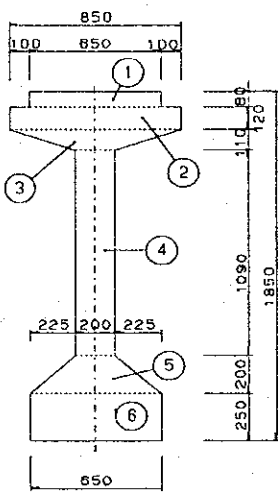
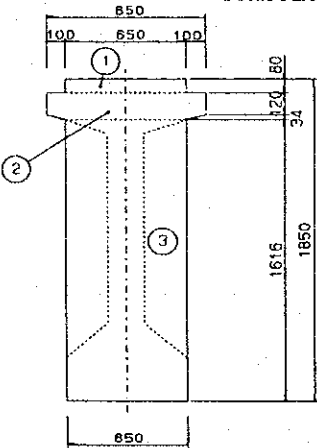


3.3. Tra On Bridge

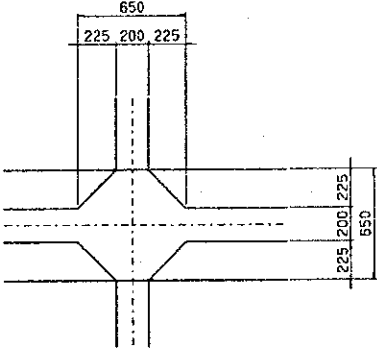
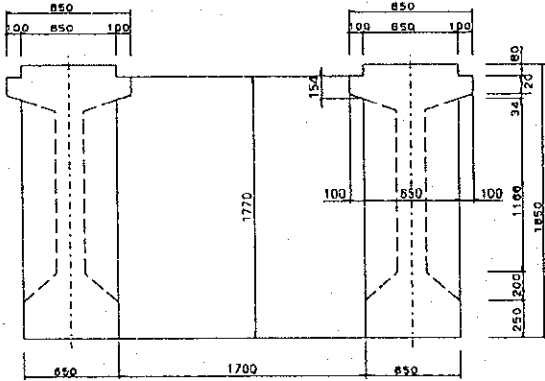
1.Quantity of Superstructure

Superstructure-Approach Bridge

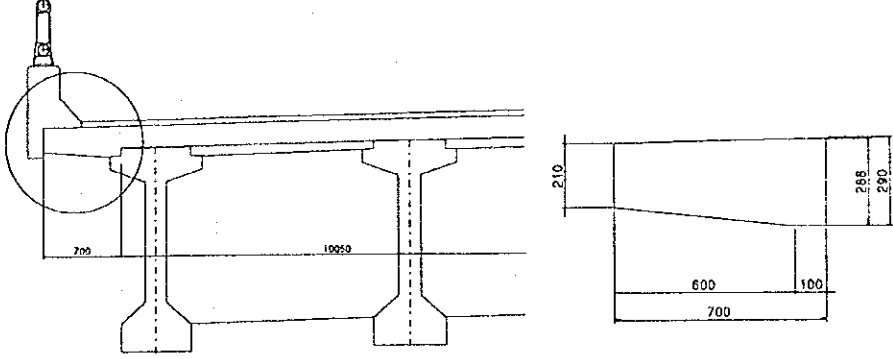
Item		Work Item		Unit	Quantity	Remarks
Concrete	CLASS B	Girder		cu.m	1004.6	$\sigma_{ck}=40\text{Mpa}$
	CLASS D-1	Deck Slab		cu.m	637.7	$\sigma_{ck}=30\text{Mpa}$
		Diaphragm		cu.m	175.9	
		Total		cu.m	813.7	
Form		Deck Slab		sq.m	739.5	
		Girder		sq.m	5619.7	
		Diaphragm		sq.m	485.0	
		Total		sq.m	6844.2	
Re-bar		Deck Slab	- D14	ton	40.1	
			D14 - D25	ton	113.4	
			D25 -	ton		
			Total	ton	153.5	
		Girder	- D14	ton	30.0	
			D14 - D25	ton	164.7	
			D25 -	ton	-	
			Total	ton	194.8	
		Diaphragm	- D14	ton	6.2	
			D14 - D25	ton	13.2	
			D25 -	ton	-	
			Total	ton	19.4	
		Total	- D14	ton	76.4	
D14 - D25	ton		291.3			
D25 -	ton		0.0			
Total	ton		367.7			
PC Cable	12S12.7B		ton	54.3	SWPR7B	
	Sheathing	3S12.7B	Transverse Tendons	ton	1.7	
		$\Phi 80/85$		m	5845.2	
		$\Phi 50/55$		m	723.6	
		Segment Grout in Sheathing		m ³	30.8	
Expansion Joint		Type A	m	43.0		
Pavement	t=70mm	Asphalt concrete surface course		sq.m	2758.5	
	t=5mm	Water proofing		sq.m	2758.5	
		Concrete		cu.m	4.3	
		Road Marking		sq.m	129.7	
Bearing		Product layer rubber bearing		nos	80	
	Anchor Bar	$\Phi 75$ L=1500	nos	160		
		$\Phi 95$ L=800	nos	48		
		$\Phi 100$ L=800	nos	8		
		$\Phi 125$ L=800	nos	8		
Concrete Form			sq.m	439.3		
			cu.m	35.1		
		D14	ton	5.0		
Shear Key			nos	450		
Erection			ton	69.9	per one girder (MAX)	
			ton	2511.5		

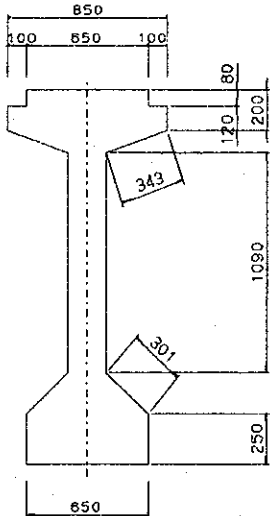
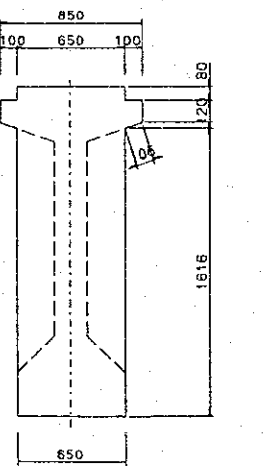
Item	Formula	Quantity																																													
1. Girder Concrete CLASS "B" 1) Girder	<p>(1) Calculation of Sectional Area</p> <p>[1] Middle Section</p> <p>Summary of Sectional Area For ONE GIRDER</p> <table border="1" data-bbox="359 448 1093 772"> <thead> <tr> <th>No</th> <th>Formula</th> <th>(m²)</th> </tr> </thead> <tbody> <tr> <td>1</td> <td>0.650×0.080</td> <td>= 0.052</td> </tr> <tr> <td>2</td> <td>0.850×0.120</td> <td>= 0.102</td> </tr> <tr> <td>3</td> <td>$1/2 \times 0.325 \times 0.110 \times 2$</td> <td>= 0.036</td> </tr> <tr> <td></td> <td>0.200×0.110</td> <td>= 0.022</td> </tr> <tr> <td>4</td> <td>1.090×0.200</td> <td>= 0.218</td> </tr> <tr> <td>5</td> <td>$1/2 \times 0.225 \times 0.200 \times 2$</td> <td>= 0.045</td> </tr> <tr> <td></td> <td>0.200×0.200</td> <td>= 0.040</td> </tr> <tr> <td>6</td> <td>0.650×0.250</td> <td>= 0.163</td> </tr> </tbody> </table> <p style="text-align: right;">Total Area 0.678 m²</p>  <p>[2] End Section</p> <p>Summary of Sectional Area For ONE GIRDER</p> <table border="1" data-bbox="359 1366 1093 1579"> <thead> <tr> <th>No</th> <th>Formula</th> <th>(m²)</th> </tr> </thead> <tbody> <tr> <td>1</td> <td>0.650×0.080</td> <td>= 0.052</td> </tr> <tr> <td>2</td> <td>0.120×0.850</td> <td>= 0.102</td> </tr> <tr> <td></td> <td>$1/2 \times 0.100 \times 0.034 \times 2$</td> <td>= 0.003</td> </tr> <tr> <td></td> <td>0.650×0.034</td> <td>= 0.022</td> </tr> <tr> <td>3</td> <td>0.650×1.616</td> <td>= 1.050</td> </tr> </tbody> </table> <p style="text-align: right;">Total Area 1.229 m²</p> 	No	Formula	(m ²)	1	0.650×0.080	= 0.052	2	0.850×0.120	= 0.102	3	$1/2 \times 0.325 \times 0.110 \times 2$	= 0.036		0.200×0.110	= 0.022	4	1.090×0.200	= 0.218	5	$1/2 \times 0.225 \times 0.200 \times 2$	= 0.045		0.200×0.200	= 0.040	6	0.650×0.250	= 0.163	No	Formula	(m ²)	1	0.650×0.080	= 0.052	2	0.120×0.850	= 0.102		$1/2 \times 0.100 \times 0.034 \times 2$	= 0.003		0.650×0.034	= 0.022	3	0.650×1.616	= 1.050	
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Total			31.600	24.626																																														
	No. of Girder 5 × 2 = 10																																																	
	V2= 24.626 × 10	= 246.260 m ³																																																
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Total			28.600	22.316																																														
	No. of Girder 5 × 2 = 10																																																	
	V3= 22.316 × 10	= 223.160 m ³																																																
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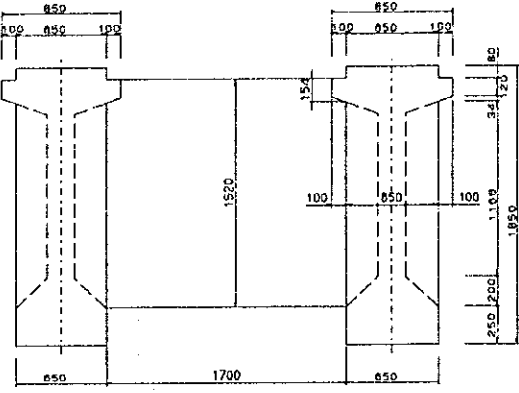
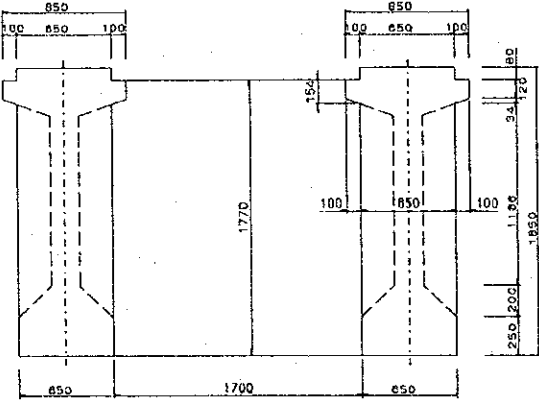
Item	Formula	Quantity
2. Diaphragm Concrete CLASS "D-1" 1) Diaphragm	 $V = \left(\frac{0.200 + 0.650}{2} \right) \times 1/2 \times 0.225 \times 2 = 0.235$ $N1 = 5 \times 3 \times 2 = 30$ $N2 = 5 \times 3 \times 2 = 30$ $N3 = 5 \times 2 \times 2 = 20$ $N4 = 5 \times 3 \times 2 = 30$ $N = 110$ $V1 = 0.235 \times 110 = 25.850 \text{ m}^3$	
2) Cross Beam	(1) End Cross Beam  $V2-1 = \left\{ 1.770 \times 1.700 - \frac{(0.120 + 0.154) \times 0.100}{2} \right\} \times 2 \times 0.5 \times 4 \times 2 \times 4 = 47.706 \text{ m}^3$	

