water Kes	ource							÷	Deep we	:11	present	numo	10	I wells	1						
II	WA	TER SUP	PLY PLAN	I		Π									1					1			
	Α	Plan Parar	neter (1)																				
	1	Water reso	ource								Deep we	11											
	1)	Altitude of	JICA dee	p w	ell		El m			:	255					x		010(07					
	2)	Latitude					N		()	:	1418900		27			Longitude	E	813607					
	3)	Static wat	e yleid /we	II lovo	(SWI)		m3/da	ay (I	/sec)	:	320	(3.7)										
	4) 5)	Dynamic v	vater level		W L)	<u>'</u>	m			•	24												
	6)	Well diam	eter	(2.			mm			:	150												
	2	Water den	and								2001			2010				2020					
	1)	Number of	household	1			Numb	bers		:	963												
	2)	Population					Numb	pers		:	5491			8920)			10981					
	3)	Per capita	consumpti	on r do	mand		$\frac{1/s}{d}$:	30			710)			60					
	4)	Maximum	hourly wate	ter (linanu		m3/h	iy hur		•	5			50	,)			1028		<u> </u>			
	3	Required r	umber of	well	acmanu	H	no.	<i>.</i>		:	1	-		2	2		-	1	\vdash			Total	4
	В	Plan Paran	neter (2)							ŕ				1	1			-	F				· ·
	1	Water inta	ke							:	Deep We	11											
	2	Raw water	transmiss	ion		Π			-	:	GI Pipes								Γ				
	3	Reservoir								:	Ground I	Rese	ervoir wi	th elev	ated	tower			L	<u> </u>	Щ		<u> </u>
	4	Water trea	tment							:	Aeration	and	Slow fi	ltration	. Cł	ilorination,							
	5	Power sun	nlv							•	Public N	, Pu et	ione taps	s, nouse	COI	Intection				<u> </u>			
	v	rower sup								•	i uone iv												
									1														
III	WA	ATER SUP	PLY FACI	LIT	Y DESIC	GN																	
		~																					
	A	Source	Deep We	ells	مما نه مرا	4.4	ion to		tina lisa		.11												
			2002 201	nee			1011 10	exis	sting fica	we													
			2010-202	20	1																		
	B	Pump	Submers	sible	pump									Boost	er j	pumps							
			Yield:	3.7	1/s			Lif	40	m						Yield				Lift			
			2002-201	10	3	n	os							2002-2	2010	59		m3/hr		15	m		
			2010-202	20	I	ne	os							2010-2	2020	. 27		m3/hr		15	m		
_	С	Power	Public n	et																			
	Č	10.001	i uone n																				
	D	Reservoir	Ground	rese	rvoir									Eleva	ted	Tower							
			2002-201	10	207	m	13							2002-2	2010	30	m	13					
			2010-202 Total	20	200	m	13							2010-2	2020	. 13	m	13					
			Total		300		15							Total		43	111	15		<u> </u>			
	Е	Treatment	Aeration	, Slo	ow sand f	filt	ratio	ı an	d chloriı	ati	ion.	-			+	-	⊢						
			Aeration	are	a:	Π				Re	eaction ta	nk,	volume	:		Filter area	:		F				
			2002-201	0	6	m	12			20	02-2010	30	m3			2002-2010	:	11	m	12			
		-	2010-202	20	3	m	12			20	10-2020	13	m3			2010-2020	:	7	m	12	Ц		
			Total	tion	9	m	12	na t	ml	To	otal	43	m3	-	-	Total	-	18	m	12			
			Cinorina	non	via concre	cie	/ 1111X11	ng ti	uik	\vdash		-		-	+		-		\vdash	<u> </u>	\square		
	F	Pipeline, r	w water			\parallel				\vdash		-			+	1	F			<u> </u>			-
	-	- <u>r</u> , i	Diameter		100	m	m							1			-						
			2002-201	0	4.5	kı	m			L									L				
			2010-202	20	1.5	kı	m														Ц		<u> </u>
	C	Distribut				Н				-		-		-	-	l	_		-	<u> </u>			
	G	Distributio	n pipes			$\left \right $	2002	201	0	-	2010-202	20		-	+		-			<u> </u>			-
			25-65	mm	1	Η	15.5	km		-	2.8	km	<u> </u>	-	-		-		ŀ				
	-		75-125	mm	1	Ħ	5	km		1	2.6	km	1		1		F		t				
			150-200	mm	1	Π	4.5	km				km	ı										
																	L						
						Ц																	
	1	Public taps	Туре	:	1				Number	! :	11	<u> </u>			-		-			<u> </u>			
			1							<u> </u>	1				1	1	L	1	L	<u> </u>			I
-	NT .	has																					
IV	N01	es																					
IV	NO	les																					
IV	NO	les																					

