7.1.6 Water Supply Facility

### CALCULATION FOR WATER SUPPLY FACILITY (FOR UNDER GROUND PART)

(The calculation based on Japanese standard)

1-GEOMETRY DATUM FOR CALCULATION (As shown on drawings attached):

(in one and and and	und all.	
Ground level:	GL	2.2
Ground water level:	GW	0.0
Bottom level of chlorination	house: BL	-3.1
Bottom level of sand filtratio	n tank: BL	-2.23

Water level in filtration works: 3.28

Thickness of chlorination house bottom 0.70 m

Thickness of filtration works bottom: 0.70 m

Average thickness of non-reinforced concrete layer (Chlorination house): 0.25 m

2-PARAMETERS FOR CALCULATION:

Concrete: Grade C21,	Rn =	70
	RS=	3.6
Reinforcement type JIS:	Ra=	1600

Back fill sand: Ye 1.80T/m3 ; Coefficient of earth pressure at 0.5

### Internal friction 20deg 2.1-BOUNDARY CONDITION:

Considering R.C piles as springs for analysing. Please refer to pile calculation part for further information about spring coefficients

# 3-LOAD CALCULATION (BASE ON JAPANESE STANDARD):

3.1- Maximum loads from architect part to be taken in calculation as in analysis 3.2- Soil load:

In case of ground water level at 0.00 (Permanent case):

-Horizontal triangle distributed load due to earth under ground water level (Outside of chlorination house): p<sub>h1</sub>=(γ<sub>s</sub>-1)x(GWL-BL)xK<sub>o</sub>+(GWL-BL)x1.0+p<sub>h3</sub> = 6.32T/m2

-Horizontal triangle distributed load due to earth under ground water level (Outside of filtration works): p<sub>h2</sub>=(γ<sub>ε</sub>-1)x(GWL-BL)xK<sub>6</sub>+(GWL-BL)x1.0+p<sub>h3</sub> = 5.10T/m2

Horizontal triangle distributed load due to earth above ground water level

ph3=94(GL-GWL)K0= 1.98T/m2

-Horizontal uniform load due to surcharge load on ground surface:

Ph-surch<sup>=</sup> 0.50T/m2xKo = 0.25T/m2

-Horizontal uniform load due to cover soil:

Ph-coversoi = 1.8x1.20 xKo = 1.08T/m2

-Uplift pressure to bottom of chlorination house for this case:

Pupili=(Hground water)x1.0= 3.80T/m2

-Uplift pressure to bottom of filtration work for this case: Pupin=(Hground water)x1.0= 2.93T/m2

# 3.3-Water load in filtration works:

-Water load in side tanks:

# Pwater = 5.51T/m2

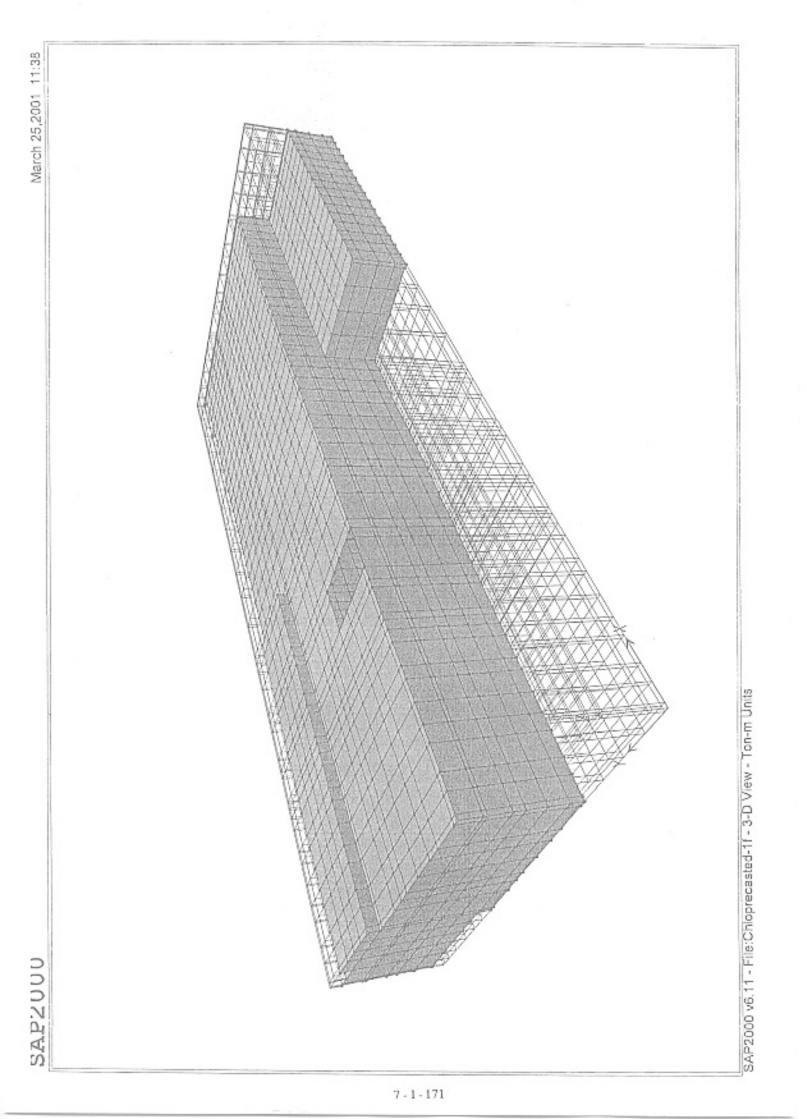
# 3.4-Load of non-reinforced concrete layer on bottom slab:

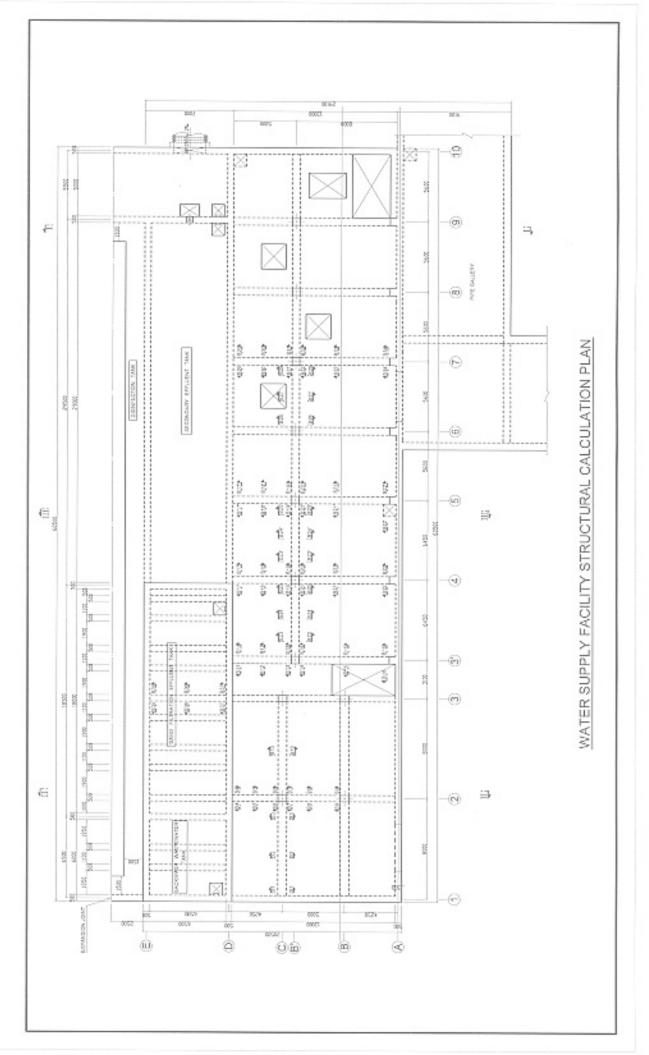
(Due to machanical supports and cinder concrete) -Uniform load: q<sub>conta</sub> 2.5x0.25= 0.625T/m2

### 3.5-Live load:

-Live load for all operating floor and walking way : q<sub>ive</sub> = 0.50T/m2 -Live load for electric room: 1.00T/m2

-Live load due to chlorination tank : 4.40T/m2





# 4-ANALYSING BY SAP 2000: THERE ARE 4 COMBOS FOR ANALYSING AS ATTACHED HEREAFTER: ALL THE LOADS, FACTORS, AND OTHER INPUT DATUM TO BE TAKEN IN ANALYSIS AND CALCULATED BY SAP 2000

LOAD COMBINATION MULTIPLIERS

COMBO TYPE CASE FACTOR TYPE TITLE

COMB1 ADD

Disinfection tanks full of water, water supply facility tanks empty, and right wind load

SELF 1.0000 STATIC(DEAD) MANBOTTO 1.0000 STATIC(LIVE) WATERDIS 1.0000 STATIC(LIVE) EARTH 1.0000 STATIC(OTHER) ELECFLOO 1.0000 STATIC(OTHER) UPLIFT 1.0000 STATIC(OTHER) VVEHICLE 1.0000 STATIC(OTHER) VVEHICLE 1.0000 STATIC(UVE) ACHITEC2 1.0000 STATIC(UVE) FILTER 1.0000 STATIC(UVE) CHLOTANK 1.0000 STATIC(UVE) BRICKWAL 1.0000 STATIC(DEAD) MANFFLOO 1.0000 STATIC(UVE) COVERSOI 1.0000 STATIC(OTHER)

COMB2 ADD

Water supply facility tanks full of water, disinfection tanks empty, and right wind load

SELF 1.0000 STATIC(DEAD) MANBOTTO 1.0000 STATIC(LIVE) EARTH 1.0000 STATIC(OTHER) ELECFLOO 1.0000 STATIC(OTHER) UPLIFT 1.0000 STATIC(OTHER) VVEHICLE 1.0000 STATIC(OTHER) ACHITEC2 1.0000 STATIC(LIVE) ACHITEC2 1.0000 STATIC(LIVE) FILTER 1.0000 STATIC(LIVE) CHLOTANK 1.0000 STATIC(LIVE) BRICKWAL 1.0000 STATIC(DEAD) MANFFLOO 1.0000 STATIC(LIVE) WATERFIL 1.0000 STATIC(LIVE) COVERSOI 1.0000 STATIC(LIVE)

COMB3 ADD

Disinfection tanks full of water water supply facility tanks empty, and left wind load

SELF 1.0000 STATIC(DEAD) MANBOTTO 1.0000 STATIC(LIVE) WATERDIS 1.0000 STATIC(LIVE) EARTH 1.0000 STATIC(OTHER) ELECFLOO 1.0000 STATIC(OTHER) UPLIFT 1.0000 STATIC(OTHER) VVEHICLE 1.0000 STATIC(LIVE) FILTER 1.0000 STATIC(LIVE) CHLOTANK 1.0000 STATIC(LIVE) BRICKWAL 1.0000 STATIC(DEAD) MANFFLOO 1.0000 STATIC(LIVE) ACHITEC1 1.0000 STATIC(OTHER) COVERSOI 1.0000 STATIC(OTHER)

COMB4 ADD

Water supply facility tanks full of water, disinfection tanks empty, and left wind load

SELF 1.0000 STATIC(DEAD) MANBOTTO 1.0000 STATIC(LIVE) EARTH 1.0000 STATIC(UVE) ELECFLOO 1.0000 STATIC(UVE) SURCHARG 1.0000 STATIC(UVE) VVEHICLE 1.0000 STATIC(LIVE) FILTER 1.0000 STATIC(LIVE) CHLOTANK 1.0000 STATIC(UVE) BRICKWAL 1.0000 STATIC(DEAD) MANFFLOO 1.0000 STATIC(DTHER) WATERFIL 1.0000 STATIC(UVE) COVERSOI 1.0000 STATIC(UVE)

REINENVE ENVE

MAX MIN OF ALL COMBOS TO CALCULATE FOR REINFORCEMENT

COMB1 1,0000 COMB0 COMB2 1,0000 COMB0 COMB3 1,0000 COMB0 COMB4 1,0000 COMB0

# 5-CALCULATION FOR BAR ARRANGEMENT:

Base on attached results of shell forces analised by SAP2000, choosing the most dangerous forces for calculation:

 $A_o = M/R_obh_o^2$  Where, M: Maximum bending moment(T.m)  $h_o$ : Effective depth of bearing area(cm)  $h_o=$  (Element thickness-Cover thickness) b: Width of calculated area(cm) Required area of reinforcement: Fa= M/yRaho Where:  $y = 0.5 + ((1-2Ao)^{1/2})/2$ 

### 5.1-SLABS AND WALLS:

Moments	Values	Ao	γ	Fa	Bar arra	ngement	Remarks
	(T.m)			(cm <sup>2</sup> )	¢(mm)	a(mm)	1
	17.700	0.0752	0.961	19.85	18	125	
	36.300	0.1542	0.916	42.71	28	125	
	12.000	0.0510	0.974	13.28	16	125	
sect. 1-1	11.400	0.0484	0.975	12.60	16	125	
(Bottom)	8.500	0.0361	0.982	9.33	18	250	
t=0.70	14.400	0.0612	0.968	16.02	16	125	
	12.400	0.0527	0.973	13.73	16	125	
	13.600	0.0578	0.970	15.10	16	125	1
	14,900	0.0633	0.967	16.60	18	125	
	6.900	0.0293	0.985	7.55	18	250	
	8.400	0.0357	0.982	9.22	18	250	
	-17.600	0.0633	0.967	18.05	20	150	125
	-12.700	0.0457	0.977	12.90	16	125	
1	-9.200	0.0331	0.983	9.28	14	125	

