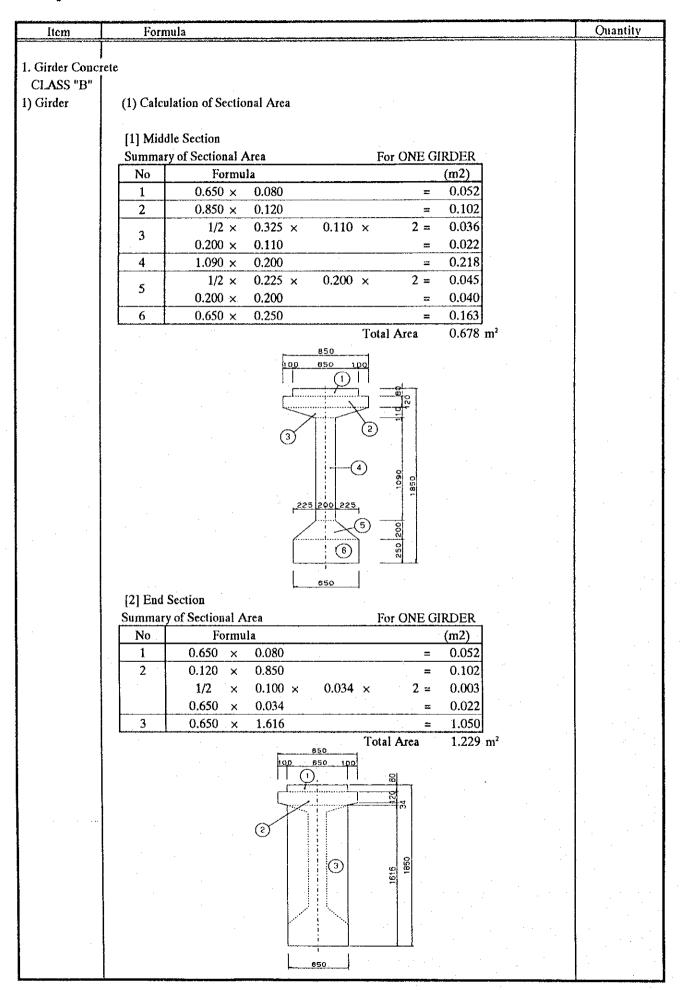
3.3. Tra On Bridge

# 1.Quantity of Superstructure

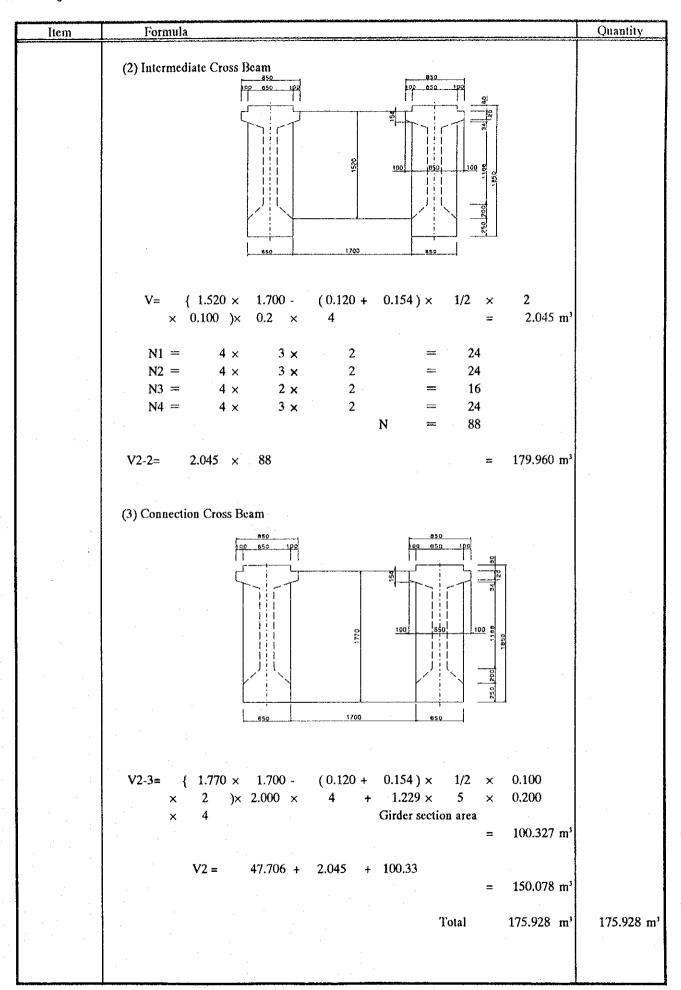
Superstructure-Approach Bridge

Ite	m	Work Item		Unit	Quantity	Remarks
Concrete	CLASS B	Girder		cu.m	1004.6	σck=40Mpa
J551 575	CLASS D-1	Deck Slab			637.7	<del> </del>
		Diaphragm			175.9	1
	ľ	Total	<del> </del>	cu.m	813.7	о ск-зыпра
Form	· · · · · · · · · · · · · · · · · · ·	Deck Slab		sq.m	739.5	
rom	-	Girder			5619.7	
				sq.m	485.0	
	-	Diaphragm	· · · · · · · · · · · · · · · · · · ·	sq.m	6844.2	 
D 1		Total	1 514	sq.m		
Re-bar		75 1 69 1	- D14	ton	40.1	
	İ	Deck Slab	D14 - D25	ton	113.4	
			D25 -	ton		
			Total	ton	153.5	
			- D14	ton	30.0	
		Girder	D14 - D25	ton	164.7	
			D25 -	ton	•	
			Total	ton	194.8	
		÷	- D14	ton	6.2	
		Diaphragm	D14 - D25	ton	13.2	
		•	D25 -	ton	-	
			Total	ton	19.4	
			- D14	ton	76.4	
		Total	D14 - D25	ton	291.3	
			D25 -	ton	0.0	
		•	Total	ton	367.7	
PC Cable	12S12.7B			ton		SWPR7B
	3S12.7B	Transverse Te	endons	ton	1.7	
Sheathing	Φ 80/85			m	5845.2	
2	Φ50/55			m	723.6	<del></del>
	Segment Grout	t in Sheathing		m3	30.8	
Expansion Joint	Segment Groun	Type A		m	43.0	
Expunsion some		1,10011	···-		15.0	
Pavement	t=70mm	Asphalt concrete su	rface course	sq.m	2758.5	
1 d Chilont	t=5mm	Water proo		sq.m	2758.5	
. *		Concrete		cu.m	4.3	
		Road Mark		sq.m	129.7	
Bearing		Product layer rubb		nos	80	
		Anchor Bar	Φ75 L=1500	nos	160	L
	}	7 MICHOL Dai	Φ95 L=800	nos	48	
:		Anchor Cap	Φ100 L=800	nos	8	<del></del>
		Allehor Cap	Φ100 L=800 Φ125 L=800		8	
Concrete Form			1 4 123 L=000	nos	439.3	
College to the	· · · · · · · · · · · · · · · · · · ·			sq.m cu.m	439.3 35.1	
		Ft.1.4				
		D14		ton	5.0	
<u> </u>			<del> </del>		A \$14.00	
Shear Key			·	nos	450	
	·		<u> </u>			
Erection		<u> </u>		ton	<del> </del>	per one girder (MAX)
6 v	.			ton	2511.5	



Item	Fori	nula						1	Quantity
	(2) Calc	ulation of Co	ncrete Voli	ume					
	(2) Care	mation of ex	delete von		rder Length	36,000	m		
1			Section	Average of	Length of	Concrete			
		Section No.	(m2)	(m2)	(m)	(m3)	Remark		
		END	1.229						
		END	1.229	1.229	0.500	0.615			
		MIDDLE	0.678	0.954	5.400	5.152			
•		MIDDLE	0.678	0.678	24.200	16.408			
1		END	1.229	0.954	5.400	5.152			
		END	1.229	1.229	0.500	0.615			
·		Total			36.000	27.942			
		No	. of Girder		5	× 2	= 10		
	V1=	27.942	× 10				= 279.420	m³	
			····	Gi	rder Length	31.600	m		
		Section No.	Section	Average of	Length of	Concrete	Remark		
ļ			(m2)	(m2)	(m)	(m3)	Windly		
Į ;		END	1.229						
		END	1.229	1.229	0.500	0.615			
		MIDDLE	0.678	0.954	4.800	4.579			
		MIDDLE	0.678	0.678	21.000	14.238			
		END	1.229	0.954	4.800	4.579			
ļ		END	1.229	1.229	0.500	0.615			
·		Total	· · · · · · · · · · · · · · · · · · ·		31.600	24.626			
			. of Girder		. 5	× . 2			
·	V2=	24.626	× 10				= 246.260	m,	
				a.		20, 600			
		· · · · · · · · · · · · · · · · · · ·	Continu		rder Length	28.600	m		
		Section No.	Section	Average of	- 1	ı	Remark		
		END	(m2) 1.229	(m2)	(m)	(m3)			
		END	1.229	1.229	0.500	0.615			
		MIDDLE	0.678	0.954	4.300	4.102			
		MIDDLE	0.678	0.678	19.000	12.882			
<u> </u>		END	1.229	0.954		4.102			
		END	1.229	1.229		0.615			
		Total	1.667	1.427	28.600	22.316			
			. of Girder	1	5	<del></del>	= 10		
	V3=				,		= 223.160	m³	
			••	Gi	rder Length	33,000		-	
			Section	Average of		Concrete			
		Section No.	(m2)	(m2)	(m)	(m3)	Remark		
		END	1.229						
		END	1.229	1.229	0.500	0.615			
		MIDDLE	0.678	0.954		4.579			
		MIDDLE	0.678	0.678		15.187			
		END	1.229	0.954	<u> </u>	4.579	······································		
		END	1.229	1.229					•
		Total			33,000			.	
	,	<u></u>	o. of Girder		5	<del></del>	= 10		
	V4=		× 10		,		= 255.750	m³	
			:		· ·	ΣΑ	= 1004.590		1004.59 m <sup>3</sup>
	•							- 1	

Item	Formula	Quantity
2. Diaphragm		
Concrete		
CLASS "D-1"		
1) Diaphragm	650	
	225, 200, 225	
	0 G 0 G	
	25.25.25.25.25.25.25.25.25.25.25.25.25.2	
	$V = (0.200 + 0.650) \times 1/2 \times 0.225 \times$	
	$(1.090 + 1.366) \times 1/2 \times 2 = 0.235$	
	$N1 = 5 \times 3 \times 2 = 30$	
	$N2 = 5 \times 3 \times 2 = 30$	
	$N3 = 5 \times 2 \times 2 = 20$	
	$N4 = 5 \times 3 \times 2 = 30$	
·	N = 110	
	$V1 = 0.235 \times 110 = 25.850 \text{ m}^3$	
	- 25.650 III	•
2) Cross Beam	(1) End Cross Beam	
2, 2.200 2000	00 esc 100 100 100 100	<i>:</i>
	100 550 100 5	
<b>!</b>		
<b>[</b>	650 1700 650	
		1
	$V2-1= \{ 1.770 \times 1.700 - (0.120 + 0.154) \times 1/2 \times 0.100 \}$	•
]	$\times$ 2 )× 0.5 × 4 × 2 × 4 = 47.706 m <sup>3</sup>	
1		
]		
1		

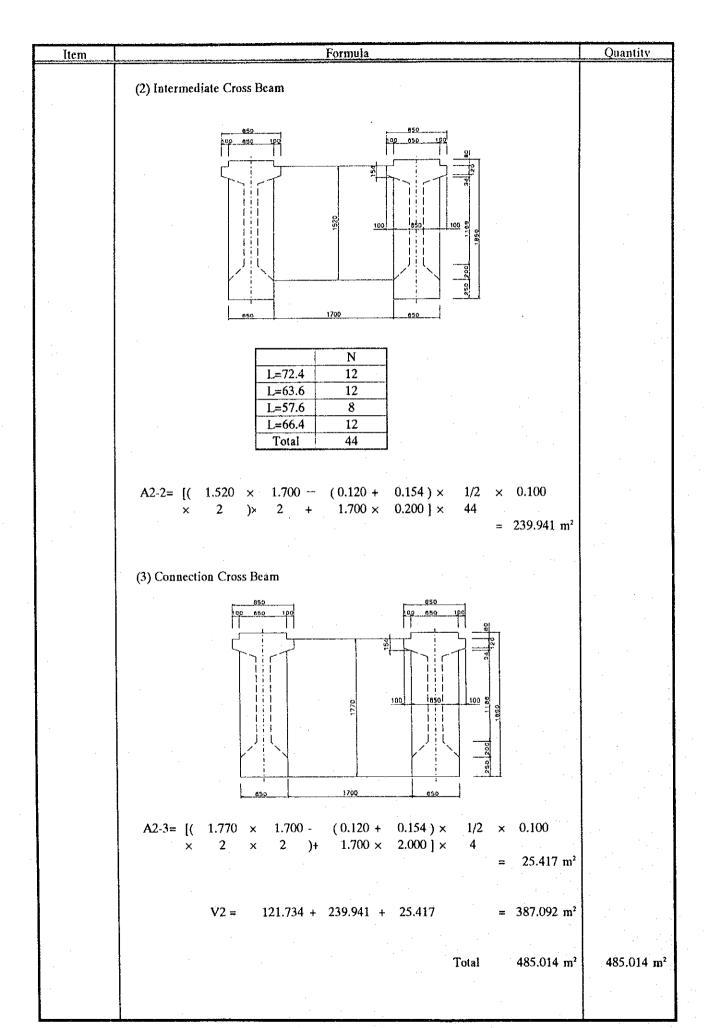


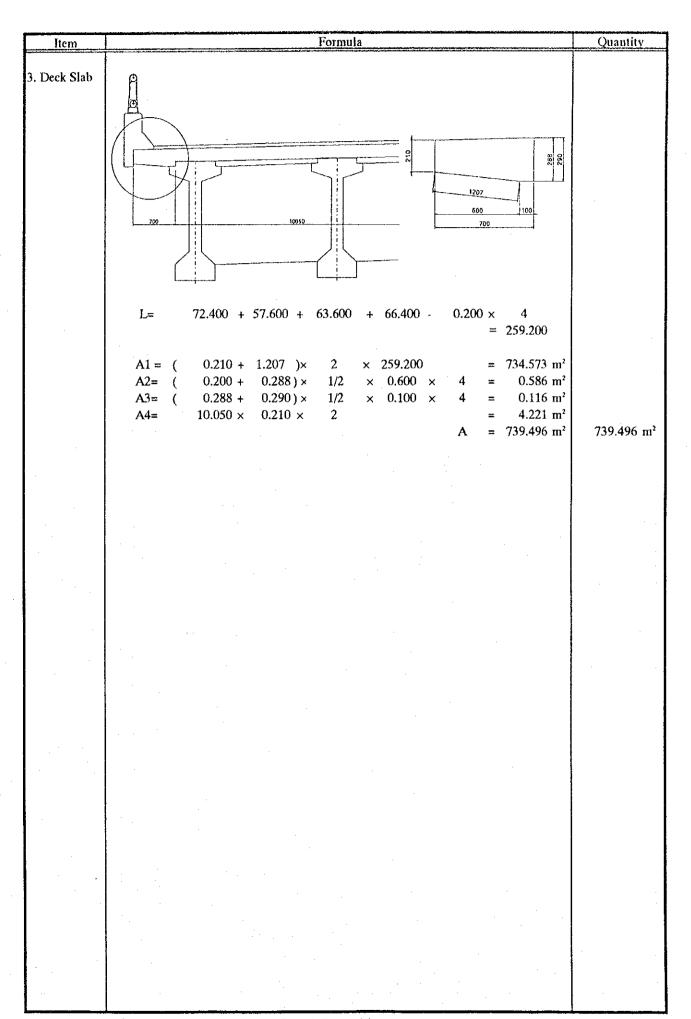
Item_	Formula	Quantity
3. Deck Slab Concrete CLASS "D-1"	50 00 100 700	
	A 1= $(0.21 + 0.288) \times 1/2 \times 0.600 \times 2 = 0.299$ A 2= $(0.288 + 0.290) \times 1/2 \times 0.100 \times 2 = 0.058$ A 3= $10.050 \times 0.210$ = $2.111$ $\Sigma A = 2.468 \text{ m}^2$	
	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	
	V = 2.468 × 258.400 = 637.731	637.731 m³