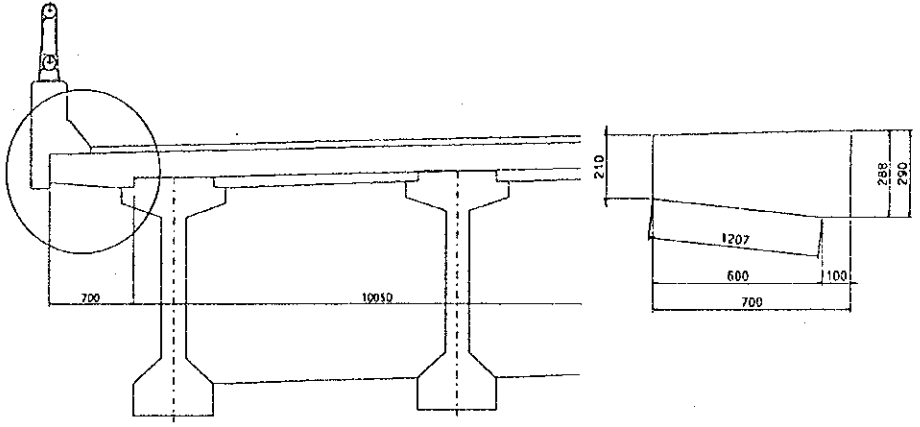
Item	Formula	Quantity
	<p>(2) Intermediate Cross Beam</p> $V = \{ 1.520 \times 1.700 - (0.120 + 0.154) \times \frac{1}{2} \times 2 \times 0.100 \} \times 0.2 \times 4 = 2.045 \text{ m}^3$ <p> $N1 = 4 \times 3 \times 2 = 24$ $N2 = 4 \times 3 \times 2 = 24$ $N3 = 4 \times 2 \times 2 = 16$ $N4 = 4 \times 3 \times 2 = 24$ $N = 88$ </p> $V2-2 = 2.045 \times 88 = 179.960 \text{ m}^3$ <p>(3) Connection Cross Beam</p> $V2-3 = \{ 1.770 \times 1.700 - (0.120 + 0.154) \times \frac{1}{2} \times 0.100 \times 2 \} \times 2.000 \times 4 + \text{Girder section area} \times 5 \times 0.200 = 100.327 \text{ m}^3$ $V2 = 47.706 + 2.045 + 100.33 = 150.078 \text{ m}^3$ <p style="text-align: right;">Total 175.928 m³</p>	<p style="text-align: right;">175.928 m³</p>

Item	Formula	Quantity
3. Deck Slab Concrete CLASS "D-1"	 $ \begin{aligned} A1 &= (0.21 + 0.288) \times \frac{1}{2} \times 0.600 \times 2 = 0.299 \\ A2 &= (0.288 + 0.290) \times \frac{1}{2} \times 0.100 \times 2 = 0.058 \\ A3 &= 10.050 \times 0.210 = 2.111 \\ \Sigma A &= 2.468 \text{ m}^2 \\ \\ L1 &= 72.400 - 0.200 \times 2 = 72.000 \\ L2 &= 57.600 - 0.200 \times 2 = 57.200 \\ L3 &= 63.600 - 0.200 \times 2 = 63.200 \\ L4 &= 66.400 - 0.200 \times 2 = 66.000 \\ L &= 258.400 \\ \\ V &= 2.468 \times 258.400 = 637.731 \end{aligned} $	<p style="text-align: right;">637.731 m³</p>

Item	Formula	Quantity
1. Girder		
1) Girder	(1) Calculation of Sectional Area	
	[1] Middle Section	
	Summary of Sectional Length	For ONE GIRDER
No	Formula	(m)
1	0.080×2	= 0.160
2	0.120×2	= 0.240
3	0.343×2	= 0.686
4	1.090×2	= 2.180
5	0.301×2	= 0.602
6	0.250×2	= 0.500
		Total Area 4.368 m
		
	[2] End Section	
	Summary of Sectional Length	For ONE GIRDER
No	Formula	(m)
1	0.080×2	= 0.160
2	0.120×2	= 0.240
3	0.106×2	= 0.212
4	1.616×2	= 3.232
		Total Area 3.844 m
		

Item	Formula					Quantity
(2) Calculation of Form Area						
Girder Length 36.000 m						
Section No.	Section Length (m)	Average of Section (m)	Length of Block (m)	Form Area (m ²)	Remark	
END	3.844			1.229	End-sec	
END	3.844	3.844	0.500	1.922		
MIDDLE	4.368	4.106	5.400	22.172		
MIDDLE	4.368	4.368	24.200	105.706		
END	3.844	4.106	5.400	22.172		
END	3.844	3.844	0.500	1.922		
				1.229	End-sec	
Total			36.000	156.352		
No. of Girder 5 × 2 = 10						
A1	= 156.352 × 10					= 1563.520 m ²
Girder Length 31.600 m						
Section No.	Section Length (m)	Average of Section (m)	Length of Block (m)	Form Area (m ²)	Remark	
END	3.844			1.229	End-sec	
END	3.844	3.844	0.500	1.922		
MIDDLE	4.368	4.106	4.800	19.709		
MIDDLE	4.368	4.368	21.000	91.728		
END	3.844	4.106	4.800	19.709		
END	3.844	3.844	0.500	1.922		
				1.229	End-sec	
Total			31.600	137.448		
No. of Girder 5 × 2 = 10						
A2	= 137.448 × 10					= 1374.480 m ²
Girder Length 28.600 m						
Section No.	Section Length (m)	Average of Section (m)	Length of Block (m)	Form Area (m ²)	Remark	
END	3.844			1.229	End-sec	
END	3.844	3.844	0.500	1.922		
MIDDLE	4.368	4.106	4.300	17.656		
MIDDLE	4.368	4.368	19.000	82.992		
END	3.844	4.106	4.300	17.656		
END	3.844	3.844	0.500	1.922		
				1.229	End-sec	
Total			28.600	124.606		
No. of Girder 5 × 2 = 10						
A3	= 124.606 × 10					= 1246.060 m ²
Girder Length 33.000 m						
Section No.	Section Length (m)	Average of Section (m)	Length of Block (m)	Form Area (m ²)	Remark	
END	3.844			1.229	End-sec	
END	3.844	3.844	0.500	1.922		
MIDDLE	4.368	4.106	4.800	19.709		
MIDDLE	4.368	4.368	22.400	97.843		
END	3.844	4.106	4.800	19.709		
END	3.844	3.844	0.500	1.922		
				1.229	End-sec	
Total			33.000	143.563		
No. of Girder 5 × 2 = 10						
A4	= 143.563 × 10					= 1435.630 m ²
A = 5619.69						5,619.7 m ²

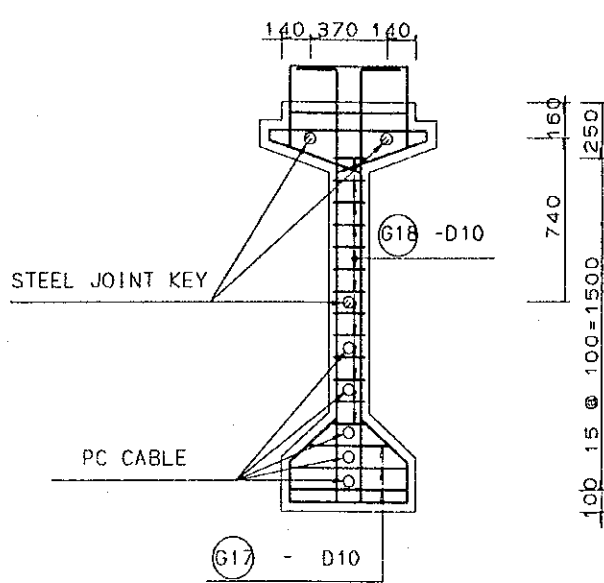
Item	Formula	Quantity												
	<p data-bbox="363 241 667 275">(2) Intermediate Cross Beam</p>  <table border="1" data-bbox="608 779 874 981"> <thead> <tr> <th></th> <th>N</th> </tr> </thead> <tbody> <tr> <td>L=72.4</td> <td>12</td> </tr> <tr> <td>L=63.6</td> <td>12</td> </tr> <tr> <td>L=57.6</td> <td>8</td> </tr> <tr> <td>L=66.4</td> <td>12</td> </tr> <tr> <td>Total</td> <td>44</td> </tr> </tbody> </table> $A2-2 = \left[\left(1.520 \times 1.700 - (0.120 + 0.154) \times \frac{1}{2} \times 0.100 \right) \times 2 \right] \times 2 + 1.700 \times 0.200 \times 44 = 239.941 \text{ m}^2$ <p data-bbox="363 1220 657 1254">(3) Connection Cross Beam</p>  $A2-3 = \left[\left(1.770 \times 1.700 - (0.120 + 0.154) \times \frac{1}{2} \times 0.100 \right) \times 2 \times 2 \right] + 1.700 \times 2.000 \times 4 = 25.417 \text{ m}^2$ $V2 = 121.734 + 239.941 + 25.417 = 387.092 \text{ m}^2$ <p data-bbox="1007 1989 1437 2022">Total 485.014 m² 485.014 m²</p>		N	L=72.4	12	L=63.6	12	L=57.6	8	L=66.4	12	Total	44	
	N													
L=72.4	12													
L=63.6	12													
L=57.6	8													
L=66.4	12													
Total	44													

Item	Formula	Quantity
3. Deck Slab	 <p>The drawing shows a cross-section of a deck slab supported by two columns. The total length of the slab is divided into four segments: 72.400, 57.600, 63.600, and 66.400. A circular detail on the left shows a corner with a radius of 0.210. A trapezoidal detail on the right shows a top width of 0.210, a bottom width of 0.700, a height of 0.600, and a top edge with a slope of 1:2 (1207). The bottom edge of the trapezoid has a width of 0.100. The total length of the slab is calculated as L = 72.400 + 57.600 + 63.600 + 66.400 - 0.200 x 4 = 259.200. The area calculations are: A1 = (0.210 + 1.207) x 2 x 259.200 = 734.573 m²; A2 = (0.200 + 0.288) x 1/2 x 0.600 x 4 = 0.586 m²; A3 = (0.288 + 0.290) x 1/2 x 0.100 x 4 = 0.116 m²; A4 = 10.050 x 0.210 x 2 = 4.221 m². The total area A = 739.496 m².</p> $L = 72.400 + 57.600 + 63.600 + 66.400 - 0.200 \times 4 = 259.200$ $A1 = (0.210 + 1.207) \times 2 \times 259.200 = 734.573 \text{ m}^2$ $A2 = (0.200 + 0.288) \times \frac{1}{2} \times 0.600 \times 4 = 0.586 \text{ m}^2$ $A3 = (0.288 + 0.290) \times \frac{1}{2} \times 0.100 \times 4 = 0.116 \text{ m}^2$ $A4 = 10.050 \times 0.210 \times 2 = 4.221 \text{ m}^2$ $A = 739.496 \text{ m}^2$	739.496 m ²

Item	Formula						Quantity
PC CABLE 1) 12S12.7	L=36.0m For One Girder						
	CABLE VAR.	EACH LENGTH	CABLE NO.	EACH	TOTAL LENGTH	UNIT WEIGHT	WEIGHT
	1	35.702	C1	1	34.702	9.290	322.4
	2	35.714	C2	1	34.714	9.290	322.5
	3	35.754	C3	1	35.754	9.290	332.2
	4	35.784	C4	1	35.784	9.290	332.4
	5	35.822	C5	1	35.822	9.290	332.8
	TOTAL			5	176.776		1642.3
	SUB-TOTAL WEIGHT OF PC CABLES per BRIDGE						
	$W_p = 1642 \times 5 \times 2 =$						16,423.0 kgf 16.4 ton
	TENSION UNIT						
	$N_s = 5 \times 2 \times 2 =$						20.0 nos
	HEATING Φ 80/85						
	$L = 176.776 \times 5 \times 2 =$						1,767.8 m
	CENEBT GROUT						
	$V = 0.888 \times 5 \times 2 =$						8.9 m3
	L=31.6m For One Girder						
	CABLE VAR.	EACH LENGTH	CABLE NO.	EACH	TOTAL LENGTH	UNIT WEIGHT	WEIGHT
	1	31.302	C1	1	31.302	9.290	290.8
2	31.332	C2	1	31.332	9.290	291.1	
3	31.394	C3	1	31.394	9.290	291.7	
4	31.456	C4	1	31.456	9.290	292.2	
TOTAL			4	125.484		1165.8	
SUB-TOTAL WEIGHT OF PC CABLES per BRIDGE							
$W_p = 1166 \times 5 \times 2 =$						11,658.0 kgf 11.7 ton	
TENSION UNIT							
$N_s = 4 \times 2 \times 2 =$						16.0 nos	
HEATHING Φ 80/85							
$L = 125.484 \times 5 \times 2 =$						1,254.8 m	
CENEBT GROUT							
$V = 0.630 \times 5 \times 2 =$						6.3 m3	