Moments	Values	Ao	γ	Fa	Bar arra	ngement	Remarks
	(T.m)			(cm <sup>2</sup> )	φ(mm)	a(mm)	1
	-8.500	0.0306	0.984	8.57	14	150	125not in Dwg
	-10.500	0.0378	0.981	10.62	14	125	
	-9.800	0.0353	0.982	9.90	14	125	1 N.
	-7.700	0.0277	0.986	7.75	14	150	125
	-1.830	0.1162	0.938	8.13	12	125	1
sect. 1-1	1.000	0.0635	0.967	4.31	12	250	
(First floor)	-1.310	0.0832	0.957	5.71	12	150	125
t=0.20	-2.560	0.1625	0.911	11.71	14	125	
	1.330	0.0844	0.956	5.80	14	250	1
	-2.100	0.1333	0.928	9.43	14	125	1
	2.630	0.1670	0,908	12.07	14	125	1
	-2.350	0.1492	0.919	10.66	14	125	1
	0.900	0.0571	0.971	3.86	12	250	
	1.640	0.1041	0.945	7.23	12	125	not in Dwg
	-4.300	0.2730	0.837	21.41	18	100	125not in Dwg
	-2.600	0.2750	0.837	11.91	14	125	LCONDER DWG
	-14.200	0.1097	0.942	21.92	20	125	-
	12.500	0.0966	0.949	19.14	18	125	
sect. 1-1	4.000	0.0309	0.984	5.91	14	250	
(Wall A)	-2.600	0.0201	0.990	3.82	16	250	1
t=0.50	-12.000	0.0927	0.951	18.34	18	125	not in Dwg
	14.000	0.1082	0.943	21.59	20	125	- Inor in Dwg
sect. 1-1	-6,000	0.0464	0.976	8.93	14	150	125
(Wall D)	3.400	0.0263	0.987	5.01	14	250	125
t=0.50	-1.750	0.0135	0.993	2.56	12	250	1
sect. 1-1	10.470	0.0809	0.958	15.89	18	125	-
(Wall E)	-12.000	0.0927	0.951	18.34	18	125	
t=0.50	2.800	0.0216	0.989	4.11	14	250	1
1-0.50	-5.200	0.0402	0.979	7.72	16	250	
No. of Concession, Name	15.700	0.0667	0.965	17.52	24	250	-
	15.400	0.0654	0.966	17.18	24	250	
	26.000	0.1104	0.941	29.76	24	125	
sect. 2-2	16.190	0.0688	0.964	18.09	24	250	
(Bottom)	25.200	0.1070	0.943	28.79	24	125	
t=0.70	28.800	0.1223	0.935	33.21	24	125	1
	14.600	0.0620	0.968	16.25	18	125	
	9.400	0.0399	0.980	10.34	18	250	
	23.700	0.1006	0.947				ant in Dur
				26.97	22	125	not in Dwg
	18.700	0.0794	0.959	21.02	20	125	
	8.520	0.0362	0.982	9.35	20	250	
	-9.400	0.0338	0.983	9.49	14	125	
	-14.400	0.0518	0.973	14.68	16	125	
	-13.500	0.0486	0.975	13.74	16	125	
	-9.100	0.0328	0.983	9.18	14	150	
	-9.000	0.0324	0.984	9.08	14	150	
	-14.000	0.0504	0.974	14.26	16	125	not in Dwg
	-10.300	0.0371	0.981	10.42	14	125	
	-8.800	0.0317	0.984	8.87	14	150	125
	-5.230	0.1195	0.936	13.97	16	125	
	3.100	0.0709	0.963	8.05	12	125	1

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Moments	Values	Ao	γ	Fa	Bar arrai	ngement	Remarks
	(T.m)			(cm <sup>2</sup> )	¢(mm)	a(mm)	
	-2.620	0.0599	0.969	6.76	12	150	
sect. 2-2	1.250	0.0285	0.986	3.17	14	250	
(firstfloor)	-6.400	0.1463	0.921	17.38	18	125	
	-4.920	0.1125	0.940	13.08	16	125	not in Dwg
	-2.940	0.0672	0.965	7.62	16	250	
0.3	1.900	0.0434	0.978	4.86	14	250	1
0.25	-3.400	0.1214	0.935	11.36	14	125	
Sand filtration	1.820	0.0650	0.966	5.89	14	250	
cover slab)	-3.300	0.1179	0.937	11.00	14	125	
	1.500	0.0536	0.972	4.82	14	250	
	1.820	0.0650	0.966	5.89	14	250	
	-5.200	0.1857	0.896	18.13	18	125	not in Dwg
	-3.800	0.1357	0.927	12.81	16	125	
	-3.100	0.1107	0.941	10.29	16	150	
sect.2-2	-15.900	0.1228	0.934	24.74	20	125	
wall A	4.100	0.0317	0.984	6.06	14	250	
0.5	-9.100	0.0703	0.964	13.73	20	200	
	-4.000	0.0309	0.984	5.91	14	250	
sect.2-2	17.500	0.1352	0.927	27.44	22	125	
wall D	-12.000	0.0927	0.951	18.34	18	125	
0.5	5.300	0.0409	0.979	7.87	16	250	
	-2.100	0.0162	0.992	3.08	12	250	
sect.2-2	-14.200	0.1097	0.942	21.92	20	125	
wall E	9.000	0.0695	0.964	13.57	18	150	
0.5	-5.400	0.0417	0.979	8.02	16	250	
	4.520	0.0349	0.982	6.69	16	250	-

### 5.1-BEAMS:

SECTION	LOCATION	Values	Ao	Y	Fa	Bar arra	angement	Remarks
m2		(T.m)			(cm <sup>2</sup> )	φ(mm)	quantity	
b=0.70	B1	-15.520	0.0209	0.989	7.97	25	2	
h=1.30	B2	37.860	0.0511	0,974	19.76	25	5	
	B3	-62.820	0.0847	0.956	33.40	24	8	
	B10	45.430	0.0613	0.968	23.84	25	5	1
b=0.50	B4	-5.760	0.0586	0.970	7.00	22	2	
h=0.60	B5	5.200	0.0529	0.973	6.30	18	3	
	B6	-6.630	0.0674	0,965	8.10	22	3	
	B7	-5.870	0.0597	0.969	7.14	18	3	
	B8	2.940	0.0299	0.985	3.52	18	2	1
	B9	-6.830	0.0695	0.964	8.36	22	3	
b=0.60	B11	-11.700	0.0218	0.989	6.54	22	2	
h=1.20	B12	2.840	0.0053	0.997	1.57	20	2	
	B13	-41.480	0.0773	0.960	23.91	24	6	
	B14	-39.620	0.0739	0.962	22.79	24	6	
	B15	22.420	0.0418	0.979	12.67	20	5	
	B16	-23.750	0.0443	0.977	13.44	24	3	
1	B17	-4.850	0.0090	0.995	2.69	24	2	1

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SECTION	LOCATION	Values	Ao	у	Fa	Bar arr	angement	Remarks
m2	1 (C)	(T.m)			(cm <sup>2</sup> )	ộ(mm)	quantity	
	B18	7.190	0.0134	0.993	4.00	22	2	
	B19	-37.580	0.0701	0.964	21.57	24	5	1
	B20	-37.640	0.0702	0.964	21.61	24	5	
	B21	30.930	0.0577	0.970	17.63	22	5	1
	B22	-31.750	0.0592	0.969	18.11	24	5	
	B17'	-11.470	0.0214	0.989	6.41	24	2	
	B18'	2.050	0.0038	0.998	1.14	22	2	1
	B19'	-45.150	0.0842	0.956	26.12	24	6	1
	B20'	-46.420	0.0866	0.955	26.89	24	6	1
	B21'	40.970	0.0764	0.960	23.60	22	7	
	B22'	-39.660	0.0740	0.962	22.81	24	6	
b=0.60	B23	-9.490	0.0424	0.978	8.30	24	2	
h=0.80	B24	19,880	0.0888	0.953	17.85	24	4	1
	825	-29.990	0.1340	0.928	27.67	24	7	
	B23'	-29.690	0.1327	0.929	27.37	24	7	1
	B24'	16.210	0.0724	0.962	14.42	24	4	1
	B25'	-18.740	0.0837	0.956	16.78	24	4	1
b=0.60	B26	-11.700	0.0992	0.948	14.56	24	.4	
h=0.60	B27	5.810	0.0492	0,975	7.03	20	3	
	B28	-11.020	0.0934	0.951	13.67	24	4	1
b=0.60	B29	-7.670	0.0211	0,989	5.21	24	2	
h=1.00	B30	0.000	0.0000	1.000	0.00	22	2	1
	B31	-30.370	0.0836	0,956	21.34	24	5	
b=0.60	B32	-31.240	0.0860	0.955	21.98	24	5	
h=1.00	B33	25.090	0.0691	0.964	17.49	22	5	
	B34	-27.080	0.0745	0.961	18.93	24	5	
b=0.50	B35	-18.600	0.0771	0.960	14.59	20	5	
h=0.90	B36	31.110	0.1290	0.931	25.17	24	6	
	B37	-15.900	0.0659	0.966	12.40	20	4	· · · · ·

# 5.1.1-CHECKING SHEAR FORCES:

-Height of hand for supporting beam s, so the section need to be checked - In case Q >= Rsxbxd so the below case is to be considered shear bearing capacity is [c/2+(h+s)/2]

- In case concrete is not enough to bear shearing force, stirrups will be considered

Sc/2 + Ss >= Q (shearing force at section calculated) Sc: shearing bearing capacity of concrete (kg) Where

Ss: shearing bearing capacity of reinforcement (kg)

Ss=AsxRaxjxd/a = Q - Sc/2

As: area of all stirrup in section considered

d: effective height of beam

a: pitch of stirrup (distance between two stirrups)

=0.87 ]: coefficient that consider safety factor (=1/1.15)

Frame	height of	Width	height of	height of	c/2+	Values	Capacity of	Shearing	Degn Shearin	Compare	Number	Dia. Of	pitch
	beam	of beam	hand	column	+(h+s)/2	(T.m)	concrete	stresses	stress (Sc)	&Conclud	of stirrup	stirrup	of stimup
	h (m)	(m) d	s (m)	c (m)		(Ø)	(tan)	(Kg/cm2)	(Kg/cm2)		branches	(ww)	(cm)
Frame C	1.3	0.7	0	0.5	0.00	36.63	31.00	4.25	3.6	NOT OK!	2	12	18.3
Frame 2	0.6	0.5	0	0.7	0.65	8.32	9.54	3,14	3.6	OKIII			
Frame 4	1.2	9'0	0	0.6	0.90	22.45	24.41	3.31	3.6	OKIII			
Frame 5	1.2	0.6	0	0.6	0.90	29.83	24.41	4.40	3.6	NOT OK!	2	12	20.2
Frame 7	1.0	0.6	0	0.6	0.80	19.56	20.09	3.51	3.6	OKIII			
Frame B'	1.0	0.6	0	0.6	0.80	19.72	20,09	3,53	3.6	OKIII			

Water supply facility - 3/25/01

6-COLUMNS:

	70 (Kg'cm2)	230000 (Kg/cm2)	2000000 (Kg/cm2)	1600 (Kg/cm2)
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	â	Ê	Ea	Ra = Ra'
	E21		Ē	
	Grade		Grada	
A. MATERIAL PROPERTIES:	_ Concrete		_ Reinforcement	

B. CALCULATION:

160/0 0.1106 0.097

6.8

5.8 4

41270 \$0000100 2.42E404 2.30E404 1 3364 10000010 2.25E403 1.10E405 3364 10000010 4.02E403 1.17E405 4939 142916657 4.59E403 1.17E405

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Water supply facility - 3/25/01

Same arrangement	Fa=Fa'	4 f25	3 125	-4 f26	-3 f27	-3 f28	-3 f29
CHECK		OK!	OK!	RECH	RECH	RECH	RECH
	(%)	0.00	0000	0.03	0.00	0.00	0.00
a chosen	(cm2)	-					-
11 <sup>11</sup>	(%)	0.63	0.40	-1.15	-1.00	-0.94	-0.85
=Fa,	cm2)	17.4	1.11	-18.4	-15.8	-14.8	-13.4
Far	(CI	TET	LTL.	171.	1.TL	LTI.	LTI.
×	(cm)	213	57.7	53.1	50.8	59.6	58.5
αn.h.		21.7	57.7	32.9	32.9	39.1	39.1
×	(cm)	4.9	5.5	26.3	27.8	33.1	34.9
Θ	(cm)	117.3	66.5	27.6	29.1	35.0	35.7
600		26.94	26.94	16,85	16.85	19.38	19.38
D.e.u	(cm)	160.27	109.54	4.62	6.12	1.00	127
E		1,003	1,004	1,040	1.048	1.035	1.038
Hi N	(Kg)	6,48E+06	5.62E+06	2.84E+06	2.576+05	3.44E+06	3.328+05
ź		2.00	2.00	2.00	2.00	2.00	2.00
-File	(Kg)	2.056+04	2.30E+04	1.108405	1.17E+05	1.16E+05	1.22E+05
Mah	(Kg.m)	3.196+04	2.42E+04	2.25E+03	4.02E+03	4.59E+03	5.65E+03
'n		0.10	0.19	0.73	0.66	0.66	0.63
PUSITION		Upper	Lower	Upper	Lower	Upper	Lower
		Å0		B'4		C2	

# 7- CALCULATION FOR PILE NUMBER

(Pile number to be decided by pile capacity as calculated, please refer to pile calculation part in the "Design report" for more information)

