

Ultimate limit state (Under ordinary conditions)

Sidewall (parallel to centerline:landside) — Perpendicular outside steel reinforcement

B = 100cm

NO	Md (kN·m)	d (cm)	Asn (cm <sup>2</sup> )	Diameter (mm)	Pitch (cm)	As (cm <sup>2</sup> )	Mud (kN·m)	$\gamma_i \cdot Md/Mud$	
I	7	0.000	29.0	0.00	D13	20.0	6.34	53.833	0.00
	6	14.361	29.0	1.83	D13	20.0	6.34	53.833	0.29
	5	15.111	29.0	1.92	D13	20.0	6.34	53.833	0.31
	4	16.033	29.0	2.04	D13	20.0	6.34	53.833	0.33
	3	19.686	29.0	2.51	D13	20.0	6.34	53.833	0.40
	2	33.275	29.0	4.28	D13, D13	10.0	12.67	104.945	0.35
	1	100.690	35.6	10.73	D13, D13	10.0	12.67	130.031	0.85
II	7	0.000	29.0	0.00	D13	40.0	3.17	27.249	0.00
	6	3.223	29.0	0.41	D13	40.0	3.17	27.249	0.13
	5	3.339	29.0	0.42	D13	40.0	3.17	27.249	0.13
	4	3.874	29.0	0.49	D13	40.0	3.17	27.249	0.16
	3	5.938	29.0	0.75	D13	40.0	3.17	27.249	0.24
	2	15.244	29.0	1.94	D13	20.0	6.34	53.833	0.31
	1	61.307	35.6	6.44	D13, D13	10.0	12.67	130.031	0.52
III	7	0.000	35.6	0.00	D13	40.0	3.17	33.525	0.00
	6	3.857	35.6	0.40	D13	40.0	3.17	33.525	0.13
	5	9.135	35.6	0.94	D13	40.0	3.17	33.525	0.30
	4	14.413	35.6	1.49	D13	40.0	3.17	33.525	0.47
	3	19.691	35.6	2.04	D13	40.0	3.17	33.525	0.65
	2	20.097	35.6	2.08	D13	40.0	3.17	33.525	0.66
	1	0.000	35.6	0.00	D13	40.0	3.17	33.525	0.00

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CHECKED BY	R. NISHIMURA	09/08/2002

Serviceability limit state

Sidewall (parallel to centerline: landside) -- Perpendicular outside steel reinforcement

B = 100cm

NO	Ms (kN·m)	d (cm)	Diameter (mm)	Pitch (cm)	As (cm <sup>2</sup> )	$\sigma_{se}$ (N/mm <sup>2</sup> )	Crack width W (cm)	Permission crack width $W_{lim}$ (cm)
I	7	0.000	29.0	D13	20.0	6.34	0.0000	0.0035 × 10.0 = 0.0350
	6	13.055	29.0	D13	20.0	6.34	0.0200	
	5	13.738	29.0	D13	20.0	6.34	0.0210	
	4	14.575	29.0	D13	20.0	6.34	0.0223	
	3	17.897	29.0	D13	20.0	6.34	0.0274	
	2	30.251	29.0	D13, D13	10.0	12.67	0.0206	
	1	45.766	35.6	D13, D13	10.0	12.67	0.0252	
II	7	0.000	29.0	D13	40.0	3.17	0.0000	0.0035 × 10.0 = 0.0350
	6	2.930	29.0	D13	40.0	3.17	0.0112	
	5	3.036	29.0	D13	40.0	3.17	0.0116	
	4	3.522	29.0	D13	40.0	3.17	0.0134	
	3	5.399	29.0	D13	40.0	3.17	0.0206	
	2	13.858	29.0	D13	20.0	6.34	0.0212	
	1	27.866	35.6	D13, D13	10.0	12.67	0.0153	
III	7	0.000	35.6	D13	40.0	3.17	0.0000	0.0035 × 10.0 = 0.0350
	6	1.753	35.6	D13	40.0	3.17	0.0054	
	5	4.152	35.6	D13	40.0	3.17	0.0128	
	4	6.551	35.6	D13	40.0	3.17	0.0202	
	3	8.950	35.6	D13	40.0	3.17	0.0276	
	2	9.135	35.6	D13	40.0	3.17	0.0282	
	1	0.000	35.6	D13	40.0	3.17	0.0000	

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CHECKED BY	R. NISHIMURA	07/08/2002

Examination of as opposed to slip out and load of a partition wall  
 (Ultimate limit state is examined)

Partition wall(perpendicular to levee normal)

B = 100cm

Section	Td (kN/m)	$\gamma_b$	$\gamma_i$	Asn (cm <sup>2</sup> )	Diameter (mm)	Pitch (cm)	As (cm <sup>2</sup> )	Nud (kN/m)	$\gamma_i \cdot Td / Nud$
Horizontal	505.61	1.15	1.10	18.54	D16, D19	10.0	24.26	727.800	0.76
Perpendicular	754.35	1.00	1.00	21.87	D16, D22	10.0	29.29	1010.505	0.75

Partition wall(parallel to centerline)

B = 100cm

Section	Td (kN/m)	$\gamma_b$	$\gamma_i$	Asn (cm <sup>2</sup> )	Diameter (mm)	Pitch (cm)	As (cm <sup>2</sup> )	Nud (kN/m)	$\gamma_i \cdot Td / Nud$
Horizontal	487.13	1.15	1.10	17.86	D16, D19	10.0	24.26	727.800	0.74
Perpendicular	784.51	1.00	1.00	22.74	D16, D22	10.0	29.29	1010.505	0.78

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CHECKED BY	K. NISHIMURA	07/08/2002



Serviceability limit state  
 Partition wall (perpendicular to levee normal) — Horizontal steel reinforcement  
 B = 100cm

NO	Ms (kN·m)	d (cm)	Diameter (mm)	Pitch (cm)	As (cm <sup>2</sup> )	$\sigma_{se}$ (N/mm <sup>2</sup> )	Crack width W (cm)	Permission crack width $W_{lim}$ (cm)
I	7	4.819	10.0	D16, D19	10.0	24.26	23.447	0.0054
	6	4.630	10.0	D16, D19	10.0	24.26	22.528	0.0052
	5	4.641	10.0	D16, D19	10.0	24.26	22.581	0.0052
	4	4.652	10.0	D16, D19	10.0	24.26	22.635	0.0052
	3	4.652	10.0	D16, D19	10.0	24.26	22.635	0.0052
	2	3.826	10.0	D16, D19	10.0	24.26	18.616	0.0043
	1	1.049	10.0	D16, D19	10.0	24.26	5.104	0.0012
								0.0040 × 10.0 = 0.0400
II	7	1.171	10.0	D16, D19	10.0	24.26	5.698	0.0013
	6	1.160	10.0	D16, D19	10.0	24.26	5.644	0.0013
	5	1.160	10.0	D16, D19	10.0	24.26	5.644	0.0013
	4	1.160	10.0	D16, D19	10.0	24.26	5.644	0.0013
	3	1.194	10.0	D16, D19	10.0	24.26	5.809	0.0013
	2	1.171	10.0	D16, D19	10.0	24.26	5.698	0.0013
	1	0.625	10.0	D16, D19	10.0	24.26	3.041	0.0007
								0.0040 × 10.0 = 0.0400
III	7	9.783	10.0	D16, D19	10.0	24.26	47.600	0.0109
	6	9.493	10.0	D16, D19	10.0	24.26	46.189	0.0106
	5	9.449	10.0	D16, D19	10.0	24.26	45.975	0.0105
	4	9.438	10.0	D16, D19	10.0	24.26	45.921	0.0105
	3	9.538	10.0	D16, D19	10.0	24.26	46.408	0.0106
	2	8.367	10.0	D16, D19	10.0	24.26	40.710	0.0093
	1	0.000	10.0	D16, D19	10.0	24.26	0.000	0.0000
								0.0040 × 10.0 = 0.0400

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CHECKED BY	E. WISHINAGA	09/08/002

Ultimate limit state (Under ordinary conditions)

Partition wall(perpendicular to levee normal)—Perpendicular steel reinforcement  
B = 100cm

NO	Md (kN·m)	d (cm)	Asn (cm <sup>2</sup> )	Diameter (mm)	Pitch (cm)	As (cm <sup>2</sup> )	Mud (kN·m)	$\gamma_i \cdot Md/Mud$	
I	7	0.000	10.0	0.00	D16	40.0	4.97	14.097	0.00
	6	1.644	10.0	0.61	D16	40.0	4.97	14.097	0.13
	5	1.669	10.0	0.62	D16	40.0	4.97	14.097	0.13
	4	1.693	10.0	0.63	D16	40.0	4.97	14.097	0.13
	3	1.939	10.0	0.72	D16	40.0	4.97	14.097	0.15
	2	3.240	10.0	1.20	D16	40.0	4.97	14.097	0.25
	1	13.842	10.0	5.39	D16, D22	10.0	29.29	59.637	0.26
II	7	0.000	10.0	0.00	D16	40.0	4.97	14.097	0.00
	6	0.368	10.0	0.14	D16	40.0	4.97	14.097	0.03
	5	0.368	10.0	0.14	D16	40.0	4.97	14.097	0.03
	4	0.393	10.0	0.14	D16	40.0	4.97	14.097	0.03
	3	0.540	10.0	0.20	D16	40.0	4.97	14.097	0.04
	2	1.423	10.0	0.52	D16	40.0	4.97	14.097	0.11
	1	8.222	10.0	3.12	D16, D22	10.0	29.29	59.637	0.15
III	7	0.000	10.0	0.00	D16	40.0	4.97	14.097	0.00
	6	3.485	10.0	1.30	D16	40.0	4.97	14.097	0.27
	5	3.460	10.0	1.29	D16	40.0	4.97	14.097	0.27
	4	3.460	10.0	1.29	D16	40.0	4.97	14.097	0.27
	3	3.510	10.0	1.31	D16	40.0	4.97	14.097	0.27
	2	3.068	10.0	1.14	D16	40.0	4.97	14.097	0.24
	1	0.000	10.0	0.00	D16, D22	10.0	29.29	59.637	0.00

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CHECKED BY	E. Nishimura	09/08/2002

Serviceability limit state

Partition wall (perpendicular to levee normal) — Perpendicular steel reinforcement  
B = 100cm

NO	Ms (kN·m)	d (cm)	Diameter (mm)	Pitch (cm)	As (cm <sup>2</sup> )	$\sigma_{se}$ (N/mm <sup>2</sup> )	Crack width W (cm)	Permission crack width $W_{lim}$ (cm)
I	7	0.000	10.0	D16	40.0	4.97	0.0000	0.0040 × 10.0 = 0.0400
	6	0.747	10.0	D16	40.0	4.97	16.367	
	5	0.759	10.0	D16	40.0	4.97	16.630	
	4	0.770	10.0	D16	40.0	4.97	16.871	
	3	0.881	10.0	D16	40.0	4.97	19.303	
	2	1.473	10.0	D16	40.0	4.97	32.274	
	1	6.292	10.0	D16, D22	10.0	29.29	25.668	
II	7	0.000	10.0	D16	40.0	4.97	0.0000	0.0040 × 10.0 = 0.0400
	6	0.167	10.0	D16	40.0	4.97	3.659	
	5	0.167	10.0	D16	40.0	4.97	3.659	
	4	0.178	10.0	D16	40.0	4.97	3.900	
	3	0.245	10.0	D16	40.0	4.97	5.368	
	2	0.647	10.0	D16	40.0	4.97	14.176	
	1	3.737	10.0	D16, D22	10.0	29.29	15.245	
III	7	0.000	10.0	D16	40.0	4.97	0.0000	0.0040 × 10.0 = 0.0400
	6	1.584	10.0	D16	40.0	4.97	34.706	
	5	1.573	10.0	D16	40.0	4.97	34.465	
	4	1.573	10.0	D16	40.0	4.97	34.465	
	3	1.595	10.0	D16	40.0	4.97	34.947	
	2	1.394	10.0	D16	40.0	4.97	30.543	
	1	0.000	10.0	D16, D22	10.0	29.29	0.0000	

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PREPARED BY	<i>Y. Ando</i>	<i>26/07/02</i>
	<i>E. NISHIHARA</i>	<i>09/08/2002</i>

Ultimate limit state (Under ordinary conditions)  
 Partition wall (parallel to centerline) — Horizontal steel reinforcement  
 B = 100cm

NO	Md (kN·m)	d (cm)	Asn (cm <sup>2</sup> )	Diameter (mm)	Pitch (cm)	As (cm <sup>2</sup> )	Mud (kN·m)	$\gamma_i \cdot Md/Mud$	
I	7	11.524	10.0	4.44	D16, D19	10.0	24.26	53.411	0.24
	6	11.043	10.0	4.25	D16, D19	10.0	24.26	53.411	0.23
	5	11.097	10.0	4.27	D16, D19	10.0	24.26	53.411	0.23
	4	11.150	10.0	4.29	D16, D19	10.0	24.26	53.411	0.23
	3	11.070	10.0	4.26	D16, D19	10.0	24.26	53.411	0.23
	2	8.696	10.0	3.31	D16, D19	10.0	24.26	53.411	0.18
	1	2.507	10.0	0.93	D16, D19	10.0	24.26	53.411	0.05
II	7	2.801	10.0	1.04	D16, D19	10.0	24.26	53.411	0.06
	6	2.774	10.0	1.03	D16, D19	10.0	24.26	53.411	0.06
	5	2.774	10.0	1.03	D16, D19	10.0	24.26	53.411	0.06
	4	2.801	10.0	1.04	D16, D19	10.0	24.26	53.411	0.06
	3	2.854	10.0	1.06	D16, D19	10.0	24.26	53.411	0.06
	2	2.748	10.0	1.02	D16, D19	10.0	24.26	53.411	0.06
	1	1.494	10.0	0.55	D16, D19	10.0	24.26	53.411	0.03
III	7	23.261	10.0	9.52	D16, D19	10.0	24.26	53.411	0.48
	6	22.594	10.0	9.22	D16, D19	10.0	24.26	53.411	0.47
	5	22.540	10.0	9.19	D16, D19	10.0	24.26	53.411	0.46
	4	22.567	10.0	9.20	D16, D19	10.0	24.26	53.411	0.46
	3	22.701	10.0	9.27	D16, D19	10.0	24.26	53.411	0.47
	2	19.206	10.0	7.69	D16, D19	10.0	24.26	53.411	0.40
	1	0.000	10.0	0.00	D16, D19	10.0	24.26	53.411	0.00

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	R. NISHIMURA	09/08/2002



Serviceability limit state

Partition wall (parallel to centerline) -- Horizontal steel reinforcement

B = 100cm

NO	Ms (kN-m)	d (cm)	Diameter (mm)	Pitch (cm)	As (cm <sup>2</sup> )	$\sigma_{se}$ (N/mm <sup>2</sup> )	Crack width W (cm)	Permission crack width $W_{lim}$ (cm)
I	7	5.238	10.0	D16, D19	10.0	24.26	25.486	0.0058
	6	5.020	10.0	D16, D19	10.0	24.26	24.425	0.0056
	5	5.044	10.0	D16, D19	10.0	24.26	24.542	0.0056
	4	5.068	10.0	D16, D19	10.0	24.26	24.659	0.0057
	3	5.032	10.0	D16, D19	10.0	24.26	24.484	0.0056
	2	3.953	10.0	D16, D19	10.0	24.26	19.234	0.0044
	1	1.140	10.0	D16, D19	10.0	24.26	5.547	0.0013
II	7	1.273	10.0	D16, D19	10.0	24.26	6.194	0.0014
	6	1.261	10.0	D16, D19	10.0	24.26	6.135	0.0014
	5	1.261	10.0	D16, D19	10.0	24.26	6.135	0.0014
	4	1.273	10.0	D16, D19	10.0	24.26	6.194	0.0014
	3	1.297	10.0	D16, D19	10.0	24.26	6.311	0.0014
	2	1.249	10.0	D16, D19	10.0	24.26	6.077	0.0014
	1	0.679	10.0	D16, D19	10.0	24.26	3.304	0.0008
III	7	10.573	10.0	D16, D19	10.0	24.26	51.444	0.0118
	6	10.270	10.0	D16, D19	10.0	24.26	49.969	0.0115
	5	10.246	10.0	D16, D19	10.0	24.26	49.853	0.0114
	4	10.258	10.0	D16, D19	10.0	24.26	49.911	0.0115
	3	10.318	10.0	D16, D19	10.0	24.26	50.203	0.0115
	2	8.730	10.0	D16, D19	10.0	24.26	42.476	0.0097
	1	0.000	10.0	D16, D19	10.0	24.26	0.000	0.0000

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PREPARED BY	Y. Ando	24/07/02
CHECKED BY	E. BISHINGRA	09/08/2002

Ultimate limit state (Under ordinary conditions)

Partition wall (parallel to centerline) — Perpendicular steel reinforcement

B = 100cm

NO	Md (kN·m)	d (cm)	Asn (cm <sup>2</sup> )	Diameter (mm)	Pitch (cm)	As (cm <sup>2</sup> )	Mud (kN·m)	$\gamma_i \cdot Md/Mud$	
I	7	0.000	10.0	0.00	D16	40.0	4.97	14.097	0.00
	6	1.787	10.0	0.66	D16	40.0	4.97	14.097	0.14
	5	1.814	10.0	0.67	D16	40.0	4.97	14.097	0.14
	4	1.867	10.0	0.69	D16	40.0	4.97	14.097	0.15
	3	2.214	10.0	0.82	D16	40.0	4.97	14.097	0.17
	2	3.628	10.0	1.35	D16	40.0	4.97	14.097	0.28
	1	15.071	10.0	5.91	D16, D22	10.0	29.29	59.637	0.28
II	7	0.000	10.0	0.00	D16	40.0	4.97	14.097	0.00
	6	0.400	10.0	0.15	D16	40.0	4.97	14.097	0.03
	5	0.400	10.0	0.15	D16	40.0	4.97	14.097	0.03
	4	0.453	10.0	0.17	D16	40.0	4.97	14.097	0.04
	3	0.667	10.0	0.25	D16	40.0	4.97	14.097	0.05
	2	1.654	10.0	0.61	D16	40.0	4.97	14.097	0.13
	1	9.016	10.0	3.44	D16, D22	10.0	29.29	59.637	0.17
III	7	0.000	10.0	0.00	D16	40.0	4.97	14.097	0.00
	6	3.761	10.0	1.40	D16	40.0	4.97	14.097	0.29
	5	3.761	10.0	1.40	D16	40.0	4.97	14.097	0.29
	4	3.761	10.0	1.40	D16	40.0	4.97	14.097	0.29
	3	3.788	10.0	1.41	D16	40.0	4.97	14.097	0.30
	2	3.201	10.0	1.19	D16	40.0	4.97	14.097	0.25
	1	0.000	10.0	0.00	D16, D22	10.0	29.29	59.637	0.00

