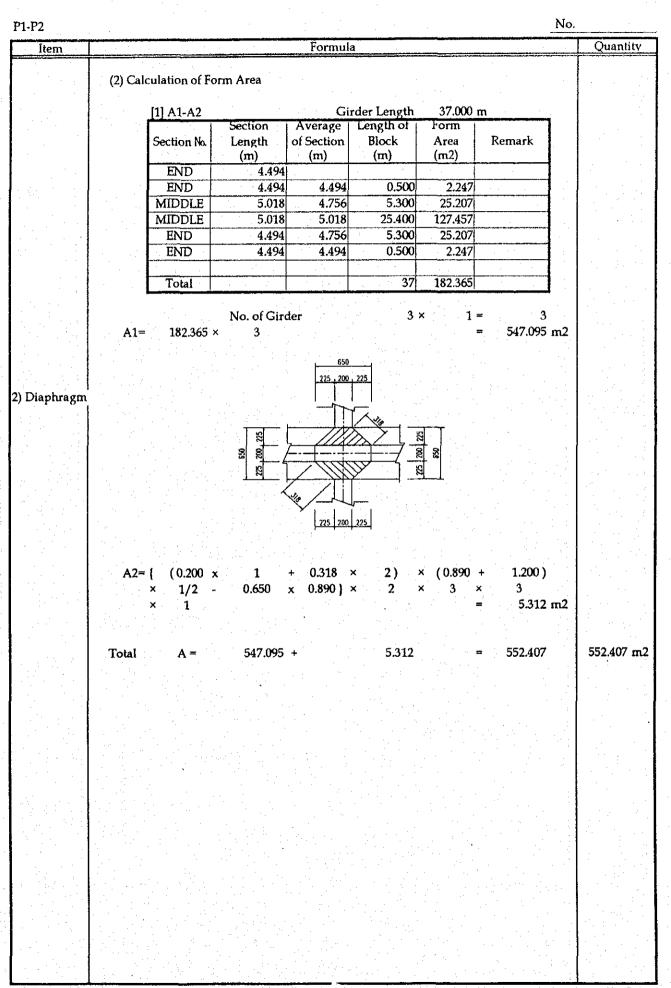
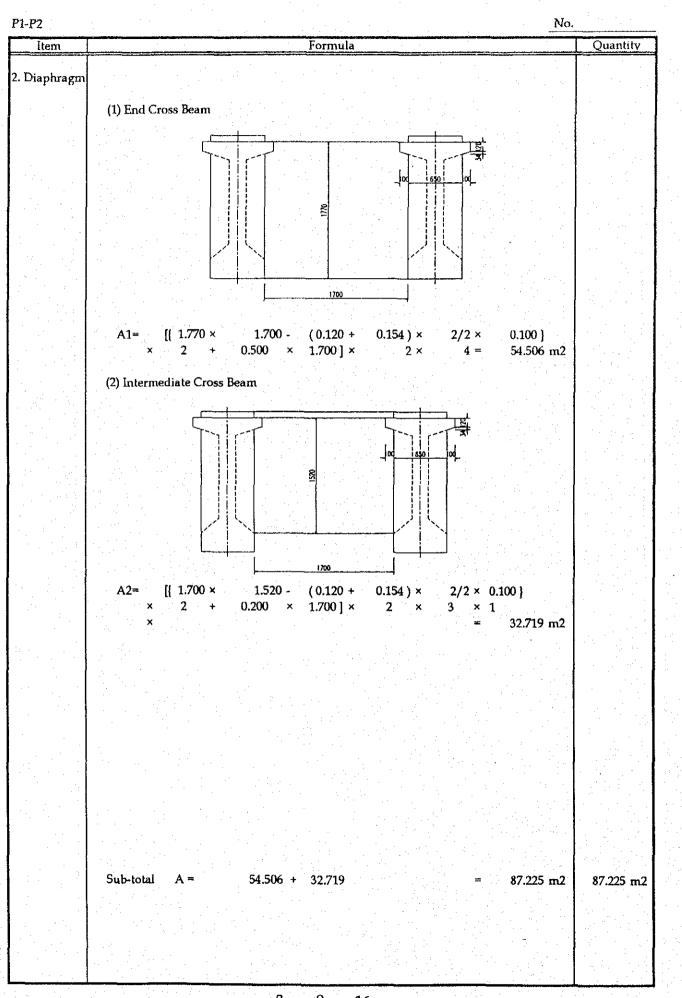
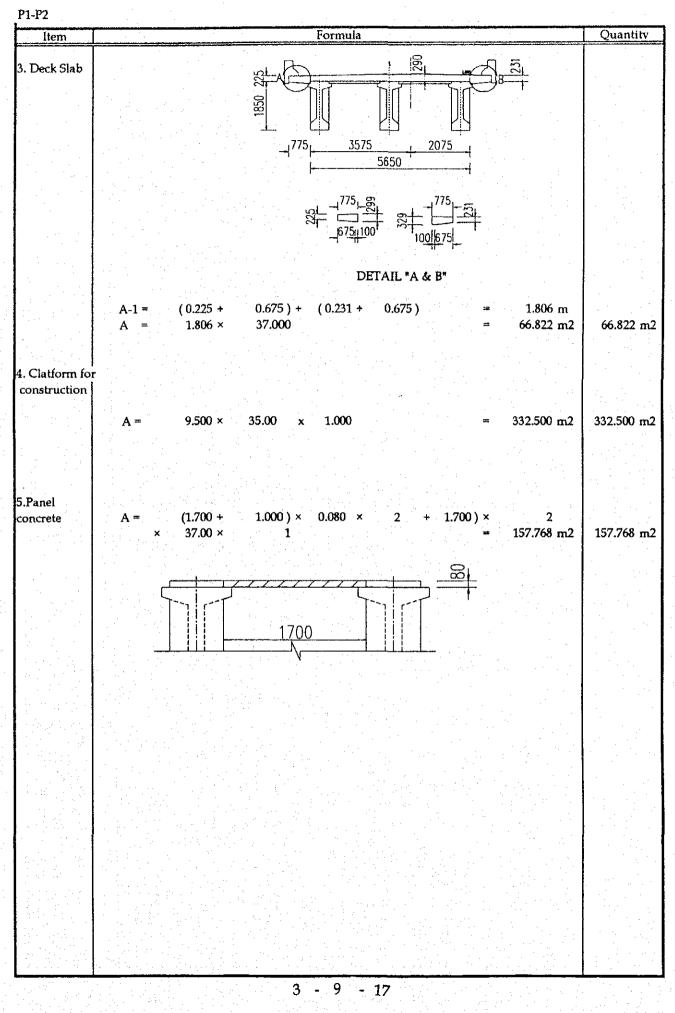
P1-P2				1. J.	No.	·	
Item		Form	nula			Quantity]
Form			-]
1. Girder		and and the second s					
1) Girder	(1) Calculation of Section	inal Area					
	[1] Middle Section	Г ц ь		מערוכ			
	Summary of Sectional	nula	For ONE GI	(m)			
	1 0.080 ×	2		0.160			l
	2 0.120 ×	2	=	0.240			1
	3 0.343 × 4 1.090 ×	2 2		0.686		n n in	
	5 0.301 ×	2		0.602			
	6 0.250 ×	2 + 0.65	50 =	1.150			ļ
			<u> </u>	5.010			
	850		Total Length	5.018 m			
					·		
							1
	325 200	342					
	(()	1090			· · ·		
	225 200						1
	<u> (</u>	50					ļ
te de la composition	6	520					
	(
	<u> </u>						
	[2] End Section						
	Summary of Sectional L	ength nula	For ONE GIF	(m)			
	1 0.080 ×	2	=	0.160			1.
	2 0.120 ×	2		0.240			
	2 0.106 × 3 1.616 ×	2 2 + 0.65	=	0.212 3.882			
		2 + 0.00	~	5.082			
	<u></u>) (1) (1) (1) (1) (1) (1) (1) (1) (1) (1	Total Length	4.494 m			
	00	00					l
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	and 1						
· ·							1
				· · · · · · · · · ·			
		1616					1
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· · ·]							
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	<u> </u>						
	δ	⊻					
		an a					ſ
) - 14				ł



3 - 9 - 15



3 - 9 - 16



. .	Earrada											
Item	:			Formula				Quantity				
PC CABLE			· .			· · ·						
1) 12S12.7(B)	1.A1-P1					For ONE C		ан Тайтаан ал				
· · · · · · · · · · · · · · · · · · ·	CABLE VAR.	EACH	CABLE	EACH	TOTAL	UNIT	WEIGHT					
		LENGTH	NO.		LENGTH	WEIGHT						
	1	27.702	C1	1	27.702	9.290	257.352	1 				
	2	27.746	C2	1	27.746	9.290	257.760					
	3	27.848	C3	1	27.848	9.290	258.708					
				·								
			· · · · · · · · · · · · · · · · · · ·									
	TOTAL			3	83.296		773.820					
				07.00								
	કા	and the second se			ABLES per	•		k and the				
		Wp=	773.820	×	3 ×.	1	ㅋ	2,321.460				
	711	ENSION UN	JTT .					EACH				
	11	Ns=		2 ×	3 ×	1		LACH 18				
		143-		4	J ^	.		10				
			· · · ·									
			· ·									
2) 3512.7												
	PC CABLE O					For A1-P1						
	LOCATION		CABLE	EACH	TOTAL	UNIT	WEIGHT					
		LENGTH	NO.		LENGTH	WEIGHT						
	Connection Or	5.650		8								
I				-	45.2	2.320	104.864					
1					40.2	2.320	104.804					
	TOTAL					2.320						
	TOTAL			8	45.200	2.320	104.864					
	TOTAL					2.320						
	TOTAL					2.320						
	994-0-2044-0-000-0-000-0-0	DTAL WEIG	GHT OF P	8	45.200			k				
	994-0-2044-0-000-0-000-0-0			8 C CABL				k 104.864				
	994-0-2044-0-000-0-000-0-0	OTAL WEIG Wp=	GHT OF P 104.864	8 C CABL	45.200							
	T		104.864	8 C CABL	45.200							
	T	Wp=	104.864	8 C CABL	45.200			104.864				
	T(T)	Wp= ENSION UN Ns=	104.864 NIT 8 *	8 C CABL x	45.200 ES per BRID 1	GE(A1-P1)		104.864 EACH 16				
	T	Wp= ENSION UN Ns=	104.864 NIT 8 *	8 C CABL x	45.200			104.864 EACH 16				
	T T SHEATING	Wp= ENSION UN Ns= F 80/85MN	104.864 NIT 8 *	8 C CABL x	45.200 ES per BRID 1 83.296 x	GE(A1-P1) 3 x	104.864 = = 1 =	104.864 ЕАСН 16 249.888 п				
	T(T)	Wp= ENSION UN Ns= F 80/85MN	104.864 NIT 8 *	8 C CABL x	45.200 ES per BRID 1	GE(A1-P1)	104.864	EACH				
	T T SHEATING SHEATING	Wp= ENSION UN Ns= F 80/85MN F 50/55MN	104.864 NIT 8 *	8 C CABL x	45.200 ES per BRID 1 83.296 x 45.200 x	GE(A1-P1) 3 x 1 x	104.864 = 1 = 1 =	104.864 ЕАСН 16 249.888 п 45.200 п				
	T T SHEATING	Wp= ENSION UN Ns= F 80/85MN F 50/55MN	104.864 NIT 8 *	8 C CABL x	45.200 ES per BRID 1 83.296 x	GE(A1-P1) 3 x	104.864 = = 1 =	104.864 ЕАСН 16 249.888 п				
	TO TI SHEATING SHEATING STEEL SHEA	Wp= ENSION UN Ns= F 80/85MN F 50/55MN AR KEY	104.864 NIT 8 * M	8 C CABL x	45.200 ES per BRID 1 83.296 x 45.200 x	GE(A1-P1) 3 x 1 x	104.864 = 1 = 1 =	104.864 EACH 16 249.888 п 45.200 п				
	TO TI SHEATING SHEATING STEEL SHEA CEMENT GR	Wp= ENSION UN Ns= F 80/85MN F 50/55MN AR KEY	104.864 NIT 8 * M M IEATING	8 C CABL x 2	45.200 ES per BRID 1 83.296 x 45.200 x 6 x	GE(A1-P1) 3 x 1 x	104.864 = 1 = 1 =	104.864 EACH 16 249.888 n 45.200 n 18 s				
	TO TI SHEATING SHEATING STEEL SHEA CEMENT GR 3.14 x	Wp= ENSION UN Ns= F 80/85MN F 50/55MN AR KEY OUT IN SH 0.08 x	104.864 NIT 8 * M M IEATING 0.080 /	8 C CABL x 2	45.200 ES per BRID 1 83.296 x 45.200 x 6 x 249.888	GE(A1-P1) 3 x 1 x	104.864 = 1 = 1 =	104.864 EACH 16 249.888 n 45.200 n 18 s 1.255 n				
	TO TI SHEATING SHEATING STEEL SHEA CEMENT GR 3.14 x 3.14 x	Wp= ENSION UN Ns= F 80/85MN F 50/55MN AR KEY OUT IN SH 0.08 x 0.05 x	104.864 NIT 8 * M M IEATING 0.080 / 0.050 /	8 C CABL x 2	45.200 ES per BRID 1 83.296 x 45.200 x 6 x 249.888 45.200	GE(A1-P1) 3 x 1 x 3 x	104.864 = 1 = 1 = 1 =	104.864 EACH 16 249.888 m 45.200 m 18 s 1.255 m 0.089 m				
	TO TI SHEATING SHEATING STEEL SHEA CEMENT GR 3.14 x	Wp= ENSION UN Ns= F 80/85MN F 50/55MN AR KEY OUT IN SH 0.08 x 0.05 x	104.864 NIT 8 * M M IEATING 0.080 / 0.050 / 12S12.7	8 C CABL x 2	45.200 ES per BRID 1 83.296 x 45.200 x 6 x 249.888	GE(A1-P1) 3 x 1 x	104.864 = 1 = 1 =	104.864 EACH 16 249.888 n 45.200 n 18 s 1.255 n				