# SAP2000 v6.11 File: CHLO-PILE-CAL Ton-m Units March 25, 2001 13:33

# JOINT REACTIONS

JOINT	LOAD	FL	F2	F3	ML	M2	М3
z	Minima	0.0000	0.000	1138,4926	0.0000	0.0000	0,0000
		PILEENVE	PILEENVE	PILEENVE	PILEENVE	PILEENVE	FILEENVE
2	Maxima	0.0000	0.0000	1148.0933	0.0000	0.0000	0.0000
		PILEENVE	PILEENVE	PILEENVE	PILEENVE	PILEENVE	PILEENVE
13	Minima	0.0000	238.0364	797,2947	0.0000	0.0000	0.0000
		PILEENVE	PILEENVE	PILEENVE	PILEENVE	PILEENVE	PILEENVE
13	Maxina	0.0000	241.0088	797.5787	0.0000	0.0000	0.0000
		PILEENVE	PILEENVE	FILEENVE	PILEENVE	PILEENVE	PILEENVE
44	Minima	-137.6070	0,0000	117.1416	0.0000	0,0000	0.0000
		PILEENVE	PILEENVE	PILEENVE	PILEENVE	PILEENVE	PILEENVE
4.4	Maxima	-45.2618	0.0000	134.6723	0.0000	0.0000	0.0000
		PILEENVE	PILEENVE	PILEENVE	PILEENVE	PILEENVE	PILEENVE
219	Minima	0.0000	0.0000	1866.9232	0.0000	0.0000	0.0000
		PILEENVE	FILEENVE	PILEENVE	PILEENVE	PILEENVE	PILEENVE
219	Maxima	0.0000	0.0000	1874.2565	0.0000	0.0000	0.0000
		PILEBNVE	PILEENVE	PILEENVE	PILEENVE	FILEENVE	PILEENVE
303	Minima	0.0000	0.0000	473.2465	0.0000	0.0000	0.0000
		PILEENVE	PILEENVE	PILEENVE	PILEENVE	PILSENVE	FILEENVE
303	Maxima	0.0000	0.0000	475.9105	D.0000	0.0000	0.0000
		PILEENVE	PILEENVE	PILEENVE	PILEENVE	PILEENVE	PILEENVE
450	Minima	0.0000	0.0000	233.7308	0.0000	0.0000	0.0000
		PILEENVE	PILEENVE	PILEENVE	PILEENVE	PILEENVE	PILEENVE
450	Maxina	0.0000	0.0000	233.8615	0.0000	0.0000	0.0000
		PILEENVE	PILEENVE	FILEENVE	PILEENVE	PILEENVE	PILEENVE

 $\sum_{i=2-40} R_i = 4664.4T \text{ So reaction pressure is: } 4664.4/(25.5x23) = 7.953 \text{ (T/m2)}$ 

Pile capacity= 45 ton for one pile so one pile can bear for an area less tha 45/7.953= 5.66m2 So the R.C piles to be arranged as on DWG No. PE-WWTP-254-01

12

# 8- CHECKING UPLIFT IN CASE GROUND WATER LEVEL UPTO +2.2

SAP2000 v6.11 File: CHLO-PILE-CAL Ton-m Units PAGE 1 March 25, 2001 14:16

### .....

# LOAD COMBINATION MULTIPLIERS

COMBO TYPE CASE FACTOR TYPE TITLE

UPLIFT ADD Checking uplift, when ground water level upto +2.2 SELF 1.0000 STATIC(DEAD) ELECFLOO 1.0000 STATIC(LIVE) ACHITEC2 1.0000 STATIC(OTHER) FILTER 1.0000 STATIC(LIVE) CHLOTANK 1.0000 STATIC(LIVE) BRICKWAL 1.0000 STATIC(DEAD) COVERSOI 1.0000 STATIC(OTHER) MAXUPLIF 1.0000 STATIC(OTHER)

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## JOINT REACTIONS

JOINT	LOAD	P1	F2	F3	Ml	м2	М3
2	Minima	0.0000	0.0000	248.5001	0.0000	0.0000	0.0000
		UPLIFT	UPLIFT	OPLIFT	UPLIFT	UPLIFT	UPLIFT
2	Maxima	0.0000	0.0000	248,5001	0.0000	0.0000	0.0000
		UPLIET	UPLIFT	UPLIFT	UPLIFT	<b>UPLIFT</b>	UPLIFT
4	Minima	0.0000	334.3679	38.4905	0.0000	0.0000	0.0000
		UPLIFT	UPLIFT	UPLIFT	OPLIFT	UPLIFT	UPLIFT
4	Maxima	0.0000	334.3679	38.4905	0.0000	0.0000	0.0000
		UPLIFT	UPLIFT	UPLIFT	OPLIFT	UPLIFT	UPLIF7
13	Minima	0.0000	290.5133	-5.0094	0.0000	0.0000	0.0000
		UPLIFT	UPLIFT	UPLIFT	OPLIFT	UPLIFT	OPLIFT
13	Maxima	0.0000	290.5133	-5.0094	0.0000	0.0000	0.0000
		UPLIFT	UPLIFT	OPLIFT	UPLIFT	UPLIFT	UPLIFT
14	Minima	-21,9829	0.0000	-26.8673	0.0000	0.0000	0.0000
		OPLIFT	UPLIFT	UPLIFT	OPLIFT	UPLIFT	UPLIFT
14	Maxima	-21.9829	0.0000	-26.8673	0.0000	0.0000	0.0000
		UPLIFT	UPLIFT	OPLIFT	OPLIFT	UPLIET	UPLIFT
4.4	Minima	92.4739	0.0000	110.2682	0.0000	0.0000	0.0000

### Water supply facility - 3/25/04

		UPLIFT	UPLIFT	UPLIFT	UPLIFT	UPLIFT	UPLIFT
44	Махіта	92.4739	0.0000	110.2682	0.0000	0.0000	0.0000
		UPLIFT	UPLIFT	UPLIFT	UFLIFT	UPLIFT	UPLIET
219	Minina	0.0000	0.0000	-132.7863	D.0000	0.0000	0.0000
		OPLIFT	UPLIFT	UPLIFT	UFLIFT	UPLIFT	UPLIFT
219	Maxima	0.0000	0.0000	-132.7863	0.0000	0.0000	0.0000
		UPLIFT	UPLIFT	UPLIFT	UPLIFT	VPLIFT	UPLIFT
220	Minima	0.0000	0.0000	-280.6315	0.0000	0.0000	0.0000
		UPLIFT	UPLIET	UPLIFT	UPLIFT	UPLIFT	UPLIFT
220	Maxima	0.0000	0.0000	-280.6315	0.0000	0.0000	0.0000
		UPLIFT	UPLIFT	UPLIFT	UPLIFT	UFLIFT	OPLIFT
303	Minipa	0.0000	0.0000	-22.0051	0.0000	0.0000	0.0000
		UPLIFT	UFLIFT	UPLIFT	UFLIFT	UPLIFT	UPLIFT
303	Maxima	0.0000	0.0000	-22.8851	0.0000	0.0000	0.0000
		UPLIFT	UPLIFT	UPLIFT	UPLIFT	UPLIFT	UPLIFT
304	Minima	0.0000	0.0000	-224.3280	0.0000	0.0000	D.00DD
		UPLIFT	UPLIFT	UPLIFT	UPLIFT	UPLIFT	UPLIFT
304	Naxima	0.0000	0.0000	-224.3280	0.0000	0.0000	0.0000
		UPLIFT	UPLIFT	UPLIFT	UPLIFT	UPLIFT	OPLIFT
429	Minima	0.0000	D.DDDD	59.6100	0.0000	0.0000	0.0000
		UPLIFT	UPLIFT	UPLIFT	UPLIFT	UPLIFT	UPLIFT
429	Maxima	0.0000	0.0000	58.6100	0.0000	0.0000	0.0000
		UPLIFT	UPLIFT	UPLIFT	UPLIFT	UPLIFT	UPLIFT
450	Minima	0.0000	0,0000	-11.97HD	0.0000	0.0000	0.0000
		OPLIFT	UPLIFT	UPLIFT	UPLIFT	UPLIFT	UPLIFT
450	Maxima	0.0000	0.0000	-11,9780	D.0000	0.0000	0.0000
		UPLIET	OPLIFT	UPLIFT	UPLIFT	UPLIFT	UPLIFT
451	Hinima	0.0000	0.0000	-44.6212	0.0000	0.0000	0.0000
		UPLIFT	UPLIFT	UPLIFT	UFLIFT	UPLIFT	UPLIFT
451	Махіпа	0.0000	0.0000	-44.6212	0.0000	0.000	0.0000
		UPLIFT	UFLIFT	OPLIFT	UPLIFT	UPLIFT	UPLIFT
2822	Minima	0.0000	0.0000	-342.6094	0.0000	0.0000	0.0000
		UPLIFT	UPLIFT	UPLIFT	UPLIFT	UPLIFT	UPLIFT
2622	Maxima	0.0000	0.0000	-342.6094	0.0000	D.000D	0.0000
		UPLIET	UPLIFT	UPLIFT	UPLIFT	UPLIFT	UPLIF7

Total uplift upto the bottom slab =

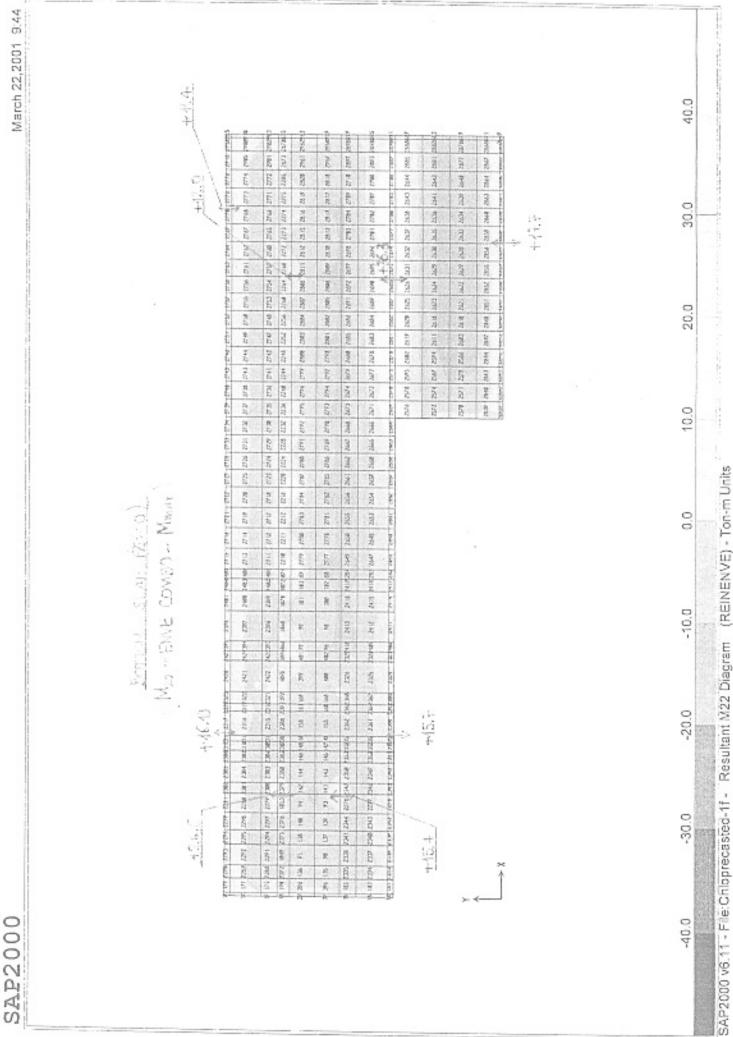
-635.85 Ton

Tensile capacity of one pile as calculated is 20 ton (In case the pile be pulled up)

So the number of piles needed for preventing structure from uplifting is : 31.8 piles, but as numbe piles calculated in part 7 above is 352 piles for water supply facility foundation, so uplift will never occur

