

Table 4.1.8 (2/7) CALCULATION SHEET FOR TEMPORARY WORK AND RAIL WORK-1

ID No.	Working Name		Calculation Quantity		Remarks		Unit Cost			Cost			Remarks
CW-4-7	Sand Bags		10 m				PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Major Item	ID No.	Description	Unit	Quantity	PF/C	IF/C	L/C	PF/C	IF/C	L/C			
Labour	L-2-1	Foreman	day	1.6	0	0	48800	0	0	78,080			
	L-2-23	Common Labour	day	8	0	0	35100	0	0	280,800			
Material	M-B-9	Soil for Backfilling	m ³	3	0	400	7600	0	1,200	22,800	50% of Soil is purchased		
	M-M-15	Plastic Sack	nos.	160	0	750	1750	0	120,000	280,000			
Working Base Cost													
	CW-1-46	Excavation A	m ³	6	2361	39	1711	14,166	234	10,266			
Total for			10 m					14,166	121,434	671,946			
Unit Cost for			1 m					1,417	12,143	67,195			
Unit Cost for			1 nos					89	759	4,200			

* 1: Labor Rate : Foreman : 1
Common Labor : 5
Daily Work Volume : 20 nos. / 1 common labor

ID No.	Working Name		Calculation Quantity		Remarks		Unit Cost			Cost			Remarks
CW-4-8	Temporary Steel Sheet Pile (Type-C)		4 m		L=9.0 long and 6.0m of Driving and Pulling Out (Type-II)		PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Major Item	ID No.	Description	Unit	Quantity	PF/C	IF/C	L/C	PF/C	IF/C	L/C			
Material	M-E-30	Steel Sheet Pile (Lease)	kg day	861000	16.30658	0	10.87105	14,088.884	0	9,392.589			
Working Base Cost													
	CW-3-9	Driving In of Steel Sheet Pile (Type-II)	m	6	9909.067	76.17755	8578.669	59,454	457	51,472			
	CW-3-10	Pulling Out of Steel Sheet Pile (Type-II)	m	6	9754	67	8548	58,524	402	51,288			
Total for			4 m					14,206,863	859	9,495,349			
Unit Cost for			1 m					3,551,716	215	2,373,837			
Unit Cost for			1 nos					1,420,686	86	949,535			

* 1: Steel Sheet Pile : 48 kg/m x 9 m long x 4 m dstnc / 0.4 m wide
4320 kg
* 2: Leasing Term : 200 days

ID No.	Working Name		Calculation Quantity		Remarks		Unit Cost			Cost			Remarks
CW-4-9	Installation of Tierod and Wale (Temporary)		10 ton		Excluding Material		PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Major Item	ID No.	Description	Unit	Quantity	PF/C	IF/C	L/C	PF/C	IF/C	L/C			
Equipment	A-2-2-58	Shotcrete Machine Wet Type : 0.8-1.2	hourly	5	68498.33	0	40476.29	342,492	0	202,381			
	A-2-1-72	Truck Crane; 16 ton, Oil Pressure	hourly	33	135641.1	1020	115858.5	4,476,156	33,660	3,823,330			
	L-2-1	Foreman	day	5	0	0	48800	0	0	244,000			
Labour	L-2-13	Rigger	day	10	0	0	39000	0	0	390,000			
	L-2-6	Welder	day	5	0	0	39000	0	0	195,000			
	L-2-23	Common Labour	day	5	0	0	35100	0	0	175,500			
Others		Miscellaneous	L.S.					385,552	2,740	402,489	8%		
Total for			10 ton					5,204,200	36,400	5,432,700			
Unit Cost for			1 ton					520,420	3,640	543,270			

* 1: All rates are quoted from Japanese Manual (P294)

ID No.	Working Name		Calculation Quantity		Remarks		Unit Cost			Cost			Remarks
CW-4-10	Removal of Tierod and Wale (Temporary)		10 ton		Excluding Material		PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Major Item	ID No.	Description	Unit	Quantity	PF/C	IF/C	L/C	PF/C	IF/C	L/C			
Equipment	A-2-1-72	Truck Crane; 16 ton, Oil Pressure	hourly	20	135641.1	1020	115858.5	2,712,822	20,400	2,317,170			
Labour	L-2-1	Foreman	day	3	0	0	48800	0	0	146,400			
	L-2-13	Rigger	day	5	0	0	39000	0	0	195,000			
	L-2-23	Common Labour	day	3	0	0	35100	0	0	105,300			
Others		Miscellaneous	L.S.					244,178	1,900	248,830	9%		
Total for			10 ton					2,957,000	22,300	3,012,700			
Unit Cost for			1 ton					295,700	2,230	301,270			

* 1: All rates are quoted from Japanese Manual (P294)

Table 4.1.8 (3/7) CALCULATION SHEET FOR TEMPORARY WORK AND RAIL WORK-1

ID No.	Working Name	Calculation Quantity	Remarks								
CW-4-11	Temporary Double Steel Sheet Pile	4 m	L=9.0 and 15.0m long and 4.7 and 10.7m of Driving and Pulling Out (Type-II)								
Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Material											
	M-B-9	Soil for Backfilling	m ³	55.9	0	400	7600	0	22,360	424,840	50% of Total Volume
		Structural Steel(Purchasing), SS41	kg	114.9075	5225	0	275	600,392	0	31,600	
	M-E-12	Tierod (Lease)	kg day	56250	60	0	40	3,375,000	0	2,250,000	
	M-E-77	C-beam (Lease), SS41	kg day	96960	3,623,684	0	2,415,789	351,352	0	234,235	
	M-E-30	Steel Sheet Pile (Lease)	kg day	2304000	16,30658	0	10,87105	37,570,358	0	25,046,905	
Working Base Cost											
	CW-1-46	Excavation A	m ³	111.8	2361	39	1711	261,960	4,360	191,290	
	CW-1-8	Tamper Loading	m ³	111.8	1760	60	2600	196,768	6,708	290,680	
	CW-4-9	Installation of Tierod and Wale (Temporary)	ton	4.7	520420	3640	543270	2,445,974	17,108	2,553,369	
	CW-4-10	Removal of Tierod and Wale (Temporary)	ton	4.7	295700	2230	301270	1,389,790	10,481	1,415,969	
	CW-3-9	Driving In of Steel Sheet Pile (Type-II)	m	4.7	9909.067	76.17755	8578.669	46,573	358	40,320	River Side
	CW-3-10	Pulling Out of Steel Sheet Pile (Type-II)	m	4.7	9754	67	8548	45,844	315	40,176	River Side
	CW-3-9	Driving In of Steel Sheet Pile (Type-II)	m	10.7	9909.067	76.17755	8578.669	106,027	815	91,792	Field Side
	CW-3-10	Pulling Out of Steel Sheet Pile (Type-II)	m	10.7	9754	67	8548	104,368	717	91,464	Field Side
Total for		4 m						46,496,405	63,222	32,702,638	
Unit Cost for		1 m						11,624,101	15,806	8,175,660	
Unit Cost for		1 set						4,649,641	6,322	3,270,264	

- * 1 : Steel Sheet Pile : 48 kg/m x 24 m long x 4 m dstnc / 0.4 m wide
= 11520 kg
- * 2 : Tie Rod Dia 42mm : 15 kg/m x 7.5 m long x 4 m dstnc / 1.6 m pitch
= 281.25 kg
- * 3 : Channel Beam 200x90 : 30.3 kg/m x 4 m dstnc x 4 pieces = 484.8 kg
- * 4 : Structural Steel SS400 : 15 % of Total of Tierod and Beam = 114,9075 kg
- * 5 : Soil Volume : 6.5 m wide x 4.3 m high x 4 m = 111.8 m³
- * 6 : Leasing Term : 200 days

ID No.	Working Name	Calculation Quantity	Remarks								
CW-4-12	Temporary Dewatering by D100mm	50 m	Width is 3m. Number of Working Day is 180 days including Installation and Removal								
Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Equipment											
	A-2-2-47	Submergible Pump; D100mm 5.5 kW	daily	60	26,903	0	13,589	1,614,187	0	815,329	for Driving
	A-2-2-15	Generator; 100 kVA	daily	60	215064	10800	160745.1	12,903,843	648,000	9,644,708	for Driving
	A-2-1-75	Truck Crane; 4.9 ton, Oil Pressure	hourly	1	55145.89	720	48324.47	55,146	720	48,324	for Installation and Removal
Labour											
	L-2-1	Foreman	day	0.2	0	0	48800	0	0	9,760	for Installation and Removal
	L-2-23	Common Labour	day	2.8	0	0	35100	0	0	98,280	for Installation and Removal
	L-2-4	Electrician	day	0.17	0	0	39000	0	0	6,630	for Driving
Total for		50 m						14,573,175	648,720	10,623,031	
Unit Cost for		1 m						291,464	12,974	212,461	
Unit Cost for		1 place						14,573,175	648,720	10,623,031	

Table 4.1.8 (4/7) CALCULATION SHEET FOR TEMPORARY WORK AND RAIL WORK-1

ID No.	Working Name	Calculation Quantity	Remarks									
CW-4-17	Temporary Double Steel Sheet Pile for Drainage Component	4 m	L=8m long and 3.3m of Driving and Pulling Out (Type-II)									
Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks	
Working Base Cost												
	CW-1-1	Backfill (Soil) A	m ³	165.12	6076	87	5043	1,003,269	14,365	832,700		
	CW-1-46	Excavation A	m ³	220.16	2361	39	1711	519,798	8,586	376,694		
	CW-1-46	Excavation A	m ³	55.04	2361	39	1711	129,949	2,147	94,173	for Replacement	
	CW-1-8	Tamper Loading	m ³	55.04	1760	60	2600	96,870	3,302	143,104		
	CW-4-9	Installation of Tierod and Wale (Temporary)	ton	0.66102	520420	3640	543270	344,008	2,406	359,112		
	CW-4-10	Removal of Tierod and Wale (Temporary)	ton	0.66102	295700	2230	301270	195,464	1,474	199,145		
	CW-3-9	Driving In of Steel Sheet Pile (Type-II)	m	37	9909.067	76.17755	8578.669	366,635	2,819	317,411	River Side	
	CW-3-10	Pulling Out of Steel Sheet Pile (Type-II)	m	37	9754	67	8548	360,898	2,479	316,276	River Side	
	CW-3-9	Driving In of Steel Sheet Pile (Type-II)	m	37	9909.067	76.17755	8578.669	366,635	2,819	317,411	Field Side	
	CW-3-10	Pulling Out of Steel Sheet Pile (Type-II)	m	37	9754	67	8548	360,898	2,479	316,276	Field Side	
Total for								4 m	3,744,425	42,876	3,272,303	
Unit Cost for								1 m	936,106	10,719	818,076	

- * 1 : Steel Sheet Pile : 48 kg/m x 16 m long x 4 m dstnc / 0.4 m wide = 7680 kg
- * 2 : Tie Rod Dia 42mm : 9 kg/m x 4 m long x 4 m dstnc / 1.6 m pitch = 90 kg
- * 3 : Channel Beam 200x90 : 30.3 kg/m x 4 m dstnc x 4 pieces = 484.8 kg
- * 4 : Structural Steel SS400 : 15 % of Total of Tierod and Beam = 86.22 kg
- * 5 : Soil Volume : 3.2 m wide x 4.3 m high x 4 m = 55.04 m³