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Itam				Formula				Quantity
Item			<u> </u>	cormula				Quality
PC CABLE				1.1	•			
1) 12S12.7(B)	1.P1-P2					For ONE C	IRDER	
	CABLE VAR.	EACH	CABLE	EACH	TOTAL	UNIT	WEIGHT	
	CIEDE FINA	LENGTH			LENGTH	WEIGHT		
	1	36.702	C1	1	36.702	9.290	340.962	
	2	36.714	C2	1	36.714	9.290	341.073	
· · ·	3	36.754	C3	1	36,754	9.290	341.445	
	4	36.784	- <u>C</u>	1	36.784	9.290	341.723	
22	5	36.822	C5	1	36.822	9.290	342.076	
	TOTAL	30.022		5	183.776	9.290	1707.279	
		ليستعدد والمستحد		L	105.770		1/0/.2/7	
	CT		WEICHT		CABIES	BDIDCE/P	(and the second
	50				CABLES per		-12) _	5 101 927
		Wp=	1707.279	×	3 ×	1		5,121.837
	T		אזזידי					TACII
	11	ENSION U		•	•			EACH
		Ns≖	5 ×	2 ×	3 ×	. 1 .	· . •	30
			· . ·			•		
			an a		t de la composición d	•	an an an Array An Anna Anna	
			forgitus e R	1	an a			
) 3S12.7	and a second	n tan san san san Ang san san san san				sa si <u>si si</u> ji	a a transfer	
	PC CABLE OF					For P1-P2		
	LOCATION			EACH	TOTAL	UNIT	WEIGHT	
		LENGTH	NO.		LENGTH	WEIGHT		
	Connection Or	5.650		10	56.50	2.320	131.080	
			· · · · · · · · · · · ·			· · · ·	and the second	
	TOTAL	· ·		10				
	and the second			10	56.500		131.080	
				. 10]	56.500	<u> </u>	131.080	
				<u> </u>	56.500		131.080	
	тс			PC CABI	56.500 LES per BRIE))GE(A1-P4)		1
	т	OTAL WEI Wp=	GHT OF 1 131.080	PC CABI))GE(A1-P4)		1 131.080
		Wp=	131.080	PC CABI))GE(A1-P4)		131.080
		Wp= ENSION U	131.080 NIT	PC CABI x))GE(A1-P4)		131.080 EACH
		Wp=	131.080	PC CABI x))GE(A1-P4)		131.080
	TI	Wp= ENSION U Ns=	131.080 NIT 10 x	PC CABI x	LES per BRIL 1			131.080 EACH 20
		Wp= ENSION U Ns=	131.080 NIT 10 x	PC CABI x)GE(A1-P4) 3 x		131.080 EACH
	TI SHEATING	Wp= ENSION U Ns= F 80/85M	131.080 NIT 10 × M	PC CABI x	LES per BRII 1 183.776 x	3 x	= 1 =	131.080 EACH 20 551.328 r
	TI	Wp= ENSION U Ns= F 80/85M	131.080 NIT 10 × M	PC CABI x	LES per BRIL 1	3 x		131.080 EACH 20 551.328 1
	TI SHEATING SHEATING	Wp= ENSION U Ns= F 80/85M F 50/55M	131.080 NIT 10 × M	PC CABI x	LES per BRIL 1 183.776 x 56.500 x	3 x 2 x	= 1 = 1 =	131.080 EACH 20 551.328 1 113.000 1
	TI SHEATING	Wp= ENSION U Ns= F 80/85M F 50/55M	131.080 NIT 10 × M	PC CABI x	LES per BRII 1 183.776 x	3 x 2 x	= 1 =	131.080 EACH 20 551.328 r
	TI SHEATING SHEATING	Wp= ENSION U Ns= F 80/85M F 50/55M	131.080 NIT 10 × M	PC CABI x	LES per BRIL 1 183.776 x 56.500 x	3 x 2 x	= 1 = 1 =	131.080 EACH 20 551.328 1 113.000 1
	TI SHEATING SHEATING	Wp= ENSION U Ns= F 80/85M F 50/55M AR KEY	131.080 NIT 10 x M	PC CABI X 2	LES per BRIL 1 183.776 x 56.500 x	3 x 2 x	= 1 = 1 =	131.080 EACH 20 551.328 1 113.000 1
	TI SHEATING SHEATING STEEL SHEA CEMENT GRO	Wp= ENSION U Ns= F 80/85M F 50/55M AR KEY OUT IN SH	131.080 NIT 10 × M M	PC CABI x 2	LES per BRII 1 183.776 x 56.500 x 12 x	3 x 2 x	= 1 = 1 =	131.080 EACH 20 551.328 1 113.000 1
	TI SHEATING SHEATING STEEL SHEA CEMENT GRO 3.14 x	Wp= ENSION U Ns= F 80/85M F 50/55M AR KEY OUT IN SH 0.08 x	131.080 NIT 10 × M M HEATING 0.080 /	PC CABI x 2 4 x	LES per BRII 1 183.776 x 56.500 x 12 x 551.328	3 x 2 x	= 1 = 1 =	131.080 EACH 20 551.328 1 113.000 1 36 s
	TH SHEATING SHEATING STEEL SHEA CEMENT GRO 3.14 x 3.14 x	Wp= ENSION U Ns= F 80/85M F 50/55M AR KEY OUT IN SH 0.08 x 0.05 x	131.080 NIT 10 x M HEATING 0.080 / 0.050 /	PC CABI x 2 4 x 4 x 4 x	LES per BRIL 1 183.776 x 56.500 x 12 x 551.328 113.000	3 x 2 x 3 x	= 1 = 1 =	131.080 EACH 20 551.328 1 113.000 1 36 s 2.770 1 0.222 1
	TI SHEATING SHEATING STEEL SHEA CEMENT GRO 3.14 x 3.14 x ANCHOR	Wp= ENSION U Ns= F 80/85M F 50/55M AR KEY OUT IN SH 0.08 x 0.05 x CABLES	131.080 NIT 10 x M M HEATING 0.080 / 0.050 / 12S12.7	PC CABI x 2 4 x 4 x 4 x	LES per BRIL 1 183.776 x 56.500 x 12 x 551.328 113.000 10 x	3 x 2 x 3 x 3 x	= 1= 1=	131.080 EACH 20 551.328 1 113.000 1 36 s 2.770 1 0.222 1 30 s
	TI SHEATING SHEATING STEEL SHEA CEMENT GRO 3.14 x 3.14 x ANCHOR	Wp= ENSION U Ns= F 80/85M F 50/55M AR KEY OUT IN SH 0.08 x 0.05 x	131.080 NIT 10 x M M HEATING 0.080 / 0.050 / 12S12.7	PC CABI x 2 4 x 4 x 4 x	LES per BRIL 1 183.776 x 56.500 x 12 x 551.328 113.000	3 x 2 x 3 x	= 1= 1= 1= 1=	131.080 EACH 20 551.328 1 113.000 1 36 s 2.770 1 0.222 1

P2-A2

ltem				Formula				Quantity
No cum -						· · · · · · · · · · · · · · · · · · ·		
PC CABLE 1) 12S12.7(B)	1.P2-A2			· · ·		For ONE C	משרוקו	
(0)/ 21021 (1 	CABLE VAR.	EACH	CABLE	EACH	TOTAL	UNIT	WEIGHT	
		LENGTH	NO.	j [LENGTH	WEIGHT	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	
. [1	27.702	C1	1	27.702	9.290	257.352	
	2	27.746	C2	1	27.746	9.290	257.760	
	3	27.848	C3	1	27.848	9.290	258.708	
	TOTAL		<u> </u>	3	83.296	<u></u>	773.820	
	SI	IR.TOTAT	WEICHT		ABLES per	מ) שייירווקא	4.21	ka
		Wp=	773.820		3 ×		±	kg 2,321.460
		· P	//0.020		.			2,521,100
	Т	ENSION UN	VIT					EACH
		Ns=	3 ×	2 ×	3 ×	. 1 1	=	18
				1.5 1.1			engan di Ka	
				÷				
			and a star of the		e de la composition de la comp			
				e de la composition de la comp				
2) 3512.7		n an an Arta. An tha an Arta						
	PC CABLE O	F DIAPHRA	GMS			For P2-A2		
	LOCATION	EACH	CABLE	EACH	TOTAL	UNIT	WEIGHT	
		LENGTH	NO,		LENGTH	WEIGHT		
	Connection Or	5.650	<u></u>	. 8	45.20	2.320	104.864	
	TOTAL			8	45.000		104.964	
				<u> </u>	45.200		104.864	
- 1 ⁰ - 1	· · ·		e di terese			and the second		
	T	OTAL WEIG	CHT OF P	C CABL	ES per BRID	GE(A1-P1)		kg
			104.864				=	104.864
				·				
	Т	ENSION UN				un de la deservation Notae de la deservation		EACH
		Ns=	8*	2			7	16
	SHEATING	E 80 / 95 M	J.	an san Nga	82 204	3		240 890
	JUEATING		ана стала АТ — стала стала		83.296 x	3 x	1=	249.888 m
	SHEATING	F 50/55M	ví		45.200 x	1 x	1=	45.200 m
		1 00/ 001411	T4 ·		чэ.400 X	**	· · · · · ·	-1J.200 III
	STEEL SHEA	AR KEY			6 x	3 x	1 =	18 set
					~~	~~~		
· · .]	CEMENT GR	OUT IN SH	EATING					
		0.08 x		4 x	249.888		=	1.255 m3
	3.14 x	0.05 x	0.050 /	4 x	45.200		=	0.089 m3
	ANCHOR				6 x	3 x	1 =	18 set
		CABLES	3S12.7		8 x	1 x	1 =	8 set
					an an Christian An Christian			
		1	1. Contract (1997)	1. J. S.	e de la companya de l	1	12 M 14 1 1 1 1 1 1 1 1	and the second

No.

9 - 20 3 -

Item	Formula		Quantity
ETC.	1. JOINT		
	EACH LENGTH		
	L = 6.500		·
	EACH = 4		
			·
· · · · ·	TOTAL LENGTH		· · ·
	$L = 6.500 \times 4$		26 m
	2. BEARING PAD ELASTOMERIC 600x300x57		
	EACH for One SPAN		
	EACH = 3		
	TOTAL EACH		
			EAC
	$EACH = 3 \times 2$ ELASTOMERIC 500×250×50	·] =	6
	EACH for One SPAN		
	EACH = 3		
	TOTAL EACH		
			EAC
	$EACH = 3 \times 4$		12
	3. ANCHOR BAR		· .
	EACH for One SPAN		
			EAC
	EACH = 12		12
	TOTAL EACH (FIX)		
	$EACH = 2 \times 4 = 8$		
			-
	TOTAL EACH (MOVE)		
	$EACH = 2 \times 2 = 4$		la estador General
	4. PAVEMENT	:	
	a. WATER PROOFING 5MM 6.5 X 93.5	≕ 608 m2	608 m
	b. WATER PROOFING 70MM 6.5 X 93.5	$= 608 \text{ m}^2$	608 m
			r Ar an an an
	[철 제 비슷 소문은 소문을 가지 않는 것 ㅋㅋ ㅋㅋ		n de la composition de

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3 - 9 - 21

A1-P1

No.

			·		·	1.1						For ON	E GIRDER
				SCHE	DULE	OF RE	INFOR	CEME	ENT (OF G	IRDER)	· . ·		
				-					LENGTH	UNIT	NO.		
BAR	SIZE		. 1	DIME	VSION	(5 (mm)	÷.,			WEIGHT	OF	WEIGHT	REMARKS
MARK	(mm)	a	b	с	d	e	f	g	(mm)	(kg/m)	BARS		
G1	14	190	1757	117	1757	190			4011	1.208	138	668.9	
G2	14	190	1757	<u>342</u>	1757	190			4236	1.208	56	286.7	Average
G3	14	190	1757	567	1757	190			4461	1.208	6	32.3	
G4 :	14	190	1757	567	1757	190	-		4461	1.208	- 4	21.6	
G5	14	200	238	154	567	154	238	200	1751	1.208	148	313.2	
G6	14	200	42	759	42	200		· · .	1243	1.208	148		
G7	· 14	200	301	183	567	183	301	200	1935	1.208	86	201.1	and
C8	- 14	200	<u>151</u>	183	567	183	<u>151</u>	200	1635	1.208	56		<u>Average</u>
G9	14	389	567	389					1345	1.208	6	9.8	the state of the
G10	22	8695	10450	8695	1	: · ·			27840	2.984	6	498.5	
G11	14	8695	10450	8695					27840	1.208	20	672.8	
G12	14	8700	10450	8700					27850	1.208	10	336.5	
G13	14	100	338	151	302	100			991	1.208	20	24.0	
G14	14	200	800						1000	1	12		<u> </u>
G15	16	1600							1600		50		Interior
G16	16	1100				[.]			1100	1.578	50		Exterior
G17	10	570				·			570	0.617	12	4.2	<u></u>
G18	10	150							150	0.617	40		
G19	14	361	364	567	364	361			2017	1.208	138		
Total	C	an	Interio	r gride	er	·						3883.2	
TULAL		all	Exterio	or gird	er							3843.7	

3) Total Weight

Span	Int/Ext	Nos.	Weight/C	Total	Remark
A 1 D1	Interior Beam	· 1	3883.21	3883.21	
A1-P1	Exterior Beam	2	3843.75	7687.50	
					a de la composición d
	Total			11570.70	

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P1-P2

No.