Item	Formula	Quantity
L C!		
l. Girder		
1) Girder	(1) Calculation of Sectional Area	
-,		
	[1] Middle Section	
	Summary of Sectional Length For ONE GIRDER	
	No Formula (m)	
	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	
	$\begin{array}{ c cccccccccccccccccccccccccccccccccc$	
	$ \begin{array}{ c c c c c c c c c c c c c c c c c c c$	
	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	
	$6  0.250 \times 2 = 0.500$	
	Total Area 4.368 m	·
	, 850	
	100 550 100	
	5000	
	1	
	343	
* .		
	\%,	
* .		
	000	
	650	
	[2] End Section	
	Summary of Sectional Length For ONE GIRDER	
	No Formula (m)	
	$1   0.080 \times 2   = 0.160$	
	$2 \qquad 0.120 \times 2 \qquad = 0.240$	
	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	
	$\begin{array}{ c c c c c c c c c c c c c c c c c c c$	
	Total Area 3.844 m	
	850	
	100 650 100	
	1 8	
•		
	[][ ][ ][ ][ ][ ][ ][ ][ ][ ][ ][ ][ ][	
•		
	850	

ltem				Formula					Quantity
	(2) (2-1-								
	(2) Calc	ulation of Foi	m Area	Gi	rder Length	36.000	m		
			Section	Average of		Form		1	•
		Section No.	Length	Section	Block	Area	Remark		
			(m)	(m)	(m)	(m2)			
		END	3.844			1.229	End-sec		
İ	÷	END	3.844	3.844	0.500	1.922			
		MIDDLE	4.368	4.106	5.400				
		MIDDLE	4.368	4.368	24.200				
<b>!</b>		END	3.844	4.106	5.400		====		
		END	3.844	3.844	0.500	1.922	End-sec		
1		Total			36.000	1.229 156.352	Enn-sec		
		10tai 1	No. of G	irder		× 2	= 10	1	·
	A1	= 156.352		IIdei	3		= 1563.520		
	Ai	- 130.332	^ 10	C:	rder Length	31.600		• • • •	
		<u> </u>	Section	Average of	Length of	Form	111	1.	
		Section No.	Length	Section	Block	Area	Remark		
			(m)	(m)	(m)	(m2)			
		END	3.844			1.229	End-sec		
		END	3.844	3.844	0.500	1.922			
		MIDDLE	4.368		4.800				
1		MIDDLE	4.368	4.368	21.000	91.728			
	•	END	3.844		4.800				·
		END	3,844	3.844	0.500		TO 1		
		T. 1			21.600	1.229	End-sec		,
		Total	N CC	• 1	31.600		= 10	·	
		105 110	No. of G	iraer	3	× 2		- 1	
	A2	= 137.448	× 10	C.	. 1 . 1		= 1374.480	m,	
		<del></del>	Section	Average of	rder Length Length of	28.600 Form	m	,	
		Section No.	Length	Section Section	Block	Area	Remark		
		Section 140.	(m)	(m)	(m)	(m2)	Killalk		·
		END	3.844			1.229	End-sec		
		END	3.844		0.500	<del></del>			
		MIDDLE	4.368		4.300			1	
1		MIDDLE	4.368					]	
		END	3.844						
		END	3.844	3.844	0.500				
l l					20.400	1.229			
		Total		<u> </u>	28.600	<del></del>		]	
1		101.00	No. of C			× 2	= 10		
	A3	= 124.606	× 10			<b>77.000</b>	= 1246.060	111,	
. !			Section	Average of	irder Length	33.000 Form	<u>m</u> .	1	
		Santian No.		Section	Block	Area	Domonk		
		Section No.	Length (m)	(m)	(m)	(m2)	Remark		·
		END	3.844		()	1.229	End-sec	1	
		END	3.844	<del></del>	0.500	4		1	
		MIDDLE	4.368		<del></del>			1	*
[		MIDDLE	4.368	4.368	22.400	97.843			·
• [		END	3.844			19.709			
1		END	3.844	3.844	0.500	<del> </del>			
				<u> </u>		1.229			
		Total	<u> </u>	<u> </u>	33.000	<del> </del>		j	
		1	No. of C		. 5	× 2	= 10		[ .
	A4	= 143.563	× 10	)			= 1435.630	m²	
<b>[</b> ]									]
1			4			A	= 5619.69	)	5,619.7 m <sup>2</sup>
1									1

Item	Formula	Quantity
2. Diaphragm		
	1 0218 2 (1000 1266) 17	
1) Diaphragm	a1= 0.318 × 2 × (1.090 + 1.366) × 1/2 + 0.200 × 1.366) = 1.054 m <sup>2</sup>	
	a2= $0.318 \times 2 \times (1.090 + 1.366) \times 1/2 = 0.781 \text{ m}^2$	
}		
	N1 N2 L=72.4 12 18	
	L=63.6 12 18	
	L=57.6 8 12 L=66.4 12 18	
	Total 44 66	
	$A1 = (1.054 \times 44 + 0.781 \times 66)$	
	$= 97.922 \text{ m}^2$	
	650 225 200 225	
	300	
	250	
	7, 2 0 10 10 10 10 10 10 10 10 10 10 10 10 1	
2) Cross Beam	(1) End Cross Beam	
	100 650 100 100 650 100 100 100 100 100 100 100 100 100 1	
·		
	100	
	9 2 2	
	850. 1700 630	
	A2-1= $[(1.770 \times 1.700 - (0.120 + 0.154) \times 1/2 \times 0.100]$	
	$\times$ 2 $\times$ 2 )+ 1.700 $\times$ 0.500 ] $\times$ 4 $\times$ 2 $\times$ 4 = 121.734 m <sup>2</sup>	
,		
. "		





Item			)	Formula				Quantity
DO CLEVE								
PC CABLE 1) 12812.7	L=36.0m							
1) 12312.7	CABLE VAR.	EACH	CABLE	EACH	TOTAL	For One Gire	WEIGHT	
	0.22	LENGTH	NO.		LENGTH	WEIGHT		·
	- 1	35.702	C1	1	34.702	9.290	322.4	
	2	35.714	C2	1	34.714	9.290	322.5	
	3	35.754	C3	1	35.754	9.290	332.2	
	4	35.784	C4	1	35.784	9.290 9.290	332.4 332.8	
	5	35.822	C5	1	35.822	9.290	332.0	
	TOTAL			5	176.776		1642.3	
		SUB-TOTA	L WEIGH			r BRIDGE		
		Wp=	1642 ×				=	16,423.0 kgf
·			•					16.4 ton
		TENSION U	_					20.0
	*	Ns= HEATING <sup>4</sup>	5 x ศาสต/สร	2 ×	2		. =	20.0 nos
			4 60/63 176.776 ×	5 x	2		=	1,767.8 m
		CENEBT G			•••			
		V=		5 ×	2		=	8.9 m3
	L=31.6m		0.010	l		For One Gire		
	CABLE VAR.	EACH	CABLE NO.	EACH	TOTAL	UNIT	WEIGHT	
	1	1ENGTH 31.302	C1	1	31.302	9.290	290.8	
j	2	31.332	C2	1	31.332	9.290	291.1	
	3	31.394	СЗ	1	31.394	9.290	291.7	
	4	31.456	C4	1	31.456	9.290	292.2	
	TOTAL	IID TOTAL	WEIGHT	OF BC C	125.484	DRIDGE	1165.8	
	31	UB-TOTAL Wp=		OF PC C.		BRIDGE	. =	11,658.0 kgf
			7100 /		-			11.7 ton
1	Т	ENSION UN	NIT					
<u> </u>		Ns=		2 ×	2		=	16.0 nos
		HEATHING		_	_			4.5-4.6
		L= CENEBT G	125.484 ×	5 ×	2		=	1,254.8 m
		V=		: 5 x	2		· =	6.3 m3
ļ		•-	01050 7	. , ,	-			
,								
1					•			
1	1						•	
								·
Į	<u>                                     </u>							<u> </u>

Item			]	Formula				Quantity
	· ·							
						n 0 ~:	•	
l.	L=28.6m			· · · · · · · · · · · · · · · · · · ·		For One Gir		
	CABLE VAR.	EACH	3	EACH		UNIT	WEIGHT	
		LENGTH	NO.		LENGTH	WEIGHT	262.0	
	1	28.302	C1	11_	28.302	9.290	262.9 263.2	
	2	28.332	C2	1	28.332	9.290		
	3	28.394	<u>C3</u>	11	28.394	9.290 9.290	263.8 264.4	
	4	28.456	C4	1	28.456	9.290	204.4	
	TOTAL			4	113.484		1054.3	
	TOTAL	UB-TOTAL	WEIGHT			BRIDGE	1034.5	!
	31	Wp=		5 x	-	DRIDOL	<b>:=</b>	10,543.0 kgf
		11 p-	1054 7	3 ^	<u>.</u>			10.5 ton
	ידי	ENSION UN	JTT					
		Ns=		2 ×	2		=	16.0 nos
		HEATING	. ^		- :			
			113.484 ×	5 ×	2		22	1,134.8 m
	·	CENEBT G						
		V=		5 ×	2		=	5.7 m3
i								
İ	•							
1		i						
	L=33.2m					For One Gir		
	CABLE VAR.	EACH	CABLE	EACH	TOTAL	UNIT	WEIGHT	
		LENGTH	NO.		LENGTH	WEIGHT	040.1	
	1	33.702	C1	1	33.702	9.290	313.1	
	2	33.714	C2	1	33.714	9.290 9.290	313.2 313.6	
	3	33.754	C3	1	33.754 33.784	9.290	313.9	
	4	33.784 33.822	C4 C5	1	33.764	9.290	314.2	
	5	33.022	<u> </u>	L	33.022	7.270	317.4	
1	TOTAL			5	168.776		1568.0	
		UB-TOTAL	WEIGHT			BRIDGE.	1300.0	
		Wp=		5 ×	_	Dinie	=	15,680.0 kgf
ļ		, · · · · · · · · · · · · · · · · · · ·	1000	, ,	-			15.7 ton
	T	ENSION UI	VIT					
I .		Ns≃		2 ×	2		=	20.0 nos
		HEATHING						
			168.776 ×	5 ×	2		=	1,687.8 m
		CENEBT G						
		V=	0.848 ×	: 5 ×	2		_ =	8.5 m3
						•		,
1						Total	$\mathbf{w}_{\mathbf{p}} =$	54,304.0 kgf
								54.3 ton
							$N_S =$	72.0 nos
}				-			_	
							L =	5,845.2 m
							v =	29.4 m3
							-	1
	<u> </u>				·			<u> </u>

Item			)	Formula				Quantity
2012 2								
3S12.7	PC CABLE OF	DIAPHRAG						
	LOCATION	EACH LENGTH	CABLE NO.	EACH	TOTAL LENGTH	UNIT WEIGHT	WEIGHT	
	Connection One	10.050	110.	72	723.6	2.322	1680.199	
			· · · · · · · · · · · · · · · · · · ·		·····			
	TOTAL			72	723.600		1680.199	
		OTAL WEIC	1680.1		S per BRID	GE(A1-P12)	) =	1,680.2 kg 1.7 to
	T	ENSION UN Ns=	NT 72 ×	2			<del></del>	144.0 no
		HEATHING		-			=	723.6 m
		CENEBT G V=	ROUT 0.002 ×	722 600	٠		=	1.4 m
		<b>v</b> =	0.002 X	723.000			_	1.7 #
	·	*			•		•	
			÷					
				* *				
								·
								·
	·							
		• .		•	•			
							-	
			*					
			•	4				
					•			

Item	Formula	Quantity
Shear Key		
	STEEL JOINT KEY  PC CABLE  O  O  O  O  O  O  O  O  O  O  O  O  O	
	N1 3 $\times$ 4 $\times$ 10 $\times$ 3 = 360 N2 3 $\times$ 3 $\times$ 10 $\times$ 1 = 90 Total 450	

ltem	Formula	Quantity
Erection		
Prection		
	(1) Calculation of Sectional Area	
	fil Middle Castion	
	[1] Middle Section Summary of Sectional Area For ONE GIRDER	
	No Formula (m2)	
	$1 \qquad 0.650 \times 0.080 \qquad = 0.052$	
	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	
	$ \begin{array}{ c c c c c c } \hline & 0.200 \times & 0.110 & = & 0.022 \\ \hline 4 & 1.090 \times & 0.200 & = & 0.218 \\ \hline \end{array} $	
	$0.500 \times 0.225 \times 0.200 \times 2 = 0.045$	
	$\begin{array}{ c c c c c c c c c c c c c c c c c c c$	
	$6 \qquad 0.650 \times 0.250 \qquad = 0.163$	
	Total Area 0.678 m <sup>2</sup>	
	850	
	100 850 100	
·		•
	1000	
÷	225 200 225,	
		· ·
	(6) (8) (8) (9) (9) (9) (9) (9) (9) (9) (9) (9) (9	ļ
	650	
	·	1
	[2] End Section Summary of Sectional Area For ONE GIRDER	
	No Formula (m2)	
	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	
	$\begin{array}{ c c c c c c c c c c c c c c c c c c c$	
	$1/2 \times 0.100 \times 0.034 \times 2 = 0.003$	
	$0.650 \times 0.034 = 0.022$	
	$\begin{array}{ c c c c c c c c c c c c c c c c c c c$	
	10tal Arca 1.225 m	
	100 650 100	
	2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	
	3) 600	
	31816	
		;
	850	

ltem				Formul	a				Quantity	
	_									
	Concret	te Volume		C:	ndon I on oth	28.600		ŀ		
			Section	Average of	rder Length Length of	Concrete	111	1		
		Section No.	Area	Section	Block	Volume	Remark			
			(m2)_	(m2)	(m)	(m3)				
		END	1.229							
		END	1.229	1.229	0.500	0.615				
		MIDDLE	0.678	0.954	4.300	4.102				
	* *	MIDDLE	0.678	0.678		12.882				
		END	1.229	0.954	4.300	4.102				
		END	1.229	1.229	0.500	0.615				
•		Total			28.600	22.316		<u> </u>		
	Erection	_	0.500				per one gir	der	5.5500.0.1	
	W 1	= 22.316	× 2500	C!	1 7 49	21.600		=	55790.0 k	.gi
	•		Section	Average of	rder Length	31.600 Concrete	m	, 1		
l		Section No.	Area	Section	Block	Volume	Remark			
		Section 1 w.	(m2)	(m2)	(m)	(m3)	144111111			
1		END	1.229				<del></del>			
1		END	1.229	1.229	0.500	0.615				
·		MIDDLE	0.678	0.954	4.800	4.579				
	•	MIDDLE	0.678	0.678	21.000	14.238		]		
		END	1.229	0.954	4.800	4.579				
		END	1.229	1.229	0.500	· · · · · · · · · · · · · · · · · · ·				
		Total	,,		31.600	24.626		]		
	Erection	_	•				per one gir	der		
	W 2	= 24.626	× 2500					=	61565.0 k	.gf
			C4'		rder Length	33.000	m	,		
		Section No.	Section	Average of Section	Length of Block	Concrete   Volume	Remark			
. [		Section No.	Area (m2)	(m2)	(m)	(m3)	Kemark			
İ		END	1.229		\ <u>\\</u>	()				
		END	1.229		0.500	0.615		1		
		MIDDLE	0.678		4.800	4.579		1		
		MIDDLE	0.678	0.678	22.400	15.187	, ,,,,, , ,,-,-	1		
		END	1.229		4.800	4.579				
	•	END	1.229	1.229	0.500	0.615				
		Total			33.000	25.575				
	Erection	Weight					per one gir	der		
	W 3	= 25.575	× 2500					=	63937.5 k	gf
٠					rder Length	36.000	m	,		
		N 1	Section	Average of		Concrete Volume	Domest			
		Section No.	Area (m2)	Section (m2)	Block (m)	(m3)	Remark			
		END	1.229		(111)	(111.5)				
		END	1.229		0.500	0.615				
		MIDDLE	0.678							
		MIDDLE	0.678			<del> </del>				
		END	1.229			5.152				
]		END	1.229		<del></del>	<del></del>				
		Total			36.000					
	Erection	Weight		·	<del> </del>	·	per one gir	der		
	W 4	= 27.942	× 2500		¥		. ·· •	=	69855.0 k	gf
				44				١		_
	W =	( 55790.0	+ 61565.0	+ 63937.5	+ 69855.0	)× 5	× 2	=	2511475.0 k	gf
		•						ļ	2511.5 t	on

Total Weight of Girder

	- D14	D14 - D25	Total	Note
L=36.0	8,279.8	45,929.0	54,208.8	
L=31.6m	7,463.8	40,517.0	47,980.8	
L=28.6m	6,581.2	35,806.0	42,387.2	
L=33.0m	7,715.8	42,468.0	50,183.8	
Total	30,040.6	164,720.0	194,760.6	(kgf)
	30.0	164.7	194.8	(ton)