ID No.	Working Name	Calculation Quantity	Remarks									
CW-4-18	Temporary Steel Sheet Pile with Support for Drainage	6 m	L=7.5m long and 7.5 and 3.0m of Driving and Pulling Out (Type-II)									
Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks	
Working Base Cost												
	CW-2-7	Cutting Muddy Earth, 1m	m ³	13.5	0	0	7400	0	0	99,900		
	CW-1-46	Excavation A	m ³	40.5	2361	39	1711	95,621	1,580	69,296		
	CW-1-47	Excavation B	m ³	40.5	2951	48	2138	119,516	1,944	86,589		
	CW-1-48	Excavation C	m ³	40.5	3943	65	2857	159,692	2,633	115,709	for Replacement	
	CW-3-17	Wale Work-B (Temporary)	ton	2.340365	558950	3200	581050	1,308,147	7,489	1,359,869		
	CW-4-10	Removal of Tierod and Wale (Temporary)	ton	2.340365	295700	2230	301270	692,046	5,219	705,082		
	CW-3-9	Driving In of Steel Sheet Pile (Type-II)	m	225	9909.067	76.17755	8578.669	2,229,540	17,140	1,930,201	River Side	
	CW-3-10	Pulling Out of Steel Sheet Pile (Type-II)	m	90	9754	67	8548	877,860	6,030	769,320	River Side	
Total for								6 m	5,482,420	42,034	5,135,964	
Unit Cost for								1 m	913,737	7,006	855,994	
Unit Cost for								1 set	365,495	2,802	342,398	

- * 1 : Steel Sheet Pile : 48 kg/m x 15 m long x 6 m dstnc / 0.4 m wide = 10800 kg
- * 3 : H-beam for Waling 350-12x9 : 135 kg/m x 6 m dstnc x 2 pieces = 1620 kg
- * 3 : H-beam for Support 150-7x10 : 31.1 kg/m x 4.5 m long x 2 pieces = 279.9 kg
- * 3 : H-beam for Others 100-6x8 : 16.9 kg/m x 1 m long x 8 pieces = 135.2 kg
- * 4 : Structural Steel SS400 : 15 % of Total of Tierod and Beam = 305.265 kg
- * 5 : Soil Volume : 5 m wide x 4.5 m high x 6 m = 135 m³

ID No.	Working Name	Calculation Quantity	Remarks									
CW-4-21	Temporary Dewatering by D200mm	180 day	Assumption : Working Day is 180 days including Installation and Removal									
Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks	
Equipment												
	A-2-2-49	Submergible Pump; D200mm 22kW	daily	180	81,666	0	41,250	14,699,859	0	7,424,929	for Driving	
	A-2-2-16	Generator; 125 kVA	daily	180	271912.2	15120	209096	48,944,194	2,721,600	37,637,275	for Driving	
	A-2-1-75	Truck Crane; 4.9 ton, Oil Pressure	hourly	1	55145.89	720	48324.47	55,146	720	48,324	for Installation and Removal	
Labour												
	L-2-1	Foreman	day	0.2	0	0	48800	0	0	9,760	for Installation and Removal	
	L-2-23	Common Labour	day	2.8	0	0	35100	0	0	98,280	for Installation and Removal	
	L-2-4	Electrician	day	0.17	0	0	39000	0	0	6,630	for Driving	
Total for								180 day	63,699,199	2,722,320	45,225,199	
Unit Cost for								1 day	353,884	15,124	251,251	

Table 4.1.8 (5/7) CALCULATION SHEET FOR TEMPORARY WORK AND RAIL WORK-1

ID No.	Working Name	Calculation Quantity	Remarks								
CW-4-22	Temporary Dewatering by D180mm	180 day	Assumption : Working Day is 180 days including Installation and Removal								
Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Equipment											
	A-2-2-49	Submergible Pump; D200mm 22kW	daily	120	81,666	0	41,250	9,799,906	0	4,949,953	for Driving
	A-2-2-16	Generator; 125 kVA	daily	120	271912.2	15120	209096	32,629,463	1,814,400	25,091,517	for Driving
	A-2-2-48	Submergible Pump; D150mm 10.6 kW	daily	60	39,099	0	19,749	2,345,951	0	1,184,945	for Driving
	A-2-2-16	Generator; 125 kVA	daily	60	271912.2	15120	209096	16,314,731	907,200	12,545,758	for Driving
	A-2-1-75	Truck Crane; 4.9 ton, Oil Pressure	hourly	1	55145.89	720	48324.47	55,146	720	48,324	for Installation and Removal
Labour											
	L-2-1	Foreman	day	0.2	0	0	48800	0	0	9,760	for Installation and Removal
	L-2-23	Common Labour	day	2.8	0	0	35100	0	0	98,280	for Installation and Removal
	L-2-4	Electrician	day	0.17	0	0	39000	0	0	6,630	for Driving
Total for		180 day						61,145,198	2,722,320	43,935,167	
Unit Cost for		1 day						339,696	15,124	244,084	

ID No.	Working Name	Calculation Quantity	Remarks								
CW-4-23	Temporary Dewatering by D160mm	180 day	Assumption : Working Day is 180 days including Installation and Removal								
Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Equipment											
	A-2-2-49	Submergible Pump; D200mm 22kW	daily	36	81,666	0	41,250	2,939,972	0	1,484,986	for Driving
	A-2-2-16	Generator; 125 kVA	daily	36	271912.2	15120	209096	9,788,839	544,320	7,527,455	for Driving
	A-2-2-48	Submergible Pump; D150mm 10.6 kW	daily	144	39,099	0	19,749	5,630,283	0	2,843,867	for Driving
	A-2-2-16	Generator; 125 kVA	daily	144	271912.2	15120	209096	39,155,355	2,177,280	30,109,820	for Driving
	A-2-1-75	Truck Crane; 4.9 ton, Oil Pressure	hourly	1	55145.89	720	48324.47	55,146	720	48,324	for Installation and Removal
Labour											
	L-2-1	Foreman	day	0.2	0	0	48800	0	0	9,760	for Installation and Removal
	L-2-23	Common Labour	day	2.8	0	0	35100	0	0	98,280	for Installation and Removal
	L-2-4	Electrician	day	0.17	0	0	39000	0	0	6,630	for Driving
Total for		180 day						57,569,595	2,722,320	42,129,123	
Unit Cost for		1 day						319,831	15,124	234,051	

Table 4.1.8 (6/7) CALCULATION SHEET FOR TEMPORARY WORK AND RAIL WORK-1

ID No.		Working Name		Calculation Quantity		Remarks		Cost			Remarks
CW-4-13		Angsana Species		1 tree		Total height from the root is 220cm					
Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Labour	L-2-1	Foreman	day	0.05	0	0	48800	0	0	2,440	
	L-2-23	Common Labour	day	0.2	0	0	35100	0	0	7,020	
Material	M-M-17	Manila Rope	kg	0.2	0	750	14250	0	150	2,850	
	M-D-3	Bamboo Pile, Dia. 3cm)	m	5	0	0	650	0	0	3,250	
	M-I-1	Angsana	tree	1	0	0	15000	0	0	15,000	
Others	Ground Preparation including natural fertilizer, Red Soil and peat moss		L.S.	1			18000	0	0	18,000	
	Maintenance and Watering		L.S.	1			45000	0	0	45,000	
Total for			1 tree					0	150	93,560	
Unit Cost for			1 tree					0	150	93,560	

* 1 : All rates and Costs are based on quotation from Private firms.

ID No.		Working Name		Calculation Quantity		Remarks		Cost			Remarks
CW-4-14		Glodogan Species		1 tree		Total height from the root is 170cm					
Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Labour	L-2-1	Foreman	day	0.05	0	0	48800	0	0	2,440	
	L-2-23	Common Labour	day	0.2	0	0	35100	0	0	7,020	
Material	M-M-17	Manila Rope	kg	0.2	0	750	14250	0	150	2,850	
	M-D-3	Bamboo Pile, Dia. 3cm)	m	5	0	0	650	0	0	3,250	
	M-I-2	Glodogan	tree	1	0	0	50000	0	0	50,000	
Others	Ground Preparation including natural fertilizer, Red Soil and peat moss		L.S.	1			18000	0	0	18,000	
	Maintenance and Watering		L.S.	1			45000	0	0	45,000	
Total for			1 tree					0	150	128,560	
Unit Cost for			1 tree					0	150	128,560	

* 1 : All rates and Costs are based on quotation from Private firms.

ID No.		Working Name		Calculation Quantity		Remarks		Cost			Remarks
CW-4-15		Flamboyant Species		1 tree		Total height from the root is 220cm					
Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Labour	L-2-1	Foreman	day	0.05	0	0	48800	0	0	2,440	
	L-2-23	Common Labour	day	0.2	0	0	35100	0	0	7,020	
Material	M-M-17	Manila Rope	kg	0.2	0	750	14250	0	150	2,850	
	M-D-3	Bamboo Pile, Dia. 3cm)	m	5	0	0	650	0	0	3,250	
	M-I-3	Flamboyant	tree	1	0	0	150000	0	0	150,000	
Others	Ground Preparation including natural fertilizer, Red Soil and peat moss		L.S.	1			18000	0	0	18,000	
	Maintenance and Watering		L.S.	1			45000	0	0	45,000	
Total for			1 tree					0	150	228,560	
Unit Cost for			1 tree					0	150	228,560	

* 1 : All rates and Costs are based on quotation from Private firms.

Table 4.1.8 (7/7) CALCULATION SHEET FOR TEMPORARY WORK AND RAIL WORK-1

ID No.		Working Name		Calculation Quantity		Remarks					
CW-4-16		Relocating Trees		1 tree		Total height from the root is 220cm					
Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Labour	L-2-1	Foreman	day	1	0	0	48800	0	0	48,800	
	L-2-23	Common Labour	day	2	0	0	35100	0	0	70,200	
Material	M-M-17	Manila Rope	kg	0.5	0	750	14250	0	375	7,125	
	M-D-3	Bamboo Pile, Dia. 3cm)	m	12	0	0	650	0	0	7,800	
Others	Ground Preparation including natural fertilizer, Red Soil and peat moss		L.S.	1			36000	0	0	36,000	
	Trees Preparation and Equipment		L.S.	1			25000	0	0	25,000	
	Maintenance and Watering		L.S.	1			45000	0	0	45,000	
Total for		1 tree						0	375	239,925	
Unit Cost for		1 tree						0	375	239,925	

* 1 : All rates and Costs are based on quotation from Private firms.

ID No.		Working Name		Calculation Quantity		Remarks					
CW-4-19		Palm Botoi Planting		1 tree		Total height from the root is minimum 200cm					
Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Labour	L-2-1	Foreman	day	0.05	0	0	48800	0	0	2,440	
	L-2-23	Common Labour	day	0.2	0	0	35100	0	0	7,020	
Material	M-M-17	Manila Rope	kg	0.2	0	750	14250	0	150	2,850	
	M-D-3	Bamboo Pile, Dia. 3cm)	m	5	0	0	650	0	0	3,250	
		Palm Botoi	tree	1	0	0	250000	0	0	250,000	
Others	Ground Preparation including natural fertilizer, Red Soil and peat moss		L.S.	1			18000	0	0	18,000	
	Maintenance and Watering		L.S.	1			45000	0	0	45,000	
Total for		1 tree						0	150	328,560	
Unit Cost for		1 tree						0	150	328,560	

* 1 : All rates and Costs are based on quotation from Private firms.

ID No.		Working Name		Calculation Quantity		Remarks					
CW-4-20		Bougainvillea Planting		1 tree		Total height from the root is minimum 100cm					
Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Labour	L-2-1	Foreman	day	0.05	0	0	48800	0	0	2,440	
	L-2-23	Common Labour	day	0.2	0	0	35100	0	0	7,020	
Material	M-M-17	Manila Rope	kg	0.2	0	750	14250	0	150	2,850	
	M-D-3	Bamboo Pile, Dia. 3cm)	m	5	0	0	650	0	0	3,250	
		Bougainvillea	tree	1	0	0	250000	0	0	250,000	
Others	Ground Preparation including natural fertilizer, Red Soil and peat moss		L.S.	1			18000	0	0	18,000	
	Maintenance and Watering		L.S.	1			45000	0	0	45,000	
Total for		1 tree						0	150	103,560	
Unit Cost for		1 tree						0	150	103,560	

* 1 : All rates and Costs are based on quotation from Private firms.

