

JAPAN INTERNATIONAL COOPERATION AGENCY (JICA)

PEOPLE'S COMMITTEE OF HO CHI MINH CITY (PCHCMC)

MINISTRY OF PLANNING AND INVESTMENT (MPI)

THE SOCIALIST REPUBLIC OF VIET NAM

**THE DETAILED DESIGN STUDY
ON
HO CHI MINH CITY
WATER ENVIRONMENT IMPROVEMENT PROJECT
IN
THE SOCIALIST REPUBLIC OF VIET NAM**

FINAL REPORT

MAIN REPORT

VOLUME 2

JUNE 2001

PACIFIC CONSULTANTS INTERNATIONAL

Foreign Currency Exchange Rate Applied in this Report

Currency	Exchange Rate/US\$
Vietnamese Dong (VND)	14,500
Japanese Yen (JPY)	110.00

(As of December, 2000)

**MAIN REPORT : VOLUME 2
DETAILED DESIGN**

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**MAIN REPORT : VOLUME 1
DEFINITIVE PLAN**

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ABBREVIATIONS

1. ORGANIZATIONS

ADB	Asian Development Bank
CDM	Camp Dresser & MaKee International Inc.
CEC	City Environment Company
CITENCO	Ho Chi Minh City Environmental Company
DARD	Department of Agriculture and Rural Development
DFP	Department of Finance-Pricing
DHI	Danish Hydraulics Institute
DOF	Department of Finance
DOSTE	Department of Science, Technology, and Environment
DPI	Department of Planning and Investment
DTPW	Department of Transport and Public Works
ENCO	Environmental Committee (renamed recently to the Environmental Management Section)
GOJ	Government of Japan
GOV	Government of Viet Nam
HCMC	Ho Chi Minh City
IDA	International Development Association
JICA	Japan International Cooperation Agency
JBIC	Japan Bank for International Cooperation (previous name: Overseas Economic Cooperation Fund: OECF)
MARD	Ministry of Agriculture and Rural Development
MOC	Ministry of Construction
MOF	Ministry of Finance
MOSTE	Ministry of Science, Technology, and Environment
MPI	Ministry of Planning and Investment
OECF	Overseas Economic Cooperation Fund (renamed to Japan Bank for International Cooperation: JBIC)
OWM	Office of Waterway Management
PC	People's Committee
PCHCMC	People's Committee of Ho Chi Minh City
PMU	Project Management Unit
SDC	Sewerage and Drainage Company
SOE	State Owned Enterprise
UDC	Urban Drainage Company
UPI	Urban Planning Institute
URENCO	Urban Environment Company
USAID	United State International Assistance Department
WB	World Bank
WSC	Water Supply Company

2. TERMINOLOGY

ATP	Affordability to Pay	BOD	Biochemical Oxygen Demand
B/C	Benefit by Cost Ratio	cm	Centimeter
CIF	Cost, Insurance and Freight	COD	Chemical Oxygen Demand
DHWL	Design High Water Level	DLWL	Design Low Water Level
DO	Dissolved Oxygen	FC	Foreign Currency
EIRR	Economic Internal Rate of Return	FOB	Free on Board
FIRR	Financial Internal Rate of Return	GIS	Geographic Information System
GRDP	Gross Regional Domestic Products	GDP	Gross Domestic Products
ha	Hectare	HH/HHs	Household or Households
IDF	Intensity-Duration-Frequency	kg	Kilogram
km	Kilometer	km ²	Square kilometer
kw	Kilowatt	l	Litter
m	Meter	m/s	Meter per second
m ²	Square meter	m ³	Cubic meter
m ³ /s	Cubic meter per second	mg	Milligram
mg/l	Milligram per litter	mm	Millimeter
LC	Local Currency	NLTN	Nhieu Loc Thi Nghe
NPV	Net Present Value	OM or O/M	Operation and Maintenance
s	Second	SCF	Standard Conversion Factor
SE	South East	SS	Suspended Solid
SAPROF	Special Assistance for Project Formation		
THBNDT	Tau Hu Ben Nghe Doi Te	THLG	Tan Hoa Lo Gom
TLBC	Thanh Long Binh chang	TQC	Total Quality Control
US\$	United States Dollar	VND	Vietnamese Dong
%	Percent		

Chapter 1
GENERAL

CHAPTER 1 GENERAL

1.1 General

Construction works for HCMC Water Environment Improvement Project in Phase I are divided into following five (5) packages;

- Package A: Tau Hu-Ben Nghe Canal Improvement
- Package B: Pump Drainage Improvement
- Package C: Interceptor Sewer Construction, Intermediate Wastewater Pumping Station Construction and Procurement of Sewer Cleaning Equipment
- Package D: Conveyance Sewer Construction and Existing Combined Sewer Improvement
- Package E: Wastewater Treatment Plant Construction

Detailed design report is compiled based on the construction packages mentioned above.

Detailed design report consists of (1) Summary report, (2) Main report, (3) Design report, (4) Data book, and (5) Draft bidding documents.

Main report indicates the applicable design code and standard, design condition and criteria, and design concept of each facility. In the design report, all structural calculation sheets and basic data are compiled. Topographic survey data is compiled in data book. And bidding document is composed of (i) Vol. 1: Invitation of bid, Instruction of bidders, Conditions of contract, Bill of quantities and etc, (ii) Vol. 2 Specification and (iii) Vol. 3 Tender drawings.

The following shows the composition of the detail for the report.

- (1) Summary
 - Japanese
 - English
- (2) Main Report
 - Volume 1 (Definitive Design)
 - Volume 2 (Detailed Design)
- (3) Design Report
 - Volume 1 (Structural Calculation)
 - Volume 2 (Structural Calculation)
 - Volume 3 (Quantity Estimation)
- (4) Data Book



Volume 1 (Water quality survey, EIA, etc)
Volume 2 (Topographic Survey and Soil Investigation)

(5) Draft Bidding Documents

Package A	Prequalification Documents Volume 1 Volume 2 (Specification) Volume 3 (Tender Drawings)
Package B	Prequalification Documents Volume 1 Volume 2 (Specification) Volume 3 (Tender Drawings)
Package C	Prequalification Documents Volume 1 Volume 2 (Specification) Volume 3 (Tender Drawings)
Package D	Prequalification Documents Volume 1 Volume 2 (Specification) Volume 3 (Tender Drawings)
Package E	Prequalification Documents Volume 1 Volume 2 (Specification) Volume 3 (Tender Drawings 1/3) Volume 3 (Tender Drawings 2/3) Volume 3 (Tender Drawings 3/3)

Chapter 2

***TAU HU-BEN NGHE CANAL
IMPROVEMENT
(PACKAGE A)***

CHAPTER 2 TAU HU - BEN NGHE CANAL IMPROVEMENT (PACKAGE A)

2.1 General

Tau Hu- Ben Nghe Canal locates along southern edge of the central business area of HCMC and flow into Saigon River to the east. Total length of the canal is more than 12.0km. The canal formerly had three functions, such as urban drainage for storm water and sewer from 8 Districts of the city, waterway transportation between Mekong Delta and HCMC, and water body open space. Due to the recent rapid growth of the city, however, these functions have being deteriorated drastically by much sedimentation of garbage, waste disposal and soil deposit, encroachment of illegal houses and buildings on/along the canal, and direct inflow of domestic and home industry wastewater from its basin. The canal improvement work is necessary to create its primary functions.

This report covers the further study of Basic Design on Tau Hu – Ben Nghe Canal Improvement Project (Phase I).

2.2 Scope of Project

Package A: Tau Hu – Ben Nghe Canal Improvement Project (Phase I) (hereinafter referred to as “THBNCIP”) consists of the following components:

- (a) Ben Nghe canal improvement: L = 3,158m
- (b) Tau Hu canal improvement: L = 4,128m
- (c) Landscape work on both banks of Tau Hu – Ben Nghe canals
- (d) New road construction of about 360m long including replacement of utility services, sewer, water supply pipe, electricity and telecommunication line along south bank of Tau Hu canal

2.3 Design of Channel Improvement

2.3.1 General

In the previous study stage, channel improvement plan of Tau Hu – Ben Nghe canal system of about 13.38km including Lo Gom and Ngang No.1 to No.3 canals, which was formulated in the feasibility study, has been reviewed based on the recent additional data and information.

This study stage includes the detailed design on Tau Hu – Ban Nghe Canal Improvement Project (Phase I), of which channel section is between the junctions with Saigon River to the east and Ngang No.1 Canal to the west. Objective channel length for the detailed design is about 7.3km in total as shown in Fig. 2.1.

2.3.2 Channel Alignment

Through several meetings between JICA Study Team and PMU of HCMC East-West Highway Project (EWHP), the channel alignment of Tau Hu – Ben Nghe Canal has been revised and finalized. Main revised channel alignments in Phase I project are mentioned below:

- (a) Total length of canal improvement of Ben Nghe and Tau Hu (Downstream) canals in the feasibility study is revised as follows:
 - Ben Nghe Canal: 3,140m in F/S to 3,158m in D/D
 - Tau Hu (Downstream): 4,220m in F/S to 4,128m in D/D
- (b) Alignment of Ben Nghe Canal between Khan Hoi and Mong bridges is shifted about 10m to the south in order to acquire the additional land for relocation of existing access road of Mong Bridge, which is to be left as a historical structure.
- (c) Radii of previous S-curve alignment of Tau Hu canal nearby Cho Quan Hospital is revised from 100 and 175m to 160 and 250m in order to acquire the minimum road curb radius of 200m, which is proposed in EWHP. Accordingly, the existing city road (Ben Ba Dinh Road) of about 350m located along south bank (right bank) of Tau Hu Canal is necessary to replace. Existing public utility services, such as sewer, water supply pipe, electricity and telecommunication line are also simultaneously required to replace.

Proposed channel alignment is shown in Fig. 2.1. Detailed channel alignments with scale of 1:1000 are compiled as PA-THBNCI-201 to 214 in Tender Document, Volume III Tender Drawing (Package B Tau Hu – Ben Nghe Canal Improvement).

2.3.3 Profile and Cross Section

The canal bed elevation, bed slope and cross sections designed in Definitive Plan are basically not changed. Namely,

- (a) Canal bed elevation is designed to meet the requirements of navigation boat of 100 – 200ton. So, highest canal bed elevation of Ben Nghe and Tau Hu (Downstream) canals shall be EL. -3.30m.
- (b) Canal bed slope is designed at 1:20,000 to be more gentle hydraulic gradient of DHWL, which is principally almost the same or lower than the existing ground elevation along the canals not to create newly drainage problems.
- (c) Following two (2) types of single cross sections are designed mainly in view of

difficulty of land acquisition.

- Type A: Trapezoidal in shape channel with 1:1.5 in slope lined by revetment of stone masonry
- Type B: Trapezoidal in shape channel with 1:0.5 in slope lined by revetment of stone masonry

Table 2.1 (1/4) to (4/4) and Fig.2.2 show the proposed profile of THBNCI (Phase I). Table 2.2 (1/7) to (7/7) show the Proposed cross section of THBCI (Phase I). Detailed profile (1:100 in vertical, 1:1000 in horizontal scale) and cross sections with scale of 1:200 are also compiled as PA-THBNCI 201 to 214 and PA-THBNCI-215 to 244 respectively in Tender Document, Volume III Tender Drawing (Package B Tau Hu – Ben Nghe Canal Improvement).

2.4 Design of Revetment

2.4.1 General

Revetments along Tau Hu – Ben Nghe canal have been constructed at some sections along in different times to develop waterway transport facility and to prevent scouring and sliding of the canal banks as well as to improve the landscape of the canal. Execution agency of these constructions was Office of Water Management (OWM). However, the discontinuous revetment has created negative effects to protection purpose and landscape. On the other hand, there are still many portions without any bank protection facility, so that the construction of revetment for Tau Hu – Ben Nghe canal is necessary.

Revetment construction is an important component of Tau Hu – Ben Nghe Canal improvement project. It is carries out to create the following functions:

- Prevent the scouring and sliding for both banks of the canal
- Clean the canal banks caused by dumping waste along both bank of the canal
- Improve general landscape in line with the City Development
- Decrease roughness coefficient of the canal banks (to increase discharge capacity)

2.4.2 Applicable Codes and Standards

The detailed design of channel revetment is conducted based on the following Vietnamese codes and standards:

- Flood Control Embankments, Design Procedure: 14 TNC 84-91, Ministry of Agriculture and Rural Development
- TCVN 4612-1998 and TCVN 5574-1991 Vietnamese design standard for reinforced concrete structure
- TCVN 5573 – 1991 Vietnamese design standard for masonry structures

- Bearing capacity of wooden pile, concrete material is referred on available Vietnamese hand books

As reference information, the following Japanese standards are also applied.

- Manual for River Works in Japan: Published by River Bureau, Ministry of Construction, Japan
- Standard Specification for Design and Construction of Concrete Structures, 1996, Japan Society of Civil Engineering

2.4.3 Design Condition and Criteria

(1) Design Length in Phase I Project

Design length in Phase I project includes Ben Nghe Canal of 3.16km from confluence with Saigon River to Chu Y Bridge and Tau Hu (Downstream) Canal of 4.12km from Chu Y bridge to T-junction with Ngang No.1 Canal. Total design length is about 7.28km.

(2) Hydraulic Parameter

Design water levels (DHHWL, DHWL, DLWL) of Tau Hu – Ben Nghe canal system are as follows:

Canal	DHHWL (m)	DHWL (m)	DLWL (m)
Ben Nghe	+1.54	+1.35	-2.05
Tau Hu	+1.54	+1.42	-2.00

- Note:
1. DHHWL: Design highest high water level in Mui Nai benchmark (recorded historical maximum water level at Phu An station at Saigon River):
 2. DHWL: Design high water level in Mui Nai benchmark (Average of monthly maximum water levels for the month August and November)
 3. DLWL: Design low water level in Mui Nai benchmark (Average of annual minimum water level)

(3) Geological Condition

Soil condition along Tau Hu – Ben Nghe canal was surveyed and analyzed for 14 boreholes. Geologic stratum at each borehole is very similar except thickness of each layer, as shown in Fig.2.3. Soil characteristics of each layer are summarized below

- (a) Layer 1: Made ground – clay sand or gravelly sand with cobbles of stone, brick, concrete (OH layer)

This layer is occurred at all boreholes with the thickness ranging from 0.7m (borehole

SC-08) to 3.0m (borehole SS-11).

- (b) Layer 2: Very soft, high plasticity blackish or gravelly sand with cobble of stone, brick and concrete

This layer is occurred at most boreholes with the thickness from 1.5m (boreholes DBN – 04) to 6.4m (borehole SC-02). Main soil characteristics of this layer is as follows:

- Wet density: $\gamma_w = 1.461 \text{ g/cm}_3$
- Natural moisture: 95%
- Unconfined compressive strength: $q_u = 0.175 \text{ kg/cm}_2$
- Compression index: $C_c = 1.274$
- Coefficient of consolidation: $C_v = 3.65 \times 10^{-4}$

- (c) Layer 4: Stiff, low plasticity yellowish brown, grey clay (CL)

It is also found at most boreholes with the thickness from 1.5m (borehole DBN-02) to 5.0m (borehole SC-09). The main characteristics are as follows:

- Wet density: $\gamma_w = 1.963 \text{ g/cm}_3$
- Natural moisture: 20 - 35%
- Unconfined compressive strength: $q_u = 1.202 \text{ kg/cm}_2$
- Compression index: $C_c = 0.181$
- Coefficient of consolidation: $C_v = 6.88 \times 10^{-4}$

- (d) Layer4b: Medium dense, whitish gray, yellowish brown clayey sand (SC)

This is occurred at most boreholes with the thickness from 4.0m (borehole DBN-04) to 14.0m (borehole SC-01). The main characteristics are mentioned below:

- Wet density: $\gamma_w = 1.98 \text{ g/cm}_3$
- Natural moisture: 12 - 25%
- Unconfined compressive strength: $q_u = 0.928 \text{ kg/cm}_2$
- Compression index: $C_c = 0.155$
- Coefficient of consolidation: $C_v = 9.28 \times 10^{-4}$

Lower layers does not concern to stability of revetment. The above soil data are used for analysis of anti-sliding of the revetment and the canal banks

- (4) Bank elevation

Top elevation of the bank (EL.+2.00m) proposed in the feasibility study is revised at EL.+2.20m in Mui Nai benchmark to harmonize the future road improvement plan (HCMC East-West Highway Project). Accordingly, freeboard of the canal is able to ensure about 0.6 to 0.9m.

- (5) Quality requirement for materials

The following requirements for quality of materials are applied.

- Concrete block of revetment foundation: Cast-in situ reinforced concrete blocks are made using Concrete Class E (cylinder 28 days compressive strength is 210 kgf/cm²)
- Cast in-situ reinforced concrete: Concrete Class E is applied for apron, wing wall, headwall of outlets
- Concrete class F (cylinder 28 days compressive strength is approximately 180 kgf/cm²) is used for plain concrete of revetment including stone masonry
- Concrete class G (cylinder 28 days compressive strength is approximately 100 kgf/cm²) is applied for leveling of foundation and outlets
- Cobble stone of average diameter 250mm – 300mm is acceptable for stone masonry
- Timber pile of $\phi 80\text{mm}$ - $\phi 100\text{mm}$ and 4.5m long is applied.

2.4.4 Design of Revetment

(1) Revetment Type A

Revetment type A is designed to satisfy the following requirement

- To harmonize with proposed canal cross section
- To be strong structure and economical
- To be easy for land acquisition and house evacuation
- To be easy construction
- To Beautify

As shown in Fig.2.4, this type is lined by stone masonry with 1:1.5 slope, which is the most typical type revetment in HCMC and applied for bank protection having no land acquisition and house relocation problems. Stone with mortar of 300mm thick is supported and fixed by plain concrete layer with thickness of 100mm. Free draining gravel of 300mm thick in average is placed between plain concrete layer and sandy soil filling. In front of sandy soil filling, geotextile sheet is provided to protect outflow of filling material. PVC pipe of 100mm diameter (weep hole) is provided at 2.0m interval in 3 stages to drain out groundwater to the canal in short term during low tide. These weep holes are utilized for stability of revetment. The foundation of revetment is made of cast-in-situ reinforced concrete block of 600mm wide, 400mm high and 4.0m long. This foundation concrete block is supported by wooden piles of $\phi 80$ - $\phi 100\text{mm}$ diameter, $l = 4.50$ and 25pcs/m². For some low-lying area to be filled up more than 2.0m, the same wooden piles are also driven with density of 9 pcs/m² to prevent from sliding risk due to filling. To avoid the cracks by expansion and contraction due to temperature variation, elastic joint filler of 10mm thick is designed to provide in front and behind of one lot revetment of 26m long.

As shown in Fig.2.1, this type is applied for all along Ben Nghe canal of about 3.16km and downstream channel of Tau Hu canal of about 2.69km. The construction of revetment is carried out under dry condition. Therefore, cofferdam is required for drying up construction site.

(2) Revetment type B

As shown in Fig.2.4, This type is lined by stone masonry with slope of 1:0.5 and is applied for channel having land restriction. The structure of this type is basically the same as that of Type A except the following some changes:

- Thickness of stone masonry is 450mm
- Thickness of concrete layer is 150mm
- Dimension of RC block foundation is 800mmW x 400mmH x 4mL
- Thickness of free draining gravel layer is 300 – 375mm

This type revetment is applied for up and downstream channel of about 1.34km long at Cha Ba Bridge of Tau Hu canal, as shown in Fig.2.1.

(3) Proposed Length of Revetment to be Newly Constructed

Length of Type A and B revetments by left and right bank of Tau Hu – Ben Nghe canal is summarized in the table below:

REVETMENT LENGTH BY REVETMENT TYPE						
Revetment type	Ben Ngeh (m)			Tau Hu (m)		
	Left Bank	Right Bank	Total	Left Bank	Right Bank	Total
Type A	2,777	2,910	5,687	2,654	2,539	5,193
Type B	0	0	0	1,343	1,389	2,732
Total	2,777	2,910	5,687	3,997	3,928	7,925

Note: All figures are not included revetment length of 223m to be constructed by other related projects, such as HCMC East-West Highway Project, Hang Bang Drainage Improvement and Package D in JBIC Project.

2.4.5 Countermeasure for Sliding of Bank due to Filling

Prevention work for sliding of the canal banks due to filling work is studied based on the following conditions

- Existing soil condition along Tau Hu – Ben Nghe canal
- Filling volume
- Filling materials

Existing soil condition used on this study is based on the results of soil investigation for 4 boreholes, SS-02, SS-09, SS-08, SC-01 along Tau Hu canal and 10 boreholes, SC-01, DBN-03, DNB-02, DBN-01, SS-10, SS-12, SS-11, DBN-04 along Tau Hu (Downstream)

Filling volume of each section is estimated based on the channel alignment, profile and cross section plan, which are compiled in Data Book. Soil characteristics for filling material of sandy soil/clay (mixing soil with black sand and clay) are as follows:

<u>Item</u>	<u>Sandy Soil/Clay</u>
- Specific gravity:	$\gamma = 1.6 \text{ (t/m}^3\text{)}$
- Cohesion:	$C_u = 1.5 \text{ (t/m}^2\text{)}$
- Internal friction angle:	$\phi = 0^\circ$

(1) Selection of typical section

Typical risky sections for filling work are selected based on the following conditions:

- Weak soil condition. It is the most important factor.
- Big filling volume, especially high depth and large width
- Steep slope revetment
- All above conditions

Weak soil layer, according to soil investigation report, is confirmed to be OH layer (Very soft, high plasticity blackish gray organic clay). The main soil characteristics of OH layer are shown below:

- Wet density $\gamma_w = 1.4 - 1.7 \text{ (t/m}^3\text{)}$
- Cohesion $C_u = 0.65 - 1.10 \text{ (t/m}^2\text{)}$

With these requirements, there are four (4) cases can be selected to evaluate for prevention of sliding. These are:

- (a) Case 1 (Cross section No.7): This case is very weak soil condition and high depth of filling. As mentioned in Fig.2.5, this case is for soil condition at boreholes SS-02 and SC-01.
- (b) Case 2 (Cross section No.24): This case is weak soil condition and high depth of filling. As shown in Fig.2.5, This case is for soil condition at borehole SS-08.
- (c) Case 3 (Cross section No.82): This case is weak soil condition and very high depth of filling. As shown in Fig.2.6, this case is soil condition at borehole SS-10.
- (d) Case 4 (Cross section No.120): This case is stiff revetment and high depth of filling. As shown in Fig.2.6, this case is for soil condition at

borehole SS-11

(2) Sliding Analysis

Wooden pile of $\phi 80 - 100\text{mm}$, $l = 4.50\text{m}$ is driven to strengthen the soil condition. The cohesion coefficient of OH Layer depend on number of wooden pile is indicated below:

Number of pile (Pcs/m ²)	0	6	9	16	25
C _u of 1:1.5slope Revetment	0.65	0.90	0.995	1.225	1.50
C _u of 1:0.5slope Revetment	0.65	0.70	0.80	0.90	1.02

Note: The above cohesion coefficient was proposed by Dr. CHAU NGOC, Technical University.

Software FORUM-8, which was developed in Japan, is used for sliding analysis of canal banks due to filling work.

Wooden pile driven is separated into two parts

- (a) Part1: Wooden pile driven for revetment foundation with the density of 25pcs/m² is constant
- (b) Part 2: Penetration of wooden pile in filling area with the various pile density of 0, 6, 9, 16 and 25 pcs/m²

(3) Calculation Result

The canal banks are stable when $F_s = (M1 : M2) > 1.2$

Where, M1 is anti-sliding moment

M2 is sliding moment respectively

According to the results of several sliding calculation cases, if the wooden piles with 9 pcs/m² are penetrate in the part 2 area, it is found that safety value of F_s is more than 1.2, as shown in Table 2.3 (1/4) to (4/4).

(4) Conclusion

Base on the result of sliding analysis, it is concluded as follows:

- In case of more than 2.0m filling height, it is necessary to penetrate wooden pile before filling to strengthen the subsoil for sliding.
- Penetration of wooden pile is separated into two parts: Foundation of revetment with density of 25pcs/m² and filling area with density of 9 pcs/m². Penetration boundary is limited by outer edge of O/M road
- Wooden pile of $\phi 80 - \phi 100$, $L = 4.5\text{m}$ is applicable.

2.4.6 Temporary Cofferd Dam

Tau Hu – Ben Nghe Canal is always affected by tidal effect occurring two times per day. Considering the low excavation level of EL.-2.40m for construction of concrete foundation of revetment, temporary cofferdam is necessary to provide in order to obtain a sufficient dry construction condition. Top elevation of cofferdam is designed at EL.+2.00m, which is about 60cm higher than average monthly maximum water levels of Tau Hu – Ben Nghe Canal.

Temporary cofferdam is designed to use steel sheet pile of PU 8, $l = 8.0\text{m}$, and H section steel beam of $200 \times 200 \times 8 \times 12$, $l = 3$ to 8.5m . The proposed temporary cofferdam is shown in Fig.2.7.

One lot length of revetment construction is designed at 26.0m due consideration of easiness for making dry up condition in the construction site. Accordingly, length of one lot cofferdam is designed at 30.0m to get 2.0m allowance in front and behind of revetment.

2.5 Design of Sewer Outlet

2.5.1 General

Rainwater, domestic and industrial wastewater from districts 1, 4, 5, 6, 8, 10, 11 and Binh Chanh is discharged into Tau Hu – Ben Nghe canal system through a large number of existing sewer outlets. The existing canal banks are designed to move right and left and to be protected by stone masonry revetment in the canal improvement works. Therefore, existing sewer outlets were proposed to reconstruct in Definitive Plan.

Some existing sewers connecting with outlet are clogged, encroached or broken. These are also designed to simultaneously replace.

2.5.2 Existing Sewer Outlet

There are about 96 existing sewer outlets along both banks of Tau Hu – Ben Nghe canal. They are connected with some kinds of sewers, such as concrete pipes of $D400\text{mm}$ – $D1500\text{mm}$, box culverts from dimension $1000\text{mmW} \times 1000\text{mmH}$ to $2800\text{mmW} \times 2500\text{mmH}$ single cell, double cell or brick arch sewers. Many of them have been constructed long time ago. Some sewers made from brick have been collapsed already. All of sewers and outlets are managed and maintained by Urban Drainage Company (UDC).

Some sewers have been clogged by soil and debris and out of work and some others are under the illegal houses occupying both bank of the channel. Most of outlets are broken or damaged. Existing sewers and outlets is listed in Table 2.4.

2.5.3 Applicable Codes and Standards

The following codes and standards are applied for design of sewer outlet including sewer.

- Flood Control Embankments, Design Procedure: 14 TNC 84-91, Ministry of Agriculture and Rural Development
- Vietnamese Design Standard 20 TCN – 51 – 84 from Ministry of Construction on “Outside Sewer Network” .
- Proceedings of Vietnam Construction Standards, Volume III (Ministry of Construction 1997)
- TCVN 4612-1998 and TCVN 5574-1991 Vietnamese design standard for reinforced concrete structure
- Standard Specification for Design and Construction of Concrete Structure (The Japan Society of Civil Engineers 1996)
- Standard Specification for Design and Construction of Concrete Structure (The Japan Society of Civil Engineers 1996)

2.5.4 Design Condition and Criteria

(1) Hydraulic Conditions

(a) Design water levels

Design water levels (DHHWL, DHWL, DLWL) of Tau Hu – Ben Nghe canal system are as follows:

Canal	DHHWL (m)	DHWL (m)	DLWL (m)
Ben Nghe	+1.54	+1.35	-2.05
Tau Hu	+1.54	+1.42	-2.00

- Note:
1. DHHWL: Design highest high water level (recorded historical maximum water level at Phu An station at Saigon River)
 4. DHWL: Design high water level (Average of monthly maximum water levels for the month August and November)
 5. DLWL: Design low water level (Average of annual minimum water level)

(b) Minimum velocity in drainage pipe

Velocity in additional pipe is not lower than 0.8m/s in case of maximum discharge flow. It is consist with criteria in package C and D

(2) Geological Condition

Geological conditions are explained in the previous Section 2.4.3 (2).