Item	Formula						Quantity
	L=28.6m						
	For One Girder						
	CABLE VAR.	EACH LENGTH	CABLE NO.	EACH	TOTAL LENGTH	UNIT WEIGHT	WEIGHT
	1	28.302	C1	1	28.302	9.290	262.9
	2	28.332	C2	1	28.332	9.290	263.2
	3	28.394	C3	1	28.394	9.290	263.8
	4	28.456	C4	1	28.456	9.290	264.4
	TOTAL			4	113.484		1054.3
	SUB-TOTAL WEIGHT OF PC CABLES per BRIDGE						
	$W_p = 1054 \times 5 \times 2$						= 10,543.0 kgf 10.5 ton
	TENSION UNIT						
	$N_s = 4 \times 2 \times 2$						= 16.0 nos
	HEATING						
	$L = 113.484 \times 5 \times 2$						= 1,134.8 m
	CENEPT GROUT						
	$V = 0.570 \times 5 \times 2$						= 5.7 m3
	L=33.2m						
	For One Girder						
	CABLE VAR.	EACH LENGTH	CABLE NO.	EACH	TOTAL LENGTH	UNIT WEIGHT	WEIGHT
	1	33.702	C1	1	33.702	9.290	313.1
	2	33.714	C2	1	33.714	9.290	313.2
	3	33.754	C3	1	33.754	9.290	313.6
	4	33.784	C4	1	33.784	9.290	313.9
	5	33.822	C5	1	33.822	9.290	314.2
	TOTAL			5	168.776		1568.0
	SUB-TOTAL WEIGHT OF PC CABLES per BRIDGE						
	$W_p = 1568 \times 5 \times 2$						= 15,680.0 kgf 15.7 ton
	TENSION UNIT						
	$N_s = 5 \times 2 \times 2$						= 20.0 nos
	HEATING $\Phi 80/85$						
	$L = 168.776 \times 5 \times 2$						= 1,687.8 m
	CENEPT GROUT						
	$V = 0.848 \times 5 \times 2$						= 8.5 m3
	Total						$W_p =$ 54,304.0 kgf 54.3 ton
							$N_s =$ 72.0 nos
							$L =$ 5,845.2 m
							$V =$ 29.4 m3

Item	Formula						Quantity	
2) 3S12.7	PC CABLE OF DIAPHRAGMS							
	LOCATION	EACH LENGTH	CABLE NO.	EACH	TOTAL LENGTH	UNIT WEIGHT		WEIGHT
	Connection One	10.050		72	723.6	2.322		1680.199
	TOTAL			72	723.600			1680.199
	TOTAL WEIGHT OF PC CABLES per BRIDGE(A1-P12)							
	$W_p = 1680.199$							=
								1,680.2 kgf
								1.7 ton
TENSION UNIT								
$N_s = 72 \times 2$						=		
HEATHING $\Phi 50/55$						144.0 nos		
$L = 723.600$						=		
CENEBT GROUT						723.6 m		
$V = 0.002 \times 723.600$						=		
						1.4 m ³		

Item	Formula	Quantity
Shear Key	 <p data-bbox="446 616 654 660">STEEL JOINT KEY</p> <p data-bbox="518 795 638 840">PC CABLE</p> <p data-bbox="654 896 813 940">G17 - D10</p> <p data-bbox="821 571 933 616">G18 - D10</p> <p data-bbox="702 369 861 414">140 370 140</p> <p data-bbox="973 459 1053 504">160</p> <p data-bbox="973 548 1053 593">740</p> <p data-bbox="973 638 1053 683">250</p> <p data-bbox="1021 728 1053 884">100 15 @ 100 = 1500</p>	
N1	3 x 4 x 10 x 3	= 360
N2	3 x 3 x 10 x 1	= 90
Total		= 450

Item	Formula	Quantity
Erection		
(1) Calculation of Sectional Area		
[1] Middle Section		
Summary of Sectional Area For ONE GIRDER		
No	Formula	(m ²)
1	0.650×0.080	= 0.052
2	0.850×0.120	= 0.102
3	$\frac{1}{2} \times 0.325 \times 0.110 \times 2$	= 0.036
4	0.200×0.110	= 0.022
4	1.090×0.200	= 0.218
5	$0.500 \times 0.225 \times 0.200 \times 2$	= 0.045
6	0.200×0.200	= 0.040
6	0.650×0.250	= 0.163
Total Area		0.678 m ²
[2] End Section		
Summary of Sectional Area For ONE GIRDER		
No	Formula	(m ²)
1	0.650×0.080	= 0.052
2	0.120×0.850	= 0.102
	$\frac{1}{2} \times 0.100 \times 0.034 \times 2$	= 0.003
	0.650×0.034	= 0.022
3	0.650×1.616	= 1.050
Total Area		1.229 m ²

Item	Formula	Quantity																																																
Concrete Volume	Girder Length 28.600 m																																																	
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	Total			28.600	22.316																																													
Erection Weight	per one girder																																																	
W 1 = 22.316 × 2500	=	55790.0 kgf																																																
Concrete Volume	Girder Length 31.600 m																																																	
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W 2 = 24.626 × 2500	=	61565.0 kgf																																																
Concrete Volume	Girder Length 33.000 m																																																	
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	Total			33.000	25.575																																													
Erection Weight	per one girder																																																	
W 3 = 25.575 × 2500	=	63937.5 kgf																																																
Concrete Volume	Girder Length 36.000 m																																																	
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	Total			36.000	27.942																																													
Erection Weight	per one girder																																																	
W 4 = 27.942 × 2500	=	69855.0 kgf																																																
W = (55790.0 + 61565.0 + 63937.5 + 69855.0) × 5 × 2	=	2511475.0 kgf 2511.5 ton																																																

Total Weight of Girder

	- D14	D14 - D25	Total	Note
L=36.0	8,279.8	45,929.0	54,208.8	
L=31.6m	7,463.8	40,517.0	47,980.8	
L=28.6m	6,581.2	35,806.0	42,387.2	
L=33.0m	7,715.8	42,468.0	50,183.8	
Total	30,040.6	164,720.0	194,760.6	(kgf)
	30.0	164.7	194.8	(ton)

Total Weight of Slab

	- D14	D14 - D25	Total	Note
L=28.6m	8,702.7	25,489.4	34,192.1	
L=31.6m	9,826.6	27,900.6	37,727.2	
L=33.0m	10,308.4	28,996.6	39,305.0	
L=36.0m	11,311.8	30,969.4	42,281.2	
Total	40,149.5	113,356.0	153,505.5	(kgf)
	40.1	113.4	153.5	(ton)

LIST OF REINFORCEMENT

I=57.4

SIGN	DIAMETER	LENGTH	NOS.	UNIT WEIGHT	PIECE WEIGHT	TOTAL WEIGHT	NOTE
S	1	D20	11,350	353	2.466	28.000	9,884.0
	2	D20	10,850	353	2.466	26.800	9,460.4 (AVE)
	3	D14	963	766	1.208	1.160	888.6
	4	D22	12,000	100	2.984	35.800	3,580.0
	5	D14	22,780	100	1.208	27.500	2,750.0
	5'	D14	25,206	100	1.208	30.400	3,040.0
	6	D25	11,350	30	3.854	43.700	1,311.0 (AVE)
	7	D25	10,850	30	3.854	41.800	1,254.0
	8	D12	565	4,032	0.888	0.502	2,024.1
						TOTAL	34,192.1
						D25	2565.0
						D22	3580.0
						D20	19344.4
						D14	6678.6
						D12	2024.1
						Total	34,192.1
							(kgf)
						- D14	8,702.7
						D14 - D25	25,489.4
						Total	34,192.1

LIST OF REINFORCEMENT

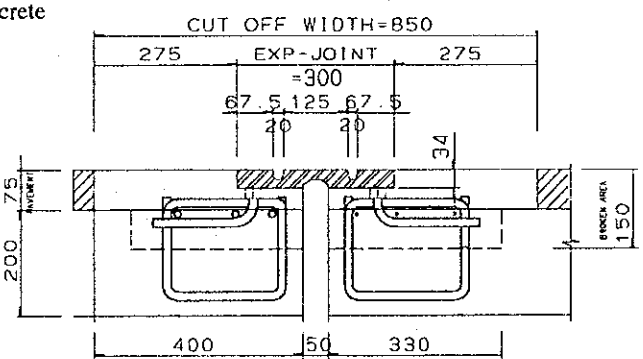
L=63.4

SIGN	DIAMETER	LENGTH	NOS.	UNIT WEIGHT	PIECE WEIGHT	TOTAL WEIGHT	NOTE
S	1	D20	11,350	397	2.466	28.000	11,116.0
	2	D20	10,850	397	2.466	26.800	10,639.6 (AVE)
	3	D14	963	854	1.208	1.160	990.6
	4	D22	12,000	100	2.984	35.800	3,580.0
	5	D14	26,206	100	1.208	31.700	3,170.0
	5'	D14	28,206	100	1.208	34.100	3,410.0
	6	D25	11,350	30	3.854	43.700	1,311.0 (AVE)
	7	D25	10,850	30	3.854	41.800	1,254.0
	8	D12	565	4,494	0.888	0.502	2,256.0
					TOTAL	37,727.2	
						D25	2565.0
						D22	3580.0
						D20	21755.6
						D14	7570.6
						D12	2256.0
						Total	37727.2
							(kgf)
						- D14	9,826.6
						D14 - D25	27,900.6
						Total	37,727.2

LIST OF REINFORCEMENT

L=72.2

SIGN	DIAMETER	LENGTH	NOS.	UNIT WEIGHT	PIECE WEIGHT	TOTAL WEIGHT	NOTE
S	1	D20	11,350	453	2.466	28.000	12,684.0
	2	D20	10,850	453	2.466	26.800	12,140.4 (AVE)
	3	D14	963	966	1.208	1.160	1,120.6
	4	D22	12,000	100	2.984	35.800	3,580.0
	5	D14	30,606	100	1.208	37.000	3,700.0
	5'	D14	32,606	100	1.208	39.400	3,940.0
	6	D25	11,350	30	3.854	43.700	1,311.0 (AVE)
	7	D25	10,850	30	3.854	41.800	1,254.0
	8	D12	565	5,082	0.888	0.502	2,551.2
					TOTAL	42,281.2	
					D25	2565.0	
					D22	3580.0	
					D20	24824.4	
					D14	8760.6	
					D12	2551.2	
					Total	42,281.2	
						(kgf)	
					- D14	11,311.8	11.3
					D14 - D25	30,969.4	31.0
					Total	42,281.2	42.3

Item	Formula	Quantity
9 Pavement		
1) Pavement t=75mm	$L = 72.400 + 63.600 + 57.600 + 66.400 = 260.0 \text{ m}$ $A = 10.750 \times (260.00 - 0.850 \times 4) = 2758.5 \text{ m}^2$	2758.5 m ²
2) Water Proofing t=5mm	$A = 2758.5 \text{ m}^2$	2758.5 m ²
3) Concrete	 <p style="text-align: right;">per one side</p> $A1 = 0.400 \times 0.075 - 0.1 \times 0.034 = 0.027$ $A2 = 0.075 \times 0.300 = 0.023$ <p style="text-align: right;">Total 0.050 m²</p> $V = 0.050 \times 10.750 \times 8 = 4.300 \text{ m}^3$	4.300 m ³
4) Road marking	$\text{Bridge Length } L = 260.000 - 0.600 = 259.400 \text{ m}$ <p style="text-align: center;">Side Line</p> $A1 = 259.400 \times 0.100 \times 4 = 103.760 \text{ m}^2$ <p style="text-align: center;">Center Line</p> $L = 259.400 / 5.000 = 51.9 \text{ m}$ $A2 = 2.500 \times 0.100 \times 51.9 \times 2 = 25.950 \text{ m}^2$ $\text{Total } 103.760 + 25.950 = 129.7 \text{ m}^2$	129.7 m ²

Item	Formula	Quantity
1. Expansion Joint (TYPE A)	<p>EACH LENGTH $L = 10.750$</p> <p>$N = 1 \times 4 = 4$</p> <p>TOTAL LENGTH $L = 10.750 \times 4 = 43.000 \text{ m}$</p>	
2. BEARING PAD ELASTOMERIC 600*300*57	<p>EACH for One SPAN</p> <p>$N = 5$</p> <p>TOTAL EACH $N = 5 \times 4 \times 4 = 80$</p>	nos
3. ANCHOR BAR	<p>$\Phi 75 \quad L=1520 \quad (\text{MOVE})$ $N = 4 \times 8 \times 2 = 64$</p> <p>$\Phi 75 \quad L=1520 \quad (\text{FIX})$ $N = 8 \times 3 \times 4 = 96$</p> <p style="text-align: right;">Total = 160 nos</p>	nos
4. ANCHOR CAP (SGP)	<p>$\Phi 95 \quad L=800 \quad (\text{FIX})$ $N = 4 \times 3 \times 4 = 48$</p> <p>$\Phi 125 \quad L=800 \quad (\text{MOVE})$ $N = 4 \times 2 = 8$</p> <p>$\Phi 150 \quad L=800 \quad (\text{MOVE})$ $N = 4 \times 2 = 8$</p>	nos
5. Railing	<p>$L = 72.400 + 63.600 + 57.600 + 66.400 = 260.0 \text{ m}$</p> <p>$L = 260.000 \times 2 \times 2 = 1040 \text{ m}$</p>	