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CHECKED BY	P. NISHIHARA	07/08/2002

Serviceability limit state

Partition wall (parallel to centerline) — Perpendicular steel reinforcement

B = 100cm

NO	Ms (kN·m)	d (cm)	Diameter (mm)	Pitch (cm)	As (cm <sup>2</sup> )	$\sigma_{se}$ (N/mm <sup>2</sup> )	Crack width W (cm)	Permission crack width $W_{lim}$ (cm)
I	7	0.000	10.0	D16	40.0	4.97	0.0000	0.0040 × 10.0 = 0.0400
	6	0.812	10.0	D16	40.0	4.97	17.791	
	5	0.825	10.0	D16	40.0	4.97	18.076	
	4	0.849	10.0	D16	40.0	4.97	18.602	
	3	1.006	10.0	D16	40.0	4.97	22.042	
	2	1.649	10.0	D16	40.0	4.97	36.130	
	1	6.851	10.0	D16, D22	10.0	29.29	27.948	
II	7	0.000	10.0	D16	40.0	4.97	0.0000	0.0040 × 10.0 = 0.0400
	6	0.182	10.0	D16	40.0	4.97	3.988	
	5	0.182	10.0	D16	40.0	4.97	3.988	
	4	0.206	10.0	D16	40.0	4.97	4.513	
	3	0.303	10.0	D16	40.0	4.97	6.639	
	2	0.752	10.0	D16	40.0	4.97	16.476	
	1	4.098	10.0	D16, D22	10.0	29.29	16.717	
III	7	0.000	10.0	D16	40.0	4.97	0.0000	0.0040 × 10.0 = 0.0400
	6	1.710	10.0	D16	40.0	4.97	37.466	
	5	1.710	10.0	D16	40.0	4.97	37.466	
	4	1.710	10.0	D16	40.0	4.97	37.466	
	3	1.722	10.0	D16	40.0	4.97	37.729	
	2	1.455	10.0	D16	40.0	4.97	31.879	
	1	0.000	10.0	D16, D22	10.0	29.29	0.000	

<b>CALCULATION</b>	
Detailed Design on Port Reactivation Project in La Union Province	
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CALC INDEX No	PAGE 178
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PREPARED BY	<i>Y. Ando</i> 25/02/02
CHECKED BY	<i>E. NISHIHARA</i> 07/03/2002

Ultimate limit state (Under ordinary conditions)  
 Bottom slab(A Room) — Perpendicular to levee normal An upper steel reinforcement  
 B = 100cm

NO	Md (kN·m)	d (cm)	Asn (cm <sup>2</sup> )	Diameter (mm)	Pitch (cm)	As (cm <sup>2</sup> )	Mud (kN·m)	$\gamma_i \cdot Md/Mud$	
I	5	0.000	53.0	0.00	D13	20.0	6.34	99.479	0.00
	4	27.163	53.0	1.89	D13	20.0	6.34	99.479	0.30
	3	62.811	53.0	4.39	D13, D13	10.0	12.67	196.171	0.35
	2	31.445	53.0	2.19	D13	20.0	6.34	99.479	0.35
	1	0.000	53.0	0.00	D13	20.0	6.34	99.479	0.00
II	5	0.000	53.0	0.00	D13	20.0	6.34	99.479	0.00
	4	16.508	53.0	1.14	D13	20.0	6.34	99.479	0.18
	3	35.369	53.0	2.46	D13	20.0	6.34	99.479	0.39
	2	19.538	53.0	1.36	D13	20.0	6.34	99.479	0.22
	1	0.000	53.0	0.00	D13	20.0	6.34	99.479	0.00
III	5	0.000	53.0	0.00	D13	20.0	6.34	99.479	0.00
	4	0.000	53.0	0.00	D13	20.0	6.34	99.479	0.00
	3	0.000	53.0	0.00	D13	20.0	6.34	99.479	0.00
	2	0.000	53.0	0.00	D13	20.0	6.34	99.479	0.00
	1	0.000	53.0	0.00	D13	20.0	6.34	99.479	0.00

<b>CALCULATION</b>	
Detailed Design	
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REVISION	DATE
PREPARED BY	Y. Andu 26/07/02
CHECKED BY	E. NISHIMURA 09/08/2002

Ultimate limit state (During an earthquake)  
 Bottom slab(A Room) —Perpendicular to levee normal An upper steel reinforcement  
 B = 100cm

NO	Md (kN·m)	d (cm)	Asn (cm <sup>2</sup> )	Diameter (mm)	Pitch (cm)	As (cm <sup>2</sup> )	Mud (kN·m)	$\gamma_i \cdot Md/Mud$	
I	5	0.000	53.0	0.00	D13	20.0	6.34	114.401	0.00
	4	41.050	53.0	2.26	D13	20.0	6.34	114.401	0.36
	3	111.361	53.0	6.17	D13, D13	10.0	12.67	225.597	0.49
	2	63.082	53.0	3.47	D13	20.0	6.34	114.401	0.55
	1	0.000	53.0	0.00	D13	20.0	6.34	114.401	0.00
II	5	0.000	53.0	0.00	D13	20.0	6.34	114.401	0.00
	4	24.269	53.0	1.33	D13	20.0	6.34	114.401	0.21
	3	62.708	53.0	3.45	D13	20.0	6.34	114.401	0.55
	2	39.860	53.0	2.19	D13	20.0	6.34	114.401	0.35
	1	0.000	53.0	0.00	D13	20.0	6.34	114.401	0.00
III	5	0.000	53.0	0.00	D13	20.0	6.34	114.401	0.00
	4	0.000	53.0	0.00	D13	20.0	6.34	114.401	0.00
	3	0.000	53.0	0.00	D13	20.0	6.34	114.401	0.00
	2	0.000	53.0	0.00	D13	20.0	6.34	114.401	0.00
	1	0.000	53.0	0.00	D13	20.0	6.34	114.401	0.00

<b>CALCULATION</b>		
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	INITIAL	DATE
PREPARED BY	T. Ando	26/07/02
CHECKED BY	P. NISHIMURA	09/08/2002

Serviceability limit state

Bottom slab(A Room) —Perpendicular to levee normal An upper steel reinforcement  
B = 100cm

NO	Ms (kN·m)	d (cm)	Diameter (mm)	Pitch (cm)	As (cm <sup>2</sup> )	$\sigma_{se}$ (N/mm <sup>2</sup> )	Crack width W(cm)	Permission crack width $W_{lim}$ (cm)
I	5	0.000	53.0	D13	20.0	6.34	0.0000	0.0040 × 6.0 = 0.0240
	4	10.709	53.0	D13	20.0	6.34	33.306	
	3	27.092	53.0	D13, D13	10.0	12.67	42.880	
	2	14.602	53.0	D13	20.0	6.34	45.413	
	1	0.000	53.0	D13	20.0	6.34	0.0000	
II	5	0.000	53.0	D13	20.0	6.34	0.0000	0.0040 × 6.0 = 0.0240
	4	6.412	53.0	D13	20.0	6.34	19.942	
	3	15.256	53.0	D13	20.0	6.34	47.447	
	2	9.167	53.0	D13	20.0	6.34	28.510	
	1	0.000	53.0	D13	20.0	6.34	0.0000	
III	5	0.000	53.0	D13	20.0	6.34	0.0000	0.0040 × 6.0 = 0.0240
	4	0.000	53.0	D13	20.0	6.34	0.0000	
	3	0.000	53.0	D13	20.0	6.34	0.0000	
	2	0.000	53.0	D13	20.0	6.34	0.0000	
	1	0.000	53.0	D13	20.0	6.34	0.0000	

REGULATION		
Detailed Design		
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PREPARED BY	Y. Ando	26/07/03
CHECKED BY	R. NISHIMURA	09/08/2002

Ultimate limit state (Under ordinary conditions)  
 Bottom slab (A Room) — Perpendicular to levee normal A lower steel reinforcement  
 B = 100cm

NO	Md (kN·m)	d (cm)	Asn (cm <sup>2</sup> )	Diameter (mm)	Pitch (cm)	As (cm <sup>2</sup> )	Mud (kN·m)	$\gamma_i \cdot Md / Mud$	
I	5	151.313	57.6	9.82	D13, D16	10.0	16.27	272.448	0.61 ※
	4	0.000	51.0	0.00	D13	20.0	6.34	95.683	0.00
	3	0.000	51.0	0.00	D13	20.0	6.34	95.683	0.00
	2	0.000	51.0	0.00	D13	20.0	6.34	95.683	0.00
	1	161.523	57.6	10.49	D13, D19	10.0	20.66	342.945	0.52
II	5	94.772	57.6	6.10	D13, D13	10.0	12.67	213.654	0.49
	4	0.000	51.0	0.00	D13	20.0	6.34	95.683	0.00
	3	0.000	51.0	0.00	D13	20.0	6.34	95.683	0.00
	2	0.000	51.0	0.00	D13	20.0	6.34	95.683	0.00
	1	102.809	57.6	6.63	D13, D13	10.0	12.67	213.654	0.53
III	5	0.000	57.6	0.00	D13	20.0	6.34	108.233	0.00
	4	15.873	57.6	1.01	D13	20.0	6.34	108.233	0.16
	3	26.222	57.6	1.67	D13	20.0	6.34	108.233	0.27
	2	16.992	57.6	1.08	D13	20.0	6.34	108.233	0.17
	1	0.000	57.6	0.00	D13	20.0	6.34	108.233	0.00

※ It determines from serviceability limit state.

<b>CALCULATION</b>		
Detailed Design		
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CHECKED BY	12.0154/11/02 19/03/2002	

Ultimate limit state (During an earthquake)

Bottom slab(A Room) —Perpendicular to levee normal A lower steel reinforcement  
B = 100cm

NO	Md (kN·m)	d (cm)	Asn (cm <sup>2</sup> )	Diameter (mm)	Pitch (cm)	As (cm <sup>2</sup> )	Mud (kN·m)	$\gamma_i \cdot Md/Mud$	
I	5	251.052	57.6	12.95	D13, D16	10.0	16.27	313.316	0.80 ※
	4	0.000	51.0	0.00	D13	20.0	6.34	110.035	0.00
	3	0.000	51.0	0.00	D13	20.0	6.34	110.035	0.00
	2	0.000	51.0	0.00	D13	20.0	6.34	110.035	0.00
	1	303.589	57.6	15.75	D13, D19	10.0	20.66	394.387	0.77
II	5	154.474	57.6	7.89	D13, D13	10.0	12.67	245.702	0.63
	4	0.000	51.0	0.00	D13	20.0	6.34	110.035	0.00
	3	0.000	51.0	0.00	D13	20.0	6.34	110.035	0.00
	2	0.000	51.0	0.00	D13	20.0	6.34	110.035	0.00
	1	195.826	57.6	10.05	D13, D13	10.0	12.67	245.702	0.80
III	5	0.000	57.6	0.00	D13	20.0	6.34	124.468	0.00
	4	26.141	57.6	1.32	D13	20.0	6.34	124.468	0.21
	3	46.490	57.6	2.35	D13	20.0	6.34	124.468	0.37
	2	31.903	57.6	1.61	D13	20.0	6.34	124.468	0.26
	1	0.000	57.6	0.00	D13	20.0	6.34	124.468	0.00

※ It determines from serviceability limit state.

<b>CALCULATION</b>		
Detailed Design		
on Port Reactivation Project		
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CALC INDEX No.:	PAGE/03	
PREPARED BY	INITIAL	DATE
CHECKED BY	Y. Ando	26/07/02
	R. BISH/HUAA	09/08/2002



Serviceability limit state

Bottom slab (A Room) -- Perpendicular to levee normal A lower steel reinforcement  
 B = 100cm

NO	Ms (kN·m)	d (cm)	Diameter (mm)	Pitch (cm)	As (cm <sup>2</sup> )	$\sigma_{se}$ (N/mm <sup>2</sup> )	Crack width W(cm)	Permission crack width $W_{lim}$ (cm)
I	5	62.825	57.6	D13, D16	10.0	16.27	71.598	0.0136
	4	0.000	51.0	D13	20.0	6.34	0.000	0.0035 × 8.0 = 0.0280
	3	0.000	51.0	D13	20.0	6.34	0.000	
	2	0.000	51.0	D13	20.0	6.34	0.000	
	1	72.107	57.6	D13, D19	10.0	20.66	65.217	0.0124
II	5	38.957	57.6	D13, D13	10.0	12.67	56.605	0.0108
	4	0.000	51.0	D13	20.0	6.34	0.000	0.0035 × 8.0 = 0.0280
	3	0.000	51.0	D13	20.0	6.34	0.000	
	2	0.000	51.0	D13	20.0	6.34	0.000	
	1	46.264	57.6	D13, D13	10.0	12.67	67.222	0.0128
III	5	0.000	57.6	D13	20.0	6.34	0.000	0.0035 × 8.0 = 0.0280
	4	6.563	57.6	D13	20.0	6.34	18.748	
	3	11.310	57.6	D13	20.0	6.34	32.308	
	2	7.581	57.6	D13	20.0	6.34	21.656	
	1	0.000	57.6	D13	20.0	6.34	0.000	

CALCULATION		
Detailed Design		
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CALC INDEX No.:		PAGE 184
	INITIAL	DATE
PREPARED BY	<i>Y. Ando</i>	<i>26/07/02</i>
CHECKED BY	<i>R. NISHIMURA</i>	<i>07/08/2002</i>

Ultimate limit state (Under ordinary conditions)  
 Bottom slab(A Room) —Parallel to centerline An upper steel reinforcement  
 B = 100cm

NO	Md (kN·m)	d (cm)	Asn (cm <sup>2</sup> )	Diameter (mm)	Pitch (cm)	As (cm <sup>2</sup> )	Mud (kN·m)	$\gamma_i \cdot Md/Mud$	
I	5	0.000	51.0	0.00	D13	20.0	6.34	95.683	0.00
	4	34.645	51.0	2.50	D13	20.0	6.34	95.683	0.40
	3	62.811	51.0	4.56	D13, D13	10.0	12.67	188.577	0.37
	2	36.094	51.0	2.61	D13	20.0	6.34	95.683	0.41
	1	0.000	51.0	0.00	D13	20.0	6.34	95.683	0.00
II	5	0.000	51.0	0.00	D13	20.0	6.34	95.683	0.00
	4	17.298	51.0	1.25	D13	20.0	6.34	95.683	0.20
	3	29.271	51.0	2.11	D13	20.0	6.34	95.683	0.34
	2	18.747	51.0	1.35	D13	20.0	6.34	95.683	0.22
	1	0.000	51.0	0.00	D13	20.0	6.34	95.683	0.00
III	5	0.000	51.0	0.00	D13	20.0	6.34	95.683	0.00
	4	0.000	51.0	0.00	D13	20.0	6.34	95.683	0.00
	3	0.000	51.0	0.00	D13	20.0	6.34	95.683	0.00
	2	0.000	51.0	0.00	D13	20.0	6.34	95.683	0.00
	1	0.000	51.0	0.00	D13	20.0	6.34	95.683	0.00

CALCULATION		
Detailed Design		
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CALC INDEX No.:	PAGE 185	
	INITIAL	DATE
PREPARED BY	Y. Ando	26/07/02
CHECKED BY	E. NUBA/HUOA	07/08/2002

Ultimate limit state (During an earthquake)  
 Bottom slab (A Room) --Parallel to centerline An upper steel reinforcement  
 B = 100cm

NO	Md (kN·m)	d (cm)	Asn (cm <sup>2</sup> )	Diameter (mm)	Pitch (cm)	As (cm <sup>2</sup> )	Mud (kN·m)	$\gamma_i \cdot Md / Mud$	
I	5	0.000	51.0	0.00	D13	20.0	6.34	110.035	0.00
	4	58.980	51.0	3.38	D13	20.0	6.34	110.035	0.54
	3	111.361	51.0	6.42	D13, D13	10.0	12.67	216.864	0.51
	2	66.436	51.0	3.81	D13	20.0	6.34	110.035	0.60
	1	0.000	51.0	0.00	D13	20.0	6.34	110.035	0.00
II	5	0.000	51.0	0.00	D13	20.0	6.34	110.035	0.00
	4	28.336	51.0	1.62	D13	20.0	6.34	110.035	0.26
	3	51.896	51.0	2.97	D13	20.0	6.34	110.035	0.47
	2	35.793	51.0	2.04	D13	20.0	6.34	110.035	0.33
	1	0.000	51.0	0.00	D13	20.0	6.34	110.035	0.00
III	5	0.000	51.0	0.00	D13	20.0	6.34	110.035	0.00
	4	0.000	51.0	0.00	D13	20.0	6.34	110.035	0.00
	3	0.000	51.0	0.00	D13	20.0	6.34	110.035	0.00
	2	0.000	51.0	0.00	D13	20.0	6.34	110.035	0.00
	1	0.000	51.0	0.00	D13	20.0	6.34	110.035	0.00

<b>CALCULATION</b>		
Detailed Design		
on Port Reactivation Project		
in La Union Province		
CALC FILE NO.		
CALC INDEX NO.	PAGE 186	
	INITIAL	DATE
PREPARED BY	Y. Ando	26/07/00
CHECKED BY	E. NISHIMURA	07/08/2002

Serviceability limit state

Bottom slab (A Room) — Parallel to centerline An upper steel reinforcement  
 B = 100cm

NO	Ms (kN·m)	d (cm)	Diameter (mm)	Pitch (cm)	As (cm <sup>2</sup> )	$\sigma_{se}$ (N/mm <sup>2</sup> )	Crack width W (cm)	Permission crack width $W_{lim}$ (cm)
I	5	0.000	51.0	D13	20.0	6.34	0.000	0.0040 × 8.0 = 0.0320
	4	14.597	51.0	D13	20.0	6.34	47.210	
	3	27.092	51.0	D13, D13	10.0	12.67	44.608	
	2	15.914	51.0	D13	20.0	6.34	51.470	
	1	0.000	51.0	D13	20.0	6.34	0.000	
II	5	0.000	51.0	D13	20.0	6.34	0.000	0.0040 × 8.0 = 0.0320
	4	7.131	51.0	D13	20.0	6.34	23.063	
	3	12.625	51.0	D13	20.0	6.34	40.832	
	2	8.448	51.0	D13	20.0	6.34	27.323	
	1	0.000	51.0	D13	20.0	6.34	0.000	
III	5	0.000	51.0	D13	20.0	6.34	0.000	0.0040 × 8.0 = 0.0320
	4	0.000	51.0	D13	20.0	6.34	0.000	
	3	0.000	51.0	D13	20.0	6.34	0.000	
	2	0.000	51.0	D13	20.0	6.34	0.000	
	1	0.000	51.0	D13	20.0	6.34	0.000	

<b>CALCULATION</b>		
Detailed Design on Port Reactivation Project in La Union Province		
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CALC INDEX No.	PAGE 187	
	INITIAL	DATE
PREPARED BY	Y. Ando	26/07/02
CHECKED BY	R. NISHIMURA	09/08/2002

Ultimate limit state (Under ordinary conditions)

Bottom slab (A Room) — Parallel to centerline A lower steel reinforcement

B = 100cm

NO	Md (kN·m)	d (cm)	Asn (cm <sup>2</sup> )	Diameter (mm)	Pitch (cm)	As (cm <sup>2</sup> )	Mud (kN·m)	$\gamma_i \cdot Md / Mud$	
I	5	25.366	55.6	1.68	D13	20.0	6.34	104.429	0.27
	4	0.000	49.0	0.00	D13	20.0	6.34	91.879	0.00
	3	0.000	49.0	0.00	D13	20.0	6.34	91.879	0.00
	2	0.000	49.0	0.00	D13	20.0	6.34	91.879	0.00
	1	27.079	55.6	1.79	D13	20.0	6.34	104.429	0.29
II	5	15.807	55.6	1.04	D13	20.0	6.34	104.429	0.17
	4	0.000	49.0	0.00	D13	20.0	6.34	91.879	0.00
	3	0.000	49.0	0.00	D13	20.0	6.34	91.879	0.00
	2	0.000	49.0	0.00	D13	20.0	6.34	91.879	0.00
	1	17.124	55.6	1.13	D13	20.0	6.34	104.429	0.18
III	5	0.000	55.6	0.00	D13	20.0	6.34	104.429	0.00
	4	95.761	55.6	6.40	D13, D13	10.0	12.67	206.050	0.51
	3	156.385	55.6	10.53	D13, D19	10.0	20.66	330.546	0.52
	2	101.821	55.6	6.81	D13, D13	10.0	12.67	206.050	0.54
	1	0.000	55.6	0.00	D13	20.0	6.34	104.429	0.00

※ It determines from serviceability limit state.

