## Appendix 1

Standard Well Design (type-1 and type-2)


Type-1
(Solar pump + Generator pump system)


Type-2
(Public power pump system only)

| Example of Wells | Well Dimension |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | A: concrete base (size) | $\begin{aligned} & \text { B: Clay packing } \\ & \text { (m) } \end{aligned}$ | C: Installation depth of 12 " pipes for pump chamber of 2 pumps | D: Installation depth of $6^{\prime \prime}$ casing/screen pipes | E: Drilling depth (m) |
| K1-1 | $\left\{\begin{array}{l} \mathrm{L} x \mathrm{x} \text { W x D }=1.5 \\ \mathrm{x} 1.5 \times 1.0 \end{array}\right.$ | Average depth: around 10 m in general. Below the clay packing, gravel pack will be done | 42 | 120 | 125 |
| K2-3 |  |  | 32 | 120 | 125 |
| G-3 |  |  | 67 | 150 | 155 |
| D4-1 |  |  | 58 | 150 | 155 |
| D4-2 |  |  | 58 | 150 | 155 |
| Others |  |  | 40-60 | 50-150 | 55-155 |
| Note: 1 Exploratory wells of 17 of the first investigation shall be used for <br> production wells of stage- 1 implementation <br>  2 Remaining casing/screnn pipes in the first investigation stage will be used |  |  |  |  |  |

## Appendix 2

Spread Sheet on Plan and Design for Rural Water Supply

Appendix 2
SPREAD SHEET ON PLAN AND DESIGN FOR RURAL WATER SUPPLY



## IV Notes

A well yield of $346 \mathrm{~m} 3 /$ day is used for planning

## SPREAD SHEET ON PLAN AND DESIGN FOR RURAL WATER SUPPLY




## IV Notes

The well drilled by the study team only yielded $0,5 \mathrm{l} / \mathrm{s}$. For calculation purposes a yield of $216 \mathrm{~m} 3 /$ day has been used for future wells

## SPREAD SHEET ON PLAN AND DESIGN FOR RURAL WATER SUPPLY




## IV Notes

## SPREAD SHEET ON PLAN AND DESIGN FOR RURAL WATER SUPPLY




## IV Notes

No well was constructed in the area during the first phase of the study.

## SPREAD SHEET ON PLAN AND DESIGN FOR RURAL WATER SUPPLY





IV Notes

## SPREAD SHEET ON PLAN AND DESIGN FOR RURAL WATER SUPPLY




[^0]
## SPREAD SHEET ON PLAN AND DESIGN FOR RURAL WATER SUPPLY




IV Notes

## SPREAD SHEET ON PLAN AND DESIGN FOR RURAL WATER SUPPLY




## IV Notes

The well constucted during the first phase of the study only yielded $0,41 / \mathrm{s}$, however a higher yield has been used for design purposes Two existing wells will be included in the system. These reportedly give a yield of more than $31 / \mathrm{s}$ each

## SPREAD SHEET ON PLAN AND DESIGN FOR RURAL WATER SUPPLY




## IV Notes

One existing well will be included in the systen
Existing network will be included in the system

## Appendix 3

Standard Designs of Water Supply Facilities













CAD Filc : Gencral-LayoutPlan-K41.dwg



## Surface Water Intake

$S=1 / 100$


Section

















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Plan of Vertical Bend



Plan of Reducer


| Table of Dimensions for Reducer |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { DN } 1 \times N D 2 \\ & (m m \times m m) \end{aligned}$ | $\underset{(m m)}{A}$ | $\begin{gathered} B \\ (\mathrm{~mm}) \end{gathered}$ | $\underset{(\mathrm{mm})}{\mathrm{C}}$ | $\underset{(m m)}{\mathrm{H}}$ | $\begin{gathered} \text { VOLUME } \\ \mathrm{m}^{3} \end{gathered}$ |
| $80 \times 65$ | 250 | 150 | 150 | 300 |  |
| 100×80 | 250 | 150 | 150 | 300 |  |
| $150 \times 100$ | 500 | 250 | 350 | 500 | 0.08 |
| 200×150 | 700 | 260 | 450 | 700 | 0.2 |
| $250 \times 200$ | 800 | 270 | 550 | 800 | 0.3 |
|  |  |  |  |  |  |