Item	Formula	Quantity																														
Concrete Form	$L = 28.600 + 31.600 + 33.000 + 36.000 = 129.200$																															
	$A = 1.700 \times 129.200 \times 2 = 439.280 \text{ m}^2$	439.280 m ²																														
	$V = 439.28 \times 0.080 = 35.142 \text{ m}^3$	35.142 m ³																														
<table border="1"> <thead> <tr> <th>L</th> <th>L1</th> <th>L2</th> <th>L3</th> <th>L4</th> <th>N</th> </tr> </thead> <tbody> <tr> <td>28,600</td> <td>28,520</td> <td>160</td> <td>28,200</td> <td>5,500</td> <td>141</td> </tr> <tr> <td>31,600</td> <td>31,520</td> <td>160</td> <td>31,200</td> <td>8,500</td> <td>156</td> </tr> <tr> <td>33,000</td> <td>32,920</td> <td>160</td> <td>32,600</td> <td>9,900</td> <td>163</td> </tr> <tr> <td>36,000</td> <td>35,920</td> <td>160</td> <td>35,600</td> <td>12,900</td> <td>178</td> </tr> </tbody> </table>			L	L1	L2	L3	L4	N	28,600	28,520	160	28,200	5,500	141	31,600	31,520	160	31,200	8,500	156	33,000	32,920	160	32,600	9,900	163	36,000	35,920	160	35,600	12,900	178
L	L1	L2	L3	L4	N																											
28,600	28,520	160	28,200	5,500	141																											
31,600	31,520	160	31,200	8,500	156																											
33,000	32,920	160	32,600	9,900	163																											
36,000	35,920	160	35,600	12,900	178																											

Item	Formula						Quantity
Re-bar Per 28.6m							
BAR MARK	SIZE (mm)	LENGTH (mm)	WEIGHT/M (kgf/m)	WEIGHT /One (kgf)	NO. of BARS	WEIGHT (kgf)	
S1	D 14	1620	1.042	1.688	144	243	
S2	D 14	12000	1.042	12.504	20	250	
S3	D 14	5500	1.042	5.731	10	57	
						550	
$w_1 = 550.0 \times 2 = 1100 \text{ kgf}$							
Re-bar Per 31.6m							
BAR MARK	SIZE (mm)	LENGTH (mm)	WEIGHT/M (kgf/m)	WEIGHT /One (kgf)	NO. of BARS	WEIGHT (kgf)	
S1	D 14	1620	1.042	1.688	159	268	
S2	D 14	12000	1.042	12.504	20	250	
S3	D 14	8500	1.042	8.857	10	89	
						607	
$w_2 = 607.0 \times 2 = 1214 \text{ kgf}$							
Re-bar Per 33.0m							
BAR MARK	SIZE (mm)	LENGTH (mm)	WEIGHT/M (kgf/m)	WEIGHT /One (kgf)	NO. of BARS	WEIGHT (kgf)	
S1	D 14	1620	1.042	1.688	166	280	
S2	D 14	12000	1.042	12.504	20	250	
S3	D 14	9900	1.042	10.316	10	103	
						633	
$w_3 = 633.0 \times 2 = 1266 \text{ kgf}$							
Re-bar Per 36.0m							
BAR MARK	SIZE (mm)	LENGTH (mm)	WEIGHT/M (kgf/m)	WEIGHT /One (kgf)	NO. of BARS	WEIGHT (kgf)	
S1	D 14	1620	1.042	1.688	181	306	
S2	D 14	12000	1.042	12.504	20	250	
S3	D 14	12900	1.042	13.442	10	134	
						690	
$w_4 = 690.0 \times 2 = 1380 \text{ kgf}$							
$W = 4960 \text{ kgf}$							
						4,960 kgf	
						5.0 ton	

1.Quantity of Superstructure

Supersuructure Main Bridge

per one side

Item	CLASS B	Work Item	Unit	Quantity	Remarks	
Concrete	CLASS B	Total	cn.m	1439.9		
Form		External	Cantilever	sq.m	1420.4	
			Pier Head	sq.m	322.6	
			Support	sq.m	369.3	
			Total	sq.m	2112.4	
		Internal	Cantilever	sq.m	1033.6	
			Pier Head	sq.m	295.7	
			Support	sq.m	169.3	
			Total	sq.m	1498.6	
Total			sq.m	3611.0		
Re-bar		Total	~D14	ton	64.0	
			D14~D25	ton	264.5	
			D25~	ton		
			Total	ton	328.5	
PC Steel	12S12.7B	Internal Longitudinal Prestressing Ten	ton	68.8	SWPR7B	
	19S15.2B	External Longitudinal Prestressing Ten	ton	22.7	SWPR7B	
	3S12.7	Internal Transverse Tendons	ton	3.6		
Duct	12S12.7B		nos	428		
	19S15.2B		nos	108		
	3S12.7		nos	272		
	Φ80/85		m	4963.8		
	Φ90/100		m	2448.0		
	FlatDuct25x80		m	1557.5		
Cement grout in sheathng			m3	40.6		
Bearing	Product layer rubber bearing	660*560*125	nos	4		
		1400*1500*214	nos	4		
		Anchor Bar	Φ75 L=1250	nos	8	
			Φ75 L=2500	nos	8	
		Anchor Cap	Φ80 L=1350	nos	8	
			Φ150 L=850	nos	8	
Expansion joint		100mm	m	21.5		
Pavement	t=75mm	Asphalt concrete surface course	sq.m	1386.8		
	t= 5mm	Water Proofing	sq.m	115.6		
		Concrete	cu.m	1.054		
		Road Marking	sq.m	58.1		

1. Concrete

Total Quantity

Item	Quantity						(m ³)
	Main part	Diaphragm	Manhole	Haunch	Expansion	Anchorage	Total
Cantilever part	824.136	17.180				14.528	855.844
Pier Head part	222.606	149.368		1.640			373.614
Supporting Part	193.452	18.825	-1.650	0.828	-1.011		210.444
Total	1240.194	185.373	-1.650	2.468	-1.011	14.528	1439.902

(1) Supporting Part on P2 Side

Block	Section	Section Area (m ²)	Ave. of	Length of (m)	Concrete (m ³)		Total of each (m ³)
			Area (m ²)				
1	1	16.930					
	2	16.930	16.930	1.650	27.935		27.935
	3	8.150	12.540	0.000	0.000		0.000
	4	7.120	7.635	5.000	38.175		38.175
	5	7.120	7.120	2.800	19.936		19.936
Total				9.450	86.046		86.046

(2) Cantilever part on P2 -> P3

Block	Section	Section Area (m ²)	Average of (m ²)	Length of (m)	Concrete (m ³)		Total of each (m ³)
2	5	7.120					
	6	7.408	7.264	3.500	25.424		25.424
3	7	8.332	7.870	3.500	27.545		27.545
4	8	8.850	8.591	3.500	30.069		30.069
5	9	9.410	9.130	3.000	27.390		27.390
6	10	10.031	9.721	3.000	29.162		29.162
7	11	11.137	10.584	3.000	31.752		31.752
8	12	11.991	11.564	3.000	34.692		34.692
Total				22.500	206.034		206.034

(3) Pier Head part on P3

Block	Section	Section Area (m ²)	Average of (m ²)	Length of (m)	Concrete (m ³)		Total of each (m ³)
9	12	11.991					
	13	12.555	12.273	3.000	36.819		36.819
	14	12.555	12.555	0.000			
10	15	12.555	12.555	3.000	37.665		37.665
	16	12.555	12.555	0.000			
9	17	11.991	12.273	3.000	36.819		36.819
Total				9.000	111.303		111.303

(4) Cantilever part on P3 -> P4

Block	Section	Section Area (m ²)	Average of (m ²)	Length of (m)	Concrete (m ³)		Total of each (m ³)
8	17	11.991					
	18	11.137	11.564	3.000	34.692		34.692
7	19	10.031	10.584	3.000	31.752		31.752
6	20	9.410	9.721	3.000	29.162		29.162
5	21	8.850	9.130	3.000	27.390		27.390
4	22	8.332	8.591	3.500	30.069		30.069
6	23	7.408	7.870	3.500	27.545		27.545
2	24	7.120	7.264	3.500	25.424		25.424
Total				22.500	206.034		206.034

(5) Supporting Part on center of Span

Block	Section	Section Area (m ²)	Average of (m ²)	Length of (m)	Concrete (m ³)		Total of each (m ³)
11	24	7.120					
	24	7.120	7.120	3.000	21.360		21.360
Total				3.000	21.360		21.360

(6) Cantilever part on P3 -> P4

Block	Section	Section Area (m ²)	Average of (m)	Length of (m)	Concrete (m ³)	Total of each (m ³)
2	24	7.120	7.264	3.500	25.424	25.424
	23	7.408				
3	22	8.332	7.870	3.500	27.545	27.545
4	21	8.850	8.591	3.500	30.069	30.069
5	20	9.410	9.130	3.000	27.390	27.390
6	19	10.031	9.721	3.000	29.162	29.162
7	18	11.137	10.584	3.000	31.752	31.752
8	17	11.991	11.564	3.000	34.692	34.692
Total				22.500	206.034	206.034

(7) Pier Head part on P4

Block	Section	Section Area (m ²)	Average of (m)	Length of (m)	Concrete (m ³)	Total of each (m ³)
9	17	11.991	12.273	3.000	36.819	36.819
	16	12.555				
10	15	12.555	12.555	0.000		
	14	12.555	12.555	3.000	37.665	37.665
9	13	12.555	12.555	0.000		
	12	11.991	12.273	3.000	36.819	36.819
Total				9.000	111.303	111.303

(8) Cantilever part on P4 -> P5

Block	Section	Section Area (m ²)	Average of (m)	Length of (m)	Concrete (m ³)	Total of each (m ³)
8	12	11.991	11.564	3.000	34.692	34.692
	11	11.137				
7	10	10.031	10.584	3.000	31.752	31.752
6	9	9.410	9.721	3.000	29.162	29.162
5	8	8.850	9.130	3.000	27.390	27.390
4	7	8.332	8.591	3.500	30.069	30.069
3	6	7.408	7.870	3.500	27.545	27.545
2	5	7.120	7.264	3.500	25.424	25.424
Total				22.500	206.034	206.034

(9) Supporting Part on P6 Side

Block	Section	Section Area (m ²)	Ave. of Area (m ²)	Length of (m)	Concrete (m ³)	Total of each (m ³)				
1	5	7.120	7.120	2.800	19.936	19.936				
	4	7.120								
	3	8.150					7.635	5.000	38.175	38.175
	2	16.930					12.540	0.000	0.000	0.000
	1	16.930					16.930	1.650	27.935	27.935
Total				9.450	86.046	86.046				

Total

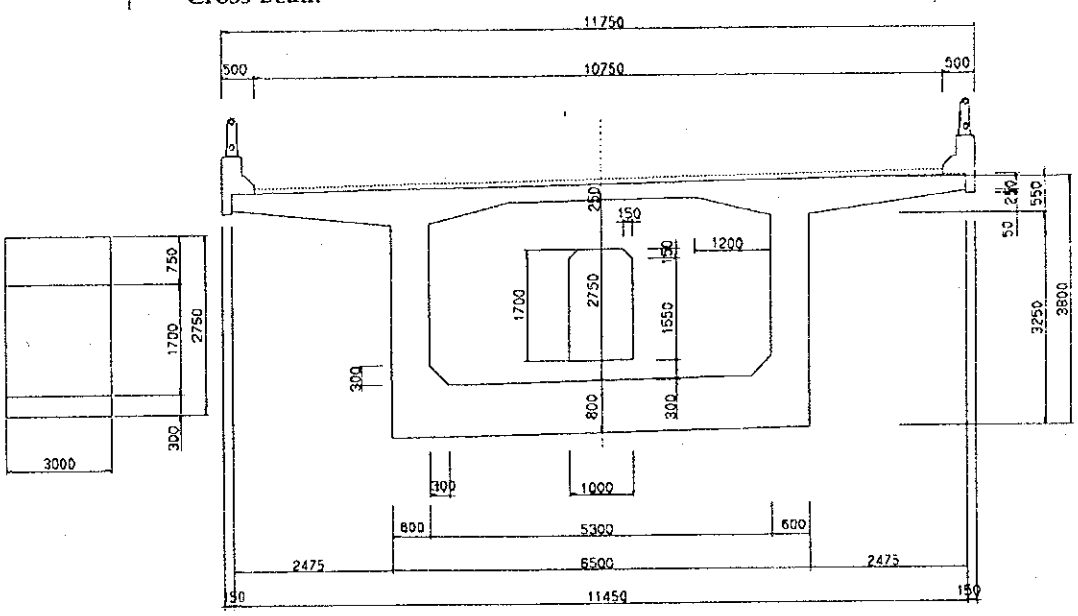
Item	Sub-Total
Pier Head part	222.606
Cantilever part	824.136
Supporting Part	193.452
Total	1240.194

