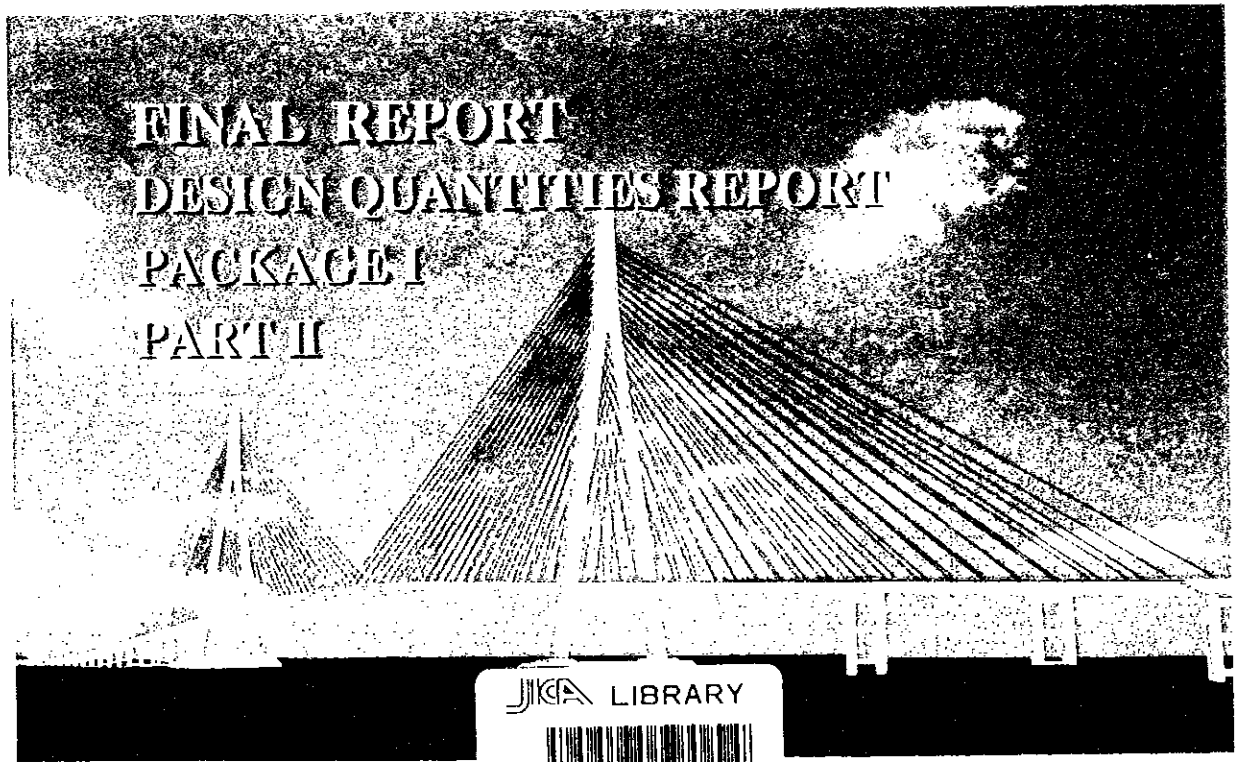


JAPAN INTERNATIONAL COOPERATION AGENCY (JICA)
MINISTRY OF TRANSPORT
SOCIALIST REPUBLIC OF VIET NAM

THE DETAILED DESIGN
ON
THE CAN THO BRIDGE CONSTRUCTION
IN
SOCIALIST REPUBLIC OF VIET NAM



FINAL REPORT
DESIGN QUANTITIES REPORT
PACKAGE I
PART II

JICA LIBRARY
J 1161253 (8)

OCTOBER 2000

0-43

APPROVED BY THE DIRECTOR GENERAL

3
5
F
ARY

**JAPAN INTERNATIONAL COOPERATION AGENCY (JICA)
MINISTRY OF TRANSPORT
SOCIALIST REPUBLIC OF VIET NAM**

**THE DETAILED DESIGN
ON
THE CAN THO BRIDGE CONSTRUCTION
IN
SOCIALIST REPUBLIC OF VIET NAM**

**FINAL REPORT
DESIGN QUANTITIES REPORT
PACKAGE I
PART II**

OCTOBER 2000

NIPPON KOEI CO., LTD.



1161253【8】

Table of Contents

Package I

Part - 2

3.	Bridges	
3.1.	Large Tra Va Bridge	
	Superstructure	3-1-1
	Substructure (Abutments)	3-1-21
	Substructure (Piers)	3-1-35
	Miscellaneous	3-1-56
3.2	Small Tra Va Bridge	
	Superstructure	3-2-1
	Substructure (Abutments)	3-2-30
	Substructure (Piers)	3-2-42
3.2.	Tra On Bridge	
	Superstructure (I-Girder)	3-3-1
	Superstructure (Box Girder)	3-3-33
	Substructure (Abutments)	3-3-76
	Substructure (Piers)	3-3-88
	Miscellaneous	3-3-106
3.3	Interchange Bridge with NH.54	
	Superstructure	3-4-1
	Substructure (Abutments)	3-4-11
	Substructure (Piers)	3-4-21

Notes

1. General

Unless otherwise noted these notes are applied to all design quantities.

2. Concrete

Concrete strengths are specified as followings base on 28 days cylinder strength.

Concrete class	Strength	Typical use
Concrete class B	40MPa	Cast in place PC box girder, Precast PC I girder
Concrete class C	35MPa	Hollow slab
Concrete class D	30MPa	Cast in place bored concrete pile, Deck slab, Diaphragm, Precast RC panel
Concrete class E	24MPa	Abutment, Pier, Pile cap, Footing
Concrete class G	15MPa	Lean concrete, Plain concrete

3. Reinforcement

Reinforcements are specified as SD345.

4. Prestressing

Prestressed tendons are specified as SWPR7B.

5. Anchor Bar

Anchor bars are specified as SS400.

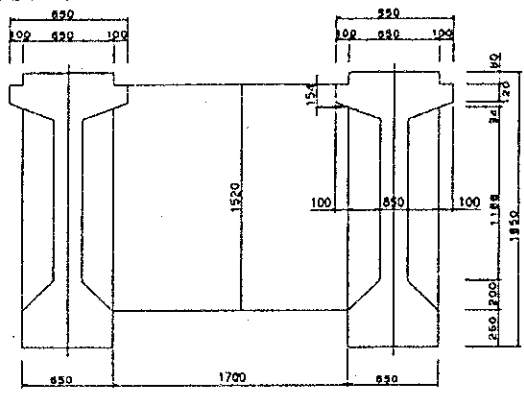
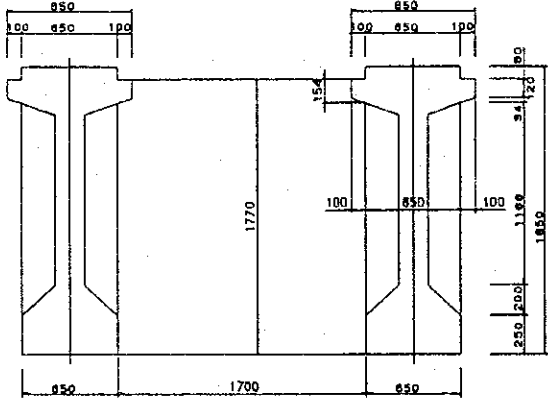
3. Bridges

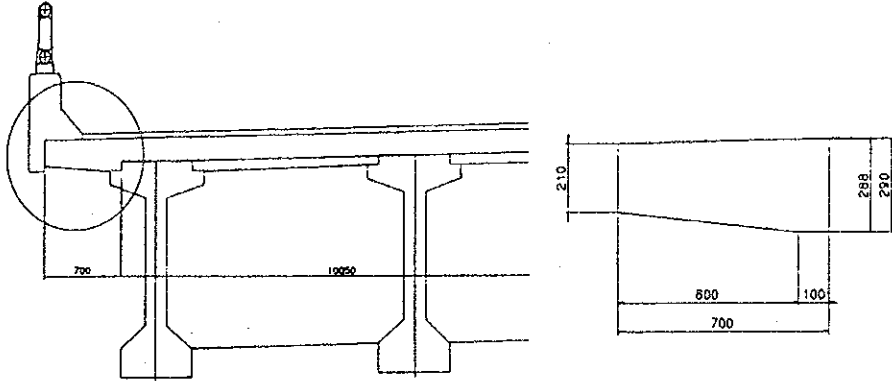
3.1. Large Tra Va Bridge

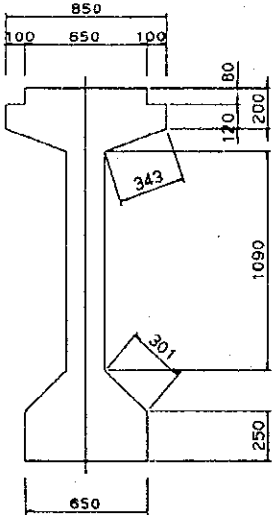
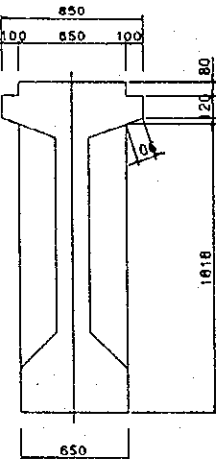
1.Quantity of Superstructure

		Superstructure		Unit	Quantity	Remarks
Item		Work Item				
Concrete	CLASS B	Girder		cu.m	2172.2	$\sigma_{ck}=40\text{Mpa}$
	CLASS D-1	Deck Slab		cu.m	1389.0	$\sigma_{ck}=30\text{Mpa}$
		Diaphragm		cu.m	503.3	
		Total		cu.m	1892.3	
Form		Deck Slab		sq.m	1604.8	
		Girder		sq.m	12167.1	
		Diaphragm		sq.m	1580.6	
		Total		sq.m	15352.5	
Re-bar	Deck Slab	- D14		ton	42.2	
		D16 - D25		ton	134.0	
		Total		ton	176.1	
	Girder	- D14		ton	175.1	
		D16 - D25		ton	31.2	
		Total		ton	206.3	
	Diaphragm	- D14		ton	8.7	
		D16 - D25		ton	13.8	
		Total		ton	22.5	
	Total	- D14		ton	226.0	
		D16 - D25		ton	179.0	
		Total		ton	405.0	
PC Cable	12S12.7B			ton	129.2	SWPR7B
	3S12.7B	Transverse Tendons		ton	5.0	
ANCHOR	12S12.7B			set	400.0	
	3S12.7B			set	432.0	
SHEATHING	$\Phi 80/85$			m	13902.1	
	$\Phi 50/55$			m	723.6	
	Sement Grout in Sheathing			cu.m	69.8	
Bearing	Product layer rubber bearing		nos	160		
	Anchor Bar	$\Phi 75$ L=1520	nos	128		
		$\Phi 100$ L=800	nos	96		
	Anchor Cap	$\Phi 125$ L=800	nos	32		
Expansion Joint	t=50mm		m	64.5		
Pavement	t=70mm	Asphalt concrete surface course		sq.m	6017.9	
	t=5mm	Water proofing		sq.m	6017.9	
	Concrete		cu.m	4.3		
	Road Marking		sq.m	140.5		
Concrete Form			sq.m	7450.6		
			cu.m	596.0		
	D14		ton	71.5		
Shear Key			nos	960		

Item	Formula	Quantity
1. Girder Concrete CLASS "B"		
1) Girder	(1) Calculation of Sectional Area	
	[1] Middle Section	
	Summary of Sectional Area	For ONE GIRDER
No	Formula	(m ²)
1	0.650×0.080	= 0.052
2	0.850×0.120	= 0.102
3	$\frac{1}{2} \times 0.325 \times 0.110 \times 2$	= 0.036
4	0.200×0.110	= 0.022
4	1.090×0.200	= 0.218
5	$\frac{1}{2} \times 0.225 \times 0.200 \times 2$	= 0.045
6	0.200×0.200	= 0.040
6	0.650×0.250	= 0.163
	Total Area	0.678 m ²
	[2] End Section	
	Summary of Sectional Area	For ONE GIRDER
No	Formula	(m ²)
1	0.650×0.080	= 0.052
2	0.120×0.850	= 0.102
3	$\frac{1}{2} \times 0.100 \times 0.034 \times 2$	= 0.003
3	0.650×0.034	= 0.022
3	0.650×1.616	= 1.050
	Total Area	1.229 m ²
	3 - 1 - 2	

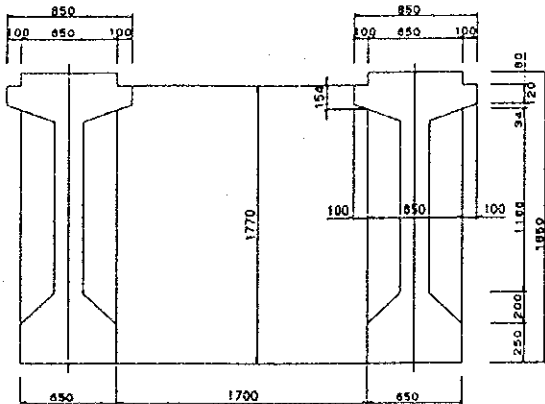
Item	Formula	Quantity
	<p>(2) Intermediate Cross Beam</p>  $V2-2 = \left\{ \begin{array}{l} 1.520 \times 1.700 - (0.120 + 0.154) \times \frac{1}{2} \times 0.100 \\ \times 2 \end{array} \right\} \times 0.2 \times 4 \times 3 \times 8 = 49.087 \text{ m}^3$ <p>(3) Connection Cross Beam</p>  $V2-3 = \left\{ \begin{array}{l} 1.770 \times 1.700 - (0.120 + 0.154) \times \frac{1}{2} \times 0.100 \\ \times 2 \end{array} \right\} \times 2.000 \times 4 + 1.229 \times 5 \times 0.200 \} \times 6$ <p style="text-align: center;">Girder section area</p> $= 150.491 \text{ m}^3$ $V2 = (23.853 + 49.087 + 150.49) \times 2 = 446.862 \text{ m}^3$ <p style="text-align: right;">Total 503.262 m³</p>	<p style="text-align: right;">503.262 m³</p>

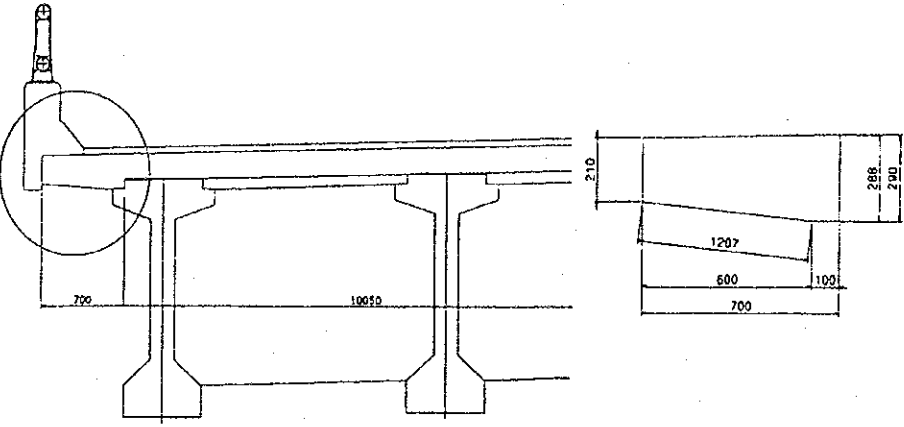
Item	Formula	Quantity
3. Deck Slab Concrete CLASS "D-1"	 $ \begin{aligned} A_1 &= (0.21 + 0.288) \times \frac{1}{2} \times 0.600 \times 2 = 0.299 \\ A_2 &= (0.288 + 0.290) \times \frac{1}{2} \times 0.100 \times 2 = 0.058 \\ A_3 &= 10.050 \times 0.210 = 2.111 \\ \Sigma A &= 2.468 \text{ m}^2 \\ V &= 2.468 \times (281.600 - 0.200) \times 2 = 1388.990 \end{aligned} $	1388.990 m ³

Item	Formula	Quantity																																										
1. Girder																																												
1) Girder	<p>(1) Calculation of Sectional Area</p> <p>[1] Middle Section</p> <p>Summary of Sectional Length For ONE GIRDER</p> <table border="1" data-bbox="368 445 1094 719"> <thead> <tr> <th>No</th> <th>Formula</th> <th>(m)</th> </tr> </thead> <tbody> <tr> <td>1</td> <td>0.080×2</td> <td>= 0.160</td> </tr> <tr> <td>2</td> <td>0.120×2</td> <td>= 0.240</td> </tr> <tr> <td>3</td> <td>0.343×2</td> <td>= 0.686</td> </tr> <tr> <td>4</td> <td>1.090×2</td> <td>= 2.180</td> </tr> <tr> <td>5</td> <td>0.301×2</td> <td>= 0.602</td> </tr> <tr> <td>6</td> <td>0.250×2</td> <td>= 0.500</td> </tr> <tr> <td colspan="2"></td> <td style="text-align: right;">Total Area 4.368 m</td> </tr> </tbody> </table>  <p>[2] End Section</p> <p>Summary of Sectional Length For ONE GIRDER</p> <table border="1" data-bbox="368 1391 1094 1592"> <thead> <tr> <th>No</th> <th>Formula</th> <th>(m)</th> </tr> </thead> <tbody> <tr> <td>1</td> <td>0.080×2</td> <td>= 0.160</td> </tr> <tr> <td>2</td> <td>0.120×2</td> <td>= 0.240</td> </tr> <tr> <td>3</td> <td>0.106×2</td> <td>= 0.212</td> </tr> <tr> <td>4</td> <td>1.616×2</td> <td>= 3.232</td> </tr> <tr> <td colspan="2"></td> <td style="text-align: right;">Total Area 3.844 m</td> </tr> </tbody> </table> 	No	Formula	(m)	1	0.080×2	= 0.160	2	0.120×2	= 0.240	3	0.343×2	= 0.686	4	1.090×2	= 2.180	5	0.301×2	= 0.602	6	0.250×2	= 0.500			Total Area 4.368 m	No	Formula	(m)	1	0.080×2	= 0.160	2	0.120×2	= 0.240	3	0.106×2	= 0.212	4	1.616×2	= 3.232			Total Area 3.844 m	
No	Formula	(m)																																										
1	0.080×2	= 0.160																																										
2	0.120×2	= 0.240																																										
3	0.343×2	= 0.686																																										
4	1.090×2	= 2.180																																										
5	0.301×2	= 0.602																																										
6	0.250×2	= 0.500																																										
		Total Area 4.368 m																																										
No	Formula	(m)																																										
1	0.080×2	= 0.160																																										
2	0.120×2	= 0.240																																										
3	0.106×2	= 0.212																																										
4	1.616×2	= 3.232																																										
		Total Area 3.844 m																																										

Item	Formula	Quantity																																																						
	<p>(2) Calculation of Form Area</p> <p style="text-align: center;">Girder Length 35.000 m</p> <table border="1" style="margin-left: auto; margin-right: auto; border-collapse: collapse;"> <thead> <tr> <th style="width: 10%;">Section No</th> <th style="width: 10%;">Section Length (m)</th> <th style="width: 10%;">Average of Section (m)</th> <th style="width: 10%;">Length of Block (m)</th> <th style="width: 10%;">Form Area (m²)</th> <th style="width: 10%;">Remark</th> </tr> </thead> <tbody> <tr> <td>END</td> <td>3.844</td> <td></td> <td></td> <td>1.229</td> <td>End-sec</td> </tr> <tr> <td>END</td> <td>3.844</td> <td>3.844</td> <td>0.500</td> <td>1.922</td> <td></td> </tr> <tr> <td>MIDDLE</td> <td>4.368</td> <td>4.106</td> <td>5.200</td> <td>21.351</td> <td></td> </tr> <tr> <td>MIDDLE</td> <td>4.368</td> <td>4.368</td> <td>23.600</td> <td>103.085</td> <td></td> </tr> <tr> <td>END</td> <td>3.844</td> <td>4.106</td> <td>5.200</td> <td>21.351</td> <td></td> </tr> <tr> <td>END</td> <td>3.844</td> <td>3.844</td> <td>0.500</td> <td>1.922</td> <td></td> </tr> <tr> <td></td> <td></td> <td></td> <td></td> <td>1.229</td> <td>End-sec</td> </tr> <tr> <td>Total</td> <td></td> <td></td> <td>35.000</td> <td>152.089</td> <td></td> </tr> </tbody> </table> <p style="text-align: center;">No. of Gir 5 × 8 × 2 = 80</p> <p>A = 152.089 × 80 = 12167.1 m² 12167.120 m²</p>	Section No	Section Length (m)	Average of Section (m)	Length of Block (m)	Form Area (m ²)	Remark	END	3.844			1.229	End-sec	END	3.844	3.844	0.500	1.922		MIDDLE	4.368	4.106	5.200	21.351		MIDDLE	4.368	4.368	23.600	103.085		END	3.844	4.106	5.200	21.351		END	3.844	3.844	0.500	1.922						1.229	End-sec	Total			35.000	152.089		
Section No	Section Length (m)	Average of Section (m)	Length of Block (m)	Form Area (m ²)	Remark																																																			
END	3.844			1.229	End-sec																																																			
END	3.844	3.844	0.500	1.922																																																				
MIDDLE	4.368	4.106	5.200	21.351																																																				
MIDDLE	4.368	4.368	23.600	103.085																																																				
END	3.844	4.106	5.200	21.351																																																				
END	3.844	3.844	0.500	1.922																																																				
				1.229	End-sec																																																			
Total			35.000	152.089																																																				

Item	Formula	Quantity
2. Diaphragm		
1) Diaphragm	$a1 = 0.318 \times 2 \times (1.090 + 1.366) \times 1/2 + 0.200 \times 1.366 = 1.054 \text{ m}^2$ $a2 = 0.318 \times 2 \times (1.090 + 1.366) \times 1/2 = 0.781 \text{ m}^2$ $A1 = (1.054 \times 6 + 0.781 \times 9) \times 8 = 106.824 \text{ m}^2$	
2) Cross Beam	(1) End Cross Beam	
	$A2-1 = [(1.770 \times 1.700 - (0.120 + 0.154) \times 1/2 \times 0.100 \times 2 \times 2) + 1.700 \times 0.500] \times 4 \times 4 = 60.867 \text{ m}^2$	
	(2) Intermediate Cross Beam	
	$A2-2 = [(1.520 \times 1.700 - (0.120 + 0.154) \times 1/2 \times 0.100 \times 2) \times 2 + 1.700 \times 0.200] \times 4 \times 3 \times 8 = 523.507 \text{ m}^2$	

Item	Formula	Quantity
	<p data-bbox="347 286 639 320">(3) Connection Cross Beam</p>  $A2-3 = \left[\left(1.770 \times 1.700 - (0.120 + 0.154) \times \frac{1}{2} \times 0.100 \right) \times 2 \times 2 \right] + 1.700 \times 2.000 \times 4 \times 6 = 152.501 \text{ m}^2$ $V2 = (60.867 + 523.507 + 152.501) \times 2 = 1473.75 \text{ m}^2$ <p data-bbox="979 1064 1412 1097" style="text-align: right;">Total 1580.57 m² 1580.574 m²</p>	

Item	Formula	Quantity
3. Deck Slab	 <p data-bbox="392 792 1262 958"> $A1 = (0.210 + 1.207) \times 2 \times (281.600 - 0.200) = 797.488 \text{ m}^2$ $A2 = (0.200 + 0.288) \times 1/2 \times 0.600 \times 4 = 0.586 \text{ m}^2$ $A3 = (0.288 + 0.290) \times 1/2 \times 0.100 \times 4 = 0.116 \text{ m}^2$ $A4 = 10.050 \times 0.210 \times 2 = 4.221 \text{ m}^2$ $\text{Sub-total} = 802.411 \text{ m}^2$ </p> <p data-bbox="392 994 1442 1025"> $\text{Total } A = 802.411 \times 2 = 1604.822 \text{ m}^2 \quad 1604.822 \text{ m}^2$ </p>	

LIST OF REINFORCEMENT

SLAB

SIGN	DIACETER	LENGTH	NOS.	UNIT WEIGHT	PIECE WEIGHT	TOTAL WEIGHT	NOTE
S	1	D20	11,350	1,678	2.466	28.000	46,984.0
	2	D20	10,850	1,678	2.466	26.800	44,970.4 (AVE)
	3	D14	963	3,836	1.208	1.160	4,449.8
	4	D22	12,000	600	2.984	35.800	21,480.0
	5	D14	25,460	400	1.208	30.800	12,320.0
	6	D14	30,606	200	1.208	37.000	7,400.0
	6'	D14	32,606	200	1.208	39.400	7,880.0
	7	D25	11,350	240	3.853	43.700	10,488.0 (AVE)
	8	D25	10,850	240	3.853	41.800	10,032.0
	9	D12	565	20,160	0.888	0.502	10,120.3
					TOTAL	176,124.5	
					D25	20520.0	
					D22	21480.0	
					D20	91954.4	
					D14	32049.8	
					D12	10120.3	
						176,124.5	
					- D14	42,170.1	42.2
					D16-D25	133,954.4	134.0
					ToTAL	176,124.5	176.1

LIST OF REINFORCEMENT

Diaphragm

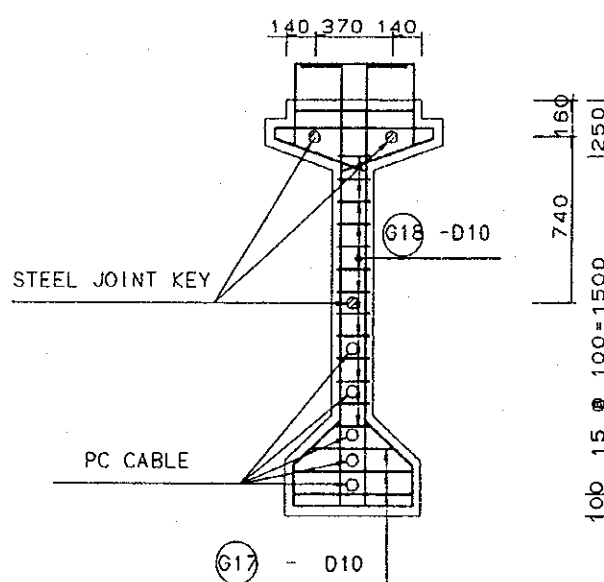
SIGN	DIACETER	LENGTH	NOS.	UNIT WEIGHT	PIECE WEIGHT	TOTAL WEIGHT	NOTE
D	1	D16	1,700	288	1.578	2.680	771.8
	2	D14	1,700	112	1.208	2.050	229.6 (AVE)
E	1	D16	4,724	1,536	1.578	7.460	11,458.6
	2	D14	3,924	672	1.208	4.740	3,185.3
C	1	D16	1,700	528	1.578	2.680	1,415.0
	2	D14	2,400	672	1.208	2.900	1,948.8
	3	D14	5,442	336	1.208	6.580	2,210.9
	4	D14	5,514	168	1.208	6.660	1,118.9 (AVE)
	5	D16	9,900	12	1.578	15.600	187.2
TOTAL						22,526.1	
					D16	13832.6	
					D14	8693.5	
						22,526.1	
					- D14	8,694	8.7
					D16-D25	13,833	13.8
					ToTAL	22,526	22.5