L=36.0m

•	SIGN	DIAMETER	LENGTH	NOS.	UNIT WEIGHT	PIECE WEIGHT	OTAL WEIGH	NOTE
G-	1	D14	4,411	190	1.208	5.330	1,012.7	
	2	D14	4,636	72	1.208	5.600	403.2	(AVE)
	3	D14	4,861	6	1.208	5.870	35.2	
	4	D14	4,861	6	1.208	5.870	35.2	
	5	D14	1,751	190	1.208	2.120	402.8	
	6	D14	1,243	190	1.208	1.500	285.0	
	7	D14	1,935	112	1.208	2.340	262.1	
	8	D14	1,635	72	1.208	1.980	142.6	(AVE)
	9	D14	1,345	6	1.208	1.620	9.7	
	10	D22	35,700	6	2.984	107.000	642.0	
	11	D14	35,700	22	1.208	43.100	948.2	
	12	D14	35,708	12	1.208	43.100	517.2	
	13	D14	987	36	1.208	1.190	42.8	
	14	D14	1,000	12	1.208	1.210	14.5	
	15	D16	1,600	84	1.579	2.530		Interior girder
	16	D16	1,100	84	1.579	1.740	146.2	Exterior girder
•	. 17	D10	570	24	0.617	0.352	8.4	
	18	D10	150	104	0.617	0.093	9.7	
	19	D14	2,017	190	1.208	2.440	463.6	
				<u> </u>		***************************************		
					Interior	Exterior	Total	
					6 поз.	4 nos.		
		<del>                                     </del>	• 1	D22	642.0	642.0	6,420.0	
	-			D16	212.5	146.2	1,859.8	
		1		D14	4,574.8	4,574.8	45,748.0	
				D10	18.1	18.1	181.0	
				Sub-Total	5,447.4	5,381.1	27,104.4	
					- D14		8,280	for one span
					D14 - D25	•	45,929	

L=31.6m

#### LIST OF REINFORCEMENT

SIGN	DIAMETER	LENGTH	NOS.	UNIT WEIGHT	PIECE WEIGHT	OTAL WEIGH	NOTE
G- 1	D14	4,411	166	1,208	5.330	884.8	
2	D14	4,636	66	1,208	5.600	369.6	(AVE)
3	D14	4,861	6	1.208	5.870	35.2	
4	D14	4,861	6	1.208	5.870	35.2	
5	D14	1,751	166	1.208	2.120	351.9	
6	D14	1,243	166	1.208	1.500	249.0	
7	D14	1,935	94	1.208	2.340	220.0	
8	D14	1,635	66	1.208	1.980		(AVE)
9	D14	1,345	6	1.208	1.620	9.7	
10	D22	31,300	6	2.984	93.400	560.4	
11	D14	31,300	22	1.208	37.800	831.6	<u> </u>
12	- D14	31,308	12	1.208	37.800	453.6	
13	D14	987	36	1.208	1.190	42.8	
14	D14	1,000	12	1.208	1.210	14.5	
15	D16	1,600	84	1.579	2.530		Interior girder
16	D16	1,100	84	1.579	1.740		Exterior girder
17	D10	570	24	0.617	0.352	8.4	
- 18	D10	150	104	0.617	0.093	9.7	
19	D14	2,017	166	1.208	2.440	405.0	
			·				
		-					
			<u> </u>			1	
				Interior	Exterior	Total	
				6 nos.	4 nos.		
			D22	560.4	560.4	5,604.0	
			D16	212.5	146.2	1,859.8	
			D14	4,033.6	<del>}</del>	<del> </del>	
		<del> </del>	D10	18.1	18.1	·	
			Sub-Total	4,824.6	4,758.3	23,990.4	
						7.44	6
<u> </u>				- D14		7,464	
				D14 - D25	ļ	40,517	

L=28.6m

S	SIGN	DIAMETER	LENGTH	NOS.	UNIT WEIGHT	PIECE WEIGHT	OTAL WEIGH	' NOTE
G۰	1	D14	4,411	146	1.208	5.330	778.2	
	2	D14	4,636	60	1.208	5.600	336.0	(AVE)
	3	D14	4,861	6	1.208	5.870	35.2	·
	4	D14	4,861	4	1.208	5.870	23.5	
	5	D14	1,751	146	1.208	2.120	309.5	
	6	D14	1,243	146	1.208	1.500	219.0	
	7	D14	1,935	80	1.208	2.340	187.2	
	8	D14	1,635	60	1.208	1.980	118.8	(AVE)
	9	D14	1,345	6	1.208	1.620	9.7	
	10	D22	28,360	6	2.984	84.600	507.6	
	11	D14	28,360	22	1.208	34.300	754.6	
	12	D14	27,370	12	1.208	33.100	397.2	
	13	D14	987	23	1.208	1.190	27.4	
	14	D14	1,000	12	1.208	1.210	14.5	
	15	D16	1,600	68	1.579	2.530	172.0	Interior girder
	16	D16	1,100	68	1.579	1.740	118.3	Exterior girder
	17	D10	570	18	0.617	0.352	6.3	
	18	D10	150	78	0.617	0.093	7.3	
	19	D14	2,017	. 146	1.208	2.440	356.2	
					Interior	Exterior	Total	
					6 nos.	4 nos.	10(4)	
ļ				D22	507.6	507.6	5,076.0	
				D16	172.0	118.3	1,505.2	
<u> </u>			· · · · · · · · · · · · · · · · · · ·	D10	3,567.0	3,567.0	35,670.0	
				D10	13.6	13.6	136.0	
				Sub-Total	4,260.2	4,206.5	21,193.6	· · · · · · · · · · · · · · · · · · ·
				540-10441	7,200.2	1,200.5	21,175.0	<u> </u>
					- D14		6,581	for one span
					D14 - D25		35,806	

L=33.0m

SIG	N	DIAMETER	LENGTH	NOS.	UNIT WEIGHT	PIECE WEIGHT	OTAL WEIGH	NOTE
G-	1	D14	4,411	176	1.208	5.330	938.1	
	2	D14	4,636	66	1.208	5.600	369.6	(AVE)
	3	D14	4,861	6	1.208	5.870	35.2	·
	4	D14	4,861	6	1.208	5.870	35.2	
	5	D14	1,751	176	1.208	2.120	373.1	
	6	D14	1,243	176	1.208	1.500	264.0	
	7	D14	1,935	104	1.208	2,340	243.4	
	8	D14	1,635	66	1.208	1.980	130.7	(AVE)
	9	D14	1,345	6	1.208	1.620	9.7	
1	10	D22	32,700	6	2.984	97.600	585.6	
	11	D14	32,700	22	1.208	39.500	869.0	
1	2	D14	32,706	12	1.208	39.500	474.0	
1	13	D14	987	36	1.208	1.190	42.8	
1	 14	D14	1,000	12	1.208	1.210	14.5	
1	15	D16	1,600	84	1.579	2.530	212.5	Interior girder
1	16	D16	1,100	84	1.579	1.740	146.2	Exterior girder
	17	D10	570	24	0.617	0.352	8.4	
	18	D10	150	104	0.617	0.093	9.7	
	19	D14	2,017	176	1.208	2.440	429.4	
,								
<u></u>	·- · · · · ·				Interior	Exterior	Total	
		1	·		6 nos.	4 nos.	2 0141	
				D22	585.6	585.6	5,856.0	
				D16	212.5	<del></del>		
<del></del>				D10	4,228.7	4,228.7		
		<del>                                     </del>		D10	18.1	18.1	181.0	
				Sub-Total	5,044.9	4,978.6	<del></del>	<del></del>
		<u> </u>			_,		,	
		<u> </u>			- D14		7,716	for one span
					D14 - D25		42,468	<del></del>
		1.					-	

Diaphragm

	SIGN	DIAMETER	LENGTH	NOS.	UNIT WEIGHT	PIECE WEIGHT	TOTAL WEIGHT	NOTE
D	1	D16	1,700	576	1.579	2.680	1,543.7	
	2	D14	1,700	224	1.208	2.050	459.2	(AVE)
E	1	D16	4,724	1,408	1.579	7.460	10,503.7	
	2	D14	3,924	616	1.208	4.740	2,919.8	
С	1	D16	1,700	384	1.579	2.680	1,029.1	
	2	D14	2,400	192	1.208	2.900	556.8	
	3	D14	5,442	224	1.208	6.570	1,471.7	
	4	D14	5,514	112	1.208	6.660	745.9	(AVE)
	5	D16	9,900	8	1.579	15.600	124.8	
						TOTAL	19,354.7	
					END	INTERMEDIAT	CONNECTION	
	-			D16	1543.7	10503.7	1153.9	
				D14	459.2	2919.8	2774.4	
				Total	2,002.9	13423.5	3928,3	
							(kg)	(ton)
						- D14	6,153.4	6.2
						D14 - D25	13,201.30	13.2

Total Weight of Slab

	- D14	D14 - D25	Total	Note
L=28.6m	8,702.7	25,489.4	34,192.1	
L=31.6m	9,826.6	27,900.6	37,727.2	
L=33.0m	10,308.4	28,996.6	39,305.0	
L=36.0m	11,311.8	30,969.4	42,281.2	
Total	40,149.5	113,356.0	153,505.5	(kgf)
	40.1	113.4	153.5	(ton)

L=57.4

Γ	SIGN	DIAMETER	LENGTH	NOS.	UNIT WEIGHT	PIECE WEIGHT	TOTAL WEIGHT	NOTE
S	1	D20	11,350	353	2.466	28.000	9,884.0	
	2	D20	10,850	353	2.466	26.800	9,460.4	(AVE)
	3	D14	963	766	1.208	1.160	888.6	
	4	D22	12,000	100	2.984	35.800	3,580.0	
	5	D14	22,780	100	1.208	27.500	2,750.0	
	5'	D14	25,206	100	1.208	30.400	3,040.0	
	6	D25	11,350	30	3.854	43.700	1,311.0	(AVE)
	7	D25	10,850	30	3.854	41.800	1,254.0	
	8	D12	565	4,032	0.888	0.502	2,024.1	
						TOTAL	34,192.1	
						D25	2565.0	
	· · · · · · · · · · · · · · · · · · ·					D22	3580.0	
		Ì				D20	19344.4	
L						D14	6678.6	
						D12		
						Total	34,192.1	
							(kgf)	
					- D14		8,702.7	
					D14 - D25		25,489.4	
						Total	34,192.1	

L=63.4

S	GN	DIAMETER	LENGTH	NOS.	UNIT WEIGHT	PIECE WEIGHT	TOTAL WEIGHT	NOTE
S	1	D20	11,350	397	2.466	28.000	11,116.0	
	2	D20	10,850	397	2.466	26.800	10,639.6	(AVE)
-,,-	3	D14	963	854	1.208	1.160	990.6	
	4	D22	12,000	100	2.984	35.800	3,580.0	
	5	D14	26,206	100	1.208	31.700	3,170.0	
	5'	D14	28,206	100	1.208	34.100	3,410.0	
	6	D25	11,350	30	3.854	43.700	1,311.0	(AVE)
	7	D25	10,850	30	3.854	41.800	1,254.0	
	8	D12	565	4,494	0.888	0.502	2,256.0	
						TOTAL	37,727.2	
						D25	2565.0	,
						D22	3580.0	
						D20	21755.6	
						D14	7570.6	
						D12	2256.0	
						Total	37727.2	
							(kgf)	
					- D14		9,826.6	
				·	D14 - D25		27,900.6	
						Total	37,727.2	
		1						

L=66.2

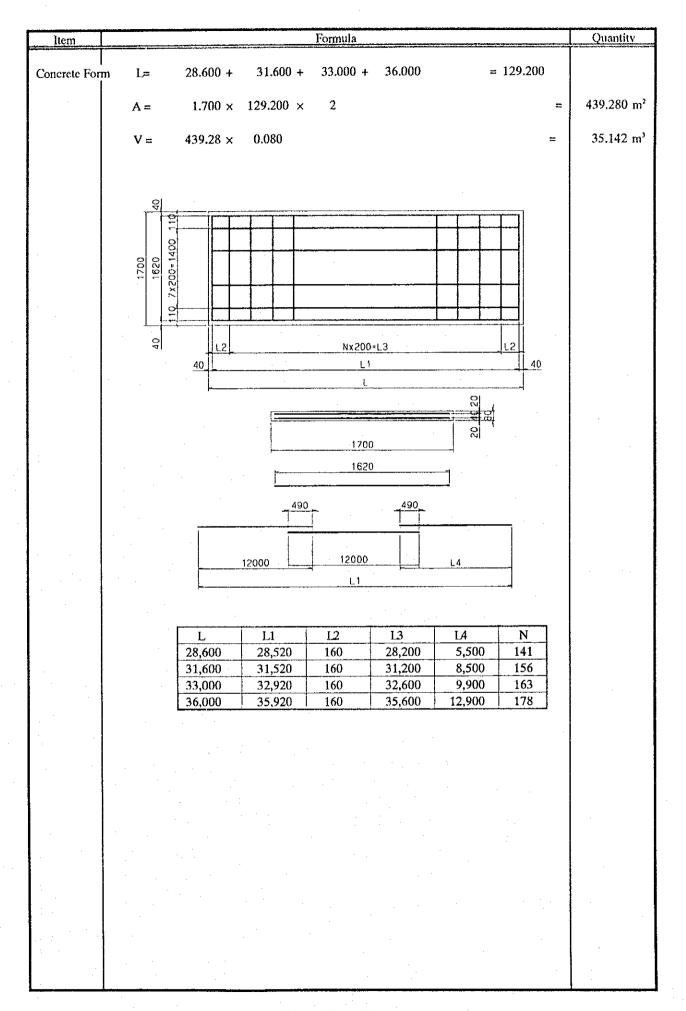
SI	GN	DIAMETER	LENGTH	NOS.	UNIT WEIGHT	PIECE WEIGHT	TOTAL WEIGHT	NOTE
S	1	D20	11,350	417	2.466	28.000	11,676.0	
	2	D20	10,850	417	2.466	26.800	11,175.6	(AVE)
	3	D14	963	894	1.208	1.160	1,037.0	
	4	D22	12,000	100	2.984	35.800	3,580.0	
	5	D14	27,606	100	1.208	33.300	3,330.0	
	5'	D14	29,606	100	1.208	35.800	3,580.0	
	6	D25	11,350	30	3.854	43.700	1,311.0	(AVE)
	7	D25	10,850	30	3.854	41.800	1,254.0	
	8	D12	565	4,704	0.888	0.502	2,361.4	
						TOTAL	39,305.0	
		]						
				Ì	i			
						D25	2565.0	
						D22	3580.0	
						D20	22851.6	
						D14	7947.0	
		·		·		D12	2361.4	
						Total	39,305.0	
							(kgf)	
					- D14		10,308.4	
					D14 - D25		28,996.6	
						Total	39,305.0	

L=72.2

SI	GN	DIAMETER	LENGTH	NOS.	UNIT WEIGHT	PIECE WEIGHT	TOTAL WEIGHT	NOTE
S	1	D20	11,350	453	2.466	28.000	12,684.0	
	2	D20	10,850	453	2,466	26.800	12,140.4	(AVE)
	3	D14	963	966	1.208	1.160	1,120.6	
	4	D22	12,000	100	2.984	35.800	3,580.0	
	5	D14	30,606	100	1.208	37.000	3,700.0	
	5'	D14	32,606	100	1.208	39.400	3,940.0	
	6	D25	11,350	30	3.854	43.700	1,311.0	(AVE)
	7	D25	10,850	30	3.854	41.800	1,254.0	
	8	D12	565	5,082	0.888	0.502	2,551.2	
						TOTAL	42,281.2	
						D25		
				·		D22	3580.0	
						D20		
			1			D14		
						D12	2551.2	
						Total	42,281.2	
							(kgf)	
					- D14		11,311.8	
					D14 - D25		30,969.4	31.0
						Total	42,281.2	42.3