Table 4.1.9 (1/5) CALCULATION SHEET FOR BRIDGE WORK

ID No.		Working Name			Calculation Quantity			Remarks			
CW-6-1		Furnishing of Main Beam with Reinforcing Bar			1 Beam			L = 21.8 m long			
Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Working Base Cost											
	CW-1-31	Reinforcing Bar Setup 2	t	1.5	0	2808810	2992590	0	4,213,215	4,488,885	
	CW-6-6	Setup of PC Cable	kg	622	6250	64,708875	21799.273	3,887,500	40,249	13,559,148	
	CW-6-10	Stringing Work	cable	3	652536	108756	390495.52	1,957,608	326,268	1,171,487	
	CW-6-20	Concrete Work of Beam at A2 by Crane	m3	10.77	120	50980	223080	1,292	549,055	2,402,572	
	CW-6-9	Hole Work for PC Cable	m	21.6	11589.75	0	1713.9168	250,339	0	37,021	
	CW-6-11	Temporary Placing for Beam	beam	1	0	0	282500	0	0	282,500	
	CW-1-23	Form Work A	m2	86.501	60	0	44798	5,190	0	3,875,072	
Total for								1 Beam	6,101,929	5,128,787	25,816,683
Unit Cost for								1 Beam	6,101,929	5,128,787	25,816,683

ID No.		Working Name			Calculation Quantity			Remarks			
CW-6-2		Temporary Work for Furnishing of Main Beam with Reinforcing Bar			1 Beam			L = 21.8 m long			
Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Working Base Cost											
	CW-6-5	Depreciation of Equipment for Construction	piece	1	28375138	3096	28305763	28,375,138	3,096	28,305,763	
	CW-6-12	Clean-up of Board for Furnishing Beam	beam	1	0	0	32799	0	0	32,799	
	CW-6-13	Furnishing Beam Removing Board for	m	50	0	0	33161.34	0	0	1,658,067	
	CW-6-14	Installing and Removing Derrick Crane	crane	0.07	0	0	443810	0	0	31,701	1 crane / 14 beams
	CW-6-15	Installing and Removing Railing System for Derrick Crane	m	0.71	0	0	9847.995	0	0	7,034	10 m / 14 beams
Total for								1 Beam	28,375,138	3,096	30,035,364
Unit Cost for								1 Beam	28,375,138	3,096	30,035,364

ID No.		Working Name			Calculation Quantity			Remarks			
CW-6-3		Erection of Main Beam with Anchoring Work			1 Beam			L = 21.8 m long			
Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Working Base Cost											
	CW-6-16	Erection of Beam	tun	27	0	0	12462.6	0	0	336,490	
	CW-6-17	Installation and Removal of Equipment for Erection	L.S.	0.07	1534300.8	8640	6433438.6	109,593	617	459,531	1 L.S. / 14 beams
	CW-6-15	Installing and Removing Railing System for Derrick Crane	m	1.43	0	0	9847.995	0	0	14,069	
	CW-6-18	Cost of Equipment and Tools	Bridge	0.2	14870000	2978000	11892000	2,974,000	595,600	2,378,400	Assumption
	CW-6-19	Anchoring for Bridge Work	place	3	67545	0	447245	202,635	0	1,341,735	1 bridge = 5 beams
Total for								1 Beam	3,286,228	596,217	4,530,225
Unit Cost for								1 Beam	3,286,228	596,217	4,530,225

ID No.		Working Name			Calculation Quantity			Remarks			
CW-6-4		Furnishing of Diaphragm with Reinforcing Bar			1 Piece						
Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Working Base Cost											
	CW-1-31	Reinforcing Bar Setup 2	t	0.025	0	2808810	2992590	0	70,220	74,815	
	CW-1-38	Prefabricated Scaffold for Re-Con II	m2	30	14739	62	15629	442,170	1,860	468,870	7.5 m x 22m x 2 places / 11 pieces on average
		Tightening for Cross Direction	L.S.	0.090909	16650000	3330000	13320000	1,513,636	302,727	1,210,909	1 L.S. / 11 pieces on average
	CW-6-7	Grout Work	m3	0.004091	0	19173	545192	0	78	2,230	0.045m3 / 11 pieces on average
		Stringing for Cross Direction	piece	0.545455	47610	9520	38090	25,969	5,193	20,776	6 pieces / 11 pieces on average
	CW-6-21	Concrete Work for Diaphragm at Type-A2 by Pump	m3	0.245	20270	51140	238540	4,966	12,529	58,442	
Others											
Total for								1 Piece	1,986,742	392,608	1,836,043
Unit Cost for								1 Piece	1,986,742	392,608	1,836,043

Table 4.1.9 (2/5) CALCULATION SHEET FOR BRIDGE WORK

ID No.		Working Name		Calculation Quantity		Remarks					
CW-6-5		Depreciation of Equipment for Construction		1 piece							
Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Equipment	A-2-1-73	Truck Crane; 22 ton, Oil Pressure	hourly	3	154,913	1,032	131,788	464,738	3,096	395,363	
Working Base Cost		Stringing Jack	set day	32	52200	0	52200	1,670,400	0	1,670,400	(1+(2-1)) x 16 days
		Derrick Crane	set day	32	80000	0	80000	2,560,000	0	2,560,000	(1+(2-1)) x 16 days
		Rail Facility	m day	960	19000	0	19000	18,240,000	0	18,240,000	(1+(2-1)) x 16 days x 30m
		Tools for Furnishing of Beam	day	32	170000	0	170000	5,440,000	0	5,440,000	(1+(2-1)) x 16 days
Total for		1 piece						28,375,138	3,096	28,305,763	
Unit Cost for		1 piece						28,375,138	3,096	28,305,763	

* 1: All Rates and Cost are quoted from Japanese Standard.

ID No.		Working Name		Calculation Quantity		Remarks					
CW-6-6		Setup of PC Cable		80 kg							
Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Labour	L-2-32	Chief of Bridge	day	1.7	0	0	68300	0	0	116,110	
	L-2-33	Bridge Worker	day	7.5	0	0	58600	0	0	439,500	
	L-2-23	Common Labour	day	6.3	0	0	35100	0	0	221,130	
Material		PC Cable	kg	80	0	0	4000	0	0	320,000	
		PC Hole	m	100	5000	0	5000	500,000	0	500,000	
Working Base Cost	CW-6-7	Grout Work	m ³	0.27	0	19173	545192	0	5,177	147,202	
Others											
Total for		80 kg						500,000	5,177	1,743,942	
Unit Cost for		1 kg						6,250	65	21,799	

* 1: All Rates and Cost are quoted from Japanese Standard.

ID No.		Working Name		Calculation Quantity		Remarks					
CW-6-7		Grout Work		1 m ³							
Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Labour	L-2-33	Bridge Worker	day	5	0	0	58600	0	0	293,000	
	L-2-23	Common Labour	day	5	0	0	35100	0	0	175,500	
Material	M-C-45	Non Shrinkage Mortar	m ³	1.05	0	18260	73040	0	19,173	76,692	
Total for		1 m ³						0	19,173	545,192	
Unit Cost for		1 m ³						0	19,173	545,192	

ID No.		Working Name		Calculation Quantity		Remarks					
CW-6-8		Concrete Work for Beam		10 m ³		Utilization of Derrick Crane					
Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Labour	L-2-1	Foreman	day	0.3	0	0	48800	0	0	14,640	
	L-2-2	Operator	day	2.1	0	0	46900	0	0	98,490	
	L-2-23	Common Labour	day	2.5	0	0	35100	0	0	87,750	
Material	M-C-8	Ready Mixed Concrete; 400kg/cm ² , 25mm (A2)	m ³	10.2	0	49000	196000	0	499,800	1,999,200	10x (1+0.02)
Others		Small Tools	%	0.3				0	1,499	6,600	
Total for		10 m ³						0	501,299	2,206,680	
Unit Cost for		1 m ³						0	50,130	220,668	

* 1: All Rates and Cost are quoted from Japanese Standard.

ID No.		Working Name		Calculation Quantity		Remarks					
CW-6-9		Hole Work for PC Cable		100 m		Cross Direction					
Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Labour	L-2-32	Chief of Bridge	day	0.4	0	0	68300	0	0	27,320	
	L-2-33	Bridge Worker	day	0.8	0	0	58600	0	0	46,880	
	L-2-23	Common Labour	day	1	0	0	35100	0	0	35,100	
Material	M-E-14	Steel Pile, Dia. 38mm (1.5ch), incl. Coating & Lining	m	100	11475	0	603,94737	1,147,500	0	60,395	
Others		Small Tools	%	1				11,475	0	1,697	
Total for		100 m						1,158,975	0	171,392	
Unit Cost for		1 m						11,590	0	1,714	

* 1: All Rates and Cost are quoted from Japanese Standard.

Table 4.1.9 (3/5) CALCULATION SHEET FOR BRIDGE WORK

ID No.	Working Name	Calculation Quantity	Remarks	Unit Cost			Cost			Remarks
CW-6-10	Stringing Work	10 cable	Type 195ton	PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Major Item	ID No.	Description	Unit	Quantity						
Labour	L-2-32	Chief of Bridge	day	1.6	0	0	68300	0	0	109,280
	L-2-33	Bridge Worker	day	5.6	0	0	58600	0	0	328,160
	L-2-23	Common Labour	day	4.8	0	0	35100	0	0	168,480
Material		Stringing Material	set	20	307800	51300	153900	6,156,000	1,026,000	3,078,000
Others		Small Tools	%	6				369,360	61,560	221,035
Total for		10 cable						6,525,360	1,087,560	3,904,935
Unit Cost for		1 cable						652,536	108,756	390,493

* 1: All Rates and Cost are quoted from Japanese Standard.

ID No.	Working Name	Calculation Quantity	Remarks	Unit Cost			Cost			Remarks
CW-6-11	Temporary Placing for Beam	1 beam		PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Major Item	ID No.	Description	Unit	Quantity						
Labour	L-2-32	Chief of Bridge	day	0.5	0	0	68300	0	0	34,150
	L-2-33	Bridge Worker	day	3.1	0	0	58600	0	0	181,660
	L-2-23	Common Labour	day	1.9	0	0	35100	0	0	66,690
Total for		1 beam						0	0	282,500
Unit Cost for		1 beam						0	0	282,500

* 1: All Rates and Cost are quoted from Japanese Standard.

ID No.	Working Name	Calculation Quantity	Remarks	Unit Cost			Cost			Remarks
CW-6-12	Clean-up of Board for Furnishing Beam	1 beam		PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Major Item	ID No.	Description	Unit	Quantity						
Labour	L-2-18	Form Worker	day	0.58	0	0	39000	0	0	22,620
	L-2-23	Common Labour	day	0.29	0	0	35100	0	0	10,179
Total for		1 beam						0	0	32,799
Unit Cost for		1 beam						0	0	32,799

* 1: All Rates and Cost are quoted from Japanese Standard.

ID No.	Working Name	Calculation Quantity	Remarks	Unit Cost			Cost			Remarks
CW-6-13	Furnishing, Installing and Removing Board for Furnishing Beam	10 m		PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Major Item	ID No.	Description	Unit	Quantity						
Labour	L-2-32	Chief of Bridge	day	0.6	0	0	68300	0	0	40,980
	L-2-18	Form Worker	day	2.2	0	0	39000	0	0	85,800
	L-2-23	Common Labour	day	3.6	0	0	35100	0	0	126,360
Others		Small Tools	%	31				0	0	78,473
Total for		10 m						0	0	331,613
Unit Cost for		1 m						0	0	33,161

* 1: All Rates and Cost are quoted from Japanese Standard.