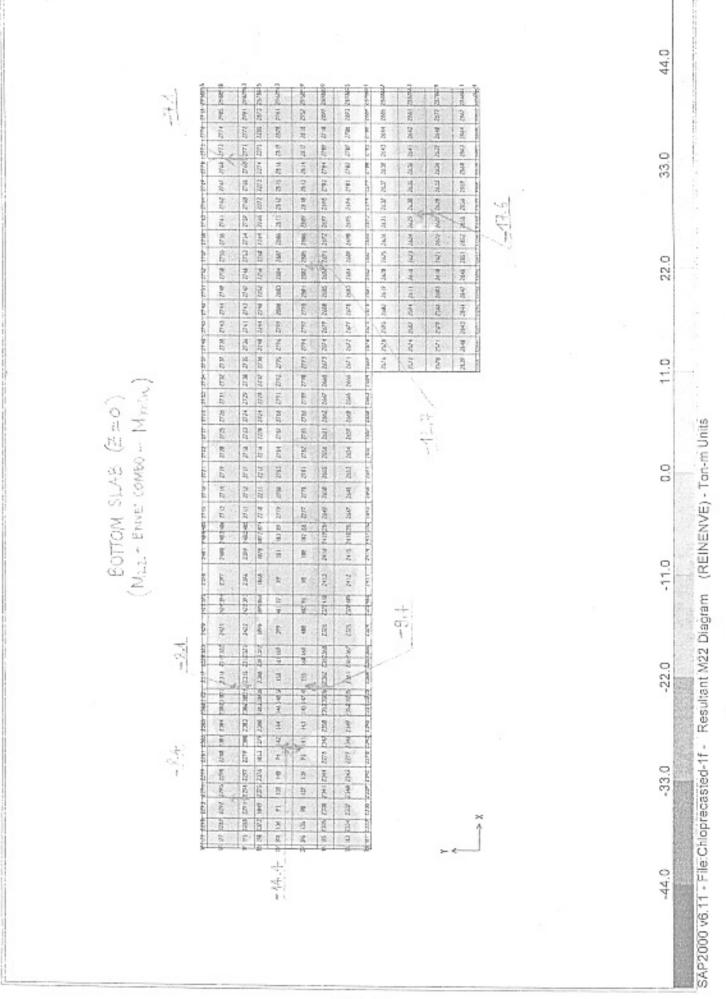
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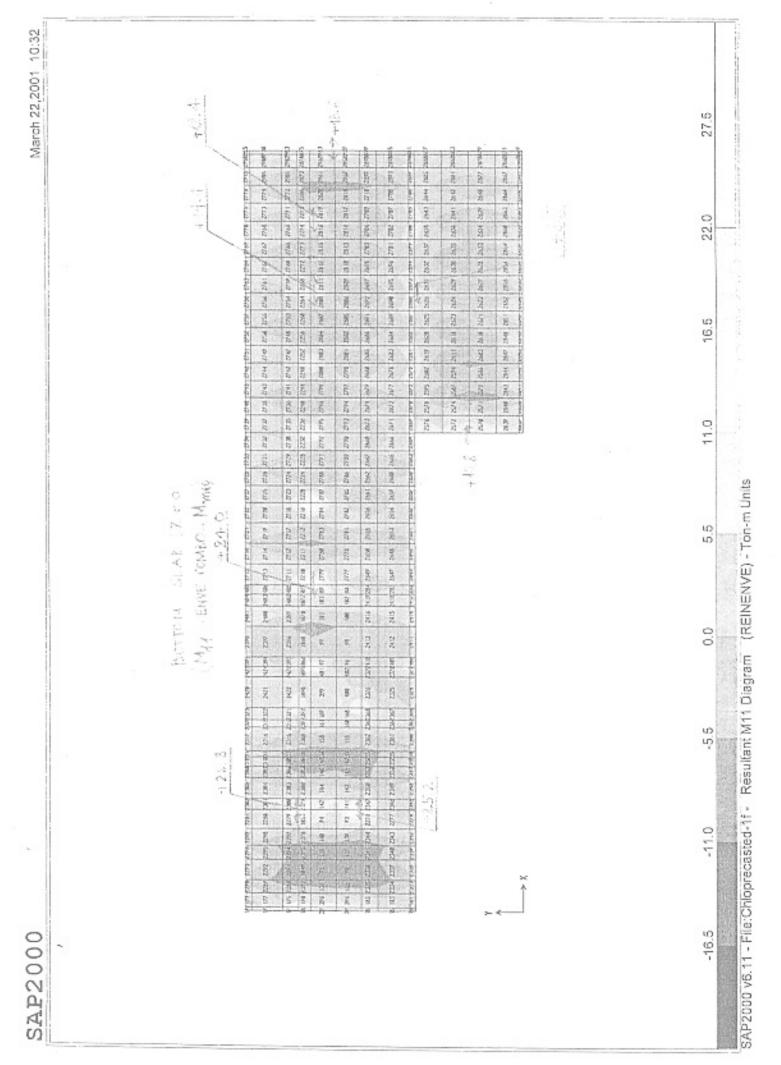
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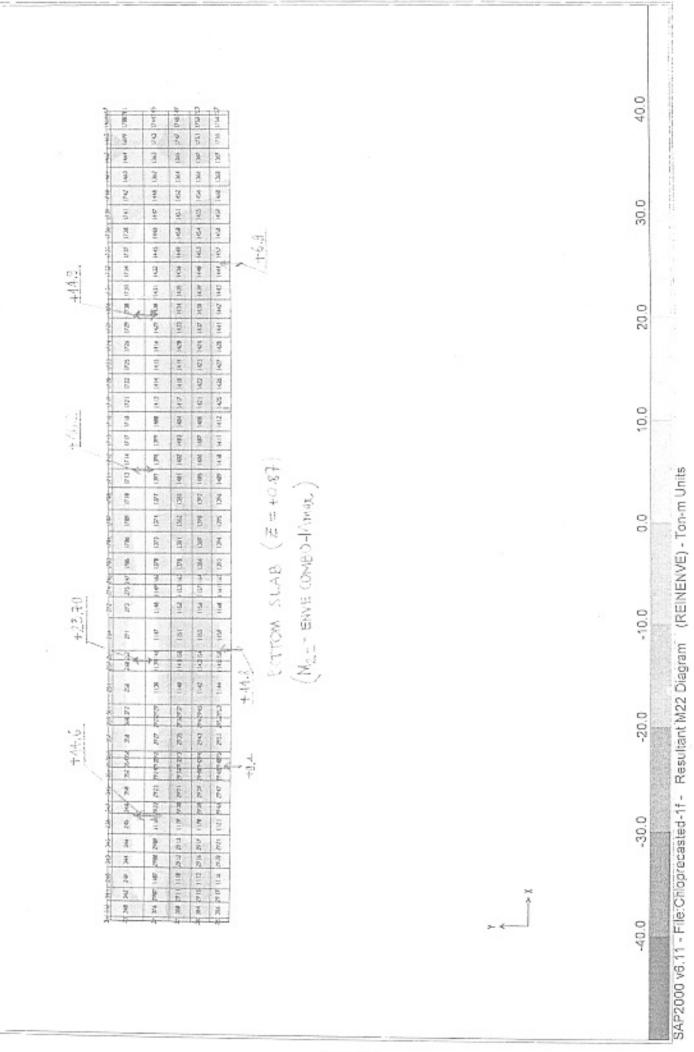
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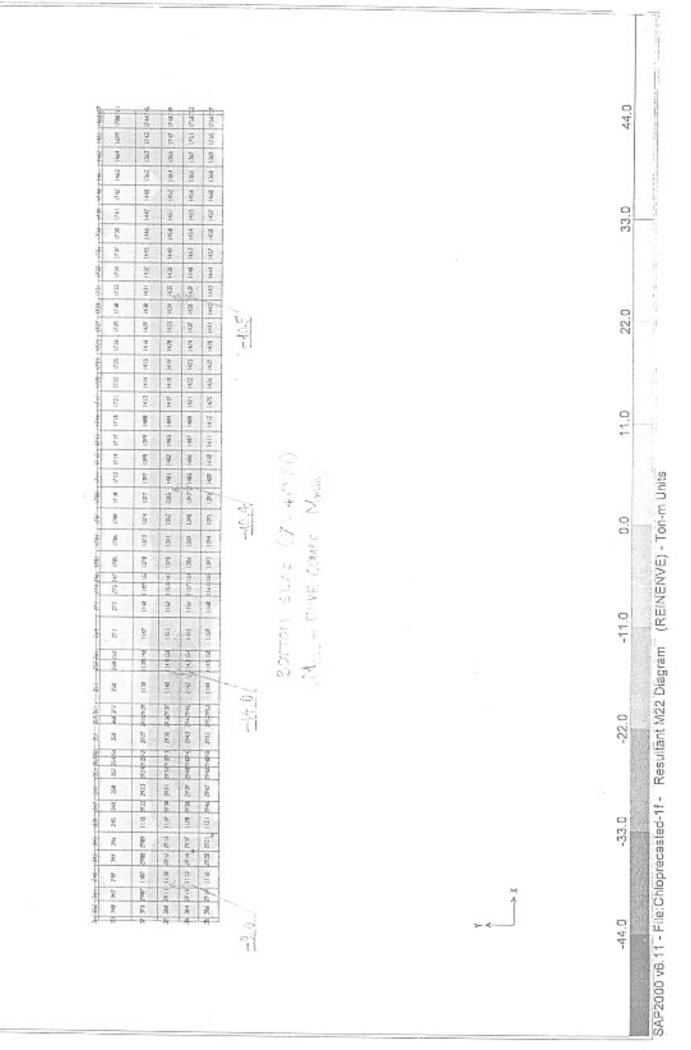
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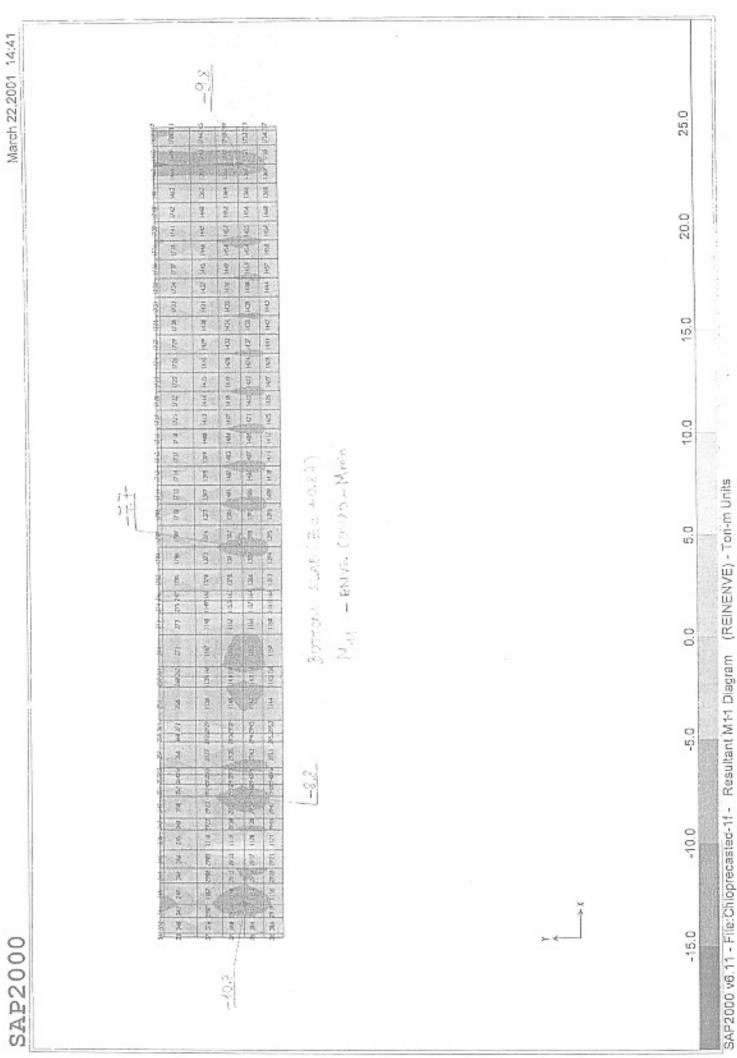
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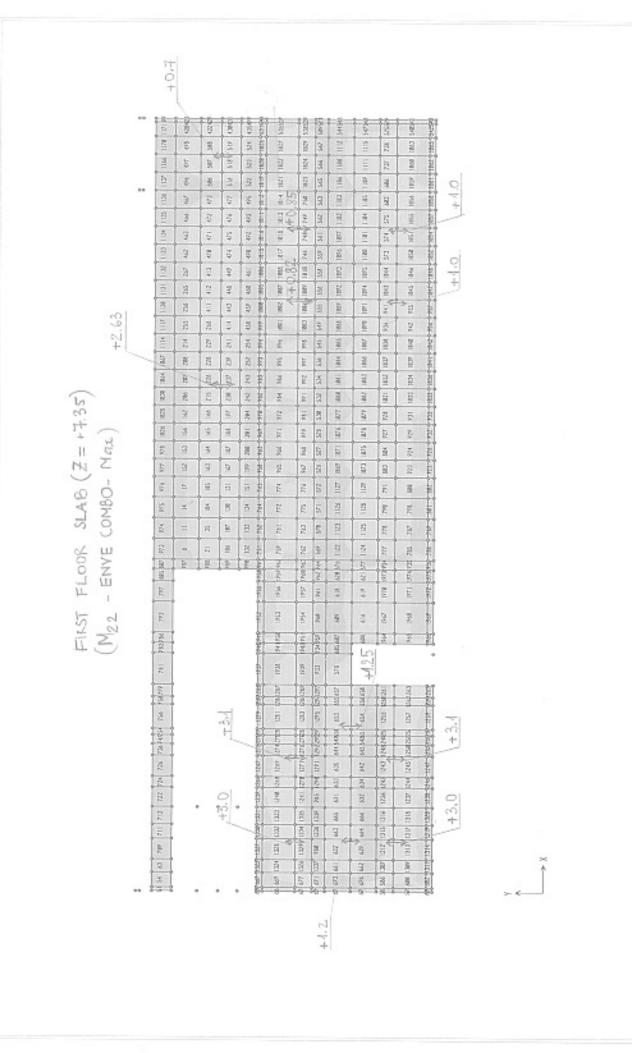
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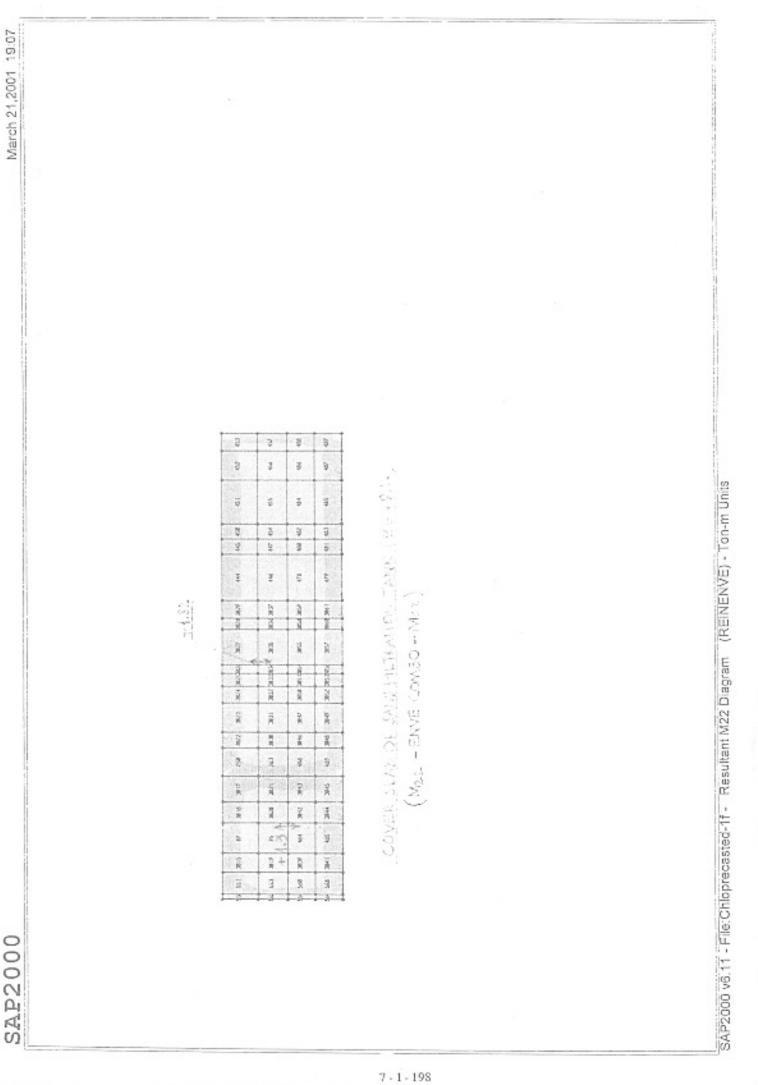
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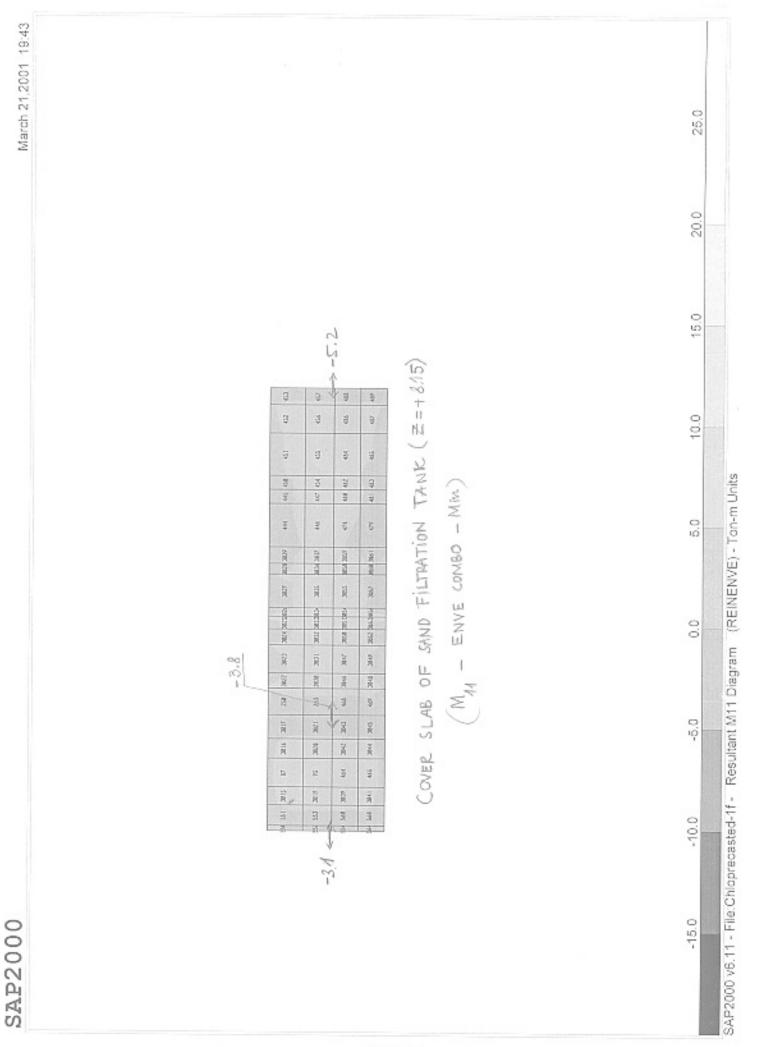
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COVER SLAB OF SAND FILTRATION TANK (2=+8.15) (M44 - ENVE COMBO - Max)