(3) Structural Design Condition and Criteria

(a) Allowable Stress

Concrete

Item	Concrete Type D	Concrete Type E	Concrete Type F	Concrete Type G
28-day compressive strength by cylinder test (300mmx150mm dia.)	250	210	180	100
Bending compressive stress (σ_{ca})	83.3	70	60	-
Shearing stress (τ_{ca})	4.0	3.6	3.3	-

Note: 1. Unit: kg/cm²

2. Concrete Type D: Precast concrete, Concrete Type E: Reinforced concrete
Concrete Type F: Plain concrete, Concrete Type G: Leveling concrete

Reinforcing Bar

Reinforcing Bar	Deformed Bar
Tensile and compressive stress (τ_{ca})	1,600 kg/cm ²

(b) Design Loads

- Design ground level: + 2.200 m
- Design ground water level: + 0.200 m
- Unit weight of soil: $\gamma = 1.8(t/m^3)$, friction angle $\phi = 35^\circ$
- Under ground water $\gamma = 0.8(t/m^3)$
- (Backfill is used sandy soil, locally called "black sand")
- Under ground water load: Pw (unit weight is 1.0t/m³)
- Soil pressure: P = (vertical soil load) x Ko
Ko = 0.5 (Earth pressure at rest)
- Horizontal load pressure: Pv = (Vertical load) x Ko
- Surcharge Pa = 1.0 t/m²

(4) Others

Navigation requirements are considered in the detailed design, other criteria reported in definitive plan report is also considered

2.5.5 Design of Sewer Outlet

Generally, sewers to be replaced are designed to provide the same shape and dimension

as existing one. All of additional sewers are made of pre-cast reinforced concrete pipe. Centrifugal RC pipes are supplied from factories while box culvert could be pre-cast in construction site to reduce transportation cost. The main comparison between cast –on –situ and pre-cast concrete box culvert has been done and shown in the table below:

COMPARISON OF BOX CULVERT

Item	Cast –on –situ box culvert	Pre-cast box culvert
Advantage	Low transportation cost, low material and machinery cost easy to adjust the length	Economize materials, time for construction easy to mechanize, low labor cost, good quality production
Disadvantage	High labor cost, long construction time, quality is unstable	High transportation, labor and machinery cost. Difficult to adjust the length,
Construction cost for 1m ³ Concrete	2,455,000 VND	2,100,000 VND

Based on the above comparison result, the pre-cast box culverts fabricated at the project site is applied for additional sewer box culvert along Tau Hu – Ben Nghe canal.

Drainage pipe: As shown in Fig.2.8, 4m long centrifugal RC pipes with cup end are laid on sand bedding of 350mm in thickness, which supported by 100mm thick leveling concrete layer. Wooden piles ϕ 80mm – 100mm, 4,5m long are penetrated under the additional sewers with the density of 16pcs/m². The second sanding layer covers pilling area with the thickness of 200mm.

Box culvert: As shown in Fig. 2.9, pre-cast box culverts are laid on a leveling concrete, which is supported by wooden piles ϕ 80mm – 100mm, 4,5m, penetrated under the box culverts with the density of 16pcs/m². Yellow sand is compacted on top of pilling area with the thickness of 200mm.

Outlet structures are designed to construct using cast –on –situ reinforced concrete (Concrete Type E). Outlet structure is likely U channel. It is constructed with head wall of 300mm thick, wing wall of 300mm thick and apron (bottom slab) of 350mm thick. Wooden piles of ϕ 80mm – 100mm, 4,5m and 25pcs./m² are driven to support the outlet. 100mm thick leveling concrete and 200mm thick yellow sand should be placed.

2.5.6 Temporary Cofferd Dam

Because of low invert level of sewers and high tidal effect of canal, temporary cofferdam is necessary for construction of the outlets. Generally, sewer outlet is completed before the construction of revetment. Temporary cofferdam is provided by steel sheet pile of PU8, 8.0m long and H section steel beam of 200x200x8x12, L = 3 to 8m as shown in Fig.

2.7, and is installed around the outlet to make dry up condition in the site. Top elevation of cofferdam is designed at EL.+2.00m. It is larger than outlet width about 1m. After completion of sewer outlet, temporary cofferdam shall be removed to construct the canal revetment

2.6 Earth Works

2.6.1 Earth Works for Revetment and Sewer Outlet Construction

Earth works for the construction of revetment and sewer outlet shall be conducted under the dry condition after construction of the temporary cofferdam along the canal, which was mentioned in previous Section 2.5 and 2.6. Mud Soil, sludge, low quality soil and surplus soil excavated by clamshell, backhoe or other excavator are loaded to the barge with capacity of 200 to 500ton moored near the site and transported to the designed dumping site, which will be mentioned later. If the quality of excavated soil is available for filling to both low banks of the canal, these are directly used as filling materials or hauled to stockpile provided in vicinity of the site.

Filling material along both low banks of the canal is basically used the sandy soil, which shall be adequate for the requirement mentioned in technical specification. This material is locally known as “black sand”, which is cheaper than natural sand (locally called as “yellow sand”) for sand bedding. Before filling the sandy soil for some sections, timber piles of 80 – 100mm in diameter and 4,500mm long shall be penetrated to

2.6.2 Channel Excavation

After completion of the construction of revetment and sewer outlet, channel excavation shall be commenced to increase waterway channel of Tau Hu and Ben Nghe canals. Channel excavation shall be conducted below water level. There are many types of dredger; pump type, bucket type, grab type and dipper type. In Ho Chi Minh City and its outskirts, ordinary barge (200 to 500ton) with clamshell or grab equipment is generally used for canal dredging. Grab bucket capacity is 1.2m³. Excavation around the existing bridge pier is proposed to use a suction cutter.

Dredged and dug soil and materials are loaded into the barge, of which capacity is from 140 – 300m³ and its bottom can be opened to dump. The fully loaded barge is towed to the designed dumping site by a tugboat with capacity of 150 to 250hp. During the barge is being towed to dump site, it should be covered by sheet to prevent from odor and dropping of soil.

Joint meeting with DTPW, OWM, PMU of WEIP and JICA Study Team held on 18th October 2000, OWM agree that the dumping sites in Can Gio and Nha Be districts shown in Fig. 2.10 can be used for dumping sludge dredged in THBNCIP. The proposed

dumping sites are summarized below:

(a) Tam Thon Hiep, Can Gio District

This dumping site with area of about 50ha is located at the right bank of Long Tau River, nearby T-junction with Tac Roi Canal. This site is also surrounded with Cha La and Duoc canals of 2 to 3m depth and 10 to 12m wide, as shown in Fig. 2.11. The site is low-lying area consisting of mixed land use with paddy field, papyrus field and water coconut palm field. The average tidal inundation depth is surveyed to be 0.5 to 0.7m. This dumping site has potential to receive a great amount of dredged soil and sludge. People's Committee of Tam Thon Hiep commune agree to receive light polluted dredged soil and sludge of maximum 480,000m³ from Tau Hu – Ben Nghe canals for filling up the low-lying site in order to construct the resettlement site for the residents from Tac Roi Island in line with relocation program of Can Gio District.

This dumping site is about 45 to 50km far from Chu-Y Bridge. Two navigation routes will be considered: (i) Saigon River – Nhabe River – Long Tau River, (ii) Rach Ong – rach Dia – Nha Be river – Long Tau River. Distance of both routes is not so different.

(b) Hiep Phuoc Commune, Nha Be District

This dumping site of about 25ha is located along the right bank of Lo Canal, which is a tributary of Nha Be River. It is 30 to 35km far from Chu Y-Bridge as shown in Fig. 2.12. Land use of the site is mixed one with resident and garden in filling up land, paddy field and water coconut palm field in low-lying area. An average tidal inundation depth is only 0.2 to 0.5m. People's Committee of Hiep Phuoc Commune and other households agreed to fill up light polluted dredged sludge of 50,000m³ each from Tau Hu – Bne Nghe canals.

There are also two considerable navigation routes: (i) Rach Can Guioc – Raci Doi – Dong Dien Canal – Nha Be River, (ii) Rach Can Guioc – Rach Dua – Giong River – Lo Canal.

2.7 New Road and Sewer Line Construction along Tau Hu Canal

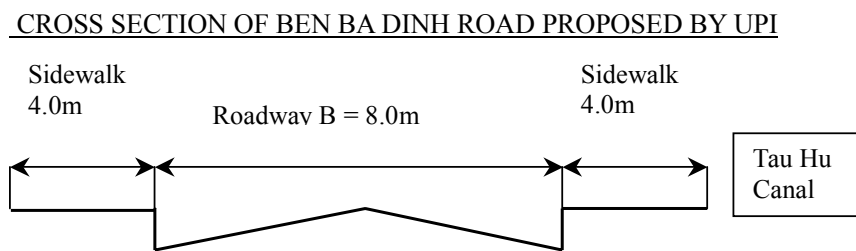
2.7.1 General

The existing Tau Hu canal alignment of about 340m long between Section 77 and 85 is planned to shift to the south with the maximum length of about 60m in order to obtain the minimum curve length of 200m for East-West Highway. As a result, the existing city road of Ben Ba Dinh Road including water supply pipe, small sewers electricity and telecommunication line shall be replaced. 74 houses shall be relocated and 30 households shall be acquitted a part of building and land. In this Section, detailed design of new Ben Ba Dinh Road and sewer line construction are mentioned below.

2.7.2 Road Construction

(1) Future Plan of Ben Ba Dinh Road

Existing Ben Ba Dinh Road is classified into “Collection Street (secondary street)”, however, it is very narrow (only 4.0 to 5.0m) and no sidewalk. According to the City Master Plan prepared by Urban Planning Institute (UPI), Ben Ba Dinh Road between Chu Y-Bridge and U Cay Canal is planned to expand to 8.0m of roadway and 4.0m of both sidewalk as shown below:



Replacement of Ben Ba Dinh Road is to be designed under the future plan prepared by UPI.

(2) Applicable Codes and Standards

The following Codes and Standard of Vietnam are applied:

- (a) 20 TCN 104-83 “Standard for Urban Highway Design” approved by Ministry of Construction, (MOC)
- (b) TCVN 4054-98 “Standard for Rural Highway Design” approved by Ministry of Transportation, (MOT)
- (c) 22 TCN 211-93 “Standard for Flexible Pavement Design” approved by MOT
- (d) TCVN 2201-1993 “ Soil for Construction – Testing method for compacting index in laboratory” approved by MOC
- (e) 22 TCN 249-98 “Specification for asphalt concrete construction, hot laid“ approved by MOT

References have been made to the following publication additionally:

- (a) A policy on Geometric Design of Highways and Streets – AASHTO 1990
- (b) Guide for Design of Pavement Structures – AASHTO 1986
- (c) Standard specification for transportation materials and methods of sampling and testing. AASHTO 1990

(3) Design Conditions and Parameters

According to 20 TCN 104-83, Ben Ba Dong Road is classified into “Collection Street”.



(a) Basic Design Parameters.

- Design load for pavement: Axle load 12 ton
Tire pressure 6 Kg/cm²
Diameter of tire mark 36 cm
- Design Traffic Volume (H10): 300 vehicle/Day
- Design speed: 60 Km/h
- Pavement structure: Asphalt concrete Type P4

(b) Basic Design Geometric Parameters

- Existing Ground Level: EL.+1.50 ~ +1.70 m
- Design new pavement level: EL.+1.60 ~ +1.80 m
- DHWL of Tau Hu canal: EL.+1.50 m
- Road bed width: 17.00 m
- Design width of pavement: 8.00 m
- Sidewalk width: North side: 5.00m and south side: 4.00 m
- Total Length of Road: 362m
- Profile slope: 0007

(c) Geological Condition

According to the soil survey results based on borehole number DBN-01 conducted on June 2000, geological conditions of Ben Ba Dong Street are summarized in the table below:

Geological Condition near Ben Ba Dong Road Replacement Site

Properties	Unit	Layer 2	Layer 2a	Layer 2b	Layer 4
Thickness of layer	m	1.8-3.5	2.0	1.5	1.2-4.0
Wet density, γ_w	g/cm ³	1.481	1.728	1.979	2.059
Unconfined compressive strength q_u	Kg/cm ²	0.241	0.371	0.621	1.198
Compression index, C_c	cm ² /Kg	0.866	0.3935	0.1946	0.194
Coefficient of consolidation, C_v	cm ² /s	3.5×10^{-4}	3.7×10^{-4}	7.8×10^{-4}	5.9×10^{-4}
Coefficient of volume compressibility, m_v	cm ² /g	9.3×10^{-5}	5.9×10^{-5}	2.3×10^{-5}	3.7×10^{-5}
Standard penetration resistance, SPT, N	Blow	0-2	4-5	4	7-15

Note: Soil classification of each layer is as follows:

- Layer 1: right on surface, thickness is from 1.00 to 3.00m.
- Layer 2: Blackish gray, ORGANIC CLAY (OH)
- Layer 2a: Yellowish brow CLAY (CH)
- Layer 2b: Blackish gray CLAYEY SAND (SC)
- Layer 4: Stiff, yellowish gray SANDY CLAY (CL)

(4) Design Layout, Profile and Cross Section

(a) Layout

Local road in THBNCI project location on a curve with radius R = 126 m. Road bed wide is 17 m, compose 8 m asphalt concrete pavement and 5+4 m sidewalk. Elevation of concrete curb at both side of pavement is 20 cm higher than that of pavement surface.

(b) Longitudinal Slope

Following existing level, profile of road designed 0.07%. Drainage water will be concentrated by cross section slope 2% to rainwater inlet at both sides.

(c) Cross Section

Cross section of road composition:

- Asphalt concrete Pavement, between two concrete curbs, have cross slope of 2%
- Design width of pavement surface 8.00 m
- Side walk at two sides of pavement, (5 + 4 m) wide

(5) Pavement Structure and Concrete Curb

(a) Pavement structure

The following asphalt concrete pavement (Type P4) is designed for the replacement of Ben Ba Dong Road.

- 5cm Wearing course, Asphalt concrete (gravel is small size)
- Emulsified asphalt Tack coat 0.2 ~ 0.7 l/m²
- 7cm Binder course, Asphalt concrete (gravel is middle size)
- Emulsified asphalt Prime coat 1.0 ~ 2.0 l/m²
- 26cm Base course, crush aggregate (size 2 ~ 4 cm)
- 15cm Sub-base course, mixture sand + 10% cement

Total thickness of pavement is 53cm.

(b) Concrete Curb

Both sides of asphalt concrete pavement are connected with concrete curb with 180W x 260H and 180W x 300H, which is constructed by cast-in-situ concrete (Concrete Type E). Top elevation of curb is 20cm higher than that of pavement surface.

2.7.3 Sewer Line Construction

(1) General

Tau Hu canal alignment will be improved in the section of nearly 400m belonging to District 8 in the south and District 5 in the north. In this section of canal, a part of the existing road – Ben Ba Dinh – will be omitted. It is necessary to construct a new road in order to replace the omitted road as mentioned above. This new road will be also constructed with the attached infrastructure, such as electric, water supply, telephone, drainage, etc. For the drainage of the new road, a sewer line is designed.

(2) Applicable Codes and Standards.

The following codes and standards are applied for this detailed design.

- (a) Vietnamese Design Standard 20 TCN – 51 – 84 from Ministry of Construction on “Outside Sewer Network” .
- (b) Standard Specification for Design and Construction of Concrete Structure (The Japan Society of Civil Engineers 1996)
- (c) Proceedings of Vietnam Construction Standards, Volume III (Ministry of Construction 1997)
- (d) Japanese Industrial Standard (Japan Standards Association 2000)
- (e) Typical Sewer Design from HCMC Urban Drainage Company for reference.
- (f) Japanese Typical Sewer Design Sample for reference.

(3) Design Conditions and Parameters.

(a) Design Conditions:

There is no existing sewer pipes along the omitted road. But there are many small pipes with the size from 200mm to 400mm, which drain storm water and wastewater for the small inner road in the area. They discharge directly into Tau Hu canal. Therefore, the designed sewer on the new road has to connect with all these small existing sewer pipes.

(b) Design Parameters:

Design parameters applied in this design are as follows:

- Service Area: 3,9 ha, including the area of 1.7ha, which is outside this section will be connected in the future.
- Flow system: Gravity flow
- Sewer system: Combined system
- Flow equation: Manning formula
- Design velocity: 1,4m/s.
- Design flows of storm water and wastewater: 1.28m³/s.
- High water tide level on Tau Hu canal: +1.320
- Pipe size: 2 ϕ 800, $i = 2.5\%$, ϕ 1,200, $I = 1.2\%$
- Length of pipe : ϕ 800, $L = 362\text{m}$, ϕ 1200, $L = 11\text{m}$

(4) Design for Layout, Profile of Sewer Line:

(a) Layout Design: (refer to DWG.No PA-THBNCI-260,261 in Tender Drawings)

Because the sidewalk has no space enough to install for all facilities; the center of sewer line ϕ 800 is defined under the road, in the side which is opposite Tau Hu canal bank, and about 0 – 1.3m far from the curb based on the radius of the new road.

There are 14 manholes above this line with the step from 20m to 37m, based on the radius of the new road. The first and the last manhole will use grid cover to collect surface water because there is no inlet installed here.

There are 12 inlets on the sidewalk, which is opposite Tau Hu canal bank because the new

road is 1-slope road. They are connected with the 12 remain new manholes by connection pipes $\phi 400$ and connected with all the existing crossing small pipes of the area

Center of sewer line $\phi 1200$ is perpendicularly with the road, nearly at the middle of the line $\phi 800$.

An outlet for the pipe $\phi 1200$ will be constructed.

(b) Profile Design: (refer to DWG.No PA-THBNCI-260,261 in Volume III, Tender Drawings)

Based on (i) The design elevation of the new road mentioned in previous section, (ii) The hydraulic calculation results, (iii) The preparation of the future connection for the 100m – 150m existing continuously pipes both sides of this designed section.

Profile is designed as follow:

- Invert elevation at the manhole M1: IE = -0.200
- Invert elevation at the manhole M7: IE = -0.695 for the pipe $\phi 800$ and IE = -1.100 for the pipe $\phi 1200$.
- Invert elevation at the manhole M14: IE = -0.206
- Gradient for $\phi 800$: 2.5‰
- Gradient for $\phi 120$: 1.2‰

(5) Design for Structure

- (a) Pipes: Precast centrifugal reinforced concrete Type D – 250kg/cm².
- (b) Foundation of sewer pipes: Driving timber piles $\phi 80$ - $\phi 100$ with 16 trees/m², L = 4.5m/tree, Cast in situ concrete Type G – 100kg/cm²
- (c) Manholes: Cast in situ reinforced concrete Type E – 210kg/cm². (refer to DWG. No. PA - THBNCI-264)
- (d) Inlets: Cast in situ reinforced concrete Type E – 210kg/cm². (refer to DWG. No. PA - THBNCI-264)
- (e) Manhole cover: 12 circle cast iron covers with the load 30 Tons (Type A) and 02 grid cast Iron cover with the load 30 Tons (Type C). (refer to Drawing DWG. No.PA-THBNCI-265)
- (f) Inlet cover: 12 square cast iron covers with the load 6 Tons (Type E). (refer to DWG. No. PA-THBNCI-265)
- (g) Outlet: Cast in situ reinforced concrete Type E – 210kg/cm Drawing DWG. No. PA - THBNCI-264)

After installation of the pipes, the trench will be backfilled by yellow sand until 0.2m higher than the top of pipes and by sandy soil until the layers of new road.

(6) Construction method: using machine in combination with manual.

Cutting the road first by road cutter, then excavate the soil by using backhoe, because the excavation depth is below 6.0m. Installation of the pipes is to be conducted by the suitable crane. The trench in this section is lower than 4.0m deep, so, the trench shall be shored by the timber and the timber board. Timber board pile with section of 30mm x

300mm and timber strut and wailing of 60mm x 120mm (double) are to be used.

The excavation width is proposed as follows:

Diameter (mm)	φ800	φ1200
Width (m)	2.0	2.6

The works, such as construction of the manholes, inlets, outlets, adjustment work for the position of pipes will be conducted mostly by manual.

2.7.4 Other Utility Services

There are three utility services to be replaced simultaneously with the reconstruction of Ben Ba Dinh Road. These are water supply pipe of 150mm diameter, electric line of low voltage and 15kv in voltage and telecommunication line.

Water supply pipe of 150mm diameter is planned to install at 1.0m from the road boundary and 1.2m below ground along the southern sidewalk. 12 electric RC poles shall be newly provided at 1.0m from the edge of the roadway along the southern sidewalk to install low voltage and 15kv cables. Telecommunication lines are to be install at same time to electric RC poles.

Construction of these public utility services will be executed by the agency concerned, Ho chi Minh City Water Supply Company, Power Company and Telecommunication Company.

2.8 Landscape Work

For establishment of beautiful canal side scenery, objective area for landscaping is 7.4km in Tau Hu – Ben Nghe canal section and 6.0km in up-stream of Tau Hu canal section and total length becomes to 13.4km in length.

(1) Landscape facility

Paving of operation and maintenance space shall be colored concrete block. Concrete curb of the planting area is 1.8m square type and rectangular type of 1.8m x 9.8m. The curb rises 20cm in height from the paving level and width of top to be 20cm.

(2) Plantings

Medium size canopy trees are planted along the revetment top of operation / maintenance space. Interval of each planting tree is 8.0 m in general for consideration of future growth of crown of the trees. 1.8m x 1.8m square planting shall be applied for each planting location. Each interval of 24.0m rectangular formed planting areas is to be as for a visual accent though the O/M space, in which grass sods is planted. Introduced plants species are shown as follows.

(a) Medium size canopy tree:

Lagaestromea tomentosa 710 Nos. and *Mimusops elengi* 990 Nos shall be planted in total. Both species are H=4.0m, stem diameter is 4cm in 1.2m height from the ground. All trees shall be supported by tripod type support with pole individually to sustain trees in stable.

(b) Flowering shrub:

Ixora chinensis rosa 497 m², *Ixora macrophyssa* 497m², *Hibiscus rosa - chinensis* 693m², and *Musaenda erythrophylla* 693m² shall be planted in total.

All flowering shrub is 40cm in height and crown width is 25cm, they are planted in group with 10 Nos/m².

(c) Ground cover plant:

Wedelia trilobata 1,645m² shall be planted in total. Lower growing plants with yellow flower, height of plant is about 10cm, and 16 Nos/ m² shall be applied when planted.

(d) Grass sods:

Axonopus compressus 1,611.6m² shall be planted in total. Broad leafed type grass is planted with sods within the square planting area.

All planting work shall required proper size of planting hole and existing soil to be replaced agriculture soil.

Proposed Landscape Development Scheme along Tau Hu – Ben Nghe Canal is shown in Fig.2.13.

2.9 Allotment of a Part of Canal Improvement Works to Ongoing Relevant Project

There are three ongoing projects related to THBNCIP, such as

- (a) Dredge and Rehabilitation Project on Ben Nghe - Tau Hu - Lo Gom for Navigation managed by Office of Waterway Management (OWM)
- (b) Ho Chi Minh City East-West Highway Project (EWHP) managed by Department of Transportation and Public Works (DTPW)
- (c) Ho Chi Minh City Environmental Improvement Project (Hang Bang Canal Rehabilitation: HBCRP) assisted by Asian development Bank (ADB)

(1) Channel Excavation

According to OWM, Ben Nghe and Tau Hu canals between the junctions with Saigon River and Ngang No.1 Canal will be commenced to dredge from the end of 2000 without any house relocation along the canal. Total dredging volume of Ben Nghe (Saigon River to Chu Y-Bridge) and Tau Hu (Chu Y-Bridge to Junction with Ngang No.1 Canal) is estimated at about 192,000 and 158,000m³ respectively.

Through the meeting with PMU, OWM and JICA Study Team, it has been concluded that total dredging volume of about 350,000m³ estimated by OWM for Tau Hu – Ben Nghe canals shall be deducted from the estimated volume of about 700,000m³ in THBNCIP.

(2) Construction of Sewer Outlet and Revetment

For existing four road bridges along the canal of THBNCIP, such as Khanh Hoi, Calmette, Chu Y and Cha Ba bridges, are recommended to reconstruct in EWHP. Therefore, construction of revetment and sewer outlet proposed in up and downstream stretch of 20 to 25m from the center of the existing bridge is planned to implement in EWHP.

Moreover, six combined sewers including their outlets, which exist from Section 130 to 146 of Tau Hu Canal are proposed to reconstruct. So, construction of the revetment from 5 to 10m shall be executed simultaneously with these outlet constructions.

Table 2.5 Shows the construction works to be executed by other relevant projects.

2.10 Work Quantity

Work Quantities of Tau Hu – Ben Nghe Canal Improvement Project (Package A) are summarized in Table 2.6 (1/3) to (3/3). Details of Work Quantities are compiled in Design Report, Volume III.

2.11 Construction Plan and Schedule

This section deals with the construction plan and schedule of Package A :Tau Hu – Ben Nghe Canal Improvement.

2.11.1 Basic Condition

For preparation of construction plan and schedule, the following considerations have been taken as a basic concept of construction works.

(1) Scope of Package A Project

Package A: Tau Hu – Ben Nghe Canal Improvement Project consists of the following main construction component:

- (a) Construction of revetment
 - Type A: 5,687m for Ben Nghe canal, 5,193m for Tau Hu canal
 - Type B: 0m for Ben Nghe canal, 2,735m for tau Hu canal
 - Excavation for revetment: $V = 177,860\text{m}^3$
 - Bank filling: $V = 207,640\text{m}^3$

- Timber piling: $L = 3,102,350\text{m}$ for foundation of revetment and sliding protection

(b) Channel Excavation: $V = 201,310 \text{ m}^3$

(2) Other Conditions

(a) Mode of construction:

The construction shall be carried out by sufficient contractor(s) selected through international competitive bidding (ICB).

(b) Availability of Construction Plant and Equipment:

The major construction work shall be conducted by applying heavy equipment, which mostly procured locally, due to limited construction period and keeping good quality of construction.

(c) Construction Materials

Most of basic construction materials could be found available in this country. Some particular processed materials (steel sheet pile, H type steel beam, etc) are to be procured from outside.

(d) Pattern of Construction Method

Main work comprises of temporary cofferdam work, earth work, concrete work and stone masonry work. Temporary coffer dam and earth works are planned to conduct by heavy machinery (pile driver, clamshell, backhoe, barge, tug boat, etc.) in combination with manpower. While, concrete and stonemasonry works will be conducted by mainly manpower.

2.11.2 Major Works of the Project

Major works of Tau Hu – Ben Nghe Canal Improvement Project consists of the followings:

- (a) Construction of Sewer Outlet
- (b) Construction of Revetment including Landscape Work
- (c) Channel Excavation

The construction methods for the above major works are designed as follows:

(1) Sewer Outlet Construction

Out of 96 existing sewer outlet along the canal, 87 sewer outlet connecting with concrete pipe from $\phi 400\text{mm}$ to $\phi 2000\text{mm}$ and box culvert from $1000\text{mmW} \times 1000\text{mmH}$ to

2800mmW x 2500mmH, shall be reconstructed in this project. Basically, sewer outlet construction consists of outlet construction and sewer line construction to connect with outlet and the existing sewer.

Sewer outlet construction shall be conducted prior to revetment works. Firstly, steel sheet pile of PU8, $l = 8.0\text{m}$ shall be driven in the canal in front of the outlet to be constructed in order to provide the temporary coffer dam. The steel sheet piles, of which top elevation is 1.5 to 2.0m, shall be fixed rigidly by H section steel beam of 200x200x8x12. Then, excavation work could be conducted under the dry condition. After completion of the excavation around the designed outlet, foundation piling (wooded pile of $\phi 80 - \phi 100$, $l = 4.5\text{m}$, 25 pcs/m²) and reinforced concrete work shall be conducted. While, sewer pipe or box culvert shall be constructed simultaneously by open cut method shoring steel sheet pile and connected between the existing sewer and newly constructed outlet.

(2) Construction of Revetment

In succession of sewer outlet construction, the revetment construction would be commenced. Main works of revetment construction are (i) temporary coffer dam work, (ii) earth work, (iii) foundation work, (iv) revetment work, and (v) crest treatment work (landscape work). Fig. 2.14 shows the construction plan of revetment work.

(a) Temporary Cofferdam Work:

As mentioned in Section 2.4.6, at the commencement of the revetment work, temporary coffer dam, which is designed by steel sheet pile of PU 8, $l = 8.0\text{m}$, shall be constructed to protect the construction site from the tidal effect occurring two times per day and to obtain a sufficient dry condition. Steel sheet piles are driven by pile driver on barge and shall be supported and fixed rigidly by H section steel beam of 200x200 and 260x260. One lot length of the temporary coffer dam is designed at 30m to be able to construct the revetment of one lot length of 26.0m.

(b) Earth Work:

After completion of the coffer dam, earth work, such as cutting and filling for bank slope preparation and excavation of canal bed for revetment foundation work shall be conducted. Earth work would be conducted by backhoe, clamshell, bulldozer in combination with man power. As shown in Section 2.6.1, mud soil, low quality soil and surplus soil excavated are loaded to the barge (200 – 500 ton) and transported to the proposed dumping sites. Filling materials shall be used good quality of sandy soil/clay (mixed soil with black sand and clay).

(c) Foundation Work:

After completion of excavation of canal bed up to the design elevation, foundation piling (wooden pile of $\phi 80 - \phi 100$, $l = 4.5\text{m}$, $n = 25\text{pcs/m}^2$) shall be conducted. Wooden piles are driven by mainly manpower wooden driver. Simultaneously,

wooden piles for protection from sliding by filling are also to be driven. After piling, gravel bedding and leveling concrete work shall be conducted. Foundation of revetment is provided by reinforced concrete block (600W x 400H x 4000L for Type A revetment and 800W x 400H x 4000L for Type B revetment), which are made at the construction site in advance to shorten the construction period. Rip rap work shall be also done.

(d) Revetment Work:

In succession of foundation work, revetment work consisting of geotextile sheet, stone masonry and gravel filling works including installation of weep hole. Geotextile sheet shall be laid out uniformly taking care not to puncture the cloth. The mortar shall be well driven into gaps between stones. Gravel shall be placed between plain concrete behind stone masonry and geotextile cloth and compacted in continuous horizontal layers. During stone masonry work, weep hole shall be provided at the designated place. Sandy soil/clay filling shall be simultaneously conducted. Filling adjacent to revetment shall be placed and compacted to avoid damage to such structure. Compaction adjacent to revetment shall be carried out by suitable hand operated equipment.