ID No.	Working Name	Calculation Quantity	Remarks	Unit Cost			Cost			Remarks
CW-6-14	Installing and Removing Derrick Crane	1 crane	Type : 3ton	PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Major Item	ID No.	Description	Unit	Quantity						
Labour	L-2-33	Bridge Worker	day	3.8	0	0	58600	0	0	222,680
	L-2-4	Electrician	day	1.8	0	0	39000	0	0	70,200
	L-2-23	Common Labour	day	4.3	0	0	35100	0	0	150,930
Total for		1 crane						0	0	443,810
Unit Cost for		1 crane						0	0	443,810

* 1: All Rates and Cost are quoted from Japanese Standard.

ID No.	Working Name	Calculation Quantity	Remarks	Unit Cost			Cost			Remarks
CW-6-15	Installing and Removing Railing System for Derrick Crane	10 m		PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Major Item	ID No.	Description	Unit	Quantity						
Labour	L-2-32	Chief of Bridge	day	0.2	0	0	68300	0	0	13,660
	L-2-33	Bridge Worker	day	0.9	0	0	58600	0	0	52,740
	L-2-23	Common Labour	day	0.9	0	0	35100	0	0	31,590
Others		Small Tools	%	0.5				0	0	490
Total for		10 m						0	0	98,480
Unit Cost for		1 m						0	0	9,848

* 1: All Rates and Cost are quoted from Japanese Standard.

Table 4.1.9 (4/5) CALCULATION SHEET FOR BRIDGE WORK

ID No.	Working Name	Calculation Quantity		Remarks							
CW-6-16	Erection of Beam	300 ton									
Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Labour	L-2-32	Chief of Bridge	day	6.7	0	0	68300	0	0	457,610	
	L-2-33	Bridge Worker	day	40	0	0	58600	0	0	2,344,000	
	L-2-23	Common Labour	day	26.7	0	0	35100	0	0	937,170	
Total for	300 ton							0	0	3,738,780	
Unit Cost for	1 ton							0	0	12,463	

- * 1: All Rates and Cost are quoted from Japanese Standard.
- * 2:

Chief of Bridge	1 person x 300 ton	=	6.7
	45 ton/day		
Bridge Worker	6 person x 300 ton	=	40
	45 ton/day		
Common Labour	4 person x 300 ton	=	26.7
	45 ton/day		

ID No.	Working Name	Calculation Quantity		Remarks							
CW-6-17	Installation and Removal of Equipment for Erection	1 L.S.									
Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Equipment	A-2-1-74	Truck Crane; 35 ton, Oil Pressure	hourly	6	255716.81	14.40	216489.77	1,534,301	8,640	1,298,939	
Labour	L-2-32	Chief of Bridge	day	10	0	0	68300	0	0	683,000	
	L-2-33	Bridge Worker	day	55	0	0	58600	0	0	3,223,000	
	L-2-23	Common Labour	day	35	0	0	35100	0	0	1,228,500	
Total for	1 L.S.							1,534,301	8,640	6,433,439	
Unit Cost for	1 L.S.							1,534,301	8,640	6,433,439	

- * 1: All Rates and Cost are quoted from Japanese Standard.

ID No.	Working Name	Calculation Quantity		Remarks							
CW-6-18	Cost of Equipment and Tools	1 Bridge		Application : 20 - 30 m							
Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Equipment		Equipment for Erection	day	20	147,600	29,500	118,100	2,932,000	590,000	2,362,000	
		Equipment for Beam Hanging	day	40	141,300	28,300	113,000	5,652,000	1,132,000	4,520,000	
		Equipment for Horizontal Moving	day	20	246,600	49,300	197,300	4,932,000	986,000	3,946,000	
		Tools for Erection	day	20	47,700	9,500	38,200	954,000	190,000	764,000	
		Equipment of Railing System	m day	2000	190	40	150	380,000	80,000	300,000	
Total for	1 Bridge							14,870,000	2,978,000	11,892,000	
Unit Cost for	1 Bridge							14,870,000	2,978,000	11,892,000	

- * 1: All Rates and Cost are quoted from Japanese Standard.

ID No.	Working Name	Calculation Quantity		Remarks							
CW-6-19	Anchoring for Bridge Work	1 place									
Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Labour	L-2-32	Chief of Bridge	day	0.2	0	0	68300	0	0	13,660	
	L-2-33	Bridge Worker	day	0.4	0	0	58600	0	0	23,440	
	L-2-23	Common Labour	day	0.9	0	0	35100	0	0	31,590	
Material	M-M-5	Wooden Sleeper 13x22x200cm	nos.	3	0	0	125000	0	0	375,000	
	M-E-35	Galvanized Steel Wire	kg	23.7	2850	0	150	67,545	0	3,555	
Total for	1 place							67,545	0	447,245	
Unit Cost for	1 place							67,545	0	447,245	

- * 1: All Rates and Cost are quoted from Japanese Standard.
- * 2: Wire Rope 15 m x 0.000201 m² x 7.85 ton/m³ = 23.7 kg

Table 4.1.9 (5/5) CALCULATION SHEET FOR BRIDGE WORK

ID No.		Working Name		Calculation Quantity			Remarks				
CW-6-20		Concrete Work of Beam at A2 by Crane		10 m ³							
Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Labour	L-2-1	Foreman	day	0.3	0	0	48800	0	0	14,640	
	L-2-17	Concrete Worker	day	2.1	0	0	39000	0	0	81900	
	L-2-23	Common Labour	day	2.5	0	0	35100	0	0	87750	
Material	M-C-8	Ready Mixed Concrete; 400kg/cm ² , 25mm (A2)	m ³	10.2	0	49000	196000	0	499800	1999200	
	Others	CW-1-45	Curing Work	m ³	10	110	0	350	1100	0	3500
		Miscellaneous	L.S.	1				100	10,000	43,810	
Total for				10 m ³				1,200	509,800	2,230,800	
Unit Cost for				1 m ³				120	50980	223080	

Refer to Japanese Standard for All rates
Concrete : 10 x (1 + 0.02) = 10.2m³

ID No.		Working Name		Calculation Quantity			Remarks				
CW-6-21		Concrete Work for Diaphragm at Type-A2 by Pump		10 m ³			by Boom, Standard Concreting Volume = 75m ³				
Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Equipment	A-2-1-36-1	Concrete Pump Truck; 90-110 m ³ /hr	Time	0.15	45418.325	340	28935.435	6,813	51	4,340	
	A-2-1-36-2	Concrete Pump Truck; 90-110 m ³ /hr	hourly	0.7	272509.95	2040	173612.61	190,757	1,428	121,529	
Labour	L-2-1	Foreman	day	0.3	0	0	48800	0	0	14,640	
	L-2-17	Concrete Worker	day	1.5	0	0	39000	0	0	58,500	
	L-2-23	Common Labour	day	3.9	0	0	35100	0	0	136,890	
Material	M-C-8	Ready Mixed Concrete; 400kg/cm ² , 25mm (A2)	m ³	10.2	0	49000	196000	0	499,800	1,999,200	
	Others	CW-1-45	Curing Work	m ³	10	110	0	350	1,100	0	3,500
		Miscellaneous	L.S.	1				4,030	10,121	46,801	
Total for				10 m ³				202,700	511,400	2,385,400	
Unit Cost for				1 m ³				20,270	51,140	238,540	

Refer to Japanese Standard for All rates
Concrete : 10 x (1 + 0.02) = 10.2m³

ID No.		Working Name		Calculation Quantity			Remarks				
CW-6-22		Concrete Work of Type-B by Pump		10 m ³			by Boom, Standard Concreting Volume = 75m ³				
Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Equipment	A-2-1-36-1	Concrete Pump Truck; 90-110 m ³ /hr	Time	0.15	45418.325	340	28935.435	6,813	51	4,340	
	A-2-1-36-2	Concrete Pump Truck; 90-110 m ³ /hr	hourly	0.7	272509.95	2040	173612.61	190,757	1,428	121,529	
Labour	L-2-1	Foreman	day	0.3	0	0	48800	0	0	14,640	
	L-2-17	Concrete Worker	day	1.5	0	0	39000	0	0	58,500	
	L-2-23	Common Labour	day	3.9	0	0	35100	0	0	136,890	
Material	M-C-10	Ready Mixed Concrete; 250kg/cm ² , 25mm (B)	m ³	10.2	0	42000	168000	0	428,400	1,713,600	
	Others	CW-1-45	Curing Work	m ³	10	110	0	350	1,100	0	3,500
		Miscellaneous	L.S.	1				4,030	8,621	41,101	
Total for				10 m ³				202,700	438,500	2,094,100	
Unit Cost for				1 m ³				20,270	43,850	209,410	

Refer to Japanese Standard for All rates
Concrete : 10 x (1 + 0.02) = 10.2m³

Table 4.2.1 SCHEDULE OF TRUCK IN GENERAL NEEDED FOR MOBILIZATION AND DEMOBILIZATION

Construction Equipment		Number of Equipment																																				
		2001												2002												2003												
	Capacity/ Specification	1	2	3	4	5	6	7	8	9	10	11	12	1	2	3	4	5	6	7	8	9	10	11	12	1	2	3	4	5	6	7	8	9	10	11	12	
Dump Truck A	4 ton																																					
Dump Truck B	10 ton																																					
Truck with Crane A	4 ton																																					

Construction Equipment		Number of Equipment																																				
		2001												2002												2003												
	Capacity/ Specification	1	2	3	4	5	6	7	8	9	10	11	12	1	2	3	4	5	6	7	8	9	10	11	12	1	2	3	4	5	6	7	8	9	10	11	12	
Pontoon	100 m3																																					
Barge	100 m3																																					
Tug Boat	15 ton																																					
Dump Truck B	10 ton																																					
Truck with Crane A	4 ton																																					

Construction Equipment		Number of Equipment																																				
		2001												2002												2003												
	Capacity/ Specification	1	2	3	4	5	6	7	8	9	10	11	12	1	2	3	4	5	6	7	8	9	10	11	12	1	2	3	4	5	6	7	8	9	10	11	12	
Dump Truck B	10 ton																																					
Truck with Crane A	4 ton																																					

Mobilization
Demobilization

**Table 4.2.3 NUMBER OF TRUCK IN GENERAL TRANSPORTATION FOR
MOBILIZATION AND DEMOBILIZATION**

MOBILIZATION AND DEMOBILIZATION OF SEMARANG RIVER DRAINAGE SYSTEM IMPROVEMENT

Construction Equipment	Capacity/ Specification	Number of Equipment						Total	
		2001		2002		2003		M	D
		M	D	M	D	M	D		
Dump Truck A	4 ton			2	2	3	3	5	5
Dump Truck B	10 ton			8	8	8	8	16	16
Truck with Crane A	4 ton			1	1	1	1	2	2

MOBILIZATION AND DEMOBILIZATION OF ASIN RIVER DRAINAGE SYSTEM IMPROVEMENT

Construction Equipment	Capacity/ Specification	Number of Equipment						Total	
		2001		2002		2003		M	D
		M	D	M	D	M	D		
Pontoon	100 m3	1	1					1	1
Barge	100 m3	2	2					2	2
Tug Boat	15 ton	1	1					1	1
Dump Truck B	10 ton	17	17	20	20	17	17	54	54
Truck with Crane A	4 ton	5	5	5	4	6	7	16	16

MOBILIZATION AND DEMOBILIZATION OF BANDARHARJO DRAINAGE SYSTEM IMPROVEMENT

Construction Equipment	Capacity/ Specification	Number of Equipment						Total	
		2001		2002		2003		M	D
		M	D	M	D	M	D		
Dump Truck A	4 ton	12	12	11	11	13	13	36	36
Truck with Crane A	4 ton	3	3	5	4	1	2	9	9