Item	<u>Formula</u>	Quantity
9 Pavement 1)	Pavement t=75mm	
	L = 72.400 + 63.600 + 57.600 + 66.400 = 260.0 m	
	$A = 10.750 \times (260.00 - 0.850 \times 4)$ = 2758.5 m <sup>2</sup>	2758.5 <b>m</b> ²
2)	Water Proofing t=5mm	
	$A = 2758.5 \text{ m}^2$	2758.5 <b>m</b>
3)	275 EXP-JOINT 275	
	=300 67.5,125.67.5 20 20 75	
	200 Z 200 MINISTER OF THE PROPERTY OF THE PROP	
	400 50 330	
	per one side	
	$A1 = 0.400 \times 0.075 - 0.1 \times 0.034$ = 0.027 $A2 = 0.075 \times 0.300$ = 0.023	
	$A2 = 0.075 \times 0.300 = 0.023$ Total 0.050 m <sup>2</sup>	
	$V = 0.050 \times 10.750 \times 8 = 4.300 \text{ m}^3$	4.300 m³
4)	Road marking	
	Bridge Length L= 260.000 - 0.600 = 259.400 m	
	Side Line	·
	$A1 = 259.400 \times 0.100 \times 4 = 103.760 \text{ m}$	
	Center Line L = 259.400 / 5.000 = 51.9 m	
	$A2 = 2.500 \times 0.100 \times 51.9 \times 2$ = 25.950 m <sup>2</sup>	
	Total $103.760 + 25.950 = 129.7 \text{ m}^3$	129.7 m²

Item	Formula		Quantity
1. Expansion J	oint (TYPE A)		
	EACH LENGTH		
	EACH LENGTH $L = 10.750$		
	$N = 1 \times 4 = 4$		
,	TOTALIENGTH		
	$L = 10.750 \times 4$	=	43.000 m
			4
2. BEARING	PAD ELASTOMERIC 600*300*57		
	EACH for One SPAN		
	N = 5		
	TOTALEACH		
			nos
	$N = 5 \times 4 \times 4$	=	80
3. ANCHOR I	BAR		
	Φ75 L=1520 (MOVE)		nos
	$N = 4 \times 8 \times 2$ $\Phi 75  L=1520  (FIX)$	· =	64 nos
	$N = 8 \times 3 \times 4$	=	96
		Total =	160 nos
4 ANICHOD	CAD (GCD)		· .
4. ANCHOR		•	
	$\Phi 95  L=800  (FIX)$ $N = 4  \times  3  \times  4$	=	nos 48
			•
	$\Phi 125 = 800  (MOVE)$ $N = 4 \times 2$	=	nos 8
	Φ150 L=800 (MOVE)		nos
	$N = 4 \times 2$	=	8
6 D - 10	L = 72.400 + 63.600 + 57.600 + 66.400	_ 260.0	
5.Railing		= 260.0 m	·
	L= 260.000 × 2 × 2	<u> </u>	1040 m
•		* *	
		·	



Item		Formula				Quantity
	Re-bar				Per 28.6m	
			WEIGHT	NO.		
	BAR SIZE LEN	IGTH WEIGHT/M	/One	of	WEIGHT	
	MARK (mm) (mn		(kgf)	BARS	(kgf)	
		520 1.042	1.688	144	243	
		000 1.042	12.504	20	250	
	S3 D 14 5	500   1.042	5.731	10	57	
ļ					550	
-		1		<del>,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,</del>	550	
	$w1 = 550.0 \times$	2			= 1100 kgf	
	W1 = 330.0 X	2		•	- 1100 kgr	
						•
ļ	Re-bar				Per 31.6m	
	TO CALL		WEIGHT	NO.		
	BAR SIZE LEI	IGTH WEIGHT/M	/One	of	WEIGHT	
	MARK (mm) (mm	1	(kgf)	BARS	(kgf)	
		620   1.042	1.688	159	268	*
		2000 1.042	12.504	20	250	
	S3 D 14 8	500 1.042	8.857	10	89	
					607	
					1	
	$w 2 = 607.0 \times$	2			= 1214 kgf	
		•	•			
	Re-bar				Per 33.0m	
		LOWER STREET	WEIGHT	NO.	WEIGHT	* .
	i   1	NGTH WEIGHT/M	/One	of BARS	WEIGHT	
	MARK (mm) (mr		(kgf) 1.688	166	(kgf) 280	
		620 1.042 2000 1.042	12.504	20	250	
	1	900 1.042	10.316	10	103	
	33 12 14 3	1.042	10.510		1 1	
					633	
						-
	$w3 = 633.0 \times$	2			= 1266 kgf	
				•		
					Ì	
	Re-bar				Per 36.0m	*
			WEIGHT	NO.		
	BAR SIZE LE	NGTH WEIGHT/M	/One	oí	WEIGHT	
	MARK (mm) (m		(kgf)	BARS	(kgf)	
	1	1.042	1.688	181	306	
		2000 1.042	12.504	20	250	
1	S3 D 14 1	2900 1.042	13.442	10	134	
		·			600	
				<u> </u>	690	•
	4				1200 10	
	$w4 = 690.0 \times$	2			= 1380 kgf	
				w	= 4960 kgf	4,960 kgf
		•		. **	~ 4700 KgJ	4,960 kgr 5.0 ton
						ว.บ เดก
<u> </u>						<del> </del>

# 1.Quantity of Superstructure

			re Main Bridge			per one side
Item		Work Item		Unit	Quantity	Remarks
Concrete	CLASS B	Total		cn.m	1439.9	
			Cantilever	sq.m	1420.4	
		External	Pier Head	sq.m	322.6	
			Support	sq.m	369.3	
			Total	sq.m	2112.4	
Form			Cantilever	sq.m	1033.6	
		Internal	Pier Head	sq.m	295.7	
			Support	sq.m	169.3	
			Total	sq.m	1498.6	
		Total		sq.m	3611.0	
	·		~D14	ton	64.0	
Re-bar		Total	D14~D25	ton	264.5	
Ke-bai			D25~	ton		
			Total	ton	328.5	
	12S12.7B	Internal Longitudinal	Prestressing Ten	ton	68.8	SWPR7B
PC Steel	19S15.2B	External Longitudinal	ton	22.7	SWPR7B	
	3S12.7	Internal Transverse Tendons			3.6	
	12S12.7B			nos	428	
	19S15.2B		nos	108		
Duct	3S12.7			nos	272	
Duci	Φ80/85			m	4963.8	
	Φ90/100	0			2448.0	
	FlatDuct25	5x80			1557.5	
Cement grout	in sheathng	5		m3	40.6	
Bearing	Product lay	er rubber bearing	660*560*125	nos	4	
			1400*1500*214	nos	4	
-		Anchor Bar	Φ75 L≃1250	nos	8	
		Anction bai	Φ75 L=2500	nos	8	
		Anchor Cap		nos	. 8	
·		Anchor Cap	Φ150 L=850	nos	8	
Expansion joint		100mm		m	21.5	<u> </u>
Pavement	t=75mm	Aspfalt concrete surface course		sq.m	1386.8	
	t= 5mm	Water Prod	sq.m	115.6		
· · · · · · · · · · · · · · · · · · ·		Concrete			1.054	
		Road Mar	cu.m sq.m	58.1		

# 1. Concrete

# Total Quantity

	Quantity						(m³)
Item	Main part	Diaphragm	Manhole	Haunch	Expansiom	Anchorage	Total
Cantilever part	824.136	17.180				14.528	855.844
Pier Head part	222.606	149.368		1.640			373.614
Supporting Part	193.452	18.825	-1.650	0.828	-1.011		210.444
Total	1240.194	185.373	-1.650	2.468	-1.011	14.528	1439.902

(1) Supporting Part on P2 Side

Block	Section	Section Area	Ave. of	Length of	Concrete	Total of each
		(m <sup>2</sup> )	Area (m2)	(m)	(m <sup>3</sup> )	(m <sup>3</sup> )
1	1	16.930				
	2	16.930	16.930	1.650	27.935	27.935
	3	8.150	12.540	0.000	0,000	0.000
	4	7.120	7.635	5,000	38.175	38.175
	5	7.120	7.120	2,800	19.936	19.936
Total				9.450	86.046	86.046

(2) Cantilever part on P2 -> P3

Block	Section	Section Area	Average of	Length of	Concrete	Total of each
		(m³)	(m <sup>†</sup> )	(m)	(m <sup>3</sup> )	(m <sup>3</sup> )
	5	7.120				
2	6	7.408	7.264	3.500	25.424	25.424
3	7	8.332	7.870	3.500	27.545	27,545
4	8	8.850	8.591	3,500	30.069	30.069
5	9	9.410	9.130	3.000	27.390	27.390
6	10	10.031	9.721	3.000	29.162	29.162
7	11	11.137	10.584	3.000	31.752	31.752
8	12	11.991	11.564	3.000	34.692	34.692
Total				22,500	206.034	206,034

(3) Pier Head part on P3

Block	Section	Section Area (m³)	Average of (m)	Length of (m)	Concrete (m³)	Total of each (m³)
	12	11.991				
ģ	13	12.555	12.273	3.000	36.819	36.819
- "	14	12.555	12.555	0.000		
10	15	12.555	12.555	3.000	37.665	37.665
	16	12.555	12.555	0.000		
9	17	11.991	12.273	3.000	36.819	36.819
Total				9.000	111.303	111.303

(4) Cantilever part on P3 -> P4

Block	Section	Section Area	Average of	Length of	Concrete	Total of each
		(m²)	<u>(m²)</u>	(m)	(m <sup>3</sup> )	(m <sup>3</sup> )
	17	11.991		Į		2
8	18	11.137	11.564	3.000	34.692	34.692
7	19	10.031	10.584	3.000	31.752	31.752
6	20	9.410	9.721	3.000	29.162	29.162
.5	21	8.850	9.130	3.000	27.390	27.390
4	22	8.332	8.591	3.500	30.069	30.069
6	23	7.408	7.870	3.500	27.545	27.545
2	24	7.120	7.264	3.500	25.424	25.424
Total				22.500	206.034	206.034

(5) Supporting Part on center of Span

Block	Section	Section Area (m²)	Average of (m)	Length of (m)	Concrete (m³)	Total of each (m <sup>3</sup> )
	24	7.120				
11	24	7.120	7.120	3.000	21.360	21.360
Total				3.000	21.360	21.360

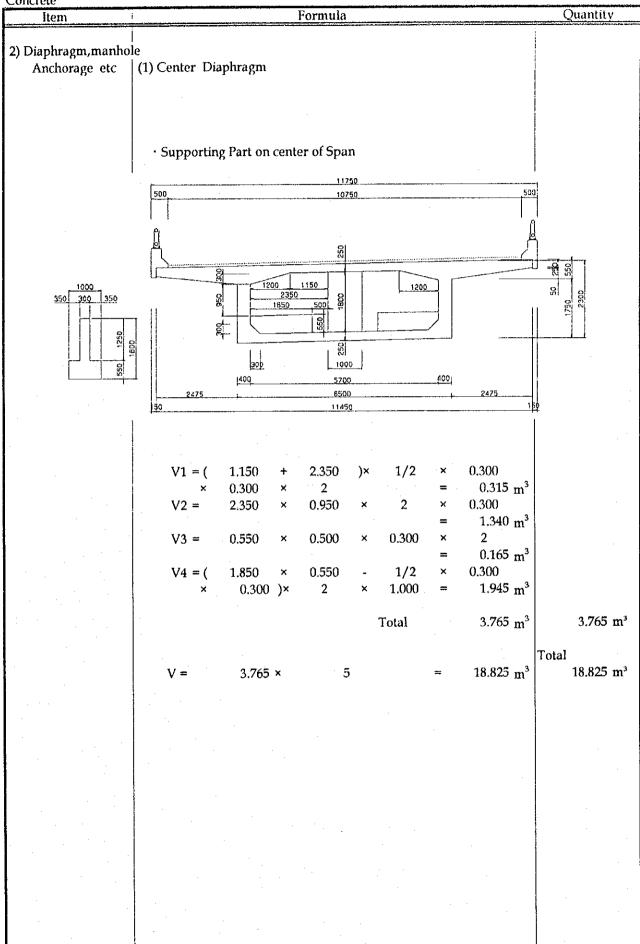
(6) Cant	ilever par	t on P3 -> P4				
Block	Section	Section Area	Average of	Length of	Concrete	Total of each
		(ml)	(m²)	(m)	(m <sup>3</sup> )	(m <sup>3</sup> )
	24	7.120				
2	23	7.408	7.264	3.500	25.424	25,424
3	22	8.332	7.870	3.500	27.545	27.545
<u> </u>	21	8.850	8.591	3.500	30.069	30.069
	20		9.130	3.000	27.390	27.390
	19		9.721	3.000	29.162	29.162
<del>-</del> 7			10.584	3.000	31.752	31.752
8		11.991	11.564	3.000	34.692	34.692
<u>-</u>						
Total	<u> </u>			22,500	206.034	206.034

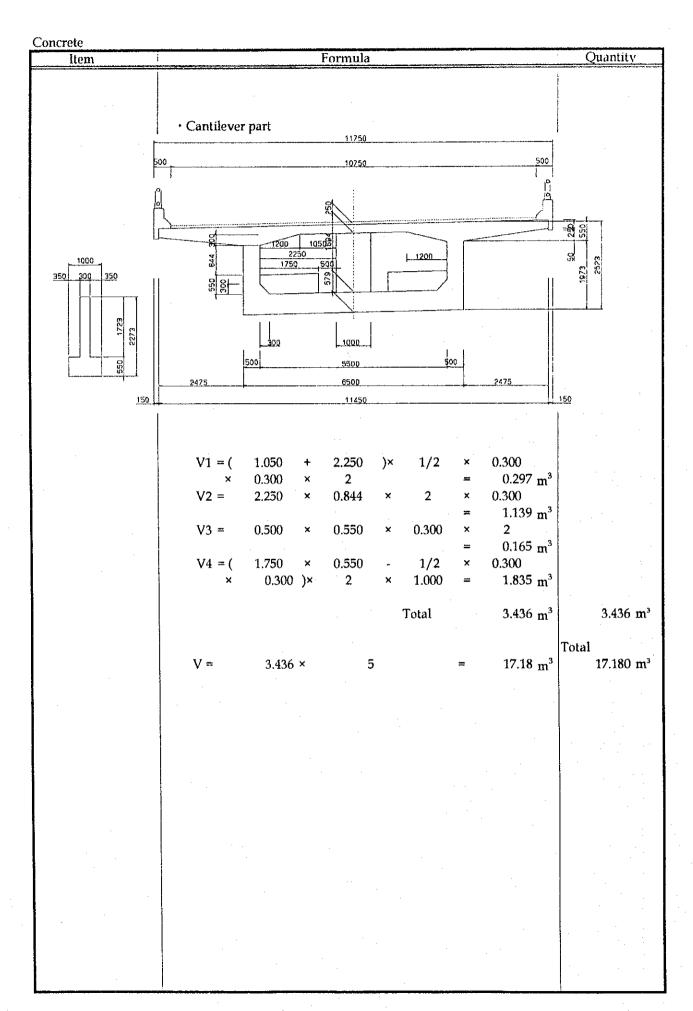
Block	Section	Section Area	Average of (m <sup>2</sup> )	Length of (m)	Concrete (m <sup>3</sup> )	Total of each (m <sup>3</sup> )
	17	11.991				
. 9	16	12.555	12.273	3.000	36.819	 36.819
	15	12.555	12.555	0.000		
10		12.555	12.555	3.000	37.665	 37.665
	13	12.555	12.555	0.000		
. 9	12		12.273	3.000	36.819	36.819
Total	<del> </del>			9.000	111.303	111.303