SAP2000 v6.11 - File:Chloprecasted-1f - Resultant M11 Diagram (REINENVE) - Ton-m Units



SAP2000

March 21,2001 19:49

VALL AXIS (E)  $(\gamma = 28.55 \text{ m})$  $(M_{22} = ENVE COMBO = MOC)$ 

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March 21,2001 19:54

VALL AXIS ( Y=+28.55 m) (M22 - ENVE COMBO - Min)

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March 22,2001 8:09

WALL AXIS () ( $\gamma_{\pm\pm}22.05$ ) ( $M_{22}$ - ENVE COMBO- Max)

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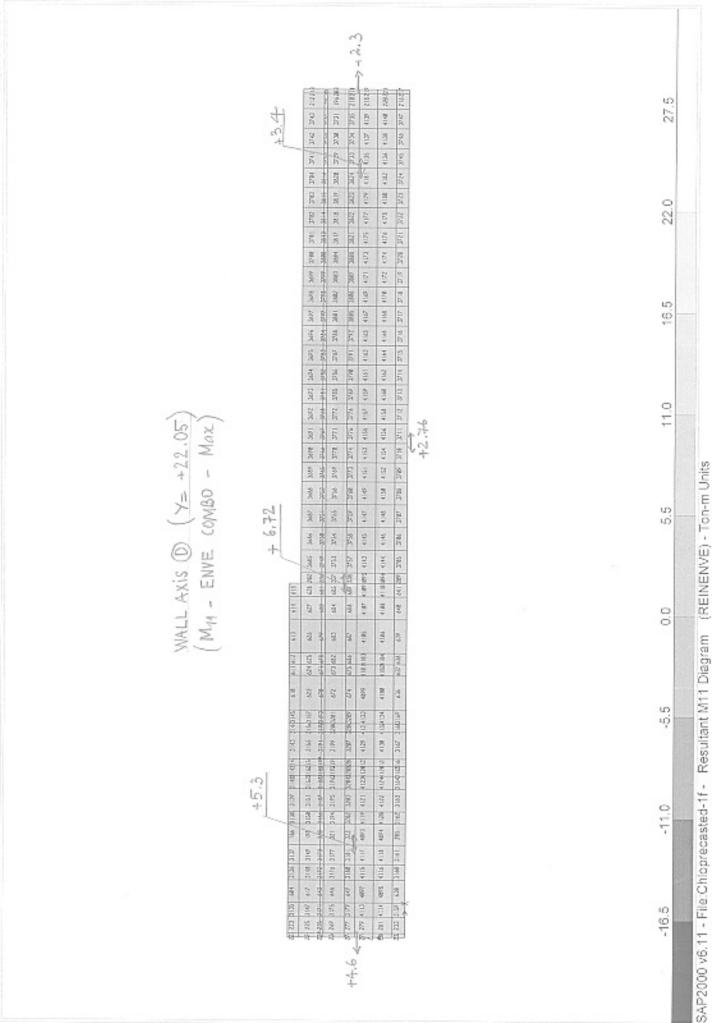
WALL AXIS ( Y = +22.05) (M22 - ENVE COMBO - Min )

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WALL AXIS (D) (7= +22.05)

WALL AXIS ( 7= +22.05) ( My - ENVE COMBO - Min. )

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WALL AXIS  $\bigoplus (\gamma = + 8.55)$ ( $M_{2.8}$  - ENVE COMBO -  $M_{inv}$ )

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March 22,2001 9:04

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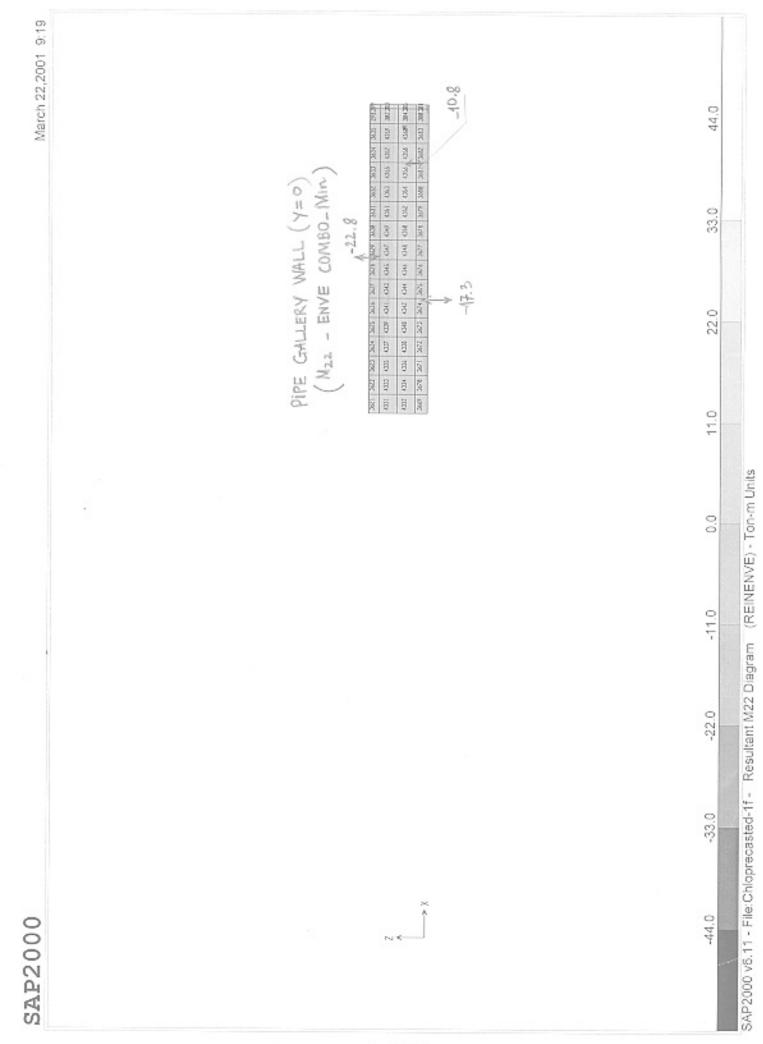
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WALL AXIS (Y= +8.55) (My - ENVE COMBO- MAK)

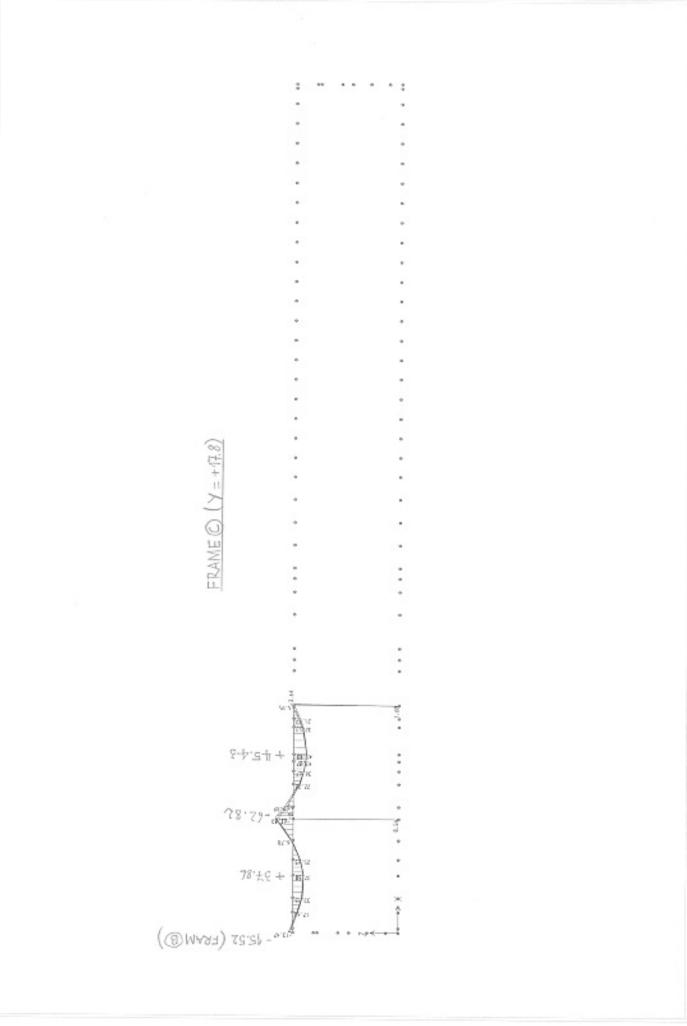
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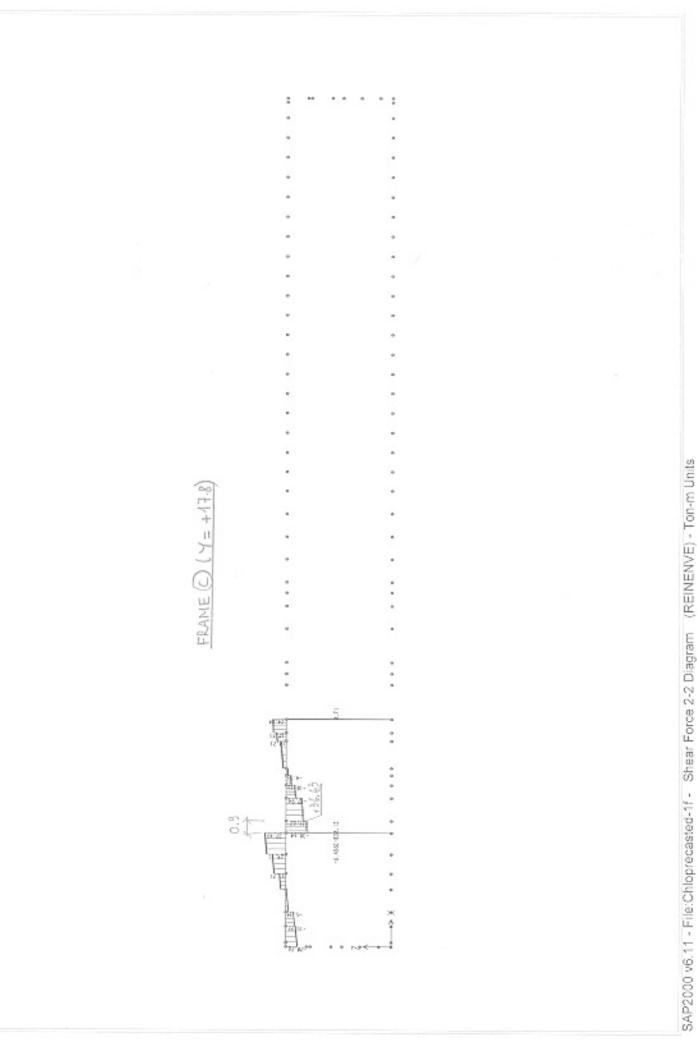