				SPI	KEAD SI	шĿ																	
(E	NFRAL IN	FORMAT	ION	J	ПТ		1				1	<u> </u>						1		—		
-	1	Province	UT OKMAI							:	Dak Lak								-				
	2	Name of C	ommune/T	`owr	1					:	Kien Du	c (C	ommuni	ities N	os.1	, 2, 3, 4, 5, 6,	7	, 8)					
	3	System Nu	mber							:	D 6-1												
	4	Water Res	ource							:	Deep We	ell	(presen	t num	ber o	of wells	1						
		TED OVD		×		T T				1	1	1		-	-				1				1
	V A	TER SUP	PLY PLAN																_		_		
P	1	Water res	neter (1)								Deen we	11									_		
+	1)	Altitude of	IICA dee	n w	ell		Fl m				691	11											
	2)	Latitude	bientuce	P	cii		N			:	1325577					Longitude	E	772292					
	3)	Permissibl	e vield /we	1			m3/da	ay (1	/sec)	:	259	(3) (See	note	20181000	_						
	4)	Static wat	er surface	level	l (S.W.L))	m			:	32		ĺ										
	5)	Dynamic	water level	(D.)	W.L)		m			:	54												
	6)	Well diam	eter				mm			:	150												
	2	Water den	nand				NT 1				2001			201	0			2020			_		
_	1)	Number of	nousenoic	1			Numt	bers		:	1492			106	10			12071			+-'		
-	2) 3)	Population	consumnti	on			1/s/d	Jers			30			100	50			130/1			+		
	3) 4)	Maximum	daily wate	r de	mand		m3/dz	av		•	103			8	15			1223					
	5)	Maximum	hourly wa	ter d	lemand		m3/ho	our			9				71			102					
1	3́	Required	umber of	well		\square	no.			:	1				3			1	T		T	Total	
F	3	Plan Para	neter (2)																				
	1	Water inta	ike			μŢ				:	Deep We	ell									F		
	2	Raw water	• transmiss	ion		$\left \right $:	Gi Pipes		L						L	-	+		
+	5	Keservoir	tmont			+				:	Ground I	kes	ervoir			-	-		-		+		-
+	+ 5	Water dief	ribution			+		-			PVC PF	uon Pi	hlic tan	s hou	se co	nnection	-		-	+	+		-
+	6	Power sun	ply			+		-		:	Public N	et	ione tap	, nou	,	meenon	-		┢		+		-
+	-		• ř			††				f				1					t		+		1
IV	VA	TER SUP	PLY FACI	LIT	Y DESIC	GN	[
														_							_		
A	۱	Source	Deep We	ells	1 1 . 1	114					11												
			Numbers	nee	ded in ad	diti	ion to	exis	sting Jica	ı we													
+			2002-201	20	5	+															+		
+			2010-202	.0	1																		
E	3	Pump	Submers	ible	pump																		
			Yield:	3.0	1/s			Lif	120	m													
			2002-201	0	4	no	os																
			2010-202	20	1	no	os																
(2	Power	Public n	et																			
-																					_		
r)	Reservoir	Ground	rese	rvoir																		
T		iteser von	2002-201	0	282	m	3																
			2010-202	20	126	m	3																
T	-		Total		408	m	3																
													L T						L		\perp		
F	Ľ.	Treatment	Aeration	, Slo	ow sand f	tilt	ratior	n an	a chlori	nat	ion		rele	<u> </u>		Filtor			-		+		-
+	_		2002 201	are	a: 7		2	-		20	eaction ta	11K,	voiume	:	_	2002 2010	:	1.4		12	+		-
+			2010-202	20	3	m	2			20)10-2020	16	m3	-	+	2010-2020	•	14	m	12	+		-
+			Total		10	m	2			To	otal	51	m3			Total	ŀ	21	n	12	+		1
+			Chlorina	tion	via concr	ete	mixi	ng ta	ank	1		1							1	-	+		1
						Π		Ľ		L		L					L		L				
F	1	Pipeline, r	aw water				-																
_			Diameter		100	m	m			-			\vdash				-				\perp		
_			2002-201	0	6	kr	n	-		-			├				-				+		-
+	-		2010-202	.0	1.5	KI	ш			+	-	-	+ +		_		-		┝		+		+
1	2	Distributio	n nines			+				\vdash	-	-		-	+		-		┢	-	+		+
╞	-	2 ISCI IDUCI	Diameter			+	2002-	201	0	\vdash	2010-202	20				1	-		:	1	+		+
T			25-65	mm	1	\square	15.5	km		\square	8.5	km	i						Ē		T		\mathbf{T}
			75-125	mm	1		8.5	km			1.5	kn	1						L				
Г			150-200	mm	1	П	0.5	km				kn	1						[
_						μĮ							L T						L		\perp		
		D 11	T					<u> </u>	NT 1												\perp		
		Public tane	Туре	1:	1				Number	19 :	10	<u> </u>									+		-
I		i uone taps				1 1										1							
Ι		r uone taps																					
I	lo*	r uone taps																					
I	Not	es The well or	onstucted d	ırino	the first	ph	ase of	f the	study or	ılv '	vielded 0	4 1/9	s. howev	erah	ighei	vield has be	en	used for	de	esign nu	rpos	es	