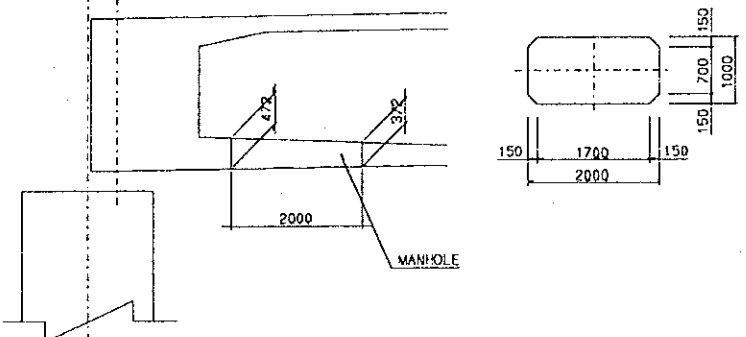
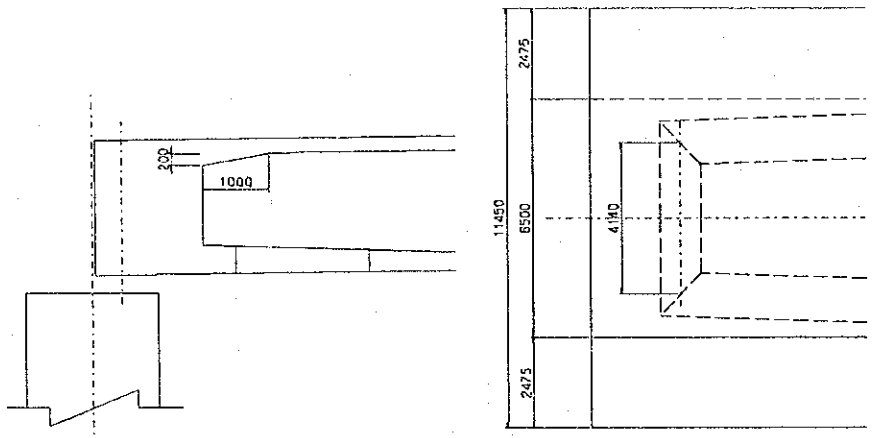
Concrete

Item	Formula	Quantity
2) Diaphragm, manhole Anchorage etc	(1) Center Diaphragm	
	· Supporting Part on center of Span	
	$V1 = (1.150 + 2.350) \times \frac{1}{2} \times 0.300$ $\times 0.300 \times 2 = 0.315 \text{ m}^3$ $V2 = 2.350 \times 0.950 \times 2 \times 0.300$ $= 1.340 \text{ m}^3$ $V3 = 0.550 \times 0.500 \times 0.300 \times 2$ $= 0.165 \text{ m}^3$ $V4 = (1.850 \times 0.550 - \frac{1}{2} \times 0.300) \times 2 \times 1.000$ $= 1.945 \text{ m}^3$	
	Total	3.765 m ³ 3.765 m ³
	V = 3.765 × 5 = 18.825 m ³	Total 18.825 m ³

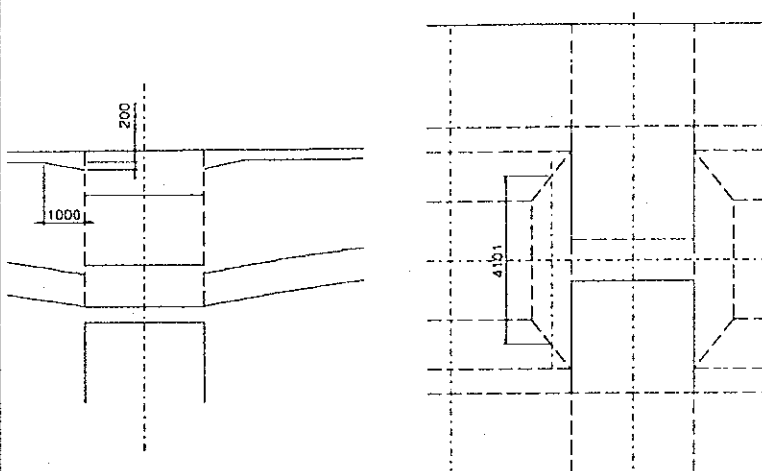
Concrete

Item	Formula	Quantity
	<p>• Cantilever part</p>	
	$V1 = (1.050 + 2.250) \times \frac{1}{2} \times 0.300$ $\times 0.300 \times 2 = 0.297 \text{ m}^3$ $V2 = 2.250 \times 0.844 \times 2 \times 0.300$ $= 1.139 \text{ m}^3$ $V3 = 0.500 \times 0.550 \times 0.300 \times 2$ $= 0.165 \text{ m}^3$ $V4 = (1.750 \times 0.550 - \frac{1}{2} \times 0.300) \times 2 \times 1.000$ $= 1.835 \text{ m}^3$	
	<p style="text-align: right;">Total</p>	<p>3.436 m³ 3.436 m³</p>
	$V = 3.436 \times 5 = 17.18 \text{ m}^3$	<p>Total 17.180 m³</p>

Item	Formula	Quantity
	<p>(2) Intermediate Cross Beam on Pier Head · Cross Beam</p> 	
	$V1 = 5.300 \times 2.750 \times 3.000 = 43.725 \text{ m}^3$ $V2 = - \frac{1.200 \times 0.300 \times 1/2 \times 3.000}{2} = -1.080 \text{ m}^3$ $V3 = - \frac{0.300 \times 0.300 \times 1/2 \times 3.000}{2} = -0.270 \text{ m}^3$ $V4 = - \left(\frac{0.700 + 1.000}{2} \times 1/2 \times 0.150 + \frac{1.550 \times 1.000}{2} \times 3.000 \right) = -5.033 \text{ m}^3$ <p style="text-align: right;">Total 37.342 m^3</p>	<p style="text-align: right;">per each 37.342 m^3</p>
	$V = 37.342 \times 4 = 149.368 \text{ m}^3$	<p style="text-align: right;">Total 149.368 m^3</p>

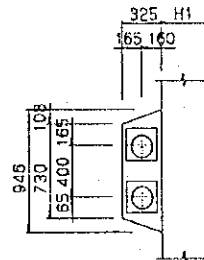
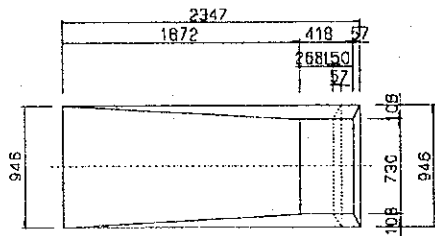
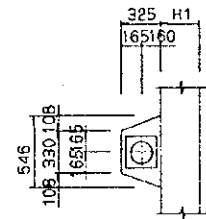
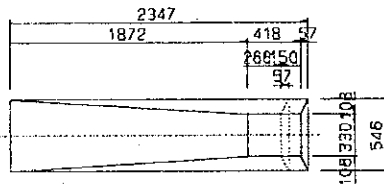
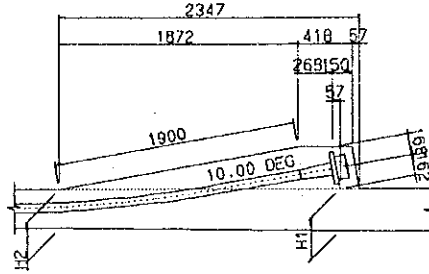
Item	Formula	Quantity
	<p>(3) Supporting Part on Side of Span (Manhole, Haunch etc)</p> <p>• Manhole-A (Subtraction) -Bottom Slab</p>  $A = -1.000 \times 2.000 + 4 \times \frac{1}{2} \times 0.150 = -1.955 \text{ m}^2 \text{ per each}$ $V = -1.955 \times \frac{1}{2} \times (0.472 + 0.372) = -0.825 \text{ m}^3$ $V = -0.825 \times 2 = -1.650 \text{ m}^3$ <p>• Haunch (Top Slab)</p>  $A = \frac{1}{2} \times 1.000 \times 0.200 = 0.100 \text{ m}^2 \text{ per each}$ $V = 0.100 \times 4.140 = 0.414 \text{ m}^3$ $V = 0.414 \times 2 = 0.828 \text{ m}^3$	<p>Total</p> <p>-1.650 m³</p> <p>Total</p> <p>0.828 m³</p>

Concrete

Item	Formula	Quantity
		
	$A = \frac{1}{2} \times 1.000 \times 0.200 = 0.100 \text{ m}^2$	per each
	$V = 0.100 \times 4.101 = 0.410 \text{ m}^3$	0.410 m ³
	$V = 0.410 \times 4 = 1.6404 \text{ m}^3$	Total 1.640 m ³
	<p>• Expansion Joint Part</p> $V = -10.750 \times 0.336 \times 0.070 \times 4 = -1.011$	-1.011 m ³

(4)Anchorage

TYPE A
SCALE 1:50



• TYPE-A1

$$V = \frac{1}{6} \times 0.325 \times (2.347 \times 0.546 + (2.347 + 0.418) \times 0.546 + 0.418 \times 0.330) = 0.208 \text{ m}^3$$

per each
0.208 m³

• TYPE-A2

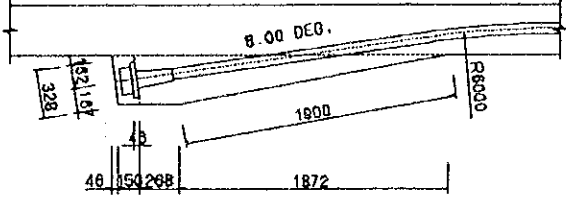
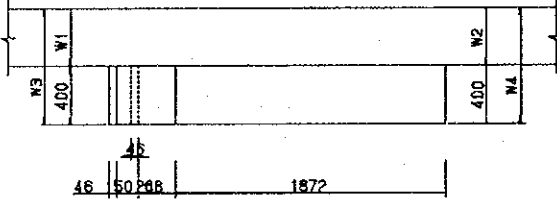
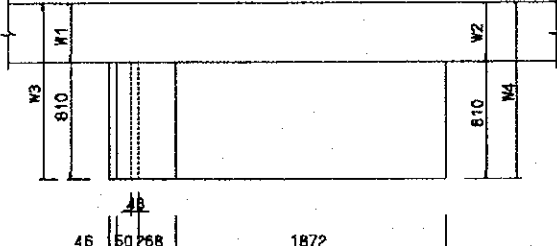
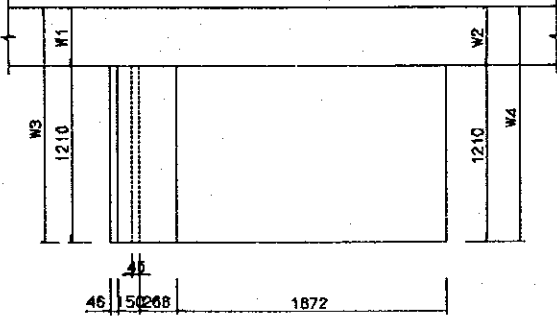
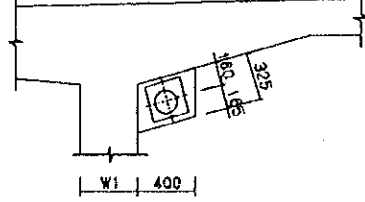
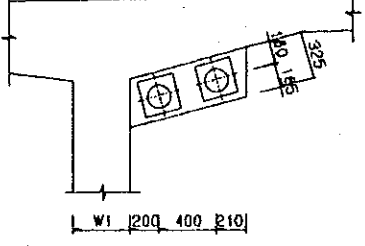
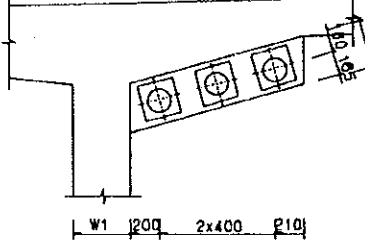
$$V = \frac{1}{6} \times 0.325 \times (2.347 \times 0.946 + (2.347 + 0.418) \times 0.946 + 0.418 \times 0.730) = 0.388 \text{ m}^3$$

per each
0.388 m³

Block No.	Anchorage Type		Concrete m ³	Remarks
	A1(0.208m3)	A2(0.388m3)		
1	4		0.832	
3	4		0.832	
5		4	1.552	
6	4		0.832	
7	4		0.832	
8	4	4	2.384	
Sub-Total	20	8	7.264	per 1-11 block