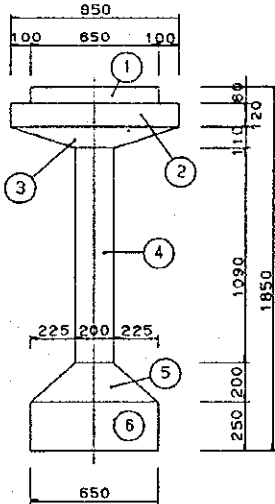
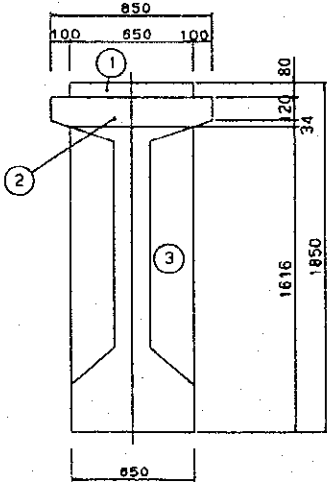
Item	Formula						Quantity	
PC CABLE 1) 12S12.7	For ONE GIRDER							
	CABLE VAR.	EACH LENGTH	CABLE NO.	EACH	TOTAL LENGTH	UNIT WEIGHT	WEIGHT	
	1	34.702	C1	1	34.702	9.290	322.4	
	2	34.714	C2	1	34.714	9.290	322.5	
	3	34.754	C3	1	34.754	9.290	322.9	
	4	34.784	C4	1	34.784	9.290	323.1	
	5	34.822	C5	1	34.822	9.290	323.5	
	TOTAL			5	173.776		1614.4	
	SUB-TOTAL WEIGHT OF PC CABLES per BRIDGE							
	$W_p = 1614.4 \times 5 \times 8 \times 2 =$						129152 kgf 129.2 ton	
	TENSION UNIT							
	$N_s = 5 \times 2 \times 5 \times 8 \times 2 =$						400 nos	
	HEATHING $\Phi 80/85$							
$L = 173.776 \times 5 \times 8 \times 2 =$						13902.08 m		
CENEBT GROUT								
$V = 0.873 \times 5 \times 8 \times 2 =$						69.844 m ³		
2) 3S12.7	PC CABLE OF DIAPHRAGMS							
	LOCATION	EACH LENGTH	CABLE NO.	EACH	TOTAL LENGTH	UNIT WEIGHT	WEIGHT	
	Connection One	10.050		216	2170.8	2.322	5040.598	
	TOTAL			216	2170.800		5040.598	
	TOTAL WEIGHT OF PC CABLES per BRIDGE(A1-P12)							
	$W_p = 5040.598 =$						5040.598 kgf 5.0 ton	
	TENSION UNIT							
	$N_s = 216 \times 2 =$						432 nos	
	HEATHING $\Phi 50/55$							
	$L = 723.600 =$						723.600 m	
	CENEBT GROUT							
	$V = 0.002 \times 723.600 =$						1.42 m ³	

Item	Formula	Quantity
9 Pavement	1) Pavement t=70mm	
	$A = \frac{10.750 \times (281.60 - 0.850 \times 2)}{2} = 6017.9 \text{ m}^2$	6017.9 m ²
	2) Water Proofing t=5mm	
	$A = 2 = 6017.9 \text{ m}^2$	6017.9 m ²
3) Concrete		
	<p style="text-align: right;">per one side</p> $A1 = 0.400 \times 0.075 - 0.1 \times 0.034 = 0.027$ $A2 = 0.075 \times 0.300 = 0.023$ <p style="text-align: right;">Total 0.050 m²</p>	
	$V = 0.050 \times 10.750 \times 8 = 4.300 \text{ m}^3$	4.300 m ³
	4) Road marking	
	Bridge Length L= 281.600 - 0.600 = 281.000 m	
	Side Line	
	$A1 = 281.000 \times 0.100 \times 4 = 112.400 \text{ m}^2$	112.400 m ²
	Center Line	
	$L = 281.000 / 5.000 = 56.2 \text{ m}$	56.2 m
	$A2 = 2.500 \times 0.100 \times 56.2 \times 2 = 28.100 \text{ m}^2$	28.100 m ²
	Total 112.400 + 28.100 = 140.5 m ²	140.5 m ²

Item	Formula	Quantity
1. Expansion Joint (TYPE A)	<p>EACH LENGTH $L = 10.750$</p> <p>$N = 3 \times 2 = 6$</p> <p>TOTAL LENGTH $L = 10.750 \times 6 = 64.500 \text{ m}$</p>	
2. BEARING PAD ELASTOMERIC 550*300*57	<p>EACH for One SPAN $N = 5$</p> <p>TOTAL EACH $N = 5 \times 16 \times 2 = 160 \text{ nos}$</p>	
3. ANCHOR BAR	<p>$\Phi 75 \quad L=1520 \quad (\text{FIX})$ $N = 4 \times 12 \times 2 = 96 \text{ nos}$</p> <p>$\Phi 75 \quad L=1520 \quad (\text{MOVE})$ $N = 4 \times 4 \times 2 = 32 \text{ nos}$</p> <p style="text-align: right;">Total = 128 nos</p>	
4. ANCHOR CAP (SGP)	<p>$\Phi 100 \quad L=800 \quad (\text{FIX})$ $N = 4 \times 12 \times 2 = 96 \text{ nos}$</p> <p>$\Phi 125 \quad L=800 \quad (\text{MOVE})$ $N = 4 \times 4 \times 2 = 32 \text{ nos}$</p>	
5. Railing	<p>$L = 281.600 \times 2 \times 2 = 1126.4 \text{ m}$</p>	

Item	Formula	Quantity
Slab Plate	$A = 1.950 \times 8 \times 39.800 \times 3 \times 4 =$	7450.560 m ²
	$V = 7450.56 \times 0.080 =$	596.045 m ³
	$W = 596.045 \times 120 \text{ (kg/M3)} =$	71525.4 kg 71.525 ton

Item	Formula	Quantity
Shear Key	<p>per one girder</p> $N = 3 \times 4 \times 5 \times 16 = 960$ 	each

Item	Formula	Quantity																											
Efection	<p>(1) Calculation of Sectional Area</p> <p>[1] Middle Section</p> <p>Summary of Sectional Area For ONE GIRDER</p> <table border="1" data-bbox="379 454 1098 757"> <thead> <tr> <th>No</th> <th>Formula</th> <th>(m2)</th> </tr> </thead> <tbody> <tr> <td>1</td> <td>0.650×0.080</td> <td>= 0.052</td> </tr> <tr> <td>2</td> <td>0.850×0.120</td> <td>= 0.102</td> </tr> <tr> <td>3</td> <td>$\frac{1}{2} \times 0.325 \times 0.110 \times 2$</td> <td>= 0.036</td> </tr> <tr> <td></td> <td>0.200×0.110</td> <td>= 0.022</td> </tr> <tr> <td>4</td> <td>1.090×0.200</td> <td>= 0.218</td> </tr> <tr> <td>5</td> <td>$0.500 \times 0.225 \times 0.200 \times 2$</td> <td>= 0.045</td> </tr> <tr> <td></td> <td>0.200×0.200</td> <td>= 0.040</td> </tr> <tr> <td>6</td> <td>0.650×0.250</td> <td>= 0.163</td> </tr> </tbody> </table> <p style="text-align: right;">Total Area 0.678 m²</p> 	No	Formula	(m2)	1	0.650×0.080	= 0.052	2	0.850×0.120	= 0.102	3	$\frac{1}{2} \times 0.325 \times 0.110 \times 2$	= 0.036		0.200×0.110	= 0.022	4	1.090×0.200	= 0.218	5	$0.500 \times 0.225 \times 0.200 \times 2$	= 0.045		0.200×0.200	= 0.040	6	0.650×0.250	= 0.163	
No	Formula	(m2)																											
1	0.650×0.080	= 0.052																											
2	0.850×0.120	= 0.102																											
3	$\frac{1}{2} \times 0.325 \times 0.110 \times 2$	= 0.036																											
	0.200×0.110	= 0.022																											
4	1.090×0.200	= 0.218																											
5	$0.500 \times 0.225 \times 0.200 \times 2$	= 0.045																											
	0.200×0.200	= 0.040																											
6	0.650×0.250	= 0.163																											
	<p>[2] End Section</p> <p>Summary of Sectional Area For ONE GIRDER</p> <table border="1" data-bbox="379 1395 1098 1601"> <thead> <tr> <th>No</th> <th>Formula</th> <th>(m2)</th> </tr> </thead> <tbody> <tr> <td>1</td> <td>0.650×0.080</td> <td>= 0.052</td> </tr> <tr> <td>2</td> <td>0.120×0.850</td> <td>= 0.102</td> </tr> <tr> <td></td> <td>$\frac{1}{2} \times 0.100 \times 0.034 \times 2$</td> <td>= 0.003</td> </tr> <tr> <td></td> <td>0.650×0.034</td> <td>= 0.022</td> </tr> <tr> <td>3</td> <td>0.650×1.616</td> <td>= 1.050</td> </tr> </tbody> </table> <p style="text-align: right;">Total Area 1.229 m²</p> 	No	Formula	(m2)	1	0.650×0.080	= 0.052	2	0.120×0.850	= 0.102		$\frac{1}{2} \times 0.100 \times 0.034 \times 2$	= 0.003		0.650×0.034	= 0.022	3	0.650×1.616	= 1.050										
No	Formula	(m2)																											
1	0.650×0.080	= 0.052																											
2	0.120×0.850	= 0.102																											
	$\frac{1}{2} \times 0.100 \times 0.034 \times 2$	= 0.003																											
	0.650×0.034	= 0.022																											
3	0.650×1.616	= 1.050																											

Item	Formula	Quantity																																																
	<p>Concrete Volume</p> <p style="margin-left: 200px;">Girder Length 35.000 m</p> <table border="1" style="margin-left: auto; margin-right: auto; border-collapse: collapse;"> <thead> <tr> <th style="width: 10%;">Section No.</th> <th style="width: 10%;">Section Area (m²)</th> <th style="width: 10%;">Average of Section (m²)</th> <th style="width: 10%;">Length of Block (m)</th> <th style="width: 10%;">Concrete Volume (m³)</th> <th style="width: 10%;">Remark</th> </tr> </thead> <tbody> <tr> <td>END</td> <td>1.229</td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>END</td> <td>1.229</td> <td>1.229</td> <td>0.400</td> <td>0.492</td> <td></td> </tr> <tr> <td>MIDDLE</td> <td>0.678</td> <td>0.954</td> <td>6.000</td> <td>5.724</td> <td></td> </tr> <tr> <td>MIDDLE</td> <td>0.678</td> <td>0.678</td> <td>22.200</td> <td>15.052</td> <td></td> </tr> <tr> <td>END</td> <td>1.229</td> <td>0.954</td> <td>6.000</td> <td>5.724</td> <td></td> </tr> <tr> <td>END</td> <td>1.229</td> <td>1.229</td> <td>0.400</td> <td>0.492</td> <td></td> </tr> <tr> <td>Total</td> <td></td> <td></td> <td>35.000</td> <td>27.484</td> <td></td> </tr> </tbody> </table>	Section No.	Section Area (m ²)	Average of Section (m ²)	Length of Block (m)	Concrete Volume (m ³)	Remark	END	1.229					END	1.229	1.229	0.400	0.492		MIDDLE	0.678	0.954	6.000	5.724		MIDDLE	0.678	0.678	22.200	15.052		END	1.229	0.954	6.000	5.724		END	1.229	1.229	0.400	0.492		Total			35.000	27.484		
Section No.	Section Area (m ²)	Average of Section (m ²)	Length of Block (m)	Concrete Volume (m ³)	Remark																																													
END	1.229																																																	
END	1.229	1.229	0.400	0.492																																														
MIDDLE	0.678	0.954	6.000	5.724																																														
MIDDLE	0.678	0.678	22.200	15.052																																														
END	1.229	0.954	6.000	5.724																																														
END	1.229	1.229	0.400	0.492																																														
Total			35.000	27.484																																														
	<p>Erection Weight</p> <p style="text-align: right;">per one girder</p> <p>W = 27.484 × 2500 = 68710 (kgf)</p> <p style="text-align: right;">68.7 (ton)</p> <p>W = 27.484 × 2500 × 9 × 3 × 4 = 7420680 (kgf)</p> <p style="text-align: right;">7420.7 (ton)</p>																																																	

QUANTITY TABLE OF BRIDGE

Large Tra Va Bridge

Item		UNIT	ABUTMENT A1	ABUTMENT A2	Total	
A - ABUTMENT						
PILES	NUMBER OF PILES	pile	17	17	34	
	TOTAL LENGTH OF BORED ϕ 1500	m	1190.0	1190.0	2380.0	
	CONCRETE DLASS D	m3	2102.9	2102.9	4205.8	
	REINFORCEMENT	32	kg			
		28	kg	1624.0	1624	3248.0
		25	kg	3509.8	3509.8	7019.6
		22	kg	387.0	387	774.0
		16	kg	29.0	29	58.0
10		kg	1077.6	1077.6	2155.2	
TOTAL	kg	6627.4	6627.4	13254.8		
ABUTMENT	CONCRETE DLASS E	m3	671.6	670.9	1342.5	
	REINFORCEMENT	25	kg	16020.6	16024.8	32045.4
		22	kg	5052.6	5051.8	10104.4
		20	kg	6814.2	6814.2	13628.4
		18	kg	802.5	802.0	1604.5
		16	kg	4813.0	4814.6	9627.6
		14	kg	2967.5	2967.5	5935.0
		10	kg	159.3	159.3	318.6
		TOTAL	kg	36629.7	36634.2	73263.9
	BLINDING STONE	m3	45.7	45.7	91.4	
	LEAN CONCRETE CLASS G	m3	22.8	22.8	45.7	
	EXCAVATION	m3	420.6	567.8	988.4	
BACK FILL	m3	97.1	144.5	241.6		
B - APPROACH SLAB						
	CONCRETE DLASS E	m3	45.1	45.1	90.2	
	LEAN CONCRETE CLASS G	m3	13.1	13.1	26.3	
	FORM	m2	20.4	20.4	40.8	
	ASPHANTIC JOINT FILLTER T=20	m3	0.4	0.4	0.8	
	REINFORCEMENT	20	kg	3421.6	3421.6	6843.2
		16	kg	3402.3	3402.3	6804.6
		10	kg	255.8	255.8	511.6
		TOTAL	kg	7158.7	7158.7	14317.4
C - SLOPE PROTECTION						
SIDE SLOPE	STONE MANSORY T=300mm	m3	829.7	860.5	1690.2	
	BLINDING AGGREGATE T=100mm	m3	276.2	286.8	563.0	
	GEOTEXTILE	m2	831.0	955.0	1786.0	
	PVC PILE Φ 50mm DIA. L=1000mm	m	75.0	76.0	151.0	
FOOTING	LENGTH OF FOOTING	m	149.9	156.1	306.0	
	WOODEN PILE L=3.0m	m	8996.0	9094.0	18090.0	
	BLINDING	m3	26.0	12.1	38.1	
	STONE MANSORY	m3	40.0	54.8	94.8	
	EXCAVATION	m3	648.0	655.0	1303.0	
	BACK FILL	m3	450.0	455.0	905.0	

Large Tra Va Bridge

Item	Formula	Quantity
Abutment A1		
1. Concrete (Class E)		
1) Paramet	$V1 = 24.100 \times 0.400 \times 2.335 =$	22.509 m3
2) Corbel	$V2 = (0.300 + 0.600) \times \frac{1}{2} \times 0.300$ $\times (22.100 + 22.700) \times \frac{1}{2}$	3.024 m3
3) Wall	$V3 = 24.100 \times 1.500 \times 4.640 =$	167.736 m3
4) Footing	$V4 = 24.100 \times 9.200 \times 2.000 =$	443.440 m3
5) Wing Wall	$V5-1 = (6.975 + 7.140) \times \frac{1}{2} \times 5.050$ $\times 0.500 \times 2 =$	35.640 m3
	$V5-2 = (5.000 + 5.800) \times \frac{1}{2} \times 0.650$ $\times 0.500 \times 2 =$	-3.510 m3
6) Wall Haunch	$V6 = 0.500 \times 0.500 \times \frac{1}{2} \times 6.975$ $\times 2 =$	1.744 m3
7) Railing	$V7 = (0.300 \times 0.300 - 0.02 \times 0.020)$ $\times \frac{1}{2} \times 2 + (0.35 + 0.500)$ $\times \frac{1}{2} \times 0.150 + 0.500 \times 0.05$ $\times 5.350 \times 2 =$	0.976 m3
	Total (One Abutment)	671.559 m3
2. Form		
1) Paramet	$A1 = 24.100 \times 2.335 + (2.335 - 0.600)$ $\times 22.100 =$	94.617 m2
2) Corbel	$A2 = (0.300 + 0.300 \times 1.414) \times (22.100$ $+ 22.700) \times \frac{1}{2} =$	16.222 m2
3) Wall	$A3 = (24.100 + 22.100) \times 4.640 =$	214.368 m2
4) Footing	$A4 = (24.100 + 9.200) \times 2.000 \times 2 =$	133.200 m2
5) Wing Wall	$A5-1 = (6.975 + 7.140) \times \frac{1}{2} \times 5.050$ $\times 4 =$	142.562 m2
	$A5-2 = (5.000 + 5.800) \times \frac{1}{2} \times 0.650$ $\times 4 - 0.500 \times 6.975 \times 2.000 =$	-7.065 m2
	$A5-3 = 0.500 \times 2.335 \times 2 =$	2.335 m2
	$A5-4 = (1.040 + (0.800^2 + 0.650^2)^{0.5} + 5.000)$ $\times 0.500 \times 2 =$	7.071 m2
6) Wall Haunch	$A6 = (0.300^2 + 0.300^2)^{0.5} \times 6.975 \times 2 =$	5.918 m2

Large Tra Va Bridge

Item	Formula	Quantity
7) Railing	$A7-1 = (0.300 \times 0.300 - 0.02 \times 0.020)$ $\times \frac{1}{2} \times 2 + (0.35 + 0.500)$ $\times \frac{1}{2} \times 0.150 + 0.500 \times 0.05)$ $\times 2$	0.182 m2
	$A7-2 = (0.050 + 0.150 \times 1.414 + 0.280$ $+ 0.480 + 0.020 \times 1.414 \times 2)$ $\times 5.350 \times 2 =$	11.542 m2
	Total (One Abutment)	620.952 m2

Large Tra Va Bridge

Item	Formula	Quantity
4. Lean Concrete	$V = 24.300 \times 9.400 \times 0.1000 =$	22.842 m ³
5. Blinding Stone	$V = 24.300 \times 9.400 \times 0.2000 =$	45.684 m ³
6. Excavation	<p data-bbox="558 492 861 526">Excavation Height = 1.450</p> $V = \left(25.300 \times 10.400 + 26.750 \times 11.850 \right) \times \frac{1}{2} \times 1.450 =$	420.578 m ³
7. Back Fill	$V = 420.578 - 45.684 - 22.842 - 24.100 \times 9.200 \times 1.150 =$	97.074 m ³

Large Tra Va Bridge

Item	Formula	Quantity
Abutment A2		
1. Concrete (Class E)		
1) Paramet	$V1 = 24.100 \times 0.400 \times 2.335 =$	22.509 m3
2) Corbel	$V2 = (0.300 + 0.600) \times \frac{1}{2} \times 0.300$ $\times (22.100 + 22.700) \times \frac{1}{2}$	3.024 m3
3) Wall	$V3 = 24.100 \times 1.500 \times 4.640 =$	167.736 m3
4) Footing	$V4 = 24.100 \times 9.200 \times 2.000 =$	443.440 m3
5) Wing Wall	$V5-1 = (6.975 + 6.893) \times \frac{1}{2} \times 5.050$ $\times 0.500 \times 2 =$	35.017 m3
	$V5-2 = (5.000 + 5.800) \times \frac{1}{2} \times 0.650$ $\times 0.500 \times 2 =$	-3.510 m3
6) Wall Haunch	$V6 = 0.500 \times 0.500 \times \frac{1}{2} \times 6.975$ $\times 2 =$	1.744 m3
7) Railing	$V7 = (0.300 \times 0.300 - 0.02 \times 0.020)$ $\times \frac{1}{2} \times 2 + (0.35 + 0.500)$ $\times \frac{1}{2} \times 0.150 + 0.500 \times 0.05$ $\times 5.350 \times 2 =$	0.976 m3
	Total (One Abutment)	670.936 m3
2. Form		
1) Paramet	$A1 = 24.100 \times 2.335 + (2.335 - 0.600)$ $\times 22.100 =$	94.617 m2
2) Corbel	$A2 = (0.300 + 0.300 \times 1.414) \times (22.100$ $+ 22.700) \times \frac{1}{2} =$	16.222 m2
3) Wall	$A3 = (24.100 + 22.100) \times 4.640 =$	214.368 m2
4) Footing	$A4 = (24.100 + 9.200) \times 2.000 \times 2 =$	133.200 m2
5) Wing Wall	$A5-1 = (6.975 + 6.893) \times \frac{1}{2} \times 5.050$ $\times 4 =$	140.067 m2
	$A5-2 = (5.000 + 5.800) \times \frac{1}{2} \times 0.650$ $\times 4 - 0.500 \times 6.975 \times 2.000 =$	-7.065 m2
	$A5-3 = 0.500 \times 2.335 \times 2 =$	2.335 m2
	$A5-4 = (1.040 + (0.800^2 + 0.650^2)^{0.5} + 5.053)$ $\times 0.500 \times 2 =$	7.124 m2
6) Wall Haunch	$A6 = (0.300^2 + 0.300^2)^{0.5} \times 6.975 \times 2 =$	5.918 m2

Large Tra Va Bridge

Item	Formula	Quantity
7) Railing	$A7-1 = (0.300 \times 0.300 - 0.02 \times 0.020$ $\times \frac{1}{2} \times 2 + (0.35 + 0.500)$ $\times \frac{1}{2} \times 0.150 + 0.500 \times 0.05)$ $\times 2$	0.182 m2
	$A7-2 = (0.050 + 0.150 \times 1.414 + 0.280$ $+ 0.480 + 0.020 \times 1.414 \times 2)$ $\times 5.350 \times 2 =$	11.542 m2
	Total (One Abutment)	618.510 m2

Large Tra Va Bridge

Item	Formula	Quantity
4. Lean Concrete	$V = 24.300 \times 9.400 \times 0.1000 =$	22.842 m ³
5. Blinding Stone	$V = 24.300 \times 9.400 \times 0.2000 =$	45.684 m ³
6. Excavation	$\text{Excavation Height} = 1.900$ $V = \left(\frac{25.300 \times 10.400 + 27.200 \times 12.300}{2} \right) \times 1.900 =$	567.796 m ³
7. Back Fill	$V = \frac{567.796 - 45.684 - 22.842 - 24.100}{9.200 \times 1.600} =$	144.518 m ³

Large Tra Va Bridge

Item	Formula	Quantity
Pile	A1,A2	
1) Number of Pile		
	$N = 17 = 17$	17 Pill
2) Total Length of Bored Piles ϕ 1500		
	$L = 17 \times 70.000 = 1190.0$	1190.0 m
3) Concrete Class D		
	$V = 0.750^2 \times \pi \times 1190.0 = 2102.904$	2102.9 m ³
4) Reiforcemento		
	D28 1624.0	
	D25 3509.8	
	D22 387.0	
	D16 29.0	
	D10 1077.6	
	Total 6627.4	

Large Tra Va Bridge
LIST OF REINFORCEMENT

A1

	SIGN	DIACETER	LENGTH	NOS.	UNIT WEIGHT	PIECE WEIGHT	TOTAL WEIGHT	NOTE
A1	1	D14	2,611	289	1.208	3.154	911.6	
	2	D14	24,752	18	1.208	29.900	538.3	
	3	D14	720	196	1.208	0.870	170.5	
	4	D16	1,837	97	1.579	2.901	281.4	
	5	D14	690	97	1.208	0.834	80.9	
	6	D14	23,552	4	1.208	28.451	113.9	
F	1	D25	12,500	97	3.854	48.175	4,673.0	
	2	D25	9,000	96	3.854	34.686	3,329.9	
	3	D25	9,874	97	3.854	38.054	3,691.3	
	4	D25	6,200	96	3.854	23.895	2,294.0	
	5	D20	9,655	16	2.466	23.809	381.0	
	6	D20	25,200	16	2.466	62.143	994.3	
	7	D16	4,830	176	1.579	7.627	1,342.3	
	8	D20	28,700	37	2.466	70.774	2,618.7	
	9	D20	25,900	37	2.466	63.869	2,363.2	
W	1	D22	5,485	94	2.984	16.367	1,538.6	
	2	D22	6,955	97	2.984	20.754	2,013.2	
	3	D16	6,850	97	1.579	10.816	1,049.2	
	4	D14	1,822	221	1.208	2.201	486.5	
	5	D16	24,856	20	1.579	39.248	785.0	
	6	D14	24,752	20	1.208	29.900	598.1	
	7	D16	1,860	97	1.579	2.937	284.9	
K	1	D22	5,955	8	2.984	17.770	142.2	
	2	D22	3,085	18	2.984	9.206	165.8	
	3	D22	4,185	16	2.984	12.488	199.9	
	4	D25	9,649	42	3.854	37.187	1,561.9	
	5	D25	4,587	24	3.854	17.678	424.3	
	6	D22	6,045	38	2.984	18.038	685.5	
	7	D18	6,770	8	1.998	13.526	108.3	
	8	D16	6,745	10	1.579	10.650	106.6	
	9	D16	1,731	48	1.579	2.733	131.2	
	10	D16	2,370	2	1.579	3.742	7.5	
	11	D20	2,250	18	2.466	5.549	99.9	
	12	D16	9,492	42	1.579	14.988	629.5	
	13	D18	6,335	38	1.998	12.657	481.0	
	14	D10	690	290	0.617	0.426	123.5	
	15	D22	5,681	6	2.984	16.952	101.8	
	16	D18	6,496	6	1.998	12.979	77.9	
	17	D16	2,091	4	1.579	3.302	13.3	
	18	D16	2,091	4	1.579	3.302	13.3	
	19	D22	5,305	4	2.984	15.830	63.4	
	20	D18	5,235	4	1.998	10.460	41.9	
	21	D25	2,994	4	3.854	11.539	46.2	
	22	D14	875	64	1.208	1.057	67.7	
	23	D20	3,810	38	2.466	9.395	357.1	
	24	D16	2,227	48	1.579	3.516	168.8	
	25	D22	5,956	8	2.984	17.773	142.2	
	26	D18	5,841	8	1.998	11.670	93.4	
R	1	D10	5,800	10	0.617	3.579	35.8	
						TOTAL	36,629.7	
						D25	16020.6	
						D22	5052.6	
						D20	6814.2	
						D18	802.5	
						D16	4813.0	
						D14	2967.5	
						D12	0.0	
						D10	159.3	
							36629.7	