				SPI	READ SH	HE I	ет о	N P	LAN AN	D	DESIGN	FC	OR RUR	AL	WA	TF	ER SUPPLY	Z						
I	GE	NERAL IN	FORMAT	TION	Ň																			
	1	Province		-						:	Dak Lak		C		N	- 1	71 K 2 K 2	T	ZA V. E	V				
	2	Name of C System Nu	ommune/ I	lowi	n					:	Krong M	lar (Commu	nities	s No	s.r	KI, K 2, K 3	, r	Κ 4, Κ 5,	ĸ	6, K /, K	. 8)		
	4	Water Res								•	Deen We	-11	(presen	t nun	iher	of	wells	1		-				
	-	water nes	ource	- I						·	Beep in		presen	t man	1001	01	wens	1		1	1			
II	WA	TER SUPI	PLY PLAN	Ň		П														Γ		1		
	Α	Plan Parar	neter (1)																					
	1	Water reso	ource								Deep we	11												
	1)	Altitude of	JICA dee	ep w	ell	1	El m			:	436													
	2)	Latitude				1	N	(1		:	1384752						Longitude	E	210996					
	3)	Permissible	e yield /we	1		1	n3/da	ay (I	/sec)	:	553	(6.4)										
	4)	Static wat	er surface	leve	$\frac{1}{(S.W.L)}$		n			:	4													
	5)	Woll diam	water level	ц р .	w.L)	1	11 mm			•	11			_										
	2	Water dem	and				11111			•	2001			20	10				2020					
	1)	Number of	'householo	d		1	Numł	bers		:	1169			20	10				2020					
	2)	Population		Ĩ		1	Numl	bers		:	5930			74	184				9123					
	3)	Per capita	consumpti	ion		1	/s/d			:	30				60				60					
	4)	Maximum	daily wate	er de	mand	I	m3/da	ay		:	69			5	595				854					
	5)	Maximum	hourly wa	ter d	demand	1	n3/ho	our			6				50				71					
	3	Required r	number of	well		1	10.			:	1				1	_			0				Total	2
	B	Plan Parar	neter (2)			Ц						L								_				
	1	Water inta	ke	Ļ		\square				:	Deep We	ell										1		
	2	Raw water	transmiss	sion		\square				:	GI Pipes		L							-		-		
	3	Reservoir Water tree	tmant							:	Chloring	kese	ervoir											
\vdash	4	water trea Water dist	ribution			+				•	PVC PF	uon pr	, ihlic tan	s her	156.0	01	nection			╞		+		-
<u> </u>	6	Power sun	ply			+				:	Public N	et	ione tap	5, 1101		.01	meenom			┢	-	+		
	-		r-v							-		1												
			1			<u></u>			1	۰				-						1	1	<u> </u>		
Ш	WA	TER SUPI	PLY FACI	LIT	Y DESIC	ΞN												_		L				
						IT	-									_		_		[1		
	A	Source	Deep W	ells																_				
L			Numbers	s nee	ded in add	diti	on to	exis	sting Jica	we	1			-						L		1		
			2002-20	10	1	\parallel				<u> </u>		<u> </u>								-				
⊨			2010-202	20	0	+				-		-		_						-		-		
⊢	B	Pump	Submer	sible	numn	+				-		-		Ree	oste	r r	umne			╞		1		-
	0	rump	Yield.	64	1/s			Lif	45	m					USIC:	. 1	Yield				Lif	H		
			2002-20	10	2	no	s	2						200	2-20	010	50		m3/hr	Г	60	m		
			2010-202	20	0	no	s							201	0-20	020	21		m3/hr		60	m		
	С	Power	Public n	et																				
	_		~ .																					
	D	Reservoir	Ground	rese	rvoir		<u>,</u>																	
			2002-20	10	199	m.	5																	
			2010-202 Total	20	285	m	2							_										
			Total		285	111.)																	
	Е	Treatment	Aeration	ı, Slo	ow sand f	iltr	atio	ı an	d chloriı	ati	on.	1								┢		1		
			Aeration	1 are	a:					Re	action ta	nk,	volume	:			Filter area	:		t		1		
			2002-20	10	5	mź	2			20	02-2010	25	m3				2002-2010	:	9	n	n2	L		
			2010-202	20	3	mź	2			20	10-2020	11	m3			_	2010-2020	:	6	n	12	1		
			Total		7	mź	2			To	tal	36	m3		[Total		15	n	12			
			Chlorina	tion	via concre	ete	mixi	ng ta	ank											L		-		
	F	Din alt				\parallel				-				-						L		-		
	r	r ipeline, ra	aw water		100		m			⊢		-								╞		$\left - \right $		
		-	2002 201	10	100	hui ku	ш 1			-		-		-						\vdash		+		
			2002-20	20	1.5	kn kn	1			-				-						╞		-		
-		-								\vdash		1								┢	1	1		
	G	Distributio	n pipes			Ħ														t	1	1		
			Diameter	r		2	2002-	201	0		2010-202	20								:	1	1		
			25-65	mm	1		13	km		L	5	km	l I							L				
			75-125	mm	1	П	5	km			3	km	1			_								
			150-200	mm	1	\prod	1	km				km	ı T							Ĺ				
						\prod														L				
	r	D 11	Ŧ			\square			NT 1	_		<u> </u>										1		
	1	Public taps	Type	1	1	\square			Number	:	10									-		-		
L						Ш		I		L		<u> </u>												
W	Not	66																						
1 V	1101	One existin	g well will	he ii	ncluded in	ן th	e svs	ten																
I		Existing ne	twork will	be in	ncluded in	the	e syst	en																
		· · · · ·				-																		
		U																						

