

4.4.5. Slope Stability

Slopes in the Study Area are categorised as follows:

Large-scale slope

- The northern edge of the Study Area is at the foot of the Alborz Mountain and steep slope is distributed throughout.
- The Northern half of the Study area consists of alluvial fans. A gentle slope with the same gradient is prevailing in the area.
- A deep valley is distributed alongside a major river in the northern part of the Study area.
- Elevation data from the Beautification Organisation is available. It is possible to determine the slope gradient within every 50-mesh unit. Statistical treatment is applicable.

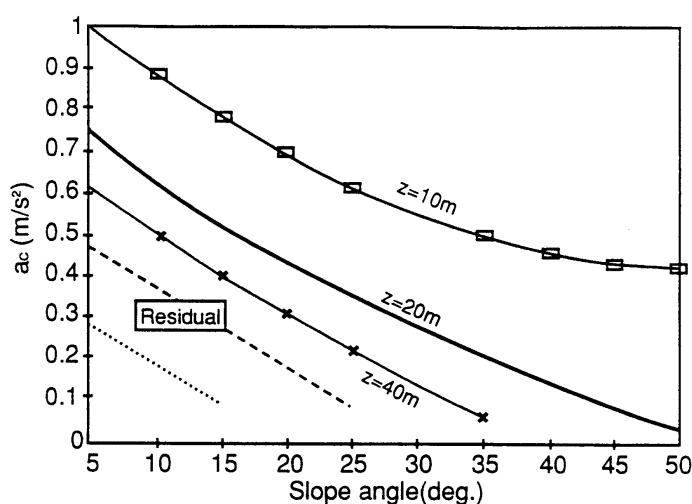
Small-scale slope

- Cut slopes are distributed alongside major highways in the northern part of the Study Area. Most of these have no slope protection and tall buildings are constructed on top of the slopes.
- Information on location of slope, shape of slope and soil strength is not available.

Considering the above condition, only large-scale slopes were examined in the Study. For case of small-scale slopes, it is recommended that the stability of each slope be analysed.

(1) Method of the Slope Failure Evaluation

Komak Panah and Hefezi Moghaddas (1992) studied slope failure caused by the 1990 Majil earthquake in Iran and compiled slope stability chart as shown in Figure 4.4.6.



z: depth from ground surface to sliding surface
 a_c : peak ground acceleration (m/sec²)

Figure 4.4.6 Judgement for the Slope Stability, Panah et al.(1993)

This chart indicates the relationship between stable slope gradient and peak ground acceleration. The studied slopes are distributed in the western Alborz Mountain, which belong to the same mountain range of the Study area.

In this study, the slope gradient for each 50-m mesh was calculated first. Then the slope stability of each mesh was judged based upon the chart, taking into account the peak ground acceleration value for the mesh. The $z = 20\text{m}$ curve in the chart was used in this Study.

In stability judgement, a score $F = 0$ for a stable mesh or $F = 1$ for an unstable mesh was given. Based on this judgement, the stability ranking for each 500 m mesh was estimated by using following formula:

$$Score = \sum_{i=1}^{100} F_i$$

Rank 3	Score 60-100	60-100% of the area is unstable
Rank 2	Score 30-60	30-60% of the area is unstable
Rank 1	Score 0 – 30	0-30% of the area is unstable
Rank 0	Score 0	All of the area is stable

(2) Slope Stability

Ray Fault model, Mosha Fault model, Floating model

Most of the meshes are judged as stable.

NTF model

Many meshes at the edge of the Alborz Mountain are judged as unstable.