**Table 4.2.4 (1/2) NUMBER OF TRAILER TRANSPORTATION FOR
MOBILIZATION AND DEMOBILIZATION**

**MOBILIZATION AND DEMOBILIZATION OF SEMARANG RIVER
DRAINAGE SYSTEM IMPROVEMENT**

Construction Equipment	Capacity/ Specification	Number of Transportation			Total
		2001	2002	2003	
Buldozer B	15 ton	0	2	0	2
Backhoe/Excavator A	0.20 m3	0	2	2	4
Backhoe/Excavator B	0.35 m3	0	0	2	2
Backhoe/Excavator C	0.60 m3	0	2	2	4
Backhoe D (Long Arm)	0.60 m3	0	2	2	4
Portable Concrete Mixer A	0.5 m3	0	2	0	2
Tamper	60/100 kg	0	2	0	2
Vibrating Roller B	10 ton	0	2	2	4
Tire Roller	40 ton	0	0	2	2
Motor Grader	50 ton	0	0	2	2
Diesel Engine Generator A	125 KVA	0	0	2	2
Air Compressor A	5.0 m3	0	2	2	4
Total Number of Trailer for Mobilization		0	3	5	8
Total Number of Trailer for Demobilization		0	3	5	8

**MOBILIZATION AND DEMOBILIZATION OF ASIN RIVER
DRAINAGE SYSTEM IMPROVEMENT**

Construction Equipment	Capacity/ Specification	Number of Transportation			Total
		2001	2002	2003	
Buldozer A	15 ton	2	3	0	5
Backhoe/Excavator B	0.35 m3	2	2	0	4
Backhoe/Excavator C	0.60 m3	10	10	7	27
Giant Breaker	600/800 kg	0	0	2	2
Clamshell Grabbing	1.0 m3	2	0	0	2
Truck Crane A	20 ton	0	2	2	4
Truck Crane B	25 ton	0	2	4	6
Crawler Crane A	50 ton	0	0	4	4
Crawler Crane C	100 ton	0	2	0	2
Vibratory Pile Driver A	60 kW	2	2	4	8
Vibratory Pile Driver B	90 kW	2	3	0	5
Diesel Pile Hammer	3.5 ton	0	2	2	4
Portable Concrete Mixer A	0.20 m3	2	2	2	6
Tamper	60/100 kg	2	2	2	6
Vibrating Roller B	10 ton	2	2	4	8
Tire Roller	8/12 ton	2	0	0	2
Tandem Roller	8/12 ton	2	0	0	2
Motor Grader	2.8 m	2	0	4	6
Asphalt Sprayer	30 lit/min	2	0	2	4
Asphalt Finisher	2.4 m	2	0	2	4
Diesel Engine Generator A	35 kVA	2	0	0	2
Air Compressor A	5 m3	2	1	3	6
Total Number of Trailer for Mobilization		10	11	16	37
Total Number of Trailer for Demobilization		12	13	15	40

**Table 4.2.4 (2/2) NUMBER OF TRAILER TRANSPORTATION FOR
MOBILIZATION AND DEMOBILIZATION**

**MOBILIZATION AND DEMOBILIZATION OF BANDARHARJO
DRAINAGE SYSTEM IMPROVEMENT**

Construction Equipment	Capacity/ Specification	Number of Transportation			Total
		2001	2002	2003	
Buldozer A	15 ton	2	2	0	4
Backhoe/Excavator B	0.35 m3	2	0	0	2
Backhoe/Excavator C	0.60 m3	6	5	8	19
Backhoe D (Long Arm)	0.60 m3	0	2	0	2
Truck Crane A	20 ton	0	4	2	6
Crawler Crane C	100 ton	0	2	0	2
Vibratory Pile Driver A	60 kW	4	4	2	10
Vibratory Pile Driver B	90 kW	0	2	0	2
Diesel Pile Hammer	3.5 ton	0	2	0	2
Portable Concrete Mixer A	0.20 m3	2	2	2	6
Tamper	60/100 kg	2	2	2	6
Vibrating Roller B	10 ton	0	2	4	6
Tire Roller	8/12 ton	0	0	4	4
Tandem Roller	8/12 ton	0	0	4	4
Motor Grader	2.8 m	0	0	4	4
Asphalt Sprayer	30 lit/min	0	0	2	2
Asphalt Finisher	2.4 m	0	0	2	2
Air Compressor B	11 m3	0	2	0	2
Total Number of Trailer for Mobilization		8	11	0	29
Total Number of Trailer for Demobilization		7	10	0	28

Table 4.2.5 (1/119) CALCULATION SHEET FOR UNIT COST OF EACH PAYMENT ITEM OF THREE PACKAGES

ID No.	Working Name	Calculation	Quantity	Remarks	Unit Cost			Cost			Remarks			
Major Item	ID No.	Description	Unit	Quantity	PF/C	IF/C	L/C	PF/C	IF/C	L/C				
Direct Cost														
Equipment														
	A-2-1-69	Trailer; 20 ton	hourly	64	119,879	2,160	102,572	7,672,283	138,240	6,564,587				
	A-2-1-48	Dumptruck; 10 ton	hourly	128	77269	3060	70744.12	9,890,431	391,680	9,055,247				
	A-2-1-80	Truck; 11 ton	hourly	40	96,932	1,560	95,161	3,877,271	62,400	3,806,445				
	A-2-1-31	Truck with crane; 4 ton, Crane: 2.9 ton	hourly	16	48669.75	780	47767.65	778,716	12,480	764,282				
	A-2-1-32	Truck with crane; 6 ton	hourly	0	62783.98	912	61243.41	0	0	0				
	A-2-2-37	Pontoon Barge; 100 ton	daily	0	314821.3	0	237790.6	0	0	0				
	A-2-2-41	Drifter Air Type : 150kg class	daily	0	257007.1	0	88827.32	0	0	0				
	A-2-1-84	Tugboat; 15 ton	hourly	0	129433.1	4440	140042.2	0	0	0				
Indirect Cost														
	Site Expense		%	15	0.8		0.2	5,161,688	0	1,290,422				
	Profit and Overhead Cost		%	10	0.8		0.2	3,957,294	0	989,323				
	Miscellaneous		L.S.					17	0	92	Round Up			
Total for								1 L.S.			31,337,700	604,800	22,470,400	
Unit Cost for								1 L.S.			31,337,700	604,800	22,470,400	

* 1 : All Equipment : Land Transportation : hours land transportation. hours ship transportation
 All Equipment : Ship Transportation : Though it takes longer time than land transportation, it assumes that the cost is same.

* 2 : Number of Mobilized and Demobilized Equipment

	Mobilization	Demobilization
Trailer	8	8
Dump Truck 10t	16	16
Ordinary Truck 10t	5	5
Truck with Crane 4 t	2	2
Truck with Crane 6 t		
Pontoon Barge 100 t		
Soil Carriage 100m3		
Tag Boat 15 ton		

ID No.	Working Name	Calculation	Quantity	Remarks	Unit Cost			Cost			Remarks			
Major Item	ID No.	Description	Unit	Quantity	PF/C	IF/C	L/C	PF/C	IF/C	L/C				
Material														
	M-B-12	Crushed Stone for Pavement and Concrete	m3	207.5	0	3250	61750	0	674,375	12,813,125	for Excavation			
	M-B-13	Solid Soil	m3	944	0	600	11400	0	566,400	10,761,600	for Embankment			
	M-B-12	Crushed Stone for Pavement and Concrete	m3	489.7	0	3250	61750	0	1,591,525	30,238,975	for Embankment			
Working Base Cost														
	CW-1-46	Excavation A	m3	718.75	2361	39	1711	1,696,969	28,031	1,229,781	for Excavation			
	CW-1-12	Slope Clearing of Excavation by Machine	m2	325	4018	66	3760	1,305,850	21,450	1,222,000	for Excavation			
	CW-4-1	Temporary Bridge	m2	0	917231.6	19318.14	861665.6	0	0	0	for Embankment			
	CW-1-56	Spreading and Compaction for Gravel Pavement	m3	442.5	5117.168	43.2	16431.17	2,264,347	19,116	7,270,791	for Embankment			
	CW-1-59	Spreading and Compaction-D	m3	1696.25	1509	19	1473	2,559,641	32,229	2,498,576	for Embankment			
Indirect Cost														
	Site Expense		%	15	0.8		0.2	9,215,374	0	2,303,843				
	Profit and Overhead Cost		%	10	0.8		0.2	7,065,120	0	1,766,280				
	Miscellaneous		L.S.					99	74	28	Round Up			
Total for								1 L.S.			24,107,400	2,933,200	70,105,000	
Unit Cost for								1 L.S.			24,107,400	2,933,200	70,105,000	

for Embankment

* 1 : Temporary Road Body Volume (V1) : $0.5 \times (5.0 \text{ m wide} + 6.50 \text{ m wide}) \times 0.5 \text{ m high}$
 $= 2.875 \text{ m}^3/\text{m}$ (Purchased Soil Volume = 50% of Total Volume)
 Purchased Soil for Road : $1.4375 \text{ m}^3/\text{m} / 0.9 \text{ loss} = 1.6 \text{ m}^3/\text{m}$

* 2 : Gravel Pavement Volume (V2) : $0.15 \text{ m thick} \times 5.0 \text{ m wide} = 0.75 \text{ m}^3/\text{m}$
 Purchased Crushed Stone : $0.75 \text{ m}^3/\text{m} / 0.9 \text{ loss} = 0.83 \text{ m}^3/\text{m}$

* 3 : Temporary Road Length (l1) : m from Construction Planning roundup m

* 4 : Temporary Bridge Length (l2) : m from Construction Planning roundup m

for Excavated Road

* 5 : Excavation Volume (V3) : $2.875 \text{ m}^3/\text{m}$ from Construction Planning

* 6 : Gravel Pavement Volume (V4) : $0.83 \text{ m}^3/\text{m}$

* 7 : Slope Clearing : $1.3 \text{ m}^3/\text{m}$

* 8 : Temporary Road Length (l3) : m from Construction Planning roundup m

Table 4.2.5 (2/119) CALCULATION SHEET FOR UNIT COST OF EACH PAYMENT ITEM OF THREE PACKAGES

ID No.	Working Name		Calculation Quantity		Remarks						
U-P1-Bq-5	Contractor's Site Office and Facilities		1 L.S.								
Major Item	ID No.	Description	Unit	Quantity	PF/C	IF/C	L/C	PF/C	IF/C	L/C	Remarks
Direct Cost											
		Install of Office and Others	m2	574	80000	80000	915000	45,920,000	45,920,000	525,210,000	including all facilities
		Removal of Office	%	30				13,776,000	13,776,000	157,563,000	
		Rental of Land	%	10				5,969,600	5,969,600	68,277,300	
Indirect Cost											
Site Expense			%	15	0.8		0.2	105,885,780	0	26,471,445	
Profit and Overhead Cost			%	10	0.8		0.2	81,179,098	0	20,294,775	
		Miscellaneous	L.S.					22	0	81	
Total for		1 L.S.						252,730,500	65,665,600	797,816,600	
Unit Cost for		1 L.S.						252,730,500	65,665,600	797,816,600	

ID No.	Working Name		Calculation Quantity		Remarks					
U-P1-Bq-6	Engineer's Site Office and Facilities		1 L.S.							
Major Item	ID No.	Description	Unit	Qua	Remarks					