CALCULATION		
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PREPARED BY	Y. Ando	26/07/02
CHECKED BY	R. NISHIHARA	09/08/2002

Ultimate limit state (During an earthquake)

Bottom slab(A Room) --Parallel to centerline A lower steel reinforcement

B = 100cm

NO	Md (kN·m)	d (cm)	Asn (cm <sup>2</sup> )	Diameter (mm)	Pitch (cm)	As (cm <sup>2</sup> )	Mud (kN·m)	$\gamma_i \cdot Md/Mud$	
I	5	42.084	55.6	2.20	D13	20.0	6.34	120.093	0.35
	4	0.000	49.0	0.00	D13	20.0	6.34	105.661	0.00
	3	0.000	49.0	0.00	D13	20.0	6.34	105.661	0.00
	2	0.000	49.0	0.00	D13	20.0	6.34	105.661	0.00
	1	50.897	55.6	2.67	D13	20.0	6.34	120.093	0.42
II	5	25.802	55.6	1.35	D13	20.0	6.34	120.093	0.21
	4	0.000	49.0	0.00	D13	20.0	6.34	105.661	0.00
	3	0.000	49.0	0.00	D13	20.0	6.34	105.661	0.00
	2	0.000	49.0	0.00	D13	20.0	6.34	105.661	0.00
	1	32.581	55.6	1.70	D13	20.0	6.34	120.093	0.27
III	5	0.000	55.6	0.00	D13	20.0	6.34	120.093	0.00
	4	159.559	55.6	8.46	D13, D13	10.0	12.67	236.957	0.67
	3	277.151	55.6	14.89	D13, D19	10.0	20.66	380.128	0.73
	2	190.742	55.6	10.15	D13, D13	10.0	12.67	236.957	0.80
	1	0.000	55.6	0.00	D13	20.0	6.34	120.093	0.00

※ It determines from serviceability limit state.

CALCULATION		
Detailed Design		
on Port Reactivation Project		
in La Union Province		
CALC FILE No.:		
CALC INDEX No.	PAGE 189	
	INITIAL	DATE
PREPARED BY	<i>Y. Ando</i>	<i>26/07/02</i>
CHECKED BY	<i>E. NISHIMURA</i>	<i>09/08/2002</i>

Serviceability limit state

Bottom slab(A Room) —Parallel to centerline A lower steel reinforcement

B = 100cm

NO	Ms (kN·m)	d (cm)	Diameter (mm)	Pitch (cm)	As (cm <sup>2</sup> )	$\sigma_{se}$ (N/mm <sup>2</sup> )	Crack width W(cm)	Permission crack width $W_{lim}$ (cm)
I	5	10.531	55.6	D13	20.0	6.34	31.187	0.0083
	4	0.000	49.0	D13	20.0	6.34	0.000	0.0000
	3	0.000	49.0	D13	20.0	6.34	0.000	0.0000
	2	0.000	49.0	D13	20.0	6.34	0.000	0.0000
	1	12.088	55.6	D13	20.0	6.34	35.798	0.0095
II	5	6.503	55.6	D13	20.0	6.34	19.258	0.0051
	4	0.000	49.0	D13	20.0	6.34	0.000	0.0000
	3	0.000	49.0	D13	20.0	6.34	0.000	0.0000
	2	0.000	49.0	D13	20.0	6.34	0.000	0.0000
	1	7.701	55.6	D13	20.0	6.34	22.806	0.0061
III	5	0.000	55.6	D13	20.0	6.34	0.000	0.0000
	4	39.856	55.6	D13, D13	10.0	12.67	60.053	0.0138
	3	67.436	55.6	D13, D19	10.0	20.66	63.262	0.0146
	2	45.365	55.6	D13, D13	10.0	12.67	68.354	0.0158
	1	0.000	55.6	D13	20.0	6.34	0.000	0.0000

CALCULATION		
Detailed Design on Port Reactivation Project in La Union Province		
CALC FILE No.:		
CALC INDEX No.	PAGE 190	
	INITIAL	DATE
PREPARED BY	Y. Ardo	26/07/02
CHECKED BY	E. NUSAMUDA	09/08/2002

Ultimate limit state (Under ordinary conditions)  
 Bottom slab(B Room) —Perpendicular to levee normal An upper steel reinforcement  
 B = 100cm

NO	Md (kN·m)	d (cm)	Asn (cm <sup>2</sup> )	Diameter (mm)	Pitch (cm)	As (cm <sup>2</sup> )	Mud (kN·m)	$\gamma_i \cdot Md/Mud$	
I	5	0.000	53.0	0.00	D13	20.0	6.34	99.479	0.00
	4	20.072	53.0	1.39	D13	20.0	6.34	99.479	0.22
	3	47.310	53.0	3.30	D13	20.0	6.34	99.479	0.52 ※
	2	24.084	53.0	1.67	D13	20.0	6.34	99.479	0.27
	1	0.000	53.0	0.00	D13	20.0	6.34	99.479	0.00
II	5	0.000	53.0	0.00	D13	20.0	6.34	99.479	0.00
	4	12.161	53.0	0.84	D13	20.0	6.34	99.479	0.13
	3	26.641	53.0	1.85	D13	20.0	6.34	99.479	0.29
	2	15.001	53.0	1.04	D13	20.0	6.34	99.479	0.17
	1	0.000	53.0	0.00	D13	20.0	6.34	99.479	0.00
III	5	0.000	53.0	0.00	D13	20.0	6.34	99.479	0.00
	4	0.000	53.0	0.00	D13	20.0	6.34	99.479	0.00
	3	0.000	53.0	0.00	D13	20.0	6.34	99.479	0.00
	2	0.000	53.0	0.00	D13	20.0	6.34	99.479	0.00
	1	0.000	53.0	0.00	D13	20.0	6.34	99.479	0.00

※ It determines from serviceability limit state.

CALCULATION		
Detailed Design		
on Port Reactivation Project		
in La Union Province		
CALC FILE No.		
CALC INDEX	PAGE 19	
	INITIAL	DATE
PREPARED BY	Y. Ando	26/07/02
CHECKED BY	E. NISHIHARA	09/08/2002



Ultimate limit state (During an earthquake)

Bottom slab (B Room) -- Perpendicular to levee normal An upper steel reinforcement  
B = 100cm

NO	Md (kN·m)	d (cm)	Asn (cm <sup>2</sup> )	Diameter (mm)	Pitch (cm)	As (cm <sup>2</sup> )	Mud (kN·m)	$\gamma_i \cdot Md/Mud$	
I	5	0.000	53.0	0.00	D13	20.0	6.34	114.401	0.00
	4	8.705	53.0	0.48	D13	20.0	6.34	114.401	0.08
	3	40.500	53.0	2.23	D13	20.0	6.34	114.401	0.35 ※
	2	29.360	53.0	1.61	D13	20.0	6.34	114.401	0.26
	1	0.000	53.0	0.00	D13	20.0	6.34	114.401	0.00
II	5	0.000	53.0	0.00	D13	20.0	6.34	114.401	0.00
	4	4.449	53.0	0.24	D13	20.0	6.34	114.401	0.04
	3	22.806	53.0	1.25	D13	20.0	6.34	114.401	0.20
	2	19.067	53.0	1.05	D13	20.0	6.34	114.401	0.17
	1	0.000	53.0	0.00	D13	20.0	6.34	114.401	0.00
III	5	0.000	53.0	0.00	D13	20.0	6.34	114.401	0.00
	4	0.000	53.0	0.00	D13	20.0	6.34	114.401	0.00
	3	0.000	53.0	0.00	D13	20.0	6.34	114.401	0.00
	2	0.000	53.0	0.00	D13	20.0	6.34	114.401	0.00
	1	0.000	53.0	0.00	D13	20.0	6.34	114.401	0.00

※ It determines from serviceability limit state.

CALCULATION		
Detailed Design		
on Port Reactivation Project		
in La Union Province		
CALC FILE No.:		
CALC INDEX No.:	PAGE 192	
	INITIAL	DATE
PREPARED BY	Y. Ando	26/07/02
	R. NISHIHARA	09/08/2002

Serviceability limit state

Bottom slab(B Room) —Perpendicular to levee normal An upper steel reinforcement  
B = 100cm

NO	Ms (kN·m)	d (cm)	Diameter (mm)	Pitch (cm)	As (cm <sup>2</sup> )	$\sigma_{se}$ (N/mm <sup>2</sup> )	Crack width W(cm)	Permission crack width $W_{lim}$ (cm)
I	5	0.000	53.0	D13	20.0	6.34	0.0000	0.0040 × 6.0 = 0.0240
	4	6.653	53.0	D13	20.0	6.34	20.691	
	3	14.276	53.0	D13	20.0	6.34	44.399	
	2	8.498	53.0	D13	20.0	6.34	26.429	
	1	0.000	53.0	D13	20.0	6.34	0.0000	
II	5	0.000	53.0	D13	20.0	6.34	0.0000	0.0040 × 6.0 = 0.0240
	4	4.089	53.0	D13	20.0	6.34	12.717	
	3	8.039	53.0	D13	20.0	6.34	25.002	
	2	5.403	53.0	D13	20.0	6.34	16.804	
	1	0.000	53.0	D13	20.0	6.34	0.0000	
III	5	0.000	53.0	D13	20.0	6.34	0.0000	0.0040 × 6.0 = 0.0240
	4	0.000	53.0	D13	20.0	6.34	0.0000	
	3	0.000	53.0	D13	20.0	6.34	0.0000	
	2	0.000	53.0	D13	20.0	6.34	0.0000	
	1	0.000	53.0	D13	20.0	6.34	0.0000	

<b>CALCULATION</b>	
Detailed Design on Port Reactivation Project in La Union Province	
CALC FILE No.:	
CALC INDEX No.	PAGE 193
INITIAL : DATE	
PREPARED BY	<i>Y. A. ... 26/07/02</i>
<i>E. NISHIHARA 09/08/2002</i>	

Ultimate limit state (Under ordinary conditions)  
 Bottom slab (B Room) — Perpendicular to levee normal A lower steel reinforcement  
 B = 100cm

NO	Md (kN·m)	d (cm)	Asn (cm <sup>2</sup> )	Diameter (mm)	Pitch (cm)	As (cm <sup>2</sup> )	Mud (kN·m)	$\gamma_i \cdot Md/Mud$
I	5	113.032	57.6	7.30	D13, D13	10.0	12.67	213.654
	4	0.000	51.0	0.00	D13	20.0	6.34	95.683
	3	0.000	51.0	0.00	D13	20.0	6.34	95.683
	2	0.000	51.0	0.00	D13	20.0	6.34	95.683
	1	122.599	57.6	7.92	D13, D13	10.0	12.67	213.654
								0.63 ※
II	5	70.645	57.6	4.54	D13	20.0	6.34	108.233
	4	0.000	51.0	0.00	D13	20.0	6.34	95.683
	3	0.000	51.0	0.00	D13	20.0	6.34	95.683
	2	0.000	51.0	0.00	D13	20.0	6.34	95.683
	1	78.175	57.6	5.02	D13	20.0	6.34	108.233
								0.79 ※
III	5	0.000	57.6	0.00	D13	20.0	6.34	108.233
	4	11.846	57.6	0.76	D13	20.0	6.34	108.233
	3	19.751	57.6	1.26	D13	20.0	6.34	108.233
	2	12.895	57.6	0.82	D13	20.0	6.34	108.233
	1	0.000	57.6	0.00	D13	20.0	6.34	108.233
								0.00

※ It determines from serviceability limit state.

<b>CALCULATION</b>	
Detailed Design on Port Reactivation Project in La Union Province	
CALC FILE No.:	
CALC INDEX No.:	PAGE 194
	INITIAL
PREPARED BY	Y. Ardo 24/07/2002
CHECKED BY	Z. NISHIMURA 29/08/2002

Ultimate limit state (During an earthquake)

Bottom slab (B Room) — Perpendicular to levee normal A lower steel reinforcement  
B = 100cm

NO	Md (kN·m)	d (cm)	Asn (cm <sup>2</sup> )	Diameter (mm)	Pitch (cm)	As (cm <sup>2</sup> )	Mud (kN·m)	$\gamma_i \cdot Md / Mud$	
I	5	76.226	57.6	3.86	D13, D13	10.0	12.67	245.702	0.31
	4	0.000	51.0	0.00	D13	20.0	6.34	110.035	0.00
	3	0.000	51.0	0.00	D13	20.0	6.34	110.035	0.00
	2	0.000	51.0	0.00	D13	20.0	6.34	110.035	0.00
	1	125.482	57.6	6.39	D13, D13	10.0	12.67	245.702	0.51 ※
II	5	44.313	57.6	2.24	D13	20.0	6.34	124.468	0.36
	4	0.000	51.0	0.00	D13	20.0	6.34	110.035	0.00
	3	0.000	51.0	0.00	D13	20.0	6.34	110.035	0.00
	2	0.000	51.0	0.00	D13	20.0	6.34	110.035	0.00
	1	83.082	57.6	4.21	D13	20.0	6.34	124.468	0.67 ※
III	5	0.000	57.6	0.00	D13	20.0	6.34	124.468	0.00
	4	7.756	57.6	0.39	D13	20.0	6.34	124.468	0.06
	3	16.907	57.6	0.85	D13	20.0	6.34	124.468	0.14
	2	13.158	57.6	0.66	D13	20.0	6.34	124.468	0.11
	1	0.000	57.6	0.00	D13	20.0	6.34	124.468	0.00

※ It determines from serviceability limit state.

Detailed L.....		
on Port Reactivation Project in La Union Province		
CALC FILE No.:		
CALC INDE. No.:	PAGE 195	
	INITIAL	DATE
PREPARED BY	Y. Ando	26/07/02
CHECKED BY	P. MISHIMOTO	09/08/2002

Serviceability limit state

Bottom slab (B Room) — Perpendicular to levee normal A lower steel reinforcement  
B = 100cm

NO	Ms (kN·m)	d (cm)	Diameter (mm)	Pitch (cm)	As (cm <sup>2</sup> )	$\sigma_{se}$ (N/mm <sup>2</sup> )	Crack width W(cm)	Permission crack width $W_{lim}$ (cm)
I	5	35.550	57.6	D13, D13	10.0	12.67	51.654	0.0098
	4	0.000	51.0	D13	20.0	6.34	0.000	0.0000
	3	0.000	51.0	D13	20.0	6.34	0.000	0.0000
	2	0.000	51.0	D13	20.0	6.34	0.000	0.0000
	1	39.867	57.6	D13, D13	10.0	12.67	57.927	0.0110
								0.0035 × 8.0 = 0.0280
II	5	22.453	57.6	D13	20.0	6.34	64.139	0.0145
	4	0.000	51.0	D13	20.0	6.34	0.000	0.0000
	3	0.000	51.0	D13	20.0	6.34	0.000	0.0000
	2	0.000	51.0	D13	20.0	6.34	0.000	0.0000
	1	25.855	57.6	D13	20.0	6.34	73.857	0.0167
								0.0035 × 8.0 = 0.0280
III	5	0.000	57.6	D13	20.0	6.34	0.000	0.0000
	4	3.742	57.6	D13	20.0	6.34	10.689	0.0024
	3	5.960	57.6	D13	20.0	6.34	17.025	0.0038
	2	4.188	57.6	D13	20.0	6.34	11.963	0.0027
	1	0.000	57.6	D13	20.0	6.34	0.000	0.0000
								0.0035 × 8.0 = 0.0280

<b>CALCULATION</b>	
Detailed Design on Port Reactivation Project in La Union Province	
CALC FILE No.:	
CALC INDEX No.:	PAGE 196
INITIAL DATE	
PREPARED BY	Y. Ando 26/07/02
CHECKED BY	E. NISHIMURA 09/08/2002

Ultimate limit state (Under ordinary conditions)

Bottom slab (B Room) -- Parallel to centerline An upper steel reinforcement  
B = 100cm

NO	Md (kN·m)	d (cm)	Asn (cm <sup>2</sup> )	Diameter (mm)	Pitch (cm)	As (cm <sup>2</sup> )	Mud (kN·m)	$\gamma_i \cdot Md/Mud$	
I	5	0.000	51.0	0.00	D13	20.0	6.34	95.683	0.00
	4	25.962	51.0	1.87	D13	20.0	6.34	95.683	0.30
	3	47.310	51.0	3.43	D13	20.0	6.34	95.683	0.54
	2	27.320	51.0	1.97	D13	20.0	6.34	95.683	0.31
	1	0.000	51.0	0.00	D13	20.0	6.34	95.683	0.00
II	5	0.000	51.0	0.00	D13	20.0	6.34	95.683	0.00
	4	12.902	51.0	0.93	D13	20.0	6.34	95.683	0.15
	3	22.047	51.0	1.59	D13	20.0	6.34	95.683	0.25
	2	14.260	51.0	1.03	D13	20.0	6.34	95.683	0.16
	1	0.000	51.0	0.00	D13	20.0	6.34	95.683	0.00
III	5	0.000	51.0	0.00	D13	20.0	6.34	95.683	0.00
	4	0.000	51.0	0.00	D13	20.0	6.34	95.683	0.00
	3	0.000	51.0	0.00	D13	20.0	6.34	95.683	0.00
	2	0.000	51.0	0.00	D13	20.0	6.34	95.683	0.00
	1	0.000	51.0	0.00	D13	20.0	6.34	95.683	0.00