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## Appendix 4

Technical Specifications

## TECHNICAL SPECIFICATIONS

## 1. Introduction

This appendix includes detailed specification for delivery and installation of all relevant items of implementation of a piped network. The specifications include delivery and laying of Pipes (GI, PVC, PE, valves, airvalves, checkvalves and water meters).

The structure can be directly used for preparation of specifications in detailed design. British Standard has been used as basis everywhere.

## 2. Galvanised Steel Pipes

## Standards

Except where otherwise stated, steel pipes and fittings shall be in accordance with BS 534: Specification for Steel Pipes and Specials for Water and Sewage, BS 3601: Carbon Steel Pipes and Tubes with specified Room Temperature Properties for Pressure Purposes and BS 1387: Screwed and Socketed Steel Tubes and Tubulars and for Plain End Steel Tubes suitable for Welding or for Screwing to BS 21 Pipe Threads.

## Joints

Mild steel pipes and fittings shall be supplied with either flanged ends or with screwed ends as specified in the Bill of Quantities.

Flange joints shall have flanges in accordance with BS 4504: Section 3.1. Where pipes supplied with flanges welded on, the welding shall be carried out in accordance with BS 2633 or BS 2971.

Bolts and nuts, unless otherwise specified shall be of mild steel and the dimensions and finish shall comply with BS 3692. Where washers are used they shall be of appropriate material and the dimensions shall comply with BS 4320.

Where screwed ends are specified each pipe shall be supplied complete with one screwed socket to complete the joint.

Gaskets for flange joints shall be of the inside-bolt-circle type. The dimensions of gaskets shall comply with BS 4865:Part 1. Gaskets shall be manufactured from material complying with the provisions of BS 2494 for type W rings.

## Tests

Before dispatch all mild steel straight pipes shall be hydraulically tested by the manufacturer in accordance with BS 3601.

The hydraulic test pressure to be applied to each straight pipe shall be the greater of:

- either the pressure given in the relevant clause of Section 2 of BS 3601, or
- twice the specified working pressure.

Hydraulic testing of mild steel fittings and specials shall be carried out in accordance with the recommendations given in Clause 4.3 of BS 534 .

## Galvanising

After being tested the mild steel pipes and fittings shall be galvanised in accordance with BS 729.

## 3. Screwed Stainless Steel Pipes

Screwed stainless steel pipes shall comply with BS 6362: Specification for Stainless Steel Tubes suitable for Screwing in accordance with BS 21 Pipe Threads for Tubes and Fittings where pressure tight Joints are made on the Threads.

## Welded Stainless Steel Tubes

Welded stainless steel tubes shall comply with BS 6323, Part 8: Specific Requirements for longitudinally welded Stainless Steel Tubes. Welded pipe joints shall be welded in compliance with the procedures laid down in BS 4870 and welders shall be approval tested in accordance with BS 4871 and BS 4872.

## 4. Unplasticized Polyvinyl Chloride (PVC) Pipes

## PVC Pipes for Pressure Applications

PVC pipes for pressure applications shall comply with BS 3505. Joints and fittings to be used therewith shall comply with BS 4346. The pressure classification shall be PN 6 or PN 10.

All buried pipes shall, unless otherwise indicated, be of the socket and spigot type made water tight in the joints by the compression of rubber gaskets with two thick lips.

## Common Requirements

PVC pipes shall be capable of withstanding ultraviolet degradation. A rodent inhibitor shall be incorporated in the material of the pipe.