Large Tra Va Bridge
LIST OF REINFORCEMENT

A2

	SIGN	DIACETER	LENGTH	NOS.	UNIT WEIGHT	PIECE WEIGHT	TOTAL WEIGHT	NOTE
A1	1	D14	2,611	289	1.208	3.154	911.6	
	2	D14	24,752	18	1.208	29.900	538.3	
	3	D14	720	196	1.208	0.870	170.5	
	4	D16	1,837	97	1.579	2.901	281.4	
	5	D14	690	97	1.208	0.834	80.9	
	6	D14	23,552	4	1.208	28.451	113.9	
F	1	D25	12,500	97	3.854	48.175	4,673.0	
	2	D25	9,000	96	3.854	34.686	3,329.9	
	3	D25	9,874	97	3.854	38.054	3,691.3	
	4	D25	6,200	96	3.854	23.895	2,294.0	
	5	D20	9,655	16	2.466	23.809	381.0	
	6	D20	25,200	16	2.466	62.143	994.3	
	7	D16	4,830	176	1.579	7.627	1,342.3	
	8	D20	28,700	37	2.466	70.774	2,618.7	
	9	D20	25,900	37	2.466	63.869	2,363.2	
W	1	D22	5,485	94	2.984	16.367	1,538.6	
	2	D22	6,955	97	2.984	20.754	2,013.2	
	3	D16	6,850	97	1.579	10.816	1,049.2	
	4	D14	1,822	221	1.208	2.201	486.5	
	5	D16	24,856	20	1.579	39.248	785.0	
	6	D14	24,752	20	1.208	29.900	598.1	
	7	D16	1,860	97	1.579	2.937	284.9	
K	1	D22	5,955	8	2.984	17.770	142.2	
	2	D22	3,085	18	2.984	9.206	165.8	
	3	D22	4,185	16	2.984	12.488	199.9	
	4	D25	9,675	42	3.854	37.287	1,566.1	
	5	D25	4,587	24	3.854	17.678	424.3	
	6	D22	6,045	38	2.984	18.038	685.5	
	7	D18	6,770	8	1.998	13.526	108.3	
	8	D16	6,745	10	1.579	10.650	106.6	
	9	D16	1,731	48	1.579	2.733	131.2	
	10	D16	2,370	2	1.579	3.742	7.5	
	11	D20	2,250	18	2.466	5.549	99.9	
	12	D16	9,518	42	1.579	15.029	631.3	
	13	D18	6,335	38	1.998	12.657	481.0	
	14	D10	690	290	0.617	0.426	123.5	
	15	D22	5,637	6	2.984	16.821	101.0	
	16	D18	6,452	6	1.998	12.891	77.4	
	17	D16	2,087	4	1.579	3.295	13.2	
	18	D16	2,087	4	1.579	3.295	13.2	
	19	D22	5,305	4	2.984	15.830	63.4	
	20	D18	5,235	4	1.998	10.460	41.9	
	21	D25	2,994	4	3.854	11.539	46.2	
	22	D14	875	64	1.208	1.057	67.7	
	23	D20	3,810	38	2.466	9.395	357.1	
	24	D16	2,227	48	1.579	3.516	168.8	
	25	D22	5,956	8	2.984	17.773	142.2	
	26	D18	5,841	8	1.998	11.670	93.4	
R	1	D10	5,800	10	0.617	3.579	35.8	
						TOTAL	36,634.2	
						D25	16,024.8	
						D22	5,051.8	
						D20	6,814.2	
						D18	802.0	
						D16	4,814.6	
						D14	2,967.5	
						D10	159.3	
							36,634.2	

Large Tra Va Bridge

Item	Formula	Quantity
Approach Slab		
1. Concrete (Class E)	$V1 = 23.040 \times 6.000 \times 0.300 =$	41.472 m3
	$V2 = (0.300 + 0.500) \times 23.040 \times \frac{1}{2} \times 0.200 =$	3.686 m3
	$V3 = - \frac{0.500 \times 0.500 \times 1}{2} \times \frac{1}{2} \times 0.300 =$	-0.075 m3
	Total	45.083 m3
2. Lean Concrete	$V = 23.040 \times 5.700 \times 0.100 =$	13.133 m3
3. Form	$A1 = (0.300 + 0.500) \times 23.040 =$	18.432 m2
	$A2 = 0.300 \times 6.000 =$	1.800 m2
	$A3 = (0.300 + 0.500) \times \frac{1}{2} \times 0.200 =$	0.160 m2
	Total	20.392 m2
4. Asphtic joint	$V = ((0.060 + 0.020) \times \frac{1}{2} \times 0.300 + 0.020 \times 0.300) \times 23.040 =$	0.415 m3
5. Re-bar	D10 255.8 D16 3402.3 D20 3421.6 Total 7079.7	7079.700 kg

QUANTITIES FOR EARTHWORKS SLOPE PROTECTION

Bridge Abutment **LARGE TRA VA**
A1

h1	h2	h3	h4	h5	h6	d1	d2
8.22	3.5	2.9	0.45	0.95	0.26	24.1	12

Block	a (m)	b (m)	Dh (m)	R (m)	r (m)	L (m)	Sxq (m ²)	W (m)	Masonry (m ³)	Blinding (m ³)
1	9.940	6.400	4.720	8.170	0.500	9.435	60.514		18.15	6.05
2	12.000	12.000	0.600	20.170	8.170	12.015	267.296		80.19	26.73
3	4.900	3.063	2.450	24.151	20.170	4.675	162.643		48.79	16.26
4	24.100	12.000	0.600				289.561		86.87	28.96
5	24.100	3.063	2.450				94.518		28.36	9.45
6	9.940	6.400	4.720	8.170	0.000	9.435	60.514		18.15	6.05
7	12.000	12.000	0.600	20.170	8.170	12.015	267.296		80.19	26.73
8	4.900	3.063	2.450	24.151	20.170	4.675	162.643		48.79	16.26
9			1.200				4.320	99.935		
Extra								87.434	409	136
FOOTING	Wood pile		(m)	5996						
	Binding		(m ³)	8.0						
	Stone masonry		(m ³)	36.0						
	Excavation		(m ³)	432						
	Back fill		(m ³)	300						
LENGTH OF FOOTING : L = 99.93 (m)										
SIDE SLOPE	Masonry		(m ³)	408						
	Blinding		(m ³)	136						
	Geotextile		(m ²)	507						
	PVC Pipe		(m)	50						

Abutment **A2**

h1	h2	h3	h4	h5	h6	d1	d2
8.11	3.5	2.9	0.02	0.52	0.68	24.1	12

Block	a (m)	b (m)	Dh (m)	R (m)	r (m)	L (m)	Sxq (m ²)	W (m)	Masonry (m ³)	Blinding (m ³)
1	9.720	6.263	4.610	7.991	0.000	9.226	57.874		17.36	5.79
2	12.000	12.000	0.600	19.991	7.991	12.015	263.924		79.18	26.39
3	5.760	3.600	2.880	24.671	19.991	5.495	192.661		57.80	19.27
4	24.100	12.000	0.600				289.561		86.87	28.96
5	24.100	3.600	2.880				111.107		33.33	11.11
6	9.720	6.263	4.610	7.991	0.000	9.226	57.874		17.36	5.79
7	12.000	12.000	0.600	19.991	7.991	12.015	263.924		79.18	26.39
8	5.760	3.600	2.880	24.671	19.991	5.495	192.661		57.80	19.27
9			1.200				4.320	101.568		
Extra								86.873	429	143
FOOTING	Wood pile		(m)	6094						
	Binding		(m ³)	8.1						
	Stone masonry		(m ³)	36.5						
	Excavation		(m ³)	438.8						
	Back fill		(m ³)	305						
LENGTH OF FOOTING : L = 101.57 (m)										
SIDE SLOPE	Masonry		(m ³)	428.9						
	Blinding		(m ³)	143.0						
	Geotextile		(m ²)	583						
	PVC Pipe		(m)	51						

NOTES :

* INPUT VALUES : GRAY CELLS ; TOTAL VALUES : BOLD NUMBERS

* No. of Blocks : see drawing. Extra block used for calculation only.

- h : Elevations (see drawing)
- d : Distances (see drawing)
- a : Long edge of ellipse (or rectangle) : a=Dh*slope
- b : Short edge of ellipse (or rectangle) : b=Dh*slope
- Dh : Height of cone (or truncated cone)
- R : Average radius of lower ellipse : R=(Sa+Sb)/2
- r : Average radius of upper ellipse : r=(Sa+Sb)/2
- L : Generatrix of cone : L=sqrt(Dh²+(R-r)²)
- Sxq : Area of cone's side (or plane before abutment) :
 Sxq=3.14/4*(R+r)*L for block 1,2,3,6,7,8
 Sxq=a*sqrt(b²+Dh²) for block 4,5
 Sxq=(1.8+1.8+Dh₁+2*Dh₂)*Dh₂/2 for block 9

- W : Length of masonry's footing : W₃
 W₃=2*3.14*R₂/4+a₃+2*3.14*R₂/4 , and
 W₃=2*3.14*R₂/4+a₃+2*3.14*R₂/4
- Masonry₃ = 0.3*Sxq₃
- Blinding₃ = 0.1*Sxq₃
- Geotextile = Sxq₃ + Sxq₂ + Sxq₁ + W₃*1
- PVC Pipe = W₃/2
- Wood Pile = W₃*0.8*25*3
- Excavation = W₃*Sxq₃
- Filling = Excavation-W₃*Dh₃*1.1

LARGE TRA VA BRIDGE : QUANTITIES OF EARTHWORKS SLOPE PROTECTION OF 25M BEHIND HEAD WALL

ABUTMENT A1:

Block	B (m)	L (m)	S (m2)	Thick (m)	Masonry (m3)	Blinding (T=0.1m) (m3)	
10	10.55	25	263.86	0.30	79.16	26.39	
11	12.01	25	300.37	0.30	90.11	30.04	
12	5.48	25	136.96	0.30	41.09	13.70	Geotextile (m2)
15	10.55	25	263.86	0.30	79.16	26.39	= 2 x (L*1 + S12) = 324
16	12.01	25	300.37	0.30	90.11	30.04	PVC pipe (m)
17	5.48	25	136.96	0.30	41.09	13.70	= 2 x L/2 x 1m = 25
FOOTING	Wood pile		(m)		3000		LENGTH OF FOOTING L = 50 (m)
	Blinding		(m3)		4		
	Stone masonry		(m3)		18.00		
	Excavation		(m3)		216.00		
SIDE SLOPE	Back fill		(m3)		150.00		
	Stone masonry		(m3)		420.71		
	Blinding		(m3)		140.24		
	Geotextle		(m2)		324		
	PVC pile		(m)		25		

No	h (m)	Dh (m)	S (m2)	L (m)	Total (m3)		
					Excavation	Back fill	
5	0.95						Excavation = 2 x S x L
6	-0.25	1.2	4.32	25	216	150	Filling = Excavation - 2 x 25 x Dh x 1.1

ABUTMENT A2:

Block	B (m)	L (m)	S (m2)	Thick (m)	Masonry (m3)	Blinding (T=0.1m) (m3)	
10	10.31	25	257.71	0.30	77.31	25.77	
11	12.01	25	300.37	0.30	90.11	30.04	
12	6.44	25	161.00	0.30	48.30	16.10	Geotextile (m2)
15	10.31	25	257.71	0.30	77.31	25.77	= 2 x (L*1 + S12) = 372
16	12.01	25	300.37	0.30	90.11	30.04	PVC pipe (m)
17	6.44	25	161.00	0.30	48.30	16.10	= 2 x L/2 x 1m = 25
FOOTING	Wood pile		(m)		3000		LENGTH OF FOOTING L = 50 (m)
	Blinding		(m3)		4		
	Stone masonry		(m3)		18.00		
	Excavation		(m3)		216.00		
SIDE SLOPE	Back fill		(m3)		150.00		
	Stone masonry		(m3)		431.45		
	Blinding		(m3)		143.82		
	Geotextle		(m2)		372		
	PVC pile		(m)		25		

QUANTITY TABLE OF PIER

Large Tra Va Bridge

Item	UNIT	Pier 1	Pier 2	Pier 3	Pier 4	Pier 5	Pier 6	Pier 7	Total (1dir)	Total (2dir)
NUMBER OF PILES	pile	6	6	6	6	6	6	6	42	84
TOTAL LENGTH OF BORED ϕ 1500m	m	420.0	420.0	420.0	420.0	444.0	444.0	420.0	2988.0	5976.0
CONCRETE DLASS D	m3	742.2	742.2	742.2	742.2	784.6	784.6	742.2	5280.2	10560.5
25	kg	4524.4	4524.4	4524.4	4524.4	4848.0	4848.0	4524.4	32318	64636.0
22	kg	418.3	418.3	418.3	418.3	380.3	380.3	418.3	2852.1	5704.2
16	kg	33.4	33.4	33.4	33.4	33.4	33.4	33.4	233.8	467.6
10	kg	1154.4	1154.4	1154.4	1154.4	1084.1	1,084.1	1154.4	7940.2	15880.4
TOTAL	kg	6130.5	6130.5	6130.5	6130.5	7130.4	7,130.4	6130.5	44913.326	89826.7
CONCRETE DLASSE	m3	212.5	210.2	217.1	217.1	195.9	193.6	210.2	1456.6	2913.2
32	kg	3576.3	3576.3	3576.3	3576.3	3715.2	3715.2	3576.3	25312.0	50624.0
28	kg	1745.1	1745.1	1745.1	1745.1	1745.1	1745.1	1745.1	12215.9	24431.7
25	kg	3077.9	2916.1	3401.6	3077.9	1912.8	1751.0	2916.1	19053.4	38106.8
22	kg	3812.4	3812.4	3812.4	3812.4	3946.6	3946.6	3812.4	26955.3	53910.7
20	kg	722.0	722.0	722.0	722.0	737.8	737.8	722.0	5085.9	10171.8
16	kg	869.1	869.1	869.1	869.1	869.1	869.1	869.1	6083.8	12167.7
14	kg	1801.7	1752.8	1915.8	1801.7	1410.5	1361.6	1752.8	11796.9	23593.7
10	kg	17.9	17.9	17.9	35.8	35.8	35.8	17.9	178.9	357.9
TOTAL	kg	15622.6	15411.8	16060.3	15640.5	14372.9	14162.1	15411.8	106682.1	213364.2
BLINDING STONE	m3	16.1	16.1	16.1	16.1			16.1		161.3
LEAN CONCRETE CLASS G	m3	8.1	8.1	8.1	8.1			8.1		80.6
EXCAVATION	m3	470.3	466.6	464.7	500.3			466.6		4736.8
BACK FILL	m3	287.2	283.6	281.8	316.5			283.6		2905.1

PIER

Large Tra Va Bridge P1

Item	Formula	Quantity
Pier 1. Concrete (Class E)	$V1 = 11.400 \times 2.000 \times 0.700 = 15.960 \text{ m}^3$ $V2 = (11.400 + 9.800) \times \frac{1}{2} \times 2.000 \times 0.700 = 14.840 \text{ m}^3$ $V3 = 0.700^2 \times \pi \times 6.000 \times 3 = 27.709 \text{ m}^3$ $V4 = 11.000 \times 7.000 \times 2.000 = 154.000 \text{ m}^3$ <p style="text-align: right;">Total (One Pier)</p>	<p style="text-align: right;">212.509 m³</p>
2. Form	$A1 = 11.400 \times 0.700 \times 2 = 15.960 \text{ m}^2$ $A2 = (11.400 + 9.800) \times \frac{1}{2} \times 0.700 \times 2 = 14.840 \text{ m}^2$ $A3 = 0.700 \times 2.000 \times 2 = 2.800 \text{ m}^2$ $A4 = (0.700^2 + 0.800^2)^{0.5} \times 2.000 \times 2 = 4.252 \text{ m}^2$ $A5 = \frac{9.800 \times 2.000 - 0.700^2 \times \pi}{3} = 14.982 \text{ m}^2$ $A6 = 1.400 \times \pi \times 6.000 \times 3 = 79.168 \text{ m}^2$ $A7 = (11.000 + 7.000) \times 2 \times 2.000 = 72.000 \text{ m}^2$	
3. Re-Bar	<p style="text-align: center;">D32 3576.3 D28 1745.1 D25 3077.9 D22 3812.4 D20 722.0 D18 0.0 D16 869.1 D14 1801.7 D12 0.0 D10 17.9 Total 15622.6</p>	

Large Tra Va Bridge P1

Item	Formula	Quantity
4. Lean Concrete	$V = 11.200 \times 7.200 \times 0.1000 =$	8.064 m3
5. Blinding Stone	$V = 11.200 \times 7.200 \times 0.2000 =$	16.128 m3
6. Excavation	Excavation Hight= 3.360	
	$V = (12.200 \times 8.200 + 15.560 \times 11.560)$ $\times \frac{1}{2} \times 3.360 =$	470.255 m3
7. Back Fill	$V = 470.255 - 16.128 - 8.064 - 11.000$	
	$\times 7.000 \times 2.000 - 0.700^2 \times \pi$	
	$\times 3 \times 1.060 =$	287.168 m3

Large Tra Va Bridge P2

Item	Formula	Quantity
Pier 1. Concrete (Class E)	$V1 = 11.400 \times 2.000 \times 0.700 = 15.960 \text{ m}^3$ $V2 = \left(\frac{11.400 + 9.800}{2} \right) \times 1/2 \times 2.000 \times 0.700 = 14.840 \text{ m}^3$ $V3 = 0.700^2 \times \pi \times 5.500 \times 3 = 25.400 \text{ m}^3$ $V4 = 11.000 \times 7.000 \times 2.000 = 154.000 \text{ m}^3$ $\text{Total (One Pier)} = 210.200 \text{ m}^3$	
2. Form	$A1 = 11.400 \times 0.700 \times 2 = 15.960 \text{ m}^2$ $A2 = \left(\frac{11.400 + 9.800}{2} \right) \times 1/2 \times 0.700 = 14.840 \text{ m}^2$ $A3 = 0.700 \times 2.000 \times 2 = 2.800 \text{ m}^2$ $A4 = \left(0.700^2 + 0.800^2 \right)^{0.5} \times 2.000 \times 2 = 4.252 \text{ m}^2$ $A5 = \frac{9.800 \times 2.000 - 0.700^2 \times \pi}{3} = 14.982 \text{ m}^2$ $A6 = 1.400 \times \pi \times 5.500 \times 3 = 72.571 \text{ m}^2$ $A7 = (11.000 + 7.000) \times 2 \times 2.000 = 72.000 \text{ m}^2$	
3. Re-Bar	D32 3576.3 D28 1745.1 D25 2916.1 D22 3812.4 D20 722.0 D18 0.0 D16 869.1 D14 1752.8 D12 0.0 D10 17.9 Total 15411.8	

Large Tra Va Bridge P2

Item	Formula	Quantity
4. Lean Concrete	$V = 11.200 \times 7.200 \times 0.1000 =$	8.064 m ³
5. Blinding Stone	$V = 11.200 \times 7.200 \times 0.2000 =$	16.128 m ³
6. Excavation	<p>Excavation Hight= 3.340</p> $V = \left(\frac{12.200 \times 8.200 + 15.540 \times 11.540}{2} \right) \times 3.340 =$	466.551 m ³
7. Back Fill	$V = 466.551 - 16.128 - 8.064 - 11.000$ $\times 7.000 \times 2.000 - 0.700^2 \times \pi$ $\times 3 \times 1.040 =$	283.556 m ³

Large Tra Va Bridge P3

Item	Formula	Quantity
Pier 1. Concrete (Class E)	$V1 = 11.400 \times 2.000 \times 0.700 =$	15.960 m3
	$V2 = (11.400 + 9.800) \times \frac{1}{2} \times 2.000 \times 0.700 =$	14.840 m3
	$V3 = 0.700^2 \times \pi \times 7.000 \times 3 =$	32.327 m3
	$V4 = 11.000 \times 7.000 \times 2.000 =$	154.000 m3
	Total (One Pier)	217.127 m3
2. Form	$A1 = 11.400 \times 0.700 \times 2 =$	15.960 m2
	$A2 = (11.400 + 9.800) \times \frac{1}{2} \times 0.700 \times 2 =$	14.840 m2
	$A3 = 0.700 \times 2.000 \times 2 =$	2.800 m2
	$A4 = (0.700^2 + 0.800^2)^{0.5} \times 2.000 \times 2 =$	4.252 m2
	$A5 = \frac{9.800 \times 2.000 - 0.700^2 \times \pi}{3} =$	14.982 m2
	$A6 = 1.400 \times \pi \times 7.000 \times 3 =$	92.363 m2
	$A7 = (11.000 + 7.000) \times 2 \times 2.000 =$	72.000 m2
3. Re-Bar	D32	3576.3
	D28	1745.1
	D25	3401.6
	D22	3812.4
	D20	722.0
	D18	0.0
	D16	869.1
	D14	1915.8
	D12	0.0
	D10	17.9
Total	16060.3	

Large Tra Va Bridge P3

Item	Formula	Quantity
4. Lean Concrete	$V = 11.200 \times 7.200 \times 0.1000 =$	8.064 m3
5. Blinding Stone	$V = 11.200 \times 7.200 \times 0.2000 =$	16.128 m3
6. Excavation	$\text{Excavation Hight} = 3.330$ $V = \left(\frac{12.200 \times 8.200 + 15.530 \times 11.530}{2} \right) \times 3.330 =$	464.703 m3
7. Back Fill	$V = 464.703 - 16.128 - 8.064 - 11.000$ $\times 7.000 \times 2.000 - 0.700^2 \times \pi$ $\times 3 \times 1.030 =$	281.754 m3

LIST OF REINFORCEMENT

SIGN	DIACETER	LENGTH	NOS.	UNIT WEIGHT	PIECE WEIGHT	TOTAL WEIGHT	NOTE
R	1	D10	5,800	5	0.617	3.579	17.9
B	1	D28	11,200	13	4.834	54.141	703.8
	2	D28	9,800	13	4.834	47.373	615.9
	3	D16	11,200	6	1.578	17.674	106.0
	3 -a	D16	10,490	4	1.578	16.553	66.2
	4	D14	4,916	49	1.208	5.939	291.0
	4 -a	D14	3,566	98	1.208	4.308	422.2
	4 -b	D14	2,332	49	1.208	2.817	138.0
	5	D14	4,157	6	1.208	5.022	30.1
	5 -a	D14	2,728	12	1.208	3.295	39.5
	5 -b	D14	2,332	6	1.208	2.817	16.9
	6	D28	3,393	13	4.834	16.402	213.2
	7	D28	3,377	13	4.834	16.324	212.2
C	1	D25	10,510	84	3.853	40.495	3,401.6
	2	D14	4,498	180	1.208	5.434	978.0
F	1	D22	11,570	25	2.984	34.525	863.1
	2	D22	15,070	47	2.984	44.969	2,113.5
	3	D32	10,300	55	6.313	65.024	3,576.3
	4	D22	7,570	37	2.984	22.589	835.8
	5	D20	7,500	16	2.466	18.495	295.9
	6	D20	10,800	16	2.466	26.633	426.1
	7	D16	4,698	94	1.578	7.413	696.9
					TOTAL	16,060.3	
					D32	3576.3	
					D28	1745.1	
					D25	3401.6	
					D22	3812.4	
					D20	722.0	
					D18	0.0	
					D16	869.1	
					D14	1915.8	
					D12	0.0	
					D10	17.9	
						16060.3	

Large Tra Va Bridge P4

Item	Formula	Quantity
Pier 1. Concrete (Class E)		
	$V1 = 11.400 \times 2.000 \times 0.700 =$	15.960 m3
	$V2 = (11.400 + 9.800) \times \frac{1}{2} \times 2.000 \times 0.700 =$	14.840 m3
	$V3 = 0.700^2 \times \pi \times 6.000 \times 3 =$	27.709 m3
	$V4 = 11.000 \times 7.000 \times 2.000 =$	154.000 m3
	Total (One Pier)	212.509 m3
2. Form		
	$A1 = 11.400 \times 0.700 \times 2 =$	15.960 m2
	$A2 = (11.400 + 9.800) \times \frac{1}{2} \times 0.700 \times 2 =$	14.840 m2
	$A3 = 0.700 \times 2.000 \times 2 =$	2.800 m2
	$A4 = (0.700^2 + 0.800^2)^{0.5} \times 2.000 \times 2 =$	4.252 m2
	$A5 = \frac{9.800 \times 2.000 - 0.700^2 \times \pi}{3} =$	14.982 m2
	$A6 = 1.400 \times \pi \times 6.000 \times 3 =$	79.168 m2
	$A7 = (11.000 + 7.000) \times 2 \times 2.000 =$	72.000 m2
3. Re-Bar		
	D32 3576.3	
	D28 1745.1	
	D25 3077.9	
	D22 3812.4	
	D20 722.0	
	D18 0.0	
	D16 869.1	
	D14 1801.7	
	D12 0.0	
	D10 35.8	
	Total 15640.5	

Large Tra Va Bridge P4

Item	Formula	Quantity
4. Lean Concrete	$V = 11.200 \times 7.200 \times 0.1000 =$	8.064 m3
5. Blinding Stone	$V = 11.200 \times 7.200 \times 0.2000 =$	16.128 m3
6. Excavation	<p>Excavation Hight= 3.520</p> $V = (12.200 \times 8.200 + 15.720 \times 11.720) \times \frac{1}{2} \times 3.520 =$	500.330 m3
7. Back Fill	$V = 500.330 - 16.128 - 8.064 - 11.000 \times 7.000 \times 2.000 - 0.700^2 \times \pi \times 3 \times 1.220 =$	316.504 m3

Large Tra Va Bridge P5

Item	Formula	Quantity
Pier 1. Concrete (Class E)	$V1 = 11.400 \times 2.000 \times 0.700 = 15.960 \text{ m}^3$ $V2 = \left(\frac{11.400 + 9.800}{2} \right) \times 1/2 \times 2.000 \times 0.700 = 14.840 \text{ m}^3$ $V3 = 0.700^2 \times \pi \times 2.400 \times 3 = 11.084 \text{ m}^3$ $V4 = 11.000 \times 7.000 \times 2.000 = 154.000 \text{ m}^3$ <p style="text-align: right;">Total (One Pier)</p>	<p style="text-align: right;">195.884 m³</p>
2. Form	$A1 = 11.400 \times 0.700 \times 2 = 15.960 \text{ m}^2$ $A2 = \left(\frac{11.400 + 9.800}{2} \right) \times 1/2 \times 0.700 = 14.840 \text{ m}^2$ $A3 = 0.700 \times 2.000 \times 2 = 2.800 \text{ m}^2$ $A4 = \left(0.700^2 + 0.800^2 \right)^{0.5} \times 2.000 \times 2 = 4.252 \text{ m}^2$ $A5 = \frac{9.800 \times 2.000 - 0.700^2 \times \pi}{3} = 14.982 \text{ m}^2$ $A6 = 1.400 \times \pi \times 2.400 \times 3 = 31.667 \text{ m}^2$ $A7 = (11.000 + 7.000) \times 2 \times 2.000 = 72.000 \text{ m}^2$	
3. Re-Bar	<p style="text-align: center;">D32 3715.2 D28 1745.1 D25 1912.8 D22 3946.6 D20 737.8 D18 0.0 D16 869.1 D14 1410.5 D12 0.0 D10 35.8 Total 14372.9</p>	