<b>CALCULATION</b>		
Detailed Design on Port Reactivation Project in La Union Province		
CALC FILE No.:		
CALC INDEX No.:	PAGE 197	
	INITIAL	DATE
PREPARED BY	Y. Ando	26/07/02
CHECKED BY	K. NISHINURA	09/08/2002

Ultimate limit state (During an earthquake)  
 Bottom slab(B Room)—Parallel to centerline An upper steel reinforcement  
 B = 100cm

NO	Md (kN·m)	d (cm)	Asn (cm <sup>2</sup> )	Diameter (mm)	Pitch (cm)	As (cm <sup>2</sup> )	Mud (kN·m)	$\gamma_i \cdot Md/Mud$	
I	5	0.000	51.0	0.00	D13	20.0	6.34	110.035	0.00
	4	19.310	51.0	1.10	D13	20.0	6.34	110.035	0.18
	3	40.500	51.0	2.31	D13	20.0	6.34	110.035	0.37
	2	26.301	51.0	1.50	D13	20.0	6.34	110.035	0.24
	1	0.000	51.0	0.00	D13	20.0	6.34	110.035	0.00
II	5	0.000	51.0	0.00	D13	20.0	6.34	110.035	0.00
	4	8.263	51.0	0.47	D13	20.0	6.34	110.035	0.08
	3	18.874	51.0	1.08	D13	20.0	6.34	110.035	0.17
	2	15.254	51.0	0.87	D13	20.0	6.34	110.035	0.14
	1	0.000	51.0	0.00	D13	20.0	6.34	110.035	0.00
III	5	0.000	51.0	0.00	D13	20.0	6.34	110.035	0.00
	4	0.000	51.0	0.00	D13	20.0	6.34	110.035	0.00
	3	0.000	51.0	0.00	D13	20.0	6.34	110.035	0.00
	2	0.000	51.0	0.00	D13	20.0	6.34	110.035	0.00
	1	0.000	51.0	0.00	D13	20.0	6.34	110.035	0.00

CALCULATION	
Detailed Design on Port Reactivation Project in La Union Province	
CALC FILE #	
CALC NO	PAGE 198
DATE	
DESIGNED BY	Y. Ardo 26/07/02
CHECKED BY	E. WISHNUPA 07/08/2002

Serviceability limit state

Bottom slab (B Room) -- Parallel to centerline An upper steel reinforcement  
 B = 100cm

NO	Ms (kN·m)	d (cm)	Diameter (mm)	Pitch (cm)	As (cm <sup>2</sup> )	$\sigma_{se}$ (N/mm <sup>2</sup> )	Crack width W (cm)	Permission crack width $W_{lim}$ (cm)
I	5	0.000	51.0	D13	20.0	6.34	0.0000	0.0040 × 8.0 = 0.0320
	4	8.039	51.0	D13	20.0	6.34	26.000	
	3	14.276	51.0	D13	20.0	6.34	46.172	
	2	8.649	51.0	D13	20.0	6.34	27.973	
	1	0.000	51.0	D13	20.0	6.34	0.0000	
II	5	0.000	51.0	D13	20.0	6.34	0.0000	0.0040 × 8.0 = 0.0320
	4	4.089	51.0	D13	20.0	6.34	13.225	
	3	6.653	51.0	D13	20.0	6.34	21.517	
	2	4.730	51.0	D13	20.0	6.34	15.298	
	1	0.000	51.0	D13	20.0	6.34	0.0000	
III	5	0.000	51.0	D13	20.0	6.34	0.0000	0.0040 × 8.0 = 0.0320
	4	0.000	51.0	D13	20.0	6.34	0.0000	
	3	0.000	51.0	D13	20.0	6.34	0.0000	
	2	0.000	51.0	D13	20.0	6.34	0.0000	
	1	0.000	51.0	D13	20.0	6.34	0.0000	

<b>CALCULATION</b>	
Detailed Design on Port Resactivation Project in La Union Province	
CALC FILE NO.	
CALC INSTRUCTIONS	PAGE 199
DATE	
PREPARED BY	Y. Ando 26/07/02
CHECKED BY	E. N. B. H. H. H. 07/08/2002



Ultimate limit state (Under ordinary conditions)  
 Bottom slab (B Room) -- Parallel to centerline A lower steel reinforcement  
 B = 100cm

NO	Md (kN·m)	d (cm)	Asn (cm <sup>2</sup> )	Diameter (mm)	Pitch (cm)	As (cm <sup>2</sup> )	Mud (kN·m)	$\gamma_i \cdot Md/Mud$	
I	5	18.948	55.6	1.25	D13	20.0	6.34	104.429	0.20
	4	0.000	49.0	0.00	D13	20.0	6.34	91.879	0.00
	3	0.000	49.0	0.00	D13	20.0	6.34	91.879	0.00
	2	0.000	49.0	0.00	D13	20.0	6.34	91.879	0.00
	1	20.553	55.6	1.36	D13	20.0	6.34	104.429	0.22
II	5	11.784	55.6	0.78	D13	20.0	6.34	104.429	0.12
	4	0.000	49.0	0.00	D13	20.0	6.34	91.879	0.00
	3	0.000	49.0	0.00	D13	20.0	6.34	91.879	0.00
	2	0.000	49.0	0.00	D13	20.0	6.34	91.879	0.00
	1	13.019	55.6	0.86	D13	20.0	6.34	104.429	0.14
III	5	0.000	55.6	0.00	D13	20.0	6.34	104.429	0.00
	4	71.570	55.6	4.76	D13	20.0	6.34	104.429	0.75
	3	117.785	55.6	7.89	D13, D13	10.0	12.67	206.050	0.63
	2	77.249	55.6	5.15	D13, D13	10.0	12.67	206.050	0.41
	1	0.000	55.6	0.00	D13	20.0	6.34	104.429	0.00

※ It determines from serviceability limit state.

<b>CALCULATION</b>	
Detailed Design on Port Reactivation Project in La Union Province	
ALC FILE No.	
OLD INSTR. No.	PAGE 200
PREPARED BY Y. Ando 24/07/02	
CHECKED BY T. KISHIMOTO 18/08/2002	

Ultimate limit state (During an earthquake)  
 Bottom slab(B Room) —Parallel to centerline A lower steel reinforcement  
 B = 100cm

NO	Md (kN·m)	d (cm)	Asn (cm <sup>2</sup> )	Diameter (mm)	Pitch (cm)	As (cm <sup>2</sup> )	Mud (kN·m)	$\gamma_i \cdot Md / Mud$	
I	5	12.776	55.6	0.67	D13	20.0	6.34	120.093	0.11
	4	0.000	49.0	0.00	D13	20.0	6.34	105.661	0.00
	3	0.000	49.0	0.00	D13	20.0	6.34	105.661	0.00
	2	0.000	49.0	0.00	D13	20.0	6.34	105.661	0.00
	1	21.039	55.6	1.10	D13	20.0	6.34	120.093	0.18
II	5	7.438	55.6	0.39	D13	20.0	6.34	120.093	0.06
	4	0.000	49.0	0.00	D13	20.0	6.34	105.661	0.00
	3	0.000	49.0	0.00	D13	20.0	6.34	105.661	0.00
	2	0.000	49.0	0.00	D13	20.0	6.34	105.661	0.00
	1	13.794	55.6	0.72	D13	20.0	6.34	120.093	0.11
III	5	0.000	55.6	0.00	D13	20.0	6.34	120.093	0.00
	4	49.080	55.6	2.57	D13	20.0	6.34	120.093	0.41
	3	100.695	55.6	5.30	D13, D13	10.0	12.67	236.957	0.42 ※
	2	78.315	55.6	4.12	D13, D13	10.0	12.67	236.957	0.33
	1	0.000	55.6	0.00	D13	20.0	6.34	120.093	0.00

※ It determines from serviceability limit state.

<b>CALCULATION</b>	
Detailed Design on Port Reactivation Project In La Union Province	
CALC FILE No:	
CALC INDEX No	PAGE 20
	INITIAL    DATE
PREPARED BY	Y. Ando 24/07/02
CHECKED BY	R. NISHIMURA 03/08/2002

Serviceability limit state

Bottom slab(B Room) —Parallel to centerline A lower steel reinforcement

B = 100cm

NO	Ms (kN·m)	d (cm)	Diameter (mm)	Pitch (cm)	As (cm <sup>2</sup> )	$\sigma_{se}$ (N/mm <sup>2</sup> )	Crack width W(cm)	Permission crack width $W_{lim}$ (cm)
I	5	5.960	55.6	D13	20.0	6.34	17.650	0.0047
	4	0.000	49.0	D13	20.0	6.34	0.000	0.0000
	3	0.000	49.0	D13	20.0	6.34	0.000	0.0000
	2	0.000	49.0	D13	20.0	6.34	0.000	0.0000
	1	6.684	55.6	D13	20.0	6.34	19.794	0.0053
II	5	3.742	55.6	D13	20.0	6.34	11.082	0.0029
	4	0.000	49.0	D13	20.0	6.34	0.000	0.0000
	3	0.000	49.0	D13	20.0	6.34	0.000	0.0000
	2	0.000	49.0	D13	20.0	6.34	0.000	0.0000
	1	4.300	55.6	D13	20.0	6.34	12.734	0.0034
III	5	0.000	55.6	D13	20.0	6.34	0.000	0.0000
	4	22.453	55.6	D13	20.0	6.34	66.494	0.0177
	3	35.550	55.6	D13, D13	10.0	12.67	53.565	0.0123
	2	25.014	55.6	D13, D13	10.0	12.67	37.690	0.0087
	1	0.000	55.6	D13	20.0	6.34	0.000	0.0000

<b>CALCULATION</b>	
Detailed Design on Port Reactivation Project in La Union Province	
CALC FILE NO.:	
CALC INDEX NO.	PAGE 202
INITIAL	DATE
PREPARED BY: Y. Ando	26/07/02
CHECKED BY: E. NISHIMURA	09/08/2002

Ultimate limit state (Under ordinary conditions)  
 Bottom slab (C Room) -- Perpendicular to levee normal An upper steel reinforcement  
 B = 100cm

NO	Md (kN·m)	d (cm)	Asn (cm <sup>2</sup> )	Diameter (mm)	Pitch (cm)	As (cm <sup>2</sup> )	Mud (kN·m)	$\gamma_i \cdot Md/Mud$	
I	5	0.000	53.0	0.00	D13, D13	10.0	12.67	196.171	0.00
	4	15.219	53.0	1.06	D13	20.0	6.34	99.479	0.17
	3	34.590	53.0	2.41	D13	20.0	6.34	99.479	0.38
	2	18.159	53.0	1.26	D13	20.0	6.34	99.479	0.20
	1	0.000	53.0	0.00	D13	20.0	6.34	99.479	0.00
II	5	0.000	53.0	0.00	D13	20.0	6.34	99.479	0.00
	4	9.353	53.0	0.65	D13	20.0	6.34	99.479	0.10
	3	19.479	53.0	1.35	D13	20.0	6.34	99.479	0.22
	2	11.358	53.0	0.79	D13	20.0	6.34	99.479	0.13
	1	0.000	53.0	0.00	D13	20.0	6.34	99.479	0.00
III	5	0.000	53.0	0.00	D13	20.0	6.34	99.479	0.00
	4	0.000	53.0	0.00	D13	20.0	6.34	99.479	0.00
	3	0.000	53.0	0.00	D13	20.0	6.34	99.479	0.00
	2	0.000	53.0	0.00	D13	20.0	6.34	99.479	0.00
	1	0.000	53.0	0.00	D13	20.0	6.34	99.479	0.00

CALCULATION	
Detailed Design	
on Port Reactivation Project	
in La Union Province	
CALC FILE No.	
CALC INCH	PAGE 203
PREPARED BY	Y. Ando 26/07/02
CHECKED BY	E. NISHIMURA 09/08/2002

Ultimate limit state (During an earthquake)  
 Bottom slab(C Room) —Perpendicular to levee normal An upper steel reinforcement  
 B = 100cm

NO	Md (kN·m)	d (cm)	Asn (cm <sup>2</sup> )	Diameter (mm)	Pitch (cm)	As (cm <sup>2</sup> )	Mud (kN·m)	$\gamma_i \cdot Md/Mud$	
I	5	93.545	53.0	5.17	D13, D13	10.0	12.67	225.597	0.41
	4	0.603	53.0	0.03	D13	20.0	6.34	114.401	0.01
	3	3.884	53.0	0.21	D13	20.0	6.34	114.401	0.03
	2	3.054	53.0	0.17	D13	20.0	6.34	114.401	0.03
	1	50.133	53.0	2.76	D13	20.0	6.34	114.401	0.44
II	5	62.456	53.0	3.44	D13	20.0	6.34	114.401	0.55
	4	0.264	53.0	0.01	D13	20.0	6.34	114.401	0.00
	3	2.187	53.0	0.12	D13	20.0	6.34	114.401	0.02
	2	1.999	53.0	0.11	D13	20.0	6.34	114.401	0.02
	1	28.287	53.0	1.55	D13	20.0	6.34	114.401	0.25
III	5	0.000	53.0	0.00	D13	20.0	6.34	114.401	0.00
	4	10.083	53.0	0.55	D13	20.0	6.34	114.401	0.09
	3	12.043	53.0	0.66	D13	20.0	6.34	114.401	0.11
	2	5.321	53.0	0.29	D13	20.0	6.34	114.401	0.05
	1	0.000	53.0	0.00	D13	20.0	6.34	114.401	0.00

<b>CALCULATION</b>		
Detailed Design on Port Reactivation Project in La Union Province		
CALC FILE NO.		
CALC INDEX NO.		PAGE 204
	INITIAL	DATE
PREPARED BY	Y. Ando	26/07/02
CHECKED BY	E. NISHIMURA	09/08/02

Serviceability limit state

Bottom slab (C Room) — Perpendicular to levee normal An upper steel reinforcement  
B = 100cm

NO	Ms (kN·m)	d (cm)	Diameter (mm)	Pitch (cm)	As (cm <sup>2</sup> )	$\sigma_{se}$ (N/mm <sup>2</sup> )	Crack width W (cm)	Permission crack width $W_{lim}$ (cm)
I	5	4.997	53.0	D13, D13	10.0	12.67	7.909	0.0012
	4	6.653	53.0	D13	20.0	6.34	20.691	0.0038
	3	14.276	53.0	D13	20.0	6.34	44.399	0.0082
	2	6.653	53.0	D13	20.0	6.34	20.691	0.0038
	1	2.678	53.0	D13	20.0	6.34	8.329	0.0015
								0.0040 × 6.0 = 0.0240
II	5	3.336	53.0	D13	20.0	6.34	10.375	0.0019
	4	4.089	53.0	D13	20.0	6.34	12.717	0.0024
	3	8.039	53.0	D13	20.0	6.34	25.002	0.0046
	2	4.089	53.0	D13	20.0	6.34	12.717	0.0024
	1	1.511	53.0	D13	20.0	6.34	4.699	0.0009
								0.0040 × 6.0 = 0.0240
III	5	0.000	53.0	D13	20.0	6.34	0.000	0.0000
	4	0.539	53.0	D13	20.0	6.34	1.676	0.0003
	3	0.643	53.0	D13	20.0	6.34	2.000	0.0004
	2	0.284	53.0	D13	20.0	6.34	0.883	0.0002
	1	0.000	53.0	D13	20.0	6.34	0.000	0.0000
								0.0040 × 6.0 = 0.0240

<b>CALCULATION</b>	
Detailed Design	
on Port Reactivation in La Union Province	
CALC FILE NO.	
CALC NO.	PAGE 205
PREPARED BY	Y. Arado 26/07/02
CHECKED BY	P. NISHIHARA 09/08/2002

Ultimate limit state (Under ordinary conditions)  
 Bottom slab (C Room) — Perpendicular to levee normal A lower steel reinforcement  
 B = 100cm

NO	Md (kN·m)	d (cm)	Asn (cm <sup>2</sup> )	Diameter (mm)	Pitch (cm)	As (cm <sup>2</sup> )	Mud (kN·m)	$\gamma_i \cdot Md / Mud$		
I	5	81.353	57.6	5.23	D13, D13	10.0	12.67	213.654	0.42	※
	4	0.000	51.0	0.00	D13	20.0	6.34	95.683	0.00	
	3	0.000	51.0	0.00	D13	20.0	6.34	95.683	0.00	
	2	0.000	51.0	0.00	D13	20.0	6.34	95.683	0.00	
	1	90.927	57.6	5.85	D13, D13	10.0	12.67	213.654	0.47	
II	5	51.364	57.6	3.29	D13	20.0	6.34	108.233	0.52	※
	4	0.000	51.0	0.00	D13	20.0	6.34	95.683	0.00	
	3	0.000	51.0	0.00	D13	20.0	6.34	95.683	0.00	
	2	0.000	51.0	0.00	D13	20.0	6.34	95.683	0.00	
	1	58.173	57.6	3.73	D13	20.0	6.34	108.233	0.59	
III	5	0.000	57.6	0.00	D13	20.0	6.34	108.233	0.00	
	4	8.561	57.6	0.55	D13	20.0	6.34	108.233	0.09	
	3	14.441	57.6	0.92	D13	20.0	6.34	108.233	0.15	
	2	9.561	57.6	0.61	D13	20.0	6.34	108.233	0.10	
	1	0.000	57.6	0.00	D13	20.0	6.34	108.233	0.00	

※ It determines from serviceability limit state.