The Contractor shall advise the manufacturer of the climatic and conditions at the site of the Works and shall seek his advice on the storage of PVC materials on site. Subject to the Engineer's approval this advice shall be followed at all times.

Except in the case of flanged joints and where otherwise specified or approved by the Engineer pipe joints shall be flexible and sealed with a rubber gasket to the approval of the Engineer and shall withstand the various tests specified in the applicable standards. For pipes with a nominal bore of 100 to 230 mm the joints shall be capable of withstanding a deflection of not less than 3.0 degrees in any direction and for pipes with a nominal bore of 250 to 330 mm a deflection of 2.0 degrees and from 350 to 600 mm 0.5 degrees in any direction. All pipes shall be capable of
withstanding a "draw" of 13 mm over and above the initial jointing allowance. The initial jointing allowance is the gap measured parallel to the centre line of the pipeline and shall not be less than 6 mm or more than 13 mm or as otherwise recommended by the pipe manufacturer and approved by the Engineer. Pipes and fittings shall be indelibly marked prior to laying to indicate the correct initial jointing allowance.

## Gaskets

The gasket consists of a substantial body, which is extended into two thick lips directed towards the bottom of the socket. The outer edge of the body is formed into an annular shoulder, which fits into the recess in the socket of the pipe. The side of the shoulder facing the entry to the socket is given a chamfer, which centres the gasket in its recess.

The standard gaskets are moulded components with a generous cross section providing large seating surfaces to ensure fluid tightness and a substantial reserve of elasticity. The maximum continuous working temperature for these gaskeks is $70^{\circ} \mathrm{C}$. In natural rubber or an equivalent elastomer the gaskets for ND 60 to 600 are homogenous. It is preferred that the gaskets are factory fitted and factory lubricated with a long lasting silicone lubricant. The pipes must be supplied with end caps protecting the pipes effectively against fouling etc.

The storage conditions for the gaskets are:
. the storage temperature should be between $+5^{\circ} \mathrm{C}$ and $+25^{\circ} \mathrm{C}$. They should be brought to about $30^{\circ} \mathrm{C}$ for long enough to give them their original flexibility before being used.
. for vulcanised elastomer based products the following should be avoided:

- $\quad$ storage in a too damp or too dry atmosphere;
- direct sunlight or high ultra-violet artificial light;
- protect them from ambient air and the especially harmful effects of ozone.


## Transport and Storage

During transport pipes shall be solidly supported under the lower layer and at the sides to prevent accidental damage. Storage on site shall be on a raised floor with support frames so that the sockets do not touch the ground and shall be head to tail to prevent pipes from resting on the sockets.

Pipes must at all times during storage be protected against ultraviolet light from the sun.

## Excavation of Trenches

Trenches shall be excavated to the width and the depth and to the lines shown on the drawings and in conformity with Clause "Earthworks". The earth cover shall be at least 0.6 metre above the pipeline and must never exceed 3 metres.

## Laying PVC Pipes

The Contractor shall submit for the Engineer's approval the pipe manufacturer's complete and detailed recommendations for the handling and installation of pipes and fittings in open trench.

Flexible pipes shall be laid and bedded in approved granular materials except where concrete protection is required. The particle size in the material must not exceed 16 mm and the contents of particles between 8 and 16 mm must not exceed $10 \%$. Sharp stones or crushed material must not be present in the material. The granular material shall be placed over the full with of the bottom of
the trench and shall extend from a level 150 mm below the underside of the sockets or couplings on the pipeline to 200 mm above the crown of the pipes for the full length of the pipeline. The granular material shall be carefully compacted by hand.

## Jointing of PVC Pipes

Before lowering the pipes into the trench the Contractor shall carefully inspect and clean the pipe to ensure that it has been freed for all foreign matter.

The alignment of the pipe in the trench shall be controlled by level instrument or boning rods to ensure correct levels. For water supply the minimum slope shall be 1 mm per metre pipe length. No zero slope will be tolerated.