Table 4.2.5 (3/119) CALCULATION SHEET FOR UNIT COST OF EACH PAYMENT ITEM OF THREE PACKAGES

ID No.	Working Name	Calculation	Quantity	Remarks	Unit Cost			Cost			Remarks	
Major Item	ID No.	Description	Unit	Quantity	PF/C	IF/C	L/C	PF/C	IF/C	L/C		
Direct Cost												
Labour												
	L-2-34	Cad Operator	day	200	0	0	54700	0	0	10,940,000		
	L-2-35	Draft Man	day	200	0	0	39000	0	0	7,800,000		
Material												
	M-L-15	Drawing Paper (A1)	sheet	100	8000	0	2000	800,000	0	200,000		
	M-L-16	Blue Copy (A1)	sheet	100	0	2500	2500	0	250,000	250,000		
Others												
		Tools	%	20				160,000	50,000	3,838,000	Computer, Plotter, Drafer and etc.	
Indirect Cost												
		Site Expense	%	15	0.8		0.2	2,914,560	0	728,640		
Profit and Overhead Cost												
		Miscellaneous	L.S.	10	0.8		0.2	2,234,496	0	558,624		
								44	0	36	Round Up	
Total for								1 L.S.				
Unit Cost for								1 L.S.				
									6,109,100	300,000	24,315,300	
									6,109,100	300,000	24,315,300	

ID No.	Working Name	Calculation	Quantity	Remarks	Unit Cost			Cost			Remarks	
Major Item	ID No.	Description	Unit	Quantity	PF/C	IF/C	L/C	PF/C	IF/C	L/C		
Direct Cost												
		Drawing Plan of River Scale 1:500	ha	10			112,000	0	0	1,120,000		
		River Cross Section Survey Scale 1:100										
		a. Surveying	section	20			232000	0	0	4,640,000		
		b. Drawing & Processing	section	20			62000	0	0	1,240,000		
Indirect Cost												
		Site Expense	%	15	0.8		0.2	840,000	0	210,000		
Profit and Overhead Cost												
		Miscellaneous	L.S.	10	0.8		0.2	644,000	0	161,000		
								0	0	0	Round Up	
Total for								1 L.S.				
Unit Cost for								1 L.S.				
									1,484,000	0	7,371,000	
									1,484,000	0	7,371,000	

Table 4.2.5 (4/119) CALCULATION SHEET FOR UNIT COST OF EACH PAYMENT ITEM OF THREE PACKAGES

ID No.	Working Name		Calculation Quantity							Remarks	
U-P1-Bq-11	Clearing of Garbage		1 L.S.								
Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Direct Cost											
Equipment											
	A-2-1-54	Dumptruck; 4 ton	hourly	58.62	30203.66	1376	28631.77	1770538.83	80661.12	1678394.57	
	A-2-1-7	Backhoe; 0.6 m3	hourly	7	125542.9	2040	90965.08	878800.353	14280	636755.535	
Labour											
	L-2-1	Foreman	day	1	0	0	48800	0	0	48800	
	L-2-23	Common Labour	day	10	0	0	35100	0	0	351000	
Material											
		Plastic Bag	sheet	100	0	0	1000	0	0	100000	
Indirect Cost											
		Site Expense	%	15	0.8		0.2	667107.649	0	166776.912	
		Profit and Overhead Cost	%	10	0.8		0.2	511449.197	0	127862.299	
		Miscellaneous	L.S.					4	59	11	Round Up
Total for								1 L.S.			
Unit Cost for								1 L.S.			
								3,827,900	95,000	3,109,600	
								3,827,900	95,000	3,109,600	

* 1 : Total Volume of Gabege = 565 m3
 * 2 : Dump Truck : = 565 m3 / 1 ton/m3 / 10 ton truck / 0.8 loss
 = 70.63 truck round
 10 km/rnd / 20 km/hr + 20 mnts(loss) = 0.83 hours
 70.63 Truck x 0.83 hours = 58.62 hours

ID No.	Working Name		Calculation Quantity							Remarks	
U-P1-Bq-13	Excavation below water level including hauling and treatment of contaminated soil		100 m3								
Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Direct Cost											
Equipment											
	A-2-1-48	Dumptruck; 10 ton	hourly	21.89	77269	3060	70744.12	1,691,418	66,983	1,548,589	from Hauling
	A-2-2-2	Stabilizer	hourly	0.683761	743275.5	1092	502607.8	508,223	747	343,663	for Mixing
Labour											
	L-2-1	Foreman	day	0.008686	0	0	48800	0	0	424	for water proof sheet
	L-2-23	Common Labour	day	0.028952	0	0	35100	0	0	1,016	for water proof sheet
	L-2-23	Common Labour	day	6.666667	0	0	35100	0	0	234,000	for Spreading of Concrete
Material											
		Drain Pipe	m	0.173712	108000	0	12000	18,761	0	2,085	for Hauling
		Water Proof Sheet for Disposal Site	m2	47.57788	27000	0	3000	1,284,603	0	142,734	for Disposal Site
		Water Proof Sheet for Dump Truck	sheet	0.347423	0	0	50000	0	0	17,371	for Hauling
	M-B-3	Sand for Mortar (Masonry)	m3	21.74986	0	2250	42750	0	48,937	929,806	for Backfill of Drain
	M-C-1	Portland Cement	kg	7000	0	100	400	0	700,000	2,800,000	for Backfill of Drain
	M-B-5	Cobble Stone	m3	0.496526	0	1850	35150	0	919	17,453	for Backfill of Drain
	M-B-13	Solid Soil	m3	33.42373	0	600	11400	0	20,054	381,030	for Banking
Working Base Cost											
	CW-1-64	Excavation by Backhoe 0.35m3	m3	0.496526	2687.773	45.24	1953.753	1,335	22	970	for Excavation
	CW-1-64	Excavation by Backhoe 0.35m3	m3	0.551695	2687.773	45.24	1953.753	1,483	25	1,078	for Backfill
		Spreading by Swamp									
	CW-1-65	Bulldozer	m3	108	4284.32	54.264	4047.348	462,707	5,861	437,114	for Spreading
	CW-1-5	Spreading A	m3	80.74117	2941	35	2823	237,460	2,826	227,932	for Disposal Site
		Slope Clearing for									
	CW-1-10	Embankment 2	m2	3.420382	3265	54	2660	11,168	185	9,098	for Banking
	CW-1-5	Spreading A	m3	20.26636	2941	35	2823	59,603	709	57,212	for Final Spreading
	CW-1-1	Backfill (Soil) A	m3	30.57788	6076	87	5043	185,791	2,660	154,204	for Banking
		Spreading and Compaction for									
	CW-1-58	Earth Filling	m3	30.57788	2833.803	36.252	2632.618	86,652	1,109	80,500	for Banking
	CW-1-48	Excavation C	m3	100	3943	65	2857	394,300	6,500	285,700	for Dredging
	CW-1-47	Excavation B	m3	1.022583	2951	48	2138	3,018	49	2,186	for Pit
		Concrete Work for Small									
	CW-1-21	Structure : Type-D	m3	0.348871	120	42570	193500	42	14,851	67,507	for Pit
	CW-1-23	Form Work A	m2	0.318471	60	0	44798	19	0	14,267	for Pit
	CW-1-29	Reinforcing Bar Setup 1	t	0.006977	0	3120900	3325100	0	21,776	23,201	for Pit
	CW-1-15	Gravel Bedding	m3	0.056167	0	1360	31260	0	76	1,756	for Pit
	CW-1-2	Backfill (Soil) B	m3	0.132021	7022	103	6326	927	14	835	for Pit
Indirect Cost											
		Site Expense	%	15	0.8		0.2	1,634,825	0	408,706	
		Profit and Overhead Cost	%	10	0.8		0.2	1,253,366	0	313,341	
		Miscellaneous	L.S.					1	97	22	
Total for								100 m3			
Unit Cost for								1 m3			
								7,835,700	894,400	8,503,800	
								78,357	8,944	85,038	See Next Page

Table 4.2.5 (5/119). CALCULATION SHEET FOR UNIT COST OF EACH PAYMENT ITEM OF THREE PACKAGES

* 1 :	Dump Truck for Hauling:	107 m3	/	6.5 m3/truck	=	16.46 Truck	
		20 km/hrd /		20 km/hr +	20 trnts(loss)=	1.33 hours	
		16.46 Truck	x	1.33 hours	=	21.89 hours	
	Water Proof Sheet	10 m2/truckx	20	trucks x	3 changes =	600	
	Unit Quantity	600 m2 /	172700	=	0.003474	m3	
* 2 :	Excavation/Backfill by Backhoe	0.35m3 for Drain					
	Dimension of Drain	0.5 m wide x	0.5	m depthx(490	x	350)m length
		x	2	/	100 m pitch	=	857.5 m3
	Unit excavation/Backfilling Volume	857.5 m3 /	172700	=	0.004965	m3	
	Cobble Stone	0.004965 /	0.9	loss =	0.005517		
* 3 :	Spreading by Swamp Bull	100 m3 x	1.2 (L) x	0.9 (C) =		108 m3	
* 4 :	Banking	52808 m3					
	Unit Volume	52808 /	172700	=	0.305779	m3	
	Soil	(52808	-	857.5)/	0.9 loss =	57722.7778 m3	
	Unit Volume	57722.78	/	172700	=	0.334237	m3
* 5 :	Labor Rate and Cement:	Cement : 70 kg/m3 is necessary					
	Labor :	2 person/party for Spreading at 50kg (1bag) of cement					
		It takes	10 minutes.				
		Hence,	70 kg/m3 /	50 kg/bag x	2 person/party		
		x	10 minutes /	60 minutes/hour /	7 hours/day		
		=	0.066667	person/m3			
* 6 :	Mixing	$Q = \frac{60 \times q \times E}{Cm}$ (m3/hour)	$Cm = ($	0.027	x	30 m +	
		$q =$	3 m wide x	0.2 m depth x	30 m long =	18 m3/time	4.8 minutes
		$Q =$	146.25 m3/hour	Hence,	$Tm =$	0.00683761 hour/m3	
* 7 :	Final Spreading	500 m x	350 m x	0.2 m depth =	35000 m3		
		35000 m3 /	172700	=	0.202664	m3	
* 8 :	Water Proof Sheet for Disposal Site :	82167 m2 /	172700	m3 =	0.47577881		
	including Drain Pipe and	Foreman :	3 persons x	5 days /	172700	m3 =	8.68558E-05
	Coarse Sand	Common Labor :	10 persons x	5 days /	172700	m3 =	0.000289519
* 9 :	Spreading for Disposal Site :	139440 m2	from Construction Planning /	172700	m3 =	0.8074117	
* 10 :	Slope Clearing :	5907 m2	from Construction Planning /	172700	m3 =	0.03420382	
* 11 :	Drain Pipe :	300 m	from Construction Planning /	172700	m3 =	0.00173712	
* 12 :	Coarse Sand :	37562 m3 (from Construction Planning) /	172700	m3 =	0.21749855		
Reservoir Pit							
* 13 :	Excavation :	1766 m3 (from Construction Planning) /	172700	m3 =	0.01022583		
* 14 :	Concrete :	602.5 m3 (from Construction Planning) /	172700	m3 =	0.00348871		
* 15 :	Formwork :	550 m2 (from Construction Planning) /	172700	m3 =	0.00318471		
* 16 :	Reinforcing Bar :	12050 kg (from Construction Planning) /	172700	m3 =	0.06977417		
* 17 :	Backfilling Gravel :	97 m3 (from Construction Planning) /	172700	m3 =	0.00056167		
* 18 :	Backfilling :	228 m3 (from Construction Planning) /	172700	m3 =	0.00132021		