CALCULATION		
Detailed Design		
on Port Reactivation Project		
in La Union Province		
ALU FILE No.:		
CALC. No.:	PAGE 206	
	INITIAL	DATE
PREPARED BY	Y. Ando	26/07/02
CHECKED BY	P. NISHIMURA	07/08/2002

Ultimate limit state (During an earthquake)  
 Bottom slab (C Room) — Perpendicular to levee normal A lower steel reinforcement  
 B = 100cm

NO	Md (kN·m)	d (cm)	Asn (cm <sup>2</sup> )	Diameter (mm)	Pitch (cm)	As (cm <sup>2</sup> )	Mud (kN·m)	$\gamma_i \cdot Md / Mud$	
I	5	6.750	57.6	0.34	D13, D13	10.0	12.67	245.702	0.03 ※
	4	22.406	51.0	1.28	D13	20.0	6.34	110.035	0.20
	3	28.848	51.0	1.65	D13	20.0	6.34	110.035	0.26
	2	4.201	51.0	0.24	D13	20.0	6.34	110.035	0.04
	1	12.594	57.6	0.63	D13, D13	10.0	12.67	245.702	0.05
II	5	3.808	57.6	0.19	D13	20.0	6.34	124.468	0.03 ※
	4	14.564	51.0	0.83	D13	20.0	6.34	110.035	0.13
	3	16.244	51.0	0.93	D13	20.0	6.34	110.035	0.15
	2	1.680	51.0	0.10	D13	20.0	6.34	110.035	0.02
	1	8.409	57.6	0.42	D13	20.0	6.34	124.468	0.07
III	5	0.000	57.6	0.00	D13	20.0	6.34	124.468	0.00
	4	0.679	57.6	0.03	D13	20.0	6.34	124.468	0.01
	3	1.622	57.6	0.08	D13	20.0	6.34	124.468	0.01
	2	1.320	57.6	0.07	D13	20.0	6.34	124.468	0.01
	1	0.000	57.6	0.00	D13	20.0	6.34	124.468	0.00

※ It determines from serviceability limit state.

<b>CALCULATION</b>	
Detailed Design on Port Reactivation Project in La Union Province	
ALC FILE NO.	
DATE	PAGE # 207
PREPARED BY	Y. Ando 26/07/02
CHECKED BY	R. AISHIMURA 09/08/2002



Serviceability limit state

Bottom slab (C Room) -- Perpendicular to levee normal A lower steel reinforcement  
B = 100cm

NO	Ms (kN·m)	d (cm)	Diameter (mm)	Pitch (cm)	As (cm <sup>2</sup> )	$\sigma_{se}$ (N/mm <sup>2</sup> )	Crack width W (cm)	Permission crack width $W_{lim}$ (cm)
I	5	35.550	57.6	D13, D13	10.0	12.67	51.654	0.0098
	4	1.197	51.0	D13	20.0	6.34	3.871	0.0009
	3	1.541	51.0	D13	20.0	6.34	4.984	0.0011
	2	0.224	51.0	D13	20.0	6.34	0.724	0.0002
	1	35.550	57.6	D13, D13	10.0	12.67	51.654	0.0098
								0.0035 × 8.0 = 0.0280
II	5	22.453	57.6	D13	20.0	6.34	64.139	0.0145
	4	0.778	51.0	D13	20.0	6.34	2.516	0.0006
	3	0.868	51.0	D13	20.0	6.34	2.807	0.0006
	2	0.090	51.0	D13	20.0	6.34	0.291	0.0001
	1	22.453	57.6	D13	20.0	6.34	64.139	0.0145
								0.0035 × 8.0 = 0.0280
III	5	0.000	57.6	D13	20.0	6.34	0.000	0.0000
	4	3.742	57.6	D13	20.0	6.34	10.689	0.0024
	3	5.960	57.6	D13	20.0	6.34	17.025	0.0038
	2	3.742	57.6	D13	20.0	6.34	10.689	0.0024
	1	0.000	57.6	D13	20.0	6.34	0.000	0.0000
								0.0035 × 8.0 = 0.0280

<b>CALCULATION</b>	
Detailed Design	
on Port Reactivation Project in La Union Province	
ALC FILE NO.	
CALC BY	PAGE 208
DATE	
PREP BY	Y. Ando 26/07/02
CHECKED BY	E. NISHIKIYAMA 07/08/2002

Ultimate limit state (Under ordinary conditions)  
 Bottom slab (C Room) — Parallel to centerline An upper steel reinforcement  
 B = 100cm

NO	Md (kN·m)	d (cm)	Asn (cm <sup>2</sup> )	Diameter (mm)	Pitch (cm)	As (cm <sup>2</sup> )	Mud (kN·m)	$\gamma_i \cdot Md / Mud$	
I	5	0.000	51.0	0.00	D13	20.0	6.34	95.683	0.00
	4	18.799	51.0	1.36	D13	20.0	6.34	95.683	0.22
	3	34.590	51.0	2.50	D13	20.0	6.34	95.683	0.40
	2	20.158	51.0	1.45	D13	20.0	6.34	95.683	0.23
	1	0.000	51.0	0.00	D13	20.0	6.34	95.683	0.00
II	5	0.000	51.0	0.00	D13	20.0	6.34	95.683	0.00
	4	9.353	51.0	0.67	D13	20.0	6.34	95.683	0.11
	3	16.120	51.0	1.16	D13	20.0	6.34	95.683	0.19
	2	10.617	51.0	0.76	D13	20.0	6.34	95.683	0.12
	1	0.000	51.0	0.00	D13	20.0	6.34	95.683	0.00
III	5	0.000	51.0	0.00	D13	20.0	6.34	95.683	0.00
	4	0.000	51.0	0.00	D13	20.0	6.34	95.683	0.00
	3	0.000	51.0	0.00	D13	20.0	6.34	95.683	0.00
	2	0.000	51.0	0.00	D13	20.0	6.34	95.683	0.00
	1	0.000	51.0	0.00	D13	20.0	6.34	95.683	0.00

CALCULATION	
Detailed Design on Port Reactivation Project in La Union Province	
CALC FILE No :	
CALC INDE	PAGE 209
INITIAL	DATE
PREPARED BY	Y. Amde 26/07/02
CHECKED BY	E. NISHIMURA 29/08/2002

Ultimate limit state (During an earthquake)  
 Bottom slab (C Room) --- Parallel to centerline An upper steel reinforcement  
 B = 100cm

NO	Md (kN·m)	d (cm)	Asn (cm <sup>2</sup> )	Diameter (mm)	Pitch (cm)	As (cm <sup>2</sup> )	Mud (kN·m)	$\gamma_i \cdot Md/Mud$	
I	5	15.684	51.0	0.89	D13	20.0	6.34	110.035	0.14
	4	1.772	51.0	0.10	D13	20.0	6.34	110.035	0.02
	3	3.884	51.0	0.22	D13	20.0	6.34	110.035	0.04
	2	2.602	51.0	0.15	D13	20.0	6.34	110.035	0.02
	1	8.402	51.0	0.48	D13	20.0	6.34	110.035	0.08
II	5	10.363	51.0	0.59	D13	20.0	6.34	110.035	0.09
	4	0.717	51.0	0.04	D13	20.0	6.34	110.035	0.01
	3	1.810	51.0	0.10	D13	20.0	6.34	110.035	0.02
	2	1.546	51.0	0.09	D13	20.0	6.34	110.035	0.01
	1	4.761	51.0	0.27	D13	20.0	6.34	110.035	0.04
III	5	0.000	51.0	0.00	D13	20.0	6.34	110.035	0.00
	4	58.255	51.0	3.33	D13	20.0	6.34	110.035	0.53
	3	71.979	51.0	4.13	D13	20.0	6.34	110.035	0.65
	2	32.489	51.0	1.85	D13	20.0	6.34	110.035	0.30
	1	0.000	51.0	0.00	D13	20.0	6.34	110.035	0.00

<b>CALCULATION</b>	
Detailed Design on Port Revitalization Project in La Union Province	
CALC FILE No:	
CALC INDEX IV	PAGE 2/0
INITIAL	DATE
PREPARED BY	Y. Ando 26/07/02
CHECKED BY	P. NISHIHARA 09/08/2002

Serviceability limit state

Bottom slab (C Room) -- Parallel to centerline An upper steel reinforcement

B = 100cm

NO	Ms (kN·m)	d (cm)	Diameter (mm)	Pitch (cm)	As (cm <sup>2</sup> )	$\sigma_{se}$ (N/mm <sup>2</sup> )	Crack width W (cm)	Permission crack width $W_{lim}$ (cm)
I	5	0.838	51.0	D13	20.0	6.34	2.710	0.0006
	4	8.039	51.0	D13	20.0	6.34	26.000	0.0059
	3	14.276	51.0	D13	20.0	6.34	46.172	0.0104
	2	8.039	51.0	D13	20.0	6.34	26.000	0.0059
	1	0.449	51.0	D13	20.0	6.34	1.452	0.0003
								0.0040 × 8.0 = 0.0320
II	5	0.554	51.0	D13	20.0	6.34	1.792	0.0004
	4	4.089	51.0	D13	20.0	6.34	13.225	0.0030
	3	6.653	51.0	D13	20.0	6.34	21.517	0.0049
	2	4.089	51.0	D13	20.0	6.34	13.225	0.0030
	1	0.254	51.0	D13	20.0	6.34	0.822	0.0002
								0.0040 × 8.0 = 0.0320
III	5	0.000	51.0	D13	20.0	6.34	0.000	0.0000
	4	3.112	51.0	D13	20.0	6.34	10.065	0.0023
	3	3.845	51.0	D13	20.0	6.34	12.436	0.0028
	2	1.735	51.0	D13	20.0	6.34	5.611	0.0013
	1	0.000	51.0	D13	20.0	6.34	0.000	0.0000
								0.0040 × 8.0 = 0.0320

CALCULATION		
Detailed Design		
on Port Renovation Project		
in La Haha Port Area		
CALC FILE No.		
CALC INDEX No.	PAGE 2/11	
	INITIAL	DATE
PREPARED BY	Y. Ando	26/07/02
CHECKED BY	R. NISHIMURA	09/08/2002

Ultimate limit state (Under ordinary conditions)

Bottom slab (C Room) -- Parallel to centerline A lower steel reinforcement

B = 100cm

NO	Md (kN·m)	d (cm)	Asn (cm <sup>2</sup> )	Diameter (mm)	Pitch (cm)	As (cm <sup>2</sup> )	Mud (kN·m)	$\gamma_i \cdot Md/Mud$	
I	5	13.638	55.6	0.90	D13	20.0	6.34	104.429	0.14
	4	0.000	49.0	0.00	D13	20.0	6.34	91.879	0.00
	3	0.000	49.0	0.00	D13	20.0	6.34	91.879	0.00
	2	0.000	49.0	0.00	D13	20.0	6.34	91.879	0.00
	1	15.244	55.6	1.01	D13	20.0	6.34	104.429	0.16
II	5	8.561	55.6	0.57	D13	20.0	6.34	104.429	0.09
	4	0.000	49.0	0.00	D13	20.0	6.34	91.879	0.00
	3	0.000	49.0	0.00	D13	20.0	6.34	91.879	0.00
	2	0.000	49.0	0.00	D13	20.0	6.34	91.879	0.00
	1	9.685	55.6	0.64	D13	20.0	6.34	104.429	0.10
III	5	0.000	55.6	0.00	D13	20.0	6.34	104.429	0.00
	4	51.564	55.6	3.42	D13	20.0	6.34	104.429	0.54
	3	86.109	55.6	5.74	D13, D13	10.0	12.67	206.050	0.46
	2	57.246	55.6	3.80	D13	20.0	6.34	104.429	0.60
	1	0.000	55.6	0.00	D13	20.0	6.34	104.429	0.00

CALCULATION	
Detailed Design	
on Port Reactivation Project	
in La Union Province	
ALC FILE No.:	
CALC. NO.:	PAGE 212
PREPARED BY	INITIAL DATE
CHECKED BY	P. NISHIMURA 09/08/2002

Ultimate limit state (During an earthquake)

Bottom slab (C Room) — Parallel to centerline A lower steel reinforcement

B = 100cm

NO	Md (kN·m)	d (cm)	Asn (cm <sup>2</sup> )	Diameter (mm)	Pitch (cm)	As (cm <sup>2</sup> )	Mud (kN·m)	$\gamma_i \cdot Md/Mud$	
I	5	1.131	55.6	0.06	D13	20.0	6.34	120.093	0.01
	4	19.325	49.0	1.15	D13	20.0	6.34	105.661	0.18
	3	28.848	49.0	1.71	D13	20.0	6.34	105.661	0.27
	2	13.163	49.0	0.78	D13	20.0	6.34	105.661	0.12
	1	2.112	55.6	0.11	D13	20.0	6.34	120.093	0.02
II	5	0.641	55.6	0.03	D13	20.0	6.34	120.093	0.01
	4	11.203	49.0	0.66	D13	20.0	6.34	105.661	0.11
	3	13.444	49.0	0.80	D13	20.0	6.34	105.661	0.13
	2	5.041	49.0	0.30	D13	20.0	6.34	105.661	0.05
	1	1.395	55.6	0.07	D13	20.0	6.34	120.093	0.01
III	5	0.000	55.6	0.00	D13	20.0	6.34	120.093	0.00
	4	4.374	55.6	0.23	D13	20.0	6.34	120.093	0.04
	3	9.653	55.6	0.50	D13, D13	10.0	12.67	236.957	0.04
	2	7.843	55.6	0.41	D13	20.0	6.34	120.093	0.07
	1	0.000	55.6	0.00	D13	20.0	6.34	120.093	0.00

CALCULATION		
Detailed Design		
on Port Reactivation Project		
in La Union Province		
CALC FILE No.:		
CALC INDEX No.:		PAGE 213
	INITIAL	DATE
PREPARED BY	Y. Ando	26/07/02
CHECKED BY	R. NISHIMURA	07/08/2002

Serviceability limit state

Bottom slab (C Room) -- Parallel to centerline A lower steel reinforcement

B = 100cm

NO	Ms (kN·m)	d (cm)	Diameter (mm)	Pitch (cm)	As (cm <sup>2</sup> )	$\sigma_{se}$ (N/mm <sup>2</sup> )	Crack width W (cm)	Permission crack width $W_{lim}$ (cm)
I	5	5.960	55.6	D13	20.0	6.34	17.650	0.0035 × 10.0 = 0.0350
	4	1.032	49.0	D13	20.0	6.34	3.477	
	3	1.541	49.0	D13	20.0	6.34	5.192	
	2	0.703	49.0	D13	20.0	6.34	2.368	
	1	5.960	55.6	D13	20.0	6.34	17.650	
II	5	3.742	55.6	D13	20.0	6.34	11.082	0.0035 × 10.0 = 0.0350
	4	0.598	49.0	D13	20.0	6.34	2.015	
	3	0.718	49.0	D13	20.0	6.34	2.419	
	2	0.269	49.0	D13	20.0	6.34	0.906	
	1	3.742	55.6	D13	20.0	6.34	11.082	
III	5	0.000	55.6	D13	20.0	6.34	0.000	0.0035 × 10.0 = 0.0350
	4	22.453	55.6	D13	20.0	6.34	66.494	
	3	35.550	55.6	D13, D13	10.0	12.67	53.565	
	2	22.453	55.6	D13	20.0	6.34	66.494	
	1	0.000	55.6	D13	20.0	6.34	0.000	

<b>CALCULATION</b>	
Detailed Design on Port Reactivation Project in La Union Province	
CALC FILE No:	
CALC INDEX No.	PAGE 214
	INITIAL   DATE
PREPARED BY	Y. Ando 25/07/02
CHECKED BY	E. NISHIMURA 09/08/2002

Ultimate limit state (Under ordinary conditions)

Bottom slab(D Room) —Perpendicular to levee normal An upper steel reinforcement  
B = 100cm

NO	Md (kN·m)	d (cm)	Asn (cm <sup>2</sup> )	Diameter (mm)	Pitch (cm)	As (cm <sup>2</sup> )	Mud (kN·m)	$\gamma_i \cdot Md/Mud$	
I	5	0.000	53.0	0.00	D13, D19	10.0	20.66	314.440	0.00
	4	15.888	53.0	1.10	D13	20.0	6.34	99.479	0.18
	3	34.093	53.0	2.37	D13	20.0	6.34	99.479	0.38
	2	15.888	53.0	1.10	D13	20.0	6.34	99.479	0.18
	1	0.000	53.0	0.00	D13, D16	10.0	16.27	249.981	0.00
II	5	0.000	53.0	0.00	D13, D13	10.0	12.67	196.171	0.00
	4	9.764	53.0	0.68	D13	20.0	6.34	99.479	0.11
	3	19.198	53.0	1.33	D13	20.0	6.34	99.479	0.21
	2	9.764	53.0	0.68	D13	20.0	6.34	99.479	0.11
	1	0.000	53.0	0.00	D13, D13	10.0	12.67	196.171	0.00
III	5	0.000	53.0	0.00	D13	20.0	6.34	99.479	0.00
	4	0.000	53.0	0.00	D13	20.0	6.34	99.479	0.00
	3	0.000	53.0	0.00	D13	20.0	6.34	99.479	0.00
	2	0.000	53.0	0.00	D13	20.0	6.34	99.479	0.00
	1	0.000	53.0	0.00	D13	20.0	6.34	99.479	0.00

<b>CALCULATION</b>	
Detailed Design	
on Port Revetment Project in La Union Province	
NO. FILE	
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PREPARED BY	<i>Y. Ando</i> 26/07/02
CHECKED BY	<i>E. NISHIMURA</i> 09/08/2002



Ultimate limit state (During an earthquake)

Bottom slab(D Room) — Perpendicular to levee normal An upper steel reinforcement  
B = 100cm

NO	Md (kN·m)	d (cm)	Asn (cm <sup>2</sup> )	Diameter (mm)	Pitch (cm)	As (cm <sup>2</sup> )	Mud (kN·m)	$\gamma_i \cdot Md/Mud$	
I	5	242.549	53.0	13.65	D13, D19	10.0	20.66	361.606	0.67
	4	0.000	53.0	0.00	D13	20.0	6.34	114.401	0.00
	3	0.000	53.0	0.00	D13	20.0	6.34	114.401	0.00
	2	0.000	53.0	0.00	D13	20.0	6.34	114.401	0.00
	1	216.336	53.0	12.14	D13, D16	10.0	16.27	287.478	0.75
II	5	155.227	53.0	8.64	D13, D13	10.0	12.67	225.597	0.69
	4	0.000	53.0	0.00	D13	20.0	6.34	114.401	0.00
	3	0.000	53.0	0.00	D13	20.0	6.34	114.401	0.00
	2	0.000	53.0	0.00	D13	20.0	6.34	114.401	0.00
	1	134.594	53.0	7.48	D13, D13	10.0	12.67	225.597	0.60
III	5	0.000	53.0	0.00	D13	20.0	6.34	114.401	0.00
	4	25.674	53.0	1.41	D13	20.0	6.34	114.401	0.22
	3	38.464	53.0	2.11	D13	20.0	6.34	114.401	0.34
	2	22.799	53.0	1.25	D13	20.0	6.34	114.401	0.20
	1	0.000	53.0	0.00	D13	20.0	6.34	114.401	0.00