Standard Designs of Water Supply Facilities



















9

ς

Þ

٤

2

I

3

a

6

8

L







н

D

ł

9

σ

Э

В

۲



























Typical Elevated Tower S : 1/100

hlands in the s	Socialist Repu	blic of Viet Nam
Typical E	levated	ſower
Scale. 1:100	For A3 size	Draw.No.
	thlands in the S Typical E Scale. 1:100	thlands in the Socialist Repu Typical Elevated Scale. 1:100 For A3 size

CAD File : ElevatedTower.dwg

G

٨

C

n

CAD File : Typ_Thrustblock.dwg

....

ъ

C

5

Ð

>

CAD File : Typ_Thrustblock.dwg ы бл С 0.33 0.71 1.25 1.90 **Table of Dimensions for Vertical Bend** Н (% тĘ ğ 8 1100 1250 300 906 oncrete **Plan of Vertical Bend** (Lun 150 150 g 250 260 Δ 330 2 Section 3 (mm) 150 **6** C B **4**20 550 Clamp шĘ 1100 300 lõn 700 006 1250 (mm) 900 1100 1250 300 200 300 ∢ w 8x8.0 ND MU 9 150 250 200 83 80 Na 4 VOLUME 0.08 0.2 0.3 ٣E **Table of Dimensions for Reducer** Ξ 300 200 700 B 800 s **Plan of Reducer** ပဋိ 350 150 150 550 Section2 шÊ 150 250 150 260 m 270 (E E 250 250 500 200 800 ≺ 6 DN1×ND2 250×200 150×100 200×150 (ттхтт) 100×80 80×65 7 a VOLUM 0.06 0.45 0.90 ٣E шÊ • **Table of Dimensions for Tees** 100 ē 100 150 200 250 Plan of Tee 150 150 30 300 350 250 Section 4 ပဋိ Concrete 202 ş 1000 800 600 ۵Ê 9 300 B 400 600 1000 800 ۲ (m 300 ß 1000 400 600 800 N <u>1</u>0 150 200 250 65 8 ō The Study on Groundwater Development in the Rural Provinces of Central Highlands in the Socialist Republic of Viet Nam Draw.Name Thrust Blocks 2/2 Ξ Date. Nov. 2001 Scale. NTS For A3 size Draw.No. JAPAN INTERNATIONAL COOPERATION AGENCY (JICA) Date. Nov. 2001 Scale. NTS Ξ ດ ۰τ, 5 D c 8 >

Technical Specifications

TECHNICAL SPECIFICATIONS

1. Introduction

This appendix includes detailed specification for delivery and installation of all relevant items of implementation of a piped network. The specifications include delivery and laying of Pipes (GI, PVC, PE, valves, airvalves, checkvalves and water meters).

The structure can be directly used for preparation of specifications in detailed design. British Standard has been used as basis everywhere.

2. Galvanised Steel Pipes

<u>Standards</u>

Except where otherwise stated, steel pipes and fittings shall be in accordance with BS 534: Specification for Steel Pipes and Specials for Water and Sewage, BS 3601: Carbon Steel Pipes and Tubes with specified Room Temperature Properties for Pressure Purposes and BS 1387: Screwed and Socketed Steel Tubes and Tubulars and for Plain End Steel Tubes suitable for Welding or for Screwing to BS 21 Pipe Threads.

<u>Joints</u>

Mild steel pipes and fittings shall be supplied with either flanged ends or with screwed ends as specified in the Bill of Quantities.

Flange joints shall have flanges in accordance with BS 4504: Section 3.1. Where pipes supplied with flanges welded on, the welding shall be carried out in accordance with BS 2633 or BS 2971.

Bolts and nuts, unless otherwise specified shall be of mild steel and the dimensions and finish shall comply with BS 3692. Where washers are used they shall be of appropriate material and the dimensions shall comply with BS 4320.

Where screwed ends are specified each pipe shall be supplied complete with one screwed socket to complete the joint.

Gaskets for flange joints shall be of the inside-bolt-circle type. The dimensions of gaskets shall comply with BS 4865:Part 1. Gaskets shall be manufactured from material complying with the provisions of BS 2494 for type W rings.