(e) Crest Treatment Work (Landscape Work):

After completion of the revetment work including filling work of both banks, crest treatment works (landscape works). This work consists of the followings:

- Cable wiring and lighting work
- Concrete block pavement work including sand bedding and concrete curb works
- Planting works

(3) Channel Excavation

After completion of construction of sewer outlet and revetment in some section, channel excavation shall be commenced to increase the flow area of Tau Hu – Ben Nghe canal. Channel excavation volume is estimated at more than 71,000m³ for Ben Nghe canal and about 130,000 m³ for Tau Hu canal. (Note: This figure excludes the dredging volume of Tau Hu – Ben Nghe rehabilitation project by OWM)

As described in Section 2.6.2, channel excavation method is designed to apply local dredging method. For ordinary channel excavation, 200 to 500 ton barge with clamshell or grab equipment (bucket capacity is 1.2m³) shall be employed. Excavation around the existing bridge pier is proposed to use a suction cutter. Dredged and dug soil and materials are loaded into the barge, of which capacity is from 140 – 300m³ and its bottom can be opened to dump. The fully loaded barge is towed to the designed dumping site by a tugboat with capacity of 150 to 250hp. During the barge is being towed to dump site, it should be covered by sheet to prevent from odor and dropping of soil. The dumping site

is planned to use two places, Tam Thon Hiep (480,000m³) , Can Gio District and Hiep Phuoc Commun (100,000m³) , Nha Be District.

2.11.3 Construction Schedule

Construction schedule is prepared based on the following assumptions:

- (a) Construction could be commenced at the beginning of February 2002 after due consideration of extended over a long period of relocation implementation and tender process for construction.
- (b) Preparatory work period is considered at least two (2) months.
- (c) Construction of sewer outlet shall be commenced first, then revetment construction, of which total construction period would be 26 months, is to be followed. Channel excavation work could be commenced the beginning of the year 2004.
- (d) The Project will be completed at the end of July 2005.

The proposed construction schedule is shown in Table 2.7.

TABLE 2.1 (1/4) PROPOSED LONGITUDINAL PROFILE OF TAU HU - BEN NGHE CANAL

Station	Distance (m)	Accumu-late Distance (m)	Existing			Design			
			Bed Elevation EL1 (m)	Left Bank Elevation EL2 (m)	Right Bank Elevation EL3 (m)	Canal Bed Elevation EL4 (m)	High Water Level EL5 (m)	Left Dike Crown Elevation EL6(m)	Right Dike Crown Elevation EL7(m)
No.0 (BP)	0.00	0.00	-8.75	-	-	-3.450	1.320	-	-
No.1	50.00	50.00	-2.68	2.15	1.98	-3.448	1.321	2.150	1.980
No.2	50.00	100.00	-2.57	2.20	1.52	-3.445	1.321	2.200	1.520
No.3	50.00	150.00	-2.35	2.20	1.96	-3.443	1.322	2.350	1.960
BC1	19.47	169.47	-2.27	2.13	2.19	-3.442	1.322	2.350	1.960
SP1	20.33	189.80	-2.19	2.06	2.42	-3.441	1.322	2.350	1.960
No.4	10.20	200.00	-2.15	2.02	2.54	-3.440	1.323	2.200	2.200
EC1	10.13	210.13	-2.11	2.02	2.43	-3.439	1.323	2.200	2.200
No.5	39.87	250.00	-1.97	2.04	1.99	-3.438	1.323	2.200	2.200
No.6	50.00	300.00	-2.04	2.04	1.98	-3.435	1.324	2.200	2.200
No.7	50.00	350.00	-1.95	1.98	2.09	-3.433	1.324	2.200	2.200
No.8	50.00	400.00	-2.28	2.10	2.03	-3.430	1.325	2.200	2.200
BC2	39.47	439.47	-2.25	2.07	1.99	-3.428	1.326	2.200	2.200
No.9	10.53	450.00	-2.24	2.06	1.98	-3.428	1.326	2.200	2.200
SP2	49.78	499.78	-1.98	2.00	2.00	-3.425	1.326	2.200	2.200
No.10	0.23	500.00	-1.98	2.00	2.00	-3.425	1.326	2.200	2.200
No.11	50.00	550.00	-1.75	2.08	2.01	-3.423	1.327	2.200	2.200
EC2	10.08	560.08	-1.82	2.09	2.02	-3.422	1.327	2.200	2.200
No.12	39.92	600.00	-2.12	2.13	2.04	-3.420	1.328	2.200	2.200
No.13	50.00	650.00	-2.06	2.12	1.99	-3.418	1.328	2.200	2.200
No.14	50.00	700.00	-1.66	2.08	1.94	-3.415	1.329	2.200	2.200
No.15	50.00	750.00	-2.15	1.84	1.92	-3.413	1.330	2.200	2.200
BC3	10.73	760.73	-2.19	1.86	1.91	-3.412	1.330	2.200	2.200
No.16	39.27	800.00	-2.32	1.91	1.86	-3.410	1.330	2.200	2.200
SP3	4.44	804.44	-2.29	1.92	1.91	-3.410	1.330	2.200	2.200
EC3	43.71	848.14	-2.02	2.04	2.40	-3.408	1.331	2.200	2.200
No.17	1.86	850.00	-2.01	2.04	2.42	-3.408	1.331	2.200	2.200
No.18	50.00	900.00	-1.93	2.08	2.38	-3.405	1.332	2.200	2.200
No.19	50.00	950.00	-1.83	2.04	1.96	-3.403	1.332	2.200	2.200
BC4	6.69	956.69	-1.83	2.03	1.95	-3.402	1.332	2.200	2.200
SP4	12.90	969.59	-1.82	2.00	1.94	-3.402	1.332	2.200	2.200
EC4	12.90	982.49	-1.81	1.98	1.92	-3.401	1.333	2.200	2.200
No.20	17.51	1,000.00	-1.80	1.94	1.90	-3.400	1.333	2.200	2.200
No.21	50.00	1,050.00	-1.85	2.04	1.94	-3.398	1.333	2.200	2.200
No.22	50.00	1,100.00	-1.82	2.06	1.78	-3.395	1.334	2.200	2.200
BC5	13.28	1,113.28	-1.90	2.06	1.77	-3.394	1.334	2.200	2.200
No.23	36.72	1,150.00	-2.14	2.06	1.76	-3.393	1.335	2.200	2.200
SP5	18.23	1,168.23	-2.02	1.96	1.78	-3.392	1.335	2.200	2.200
No.24	31.77	1,200.00	-1.82	1.78	1.82	-3.390	1.335	2.200	2.200
EC5	23.18	1,223.18	-1.82	1.81	1.82	-3.389	1.336	2.200	2.200
No.25	26.82	1,250.00	-1.83	1.84	1.82	-3.388	1.336	2.200	2.200
No.26	50.00	1,300.00	-1.74	1.84	1.73	-3.385	1.337	2.200	2.200
No.27	50.00	1,350.00	-1.86	1.82	1.80	-3.383	1.337	2.200	2.200
IP6	47.66	1,397.66	-1.87	1.76	1.91	-3.380	1.338	2.200	2.200
No.28	2.34	1,400.00	-1.87	1.76	1.92	-3.380	1.338	2.200	2.200
No.29	50.00	1,450.00	-1.84	1.76	1.96	-3.378	1.339	2.200	2.200
No.30	50.00	1,500.00	-1.97	1.63	2.06	-3.375	1.339	2.200	2.200
No.31	50.00	1,550.00	-1.70	1.64	1.99	-3.373	1.340	2.200	2.200
No.32	50.00	1,600.00	-1.73	1.75	2.03	-3.370	1.341	2.200	2.200
BC7	1.07	1,601.07	-1.73	1.75	2.03	-3.370	1.341	2.200	2.200
No.33	48.93	1,650.00	-1.73	1.79	1.95	-3.368	1.341	2.200	2.200
SP7	8.43	1,658.43	-1.70	1.81	1.96	-3.367	1.341	2.200	2.200
No.34	41.57	1,700.00	-1.53	1.93	2.01	-3.365	1.342	2.200	2.200

TABLE 2.1 (2/4) PROPOSED LONGITUDINAL PROFILE OF TAU HU - BEN NGHE CANAL

Station	Distance (m)	Accumu-late Distance (m)	Existing			Design			
			Bed Elevation EL1 (m)	Left Bank Elevation EL2 (m)	Right Bank Elevation EL3 (m)	Canal Bed Elevation EL4 (m)	High Water Level EL5 (m)	Left Dike Crown Elevation EL6(m)	Right Dike Crown Elevation EL7(m)
EC7	15.79	1,715.79	-1.51	1.99	2.12	-3.364	1.342	2.200	2.200
No.35	34.21	1,750.00	-1.46	2.13	2.34	-3.363	1.342	2.200	2.200
No.36	50.00	1,800.00	-1.41	2.28	2.07	-3.360	1.343	2.200	2.200
No.37	50.00	1,850.00	-1.44	2.40	1.89	-3.358	1.344	2.200	2.200
No.38	50.00	1,900.00	-1.43	2.34	1.93	-3.355	1.344	2.200	2.200
No.39	50.00	1,950.00	-1.44	2.01	1.77	-3.353	1.345	2.200	2.200
BC8	24.33	1,974.33	-1.55	2.04	1.77	-3.351	1.345	2.200	2.200
No.40	25.67	2,000.00	-1.66	2.07	1.78	-3.350	1.346	2.200	2.200
No.41	50.00	2,050.00	-1.53	1.98	1.78	-3.348	1.346	2.200	2.200
SP7	46.09	2,096.09	-1.64	2.12	1.95	-3.345	1.347	2.200	2.200
No.42	3.92	2,100.00	-1.65	2.13	1.96	-3.345	1.347	2.200	2.200
No.43	50.00	2,150.00	-1.78	2.27	2.15	-3.343	1.348	2.200	2.200
No.44	50.00	2,200.00	-1.72	2.16	2.00	-3.340	1.348	2.200	2.200
EC8	17.84	2,217.84	-1.69	2.20	1.98	-3.339	1.348	2.200	2.200
No.45	32.16	2,250.00	-1.62	2.25	1.95	-3.338	1.349	2.200	2.200
No.46	50.00	2,300.00	-1.63	2.04	1.97	-3.335	1.349	2.200	2.200
No.47	50.00	2,350.00	-1.88	2.10	1.89	-3.333	1.350	2.200	2.200
No.48	50.00	2,400.00	-1.77	2.06	1.95	-3.330	1.351	2.200	2.200
No.49	50.00	2,450.00	-1.73	1.92	2.02	-3.328	1.351	2.200	2.200
BC9	8.00	2,458.00	-1.72	1.93	2.00	-3.327	1.352	2.200	2.200
No.50	42.00	2,500.00	-1.62	1.98	1.92	-3.325	1.352	2.200	2.200
SP9	1.94	2,501.94	-1.64	1.98	1.92	-3.325	1.352	2.200	2.200
EC9	43.94	2,545.87	-1.91	1.85	1.83	-3.323	1.353	2.200	2.200
No.51	4.13	2,550.00	-1.93	1.84	1.82	-3.323	1.353	2.200	2.200
No.52	50.00	2,600.00	-1.86	1.83	1.93	-3.320	1.353	2.200	2.200
No.53	50.00	2,650.00	-1.84	1.83	-	-3.318	1.354	2.200	2.200
No.54	50.00	2,700.00	-1.87	1.90	1.60	-3.315	1.355	2.200	2.200
No.55	50.00	2,750.00	-2.04	1.95	1.58	-3.313	1.355	2.200	2.200
No.56	50.00	2,800.00	-2.07	2.15	1.62	-3.310	1.356	2.200	2.200
No.57	50.00	2,850.00	-2.22	2.39	1.91	-3.308	1.357	2.200	2.200
BC10	26.36	2,876.36	-2.34	2.84	1.71	-3.306	1.357	2.200	2.200
No.58	23.64	2,900.00	-2.45	3.24	1.53	-3.305	1.357	2.200	2.200
SP10	21.77	2,921.77	-2.52	2.85	1.50	-3.304	1.357	2.200	2.200
No.59	28.24	2,950.00	-2.63	2.34	1.45	-3.303	1.358	2.200	2.200
EC10	17.17	2,967.17	-3.42	2.24	-	-3.302	1.358	2.200	2.200
BC11	9.84	2,977.01	-3.88	2.18	-	-3.301	1.358	2.200	2.200
No.60	22.99	3,000.00	-4.94	2.04	-	-3.300	1.358	2.200	2.200
No.61	50.00	3,050.00	-6.66	1.93	-	-3.564	1.360	2.200	-
No.62	50.00	3,100.00	-7.04	1.88	-	-5.042	1.361	2.200	-
No.63	50.00	3,150.00	-7.46	1.85	-	-5.664	1.363	2.200	-
SP11(EP,BP)	7.42	3,157.42	-7.68	1.90	-	-7.462	1.363	2.200	-
No.64	42.58	3,200.00	-10.23	1.90	-	-5.532	1.364	2.200	-
No.65	50.00	3,250.00	-10.24	1.89	-	-5.840	1.366	2.200	-
No.66	50.00	3,300.00	-8.62	2.02	-	-4.624	1.368	2.200	-
EC11	37.83	3,337.83	-4.39	2.17	1.56	-3.497	1.368	2.200	2.200
No.67	12.17	3,350.00	-3.03	2.22	1.56	-3.497	1.369	2.200	2.200
No.68	50.00	3,400.00	-2.53	2.10	1.53	-3.494	1.369	2.200	2.200
No.69	50.00	3,450.00	-2.73	2.10	1.53	-3.492	1.370	2.200	2.200
No.70	50.00	3,500.00	-2.74	2.08	1.50	-3.489	1.370	2.200	2.200
No.71	50.00	3,550.00	-2.94	2.18	1.70	-3.487	1.371	2.200	2.200
BC12	38.95	3,588.95	-2.55	2.31	1.68	-3.485	1.371	2.200	2.200
No.72	11.05	3,600.00	-2.43	2.35	1.67	-3.484	1.371	2.200	2.200

TABLE 2.1 (3/4) PROPOSED LONGITUDINAL PROFILE OF TAU HU - BEN NGHE CANAL

Station	Distance (m)	Accumu-late Distance (m)	Existing			Design			
			Bed Elevation EL1 (m)	Left Bank Elevation EL2 (m)	Right Bank Elevation EL3 (m)	Canal Bed Elevation EL4 (m)	High Water Level EL5 (m)	Left Dike Crown Elevation EL6(m)	Right Dike Crown Elevation EL7(m)
SP12	44.98	3,644.98	-2.35	2.08	1.50	-3.482	1.371	2.200	2.200
No.73	5.03	3,650.00	-2.34	2.05	1.48	-3.482	1.372	2.200	2.200
No.74	50.00	3,700.00	-2.66	2.04	1.50	-3.479	1.372	2.200	2.200
EC12	1.00	3,701.00	-2.66	2.04	1.50	-3.479	1.372	2.200	2.200
No.75	49.00	3,750.00	-2.34	2.04	1.54	-3.477	1.373	2.200	2.200
No.76	50.00	3,800.00	-3.26	2.10	1.75	-3.474	1.373	2.200	2.200
No.77	50.00	3,850.00	-2.78	2.13	1.70	-3.472	1.374	2.200	2.200
BC13	19.28	3,869.28	-2.63	2.12	1.67	-3.471	1.374	2.200	2.200
SP13	23.04	3,892.32	-2.45	2.11	1.63	-3.470	1.374	2.200	2.200
No.78	7.68	3,900.00	-2.38	2.10	1.62	-3.469	1.374	2.200	2.200
EC13	15.36	3,915.36	-2.41	2.16	1.64	-3.469	1.374	2.200	2.200
No.79	34.64	3,950.00	-2.45	2.28	1.68	-3.467	1.375	2.200	2.200
BC14	23.09	3,973.09	-2.60	2.25	1.59	-3.466	1.375	2.200	2.200
No.80	26.91	4,000.00	-2.77	2.21	1.49	-3.464	1.375	2.200	2.200
No.81	50.00	4,050.00	-2.94	2.17	1.56	-3.462	1.376	2.200	2.200
SP14	31.69	4,081.69	-2.87	2.06	1.65	-3.460	1.376	2.200	2.200
No.82	18.32	4,100.00	-2.83	2.00	1.70	-3.459	1.376	2.200	2.200
No.83	50.00	4,150.00	-2.44	2.07	1.72	-3.457	1.377	2.200	2.200
EC14	40.28	4,190.28	-2.23	2.15	1.55	-3.455	1.377	2.200	2.200
No.84	9.72	4,200.00	-2.18	2.17	1.51	-3.454	1.377	2.200	2.200
BC15	8.82	4,208.82	-2.21	2.12	1.53	-3.454	1.377	2.200	2.200
No.85	41.18	4,250.00	-2.33	1.86	1.60	-3.452	1.378	2.200	2.200
No.86	50.00	4,300.00	-2.44	1.89	1.49	-3.449	1.378	2.200	2.200
No.87	50.00	4,350.00	-2.45	1.78	1.42	-3.447	1.379	2.200	2.200
SP15	23.32	4,373.32	-2.35	1.76	1.40	-3.446	1.379	2.200	2.200
No.88	26.68	4,400.00	-2.25	1.74	1.38	-3.444	1.379	2.200	2.200
No.89	50.00	4,450.00	-2.23	1.78	1.39	-3.442	1.380	2.200	2.200
No.90	50.00	4,500.00	-2.13	1.72	1.49	-3.439	1.380	2.200	2.200
EC15	37.82	4,537.82	-2.23	1.80	1.43	-3.437	1.380	2.200	2.200
No.91	12.18	4,550.00	-2.26	1.83	1.41	-3.437	1.381	2.200	2.200
No.92	50.00	4,600.00	-2.04	1.97	1.40	-3.434	1.381	2.200	2.200
BC16	4.60	4,604.60	-2.02	1.96	1.41	-3.434	1.381	2.200	2.200
SP16	42.12	4,646.72	-1.85	1.83	1.51	-3.432	1.381	2.200	2.200
No.93	3.29	4,650.00	-1.83	1.82	1.51	-3.432	1.382	2.200	2.200
EC16	38.83	4,688.83	-1.86	1.99	1.52	-3.430	1.382	2.200	2.200
No.94	11.17	4,700.00	-1.86	2.03	1.52	-3.429	1.382	2.200	2.200
No.95	50.00	4,750.00	-1.83	2.02	1.80	-3.427	1.383	2.200	2.200
No.96	50.00	4,800.00	-1.94	2.08	1.43	-3.424	1.383	2.200	2.200
No.97	50.00	4,850.00	-1.83	1.99	1.46	-3.422	1.384	2.200	2.200
BC17	0.40	4,850.40	-1.83	1.99	1.47	-3.422	1.384	2.200	2.200
SP17	35.54	4,885.94	-1.87	2.19	1.57	-3.420	1.384	2.200	2.200
No.98	14.06	4,900.00	-1.88	2.27	1.61	-3.419	1.384	2.200	2.200
EC17	21.48	4,921.48	-1.75	2.15	1.87	-3.418	1.384	2.200	2.200
No.99	28.52	4,950.00	-1.57	2.00	2.20	-3.417	1.385	2.200	2.200
BC18	14.79	4,964.79	-1.65	2.00	2.03	-3.416	1.385	2.200	2.200
SP18	20.09	4,984.88	-1.76	2.00	1.80	-3.415	1.385	2.200	2.200
No.100	15.12	5,000.00	-1.83	2.00	1.62	-3.414	1.385	2.200	2.200
EC18	4.97	5,004.97	-1.83	2.01	1.62	-3.414	1.385	2.200	2.200
No.101	45.03	5,050.00	-1.77	2.11	1.58	-3.412	1.386	2.200	2.200
No.102	50.00	5,100.00	-1.47	2.33	1.60	-3.409	1.386	2.200	2.200
No.103	50.00	5,150.00	-1.51	2.34	1.30	-3.407	1.387	2.200	2.200

TABLE 2.1 (4/4) PROPOSED LONGITUDINAL PROFILE OF TAU HU - BEN NGHE CANAL

Station	Distance (m)	Accumu-late Distance (m)	Existing			Design			
			Bed Elevation EL1 (m)	Left Bank Elevation EL2 (m)	Right Bank Elevation EL3 (m)	Canal Bed Elevation EL4 (m)	High Water Level EL5 (m)	Left Dike Crown Elevation EL6(m)	Right Dike Crown Elevation EL7(m)
No.104	50.00	5,200.00	-1.13	2.10	1.53	-3.404	1.387	2.200	2.200
No.105	50.00	5,250.00	-1.42	2.28	1.57	-3.402	1.388	2.200	2.200
No.106	50.00	5,300.00	-1.16	2.17	1.83	-3.399	1.388	2.200	2.200
No.107	50.00	5,350.00	-1.21	2.11	1.78	-3.397	1.389	2.200	2.200
No.108	50.00	5,400.00	-1.43	1.89	1.77	-3.394	1.389	2.200	2.200
BC19	18.57	5,418.57	-1.45	1.84	1.78	-3.393	1.389	2.200	2.200
No.109	31.43	5,450.00	-1.46	1.75	1.79	-3.392	1.390	2.200	2.200
No.110	50.00	5,500.00	-1.53	1.61	1.90	-3.389	1.390	2.200	2.200
SP19	22.36	5,522.36	-1.59	1.70	1.89	-3.388	1.390	2.200	2.200
No.111	27.64	5,550.00	-1.66	1.81	1.87	-3.387	1.391	2.200	2.200
No.112	50.00	5,600.00	-1.55	1.97	1.65	-3.384	1.391	2.200	2.200
EC19	26.15	5,626.15	-1.57	1.99	1.66	-3.383	1.391	2.200	2.200
No.113	23.85	5,650.00	-1.59	2.00	1.68	-3.382	1.392	2.200	2.200
No.114	50.00	5,700.00	-1.58	2.28	1.73	-3.379	1.392	2.200	2.200
No.115	50.00	5,750.00	-1.66	2.14	1.70	-3.377	1.393	2.200	2.200
No.116	50.00	5,800.00	-1.73	2.07	1.80	-3.374	1.393	2.200	2.200
No.117	50.00	5,850.00	-1.64	2.06	1.78	-3.372	1.394	2.200	2.200
No.118	50.00	5,900.00	-1.52	2.06	1.65	-3.369	1.394	2.200	2.200
No.119	50.00	5,950.00	-1.56	2.05	1.78	-3.367	1.395	2.200	2.200
No.120	50.00	6,000.00	-1.54	2.13	1.75	-3.364	1.395	2.200	2.200
No.121	50.00	6,050.00	-1.57	2.43	2.09	-3.362	1.396	2.200	2.200
IP20	35.05	6,085.05	-1.55	2.38	2.31	-3.360	1.396	2.200	2.200
No.122	14.95	6,100.00	-1.55	2.35	2.40	-3.359	1.396	2.200	2.200
No.123	50.00	6,150.00	-1.52	3.24	2.63	-3.357	1.397	2.200	2.200
No.124	50.00	6,200.00	-1.55	2.37	2.15	-3.354	1.397	2.200	2.200
No.125	50.00	6,250.00	-1.39	1.80	1.94	-3.352	1.398	2.200	2.200
No.126	50.00	6,300.00	-1.46	1.90	1.95	-3.349	1.398	2.200	2.200
No.127	50.00	6,350.00	-1.37	1.92	1.75	-3.347	1.399	2.200	2.200
IP21	21.98	6,371.98	-1.40	1.95	1.84	-3.346	1.399	2.200	2.200
No.128	28.02	6,400.00	-1.43	1.98	1.96	-3.344	1.399	2.200	2.200
No.129	50.00	6,450.00	-1.55	2.05	1.86	-3.342	1.400	2.200	2.200
BC22	25.37	6,475.37	-1.61	2.12	1.80	-3.341	1.400	2.200	2.200
No.130	24.63	6,500.00	-1.66	2.18	1.75	-3.339	1.400	2.200	2.200
SP22	25.28	6,525.28	-1.50	2.21	1.79	-3.338	1.400	2.200	2.200
No.131	24.73	6,550.00	-1.34	2.23	1.83	-3.337	1.401	2.200	2.200
EC22	25.18	6,575.18	-1.34	2.26	1.83	-3.336	1.401	2.200	2.200
No.132	24.82	6,600.00	-1.33	2.28	1.84	-3.334	1.401	2.200	2.200
No.133	50.00	6,650.00	-1.55	2.02	1.86	-3.332	1.402	2.200	2.200
No.134	50.00	6,700.00	-1.66	1.68	1.70	-3.329	1.402	2.200	2.200
No.135	50.00	6,750.00	-1.65	1.68	1.83	-3.327	1.403	2.200	2.200
No.136	50.00	6,800.00	-1.73	1.87	1.74	-3.324	1.403	2.200	2.200
No.137	50.00	6,850.00	-1.61	1.90	1.70	-3.322	1.404	2.200	2.200
No.138	50.00	6,900.00	-1.88	1.91	1.64	-3.319	1.404	2.200	2.200
No.139	50.00	6,950.00	-1.82	1.92	1.69	-3.317	1.405	2.200	2.200
No.140	50.00	7,000.00	-1.73	1.86	1.70	-3.314	1.405	2.200	2.200
No.141	50.00	7,050.00	-1.85	1.90	1.67	-3.312	1.406	2.200	2.200
No.142	50.00	7,100.00	-1.86	1.98	1.61	-3.309	1.406	2.200	2.200
No.143	50.00	7,150.00	-1.93	1.96	1.75	-3.307	1.407	2.200	2.200
No.144	50.00	7,200.00	-2.14	1.91	1.90	-3.304	1.407	2.200	2.200
No.145	50.00	7,250.00	-2.75	1.95	3.44	-3.302	1.408	2.200	2.200
IP24 (EP)	36.72	7,286.72	-2.75	1.95	3.44	-3.300	1.408	-	-

TABLE 2.2 (17) PROPOSED CROSS SECTION OF TAU HU - BEN NGHE CANAL IMPROVEMENT

Name of Canal	Section	Length (m)	Accumulate Length (m)	Elevation (m above MSL)										Canal Width										Height					
				Left					Right					Channel					Right					L-Bank		H-Chan.		Total	
				Bank	Crown	L-Bed	C-Bed	R-Bed	Crown	Bank	Right	Left	Road	Left	Bottom	Right	Road	Total	G-total	L-Bank	H-Chan.	L-Chan.	Total	R-Bank					
				EL.1 (m)	EL.2 (m)	EL.3 (m)	EL.4 (m)	EL.5 (m)	EL.6 (m)	EL.7 (m)		B1 (m)	B2 (m)	B3 (m)	B4 (m)	B5 (m)	B6 (m)	B7 (m)	B8 (m)	B9 (m)	H1 (m)	H2 (m)	H3 (m)	H4 (m)	H5 (m)				
No.0 (BP)		0.00	0.00																										
No.1		50.00	50.00	[2.10]	[2.15]	-0.300	-3.448	0.310	[1.92]	[1.98]																			
No.2		50.00	100.00	[2.20]	[2.31]	-0.970	-3.445	-1.470	[2.03]	[2.03]																			
No.3		50.00	150.00	[2.20]	[2.04]	-1.040	-3.443	-0.840	[2.04]	[1.96]																			
BC1		19.47	169.47	-	2.20	-2.000	-3.442	-2.000	2.200	-																			
SP1		20.33	189.80	-	2.20	-2.000	-3.441	-2.000	2.200	-																			
No.4		10.20	200.00	2.020	2.200	-2.000	-3.440	-2.000	2.200	2.540																			
EC1		10.13	210.13	-	2.200	-2.000	-3.439	-2.000	2.200	-																			
No.5		39.87	250.00	2.040	2.200	-2.000	-3.438	-2.000	2.200	1.990																			
No.6		50.00	300.00	2.040	2.200	-2.000	-3.435	-2.000	2.200	1.980																			
No.7		50.00	350.00	1.980	2.200	-2.000	-3.433	-2.000	2.200	2.090																			
No.8		50.00	400.00	2.100	2.200	-2.000	-3.430	-2.000	2.200	2.030																			
BC2		39.47	439.47	-	2.200	-2.000	-3.428	-2.000	2.200	-																			
No.9		10.53	450.00	2.060	2.200	-2.000	-3.428	-2.000	2.200	1.980																			
SP2		49.78	499.78	-	2.200	-2.000	-3.425	-2.000	2.200	-																			
No.10		0.23	500.00	2.000	2.200	-2.000	-3.425	-2.000	2.200	2.000																			
No.11		50.00	550.00	2.080	2.200	-2.000	-3.423	-2.000	2.200	2.010																			
EC2		10.08	560.08	-	2.200	-2.000	-3.422	-2.000	2.200	-																			
No.12		39.92	600.00	2.130	2.200	-2.000	-3.420	-2.000	2.200	2.040																			
No.13		50.00	650.00	1.120	2.200	-2.000	-3.418	-2.000	2.200	1.990																			
No.14		50.00	700.00	2.080	2.200	-2.000	-3.415	-2.000	2.200	1.940																			
No.15		50.00	750.00	1.840	2.200	-2.000	-3.413	-2.000	2.200	1.920																			
BC3		10.73	760.73	-	2.200	-2.000	-3.412	-2.000	2.200	-																			
No.16		39.27	800.00	1.910	2.200	-2.000	-3.410	-2.000	2.200	1.860																			
SP3		4.44	804.44	-	2.200	-2.000	-3.410	-2.000	2.200	-																			
EC3		43.71	848.14	-	2.200	-2.000	-3.408	-2.000	2.200	-																			
No.17		1.86	850.00	2.040	2.200	-2.000	-3.408	-2.000	2.200	2.420																			
No.18		50.00	900.00	2.080	2.200	-2.000	-3.405	-2.000	2.200	2.380																			
No.19		50.00	950.00	2.040	2.200	-2.000	-3.403	-2.000	2.200	1.960																			
BC4		6.69	956.69	-	2.200	-2.000	-3.402	-2.000	2.200	-																			
SP4		12.90	969.59	-	2.200	-2.000	-3.402	-2.000	2.200	-																			