	•••				÷ * ,	·		÷		· ·			For ON	E GIRDER
					SCHE	DULE	E OF R	EINFC	DRCE	MENT (O	F GIRDER	.)		
				· .		a store				LENGTH	UNIT	NO.		т.
BAI	R	SIZE	1.1.1	L	DIMEN	ISION	S (mm)			WEIGHT			REMARKS
MAF	ĸ	(mm)	a	b	с	d	e	f	g	(mm)	(kgf/m)			
G1	1	14	190	1957	117	1957	190		2	4411	1.208	184	980.8	· · · · ·
G2		14	190	1957	<u>342</u>	1957	190			4636	1.208	- 74	**************************************	Average
G3		14	190	1957	567	1957	190	· .		4861	1.208	6		
G4	÷.	14	190	1957	567	1957	190			4861	1.208	- 6	35.2	
G5 -		14	200	238	154	567	154	238	200	1751	1.208	196	414.7	
G6		14	200	42	759	42	200			1243	1.208	196	L	
G7		14	200	301	183	567	183	301	200	1935	1.208	122	285.3	
G8 -	1	14	200	<u>151</u>	183	567	183	<u>151</u>	200	1635	1.208	. 74	<u></u>	<u>Average</u>
G9		14	389	567	389		E.J.		· .	1345	1.208	6	9.8	
G10	·.	22	6945	7450	7950	7450	6945			36740	2.984	6		· · · ·
G11	1.	14	6945	7450	7950	7450	6 94 5	14		36740		22	976.7	
G12	÷	14	6950	7450	7950	7450	6950	N (36750	1.208	12	1	
G13		14	100	318	151	318	100		•	987	1.208	30	<u></u>	
G14		14	200	800			4 A.			1000	1.208	12	14.5	1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1
G15		16	1600							1600	1.578	74		Interior
G16		16	1100		1.1					1100	1.578	74	L	Exterior
G17		10	570							570	0.617	24		
G18	1	10	150							150	0.617	104	9.6	
G19	- /	14	361	364	567	364	361			2017	1:208	184	448.5	
Tota	 _1	c.		Interio	or grid	ler			1.1				5487.3	
	а н	l st	ban	Exteri	or gire	der	. <u>.</u>						5428.9	

3) Total Weigl	ht						
Ŭ	Span	Mid/Side	Int/Ext	Nos.	Weight/G	Total	Remark
		C.1. C	nterior Bean	1	5487.31	5487.31	
	P1-P2	Side Span	xterior Bear	2	5428.91	10857.82	
	Total					16345.13	
		<u>h</u>					

P2-A2

No.

1. 				1 				- 1	1	1	÷ .	FOFUNI	: GIRDER
			S	CHEE	ULE (OF RE	INFO	RCEM	ENT (OF	GIRDER)			
-									LENGTH	UNIT	NO.		
BAR	SIZE		D	MEN	SIONS	(mm)		:	LEINGIT	WEIGHT	OF	WEIGHT	REMARKS
MARK	(mm)	a	b	с	d	e	f	g	(mm)	(kgf/m)	BARS	(kgf)	
G1	14	190	1757	117	1757	190			4011	1.208	138	668.9	
G2	14	190	1757	<u>342</u>	1757	190			4236	1.208	56	286.7	<u>Average</u>
G3 **	14	190	1757	567	1757	190			4461	1.208	6	32.3	1. 1
G4 :	14	190	1757	567	1757	190			4461	1.208	4	21.6	
G5	14	200	238	154	567	154	238	200	1751	1.208	148	313.2	
G6	14	200	42	759	42	200			1243	1.208	148	222.3	
G7	14	200	301	183	567	183	301	200	1935	1.208	86	201.1	
C8	14	200	<u>151</u>	183	567	183	<u>151</u>	200	1635	1.208	56	110.6	<u>Average</u>
G9	14	389	567	389					1345	1.208	6	9.8	
G10	22	8695	10450	8695					27840	2.984	6	498.5	
G11	14	8695	10450	8695		19	×.		27840	1.208	20	672.8	
G12	14	8700	10450	8700		- 4 - B - 1			27850	1.208	10	336.5	
G13	14	100	318	151	318	100	1		987	1.208	20	23.9	
G14	. 14	200	800						1000	1.208	12	14.5	
G15	16	1600	н						1600	1.578	50	126.3	Interior
G16	16	1100					5 J.		1100	1.578	50	86.8	Exterior
G17	10	570							570		12		
G18	10	150							150	0.617	40	the second s	
G19	.14	361	364	567	364	361			2017	1.208	138		
Total	S	pan	Interio							St. West	14.4	3883.1	
10.01			Exterio	r gird	er							3843.7	

3) Total Weight

Span	Int/Ext	Nos.	Weight/C	Total	Remark
P2-A2	nterior Bean	1	3883.11	3883.11	Alt de serte
r <i>1</i> -A1	xterior Bear	2	3843.65	7687.30	
		1. A. A.		in al char	
	Total	· ·		11570.41	

No.

A1-A2

Par EACH

- 10 A	•		S	CHED	ULE O	F REII	NFOR	CEMENT	' (OF DIAP	HRAGM)		
(1) END [MAPH	RAGM	1										
BAR	SIZE		· · · · · · · · · · · · · · · · · · ·	DIMI	ENSION	JS (mi	n)		LENGTH	UNIT WEIGHT	NO. OF	WEIGHT	REMARKS
MARK	(mm)	a	b	с	d	е	f	g	(mm)	(kg/m)	BARS	(kg)	
G1	16	1850						· _	1850	1.578	32	93.4	
G2	14	198	1759	410	1759	198			4324	1.208	14	73.1	
· · · · · · · · · · · · · · · · · · ·	2	e di				1.1							
Sub-Tota	1				1.2				1. A.	and the second		166.6	
(2) INTER	MEDI	ATE D	IAPH	RAGN	[·			1.1.5		
BAR	SIZE	· · ·		DIMI	ENSION	√S (mi	n)		LENGTH	UNIT WEIGHT	NO. OF	WEIGHT	REMARKS
MARK	(mm)	a	b	с	d	e	f	g	(mm)	(kgf/m)	BARS	(kgf)	
(+) INTER	MEDI	ATE D	IAPH	RAGN	1 (OF G	IRDE	R L=2		1				
H1		1850		1.2.1					1850	1.578	28	81.7	
H2	14	198	1509	110	1509	198			3524	1.208	14	59.6	
a a a tra	-	·										141.3	
(+) INTER	MEDI	ATE D	IAPH	RAGM	1 (OF G	IRDE	R L=3	7M)					
K1	16							-	1850	1.578	32	93.4	
K2	14	198	1709	110	1709	198			3924	1.208	14	66.4	
ja di se												159.8	
												a de la composición d	
Sub-Tota	1											301.1	

3) Total Weight

SECTION			Nos.	Weight/EACH	Weight
END DIAPHRAGM	e tra de letra		4	166.6	666.2
(+) INTERMEDIATE DIAPHRAGM	(OF GIRDE	R L=28M)	4	141.3	565.4
(+) INTERMEDIATE DIAPHRAGM	(OF GIRDE	R L=37M)	3	159.8	479.4
	la r				
			· ·		
Total					1711.0

1	· · ·	1 A. A.			* .				1. St. 1		Par Br	NDGE
			SCH	IEDUL	EOF	REIN	FORC	EMENT (C	F DECK SI	LAB)		· · · ·
· · · · ·								LENGTH	UNIT	NO.		
BAR	SIZE		DIME	NSIO	NS (m	m) –	2 <u>1</u>	LEINGTH	WEIGHT	OF	WEIGHT	REMARKS
MARK	(mm)	a	b	c	d	e	ſ	(mm)	(kg/m)	BARS	(kg)	
S1	20	7080						7080	2.466	592	10335.9	
52	20	7080						7080	2,466	592	10335.9	
S3	22	7000	5000					12000	2.984	34	1217.5	
S3'	22	5000	7000					12000	2.984	32	1145.9	
S4	22	12000						12000	2.984	66	2363.3	
S5	14	27638						27638	1.208	66	2203.5	
S6	14	24812						24812	1.208	34	1019.1	
S6'	14	22364				· · · · ·		22364	1.208	32	864.5	
S7	25	7080			: : :			7080	3.853	35	954.8	
S8	25	7080					1	7080	3.853	35	954.8	
S9	14	12000	6000	6606				24606	1.208	34	1010.6	
S9'	14	12000	10170					22170	1.208	32	857.0	
S10	12	210	155	210				575	0.888	4856	2479.5	
		· · · · · · · · · · · · · · · · · · ·										
Total	<u>†</u> -										35742.2	

QUANTITY TABLE OF ABUTMENTS

ITEMS	,		UNIT		ABUTMENT	TOTAL
		1	<u> </u>	A1	A2	
A- ABUTMENT						
	NUMBER OF PILES		PILE	24.0		48.
	TOTAL LENGTH OF RC PILES D450MM	<u> </u>	M	960.0	960.0	1920.
	CONCRETE CLASS D		M3	195.2	195.2	390.
		D32	KG	122.4	122.4	244.
PILE		D28	KG	1411.2	1411.2	2822.
		D22	KG	26697.6	26697.6	53395.
		D16	KG	1596.0	1596.0	3192.
		φ6	KG	4413.6	4413.6	8827
the second s		TOTAL	KG	34118.4	34118.4	68236.
	CONCRETE CLASS E	7	M3	146.3	146.3	292
		D25	KG	2724.8	2724.8	5449.
		D22	KG	1711.2	1711.2	3422
	REINFORCEMENT	D20	KG	2786.1	2776.0	5562.
ABUTMENT	ABUTMENT			1892.3	1907.5	3799.
		D14	KG	838.0	814.9	1652
		D10	KG	84.3	82.3	166.
		TOTAL	KG	10036.7	10016.7	20053.
	LEAN CONCRETE CLASS G		МЗ	3.9	3.9	7.
	BLINDING STONE		M3	7.7	7.7	15
	EXCAVATION		M3	486.8	461.7	948.
	FILLING		M3	392.9	367.8	760.
- APPROACH SLAB		<u> </u>				
	CONCRETE CLASS E	····	M3	12.1	12.1	24.
	LEAN CONCRETE CLASS G		M3	3.7	3.7	7.
a Maria a Maria a Barat Maria a Maria	ASPHANTIC JOINT FILLER T=20MM		МЗ	0,1	0.1	<u>.</u> 0.
		D20	KG	938.1	938.1	1876.
		D16	KG	975.5	965.3	1940.
	REINFORCEMENT	D10	KG	69.4	69.4	138.
		TOTAL	KG	1983.0	1972.8	3955.
- SLOPE PROTECTION	I LE CARTA CARA A A					
	STONE MASONRY T=300MM		M3	231.3	139.9	371.
	BLINDING AGGREGATE T=100MM	and the second	МЗ	77.1	46.6	123.
	GEOTEXTILE		M2	268.0	176.0	444.
	PVC PILE #50MM DIA., L=1000MM		M	40.0	27.5	67.
	EXCAVATION	· · · ·	M3	756.0	519.0	1275.
	FILLING		МЗ	582.0	400.0	982.
	WOODEN PILE L=3M	T	M	4763.0	3272.0	8035.
FOOTING	BLINDING STONE		M3	6.4	6.4	12.
An and the second second	STONE MASONRY T=300MM		M3	28.6	28.6	57.

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No.

ltem			Formula				a series en esta	Quantity
	1.							
1) Concrete								
	+ PaslaMall							
	* BackWall	7 50	0.05		, et to			
4 - 4 - ₁	v1 = (x	7.50 x 0.05 /	2.05 2.00	· · +	7.50 0.40	× <u>×</u>	6.23 m3	
	Ŷ	0.05 /	2.00) x	0.40	. –	0.23 115	
and the second sec	* Frontwall		e e di					· · ·
	v2 =	7.50 x (3.49	x	1.50	-	0.1 ^2	
				1	2	`) ≖	39.17 m3	
					· · ·			
	* Corbel	i en			n an tha she Na she she she		· · · ·	
	v3 =	0.30 x(0.30	+	0.60	:)/	2	
				x	6.50	-	0.88 m3	
	* Haunch		0.50	1.1				
	v4 =	5.54 x	0.50	x	0.50	· /	2 x 2	
	#14/1			e g		. **	1.38 m3	
	*Wingwall v5 ={	2.40 x(5 54	•	5 60	1.57		
	, v⊃ ~(2.40 x(1.80 x(5.54 1.05	+ +	5.50 2.51)/	2 2 }	
		1.00 X	0.50	x	2.51		16.45 m3	
na an a	* Parapet			. ^	•		10.40 110	
	v7 =(0.50 x	0.30	+	0.20	^2		
		0.15 ^2/	2)x	4.60	×	2	
					1.64		1.64 m3	
and the second secon	*Footing				1.		· · · · ·	
	v6 =	7.50 x	5.40	x	2.00			
		4 x	pi	x	0.60	^2 x		
					1.4	· 💻	80.55 m3	
							Total	146.3 m3
) Form	- D 1347 3				en e			
	* BackWall a1 =	2.00	0.0E		7 50	e frank		
	ai -	2.00 x 2.05 x (2.05	× +	7.50 0.50		cos 8° 2	
		2.05 X (0.50	.	0.00)×	26.95 m2	
	* Frontwall		n n Talena			1.1	20.55 1112	
	a2 =	7.5 /	nos 8º	× (3.49	+	3.56)	
	(0.5 +	0.5) x	3.56		2	
and the second second second	+	0.5 + 3.52 x	1.5	x	2	× .	56.80 m2	
					, en start			
	* Corbel				1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 -			
	a3 = {	0.3 + (0.3	^2+		^2)^		· · · · ·
		X 200	6.5	1	cos 8º	*	4.75 m2	
	* Haunch							
	a4 =	+(0.5	. ^2+		^2)^		
	*Wingwall	×	5.54	×	2		7.83	
	a5 =	2 × (2.4		5.535		5.50)×1/2	
	aj -		1.8	×(×(1.05	· + · ·	2.51)x1/2	
	+	0.5 x (1.05	+	2.31	+ '	3.00)	
			0.5	x	5.54)=	66.63 m2	
	* Parapet					1		
	a5 = {	4.6 x(0.5	· +	0.3	+	0.05	
		+	1.4142	x	0.15	+	0.05)	
		+	2	x		} x {	2	
			n de la composición d	r sini.			10.95 m2	
	*Footing					÷ js		
	a6 =	2 x(7.500	1	cos 8º	+	4.50)	
				×	. 2	-	48.29 m2	
	1 · · ·						Total	222.2 m2