Tests

Before dispatch all mild steel straight pipes shall be hydraulically tested by the manufacturer in accordance with BS 3601.

The hydraulic test pressure to be applied to each straight pipe shall be the greater of:

- either the pressure given in the relevant clause of Section 2 of BS 3601, or
- . twice the specified working pressure.

Hydraulic testing of mild steel fittings and specials shall be carried out in accordance with the recommendations given in Clause 4.3 of BS 534.

<u>Galvanising</u>

After being tested the mild steel pipes and fittings shall be galvanised in accordance with BS 729.

3. Screwed Stainless Steel Pipes

Screwed stainless steel pipes shall comply with BS 6362: Specification for Stainless Steel Tubes suitable for Screwing in accordance with BS 21 Pipe Threads for Tubes and Fittings where pressure tight Joints are made on the Threads.

Welded Stainless Steel Tubes

Welded stainless steel tubes shall comply with BS 6323, Part 8: Specific Requirements for longitudinally welded Stainless Steel Tubes. Welded pipe joints shall be welded in compliance with the procedures laid down in BS 4870 and welders shall be approval tested in accordance with BS 4871 and BS 4872.

4. Unplasticized Polyvinyl Chloride (PVC) Pipes

PVC Pipes for Pressure Applications

PVC pipes for pressure applications shall comply with BS 3505. Joints and fittings to be used therewith shall comply with BS 4346. The pressure classification shall be PN 6 or PN 10.

All buried pipes shall, unless otherwise indicated, be of the socket and spigot type made water tight in the joints by the compression of rubber gaskets with two thick lips.

Common Requirements

PVC pipes shall be capable of withstanding ultraviolet degradation. A rodent inhibitor shall be incorporated in the material of the pipe.

The Contractor shall advise the manufacturer of the climatic and conditions at the site of the Works and shall seek his advice on the storage of PVC materials on site. Subject to the Engineer's approval this advice shall be followed at all times.

Except in the case of flanged joints and where otherwise specified or approved by the Engineer pipe joints shall be flexible and sealed with a rubber gasket to the approval of the Engineer and shall withstand the various tests specified in the applicable standards. For pipes with a nominal bore of 100 to 230 mm the joints shall be capable of withstanding a deflection of not less than 3.0 degrees in any direction and for pipes with a nominal bore of 250 to 330 mm a deflection of 2.0 degrees and from 350 to 600 mm 0.5 degrees in any direction. All pipes shall be capable of

withstanding a "draw" of 13 mm over and above the initial jointing allowance. The initial jointing allowance is the gap measured parallel to the centre line of the pipeline and shall not be less than 6 mm or more than 13 mm or as otherwise recommended by the pipe manufacturer and approved by the Engineer. Pipes and fittings shall be indelibly marked prior to laying to indicate the correct initial jointing allowance.

Gaskets

The gasket consists of a substantial body, which is extended into two thick lips directed towards the bottom of the socket. The outer edge of the body is formed into an annular shoulder, which fits into the recess in the socket of the pipe. The side of the shoulder facing the entry to the socket is given a chamfer, which centres the gasket in its recess.

The standard gaskets are moulded components with a generous cross section providing large seating surfaces to ensure fluid tightness and a substantial reserve of elasticity. The maximum continuous working temperature for these gaskets is 70°C. In natural rubber or an equivalent elastomer the gaskets for ND 60 to 600 are homogenous. It is preferred that the gaskets are factory fitted and factory lubricated with a long lasting silicone lubricant. The pipes must be supplied with end caps protecting the pipes effectively against fouling etc.

The storage conditions for the gaskets are:

• the storage temperature should be between $+5^{\circ}$ C and $+25^{\circ}$ C. They should be brought to about 30° C for long enough to give them their original flexibility before being used.

• for vulcanised elastomer based products the following should be avoided:

- storage in a too damp or too dry atmosphere;
- direct sunlight or high ultra-violet artificial light;
- protect them from ambient air and the especially harmful effects of ozone.

Transport and Storage

During transport pipes shall be solidly supported under the lower layer and at the sides to prevent accidental damage. Storage on site shall be on a raised floor with support frames so that the sockets do not touch the ground and shall be head to tail to prevent pipes from resting on the sockets.

Pipes must at all times during storage be protected against ultraviolet light from the sun.

Excavation of Trenches

Trenches shall be excavated to the width and the depth and to the lines shown on the drawings and in conformity with Clause "Earthworks". The earth cover shall be at least 0.6 metre above the pipeline and must never exceed 3 metres.

Laying PVC Pipes

The Contractor shall submit for the Engineer's approval the pipe manufacturer's complete and detailed recommendations for the handling and installation of pipes and fittings in open trench.