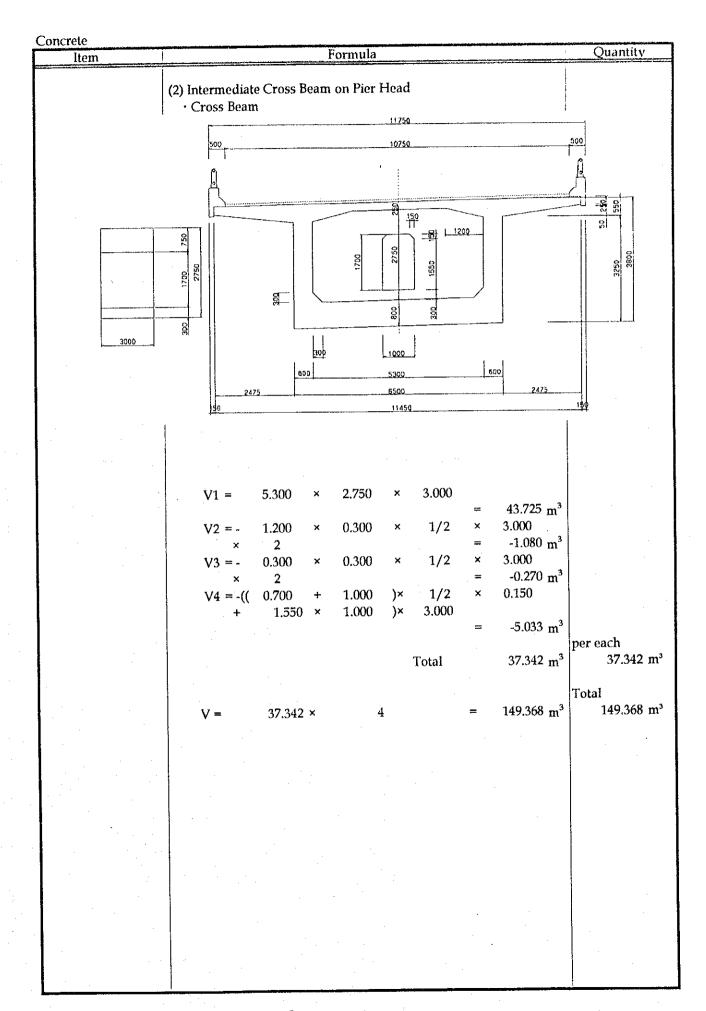
Block	Section	Section Area (m <sup>2</sup> )	Average of (m)	Length of (m)	Concrete (m³)		Total of each
- K	12	11.991					
8	11	11.137	11.564	3.000	34.692		34.692
7	10	10.031	10.584	3.000	31.752		31.752
6	9	h	9.721	3.000	29.162		29.162
5	8		9.130	3.000	27.390		27.390
4	7	8.332	8.591	3.500	30.069	M.4	30.069
3	6	7.408	7.870	3.500	27.545		27.545
2	5	7.120	7.264	3.500	25.424		25.424
Total				22.500	206.034		206.034

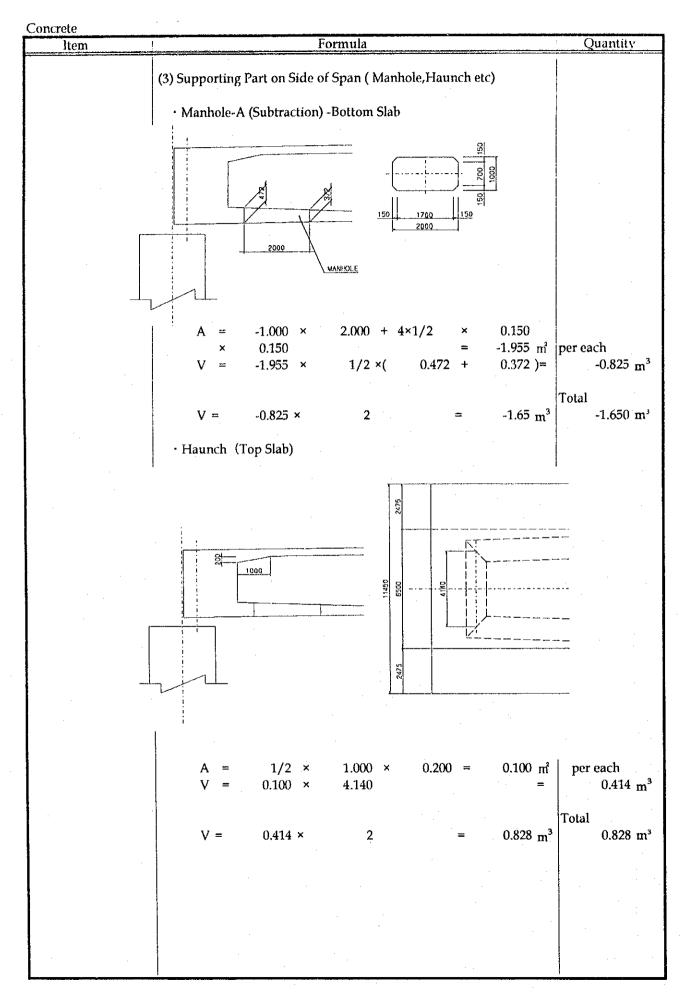
(9) Supp	orting Par	t on P6 Side					
Block	Section	Section Area	Ave. of Area (m2)	Length of (m)	Concrete (m <sup>3</sup> )		Total of each (m <sup>3</sup> )
	5	7.120 7.120	7.120	2.800	19.936	-	19.936
1	3	8.150	7.635	5.000	38.175		38.175
	2	16.930 16.930	12.540 16.930	0.000 1.650	0.000 27.935		27.935
Total	1	10.230	10.550	9.450	86.046		86.046

Total	
Item	Sub-Total
Pier Head part	222.606
Cantilever part	824.136
Supporting Part	193.452
Total	1240.194

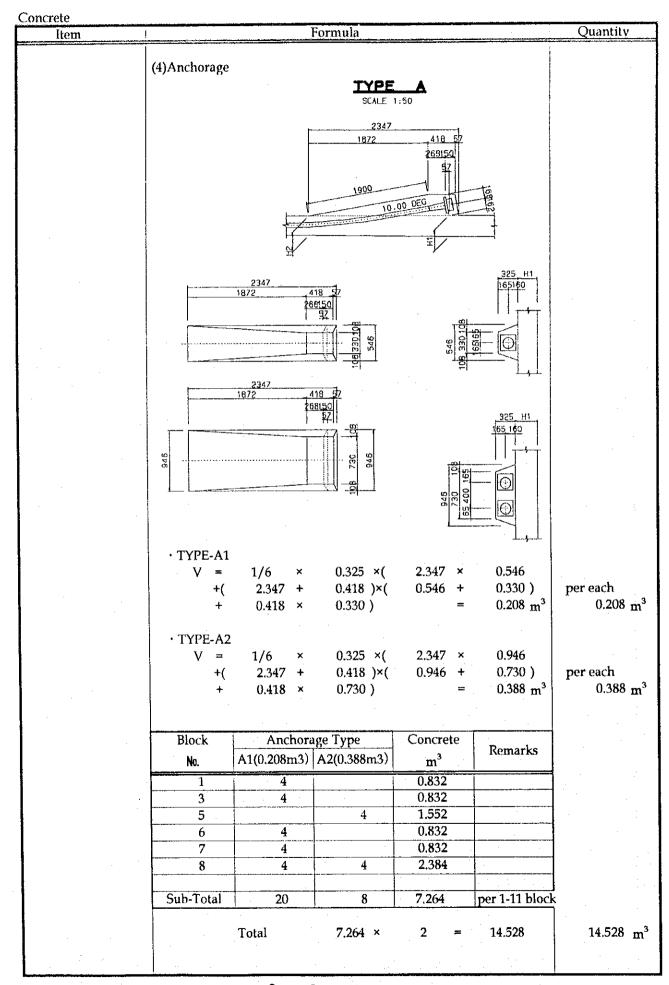


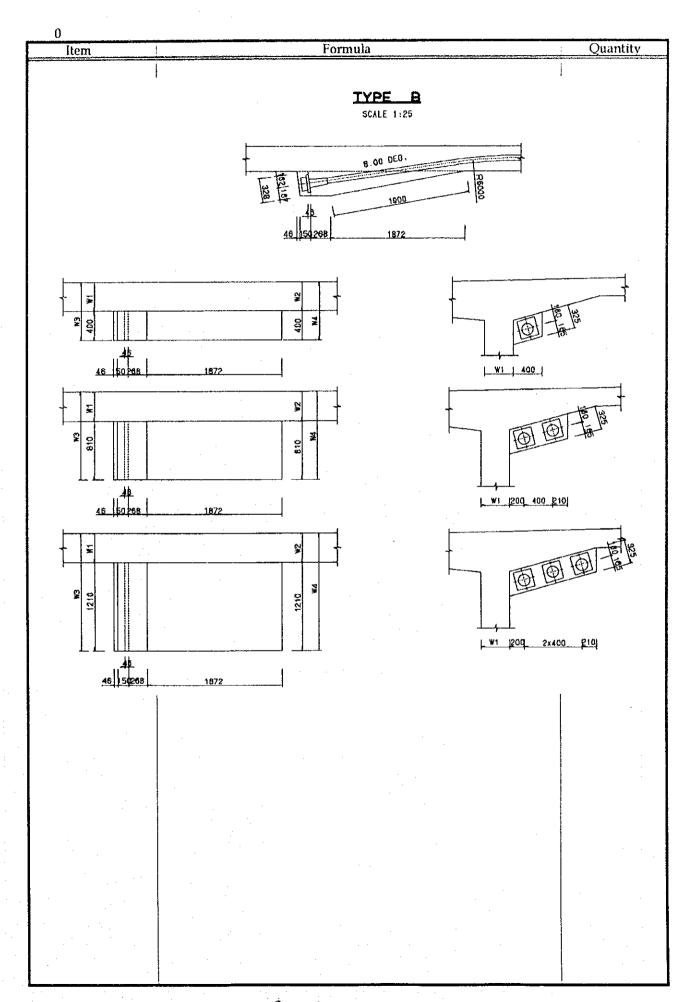






Concrete ltem	Formula	Quantity
	801	
	$A = 1/2 \times 1.000 \times 0.200 = 0.100 \text{ m}^2$ $V = 0.100 \times 4.101$	per each 0.410 m <sup>3</sup> Total
, **	$V = 0.410 \times 4 = 1.6404 \text{ m}^3$	1.640 m <sup>3</sup>
		:
	· Expansion Joint Part	
	$V = -10.750 \times 0.336 \times 0.070 \times 4$ = -1.011	-1.011 m <sup>3</sup>





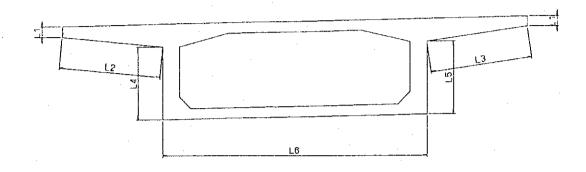
_(	20	n	re		
	-			lt€	1
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Item			Formula		:	Quantity
	1					
					1	
	· TYPE-B1	•				
		1/6 ¥	0.325 ×(	2330 x	0.400	
			0.325 ^( 0.418 )×(		0.400)	per each
	+(				$0.400 \text{ m}^3$	0.1703
	+	0.418 ×	0.400)		0.179 m	$0.179 \text{ m}^3$
	TYPE-B2		·			
	V =			2.336 ×	0.810	
	+(	2.336 +	0.418 )×(	0.810 +	0.810 ) 0.362 m <sup>3</sup>	per each
	+	0.418 ×		<del></del>	$0.362 \text{ m}^3$	$0.362 \text{ m}^3$
			•			
	· TYPE-B3	•				
		1/6 ×	0.325 ×/	2.336 ×	1.210	
			0.323 h( 0.418 )×(		1.210 )	per each
	+(				$0.542 \text{ m}^3$	$0.542 \text{ m}^3$
	+	0.418 ×	1.210)	. =	0.542 m	0.542 m
1.0						
	Block	ļ ,	Anchorage Typ	e	Concrete	
	No.	B1(0.179m3)	B2(0.362m3)	B3(0.542m3)	m <sup>3</sup>	٠
* - *		1	<u> </u>			
	3	4	4		2.164	
·	4	4		4	2.884	
	5	4	4		2.164	
	6	4		4	2.884	
•	7	4	4		2.164	
•	8		4		1.448	
	9	4			0.716	
	) 7		· ·			
	1	20	16	8	12.260	·
	Sub-Total	<del> </del>	16	8		
	1	20	<u> </u>		12.260	24.520 m <sup>3</sup>
	1	<del> </del>	16 12.260 ×	2 =		24.520 m <sup>3</sup>
	1	20	<u> </u>		12.260	24.520 m <sup>3</sup>
	1	20	<u> </u>		12.260	24.520 m <sup>3</sup>
	1	20	<u> </u>		12.260	24.520 m <sup>3</sup>
	1	20	<u> </u>		12.260	24.520 m <sup>3</sup>
	1	20	<u> </u>		12.260	24.520 m <sup>3</sup>
	1	20	<u> </u>		12.260	24.520 m <sup>3</sup>
	1	20	<u> </u>		12.260	24.520 m <sup>3</sup>
	1	20	<u> </u>		12.260	24.520 m <sup>3</sup>
	1	20	<u> </u>		12.260	24.520 m <sup>3</sup>
	1	20	<u> </u>		12.260	24.520 m <sup>3</sup>
	1	20	<u> </u>		12.260	24.520 m <sup>3</sup>
	1	20	<u> </u>		12.260	24.520 m <sup>3</sup>
	1	20	<u> </u>		12.260	24.520 m <sup>3</sup>
	1	20	<u> </u>		12.260	24.520 m <sup>3</sup>
	1	20	<u> </u>		12.260	24.520 m <sup>3</sup>
	1	20	<u> </u>		12.260	24.520 m <sup>3</sup>
	1	20	<u> </u>		12.260	24.520 m <sup>3</sup>
	1	20	<u> </u>		12.260	24.520 m <sup>3</sup>
	1	20	<u> </u>		12.260	24.520 m <sup>3</sup>
	1	20	<u> </u>		12.260	24.520 m <sup>3</sup>
	1	20	<u> </u>		12.260	24.520 m <sup>3</sup>

## 2. Form

## 2-1. External Form

# 1) Dimension Table of Girder



Section	L1	L2	L3	L4	L5	L6	ΣL
1,2	0.250	2.488	2.500	1.750	1.750	6.500	15.238
3	0.250	2.488	2.500	1.754	1.754	6.500	15.246
4	0.250	2.488	2.500	1.781	1.781	6.500	15.300
5	0.250	2.488	2.500	1.855	1.855	6.500	15.448
6	0.250	2.488	2.500	1.978	1.978	6.500	15.694
7	0.250	2.488	2.500	2.156	2.156	6.500	16.050
8	0.250	2,488	2.500	2.421	2.421	6.500	16.580
9	0.250	2.488	2.500	2.780	2.780	6.500	17.298
10	0.250	2.488	2.500	3.250	3.250	6.500	18.238

# 2) Total Quantity

	Quantity						
Item	Main part	Haunch	Man Hole		Total		
Cantilever part	1420.431		i i		1420.431		
Pier Head part	322.644				322.644		
Supporting Part	367.572	0.856	0.856		369.284		
Total	2110.647	0.856	0.856		2112.359		

# 3) Calculation of Girder Form

(1) Supporting Part on P2 Side

Block No.		Form Length	Average of Form Length	Length of Block	Form Area	Bulkhead Cross Beam	Total of each Block
		(m)	(m)	(m)	(m²)	(m²)	(m²)
	Girder I	nd			16.930		16.930
1	1	15.238					
	2	15.238	15.238	9.450	143.999		143.999
Total	<del>                                     </del>			9.450	143.999		160.929

(2) Cantilever part on P2 -> P3

Block		Form Length	Average of	Length of	Form Area	Bulkhead	Total of each
		(m)	(m)	(m)	(m²)	(m²)	<u>(m²)</u>
	2	15.238					
2	3	15.246	15.242	3.500	53.347		53.347
3	4	15.300	15.273	3.500	53.456		53.456
4	5	15.448	15.374	3.500	53.809		53.809
5	6	15.694	15.571	3.000	46.713		46.713
6	7	16.050	15.872	3.000	47.616		47.616
7	8	16.580	16.315	3.000	48.945		48.945
8	9	17.298	16.939	3.000	50.817		50.817
Total				22.500	354.703	······································	354.703

(3) Pier Head part on P3

Block		Form Length	i	Length of	Form Area	Bulkhead	Total of each
		(m)	(m)	(m)	(m)	<u>(ml)</u>	(m)
	9	17.298				- <u>-                                    </u>	
9	10	18.238	17.768	3.000	53.304		53.304
	10	18.238	18.238	0.000			
10	10	18.238	18.238	3.000	54.714		54.714
	10	18.238	18.238	0.000			
9	9	17.298	17.768	3.000	53.304		53.304
Total	<u> </u>			9.000	161.322		161.322

(4) Captilever part on P3-> P4

Block		Form Length	Average of	Length of	Form Area	Bulkhead	Total of each
		(m)	(m)	(m)	(m²) <u> </u>	(m²)	(m²)
	9	17.298					
8	8	16.580	16.939	3.000	50.817		50.817
7	7	16.050	16.315	3.000	48.945		48.945
6	6	15.694	15.872	3.000	47.616		47.616
5	5	15,448	15.571	3.000	46.713		46.713
4	4	15.300	15.374	3.500	53.809		53.809
3	3	15.246	15.273	3.500	53,456		53.456
2	2	15.238	15.242	3,500	53.347		53.347
Total				22.500	354.703		354.703

(5) Supporting Part on center of Span

Block	Section	Form Length	Average of	Length of	Form Area	Bulkhead	Total of each
		(m)	(m)	(m)	(n²)	(m²)	(m²)
	2	15.238					
11	2	15.238	15.238	3.000	45.714	·	45.714
Total				3.000	45.714		45.714