The Contractor shall keep the interior of pipes clean and free from water, dirt, stones and other foreign matter as laying proceeds, and at the end of the day's work or at other times when installation work is not proceeding the open ends of the pipes shall be sealed off by a suitable stopper or end cap. The Contractor shall take such precautions as are necessary to prevent pipes from floating.

The pipes shall be laid in straight lines both in horizontal and vertical planes. Changes of direction of less than $111 / 4^{\circ}$ shall be obtained by deviating the pipes after jointing at one or more joints. The angle of deviation at each joint depends on the ND and has the maximum values as stated above.

## General Jointing Procedure

The general jointing procedure may vary with the type of joint used, but the basic requirements are:
. Overall cleanliness

- Correct positioning of the components
- Correct centring of the spigot in the socket
- Strict observance of the manufacturer's assembling instructions.

The Contractor shall be responsible for providing the necessary copies of these instructions.
The general jointing procedure is as follows:
The spigot end of the pipe P2 to be laid must penetrate into the socket of the waiting pipe P1 fitted with its gasket.

Check that the spigot end of pipe P 2 is correctly chamfered.
Carefully clean with a rag:

- the interior of the socket and especially the seat of the gasket of pipe P1, if the gasket is not factory fitted;
- the spigot of the pipe P2 to be assembled;
- the gasket, if not.

Fit the gasket, if not factory fitted, in the socket of pipe P1 with the lips pointing towards the bottom of the socket. Check that it is correctly and uniformly placed in its groove.
Mark the normal jointing depth on the spigot of pipe P2 on each side of the pipe.
These marks should be the depth of the socket less 10 mm away from the end of the pipe.
Lower the pipe P2 into the trench carefully. Hold the spigot about 400 mm from the socket of the pipe P1.
Bring the spigot of pipe P2 into the socket of pipe P1 to the level of the gasket.
Coat with lubricant:

- the inside of the gasket;
- the spigot of pipe P2 up to about 25 mm from the two socketing depth marks. Push in place to the socketing depth marks manually.
If difficult to push the pipe in place a special jointing tackle with looped slings fitted around the two pipes can be used.

Note: It shall not be permitted to use the arm of an excavator to push the pipe P2 into position in the socket of pipe P1.
. If the pipeline lay out indicates a deviation this may now be made.
. The levelling layer must be checked to make sure the pipes are evenly supported.
. The side fill to the pipe must provide adequate support with the approved granular material in order to keep it centred in the socket. Stamping with the foot is recommended.

- The top layer of 200 mm to be equally filled and stamped with the foot.


## Polyethylene Pipes

Polyethylene pipes for cold water services shall comply with ISO 161:1988 and ISO 4065:1978 pressure class PN 6 and PN 10. Fittings shall be compression type fittings to BS 864 Part 3.

Nominal diameters are $20,25,32,50,63$ and 80 mm .
Polyethylene piping shall be laid in accordance with the pipe manufacturer's recommendations as described under PVC pipes above and to the approval of the Engineer.

## 5. CHAMBERS

## General

Chambers shall be constructed on water supply pipelines in the positions indicated at the drawings and to house valves and flow meters. Generally all chambers shall be constructed in either precast or in-situ concrete made with Portland cement to the standard details shown on the Drawings.

## Construction details

Precast concrete chamber sections shall be constructed with slabs aligned correctly. Joints shall be made so that the required jointing material fills the joint cavity. Any surplus jointing material extruded inside the chamber shall be trimmed off and joints shall be pointed on completion.

Bases for chambers shall be constructed to the standard details shown on the Drawings.
Valves and other accessories shall be seated, embedded and anchored to concrete plinths cast in concrete Class 20.