Large Tra Va Bridge P6

Item	Formula	Quantity
Pier 1. Concrete (Class E)		
	$V1 = 11.400 \times 2.000 \times 0.700 =$	15.960 m ³
	$V2 = \left(\frac{11.400 + 9.800}{2} \right) \times 1/2 \times 2.000 \times 0.700 =$	14.840 m ³
	$V3 = 0.700^2 \times \pi \times 1.900 \times 3 =$	8.774 m ³
	$V4 = 11.000 \times 7.000 \times 2.000 =$	154.000 m ³
	Total (One Pier)	193.574 m ³
2. Form		
	$A1 = 11.400 \times 0.700 \times 2 =$	15.960 m ²
	$A2 = \left(\frac{11.400 + 9.800}{2} \right) \times 1/2 \times 0.700 =$	14.840 m ²
	$A3 = 0.700 \times 2.000 \times 2 =$	2.800 m ²
	$A4 = \left(0.700^2 + 0.800^2 \right)^{0.5} \times 2.000 \times 2 =$	4.252 m ²
	$A5 = \frac{9.800 \times 2.000 - 0.700^2 \times \pi}{3} =$	14.982 m ²
	$A6 = 1.400 \times \pi \times 1.900 \times 3 =$	25.070 m ²
	$A7 = \left(11.000 + 7.000 \right) \times 2 \times 2.000 =$	72.000 m ²
3. Re-Bar		
	D32 3715.2	
	D28 1745.1	
	D25 1751.0	
	D22 3946.6	
	D20 737.8	
	D18 0.0	
	D16 869.1	
	D14 1361.6	
	D12 0.0	
	D10 35.8	
	Total 14162.1	

LIST OF REINFORCEMENT

P6

SIGN	DIACETER	LENGTH	NOS.	UNIT WEIGHT	PIECE WEIGHT	TOTAL WEIGHT	NOTE
R	1	D10	5,800	10	0.617	3.579	35.8
B	1	D28	11,200	13	4.834	54.141	703.8
	2	D28	9,800	13	4.834	47.373	615.9
	3	D16	11,200	6	1.578	17.674	106.0
	3 -a	D16	10,490	4	1.578	16.553	66.2
	4	D14	4,916	49	1.208	5.939	291.0
	4 -a	D14	3,566	98	1.208	4.308	422.2
	4 -b	D14	2,332	49	1.208	2.817	138.0
	5	D14	4,157	6	1.208	5.022	30.1
	5 -a	D14	2,728	12	1.208	3.295	39.5
	5 -b	D14	2,332	6	1.208	2.817	16.9
	6	D28	3,393	13	4.834	16.402	213.2
	7	D28	3,377	13	4.834	16.324	212.2
C	1	D25	5,410	84	3.853	20.845	1,751.0
	2	D14	4,498	78	1.208	5.434	423.8
F	1	D22	11,570	25	2.984	34.525	863.1
	2	D22	15,070	49	2.984	44.969	2,203.5
	3	D32	10,700	55	6.313	67.549	3,715.2
	4	D22	7,970	37	2.984	23.782	880.0
	5	D20	7,900	16	2.466	19.481	311.7
	6	D20	10,800	16	2.466	26.633	426.1
	7	D16	4,698	94	1.578	7.413	696.9
					TOTAL	14,162.1	
					D32	3715.2	
					D28	1745.1	
					D25	1751.0	
					D22	3946.6	
					D20	737.8	
					D18	0.0	
					D16	869.1	
					D14	1361.6	
					D12	0.0	
					D10	35.8	
						14162.1	

Large Tra Va Bridge P7

Item	Formula	Quantity
Pier 1. Concrete (Class E)	$V1 = 11.400 \times 2.000 \times 0.700 = 15.960 \text{ m}^3$ $V2 = \left(\frac{11.400 + 9.800}{2} \right) \times 1/2 \times 2.000 \times 0.700 = 14.840 \text{ m}^3$ $V3 = 0.700^2 \times \pi \times 5.500 \times 3 = 25.400 \text{ m}^3$ $V4 = 11.000 \times 7.000 \times 2.000 = 154.000 \text{ m}^3$ <p style="text-align: right;">Total (One Pier)</p>	<p style="text-align: right;">210.200 m³</p>
2. Form	$A1 = 11.400 \times 0.700 \times 2 = 15.960 \text{ m}^2$ $A2 = \left(\frac{11.400 + 9.800}{2} \right) \times 1/2 \times 0.700 = 14.840 \text{ m}^2$ $A3 = 0.700 \times 2.000 \times 2 = 2.800 \text{ m}^2$ $A4 = \left(0.700^2 + 0.800^2 \right)^{0.5} \times 2.000 \times 2 = 4.252 \text{ m}^2$ $A5 = \frac{9.800 \times 2.000}{3} - 0.700^2 \times \pi = 14.982 \text{ m}^2$ $A6 = 1.400 \times \pi \times 5.500 \times 3 = 72.571 \text{ m}^2$ $A7 = (11.000 + 7.000) \times 2 \times 2.000 = 72.000 \text{ m}^2$	
3. Re-Bar	<p style="text-align: center;">D32 3576.3 D28 1745.1 D25 2916.1 D22 3812.4 D20 722.0 D18 0.0 D16 869.1 D14 1752.8 D12 0.0 D10 17.9 Total 15411.8</p>	

Large Tra Va Bridge P7

Item	Formula	Quantity
4. Lean Concrete	$V = 11.200 \times 7.200 \times 0.1000 =$	8.064 m3
5. Blinding Stone	$V = 11.200 \times 7.200 \times 0.2000 =$	16.128 m3
6. Excavation	Excavation Hight= 3.340	
	$V = (12.200 \times 8.200 + 15.540 \times 11.540) \times \frac{1}{2} \times 3.340 =$	466.551 m3
7. Back Fill	$V = 466.551 - 16.128 - 8.064 - 11.000$	
	$\times 7.000 \times 2.000 - 0.700^2 \times \pi$	
	$\times 3 \times 1.040 =$	283.556 m3

LIST OF REINFORCEMENT

P7

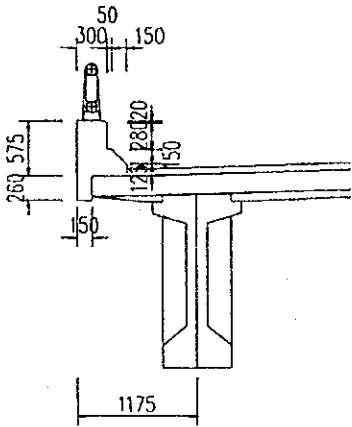
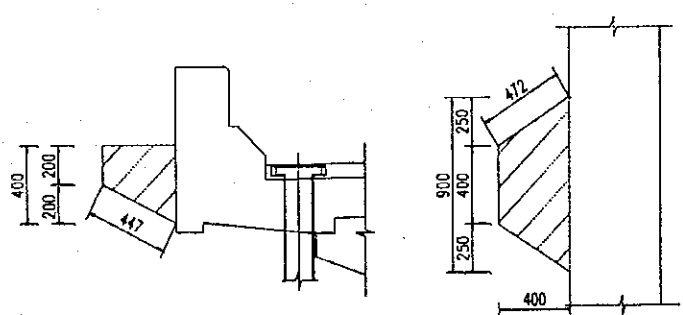
SIGN	DIACETER	LENGTH	NOS.	UNIT WEIGHT	PIECE WEIGHT	TOTAL WEIGHT	NOTE
R	1	D10	5,800	5	0.617	3.579	17.9
B	1	D28	11,200	13	4.834	54.141	703.8
	2	D28	9,800	13	4.834	47.373	615.9
	3	D16	11,200	6	1.578	17.674	106.0
	3 -a	D16	10,490	4	1.578	16.553	66.2
	4	D14	4,916	49	1.208	5.939	291.0
	4 -a	D14	3,566	98	1.208	4.308	422.2
	4 -b	D14	2,332	49	1.208	2.817	138.0
	5	D14	4,157	6	1.208	5.022	30.1
	5 -a	D14	2,728	12	1.208	3.295	39.5
	5 -b	D14	2,332	6	1.208	2.817	16.9
	6	D28	3,393	13	4.834	16.402	213.2
	7	D28	3,377	13	4.834	16.324	212.2
C	1	D25	9,010	84	3.853	34.716	2,916.1
	2	D14	4,498	150	1.208	5.434	815.0
F	1	D22	11,570	25	2.984	34.525	863.1
	2	D22	15,070	47	2.984	44.969	2,113.5
	3	D32	10,300	55	6.313	65.024	3,576.3
	4	D22	7,570	37	2.984	22.589	835.8
	5	D20	7,500	16	2.466	18.495	295.9
	6	D20	10,800	16	2.466	26.633	426.1
	7	D16	4,698	94	1.578	7.413	696.9
					TOTAL	15,411.8	
					D32	3576.3	
					D28	1745.1	
					D25	2916.1	
					D22	3812.4	
					D20	722.0	
					D18	0.0	
					D16	869.1	
					D14	1752.8	
					D12	0.0	
					D10	17.9	
						15411.8	

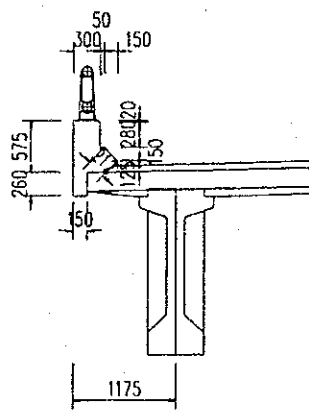
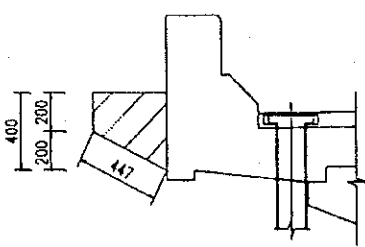
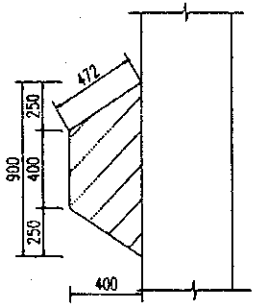
Large Tra Va Bridge

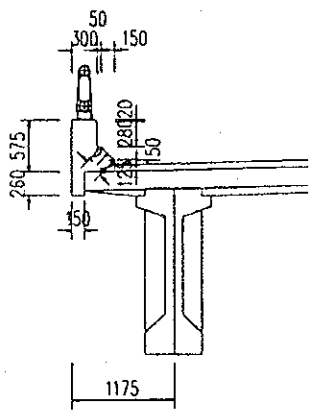
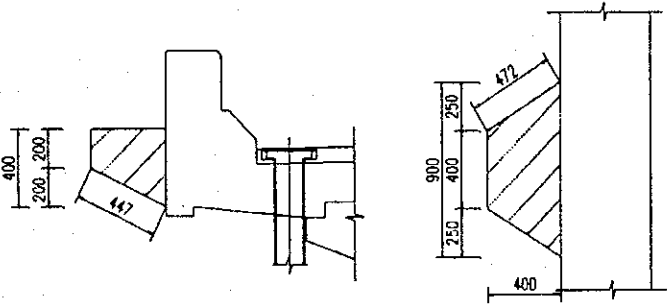
Item	Formula	Quantity
Pile	P1,P2,P3,P4,P7	
1) Number of Pile		
	$N = 6 = 6$	6 Pill
2) Total Length of Bored Piles ϕ 1500		
	$L = 6 \times 70.000 = 420.0$	420.0 m
3) Concrete Class D		
	$V = 0.750^2 \times \pi \times 420.0 = 742.201$	742.2 m ³
4) Reiforcemento		
	D10 1154.4	
	D16 33.4	
	D22 418.3	
	D25 4524.4	
	Total 6130.5	
Pile	P5,P6	
1) Number of Pile		
	$N = 6 = 6$	6 Pill
2) Total Length of Bored Piles ϕ 1500		
	$L = 6 \times 74.000 = 444.0$	444.0 m
3) Concrete Class D		
	$V = 0.750^2 \times \pi \times 444.0 = 784.613$	784.6 m ³
4) Reiforcemento		
	D10 1084.1	
	D16 33.4	
	D22 380.3	
	D25 4848.0	
	Total 6345.8	

QUANTITY OF MISCELLANEOUS

Item		Work Item	Unit	Quantity	Remarks
Concrete	CLASS E	Parapet	m3	1150.05	
		Lighting pole base	m3	0.70	
Form		Parapet	m2	7217.97	
		Lighting pole base	m2	4.88	
		Total		7222.86	
Re-bar		Parapet	ton	44.604	
		Lighting pole base	ton	0.252	
		Total	ton	44.86	
Steel Railing			m	1149.40	
Lighting	Pole		set	8	
	Pipe Φ100		m	1149.40	
Drainage	Pot		set	38	
	Pipe Φ180		m	63.22	

Item	Formula	Quantity
<p>1. Parapet CLASS "E"</p>	 $A = \left[(0.300 \times 0.575 + 0.260 \times 0.150 + (0.125 + 0.275) \times 0.150 \times \frac{1}{2} + 0.275 \times 0.050 \right] \times 2 \times 2 = 1.021 \text{ m}^2$ $V = 1.021 \times 281.600 \times 4 = 1,150.05 \text{ m}^3$	<p>1,150.05 m3</p>
<p>2. Lighting CLASS "E"</p>	 $V = (0.200 \times 0.400 + 0.400 \times 0.900) \times \frac{1}{2} \times 0.4 \times \frac{1}{8} = 0.704 \text{ m}^3$	<p>0.704 m3</p>

Item	Formula	Quantity
<p>1. Parapet</p>	<div style="text-align: center;">  </div> $A = \frac{(0.575 + 0.260 + 0.15 + 0.125 + 0.212 + 0.280) \times 2 \times 2 \times 281.600 \times 4}{1} = 7217.97 \text{ m}^2$	<p>7217.97 m2</p>
<p>2. Lighting</p>	<div style="display: flex; justify-content: space-around;"> <div style="text-align: center;">  </div> <div style="text-align: center;">  </div> </div> $A = \left[\frac{(0.900 + 0.400) \times 0.447 \times 1/2}{0.4 \times 1/2 \times 2} + \frac{(0.400 + 0.200)}{0.2 \times 0.400} \right] \times 8 = 4.884 \text{ m}^2$	<p>4.884 m2</p>

Item	Formula	Quantity
<p>1. Parapet</p>	 $A = \frac{(0.575 + 0.260 + 0.15 + 0.125 + 0.212 + 0.280) \times 2 \times 2 \times 281.600}{2} = 1804.49 \text{ m}^2$	<p>1804.49 m²</p>
<p>2. Lighting</p>	 $A = \frac{((0.900 + 0.400) \times 0.447 \times \frac{1}{2} + (0.400 + 0.200) \times 0.4 \times \frac{1}{2}) \times 2 \times 0.2 \times 8}{8} = 4.884 \text{ m}^2$	<p>4.884 m²</p>

Item	Formula	Quantity
1. Drainage	<p>TOTAL EACH</p> <p>EACH = 19 x 2 = 38</p> <p>PIPE</p> <p>L1 = 0.98 x 34 = 33.32</p> <p>L2 = 23.70 + 6.20 = 29.90</p> <p>Total 63.22 m</p>	<p>38</p> <p>EACH</p> <p>63.22 m</p>
2. Steel railing	<p>L = 281.600 x 4 = 1126.400</p> <p>Length for abutment</p> <p>L = 5.75 x 4 = 23.00 m</p> <p>TOTAL LENGTH</p> <p>L = 281.600 x 1 x 2 x 2 + 23.000 = 1149.400 m</p>	<p>1126.400</p> <p>23.00 m</p> <p>1149.400 m</p>
3. Lighting	<p>Each for one span</p> <p>Each = 8</p> <p>Total lighting poles</p> <p>8 x 1 = 8 pole</p> <p>PVC pipe Φ100mm = 1149.400 m</p>	<p>8 pole</p> <p>1149.400 m</p>

Re-bar Parapet

No. _____

Per 10m

SCHEDULE OF REINFORCEMENT (OF RAILING)													
BAR MARK	SIZE (mm)	DIMENSIONS (mm)							LENGTH (mm)	UNIT WEIGHT (kg/m)	NO. OF BARS	WEIGHT (kg)	REMARKS
		a	b	c	d	e	f	g					
P1	14	250	701	212	747	70			1980	1.208	68	162.65	
P2	14	230	297	176	275	245			1223	1.208	68	100.46	
P3	14	10000							10000	1.208	11	132.88	
Total												395.99	

Total Weight

$$\text{Total} = 395.99 / 10 \times 281.6 \times 4 = 44,604.31 \text{ (kg)}$$

Re-bar Lighting

No. _____

Per BRIDGE

SCHEDULE OF REINFORCEMENT (OF LIGHTING)													
BAR MARK	SIZE (mm)	DIMENSIONS (mm)							LENGTH (mm)	UNIT WEIGHT (kg/m)	NO. OF BARS	WEIGHT (kg)	REMARKS
		a	b	c	d	e	f	g					
P1	20	113	825	320					1258	2.466	3	9.31	
P2	16	113	380	488	280				1261	1.578	3	5.97	
P3	20	1000	300	1000					2300	2.466	2	11.34	
P4	16	282	285	282					849	1.578	2	2.68	
P5	16	208	285	208					701	1.578	2	2.21	
Total												31.51	

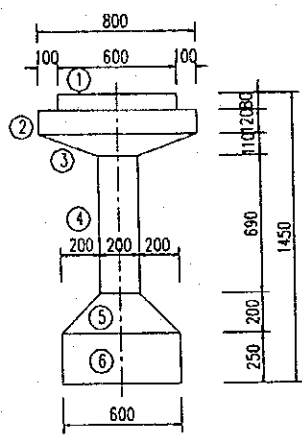
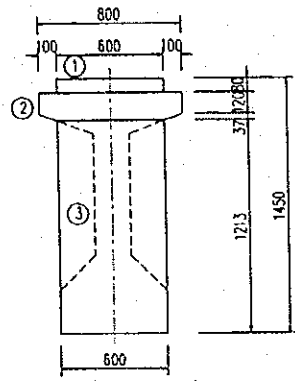
Total Weight

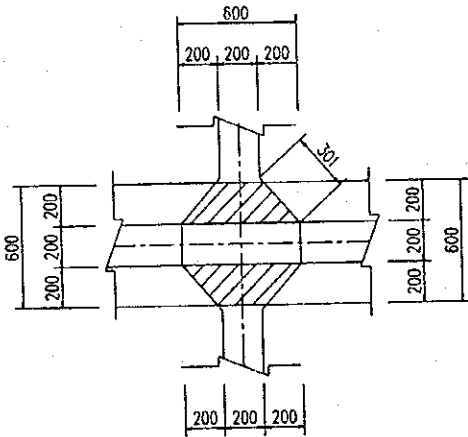
Total = 31.51 × 8 = 252.10 (kg)

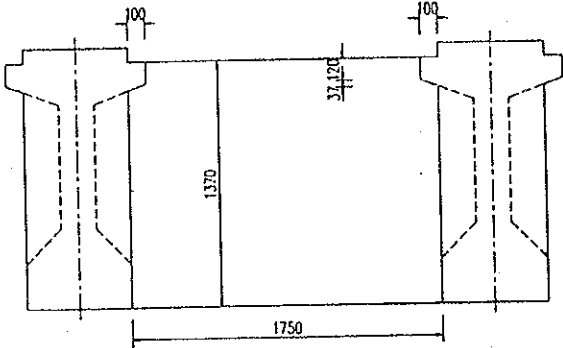
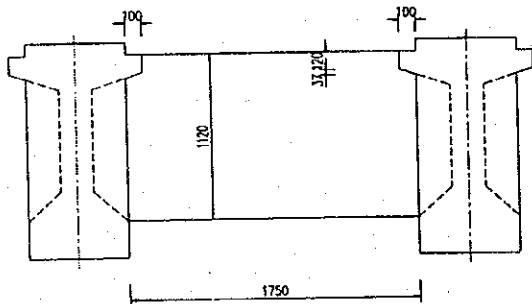
3.2 Small Tra Va Bridge

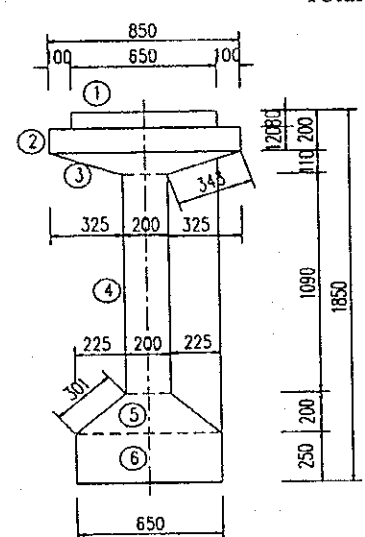
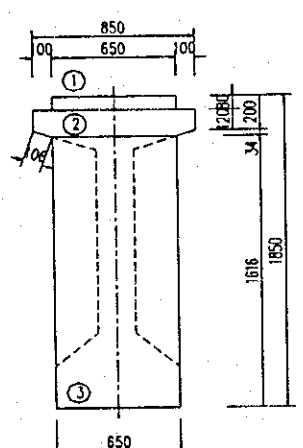
Quantity of Superstructure

Item		Work Item	Unit	Quantity	Remarks	
Concrete	CLASS B	Girder	m3	609.8	ock=400kgf/cm2	
	CLASS D	Panel	m3	96.3		
		Deck Slab	m3	419.6		
		Diaphragm	m3	129.9		
		Total		645.8		
	CLASS E	Parapet	m3	89.3	ock=300kgf/cm2	
		Lighting	m3	0.2		
		Total		89.5		
	Form		Parapet	m2	330.4	
			Lighting	m2	1.3	
		Girder	m2	3,867.6		
		Diaphragm	m2	495.1		
		Deck Slab	m2	286.9		
		Panel	m2	1,507.1		
		Total		6,488.4		
Scaffolding		Platform of construction	m2	2,114.1		
Re-bar		D25	kg	5,987.6		
		D22	kg	29,795.9		
		D20	kg	60,042.1		
		D16	kg	7,876.1		
		D14	kg	137,061.3		
		D12	kg	7,312.6		
		D10	kg	316.9		
		D6	kg	11,550.7		
	Total	kg	259,943.20			
PC Cable	12S12.7B		ton	30.87	SWPR7B	
	3S12.7B		ton	1.68		
Anchor	12S12.7B		set	220.00		
	3S12.7B		set	144.00		
Steel shear key			set	240.00		
Sheathing	φ80/85		m	3322.92		
	φ50/55		m	723.82		
Cement grout in sheathing			m3	18.11		
Expansion Joint			m	43.00		
Bearing 500x250x50			set	40.00		
Bearing 600x300x57			set	20.00		
Anchorage Bar			set	48.00		
Steel Bridge Railing			m	367.6		
Lighting pole	pole		set	2.0		
	pipe P100		m	350.0		
Pavement	70mm		m2	1,862		
Water Proofing	5mm		m2	1,862		
Drainage	pot		set	14.0		
	Pipe P165		m	24.4		

Item	Formula	Quantity																																	
1. Girder CLASS "B" 1) Girder	<p>(1) Calculation of Sectional Area</p> <p>[1] Middle Section Summary of Sectional Length For ONE GIRDER</p> <table border="1" data-bbox="351 436 1125 705"> <thead> <tr> <th>No</th> <th>Formula</th> <th>(m²)</th> </tr> </thead> <tbody> <tr> <td>1</td> <td>0.600×0.080</td> <td>= 0.048</td> </tr> <tr> <td>2</td> <td>0.800×0.120</td> <td>= 0.096</td> </tr> <tr> <td>3</td> <td>$1\sqrt{2} \times 1.000 \times 0.110$</td> <td>= 0.055</td> </tr> <tr> <td>4</td> <td>0.690×0.200</td> <td>= 0.138</td> </tr> <tr> <td>5</td> <td>$1\sqrt{2} \times 0.800 \times 0.200$</td> <td>= 0.080</td> </tr> <tr> <td>6</td> <td>0.250×0.600</td> <td>= 0.150</td> </tr> </tbody> </table> <p style="text-align: right;">Total Area 0.567 m²</p>  <p>[2] End Section Summary of Sectional Length For ONE GIRDER</p> <table border="1" data-bbox="359 1332 1141 1500"> <thead> <tr> <th>No</th> <th>Formula</th> <th>(m²)</th> </tr> </thead> <tbody> <tr> <td>1</td> <td>0.600×0.080</td> <td>= 0.048</td> </tr> <tr> <td>2</td> <td>$0.800 \times 0.120 + 0.034\sqrt{2} \times 1.400$</td> <td>= 0.122</td> </tr> <tr> <td>3</td> <td>0.600×1.213</td> <td>= 0.728</td> </tr> </tbody> </table> <p style="text-align: right;">Total Area 0.898 m²</p> 	No	Formula	(m ²)	1	0.600×0.080	= 0.048	2	0.800×0.120	= 0.096	3	$1\sqrt{2} \times 1.000 \times 0.110$	= 0.055	4	0.690×0.200	= 0.138	5	$1\sqrt{2} \times 0.800 \times 0.200$	= 0.080	6	0.250×0.600	= 0.150	No	Formula	(m ²)	1	0.600×0.080	= 0.048	2	$0.800 \times 0.120 + 0.034\sqrt{2} \times 1.400$	= 0.122	3	0.600×1.213	= 0.728	
No	Formula	(m ²)																																	
1	0.600×0.080	= 0.048																																	
2	0.800×0.120	= 0.096																																	
3	$1\sqrt{2} \times 1.000 \times 0.110$	= 0.055																																	
4	0.690×0.200	= 0.138																																	
5	$1\sqrt{2} \times 0.800 \times 0.200$	= 0.080																																	
6	0.250×0.600	= 0.150																																	
No	Formula	(m ²)																																	
1	0.600×0.080	= 0.048																																	
2	$0.800 \times 0.120 + 0.034\sqrt{2} \times 1.400$	= 0.122																																	
3	0.600×1.213	= 0.728																																	