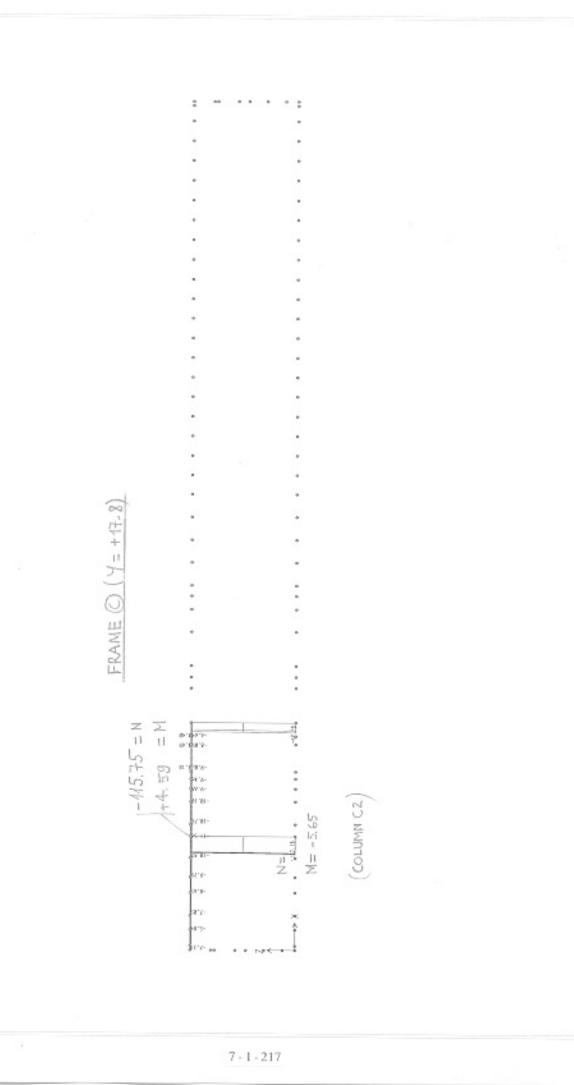
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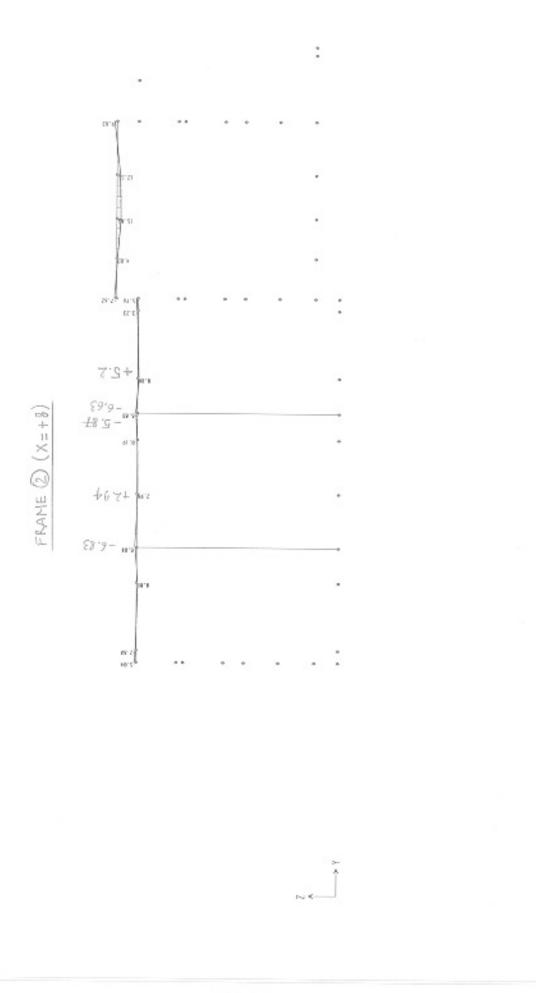


SAP2000 v6.11 - File:Chloprecasted-1f - Moment 3-3 Diagram (REINENVE) - Ton-m Units



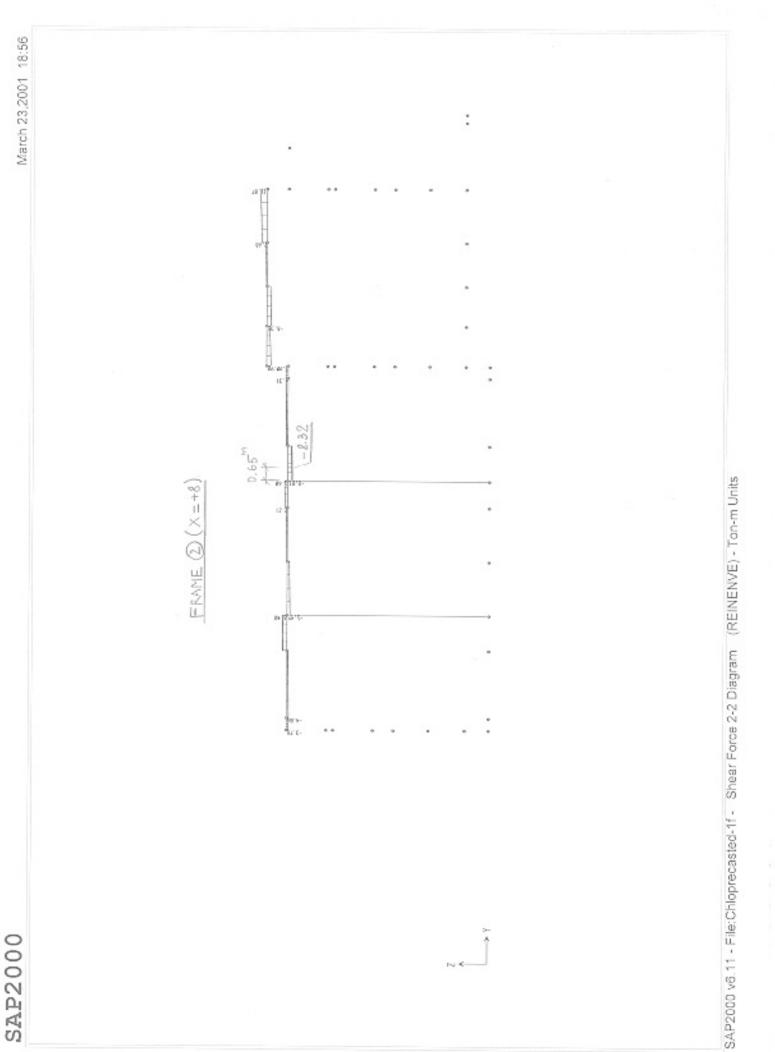


SAP2000 v6.11 - File:Chloprecasted-1f - Axial Force Diagram (REINENVE) - Ton-m Units

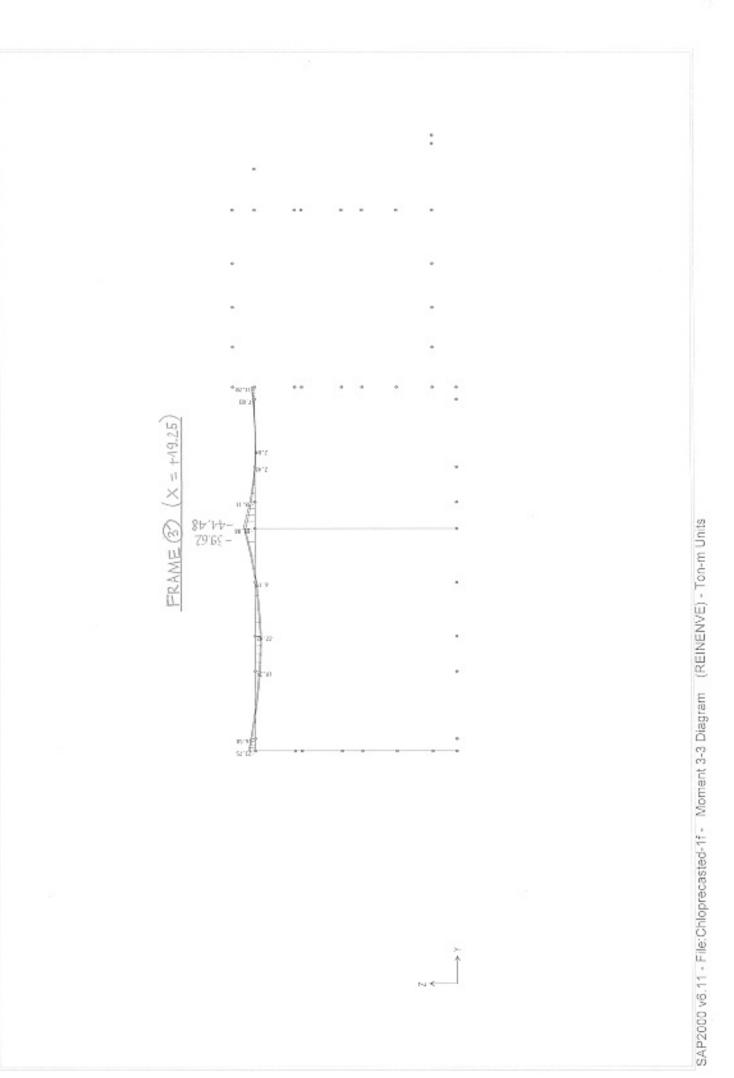


SAP2000 v6.11 - File:Chloprecasted-1f - Moment 3-3 Diagram (REINENVE) - Ton-m Units

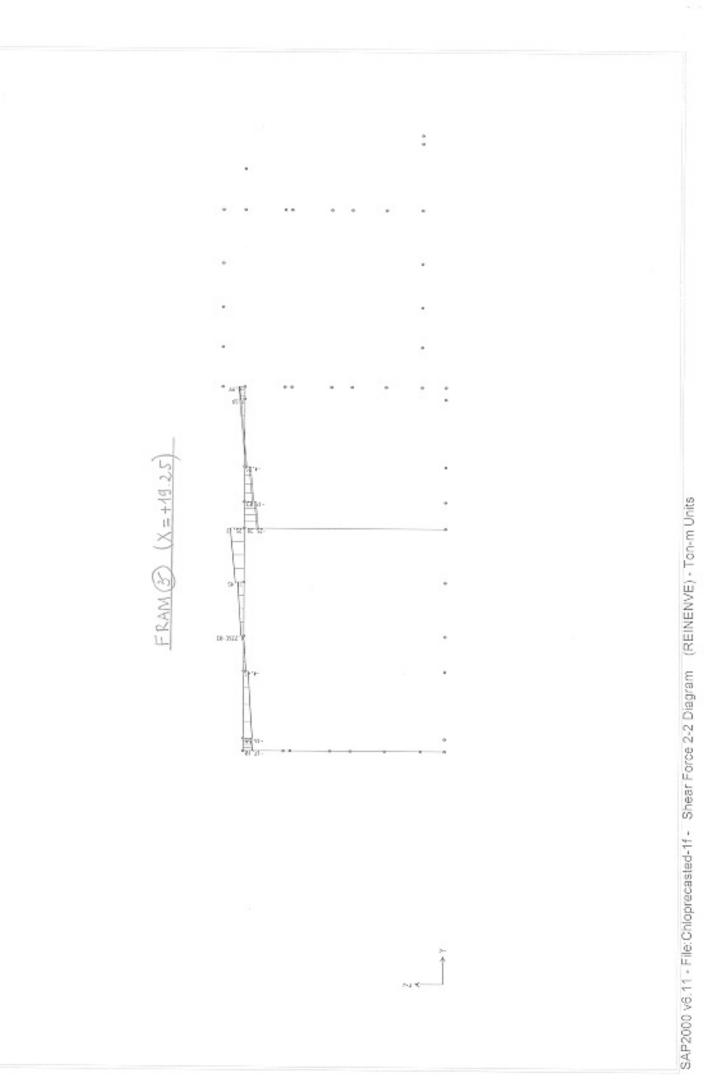
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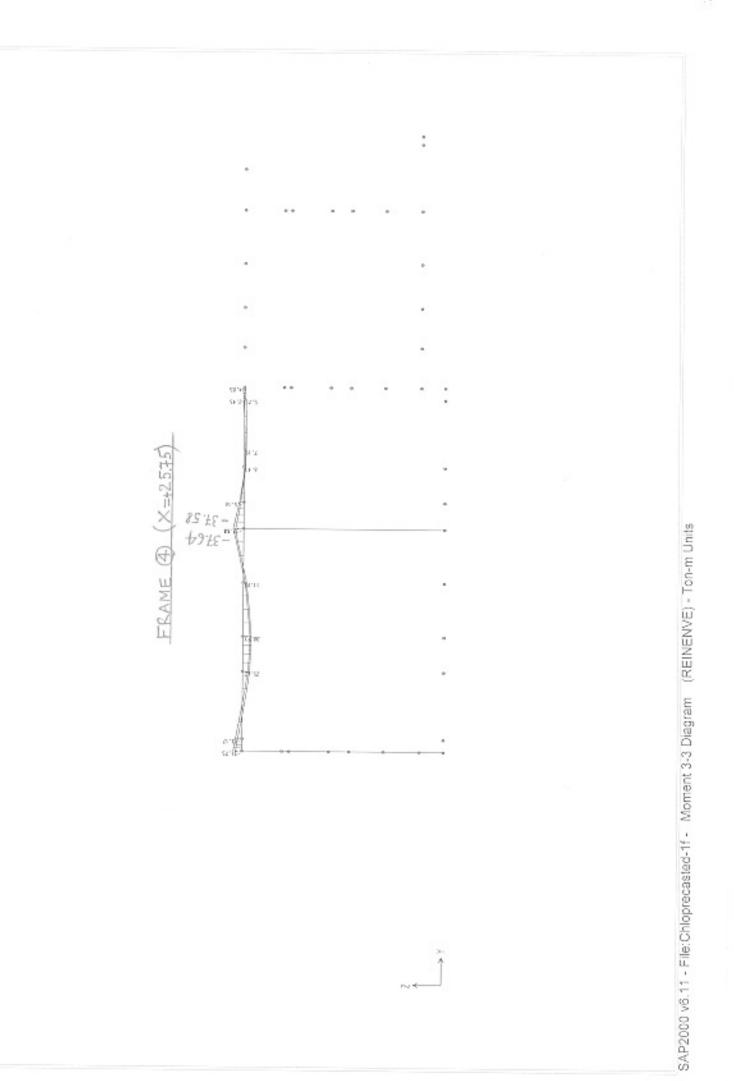