For valves with DN up to 200 mm surfaces boxes will be provided and shall be ductile iron Class A in accordance with BS 5834 and shall have a minimum clear opening of $380 \times 230 \mathrm{~mm}$. Surface boxes for other purposes shall be cast iron and shall comply with the relevant provisions of BS 5834 and be heavy duty grade A.

For large bulk flow meter chambers, covers shall be of raised pattern non-slip mild steel solid floor plates to BS 1449 Part 2, in Grade 43 steel and in the dimensions shown on the Drawings. Covers will be installed hinged as shown on the Drawings and shall be holed for and supplied with standard lifting keys. Steel plates for this purpose shall be hot dip galvanised in accordance with the relevant provisions of BS 729 and will be minimum $4,5 \mathrm{~mm}$ thick.

Covers and frames with minimum clear openings outside the ranges in BSEN 124 shall comply with the relevant provisions of that standard where applicable. All manhole covers shall have at least two closed keyways per complete cover. Keyways shall be at symmetrical points to enable lifting without tilting or jamming.

Chamber covers and frames shall in general be constructed flush with the final ground level. In cases where existing pipes are laid close to the surface and it is not possible to construct chambers with the top flush with final ground, the Contractor will propose for the Engineer's approval, the construction of the chamber to be partly above ground in order to accommodate the correct installation of valves etc.

The external surface of all chambers including roof slabs shall be protected with bituminous coating.

Chambers shall be substantially watertight, with no identifiable flow of water penetrating the permanent works. If there is any discernible flow of water entering the chamber at a point which can be located by visual inspection, the Contractor shall take such measures as are necessary to stop such infiltration. Plasticized PVC waterstops shall comply with the relevant provisions of WIs No. 4-31-02.

## Pipework

All pipes and assembling parts selected under this Contract must be of first quality, truly circular, and of uniform thickness, free from scale, lamination, honeycombs and other defects, and shall be designed and suitable for the stated pressures and temperatures.

## 6. Gate Valves and accessories

The Contractor shall submit full details of valves with manufacturer's drawings to the Engineer and obtain his approval before manufacture is commenced. All valves shall be individually tested by the manufacturer for both strength and leakage.

Gate (sluice) valves shall comply with BS 5163:1986 unless otherwise stated in the Contract.
i) Valves shall be Type B.
ii) The pressure rating shall be PN10 unless stated otherwise in the Contract.
iii) Gates shall be resilient faced up to 300 mm diameter. Gates shall be cast iron to BS 1452 GR220 min or ductile iron to BS 278973 min .

For resilient faced gates the gate shall be entirely encapsulated with rubber to BS 2494 Type W. Nitrile/EPDM with a minimum 3 mm of rubber in the seating area.

For metal seated gates both the body and the gate rings shall be gunmetal to BS 1400 LG2 and components shall be designed to provide adequate seating performance before and after wear of the seating surfaces.
iv) The body and bonnet shall be cast iron to BS 1452 GR 220 min or Ductile Iron to BS 2789 73 min .
v) Valve stems shall be threaded sufficiently to allow the gate to be raised clear of the nominal bore of the valve. Stem sealing shall be as detailed within the following table:

| Diameter (mm) | Actuator or Gearbox <br> Fitted | No Actuator or <br> Gearbox |
| :---: | :--- | :--- |
| $50-150 \mathrm{inc}$ | $0-$ seal | $0-$ seal |
| $200-300 \mathrm{inc}$ | Packed Gland | $0-$ seal |

[0-seal $=$ Double toroidal sealing ring to BS 2494 Type W]
Means shall be provided for resealing the stem under working conditions.
vi) Valves shall be designed to pass potable water and raw water.
vii) Valve caps shall be secured by hexagonal headed set screws.
viii) Operating levers will be hand wheels for $100-300 \mathrm{~mm}$ valves..
ix) Direction of closure shall be clockwise. The direction of closing shall be indicated by an arrow cast on the upper face of the gland or stem seal housing.
x) Unless indicated otherwise on the Drawings valves will be used in the closed end application.
xi) Valves with nominal diameter greater than 200 mm will be installed with ductile iron anchoring pipes on the upstream side as shown on the Drawings. Anchoring pipes will have 2 flanges and 1 puddle flange.