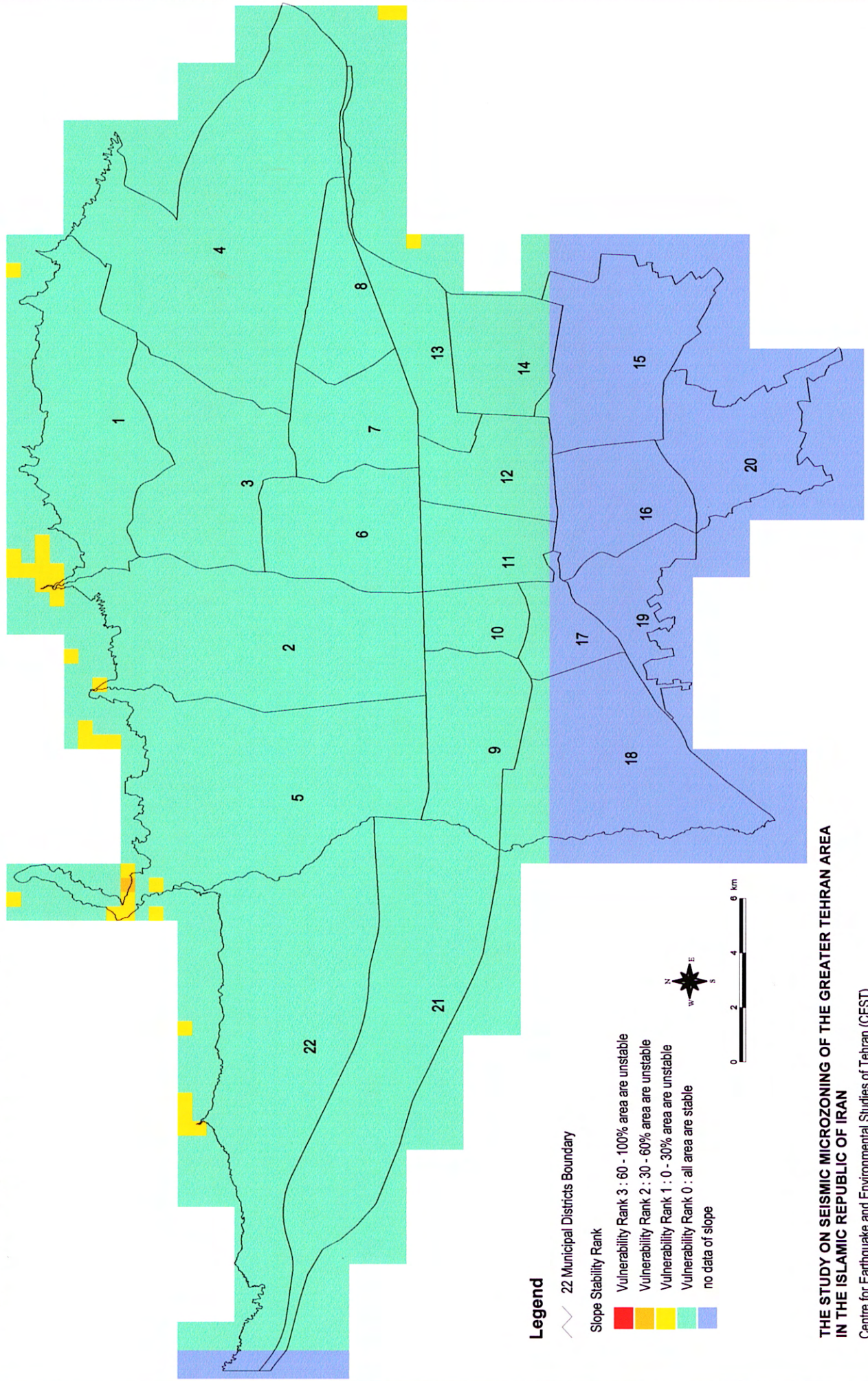
Most of these unstable meshes are not in the residential area, but many houses exist in the Rudkahneh-ye-Suleqan valley. Raw water transmission tunnels are planned to be constructed in the high-risk area. Furthermore, slopes located behind the oil tank are judged as high-risk.

The analysis shows that there is not very much slope-failure risk in the hill, terrace and fan areas, which are mainly used for residential and commercial purposes now. However, the analysis in the Study is limited to large-scale slope failures and landslides. It should be noted that many small-scale slope failures and stone failures will occur during an earthquake. In addition, cut slopes will also be unstable during an earthquake.

It is recommended that a detailed geological survey and numerical analysis for residential areas and important facilities found on slopes of high risk be conducted. Based on these results, slope protection measures should be executed for critical and unstable slopes. Further, an integrated and detailed slope study should be conducted for the entire city of Tehran. Based on the result of this study, regulation for land-use and standards for slope protection shall be established.