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No.

ltem	1		Formula					Quantity
3) Scaffolding:	* H< ≃4 m		· · ·		· ·		· · · ·	
o) ocatioiung.	A2 ={	2 x(7.50	1	cos 8°	+	5.40)+	
				+		8)x	2 =	
		· · ·				,		
	* 4m< H<=30m		144 B. A.				an a	
	A2 =	((7.5/ cos8°+2) + (4	12+1.5+2)	+ (0.5	+2) + (4.2+1	1-1)		
		+(7.5/cos8°-2 x 1.					2)) x (2.05+3.485) 237.7 m2
		e da de la seconda de la s	· · ·		÷ .			
4) Support	· =(7.98 -	1.55	+	4. 99) x	1.80 /2	2
		x	0.50	. ×	2		 	10.3 m3
		-	e de la companya de la		· ·			
5) Lean Concrete	* Concrete class		7 CO				· .	
	• V =	0.1 x ((5.40 +	7.50	+	0.2 4	`)	114	
	×(5.40 +	0.2	2)- ×		x 2 ^2)	1/4 x pi	3.86 m3
	* Form			^	1.41	4	1	3.80 m3
	A =	0.1 x ((7.50	+	0.2)/	cos 8º	
		0.1 × ((+ (5.40		0.20))x	2 **	2.68 m2
						/] ^	· .	200 112
6) Blinding Stone						1		
,	V =	0.2 × { (7.50	+ '	0.2)×(5.40 +	
and the second	+	0.2)-	4	x	1/4 x pi		1.20 ^2]	
								7.7 m3
	and the second second							
7) Rc Pile		a di barta di s						
	* Concrete D		1.1		i i state			
	N =	24			et en			24 nos
				÷ .,				per 40.0m
	V1 = (0.450 x	0.450	-	0.020	×	0.020	
	×	1/2 ×	4) x	10.000	X	4 =	8.068 m3
	V2 = (0.450 ×	0.450	•	0.020	x	0.020	a ser a station of a
	×	1/2 x	. 4	+	0.090	×	0.090)	
	×	1/2 ×	0.62		i se la		·····	0.065 m3
	v =	8.133 x	24				Toral	8.133 m3 195.192 m3
	*Form	8.133 x	47			1.2		155.152 115
	A1 =	0.020 x	1.414	x	4	x	10.000 -	1.131 m2
	A2	0.410 x	3	x	10.000			12.300 m2
	A3 =	0.450 x	0.450		0.020	x	0.020	
	x	1/2 ×	4	: :			# #	0.202 m2
	A4 = (0.450 +	0.090) x	1/2	×	0.620	
	x	3					-	0.502 m2
	A .=	A1 x	4	+	· A2	×	4	
	+	A3 x	7	+	A4		-	55.640 m2
	and the second of			• .		н. н. 1		
					1.1			
0) F J 1					an a			
8) Earthworks	* Excavation for						0.50 \	
		3.40 /		x((7.40	x	9.50)	
	+(14.2 x)+(+ ·		
	t Evener Coil - I	x (.ean Concrete + Blir	9.50 ding Store) = ma + 5		486.8 m3
		ied Volume =		e+ro ∶X		me + i x	1.20	1011C
	i ne occup		1.20		pi / 4 0.400	•	1.20	1.81 m3
	Excess Soil		1.20		0.400			93.94 m3
						1.1		73.74 110
	* Back Fill				1.1.1.1		_	392.9 m3
			$\mathcal{T}_{\mathcal{M}} = - \mathcal{L}^{2}$. • •				
				i per				
				din 1		· .		
				· .				
	The second se	the second se					A second s	(1) A set of the se

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ltem			Formula				Quantity
9) Approach Slab							
	* Concrete	and the second			·		
	=	6.46 x {	6.00 x	0.30			
	+ (0.30 +	0.50)/		2 x ·	0.20 }	
	•	0.50 x	0.50 /	2	x	0.3	
		×	2				12.1 m3
	* Lean Concrete	an an an a' a'					
	=(0.30 +	0.28 +	5.20)		
		×	6.46 x	0.1		• . •	3.7 m3
	* Asphaltic Joint Filler		a second a second		÷		
	={	0.30 x (0.02 +	0.06)/	2 .	
	+	0.30 x	0.02) x	6.46	-1	cos 8° =	0.11
					· ·		
at the second second	* Form	1 12 A	1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 -		-		
	A = {	0.30 x	0.50 + (0.50	· +	0.30)	and the second second
		2.00 x	0.20 +	1.05	x	0.30 } x	
	x	2.00 +		cos 8°	x	0.50 =	4.35 m2
والمتحر والمراجع والمراجع							
		1.1					
	 March 1999 (1999) 						
		an in the g		÷			
a she a ta a she a s		an na san san san san san san san san sa					and the second second
				1.1	5 S. S.		
		- 1		i i v			
		ga tan sa					
	la tradición de la composición de la co		e transfer de la companya de la comp		÷		
					1.1		
			a de tarte de la tra	ta di			
		ta se ta			· .		
	and the first second		1	· · · ·			
		ta di second					
				100 A.S.	. · · ·		
		1	11 - E - E - E - E - E - E - E - E - E -	1.16	1.1		

No.

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No.

~									
ltem		F	ormula						Quantity
) Concrete	a series and the	н 1							
,		· · · · · · ·	1. A.						
	* BackWall				1997 - 19				
	v1 = (7.50 x	2.05	. +	7.50	x			
	X	0.05 /	2.00) x	0.40	-	6,23	m3	
	* Frontwall								
	v2 =	7.50 × (3.49	×	1.50		0,1	^2	
			· .	1	2) =	39.17		
					· · ·		·		 A second sec second second sec
	* Corbel v3 =	0.30 x(0.20		0.40	· · ·			
n an	v3 -	0.30 X(0.30	: + . x	0.60 6,50)/	2 0.88	m3	
	* Haunch								
	v4 =	5.54 x	0.50	x	0.50	1	2 x 2	- -	
							1.38	m3	
	*Wingwall	2.10 1			5 5 0	× /			
	v5 ={ +	2.40 x(1.80 x(5.54 1.05		5.50 2.51)/ :)/	2	}	
		x	0.50	×	2	· //	16.45		
	* Parapet	terre de la composition de la	1.1.1.1.1						
	v7 =(0.50 x	0.30	+	0.20	^2	н н 1. н 1. н н		
	-	0.15 ^2/	2)x		: X	2	2	
	*Footing		•	•	1.64	3 2	1.64	m3	
and a second second Second second	v6 =	7.50 x	5.40	x	2.00				
	-	4 x		x	0.60	^2 x	0.1		
				12.5		*	80.55	m3	
			· ·.			· · ·	Total		146.3 m3
) Form	* BackWall								
	al =	2.00 x	2.05	x	7.50		÷.,		
	-	2.05 x (+)x	2		
						=	26.65	m2	
	* Frontwall	75.4		· ·	0.E/				
	a2 = - (7.5 x(0.5 +	3.49 0.5	+) x	3.56 3.56) x	2	· ·	
	↓ ·	3.52 x	1.5	x	2	Ê.	56.29		
		na i Na na ang sa				• •	1111		
	* Corbel	a te fair a star	· .	1.13					$(1,1,2,\dots,2^{n-1})$
an the second	a3 = {	0.3 + (0.3	^2+	0.3	^2)^			
	* Haunch			x	6.5	-	4.71	т2	
	a4 =	+ (0.5	^2+	0.5	^2)^	0.5		
		×	5.535	x	2	=	7.83		
	*Wingwall								
	a5 =	2 x (2.4	x(`-	5.535	. + -)x1/	
	+	+ 0.5 x (1.8 1.05	x(+	1.05 2.31	+	3.00)x1/:	<i>LXL</i>
		-	0.5	×	5.54	}=	66.63		
	* Parapet		-			•		•	
	a5 = {	4.6 x(0.5	+	0.3	+;	0.05		
		+	1.4142	×	0.15	+ ·	0.05		
		• • • •	2	×	0.17875	} x =	2 10.95		
	*Footing				· .	-	10.75	шь	
	a6 =	2 x(7,500	+	5.400)×	2.00		
						· · · ·	51.60		
						1.4 1.1	Total	· .	224.7 m2
	in the second		1997). 1997)	yst y		· · ·	an taon taon 1900. Marina		
		r - Altar Altar - Altar			n an an Na Arta a		an a	1	
			: 1						
			1211	· . *	5			•	
and the second second second	<u> </u>			· · · · · · · · · · · · · · · · · · ·	1997 - 1997 -		a a parte de la companya de la comp A companya de la comp		1

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9 - 31 3

No.