Flexible pipes shall be laid and bedded in approved granular materials except where concrete protection is required. The particle size in the material must not exceed 16 mm and the contents of particles between 8 and 16 mm must not exceed 10 %. Sharp stones or crushed material must not be present in the material. The granular material shall be placed over the full with of the bottom of

the trench and shall extend from a level 150 mm below the underside of the sockets or couplings on the pipeline to 200 mm above the crown of the pipes for the full length of the pipeline. The granular material shall be carefully compacted by hand.

Jointing of PVC Pipes

Before lowering the pipes into the trench the Contractor shall carefully inspect and clean the pipe to ensure that it has been freed for all foreign matter.

The alignment of the pipe in the trench shall be controlled by level instrument or boning rods to ensure correct levels. For water supply the minimum slope shall be 1 mm per metre pipe length. No zero slope will be tolerated.

The Contractor shall keep the interior of pipes clean and free from water, dirt, stones and other foreign matter as laying proceeds, and at the end of the day's work or at other times when installation work is not proceeding the open ends of the pipes shall be sealed off by a suitable stopper or end cap. The Contractor shall take such precautions as are necessary to prevent pipes from floating.

The pipes shall be laid in straight lines both in horizontal and vertical planes. Changes of direction of less than $111/4^{\circ}$ shall be obtained by deviating the pipes after jointing at one or more joints. The angle of deviation at each joint depends on the ND and has the maximum values as stated above.

General Jointing Procedure

The general jointing procedure may vary with the type of joint used, but the basic requirements are:

- . Overall cleanliness
- . Correct positioning of the components
- . Correct centring of the spigot in the socket
- . Strict observance of the manufacturer's assembling instructions.

The Contractor shall be responsible for providing the necessary copies of these instructions.

The general jointing procedure is as follows:

The spigot end of the pipe P2 to be laid must penetrate into the socket of the waiting pipe P1 fitted with its gasket.

- Check that the spigot end of pipe P2 is correctly chamfered.
- Carefully clean with a rag:
 - the interior of the socket and especially the seat of the gasket of pipe P1, if the gasket is not factory fitted;
 - the spigot of the pipe P2 to be assembled;
 - the gasket, if not.
- Fit the gasket, if not factory fitted, in the socket of pipe P1 with the lips pointing towards the bottom of the socket. Check that it is correctly and uniformly placed in its groove.
- Mark the normal jointing depth on the spigot of pipe P2 on each side of the pipe. These marks should be the depth of the socket less 10 mm away from the end of the pipe.
- Lower the pipe P2 into the trench carefully. Hold the spigot about 400 mm from the socket of the pipe P1.

Bring the spigot of pipe P2 into the socket of pipe P1 to the level of the gasket. Coat with lubricant:

- the inside of the gasket;

•

- the spigot of pipe P2 up to about 25 mm from the two socketing depth marks. Push in place to the socketing depth marks manually.
- If difficult to push the pipe in place a special jointing tackle with looped slings fitted around the two pipes can be used.
- Note: It shall not be permitted to use the arm of an excavator to push the pipe P2 into position in the socket of pipe P1.
- . If the pipeline lay out indicates a deviation this may now be made.
- . The levelling layer must be checked to make sure the pipes are evenly supported.
- The side fill to the pipe must provide adequate support with the approved granular material in order to keep it centred in the socket. Stamping with the foot is recommended.
 - The top layer of 200 mm to be equally filled and stamped with the foot.

Polyethylene Pipes

Polyethylene pipes for cold water services shall comply with ISO 161:1988 and ISO 4065:1978 pressure class PN 6 and PN 10. Fittings shall be compression type fittings to BS 864 Part 3.

Nominal diameters are 20, 25, 32, 50, 63 and 80mm.

Polyethylene piping shall be laid in accordance with the pipe manufacturer's recommendations as described under PVC pipes above and to the approval of the Engineer.

5. CHAMBERS

General

Chambers shall be constructed on water supply pipelines in the positions indicated at the drawings and to house valves and flow meters. Generally all chambers shall be constructed in either precast or in-situ concrete made with Portland cement to the standard details shown on the Drawings.

Construction details

Precast concrete chamber sections shall be constructed with slabs aligned correctly. Joints shall be made so that the required jointing material fills the joint cavity. Any surplus jointing material extruded inside the chamber shall be trimmed off and joints shall be pointed on completion.

Bases for chambers shall be constructed to the standard details shown on the Drawings. Valves and other accessories shall be seated, embedded and anchored to concrete plinths cast in concrete Class 20.

For valves with DN up to 200 mm surfaces boxes will be provided and shall be ductile iron Class A in accordance with BS 5834 and shall have a minimum clear opening of 380 x 230 mm. Surface boxes for other purposes shall be cast iron and shall comply with the relevant provisions of BS 5834 and be heavy duty grade A.

For large bulk flow meter chambers, covers shall be of raised pattern non-slip mild steel solid floor plates to BS 1449 Part 2, in Grade 43 steel and in the dimensions shown on the Drawings. Covers will be installed hinged as shown on the Drawings and shall be holed for and supplied with standard lifting keys. Steel plates for this purpose shall be hot dip galvanised in accordance with the relevant provisions of BS 729 and will be minimum 4,5 mm thick.