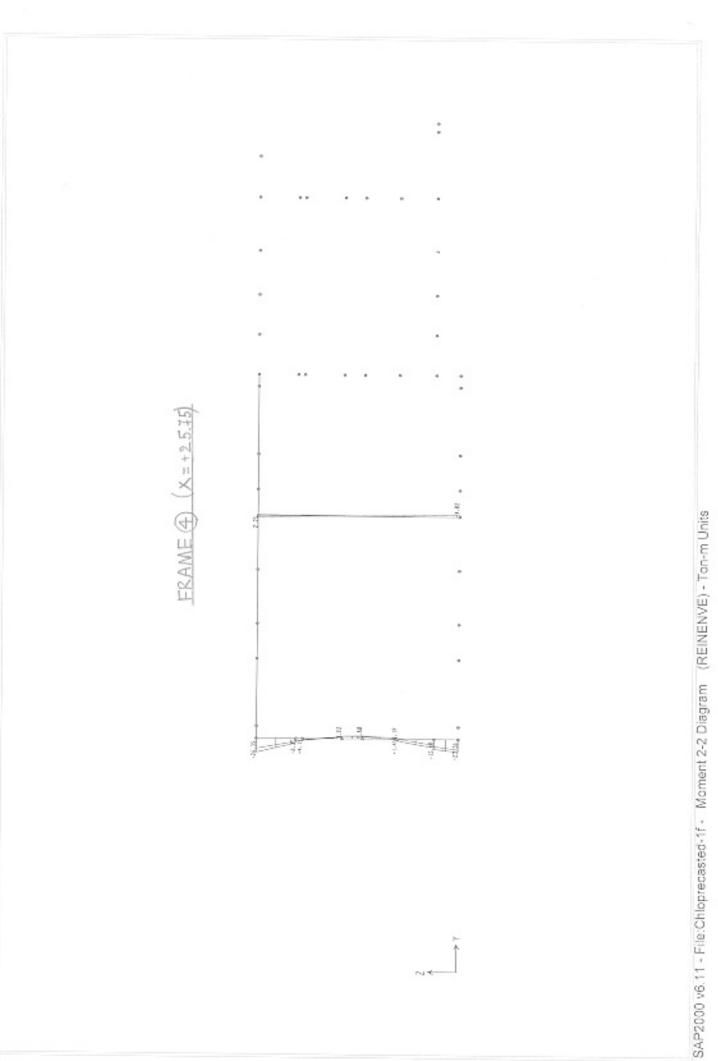
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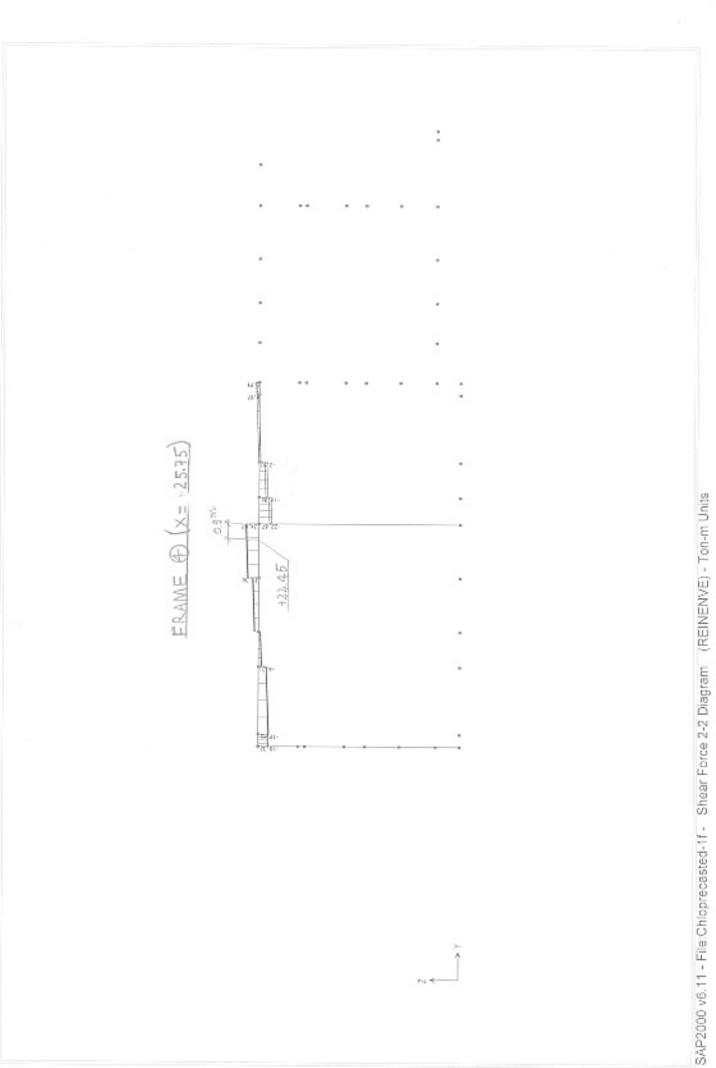


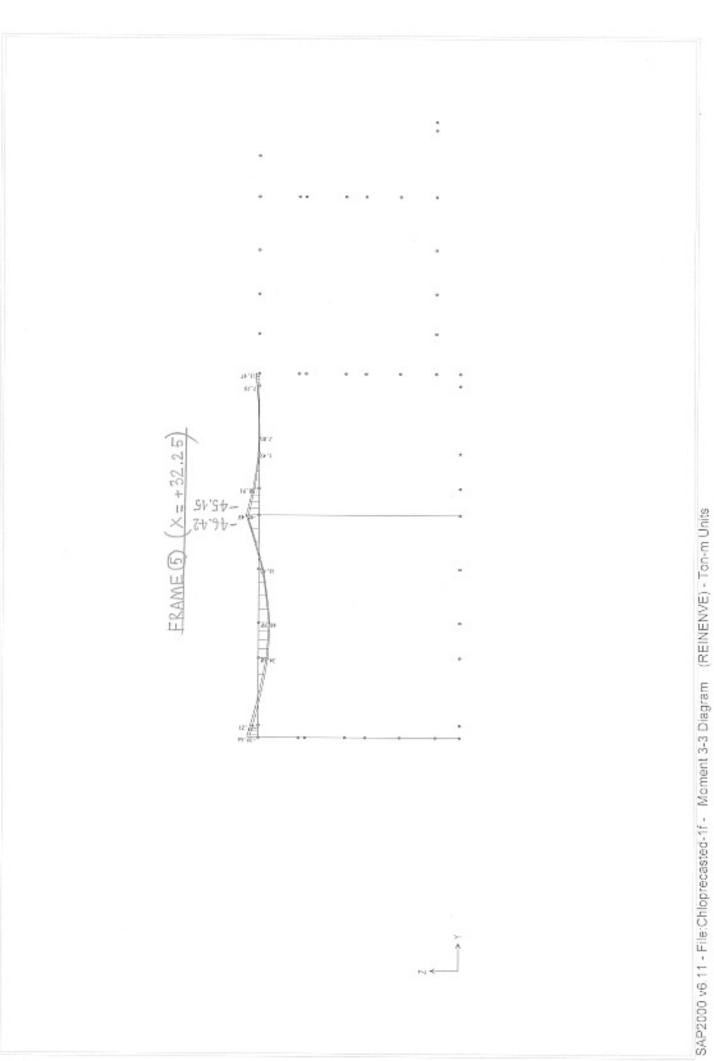
7 - 1 - 220

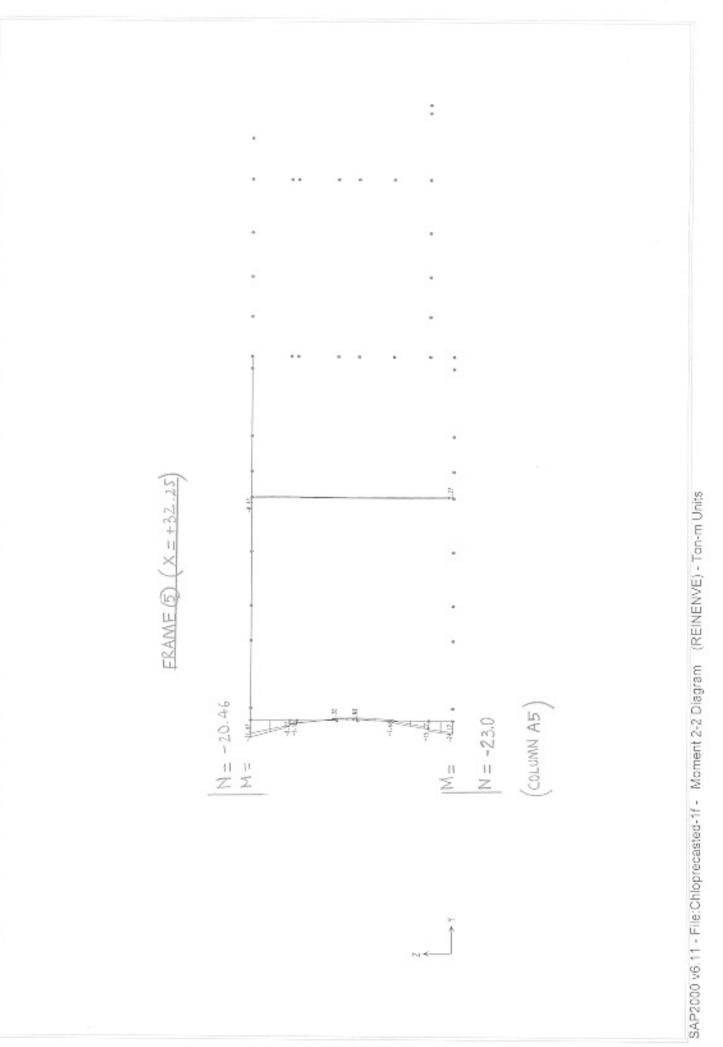


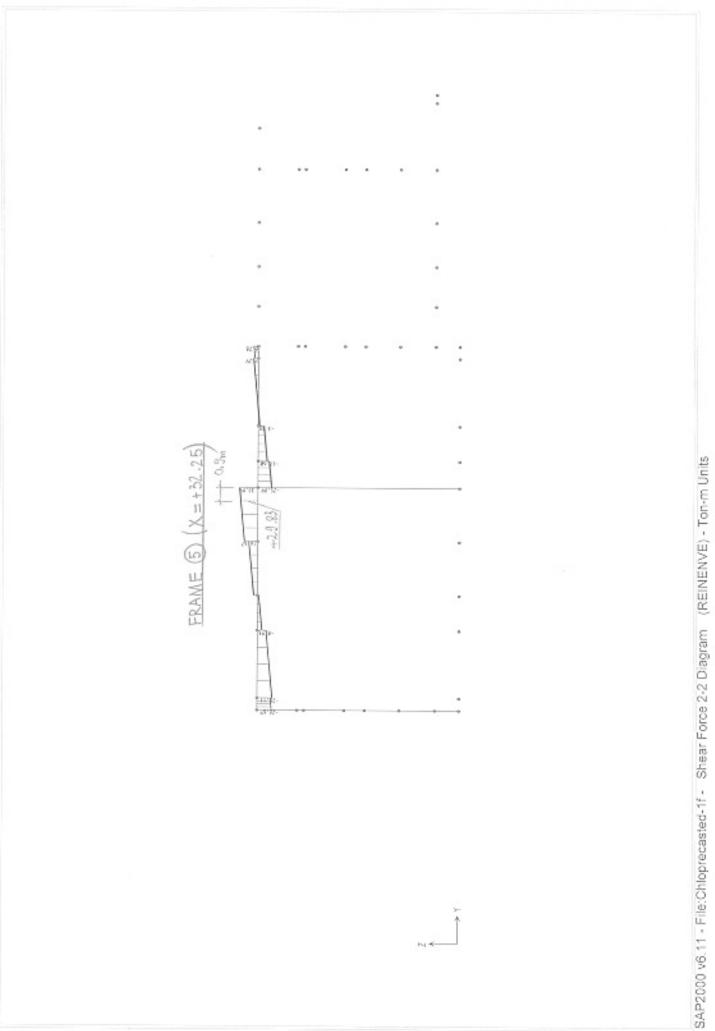








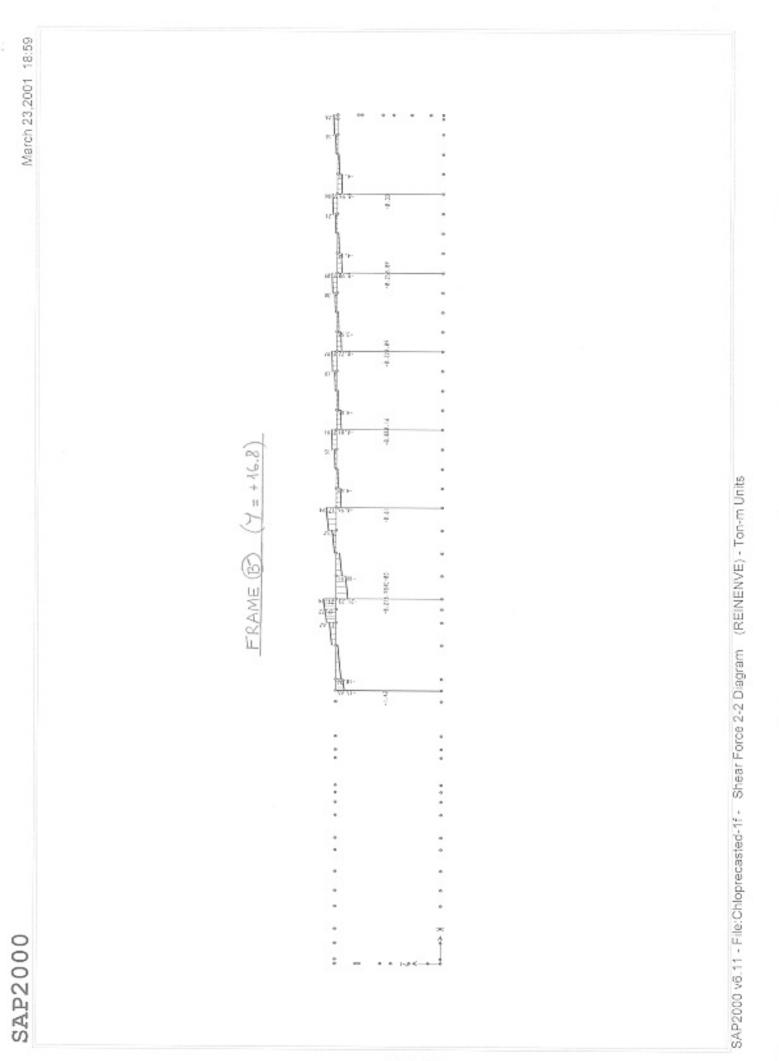




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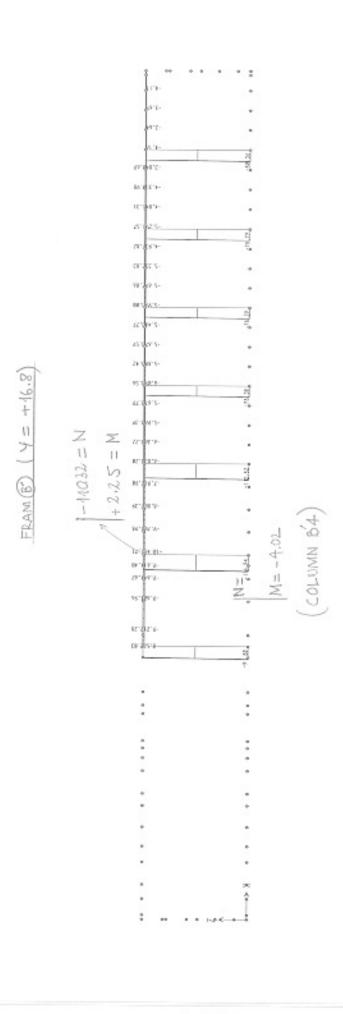
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SAP2000 v6.11 - File:Chloprecasted-1f - Moment 3-3 Diagram (REINENVE) - Ton-m Units