$\begin{array}{cccccc} 3) Scaffolding: & H = 4m \\ & A1 = (& 2 \times (& 7.50 & + & 5.40 &) + & 8 & 1 \\ & & & 2 & - & 1 \\ & & A1 = (& 2 \times (& 7.50 & + & 5.40 &) + & 8 & 1 \\ & & & A2 & - & 1 \\ & & & A2 & - & 1 \\ & & & A2 & - & 1 \\ & & & A2 & - & 1 \\ & & & A2 & - & 1 \\ & & & & A2 & - & 1 \\ & & & & A2 & - & 1 \\ & & & & A2 & - & 1 \\ & & & & A2 & - & 1 \\ & & & & & A2 & - & 1 \\ & & & & & A2 & - & 1 \\ & & & & & A2 & - & 1 \\ & & & & & A2 & - & 1 \\ & & & & & A2 & - & 1 \\ & & & & & A2 & - & 1 \\ & & & & & A2 & - & 1 \\ & & & & & A2 & - & 1 \\ & & & & & A2 & - & 1 \\ & & & & & A2 & - & 1 \\ & & & & & A2 & - & 1 \\ & & & & & A2 & - & 1 \\ & & & & & A2 & - & 1 \\ & & & & A2 & - & 1 \\ & & & & A2 & - & - & 1 \\ & & & & & A2 & - & - & 1 \\ & & & & & A2 & - & - & 2 \\ & & & & & A2 & - & - & 2 \\ & & & & & A2 & - & - & 2 \\ & & & & & A2 & - & - & - \\ & & & & & A2 & - & - & 2 \\ & & & & & A2 & - & - & - \\ & & & & & A2 & - & - & - \\ & & & & & A2 & - & - & - \\ & & & & & A2 & - & - & - \\ & & & & & A2 & - & - & - \\ & & & & & A2 & - & - & - \\ & & & & & A2 & - & - & - \\ & & & & & & A2 & - & - & - \\ & & & & & & A2 & - & - & - \\ & & & & & & A2 & - & - & - \\ & & & & & & A2 & - & - & - \\ & & & & & & A2 & - & - & - \\ & & & & & & & A2 & - & - \\ & & & & & & & A2 & - & - & - \\ & & & & & & & A2 & - & - \\ & & & & & & & A2 & - & - & - \\ & & & & & & & A2 & - & - & - \\ & & & & & & & A2 & - & - & - \\ & & & & & & & A2 & - & - & - \\ & & & & & & & A2 & - & - & - \\ & & & & & & & A2 & - & - & - \\ & & & & & & & A2 & - & - & - \\ & & & & & & & & A2 & - & - & - \\ & & & & & & & & A2 & - & - & - \\ & & & & & & & & & A2 & - & - & - \\ & & & & & & & & & & & & &$	ltem			Formula						Quantity	
$ \begin{array}{c cccc} Al = \left(\begin{array}{cccccc} 2 \times (& 7.50 & + & 5.40 &) + & 8 & 1 \\ & x & 2 & - \\ & 4m < H < 30m \\ A2 & 17.572 \times 11.57 + (4.27.14) + (0.572) \times (1.74.1572) \times 2.2057.3.4857 \\ & 7.52 \times 11.57 + (4.27.14) + (0.572) \times (1.74.1572) \times 2.2057.3.4857 \\ & 7.52 \times 11.57 + (4.27.14) + (0.572) \times (1.74.1572) \times 2.2057.3.4857 \\ & - (7.78 - & 1.55 & + & 4.99 &) \times (1.2072) \times 2 & - \\ & - (7.78 - & 1.55 & + & 4.99 &) \times (1.2072) \times 1.2072 \\ & & 0.50 \times 2 & 2 & - \\ & & 0.2 & (& - 1.4 \times 1/4 \times pi \times & 1.2072) \\ & & & 0.2 & (& - 1.4 \times 1/4 \times pi \times & 1.2072) \\ & & & - & 0.2 \times (& 7.50 & + & 0.2 &) \times (& 5.40 & + \\ & & 0.2 & (& - & 4 & \times 1/4 \times pi \times & 1.2072) \\ & & & - & - & \\ & & & 0.2 \times (& 7.50 & + & 0.2 &) \times (& 5.40 & + \\ & & & 0.2 & (& - & 4 & \times 1/4 \times pi \times & 1.2072) \\ & & & & - & - & \\ & & & & & & & & \\ & & & &$			· · ·								
$) Support = \begin{cases} A1 = \left(2 \times \left(7.50 + 5.40 \right) + 8 + 1 \\ x & 2 - 1 \\ (7.57) + (4.2 + 1) + (0.5 + 2) + (4.2 + 1.3) \\ (7.57) + (4.2 + 1) + (0.5 + 2) + (4.2 + 1.3) \\ (7.57) + (4.2 + 1) + (0.5 + 2) + (4.2 + 1.3) \\ (7.57) + (4.2 + 1) + (0.5 + 2) + (4.2 + 1.3) \\ (7.57) + (4.2 + 1) + (0.5 + 2) + (4.2 + 1.3) \\ (7.57) + (1.2 + 1.5) + (1.2 + 1.3) \\ (7.57) + (1.2 + 1.5) + (1.2 + 1.3) \\ (7.50) + (2.2) + (1.2 + 1.3) \\ (7.50) + (2.2) + (1.2 + 1.3) \\ (7.50) + (2.2) + (1.2 + 1.3) \\ (7.50) + (2.2) + (1.2 + 1.3) \\ (7.50) + (2.2) + (1.2 + 1.3) \\ (7.50) + (2.2) + (1.2 + 1.3) \\ (7.50) + (2.2) + (1.2 + 1.3) \\ (7.50) + (2.2) + (1.2 + 1.3) \\ (7.50) + (2.2) + (1.2 + 1.3) \\ (7.50) + (2.2) + (1.2 + 1.3) \\ (7.50) + (2.2) + (1.2 + 1.3) \\ (7.50) + (2.2) + (1.2 + 1.3) \\ (7.50) + (2.2) + (1.2 + 1.3) \\ (7.50) + (2.2) + (1.2 + 1.3) \\ (7.50) + (2.2) + (1.2 + 1.3) \\ (7.50) +$	a) Scoffolding	+ Headm									
$ \begin{array}{c c c c c c c c c c c c c c c c c c c $	of Scanolung.		2 ~1	7 50	+	5.40) +		a 1		
$A^{2} = \frac{17}{172} \frac{17}{2} \frac{1}{2} $			2	7.50		0.40				67.6	7
$A^{2} = \frac{[(2/5+2) + (4/2 + 1/5+2) + (2/5 + 2/1 + (2/5 + 3/45)]}{\pi (2/5/2 + (1/5) + (4/2 + 1/5) + (2/5 + 3/45)]} = 236.9 \text{ mi}}{\pi (2/5/2 + (1/5) + (4/2 + 1/5) + (2/5 + 3/45)]} = 236.9 \text{ mi}}$ $(1) \text{ Support} = - \left\{ \begin{array}{cccccc} - & - & - & - & - \\ - & - & - & - & - &$	· .					1997 - 19	~		4	07.0	- m2
$A^{2} = \frac{[(7,54) + (4,24)(5+2) + (0,5+2) + (4,24)(1) + (0,5+3)(45)]}{\pi(7,52) \times (1,24)(1,25+3)(45)} = 236.9 \text{ mi}}$ (4) Support (4) Support (5) Lean Concrete (7) Bored Crite (7) $A^{2} = (1,1) + $		* 4m< H<=30m		n An An An An				1			
$\frac{3}{12} \frac{3}{2} \frac{2}{2} \frac{1}{x} \frac{1}{x} \frac{5}{x} \frac{4}{x} \frac{4}{2} \frac{1}{x} \frac{1}{x} \frac{5}{x} \frac{2}{x} \frac{4}{x} \frac{9}{x} \frac{1}{x} \frac{1}{x} \frac{80}{x} \frac{7}{x} \frac{1}{x} $			((7 5+2) + (4 2+	1 5+7) + (I) 5	5+2) + (i	4 2+1-1)			8783		
$ \begin{aligned} \begin{array}{c} 1) \text{Support} \\ \text{i) Support} \\ \text{i) Support} \\ \text{i) Support} \\ \text{i) Lean Concrete} \\ \begin{array}{c} -\left(\begin{array}{c} 7.98 \\ - \end{array} \right) 1 \times \left\{ \left(\begin{array}{c} 7.50 \\ + \end{array} \right) 2 \right) \times \left(\begin{array}{c} 5.40 \\ + \end{array} \right) 2 \\ \text{i} \left(\begin{array}{c} 2.2 \\ - \end{array} \right) 1 \times \left\{ \left(\begin{array}{c} 7.50 \\ + \end{array} \right) 2 \\ + \end{array} \right) 2 \\ \text{i} \left(\begin{array}{c} 7.50 \\ + \end{array} \right) 2 \\ \text{i} \left(\begin{array}{c} 7.50 \\ + \end{array} \right) 2 \\ \text{i} \left(\begin{array}{c} 7.50 \\ + \end{array} \right) 2 \\ \text{i} \left(\begin{array}{c} 7.50 \\ + \end{array} \right) 2 \\ \text{i} \left(\begin{array}{c} 7.50 \\ + \end{array} \right) 2 \\ \text{i} \left(\begin{array}{c} 7.50 \\ + \end{array} \right) 2 \\ \text{i} \left(\begin{array}{c} 7.50 \\ + \end{array} \right) 2 \\ \text{i} \left(\begin{array}{c} 7.50 \\ + \end{array} \right) 2 \\ \text{i} \left(\begin{array}{c} 7.50 \\ + \end{array} \right) 2 \\ \text{i} \left(\begin{array}{c} 7.50 \\ + \end{array} \right) 2 \\ \text{i} \left(\begin{array}{c} 7.50 \\ + \end{array} \right) 2 \\ \text{i} \left(\begin{array}{c} 7.50 \\ + \end{array} \right) 2 \\ \text{i} \left(\begin{array}{c} 7.50 \\ + \end{array} \right) 2 \\ \text{i} \left(\begin{array}{c} 7.50 \\ + \end{array} \right) 2 \\ \text{i} \left(\begin{array}{c} 7.50 \\ + \end{array} \right) 2 \\ \text{i} \left(\begin{array}{c} 7.50 \\ + \end{array} \right) 2 \\ \text{i} \left(\begin{array}{c} 7.50 \\ + \end{array} \right) 2 \\ \text{i} \left(\begin{array}{c} 7.50 \\ + \end{array} \right) 2 \\ \text{i} \left(\begin{array}{c} 7.50 \\ + \end{array} \right) 2 \\ \text{i} \left(\begin{array}{c} 7.50 \\ + \end{array} \right) 2 \\ \text{i} \left(\begin{array}{c} 7.50 \\ + \end{array} \right) 2 \\ \text{i} \left(\begin{array}{c} 7.50 \\ + \end{array} \right) 2 \\ \text{i} \left(\begin{array}{c} 7.50 \\ + \end{array} \right) 2 \\ \text{i} \left(\begin{array}{c} 7.50 \\ + \end{array} \right) 2 \\ \text{i} \left(\begin{array}{c} 7.50 \\ + \end{array} \right) 2 \\ \text{i} \left(\begin{array}{c} 7.50 \\ + \end{array} \right) 2 \\ \text{i} \left(\begin{array}{c} 7.50 \\ + \end{array} \right) 2 \\ \text{i} \left(\begin{array}{c} 7.50 \\ + \end{array} \right) 2 \\ \text{i} \left(\begin{array}{c} 7.50 \\ + \end{array} \right) 2 \\ \text{i} \left(\begin{array}{c} 7.50 \\ + \end{array} \right) 2 \\ \text{i} \left(\begin{array}{c} 7.50 \\ + \end{array} \right) 2 \\ \text{i} \left(\begin{array}{c} 7.50 \\ + \end{array} \right) 2 \\ \text{i} \left(\begin{array}{c} 7.50 \\ + \end{array} \right) 2 \\ \text{i} \left(\begin{array}{c} 7.50 \\ + \end{array} \right) 2 \\ \text{i} \left(\begin{array}{c} 7.50 \\ + \end{array} \right) 2 \\ \text{i} \left(\begin{array}{c} 7.50 \\ + \end{array} \right) 2 \\ \text{i} \left(\begin{array}{c} 7.50 \\ + \end{array} \right) 2 \\ \text{i} \left(\begin{array}{c} 7.50 \\ + \end{array} \right) 2 \\ \text{i} \left(\begin{array}{c} 7.50 \\ + \end{array} \right) 2 \\ \text{i} \left(\begin{array}{c} 7.50 \\ + \end{array} \right) 2 \\ \text{i} \left(\begin{array}{c} 7.50 \\ + \end{array} \right) 2 \\ \text{i} \left(\begin{array}{c} 7.50 \\ + \end{array} \right) 2 \\ \text{i} \left(\begin{array}{c} 7.50 \\ + \end{array} \right) 2 \\ \text{i} \left(\begin{array}{c} 7.50 \\ + \end{array} \right) 2 \\ \text{i} \left(\begin{array}{c} 7.50 \\ + \end{array} \right) 2 \\ \text{i} \left(\begin{array}{c} 7.50 \\ + \end{array} \right) 2 \\ \text{i} \left(\begin{array}{c} 7.50 \\ + \end{array} \right) 2 \\ \text{i} \left(\begin{array}{c} 7.50 \\ + \end{array} \right) 2 \\ \text{i} \left(\begin{array}{c} 7.50 \\ + \end{array} \right) 2 \\ \text{i} \left(\begin{array}{c} 7.50 \\ + \end{array} \right) 2 \\ \text{i} \left(\begin{array}{c} 7.50 \\ + \end{array} \right) 2 \\ \text{i} \left(\begin{array}{c} 7.50 \\ + \end{array} \right) 2 \\ \text{i} \left(\begin{array}{c} 7.50 \\ + \end{array} \right) 2 \\ \text{i} \left(\begin{array}{c} 7.50 \\ + \end{array} \right) 2 \\ \text{i} \left(\begin{array}{c} 7.50 \\ + \end{array} \right) 2 \\ \text{i} \left(\begin{array}{c} 7.50 \\ + \end{array} \right) 2 \\ \text{i} \left(\begin{array}{c} 7.50 \\ + \end{array} \right) 2 \\ \text{i} \left(\begin{array}{c} 7.50 \\ + \end{array}$							2)) x (2	05+3 485)		236.9	m 2
$ \begin{array}{c cccc} x & 0.50 & x & 2 & - & \\ \hline & & & 0.1 \times \left\{ \left(& 7.50 & + & 0.2 \\ & & & & 0.1 \times \left\{ \left(& 7.50 & + & 0.2 \\ & & & & 1/4 \times pi \\ & & & & 1/20 \\ ^{+} & & & 0.2 \\ ^{+} & & & 0.2 \\ ^{+} & & & 0.2 \\ ^{+} & & & 0.2 \\ ^{+} & & & 0.2 \\ ^{+} & & & 0.2 \\ ^{+} & & & 0.2 \\ ^{+} & & & 0.2 \\ ^{+} & & & 0.2 \\ ^{+} & & & 0.2 \\ ^{+} & & 0.2 \\ ^$				enter frægende son en	NUTURTA:		CA.C.S.T		28:875		1116
$ \begin{array}{c cccc} x & 0.50 & x & 2 & - & \\ \hline & & & 0.1 \times \left\{ \left(& 7.50 & + & 0.2 \\ & & & & 0.1 \times \left\{ \left(& 7.50 & + & 0.2 \\ & & & & 1/4 \times pi \\ & & & & 1/20 \\ ^{+} & & & 0.2 \\ ^{+} & & & 0.2 \\ ^{+} & & & 0.2 \\ ^{+} & & & 0.2 \\ ^{+} & & & 0.2 \\ ^{+} & & & 0.2 \\ ^{+} & & & 0.2 \\ ^{+} & & & 0.2 \\ ^{+} & & & 0.2 \\ ^{+} & & & 0.2 \\ ^{+} & & 0.2 \\ ^$	4) Support	= (7.98 -	1.55	· +	4.99) x -	1 80	12	Contract of the second	÷.,
5) Lean Concrete $ \begin{array}{c} * Concrete class G \\ $	1) 0 - PP 0 10							100	*	1	m3
$ \begin{array}{c} V = 0.1 \times \left\{ \left(\begin{array}{c} 7.50 \\ + 0.2 \right) \cdot \left(\begin{array}{c} 4 \\ \times \end{array} \right) \left(1/4 \times pi \\ \times \end{array} \right) \times \left(1/4 \times pi \\ \times \end{array} \right) \left(1/4 \times pi \\ \times 1/4 \times pi \\ \times 1/2 \times 4 \\ \times 1/2 \times 1/2 \\ \times 1/2 \times 1/2 \times 1/2 \\ \times 1/2 \times 1/2 \times 1/2 \\ \times 1/2 \\ \times 1/2 \\ \times 1/2 \times 1/2 \\ \times 1/$							1.			1010	
$ \begin{array}{c} V = 0.1 \times \left\{ \left(\begin{array}{c} 7.50 \\ + 0.2 \right) \cdot \left(\begin{array}{c} 4 \\ \times \end{array} \right) \left(1/4 \times pi \\ \times \end{array} \right) \times \left(1/4 \times pi \\ \times \end{array} \right) \left(1/4 \times pi \\ \times 1/4 \times pi \\ \times 1/2 \times 4 \\ \times 1/2 \times 1/2 \\ \times 1/2 \times 1/2 \times 1/2 \\ \times 1/2 \times 1/2 \times 1/2 \\ \times 1/2 \\ \times 1/2 \\ \times 1/2 \times 1/2 \\ \times 1/$	5) Lean Concrete	* Concrete class G	3			ta da esta			1.5	· · · · ·	
$ \begin{array}{c ccccc} + & 0.2 \end{pmatrix} - & 4 & x & 1/4 x pi & x & 1.20 & ^{2} \\ \hline & & & & & & \\ & & & & & & \\ & & & &$				(7.50	+	0.2)×(5.40) +		
$ \begin{array}{c} \text{Form} & - & 0.1 \times \{(7,50 + 0.2) + (540 + 0.2) + (540 + 0.2) \} \times 2 & - \\ & + & 0.20 \) \times 2 & - \\ \text{Soluting Stone} & & & & & & & & & & & & & & & & & & &$	and the second	+			×						
$A = 0.1 \times \{(7.50 + 0.2) + (5.40 + 0.20) \times 2 - (7.50 + 0.2) \times (7.50 + 0.2) \times (7.50 + 0.2) \times (7.540 + 0.2) \times ($						· · · .	· · · .		-	3.86	m3
$ \begin{array}{c ccccc} + & 0.20 &) x & 2 & - \\ & 0.20 & (x & 2 & - \\ & 0.20 & ((7.50 + 0.2) \times (5.40 + \\ + & 0.2) & 4 & x & 1/4 \times pi & x & 1.20 \wedge 2] \\ & & & 0.20 & x & 1/4 \times pi & x & 1.20 \wedge 2] \\ \hline & & & & & & & & & & & & & & & \\ & & & & & & & & & & & & & & \\ & & & & & & & & & & & & & & & \\ & & & & & & & & & & & & & & & \\ & & & & & & & & & & & & & & \\ & & & & & & & & & & & & & & \\ & & & & & & & & & & & & & & \\ & & & & & & & & & & & & & & \\ & & & & & & & & & & & & & & \\ & & & & & & & & & & & & & & \\ & & & & & & & & & & & & & & & \\ & & & & & & & & & & & & & & \\ & & & & & & & & & & & & & & & \\ & & & & & & & & & & & & & & & \\ & & & & & & & & & & & & & & & \\ & & & & & & & & & & & & & & & \\ & & & & & & & & & & & & & & & & \\ & & & & & & & & & & & & & & & & \\ & & & & & & & & & & & & & & & & \\ & & & & & & & & & & & & & & & & \\ & & & & & & & & & & & & & & & \\ & & & & & & & & & & & & & & & & \\ & & & & & & & & & & & & & & & \\ & & & & & & & & & & & & & & & \\ & & & & & & & & & & & & & & & \\ & & & & & & & & & & & & & & & & \\ & & & & & & & & & & & & & & & \\ & & & & & & & & & & & & & & & & \\ & & & & & & & & & & & & & & & & \\ & & & & & & & & & & & & & & & & \\ & & & & & & & & & & & & & & & \\ & & & & & & & & & & & & & & & & \\ & & & & & & & & & & & & & & & & \\ & & & & & & & & & & & & & & & & & \\ & & & & & & & & & & & & & & & & & & \\ & & & & & & & & & & & & & & & & & & & \\ &$		* Form	and the second sec				an fi	1997 - 1997 1997 - 1997 - 1997 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1			
6) Blinding Stone $V = 0.2 \times \{(7.50 + 0.2) \times (5.40 + - 0.2) \times (5.40 + 0.2) \times (5.40 + $		A = 1	0.1 x {	(7.50				5.40) [1	. 1
$ \begin{array}{c} V = & 0.2 \times \left\{ \left(\begin{array}{c} 7.50 \\ + & 0.2 \right) \times \left(\begin{array}{c} 5.40 \\ + & 1/4 \times \text{pi} \times & 1.20 \\ \times & 1/2 \times 4 \end{array} \right) \times \left(\begin{array}{c} 1.20 \\ \times & 1/2 \times 1 \\ \text{v} \times & 1/2 \times 4 \end{array} \right) \times \left(\begin{array}{c} 1.20 \\ \times & 1/2 \times & 1/4 \\ \text{v} \times & 1/2 \times & 1/4 \\ \text{v} \times & 1/2 \times & 1/2 \\ \times & 1/2 \times & 4 \end{array} \right) \times \left(\begin{array}{c} 24 \\ \text{m} \end{array} \right) \times \left(\begin{array}{c$		1 · · · · · · · · · · · · · · · · · · ·	• • •	0.20)}x		2		. =	2.66	m2
$V = 0.2 \times \{(7.50 + 0.2) \times (5.40 + 1.20 \times 2) \times (5.40 + 1.20 \times 2) + 1.20 \times 2) \times (5.40 + 1.20 \times 2) \times (5.40 + 1.20 \times 2) + 1.20 \times 2) \times (5.40 + 1.20 \times$				· · · · ·							
	6) Blinding Stone	En Falles			1997 - 1997 1997 -	· · · · ·	1.1		•		· ·
7) Bored Pile *Concrete D N = 24 *Concrete D N = 24 V1 = (0.450 x 0.450 - 0.020 x 0.020 x 1/2 x 4) x 10.000 x 4 = V2 = (0.450 x 0.450 - 0.020 x 0.020 x 1/2 x 4 + 0.090 x 0.090) x 1/2 x 0.62 *Toral *Form A1 = 0.020 x 1.414 x 4 x 10.000 = 12.300 m ² A2 0.410 x 3 x 10.000 = 12.300 m ² A3 = 0.450 x 0.450 - 0.020 x 0.020 x 1/2 x 4 = 0.020 x 1.2 x 0.620 x 3 = 0.450 x 0.450 - 0.020 x 0.020 = 12.300 m ² A4 = (0.450 x 0.450 + 0.050) x 1/2 x 0.620 x 3 = 0.552 m ² A4 = A1 x 4 + A2 x 4 = 55.640 m ² * Excess Soil = Lean Concrete + Blinding Stone + Footing Volume + Pile Occupied Volume Pile Occupied Volume = 4 x pi/4 x 1.20 x 1.20 x 0.400 = 1.81 m ² Excess Soil = Lean Concrete + Blinding Stone + Footing Volume + Pile Occupied Volume Pile Occupied Volume = 4 x pi/4 x 1.20 x 1.20 x 0.400 = 33.9 m ² Excess Soil = Lean Concrete + Blinding Stone + Footing Volume + Pile Occupied Volume Pile Occupied Volume = 4 x pi/4 x 1.20 x 1.20 x 0.400 = 33.9 m ² Excess Soil = Lean Concrete + Blinding Stone + Footing Volume + Pile Occupied Volume Pile Occupied Volume = 4 x pi/4 x 1.20 x 1.20 x 0.400 = 33.9 m ² Excess Soil = Lean Concrete + Blinding Stone + Footing Volume + Pile Occupied Volume Pile Occupied Volume = 4 x pi/4 x 1.20 E		V =	0.2 × [(7.50	. +						
7) Bored Pile * Concrete D N = 24 = 24 ne per 40.0m $V1 = (0.450 \times 0.450 - 0.020 \times 0.020)$ $\times 1/2 \times 4 \times 10.000 \times 4 = 8.068 m^2$ $V2 = (0.450 \times 0.450 - 0.020 \times 0.020)$ $\times 1/2 \times 4 + 0.090 \times 0.090$ $\times 1/2 \times 0.62 = -0.065 m^2$ $V = 8.133 \times 24 = -1.000 = 1.131 m^2$ Form A1 = 0.020 $\times 1.414 \times 4 \times 10.000 = 1.131 m^2$ A2 0.410 $\times 3 \times 10.000 = 1.2300 m^2$ A3 = 0.450 $\times 0.450 - 0.020 \times 0.020$ $\times 1/2 \times 4 = -1.2300 m^2$ A4 = (0.450 $+ 0.090$) $\times 1/2 \times 0.620$ $\times 3 = -0.502 m^2$ A = A1 $\times 4 + A2 \times 4 = -1.052 m^2$ A = A1 $\times 4 + A2 \times 4 = -1.052 m^2$ A = A1 $\times 4 + A2 \times 4 = -1.0520 m^2$ A = A1 $\times -1.20 \times 0.400 = -1.181 m^2$ B) Excess Soil = Lean Concrete + Blinding Stone + Footing Volume + Pile Occupied Volume = -1.20 \times 0.400 = -1.181 m^2 B) Excess Soil = Lean Concrete + Blinding Stone + A $\times 0.400 = -1.181 m^2$ B) Excess Soil = Lean Concrete + Blinding Stone + 0.400 = -1.181 m^2 B) Excess Soil = Lean Concrete + Blinding Stone + 0.400 = -1.181 m^2 B) Excess Soil = Lean Concrete + Blinding Stone + 0.400 = -1.181 m^2 B) Excess Soil = Lean Concrete + Blindin		+	0.2)-	4	x	1/4 x pi	x	1.20) ^2}		
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	* Concrete							
	-	6.46 x {	6.00 x	0.30				
	+ (0.30 +	0.50)/		x	0.20 }		
	-	0.50 x	0.50 /	2	×	0.3		
		×	2			. =	12	.1 m3
	* Lean Concrete	and the second						
	= (0.30 +	0.28 +	5.20)		1	
		x	6.46 x	0.1		. 🛥	3	.7 m3
	* Asphaltic Joint Filler	and the second second						
		0.30 x (0.02 +	0.06)/	2		
	+	0.30 x	0.02 } x	6.46		· =	0.	11
	* Form							
	A = {	0.30 x	0.50 + (0.50	+	0.30)		
	· · ·	2.00 x	0.20 +	1.05	x	0.30 } x		
	x	2.00 +	6.46 x	0.50	(-1) = (-1)	-	4.	32 m2
		1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 -	· · · ·		1. C. C. S. S.	a de la secola de la		
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	l a serie de la							
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		la de la composición de		ta ya sa	1			
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and the second second								
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			da series de la composición de la compo					
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and the second second second			e da se da s					· · ·