TABLE 2.2 (2/7) PROPOSED CROSS SECTION OF TAU HU - BEN NGHE CANAL IMPROVEMENT

Name of Canal	Section	Length (m)	Accumulate Length (m)	Elevation (m above MSL)										Canal Width										Height					
				Left					Right					Channel					Right					L-Bank		H-Chan.		R-Bank	
				Bank EL.1 (m)	Crown EL.2 (m)	L-Bed EL.3 (m)	C-Bed EL.4 (m)	R-Bed EL.5 (m)	Crown EL.6 (m)	Bank EL.7 (m)	Left Road B1 (m)	Left B2 (m)	Left B3 (m)	Bottom B4 (m)	Right B5 (m)	Right B6 (m)	Road B7 (m)	Total B8 (m)	G-total B9 (m)	L-Bank HI (m)	H-Chan. H2 (m)	L-Chan. H3 (m)	Total H4 (m)	R-Bank H5 (m)					
	EC4	12.90	982.49	-	2.200	-2.000	-3.401	-2.000	-2.000	2.200	-	5.00	6.30	14.01	21.00	14.01	6.30	5.00	61.62	71.62	-	4.20	1.40	5.60	-				
	No.20	17.51	1,000.00	1.940	2.200	-2.000	-3.400	-2.000	-2.000	2.200	1.900	5.00	6.30	14.00	21.00	14.00	6.30	5.00	61.60	71.60	0.26	4.20	1.40	5.60	0.30				
	No.21	50.00	1,050.00	2.040	2.200	-2.000	-3.398	-2.000	-2.000	2.200	1.940	5.00	6.30	13.98	21.00	13.98	6.30	5.00	61.55	71.55	0.16	4.20	1.40	5.60	0.26				
	No.22	50.00	1,100.00	2.060	2.200	-2.000	-3.395	-2.000	-2.000	2.200	1.780	5.00	6.30	13.95	21.00	13.95	6.30	5.00	61.50	71.50	0.14	4.20	1.40	5.60	0.42				
	BC5	13.28	1,113.28	-	2.200	-2.000	-3.394	-2.000	-2.000	2.200	-	5.00	6.30	13.94	21.00	13.94	6.30	5.00	61.49	71.49	-	4.20	1.39	5.59	-				
	No.23	36.72	1,150.00	2.060	2.200	-2.000	-3.393	-2.000	-2.000	2.200	1.760	5.00	6.30	13.93	21.00	13.93	6.30	5.00	61.45	71.45	0.14	4.20	1.39	5.59	0.44				
	SP5	18.23	1,168.23	-	2.200	-2.000	-3.392	-2.000	-2.000	2.200	-	5.00	6.30	13.92	21.00	13.92	6.30	5.00	61.43	71.43	-	4.20	1.39	5.59	-				
	No.24	31.77	1,200.00	1.780	2.200	-2.000	-3.390	-2.000	-2.000	2.200	1.820	5.00	6.30	13.90	21.00	13.90	6.30	5.00	61.40	71.40	0.42	4.20	1.39	5.59	0.38				
	EC5	23.18	1,223.18	-	2.200	-2.000	-3.389	-2.000	-2.000	2.200	-	5.00	6.30	13.89	21.00	13.89	6.30	5.00	61.38	71.38	-	4.20	1.39	5.59	-				
	No.25	26.82	1,250.00	1.840	2.200	-2.000	-3.388	-2.000	-2.000	2.200	1.820	5.00	6.30	13.88	21.00	13.88	6.30	5.00	61.35	71.35	0.36	4.20	1.39	5.59	0.38				
	No.26	50.00	1,300.00	1.840	2.200	-2.000	-3.385	-2.000	-2.000	2.200	1.730	5.00	6.30	13.85	21.00	13.85	6.30	5.00	61.30	71.30	0.36	4.20	1.39	5.59	0.47				
	No.27	50.00	1,350.00	1.820	2.200	-2.000	-3.383	-2.000	-2.000	2.200	1.800	5.00	6.30	13.83	21.00	13.83	6.30	5.00	61.25	71.25	0.38	4.20	1.38	5.58	0.40				
	IP6	47.66	1,397.66	-	2.200	-2.000	-3.380	-2.000	-2.000	2.200	-	5.00	6.30	13.80	21.00	13.80	6.30	5.00	61.20	71.20	-	4.20	1.38	5.58	-				
	No.28	2.34	1,400.00	1.760	2.200	-2.000	-3.380	-2.000	-2.000	2.200	1.920	5.00	6.30	13.80	21.00	13.80	6.30	5.00	61.20	71.20	0.44	4.20	1.38	5.58	0.28				
	No.29	50.00	1,450.00	1.760	2.200	-2.000	-3.378	-2.000	-2.000	2.200	1.960	5.00	6.30	13.78	21.00	13.78	6.30	5.00	61.15	71.15	0.44	4.20	1.38	5.58	0.24				
	No.30	50.00	1,500.00	1.630	2.200	-2.000	-3.375	-2.000	-2.000	2.200	2.060	5.00	6.30	13.75	21.00	13.75	6.30	5.00	61.10	71.10	0.57	4.20	1.38	5.58	0.14				
	No.31	50.00	1,550.00	1.640	2.200	-2.000	-3.373	-2.000	-2.000	2.200	1.990	5.00	6.30	13.73	21.00	13.73	6.30	5.00	61.05	71.05	0.56	4.20	1.37	5.57	0.21				
	No.32	50.00	1,600.00	1.750	2.200	-2.000	-3.370	-2.000	-2.000	2.200	2.030	5.00	6.30	13.70	21.00	13.70	6.30	5.00	61.00	71.00	0.45	4.20	1.37	5.57	0.17				
	BC7	1.07	1,601.07	-	2.200	-2.000	-3.370	-2.000	-2.000	2.200	-	5.00	6.30	13.70	21.00	13.70	6.30	5.00	61.00	71.00	-	4.20	1.37	5.57	-				
	No.33	48.93	1,650.00	1.790	2.200	-2.000	-3.368	-2.000	-2.000	2.200	1.950	5.00	6.30	13.68	21.00	13.68	6.30	5.00	60.95	70.95	0.41	4.20	1.37	5.57	0.25				
	SP7	8.43	1,658.43	-	2.200	-2.000	-3.367	-2.000	-2.000	2.200	-	5.00	6.30	13.67	21.00	13.67	6.30	5.00	60.94	70.94	-	4.20	1.37	5.57	-				
	No.34	41.57	1,700.00	1.930	2.200	-2.000	-3.365	-2.000	-2.000	2.200	2.010	5.00	6.30	13.65	21.00	13.65	6.30	5.00	60.90	70.90	0.27	4.20	1.37	5.57	0.19				
	EC7	15.79	1,715.79	-	2.200	-2.000	-3.364	-2.000	-2.000	2.200	-	5.00	6.30	13.64	21.00	13.64	6.30	5.00	60.88	70.88	-	4.20	1.36	5.56	-				
	No.35	34.21	1,750.00	2.130	2.200	-2.000	-3.363	-2.000	-2.000	2.200	2.340	5.00	6.30	13.63	21.00	13.63	6.30	5.00	60.85	70.85	0.07	4.20	1.36	5.56	-0.14				
	No.36	50.00	1,800.00	2.280	2.200	-2.000	-3.360	-2.000	-2.000	2.200	2.070	5.00	6.30	13.60	21.00	13.60	6.30	5.00	60.80	70.80	-0.08	4.20	1.36	5.56	0.13				
	No.37	50.00	1,850.00	2.400	2.200	-2.000	-3.358	-2.000	-2.000	2.200	1.890	5.00	6.30	13.58	21.00	13.58	6.30	5.00	60.75	70.75	-0.20	4.20	1.36	5.56	0.31				
	No.38	50.00	1,900.00	2.340	2.200	-2.000	-3.355	-2.000	-2.000	2.200	1.930	5.00	6.30	13.55	21.00	13.55	6.30	5.00	60.70	70.70	-0.14	4.20	1.36	5.56	0.27				
	No.39	50.00	1,950.00	2.010	2.200	-2.000	-3.353	-2.000	-2.000	2.200	1.770	5.00	6.30	13.53	21.00	13.53	6.30	5.00	60.65	70.65	0.19	4.20	1.35	5.55	0.43				
	BC8	24.33	1,974.33	-	2.200	-2.000	-3.351	-2.000	-2.000	2.200	-	5.00	6.30	13.51	21.00	13.51	6.30	5.00	60.63	70.63	-	4.20	1.35	5.55	-				
	No.40	25.67	2,000.00	2.070	2.200	-2.000	-3.350	-2.000	-2.000	2.200	1.780	5.00	6.30	13.50	21.00	13.50	6.30	5.00	60.60	70.60	0.13	4.20	1.35	5.55	0.42				
	No.41	50.00	2,050.00	1.980	2.200	-2.000	-3.348	-2.000	-2.000	2.200	1.780	5.00	6.30	13.48	21.00	13.48	6.30	5.00	60.55	70.55	-	4.20	1.35	5.55	-				
	SP7	46.09	2,096.09	-	2.200	-2.000	-3.345	-2.000	-2.000	2.200	-	5.00	6.30	13.45	21.00	13.45	6.30	5.00	60.50	70.50	-	4.20	1.35	5.55	-				

TABLE 2.2 (3/7) PROPOSED CROSS SECTION OF TAU HU - BEN NGHE CANAL IMPROVEMENT

Name of Canal	Section	Length (m)	Accumulate Length (m)	Elevation (m above MSL)										Canal Width										Height									
				Left					Right					Channel					Right					L-Channel					R-Channel				
				Bank EL.1 (m)	Crown EL.2 (m)	L-Bed EL.3 (m)	C-Bed EL.4 (m)	R-Bed EL.5 (m)	Crown EL.6 (m)	Bank EL.7 (m)	Left Road B1 (m)	Left B2 (m)	Left B3 (m)	Left B4 (m)	Bottom B5 (m)	Right B6 (m)	Road B7 (m)	Total B8 (m)	G-total B9 (m)	L-Bank HI (m)	H-Chan. H2 (m)	L-Chan. H3 (m)	Total H4 (m)	R-Bank H5 (m)									
	No.42	3.92	2,100.00	2.130	2.200	-2.000	-3.345	-2.000	2.200	1.960	5.00	6.30	13.45	21.00	13.45	6.30	5.00	60.50	70.50	0.07	4.20	1.35	5.55	0.24									
	No.43	50.00	2,150.00	2.270	2.200	-2.000	-3.343	-2.000	2.200	2.150	5.00	6.30	13.43	21.00	13.43	6.30	5.00	60.45	70.45	-	4.20	1.34	5.54	-									
	No.44	50.00	2,200.00	2.160	2.200	-2.000	-3.340	-2.000	2.200	2.000	5.00	6.30	13.40	21.00	13.40	6.30	5.00	60.40	70.40	0.04	4.20	1.34	5.54	0.20									
	EC8	17.84	2,217.84	-	2.200	-2.000	-3.339	-2.000	2.200	-	5.00	6.30	13.39	21.00	13.39	6.30	5.00	60.38	70.38	-	4.20	1.34	5.54	-									
	No.45	32.16	2,250.00	2.250	2.200	-2.000	-3.338	-2.000	2.200	1.950	5.00	6.30	13.38	21.00	13.38	6.30	5.00	60.35	70.35	-0.05	4.20	1.34	5.54	0.25									
	No.46	50.00	2,300.00	2.040	2.200	-2.000	-3.335	-2.000	2.200	1.970	5.00	6.30	13.35	21.00	13.35	6.30	5.00	60.30	70.30	0.16	4.20	1.34	5.54	0.23									
	No.47	50.00	2,350.00	2.100	2.200	-2.000	-3.333	-2.000	2.200	1.890	5.00	6.30	13.33	21.00	13.33	6.30	5.00	60.25	70.25	0.10	4.20	1.33	5.53	0.31									
	No.48	50.00	2,400.00	2.060	2.200	-2.000	-3.330	-2.000	2.200	1.950	5.00	6.30	13.30	21.00	13.30	6.30	5.00	60.20	70.20	0.14	4.20	1.33	5.53	0.25									
	No.49	50.00	2,450.00	1.920	2.200	-2.000	-3.328	-2.000	2.200	2.020	5.00	6.30	13.28	21.00	13.28	6.30	5.00	60.15	70.15	0.28	4.20	1.33	5.53	0.18									
	BC9	8.00	2,458.00	-	2.200	-2.000	-3.327	-2.000	2.200	-	5.00	6.30	13.27	21.00	13.27	6.30	5.00	60.14	70.14	-	4.20	1.33	5.53	-									
	No.50	42.00	2,500.00	1.980	2.200	-2.000	-3.325	-2.000	2.200	1.920	5.00	6.30	13.25	21.00	13.25	6.30	5.00	60.10	70.10	0.22	4.20	1.33	5.53	0.28									
	SP9	1.94	2,501.94	-	2.200	-2.000	-3.325	-2.000	2.200	-	5.00	6.30	13.25	21.00	13.25	6.30	5.00	60.10	70.10	-	4.20	1.32	5.52	-									
	EC9	43.94	2,545.87	-	2.200	-2.000	-3.323	-2.000	2.200	-	5.00	6.30	13.23	21.00	13.23	6.30	5.00	60.05	70.05	-	4.20	1.32	5.52	-									
	No.51	4.13	2,550.00	1.840	2.200	-2.000	-3.323	-2.000	2.200	1.820	5.00	6.30	13.23	21.00	13.23	6.30	5.00	60.05	70.05	0.36	4.20	1.32	5.52	0.38									
	No.52	50.00	2,600.00	1.830	2.200	-2.000	-3.320	-2.000	2.200	1.930	5.00	6.30	13.20	21.00	13.20	6.30	5.00	60.00	70.00	0.37	4.20	1.32	5.52	0.27									
	No.53	50.00	2,650.00	1.830	2.200	-2.000	-3.318	-2.000	2.200	-	5.00	6.30	13.18	21.00	13.18	-	-	-	-	0.37	4.20	1.32	5.52	-									
	No.54	50.00	2,700.00	1.900	2.200	-2.000	-3.315	-2.000	2.200	1.600	5.00	6.30	13.15	21.00	13.15	6.30	5.00	59.90	69.90	0.30	4.20	1.32	5.52	0.60									
	No.55	50.00	2,750.00	1.950	2.200	-2.000	-3.313	-2.000	2.200	1.580	5.00	6.30	13.13	21.00	13.13	6.30	5.00	59.85	69.85	0.25	4.20	1.31	5.51	0.62									
	No.56	50.00	2,800.00	2.150	2.200	-2.000	-3.310	-2.000	2.200	1.620	5.00	6.30	13.10	21.00	13.10	6.30	5.00	59.80	69.80	0.05	4.20	1.31	5.51	0.58									
	No.57	50.00	2,850.00	2.390	2.200	-2.000	-3.308	-2.000	2.200	1.910	5.00	6.30	13.08	21.00	13.08	6.30	5.00	59.75	69.75	-0.19	4.20	1.31	5.51	0.29									
	BC10	26.36	2,876.36	-	2.200	-2.000	-3.306	-2.000	2.200	-	5.00	6.30	13.06	21.00	13.06	6.30	5.00	59.72	69.72	-	4.20	1.31	5.51	-									
	No.58	23.64	2,900.00	3.240	2.200	-2.000	-3.305	-2.000	2.200	1.530	5.00	6.30	13.05	21.00	13.05	6.30	5.00	59.70	69.70	-1.04	4.20	1.31	5.51	0.67									
	SP10	21.77	2,921.77	-	2.200	-2.000	-3.304	-2.000	2.200	-	5.00	6.30	13.04	21.00	13.04	6.30	5.00	59.68	69.68	-	4.20	1.30	5.50	-									
	No.59	28.24	2,950.00	2.340	2.200	-2.000	-3.303	-2.000	2.200	1.450	5.00	6.30	13.03	21.00	13.03	6.30	5.00	59.65	69.65	-0.14	4.20	1.30	5.50	0.75									
	EC10	17.17	2,967.17	-	2.200	-2.000	-3.302	-	-	-	5.00	6.30	13.02	21.00	-	-	-	-	-	-	-	4.20	1.30	5.50	-								
	BC11	9.84	2,977.01	-	2.200	-2.000	-3.301	-	-	-	5.00	6.30	13.01	21.00	-	-	-	-	-	-	-	4.20	1.30	5.50	-								
	No.60	22.99	3,000.00	2.040	2.200	-2.000	-3.300	-	-	-	5.00	6.30	13.00	21.00	-	-	-	-	-	0.16	4.20	1.30	5.50	-									
	No.61	50.00	3,050.00	1.930	2.200	-2.000	-3.564	-	-	-	5.00	6.30	12.29	10.50	-	-	-	29.09	34.09	0.27	4.20	1.56	5.76	-									
	No.62	50.00	3,100.00	1.880	2.200	-2.000	-5.042	-	-	-	5.00	6.30	11.59	10.50	-	-	-	28.39	33.39	0.32	4.20	3.04	7.24	-									
	No.63	50.00	3,150.00	1.850	2.200	-2.000	-5.664	-	-	-	5.00	6.30	10.88	10.50	-	-	-	27.68	32.68	0.35	4.20	3.66	7.86	-									
	SP1(EP,BP)	7.42	3,157.42	1.900	2.200	-2.000	-7.462	-	-	-	5.00	6.30	10.78	10.50	-	-	-	27.58	32.58	0.30	4.20	5.46	9.66	-									

TABLE 2.2 (4/7) PROPOSED CROSS SECTION OF TAU HU - BEN NGHE CANAL IMPROVEMENT

Name of Canal	Section	Length (m)	Accumulate Length (m)	Elevation (m above MSL)						Canal Width										Height								
				Left			Channel			Right		Left Road	B1 (m)	B2 (m)	B3 (m)	B4 (m)	B5 (m)	B6 (m)	B7 (m)	B8 (m)	B9 (m)	G-total (m)	L-Bank (m)	H-Chan. (m)	H-Chan. (m)	L-Chan. (m)	Total (m)	R-Bank (m)
				Bank EL.1 (m)	Crown EL.2 (m)	L-Bed EL.3 (m)	C-Bed EL.4 (m)	R-Bed EL.5 (m)	Crown EL.6 (m)	Bank EL.7 (m)	Channel																	
Tau Hu (Downstream)	P11 (EP, B)	0.00	3,157.42	1.900	2.200	-2.000	-7.946	-	-	5.00	6.30	10.78	10.50	-	-	-	-	-	32.58	0.30	4.00	4.00	1.51	5.51	-	-		
	No.64	42.58	3,200.00	1.900	2.200	-2.000	-5.532	-	-	5.00	6.30	10.18	10.50	-	-	-	-	-	31.98	0.30	4.00	4.00	1.51	5.51	-	-		
	No.65	50.00	3,250.00	1.890	2.200	-2.000	-5.840	-	-	5.00	6.30	9.47	10.50	-	-	-	-	-	31.27	0.31	4.20	4.20	3.84	8.04	-	-		
	No.66	50.00	3,300.00	2.020	2.200	-2.000	-4.624	-	-	5.00	6.30	8.77	10.50	-	-	-	-	-	30.57	0.18	4.20	4.20	2.62	6.82	-	-		
	EC11	37.83	3,337.83	-	2.200	-2.000	-3.497	-2.000	2.200	5.00	6.30	8.24	21.00	8.24	6.30	5.00	50.07	60.07	-	60.07	-	4.20	4.20	1.50	5.70	-	-	
	No.67	12.17	3,350.00	1.820	2.200	-2.000	-3.497	-2.000	2.200	5.00	6.30	8.23	21.00	8.23	6.30	5.00	50.07	60.07	0.38	4.20	4.20	1.50	5.70	0.40	-	-		
	No.68	50.00	3,400.00	2.100	2.200	-2.000	-3.494	-2.000	2.200	5.00	6.30	8.22	21.00	8.22	6.30	5.00	50.04	60.04	0.10	4.20	4.20	1.49	5.69	0.67	-	-		
	No.69	50.00	3,450.00	2.100	2.200	-2.000	-3.492	-2.000	2.200	5.00	6.30	8.21	21.00	8.21	6.30	5.00	50.01	60.01	0.10	4.20	4.20	1.49	5.69	0.68	-	-		
	No.70	50.00	3,500.00	2.084	2.200	-2.000	-3.489	-2.000	2.200	5.00	6.30	8.19	21.00	8.19	6.30	5.00	49.98	59.98	0.12	4.20	4.20	1.49	5.69	0.70	-	-		
	No.71	50.00	3,550.00	2.180	2.200	-2.000	-3.487	-2.000	2.200	5.00	6.30	8.18	21.00	8.18	6.30	5.00	49.96	59.96	0.02	4.20	4.20	1.49	5.69	0.50	-	-		
	BC12	38.95	3,588.95	-	2.200	-2.000	-3.485	-2.000	2.200	-	5.00	6.30	8.17	21.00	8.17	6.30	5.00	49.93	59.93	-	4.20	4.20	1.48	5.68	-	-		
	No.72	11.05	3,600.00	2.347	2.200	-2.000	-3.484	-2.000	2.200	5.00	6.30	8.16	21.00	8.16	6.30	5.00	49.93	59.93	-0.15	4.20	4.20	1.48	5.68	0.53	-	-		
	SP12	44.98	3,644.98	-	2.200	-2.000	-3.482	-2.000	2.200	-	5.00	6.30	8.15	21.00	8.15	6.30	5.00	49.90	59.90	-	4.20	4.20	1.48	5.68	-	-		
	No.73	5.03	3,650.00	2.050	2.200	-2.000	-3.482	-2.000	2.200	5.00	6.30	8.15	21.00	8.15	6.30	5.00	49.90	59.90	0.15	4.20	4.20	1.48	5.68	0.72	-	-		
	No.74	50.00	3,700.00	2.040	2.200	-2.000	-3.479	-2.000	2.200	5.00	6.30	8.14	21.00	8.14	6.30	5.00	49.87	59.87	0.16	4.20	4.20	1.48	5.68	0.70	-	-		
EC12	1.00	3,701.00	-	2.200	-2.000	-3.479	-2.000	2.200	-	5.00	6.30	8.14	21.00	8.14	6.30	5.00	49.87	59.87	-	4.20	4.20	1.48	5.68	-	-			
No.75	49.00	3,750.00	2.040	2.200	-2.000	-3.477	-2.000	2.200	5.00	6.30	8.12	21.00	8.12	6.30	5.00	49.85	59.85	-	-	-	-	-	-	-	-			
No.76	50.00	3,800.00	2.100	2.200	-2.000	-3.474	-2.000	2.200	5.00	6.30	8.11	21.00	8.11	6.30	5.00	49.82	59.82	0.10	4.20	4.20	1.47	5.67	0.45	-	-			
No.77	50.00	3,850.00	2.125	2.200	-2.000	-3.472	-2.000	2.200	5.00	6.30	8.10	21.00	8.10	6.30	5.00	49.79	59.79	0.08	4.20	4.20	1.47	5.67	0.50	-	-			
BC13	19.28	3,869.28	-	2.200	-2.000	-3.471	-2.000	2.200	-	5.00	6.30	8.09	21.00	8.09	6.30	5.00	49.78	59.78	-	4.20	4.20	1.47	5.67	-	-			
SP13	23.04	3,892.32	-	2.200	-2.000	-3.470	-2.000	2.200	-	5.00	6.30	8.08	21.00	8.08	6.30	5.00	49.77	59.77	-	4.20	4.20	1.47	5.67	-	-			
No.78	7.68	3,900.00	2.104	2.200	-2.000	-3.469	-2.000	2.200	5.00	6.30	8.08	21.00	8.08	6.30	5.00	49.76	59.76	0.10	4.20	4.20	1.47	5.67	0.58	-	-			
EC13	15.36	3,915.36	-	2.200	-2.000	-3.469	-2.000	2.200	-	5.00	6.30	8.08	21.00	8.08	6.30	5.00	49.75	59.75	-	4.20	4.20	1.47	5.67	-	-			
No.79	34.64	3,950.00	2.280	2.200	-2.000	-3.467	-2.000	2.200	5.00	6.30	8.07	21.00	8.07	6.30	5.00	49.74	59.74	-0.08	4.20	4.20	1.47	5.67	0.52	-	-			
BC14	23.09	3,973.09	-	2.200	-2.000	-3.466	-2.000	2.200	-	5.00	6.30	8.06	21.00	8.06	6.30	5.00	49.72	59.72	-	4.20	4.20	1.47	5.67	-	-			
No.80	26.91	4,000.00	2.210	2.200	-2.000	-3.464	-2.000	2.200	5.00	6.30	8.05	21.00	8.05	6.30	5.00	49.71	59.71	-0.01	4.20	4.20	1.46	5.66	0.71	-	-			
No.81	50.00	4,050.00	2.170	2.200	-2.000	-3.462	-2.000	2.200	5.00	6.30	8.04	21.00	8.04	6.30	5.00	49.68	59.68	0.03	4.20	4.20	1.46	5.66	0.64	-	-			
SP14	31.69	4,081.69	-	2.200	-2.000	-3.460	-2.000	2.200	-	5.00	6.30	8.03	21.00	8.03	6.30	5.00	49.66	59.66	-	4.20	4.20	1.46	5.66	-	-			
No.82	18.32	4,100.00	2.000	2.200	-2.000	-3.459	-2.000	2.200	5.00	6.30	8.03	21.00	8.03	6.30	5.00	49.65	59.65	0.20	4.20	4.20	1.46	5.66	0.50	-	-			
No.83	50.00	4,150.00	2.074	2.200	-2.000	-3.457	-2.000	2.200	5.00	6.30	8.01	21.00	8.01	6.30	5.00	49.63	59.63	0.13	4.20	4.20	1.46	5.66	0.48	-	-			
EC14	40.28	4,190.28	-	2.200	-2.000	-3.455	-2.000	2.200	-	5.00	6.30	8.00	21.00	8.00	6.30	5.00	49.60	59.60	-	4.20	4.20	1.45	5.65	-	-			
No.84	9.72	4,200.00	2.174	2.200	-2.000	-3.454	-2.000	2.200	5.00	6.30	8.00	21.00	8.00	6.30	5.00	49.60	59.60	0.03	4.20	4.20	1.45	5.65	0.69	-	-			
BC15	8.82	4,208.82	-	2.200	-2.000	-3.454	-2.000	2.200	-	5.00	6.30	8.00	21.00	8.00	6.30	5.00	49.59	59.59	-	4.20	4.20	1.45	5.65	-	-			