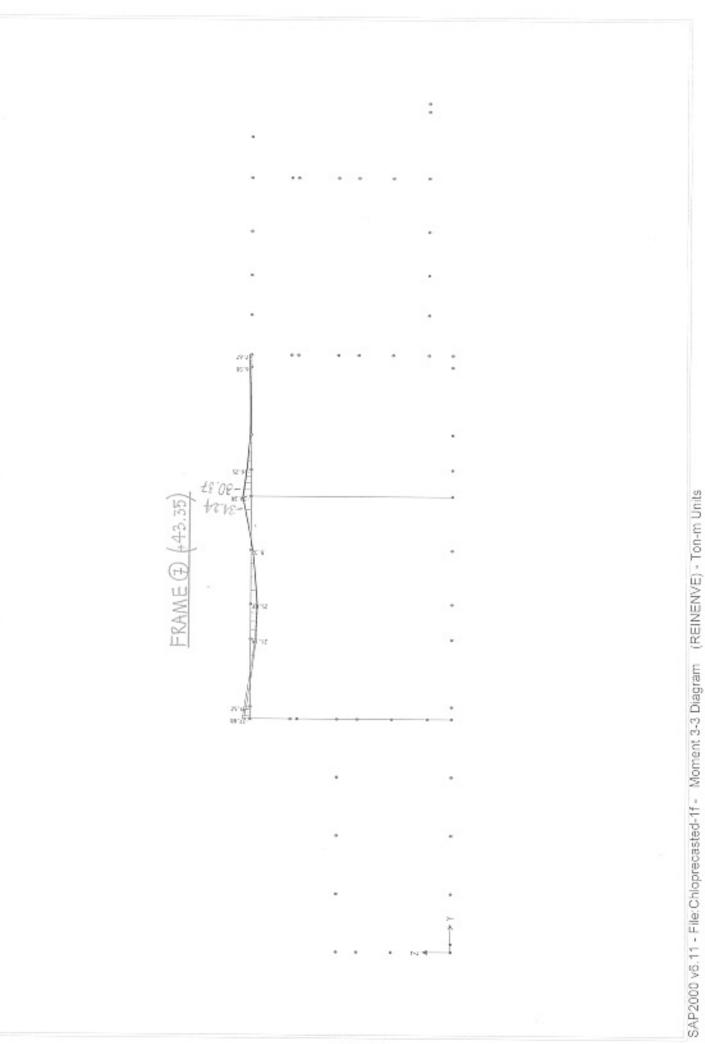


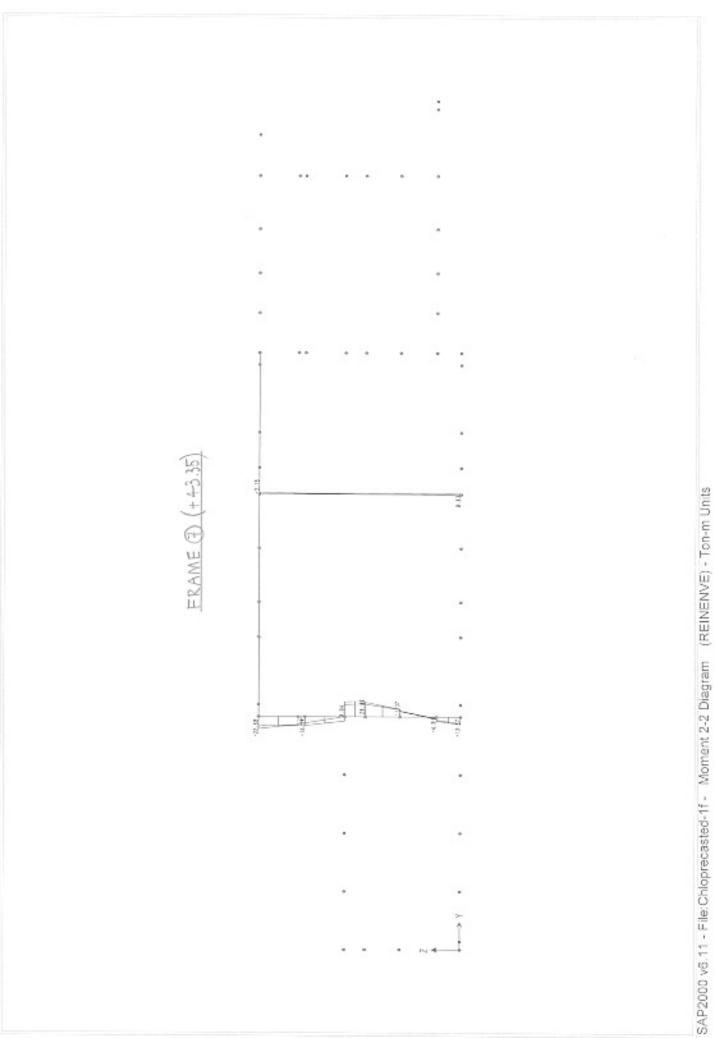
7 - 1 - 229

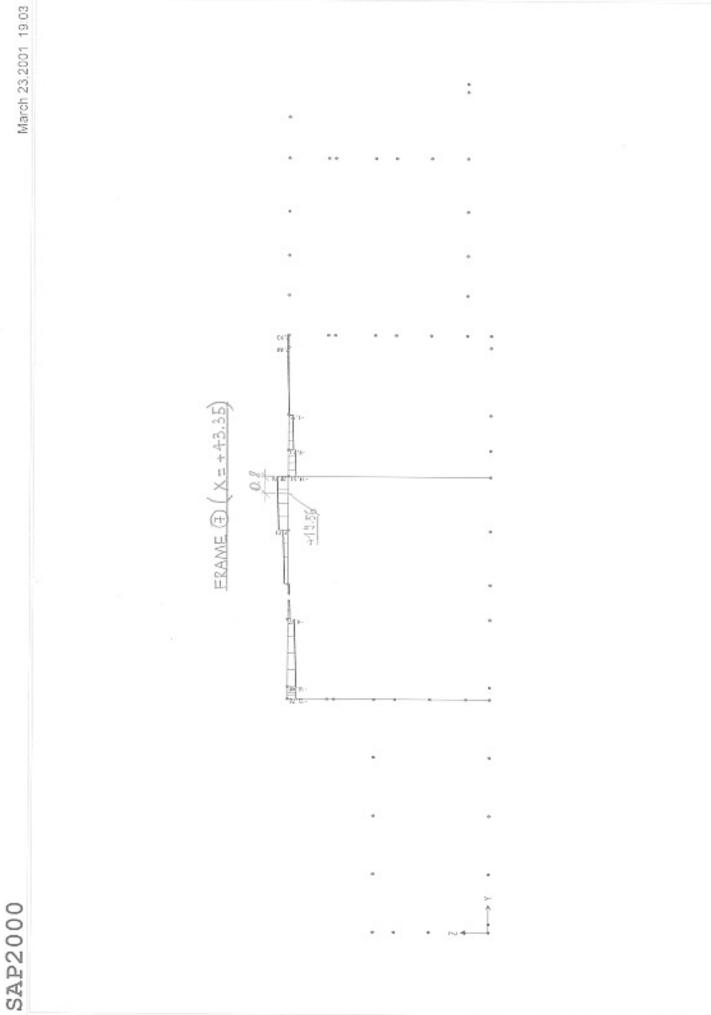
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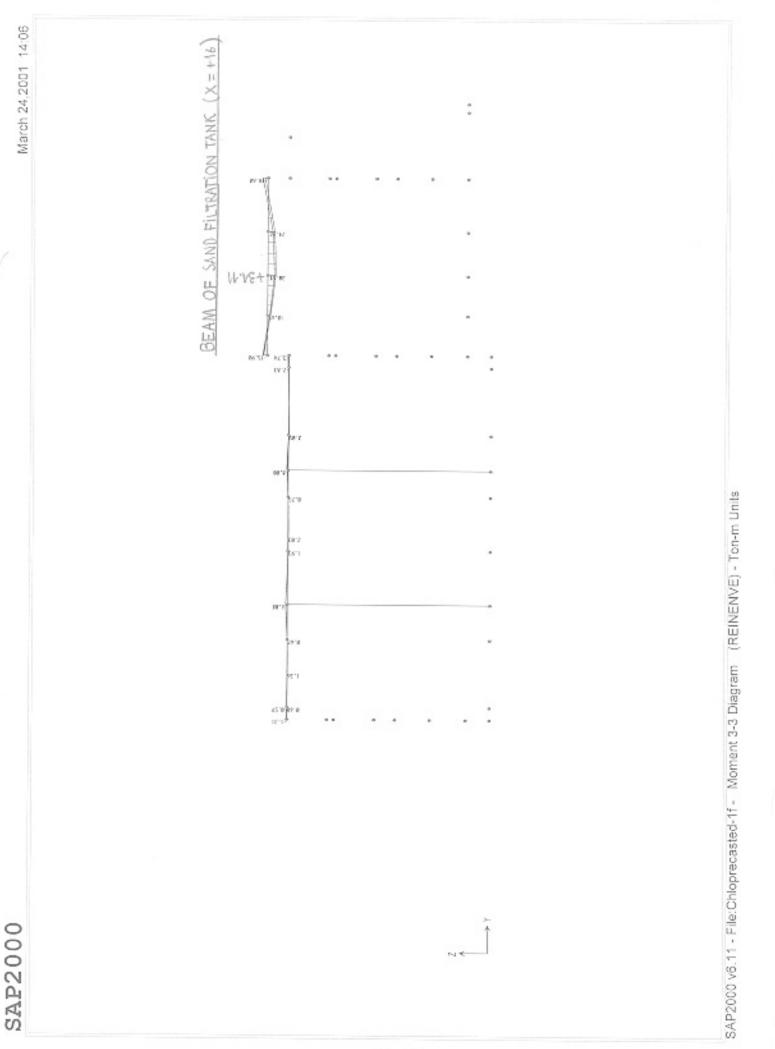
SAP2000 v6.11 - File:Chloprecasted-1f - Axial Force Diagram (REINENVE) - Ton-m Units



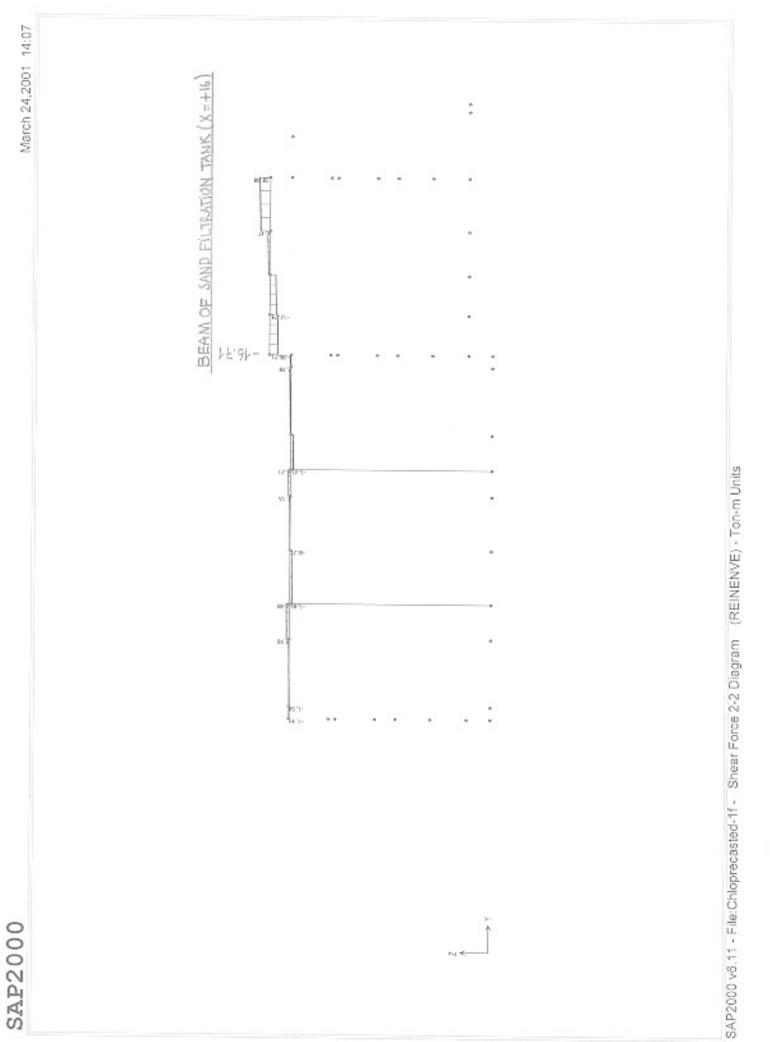




SAP2000 v6.11 - File:Chloprecasted-1f - Shear Force 2-2 Diagram (REINENVE) - Ton-m Units



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SAP2000 v6.11 - File:Chloprecasted-1f - Moment 3-3 Diagram (REINENVE) - Ton-m Units

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SAP2000 v6.11 - File:Chloprecasted-1f - Axial Force Diagram (REINENVE) - Ton-m Units