CALCULATION	
Detailed Design	
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PREPARED BY	Y. Ardo 26/02/22
CHECKED BY	E. NISHIHARA 19/06/2022

Serviceability limit state

Bottom slab(D Room) --Perpendicular to levee normal An upper steel reinforcement  
B = 100cm

NO	Ms (kN·m)	d (cm)	Diameter (mm)	Pitch (cm)	As (cm <sup>2</sup> )	$\sigma_{se}$ (N/mm <sup>2</sup> )	Crack width W(cm)	Permission crack width $W_{lim}$ (cm)
I	5	28.014	53.0	D13, D19	10.0	20.66	27.614	0.0042
	4	6.945	53.0	D13	20.0	6.34	21.599	0.0040
	3	14.903	53.0	D13	20.0	6.34	46.349	0.0086
	2	6.945	53.0	D13	20.0	6.34	21.599	0.0040
	1	18.732	53.0	D13, D16	10.0	16.27	23.262	0.0035
II	5	18.415	53.0	D13, D13	10.0	12.67	29.146	0.0044
	4	4.268	53.0	D13	20.0	6.34	13.274	0.0025
	3	8.392	53.0	D13	20.0	6.34	26.100	0.0048
	2	4.268	53.0	D13	20.0	6.34	13.274	0.0025
	1	11.108	53.0	D13, D13	10.0	12.67	17.581	0.0026
III	5	0.000	53.0	D13	20.0	6.34	0.000	0.0000
	4	2.999	53.0	D13	20.0	6.34	9.327	0.0017
	3	3.918	53.0	D13	20.0	6.34	12.185	0.0023
	2	1.981	53.0	D13	20.0	6.34	6.161	0.0011
	1	0.000	53.0	D13	20.0	6.34	0.000	0.0000

<b>CALCULATION</b>		
Detailed Design on Port Reactivation Project in La Union Province		
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	INITIAL	DATE
PREPARED BY		
CHECKED BY	E. NISHIMURA	09/06/2002

Ultimate limit state (Under ordinary conditions)  
 Bottom slab(D Room) —Perpendicular to levee normal A lower steel reinforcement  
 B = 100cm

NO	Md (kN·m)	d (cm)	Asn (cm <sup>2</sup> )	Diameter (mm)	Pitch (cm)	As (cm <sup>2</sup> )	Mud (kN·m)	$\gamma_i \cdot Md/Mud$	
I	5	84.901	57.6	5.46	D13, D13	10.0	12.67	213.654	0.44
	4	0.000	51.0	0.00	D13	20.0	6.34	95.683	0.00
	3	0.000	51.0	0.00	D13, D13	10.0	12.67	188.577	0.00
	2	0.000	51.0	0.00	D13	20.0	6.34	95.683	0.00
	1	84.901	57.6	5.46	D13, D13	10.0	12.67	213.654	0.44
II	5	53.621	57.6	3.44	D13	20.0	6.34	108.233	0.54
	4	0.000	51.0	0.00	D13	20.0	6.34	95.683	0.00
	3	0.000	51.0	0.00	D13	20.0	6.34	95.683	0.00
	2	0.000	51.0	0.00	D13	20.0	6.34	95.683	0.00
	1	53.621	57.6	3.44	D13	20.0	6.34	108.233	0.54
III	5	0.000	57.6	0.00	D13	20.0	6.34	108.233	0.00
	4	8.937	57.6	0.57	D13	20.0	6.34	108.233	0.09
	3	14.233	57.6	0.91	D13	20.0	6.34	108.233	0.14
	2	8.937	57.6	0.57	D13	20.0	6.34	108.233	0.09
	1	0.000	57.6	0.00	D13	20.0	6.34	108.233	0.00

CALCULATION		
Detailed Design		
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	INITIAL	DATE
PREPARED BY	Y. Ando	26/07/01
CHECKED BY	P. NISHIMURA	07/08/2002

Ultimate limit state (During an earthquake)  
 Bottom slab(D Room)—Perpendicular to levee normal A lower steel reinforcement  
 B = 100cm

NO	Md (kN·m)	d (cm)	Asn (cm <sup>2</sup> )	Diameter (mm)	Pitch (cm)	As (cm <sup>2</sup> )	Mud (kN·m)	$\gamma_i \cdot Md / Mud$	
I	5	0.000	57.6	0.00	D13, D13	10.0	12.67	245.702	0.00
	4	48.349	51.0	2.76	D13	20.0	6.34	110.035	0.44
	3	92.134	51.0	5.30	D13, D13	10.0	12.67	216.864	0.42
	2	37.356	51.0	2.13	D13	20.0	6.34	110.035	0.34
	1	0.000	57.6	0.00	D13, D13	10.0	12.67	245.702	0.00
II	5	0.000	57.6	0.00	D13	20.0	6.34	124.468	0.00
	4	30.193	51.0	1.72	D13	20.0	6.34	110.035	0.27
	3	51.882	51.0	2.97	D13	20.0	6.34	110.035	0.47
	2	22.414	51.0	1.28	D13	20.0	6.34	110.035	0.20
	1	0.000	57.6	0.00	D13	20.0	6.34	124.468	0.00
III	5	0.000	57.6	0.00	D13	20.0	6.34	124.468	0.00
	4	0.000	57.6	0.00	D13	20.0	6.34	124.468	0.00
	3	0.000	57.6	0.00	D13	20.0	6.34	124.468	0.00
	2	0.000	57.6	0.00	D13	20.0	6.34	124.468	0.00
	1	0.000	57.6	0.00	D13	20.0	6.34	124.468	0.00

CALCULATION		
Detailed Design		
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	INITIAL	DATE
PREPARED BY	Y. Azido	26/07/02
CHECKED BY	E. NISHIMURA	09/08/2002

Serviceability limit state  
 Bottom slab(D Room) --Perpendicular to levee normal A lower steel reinforcement  
 B = 100cm

NO	Ms (kN·m)	d (cm)	Diameter (mm)	Pitch (cm)	As (cm <sup>2</sup> )	$\sigma_{se}$ (N/mm <sup>2</sup> )	Crack width W(cm)	Permission crack width $W_{lim}$ (cm)
I	5	37.113	57.6	D13, D13	10.0	12.67	53.925	0.0103
	4	6.290	51.0	D13	20.0	6.34	20.343	0.0046
	3	9.385	51.0	D13, D13	10.0	12.67	15.453	0.0029
	2	2.397	51.0	D13	20.0	6.34	7.752	0.0017
	1	37.113	57.6	D13, D13	10.0	12.67	53.925	0.0103
II	5	23.440	57.6	D13	20.0	6.34	66.958	0.0151
	4	4.035	51.0	D13	20.0	6.34	13.050	0.0029
	3	5.285	51.0	D13	20.0	6.34	17.093	0.0039
	2	1.280	51.0	D13	20.0	6.34	4.140	0.0009
	1	23.440	57.6	D13	20.0	6.34	66.958	0.0151
III	5	0.000	57.6	D13	20.0	6.34	0.000	0.0000
	4	3.907	57.6	D13	20.0	6.34	11.161	0.0025
	3	6.222	57.6	D13	20.0	6.34	17.774	0.0040
	2	3.907	57.6	D13	20.0	6.34	11.161	0.0025
	1	0.000	57.6	D13	20.0	6.34	0.000	0.0000

<b>CALCULATION</b>	
Detailed Design on Port Reactivation Project in La Union Province	
CALC FILE NO.	
CALC NO.	PAGE 220
INITIAL   DATE	
PREPARED BY	K. Ando 26/07/02
CHECKED BY	P. NISHIMURA 07/08/2002

Ultimate limit state (Under ordinary conditions)

Bottom slab(D Room) -Parallel to centerline An upper steel reinforcement

B = 100cm

NO	Md (kN·m)	d (cm)	Asn (cm <sup>2</sup> )	Diameter (mm)	Pitch (cm)	As (cm <sup>2</sup> )	Mud (kN·m)	$\gamma_i \cdot Md/Mud$	
I	5	0.000	51.0	0.00	D13	20.0	6.34	95.683	0.00
	4	19.198	51.0	1.38	D13	20.0	6.34	95.683	0.22
	3	34.093	51.0	2.46	D13	20.0	6.34	95.683	0.39
	2	19.198	51.0	1.38	D13	20.0	6.34	95.683	0.22
	1	0.000	51.0	0.00	D13	20.0	6.34	95.683	0.00
II	5	0.000	51.0	0.00	D13	20.0	6.34	95.683	0.00
	4	9.764	51.0	0.70	D13	20.0	6.34	95.683	0.11
	3	15.888	51.0	1.15	D13	20.0	6.34	95.683	0.18
	2	9.764	51.0	0.70	D13	20.0	6.34	95.683	0.11
	1	0.000	51.0	0.00	D13	20.0	6.34	95.683	0.00
III	5	0.000	51.0	0.00	D13	20.0	6.34	95.683	0.00
	4	0.000	51.0	0.00	D13, D13	10.0	12.67	188.577	0.00
	3	0.000	51.0	0.00	D13, D19	10.0	20.66	302.055	0.00
	2	0.000	51.0	0.00	D13, D13	10.0	12.67	188.577	0.00
	1	0.000	51.0	0.00	D13	20.0	6.34	95.683	0.00

CALCULATION		
Detailed Design		
on Port Reactivation Project		
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CALC INDEX NO.	PAGE 221	
	INITIAL	DATE
PREPARED BY	Y. Andri	26/07/02
CHECKED BY	E. NISHIMURA	09/08/2002

Ultimate limit state (During an earthquake)  
 Bottom slab (D Room) —Parallel to centerline An upper steel reinforcement  
 B = 100cm

NO	Md (kN·m)	d (cm)	Asn (cm <sup>2</sup> )	Diameter (mm)	Pitch (cm)	As (cm <sup>2</sup> )	Mud (kN·m)	$\gamma_i \cdot Md/Mud$	
I	5	40.663	51.0	2.32	D13	20.0	6.34	110.035	0.37
	4	0.000	51.0	0.00	D13	20.0	6.34	110.035	0.00
	3	0.000	51.0	0.00	D13	20.0	6.34	110.035	0.00
	2	0.000	51.0	0.00	D13	20.0	6.34	110.035	0.00
	1	36.266	51.0	2.07	D13	20.0	6.34	110.035	0.33
II	5	25.843	51.0	1.47	D13	20.0	6.34	110.035	0.23
	4	0.000	51.0	0.00	D13	20.0	6.34	110.035	0.00
	3	0.000	51.0	0.00	D13	20.0	6.34	110.035	0.00
	2	0.000	51.0	0.00	D13	20.0	6.34	110.035	0.00
	1	22.461	51.0	1.28	D13	20.0	6.34	110.035	0.20
III	5	0.000	51.0	0.00	D13	20.0	6.34	110.035	0.00
	4	152.690	51.0	8.85	D13, D13	10.0	12.67	216.864	0.70
	3	229.527	51.0	13.43	D13, D19	10.0	20.66	347.363	0.66
	2	137.131	51.0	7.93	D13, D13	10.0	12.67	216.864	0.63
	1	0.000	51.0	0.00	D13	20.0	6.34	110.035	0.00

CALCULATION		
Detailed Design		
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PREPARED BY	Y. Ando	26/07/04
CHECKED BY	R. WASHIMURA	09/08/2002

Serviceability limit state

Bottom slab(D Room) —Parallel to centerline An upper steel reinforcement

B = 100cm

NO	Ms (kN·m)	d (cm)	Diameter (mm)	Pitch (cm)	As (cm <sup>2</sup> )	$\sigma_{se}$ (N/mm <sup>2</sup> )	Crack width W(cm)	Permission crack width $W_{lim}$ (cm)
I	5	4.697	51.0	D13	20.0	6.34	15.191	0.0040 × 8.0 = 0.0320
	4	8.392	51.0	D13	20.0	6.34	27.142	
	3	14.903	51.0	D13	20.0	6.34	48.200	
	2	8.392	51.0	D13	20.0	6.34	27.142	
	1	3.140	51.0	D13	20.0	6.34	10.156	
II	5	3.059	51.0	D13	20.0	6.34	9.894	0.0040 × 8.0 = 0.0320
	4	4.268	51.0	D13	20.0	6.34	13.804	
	3	6.945	51.0	D13	20.0	6.34	22.462	
	2	4.268	51.0	D13	20.0	6.34	13.804	
	1	1.861	51.0	D13	20.0	6.34	6.019	
III	5	0.000	51.0	D13	20.0	6.34	0.000	0.0040 × 8.0 = 0.0320
	4	17.516	51.0	D13, D13	10.0	12.67	28.841	
	3	23.403	51.0	D13, D19	10.0	20.66	24.005	
	2	12.007	51.0	D13, D13	10.0	12.67	19.770	
	1	0.000	51.0	D13	20.0	6.34	0.000	

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Ultimate limit state (Under ordinary conditions)  
 Bottom slab(D Room) —Parallel to centerline A lower steel reinforcement  
 B = 100cm

NO	Md (kN·m)	d (cm)	Asn (cm <sup>2</sup> )	Diameter (mm)	Pitch (cm)	As (cm <sup>2</sup> )	Mud (kN·m)	$\gamma_i \cdot Md/Mud$	
I	5	14.233	55.6	0.94	D13	20.0	6.34	104.429	0.15
	4	0.000	49.0	0.00	D13	20.0	6.34	91.879	0.00
	3	0.000	49.0	0.00	D13, D13	10.0	12.67	180.957	0.00
	2	0.000	49.0	0.00	D13	20.0	6.34	91.879	0.00
	1	14.233	55.6	0.94	D13	20.0	6.34	104.429	0.15
II	5	8.937	55.6	0.59	D13	20.0	6.34	104.429	0.09
	4	0.000	49.0	0.00	D13	20.0	6.34	91.879	0.00
	3	0.000	49.0	0.00	D13	20.0	6.34	91.879	0.00
	2	0.000	49.0	0.00	D13	20.0	6.34	91.879	0.00
	1	8.937	55.6	0.59	D13	20.0	6.34	104.429	0.09
III	5	0.000	55.6	0.00	D13	20.0	6.34	104.429	0.00
	4	53.621	55.6	3.56	D13	20.0	6.34	104.429	0.56
	3	84.901	55.6	5.66	D13, D13	10.0	12.67	206.050	0.45
	2	53.621	55.6	3.56	D13	20.0	6.34	104.429	0.56
	1	0.000	55.6	0.00	D13	20.0	6.34	104.429	0.00

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Ultimate limit state (During an earthquake)

Bottom slab(D Room) --Parallel to centerline A lower steel reinforcement

B = 100cm

NO	Md (kN·m)	d (cm)	Asn (cm <sup>2</sup> )	Diameter (mm)	Pitch (cm)	As (cm <sup>2</sup> )	Mud (kN·m)	$\gamma_i \cdot Md/Mud$	
I	5	0.000	55.6	0.00	D13	20.0	6.34	120.093	0.00
	4	53.742	49.0	3.20	D13	20.0	6.34	105.661	0.51
	3	92.134	49.0	5.52	D13, D13	10.0	12.67	208.101	0.44
	2	50.022	49.0	2.98	D13	20.0	6.34	105.661	0.47
	1	0.000	55.6	0.00	D13	20.0	6.34	120.093	0.00
II	5	0.000	55.6	0.00	D13	20.0	6.34	120.093	0.00
	4	28.164	49.0	1.67	D13	20.0	6.34	105.661	0.27
	3	42.937	49.0	2.55	D13	20.0	6.34	105.661	0.41
	2	24.443	49.0	1.45	D13	20.0	6.34	105.661	0.23
	1	0.000	55.6	0.00	D13	20.0	6.34	120.093	0.00
III	5	0.000	55.6	0.00	D13	20.0	6.34	120.093	0.00
	4	0.000	55.6	0.00	D13	20.0	6.34	120.093	0.00
	3	0.000	55.6	0.00	D13, D13	10.0	12.67	236.957	0.00
	2	0.000	55.6	0.00	D13	20.0	6.34	120.093	0.00
	1	0.000	55.6	0.00	D13	20.0	6.34	120.093	0.00

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Serviceability limit state

Bottom slab (D Room) -- Parallel to centerline A lower steel reinforcement

B = 100cm

NO	Ms (kN·m)	d (cm)	Diameter (mm)	Pitch (cm)	As (cm <sup>2</sup> )	$\sigma_{se}$ (N/mm <sup>2</sup> )	Crack width W (cm)	Permission crack width $W_{lim}$ (cm)
I	5	6.222	55.6	D13	20.0	6.34	18.426	0.0035 × 10.0 = 0.0350
	4	5.944	49.0	D13	20.0	6.34	20.026	
	3	9.385	49.0	D13, D13	10.0	12.67	16.104	
	2	4.627	49.0	D13	20.0	6.34	15.589	
	1	6.222	55.6	D13	20.0	6.34	18.426	
II	5	3.907	55.6	D13	20.0	6.34	11.570	0.0035 × 10.0 = 0.0350
	4	3.316	49.0	D13	20.0	6.34	11.172	
	3	4.374	49.0	D13	20.0	6.34	14.737	
	2	1.999	49.0	D13	20.0	6.34	6.735	
	1	3.907	55.6	D13	20.0	6.34	11.570	
III	5	0.000	55.6	D13	20.0	6.34	0.000	0.0035 × 10.0 = 0.0350
	4	23.440	55.6	D13	20.0	6.34	69.417	
	3	37.113	55.6	D13, D13	10.0	12.67	55.920	
	2	23.440	55.6	D13	20.0	6.34	69.417	
	1	0.000	55.6	D13	20.0	6.34	0.000	

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Footing

i) Examination at the time of ultimate which receives bending  
Under ordinary conditions

B = 100cm

NO	Md (kN·m)	d (cm)	Asn (cm <sup>2</sup> )	Diameter (mm)	Pitch (cm)	As (cm <sup>2</sup> )	Mud (kN·m)	$\gamma_i \cdot Md/Mud$
Seaside above	0.000	61.0	0.00	D13	20.0	6.34	114.698	0.00
below	171.815	61.0	10.53	D16, D19	10.0	24.26	424.575	0.45
Landside above	0.717	61.0	0.04	D16	20.0	9.93	178.470	0.00
below	0.582	61.0	0.03	D13	20.0	6.34	114.698	0.01

During an earthquake

B = 100cm

NO	Md (kN·m)	d (cm)	Asn (cm <sup>2</sup> )	Diameter (mm)	Pitch (cm)	As (cm <sup>2</sup> )	Mud (kN·m)	$\gamma_i \cdot Md/Mud$
Seaside above	0.000	61.0	0.00	D13	20.0	6.34	131.903	0.00
below	291.305	61.0	14.20	D16, D19	10.0	24.26	488.261	0.60
Landside above	119.445	61.0	5.73	D16	20.0	9.93	205.240	0.58
below	0.000	61.0	0.00	D13	20.0	6.34	131.903	0.00

ii) Under serviceability Examination to a crack

B = 100cm

NO	Ms (kN·m)	d (cm)	Diameter (mm)	Pitch (cm)	As (cm <sup>2</sup> )	$\sigma_{se}$ (N/mm <sup>2</sup> )	Crack width W(cm)	Permission crack width $W_{lim}$ (cm)
Seaside above	0.000	61.0	D13	20.0	6.34	0.000	0.0000	0.0035x8=0.0280
below	155.480	61.0	D16, D19	10.0	24.26	113.481	0.0215	0.0035x8=0.0280
Landside above	18.658	61.0	D16	20.0	9.93	32.412	0.0073	0.0035x8=0.0280
below	0.000	61.0	D13	20.0	6.34	0.000	0.0000	0.0035x8=0.0280

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iii) Examination to shearing

Sea side footing

NO	Vd (kN/m)	d (cm)	$\gamma_i$	$\gamma_b$	$\beta_d$	$\beta_p$	$\beta_n$	f <sub>vd</sub> (N/mm <sup>2</sup> )	V <sub>cd</sub> (kN/m)	$\gamma_i \cdot V_d/V_{cd}$
Ultimate limit state (Under ordinary conditions)										
Sea side above	0.000	54.0	1.10	1.30	1.16655	0.48910	1	0.529	125.374	0.00
below	223.408	54.0	1.10	1.30	1.16655	0.76574	1	0.529	196.287	1.25
Ultimate limit state (During an earthquake)										
Sea side above	0.000	54.0	1.00	1.15	1.16655	0.48910	1	0.529	141.727	0.00
below	378.888	54.0	1.00	1.15	1.16655	0.76574	1	0.529	221.889	1.71
Serviceability limit state										
Sea side above	0.000	54.0	—	1.00	1.16655	0.48910	1	0.577	177.775	0.00
below	202.170	54.0	—	1.00	1.16655	0.76574	1	0.577	278.326	0.73

The steel reinforcement of D13 is arranged at intervals of 200mm.