(6) Cantilever part on P3 -> P4

		11101112-714					Tail Tail
Block	Section	Form Length	Average of	Length of	Form Area	Bulkhead	Total of each
		(m)	(m)	(m)	(m³)	(m²)	(m²)
	2	15,238					
2	3	15.246	15.242	3.000	45.726		45.726
3	4	15.300	15,273	3.000	45.819		45.819
4	5	15.448	15.374	3.000	46.122		46.122
5	6	15.694	15.571	3.000	46.713		46.713
6	7	16.050	15.872	3.500	55,552		55.552
7	8	16.580	16.315	3.500	57.103		57,103
8	9	17.298	16.939	3.500	59.287		59.287
Total				22.500	356.322		356.322

(7) Pier Head part on P4

Block	Section	Form Length	Average of	Length of	Form Area	Bulkhead	Total of each
		(m)	(m)	(m)	(m²)	(m³)	(m²)
	9	17.298					
9	10	18.238	17.768	3.000	53.304		53.304
	10	18.238	18.238	0.000			
10	10	18.238	18.238	3.000	54. <i>7</i> 14		54.714
	10	18.238	18.238	0.000			
9	9	17.298	17.768	3.000	53.304		53.304
Total				9.000	161.322		161.322

(8) Cantilever part on P4 -> P5

Block	Section	Form Length	Average of	Length of	Form Area	Bulkhead	Total of each
		(m)	(m)	(m)	(m²)	(m²)	(m)
	9	17.298					
8	8	16.580	16.939	3.000	50.817		50.817
7	7	16.050	16.315	3.000	48.945		48.945
6	6	15.694	15.872	3.000	47.616		47.616
. 5	5	15.448	15.571	3.000	46.713		46.713
4	4	15.300	15.374	3.500	53.809		53.809
3	3	15.246	15.273	3.500	53.456		53.456
2	2	15.238	15.242	3.500	53.347		53.347
Total				22,500	354.703		354.703

(9) Supporting Part on P5 Side

Block No.	Section No.	Form Length	Average of Form Length	Length of Block	Form Area	Bulkhead Cross Beam	Total of each Block	
			(m)	(m)	(m)	(m²)	(m²)	(m)
1	2	15.238						
٠.	1	15.238	15.238	9.450	143.999		143.999	
	Girder e	end			16.930		16.930	
Total				9.450	143.999		160.929	

Total

Item	Quantity	Remark
Cantilever part	1420.431	
Pier Head part	322,644	
Supporting Part	367.572	
Total	2110.647	

# 2. Internal Form

# 1) Total Quantity

Item	Main part	Diaphragm	Hunch	Anchorage	Total
Cantilever part	799.378	133.664		100.536	1033.578
Pier Head part	120.216	160.932	14.592		295.740
Supporting Part	164.791	0.000	4.520		169.311
Total	1084.385	294.596	19.112	100.536	1498.629

#### 2) Calculation of Girder Form

(1) Supporting Part on P2 Side

Block No.	Section No.	Form Length	Average of Form Length	Length of Block	Form Area	Bulkhead Cross Beam	Total of each Block
NO.	AV.	(m)	(m)	(m)	(m²)	(m²)	(m³)
1	3	8.664			·		
-	4	9,574	9.119	5.000	45.5 <del>9</del> 5		45.595
1	5	9,574	9.574	2.800	26.807		26.807
1							

(2) Cantilever part on P2 -> P3

Block	Section	Form Length	Average of	Length of	Form Area	Bulkhead	Total of each
		(m)	(m)	(m)	(m²)	(m²)	(m²)
	5	9.574					
2	6	8.932	9.253	3.500	32.386	•	32.386
3	7	8.586	8.759	3.500	30.657		30.657
4	8	8,574	8.580	3.500	30.030	(	30.030
5	9	8.650	8.612	3.000	25.836		25.836
6	10	8.856	8.753	3.000	26.259		26.259
7	11	9.026	8.941	3.000	26.823		26.823
8	12	9.512	9.269	3.000	27.807		27.807
	<u> </u>						ļ
Total	İ	<u> </u>		22.500	199.798		199.798

(3) Pier Head part on P3

\	rieau pai	the same of the sa	A C T	T (1 f	E A	. D. 11.1	Total of each
Block	Section	Form Length	Average or §	Length of	Form Area	Bulkhead	1
		(m)	(m)	(m)	(m²)	(m²)	(m²)
	12	9.512					
9	13	10.524	10.018	3.000	30.054		30.054
•							
10					·		
	13	10.524	10.524	0.000			
9	12	9.512	10.018	3.000	30.054		30.054
Total			}	6.000	60.108		60.108

(4) Cantilover part on P3-> P4

(4) Can	mever pa	irt on P3-> P4					
Block	Section	Form Length	Average of	Length of	Form Area	Bulkhead	Total of each
		(m)	(m)	(m)	(m²)	(m²)	(n²)
	12	9.512					
8	11	9.026	9.269	3,000	27.807	•	27.807
7	10	8.856	8.941	3.000	26.823		26.823
6	- 9	8.650	8. <b>75</b> 3	3.000	26.259		26.259
5	8	8.574	8.612	3.000	25.836		25.836
4	7	8.586	8.580	3.500	30.030		30.030
3	6	8.932	8.759	3,500	30.657		30.657
2	5	9.574	9.253	3.500	32.386		32,386
Total				22,500	199.798		199.798

(5) Supporting Part on center of Span

Block	Section	Form Length (m)	Average of (m)	Length of (m)	Form Area (n²)	Bulkhead (m²)	Total of each (m²)
	5	9.574					
11	5	9.574	9.574	3.000	28.722		28.722
Total				3.000	28.722		28.722

(6) Cantilever part on P3 -> P4

Block		Form Length	Average of	Length of	Form Area	Bulkhead	Total of each
		(m)	(m)	(m)	(m)	(m³)	(m²)
	5	9.574					
2	6	8.932	9.253	3.000	27.759		27.759
3	7	8.586	8.759	3,000	26.277		26.277
4	8	8.574	8.580	3.000	25.740		25.740
5	9	8.650	8.612	3.000	25.836		25.836
6	10	8.856	8.753	3.500	30.636		30.636
7	11	9.026	8.941	3.500	31.294		31.294
8	12	9.512	9.269	3.500	32.442		32.442
Total				22.500	199.984		199.984

(7) Pier Head part on P4

Block	Section	Form Length	Average of	Length of	Form Area	Bulkhead	Total of each
		(m)	(m)	(m)	(m²)	<u>(m²)</u>	(m²)
	12	9.512					
9	13	10.524	10.018	3.000	30.054		30.054
10			· · · · · · · · · · · · · · · · · · ·				
	13	10.524	10.524	0.000			
9	12	9.512	10.018	3.000	30.054		30.054
Total				6.000	60.108		60.108

(8) Cantilever part on P4 -> P5

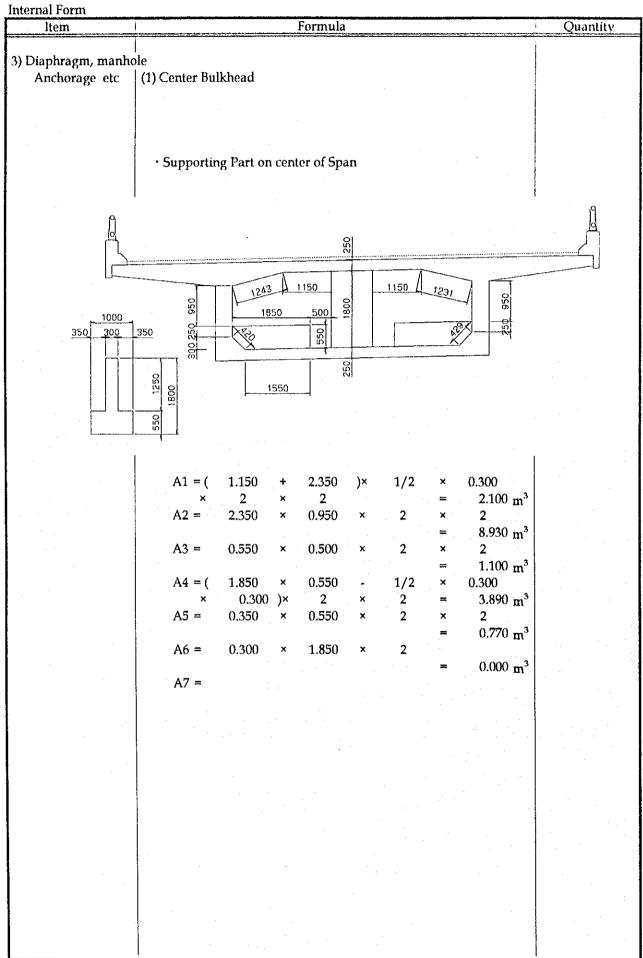
Block	Section	Form Length	Average of	Length of	Form Area	Bulkhead	Total of each
		(m)	(m)	(m)	(m²)	(㎡)	(m²)
	12	9.512					
8	11	9.026	9.269	3.000	27.807		27.807
7	10	8.856	8.941	3.000	26.823		26.823
6	9	8.650	8.753	3.000	26.259		26.259
5	8	8.574	8.612	3.000	25.836		25.836
. 4	-7	8.586	8.580	3.500	30.030		30.030
3	6	8.932	8. <i>7</i> 59	3.500	30.657		30.657
2	5	9.574	9.253	3.500	32.386		32,386
Total				22,500	199.798		199.798

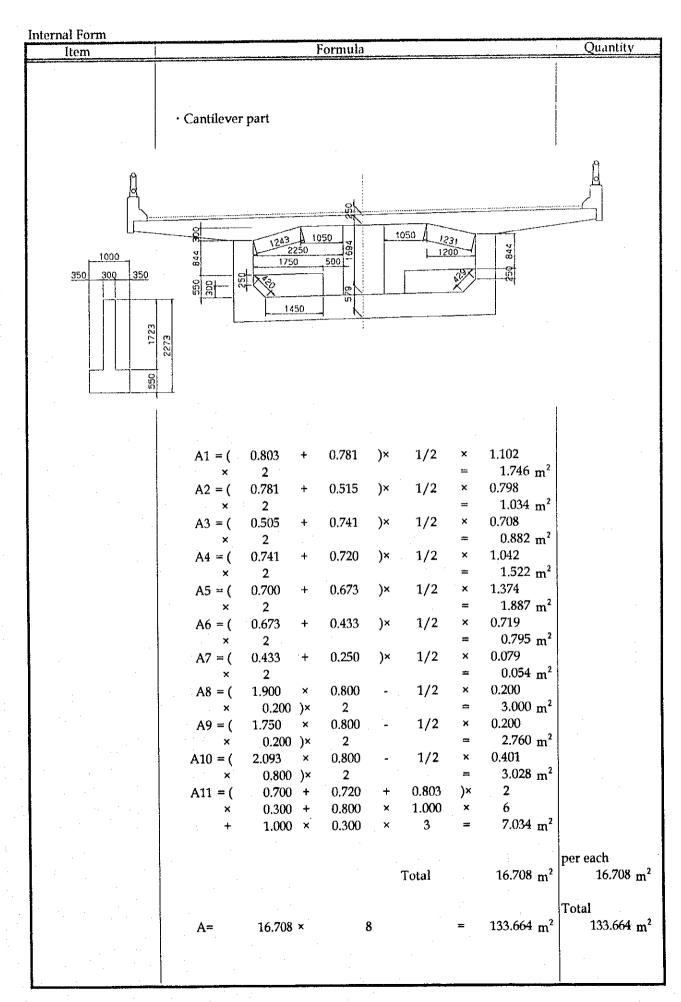
(9) Supporting Part on P5 Side

Block	Section	Form Length	Average of (m)	Length of (m)	Form Area (m²)	Bulkhead (m²)	Total of each
1	5	9.574					
	4	9.574	9,574 9,119	9.450 9.450	90.474 86.175		90.474 86.175
Total	3	8.664	7,117	9.450	90.474		90.474

Total

ltem	Quantity	Remark
Cantilever part	799.378	
Pier Head part	120.216	
Supporting Part	164.791	
Total	1084.385	





Internal Form

Internal Form Item	Formula	Quantity
	(3) Supporting Part on Side of Span (Manhole Haunch etc)  · Haunch (Top Slab)	
	08911 08911 08911	
	$L = ( 0.15^{2} \times 0.500^{2} )^{1/2} = 0.522 \text{ m}$ $A = 0.522 \times 4.330 = 0.522 \text{ m}$	per each 2.260 m <sup>2</sup>
	$A = 2.260 \times 2 = 4.52 \text{ m}^2$	Total 4.520 m <sup>2</sup>
	$L = ( 0.15^{2} \times 0.450^{2} )^{1/2} = 0.474 \text{ m}$ $A = 0.474 \times 3.848 = 0.474 \text{ m}$	per each 1.824 m <sup>2</sup>
	$A = 1.824 \times 8 = 14.592 \text{ m}^2$	Total 14.592 m <sup>2</sup>

nternal Form Item			Formula			Quantity
	(4)Anchorage	•	SCALE 1:50	A		
	1-4-1-2H		2347 1872	418 57 ±	325	
		946 730 108 5 400 165	140165 H1 325	546 108 330 108 65165	190165 H1 325	
	• TYPE-A1 A1 = ( A2 = (	0.33 + 0.418 + 2	0.546 )× 0.475 )×	1/2 × = 1/2 × =	0.33 0.145 m <sup>2</sup> 0.342 0.305 m <sup>2</sup>	
	A3 =	1.872 ×	1/2 ×	$\begin{array}{cc} 0.342 & = \\ \Sigma A & =  \end{array}$	$0.32 \text{ m}^2$ $0.77 \text{ m}^2$	per each 0.770 m <sup>3</sup>
	• TYPE-A2 A1 = (  A2 = (	0.73 + 0.418 + 2 1.872 ×	0.946 )× 0.475 )× 1/2 ×	1/2 × = 1/2 × = 0.342 = Σ A =	0.33 0.277 m <sup>2</sup> 0.342 0.305 m <sup>2</sup> 0.32 m <sup>2</sup>	per each
	Block		ige Type	Concrete	0.902 m <sup>2</sup>	0.902 m
	No. 1 3	A1 .(0.770 m <sup>3</sup> ) 4 4	A2.(0.902 m²)	3.080 3.080		
	5 6 7	4 4	4	3.608 3.080 3.080		
	8 Sub-Total	20	8	6.688		
4.2	Out 10th	Total	22.616 ×	2 =	45.232	45.232 m

Block	I A	oe :	Concrete		
No.	B1(0.595m3)	B2(0.734m3)	B3(0.868m3)	$m^3$	
3	j 4	4		5.316	
4	4		4	5.852	
5	4	4		5.316	
6	4		4	5.852	
7	4	4		5.316	
8		4		2.936	
9	4		· .	2.380	
Sub-Total	20	16	8	27.652	
		· .			
	Total	27.652 ×	2 =	55.304	

 $55.304 \text{ m}^3$ 

# 4. Reinforcement Bar

# 4-1. Total Quantity

	- D14	D16-D25	Total
Segment 1	8930.4	42387.2	51317.6
Segment 2	10865.2	46529.2	57394.4
Segment 3	26499.2	91856.8	118356.0
Segment 4	11584.8	46949.6	58534.4
Segment 5	1394.2	6572.6	7966.8
Anchor A-1		5668.8	5668.8
Anchor A-2		3885.6	3885.6
Anchor B-1		2595.0	2595.0
Anchor B-2		2880.0	2880.0
Anchor B-3		1906.8	1906.8
Deviator 1		4742.0	4742.0
Deviator 2	1072.0	1163.6	2235.6
Deviator 3	1072.0	1264.0	2336.0
Deviator 4	1072.0	1160.0	2232.0
Deviator 5	1520.4	4905.6	6426.0
Total	64010.2	264466.8	328477.0

