



## 4.2 BEARING CAPACITY OF FOUNDATION

For example, a concrete pile with the dimensions 0.4 x 0.4 x 26m is taken, the calculation is as follows:

$$\begin{aligned} P &= 1.6 \text{ m} \\ A_s &= 0.16 \text{ m}^2 \end{aligned}$$

Applying formula

$$\begin{aligned} Q_{ult} &= C N_{cs} A_s + C_A 2 \pi R L \\ \text{and } Q_{ult} &= q A_s + ((K_H/2) \gamma_{sub} L^2 \text{tg } \delta) * P \\ \text{where } q &= \gamma_{sub} R N_\gamma + K_B * \gamma_{sub} * L * N_q - \gamma_{sub} * L \end{aligned}$$

At the borehole UB(2)-03, with following parameters:

- Layer 1 + layer 2:

$$\begin{aligned} L &= 24.0 \text{ m} \\ C &= 0.074 \text{ kg/cm}^2 = 0.740 \text{ T/m}^2 \\ C_A &= 0.740 \text{ T/m}^2 \\ \gamma_{sub} &= 0.490 \text{ T/m}^3 \end{aligned}$$

- Layer 4b:

$$\begin{aligned} L &= 2.0 \text{ m} \\ \phi &= 30^\circ \\ \phi' &= 35^\circ \\ \gamma_{sub} &= 1.000 \text{ T/m}^3 \\ K_H &= 0.5 \\ \text{tan } \delta &= 0.25 \\ K_B &= 0.4 \\ N_\gamma &= 40 \\ N_q &= 40 \\ \text{tan } \delta &= 0.25 \end{aligned}$$

Because the pile gets through 3 layers so the total bearing capacity is the sum of skin friction through all layers (layer 1 + layer 2, layer 4b) and bearing capacity of tip in layer 4b:

Calculation:

$$\begin{aligned} \text{Layer 1 + 2 : } Q &= C_A 2 \pi R L \\ Q_{1+2} &= 0.465 \text{ T/m}^2 \times 1.6 \text{ m} \times 24.0 \text{ m} = 17.856 \text{ T} \end{aligned}$$

Layer 4b:

$$\begin{aligned} Q_{ult} &= q A_s + ((K_H/2) \gamma_{sub} L^2 \text{tg } \delta) * P \\ q &= \gamma_{sub} R N_\gamma + K_B * \gamma_{sub} * L * N_q - \gamma_{sub} * L \end{aligned}$$

$$\begin{aligned} Q_{4b-1} &= P_o \times K_H \times \text{tan } \delta \times L \times P \\ P_o &= 0.490 \times 24.0 + 1.000 \times 2.0/2 = 12.760 \text{ T/m}^2 \\ Q_{4b-1} &= 12.760 \times 0.5 \times 0.25 \times 4.0 \times 1.6 = 5.104 \text{ T} \end{aligned}$$

$$Q_{4b-2} = q A_s$$

$$q = \gamma_{sub} R N_\gamma + K_B * \gamma_{sub} * L * N_q - \gamma_{sub} * L$$

$$\begin{aligned} q &= 1.000 \times 0.2 \times 40 + 0.4 \times 1.000 \times 2.0 \times 40 - 1.000 \times 2.0 = 38.000 \text{ T} \\ Q_{4-2} &= q \times A_s = 38.000 \times 0.16 = 6.080 \text{ T} \end{aligned}$$

$$Q_{total} = Q_{1+2} + Q_{4b-1} + Q_{34b-2} =$$

$$Q_{total} = 17.856 \text{ T} + 5.104 \text{ T} + 6.080 \text{ T} = 29.040 \text{ T}$$

$$\text{Choosing safety factor } F_s = 3, \quad Q_{ult} = 9.68 \text{ T}$$

### Conclusion

With the above mentions, some following remarks can be made:

- Up to 30.0 m deep, the foundation is constructed by Holocene deposit layers, with thickness more 30m (very soft - soft, Organic Clay), have low bearing capacity.

- According to load of construction, foundation can be put on the layer 2 after improving, may be to use cajeput pile foundation, sand pile foundation or draining plastic stripes (for small load construction) or use concrete pile foundation to transmit the construction load to Pleistocene deposit soil layers and in this case have to drill some more deeper boreholes into the Pleistocene deposit at the borehole UB(2)-2.

Calculation for a concrete pile at borehole UB(2)-3 with section (0.4x0.4m) and length of 26.0m has following results:

$$Q_{ult} = 9.680 \text{ T}$$

#### 4.3 . OBSTACLES DURING DRILLING.

During drilling at the boreholes caught some rotten woods have not yet completely disintegrated when drill through layer 2 (Very soft, high plasticity blackish grey).

#### REFERENCES

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- 4./ W .M .Cobel - Soil mechanics ( Design manual 7-1 ) - May 1982 .