Total 7.264 × 2 = 14.528

14.528 m³

Item	Formula	Quantity
<p>TYPE B SCALE 1:25</p> 		
		
		
		
		
		
		

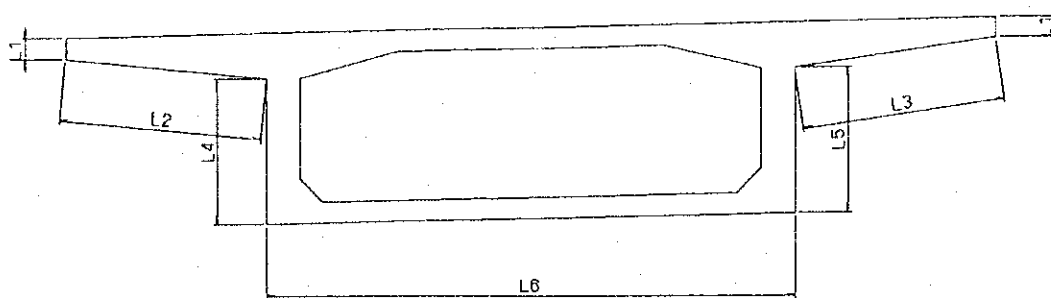
Concrete

Item	Formula				Quantity
	<p>• TYPE-B1</p> $V = \frac{1}{6} \times 0.325 \times (2.339 \times 0.400 + (2.339 + 0.418) \times 0.400 + 0.418 \times 0.400)$ <p>per each 0.179 m³</p> <p>• TYPE-B2</p> $V = \frac{1}{6} \times 0.325 \times (2.336 \times 0.810 + (2.336 + 0.418) \times 0.810 + 0.418 \times 0.810)$ <p>per each 0.362 m³</p> <p>• TYPE-B3</p> $V = \frac{1}{6} \times 0.325 \times (2.336 \times 1.210 + (2.336 + 0.418) \times 1.210 + 0.418 \times 1.210)$ <p>per each 0.542 m³</p>				
	Block No.	Anchorage Type			Concrete m ³
		B1(0.179m3)	B2(0.362m3)	B3(0.542m3)	
	3	4	4		2.164
	4	4		4	2.884
	5	4	4		2.164
	6	4		4	2.884
	7	4	4		2.164
	8		4		1.448
	9	4			0.716
	Sub-Total	20	16	8	12.260
	<p>Total 12.260 × 2 = 24.520</p>				<p>24.520 m³</p>

2. Form

2-1. External Form

1) Dimension Table of Girder



Section	L1	L2	L3	L4	L5	L6	ΣL
1,2	0.250	2.488	2.500	1.750	1.750	6.500	15.238
3	0.250	2.488	2.500	1.754	1.754	6.500	15.246
4	0.250	2.488	2.500	1.781	1.781	6.500	15.300
5	0.250	2.488	2.500	1.855	1.855	6.500	15.448
6	0.250	2.488	2.500	1.978	1.978	6.500	15.694
7	0.250	2.488	2.500	2.156	2.156	6.500	16.050
8	0.250	2.488	2.500	2.421	2.421	6.500	16.580
9	0.250	2.488	2.500	2.780	2.780	6.500	17.298
10	0.250	2.488	2.500	3.250	3.250	6.500	18.238

2) Total Quantity

Item	Quantity				Total
	Main part	Haunch	Man Hole		
Cantilever part	1420.431				1420.431
Pier Head part	322.644				322.644
Supporting Part	367.572	0.856	0.856		369.284
Total	2110.647	0.856	0.856		2112.359

3) Calculation of Girder Form

(1) Supporting Part on P2 Side

Block No.	Section No.	Form Length (m)	Average of Form Length (m)	Length of Block (m)	Form Area (m ²)	Bulkhead Cross Beam (m ²)	Total of each Block (m ²)
	Girder End				16.930		16.930
1	1	15.238					
	2	15.238	15.238	9.450	143.999		143.999
Total				9.450	143.999		160.929

(2) Cantilever part on P2 -> P3

Block	Section	Form Length (m)	Average of (m)	Length of (m)	Form Area (m ²)	Bulkhead (m ²)	Total of each (m ²)
2	2	15.238					
	3	15.246	15.242	3.500	53.347		53.347
3	4	15.300	15.273	3.500	53.456		53.456
4	5	15.448	15.374	3.500	53.809		53.809
5	6	15.694	15.571	3.000	46.713		46.713
6	7	16.050	15.872	3.000	47.616		47.616
7	8	16.580	16.315	3.000	48.945		48.945
8	9	17.298	16.939	3.000	50.817		50.817
Total				22.500	354.703		354.703

(3) Pier Head part on P3

Block	Section	Form Length (m)	Average of (m)	Length of (m)	Form Area (m ²)	Bulkhead (m ²)	Total of each (m ²)
9	9	17.298					
	10	18.238	17.768	3.000	53.304		53.304
10	10	18.238	18.238	0.000			
	10	18.238	18.238	3.000	54.714		54.714
9	10	18.238	18.238	0.000			
9	9	17.298	17.768	3.000	53.304		53.304
Total				9.000	161.322		161.322

(4) Cantilever part on P3-> P4

Block	Section	Form Length (m)	Average of (m)	Length of (m)	Form Area (m ²)	Bulkhead (m ²)	Total of each (m ²)
8	9	17.298					
	8	16.580	16.939	3.000	50.817		50.817
7	7	16.050	16.315	3.000	48.945		48.945
6	6	15.694	15.872	3.000	47.616		47.616
5	5	15.448	15.571	3.000	46.713		46.713
4	4	15.300	15.374	3.500	53.809		53.809
3	3	15.246	15.273	3.500	53.456		53.456
2	2	15.238	15.242	3.500	53.347		53.347
Total				22.500	354.703		354.703

(5) Supporting Part on center of Span

Block	Section	Form Length (m)	Average of (m)	Length of (m)	Form Area (m ²)	Bulkhead (m ²)	Total of each (m ²)
11	2	15.238					
	2	15.238	15.238	3.000	45.714		45.714
Total				3.000	45.714		45.714

(6) Cantilever part on P3 -> P4

Block	Section	Form Length (m)	Average of (m)	Length of (m)	Form Area (m ²)	Bulkhead (m ²)	Total of each (m ²)
2	2	15.238	15.242	3.000	45.726		45.726
	3	15.246					
3	4	15.300	15.273	3.000	45.819		45.819
4	5	15.448	15.374	3.000	46.122		46.122
5	6	15.694	15.571	3.000	46.713		46.713
6	7	16.050	15.872	3.500	55.552		55.552
7	8	16.580	16.315	3.500	57.103		57.103
8	9	17.298	16.939	3.500	59.287		59.287
Total				22.500	356.322		356.322

(7) Pier Head part on P4

Block	Section	Form Length (m)	Average of (m)	Length of (m)	Form Area (m ²)	Bulkhead (m ²)	Total of each (m ²)
9	9	17.298	17.768	3.000	53.304		53.304
	10	18.238					
10	10	18.238	18.238	0.000			
	10	18.238	18.238	3.000	54.714		54.714
9	10	18.238	18.238	0.000			
	9	17.298	17.768	3.000	53.304		53.304
Total				9.000	161.322		161.322

(8) Cantilever part on P4 -> P5

Block	Section	Form Length (m)	Average of (m)	Length of (m)	Form Area (m ²)	Bulkhead (m ²)	Total of each (m ²)
8	9	17.298	16.939	3.000	50.817		50.817
	8	16.580					
7	7	16.050	16.315	3.000	48.945		48.945
6	6	15.694	15.872	3.000	47.616		47.616
5	5	15.448	15.571	3.000	46.713		46.713
4	4	15.300	15.374	3.500	53.809		53.809
3	3	15.246	15.273	3.500	53.456		53.456
2	2	15.238	15.242	3.500	53.347		53.347
Total				22.500	354.703		354.703

(9) Supporting Part on P5 Side

Block No.	Section No.	Form Length (m)	Average of Form Length (m)	Length of Block (m)	Form Area (m ²)	Bulkhead Cross Beam (m ²)	Total of each Block (m ²)
1	2	15.238	15.238	9.450	143.999		143.999
	1	15.238					
	Girder end						
Total				9.450	143.999		160.929

Total

Item	Quantity	Remark
Cantilever part	1420.431	
Pier Head part	322.644	
Supporting Part	367.572	
Total	2110.647	

Item	Formula	Quantity
4) Supporting Part on Side of Span		
•Manhole		
A1 =	$-1.000 \times 2.000 + 4 \times \frac{1}{2} \times 0.150 \times 0.150 = -1.955 \text{ m}^2$	
A2 =	$0.422 \times (1.700 + 1.700 + 0.700 + 0.212 \times 4) = 2.383 \text{ m}^2$	per each
$\Sigma A =$	$-1.955 + 2.383 = 0.428 \text{ m}^2$	0.428 m ²
A =	$0.428 \times 2 = 0.856$	Total 0.856 m ²

2. Internal Form

1) Total Quantity

Item	Quantity					Total
	Main part	Diaphragm	Hunch	Anchorage		
Cantilever part	799.378	133.664		100.536		1033.578
Pier Head part	120.216	160.932	14.592			295.740
Supporting Part	164.791	0.000	4.520			169.311
Total	1084.385	294.596	19.112	100.536		1498.629

2) Calculation of Girder Form

(1) Supporting Part on P2 Side

Block No.	Section No.	Form Length (m)	Average of Form Length (m)	Length of Block (m)	Form Area (m ²)	Bulkhead Cross Beam (m ²)	Total of each Block (m ²)
1	3	8.664	9.119	5.000	45.595		45.595
	4	9.574					
	5	9.574	9.574	2.800	26.807		26.807
Total				5.000	45.595		45.595

(2) Cantilever part on P2 -> P3

Block	Section	Form Length (m)	Average of (m)	Length of (m)	Form Area (m ²)	Bulkhead (m ²)	Total of each (m ²)
2	5	9.574	9.253	3.500	32.386		32.386
	6	8.932					
3	7	8.586	8.759	3.500	30.657		30.657
4	8	8.574	8.580	3.500	30.030		30.030
5	9	8.650	8.612	3.000	25.836		25.836
6	10	8.856	8.753	3.000	26.259		26.259
7	11	9.026	8.941	3.000	26.823		26.823
8	12	9.512	9.269	3.000	27.807		27.807
Total				22.500	199.798		199.798

(3) Pier Head part on P3

Block	Section	Form Length (m)	Average of (m)	Length of (m)	Form Area (m ²)	Bulkhead (m ²)	Total of each (m ²)
9	12	9.512	10.018	3.000	30.054		30.054
	13	10.524					
10							
9	13	10.524	10.524	0.000			
	12	9.512	10.018	3.000	30.054		30.054
Total				6.000	60.108		60.108

(4) Cantilever part on P3-> P4

Block	Section	Form Length (m)	Average of (m)	Length of (m)	Form Area (m ²)	Bulkhead (m ²)	Total of each (m ²)
8	12	9.512	9.269	3.000	27.807		27.807
	11	9.026					
7	10	8.856	8.941	3.000	26.823		26.823
6	9	8.650	8.753	3.000	26.259		26.259
5	8	8.574	8.612	3.000	25.836		25.836
4	7	8.586	8.580	3.500	30.030		30.030
3	6	8.932	8.759	3.500	30.657		30.657
2	5	9.574	9.253	3.500	32.386		32.386
Total				22.500	199.798		199.798

(5) Supporting Part on center of Span

Block	Section	Form Length (m)	Average of (m)	Length of (m)	Form Area (m ²)	Bulkhead (m ²)	Total of each (m ²)
11	5	9.574	9.574	3.000	28.722		28.722
	5	9.574					
Total				3.000	28.722		28.722

(6) Cantilever part on P3 -> P4

Block	Section	Form Length (m)	Average of (m)	Length of (m)	Form Area (m ²)	Bulkhead (m ²)	Total of each (m ²)
2	5	9.574	9.253	3.000	27.759		27.759
	6	8.932					
3	7	8.586	8.759	3.000	26.277		26.277
4	8	8.574	8.580	3.000	25.740		25.740
5	9	8.650	8.612	3.000	25.836		25.836
6	10	8.856	8.753	3.500	30.636		30.636
7	11	9.026	8.941	3.500	31.294		31.294
8	12	9.512	9.269	3.500	32.442		32.442
Total				22.500	199.984		199.984