SIGN	DIACETER	LENGTH	NOS.	UNIT WEIGHT	PIECE WEIGHT	TOTAL WEIGHT	NOTE
1	D22	9,900	12	2.964	29.500	354.0	
2	D6	1,614	95	0.222	0.358	34.0	
3	D6	490	120	0.222	0.109	13.1	
4	D25.	1,911	2	3.854	7.360	14.7	
5	D22	350	8	2.984	1.040	8.3	
8	D16	1,508	8	1.579	2.380	19.0	
	1	T					
• 					TOTAL	443.1	
						· · ·	
				and the second			
				· ·	1 ma a		
					D32	0.0	
				. 1	D25	14.7	
· · ·				- · · · · · · · · · · · · · · · · · · ·	D22	362.3	
					D16	19.0	
					¢6	47.1	
						443.1	

LIST OF REINFORCEMENT

SIGN	DIACETER	LENGTH	NOS.	UNIT WEIGHT	PIÈCE WEIGHT	TOTAL WEIGHT	NOTE
1	D22	9,900	8	2.984	29.500	236.0	
2	D6	1,614	95	0.222	0.358	34.0	
3	D6	490	120	0.222	0,109	13,1	
4	D25	1,911	2	3.854	7.360	14.7	
5	D22	350	8	2.984	1.040	8.3	
8	D25	0	0	3.854	0.000	0.0	and a star
9	D16	1,508	8	1.579	2.380	19.0	
1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 -					TOTAL.	325.1	······································
· · · ·							
1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 -							· . ·
			1940 - A.		D32	0.0	
			1. A. A.	1 I I	D25	14.7	1.1.1.1
			1. A		D22	244.3	
					D16	19.0	
					Ø6	47.1	1
						325.1	
						· · ·	

SIGN	DIACETER	LENGTH	NOS.	UNIT WEIGHT	PIECE WEIGHT	TOTAL WEIGHT	NOTE
18	D22	10,589	4	2.984	31.600	126.4	
1b	D22	10,612	4	2.984	31.700	126.8	(AVE)
2	D6	1,614	95	0.222	0.358	34.0	
. 3	D6	490	60	0.222	0,109	6.5	
4	D25	1,911	2	3.854	7.360	- 14.7	and the second
5	D22	350	8	2.984	1.040	8.3	the second
6	D6	9,460	1	0.222	2.100	2.1	
7	D32	810	1	6.313	5,110	5.1	(AVE)
8	D25	0	0	3.854	0.000	0.0	
9	D16	1,508	4	1.579	2.380	9.5	a series a series a series de la
				1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 -	TOTAL	333.4	
		: ·					in the second second
		1					and the second
		1. A.	1 A.			a series a tra	
					D32	5.1	
1 A				and the second second	D25	14.7	a an an an
					D22	261.5	
					D16	9.5	
					¢8	42.6	<u>tota en e</u>
				-	· ·	333.4	
					1. S.		

3

TOT	AL .		ang		
		Pile-1	Pile-2	Pile-3	Total
ກວຣ		1	2	1	
_	D32	0.0	0.0	5.1	5.1
	D25	14.7	29.4	14.7	58.8
	D22	362.3	488.6	261.5	1112.4
	D16	19.0	38.0	9.5	66.5
	¢ 6	47.1	94.2	42.6	183,9
	19 - C	443,100	650.200	333.400	1426.7

- 34	÷	÷.,	1.1		
	-	3	<u>, 1</u>		

	Iten	15	Unit	Pier 1	Pier 2	Total
	Number of piles		Pile	25	25	25
	Total length Rc piles 1	.50mm	m	208.0	208	208
	Concrete piles class D		m³	203.3	203	203
					0	0
mit		φ6	kg	4597.5	4598	4598
Pile		D16	kg	1662.5	1663	1663
	Denterment	D22	kg	33710.0	33710	33710
	Renforcement	D25	kg	1470.0	1470	1470
		D32	kg	127.5	128	128
		Total	kg	41567.5	41568	41568
· .	Concrete class E		m ³	123.4	123.4	246.8
		D10	kg	20.7	20.7	41.4
		D14	kg	1048.8	1048.8	2097.6
		D16	kg	619.2	619.2	1238.4
	D _:_(D20	kg	485.3	485.3	970.6
	Reinforcement	D22	kg	1905.7	1905.7	3811.4
		D28	kg	4040.9	4040.9	8081.8
		D32	kg	2289.7	2289.7	4579.4
. •		Total	kg	10410.3	10410.3	20820.6
		Curve	m ²	51.0	51.0	102.0
Pier	Form	Flat	m²	81.6	81.6	163.3
		H < 4m	m²	116.0	116.0	232.0
	Scaffolingding work	4m ≤ H < 30m	m²	140.4	140.4	280.8
	Support		m ³	7.0	7.0	14.0
		Excavation for foundation	m ³	563.3	372.4	935.7
	Earth work	Blinding Concrete class G	m ³	47.7	47.7	95,5
in an		Rip rap	m ³	446.7	257.9	704.6
		Sheet pile Larsen IV	m	1248.0	1248.0	2496.0
i s Est	Cofferdams	Steel pile I 400	m	288.0	288.0	576.0
		Brace C 300	m	651.0	651.0	1302.1

QUANTITY TABLE OF PIER

QUANTITY SURVEYING FOR PIER P1 & P2

No.