S	IGN	DIACETER	LENGTH	NOS.	UNIT WEIGHT	PIECE WEIGHT [O	TAL WEIGH	NOTE
S	1- 1	D16	10,887	65	1.578	17.200	1,118.0	
	1- 2		9,887	65	1.578	15.600	1,014.0	
	1- 3		8,938	4	1.578	14.100	56.4	
	1- 4		6,148	2	1.578	9.700	19.4	
	1- 5		9,892	4	1.578	15.600	62.4	
	3	D16	11,650	77	1.578	18.400	1,416.8	
	4	D16	6,500	77	1.578	10.300	793.1	
<b></b>	6- 1		3,050	77	1.578	4.810	370.4	(AVE)
	6- 2		3,050	77	1.578	4.810	370.4	(AVE)
	7	D16	2,100	154	1.578		509.7	<del></del>
So.	1	D14	640	66	1.208	0.773	51.0	<u> </u>
30.	2	D14	805	114	1.208	0.973	110.9	(AVE)
C-	1	D22	5,480	154	2.984	16.400	2,525.6	<u> </u>
<u> </u>	2	D22	1,398	154	2.984	4.170	642.2	
Co.	1	D14	778	58		0,940	54.5	
-	4- 1	-+	10,802	12			157.2	
	4- 1		9,802	8			94.4	
	4- 2		5,457	4		<del> </del>	26.4	
L1	1- 1		10,442		·	<del></del>	627.0	(AVE)
<u> </u>	1- 1		10,442	38		<del></del>	623.2	
			6,334	. 18	ļ	<del> </del>	180.0	<u> </u>
<u> </u>			7,540	. 10	<del> </del>	<del>} </del>		(AVE)
	1- 4		4,632	4	<del></del>	<del> </del>		(AVE)
ļ	1- 5		<del></del>	2	<del></del>	<del>+</del>	15.5	(/ ()
	1- 6		4,912		<del></del>	+ ·	58.3	
	1- 7		2,050			·	70.0	
	1- 8		11,090	61	ļ	<del></del>	508.1	
ļ	2- 1		6,890		<del></del>	<u> </u>	120.6	
ļ <u>.</u>	2- 2		3,120	<del></del>	1,208		508.1	
	3 1		6,890		<del></del>		120.6	
	3- 2		3,120		<del> </del>		148.6	
ļ	4- 1		1,596	<u>.                                    </u>	<del> </del>		150.2	
	4- 2		1,617		<del></del>	<del>                                     </del>		(AVE)
Lo-		D14	765	<del></del>			25.1	(ATC)
М	1	D14	2,600		ļ. —————	<del>\</del>	44.5	
	2	D14	2,300	+	<del></del>		15.4	
<u> </u>	3	D14	1,600	8	1.208	1.930	13.4	
							12,829	
<u> </u>						<del>                                     </del>	12,029	
$\vdash$		4	<del> </del>	<u> </u>	<u> </u>			
					<u> </u>			
_		<del>.  </del>		<b> </b>		Doo	3167.8	
<b> </b>		<u> </u>		-		D22		
				ļ	<del> </del>	D16	7429.0	
<b> </b>	·-i		1	<del> </del>		D14	2232.6	
<u> </u>				ļ		TOTAL	12,829.4	
	<del></del>			<u> </u>		<del> </del>	0.000	
			ļ	<del>-</del>		-D14	2,233	
<u></u>				ļ		D16-D25	10,597	
		1		<del></del>	-	<del>  _  </del>		T-4-1
					ļ	Total/one	nos	Total
L	· · ·			<u> </u>	-D14	2232.600	4	8,930.400
					D16-D25	10596.800	4	42,387.200
1		ļ	1			Total		51,317.600

^	_	$\overline{}$	~

S	IGN	DIACETER	LENGTH	NOS.	UNIT WEIGHT		TOTAL WEIGH:	
S-	1	D16	11,100	138	1.578	17.500		
	3	D16	11,650	85	1.578	18,400	1,564.0	
	4	D16	11,000	85	1.578	17.400	1,479.0	
So-	1	D14	640	56	1.208	0.773		
	2	D14	805	96	1.208	0,973	93.4	(AVE)
s	6- 1	D16	2,950	85	1.578	4.660	396.1	
	6- 2	D16	2,950	85	1.578	4.660	396.1	
	7	D14	2,000	170	1.208	2.420	411.4	
Ċ	1- 1	D22	5,484	58	2.984	16.400	951.2	(AVE)
	1- 2	D22	5,566	56	2.984	16,600	929.6	(AVE)
	1- 3	D22	5,716	56	2.984	17.100	957.6	(AVE)
	2- 1	D22	1,398	58	2.984	4.170	241.9	
	2- 2	D22	1,448	56	2.984	4.320	241.9	(AVE)
	2- 3	D22	1,498	56	2.984	4.470	250.3	
Co-	1	D14	838	64	1.208	1.010	64.6	(AVE)
C-	4	D14	8,426	24	1.208	10.200	244.8	
L-	1	D16	11,000	104	1.578	17.400	1,809.6	
-	2	D14	6,890	85	1.208	8.330	708,1	
1	3	D14	6,890	85	1.208	8.330	708.1	
	4 1	D14	1,790	85	1.208		183.6	(AVE)
	4- 2	D14	1,853	85	1,208	2.240	190.4	(AVE)
Lo-	1	D14	645	88	1,208	0.779	68.6	(AVE)
	<del>- i</del>							<u> </u>
		!	i i					
<b> </b>				<del></del>				
<b> </b>						D22	3572.5	
		<u>!</u>	<u> </u>			D16	8059.8	
-						D14	2716.3	
1						TOTAL	14,348.6	
<b> </b>								
<b> </b>	······································					-D14	2,716	
<u> </u>						D16-D25	11,632	
<b> </b>		<del> </del>	<del>                                     </del>					
<b>—</b>		<u> </u>	<u> </u>			Total/one	nos	Total
1		1			-D14	2716.300	4	10,865.20
<b> </b>					D16-D25	11632.300	4	46,529.20
1-		-	1			Total		57,394.40

C)	IGN	DIACETER	LENGTH	NOS.	UNIT WEIGHT	PIECE WEIGHT	TOTAL WEIGHT	NOTE
			12,500	136	1.578	19,700		
S	1- 1	4	12,487	4	1.578	19.700	78.8	
	1- 2	1	11,650	97	1.578	18.400	1,784.8	
<del></del>	3	D16	6,500	97	1.578	10.300	999.1	
	4	D16	640	56	1,208	0.773	43.3	
So-	1	D14	805	96	1.208	0.973		(AVE)
	2	D14		97	1.578	4.740	459.8	(,,,,,
<u>S</u>	6- 1	D16	3,000	97	1.578	4.740	459.8	(Δ\/F)
	6- 2	<del></del>	3,000		1.208	2.480	481.1	
	7	D14	2,050	194		18,200	1,019.2	
C	1- 1	<del></del>	6,092	56	2.984	20,000	600.0	
	1- 2		6,708	30	2.984			
	1- 3		7,382	36	2.984	22.000	792.0 250.3	(VAE)
	2- 1		1,498	56	2.984	4.470		/A\/E\
	2- 2	····	1,548	30	2,984	4.620	138.6	(VAE)
	2- 3		1,598	36	2.984	4.770	171.7	/A\/E\
Co-	1	D14	878	72	1.208	1.060		(AVE)
C	4- 1		12,500	20	1.208	15.100	302.0	
	4- 2		9,288	. 4	1.208	11,200	44.8	
	4- 3		3,618	4	1.208	4.370	17.5	
	4- 4		1,830	4	1.208	2.210		
L1	1- 1		12,500	102	1.578	19.700		
	1- 2		12,487	2	1.578	19.700		
	2	D14	6,890	. 97	1.208	8,330		
· .	3	D14	6,890	97	1.208	8.330	<del>                                     </del>	
	4- 1	D14	2,667	97	1.208	3.220	<del> </del>	
	4- 2	D14	2,073	97	1.208	2,510	<del></del>	(AVE)
Lo-	1	D14	690	88	1.208	0.834	73.4	(AVE)
<u> </u>								
						[		
						D22		
						D16	<del></del>	
						D14		
						TOTAL	14,794.5	
						-D14	3,312	
	٠					D16-D25	11,482	
		<del>-</del>				Total/one	nos	Total
					-D14	3312.400	8	26,499.2
-					D16-D25	11482.100	8	91,856.8

GN	DIACETER	LENGTH	NOS.	UNIT WEIGHT	PIECE WEIGHT	TOTAL WEIGHT	NOTE
0- 1	D14	640	63	1,208	0.773	48.7	
	D14	810	109	1,208	0.979	106.7	
	D16	8,900	90	1.578	14.000	1,260.0	
1- 2	D16	9,490	90	1.578	15.000		
3	D16	11,650	73	1.578	18.400		
4	D16	6,500	73	1.578	10,300		
6- 1	D16	3,060	73	1.578	4.830		
6- 2	D16	3,060	73	1.578			
7	D16	2,100	146	1.578			·
0- 1	D14	980	114	1,208			
1- 1	D22	8,170	96				
1- 2	D22	8,670					
2	D22	1,600					
4- 1	D14	8,900					
4- 2	D14	9,440					
4- 3	D14	6,440				<u> </u>	
0- 1	D14	1,190	33	1.208			
0- 2	D14	1,130	66	1.208	1,370		
1	D16	9,560	100	1.578	15.100		
2	D14	6,890	73	1.208		1	
. 3	D14	6,890	73	1.208			
4- 1	D14	2,610	50			·	
4- 2	D14	2,510	96	1.208	3.030	290.9	(AVE)
<del></del>		<u>'                                    </u>		! 			
					D22	4333.8	
					D16	7403.6	
					D14	2896.2	
					TOTAL.	14,633.6	
							· · · · · · · · · · · · · · · · · · ·
					<del></del>		
	-		· · · · · · · · · · · · · · · · · · ·		D16-D25	11,737	
	-				Total/one	nos	Total
·	-	<u>                                     </u>		-D14	<del> </del>	4	11,584.800
		<del> </del> -		D16-D25	11737.400	4	46,949.600
	1- 1 1- 2 3 4 6- 1 6- 2 7 0- 1 1- 1 1- 2 2 4- 1 4- 2 4- 3 0- 1 0- 2 1 2 3 4- 1	0- 2 D14 1- 1 D16 1- 2 D16 3 D16 4 D16 6- 1 D16 6- 2 D16 7 D16 0- 1 D14 1- 1 D22 1- 2 D22 2 D22 4- 1 D14 4- 3 D14 4- 3 D14 0- 1 D14 0- 2 D14 1 D16 2 D14 3 D14 4- 1 D16	0- 2         D14         810           1- 1         D16         8,900           1- 2         D16         9,490           3         D16         11,650           4         D16         6,500           6- 1         D16         3,060           6- 2         D16         3,060           7         D16         2,100           0- 1         D14         980           1- 1         D22         8,670           2         D22         1,600           4- 1         D14         8,900           4- 2         D14         9,440           4- 3         D14         6,440           0- 1         D14         1,190           0- 2         D14         1,130           1         D16         9,560           2         D14         6,890           3         D14         6,890           4- 1         D14         2,610	0- 2         D14         810         109           1- 1         D16         8,900         90           1- 2         D16         9,490         90           3         D16         11,650         73           4         D16         6,500         73           6- 1         D16         3,060         73           6- 2         D16         3,060         73           7         D16         2,100         146           0- 1         D14         980         114           1- 1         D22         8,170         96           1- 2         D22         8,670         50           2         D22         1,600         146           4- 1         D14         8,900         32           4- 2         D14         9,440         32           4- 3         D14         6,440         12           0- 1         D14         1,190         33           0- 2         D14         1,130         66           1         D16         9,560         100           2         D14         6,890         73           3         D14         6,89	0- 2         D14         810         109         1.208           1- 1         D16         8,900         90         1.578           1- 2         D16         9,490         90         1.578           3         D16         11,650         73         1.578           4         D16         6,500         73         1.578           6- 1         D16         3,060         73         1.578           6- 2         D16         3,060         73         1.578           7         D16         2,100         146         1.578           0- 1         D14         980         114         1.208           1- 1         D22         8,170         96         2.984           2         D22         1,600         146         2.984           4- 1         D14         8,900         32         1.208           4- 2         D14         9,440         32         1.208           4- 3         D14         6,440         12         1.208           0- 1         D14         1,190         33         1.208           0- 2         D14         1,130         66         1.208 <tr< td=""><td>0- 2         D14         810         109         1.208         0.979           1- 1         D16         8,900         90         1.578         14,000           1- 2         D16         9,490         90         1.578         15,000           3         D16         11,650         73         1.578         10,300           4         D16         6,500         73         1.578         4.630           6- 1         D16         3,060         73         1.578         4.630           6- 2         D16         3,060         73         1.578         4.830           7         D16         2,100         146         1.578         3.310           0- 1         D14         980         114         1.208         1.180           1- 1         D22         8,170         96         2.984         24.400           1- 2         D22         1,600         146         2.984         25.900           2         D22         1,600         146         2.984         4.770           4- 1         D14         8,900         32         1.208         11.400           4- 2         D14         1,130         <t< td=""><td>0- 2         D14         810         109         1.208         0.979         106.7           1- 1         D16         8,900         90         1.578         14.000         1,260.0           1- 2         D16         9,490         90         1.578         15.000         1,350.0           3         D16         11,650         73         1.578         18.400         1,943.2           4         D16         6,500         73         1.578         10.300         751.9           6- 1         D16         3,060         73         1.578         4.830         352.6           6- 2         D16         3,060         73         1.578         4.830         352.6           7         D16         2,100         146         1.578         3.310         483.3           0- 1         D14         980         114         1.208         1.180         134.5           1- 2         D22         8,670         50         2.994         24.400         2,342.4           1- 2         D22         1,600         146         2.984         4.770         696.4           4- 1         D14         8,900         32         1.208</td></t<></td></tr<>	0- 2         D14         810         109         1.208         0.979           1- 1         D16         8,900         90         1.578         14,000           1- 2         D16         9,490         90         1.578         15,000           3         D16         11,650         73         1.578         10,300           4         D16         6,500         73         1.578         4.630           6- 1         D16         3,060         73         1.578         4.630           6- 2         D16         3,060         73         1.578         4.830           7         D16         2,100         146         1.578         3.310           0- 1         D14         980         114         1.208         1.180           1- 1         D22         8,170         96         2.984         24.400           1- 2         D22         1,600         146         2.984         25.900           2         D22         1,600         146         2.984         4.770           4- 1         D14         8,900         32         1.208         11.400           4- 2         D14         1,130 <t< td=""><td>0- 2         D14         810         109         1.208         0.979         106.7           1- 1         D16         8,900         90         1.578         14.000         1,260.0           1- 2         D16         9,490         90         1.578         15.000         1,350.0           3         D16         11,650         73         1.578         18.400         1,943.2           4         D16         6,500         73         1.578         10.300         751.9           6- 1         D16         3,060         73         1.578         4.830         352.6           6- 2         D16         3,060         73         1.578         4.830         352.6           7         D16         2,100         146         1.578         3.310         483.3           0- 1         D14         980         114         1.208         1.180         134.5           1- 2         D22         8,670         50         2.994         24.400         2,342.4           1- 2         D22         1,600         146         2.984         4.770         696.4           4- 1         D14         8,900         32         1.208</td></t<>	0- 2         D14         810         109         1.208         0.979         106.7           1- 1         D16         8,900         90         1.578         14.000         1,260.0           1- 2         D16         9,490         90         1.578         15.000         1,350.0           3         D16         11,650         73         1.578         18.400         1,943.2           4         D16         6,500         73         1.578         10.300         751.9           6- 1         D16         3,060         73         1.578         4.830         352.6           6- 2         D16         3,060         73         1.578         4.830         352.6           7         D16         2,100         146         1.578         3.310         483.3           0- 1         D14         980         114         1.208         1.180         134.5           1- 2         D22         8,670         50         2.994         24.400         2,342.4           1- 2         D22         1,600         146         2.984         4.770         696.4           4- 1         D14         8,900         32         1.208

S	IGN		DIACETER	LENGTH	NOS.	UNIT WEIGHT		TOTAL WEIGH	
S-	0-	1	D14	640	21	1.208	0.773		
	0-	2	D14	810	66	1.208	0.979		AVE
	1-	1	D16	2,900	67	1.578	4.580	306.9	
	1-	2	D16	1,410	67	1.578	2.230	149.4	
	3		D16	11,650	25	1.578	18,400	460.0	
	4		D16	6,500	25	1.578	10.300	257.5	
	6-	1	D16	2,950	25	1.578	4.660	116.5	
	6-	2	D16	2,950	25	1.578	4.660	116.5	
	7		D16	2,000	50	1.578	3.160	158.0	
С	0-	1	D14	780	25	1.208	0.943	23.6	
	4-	1	D14	3,000	12	1.208	3.630	43.6	
	4-	2	D14	1,410	12	1.208	1.700	20.4	·
	1		D22	5,470	50	2.984	16.300	815.0	
	- 2		D22	1,400	50	2,984	4.180	209.0	
L	0-	1	D14	650	18	1.208	0.785	14.1	
	1-	1	D22	3,000	53	2.984	8,950	474.4	
	1-	2	D22	1,410	53	2.984	4.210	223.1	
	2		D14	6,890	25	1.208	8.330	208.3	
	. 3		D14	6,890	25	1.208	8.330	208.3	
	4		D14	1,620	50	1.208	1,960	98.0	
			. ,						
						<u> </u>			
							D22		
							D16		·
							D14	·	
							TOTAL	3,983.4	
							-D14	697	
							D16-D25	3,286	
				·					
							Total/one	nos	Total
						-D14	697.100	2	1,394.200
						D16-D25	3286.300	2	6,572.600