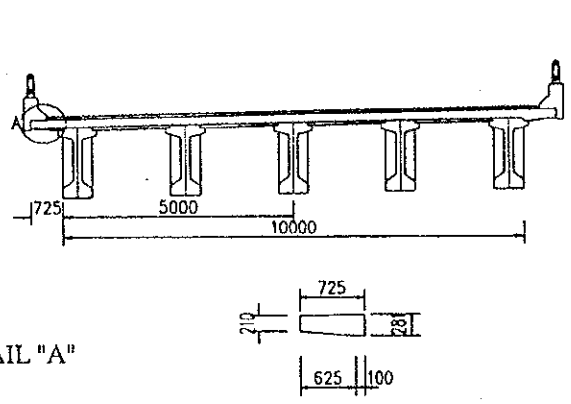
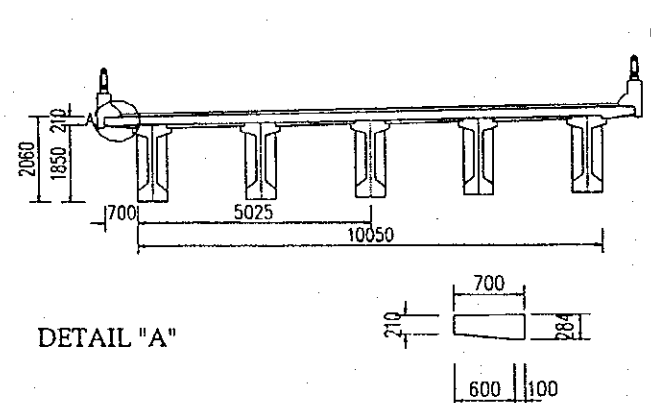
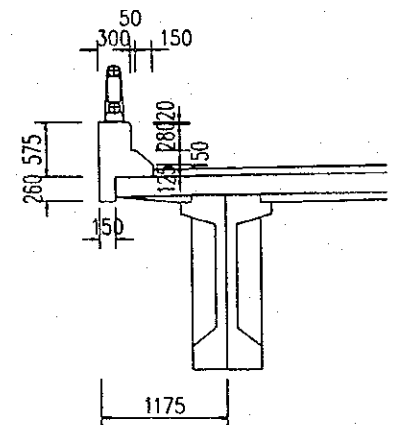
Item	Formula	Quantity																																																
	<p>(2) Calculation of Form Area</p> <p>[1] A1-A2 Girder Length 25.000 m</p> <table border="1" style="width:100%; border-collapse: collapse; margin-bottom: 10px;"> <thead> <tr> <th>Section No</th> <th>Section Area (m)</th> <th>Average of Section (m2)</th> <th>Length of Block (m)</th> <th>Concrete Volume (m3)</th> <th>Remark</th> </tr> </thead> <tbody> <tr> <td>END</td> <td>0.898</td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>END</td> <td>0.898</td> <td>0.898</td> <td>0.450</td> <td>0.404</td> <td></td> </tr> <tr> <td>MIDDLE</td> <td>0.567</td> <td>0.733</td> <td>3.500</td> <td>2.566</td> <td></td> </tr> <tr> <td>MIDDLE</td> <td>0.567</td> <td>0.567</td> <td>17.100</td> <td>9.696</td> <td></td> </tr> <tr> <td>END</td> <td>0.898</td> <td>0.733</td> <td>3.500</td> <td>2.566</td> <td></td> </tr> <tr> <td>END</td> <td>0.898</td> <td>0.898</td> <td>0.450</td> <td>0.404</td> <td></td> </tr> <tr> <td>Total</td> <td></td> <td></td> <td>25</td> <td>15.636</td> <td></td> </tr> </tbody> </table> <p style="text-align: center;">No. of Girder 10 × 2 = 20</p> <p>V1= 15.636 × 20 = 312.720 m3</p>	Section No	Section Area (m)	Average of Section (m2)	Length of Block (m)	Concrete Volume (m3)	Remark	END	0.898					END	0.898	0.898	0.450	0.404		MIDDLE	0.567	0.733	3.500	2.566		MIDDLE	0.567	0.567	17.100	9.696		END	0.898	0.733	3.500	2.566		END	0.898	0.898	0.450	0.404		Total			25	15.636		
Section No	Section Area (m)	Average of Section (m2)	Length of Block (m)	Concrete Volume (m3)	Remark																																													
END	0.898																																																	
END	0.898	0.898	0.450	0.404																																														
MIDDLE	0.567	0.733	3.500	2.566																																														
MIDDLE	0.567	0.567	17.100	9.696																																														
END	0.898	0.733	3.500	2.566																																														
END	0.898	0.898	0.450	0.404																																														
Total			25	15.636																																														
2) Diaphragm	 <p style="text-align: center;"> $V2 = \left(\frac{0.200 \times 0.963}{2} + \frac{0.600 \times 0.690}{2} \right) \times \frac{1}{2} \times 0.2 = 4.853 \text{ m}^3$ </p> <p style="text-align: center;"> Total V = 312.720 + 4.853 = 317.573 m3 </p>	<p>317.573 m3</p>																																																

Item	Formula	Quantity
2. Diaphragm CLASS "D"	<p>(1) End Cross Beam</p>  $V1 = \left\{ \frac{1.750 \times 1.370}{2} - \frac{(0.120 + 0.157) \times 0.100}{2} \right\} \times 0.450 \times 2 \times 8 = 17.063 \text{ m}^3$ <p>(2) Intermediate Cross Beam</p>  $V2 = \left[\left\{ \frac{1.750 \times 1.120}{2} - \frac{(0.120 + 0.157) \times 0.100}{2} \right\} \times 0.200 \times 2 \times 8 \right] = 12.367 \text{ m}^3$ $V = 17.063 + 12.367 = 29.430 \text{ m}^3$	<p>29.430 m³</p>

Item	Formula	Quantity																																	
1. Girder CLASS "B" 1) Girder	<p>(1) Calculation of Sectional Area</p> <p>[1] Middle Section Summary of Sectional Area For ONE GIRDER</p> <table border="1" style="width: 100%; border-collapse: collapse;"> <thead> <tr> <th style="width: 5%;">No</th> <th style="width: 70%;">Formula</th> <th style="width: 25%;">(m²)</th> </tr> </thead> <tbody> <tr> <td>1</td> <td>0.650×0.080</td> <td>= 0.052</td> </tr> <tr> <td>2</td> <td>0.850×0.120</td> <td>= 0.102</td> </tr> <tr> <td>3</td> <td>$1\sqrt{2} \times 1.050 \times 0.110$</td> <td>= 0.058</td> </tr> <tr> <td>4</td> <td>1.090×0.200</td> <td>= 0.218</td> </tr> <tr> <td>5</td> <td>$1\sqrt{2} \times 0.850 \times 0.200$</td> <td>= 0.085</td> </tr> <tr> <td>6</td> <td>0.250×0.650</td> <td>= 0.163</td> </tr> </tbody> </table> <p style="text-align: right;">Total Area 0.678 m²</p>  <p>[2] End Section Summary of Sectional Area For ONE GIRDER</p> <table border="1" style="width: 100%; border-collapse: collapse;"> <thead> <tr> <th style="width: 5%;">No</th> <th style="width: 70%;">Formula</th> <th style="width: 25%;">(m²)</th> </tr> </thead> <tbody> <tr> <td>1</td> <td>0.650×0.080</td> <td>= 0.052</td> </tr> <tr> <td>2</td> <td>$0.850 \times 0.120 + 0.034\sqrt{2} \times 1.500$</td> <td>= 0.128</td> </tr> <tr> <td>3</td> <td>0.650×1.616</td> <td>= 1.050</td> </tr> </tbody> </table> <p style="text-align: right;">Total Area 1.230 m²</p> 	No	Formula	(m ²)	1	0.650×0.080	= 0.052	2	0.850×0.120	= 0.102	3	$1\sqrt{2} \times 1.050 \times 0.110$	= 0.058	4	1.090×0.200	= 0.218	5	$1\sqrt{2} \times 0.850 \times 0.200$	= 0.085	6	0.250×0.650	= 0.163	No	Formula	(m ²)	1	0.650×0.080	= 0.052	2	$0.850 \times 0.120 + 0.034\sqrt{2} \times 1.500$	= 0.128	3	0.650×1.616	= 1.050	
No	Formula	(m ²)																																	
1	0.650×0.080	= 0.052																																	
2	0.850×0.120	= 0.102																																	
3	$1\sqrt{2} \times 1.050 \times 0.110$	= 0.058																																	
4	1.090×0.200	= 0.218																																	
5	$1\sqrt{2} \times 0.850 \times 0.200$	= 0.085																																	
6	0.250×0.650	= 0.163																																	
No	Formula	(m ²)																																	
1	0.650×0.080	= 0.052																																	
2	$0.850 \times 0.120 + 0.034\sqrt{2} \times 1.500$	= 0.128																																	
3	0.650×1.616	= 1.050																																	

Item	Formula	Quantity																																																
	(2) Calculation of Concrete Volume																																																	
	<p>[1] A1-A2 Girder Length 37.000 m</p> <table border="1" style="width:100%; border-collapse: collapse; margin-bottom: 10px;"> <thead> <tr> <th>Section No</th> <th>Section Area (m²)</th> <th>Average of Section (m²)</th> <th>Length of Block (m)</th> <th>Concrete Volume (m³)</th> <th>Remark</th> </tr> </thead> <tbody> <tr> <td>END</td> <td>1.23</td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>END</td> <td>1.23</td> <td>1.23</td> <td>0.500</td> <td>0.615</td> <td></td> </tr> <tr> <td>MIDDLE</td> <td>0.678</td> <td>0.954</td> <td>5.300</td> <td>5.056</td> <td></td> </tr> <tr> <td>MIDDLE</td> <td>0.678</td> <td>0.678</td> <td>25.400</td> <td>17.221</td> <td></td> </tr> <tr> <td>END</td> <td>1.23</td> <td>0.954</td> <td>5.300</td> <td>5.056</td> <td></td> </tr> <tr> <td>END</td> <td>1.23</td> <td>1.23</td> <td>0.500</td> <td>0.615</td> <td></td> </tr> <tr> <td>Total</td> <td></td> <td></td> <td>37</td> <td>28.563</td> <td></td> </tr> </tbody> </table> <p style="text-align: center;">No. of Girder 10 × 1 = 10</p> <p>V1= 28.563 × 10 = 285.630 m³</p>		Section No	Section Area (m ²)	Average of Section (m ²)	Length of Block (m)	Concrete Volume (m ³)	Remark	END	1.23					END	1.23	1.23	0.500	0.615		MIDDLE	0.678	0.954	5.300	5.056		MIDDLE	0.678	0.678	25.400	17.221		END	1.23	0.954	5.300	5.056		END	1.23	1.23	0.500	0.615		Total			37	28.563	
Section No	Section Area (m ²)	Average of Section (m ²)	Length of Block (m)	Concrete Volume (m ³)	Remark																																													
END	1.23																																																	
END	1.23	1.23	0.500	0.615																																														
MIDDLE	0.678	0.954	5.300	5.056																																														
MIDDLE	0.678	0.678	25.400	17.221																																														
END	1.23	0.954	5.300	5.056																																														
END	1.23	1.23	0.500	0.615																																														
Total			37	28.563																																														
2) Diaphragm	<div style="text-align: center;"> </div> <p style="margin-top: 20px;">V2= (0.200 × 1.366 + 0.65 × 1.090) × 1/2 × 0.225 × 2 × 3 × 10 × 1 = 6.626 m³</p> <p>Total V = 285.630 + 6.626 = 292.256 292.256 m³</p>																																																	

Item	Formula	Quantity
2. Diaphragm CLASS "D"	(1) Intermediate Cross Beam	
	$V1 = [\{ 1.700 \times 1.520 - (0.120 + 0.154) \times \frac{2}{2} \times 0.100 \} \times 0.200 \times 1 \times 3 \times 8 = 12.272 \text{ m}^3$	
	(2) Connection Cross Beam	
	$V3 = [\{ 1.770 \times 1.700 - (0.120 + 0.154) \times \frac{2}{2} \times 0.100 \} \times 0.900 + \{ 1.750 \times 1.370 - (0.120 + 0.157) \times \frac{2}{2} \times 0.100 \} \times 1.100] \times 2.000 \times 1 \times 8 + 0.898 \times 0.200 \times 2 = 88.236 \text{ m}^3$	
	$\text{Sub-tota } V = 12.272 + 88.236 = 100.508 \text{ m}^3$	100.508 m3

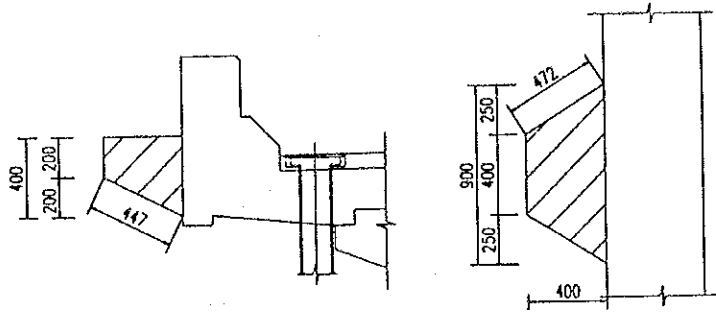
Item	Formula	Quantity
<p>2. Deck Slab CLASS "D"</p>	 <p>DETAIL "A"</p> $A-1 = \left[\frac{((0.210 + 0.281) \times 0.625 \times \frac{1}{2} + 0.100 \times 0.281)}{2 + (5.000 \times 2)} \right] \times 2 = 4.926 \text{ m}^2$ $V-1 = 4.926 \times 25.100 \times 2.000 = 247.300 \text{ m}^3$	
	 <p>DETAIL "A"</p> $A-2 = \left[\frac{((0.210 + 0.284) \times 0.600 \times \frac{1}{2} + 0.100 \times 0.284)}{2 + (5.025 \times 0.210 \times 2)} \right] \times 2 = 4.631 \text{ m}^2$ $V-2 = 4.631 \times 37.200 = 172.300 \text{ m}^3$ $V = 247.300 + 172.300 = 419.600 \text{ m}^3$	<p>419.600 m³</p>
<p>3. Railing CLASS "E"</p>	 $A = \left\{ (0.300 \times 0.575 + 0.260 \times 0.150 + (0.125 + 0.275) \times 0.150 \times \frac{1}{2} + 0.275 \times 0.050) \times 2 \right\} \times 2 = 1.021 \text{ m}^2$	

4. Lighting
Class "E"

$$V = 1.021 \times 87.500$$

$$= 89.338 \text{ m}^3$$

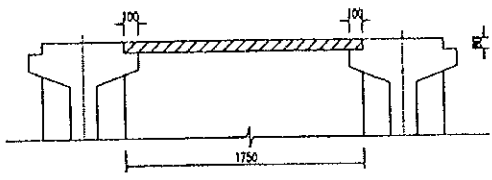
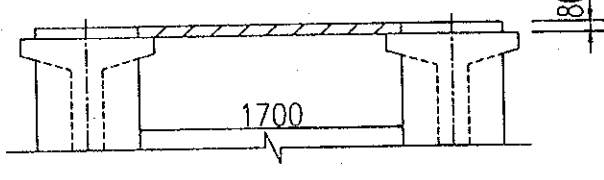
89.338 m³

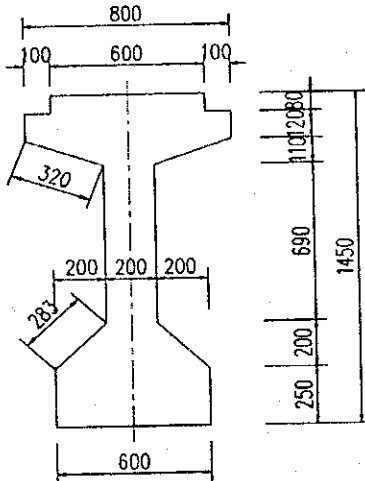
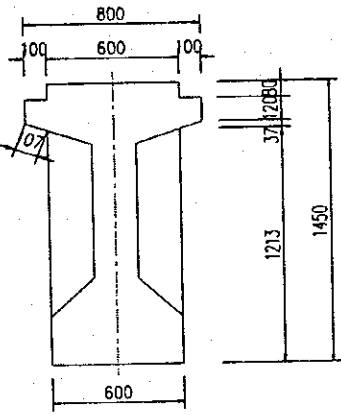


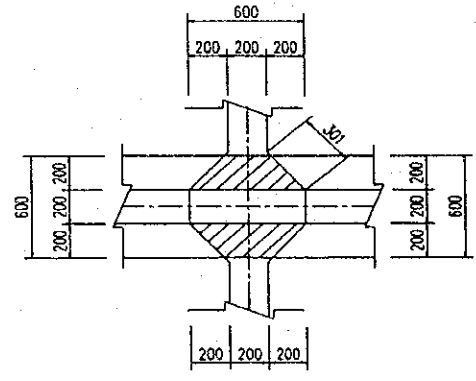
$$V = (0.200 \times 0.400 + 0.400 \times 0.900) \times \frac{1}{2} \times 0.4$$

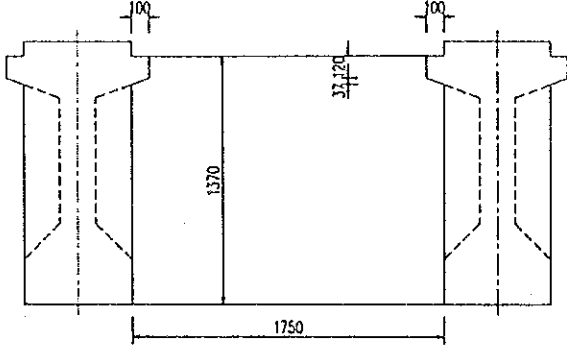
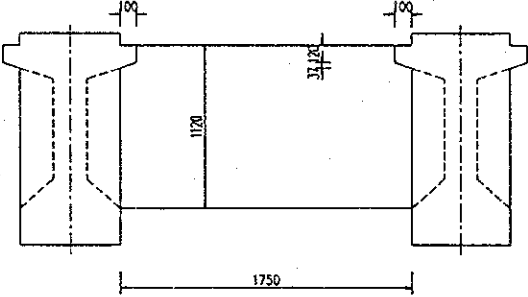
$$\times \frac{1}{2} = 0.176 \text{ m}^3$$

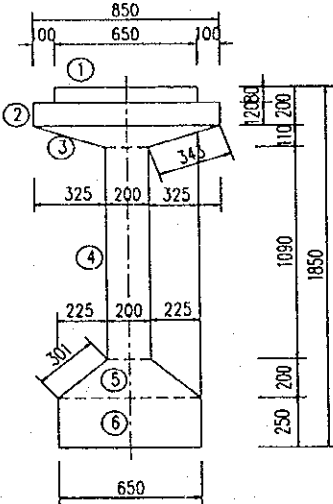
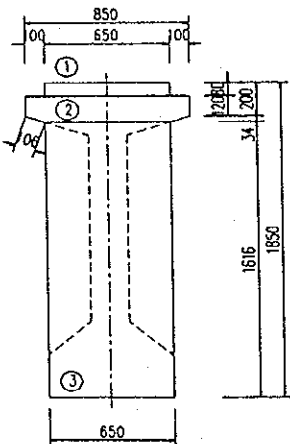
0.176 m³

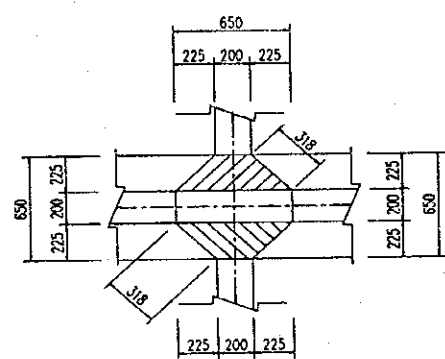
Item	Formula	Quantity
Panel concrete class D	$V-1 = 1.750 \times 0.08 \times 8 \times 25.000 \times 2 = 56.00 \text{ m}^3$	
		
	$V-2 = 1.700 \times 0.08 \times 8 \times 37.000 \times 1 = 40.256 \text{ m}^3$	
		
	$V = 56.000 + 40.256 = 96.256 \text{ m}^3$	96.256 m3

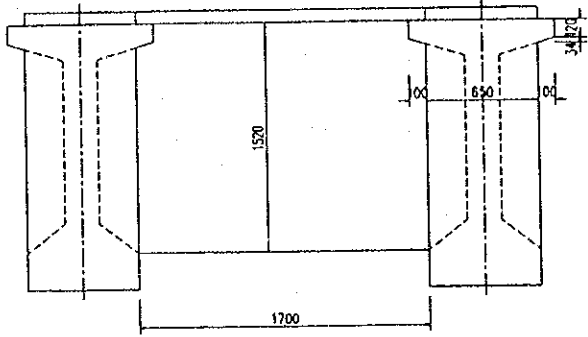
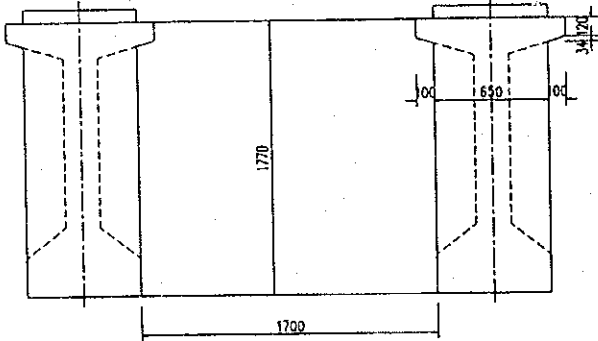
Item	Formula	Quantity																																				
1. Girder CLASS "B" 1) Girder	<p>(1) Calculation of Sectional Area</p> <p>[1] Middle Section Summary of Sectional Length For ONE GIRDER</p> <table border="1" data-bbox="279 425 1061 683"> <thead> <tr> <th>No</th> <th>Formula</th> <th>(m)</th> </tr> </thead> <tbody> <tr> <td>1</td> <td>0.080 × 2</td> <td>= 0.160</td> </tr> <tr> <td>2</td> <td>0.120 × 2</td> <td>= 0.240</td> </tr> <tr> <td>3</td> <td>0.320 × 2</td> <td>= 0.640</td> </tr> <tr> <td>4</td> <td>0.690 × 2</td> <td>= 1.380</td> </tr> <tr> <td>5</td> <td>0.283 × 2</td> <td>= 0.566</td> </tr> <tr> <td>6</td> <td>0.250 × 2 + 0.600</td> <td>= 1.100</td> </tr> </tbody> </table> <p style="text-align: right;">Total Length 4.086 m</p>  <p>[2] End Section Summary of Sectional Length For ONE GIRDER</p> <table border="1" data-bbox="295 1288 1077 1478"> <thead> <tr> <th>No</th> <th>Formula</th> <th>(m)</th> </tr> </thead> <tbody> <tr> <td>1</td> <td>0.080 × 2</td> <td>= 0.160</td> </tr> <tr> <td>2</td> <td>0.120 × 2</td> <td>= 0.240</td> </tr> <tr> <td>2</td> <td>0.107 × 2</td> <td>= 0.214</td> </tr> <tr> <td>3</td> <td>1.213 × 2 + 0.600</td> <td>= 3.026</td> </tr> </tbody> </table> <p style="text-align: right;">Total Length 3.640 m</p> 	No	Formula	(m)	1	0.080 × 2	= 0.160	2	0.120 × 2	= 0.240	3	0.320 × 2	= 0.640	4	0.690 × 2	= 1.380	5	0.283 × 2	= 0.566	6	0.250 × 2 + 0.600	= 1.100	No	Formula	(m)	1	0.080 × 2	= 0.160	2	0.120 × 2	= 0.240	2	0.107 × 2	= 0.214	3	1.213 × 2 + 0.600	= 3.026	
No	Formula	(m)																																				
1	0.080 × 2	= 0.160																																				
2	0.120 × 2	= 0.240																																				
3	0.320 × 2	= 0.640																																				
4	0.690 × 2	= 1.380																																				
5	0.283 × 2	= 0.566																																				
6	0.250 × 2 + 0.600	= 1.100																																				
No	Formula	(m)																																				
1	0.080 × 2	= 0.160																																				
2	0.120 × 2	= 0.240																																				
2	0.107 × 2	= 0.214																																				
3	1.213 × 2 + 0.600	= 3.026																																				

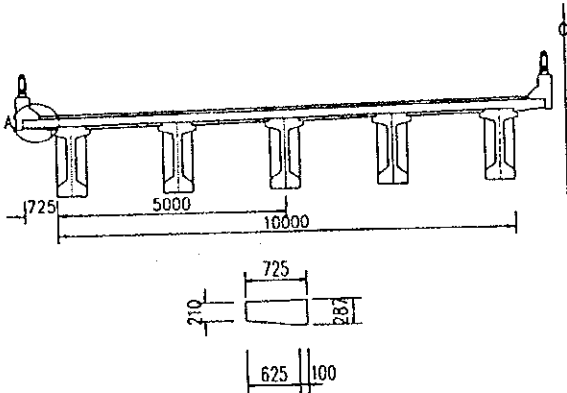
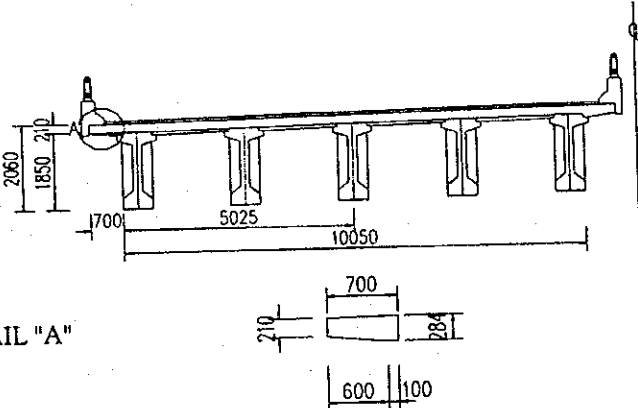
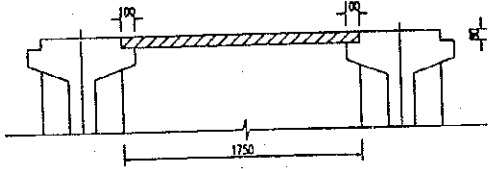
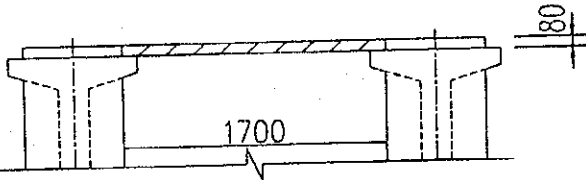
Item	Formula	Quantity																																																
	<p>(2) Calculation of Form Area</p> <p>[1] A1-A2</p> <table border="1" style="margin-left: auto; margin-right: auto;"> <thead> <tr> <th style="width: 10%;">Section No</th> <th style="width: 15%;">Section Length (m)</th> <th style="width: 15%;">Average of Section (m)</th> <th style="width: 15%;">Length of Block (m)</th> <th style="width: 15%;">Form Area (m²)</th> <th style="width: 20%;">Remark</th> </tr> </thead> <tbody> <tr> <td>END</td> <td>3.64</td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>END</td> <td>3.64</td> <td>3.64</td> <td>0.450</td> <td>1.638</td> <td></td> </tr> <tr> <td>MIDDLE</td> <td>4.086</td> <td>3.863</td> <td>3.500</td> <td>13.521</td> <td></td> </tr> <tr> <td>MIDDLE</td> <td>4.086</td> <td>4.086</td> <td>17.100</td> <td>69.871</td> <td></td> </tr> <tr> <td>END</td> <td>3.64</td> <td>3.863</td> <td>3.500</td> <td>13.521</td> <td></td> </tr> <tr> <td>END</td> <td>3.64</td> <td>3.64</td> <td>0.450</td> <td>1.638</td> <td></td> </tr> <tr> <td>Total</td> <td></td> <td></td> <td>25</td> <td>100.189</td> <td></td> </tr> </tbody> </table> <p style="text-align: center;">Girder Length 25.000 m</p> <p style="text-align: center;">No. of Girder 10 × 2 = 20</p> <p>A1 = 100.189 × 20 = 2,003.780 m²</p>	Section No	Section Length (m)	Average of Section (m)	Length of Block (m)	Form Area (m ²)	Remark	END	3.64					END	3.64	3.64	0.450	1.638		MIDDLE	4.086	3.863	3.500	13.521		MIDDLE	4.086	4.086	17.100	69.871		END	3.64	3.863	3.500	13.521		END	3.64	3.64	0.450	1.638		Total			25	100.189		
Section No	Section Length (m)	Average of Section (m)	Length of Block (m)	Form Area (m ²)	Remark																																													
END	3.64																																																	
END	3.64	3.64	0.450	1.638																																														
MIDDLE	4.086	3.863	3.500	13.521																																														
MIDDLE	4.086	4.086	17.100	69.871																																														
END	3.64	3.863	3.500	13.521																																														
END	3.64	3.64	0.450	1.638																																														
Total			25	100.189																																														
2) Diaphragm	 <p style="text-align: center;">600 200, 200, 200</p> <p style="text-align: center;">600 200, 200, 200</p> <p style="text-align: center;">600 200, 200, 200</p> <p>A2 = { (0.200 × 1 + 0.301 × 2) × (0.690 + 1.000) × 1/2 - 0.600 × 0.690 } × 10 × 2 = 21.095 m²</p> <p>Total A = 2,003.780 + 21.095 = 2,024.875 m²</p>	2,024.875 m ²																																																