Item	1				Formula					Quantii	v
1) Concr ete	}				·						
1) Concrete		÷.					· ·		• •		
	+ Headstoc	k	at an								
		v1 ={(7.00	x	1.40	-	1.15	×	0.70) [
	- 14 A					×	1.00) ^{- 1}			
		+ (7.00	x	1.60	· -	1.15	×	0.70)	
			1. N. M.			x	1.00)		= 19.3	9 m3
	* Wall ⇒		= 00		1/4		1.40				· .
		v2 =	5.80	×(1/4 x pi 2.90	×	1.40 1.40	.^2			•
	* Footing			. *	2.90	x	1.40	;)		- 324	8 m3
	100.00	v3 = (6.00	x	6.00	x	2.00				
	1 · · · · ·		4	x	pi	x	0.60	^2 x	0.1)	· .
					•					- 71.5	5 m3
		н. 1997 - Ал						- 11 - 12			
P.					ta Line e				Total	123.4	1 m3
2) Form			1. N. J. ¹⁰				·	· .			· · · · ·
	* Wall (Cui	rve form) a1-1 =	2		1.40		5 20				• · ·
and the second second	• Wall (Flat		2 x pi	x	1.40	X :	5.80		· · · · ·	- 51.0	2 m2
		a1-2 =	2	×	2.90	× .	5.80		الم الرامين	- 33.6	4 m2
	* Footing	·						•			
		a2 =	2	x (6.000	+	6.000) ×	2.00		
			•			1. 1. st				- 48.0	0 m2
				n an		n an	Total	11	Curve		2 m2
					· · ·		an a	. : •	Flat	- 81.6	4 m2
3) Scaffolding:	* (H<=4m)	10 A.			0 M	in an	9.00		2.00		s = x
		A1 = + (2	x(x(8.00 4.00	+ +	8.00 9.00) x) x	2.00		
			. •		4.00		7.00	· ^ .	2.00	- 116.0	0 m2
	* (4m< H<	=30m)		1.00				÷		1140	
		A2 =	2	×(4.00	, + '	9.00) x	5.40		
		an tara a	1997) 1997 - 1997	ar (-					•	- 140.4	0 m2
				1.5					· · · · · ·	at di second	
4) Support		- [(7.00		2.90		1.40)×_	2.00		
		+	1.15	×	0.70	×	2.00	, 1 ,		- 7.0	1 m3
5) Lean Concrete	* Concrete	class C	· · · ·			at post		1.			
of Lean Concrete	Concrete	v =	0.50	×{(6.00	• • • •	4)×(6.00	Hara and Ara	
		+			4		1/4 x pi) ^ (_ x	1.20 ^2		
						:		-			1 m3
6) Rc Pile		e i suite Second	· .					1. 			
	* Concrete						· · · · ·				
		N =	2	25	- (1 + 1)					25	nos
		V1 = (0.450		0.460	t de la c	0.020		0.000	per 40.0m	
	· .	× ×	1/2	., X X	0.450 4) x	10.000	X	0.020 4 =	8.06	8 m3
		V2 = (0.450	x	0.450		0.020	x	0.020	0.00	
		x	1/2	x	4	+	0.090	x	0.090)		
		n x 2	1/2	· x	0.6	2	ат н. Т	· .	-	0.06	5 m3
							1. 1.	1.1	Toral		3 m3
		V =	8.133	X	2	5			**	203.32	5 m3
	*Form			· · .							
		A1 = A2	0.020	X	1.414	. X	4	×	10.000 =		1 m2
		A2 A3 =	0.410 0.450	X X	3 0.450	×	10.000 0.020		0.020	12.30	v m2
		лэ - х	1/2	· · · · · · · · · · · · · · · · · · ·	0.450 4		Ų.020	×	0.020	0.20	2 m2
		A4 ≖(0.450	÷÷	0.090) x	1/2	x	0.620	0.20	- +166
		x	3	5 A 1						0.50	2 m2
	1. A.	A =	A1	X	4	+	A2	×	4		
		+	A3	x	7	+	A4		19 J. D. 🗮	55.64) m2
			1. 1. 1.			· · · ·					
	1			5 A. 19			1997 - 1997 1997 - 1997				
	1								1	1.1.1	1.1

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QUANTITY SURVEYING FOR PIER P1 & P2

No.

ltem		Fo	ormula			Quantity
				•		
) Earthwork	* Excavation for foc		· .			
		5.90	×(10.00 x	10.00)
	-	4 x pi	/4 x	1.20 ^2 x	5.90	_
	* Excess Soil					= 563.31 m3
	v1 =	Lean Concrete			÷	= 47.74 m3
	v2 =	Footing Volume	· ·			= 71.55 m ³
		Occupied volume of		· · · · · · · · · · · · · · · · · · ·		
a the standard second	- 	4 x pi	/4 x	1.200 ^2 x		= 271 m ²
	v4 =	Occupied volume of	x nf wall	0.60		= 2.71 m3
		(pi/4 x	1.400 ^2 +	2.90 x	1.40)
		x	3.400	·		= 19.04 m3
				· · · · ·		
		-			Total	= 141.04 m3
	* Riprap =	Excavation for foot 563.31 -	ing - Excess Soil 141.04			
	· · · · · · · ·			1.200 ^2 x	5.40	e de la companya de l
a de la companya de La companya de la comp		- P	, - · · ·	*^		- 446.70 m3
)Cofferdams	Later and the					
	* Sheet pile larsen I	1	•	a da da		e de la composición d La composición de la c
	* Steel pile I 400	104.00 nos	L= 12	m		= 1248.00 m
	Sieer pile 1400	24.00 nos	L= 12	m		= 258.00 m
	* Brace C 300					100,00 Ht
	L1 = (10.2 +	11.08) x	7.00 x	4	
				-		m
	L3 =	9.2 x	6			m
en en dante. Na en anterne en en en el			· · · · · ·		Total	651.04 m
		_ _	1			
] [
			10 A			and the second second
				Calling Colore	i de te	
				8		
and a second second Second second				CHARTER CLAST	12 L	
				CONCIENT DUALS C		
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		i in the second s	U			
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		1-11	H			
				LANSED N	а А	i i se se se s
				r H		
an a					5 - C	
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		<u>is quant</u>				
			000 2000			
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		eg de la contra de la		ta an		
공연 비가 가장 비행이다.						
				na statisti i i facilita. Rođanja se ostali i i i		
and the second second second				n an tha sha sha sha sha sha sha sha sha sha s		
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Q	UANTIT	Y SURVE	YING	FOR	PIER	P1 & P2	

No.

ltem	<u> </u>				Formula		· · · · · · · · · · · · · · · · · · ·			i Quantity
	1				<u> </u>					
) Concrete							· ·		· · · ·	
. •	t Lloc det -	L	1. A.	÷					÷ 1.	
· · · · ·	* Headstoc	vl ={(7.00		1 40	e î î	1 15		Δ. 7 0 \	
		vi ={(7.00	x	1.40	×	1.15 1.00	, X	0.70)	
and and a second se		. + (7.00	. x	1.60	· •	1.15	×	0.70)	
	1. A.		,		1.00	×	1.00			19.39 m3
	* Wall	-		i.		1		1		
	·	v2 🖛	5.80	x (1/4 x pi	x	1.40	^2		
in a state of the state.		· . ·		+	2,90	. x	1.40)	=	32.48 m3
	* Footing			1999 - 1997 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 -			÷.,	·		
e de la companya de l		v3 = (6.00	×	6.00	×	2.00		· · · · ·	
			4	×	pi	×	0.60	^2 x	0.1)	-
						·.		•		71.55 m3
			· · · .						Total	123.41 m3
) Form						· ·			1041	110.41 110
	* Wall (Cur	ve form)								A Constraint for the
		al-1 =	2 x pi	x	1.40	×	5.80		=	51.02 m2
	* Wall (Flat	form)	. ·	•						
		a1-2 =	2	×	2.90	x	5.80			33.64 m2
	* Footing		2 - 1999 2				2 - 2 - 2 - 2 2 - 4 - 2 2 - 2 - 2		en de la del de la del	
	· · .	a2 =	2	× (6.000	+ .	6.000) x	2.00	
							17	a tagi	-	43.00 m2
				1.1			Total	t. Serie	Curve = Flat =	51.02 m2
Scaffolding:	* (H<=4m)			11 a.					Г МІ =	81.64 m2
		A1 =	2	x (8.00	+	8.00) x	2.00	
	· ·	+ (2	x(4.00	+	9.00) x	2.00	
								· .		116.00 m2
	* (4m< H<		1.1	÷					n an	
		A2 =	2	. ×(4.00	+	9.00) x	5.40	
				1100		- 1 - 1	(1,1,1,1,1,1,1,1,1,1,1,1,1,1,1,1,1,1,1,			140.40 m2
6		: _ (/								
Support		= { (7.00	-	2.90	- :	1.40) ×	2.00	
		+	1.15	×	0.70	x	2.00	.) .	-	7.01 m3
Lean Concrete	* Concrete	class G						1.1	· ·	
		v =	0.50	×{(6,00	+	4)×(6.00 +	
	· · ·	+		4)-	4	x	1/4 x pi			
	· · · .						•	1.00		47.74 m3
Rc Pile		· · ·		N			· .			
	* Concrete		1. 1. 1.					1.		
a di seconda di setta di setta Seconda di setta di s		N =		25					=	25 nos
		$V_1 = I_1$	0.450	· · ·	0.450		0.000		0.020	per 40.0m
		V1 = (0.450 1/2	×	0.450 4	· _	0.020	×	0.020	0000 _0
· · · ·		V2 = ()	0.450	x	9 0.450) x	0.020	x x	4 - 0.020	8.068 m3
•		x	1/2	×	4	+	0.020	x	0.090)	
		×	1/2	x	0.6					0.065 m3
									Toral	8.133 m3
• •	1	V =	8.133	x	2	5				203.325 m3
	*Form	ti situ		· . :		1.1				
		A1 =	0.020	X	1.414	^{1.} x	4	x .	10.000 =	1.131 m2
		A2	0.410	ίx	3	· x	10.000		-	12.300 m2
		A3 -	0.450	x	0.450	- 1	0.020	x	0.020	
		• X (• .	1/2	×	4		4.10		a / a /	0.202 m2
· · · ·		A4 = (0.450	. +	0.090) x	1/2	×	0.620	0.000 0
e a construction de la construction Construction de la construction de la		× A =	3	· · · ·	· · · · ·	· · · ·	4.7	4 2 44		0.502 m2
		A =	A1 A3	·. X	4 7	• • •	A2 A4	. X	4 -	55.640 m2
		. F .		×	1	• +	74			00.040 m2
				1	1.1.1.1.1	e et internet. Li terrete				and the second second second

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3 -

QUANTITY SURVEYING FOR PIER P1 & P2

No.

.

ltem	Formula		Quantity
') Earthwork	* Excavation for footing		
	₩ 3.90 × (10.00 ×	10.00)	
	- 4 x pi/4 x 1.20 ^2 x	3.90	
		*	372.36 m3
and the second	* Excess Soil		
	v1 = Lean Concrete	**	47.74 m3
	$v^2 = Footing Volume$	· · ·	71.55 m3
	v3 = Occupied volume of piles = 4 x pi/4 x 1.200 ^2 x	1. A.	
	= 4 x pi/4 x 1.200 ^2 x x 0.60	-	2.71 m3
	v4 = Occupied volume of wall		2/1 00
	$=$ (pi/4 x 1.400 ^2 + 2.90 x	1.40)	
	x 1.400		7.84 m3
	이 법원 문화가 있는 것이라. 이 문화 말을 하는 것이라. 한 것이다.	Total =	129.84 m3
 A state of the second seco	* Riprap = Excavation for footing - Excess Soil		
	≈ 372.36 - 129.84	2.40	en e
	+ 4 x pi/4 x 1.200 ^2 x	3.4U =	257.90 m3
)Cofferdams		. 	257.90 m3
j~~~43~5349₩₩₩₩₩	* Sheet pile larsen IV		
	104.00 nos L= 12 m		1248.00 m
	* Steel pile I 400		
and the second state of th	24.00 nos L= 12 m		255.00 m
	* Brace C 300		
	$L1 = (10.2 + 11.08) \times 7.00 \times$	4	
	L3 = 9.2 x 6 =	595.84 m 55.2 m	
	\mathbf{I}_{i} , we can also be a set of the se	otal	651.04 m
	a da anti-a da anti-		
	n a she a ta an	e e grad	
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DITT I T		D	LENGTH	QUANTITY	UNIT WEIGH	TOTAL STEEL
DETAIL	No	(mm)	(m)	(nos.)	(kg/m)	(kg)
	R1	10	5600	6	0.617	20.7
	B1	28	7868	14	4.834	532.5
	B2	28	8790	6	4.834	254.9
	B2a	28	8390	7	4.834	283.9
	B3	16	6800	5	1.578	53.7
	B3a	16	6054	6	1.578	57.3
	B4	14	4907	23	1.208	136.3
PIER CAP	B4a	14	4257	23	1.208	118.3
PIER CAP	B4b	14	1979	23	1.208	55.0
[]	B4c	14	3557	23	1.208	98.8
	B4d	14	2329	23	1.208	64.7
	B5	14	4280	12	1.208	62.0
	B5a	14	3558	12	1.208	51.6
	.B5b	14	1630	12	1.208	23.6
	B5c	14	2858	12	1.208	41.4
	B5d	14	2329	16	1.208	45.0
	C1	28	9034	68	4.834	2969.6
COLUMN	C2	14	10794	27	1.208	352.1
COLUMIA	C3	16	3708	18	1.578	105.3
	C3a	16	3658	12	1.578	69.3
	F1	22	6570	21	2.984	411.7
	F2	22	9300	39	2.984	1082.3
	F3	22	6570	21	2.984	411.7
FOOTING	F4	32	9300	39	6.313	2289.7
	F5	20	5800	16	2.466	228.8
	F6	20	6500	16	2.466	256.5
	F7	16	4698	45	1.578	333.6
· .		D = 10	20.7	ing she water	(kg)	
		D = 14	1048.8		(kg)	
	÷ .	D = 16	619.2	tan ang Kang	(kg)	and the second
TOTAL		D = 20	485.3		(kg)	
	and a second	D = 22	1905.7	•	(kg)	
		D = 28	4040.9		(kg)	
		D = 32	2289.7		(kg)	
1		TOTAL	10410.3		(kg)	

STEEL OF PIERS P1 & P2 (FOR ONE PIER)