Covers and frames with minimum clear openings outside the ranges in BSEN 124 shall comply with the relevant provisions of that standard where applicable. All manhole covers shall have at least two closed keyways per complete cover. Keyways shall be at symmetrical points to enable lifting without tilting or jamming.

Chamber covers and frames shall in general be constructed flush with the final ground level. In cases where existing pipes are laid close to the surface and it is not possible to construct chambers with the top flush with final ground, the Contractor will propose for the Engineer's approval, the construction of the chamber to be partly above ground in order to accommodate the correct installation of valves etc.

The external surface of all chambers including roof slabs shall be protected with bituminous coating.

Chambers shall be substantially watertight, with no identifiable flow of water penetrating the permanent works. If there is any discernible flow of water entering the chamber at a point which can be located by visual inspection, the Contractor shall take such measures as are necessary to stop such infiltration. Plasticized PVC waterstops shall comply with the relevant provisions of WIs No. 4-31-02.

Pipework

All pipes and assembling parts selected under this Contract must be of first quality, truly circular, and of uniform thickness, free from scale, lamination, honeycombs and other defects, and shall be designed and suitable for the stated pressures and temperatures.

6. Gate Valves and accessories

The Contractor shall submit full details of valves with manufacturer's drawings to the Engineer and obtain his approval before manufacture is commenced. All valves shall be individually tested by the manufacturer for both strength and leakage.

Gate (sluice) valves shall comply with BS 5163:1986 unless otherwise stated in the Contract.

- i) Valves shall be Type B.
- ii) The pressure rating shall be PN10 unless stated otherwise in the Contract.
- iii) Gates shall be resilient faced up to 300mm diameter. Gates shall be cast iron to BS 1452 GR220 min or ductile iron to BS 2789 73 min.

For resilient faced gates the gate shall be entirely encapsulated with rubber to BS 2494 Type W. Nitrile/EPDM with a minimum 3mm of rubber in the seating area.

For metal seated gates both the body and the gate rings shall be gunmetal to BS 1400 LG2 and components shall be designed to provide adequate seating performance before and after wear of the seating surfaces.

iv) The body and bonnet shall be cast iron to BS 1452 GR 220 min or Ductile Iron to BS 2789 73 min.

v) Valve stems shall be threaded sufficiently to allow the gate to be raised clear of the nominal bore of the valve. Stem sealing shall be as detailed within the following table:

Diameter (mm)	Actuator or Gearbox	No Actuator or
	Fitted	Gearbox
50 - 150 inc	0 - seal	0 - seal
200 - 300 inc	Packed Gland	0 - seal
F0 1 D 11	· · · · · · · · · · · · · · · · · · ·	

[0 - seal = Double toroidal sealing ring to BS 2494 Type W]

Means shall be provided for resealing the stem under working conditions.

- vi) Valves shall be designed to pass potable water and raw water.
- vii) Valve caps shall be secured by hexagonal headed set screws.
- viii) Operating levers will be hand wheels for 100-300 mm valves.
- ix) Direction of closure shall be clockwise. The direction of closing shall be indicated by an arrow cast on the upper face of the gland or stem seal housing.
- x) Unless indicated otherwise on the Drawings valves will be used in the closed end application.
- xi) Valves with nominal diameter greater than 200 mm will be installed with ductile iron anchoring pipes on the upstream side as shown on the Drawings. Anchoring pipes will have 2 flanges and 1 puddle flange.

Gate valves shall be coated in accordance with WIS No 4-52-01 - Class A internally and Class B externally.

Extension spindles, support brackets and centring spiders shall be constructed of mild steel galvanised to BS729. Support brackets of the bolt-fixing type shall be provided to extension spindles at centres not exceeding 2 metres. The top bracket shall be located a maximum 300mm below the spindle cap, handwheel or top of wall as appropriate. Extension spindles in gate valve extension spindle chambers shall be provided with centring spiders at centres not exceeding 2 metres. The top spider shall be located a maximum 300mm below the spindle cap. All exposed universal joints shall be coated with a non-perishable material with an internal grease packing to allow flexible movement.

Check Valves

Check valves shall comply with BS 5153 unless otherwise stated in the Contract and shall be as follows:

- i) Nominal pressure shall be 10 bar (PN106) unless otherwise stated in the contract.
- ii) Valve to be swing type resilient seated.
- iii) Body ends to be flanged to BS 4504 PN10 unless otherwise stated in the contract.

Altitude valves (Level control valves)

Altitude valves shall be used to control the water level in public taps and treatment plants to avoid any overflow events. Altitude valves shall regulate the water flow in a reservoir by means of a float with 2 positions. The valve shall be able to close at a preset high water level, and open at a preset low water level. This function shall be governed by a hydraulic system connected to the float. The altitude valves shall be Danfoss C701 or similar approved.