TABLE 2.2 (5/7) PROPOSED CROSS SECTION OF TAU HU - BEN NGHE CANAL IMPROVEMENT

Name of Canal	Section	Length (m)	Accumulate Length (m)	Elevation (m above MSL)										Canal Width										Height																								
				Left					Channel					Right					Left					Channel					Right					L-Bank					H-Chan.					Total				
				Bank EL.1 (m)	Crown EL.2 (m)	L-Bed EL.3 (m)	C-Bed EL.4 (m)	R-Bed EL.5 (m)	Crown EL.6 (m)	Bank EL.7 (m)	Left Road B1 (m)	B2 (m)	B3 (m)	B4 (m)	Bottom B5 (m)	Right B6 (m)	Road B7 (m)	Total B8 (m)	G-total B9 (m)	L-Bank HI (m)	H-Chan. H2 (m)	L-Chan. H3 (m)	Total H4 (m)	R-Bank H5 (m)																								
	No.85	41.18	4,250.00	1.860	2.200	-2.000	-3.452	-2.000	2.200	1.600	5.00	6.30	7.99	21.00	7.99	6.30	5.00	49.57	59.57	0.34	4.20	1.45	5.65	0.60																								
	No.86	50.00	4,300.00	1.890	2.200	-2.000	-3.449	-2.000	2.200	1.490	5.00	6.30	7.97	21.00	7.97	6.30	5.00	49.54	59.54	0.31	4.20	1.45	5.65	0.71																								
	No.87	50.00	4,350.00	1.784	2.200	-2.000	-3.447	-2.000	2.200	1.420	5.00	6.30	7.96	21.00	7.96	6.30	5.00	49.52	59.52	0.42	4.20	1.45	5.65	0.78																								
	SP15	23.32	4,373.32	-	2.200	-2.000	-3.446	-2.000	2.200	-	5.00	6.30	7.95	21.00	7.95	6.30	5.00	49.50	59.50	-	4.20	1.45	5.65	-																								
	No.88	26.68	4,400.00	1.742	2.200	-2.000	-3.444	-2.000	2.200	1.384	5.00	6.30	7.94	21.00	7.94	6.30	5.00	49.49	59.49	0.46	4.20	1.44	5.64	0.82																								
	No.89	50.00	4,450.00	1.780	2.200	-2.000	-3.442	-2.000	2.200	1.389	5.00	6.30	7.93	21.00	7.93	6.30	5.00	49.46	59.46	0.42	4.20	1.44	5.64	0.81																								
	No.90	50.00	4,500.00	1.720	2.200	-2.000	-3.439	-2.000	2.200	1.490	5.00	6.30	7.92	21.00	7.92	6.30	5.00	49.43	59.43	0.48	4.20	1.44	5.64	0.71																								
	EC15	37.82	4,537.82	-	2.200	-2.000	-3.437	-2.000	2.200	-	5.00	6.30	7.91	21.00	7.91	6.30	5.00	49.41	59.41	-	4.20	1.44	5.64	-																								
	No.91	12.18	4,550.00	1.830	2.200	-2.000	-3.437	-2.000	2.200	1.410	5.00	6.30	7.90	21.00	7.90	6.30	3.80	49.41	58.21	0.37	4.20	1.44	5.64	0.79																								
	No.92	50.00	4,600.00	1.970	2.200	-2.000	-3.434	-2.000	2.200	1.404	5.00	6.30	7.89	21.00	7.89	6.30	4.31	49.38	58.69	0.23	4.20	1.43	5.63	0.80																								
	BC16	4.60	4,604.60	-	2.200	-2.000	-3.434	-2.000	2.200	-	5.00	6.30	7.89	21.00	7.89	6.30	4.31	49.38	58.69	-	4.20	1.43	5.63	-																								
	SP16	42.12	4,646.72	-	2.200	-2.000	-3.432	-2.000	2.200	-	5.00	6.30	7.88	21.00	7.88	6.30	5.00	49.35	59.35	-	4.20	1.43	5.63	-																								
	No.93	3.29	4,650.00	1.820	2.200	-2.000	-3.432	-2.000	2.200	1.514	5.00	6.30	7.88	21.00	7.88	6.30	5.00	49.35	59.35	0.38	4.20	1.43	5.63	0.69																								
	EC16	38.83	4,688.83	-	2.200	-2.000	-3.430	-2.000	2.200	-	5.00	6.30	7.86	21.00	7.86	6.30	5.00	49.33	59.33	-	4.20	1.43	5.63	-																								
	No.94	11.17	4,700.00	2.034	2.200	-2.000	-3.429	-2.000	2.200	1.520	5.00	6.30	7.86	21.00	7.86	6.30	5.00	49.32	59.32	0.17	4.20	1.43	5.63	0.68																								
	No.95	50.00	4,750.00	2.024	2.200	-2.000	-3.427	-2.000	2.200	1.800	5.00	6.30	7.85	21.00	7.85	6.30	5.00	49.30	59.30	0.18	4.20	1.43	5.63	0.40																								
	No.96	50.00	4,800.00	2.084	2.200	-2.000	-3.424	-2.000	2.200	1.425	5.00	6.30	11.03	21.00	11.03	6.30	5.00	55.67	65.67	0.12	4.20	1.42	5.62	0.78																								
	No.97	50.00	4,850.00	1.992	2.200	-2.000	-3.422	-2.000	2.200	1.464	5.00	6.30	14.22	21.00	14.22	6.30	5.00	62.04	72.04	0.21	4.20	1.42	5.62	0.74																								
	BC17	0.40	4,850.40	-	2.200	-2.000	-3.422	-2.000	2.200	-	5.00	6.30	14.22	21.00	14.22	6.30	4.01	62.04	71.05	-	4.20	1.42	5.62	-																								
	SP17	35.54	4,885.94	-	2.200	-2.000	-3.420	-2.000	2.200	-	5.00	6.30	14.20	21.00	14.20	6.30	4.01	62.00	71.01	-	4.20	1.42	5.62	-																								
	No.98	14.06	4,900.00	2.270	2.200	-2.000	-3.419	-2.000	2.200	1.610	5.00	6.30	14.19	21.00	14.19	6.30	4.01	61.99	71.00	-0.07	4.20	1.42	5.62	0.59																								
	EC17	21.48	4,921.48	-	2.200	-2.000	-3.418	-2.000	2.200	-	5.00	6.30	14.18	21.00	14.18	6.30	5.00	61.97	71.97	-	4.20	1.42	5.62	-																								
	No.99	28.52	4,950.00	1.998	2.200	-2.000	-3.417	-2.000	2.200	2.204	5.00	6.30	14.17	21.00	14.17	6.30	5.00	61.94	71.94	0.20	4.20	1.42	5.62	0.00																								
	BC18	14.79	4,964.79	-	2.200	-2.000	-3.416	-2.000	2.200	-	5.00	6.30	14.16	21.00	14.16	6.30	5.00	61.92	71.92	-	4.20	1.42	5.62	-																								
	SP18	20.09	4,984.88	-	2.200	-2.000	-3.415	-2.000	2.200	-	5.00	6.30	14.15	21.00	14.15	6.30	5.00	61.90	71.90	-	4.20	1.42	5.62	-																								
	No.100	15.12	5,000.00	2.004	2.200	-2.000	-3.414	-2.000	2.200	1.624	5.00	6.30	14.14	21.00	14.14	6.30	5.00	61.89	71.89	0.20	4.20	1.41	5.61	0.58																								
	EC18	4.97	5,004.97	-	2.200	-2.000	-3.414	-2.000	2.200	-	5.00	6.30	14.14	21.00	14.14	6.30	5.00	61.88	71.88	-	4.20	1.41	5.61	-																								
	No.101	45.03	5,050.00	2.110	2.200	-2.000	-3.412	-2.000	2.200	1.580	5.00	6.30	14.12	21.00	14.12	6.30	5.00	61.84	71.84	0.09	4.20	1.41	5.61	0.62																								
	No.102	50.00	5,100.00	2.147	2.200	-2.000	-3.409	-2.000	2.200	1.598	5.00	6.30	14.09	21.00	14.09	6.30	5.00	61.79	71.79	0.05	4.20	1.41	5.61	0.60																								
	No.103	50.00	5,150.00	2.340	2.200	-2.000	-3.407	-2.000	2.200	1.300	5.00	6.30	14.07	21.00	14.07	6.30	5.00	61.74	71.74	-0.14	4.20	1.41	5.61	0.90																								

TABLE 2.2 (6/7) PROPOSED CROSS SECTION OF TAU HU - BEN NGHE CANAL IMPROVEMENT

Name of Canal	Section	Length (m)	Accumulate Length (m)	Elevation (m above MSL)										Canal Width										Height															
				Left					Channel					Right					Left					Channel					Right					L-Bank		L-Chan.		R-Bank	
				Bank	Crown	L-Bed	C-Bed	R-Bed	Crown	EL.6	EL.7	Bank	Right	Left	Road	Bottom	Right	Total	G-total	L-Bank	H-Chan.	H-Chan.	Total	R-Bank	H1	H2	H3	H4	H5										
				EL.1	EL.2	EL.3	EL.4	EL.5	EL.6	EL.7	EL.7	EL.6	B1	B2	B3	B4	B5	B6	B7	B8	B9	H1	H2	H3	H4	H5	(m)	(m)	(m)	(m)	(m)								
	No.104	50.00	5,200.00	2.104	2.200	-2.000	-3.404	-2.000	2.200	1.525	5.00	6.30	14.04	21.00	14.04	6.30	61.69	71.69	0.10	4.20	1.40	5.60	5.60	0.68															
	No.105	50.00	5,250.00	2.284	2.200	-2.000	-3.402	-2.000	2.200	1.567	5.00	6.30	14.02	21.00	14.02	6.30	61.64	71.64	-0.08	4.20	1.40	5.60	5.60	0.63															
	No.106	50.00	5,300.00	2.174	2.200	-2.000	-3.399	-2.000	2.200	1.830	5.00	6.30	13.99	21.00	13.99	6.30	61.59	71.59	0.03	4.20	1.40	5.60	5.60	0.37															
	No.107	50.00	5,350.00	2.110	2.200	-2.000	-3.397	-2.000	2.200	1.784	5.00	6.30	10.83	21.00	10.83	6.30	55.26	65.26	0.09	4.20	1.40	5.60	5.60	0.42															
	No.108	50.00	5,400.00	1.890	2.200	-2.000	-3.394	-2.000	2.200	1.770	5.00	6.30	7.67	21.00	7.67	6.30	48.94	58.94	0.31	4.20	1.39	5.59	5.59	0.43															
	BC19	18.57	5,418.57	-	2.200	-2.000	-3.393	-2.000	2.200	-	5.00	6.30	7.66	21.00	7.66	6.30	48.93	58.93	-	4.20	1.39	5.59	5.59	-															
	No.109	31.43	5,450.00	1.750	2.200	-2.000	-3.392	-2.000	2.200	1.794	5.00	6.30	7.66	21.00	7.66	6.30	48.91	58.91	0.45	4.20	1.39	5.59	5.59	0.41															
	No.110	50.00	5,500.00	1.610	2.200	-2.000	-3.389	-2.000	2.200	1.900	5.00	6.30	7.64	21.00	7.64	6.30	48.88	56.44	0.59	4.20	1.39	5.59	5.59	0.30															
	SP19	22.36	5,522.36	-	2.200	-2.000	-3.388	-2.000	2.200	-	5.00	6.30	7.64	21.00	7.64	6.30	48.87	58.87	-	4.20	1.39	5.59	5.59	-															
	No.111	27.64	5,550.00	1.810	2.200	-2.000	-3.387	-2.000	2.200	1.870	5.00	6.30	7.63	21.00	7.63	6.30	48.86	58.86	0.39	4.20	1.39	5.59	5.59	0.33															
	No.112	50.00	5,600.00	1.970	2.200	-2.000	-3.384	-2.000	2.200	1.648	5.00	6.30	7.61	21.00	7.61	6.30	48.83	58.83	0.23	4.20	1.38	5.58	5.58	0.55															
	EC19	26.15	5,626.15	-	2.200	-2.000	-3.383	-2.000	2.200	-	5.00	6.30	7.61	21.00	7.61	6.30	48.81	58.81	-	4.20	1.38	5.58	5.58	-															
	No.113	23.85	5,650.00	2.000	2.200	-2.000	-3.382	-2.000	2.200	1.680	5.00	6.30	7.60	21.00	7.60	6.30	48.80	58.80	0.20	4.20	1.38	5.58	5.58	0.52															
	No.114	50.00	5,700.00	2.284	2.200	-2.000	-3.379	-2.000	2.200	1.725	5.00	6.30	7.59	21.00	7.59	6.30	48.77	58.77	-0.08	4.20	1.38	5.58	5.58	0.48															
	No.115	50.00	5,750.00	2.140	2.200	-2.000	-3.377	-2.000	2.200	1.698	5.00	6.30	7.57	21.00	7.57	6.30	48.75	58.75	0.06	4.20	1.38	5.58	5.58	0.50															
	No.116	50.00	5,800.00	2.065	2.200	-2.000	-3.374	-2.000	2.200	1.798	5.00	6.30	7.56	21.00	7.56	6.30	48.72	58.72	0.14	4.20	1.37	5.57	5.57	0.40															
	No.117	50.00	5,850.00	2.063	2.200	-2.000	-3.372	-2.000	2.200	1.784	5.00	6.30	7.55	21.00	7.55	6.30	48.69	58.69	0.14	4.20	1.37	5.57	5.57	0.42															
	No.118	50.00	5,900.00	2.064	2.200	-2.000	-3.369	-2.000	2.200	1.654	5.00	2.10	10.27	21.00	10.27	2.10	45.74	55.74	0.14	4.20	1.37	5.57	5.57	0.55															
	No.119	50.00	5,950.00	2.048	2.200	-2.000	-3.367	-2.000	2.200	1.784	5.00	2.10	10.25	21.00	10.25	2.10	45.70	55.70	0.15	4.20	1.37	5.57	5.57	0.42															
	No.120	50.00	6,000.00	2.130	2.200	-2.000	-3.364	-2.000	2.200	1.748	5.00	2.10	10.23	21.00	10.23	2.10	45.67	55.67	0.07	4.20	1.36	5.56	5.56	0.45															
	No.121	50.00	6,050.00	2.434	2.200	-2.000	-3.362	-2.000	2.200	2.092	5.00	2.10	10.21	21.00	10.21	2.10	45.63	55.63	-0.23	4.20	1.36	5.56	5.56	0.11															
	IP20	35.05	6,085.05	-	2.200	-2.000	-3.360	-2.000	2.200	-	5.00	2.10	10.20	21.00	10.20	2.10	45.60	55.60	-	4.20	1.36	5.56	5.56	-															
	No.122	14.95	6,100.00	2.351	2.200	-2.000	-3.359	-2.000	2.200	2.404	5.00	2.10	10.20	21.00	10.20	2.10	45.59	55.59	-0.15	4.20	1.36	5.56	5.56	-0.20															
	No.123	50.00	6,150.00	3.242	2.200	-2.000	-3.357	-2.000	2.200	2.625	5.00	2.10	10.18	21.00	10.18	2.10	45.55	55.55	-1.04	4.20	1.36	5.56	5.56	-0.43															
	No.124	50.00	6,200.00	2.370	2.200	-2.000	-3.354	-2.000	2.200	2.150	5.00	2.10	10.16	21.00	10.16	2.10	45.52	54.76	-0.17	4.20	1.35	5.55	5.55	0.05															
	No.125	50.00	6,250.00	1.804	2.200	-2.000	-3.352	-2.000	2.200	1.940	5.00	2.10	10.14	21.00	10.14	2.10	45.48	52.74	0.40	4.20	1.35	5.55	5.55	0.26															
	No.126	50.00	6,300.00	1.904	2.200	-2.000	-3.349	-2.000	2.200	1.950	5.00	2.10	10.12	21.00	10.12	2.10	45.44	53.72	0.30	4.20	1.35	5.55	5.55	0.23															
	No.127	50.00	6,350.00	1.924	2.200	-2.000	-3.347	-2.000	2.200	1.750	5.00	2.10	10.10	21.00	10.10	2.10	45.40	55.40	0.28	4.20	1.35	5.55	5.55	0.45															
	IP21	21.98	6,371.98	-	2.200	-2.000	-3.346	-2.000	2.200	-	5.00	2.10	10.09	21.00	10.09	2.10	45.39	55.39	-	4.20	1.35	5.55	5.55	-															
	No.128	28.02	6,400.00	1.984	2.200	-2.000	-3.344	-2.000	2.200	1.960	5.00	2.10	10.08	21.00	10.08	2.10	45.37	55.37	0.22	4.20	1.34	5.54	5.54	0.24															

TABLE 2.2 (7/7) PROPOSED CROSS SECTION OF TAU HU - BEN NGHE CANAL IMPROVEMENT

Name of Canal	Section	Length (m)	Accumulate Length (m)	Elevation (m above MSL)										Canal Width										Height															
				Left					Channel					Right					Left					Channel					Right					L-Bank		L-Chan.		R-Bank	
				Bank EL.1 (m)	Crown EL.2 (m)	L-Bed EL.3 (m)	C-Bed EL.4 (m)	R-Bed EL.5 (m)	Crown EL.6 (m)	Bank EL.7 (m)	Left Road B1 (m)	Left B2 (m)	Left B3 (m)	Left B4 (m)	Bottom B5 (m)	Right B6 (m)	Right Road B7 (m)	Total B8 (m)	G-total B9 (m)	L-Bank HI (m)	H-Chan. H2 (m)	L-Chan. H3 (m)	Total H4 (m)	R-Bank H5 (m)															
Tau Hu (Downstream)	No.129	50.00	6,450.00	2,054	2,200	-2,000	-3,342	-2,000	2,200	1,858	5.00	2.10	10.06	21.00	10.06	2.10	5.00	45.33	55.33	0.15	4.20	1.34	5.54	0.34															
	BC22	25.37	6,475.37	-	2,200	-2,000	-3,341	-2,000	2,200	-	5.00	2.10	10.05	21.00	10.05	2.10	5.00	45.31	55.31	-	4.20	1.34	5.54	-															
	No.130	24.63	6,500.00	2,190	2,200	-2,000	-3,339	-2,000	2,200	1,748	5.00	2.10	10.05	21.00	10.05	2.10	5.00	45.29	55.29	0.01	4.20	1.34	5.54	0.45															
	SP22	25.28	6,525.28	-	2,200	-2,000	-3,338	-2,000	2,200	-	5.00	2.10	10.04	21.00	10.04	2.10	5.00	45.27	55.27	-	4.20	1.34	5.54	-															
	No.131	24.73	6,550.00	2,234	2,200	-2,000	-3,337	-2,000	2,200	1,834	5.00	2.10	10.03	21.00	10.03	2.10	5.00	45.25	55.25	-0.03	4.20	1.34	5.54	0.37															
	EC22	25.18	6,575.18	-	2,200	-2,000	-3,336	-2,000	2,200	-	5.00	2.10	10.02	21.00	10.02	2.10	5.00	45.23	55.23	-	4.20	1.34	5.54	-															
	No.132	24.82	6,600.00	2,280	2,200	-2,000	-3,334	-2,000	2,200	1,835	5.00	2.10	10.01	21.00	10.01	2.10	5.00	45.22	55.22	-0.08	4.20	1.33	5.53	0.37															
	No.133	50.00	6,650.00	2,023	2,200	-2,000	-3,332	-2,000	2,200	1,860	5.00	2.10	9.99	21.00	9.99	2.10	5.00	45.18	55.18	0.18	4.20	1.33	5.53	0.34															
	No.134	50.00	6,700.00	1,680	2,200	-2,000	-3,329	-2,000	2,200	1,700	5.00	2.10	9.97	21.00	9.97	2.10	5.00	45.14	55.14	0.52	4.20	1.33	5.53	0.50															
	No.135	50.00	6,750.00	1,680	2,200	-2,000	-3,327	-2,000	2,200	1,830	5.00	2.10	9.95	21.00	9.95	2.10	5.00	45.10	55.10	0.52	4.20	1.33	5.53	0.37															
	No.136	50.00	6,800.00	1,874	2,200	-2,000	-3,324	-2,000	2,200	1,743	5.00	2.10	9.93	21.00	9.93	2.10	3.47	45.07	53.54	0.33	4.20	1.32	5.52	0.46															
	No.137	50.00	6,850.00	1,904	2,200	-2,000	-3,322	-2,000	2,200	1,698	5.00	2.10	9.91	21.00	9.91	2.10	3.49	45.03	53.52	0.30	4.20	1.32	5.52	0.50															
	No.138	50.00	6,900.00	1,914	2,200	-2,000	-3,319	-2,000	2,200	1,643	5.00	2.10	9.90	21.00	9.90	2.10	3.50	44.99	53.49	0.29	4.20	1.32	5.52	0.56															
	No.139	50.00	6,950.00	1,924	2,200	-2,000	-3,317	-2,000	2,200	1,690	5.00	2.10	9.88	21.00	9.88	2.10	3.52	44.95	53.47	0.28	4.20	1.32	5.52	0.51															
	No.140	50.00	7,000.00	1,864	2,200	-2,000	-3,314	-2,000	2,200	1,704	5.00	2.10	9.86	21.00	9.86	2.10	3.54	44.92	53.46	0.34	4.20	1.31	5.51	0.50															
	No.141	50.00	7,050.00	1,900	2,200	-2,000	-3,312	-2,000	2,200	1,674	5.00	2.10	9.84	21.00	9.84	2.10	3.56	44.88	53.44	0.30	4.20	1.31	5.51	0.53															
	No.142	50.00	7,100.00	1,984	2,200	-2,000	-3,309	-2,000	2,200	1,614	5.00	2.10	9.82	21.00	9.82	2.10	4.58	44.84	54.42	0.22	4.20	1.31	5.51	0.59															
No.143	50.00	7,150.00	1,964	2,200	-2,000	-3,307	-2,000	2,200	1,748	5.00	2.10	9.80	21.00	9.80	2.10	5.00	44.80	54.80	0.24	4.20	1.31	5.51	0.45																
No.144	50.00	7,200.00	1,912	2,200	-2,000	-3,304	-2,000	2,200	1,904	5.00	2.10	9.78	21.00	9.78	2.10	5.00	44.77	54.77	0.29	4.20	1.30	5.50	0.30																
No.145	50.00	7,250.00	1,948	2,200	-2,000	-3,302	-2,000	2,200	3,440	5.00	2.10	9.76	21.00	9.76	2.10	5.00	44.73	54.73	0.25	4.20	1.30	5.50	-1.24																
IP24 (EP)	36.72	7,286.72	1,904	2,200	-2,000	-3,300	-2,000	-	-	5.00	2.10	9.75	21.00	-	-	-	-	-	-	-	-	-	-																

TABLE 2.3 (1/4) CALCULATION OF PREVENTION FROM SLIDING – CASE 1

Safety Factor	F _s	1.20
Central Coordination of Critical Circle (Horizontal Axis) X(m)		5.00
Central Coordination of Critical Circle (Vertical Axis) Y(m)		7.00
Radius of Critical Circle	R(m)	15.00
Resisting Force	SR(tf)	38.85
Resisting Moment	MR(tf ·m)	582.74
Sliding Force	SD(tf)	35.41
Sliding Moment	MD(tf ·m)	485.25

No	Name of Block	γ (tf/m ³)	C _o (tf/m ²)	k ₁ (tf/m ³)	ϕ_u (°)
1	Fill	1.60	1.50	0.00	0.00
2	OH	1.45	1.10	0.00	0.00
3	S	1.84	0.00	0.00	26.00
4	Timber	1.70	2.60	0.00	0.00
5	OH1	1.45	1.10	0.00	0.00
6	C	2.20	4.00	0.00	0.00
7	Timber 2	1.70	1.80	0.00	0.00

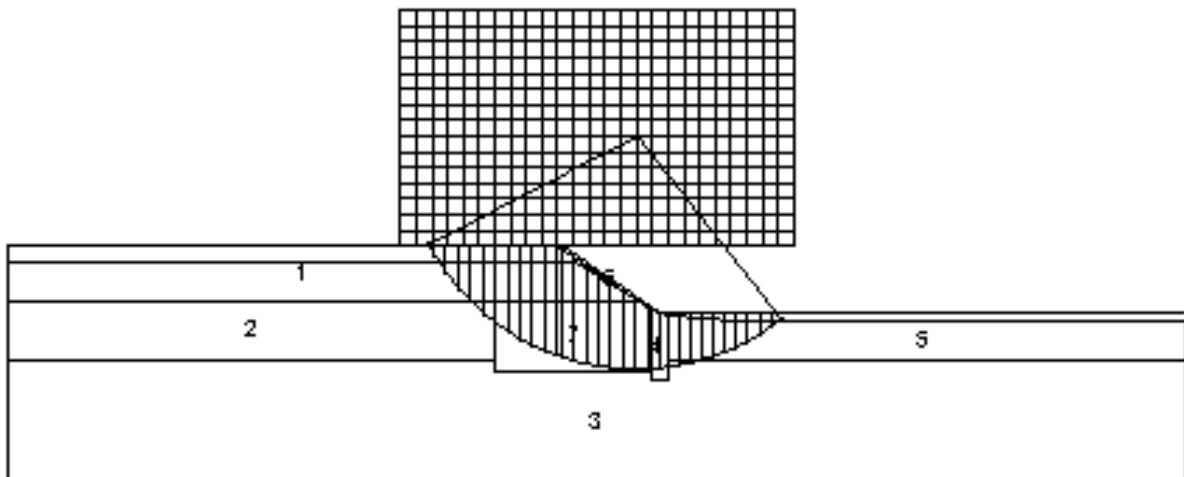


TABLE 2.3 (2/4) CALCULATION OF PREVENTION FROM SLIDING – CASE 2

Safety Factor	F _s	1.20
Central Coordination of Critical Circle (Horizontal Axis) X(m)		4.00
Central Coordination of Critical Circle (Vertical Axis) Y(m)		7.00
Radius of Critical Circle	R(m)	15.00
Resisting Force	SR(tf)	37.53
Resisting Moment	MR(tf ·m)	562.94
Sliding Force	SD(tf)	34.52
Sliding Moment	MD(tf ·m)	470.44

No	Name of Block	γ (tf/m ³)	C _o (tf/m ²)	k ₁ (tf/m ³)	ϕ_u (°)
1	Fill	1.60	1.50	0.00	0.00
2	OH	1.55	0.95	0.00	0.00
3	S	1.91	0.00	0.00	27.00
4	Timber	1.70	2.60	0.00	0.00
5	OH1	1.55	0.95	0.00	0.00
6	C	2.20	2.00	0.00	0.00
7	Timber 2	1.70	1.60	0.00	0.00

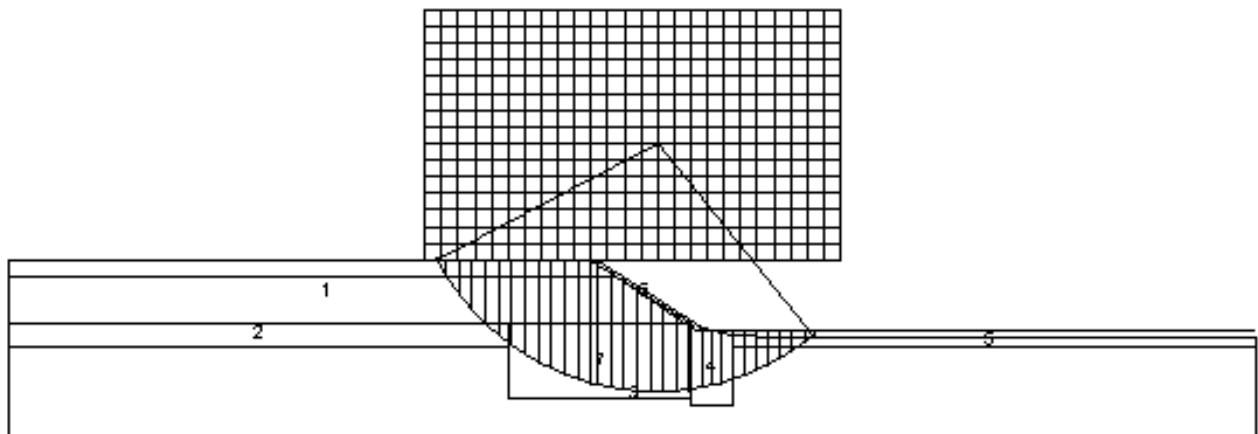


TABLE 2.3 (3/4) CALCULATION OF PREVENTION FROM SLIDING – CASE 3

Safety Factor	F _s	1.24
Central Coordination of Critical Circle (Horizontal Axis) X(m)		4.00
Central Coordination of Critical Circle (Vertical Axis) Y(m)		7.00
Radius of Critical Circle	R(m)	13.50
Resisting Force	SR(tf)	29.78
Resisting Moment	MR(tf ·m)	401.99
Sliding Force	SD(tf)	25.89
Sliding Moment	MD(tf ·m)	324.35

No	Name of Block	γ (tf/m ³)	C _o (tf/m ²)	k ₁ (tf/m ³)	ϕ_u (°)
1	Fill	1.60	1.50	0.00	0.00
2	OH	1.45	0.65	0.00	0.00
3	S	1.91	0.00	0.00	27.00
4	Timber	1.70	2.60	0.00	0.00
5	OH1	1.45	0.65	0.00	0.00
6	C	2.20	2.00	0.00	0.00
7	Timber 2	1.70	1.60	0.00	0.00

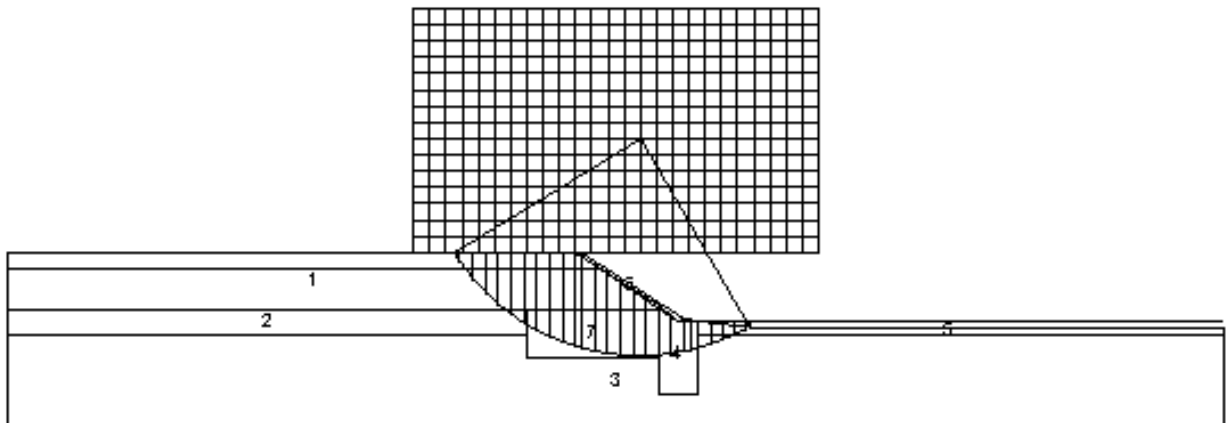
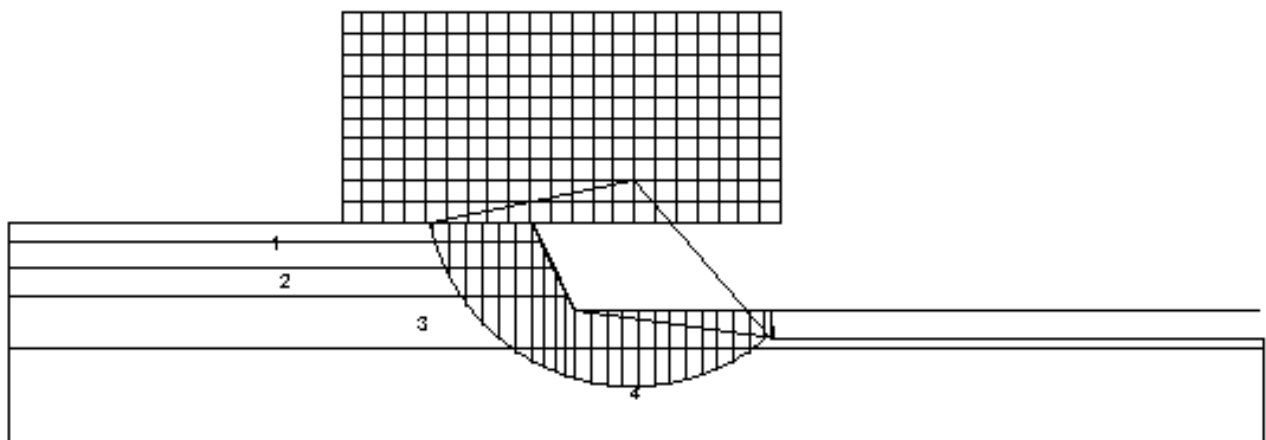