## FOR ONE ANCHOR BLOCK TYPE A

LIST	OF	REIN	NFORCEMENT

Г	SIGN	DIACETER	LENGTH	NOS.	UNIT WEIGHT	PIECE WEIGHT [	OTAL WEIGHT	NOTE
T	1	D16	2,012	12	1.578	3.180	38.2	
<b> </b>	2	D16	1,606	28	1.578	2.530	70.8	
	3	D16	2,490	4	1.578	3.930	15.7	
	4	D16	3,000	12	1.578	4.740	56.9	
	5	D16	1,106	26	1.578	1.750	45.5	
┢	6	D16	1,446	4	1.578	2.280	9.1	·
	<del></del>							
		-				-		
				·				
						D16	236.2	
	· · · · · · · · · · · · · · · · · · ·				Total/one	nos	Total	
<u> </u>					236.2	24	5,668.8	

#### LIST OF REINFORCEMENT

#### FOR TWO ANCHOR BLOCK TYPE A

SIGN	DIACETER	LENGTH	NOS.	UNIT WEIGHT	PIECE WEIGHT	TOTAL WEIGHT	NOTE
1 -1	D16	2,004	12	1.578	3.160	37.9	
1 -2	D16	1,979	12	1.578	3,120	37.4	
2 -1	D16	1,551	28	1.578	2.450	68.6	
2 -2	D16	1,526	28	1.578	2.410	67.5	
3	D16	2,490	8	1.578	3.930	31.4	
4	D16	3,000	20	1,578	4.740	94.8	
5	D16	1,506	26	1.578	2.380	61.9	
6	D16	1,133	8	1.578	1.790	14.3	
7	D16	2,500	6	1.578	3.950	23.7	
8	D16	2,177	14	1.578	3.440	48.2	
· · · · · · · · · · · · · · · · · · ·							
					D16	485.7	
				·			
				Total/one	nos	Total	
				485.7	8	3,885.6	

#### FOR ONE ANCHOR BLOCK TYPE B

SIGN	DIACETER	LENGTH	NOS.	UNIT WEIGHT	PIECE WEIGHT	TOTAL WEIGHT	NOTE
T 1	D16	2,257	6	1.578	3.560	21.4	
2	D16	1,907	14	1.578	3.010	42.1	
3	D16	2,490	4	1.578	3.930	15.7	
4	D16	3,000	6	1.578	4.740	28.4	
5	D16	1,770	26	1.578	2.790		
6	D16	1,303	4	1.578	2.060	8.2	
7	D16	2,258	20	1.578	3.560	71.2	
· <u>····</u>							
						i	
				· ·			
					D16	259.5	
		·					
				Tota/one	nos	Total	
				259.5	10	2,595.0	
,	1						

#### LIST OF REINFORCEMENT

## FOR TWO ANCHOR BLOCK TYPE B

JOI OF N	LIM ONO	L(VI)( V )					
SIGN	DIACETER	LENGTH	NOS.	UNIT WEIGHT	PIECE WEIGHT	TOTAL WEIGHT	NOTE
T 1	D16	2,257	12	1.578	3.560	42.7	
2	D16	1,907	28	1.578	3.010	84.3	
3	D16	2,490	7	1.578	3,930	27.5	
4	D16	3,000	10	1.578	4.740	47.4	
5	D16	1,770	26	1.578	2.790	72.5	
. 6	D16	1,303	7	1.578	2.060	14,4	
7	D16	2,258	20	1.578	3.560	71.2	
							·
					D16	360.0	
				Total/one	nos	Total	
				360.0	8	2,880.0	
	<del></del>	<del>                                     </del>					,

## FOR THREE ANCHOR BLOCK TYPE B

# LIST OF REINFORCEMENT

SIGN	DIACETER	LENGTH	NOS.	UNIT WEIGHT	PIECE WEIGHT	TOTAL WEIGHT	NOTE
T 1	D16	2,156	18	1.578	3.400	61.2	
2	D16	1,807	42	1.578	2.850	119.7	
3	D16	2,490	10	1.578	3.930	39.3	
4	D16	3,000	14	1.578	4.740	66.4	
5	D16	2,170	26	1.578	3.420	88.9	
6	D16	1,253	10	1.578	1.980	19.8	
7	D16	2,578	20	1,578	4.070	81.4	
					D16	476.7	
				Total/one	nos	Total	
				476.7		1,906.8	

SIG	N	DIACETER	LENGTH	NOS.	UNIT WEIGHT	PIECE WEIGHT	OTAL WEIGHT	NOTE
D	1	D16	5,960	54	1.578	9.410	508.1	
	2	D16	1,560	54	1.578	2.460	132.8	
	3	D16	6,970	30	1.578	11.000	330.0	
	4	D16	2,110	26	1.578	3.330	86.6	
	5	D16	3,000	27	1.578	4.740	128.0	····
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						D16	1185.5	
					,		· <u> </u>	
					Total/one	nos	Total	
				D16-D25	1,185.5	4	4,742.0	

# LIST OF REINFORCEMENT

SIGN	DIACETER	LENGTH	NOS.	UNIT WEIGHT	PIECE WEIGHT	TOTAL WEIGHT	NOTE
D 1-	a D16	4,110	8	1.578	6.490	51.9	
1 -	D16	2,710	- 8	1,578	4,280	34.2	
. 2	D16	3,330	8	1.578	5,260	42.1	
- 3	D16	2,060	24	1.578	3.250	78.0	
4	D16	1,600	20	1.578	2.530	50.6	
5	D16	900	24	1.578	1.420	34.1	
6	D14	2,900	32	1.208	3.500	112.0	
7	D14	2,690	48	1.208	3.250	156.0	
	<u> </u>		-				
					D14	268.0	
					D16	290,9	
					TOTAL	558,9	
					Total/one	nos	Total
				-D14	268.000	4	1,072.000
				D16-D25	290.900	4	1,163.600
					Total		2,235.600

SI	GN	DIACETER	LENGTH	NOS,	UNIT WEIGHT	PIECE WEIGHT	FOTAL WEIGHT	NOTE
D	1 -a	D16	4,740	8	1.578	7.480	59.8	
	1 -b	D16	2,170	8	1.578	3.420	27.4	
	2	D16	3,330	8	1.578	5.260	42.1	
	3	D16	2,690	24	1.578	4.250	102.0	:
	4	D16	1,600	20	1.578	2,530	50.6	
	5	D16	900	24	1.578	1.420	34.1	
	6	D14	2,900	32	1.208	3.500	112.0	
	7	D14	2,690	48	1.208	3.250	156.0	
								<u> </u>
					·			
						D14	268.0	
						D16	316.0	
						TOTAL	584.0	
						Total/one	nos	Total
					-D14	268.000	4	1,072.000
					D16-D25	316,000	4	1,264.000
						Total		2,336.000

#### LIST OF REINFORCEMENT

SI	GN	DIACETER	LENGTH	NOS.	UNIT WEIGHT	PIECE WEIGHT	TOTAL WEIGHT	NOTE
D	1 -a	D16	4,110	. 8	1.578	6.490	51.9	
	1 -b	D16	2,710	8	1.578	4.280	34.2	
	2	D16	3,260	8	1.578	5.150	41.2	
	3	D16	2,060	24	1.578	3.250	78.0	
	4	D16	1,600	20	1.578	2.530	50.6	
	5	D16	900	24	1.578	1.420	34.1	
	6	D14	2,900	32	1.208	3,500	112.0	
	7	D14	2,690	48	1.208	3.250	156.0	
							1	
								· .
						D14	268.0	
						D16	290.0	
						TOTAL	558.0	
						Total/one	nos	Total
					-D14	268,000	4	1,072.000
					D16-D25	290,000	4	1,160.000
				•		Total		2,232,000

LIST OF F	REINFORC	EMENT	<u> </u>				5
SIGN	DIACETER	LENGTH	NOS.	UNIT WEIGHT	PIECE WEIGHT	TOTAL WEIGHT	NOTE
D 1	D16	9,860	24	1.578	15.600	374.4	
2	D16	4,900	4	1.578	7.730	30.9	
3	D16	4,660	10	1.578	7.360	73.6	
4	D16	2,460	34	1.578	3,880	131.9	
5	D16	6,070	13	1.578	9,580	124.5	
6	D16	6,960	14	1.578	11.000	154.0	
7	D16	3,210	- 12	1.578	5.070	60.8	
8	D16	3,460	26	1.578	5.460	142.0	
. 9	D16	2,500	34	1.578	3.950	134.3	
10	D14	2,750	52	1.208	3.320	172.6	
- 11	D14	1,800	52	1.208	2.180	113.4	
12	D14	1,500	52	1.208	1.810	94.1	
							<b></b>
			•		D14	380.1	
					D14		
<u></u>	_				TOTAL	1,606.5	
					TOTAL	1,000.5	
					Total/one	nos	Total
<u> </u>				-D14	380,100	. 4	1,520.400
				D16-D25	1226.400	4	4,905.600
<b></b>	<del></del>				Total		6,426.000

## 4. PC Steel

# 1) Longitudinal Prestressing Internal Tendons

# 12S12.7B(SWPR7B)

Unit Weight:

9.288 kgf/m

(1) Tendons on Top Slab

PC Steel	Length of (m)	Unit (kgf)	Each	Weight of (kgf)	Remarks
S101	50.419	468.3	4	1873.2	
S102	46.925	435.8	4	1743.2	
<b>S</b> 103	39.863	370.2	4	1480.8	
S104	39.953	371.1	4	1484.4	
S105	42.935	398.8	4	1595.2	
S106	32.848	305.1	4	1220.4	
S107	32.575	302.6	4	1210.4	
S108	27.348	254.0	4	1016	
S109	26.586	246.9	4	987.6	
S110	26.220	243.5	4	974	
S111	20.804	193.2	4	772.8	
S112	19.848	184.3	4	737.2	
S113	14.754	137.0	- 4	548	
S114	11.763	109.3	4	437.2	
S115	8.700	80.8	4	323.2	
Total	441.541		60	16403.6	

(2) Tendons on Top Slab

PC Steel	Length of (m)	Unit (kgf)	Each	Weight of (kgf)	Remarks
5201	50.419	468.3	4	1873.2	
S202	46.925	435.8	4	1743.2	
S203	39.863	370.2	4	1480.8	
S204	39.953	371.1	4	1484.4	
S205	42.935	398.8	4	1595.2	
S206	32.848	305.1	4	1220.4	
S207	32.575	302.6	4	1210.4	
S208	27.348	254.0	4	1016	
S209	26.586	246.9	4	987.6	• .
S210	26.220	243.5	4	974	
S211	20.804	193.2	4	772.8	
S212	19.848	184.3	4	737.2	
S213	14.754	137.0	4	548	
5214	11.763	109.3	4	437.2	
S215	8.700	80.8	. 4	323.2	
Total	441.541		60	16403.6	

(3) Tendons on Bottom Slab

	Length of (m)	Unit (kgf)	Each	Weight of (kgf)	Remarks
5301	30.828	286.3	4	1145.2	
5302	30.828	286.3	4	1145.2	
S303	25.206	234.1	4	936.4	
Total	86.862		12	3226.8	

(4) Tendons on Bottom Slab

PC Steel	Length of (m)	Unit (kgf)	Each	Weight of (kgf)	Remarks
S401	46.036	427.6	4	1710.4	
S402	46.036	427.6	4	1710.4	
S403	46.036	427.6	4	1710.4	
S404	46.036	427.6	4	1710.4	
Total	184.144		16	6841.6	

(5) Tendons on Bottom Slab

PC Steel	Length of (m)	Unit (kgf)	Each	Weight of (kgf)	Remarks
S501	30.828	286.3	4	1145.2	
S502	30.828	286.3	4	1145.2	
S503	25,206	234,1	4	936.4	
Total	86.862		12	3226.8	

Sub-Total Weight of PC Cabl	e Wp =	(kgf) 68840	(ton) 68.8
Tension Unit	Ns =	214 × 2	= 428
Theating	L =	4963.8	m ·
Cement grout Φ80/85	V =	25.0	m3

#### 2) Longitudinal Prestressing External Tendons

PC Steel	Length of (m)	Unit (kgf)	Each	Weight of (kgf)	Remarks
C11	37.940	352.4	6	2114.4	
C12	37.974	352.7	6	2116.2	
C13	37.974	352.7	6	2116.2	
C21	60.099	558.2	6	3349.2	
C22	60.099	558.2	6	3349.2	
C23	60.099	558.2	6	3349.2	·
C31	37.940	352.4	6	2114.4	
C32	37.940	352.4	6	2114.4	
C33	37.940	352.4	6	2114.4	
Total	408.005		54	22737.6	

Sub-Total Weight of PC Cable Wp = 22737.6 22.7 Tension Unit Ns = 54 × 2 = 108Theating L =2448.03 m Cement grout Φ90/100 15.6 m3(kgf) (ton)

#### 3) Transverse Prestressing Tendons (Top Slab)

#### 3S12.7

Unit Weight:

2.322 kgf/m

PC	Length of (m)	Unit (kgf)	Each	Weight of (kgf)	Remarks
3.0m Segment	11.452	26.6	48	1276.8	
3.5m Segment	11.452	26.6	49	1303.4	
Closure Segment	11.452	26.6	3	79.8	
Pier Segment	11.452	26.6	18	478.8	
End Segment	11.452	26.6	18	478.8	
Total	57.260		136	3617.6	

Sub-Total Weight of PC Cable Wp = 
$$\begin{pmatrix} (kgf) & (ton) \\ 3617.6 & 3.6 \end{pmatrix}$$
  
Tension Unit Ns =  $\begin{pmatrix} 136 \times 2 & = 272 \\ 1557.472 & m \end{pmatrix}$ 

Item	Formula	Quantity
5. Accessories		
1) Expansion	oint	
	EACH LENGTH L = 10.750	
	L = 10.750	
·	EACH = 2	
	TOTAL LENGTH	
	$L = 10.750 \times 2$	21.500 m
2) BEARING	PAELASTOMERIC 660*560*125	
	$n = 2 \times 2 = $	4 nos
	1400*1500*214	
	$n = 2 \times 2$	4 nos
3) ANCHOR	BAR	·
	Φ75 L=1250 (MOVE)	
	$EACH = 4 \times 2 = $	8 nos
	Φ75 L=2500 (FIX)	
	$EACH = 4 \times 2 =$	8 nos
4) ANCHOR	CAP (SGP)	
	Φ80 L=1350 (MOVE)	
	$EACH = 4 \times 2$	8 nos
	Φ 150 L=850 (FIX)	
	$EACH = 4 \times 2$	8 nos

Item		Formula	Quantity
9 Pavement		L = 129.00 m	
	1)	Pavement t=75mm	
		$A = 10.750 \times 129.000 = 1,386.750 \text{ m}^3$	1,386.8 m²
	2)	Water Proofing t=5mm	
		$A = 10.750 \times 10.750 = 115.563 \text{ m}$	115.6 <b>m</b> i
	3)	Concrete	
		870 340 50 336 100 336 50 Ø D16 120 120 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9	
i .		<u>O</u> D16	
		per one side	
·		A1 = 0.336 × 0.150 - 0.12 × 0.046 = 0.045	
		$A2 = 0.080 \times 0.050 = 0.004$ Total 0.049 m <sup>2</sup>	
		$V = 0.049 \times 10.750 \times 2 = 1.054 \text{ m}^3$	1.054 m³
	2)	Road marking	
	ŕ	Bridge Length L= 129.000 = 129.000 m	·
		Side Line	
		$A1 = 129.000 \times 0.100 \times 4 = 51.600 \text{ m}^2$	a · · · · · · · · · · · · · · · · · · ·
		L = 129.000 / 5.000 = 25.8 m	
		Center Line	·
		$A2 = 2.500 \times 0.100 \times 25.8 = 6.450 \text{ m}^2$	
		Total 51.600 + 6.450 = 58.050 m <sup>2</sup>	58.1 <b>㎡</b>