7. Mechanical flow meters

Mechanical water meters shall comply with the relevant provisions of BS 5728 Class B or ISO 4064 and shall be of the in-line helical rotary type for bulk flows. They shall be designed on the basis of the expected flow rating.

Meter housings shall be of cast iron or other approved material and be double flanged to BS 4504. The meters must be capable of being installed in horizontal, vertical or inclined pipelines without loss of accuracy. The complete measurement mechanism shall be removable from the meter body and a blank cover to replace the working unit shall also be supplied. The design of the meter shall be such that water passes through it with negligible head loss and without restriction or change of direction.

Meters shall have straight reading counters registering total flow in cubic meters (m³). A centre sweep hand registering in litres (l) shall also be fitted.

Meters shall be suitable to a maximum working pressure of 16 bars and the Contractor shall ensure their suitability for use in the prevailing conditions.

Meter housings shall be coated by dipping or other equivalent means using a cold applied black bitumen material complying with BS 3416 and made from petroleum or asphaltic bitumen but not coal tar bitumen. No coating is to be applied to the casting until its surfaces are clean, dry and free from rust, oil and deleterious material.

The complete working mechanism of the meters shall be manufactured from materials offering maximum resistance to wear and corrosion.

8. Support of Pipework and Valves

All necessary supports including foundations, hangers, saddles, sliding shoes, slings, expansion pieces, fixing bolts, foundation bolts, fixing and anchor points and all other attachments shall be supplied to support the pipework and its associated equipment in an approved manner. Valves, meters, strainers and other devices mounted in the pipework shall be supported independently of the pipes to which they connect.

All brackets or other forms of support which can conveniently be so designed, shall be rigidly built up of steel sections by riveting or welding, in preference to the use of castings.

No point of passage of pipes through floors or walls shall be used as a point of support, except with the approval of the Engineer.

All brackets and fixings shall be hot-dip galvanised in accordance with BS 729.

pumped to the proposed injection point on the pipeline as shown in the Drawings. The chlorinator capacity shall be such that a free chlorine residual of at least 1 mg/l can be attained in the water after a minimum contact time of 20 minutes. This condition must be attainable even when maximum flow rates coincide with anticipated maximum chlorine demands. The equipment shall be of such design that it will operate accurately at both minimum and maximum flow rates without the use of standby equipment.

9. Dosing Pumps

In case dosing pumps are to be used for chlorination they shall fullfil the following conditions.

Dosing pumps shall be of the positive displacement reciprocating variable stroke type suitable for sodium hypochlorite solution dosing and shall be designed to. It shall be possible to regulate the pump output by adjustment of the pump stroke length or the stroke frequency. Pump housing shall be of composite material ensuring mechanical strength and chemical resistance. The pumps shall be able to draw from the solution storage tanks. Sealing materials shall be of Viton and the diaphragms of teflon. The proposed dosing pump will comply at least with the following requirements:

Max. suction lift:	3 m
Max. back pressure:	10 bar
Electrical connection:	230 V, 50 Hz
Max temperature of medium:	35° C

Pump capacity shall range between 3.5 l/h and 25 l/h.

The dosing pump shall be manual start and manual stop with automatic cut-out in the event of the feed tank being emptied.

The delivery lines shall also be provided with an overpressure cut-out to stop the pumps in the event of either the line becoming blocked or an attempt being made to run the pump against closed valve.

Comparison of Power Cost between Existing Line Use and Generator Driven Pump

COMPARISON OF POWER COST BETWEEN EXISTING LINE USE AND GENERATOR DRIVEN PUMP

1) Power cost with existing electricity line

The power cost is calculated on the following assumptions;

a) Pumping efficiency of 50%

b) Cost of electricity is 750 VND/kWh, and

Commune	Daily kWh	Monthly cost (VND/day)	Discharge rate (m^3/hr)	Cost per m ³ (VND/m ³)
K-1	30	686,250	3.6	318
K-2	54	1,235,250	6.1	338
G-3	220	5,032,500	13	524
D-4	110	2,516,250	13	323

The power cost is calculated for following 4 communes (systems);

2) Alternative power cost with generator-driven pump

The power cost is calculated on the following conditions;

- a) Cost of fuel is 4,000VND/hr
- b) Fuel consumption at 100% load is as followings;

Generator	Consumption(l/hr)	
30 kVA	7.2	
20 kVA	5.8	
15 kVA	4.6	
10.5kVA	3.3	

The power cost is calculated for following 4 communes;

Commune	Required power (kVA)	Fuel cost (VND/hr)	Discharge rate (m^3/hr)	$\frac{\text{Cost per m}^3}{(\text{VND/m}^3)}$
K-1	10.5	13,200	7.2	1,833
K-2	15	18,400	11.9	1,546
G-3	30	28,800	13.0	2,215
D-4	20	23,200	10.8	2,148

3) Conclusion

The power cost with using generator is around 6 times higher than with using electric line.