TABLE 2.3 (4/4) CALCULATION OF PREVENTION FROM SLIDING – CASE 4

Safety Factor	F _s	1.82
Central Coordination of Critical Circle (Horizontal Axis) X(m)		4.92
Central Coordination of Critical Circle (Vertical Axis) Y(m)		2.00
Radius of Critical Circle	R(m)	10.00
Resisting Force	SR(tf)	46.42
Resisting Moment	MR(tf · m)	464.16
Sliding Force	SD(tf)	29.14
Sliding Moment	MD(tf · m)	255.29

No	Name of Block	γ (tf/m ³)	C _o (tf/m ²)	k ₁ (tf/m ³)	ϕ_u (°)
1	Fill	1.70	1.50	0.00	0.00
2	OH	1.46	3.00	0.00	0.00
3	C	1.90	4.20	0.00	0.00
4	S	1.98	0.00	0.00	29.00



**TABLE 2.4 (1/2) REPLACEMENT LIST OF EXISTING SEWER OUTLETS ALONG
TAU HU - BEN NGHE CANAL**

No. of Outlet		Location	Dimension				Additional Pipe Length (m)	Remarks
(New)	(Old)		Material	Width (m)	Height (m)	Invert EL. (m)		
BN-L1	61	No.4 + 9.1 m	RC Pipe	1,000	1,000	-1.65		EWHP
BN-L2	60	No.5 + 48.4 m	RC Pipe	1,000	1,000	-0.72	14.0	
BN-L3	59	No.7 + 42.3 m	RC Pipe	1,000	1,000	-0.50	24.0	
BN-L4	58	No.9 + 48.9 m	Brick	800	1,500	-1.22	6.0	Arch
			RC (Precast)	1,600	2,000	-1.75		Box Culvert
BN-L5	57	No.12 + 42.6 m	Brick	1,800	2,000	-0.99	26.0	Arch
			RC (Precast)	1,800	2,000	-1.05		Box Culvert
BN-L6	56	No.15 + 49.3 m	RC Pipe	2,000	2,000	-1.45		EWHP
BN-L7	55	No.18 + 2.4 m	RC	1,500	1,500	-1.85	26.0	Box Culvert
BN-L8	54	No.21 + 8.4 m	RC Pipe	1,000	1,000	-1.77	30.0	
BN-L9	53	No.23 + 28.2 m	RC Pipe	1,000	1,000	-1.32	10.0	
BN-L10	52	No.33 + 19.0 m	RC Pipe	1,000	1,000	-0.95	14.0	
BN-L11	51	No.36 + 39.1 m	RC	2,000	2,000	-1.56	14.0	Box Culvert
BN-L12	50	No.40 + 45.2 m	RC Pipe	1,500	1,500	-1.56	22.0	
BN-L13	Addi.		RC(Precast)	2,000	2,000	-2.10		Box Culvert
BN-L14	49	No.48 + 3.9 m	RC Pipe	1,000	1,000	-1.18	24.0	
BN-L15	48	No.51 + 35.4 m	RC Pipe	800	800	-0.30	10.0	
BN-L16	47	No.56 + 17.0 m	RC Pipe	1,000	1,000	0.63	30.0	
BN-L17	Addi.	(Ba Do Bridge)	RC(Precast)	2,000	2,000	-2.10		Box Culvert
BN-R1	62	No.3 + 18.6 m	Brick	800	1,400	-1.35		Arch 800 x 1,400
			RC (Precast)	1,500	1,500	-1.50		Box Culvert, EWHP
BN-R2	63	No.4 + 8.2 m	RC Pipe	600	600	-1.55		EWHP
BN-R3	64	No.9 + 44.7 m	RC Pipe	600	600	-0.16	6.0	
BN-R4	65	No.10 + 23.6 m	RC Pipe	600	600	-0.18	16.0	
BN-R5	66	No.12 + 30.0 m	RC Pipe	400	400		8.0	Clogged up with soil
BN-R6	67	No.13 + 34.2 m	RC Pipe	400	400		10.0	Clogged up with soil
BN-R7	68	No.14 + 4.0 m	RC Pipe	1,200	1,200	-0.76	4.0	
BN-R8	69	No.15 + 35.1 m	RC Pipe	800	800	-1.43	10.0	
BN-R9	70	No.17 + 5.5 m	RC	3,000	1,500	-0.75	26.0	Box Culvert
BN-R10	71	No.20 + 3.0 m	RC Pipe	1,000	1,000	-1.14	2.0	
BN-R11	72	No.21 + 1.4 m	RC Pipe	400	400	0.65	2.0	Clogged up with soil
BN-R12	73	No.25 + 28.2 m	RC Pipe	800	800	-0.97	12.0	
BN-R13	74	No.28 + 28.2 m	RC Pipe	800	800		10.0	Clogged up with soil
BN-R14	75	No.32 + 25.9 m	RC Pipe	800	800		4.0	Clogged up with soil
BN-R15	76	No.36 + 47.2 m	RC Pipe	600	600	0.20	26.0	
BN-R16	77	No.38 + 24.9 m	RC Pipe	800	800	-0.46	18.0	
BN-R17	78	No.40 + 45.2 m	RC Pipe	1,000	1,000	-0.16	14.0	
BN-R18	79	No.43 + 19.2 m	RC Pipe	600	600	-0.47	12.0	
BN-R19	80	No.44 + 5.3 m	RC Pipe	400	400		12.0	Clogged up with soil
BN-R20	81	No.44 + 45.7 m	RC Pipe	400	400	-0.17	6.0	
BN-R21	82	No.46 + 15.8 m	RC Pipe	400	400	0.79	14.0	Clogged up with soil
BN-R22	83	No.46 + 35.7 m	RC Pipe	400	400	0.99	24.0	Clogged up with soil
TH-L1	46	No.67 + 5.7 m	RC Pipe	1,000	1,000	-0.31		
			RC Pipe	2,000	2,000	-2.00		EWHP & Pac.-D
TH-L2	45	No.70 + 35.9 m	Brick	2,500	2,000	-1.01		Arch 2,500 x 2,000
			RC (Precast)	2 x 2,000	2,000	-3.20		Box Culvert, Pac.-D
TH-L3	44	No.75 + 0.0 m	RC Pipe	1,000	1,000	0.06	14.0	
TH-L4	43	No.80 + 43.4 m	RC Pipe	1,000	1,000	-0.39	32.0	
TH-L5	42	No.81 + 4.3 m	RC Pipe	1,000	1,000	-0.40	44.0	
TH-L6	41	No.82 + 48.8 m	Brick	800	1,400	-1.03	48.0	Arch 800 x 1,400
			RC (Precast)	1,500	1,500	-1.10		Box Culvert
TH-L7	40	No.90 + 30.8 m	RC Pipe	1,000	1,000	-0.27	26.0	
TH-L8	39	No.93 + 8.1 m	RC Pipe	800	800	-0.90	34.0	
TH-L9	38	No.93 + 32.9 m	Brick	800	1,400			Arch, Clogged up with
			RC (Precast)	1,200	1,200	-1.40		Box Culvert
TH-L10	Addi	NT Phuong Bridge	Brick	2,500	2,200			Arch
			RC (Precast)	2,800	2,400	-2.10		Box Culvert
TH-L11	37	No.110 + 11.3 m	RC Pipe	1,500	1,500	-1.43	26.0	Arch

**TABLE 2.4 (2/2) REPLACEMENT LIST OF EXISTING SEWER OUTLETS ALONG
TAU HU - BEN NGHE CANAL**

No. of Outlet		Location	Dimension				Additional Pipe Length (m)	Remarks
(New)	(Old)		Material	Width (m)	Height (m)	Invert EL. (m)		
TH-L12	36	No.110 + 41.4 m	RC Pipe	1,000	1,000	-0.65	10.0	
TH-L13	35	No.111 + 20.3 m	RC Pipe	1,000	1,000	-0.50	20.0	
TH-L14	34	No.112 + 14.1 m	Brick	600	1,200	-0.63	18.0	Arch 600 x 1,200
			RC (Precast)	1,200	1,200	-0.70		Box Culvert
TH-L15	33	No.113 + 42.2 m	RC(Precast)	2 x 2,000	2,000	-0.89	3.0	Box Culvert
TH-L16	32	No.115 + 42.3 m	RC Pipe	600	600	-0.22	3.0	
TH-L17	31	No.117 + 14.1 m	RC Pipe	1,000	1,000	-0.06	2.0	
TH-L18	30	No.120 + 25.9 m	Brick	600	1,200	-0.30	2.0	Arch 600 x 1,200
			RC (Precast)	1,500	1,500	-0.60		Box Culvert
TH-L19	29	No.121 + 37.3 m	RC Pipe	600	600	-0.45	2.0	EWHP
TH-L20	28	No. 122 + 19.9 m	Brick	800	1,400	-1.91		Arch 800 x 1,400
			RC (Precast)	1,200	1,200	-2.00		Box Culvert, EWHP
TH-L21	27	No.123 + 11.4 m	Brick	600	1,200	-0.71	4.0	Arch 600 x 1,200
			RC	2,500	2,500	-3.30		Box Culvert, Pac.-D
TH-L22	26	No.125 + 38.0 m	Brick	600	1,200	-0.02	12.0	Arch 600 x 1,200
TH-L23	25	No.126 + 48.0 m	RC (Precast)	1,000	1,000	-0.10		Box Culvert
TH-L24	25	No.126 + 48.0 m	RC Pipe	1,000	1,000	-0.03	7.0	
TH-L24	24	No.128 + 29.1 m	RC Pipe	600	600	-0.13	2.0	
TH-L25	Addi.	No.130 + 26.1m	RC (Precast)	2,000	2,000	-2.09		Box Culvert, HBP
TH-L26	23	No.133 + 35.0 m	RC Pipe	800	800	-0.79	1.0	
			RC Pipe	1,200	1,200	-1.00		HBP
TH-L27	22	No.134 + 21.4 m	RC Pipe	400	400	0.89		Clogged up with soil
TH-L28	21	No.135 + 7.4 m	RC Pipe	400	400	0.49		
TH-L29	20	No. 135 + 25.8 m	RC Pipe	800	800	-0.41		
			RC	2,800	2,400	-1.86		HBP
TH-L30	19	No.135 + 48.7 m	RC Pipe	400	400	0.81		Clogged up with soil
TH-L31	18	No.136 + 39.8 m	RC Pipe	800	800	0.47		
TH-L32	17	No.137 + 37.6 m	RC Pipe	400	400	0.62		
TH-L33	16	No. 138 + 13.2 m	RC Pipe	600	600	0.79		
			RC Pipe	1,200	1,200	-1.30		HBP
TH-L34	15	No.139 + 4.4 m	RC Pipe	400	400	0.73		
TH-L35	14	No.140 + 16.6 m	RC Pipe	400	400	0.78		Clogged up with soil
TH-L36	13	No.140 + 41.9 m	RC Pipe	400	400	0.85		
TH-L37	12	No.141 + 44.8 m	RC Pipe	1,000	1,000	-1.17		
			RC Pipe	1,200	1,200	-1.22		HBP
TH-L38	11	No.142 + 17.7 m	RC Pipe	400	400	0.85		
TH-L39	10	No.143 + 23.3 m	RC Pipe	400	400	0.85		
TH-L40	9	No.144 + 21.6 m	RC Pipe	400	400	0.92		
TH-L41	8	No.145 + 11.3 m	RC Pipe	400	400			Clogged up with soil
			RC	4,000	2,000	-2.51		Box Culvert, HBP
TH-L42	7	No.145 + 28.8 m	RC Pipe	400	400	0.60		Clogged up with soil
TH-R1	Addi.	No.80 + 35.3m	RC Pipe	1,200	1,200	-1.12		Package B
TH-R2	84	No.106 + 10.0 m	RC Pipe	1,000	1,000	-0.03	28.0	
TH-R3	85	No.115 + 32.4 m	RC Pipe	400	400	0.11	6.0	
TH-R4	86	No.116 + 49.2 m	RC Pipe	400	400	0.00	6.0	
TH-R5	87	No.118 + 11.2 m	RC Pipe	1,000	1,000	-1.28	5.0	
TH-R6	88	No.119 + 34.3 m	RC Pipe	800	800	-2.36	4.0	
TH-R7	89	No.120 + 10.4 m	RC Pipe	800	800	-2.42	4.0	
TH-R8	90	No.120 + 31.9 m	RC Pipe	600	600	-1.58	3.0	
TH-R9	91	No.121 + 35.4 m	RC Pipe	600	600	-0.13		EWHP
TH-R10	92	No.124 + 43.7 m	RC Pipe	1,000	1,000	-0.42		
TH-R11	93	No.128 + 27.3 m	RC Pipe	600	600	0.17		
TH-R12	94	No. 128 + 34.2 m	RC Pipe	800	800	-0.12		
TH-R13	95	No. 135 + 41.7 m	RC Pipe	400	400	0.30		
TH-R14	96	No.137 + 35.8 m	RC Pipe	400	400	-0.41		
TH-R15	97	No.138 + 34.6 m	RC Pipe	400	400	-0.12		
TH-R16	98	No.139 + 25.6 m	RC Pipe	400	400	0.31		
TH-R17	99	No.140 + 27.8	RC Pipe	400	400	0.36		
TH-R18	100	No.141 + 19.0 m	RC Pipe	400	400	0.20		
TH-R19	101	No.141 + 19.5 m	RC Pipe	1,000	1,000	-0.45		

TABLE 2.5 LIST OF CONSTRUCTION OF REVETMENT AND SEWER OUTLET ALONG TAU HU - BEN NGHE CANAL TO BE EXECUTED BY OTHER PROJECT

No.	Name of Canal	Start (Station)	End (Station)	Distance (m)	Name of Project	Work Quantity to be executed by other Project					
						Revetment (m)		Outlet (No. and Name)			
						Left Bank	Right Bank	Left Bank	Right Bank	Right Bank	
1	Ben Nghe	No.3 +20.0	No.4 +10.0	40.0	Replacement of Khanh Hoi Bridge by EWHP	40.0	40.0	1 (BN-L1)	2 (BN-R1 & R2)		
2		No.15 +37.0	No.16 + 27.0	40.0	Replacement of Calmette Bridge by EWHP	40.0	40.0	1 (BN-L6)	-		
3		No.66 +26.5	No.67 + 11.5	35.0	Replacement of Chu-Y Bridge by EWHP	35.0	35.0	1 (TH-L1)	-		
4	Tau Hu	No.70 +30.9	No.70 + 40.9	10.0	Construction of Combined Sewer by Package D of WEIP	10.0	0.0	1 (TH-L2)	-		
5		No.121 + 33.0	No.122 + 33.0	50.0	Replacement of Cha Va Bridge	50.0	50.0	2 (TH-L19 & L20)	1 (TH-R8)		
6		No.121 + 7.4	No. 121 + 15.4	8.0	Construction of Combined Sewer by Package D of WEIP	8.0	0.0	1 (TH-L21)	-		
7		No.130 + 22.6	No. 130 + 29.6	7.0	Construction of Combined Sewer by HBDIP	7.0	0.0	1 (TH-L25)	-		
8		No.133 + 22.8	No.133 + 37.5	5.0	Construction of Combined Sewer by HBDIP	5.0	0.0	1 (TH-L26)	-		
9		No.135 + 18.8	No.135 + 29.8	8.0	Construction of Combined Sewer by HBDIP	8.0	0.0	1 (TH-L29)	-		
10		No.138 + 10.7	No.138 + 15.7	5.0	Construction of Combined Sewer by HBDIP	5.0	0.0	1 (TH-L33)	-		
11		No.137 + 42.3	No.137 + 47.3	5.0	Construction of Combined Sewer by HBDIP	5.0	0.0	1 (TH=L37)	-		
12		No.145 + 6.3	No.145 + 16.3	10.0	Construction of Combined Sewer by HBDIP	10.0	0.0	1 (TH=L41)	-		
Total						223.0	165.0	12	3		

Note: 1. EWHP: East-West Highway Project in Ho Chi Minh City

2. WEIP: Ho Chi Minh City Water Environmental Improvement Project

3. HBDIP: Hang Bang Drainage Improvement Project

TABLE 2.6 (1/3) B/Q OF TAU HU - BEN NGHE CANAL IMPROVEMENT PROJECT

No.	Item	Unit	Ben Nghe Canal	Tau Hu Canal	Total
1. Clearing and Grubbing			28,435	39,624	68,059
	B/Q	m2	29,290	40,810	70,100
2. Demolition of Existing Revetment			0	1,735	1,735
	B/Q	m3	0	1,790	1,790
3. Earth Work					
3.1 Excavation					
	3.1.1 Channel Excavation		71,090	130,220	201,310
	3.1.2 Revetment Type A	m3	75,390	58,050	133,440
	3.1.3 Revetment Type B		0	44,420	44,420
	Total		146,480	232,690	379,170
	B/Q		146,480	232,690	379,170
3.2 Filling					
	3.2.1 Channel		0	0	0
	3.2.2 Revetment Type A	m3	95,590	104,830	200,420
	3.2.3 Revetment Type B		0	7,220	7,220
	Total		95,590	112,050	207,640
	B/Q		95,590	112,050	207,640
4. Timber Piling					
4.1 Foundation of Revetment					
	4.1.1 Revetment Type A		512,100	467,350	979,450
	4.1.2 Revetment Type B	m	0	307,500	307,500
	Sub-total		512,100	774,850	1,286,950
4.2 Bank Protection for Sliding					
	4.2.1 Revetment Type A		929,462	885,938	1,815,400
	4.2.2 Revetment Type B	m	0	0	0
	Sub-total		929,462	885,938	1,815,400
	Total		1,441,562	1,660,788	3,102,350
	B/Q		1,456,140	1,677,410	3,102,350
5. Gravel Bedding					
	5.1.1 Revetment Type A		455	415	870
	5.1.2 Revetment Type B	m3	0	273	273
	Total		455	688	1,143
	B/Q		470	710	1,180
6. Free Draining Gravel					
	6.1.1 Revetment Type A		13,308	12,151	25,459
	6.1.2 Revetment Type B	m3	0	4,699	4,699
	Total		13,308	16,850	30,158
	B/Q		13,700	17,360	31,060
7. Geotextile Cloth					
	7.1.1 Revetment Type A		48,934	44,660	93,594
	7.1.2 Revetment Type B	m2	0	15,578	15,578
	Total		48,934	60,238	109,172
	B/Q		50,400	62,050	112,450
8. Precast Concrete Toe Block					
	8.1.1 Revetment Type A		1,423	1,298	2,721
	8.1.2 Revetment Type B	No.	0	683	683
	Total		1,423	1,981	3,404
	B/Q		1,570	2,000	3,570
9. Concrete Type F (Backfill Concrete for Stone Masonry)					
	9.1.1 Revetment Type A		4,609	4,206	8,815
	9.1.2 Revetment Type B	m3	0	1,968	1,968
	Total		4,609	6,174	10,783
	B/Q		4,750	6,360	11,110
10. Concrete type G (Leveling Concrete)					
	10.1.1 Revetment Type A		436	415	851
	10.1.2 Revetment Type B	m3	0	273	273
	Total		436	688	1,124
	B/Q		450	710	1,160
11. Wet Stone Masonry					
	11.1.1 Revetment Type A		14,111	12,879	26,990
	11.1.2 Revetment Type B	m3	0	5,931	5,931
	Total		14,111	18,810	32,921
	B/Q		14,530	19,380	33,910
12. Weep Holes					
	12.1.1 Revetment Type A		4,552	4,155	8,707
	12.1.2 Revetment Type B	No.	0	2,186	2,186
	Total		4,552	6,341	10,893
	B/Q		4,600	6,400	11,000

TABLE 2.6 (2/3) B/Q OF TAU HU - BEN NGHE CANAL IMPROVEMENT PROJECT

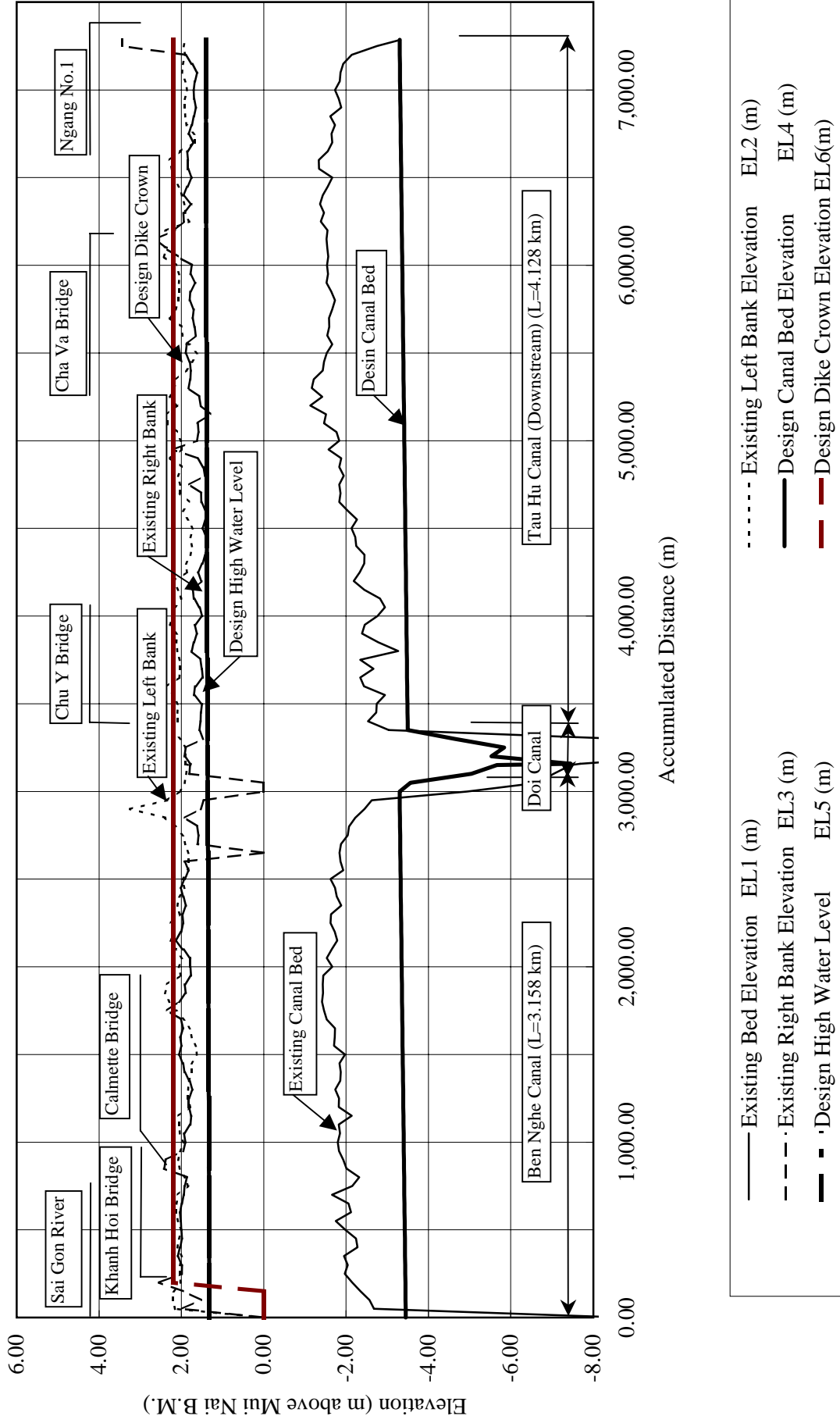
No.	Item	Unit	Ben Nghe Canal	Tau Hu Canal	Total
13. Joint Filler					
	13.1.1 Revetment Type A	m2	783	715	1,498
	13.1.2 Revetment Type B		0	343	343
	Total		783	1,058	1,841
	B/Q		810	1,090	1,900
14. Rip Rap					
	14.1.1 Revetment Type A	m3	8,679	7,817	16,496
	14.1.2 Revetment Type B		0	4,100	4,100
	Total		8,679	11,917	20,596
	B/Q		8,770	12,030	20,800
15. Road Construction					
	15.1 Excavation	m3	0	1,047	1,047
	B/Q		0	1,100	1,100
	15.2 Pavement				
	15.2.1 Subgrade Preparation	m3	0	2,500	2,500
	B/Q		0	2,530	2,530
	15.2.2 Sub Base Course	m3	0	375	375
	B/Q		0	420	420
	15.2.3 Base Course	m3	0	650	650
	B/Q		0	660	660
	15.2.4 Asphalt Binder Course	ton	0	464	464
	B/Q		0	470	470
	15.2.5 Asphalt Surface Course	ton	0	330	330
	B/Q		0	340	340
	15.2.6 Prime Coat	liter	0	3,750	3,750
	B/Q		0	3,790	3,790
	15.2.7 Tac Coat	liter	0	1,125	1,125
	B/Q		0	1,140	1,140
	15.3 Concrete Curb	m	0	362	362
	B/Q		0	365	365
	15.4 Sand Bed for Foot Path	m3	0	145	145
	B/Q		0	145	145
	15.5 Precasdt Concrete Block	m2	0	1,444	1,444
	B/Q		0	1,460	1,460
16. Storm Water Drainage System					
	16.1 Earth Work	m3	0	1839	1,839
	B/Q		0	1,930	1,930
	16.2 Pipe Installation				
	Precast RC Pipe (400dia)	m	0	25	25
	Precast RC Pipe (800dia)		0	346	346
	Precast RC Pipe (1200dia)		0	12	12
	16.3 Construction of Manhole				
	16.3.1 Concrete Type E	m3	0	84.4	84
	B/Q		0	90	90
	16.3.2 Concrete Type F	m3	0	11.3	11
	B/Q		0	12	12
	16.3.3 Concrete Type G	m3	0	8.7	9
	B/Q		0	10	10
	16.3.4 Sand Bedding	m3	0	17.4	17.4
	B/Q		0	18	18
	16.3.5 Granolitic Topping	m2	0	28.2	28.2
	B/Q		0	30	30
	16.3.6 Ladder Rungs	No.	0	155	155
	B/Q		0	160	160
	16.3.7 Deformed Reinforcing Bars	kg	0	5454	5454
	B/Q		0	5,620	5,620
	16.3.8 C.I Manhole Cover (Type A)	No.	0	12	12
	16.3.8 C.I Manhole Cover (Type C)		0	2	2
	16.3.8 C.I Manhole Cover (Type E)		0	12	12
17. Sewer Outlet					
	17.1 Excavation	m3	5,655	3,818	9,473
	B/Q		5,940	4,010	9,950
	17.2 Sandy Soil Backfill	m3	4,414	3,108	7,522
	B/Q		4,640	3,270	7,910
	17.3 Foundation Pile	m	71,224	51,917	123,141
	B/Q		71,940	52,440	124,380

TABLE 2.6 (3/3) B/Q OF TAU HU - BEN NGHE CANAL IMPROVEMENT PROJECT

No.	Item	Unit	Ben Nghe Canal	Tau Hu Canal	Total
17.4	Sand Bedding	m3	185	153	338
	B/Q		190	160	350
17.5	Reinforced Concrete (Concrete Type E)	m3	243	258	501
	B/Q		250	270	520
17.6	Leveling Concrete (Concrete Type G)	m3	92	77	169
	B/Q		100	80	180
17.7	Precast RC Pipe	m			
	17.7.1 400 dia		77	124	201
	17.7.2 600 dia		49	17	66
	17.7.3 800 dia		69	105	174
	17.7.4 1000 dia		170	206	376
	17.7.5 1200 dia		5	0	5
	17.7. 1500 dia	25	5	30	
17.8	Precast RC Box Culvert	m			
	17.8.1 1000 x 1000		0	6	6
	17.8.2 1200 x 1200		0	16	16
	17.8.3 1500 x 1500		26	12	38
	17.8.4 1600 x 2000		7	0	7
	17.8.5 1800 x 2000		7	0	7
	17.8.6 2000 x 2000		15	0	15
	17.8.7 2000 x 2000 x 2		9	0	9
	17.8.8 2800 x 2400		0	7	7
	17.8.9 3000 x 1500	27	0	27	
18. Landscape Work					
18.1	Excavation	m3	2,278	3,170	5,448
	B/Q		2,390	3,330	5,720
18.2	Sandy Soil Backfill	m3	1,481	2,061	3,542
	B/Q		1,550	2,160	3,710
18.3	Curb (Concrete Type E)	m3	1,281	1,784	3,065
	B/Q		1,320	1,840	3,160
18.4	Deformed Reinforcing Bars	kg	10,231	14,235	24,466
	B/Q		10,540	14,660	25,200
18.5	Leveling Concrete (Concrete Type G)	m3	249	347	596
	B/Q		260	360	620
18.6	Sand Bedding	m3	1,735	2,280	4,015
	B/Q		1,790	2,350	4,140
18.7	Precast Concrete Block	m2	17,348	22,800	40,148
	B/Q		17,520	23,030	40,550
18.8	Gravel Bedding	m3	498	694	1,192
	B/Q		510	715	1,225
18.9	Sodding	m2	673	939	1,612
	B/Q		680	950	1,630
18.10	Flowering Shurabs/Ground Cover Plan	m2	1,562	2,178	3,740
	B/Q		1,580	2,200	3,780
18.11	Tree	No.	710	990	1,700
	B/Q		710	990	1,700
19. Electric Work					
19.1	Installation of Power Pole	No.	2	2	4
	B/Q		2	2	4
19.2	Installation of Lighting Pole	m	145	200	345
	B/Q		145	200	345
19.3	Installation of Cable	m	5,710	7,930	13,640
	B/Q		5,880	8,170	14,050
19.4	Cable Connection	No.			
	19.4.1 Number of Connection Site		2	2	4
	19.4.2 Transformer		2	2	2
	19.4.3 Metering Cubicle		2	2	4
	19.4.4 Cable		200	200	400

Note: Bold-faced figures in the above tables include some allowance (3 to 5 % of the estimated quantity), which are applied Bill of Quantity in Tender Document.