NO	A <sub>w</sub> (mm <sup>2</sup> )	f <sub>wyd</sub> (N/mm <sup>2</sup> )	$\alpha_s$ (°)	Z (mm)	$\gamma_b$	V <sub>cd</sub> (kN/m)	V <sub>sd</sub> (kN/m)	V <sub>yd</sub> (kN/m)	$\gamma_i \cdot V_d/V_{yd}$
Ultimate limit state (Under ordinary conditions)									
Sea side above	380.1	345.00	90	469.6	1.15	125.374	267.742	393.116	0.00
below	380.1	345.00	90	469.6	1.15	196.287	267.742	464.029	0.53
Ultimate limit state (During an earthquake)									
Sea side above	380.1	345.00	90	469.6	1.00	141.727	307.904	449.631	0.00
below	380.1	345.00	90	469.6	1.00	221.889	307.904	529.793	0.72
Serviceability limit state									
Sea side above	380.1	345.00	90	469.6	1.00	177.775	307.904	485.679	0.00
below	380.1	345.00	90	469.6	1.00	278.326	307.904	586.230	0.35

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Land side Footing

NO	Vd (kN/m)	d (cm)	$\gamma_i$	$\gamma_b$	$\beta_d$	$\beta_p$	$\beta_n$	f <sub>vcd</sub> (N/mm <sup>2</sup> )	V <sub>cd</sub> (kN/m)	$\gamma_i \cdot V_d / V_{cd}$
Ultimate limit state (Under ordinary conditions)										
Land side above	0.943	54.0	1.10	1.30	1.16655	0.56877	1	0.529	145.796	0.01
below	0.738	54.0	1.10	1.30	1.16655	0.48910	1	0.529	125.374	0.01
Ultimate limit state (During an earthquake)										
Land side above	155.275	54.0	1.00	1.15	1.16655	0.56877	1	0.529	164.813	0.94
below	0.000	54.0	1.00	1.15	1.16655	0.48910	1	0.529	141.727	0.00
Serviceability limit state										
Land side above	24.284	54.0	--	1.00	1.16655	0.56877	1	0.577	206.733	0.12
below	0.000	54.0	--	1.00	1.16655	0.48910	1	0.577	177.775	0.00

The steel reinforcement of D13 is arranged at intervals of 400mm.

NO	A <sub>w</sub> (mm <sup>2</sup> )	f <sub>wd</sub> (N/mm <sup>2</sup> )	$\alpha_s$ (°)	Z (mm)	$\gamma_b$	V <sub>cd</sub> (kN/m)	V <sub>sd</sub> (kN/m)	V <sub>yd</sub> (kN/m)	$\gamma_i \cdot V_d / V_{yd}$
Ultimate limit state (Under ordinary conditions)									
Land side above	126.7	345.00	90	469.6	1.15	145.796	44.624	190.420	0.01
below	126.7	345.00	90	469.6	1.15	125.374	44.624	169.998	0.01
Ultimate limit state (During an earthquake)									
Land side above	126.7	345.00	90	469.6	1.00	164.813	51.317	216.130	0.72
below	126.7	345.00	90	469.6	1.00	141.727	51.317	193.044	0.00
Serviceability limit state									
Land side above	126.7	345.00	90	469.6	1.00	206.733	51.317	258.050	0.09
below	126.7	345.00	90	469.6	1.00	177.775	51.317	229.092	0.00

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Case.2 Date

1. Design Conditions

(1) Dimensions

Crest elevation	+ 5.000 (m)
Crest elevation of caisson	+ 2.000 (m)
Bottom end of caisson	- 14.500 (m)

(2) Tidal levels

R.W.L	± 1.040 (m)
L.W.L	- 0.130 (m)

(3) Unit Weight

Reinforced concrete	24.00 (kN/m <sup>3</sup> )
Concrete lid	22.60 (kN/m <sup>3</sup> )
Filling materials(Air)	20.00 (kN/m <sup>3</sup> )
Filling materials(submerged)	10.00 (kN/m <sup>3</sup> )
Seawater	10.10 (kN/m <sup>3</sup> )
Materials of ballast (while afloat)	18.00 (kN/m <sup>3</sup> )
Materials of ballast (after construction)	20.00 (kN/m <sup>3</sup> )
Friction increasing mat	22.60 (kN/m <sup>3</sup> )

(4) Materials

Steel reinforcements

Tensile yield strength	$f'_{yk} = 345.0$ (N/mm <sup>2</sup> )
Design tensile yield strength	$f'_{yd} = f'_{yk} / \gamma_s$
Modulus of elasticity	$E_s = 200.0$ (kN/mm <sup>2</sup> )

Concrete

Compressive yield strength	$f'_{ck} = 24.0$ (N/mm <sup>2</sup> )
Design compressive yield strength	$f'_{cd} = f'_{ck} / \gamma_c$
Modulus of elasticity	$E_c = 25.0$ (kN/mm <sup>2</sup> )

(5) Arrangement of a steel reinforcement

Coverring for steel reinforcement

Outer wall Outer side	8.0 (cm)
Outer wall Inner side	6.0 (cm)
Bottom slab Outer side	8.0 (cm)
Bottom slab Inner side	6.0 (cm)
Footing Outer side	8.0 (cm)
Footing Inner side	8.0 (cm)

Use path

D 25 ~ D 13

Steel reinforcement interval

20.0 cm or 10.0 cm

Coefficient of earth pressure of filling

$K = 0.60$

(4)

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2. Design Loads

(1) While afloat

Sidewall

hydrostatic pressure of draft +1.000 (m) is considered.

$$Sf = (6.910 + 1.000 - 0.600 / 2 - 0.000) \times 10.10 = 76.86 \text{ (kN/m}^2\text{)}$$

Ultimate limit state

$$P = 1.1 \cdot Sf = 84.55 \text{ (kN/m}^2\text{)}$$

Serviceability limit state

$$P = 0.5 \cdot Sf = 38.43 \text{ (kN/m}^2\text{)}$$

Bottom Slab

It considers as the load which pulled the bottom slab deadweight from hydrostatic pressure at the bottom of caisson

$$Sf = (6.910 + 1.000) \times 10.10 = 79.89 \text{ (kN/m}^2\text{)}$$

$$Df = (-0.600 \times 24.00) + (0.000 \times 18.00) + (0.000 \times 22.60) = -14.40 \text{ (kN/m}^2\text{)}$$

Ultimate limit state

$$P = 1.1 \cdot Sf + 0.9 \cdot Df = 74.92 \text{ (kN/m}^2\text{)}$$

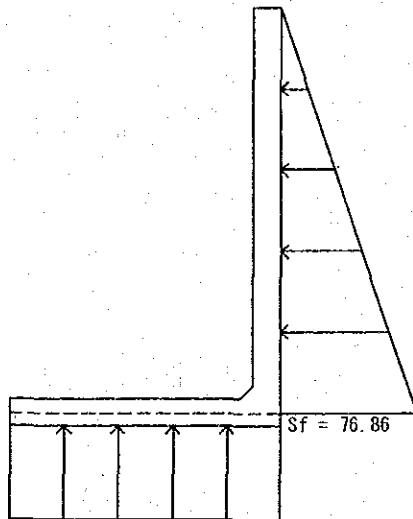
Serviceability limit state

$$P = 0.5 \cdot Sf + 0.5 \cdot Df = 32.75 \text{ (kN/m}^2\text{)}$$

Partition Walls

Although water pressure receives and compression power is received,

since it is generally safe, examination is omitted.



Ultimate limit state 74.92 Serviceability limit state 32.75

(5)

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(2) During Installation

Partition Walls

The hydrostatic head(1.00(m)) between chambers should be applied

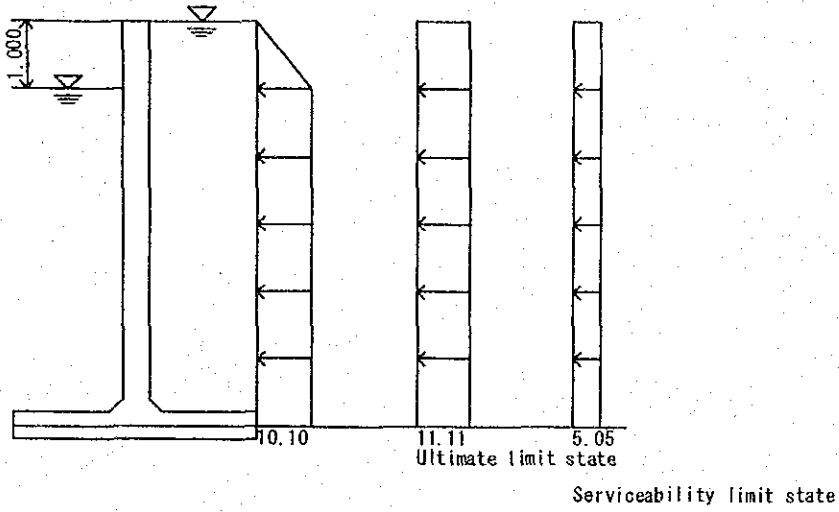
$$S = 1.000 \times 10.10 = 10.10 \text{ (kN/m}^2\text{)}$$

Ultimate limit state

$$Q = 1.1 \cdot S = 11.11 \text{ (kN/m}^2\text{)}$$

Serviceability limit state

$$Q = 0.5 \cdot S = 5.05 \text{ (kN/m}^2\text{)}$$



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(3) After Construction

Sidewall) perpendicular to levee normal

Internal earth pressure (K = 0.60)

$$P1 = (0.000 + 0.000 + 0.500 \times 22.60) \times 0.60 = 6.78 \text{ (kN/m}^2\text{)}$$

$$P2 = 6.78 + (4.600 \times 10.00) \times 0.60 = 34.38 \text{ (kN/m}^2\text{)}$$

$$D = 1/2 \times (6.78 + 34.38) \times 4.600 + 34.38 \times 11.100 = 476.29 \text{ (kN/m)}$$

Internal water pressure

$$P = 1.630 \times 10.10 = 16.46 \text{ (kN/m}^2\text{)}$$

$$S = 1/2 \times 16.46 \times 1.630 + 16.46 \times 14.070 = 245.01 \text{ (kN/m)}$$

Design loads

It converts into uniform load and triangular distribution load to which sum total load and load area become equal, and considers as design load.

Ultimate limit state

$$P = 1.1 \cdot D + 1.1 \cdot S = 793.43 \text{ (kN/m)}$$

Trapezoid load

Lower bottom

$$P = 34.38 \times 1.1 + 16.46 \times 1.1 = 55.92 \text{ (kN/m}^2\text{)}$$

Raised bottom

$$P = 2 \times \frac{793.43}{16.200} - 55.92 = 42.03 \text{ (kN/m}^2\text{)}$$

Serviceability limit state

$$P = 1.0 \cdot D + 1.0 \cdot S = 721.30 \text{ (kN/m)}$$

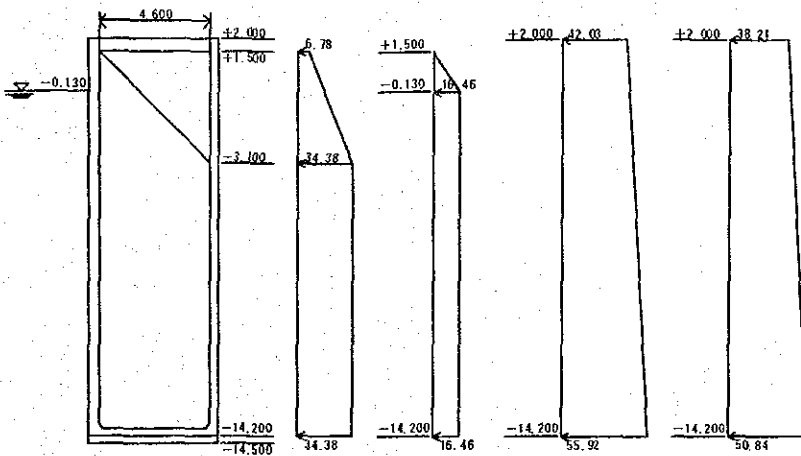
Trapezoid load

Lower bottom

$$P = 34.38 \times 1.0 + 16.46 \times 1.0 = 50.84 \text{ (kN/m}^2\text{)}$$

Raised bottom

$$P = 2 \times \frac{721.30}{16.200} - 50.84 = 38.21 \text{ (kN/m}^2\text{)}$$



Internal earth pressure D Internal water pressure S Ultimate limit state Design load U Serviceability limit state Design load S

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Front wall (parallel to centerline : seaside)

Internal earth pressure (K = 0.60)

$$P1 = (0.000 + 0.000 + 0.500 \times 22.60) \times 0.60 = 6.78 \text{ (kN/m}^2\text{)}$$

$$P2 = 6.78 + (4.400 \times 10.00) \times 0.60 = 33.18 \text{ (kN/m}^2\text{)}$$

$$D = 1/2 \times (6.78 + 33.18) \times 4.400 + 33.18 \times 11.300 = 462.85 \text{ (kN/m)}$$

Internal water pressure

$$P = 1.630 \times 10.10 = 16.46 \text{ (kN/m}^2\text{)}$$

$$S = 1/2 \times 16.46 \times 1.630 + 16.46 \times 14.070 = 245.01 \text{ (kN/m)}$$

Design loads

It converts into uniform load and triangular distribution load to which sum total load and load area become equal, and considers as design load.

Ultimate limit state

$$P = 1.1 \cdot D + 1.1 \cdot S = 778.65 \text{ (kN/m)}$$

Trapezoid load

$$P = 33.18 \times 1.1 + 16.46 \times 1.1 = 54.60 \text{ (kN/m}^2\text{)}$$

Raised bottom

$$P = 2 \times \frac{778.65}{16.200} - 54.60 = 41.53 \text{ (kN/m}^2\text{)}$$

Serviceability limit state

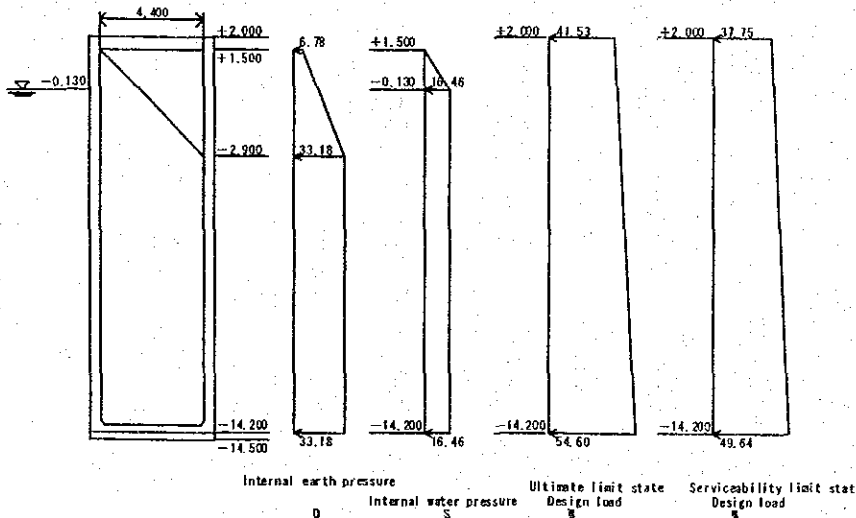
$$P = 1.0 \cdot D + 1.0 \cdot S = 707.86 \text{ (kN/m)}$$

Trapezoid load

$$P = 33.18 \times 1.0 + 16.46 \times 1.0 = 49.64 \text{ (kN/m}^2\text{)}$$

Raised bottom

$$P = 2 \times \frac{707.86}{16.200} - 49.64 = 37.75 \text{ (kN/m}^2\text{)}$$



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Rear wall (parallel to centerline : landside)

Internal earth pressure (K = 0.60)

$$P1 = (0.000 + 0.000 + 0.500 \times 22.60) \times 0.60 = 6.78 \text{ (kN/m}^2\text{)}$$

$$P2 = 6.78 + (4.400 \times 10.00) \times 0.60 = 33.18 \text{ (kN/m}^2\text{)}$$

$$D = 1/2 \times (6.78 + 33.18) \times 4.400 + 33.18 \times 11.300 = 462.85 \text{ (kN/m)}$$

Internal water pressure

$$P = 1.630 \times 10.10 = 16.46 \text{ (kN/m}^2\text{)}$$

$$S = 1/2 \times 16.46 \times 1.630 + 16.46 \times 14.070 = 245.01 \text{ (kN/m)}$$

Design loads

It converts into uniform load and triangular distribution load to which sum total load and load area become equal, and considers as design load.