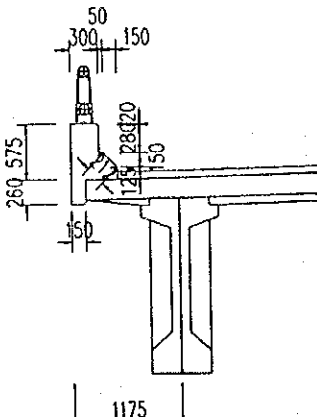
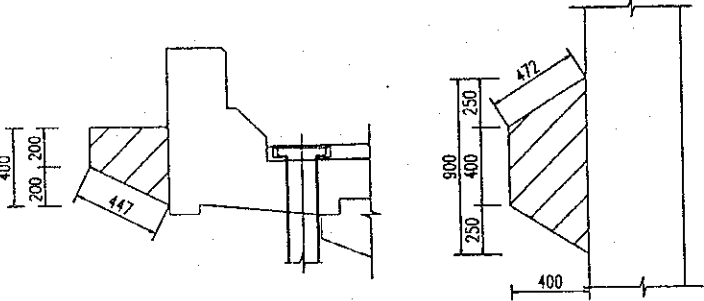
Item	Formula	Quantity
2. Diaphragm CLASS "C"	<p>(1) End Cross Beam</p>  $A1 = [\{ 1.750 \times 1.370 - (0.120 + 0.157) \times \frac{2}{2} \times 0.100 \} \times 2 + 1.75 \times 0.450] \times 8 \times \frac{2}{2} = 88.434 \text{ m}^2$ <p>(2) Intermediate Cross Beam</p>  $A1 = [\{ 1.750 \times 1.120 - (0.120 + 0.154) \times \frac{2}{2} \times 0.100 \} \times 2 + 1.75 \times 0.200] \times 8 \times \frac{2}{2} = 134.886 \text{ m}^2$ $A = 88.434 + 134.886 = 223.320 \text{ m}^2$	<p>223.320 m²</p>

Item	Formula	Quantity																																				
1. Girder CLASS "B" 1) Girder	<p>(1) Calculation of Sectional Area</p> <p>[1] Middle Section Summary of Sectional Length For ONE GIRDER</p> <table border="1" data-bbox="304 421 1093 678"> <thead> <tr> <th>No</th> <th>Formula</th> <th>(m)</th> </tr> </thead> <tbody> <tr> <td>1</td> <td>0.080×2</td> <td>= 0.160</td> </tr> <tr> <td>2</td> <td>0.120×2</td> <td>= 0.240</td> </tr> <tr> <td>3</td> <td>0.343×2</td> <td>= 0.686</td> </tr> <tr> <td>4</td> <td>1.090×2</td> <td>= 2.180</td> </tr> <tr> <td>5</td> <td>0.301×2</td> <td>= 0.602</td> </tr> <tr> <td>6</td> <td>$0.250 \times 2 + 0.650$</td> <td>= 1.150</td> </tr> </tbody> </table> <p style="text-align: right;">Total Length 5.018 m</p>  <p>[2] End Section Summary of Sectional Length For ONE GIRDER</p> <table border="1" data-bbox="316 1283 1102 1473"> <thead> <tr> <th>No</th> <th>Formula</th> <th>(m)</th> </tr> </thead> <tbody> <tr> <td>1</td> <td>0.080×2</td> <td>= 0.160</td> </tr> <tr> <td>2</td> <td>0.120×2</td> <td>= 0.240</td> </tr> <tr> <td>2</td> <td>0.106×2</td> <td>= 0.212</td> </tr> <tr> <td>3</td> <td>$1.616 \times 2 + 0.650$</td> <td>= 3.882</td> </tr> </tbody> </table> <p style="text-align: right;">Total Length 4.494 m</p> 	No	Formula	(m)	1	0.080×2	= 0.160	2	0.120×2	= 0.240	3	0.343×2	= 0.686	4	1.090×2	= 2.180	5	0.301×2	= 0.602	6	$0.250 \times 2 + 0.650$	= 1.150	No	Formula	(m)	1	0.080×2	= 0.160	2	0.120×2	= 0.240	2	0.106×2	= 0.212	3	$1.616 \times 2 + 0.650$	= 3.882	
No	Formula	(m)																																				
1	0.080×2	= 0.160																																				
2	0.120×2	= 0.240																																				
3	0.343×2	= 0.686																																				
4	1.090×2	= 2.180																																				
5	0.301×2	= 0.602																																				
6	$0.250 \times 2 + 0.650$	= 1.150																																				
No	Formula	(m)																																				
1	0.080×2	= 0.160																																				
2	0.120×2	= 0.240																																				
2	0.106×2	= 0.212																																				
3	$1.616 \times 2 + 0.650$	= 3.882																																				

Item	Formula	Quantity																																																
	(2) Calculation of Form Area																																																	
	<div style="text-align: right; margin-bottom: 5px;">Girder Length 37.000 m</div> <table border="1" style="width: 100%; border-collapse: collapse; margin-bottom: 20px;"> <thead> <tr> <th style="width: 10%;">Section No</th> <th style="width: 15%;">Section Length (m)</th> <th style="width: 15%;">Average of Section (m)</th> <th style="width: 15%;">Length of Block (m)</th> <th style="width: 15%;">Form Area (m²)</th> <th style="width: 20%;">Remark</th> </tr> </thead> <tbody> <tr><td>END</td><td>4.494</td><td></td><td></td><td></td><td></td></tr> <tr><td>END</td><td>4.494</td><td>4.494</td><td>0.500</td><td>2.247</td><td></td></tr> <tr><td>MIDDLE</td><td>5.018</td><td>4.756</td><td>5.300</td><td>25.207</td><td></td></tr> <tr><td>MIDDLE</td><td>5.018</td><td>5.018</td><td>25.400</td><td>127.457</td><td></td></tr> <tr><td>END</td><td>4.494</td><td>4.756</td><td>5.300</td><td>25.207</td><td></td></tr> <tr><td>END</td><td>4.494</td><td>4.494</td><td>0.500</td><td>2.247</td><td></td></tr> <tr><td>Total</td><td></td><td></td><td>37</td><td>182.365</td><td></td></tr> </tbody> </table> <p style="margin-left: 40px;"> No. of Girder 10 × 1 = 10 A1 = 182.365 × 10 = 1,823.650 m² </p>	Section No	Section Length (m)	Average of Section (m)	Length of Block (m)	Form Area (m ²)	Remark	END	4.494					END	4.494	4.494	0.500	2.247		MIDDLE	5.018	4.756	5.300	25.207		MIDDLE	5.018	5.018	25.400	127.457		END	4.494	4.756	5.300	25.207		END	4.494	4.494	0.500	2.247		Total			37	182.365		
Section No	Section Length (m)	Average of Section (m)	Length of Block (m)	Form Area (m ²)	Remark																																													
END	4.494																																																	
END	4.494	4.494	0.500	2.247																																														
MIDDLE	5.018	4.756	5.300	25.207																																														
MIDDLE	5.018	5.018	25.400	127.457																																														
END	4.494	4.756	5.300	25.207																																														
END	4.494	4.494	0.500	2.247																																														
Total			37	182.365																																														
2) Diaphragm	 <p style="margin-left: 40px;"> $A3 = \left\{ \left(0.200 \times 1 + 0.318 \times 2 \right) \times \left(1.090 + 1.366 \right) \right. \\ \left. \times \frac{1}{2} - 0.650 \times 1.090 \right\} \times 10 \times 3 \times 2 = 19.086 \text{ m}^2$ </p> <p style="margin-left: 40px;"> Total A = 1,823.650 + 19.086 = 1,842.736 m² </p>	1,842.736 m ²																																																

Item	Formula	Quantity
2. Diaphragm CLASS "C"	<p>(1) Intermediate Cross Beam</p>  $A1 = \left[\left(\frac{1.700 \times 1.520}{2} - \frac{(0.120 + 0.154) \times 2}{2} \right) \times 8 + 0.200 \times 1.700 \right] \times 3 = 130.877 \text{ m}^2$ <p>(2) Connection Cross Beam</p>  $A2 = \left[\left(\frac{1.770 \times 1.700}{2} - \frac{(0.120 + 0.154) \times 2}{2} \right) \times 8 + 0.900 \times 1.700 \right] + \left[\left(\frac{1.750 \times 1.370}{2} - \frac{(0.120 + 0.157) \times 2}{2} \right) \times 8 + 1.100 \times 1.750 \right] = 140.902 \text{ m}^2$ $A = 130.877 + 140.902 = 271.779 \text{ m}^2$	<p>271.779 m2</p>

Item	Formula	Quantity
<p>1. Deck Slab CLASS " "</p>	<p>a. SIDE SPAN</p>  <p>DETAIL "A"</p> $L-1 = (0.210 + 0.625) \times 2 \times 2 = 3.340 \text{ m}$ $A1 = 3.340 \times 25.000 \times 2.000 = 167.0 \text{ m}^2$ <p>b. MID SPAN</p>  <p>DETAIL "A"</p> $L-2 = (0.210 + 0.600) \times 2 \times 2 = 3.240 \text{ m}$ $A2 = 3.240 \times 37.000 \times 1.000 = 119.88 \text{ m}^2$ $A = 167.00 + 119.88 = 286.88 \text{ m}^2$	<p>286.88 m²</p>
<p>2. Panel CLASS "D"</p>	<p>a. SIDE SPAN</p>  $A1 = \left(\frac{(1.000 + 1.750) \times 0.080 \times 2 + 1.750 \times 1.000}{8 \times 25} \right) = 876.000 \text{ m}^2$ <p>b. MID SPAN</p>  $A2 = \left(\frac{(1.000 + 1.700) \times 0.080 \times 2 + 1.700 \times 1.000}{8 \times 37} \right) = 631.072 \text{ m}^2$ $A = 876.000 + 631.072 = 1,507.072 \text{ m}^2$	<p>1,507.072 m²</p>

Item	Formula	Quantity
3. Railing CLASS "D"	 $A = (0.575 + 0.280) \times 4 \times (2.000 \times 25.000 + 0.212 + 0.05 \times 37.0) = 330.4 \text{ m}^2$	330.400 m2
4. Lighting	 $A = \left\{ (0.900 + 0.400) \times 0.447 \times \frac{1}{2} + (0.400 + 0.200) \times 0.472 \times \frac{1}{2} + 0.400 \times 0.200 \right\} \times 2 = 1.308 \text{ m}^2$	1.308 m2
5. Platform for construction	$A = 26.100 \times (23.000 \times 2.000 + 35.000) = 2,114.100 \text{ m}^2$	2114.100 m2

Per bridge

No.

Item	Formula						Quantity	
1. PC CABLE 12S12.7(B)	1. A1-P1, P2-A2 For ONE GIRDER							
	CABLE VAR.	EACH LENGTH	CABLE NO.	EACH	TOTAL LENGTH	UNIT WEIGHT	WEIGHT	
	1	24.702	C1	1	24.702	9.290	229.482	
	2	24.738	C2	1	24.738	9.290	229.816	
	3	24.818	C3	1	24.818	9.290	230.559	
	TOTAL			3	74.258		689.857	
	TOTAL WEIGHT OF PC CABLES							
	$W_{p1} = 689.857 \times 10 \times 2 = 13797.140 \text{ kg}$							
	TENSION UNIT							
	$N_{s1} = 3 \times 2 \times 10 \times 2 = 120 \text{ EACH}$							
	2.P1-P2 For ONE GIRDER							
	CABLE VAR.	EACH LENGTH	CABLE NO.	EACH	TOTAL LENGTH	UNIT WEIGHT	WEIGHT	
	1	36.702	C1	1	36.702	9.290	340.962	
	2	36.714	C2	1	36.714	9.290	341.073	
	3	36.754	C3	1	36.754	9.290	341.445	
	4	36.784	C4	1	36.784	9.290	341.723	
	5	36.822	C5	1	36.822	9.290	342.076	
	TOTAL			5	183.776		1707.279	
	TOTAL WEIGHT OF PC CABLES							
	$W_{p2} = 1707.279 \times 10 \times 1 = 17072.790 \text{ kg}$							
	TENSION UNIT							
	$N_{s2} = 5 \times 2 \times 10 \times 1 = 100 \text{ EACH}$							
	SUB-TOTAL WEIGHT OF PC CABLES							
	$W_p = 13797.140 + 17072.790 = 30869.930 \text{ kg}$							30870 kg
	TOTAL TENSION UNIT							
$N_s = 120 + 100 = 220 \text{ EACH}$							220 EACH	
2. PC CABLE 3S12.7(B)	PC CABLE OF DIAPHRAGMS For A1-A2							
	LOCATION	EACH LENGTH	CABLE NO.	EACH	TOTAL LENGTH	UNIT WEIGHT	WEIGHT kg	
	Connection One	10.053		72	723.8	2.320	1679.253	
	TOTAL			72	723.816		1679.253	
							1679 kg	

Per bridge

No.

Item	Formula						Quantity	
3. SHEATHING P 80/85MM	LENGTH	74.258 x	20 +	183.776 x	10	=	3322.92 m	
3. SHEATHING P 50/55MM	LENGTH			723.816 x	1	=	723.82 m	
4. STEEL SHEAR KEY		6 x	20 +	12 x	10	=	240 set	
5. CEMENT GROUT IN SHEATHING	3.14 x x	0.08 x 0.05 /	0.080 / 4 x	4 x 723.82	3322.92 +	3.14 x 0.05	=	18.115 m ³
6. ANCHOR	a. 12S12.7 (B)	6 x	10 x	2 +	10 x	10	=	220 set
	b. 3S12.7 (B)	72 x	2			= 144 set	=	144 set

1) A1-P1, P2-A2

SCHEDULE OF REINFORCEMENT FOR GIRDER L=25M

BAR MARK	SIZE (mm)	DIMENSIONS (mm)							LENGTH (mm)	UNIT WEIGHT (kgf/m)	NO. OF BARS	WEIGHT (kgf)	REMARKS	
		a	b	c	d	e	f	g						
G1	14	190	1557	117	1557	190			3611	1.208	119	519.1		
G2	14	190	1557	317	1557	190			3811	1.208	48	221.0	Average	
G3	14	190	1557	517	1557	190			4011	1.208	8	38.8		
G4	14	190	1557	517	1557	190			4011	1.208	4	19.4		
G5	14	200	209	159	509	159	209	200	1645	1.208	131	260.3		
G6	14	200	43	709	43	200			1195	1.208	131	189.1		
G7	14	200	283	189	517	189	283	200	1861	1.208	75	168.6		
G8	14	200	141	189	517	189	141	200	1577	1.208	48	91.4	Average	
G9	14	389	517	389					1295	1.208	8	12.5		
G10	22	7445	9950	7445					24840	2.984	6	444.7		
G11	14	7445	9950	7445					24840	1.208	18	540.1		
G12	14	420	3436	17148	3436	420			24860	1.208	8	240.2		
G13	14	100	277	151	277	100			905	1.208	16	17.5		
G14	14	200	800						1000	1.208	12	14.5		
G15	16	1600							1600	1.578	48	121.2	Interior	
G16	16	1100							1100	1.578	48	83.3	Exterior	
G17	10	520							520	0.617	12	3.9		
G18	10	150							150	0.617	32	3.0		
G19	14	361	364	567	364	361			2017	1.208	119	289.9		
Total	SIDE SPAN	Interior girder											3195.2	
		Exterior girder											3157.3	

For ONE GIRDER

2) P1-P2

SCHEDULE OF REINFORCEMENT (OF GIRDER)

BAR MARK	SIZE (mm)	DIMENSIONS (mm)							LENGTH (mm)	UNIT WEIGHT (kgf/m)	NO. OF BARS	WEIGHT (kgf)	REMARKS	
		a	b	c	d	e	f	g						
G1	14	190	1957	117	1957	190			4411	1.208	184	980.4		
G2	14	190	1957	342	1957	190			4636	1.208	74	414.4	Average	
G3	14	190	1957	567	1957	190			4861	1.208	6	35.2		
G4	14	190	1957	567	1957	190			4861	1.208	6	35.2		
G5	14	200	238	154	567	154	238	200	1751	1.208	196	414.6		
G6	14	200	42	759	42	200			1243	1.208	196	294.3		
G7	14	200	301	183	567	183	301	200	1935	1.208	122	285.2		
G8	14	200	151	183	567	183	151	200	1635	1.208	74	146.2	Average	
G9	14	389	567	389					1345	1.208	6	9.8		
G10	22	6945	7450	7950	7450	6945			36740	2.984	6	657.8	Average	
G11	14	6945	7450	7950	7450	6945			36740	1.208	22	976.4		
G12	14	6950	7450	7950	7450	6950			36750	1.208	12	532.7		
G13	14	100	318	151	318	100			987	1.208	30	35.8		
G14	14	200	800						1000	1.208	12	14.5		
G15	16	1600							1600	1.578	84	2121.1	Interior	
G16	16	1100							1100	1.578	84	145.8	Exterior	
G17	10	570							570	0.617	24	8.4		
G18	10	150							150	0.617	104	9.6		
G19	14	361	364	567	364	361			2017	1.208	184	448.3		
Total	Mid span	Interior girder											5510.9	
		Exterior girder											5444.7	

Span	Mid/Side	Int/Ext	Nos.	Weight/G	Total	Remark
A1-P1	Side span	Interior Beam	6	3195.15	19170.90	
		Exterior Beam	4	3157.28	12629.12	
P2-A2	Side span	Interior Beam	6	3195.15	19170.90	
		Exterior Beam	4	3157.28	12629.12	
P1-P2	Mid Span	Interior Beam	6	5510.94	33065.64	
		Exterior Beam	4	5444.67	21778.68	
Total					118444.36	

A1-P1, P2-A2

Per EACH

SCHEDULE OF REINFORCEMENT FOR DIAPHRAGM													
(1) END DIAPHRAGM													
BAR MARK	SIZE (mm)	DIMENSIONS (mm)							LENGTH (mm)	UNIT WEIGHT (kgf/m)	NO. OF BARS	WEIGHT (kgf)	REMARKS
		a	b	c	d	e	f	g					
A1	16	1750							1750	1.578	56	154.60	
A2	14	198	1559	360	1559	198			3874	1.208	28	131.00	
Sub-Total											285.60		
(2) INTERMEDIATE DIAPHRAGM													
BAR MARK	SIZE (mm)	DIMENSIONS (mm)							LENGTH (mm)	UNIT WEIGHT (kgf/m)	NO. OF BARS	WEIGHT (kgf)	REMARKS
		a	b	c	d	e	f	g					
B1	16	1750							1750	1.578	48	132.60	
B2	14	198	1309	110	1309	198			3124	1.208	28	105.70	
Sub-Total											238.30		

Total Weight

SECTION	Nos.	Weight/EACH	Weight
END DIAPHRAGM	4	285.60	1142.40
INTERMEDIATE DIAPHRAGM	8	238.30	1906.40
Total			3048.80

SCHEDULE OF REINFORCEMENT (OF DIAPHRAGM)													
(1) INTERMEDIATE DIAPHRAGM													
BAR MARK	SIZE (mm)	DIMENSIONS (mm)							LENGTH (mm)	UNIT WEIGHT (kgf/m)	NO. OF BARS	WEIGHT (kgf)	REMARKS
		a	b	c	d	e	f	g					
K1	16	1700							1700	1.578	64	171.70	
K2	14	198	1709	110	1709	198			3924	1.208	28	132.70	
Sub-Total												304.40	
(2) CONNECTION DIAPHRAGM													
BAR MARK	SIZE (mm)	DIMENSIONS (mm)							LENGTH (mm)	UNIT WEIGHT (kgf/m)	NO. OF BARS	WEIGHT (kgf)	REMARKS
		a	b	c	d	e	f	g					
L1	16	1700							1700	1.578	56	150.20	
L2	14	245	1910	245					2400	1.208	84	243.50	
L3	14	198	1559	1128	1559	198			4642	1.208	56	314.00	
L4	14	198	1559	1200	1559	198			4714	1.208	28	159.40	
L5	16	9950							9950	1.578	2	31.40	
L6	14	1042	810	1042					2894	1.208	28	97.90	
L7	16	1750							1750	1.578	40	110.50	
Sub-Total												1106.90	

Total Weight

SECTION	Nos.	Weight/EACH	Weight
INTERMEDIATE DIAPHRAGM	6	304.40	1826.40
CONNECTION DIAPHRAGM	4	1106.90	4427.60
Total			6254.00

A1-P1, P1-P2, P2-A2

Per BRIDGE

SCHEDULE OF REINFORCEMENT FOR RAILING												
BAR MARK	SIZE (mm)	DIMENSIONS (mm)						LENGTH (mm)	UNIT WEIGHT (kgf/m)	NO. OF BARS	WEIGHT (kgf)	REMARKS
		a	b	c	d	e	f					
P1	14	250	701	212	747	70		1980	1.208	68	162.6	
P2	14	230	297	176	275	245		1223	1.208	68	100.5	
P3	14	10000						10000	1.208	11	132.9	
Total											396.0	

$$W = 396 / 10 \times 87.5 \times 4 = 13860 \text{ kg}$$

SCHEDULE OF REINFORCEMENT FOR DECK SLAB												
BAR MARK	SIZE (mm)	DIMENSIONS (mm)						LENGTH (mm)	UNIT WEIGHT (kgf/m)	NO. OF BARS	WEIGHT (kgf)	REMARKS
		a	b	c	d	e	f					
S1	20	11350						11350	2.466	1096	30676.1	
S2	20	10850						10850	2.466	1096	29324.7	
S3	14	133	623	207				963	1.208	2332	2712.8	
S4	22	12000						12000	2.984	400	14323.2	
S5	14	27460						27460	1.208	200	6634.3	
S6	14	19180						19180	1.208	200	4633.9	
S6*	14	21180						21180	1.208	200	5117.1	
S7	25	11350						11350	3.853	70	3061.2	
S8	25	10850						10850	3.853	70	2926.4	
S9	12	210	145	210				565	0.888	14575	7312.6	
Total											106722.3	106722.3

No. _____

A1-P1, P1-P2, P2-A2

Per BRIDGE

SCHEDULE OF REINFORCEMENT (OF LIGHTING)													
BAR MARK	SIZE (mm)	DIMENSIONS (mm)							LENGTH (mm)	UNIT WEIGHT (kg/m)	NO. OF BARS	WEIGHT (kg)	REMARKS
		a	b	c	d	e	f	g					
P1	20	113	825	320					1258	2.466	3	9.31	
P2	16	113	380	488	280				1261	1.578	3	5.97	
P3	20	1000	300	1000					2300	2.466	2	11.34	
P4	16	282	285	282					849	1.578	2	2.68	
P5	16	208	285	208					701	1.578	2	2.21	
Total												31.51	

Total Weight

Total = 31.51 × 2 = 63.02 (kg)

Item	Formula	Quantity
ETC.	1. JOINT	
	EACH LENGTH	
	$L = 10.750 + 10.750 = 21.500$	
	EACH = 2	
	TOTAL LENGTH	
	$L = 21.500 \times 2 = 43.000 \text{ m}$	43.000 m
	2. DRAIN	
	TOTAL EACH	
	EACH = 7 x 2 = 14	14 EACH
	PIPE = 1.74 x 14 = 24.36 m	24.36 m
	3. BEARING PAD	
	a. ELASTOMERIC 500x250x50	
	EACH for One SPAN	
	EACH = 10	
	TOTAL EACH	
	EACH = 10 x 2 x 2 = 40	EACH 40
	b. ELASTOMERIC 600x300x57	
	EACH for One SPAN	
	EACH = 10	
	TOTAL EACH	
	EACH = 10 x 2 x 1 = 20	EACH 20
	4. ANCHOR BAR	
	EACH for One SPAN	
	EACH = 4	
	TOTAL EACH (FIX)	
	EACH = 4 x 8 = 32	EACH 32
	TOTAL EACH (MOVE)	
	EACH = 4 x 4 = 16	EACH 16

Item	Formula	Quantity
	<p>4. RAILING</p> <p>EACH LENGTH for One SIDE SPAN L1 = 25.100 EACH = 2 SPAN</p> <p>EACH LENGTH for One MID SPAN L2 = 37.200 EACH = 1 SPAN</p> <p>TOTAL LENGTH</p> $L = 25.10 \times 4 \times 2 + 37.20 \times 4 \times 1 = 367.600 \text{ m}$	<p>367.600 m</p>
	<p>5. LIGHTING</p> <p>EACH for One SIDE SPAN EACH = 1</p> <p>EACH for One MID SPAN EACH = 0</p> <p>TOTAL EACH = 1.0 x 2.0 + 0 = 2.0 EACH</p>	<p>EACH 2.0</p>
	<p>6. PAVEMENT</p>	
<p>a. WATER PROOFING 5MM</p>	<p>10.75 X (24.700 x 2 + 37.200) x 2 =</p>	<p>1862 m²</p>
<p>b. ALPHALT CONCRETE 70MM</p>	<p>10.75 X (24.700 x 2 + 37.200) x 2 =</p>	<p>1862 m²</p>