(7) Pier Head part on P4

Block	Section	Form Length (m)	Average of (m)	Length of (m)	Form Area (m ²)	Bulkhead (m ²)	Total of each (m ²)
9	12	9.512	10.018	3.000	30.054		30.054
	13	10.524					
10							
9	13	10.524	10.524	0.000			
	12	9.512	10.018	3.000	30.054		30.054
Total				6.000	60.108		60.108

(8) Cantilever part on P4 -> P5

Block	Section	Form Length (m)	Average of (m)	Length of (m)	Form Area (m ²)	Bulkhead (m ²)	Total of each (m ²)
8	12	9.512	9.269	3.000	27.807		27.807
	11	9.026					
7	10	8.856	8.941	3.000	26.823		26.823
6	9	8.650	8.753	3.000	26.259		26.259
5	8	8.574	8.612	3.000	25.836		25.836
4	7	8.586	8.580	3.500	30.030		30.030
3	6	8.932	8.759	3.500	30.657		30.657
2	5	9.574	9.253	3.500	32.386		32.386
Total				22.500	199.798		199.798

(9) Supporting Part on P5 Side

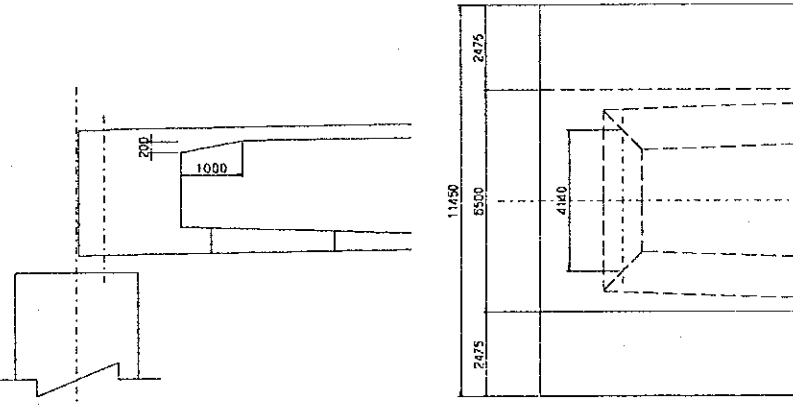
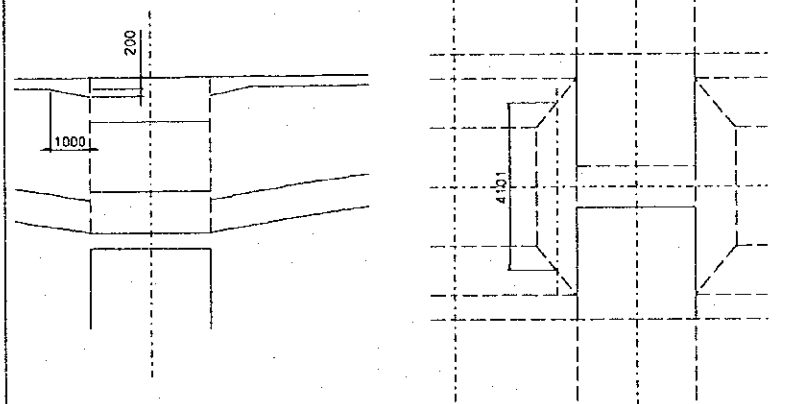
Block	Section	Form Length (m)	Average of (m)	Length of (m)	Form Area (m ²)	Bulkhead (m ²)	Total of each (m ²)
1	5	9.574	9.574	9.450	90.474		90.474
	4	9.574					
	3	8.664					
Total				9.450	90.474		90.474

Total

Item	Quantity	Remark
Cantilever part	799.378	
Pier Head part	120.216	
Supporting Part	164.791	
Total	1084.385	

Item	Formula	Quantity
<p style="text-align: center;">• Cantilever part</p>		
	$A1 = \left(\frac{0.803 + 0.781}{2} \right) \times \frac{1}{2} \times 1.102 = 1.746 \text{ m}^2$ $A2 = \left(\frac{0.781 + 0.515}{2} \right) \times \frac{1}{2} \times 0.798 = 1.034 \text{ m}^2$ $A3 = \left(\frac{0.505 + 0.741}{2} \right) \times \frac{1}{2} \times 0.708 = 0.882 \text{ m}^2$ $A4 = \left(\frac{0.741 + 0.720}{2} \right) \times \frac{1}{2} \times 1.042 = 1.522 \text{ m}^2$ $A5 = \left(\frac{0.700 + 0.673}{2} \right) \times \frac{1}{2} \times 1.374 = 1.887 \text{ m}^2$ $A6 = \left(\frac{0.673 + 0.433}{2} \right) \times \frac{1}{2} \times 0.719 = 0.795 \text{ m}^2$ $A7 = \left(\frac{0.433 + 0.250}{2} \right) \times \frac{1}{2} \times 0.079 = 0.054 \text{ m}^2$ $A8 = \left(\frac{1.900 \times 0.800 - 0.200}{2} \right) \times \frac{1}{2} \times 0.200 = 3.000 \text{ m}^2$ $A9 = \left(\frac{1.750 \times 0.800 - 0.200}{2} \right) \times \frac{1}{2} \times 0.200 = 2.760 \text{ m}^2$ $A10 = \left(\frac{2.093 \times 0.800 - 0.800}{2} \right) \times \frac{1}{2} \times 0.401 = 3.028 \text{ m}^2$ $A11 = \left(\frac{0.700 + 0.720 + 0.803}{2} \right) \times \left(\frac{0.300 + 0.800}{2} \right) \times 1.000 \times 6 + 1.000 \times 0.300 \times 3 = 7.034 \text{ m}^2$	
	<p style="text-align: right;">Total</p> $A = 16.708 \times 8 = 133.664 \text{ m}^2$	<p style="text-align: right;">per each 16.708 m²</p> <p style="text-align: right;">Total 133.664 m²</p>

Item	Formula	Quantity
(2) Intermediate Cross Beam on Pier Head • Cross Beam		
	$A1 = 0.300 \times 3.700 \times 2 = 2.220 \text{ m}^2$	
	$A2 = \left(\frac{3.700 + 3.676}{2} \right) \times 1.202 = 8.866 \text{ m}^2$	
	$A3 = \left(\left(\frac{3.676 + 3.410}{2} \right) \times 0.798 - 0.200 \times 0.200 \times \frac{1}{2} \right) \times 2 = 5.615 \text{ m}^2$	
	$A4 = \left(\left(\frac{3.396 + 3.632}{2} \right) \times 0.708 - 0.200 \times 0.200 \times \frac{1}{2} \right) \times 2 = 4.936 \text{ m}^2$	
	$A5 = \left(\frac{3.632 + 3.585}{2} \right) \times 2.361 = 17.039 \text{ m}^2$	
	$A6 = \left(\frac{3.585 + 2.106}{2} \right) \times 0.734 = 4.177 \text{ m}^2$	
	$A7 = \left(\frac{2.106 + 0.250}{2} \right) \times 0.798 = 1.880 \text{ m}^2$	
	$A8 = - 1.500 \times 1.000 \times 3 = -4.500 \text{ m}^2$	
	$A9 = (1.500 \times 2 + 1.000) \times 3 = 12.000 \text{ m}^2$	
	<p style="text-align: right;">Total</p>	<p style="text-align: right;">40.233 m² per each 40.233 m²</p>
	$A = 40.233 \times 4 = 160.932 \text{ m}^2$	<p style="text-align: right;">Total 160.932 m²</p>

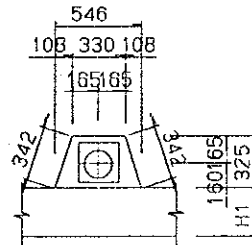
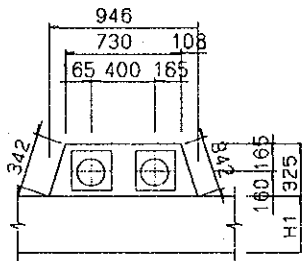
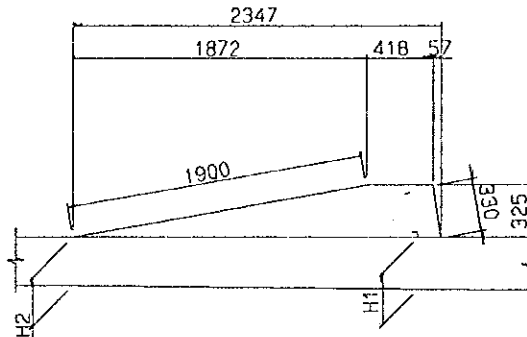
Item	Formula	Quantity
	<p>(3) Supporting Part on Side of Span (Manhole Haunch etc)</p> <p>• Haunch (Top Slab)</p> 	
	$L = (0.15^2 \times 0.500^2)^{1/2} = 0.522 \text{ m}$ $A = 0.522 \times 4.330 =$	<p>per each 2.260 m²</p>
	$A = 2.260 \times 2 = 4.52 \text{ m}^2$	<p>Total 4.520 m²</p>
		
	$L = (0.15^2 \times 0.450^2)^{1/2} = 0.474 \text{ m}$ $A = 0.474 \times 3.848 =$	<p>per each 1.824 m²</p>
	$A = 1.824 \times 8 = 14.592 \text{ m}^2$	<p>Total 14.592 m²</p>

Item	Formula	Quantity
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(4)Anchorage

TYPE A

SCALE 1:50



• TYPE-A1

$$\begin{aligned}
 A1 &= (0.33 + 0.546) \times \frac{1}{2} \times 0.33 = 0.145 \text{ m}^2 \\
 A2 &= (0.418 + 0.475) \times \frac{1}{2} \times 0.342 = 0.305 \text{ m}^2 \\
 A3 &= 1.872 \times \frac{1}{2} \times 0.342 = 0.32 \text{ m}^2 \\
 \Sigma A &= 0.77 \text{ m}^2
 \end{aligned}$$

per each
0.770 m³

• TYPE-A2

$$\begin{aligned}
 A1 &= (0.73 + 0.946) \times \frac{1}{2} \times 0.33 = 0.277 \text{ m}^2 \\
 A2 &= (0.418 + 0.475) \times \frac{1}{2} \times 0.342 = 0.305 \text{ m}^2 \\
 A3 &= 1.872 \times \frac{1}{2} \times 0.342 = 0.32 \text{ m}^2 \\
 \Sigma A &= 0.902 \text{ m}^2
 \end{aligned}$$

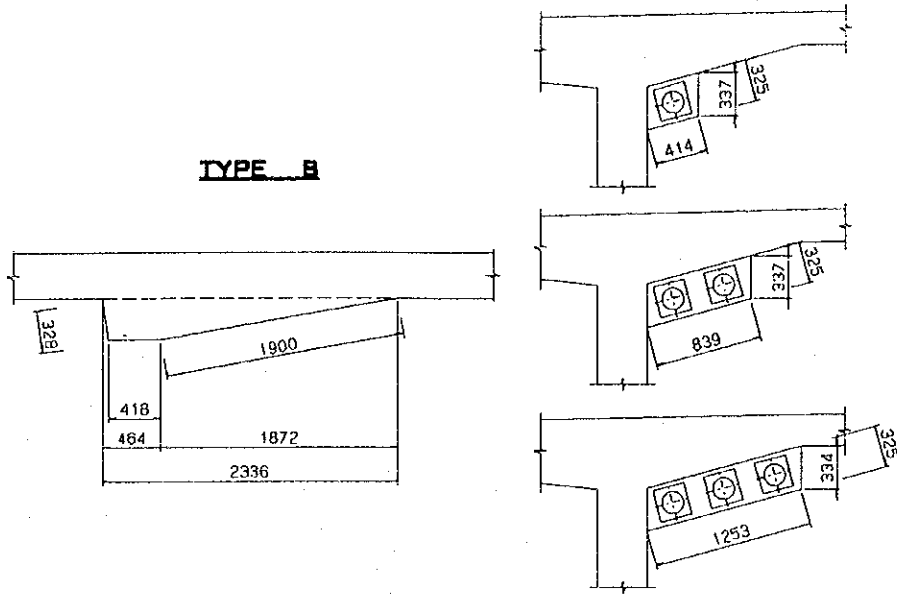
per each
0.902 m³

Block No.	Anchorage Type		Concrete m ³	Remarks
	A1 (0.770 m ³)	A2 (0.902 m ³)		
1	4		3.080	
3	4		3.080	
5		4	3.608	
6	4		3.080	
7	4		3.080	
8	4	4	6.688	
Sub-Total	20	8	22.616	