DEINICODOCNENT

ST OF RE	EINFORCE	MENT	· · · · ·	· · · · · · · · · · · · · · · · · · ·			P
SIGN	OIACETER	LENGTH	NOS.	UNIT WEIGHT	PIECE WEIGHT	TOTAL WEIGHT	NOTE
- 1	D22	9,900	12	2,984	29.500	354.01	
2	- D6	1,614	95	0.222	0.358	34.0	
3	D6	490	120	0.222	0.109	13.1	
4	D25	1,911	2	3,854	7.360	14.7	
5	D22	350	8	2.984	1.040	8.3	
8	D25	0	0	3.854	0.000	0.0	
9	D16	1,508	8	1.579	2.380	19.0	
	· ·				TOTAL	443.1	·
	[and the second		e state			
	I	1		the state of the state			
	1				2.1		
			1	1. A. A. A.	D32	0.0	
					D25	14.7	
					D22	362.3	
	1				D16	19.0	· .
			1		\$ 6	47.1	
· · ·		·.·				443,1	
	<u> </u>						

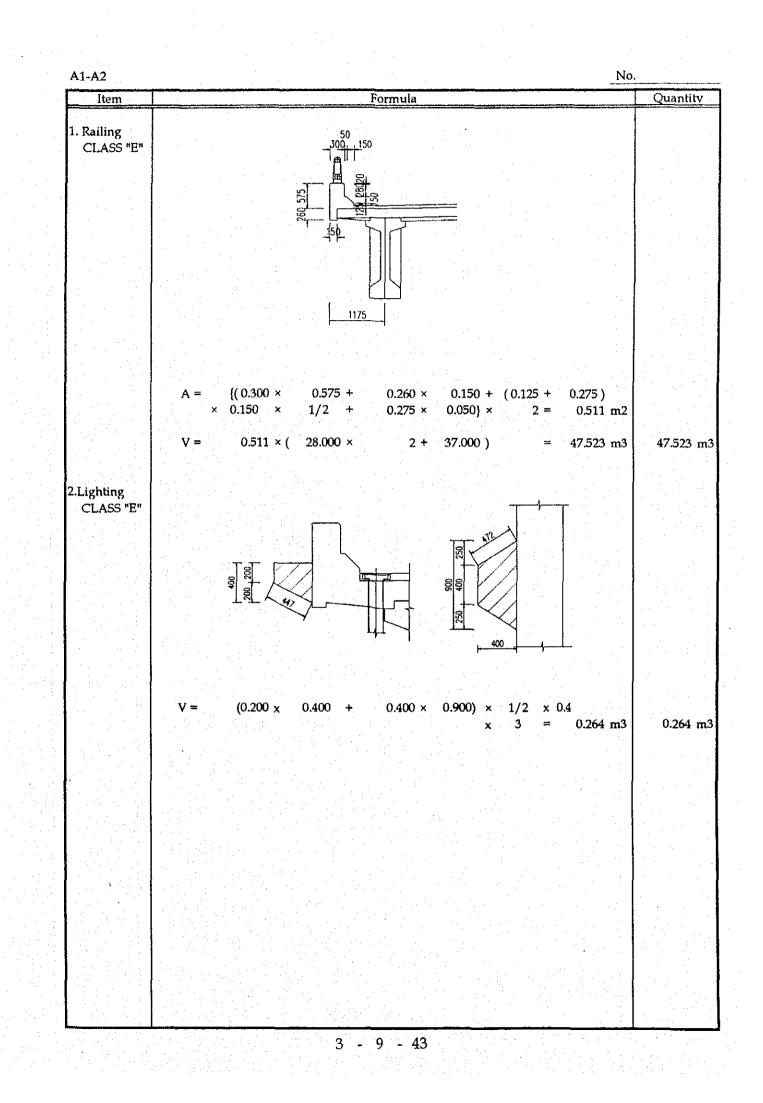
SIGN	DIACETER	LENGTH	NOS.	UNIT WEIGHT	PIECE WEIGHT	TOTAL WEIGHT	NOTE
1a	D22	10,589	4	2.984	31.600	126.4	
1b	D22	10,612	4	2.984	31.700	126.8	(AVE)
2	D6	1,614	95	0.222	0.358	34.0	
3	D6	490	60	0.222	0.109	6,5	
4	D25	1,911	2	3.854	7.360	14.7	1.1.1
5	D22	350	8	2.984	1.040	8.3	
6	D6	9,460	1	0.222	2.100	2.1	
7	D32	810	1	6.313	5,110	5.1	(AVE)
8	D25	0	0	3.854	0.000	0.0	
9	D16	1,508	4	1.579	2.380	9.5	
	<u> </u>				TOTAL	333.4	
	$(-1)^{-1} (1-\beta)^{-1} (1-\beta)^{-1}$	1.1.1		1. T	1		
					and the second		[
	·				1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 -		Γ
				ta en la composición de	D32	5.1	
		·			D25	14.7	
					022	261.5	
• . •					D16	9,5	· · ·
				· · · ·	φ6	42.6	
	1					333,4	

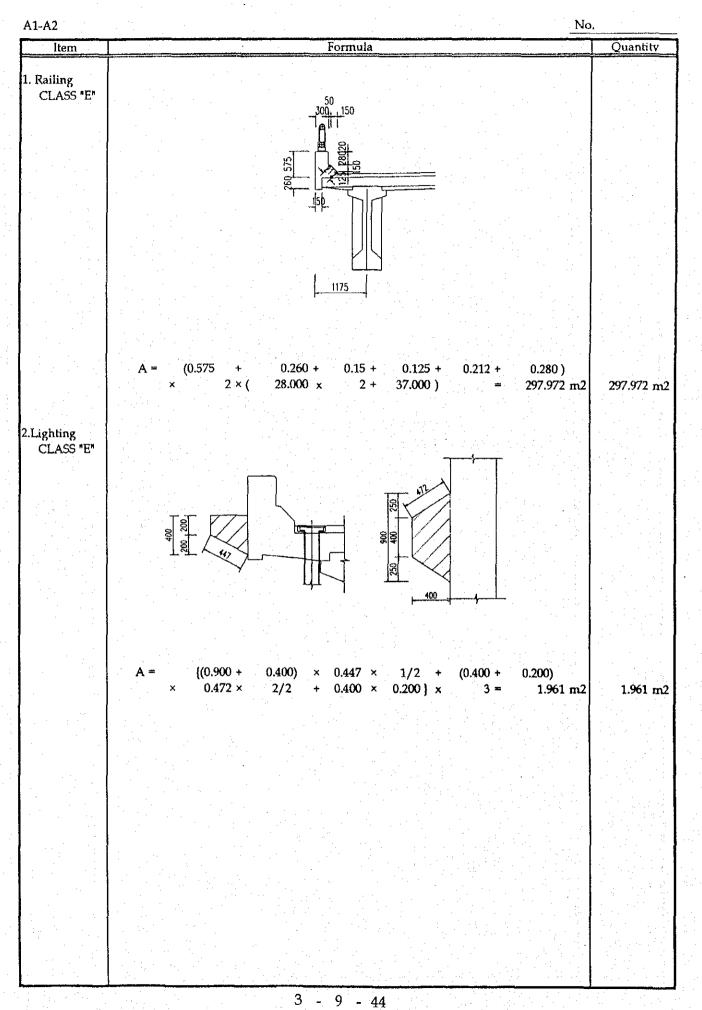
		 _	_
	· ·		
TOTAL			

TOTAL			
T	Pile-2	Pile-3	Total
กอร	3	1	
D32	0.0	5.1	5.1
D25	44.1	.14.7	58,8
D22	1086.9	261.5	1348.4
D16	57.0	9.5	66.5
¢6	141.3	42,6	183.9
	1329,300	333,400	1662.7

		11.		
	ITEMS		UNIT	TOTAL
PARAPET	CONCRETE CLASS E		m ³	48
FANALEI	REINFORCEMENT	D14	kg	7408
RAILING			m	205
	LIGHTING POLES		poles	3
	CONCRETE CLASS E		m ³	0.3
LIGHTING	REINFORCEMENT	D16	kg	32.6
		D20	kg	62.0
	PVC PILE Ø 100MM		m	187
DRAINAGE	DRAINAGE		set	14
DIAMAGE	PIPE Ø 180MM		m	24

QUANTITY FOR MISCELLANEOUS





A1-A2					К
Item					Formula
	1. DRAIN		·	с. 1. т	

TOTAL EACH	
EACH = 7 ×	2
PIPE = 1.74 ×	14

2. RAILING

EACH LE	NGTH for SI	PAN			
L≖	28.150 +	37.200 +	28.150	± • •	93.500
EACH =	3 SPA	N	teres and Alteres and		• • •
		an fan Seren en Sere			

TOTAL LENGTH

L = 93.500 × 2 205.400 m 2 + -4.6 x 2 x ==

3. LIGHTING

EACH for C	Dne SPAN
EACH =	3
PVC Φ	100 MM

3 187.00 m

=

=

No.

=

=

Quantity

EACH

m

14

24

3 - 9 - 45

A1-A2

				. 1			1.1		1997 - 1997 -			Par B	RIDGE
				SCHE	DULE (OF REII	NFOR	CEMEN	IT (OF RA	ILING)	te e gita		
BAR	SIZE			DIME	NSION	5 (mm)			LENGTH	UNIT WEIGHT	NO. OF	WEIGHT	REMARKS
MARK	(mm)	a	b	С	d	e	f	g	(mm)	(kg/m)	BARS	(kg)	· .
P1	14	250	701	212	747	70			1980	1.208	68	162.7	
P2	14	230	297	176	275	245			1223	1.208	68	100.5	andre an Andre andre and
P3	14	10000							10000	1.208	11	132.9	
										· · · ·	· .	a sa sa sa sa	
· · · · · · ·							_			:			
							• • •						
													•
										1			
Total	<u> </u>						1. <u>.</u>					396.1	

Total Weight

Total = $396.13 / 10 \times 93.5 \times 2 = 7,407.63$ (kg)

A1-A2

No.

1	. ¹		•.		А. н. н.							Par B	RIDGE
			(SCHE	DULE C	OF REI	NFOR	CEMEN	T (OF LIG	HTING)			
BAR	SIZE			DIME	INSION	5 (mi	1)	· .'	LENGTH	UNIT WEIGHT	NO. OF	WEIGHT	REMARKS
MARK	(mm)	a	b	c	d	e	f	g	(mm)	(kg/m)	BARS	(kg)	
P1	20	113	825	320					1258	2.466	3	9.31	
P2	16	113	380	488	280				1261	1.578	3	5.97	
P3	20	1000	300	1000					2300	2.466	2	11.34	
P4	16	282	285	282					849	1.578	2	2.68	1
P5	16	208	285	208					701	1.578	2	2.21	
		1 A 1					- 1 - L						
					1								
									• •				
Total		İ								at Prove		31.51	1. 1. 10. 1.

×

3

47

Total Weight

Total = 31.51

3

94.53 (kg)

QUANTITY OF RAMPWAY *D* BRIDGE - INTERCHANGE 3

IT .	EMS	UNIT	ABUTMENTS	PIERS	SUPERSTRUCTURE	MISCELLANEOUS	TOTAL
	CLASS B	МЗ		· · · ·	210.6		210.6
()) ())	CLASS D	M3	390.4	203.3	229.4		823.1
CONCRETE	CLASS E	M3	316.7	246.8		47.5	611.1
	CLASS G	M3	15.2	95.5			110.7
· · · · · · · · · · · · · · · · · · ·	12 S 12.7	tori	· · · · · · · · · · · · · · · · ·		9.8		9.8
PC - STEEL	3 S 12.7	ton			6.3		6.3
	CABLES Ø 80/85	м		• :	1051.1		1051.1
SHEATHING	CABLES Ø 50/55	M			203.2		203.2
CEMENT GROUT IN SHEATH	M3			5.7		5.7	
	CABLES 12512.7	SET			66.0		66.0
ANCHORAGE	CABLES 3S12.7	SET		·····	44.0		44.0
STEEL SHEAR KEY		SET			72.0		72.0
REINFORCEMENT	RE-BAR	TON	92.2	62.4	80.9	7.5	243.0
EXPANSION JOINT	50MM	M			26.0		26.0
	600x300x57	SET			6.0		6.0
BEARING	500x250x50	SET			12.0		12.0
ANCHORAGE BAR		SET			12.0		12.0
	Ø 50 MM	M	67.5				67.5
PVC PIPE	Ø 100 MM	M	07.0				0.0
RAILING		M				205.4	205.4
LIGHTING POLE		SET				3.0	3.0
	POT	SET			······	14.0	14.0
DRAINAGE	PIPE Ø 180 MM	M				24.4	24.4
	WATER PROOFING 5 MM	M2			607.8	41.1	607.8
PAVEMENT	ALPHALT CONCRETE 70 M	M2			607.8		607.8
Rc Pile 450mm	ALTIALI CONCILLE/VIII	M	1920.0	208.0			2128.0
GEOTEXTILE		M2	1920.0	200.0			444.0
STONE MANSORY			·····				428.4
BLINGDING AGGREGATE		M3 M3	428.4				123.7
RIP RAP		M3	143.7	704.6			704.6
BLINDING STONE		M3	28.2	703.0			28.2
WOODEN PIPE L = 3M		M	8035.0				8035.0
		M M3	2123.5	935.7			3059.2
FILLING		M3	1742.6	933.71			1742.6
	CURVE	h	1/42.0	102.0			1/42.0
FORM WORK	FLAT	M2	446.0		5054.0		5664.2
	H < 4M	M2	446.9 135.8	163.3 232.0			367.8
SCAFFOLDING WORK		M2 M2					
SUPPORT	4 M < H < 30M	M2	474.6	280.8			
		M3	20.5	14.0			34.6
COFFERDAMS	SHEET PILE LARSEN IV	M		2496.0			2496,0
COLLEND AND	STEEL PILE I 400	M		576.0			576.0
1. A	BRACE C 300	M		1302.1			1302.1

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