QUANTITY TABLE OF ABUTMENTS

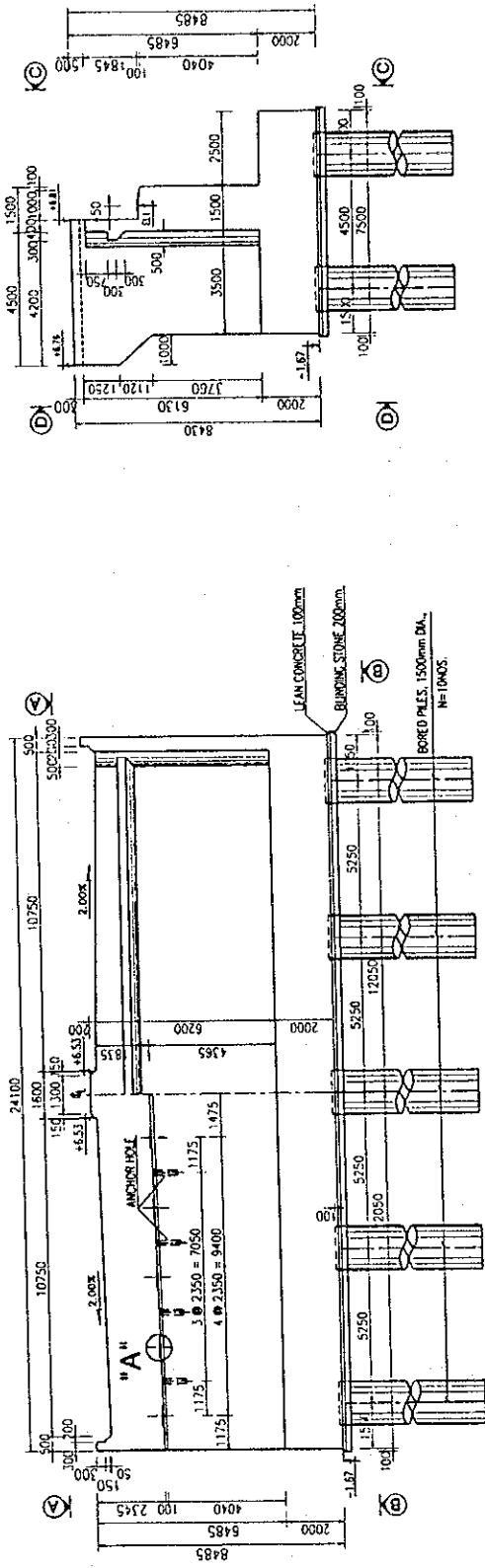
ITEMS		UNIT	ABUTMENT A1	ABUTMENT A2	TOTAL	
PILE	NUMBER OF PILES	PILE	10	10	20	
	TOTAL LENGTH BORED PILES F1500MM	M	660.0	660.0	1320	
	CONCRETE	CLASS D	M3	1166.3	1166.3	2333
	EXCAVATION MATERIALS		M3	1164.5	1164.5	2329.1
	REINFORCEMENT	D32	KG	22727.0	22727.0	45454
		D25	KG	35487.0	35487.0	70974
		D22	KG	3542.0	3542.0	7084
		D16	KG	334.0	334.0	668
		D10	KG	10386.0	10386.0	20772
		TOTAL	KG	72476.0	72476.0	144952
LEAN CONCRETE CLASS G		M3	16.9	16.9	34	
BLINDING STONE		M3	33.9	33.9	67.8	
ABUTMENT	CONCRETE	CLASS E	M3	582.8	582.8	1166
		FORM	M2	503.15	503.15	1006.3
	REINFORCEMENT	D32	KG	0.0	0.0	0
		D25	KG	12883.7	12883.7	25767
		D22	KG	4604.7	4604.7	9209
		D20	KG	5773.2	5773.2	11546
		D18	KG	701.4	701.4	1403
		D16	KG	4100.9	4100.9	8202
		D14	KG	2616.8	2616.8	5234
		D10	KG	152.7	152.7	305
		TOTAL	KG	30833.4	30833.4	61667
	EXCAVATION FOR FOUNDATION		M3	1217.2	1201.7	2418.9
	BACK FILL		M3	576.5	561.0	1137.5
	FORM		M2	503.15	503.15	1006.3
SCAFFOLDING WORK	H<4M	M2	142.40	142.40	284.8	
	4M<H<30M	M2	462.04	462.04	924.1	
APPROACH SLAB	CONCRETE	CLASS E	M3	43.2	43.2	86.5
	LEAN CONCRETE	CLASS G	M3	13.3	13.3	26.6
	ASPHTIC BIND FILLER T=20MM		M3	0.41	0.41	0.81
	FORM		M2	24.15	24.15	48.3
	REINFORCEMENT	D20	KG	3421.6	3421.6	6843
		D16	KG	3402.3	3402.3	6805
		D10	KG	255.8	255.8	512
TOTAL		KG	7079.7	7079.7	14159.4	

SLOPE PROTECTION	STONE MASONRY T=300MM		M3	563.9	660.7	1225	
	BLINDING AGGREGATE		M3	187.6	220.23	408	
	GEOTEXTILE		M2	622	946	1568	
	PVC PILE Φ 50MM DIA., L=1000MM		M	66	69	135	
	EXCAVATION		M3	568	595	1163	
	FILLING		M3	395	413	808	
	FOOTING	WOODEN PILE L=3M		M	7891	8264	16155
		BLINDING STONE		M3	10.5	11	22
		STONE MASORY		M3	47.3	49.6	97

ABUTMENT OF SMALL TRA VA BRIDGE

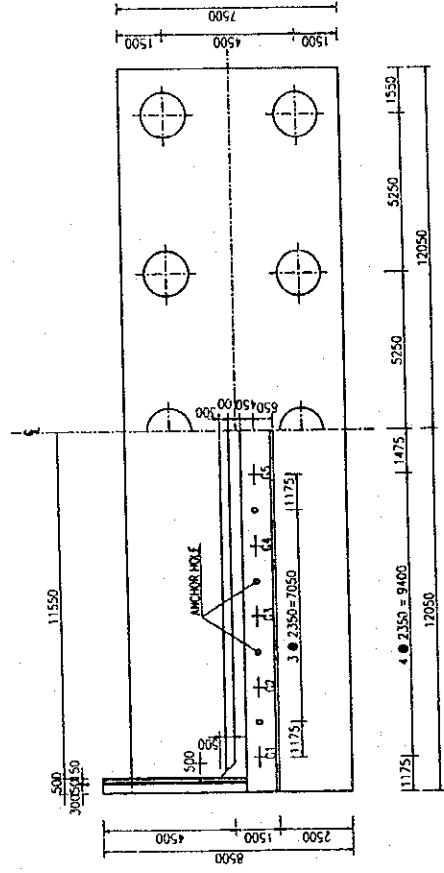
1/2 C - C

1/2 D - D



1/2 A - A

1/2 B - B



Item	Formula						Quantity
1) Concrete							
* BackWall	v1 =	24.10	x	1.85	x	0.40	= 17.79 m3
* Frontwall	v2 =	24.10	x	((4.14 + 4.38)/	2)/	153.90 m3
	x	1.50	-	0.10	^2 /	2]=
* Corbel	v3 =	0.30	x	(0.30 + 0.60)/	2)/	3.12 m3
				x	23.10	=	
* Haunch	v4 =	5.99	x	0.50	x	0.50	/ 2 x 2
							= 1.50 m3
* Wingwall	v5 = {	3.50	x	(5.99 + 5.95)/	2)/	2
	+	1.00	x	(1.05 + 2.18)/	2)/	2 }
				x	2	=	45.00 m3
* Parapet	v7 = (0.50	x	0.30	+	0.20	^2
	-	0.15	^2 /	2) x	4.90	x
							= 1.75 m3
* Footing	v6 =	24.10	x	7.50	x	2.00	
	-	10	x	pi	x	0.75	^2 x
							0.1
							= 359.73 m3
							Total
							582.8 m3
2) Form							
* BackWall	a1 =	2.00	x	1.85	x	24.10	-
	-	1.85	x	(0.50 + 0.50) x	2) x	= 85.24 m2
* Frontwall	a2 =	24.1	x	(4.14 + 4.381)	2) x	2
	-	0.5	+	0.5) x	4.381	x
	+	4.26	x	1.5	x	2	=
							209.38 m2
* Corbel	a3 = {	0.3	+	(0.3 ^2 + 0.3 ^2) ^ 0.5	x	23.1	=
							16.73 m2
* Haunch	a4 =	+	(0.5 ^2 + 0.5 ^2) ^ 0.5	x	5.985	=
							8.46
* Wingwall	a5 =	2 x {	3.5	x	(5.985 + 5.95) x 1/2	+ 2.18) x 1/2	
		+ 1	x	(1.05 + 1.50 + 3.76)	+ 45.33) =	45.33 m2
		+ 0.5	x	(1.05 + 1.50 + 3.76)	+ 45.33) =	45.33 m2
		-	0.5	x	5.99]=	45.33 m2
* Parapet	a5 = {	4.9	x	(0.5 + 0.3 + 0.05)	+ 0.05) x	2
		+	1.4142	x	0.15	+ 0.17875	} x
		+	2	x	0.17875	} x	= 11.61 m2
* Footing	a 6 =	2 x	(24.100 + 7.500) x	2.00	126.40	m2	
							Total
							503.1 m2

Item	Formula	Quantity
3) Scaffolding:	<p>* H<=4m $A2 = \{ 2 \times (24.10 + 7.50) + \frac{8}{2} \}$</p>	142.4 m2
	<p>* 4m< H<=30m $A2 = \{ (24.1+2) + (4.5+1.5+2) + (0.5+2) + (4.5+1-1) + (24.1-2 \times 1.5) + (4.5-1+1) + (0.5+2) + (4.5+1.5+2) \} \times (1.85+4.14)$</p>	462.0 m2
4) Support	<p>V = (8.43 - 1.55 + 5.76) x 1.00 / 2 x 0.50 x 2 =</p>	6.3 m3
5) Lean Concrete	<p>* Concrete class G $V = 0.1 \times \{ (24.10 + 0.2) \times (7.50 + 0.2) - 10 \times \frac{1}{4} \times \pi \times 1.50^2 \}$</p>	16.9 m3
	<p>* Form $A = 0.1 \times \{ (24.10 + 0.2) + (7.50 + 0.20) \} \times \frac{1}{2}$</p>	6.40 m2
6) Blinding Stone	<p>v = 0.2 x { (24.10 + 0.2) x (7.50 + 0.2) - 10 x 1/4 x pi x 1.50 ^2}</p>	33.9 m3
7) Bored Pile	<p>* Concrete = 10 x 1/4 x pi x 1.50 x 1.50 x (65.90 + 0.10) =</p>	1166.3 m3
	<p>* Excavation Length N<20: = 50.9 x 10 = 509.0 m 20<N<40: = 13.3 x 10 = 133.0 m 40<N: = 1.7 x 10 = 17.0 m</p>	
	<p>* Excavation Volume = 10 x 1/4 x pi x 1.50 x 1.50 x 65.90</p>	1164.5 m3
8) Earthworks	<p>* Excavation for foundation = 3.23 / 6 x { (9.50 x 26.10) + (15.96 x 32.56) + (9.50 + 15.96) x (26.10 + 32.56) }</p>	1217.2 m3
	<p>* Excess Soil = Lean Concrete + Blinding Stone + Footing Volume + Pile Occupied Volume Pile Occupied Volume = 10 x pi/4 x 1.50 x 1.50 x 0.400 = 7.07 m3 Excess Soil = 640.68 m3</p>	
	<p>* Back Fill = 576.5 m3</p>	
9) Approach Slab	<p>* Concrete = 23.04 x { 6.00 x 0.30 + (0.30 + 0.50) / 2 x 0.20 } - 0.50 x 0.50 / 2 x 0.3</p>	43.24 m3
	<p>* Lean Concrete = (0.30 + 0.28 + 5.20) x 23.04 x 0.1 = 13.3 m3</p>	
	<p>* Asphaltic Joint Filler = { 0.30 x (0.02 + 0.06) / 2 + 0.30 x 0.02 } x 23.04 = 0.41 m3</p>	
	<p>* Form = (2 x (6.00 x 0.30) + 11.52 x (0.30 + 0.50) + (0.50 + 0.30) x 0.50 x 0.20 x 2) x 6.00 x 0.30 = 24.15 m2</p>	

Item	Formula	Quantity
1) Concrete		
* BackWall		
v1 =	24.10 x 1.85 x 0.40 = 17.79 m3	
* Frontwall		
v2 =	24.10 x ((4.14 + 4.38) / 2	
x	1.50 - 0.10 ^2 / 2) = 153.90 m3	
* Corbel		
v3 =	0.30 x (0.30 + 0.60) / 2	
	x 23.10 = 3.12 m3	
* Haunch		
v4 =	5.99 x 0.50 x 0.50 / 2 x 2	
	= 1.50 m3	
*Wingwall		
v5 = {	3.50 x (5.99 + 5.95) / 2	
+	1.00 x (1.05 + 2.18) / 2 }	
	x 2 = 45.00 m3	
* Parapet		
v7 = (0.50 x 0.30 + 0.20 ^2	
-	0.15 ^2 / 2) x 4.90 x 2	
	= 1.75 m3	
*Footing		
v6 =	24.10 x 7.50 x 2.00	
-	10 x pi x 0.75 ^2 x 0.1	
	= 359.73 m3	
	Total	582.8 m3
2) Form		
* BackWall		
a1 =	2.00 x 1.85 x 24.10 -	
-	1.85 x (0.50 + 0.50) x 2	
	= 85.24 m2	
* Frontwall		
a2 =	24.1 x (4.14 + 4.381)	
-	(0.5 + 0.5) x 4.381 x 2	
+	4.26 x 1.5 x 2 = 209.38 m2	
* Corbel		
a3 = {	0.3 + (0.3 ^2 + 0.3 ^2) ^ 0.5	
	x 23.1 = 16.73 m2	
* Haunch		
a4 =	+ (0.5 ^2 + 0.5 ^2) ^ 0.5	
	x 5.985 x 2 = 8.46	
*Wingwall		
a5 =	2 x (3.5 x (5.985 + 5.95) x 1/2	
+	1 x (1.05 + 2.18) x 1/2	
+	0.5 x (1.05 + 1.50 + 3.76)	
	x 0.5 x 5.99 = 45.33 m2	
* Parapet		
a5 = {	4.9 x (0.5 + 0.3 + 0.05	
+	1.4142 x 0.15 + 0.05)	
+	2 x 0.17875 } x 2	
	= 11.61 m2	
*Footing		
a 6 =	2 x (24.100 + 7.500) x 2.00	
	126.40 m2	
	Total	503.1 m2

Item	Formula	Quantity
3) Scaffolding:	* H<=4m $A2 = \left\{ \frac{2 \times (24.10 + 7.50) + 8}{2} \right\}$	142.4 m2
	* 4m< H<=30m $A2 = \{ (24.1+2) + (4.5+1.5+2) + (0.5+2) + (4.5+1-1) + (24.1-2 \times 1.5) + (4.5-1+1) + (0.5+2) + (4.5+1.5+2) \} \times (1.85+4.14)$	462.0 m2
4) Support	V = $\left(\frac{8.43 - 1.55 + 5.76}{0.50 \times 2} \right) \times 1.00 / 2$	6.3 m3
5) Lean Concrete	* Concrete class G $V = \left\{ \frac{0.1 \times \left\{ \frac{24.10 + 0.2}{10} \times \left(\frac{7.50 + 1.50^2}{2} \right) + 0.2 \right\} - 0.2}{1} \right\}$	16.9 m3
	* Form $A = \left\{ \frac{0.1 \times \left\{ \frac{24.10 + 0.2}{10} \right\} + \left(\frac{7.50 + 0.20}{2} \right)}{1} \right\} \times 2$	6.40 m2
6) Blinding Stone	v = $\left\{ \frac{0.2 \times \left\{ \frac{24.10 + 0.2}{10} \times \left(\frac{7.50 + 1.50^2}{2} \right) + 0.2 \right\} - 0.2}{1} \right\}$	33.9 m3
7) Bored Pile	* Concrete $= 10 \times \frac{1}{4} \times \pi \times 1.50 \times \left(\frac{1.50}{65.90 + 0.10} \right) =$	1166.3 m3
	* Excavation Length N<20: = 53.3 x 10 = 533.0 m 20<N<40: = 9.2 x 10 = 92.0 m 40<N: = 3.4 x 10 = 34.0 m	
	* Excavation Volume $= 10 \times \frac{1}{4} \times \pi \times 1.50 \times 1.50 \times 65.90$	1164.5 m3
	* Excavation for foundation $= \frac{3.20}{15.9} \times \left\{ \frac{6 \times \left(\frac{9.50 \times 26.10}{32.5} \right) + \left(\frac{9.50 + 15.9}{26.10 + 32.5} \right)}{1} \right\}$	1201.7 m3
	* Excess Soil = Lean Concrete + Blinding Stone + Footing Volume + Pile Occupied Volume Pile Occupied Volume = $10 \times \frac{\pi}{4} \times 1.50 \times 0.400 = 7.07 \text{ m3}$ Excess Soil = 640.68 m3	
9) Approach Slab	* Back Fill = 561.0 m3	
	* Concrete $= \frac{23.04 \times \left\{ \frac{6.00 \times 0.30 + (0.30 + 0.50) / 2 \times 0.20}{0.50 \times 0.50 / 2 \times 0.3} \right\}}{2}$	43.24 m3
	* Lean Concrete $= (0.30 + \frac{0.28 + 5.20}{23.04 \times 0.1})$	13.3 m3
	* Asphaltic Joint Filler $= \left\{ \frac{0.30 \times (0.02 + 0.06) / 2 + 0.30 \times 0.02}{23.04} \right\} \times 2$	0.41 m3
	* Form $= \left(\frac{2 \times (6.00 \times 0.30) + 11.52}{0.50 \times 0.20 \times 2} \right) \times \left(\frac{0.50 + 0.30}{6.00 \times 0.30} \right) \times 2$	24.15 m2

LIST OF REINFORCEMENT (FOR ONE ABUTMENT)

REINF No	DIA (mm)	LENGTH (mm)	NUMBER	UNIT WEIGHT (kg/m)	WEIGHT (kg)	REMARKS
A1	14	2176	289	1.208	759.7	
A2	14	24752	16	1.208	478.6	
A3	14	720	141	1.208	122.7	
A4	16	1837	97	1.578	281.2	
A5	14	690	97	1.208	80.9	
A6	14	23126	4	1.208	111.8	
F1	25	10800	97	3.853	4036.8	
F2	25	7300	96	3.853	2700.4	
F3	25	8174	97	3.853	3055.0	
F4	25	4900	96	3.853	1812.6	
F5	20	8053	16	2.466	317.8	
F6	20	25200	16	2.466	994.4	
F7	16	4820	155	1.578	1179.2	
F8	20	28688	30	2.466	2122.5	
F9	20	25940	30	2.466	1919.2	
W1	22	5185	94	2.984	1454.4	
W2	22	6357	97	2.984	1840.0	AVERAGE
W3	16	6252	97	1.578	957.2	AVERAGE
W4	14	1822	184	1.208	405.1	
W5	16	24874	17	1.578	667.4	
W6	14	24752	22	1.208	658.0	
W7	16	1790	97	1.578	274.0	
K1	22	5129	8	2.984	122.4	
K2	22	2985	14	2.984	153.9	
K3	22	4785	16	2.984	228.5	
K4	25	8425	28	3.853	909.0	AVERAGE
K5	25	4437	16	3.853	273.6	
K6	22	6217	32	2.984	593.7	
K7	18	6232	12	1.998	149.4	
K8	16	6170	10	1.578	97.4	
K9	16	1754	40	1.578	110.7	
K10	16	2360	2	1.578	7.4	
K11	20	2250	18	2.466	99.9	
K12	16	8268	28	1.578	365.4	AVERAGE
K13	18	6152	32	1.998	393.3	
K14	10	760	208	0.617	97.5	
K15	10	750	50	0.617	23.1	
K16	20	3809	34	2.466	319.4	
K17	22	4776	4	2.984	57.0	AVERAGE
K18	25	2148	6	3.853	49.7	AVERAGE
K19	25	3025	4	3.853	46.6	
K20	18	4706	6	1.998	56.4	AVERAGE
K21	18	5358	2	1.998	21.4	
K22	16	1991	6	1.578	18.9	AVERAGE
K23	22	5428	2	2.984	32.4	AVERAGE
K24	16	2251	40	1.578	142.1	
K25	22	5129	8	2.984	122.4	
K26	18	5059	8	1.998	80.9	
R1	10	300	80	0.617	14.8	
R2	10	700	40	0.617	17.3	
TOTAL		30833.4	kg			
D10:	152.7	kg	D20:	5773.2	kg	
D14:	2616.8	kg	D22:	4604.7	kg	
D16:	4100.9	kg	D25:	12883.7	kg	
D18:	701.4	kg				

LIST REINFORCEMENT OF APPROACH SLAB

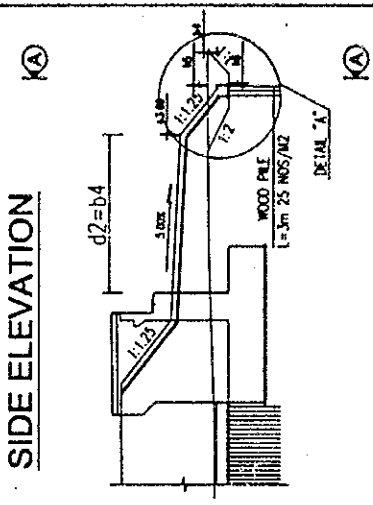
TYPE	Ø (mm)	LENGTH OF BAR (mm)	U.WEIGHT (kg/m)	NUMBER	WEIGHT (Kg)	
AS1	D20	5850	2.466	230	3318.0	
AS2	D16	11670	1.578	110	2025.7	
AS3	D16	6320	1.578	116	1156.9	
AS4	D16	1200	1.578	116	219.7	
AS5	D20	700	2.466	60	103.6	
AS6	D10	1580	0.617	60	58.5	
AS7	D10	519	0.617	616	197.3	
					D10	255.8 kg
					D16	3402.3 kg
					D20	3421.6 kg
TOTAL :						7079.7 kg
					CONCRETE :	43.4 m ³
					LEAN CONCRETE :	13.4 m ³
					ASPHATIC JOINT FILLER :	0.6 m ³

LIST REINFORCEMENT OF BORED PILES - L=66M (FOR ABUTMENTS)

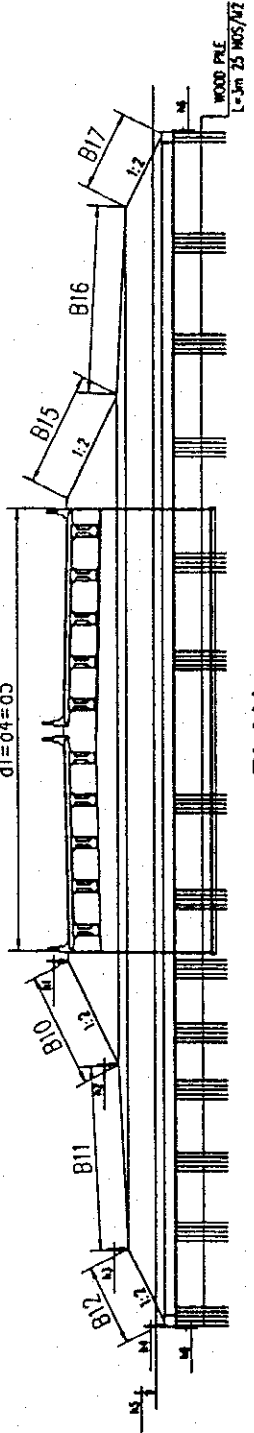
TYPE	D(mm)	LENGTH OF BAR (mm)	UNIT WEIGHT (kg/m)	NUMBER	WEIGHT (kg)	CONCRETE VOLUME (m ³)
N1	D32	12000	6.313	30	2272.7	116.63
N2	D25	12000	3.853	60	2774.2	
N3	D25	8000	3.853	15	462.4	
N4	D25	5400	3.853	15	312.1	
N5	D22	4203	2.984	6	75.3	
N6	D22	4248	2.984	22	278.9	
N7	D10	15205.3	0.617	1	93.8	
N8	D10	16725.8	0.617	1	103.2	
N9	D10	9617.36	0.617	1	593.4	
N10	D10	4105	0.617	98	248.2	
N11	D16	1322	1.578	16	33.4	
					D10	1038.6 kg
					D16	33.4 kg
					D22	354.2 kg
					D25	3548.7 kg
					D32	2272.7 kg
TOTAL						7247.6 kg

EARTHWORKS SLOPE PROTECTION

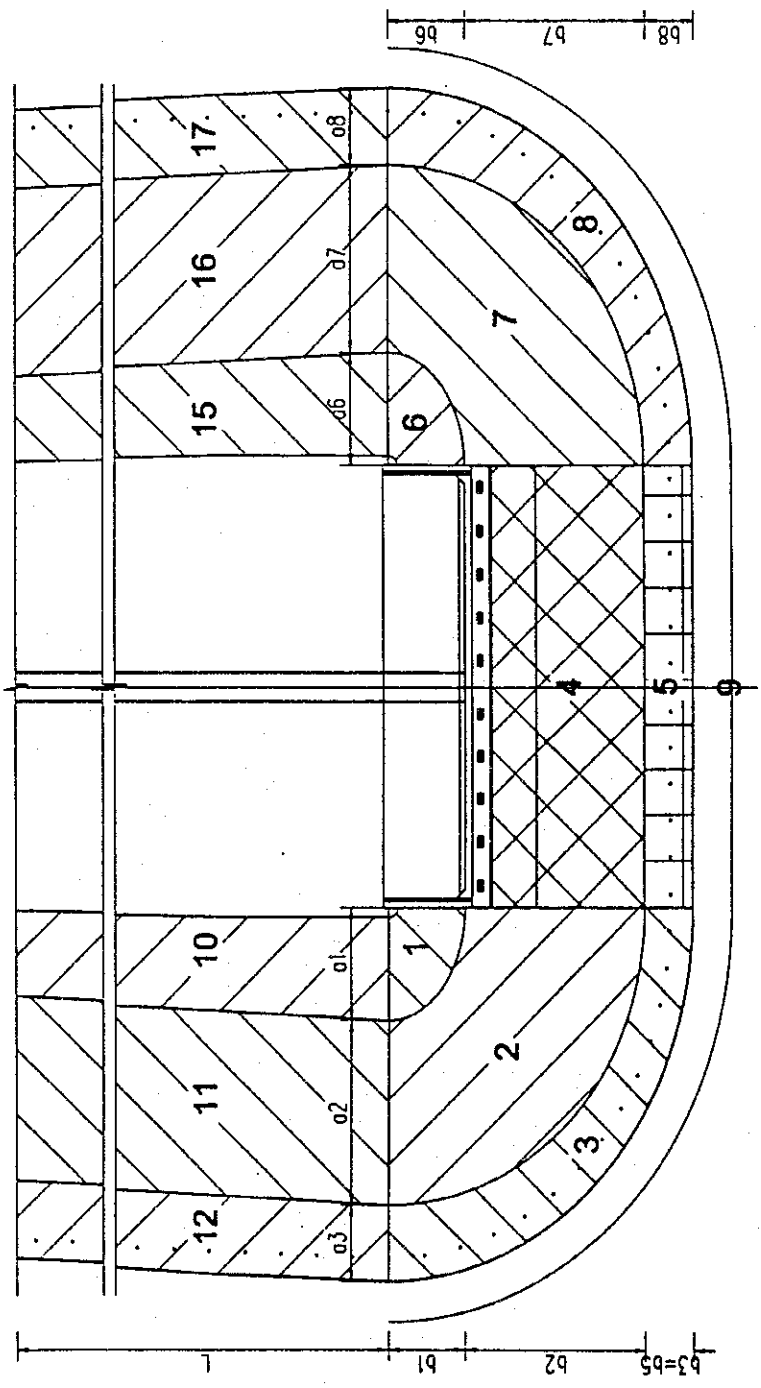
SIDE ELEVATION



A-A
d1=04=05



PLAN



QUANTITIES FOR EARTHWORKS SLOPE PROTECTION

Bridge **SMALL TRA VA**
Abutment **A1**

h1	h2	h3	h4	h5	h6	d1	d2
6.29	3.5	3	1	1.5	0.3	24.1	8.5

Block	a (m)	b (m)	Dh (m)	R (m)	r (m)	L (m)	Sxq (m ²)	W (m)	Masonry (m ³)	Blinding (m ³)
1	6.080	3.988	2.790	5.034	0.000	5.755	22.742		6.82	2.27
2	10.000	10.000	0.500	15.034	5.034	10.012	157.727		47.32	15.77
3	4.000	2.500	2.000	18.284	15.034	3.816	99.807		29.94	9.98
4	24.100	8.500	0.500				205.204		61.56	20.52
5	24.100	2.500	2.000				77.158		23.15	7.72
6	6.080	3.988	2.790	5.034	0.000	5.755	22.742		6.82	2.27
7	10.000	10.000	0.500	15.034	5.034	10.012	157.727		47.32	15.77
8	4.000	2.500	2.000	18.284	15.034	3.816	99.807		29.94	9.98
9			1.200				4.320	81.511		
Extra								71.306	253	84
							Masonry	(m ³)	253	
							Blinding	(m ³)	84	
							Geotextile	(m ²)	348	
							PVC Pipe	(m)	41	
							Excavation	(m ³)	352	
							Filling	(m ³)	245	
				ΣSxq (block 2,4,7)	(m ²)	521				
				ΣSxq	(m ²)	843				
FOOTING	WOOD PILE	(m)		4891						
	BLINDING	(m ³)	V=0.1*0.8*81.511	6.5						
	STONE MASONRY	(m ³)	V1=0.6*0.6*81.511	29.3						