Total 22.616 × 2 = 45.232

45.232 m³

Item	Formula	Quantity
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• TYPE-B1

$$\begin{aligned}
 A1 &= (0.418 + 0.464) \times \frac{1}{2} \times 0.334 \\
 &= 0.147 \text{ m}^2 \\
 A2 &= 1.872 \times \frac{1}{2} \times 0.334 = 0.313 \text{ m}^2 \\
 A3 &= 0.414 \times \frac{1}{2} = 0.135 \text{ m}^2 \\
 \Sigma A &= 0.595 \text{ m}^2
 \end{aligned}$$

per each
0.595 m³

• TYPE-B2

$$\begin{aligned}
 A1 &= (0.418 + 0.464) \times \frac{1}{2} \times 0.334 \\
 &= 0.147 \text{ m}^2 \\
 A2 &= 1.872 \times \frac{1}{2} \times 0.334 = 0.313 \text{ m}^2 \\
 A3 &= 0.839 \times \frac{1}{2} = 0.274 \text{ m}^2 \\
 \Sigma A &= 0.734 \text{ m}^2
 \end{aligned}$$

per each
0.734 m³

• TYPE-B3

$$\begin{aligned}
 A1 &= (0.418 + 0.464) \times \frac{1}{2} \times 0.334 \\
 &= 0.147 \text{ m}^2 \\
 A2 &= 1.872 \times \frac{1}{2} \times 0.334 = 0.313 \text{ m}^2 \\
 A3 &= 1.253 \times \frac{1}{2} = 0.408 \text{ m}^2 \\
 \Sigma A &= 0.868 \text{ m}^2
 \end{aligned}$$

per each
0.868 m³

Block No.	Anchorage Type			Concrete m ³
	B1(0.595m3)	B2(0.734m3)	B3(0.868m3)	
3	4	4		5.316
4	4		4	5.852
5	4	4		5.316
6	4		4	5.852
7	4	4		5.316
8		4		2.936
9	4			2.380
Sub-Total	20	16	8	27.652

Total 27.652 × 2 = 55.304

55.304 m³

4. Reinforcement Bar

4-1. Total Quantity

	- D14	D16-D25	Total
Segment 1	8930.4	42387.2	51317.6
Segment 2	10865.2	46529.2	57394.4
Segment 3	26499.2	91856.8	118356.0
Segment 4	11584.8	46949.6	58534.4
Segment 5	1394.2	6572.6	7966.8
Anchor A-1		5668.8	5668.8
Anchor A-2		3885.6	3885.6
Anchor B-1		2595.0	2595.0
Anchor B-2		2880.0	2880.0
Anchor B-3		1906.8	1906.8
Deviator 1		4742.0	4742.0
Deviator 2	1072.0	1163.6	2235.6
Deviator 3	1072.0	1264.0	2336.0
Deviator 4	1072.0	1160.0	2232.0
Deviator 5	1520.4	4905.6	6426.0
Total	64010.2	264466.8	328477.0

LIST OF REINFORCEMENT

3

SIGN	DIACETER	LENGTH	NOS.	UNIT WEIGHT	PIECE WEIGHT	TOTAL WEIGHT	NOTE
D	1 -a	D16	4,740	8	1.578	7.480	59.8
	1 -b	D16	2,170	8	1.578	3.420	27.4
	2	D16	3,330	8	1.578	5.260	42.1
	3	D16	2,690	24	1.578	4.250	102.0
	4	D16	1,600	20	1.578	2.530	50.6
	5	D16	900	24	1.578	1.420	34.1
	6	D14	2,900	32	1.208	3.500	112.0
	7	D14	2,690	48	1.208	3.250	156.0
						D14	268.0
						D16	316.0
						TOTAL	584.0
						Total/one	nos Total
						-D14	268.000 4 1,072.000
						D16-D25	316.000 4 1,264.000
						Total	2,336.000

LIST OF REINFORCEMENT

4

SIGN	DIACETER	LENGTH	NOS.	UNIT WEIGHT	PIECE WEIGHT	TOTAL WEIGHT	NOTE
D	1 -a	D16	4,110	8	1.578	6.490	51.9
	1 -b	D16	2,710	8	1.578	4.280	34.2
	2	D16	3,260	8	1.578	5.150	41.2
	3	D16	2,060	24	1.578	3.250	78.0
	4	D16	1,600	20	1.578	2.530	50.6
	5	D16	900	24	1.578	1.420	34.1
	6	D14	2,900	32	1.208	3.500	112.0
	7	D14	2,690	48	1.208	3.250	156.0
						D14	268.0
						D16	290.0
						TOTAL	558.0
						Total/one	nos Total
						-D14	268.000 4 1,072.000
						D16-D25	290.000 4 1,160.000
						Total	2,232.000

4. PC Steel

1) Longitudinal Prestressing Internal Tendons

12S12.7B(SWPR7B)

Unit Weight: 9.288 kgf/m

(1) Tendons on Top Slab

PC Steel	Length of (m)	Unit (kgf)	Each	Weight of (kgf)	Remarks
S101	50.419	468.3	4	1873.2	
S102	46.925	435.8	4	1743.2	
S103	39.863	370.2	4	1480.8	
S104	39.953	371.1	4	1484.4	
S105	42.935	398.8	4	1595.2	
S106	32.848	305.1	4	1220.4	
S107	32.575	302.6	4	1210.4	
S108	27.348	254.0	4	1016	
S109	26.586	246.9	4	987.6	
S110	26.220	243.5	4	974	
S111	20.804	193.2	4	772.8	
S112	19.848	184.3	4	737.2	
S113	14.754	137.0	4	548	
S114	11.763	109.3	4	437.2	
S115	8.700	80.8	4	323.2	
Total	441.541		60	16403.6	

(2) Tendons on Top Slab

PC Steel	Length of (m)	Unit (kgf)	Each	Weight of (kgf)	Remarks
S201	50.419	468.3	4	1873.2	
S202	46.925	435.8	4	1743.2	
S203	39.863	370.2	4	1480.8	
S204	39.953	371.1	4	1484.4	
S205	42.935	398.8	4	1595.2	
S206	32.848	305.1	4	1220.4	
S207	32.575	302.6	4	1210.4	
S208	27.348	254.0	4	1016	
S209	26.586	246.9	4	987.6	
S210	26.220	243.5	4	974	
S211	20.804	193.2	4	772.8	
S212	19.848	184.3	4	737.2	
S213	14.754	137.0	4	548	
S214	11.763	109.3	4	437.2	
S215	8.700	80.8	4	323.2	
Total	441.541		60	16403.6	

(3) Tendons on Bottom Slab

PC Steel	Length of (m)	Unit (kgf)	Each	Weight of (kgf)	Remarks
S301	30.828	286.3	4	1145.2	
S302	30.828	286.3	4	1145.2	
S303	25.206	234.1	4	936.4	
Total	86.862		12	3226.8	

(4) Tendons on Bottom Slab

PC Steel	Length of (m)	Unit (kgf)	Each	Weight of (kgf)	Remarks
S401	46.036	427.6	4	1710.4	
S402	46.036	427.6	4	1710.4	
S403	46.036	427.6	4	1710.4	
S404	46.036	427.6	4	1710.4	
Total	184.144		16	6841.6	

(5) Tendons on Bottom Slab

PC Steel	Length of (m)	Unit (kgf)	Each	Weight of (kgf)	Remarks
S501	30.828	286.3	4	1145.2	
S502	30.828	286.3	4	1145.2	
S503	25.206	234.1	4	936.4	
Total	86.862		12	3226.8	

Sub-Total Weight of PC Cable $W_p =$ (kgf) 68840 (ton) 68.8

Tension Unit $N_s =$ 214 \times 2 = 428

Theating $L =$ 4963.8 m

Cement grout $\Phi 80/85$ $V =$ 25.0 m³

2) Longitudinal Prestressing External Tendons

PC Steel	Length of (m)	Unit (kgf)	Each	Weight of (kgf)	Remarks
C11	37.940	352.4	6	2114.4	
C12	37.974	352.7	6	2116.2	
C13	37.974	352.7	6	2116.2	
C21	60.099	558.2	6	3349.2	
C22	60.099	558.2	6	3349.2	
C23	60.099	558.2	6	3349.2	
C31	37.940	352.4	6	2114.4	
C32	37.940	352.4	6	2114.4	
C33	37.940	352.4	6	2114.4	
Total	408.005		54	22737.6	

Sub-Total Weight of PC Cable $W_p = 22737.6 \quad 22.7$

Tension Unit $N_s = 54 \times 2 = 108$

Theating $L = 2448.03 \quad m$

Cement grout $\Phi 90/100 \quad V = 15.6 \quad m^3$
(kgf) (ton)

3) Transverse Prestressing Tendons (Top Slab)

3S12.7

Unit Weight: 2.322 kgf/m

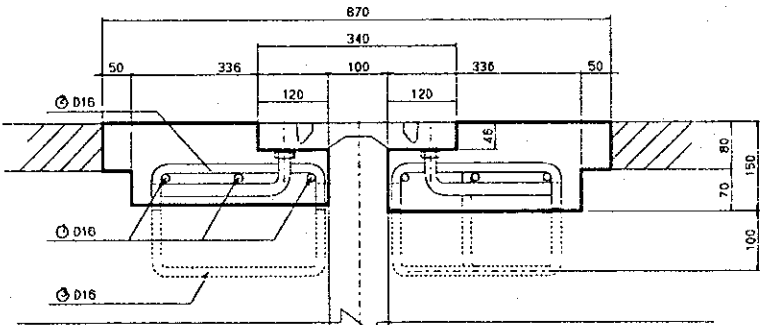
PC	Length of (m)	Unit (kgf)	Each	Weight of (kgf)	Remarks
3.0m Segment	11.452	26.6	48	1276.8	
3.5m Segment	11.452	26.6	49	1303.4	
Closure Segment	11.452	26.6	3	79.8	
Pier Segment	11.452	26.6	18	478.8	
End Segment	11.452	26.6	18	478.8	
Total	57.260		136	3617.6	

Sub-Total Weight of PC Cable $W_p = 3617.6 \quad 3.6$
(kgf) (ton)

Tension Unit $N_s = 136 \times 2 = 272$

Theating $L = 1557.472 \quad m$

Item	Formula	Quantity
5. Accessories		
1) Expansion Joint	<p>EACH LENGTH L = 10.750</p> <p>EACH = 2</p> <p>TOTAL LENGTH</p> <p>L = 10.750 × 2 = 21.500 m</p>	
2) BEARING PA ELASTOMERIC	<p>660*560*125 n = 2 × 2 = 4 nos</p> <p>1400*1500*214 n = 2 × 2 = 4 nos</p>	
3) ANCHOR BAR	<p>Φ75 L=1250 (MOVE) EACH = 4 × 2 = 8 nos</p> <p>Φ75 L=2500 (FIX) EACH = 4 × 2 = 8 nos</p>	
4) ANCHOR CAP (SGP)	<p>Φ80 L=1350 (MOVE) EACH = 4 × 2 = 8 nos</p> <p>Φ150 L=850 (FIX) EACH = 4 × 2 = 8 nos</p>	

Item	Formula	Quantity
<p>9 Pavement</p>	<p style="text-align: center;">$L = 129.00 \text{ m}$</p> <p>1) Pavement $t=75\text{mm}$</p> $A = 10.750 \times 129.000 = 1,386.750 \text{ m}^2$ <p>2) Water Proofing $t=5\text{mm}$</p> $A = 10.750 \times 10.750 = 115.563 \text{ m}^2$ <p>3) Concrete</p>  <p style="text-align: right;">per one side</p> $A1 = 0.336 \times 0.150 - 0.12 \times 0.046 = 0.045$ $A2 = 0.080 \times 0.050 = 0.004$ <p style="text-align: right;">Total 0.049 m²</p> $V = 0.049 \times 10.750 \times 2 = 1.054 \text{ m}^3$ <p>2) Road marking</p> <p>Bridge Length $L= 129.000 = 129.000 \text{ m}$</p> <p>Side Line</p> $A1 = 129.000 \times 0.100 \times 4 = 51.600 \text{ m}^2$ $L = 129.000 / 5.000 = 25.8 \text{ m}$ <p>Center Line</p> $A2 = 2.500 \times 0.100 \times 25.8 = 6.450 \text{ m}^2$ <p>Total $51.600 + 6.450 = 58.050 \text{ m}^2$</p>	<p>1,386.8 m²</p> <p>115.6 m²</p> <p>1.054 m³</p> <p>58.1 m²</p>