ABORATORY TEST ON BORING

PUMP STATION - BEN ME COC 2

.

IG	IG			Natural water content w (%)	Unit weight		ţ			ion	Atterberg limit		×		Consolidation				
#50	<u>#50 #100 #200</u>		Natural Υ (g/cm ³)		Dry Ya (g/cm ³)	Specific gravity Gs	Porcsity n (%)	17	ee saturation S (%)	Liquid limit LL (%)	Plastic limit PL (%)	Plastic index Pl (%)	Liquidity index B	Unconfined compression q. (Kg/cm ²)	Compression index Cc	Coefficient of consolidation Cv (cm ² /s)	Preconsolidation pressure Pc (Kg/cm ²)	Coefficient of	
0.3	0.15	0.075	0.005	Natural	Y (20%	Spec		×	Degree S	Liqui	Plasti	PL Plastic Pl	Liqu	Unce comp	Compres index	Coeff consc Cv	Precon pre	Coeff
16)	(17)	(18)	(19)	(20)	(21)	(22)	(23)	(24)	(25)	(26)	(27)	(29)	(29)	(30)	(31)	(32)	(33)	(34)	1
	100	96.4	30.1	71.29	1.514	0.884	2.569	0.66	1.906	96.1									
	100	95.4	29.2	68.29	1.563	0.929	2.567	0.64	1.764	99.4	65.6	36.2	29.4	1.09	0.208	1.1569	1.94E-04	1.057	-
	100	94.9	50.2	80.21	1.469	0.815	2.601	0.69	2.191	95.2									
	100	96.5	62.6	91.01	1.436	0.752	2.585	0.71	2.438	96.5	89.0	45.3	43.7	1.05	0.078	1.0109	2.11E-04	0.427	
	100	97.2	63.4	89.24	1.453	0.768	2.603	0.71	2.390	97.2									
	100	99.0	68.2	87.38	1.420	0.758	2.595	0.71	2.424	93.5	68.3	36.2	32.1	1.59					
100	90.2	83.2	50.6	80.23	1.429	0.793	2.698	0.71	2.403	90.1									
)3.2	84.2	75.2	46.3	69.91	1.454	0.856	2.601	0.67	2.039	89.2	64.7	34.3	30.4	1.17	0.153	1.2712	1.83E-04	1.048	
14.2	90.2	80,1	55.2	70.56	1.501	0.880	2.621	0.66	1.978	93.5									
7.87	94.1	93.1	63.9	84.23	1.488	0.808	2.605	0,69	2.225	98.6	70.6	37.6	33.0	1.41	0.222	1.2782	3.37E-04	1.137	
15.3	93.2	92.1	61.4	80.21	1.498	0.831	2.623	0.68	2.155	97.6									
2.6	9.7	8	2.7	15.33	1.779	1.543	2.637	0.42	0.710	57.0									
3.4	10.3	9.1	4.2	17.21	1.979	1.688	2.643	0.36	0.565	80.5									
17.9	30.9	23.6	14.5	14.87	1.995	1.737	2.645	0.34	0.523	75.2	33.5	20.4	13.1	-0.42					
i6.7	31.2	25.3	16.8	15.91	2.018	1.741	2.651	0.34	0.523	80.7									

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BORING No : UB(2) - 03							
Coefficient of volume compressibility m, (cm ² /g)	Coefficient of permeability k ₂₀ (cm/s)	REMARK					
(35)	(36)	(37)					
1.19E-04	2.21E-08						
1.66E-04	4.32E-08						
1.18E-04	2.03E-08						
1.02E-04	3.74E-08						

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4.2 BEARING CAPACITY OF FOUNDATION

For example, a concrete pile with the dimensions 0.4 x 0.4 x 26m is taken. the calculation is as follows:

P	=.	1.6 m
As	=	0.16 m^2

Applying formula

Qult = C Ncs As + $C_A 2 \pi R L$ q As + ((K_H/2) γ sub L² tg δ) * P Qult = and = $\gamma sub R N\gamma + K_B * \gamma sub * L * Nq - \gamma sub * L$ where q

At the borehole UB(2)-03, with following parameters :

- Layer 1 + layer 2 :

L = 24.0 m. $0.074 \text{ kg/cm}^2 = 0.740 \text{ T/m}^2$ C = CA 0.740 T/m^2 = 0.490 T/m^3 $\gamma sub =$

- Layer 4b :

L	=	2.0 m .
ф	=	30°
φ'	=	35°
ysub	=	1.000 T/m^3
K _H	=	0.5
tan \delta	=	0.25
KB	=	0.4
Nγ	=	40
Nq	=	40
tan \delta	=	0.25

Because the pile gets through 3 layers so the total bearing capacity is the sum of skin friction through all layers (layer 1 + layer 2, layer 4b) and bearing capacity of tip in layer 4b:

Calculation : Layer 1+2: $C_A 2 \pi R L$ Q =

 $0.465 \text{ T/m}^2 \text{ x}$ $Q_{1+2} =$ 1.6m x 24.0 m 17.856 T. =

Layer 4b q As + ((K_H/2) γ sub L² tg δ) * P Oult = γ sub R N γ + K_B * γ sub * L * Nq - γ sub * L 9 = Po $x K_H x \tan \delta x L x P$ $Q_{4b-1} =$ Po $0.490 \times 24.0 + 1.000 \times 2.0/2 = 12.760 \text{ T/m}^2$ = Q4b-1 = $12.760 \times 0.5 \times 0.25 \times 4.0 \times 1.6 =$ 5.104 T $Q_{4b-2} =$ q As $q = \gamma sub R N\gamma + K_B * \gamma sub * L * Nq - \gamma sub * L$ $q = 1.000 \times 0.2 \times 40 + 0.4 \times 1.000 \times 2.0 \times 40 - 1.000 \times 2.0 = 38.000 T$ $Q_{4-2} =$ q x As 38:000 x 0.16 = 6.080 T = $Q_{1+2} + Q_{4b-1} + Q_{34b-2} =$ $Q_{total} =$ Q_{total} = 17.856T + 5.104 T + 6.080 T =29.040 T Choosing safety factor Fs 9.68 T. = 3 Qult =

Conclusion

bearing capacity.

With the above mentions, some following remarks can be made : - Up to 30.0 m deep, the foundation is constructed by Holocene deposit layers, with thickness more 30m (very soft - soft, Organic Clay), have low

- According to load of construction, foundation can be put on the layer 2 after improving, may be to use cajeput pile foundation, sand pile foundation or draining plastic stripes (for small load construction) or use concrete pile foundation to transmit the construction load to Pleistocene deposit soil layers and in this case have to drill some more deeper boreholes into the Pleistocene deposit at the borehole UB(2)-2.

Calculation for a concrete pile at borehole UB(2)-3 with section (0.4x0.4m) and length of 26.0m has following results : Qult =9.680 T.

4.3. OBSTACLES DURING DRILLING.

During drilling at the boreholes caught some rotten woods have not yet completely disintegrated when drill through layer 2 (Very soft, high plasticity blackish grey).

1./ T.Will . Lambe - Soil testing for engineers - M.I.T , 1974 .

- 2./ American Society for testing materials ASTM 1987.
- 3./ P.Corradi Design manual soil machanics foundation and earth -1962.
- 4./W.M.Cobel Soil mechanics (Design manual 7-1) May 1982.