Figure 4.4.8

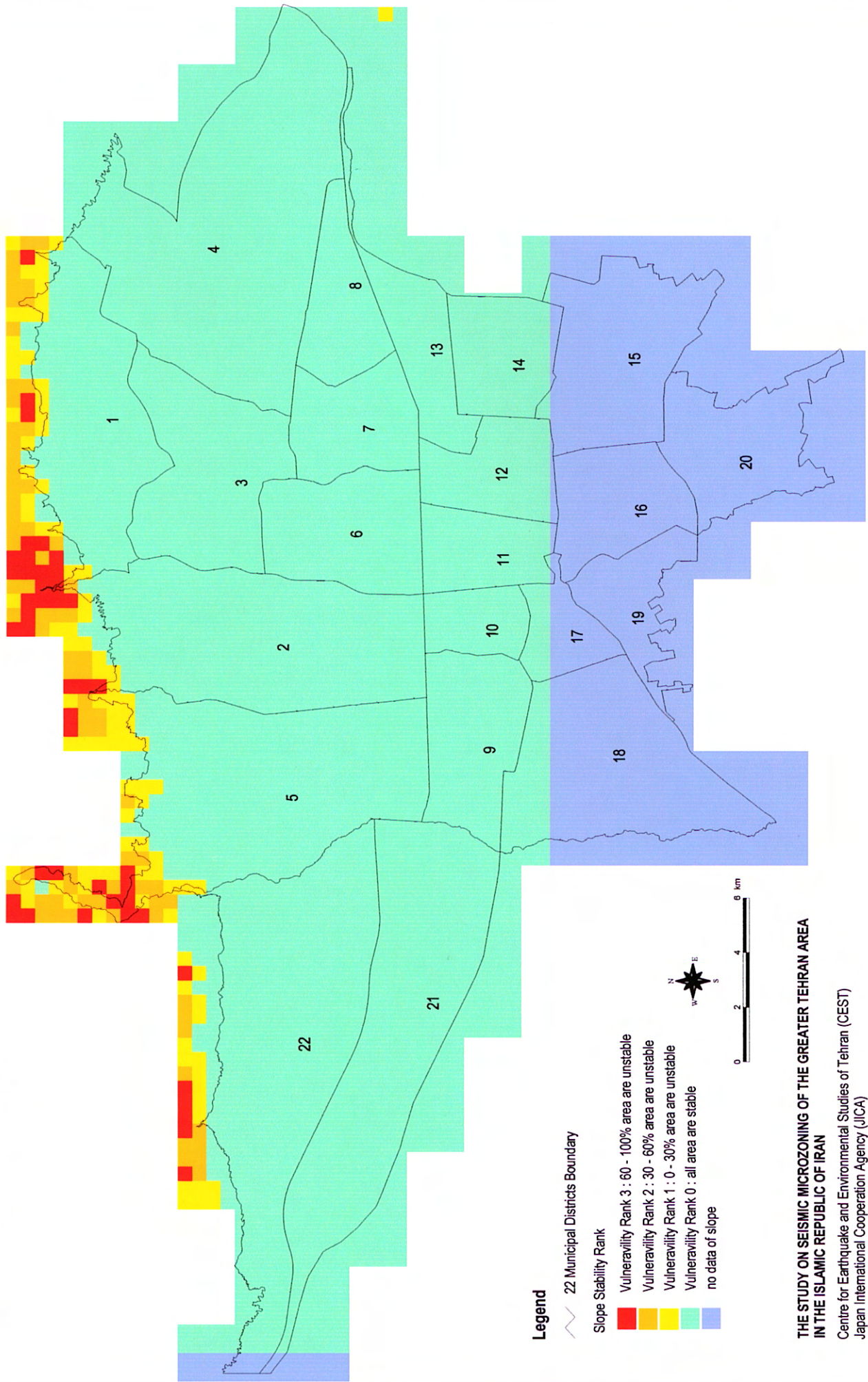
Slope Stability (Ray Fault model)



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Figure 4.4.9

Slope Stability (NTF model)



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