Gate valves shall be coated in accordance with WIS No 4-52-01 - Class A internally and Class B externally.

Extension spindles, support brackets and centring spiders shall be constructed of mild steel galvanised to BS729. Support brackets of the bolt-fixing type shall be provided to extension spindles at centres not exceeding 2 metres. The top bracket shall be located a maximum 300 mm below the spindle cap, handwheel or top of wall as appropriate. Extension spindles in gate valve extension spindle chambers shall be provided with centring spiders at centres not exceeding 2 metres. The top spider shall be located a maximum 300 mm below the spindle cap. All exposed universal joints shall be coated with a non-perishable material with an internal grease packing to allow flexible movement.

## Check Valves

Check valves shall comply with BS 5153 unless otherwise stated in the Contract and shall be as follows:
i) Nominal pressure shall be 10 bar (PN106) unless otherwise stated in the contract.
ii) Valve to be swing type resilient seated.
iii) Body ends to be flanged to BS 4504 PN10 unless otherwise stated in the contract.

## Altitude valves (Level control valves)

Altitude valves shall be used to control the water level in public taps and treatment plants to avoid any overflow events. Altitude valves shall regulate the water flow in a reservoir by means of a float with 2 positions. The valve shall be able to close at a preset high water level, and open at a preset low water level. This function shall be governed by a hydraulic system connected to the float. The altitude valves shall be Danfoss C701 or similar approved.

## 7. Mechanical flow meters

Mechanical water meters shall comply with the relevant provisions of BS 5728 Class B or ISO 4064 and shall be of the in-line helical rotary type for bulk flows. They shall be designed on the basis of the expected flow rating.

Meter housings shall be of cast iron or other approved material and be double flanged to BS 4504. The meters must be capable of being installed in horizontal, vertical or inclined pipelines without loss of accuracy. The complete measurement mechanism shall be removable from the meter body and a blank cover to replace the working unit shall also be supplied. The design of the meter shall be such that water passes through it with negligible head loss and without restriction or change of direction.

Meters shall have straight reading counters registering total flow in cubic meters $\left(\mathrm{m}^{3}\right)$. A centre sweep hand registering in litres (l) shall also be fitted.

Meters shall be suitable to a maximum working pressure of 16 bars and the Contractor shall ensure their suitability for use in the prevailing conditions.

Meter housings shall be coated by dipping or other equivalent means using a cold applied black bitumen material complying with BS 3416 and made from petroleum or asphaltic bitumen but not coal tar bitumen. No coating is to be applied to the casting until its surfaces are clean, dry and free from rust, oil and deleterious material.

The complete working mechanism of the meters shall be manufactured from materials offering maximum resistance to wear and corrosion.

## 8. Support of Pipework and Valves

All necessary supports including foundations, hangers, saddles, sliding shoes, slings, expansion pieces, fixing bolts, foundation bolts, fixing and anchor points and all other attachments shall be supplied to support the pipework and its associated equipment in an approved manner. Valves, meters, strainers and other devices mounted in the pipework shall be supported independently of the pipes to which they connect.

All brackets or other forms of support which can conveniently be so designed, shall be rigidly built up of steel sections by riveting or welding, in preference to the use of castings.

No point of passage of pipes through floors or walls shall be used as a point of support, except with the approval of the Engineer.
All brackets and fixings shall be hot-dip galvanised in accordance with BS 729.
pumped to the proposed injection point on the pipeline as shown in the Drawings. The chlorinator capacity shall be such that a free chlorine residual of at least $1 \mathrm{mg} / 1$ can be attained in the water after a minimum contact time of 20 minutes. This condition must be attainable even when maximum flow rates coincide with anticipated maximum chlorine demands. The equipment shall be of such design that it will operate accurately at both minimum and maximum flow rates without the use of standby equipment.