Ultimate limit state

$$P = 1.1 \cdot D + 1.1 \cdot S = 778.65 \text{ (kN/m)}$$

Trapezoid load

Lower bottom

$$P = 33.18 \times 1.1 + 16.46 \times 1.1 = 54.60 \text{ (kN/m}^2\text{)}$$

Raised bottom

$$P = 2 \times \frac{778.65}{16.200} - 54.60 = 41.53 \text{ (kN/m}^2\text{)}$$

Serviceability limit state

$$P = 1.0 \cdot D + 1.0 \cdot S = 707.86 \text{ (kN/m)}$$

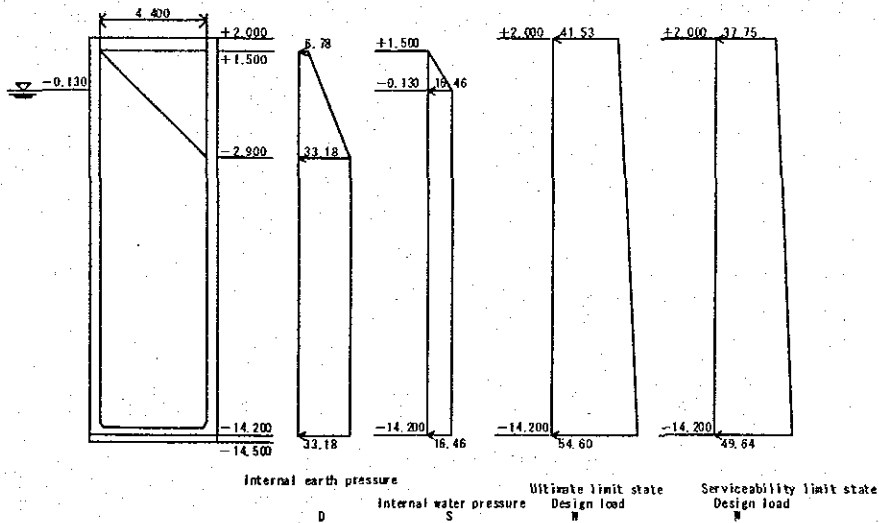
Trapezoid load

Lower bottom

$$P = 33.18 \times 1.0 + 16.46 \times 1.0 = 49.64 \text{ (kN/m}^2\text{)}$$

Raised bottom

$$P = 2 \times \frac{707.86}{16.200} - 49.64 = 37.75 \text{ (kN/m}^2\text{)}$$



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Summary of design load

Ultimate limit state

Sidewall (perpendicular to levee normal)

Load from an inner side

Raised bottom = 42.03 (kN/m<sup>2</sup>)

Lower bottom = 55.92 (kN/m<sup>2</sup>)

Load from an outside

Raised bottom = 0.00 (kN/m<sup>2</sup>)

Lower bottom = 0.00 (kN/m<sup>2</sup>)

Front wall (parallel to centerline:seaside)

Load from an inner side

Raised bottom = 41.53 (kN/m<sup>2</sup>)

Lower bottom = 54.60 (kN/m<sup>2</sup>)

Load from an outside

Raised bottom = 0.00 (kN/m<sup>2</sup>)

Lower bottom = 0.00 (kN/m<sup>2</sup>)

Front wall (parallel to centerline:landside)

Load from an inner side

Raised bottom = 41.53 (kN/m<sup>2</sup>)

Lower bottom = 54.60 (kN/m<sup>2</sup>)

Load from an outside

Raised bottom = 0.00 (kN/m<sup>2</sup>)

Lower bottom = 0.00 (kN/m<sup>2</sup>)

Serviceability limit state

Sidewall (perpendicular to levee normal)

Load from an inner side

Raised bottom = 38.21 (kN/m<sup>2</sup>)

Lower bottom = 50.84 (kN/m<sup>2</sup>)

Load from an outside

Raised bottom = 0.00 (kN/m<sup>2</sup>)

Lower bottom = 0.00 (kN/m<sup>2</sup>)

Front wall (parallel to centerline:seaside)

Load from an inner side

Raised bottom = 37.75 (kN/m<sup>2</sup>)

Lower bottom = 49.64 (kN/m<sup>2</sup>)

Load from an outside

Raised bottom = 0.00 (kN/m<sup>2</sup>)

Lower bottom = 0.00 (kN/m<sup>2</sup>)

Front wall (parallel to centerline:landside)

Load from an inner side

Raised bottom = 37.75 (kN/m<sup>2</sup>)

Lower bottom = 49.64 (kN/m<sup>2</sup>)

Load from an outside

Raised bottom = 0.00 (kN/m<sup>2</sup>)

Lower bottom = 0.00 (kN/m<sup>2</sup>)

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Bottom Slab, Footing  
 Under ordinary conditions  
 Combination of load (Design loads)

Deadweight—Partition room  
 D = concrete lid + sand of filling + materials of ballast + bottom slab + loading load  
 =  $0.50 \times 22.60 + 15.40 \times 20.00 + 0.00 \times 20.00$   
 +  $0.60 \times 24.00 + 0.00$   
 = 333.70 (kN/m<sup>2</sup>)

Deadweight—Footing  
 Deadweight + Loading load  
 The tip by the side of the sea =  $0.50 \times (24.00 - 10.10) + 0.000 = 6.95$  (kN/m<sup>2</sup>)  
 The root by the side of the sea =  $0.70 \times (24.00 - 10.10) + 0.000 = 9.73$  (kN/m<sup>2</sup>)  
 The root by the side of land =  $0.70 \times (24.00 - 10.10) + 219.680 = 229.41$  (kN/m<sup>2</sup>)  
 The tip by the side of land =  $0.50 \times (24.00 - 10.10) + 221.680 = 228.63$  (kN/m<sup>2</sup>)

Bottom slab reaction — under ordinary condition D0  
 sea side = 320.76 (kN/m<sup>2</sup>), land side = 199.65 (kN/m<sup>2</sup>), Action width = 21.000 (m)

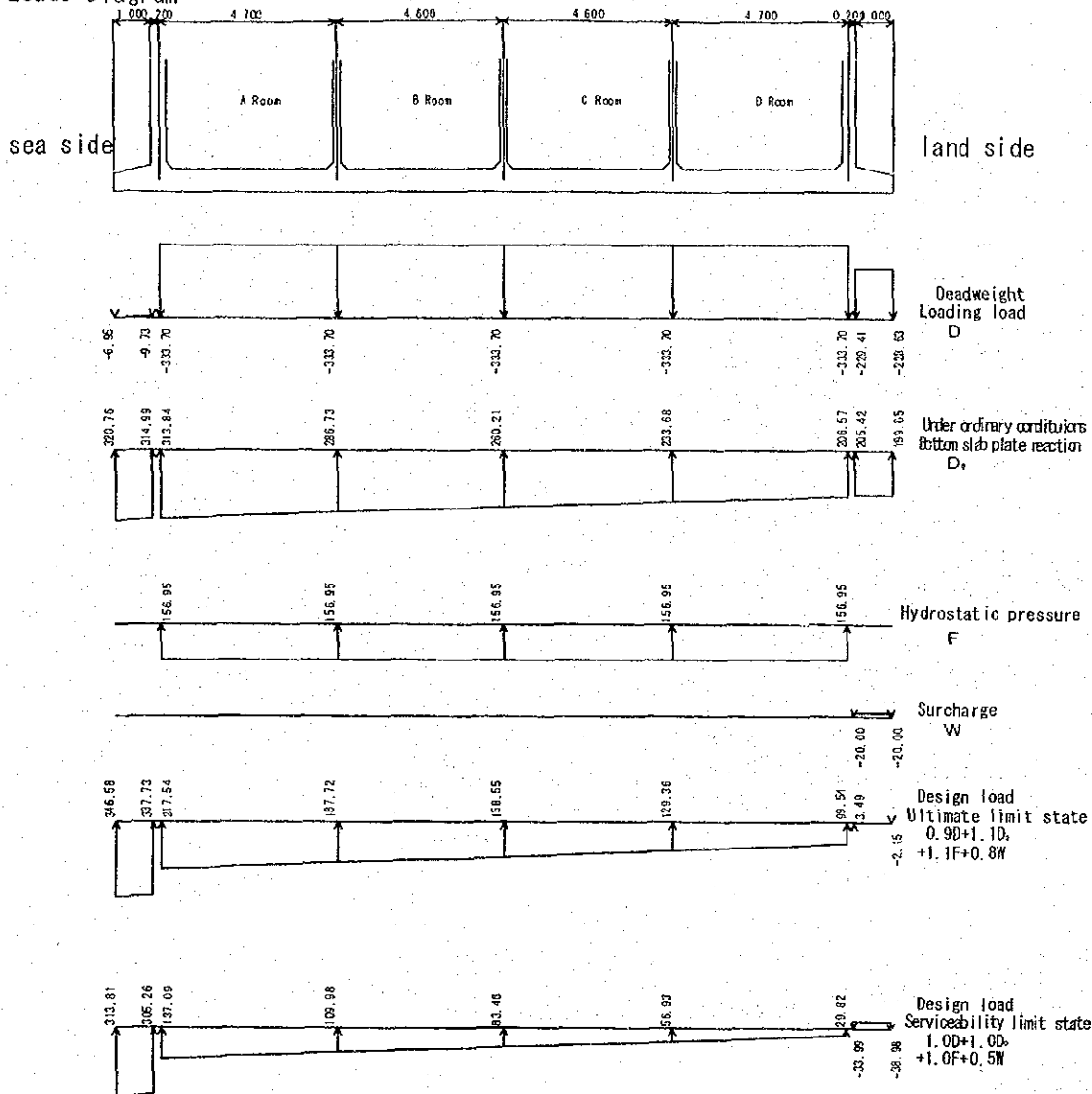
Hydrostatic pressure Under R.W.L  
 $F = (R.W.L - \text{Installation depth of water}) \cdot \gamma_w = 156.95$  (kN/m<sup>2</sup>)  
 =  $[1.040 - (-14.500)] \times 10.10$

Surcharge—Partition room  
 W = 0.00 (kN/m<sup>2</sup>)

Surcharge—Footing  
 The tip by the side of the sea = 0.00 (kN/m<sup>2</sup>)  
 The root by the side of the sea = 0.00 (kN/m<sup>2</sup>)  
 The root by the side of land = 20.00 (kN/m<sup>2</sup>)  
 The tip by the side of land = 20.00 (kN/m<sup>2</sup>)

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Loads Diagram



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During an earthquake  
Combination of load (Design loads)

Deadweight--Partition room

$$D = \text{concrete lid} + \text{sand of filling} + \text{materials of ballast} + \text{bottom slab} + \text{loading load}$$

$$= 0.50 \times 22.60 + 15.40 \times 20.00 + 0.00 \times 20.00$$

$$+ 0.60 \times 24.00 + 0.00$$

$$= 333.70 \text{ (kN/m}^2\text{)}$$

Deadweight--Footing

Deadweight+Loading load

$$\text{Sea side tip} = 0.50 \times (24.00 - 10.10) + 0.000 = 6.95 \text{ (kN/m}^2\text{)}$$

$$\text{Sea side root} = 0.70 \times (24.00 - 10.10) + 0.000 = 9.73 \text{ (kN/m}^2\text{)}$$

$$\text{Land side root} = 0.70 \times (24.00 - 10.10) + 219.680 = 229.41 \text{ (kN/m}^2\text{)}$$

$$\text{Land side tip} = 0.50 \times (24.00 - 10.10) + 221.680 = 228.63 \text{ (kN/m}^2\text{)}$$

Bottom slab reaction --During an earthquake R'

$$\text{Sea side} = 601.37 \text{ (kN/m}^2\text{)}, \text{ Land side} = 0.00 \text{ (kN/m}^2\text{)}, \text{ Action width} = 18.420 \text{ (m)}$$

Hydrostatic pressure Under R.W.L

$$F = (\text{R.W.L Hydrostatic pressure Under R.W.L})$$

$$= \{ 1.040 - (-14.500) \} \times 10.10 = 156.95 \text{ (kN/m}^2\text{)}$$

Surcharge--Partition room

$$W = 0.00 \text{ (kN/m}^2\text{)}$$

Surcharge--Footing

$$\text{The tip by the side of the sea} = 0.00 \text{ (kN/m}^2\text{)}$$

$$\text{The root by the side of the sea} = 0.00 \text{ (kN/m}^2\text{)}$$

$$\text{The root by the side of land} = 10.00 \text{ (kN/m}^2\text{)}$$

$$\text{The tip by the side of land} = 10.00 \text{ (kN/m}^2\text{)}$$

The irregular form in case of an earthquake is converted into uniform load and triangular distribution load.

• Calculation of  $\Sigma A$

$$1/2 \times (-68.360 - 176.750) \times 3.320 = -406.88 \text{ (kN/m}^2\text{)}$$

$$1/2 \times (-176.750 - 176.750) \times 1.380 = -243.92 \text{ (kN/m}^2\text{)}$$

$$\Sigma A = -650.80 \text{ (kN/m}^2\text{)}$$

• Conversion load

$$P1 = -176.75 \text{ (kN/m}^2\text{)}$$

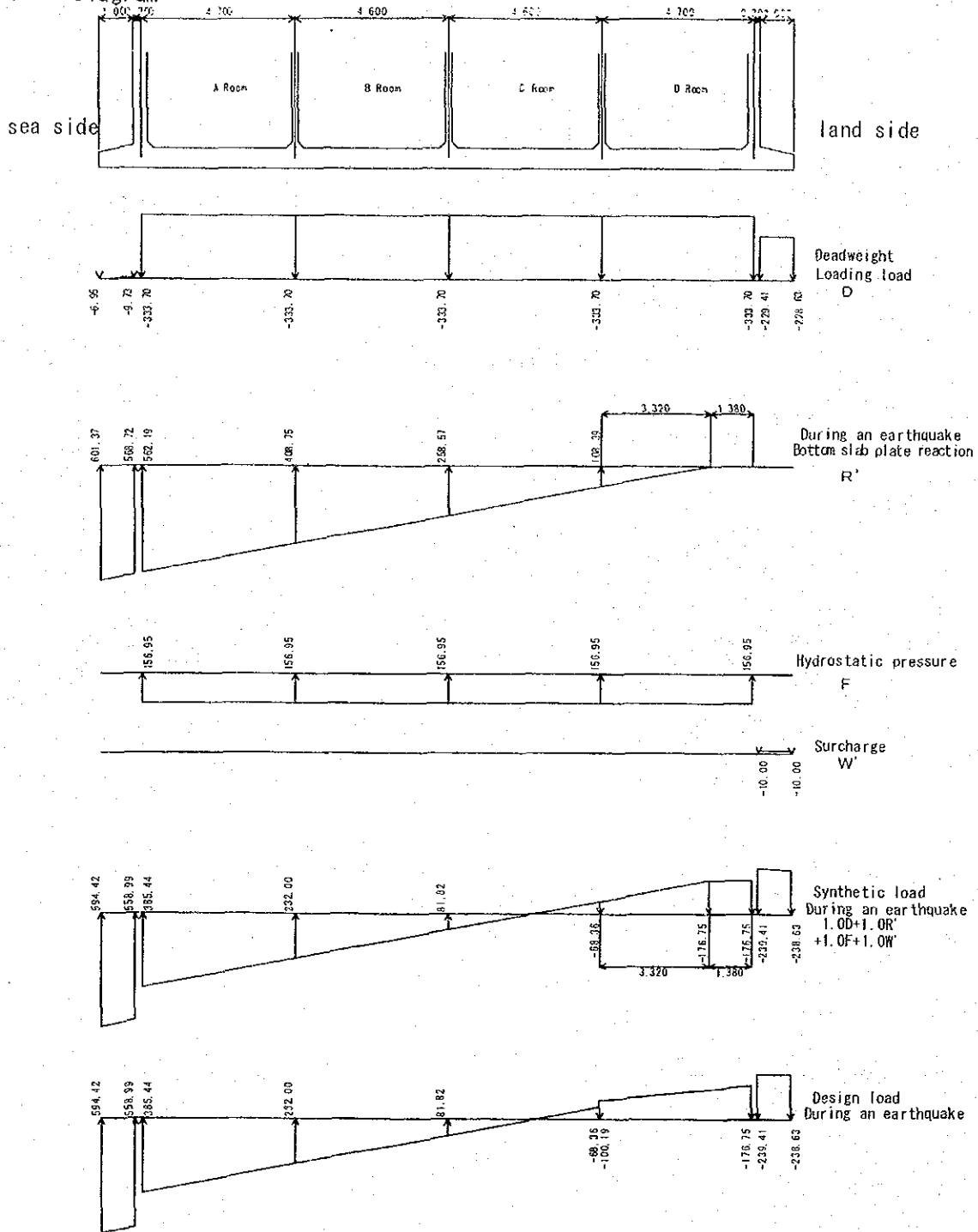
$$P2 = (2 \cdot \Sigma A / L) - P1$$

$$= (2 \times (-650.80) / 4.700) - (-176.75) = -100.19 \text{ (kN/m}^2\text{)}$$

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Loads Diagram



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(4) Dislodging of partition wall

To dislodging of a side wall, the maximum load strength of the composite load calculated on the occasion of the design of a side wall is used. (as the load per unit length)

a) Dislodging of partition wall and side wall

1) Partition wall(perpendicular to levee normal) and Side wall(parallel to centerline)

$$L = \frac{4.900 + 4.875}{2} = 4.888 \text{ (m)}$$

$$T_d = 54.60 \times 4.888 = 266.88 \text{ (kN/m)}$$

2) Partition wall(parallel to centerline) and Side wall(perpendicular to levee normal)

$$L = \frac{4.700 + 4.600}{2} = 4.650 \text{ (m)}$$

$$T_d = 55.92 \times 4.650 = 260.03 \text{ (kN/m)}$$

b) Dislodging of partition wall and bottom wall

The maximum facing-down load is used for load among the external force which acts on the bottom slab at the time of completion

Load calculation of the bottom slab is used  $W = 176.75 \text{ (kN/m}^2\text{)}$

1) Partition wall(perpendicular to levee normal) and Bottom slab

$$P1 = \frac{2}{3} \cdot W \cdot LX$$
$$= \frac{2}{3} \times 176.75 \times 4.700 = 553.82 \text{ (kN/m)}$$

2) Partition wall(parallel to centerline) and Bottom slab

$$P2 = W \cdot LX \cdot \left(1 - \frac{LX^2}{3 \times LY^2}\right)$$
$$= 176.75 \times 4.700 \times \left(1 - \frac{4.700^2}{3 \times 4.900^2}\right) = 575.96 \text{ (kN/m)}$$

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3. The component which acts on each part material calculation of power

Side wall

Calculation of bending moment uses the monography of slab

Note) The mark of bending moment (+): inside tensile  
(-): outside tensile

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