FIG. 2.2 PROPOSED LONGITUDINAL PROFILE OF TAU HU - BEN NGHE CANAL IMPROVEMENT



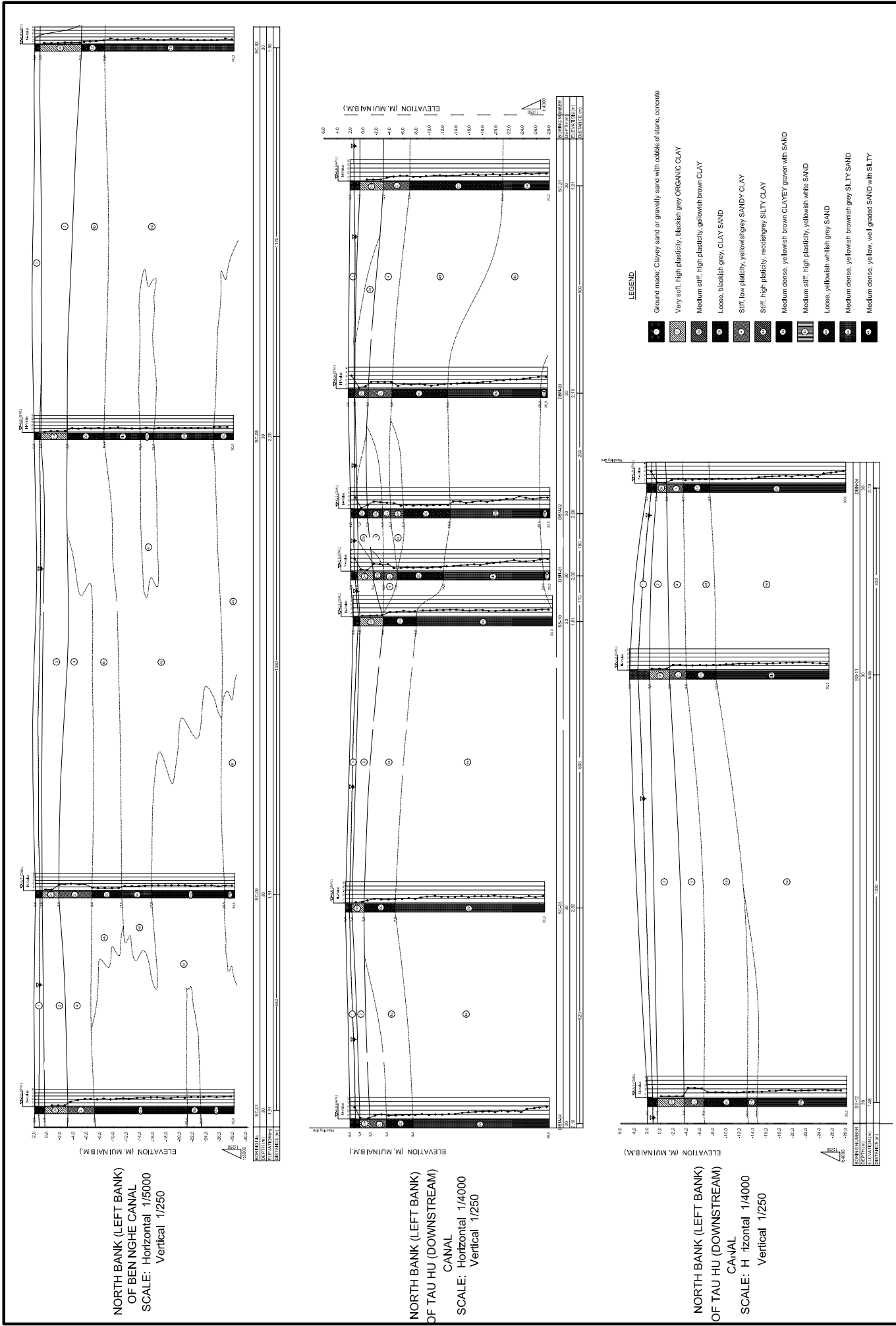


FIG. 2.3 GEOLOGICAL PROFILE ALONG TAU HU - BEN NGHE CANAL

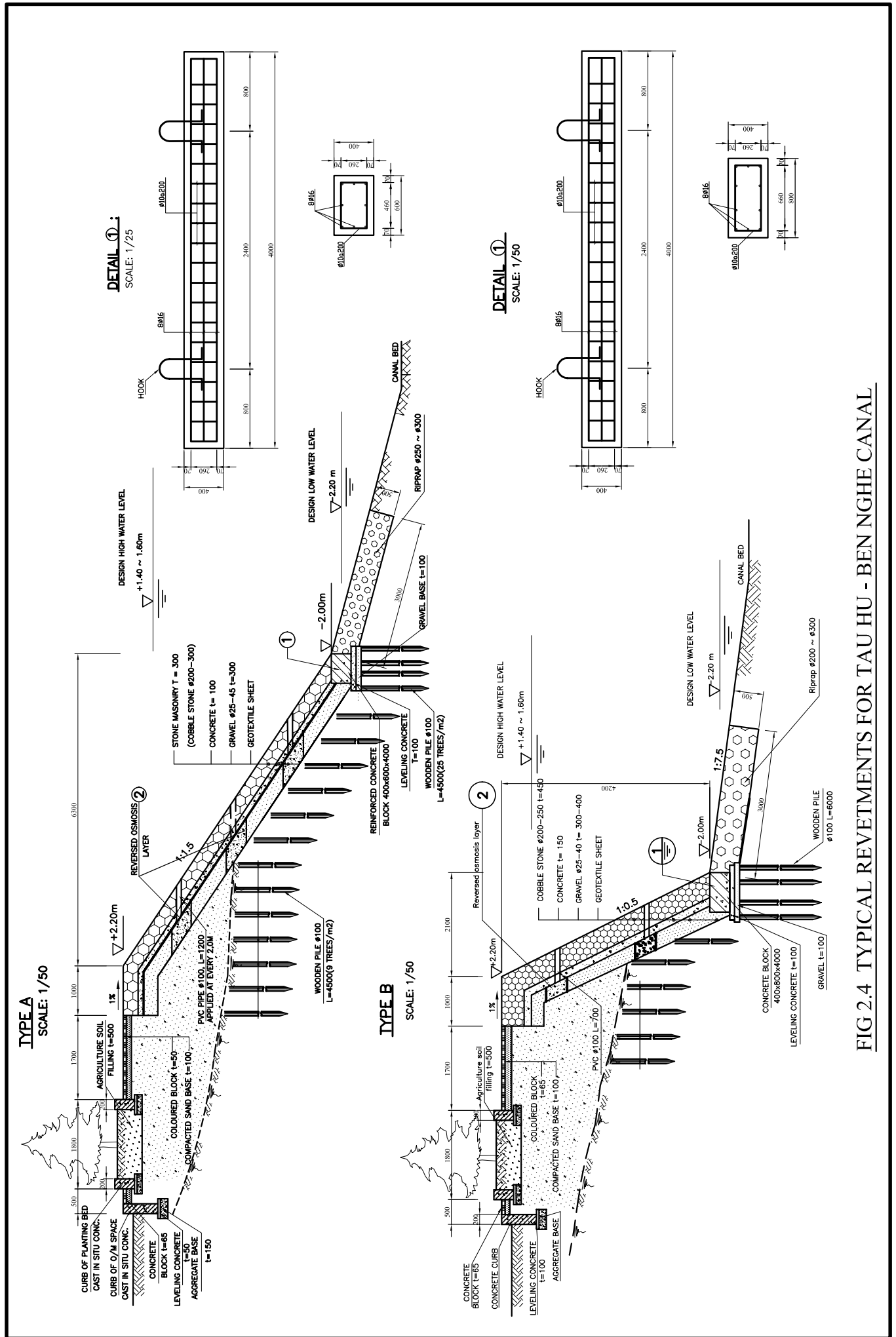
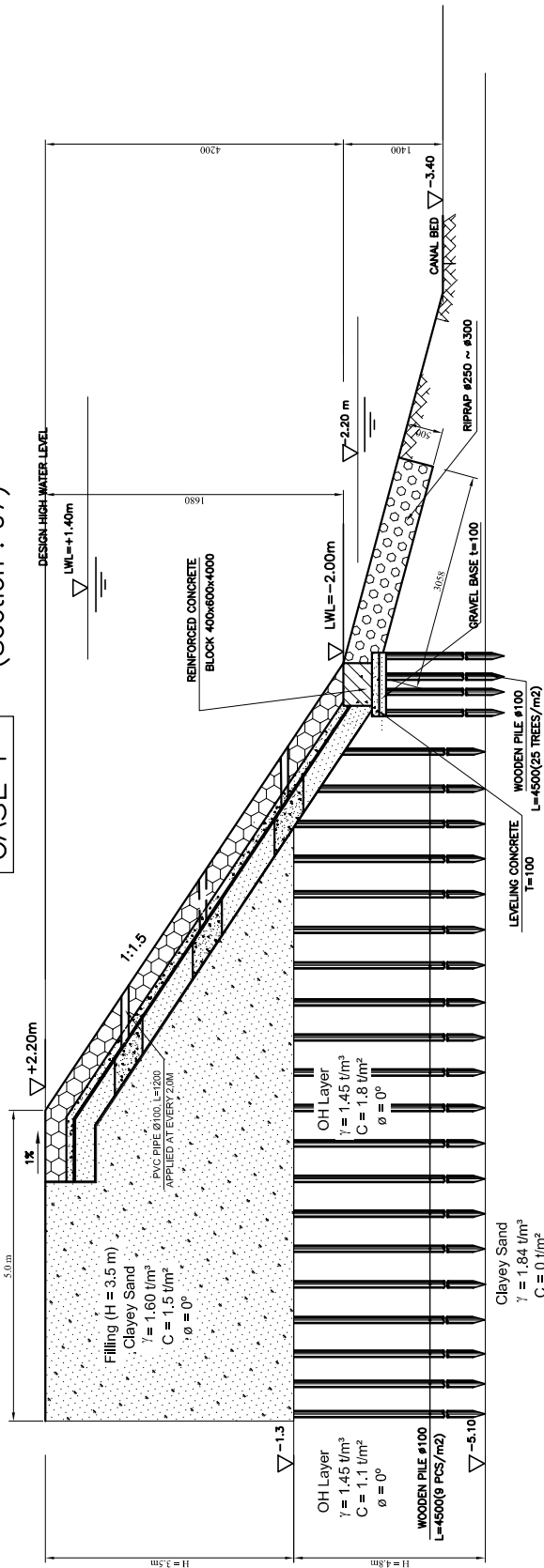


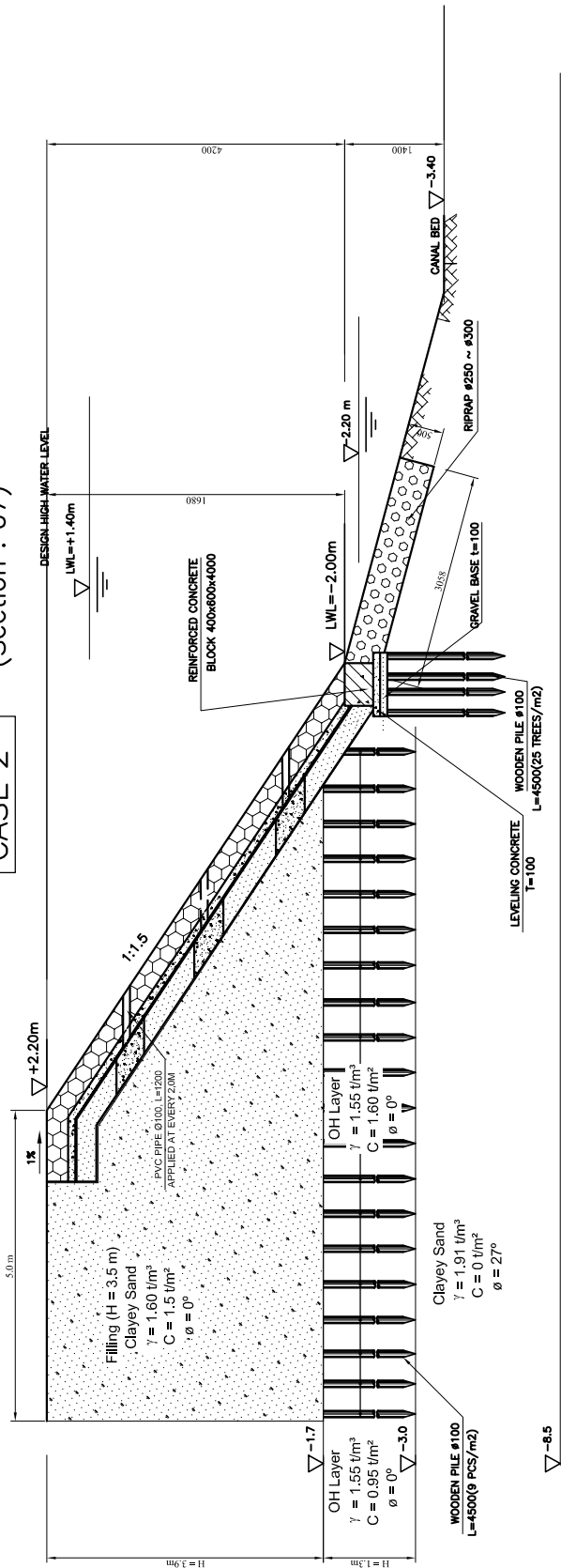
FIG 2.4 TYPICAL REVETMENTS FOR TAU HU - BEN NGHE CANAL

CASE 1 (Section . 07)



SOIL COUDITION IS FOLLOW THE DATA BORING LOG SS-02

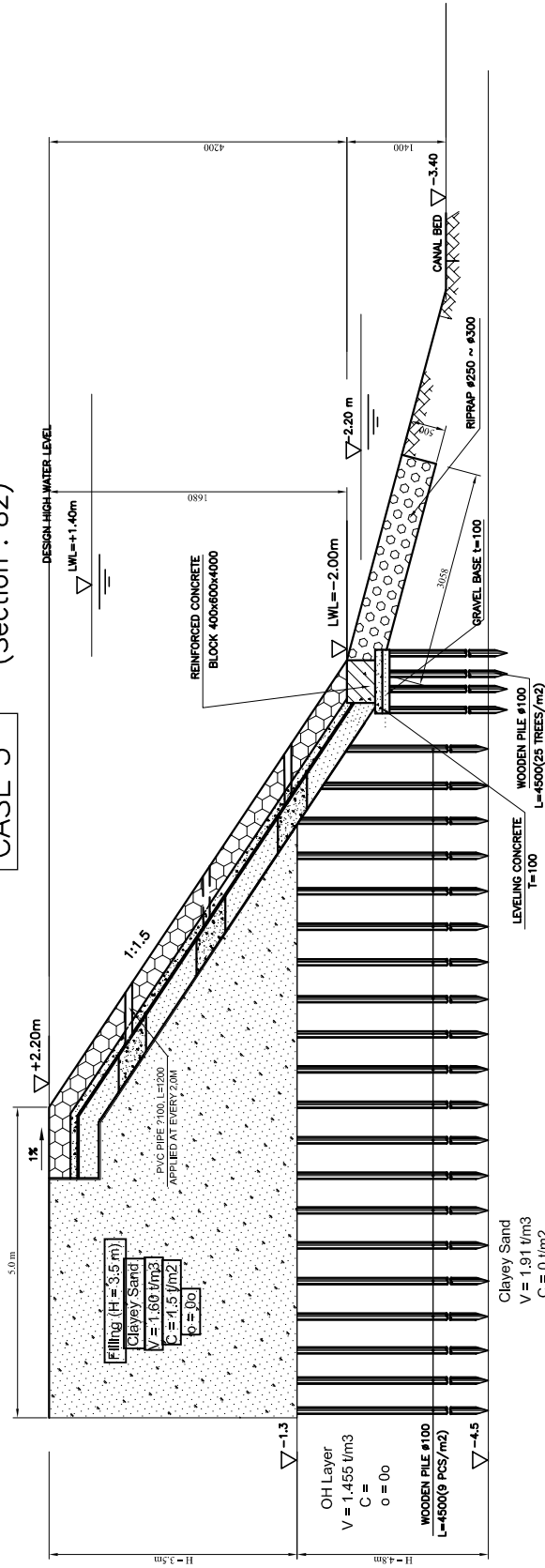
CASE 2 (Section . 07)



SOIL COUDITION IS FOLLOW THE DATA BORING LOG SS-08

FIG 2.5 PREVENTION WORK FROM SLIDING DUE TO FILLING

CASE 3 (Section . 82)



SOIL COUDDITION IS FOLLOW THE DATA BORING LOG SS-10

CASE 4

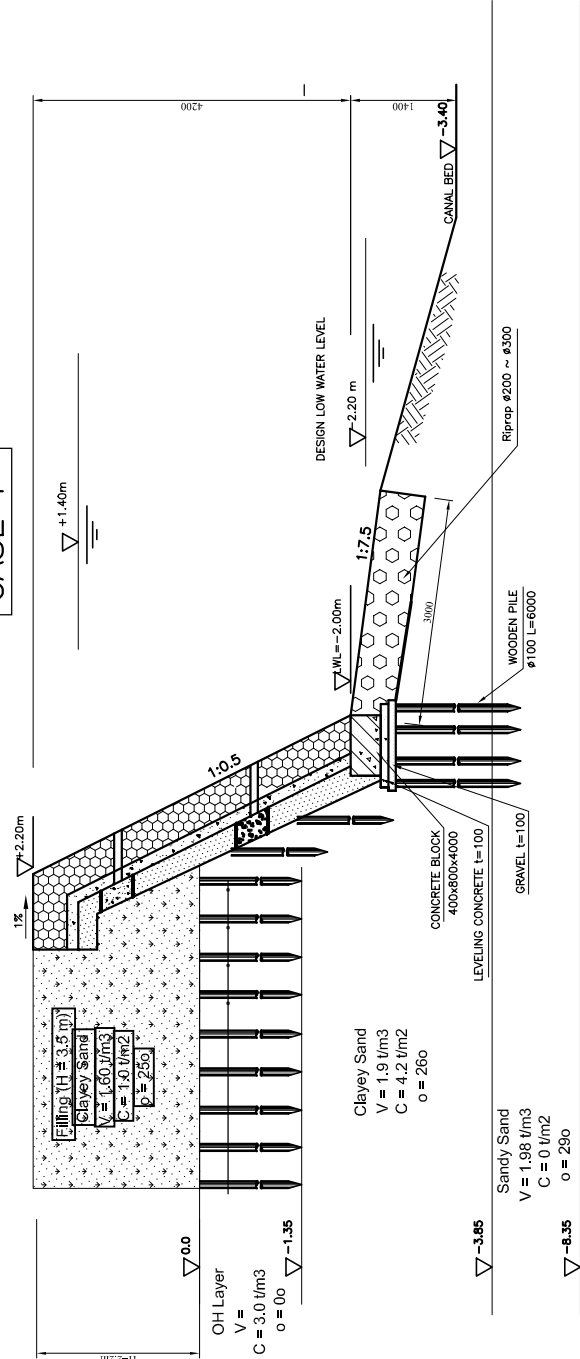


FIG 2.6 PREVENTION WORK FROM SLIDING DUE TO FILLING

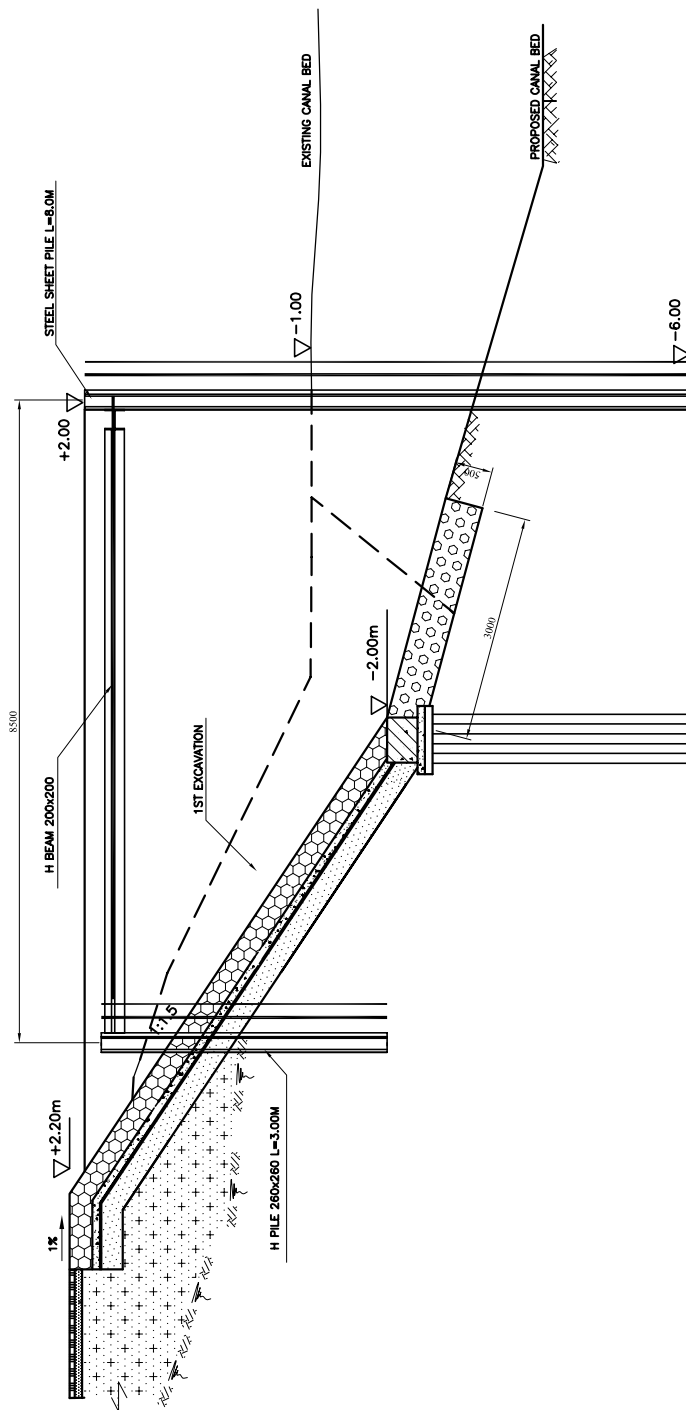
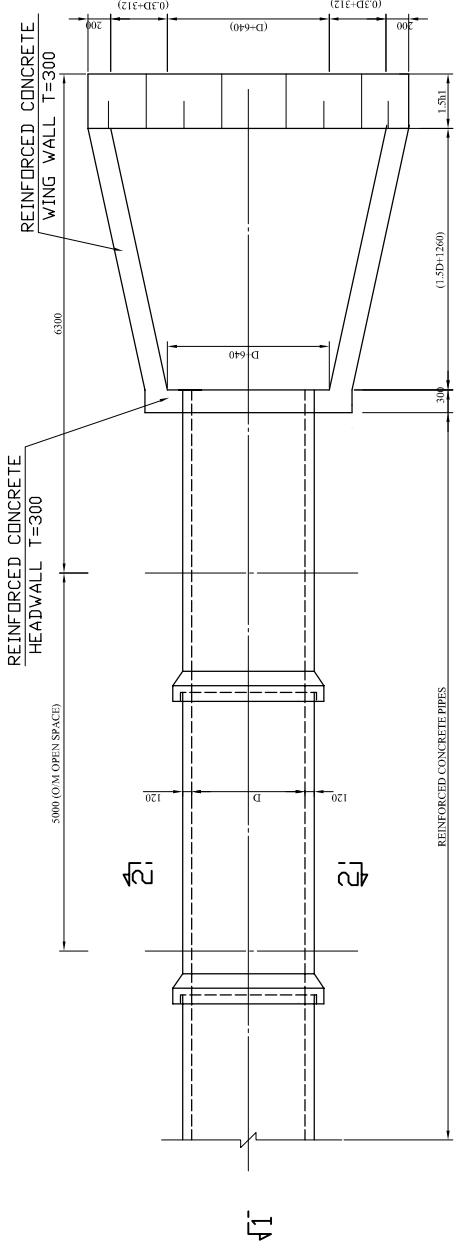
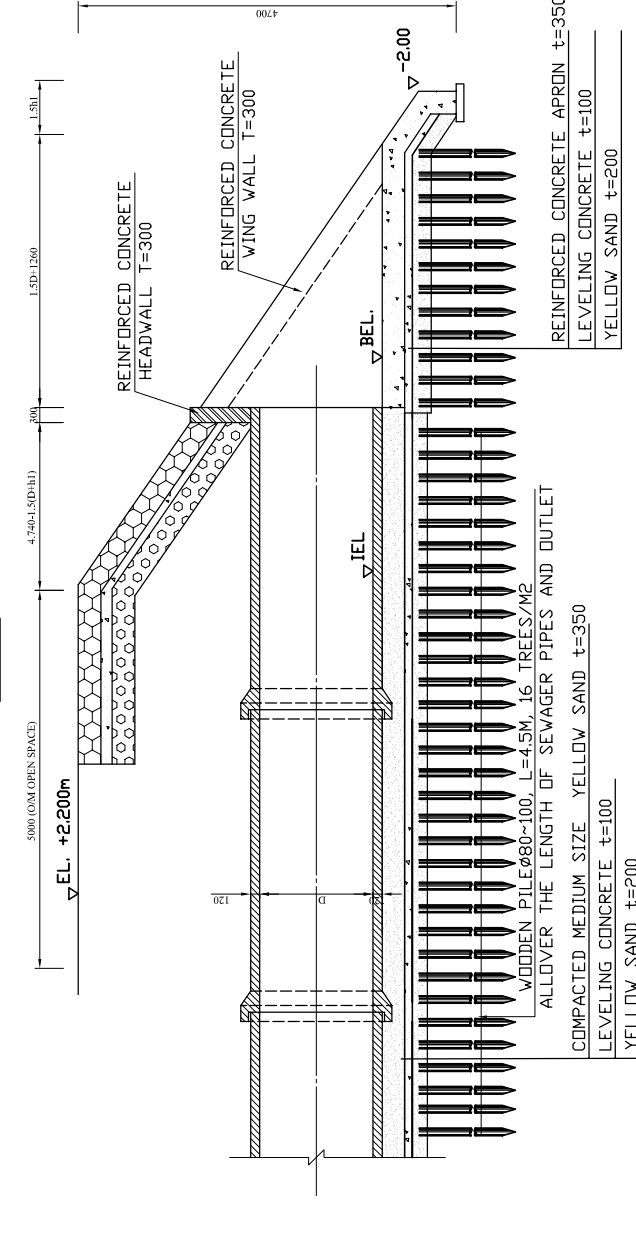