## 9. Dosing Pumps

In case dosing pumps are to be used for chlorination they shall fullfil the following conditions.
Dosing pumps shall be of the positive displacement reciprocating variable stroke type suitable for sodium hypochlorite solution dosing and shall be designed to. It shall be possible to regulate the pump output by adjustment of the pump stroke length or the stroke frequency. Pump housing shall be of composite material ensuring mechanical strength and chemical resistance. The pumps shall be able to draw from the solution storage tanks. Sealing materials shall be of Viton and the diaphragms of teflon. The proposed dosing pump will comply at least with the following requirements:

Max. suction lift: 3 m
Max. back pressure: $\quad 10$ bar
Electrical connection:
Max temperature of medium:
$230 \mathrm{~V}, 50 \mathrm{~Hz}$

Pump capacity shall range between $3.5 \mathrm{l} / \mathrm{h}$ and $25 \mathrm{l} / \mathrm{h}$.

The dosing pump shall be manual start and manual stop with automatic cut-out in the event of the feed tank being emptied.

The delivery lines shall also be provided with an overpressure cut-out to stop the pumps in the event of either the line becoming blocked or an attempt being made to run the pump against closed valve.

# Appendix 5 <br> Comparison of Power Cost between Existing Line Use and Generator Driven Pump 

## Appendix 5

## COMPARISON OF POWER COST BETWEEN EXISTING LINE USE AND GENERATOR DRIVEN PUMP

1) Power cost with existing electricity line

The power cost is calculated on the following assumptions;
a) Pumping efficiency of $50 \%$
b) Cost of electricity is $750 \mathrm{VND} / \mathrm{kWh}$, and

The power cost is calculated for following 4 communes (systems);

| Commune | Daily kWh | Monthly cost <br> $(\mathrm{VND} /$ day $)$ | Discharge rate <br> $\left(\mathrm{m}^{3} / \mathrm{hr}\right)$ | Cost per m <br>  <br> $\left(\mathrm{VND} / \mathrm{m}^{3}\right)$ |
| :--- | ---: | ---: | :--- | ---: |
| K-1 | 30 | 686,250 | 3.6 | 318 |
| K-2 | 54 | $1,235,250$ | 6.1 | 338 |
| G-3 | 220 | $5,032,500$ | 13 | 524 |
| D-4 | 110 | $2,516,250$ | 13 | 323 |

2) Alternative power cost with generator-driven pump

The power cost is calculated on the following conditions;
a) Cost of fuel is $4,000 \mathrm{VND} / \mathrm{hr}$
b) Fuel consumption at $100 \%$ load is as followings;

| Generator | Consumption $(\mathrm{l} / \mathrm{hr})$ |
| :--- | :--- |
| 30 kVA | 7.2 |
| 20 kVA | 5.8 |
| 15 kVA | 4.6 |
| 10.5 kVA | 3.3 |

The power cost is calculated for following 4 communes;

| Commune | Required <br> power (kVA) | Fuel cost <br> $(\mathrm{VND} / \mathrm{hr})$ | Discharge rate <br> $\left(\mathrm{m}^{3} / \mathrm{hr}\right)$ | Cost per m <br> $\left(\mathrm{VND} / \mathrm{m}^{3}\right)$ |
| :--- | :--- | :--- | ---: | ---: |
| K-1 | 10.5 | 13,200 | 7.2 | 1,833 |
| K-2 | 15 | 18,400 | 11.9 | 1,546 |
| G-3 | 30 | 28,800 | 13.0 | 2,215 |
| D-4 | 20 | 23,200 | 10.8 | 2,148 |

## 3) Conclusion

The power cost with using generator is around 6 times higher than with using electric line.


[^0]:    IV Notes
    No deep well was constructed in phase 1