Abutment **A2**

h1	h2	h3	h4	h5	h6	d1	d2
6.29	3.5	3	-0.218	0.282	-0.918	24.1	8.5

Block	a (m)	b (m)	Dh (m)	R (m)	r (m)	L (m)	Sxq (m ²)	W (m)	Masonry (m ³)	Blinding (m ³)
1	6.080	3.988	2.790	5.034	0.000	5.755	22.742		6.82	2.27
2	10.000	10.000	0.500	15.034	5.034	10.012	157.727		47.32	15.77
3	6.436	4.023	3.218	20.263	15.034	6.140	170.129		51.04	17.01
4	24.100	8.500	0.500				205.204		61.56	20.52
5	24.100	4.023	3.218				124.147		37.24	12.41
6	6.080	3.988	2.790	5.034	0.000	5.755	22.742		6.82	2.27
7	10.000	10.000	0.500	15.034	5.034	10.012	157.727		47.32	15.77
8	6.436	4.023	3.218	20.263	15.034	6.140	170.129		51.04	17.01
9			1.200				4.320	87.726		
Extra								71.306	309	103
							Masonry	(m ³)	309	
							Blinding	(m ³)	103	
							Geotextile	(m ²)	536	
							PVC Pipe	(m)	44	
							Excavation	(m ³)	379	
							Filling	(m ³)	263	
				ΣSxq (block 2,4,7)	(m ²)	521				
				ΣSxq	(m ²)	1031				
FOOTING	WOOD PILE	(m)		5264						
	BLINDING	(m ³)	V=0.1*0.8*87.726	7.0						
	STONE MASONRY	(m ³)	V1=0.6*0.6*87.726	31.6						

NOTES :

- * INPUT VALUES : GRAY CELLS ; TOTAL VALUES : BOLD NUMBERS
- * No. of Blocks : see drawing. Extra block used for calculation only.
- h_i : Elevations (see drawing)
- d_i : Distances (see drawing)

**SMALL TRA VA BRIDGE : QUANTITIES OF EARTHWORKS SLOPE PROTECTION OF 25M BEHIND HEAD WALL
ABUTMENT A1:**

Block	B m	L m	S m ²	Thick m	Masonry m ³	Blinding (T=0.1m) m ³	
10	6.24	25	155.97	0.30	46.79	15.60	
11	10.01	25	250.31	0.30	75.09	25.03	
12	4.47	25	111.80	0.30	33.54	11.18	Geotextile (m ²)
15	6.24	25	155.97	0.30	46.79	15.60	= 2 x (L + S) =
16	10.01	25	250.31	0.30	75.09	25.03	PVC pipe (m)
17	4.47	25	111.80	0.30	33.54	11.18	= 2 x L/2 x 1m =
TOTAL			1036.16		310.85	103.62	274
FOOTING	0.8	25	20.00	0.1		4.00	
	0.6	25	15.00	0.6	18.00		
Wood pile (m) =3000							

No	h m	Dh m	S m ²	L m	Total (m ³)		Excavation = 2 x S x L Filling = Excavation - 2 x 25 x Dh x 1.1
					Excavation	Filling	
5	1.5						
6	0.3	1.2	4.32	25	216	150	

ABUTMENT A2:

Block	B m	L m	S m ²	Thick m	Masonry m ³	Blinding (T=0.1m) m ³	
10	6.24	25	155.97	0.30	46.79	15.60	Geotextile (m ²)
11	10.01	25	250.31	0.30	75.09	25.03	= 2 x (L*1 + S) =
12	7.20	25	179.89	0.30	53.97	17.99	PVC pipe (m)
15	6.24	25	155.97	0.30	46.79	15.60	= 2 x L/2 x 1m =
16	10.01	25	250.31	0.30	75.09	25.03	
17	7.20	25	179.89	0.30	53.97	17.99	
TOTAL			1172.34		351.70	117.23	410
FOOTING	0.8	25	20.00	0.1		4.00	
	0.6	25	15.00	0.6	18.00		
Wood pile (m) =3000							

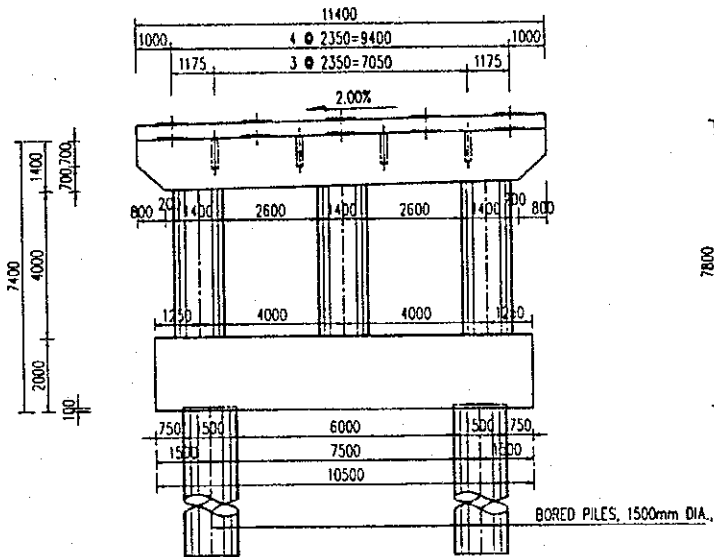
No	h m	Dh m	S m ²	L m	Total (m ³)		Excavation = 2 x S x L Filling = Excavation - 2 x 25 x Dh x 1.1
					Excavation	Filling	
5	0.282						
6	-0.918	1.2	4.32	25	216	150	

QUANTITY TABLE OF PIER

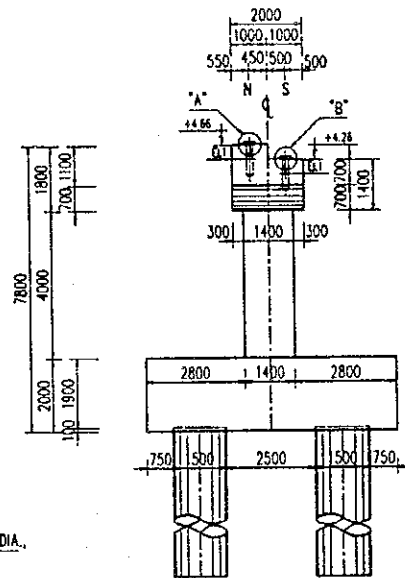
Items		Unit	Pier 1	Pier 2	Total	
Pile	Number of piles	Pile	8	8	16	
	Total length bored piles D=1500mm Dia.	m	512.0	512.0	1024.0	
	Concrete piles class D	m ³	904.8	904.8	1809.6	
	Excavation	m ³	961.3	964.2	1925.5	
	Reinforcement	D10	kg	7845.6	7845.6	15691.2
		D16	kg	267.2	267.2	534.4
		D22	kg	2839.2	2839.2	5678.4
		D25	kg	30153.6	30153.6	60307.2
Total		kg	41105.6	41105.6	82211.2	
Pier	Concrete class E	m ³	401.3	401.3	802.6	
	Reinforcement	D10	kg	34.6	34.6	69.2
		D14	kg	1829.9	1829.9	3659.8
		D16	kg	920.8	920.8	1841.6
		D20	kg	702.3	702.3	1404.6
		D22	kg	3707.4	3707.4	7414.8
		D28	kg	1786.0	1786.0	3572.0
		D32	kg	7428.8	7428.8	14857.6
		Total	kg	16409.8	16409.8	32819.6
	Form	Curve	m ²	108.6	108.6	217.1
		Flat	m ²	265.5	265.5	531.0
	Scaffoldingding work	H < 4m	m ²	254.4	254.4	508.8
		4m ≤ H < 30m	m ²	237.6	237.6	475.3
	Support		m ³	118.5	118.5	237.0
	Earth work	Excavation for foundation	m ³	1351.5	1410.6	2762.2
		Blinding Concrete class G	m ³	281.2	281.2	562.4
		Rip rap	m ³	777.7	836.8	1614.6
	Cofferdams	Sheet pile Larsen IV	m	2328.0	2328.0	4656.0
		Steel pile I 400	m	480.0	480.0	960.0
		Brace C 300	m	784.0	784.0	1568.0

PIERS P1 & P2 OF SMALL TRA VA BRIDGE

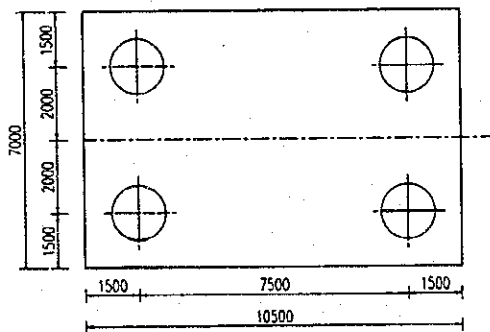
ELEVATION



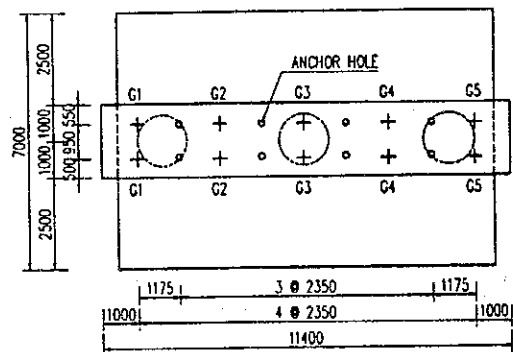
SIDE VIEW



HALF PLAN PILE FOOTING



HALF PLAN



NOTES

ALL DIMENSIONS ARE IN MILLIMETERS UNLESS OTHERWISE INDICATED.

QUANTITY SURVEYING FOR PIER P1

No. _____

Item	Formula	Quantity																		
1) Concrete	<p>* Headstock</p> $v1 = \left(\left(11.40 \times 1.40 - 0.80 \times 0.70 \right) \times 1.00 + \left(11.40 \times 1.80 - 0.80 \times 0.70 \right) \times 1.00 \right) \times 2 = 70.72 \text{ m}^3$ <p>* Column</p> $v2 = 2 \times \frac{1}{4} \times \pi \times 1.40^2 \times \left(4.03 + 4.11 + 4.19 \right) = 38.00 \text{ m}^3$ <p>* Footing</p> $v4 = 2 \times 10.50 \times 7.00 \times 2.00 - 8 \times \pi \times 0.75^2 \times 0.1 = 292.59 \text{ m}^3$ <p style="text-align: right;">Total</p>	401.30 m ³																		
2) Form	<p>* Headstock</p> $a1 = \left(\left(11.40 \times 1.40 - 0.80 \times 0.70 \right) \times 1.00 \times 2 + \left(1.06 + 0.70 \right) \times 3 \times \frac{\pi}{4} \times 1.40^2 + \left(9.80 \times 2.00 - 0.80 \times 0.70 \right) \times 2 + \left(11.40 \times 0.40 \right) \times 2 \right) = 125.51 \text{ m}^2$ <p>* Column (Curve form)</p> $a2 = 2 \times \pi \times 1.40 \times \left(4.03 + 4.11 + 4.19 \right) = 108.57 \text{ m}^2$ <p>* Footing</p> $a4 = 2 \times \left(10.50 + 7.00 \right) \times 2.00 \times 2 = 140.00 \text{ m}^2$ <p style="text-align: right;">Total</p>	108.57 m ² 265.51 m ²																		
3) Scaffolding:	<p>* H<=4m</p> $A1 = 2 \times \left(24.85 + 9.00 \right) \times 2.00 + \left(2 \times \left(4.00 + 25.75 \right) \right) \times 2.00 = 254.40 \text{ m}^2$ <p>* 4m< H<=30m</p> $A2 = 2 \times \left(4.00 + 25.75 \right) \times 3.99 = 237.64 \text{ m}^2$	254.40 m ² 237.64 m ²																		
4) Support	$= \left(11.40 - 3 \times 1.40 \right) \times 2.00 + \left(11.40 - 3 \times 4.11 \right) \times 2 = 118.48 \text{ m}^3$	118.48 m ³																		
5) Lean Concrete	<p>* Concrete class G</p> $V = 0.50 \times \left(\left(22.85 + 4 \right) \times \left(7.00 + 4 \right) - 8 \times \frac{1}{4} \times \pi \times 1.50^2 \right) \times 2 = 281.21 \text{ m}^3$	281.21 m ³																		
6) Bored Pile	<p>* Concrete</p> $= 8 \times \frac{1}{4} \times \pi \times 1.50^2 \times \left(63.90 + 0.10 \right) = 904.78 \text{ m}^3$ <p>* Excavation Length</p> <table style="width: 100%; border: none;"> <tr> <td style="width: 30%;">N<20:</td> <td style="width: 10%;">=</td> <td style="width: 20%;">50.1 x</td> <td style="width: 10%;">8</td> <td style="width: 10%;">=</td> <td style="width: 10%;">400.80 m</td> </tr> <tr> <td>20<N<40:</td> <td></td> <td>13.3 x</td> <td>8</td> <td>=</td> <td>106.40 m</td> </tr> <tr> <td>40<N:</td> <td></td> <td>0.5 x</td> <td>8</td> <td>=</td> <td>4.00 m</td> </tr> </table>	N<20:	=	50.1 x	8	=	400.80 m	20<N<40:		13.3 x	8	=	106.40 m	40<N:		0.5 x	8	=	4.00 m	904.78 m ³ 400.80 m 106.40 m 4.00 m
N<20:	=	50.1 x	8	=	400.80 m															
20<N<40:		13.3 x	8	=	106.40 m															
40<N:		0.5 x	8	=	4.00 m															

QUANTITY SURVEYING FOR PIER P1

No. _____

Item	Formula	Quantity	
7) Earthwork	* Excavation Volume = $8 \times \frac{1}{4} \times \pi \times 1.50 \times 1.50 \times 68.00$	= 961.33 m3	
	* Excavation for footing (inside cofferdam) = $4.60 \times (26.85 \times 11.00) - 8 \times \pi / 4 \times 1.50^2 \times 0.50$	= 1351.54 m3	
	* Excess Soil		
	v1 = Lean Concrete	= 281.21 m3	
	v2 = Blinding Stone	= 0.00 m3	
	v3 = Footing Volume	= 292.59 m3	
	Total	= 573.80 m3	
	* Riprap = Excavation for footing - Excess Soil = 1351.54 - 573.80	= 777.74 m3	
	8) Cofferdams	* Sheet pile larsen IV L = 12 x 194	= 2328 m
		* Steel pile I 400 L = 12 x 40	= 480 m
* Brace C 300			
L1 = 27.9 x 10 = 279 m			
L2 = 27.1 x 10 = 271 m			
L3 = 12.1 x 10 = 121 m			
L4 = 11.3 x 10 = 113 m			
Total	= 784 m		

QUANTITY SURVEYING FOR PIER P2

No. _____

Item	Formula	Quantity																		
1) Concrete	<p>* Headstock</p> $v1 = \{ (11.40 \times 1.40 - 0.80 \times 0.70) + (11.40 \times 1.80 - 0.80 \times 0.70) \} \times 1.00 \times 2 = 70.72 \text{ m}^3$ <p>* Column</p> $v2 = 2 \times \frac{1}{4} \times \pi \times 1.40^2 \times (4.03 + 4.11 + 4.19) = 38.00 \text{ m}^3$ <p>* Footing</p> $v4 = 2 \times 10.50 \times 7.00 \times 2.00 - 8 \times \pi \times 0.75^2 \times 0.1 = 292.59 \text{ m}^3$ <p style="text-align: right;">Total</p>	401.30 m ³																		
2) Form	<p>* Headstock</p> $a1 = \{ (11.40 \times 1.40 - 0.80 \times 0.70) + (1.06 + 0.70) \times 1.00 \times 2 + (9.80 \times 2.00 - 3 \times \pi / 4 \times 1.40^2) + (11.40 \times 1.80 - 0.80 \times 0.70) + (11.40 \times 0.40) \} \times 2 = 125.51 \text{ m}^2$ <p>* Column (Curve form)</p> $a2 = 2 \times \pi \times 1.40 \times (4.03 + 4.11 + 4.19) = 108.57 \text{ m}^2$ <p>* Footing</p> $a4 = 2 \times (10.500 + 7.000) \times \frac{2.00}{2} = 140.00 \text{ m}^2$ <p style="text-align: right;">Total</p>	108.57 m ² 265.51 m ²																		
3) Scaffolding:	<p>* H<=4m</p> $A1 = 2 \times (24.85 + 9.00) \times 2.00 + (2 \times (4.00 + 25.75) \times 2.00 = 254.40 \text{ m}^2$ <p>* 4m<H<=30m</p> $A2 = 2 \times (4.00 + 25.75) \times 3.99 = 237.64 \text{ m}^2$	254.40 m ² 237.64 m ²																		
4) Support	$= (11.40 - 3 \times 1.40) \times 2.00 + (4.11 \times 2) = 118.48 \text{ m}^3$	118.48 m ³																		
5) Lean Concrete	<p>* Concrete class G</p> $V = 0.50 \times \{ (22.85 + 4) \times (7.00 + 1.50) + 4 \times \frac{1}{4} \times \pi \times 1.50^2 \} = 281.21 \text{ m}^3$	281.21 m ³																		
6) Bored Pile	<p>* Concrete</p> $= 8 \times \frac{1}{4} \times \pi \times 1.50^2 \times (63.90 + 0.10) = 904.78 \text{ m}^3$ <p>* Excavation Length</p> <table style="width: 100%; border: none;"> <tr> <td style="width: 50%;">N<20:</td> <td style="width: 20%;">=</td> <td style="width: 15%;">53.8 x</td> <td style="width: 10%;">8</td> <td style="width: 10%;">=</td> <td style="width: 10%;">430.40 m</td> </tr> <tr> <td>20<N<40:</td> <td></td> <td>9.3 x</td> <td>8</td> <td>=</td> <td>74.00 m</td> </tr> <tr> <td>40<N:</td> <td></td> <td>0.9 x</td> <td>8</td> <td>=</td> <td>6.80 m</td> </tr> </table>	N<20:	=	53.8 x	8	=	430.40 m	20<N<40:		9.3 x	8	=	74.00 m	40<N:		0.9 x	8	=	6.80 m	904.78 m ³ 430.40 m 74.00 m 6.80 m
N<20:	=	53.8 x	8	=	430.40 m															
20<N<40:		9.3 x	8	=	74.00 m															
40<N:		0.9 x	8	=	6.80 m															

QUANTITY SURVEYING FOR PIER P2

No. _____

Item	Formula	Quantity	
7) Earthwork	* Excavation Volume = $8 \times \frac{1}{4} \times \pi \times 1.50 \times 1.50$ x 68.20 =	964.15 m ³	
	* Excavation for footing (inside cofferdam) = $4.80 \times (26.85 \times 11.00)$ - $8 \times \pi / 4 \times 1.50^2 \times 0.50$ =	1410.61 m ³	
	* Excess Soil		
	v1 = Lean Concrete =	281.21 m ³	
	v2 = Blinding Stone =	0.00 m ³	
	v3 = Footing Volume =	292.59 m ³	
	Total =	573.80 m ³	
	* Riprap = Excavation for footing - Excess Soil = 1410.61 - 573.80	=	836.81 m ³
	8) Cofferdams	* Sheet pile larsen IV L = 12 x 194	= 2328 m
		* Steel pile I 400 L = 12 x 40	= 480 m
* Brace C 300			
L1 = 27.9 x 10 =		279 m	
L2 = 27.1 x 10 =		271 m	
L3 = 12.1 x 10 =		121 m	
L4 = 11.3 x 10 =		113 m	
Total =	784 m		

STEEL OF PIER 1&2

DETAIL	No	D (mm)	LENGTH (m)	QUANTITY (nos.)	UNIT WEIGH (kg/m)	TOTAL STEEL (kg)
PIER CAP	R1	10	5600	10	0.617	34.6
	B1	28	11200	14	4.834	758.0
	B2	28	9800	13	4.834	615.9
	B3	16	11200	9	1.578	159.1
	B3a	16	10490	4	1.578	66.2
	B4	14	4916	49	1.208	291.0
	B4a	14	4666	49	1.208	276.2
	B4b	14	2182	49	1.208	129.2
	B4c	14	3566	49	1.208	211.1
	B4d	14	2332	49	1.208	138.0
	B5	14	4151	8	1.208	40.1
	B5a	14	3901	8	1.208	37.7
	B5b	14	1800	6	1.208	13.0
	B5c	14	2801	6	1.208	20.3
	B5d	14	2332	14	1.208	39.4
	B6	28	3393	7	4.834	114.8
	B6a	28	3793	5	4.834	91.7
	B7	28	3377	7	4.834	114.3
	B7a	28	3777	5	4.834	91.3
	COLUMN	C1	32	7510	84	6.313
C2		14	4485	117	1.208	633.9
FOOTING	F1	22	11070	25	2.984	825.8
	F2	22	14587	47	2.984	2045.8
	F3	32	10300	53	6.313	3446.3
	F4	22	7570	37	2.984	835.8
	F5	20	7500	16	2.466	295.9
	F6	20	10300	16	2.466	406.4
	F7	16	4689	94	1.578	695.5
TOTAL	D = 10		34.6			(kg)
	D = 14		1829.9			(kg)
	D = 16		920.8			(kg)
	D = 20		702.3			(kg)
	D = 22		3707.4			(kg)
	D = 28		1786.0			(kg)
	D = 32		7428.8			(kg)
	TOTAL		8981.0			(kg)

**LIST REINFORCEMENT OF BORED PILE - L= 64m.
(FOR PIER)**

TYPE	D(mm)	LENGTH OF BAR (mm)	UNIT WEIGHT (kg/m)	NUMBER	WEIGHT (kg)	CONCRETE VOLUME (m ³)
N1	D25	12000	3.853	70	3236.5	
N2	D25	9875	3.853	14	532.7	
N3	D22	4228	2.984	28	534.9	
N4	D10	152053	0.617	1	93.8	
N5	D10	182464	0.617	1	112.6	
N6	D10	938928	0.617	1	579.3	
N7	D10	4105	0.617	77	195.0	
N8	D16	1322	1.578	16	33.4	
					D10	980.7 kg
					D16	33.4 kg
					D22	354.9 kg
					D25	3789.2 kg
					TOTAL	5138.2 kg
						113.1

QUANTITY OF SMALL TRA VA BRIDGE

ITEMS		UNIT	ABUTMENTS	PIERS	SUPERSTRUCTERE	TOTAL
CONCRETE	CLASS B	M3			609.8	609.8
	CLASS D	M3	2332.6	1809.6	645.8	4788.0
	CLASS E	M3	1252.0	802.6	89.3	2144.0
	CLASS G	M3	60.5	562.4		623.0
PC - STEEL	12 S 12.7	ton			30.9	30.9
	3 S 12.7	ton			1.7	1.7
SHEATHING	CABLES P 80/85	M			3322.9	3322.9
	CABLES P 50/55	M			723.8	723.8
CEMENT GROUT IN SHEATHING		M3			18.1	18.1
ANCHORAGE	CABLES 12S12.7	SET			220.0	220.0
	CABLES 3S12.7	SET			144.0	144.0
STEEL SHEAR KEY		SET			240.0	240.0
REINFORCEMENT	RE-BAR	TON	220.8	115.0	259943.2	260279.0
EXPANSION JOINT	50MM	M			43.0	43.0
BEARING	600x300x57	SET			20.0	20.0
BEARING	500x250x50	SET			40.0	40.0
ANCHORAGE BAR		SET			48.0	48.0
PVC PIPE	P 50 MM	M	135.0			135.0
	P 100 MM	M			350.0	350.0
RAILING		M			367.6	367.6
LIGHTING POLE		SET			2.0	2.0
DRAINAGE	POT	SET			14.0	14.0
	PIPE P 165 MM	M			24.4	24.4
PAVEMENT	WATER PROOFING 5 MM	M2			1861.9	1861.9
	ALPHALT CONCRETE 70 MM	M2			1861.9	1861.9
PILE P150CM		M	1320.0	1024.0		2344.0
GEOTEXTILE		M2	1568.0			1568.0
STONE MANSORY		M3	1224.6			1224.6
BLINGDING AGGREGATE		M3	407.9			407.9
FOOTING OF SLOPE ROTECTION		M	219.2			219.2
RIP RAP		M3		1614.6		1614.6
BLINDING STONE		M3	67.8			67.8
WOODEN PIPE L = 3M		M	16155.0			16155.0
EXCAVATION		M3	3581.9	2762.2		6344.0
FILLING		M3	1945.5			1945.5
FORM WORK	CURVE	M2		217.1		217.1
	FLAT	M2	1054.6	531.0	6488.4	8074.0
SCAFFOLDING WORK	H < 4M	M2	284.8	508.8	2114.1	2907.7
	4 M < H < 30M	M2	924.1	475.3		1399.4
SUPPORT		M3		237.0		237.0
COFFERDAMS	SHEET PILE LARSEN IV	M		4656		4656.0
	STEEL PILE I 400	M		960		960.0
	BRACE C 300	M		1568		1568.0