FIG 2.7 COFFER DAM ARRANGEMENT FOR REVETMENT CONSTRUCTION

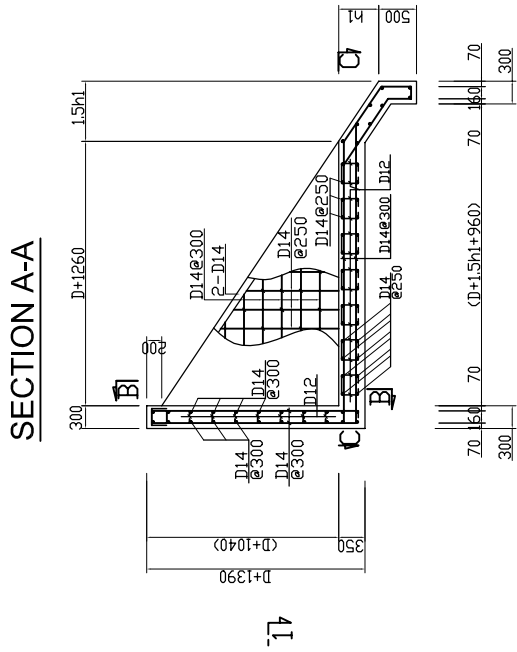
SECTION A-A



PLAN



SECTION 1-1



SECTION C-C

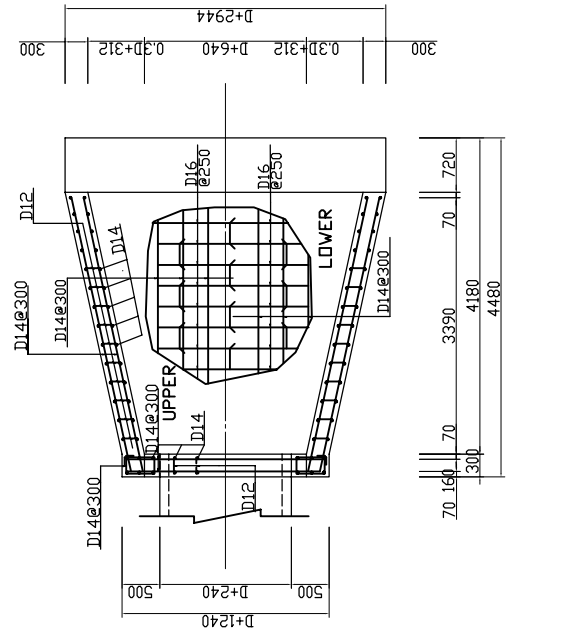
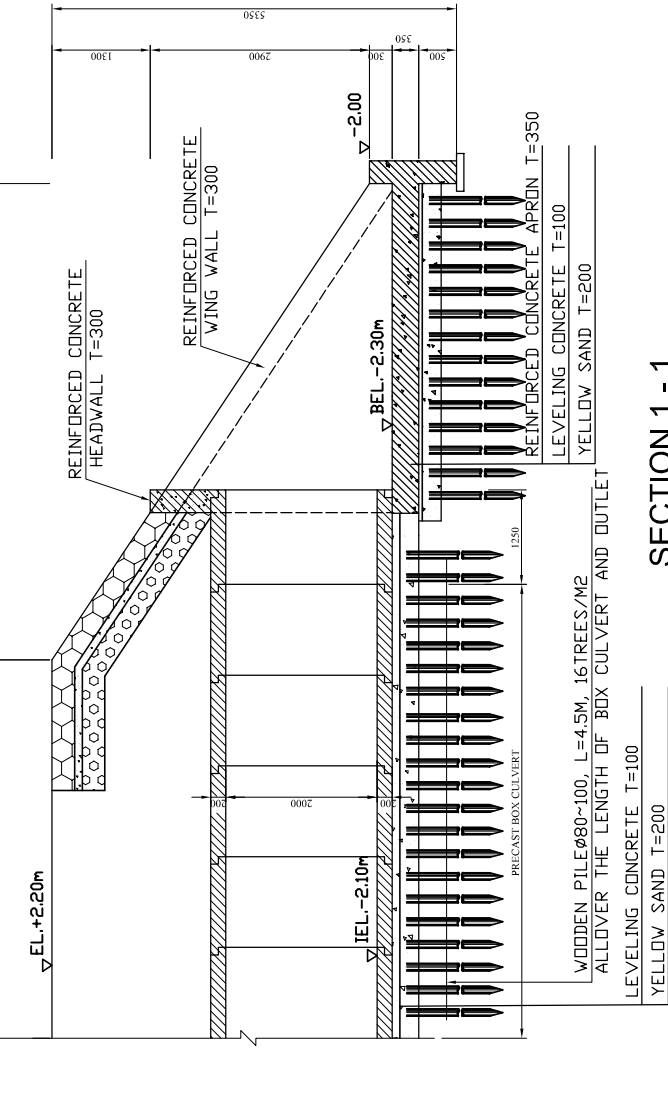
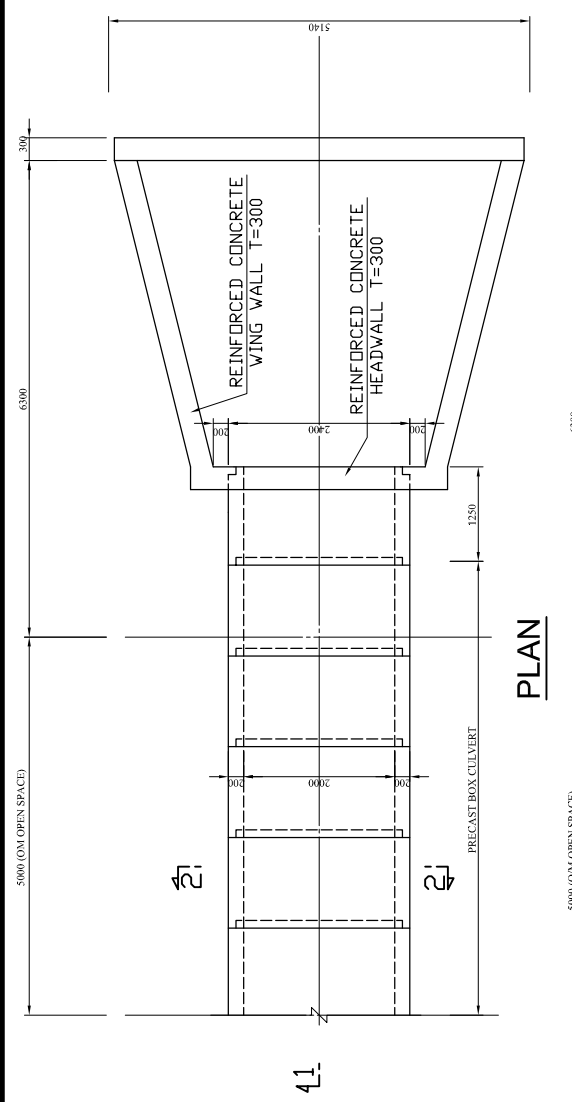
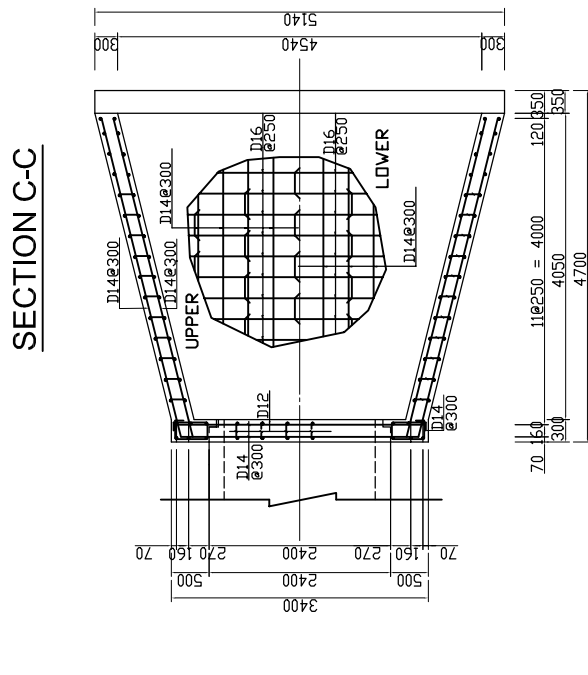
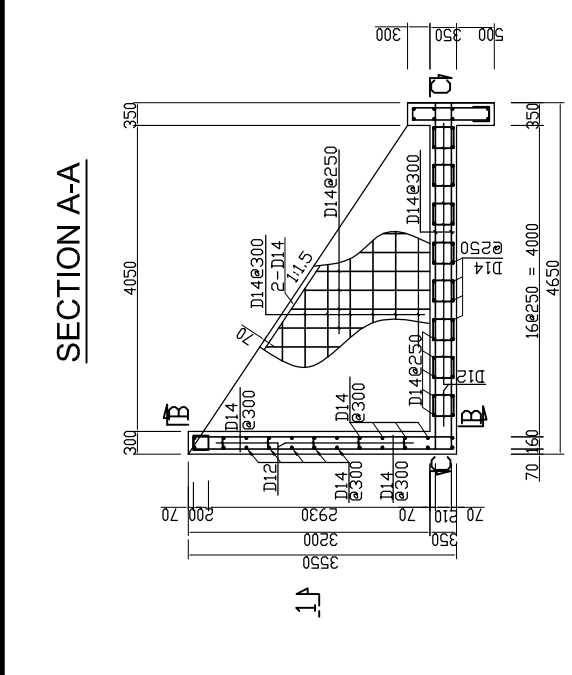


FIG 2.8 TYPICAL SEWER PIPE AND OUTLET



SECTION 1 - 1

FIG 2.9 TYPICAL BOX CULVERT AND OUTLET

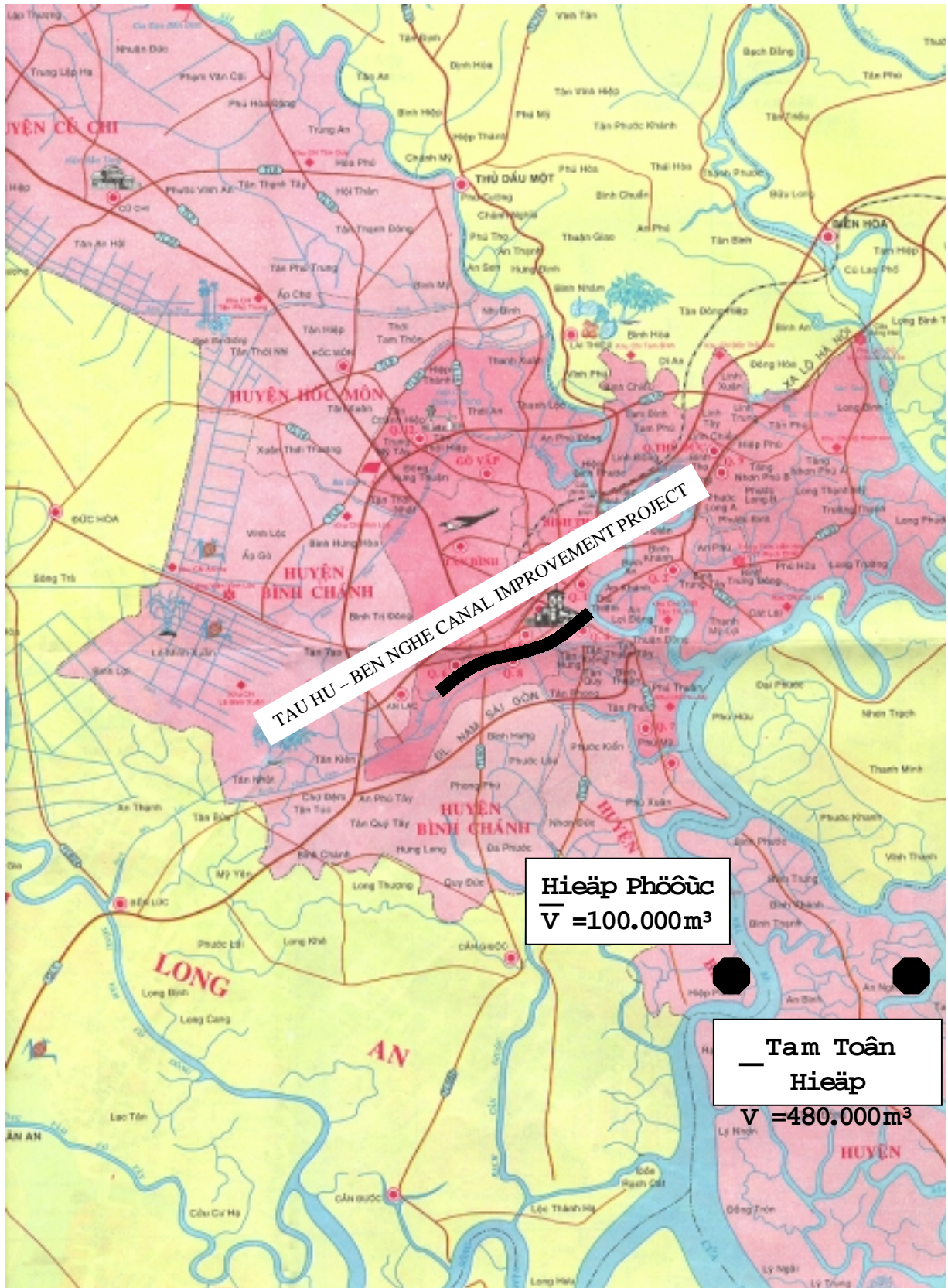


FIG 2.10 LOCATION OF DUMPING SITE FOR TAU HU – BEN NGHE CANAL IMPROVEMENT PROJECT

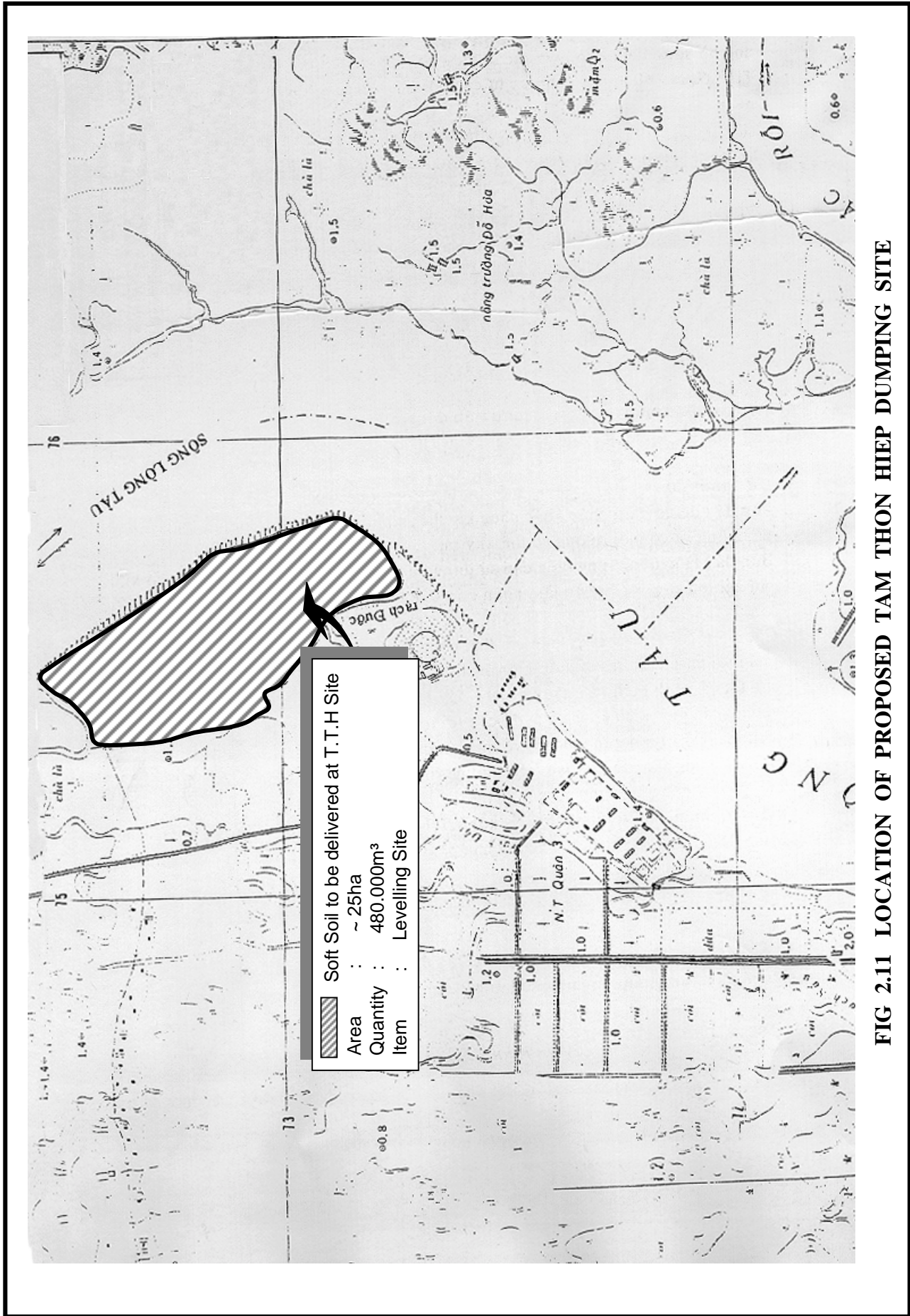


FIG 2.11 LOCATION OF PROPOSED TAM THON HIEP DUMPING SITE

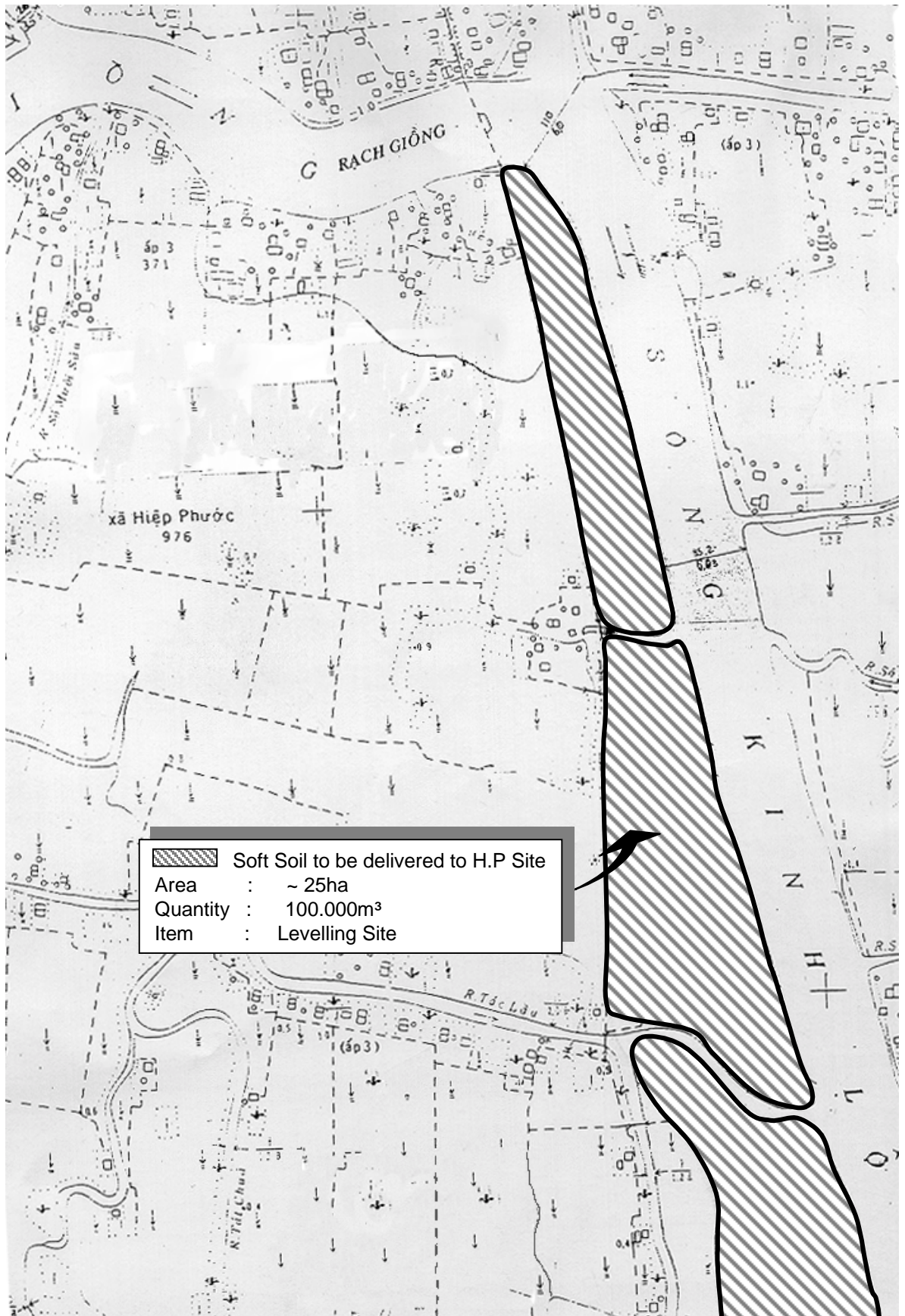
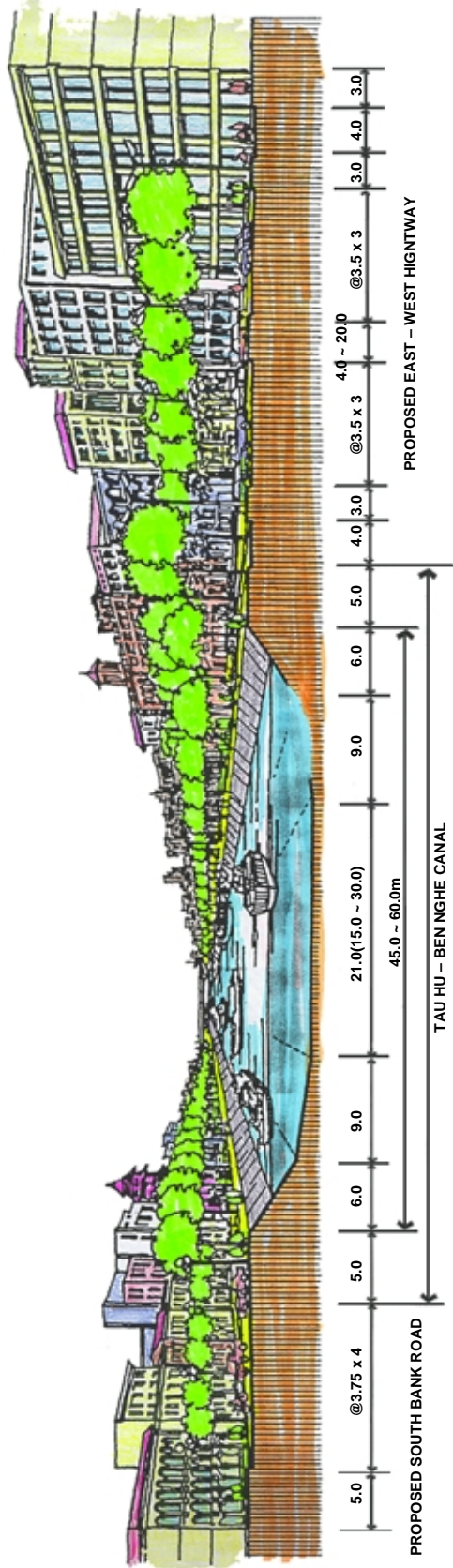


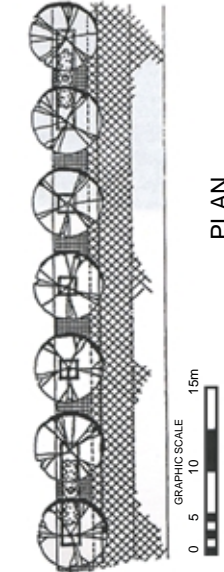
FIG 2.12 LOCATION OF PROPOSED HIEP PHUOC DUMPING SITE



Typical section of Tau Hu - Ben Nghe canal together with future development scheme of the East - West Highway and South Bank Road. Pavement of the canal shall have

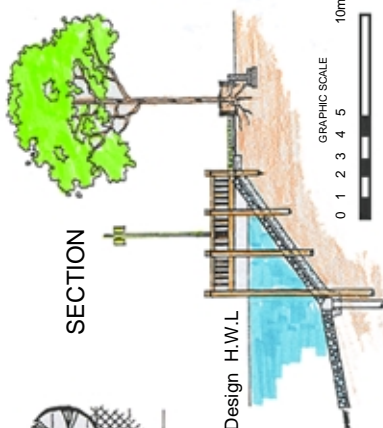
5.0m of maintenance space on the born top, this space shall be used for pedestrian walk as well as sidewalk of the both of the Highway and the Road.

This space shall be furnished with concrete block paving and canopy trees for shadow provision and street lightings.



Planting of canopy trees along the canal

Medium size canopy trees are planned along the revetment born top space, interval of each planting tree location is 8.0m in general for consideration of future growth of the planned trees. 1.8m x 1.8m square planting box shall be applied for each planting location. Each interval of 24.0m rectangular formed planting box is applied as for a visual accent, are there turf grass, flowering lower shrubs are planted. In installed center of this planting space. Street pole light is installed for safety pedestrian's walk as well as aesthetic point of view.



SECTION

Jetty as a temporary work on the revetment

Jetty and pontoon may be facilitated at some priority location of the canal side for supporting tourism activities.

also for various type of loading and unloading commercial activities in time to time during and after the canal improvement project

FIG. 2.13 PROPOSED LANDSCAPE DEVELOPMENT SCHEME ALONG TAU HU - BEN NGHE CANAL

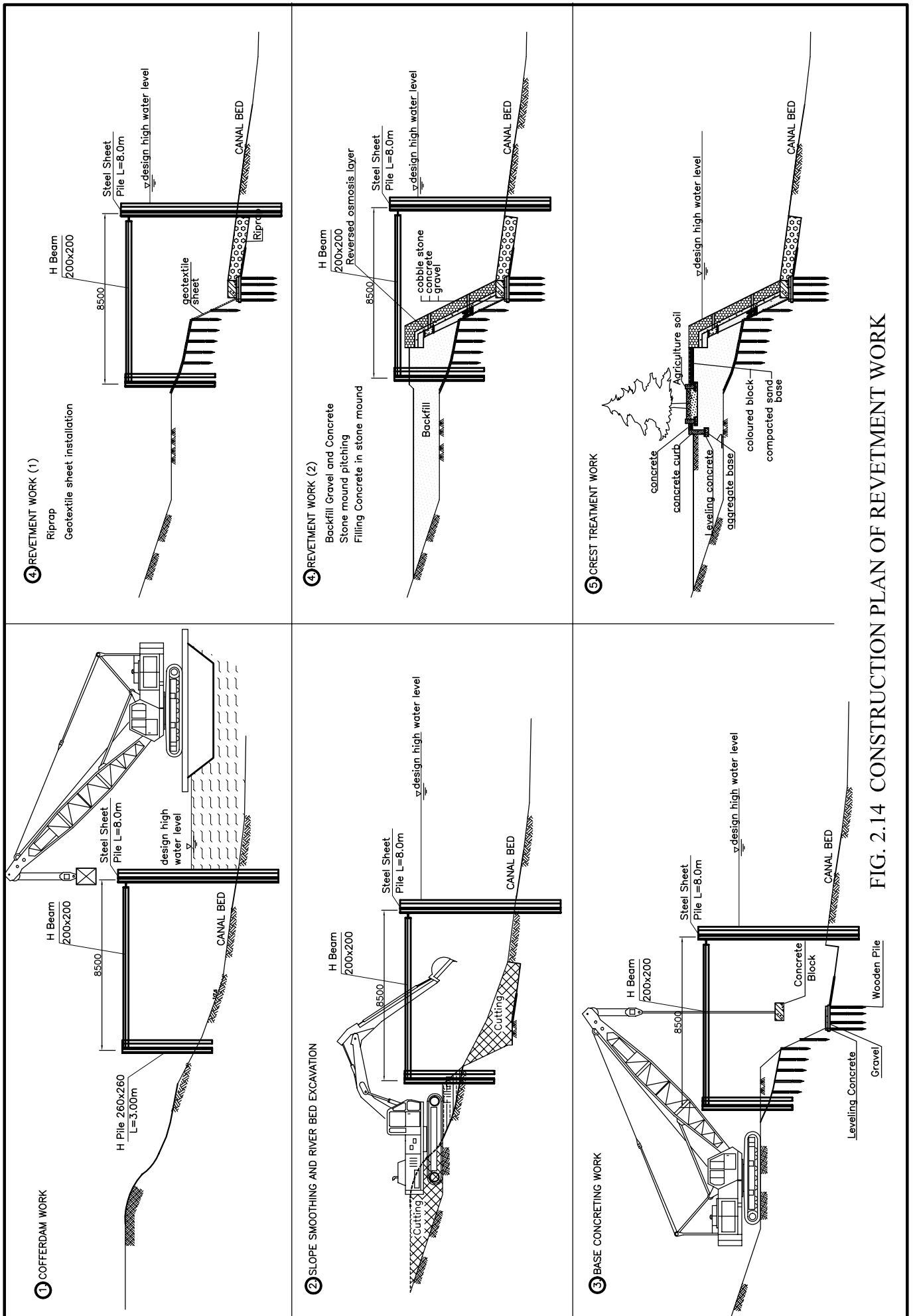


FIG. 2.14 CONSTRUCTION PLAN OF REVELTMENT WORK