Table 4.2.5 (6/119) CALCULATION SHEET FOR UNIT COST OF EACH PAYMENT ITEM OF THREE PACKAGES

ID No.	Working Name			Calculation Quantity	Remarks
U-P1-Bq-16	Structural Excavation	U-P3-Bq-21	Structural Excavation	10 m3	
U-P1-Bq-37.1	Structural Excavation	U-P3-Bq-43	Structural Excavation		
U-P2-Bq-20	Structural Excavation	U-P3-Bq-52	Structural Excavation		
U-P2-Bq-37	Structural Excavation	U-P3-Bq-63	Structural Excavation		
U-P2-Bq-61	Structural Excavation	U-P3-Bq-69	Structural Excavation		
U-P2-Bq-72	Structural Excavation	U-P3-Bq-100	Structural Excavation		
U-P2-Bq-125	Structural Excavation	U-P3-Bq-149	Structural Excavation		
U-P2-Bq-150	Structure Excavation	U-P3-Bq-159	Structural Excavation		
U-P2-Bq-163	Structural Excavation	U-P3-Bq-165	Structural Excavation		
U-P2-Bq-171	Structural Excavation	U-P3-Bq-197	Structural Excavation		
U-P2-Bq-178	Structural Excavation	U-P3-Bq-209	Structural Excavation		
U-P2-Bq-196	Structural Excavation	U-P3-Bq-220	Structural Excavation		
U-P2-Bq-253	Structure Excavation	U-P3-Bq-230	Structure Excavation		
U-P2-Bq-261	Structural Excavation	U-P3-Bq-250	Structural Excavation		
U-P2-Bq-270	Structural Excavation	U-P3-Bq-268	Structural Excavation		
U-P2-Bq-305	Structural Excavation	U-P3-Bq-286	Structural Excavation		

Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Direct Cost											
Equipment	A-2-1-48	Dumptruck; 10 ton	hourly	0.87	77269	3060	70744.12	67,224	2,662	61,547	
Working Base Cost											
	CW-1-5	Spreading A	m3	12	2941	35	2823	35,292	420	33,876	for Reclamation Site
	CW-1-46	Excavation A	m3	4	2361	39	1711	9,444	156	6,844	
	CW-1-47	Excavation B	m3	5	2951	48	2138	14,755	240	10,690	
	CW-1-6	Manpower Excavation	m3	1	0	0	15800	0	0	15,800	
Indirect Cost											
Site Expense			%	15	0.8		0.2	31,074	0	7,769	
Profit and Overhead Cost			%	10	0.8		0.2	23,823	0	5,956	
		Miscellaneous	L.S.					87	22	18	Round Up
Total for		10 m3						181,700	3,500	142,500	
Unit Cost for		1 m3						18,170	350	14,250	

*1:	Soil Volume :	10 m3							
*2:	Dump Truck :	10 ton/dump	/	1.5 m3/ton =	6.67 m3/dump				
		10 km/trrd /		40 km/hr +	20 mnts(loss) =	0.58 hours			
		10 m3	/	6.67 m3/dump	=	1.5 dp/10m3			
		1.5 dp/10m3	x	0.58 hours	=	0.87 hours			
*3:	Spreading :	10 m3	x	1.2	=	12 m3			

ID No.	Working Name			Calculation Quantity	Remarks
U-P1-Bq-17	Backfill with Selected Soil			10 m3	
U-P1-Bq-40	Backfill with Selected Soil				
U-P2-Bq-38	Backfill with Selected Soil				

Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Direct Cost											
Material	M-B-4	Sand for Filling and Base Course	m3	2.22	0	1350	25650	0	2,997	56,943	
	M-B-9	Soil for Backfilling	m3	8.89	0	400	7600	0	3,556	67,564	
Working Base Cost											
	CW-1-1	Backfill (Soil) A	m3	4	6076	87	5043	24,304	348	20,172	
	CW-1-2	Backfill (Soil) B	m3	3	7022	103	6326	21,066	309	18,978	
	CW-1-3	Backfill (Soil) C	m3	2	6392	98	6338	12,784	196	12,676	
	CW-1-4	Backfill (Soil) D	m3	2	6038	132	7114	12,076	264	14,228	
Indirect Cost											
Site Expense			%	15	0.8		0.2	32,215	0	8,054	
Profit and Overhead Cost			%	10	0.8		0.2	24,698	0	6,175	
		Miscellaneous	L.S.					56	30	11	Round Up
Total for		10 m3						127,200	7,700	204,800	
Unit Cost for		1 m3						12,720	770	20,480	

*1:	Sand :	2	m3 /	0.9	=	2.22 m3
*2:	Soil :	8	m3 /	0.9	=	8.89 m3

Table 4.2.5 (7/119) CALCULATION SHEET FOR UNIT COST OF EACH PAYMENT ITEM OF THREE PACKAGES

Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Direct Cost											
Labour	L-2-27	Chief of Mason	day	0.38	0	0	58600	0	0	22,268	
	L-2-11	Mason	day	3.8	0	0	39000	0	0	148,200	
Others		Small Tool	%	2				0	0	3,409	Graver, Hammer and etc.
Indirect Cost											
Site Expense			%	15	0.8		0.2	20,865	0	5,216	
Profit and Overhead Cost			%	10	0.8		0.2	15,997	0	3,999	
		Miscellaneous	L.S.					38	0	7	Round Up
Total for	10 m2							36,900	0	183,100	
Unit Cost for	1 m2							3,690	0	18,310	

- *1: Manpower ; Chief of Mason : 0.038 man/m2 x 10 m2 = 0.38
 Mason : 0.38 man/m2 x 10 m2 = 3.8
 Mason : 0.38 person day/m2
- *2: Rate of Mason is quoted from Japanese Standard. (P2109)
- *3: Rate of Chief of Mason is assumption.

Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Direct Cost											
Labour	L-2-23	Common Labour	day	1.5	0	0	35100	0	0	52650	
Material	M-B-4	Sand for Filling and Base Course	m3	11	0	1350	25650	0	14850	282150	
Indirect Cost											
Site Expense			%	15	0.8		0.2	41958	0	10489.5	
Profit and Overhead Cost			%	10	0.8		0.2	32167.8	0	8041.95	
		Miscellaneous	L.S.					74	50	69	Round Up
Total for	10 m3							74,200	14,900	353,400	
Unit Cost for	1 m3							7,420	1,490	35,340	

- * 1 : Sand : 10 m3 / 0.9 loss = 11
- * 2 : Labor Rates are quoted from Japanese Standards.

Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Direct Cost											
Working Base Cost	CW-2-15	Masonry of Crushed Stone, Iceement : 3sand	m3		0	22400	207600	0	0	0	
	CW-2-14	Masonry of Crushed Stone/Riverstone with Iceement : 2 sand	m3	10	0	28800	188500	0	288,000	1,885,000	
Indirect Cost											
Site Expense			%	15	0.8		0.2	260,760	0	65,190	
Profit and Overhead Cost			%	10	0.8		0.2	199,916	0	49,979	
		Miscellaneous	L.S.					24	0	31	Round Up
Total for	10 m3							460,700	288,000	2,000,200	
Unit Cost for	1 m3							46,070	28,800	200,020	

Table 4.2.5 (8/119) CALCULATION SHEET FOR UNIT COST OF EACH PAYMENT ITEM OF THREE PACKAGES

ID No.	Working Name	Calculation Quantity	Remarks
U-P1-Bq-21	Joint Filler, 10 mm thick (Elastic Material)	10 m2	
U-P2-Bq-41	Joint Filler, 10mm thick (Elastic Material)		
U-P2-Bq-70	Joint Filler, 10mm thick (Elastic Material)		

Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Direct Cost											
Labour											
	L-2-1	Foreman	day	0.05	0	0	48800	0	0	2,440	
	L-2-23	Common Labour	day	0.2	0	0	35100	0	0	7,020	
Material											
	M-G-12	Elastic Joint Filler 10mm thick	m2	11.11	0	8250	19250	0	91,658	213,868	
Others											
		Small Tools and Material	%	1				0	917	2,233	Glue, Cutter and etc.
Indirect Cost											
Site Expense											
			%	15	0.8		0.2	38,176	0	9,544	
Profit and Overhead Cost											
		Miscellaneous	L.S.	10	0.8		0.2	29,268	0	7,317	
								53	26	78	Round Up
Total for	10 m2							67,500	92,600	242,500	
Unit Cost for	1 m2							6,750	9,260	24,250	

*1:	Manpower ; Foreman :	1 man/day	/	200	m2/day x	10	m2	=	0.05
	Common Labor :	4 man/day	/	200	m2/day x	10	m2	=	0.2
	Common Labor :	1 manpower	=	50	m2/day				
*2:	Elastic Joint Filler :	10	m2	/	0.9	=	11.11		

ID No.	Working Name	Calculation Quantity	Remarks
U-P1-Bq-23	Pointing	U-P3-Bq-29	Pointing
U-P2-Bq-28	Pointing	U-P3-Bq-57.2	Pointing
U-P2-Bq-42	Pointing	U-P3-Bq-81.1	Pointing
U-P2-Bq-78	Pointing	U-P3-Bq-113.2	Pointing
U-P2-Bq-105	Pointing	U-P3-Bq-162	Pointing
U-P2-Bq-135	Pointing	U-P3-Bq-205	Pointing
U-P2-Bq-160	Pointing	U-P3-Bq-217.1	Pointing
U-P2-Bq-184	Pointing	U-P3-Bq-228.1	Pointing
U-P2-Bq-213	Pointing	U-P3-Bq-238.1	Pointing
U-P2-Bq-313	Pointing	U-P3-Bq-272	Pointing

Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Direct Cost											
Working Base Cost											
	CW-2-44	Plastering 15mm thickness with Cement : 2sand	m2	7	0	1200	10000	0	8,400	70,000	
Others											
		Extra Cost for Labor	%	20				0	1,680	14,000	
Indirect Cost											
Site Expense											
			%	15	0.8		0.2	11,290	0	2,822	
Profit and Overhead Cost											
		Miscellaneous	L.S.	10	0.8		0.2	8,655	0	2,164	
								55	20	14	Round Up
Total for	10 m2							20,000	10,100	89,000	
Unit Cost for	1 m2							2,000	1,010	8,900	

*1 : Pointing 70 % of All Area of Wall

Table 4.2.5 (9/119) CALCULATION SHEET FOR UNIT COST OF EACH PAYMENT ITEM OF THREE PACKAGES

U-P1-Bq-33.1 Sand Bedding is equal unit cost with U-P1-Bq-19

ID No.	Working Name	Calculation Quantity	Remarks
U-P1-Bq-33.2	Concrete Block Pavement	10 m2	
U-P2-Bq-49	Concrete Block Pavement	U-P3-Bq-245.2	Concrete Block Pavement
U-P2-Bq-87	Concrete Block Pavement	U-P3-Bq-262.2	Concrete Block Pavement
U-P2-Bq-323	Concrete Block Pavement	U-P3-Bq-281.2	Concrete Block Pavement
U-P3-Bq-41.2	Concrete Block Pavement		

Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Working Base Cost											
	CW-2-112	Masonry of Paving Block	m2	10	0	5700	44800	0	57,000	448,000	for Embankment
Indirect Cost											
Site Expense			%	15	0.8		0.2	60,600	0	15,150	
Profit and Overhead Cost			%	10	0.8		0.2	46,460	0	11,615	
		Miscellaneous	L.S.					40	0	35	Round Up
Total for				10 m2				107,100	57,000	474,800	
Unit Cost for				1 m2				10,710	5,700	47,480	

ID No.	Working Name	Calculation Quantity	Remarks
U-P1-Bq-33.3	Cement Mortar	U-P3-Bq-41.3	Cement Mortar
U-P2-Bq-50	Cement Mortar	U-P3-Bq-245.3	Cement Mortar
U-P2-Bq-88	Cement Mortar	U-P3-Bq-262.3	Cement Mortar
U-P2-Bq-324	Cement Mortar	U-P3-Bq-271	Cement Mortar Plastering
		U-P3-Bq-281.3	Cement Mortar

Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Working Base Cost											
	CW-2-44	Plastering 15mm thickness with cement : 2sand	m2	10	0	1200	10000	0	12,000	100,000	
Indirect Cost											
Site Expense			%	15	0.8		0.2	13,440	0	3,360	
Profit and Overhead Cost			%	10	0.8		0.2	10,304	0	2,576	
		Miscellaneous	L.S.					56	0	64	Round Up
Total for				10 m2				23,800	12,000	106,000	
Unit Cost for				1 m2				2,380	1,200	10,600	
Unit Cost for				1 m3	6.666667 m2			15,900	8,000	70,700	

ID No.	Working Name	Calculation Quantity	Remarks
U-P1-Bq-33.4	Concrete Kerb	U-P3-Bq-41.4	Concrete Kerb
U-P2-Bq-51	Concrete Kerb	U-P3-Bq-245.4	Concrete Kerb
U-P2-Bq-89	Concrete Kerb	U-P3-Bq-262.4	Concrete Kerb
		U-P3-Bq-281.4	Concrete Kerb

Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Direct Cost											
Equipment	A-2-1-79	Truck Mixer, 4.5 m3	hourly	3.17	77957.88	1560	60651.61	247,126	4,945	192,266	
Working Base Cost											
	CW-1-23	Form Work A	m2	68.67	60	0	44798	4,120	0	3,076,129	
	CW-2-40	Breaking-up the Concrete Form Concrete Work for Small Structure : Type-D	m2	68.67	0	0	3700	0	0	254,067	
	CW-1-21	Structure : Type-D	m3	10	120	42570	193500	1,200	425,700	1,935,000	
Indirect Cost											
Site Expense			%	15	0.8		0.2	736,866	0	184,217	
Profit and Overhead Cost			%	10	0.8		0.2	564,931	0	141,233	
		Miscellaneous	L.S.					56	53	89	Round Up
Total for				10 m3				1,554,300	430,700	5,783,000	
Unit Cost for				1 m3				155,430	43,070	578,300	

*1: Assumed Concrete Volume: 1.5 m3
 *2: Total Formwork Area: 10.3 m2
 Average Formwork Area: 68.67 m2/unit m3
 *3: Truck Mixer: 10.7 m3 / 4.5 m3/truck = 2.38 Truck
 10 km/rnd / 30 km/hr + 60 mnts(loss)* = 1.33 hours
 2.38 Truck x 1.33 hours = 3.17 hours

Table 4.2.5 (10/119) CALCULATION SHEET FOR UNIT COST OF EACH PAYMENT ITEM OF THREE PACKAGES

ID No.		Working Name		Calculation Quantity		Remarks					
U-P1-Bq-36		Coffering and Dewatering		1 L.S.							
Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Direct Cost											
	CW-4-7	Sand Bags	nos	300	88.5375	758.9625	4199.663	26,561	227,689	1,259,899	
	CW-4-23	Temporary Dewatering by D160mm	day	1,000	319831.1	15124	234050.7	319,831,082	15,124,000	234,050,683	
Others											
Indirect Cost											
		Site Expense	%	15	0.8		0.2	68,462,390	0	17,115,597	
		Profit and Overhead Cost	%	10	0.8		0.2	52,487,832	0	13,121,958	
		Miscellaneous	L.S.					34	11	62	
Total for		1 L.S.						440,807,900	15,351,700	265,548,200	
Unit Cost for		1 L.S.						440,807,900	15,351,700	265,548,200	

- * 1 : Sand Bags : $\frac{300 \text{ nos}}{5 \text{ months} \times 4 \text{ days/month} \times 50 \text{ places}} = 1,000 \text{ days/work}$
- * 2 : Drainage Pump Dia.160 :

ID No.		Working Name		Calculation Quantity		Remarks					
U-P1-Bq-36.1		Demolition of existing Concrete		1 L.S.							
Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Direct Cost											
Equipment											
	A-2-2-35	Pick Hammer	daily	2.32	5716.911	0	2030.087	13,257	0	4,707	
	A-2-1-48	Dumptruck; 10 ton	hourly	1.57	77269	3060	70744.12	121,312	4,804	111,068	
	A-2-2-17	Generator; 15 kVA	daily	0.58	82875	1800	52496.05	48,068	1,044	30,448	
Labour											
	L-2-1	Foreman	day	0.58	0	0	48800	0	0	28,290	
	L-2-10	Drill Worker	day	2.32	0	0	39000	0	0	90,435	
	L-2-23	Common Labour	day	2.32	0	0	35100	0	0	81,391	
Working Base Cost											
	CW-1-54	Excavation I	m3	100	5072	83	3675	507,200	8,300	367,500	
Indirect Cost											
		Site Expense	%	15	0.8		0.2	170,139	0	42,535	
		Profit and Overhead Cost	%	10	0.8		0.2	113,426	0	28,356	
		Miscellaneous	L.S.					599	852	269	Round Up
Total for		1 L.S.						974,000	15,000	785,000	
Unit Cost for		1 L.S.						974,000	15,000	785,000	

Manpower Composition, Foreman : 1 man/day
 Common Labor : 4 man/day
 Drill Worker : 4 man/day

* 1 : Demolition Volume = 8 m3

* 2 $8 \text{ m}^3 \times \frac{T_a}{T \times 60} \times \text{Composition of Manpower} = \begin{matrix} \text{Foreman} & \text{Drill Worker} & \text{Common} \\ 0.58 & 2.32 & 2.32 \end{matrix}$

* 3 $8 \text{ m}^3 \times \frac{T_b}{60} = \text{Dump Truck} = 1.57 \text{ hour}$

* 4 $8 \text{ m}^3 \times \frac{T_a}{T \times 60} = \text{Generator} = 0.58 \text{ days}$

* 5 : Average Daily Working Time of Generator, Labor, Breaker $T = \frac{690}{100} = 6.9 \text{ (hour/day)}$

* 6 : Working Time by Hand Breaker / 1m3 (Ta) $T_a = 60 \text{ minutes/m}^3$

* 7 : Working Time by Dump Truck / 1m3 (Tb) $T_b = \frac{5 \text{ km(one way)} \times 2}{20 \text{ minutes} / 10 \text{ ton truck}} = 11.75 \text{ minutes/m}^3$

* 8 : Working Time by Pick Hammer / 1m3 (Tc) $T_c = 60 \text{ minutes/m}^3$

U-P1-Bq-38 Chipping of Existing Outlet Surface is equal unit cost with U-P1-Bq-18.

Table 4.2.5 (11/119) CALCULATION SHEET FOR UNIT COST OF EACH PAYMENT ITEM OF THREE PACKAGES

ID No.		Working Name		Calculation Quantity			Remarks				
U-P1-Bq-39		Concrete, Type C1 including Formwork		10 m3							
Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Direct Cost											
Equipment	A-2-1-79	Truck Mixer; 4.5 m3	hourly	3.17	77957.88	1560	60651.61	247,126	4,945	192,266	
Working Base Cost											
	CW-1-23	Form Work A	m2	19.20354	60	0	44798	1,152	0	860,280	
	CW-2-40	Breaking-up the Concrete Form	m2	19.20354	0	0	3700	0	0	71,053	
		Concrete Work for Type-C by									
	CW-1-60	Shoot Hopper	m3	10	120	43660	197860	1,200	436,600	1,978,600	
Indirect Cost											
Site Expense			%	15	0.8		0.2	455,187	0	113,797	
Profit and Overhead Cost			%	10	0.8		0.2	348,976	0	87,244	
		Miscellaneous	L.S.					58	55	60	Round Up
Total for	10 m3							1,053,700	441,600	3,303,300	
Unit Cost for	1 m3							105,370	44,160	330,330	

*1: Total Concrete Volume : 113 m3
 *2: Total Formwork Area : 217 m2
 Average Formwork Area : 19.20354 m2/unit m3
 *3: Dump Truck : 10.7 m3 / 4.5 m3/truck = 2.38 Truck
 10 km/round / 30 km/hr + 60 mins(loss) = 1.33 hours
 2.38 Truck x 1.33 hours = 3.17 hours
 Cn Fm
 U-P1-Bq-39 53 114

Table 4.2.5 (12/119) CALCULATION SHEET FOR UNIT COST OF EACH PAYMENT ITEM OF THREE PACKAGES

ID No.	Working Name	Calculation Quantity	Remarks								
U-P2-Bq-2	Mobilization and Demobilization	1 L.S.									
Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Direct Cost											
Equipment	A-2-1-69	Trailer; 20 ton	hourly	348	119,879	2,160	102,572	41,718,037	751,680	35,694,941	
	A-2-1-48	Dumptruck; 10 ton	hourly	432	77268.9961	3060	70744.1207	33,380,206	1,321,920	30,561,460	
	A-2-1-80	Truck; 11 ton	hourly	0	96,932	1,560	95,161	0	0	0	
	A-2-1-31	Truck with crane; 4 ton, Crane : 2.9 ton	hourly	128	48669.7513	780	47767.6478	6,229,728	99,840	6,114,259	
	A-2-1-32	Truck with crane; 6 ton	hourly	0	62783.9792	912	61243.4106	0	0	0	
	A-2-2-37	Pontoon Barge; 100 ton	daily	7	314821.292	0	237790.55	2,203,749	0	1,664,534	
	A-2-2-41	Drifter Air Type : 150kg class	daily	14	257007.057	0	88827.324	3,598,099	0	1,243,583	
	A-2-1-84	Tugboat; 15 ton	hourly	48	129433.062	4440	140042.205	6,212,787	213,120	6,722,026	
Indirect Cost											
Site Expense			%	15	0.8		0.2	21,327,596	0	5,331,899	
Profit and Overhead Cost			%	10	0.8		0.2	16,351,157	0	4,087,789	
		Miscellaneous	L.S.					41	40	9	Round Up
Total for	1 L.S.							131,021,400	2,386,600	91,420,500	
Unit Cost for	1 L.S.							131,021,400	2,386,600	91,420,500	

- * 1 : All Equipment : Land Transportation : hours land transportation. hours ship transportation
 All Equipment : Ship Transportation : Though it takes longer time than land transportation, it assumes that the cost is same.
- * 2 : Number of Mobilized and Demobilized Equipment

	Mobilization	Demobilization
Trailer	37	50
Dump Truck 10t	54	54
Ordinary Truck 10t		
Truck with Crane 4 t	16	16
Truck with Crane 6 t		
Pontoon Barge 100 t	1	1
Soil Carriage 100m3	2	2
Tag Boat 15 ton	1	1

ID No.	Working Name	Calculation Quantity	Remarks								
U-P2-Bq-4	Contractor's Site Office and Facilities	1 L.S.									
Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Direct Cost											
		Install of Office and Others	m2	1148	80000	80000	915000	91,840,000	91,840,000	1,050,420,000	including all facilities
		Removal of Office	%	30				27,552,000	27,552,000	315,126,000	
		Rental of Land	%	10				11,939,200	11,939,200	136,554,600	
Indirect Cost											
Site Expense			%	15	0.8		0.2	211,771,560	0	52,942,890	
Profit and Overhead Cost			%	10	0.8		0.2	162,358,196	0	40,589,549	
		Miscellaneous	L.S.					44	0	61	
Total for	1 L.S.							505,461,000	131,331,200	1,595,633,100	
Unit Cost for	1 L.S.							505,461,000	131,331,200	1,595,633,100	

ID No.	Working Name	Calculation Quantity	Remarks								
U-P2-Bq-5	Engineer's Site Office and Facilities	1 L.S.									
Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Direct Cost											
		Install of Office	m2	88	80000	80000	915000	7,040,000	7,040,000	80,520,000	including all facilities
		Removal of Office	%	30				2,112,000	2,112,000	24,156,000	
		Rental of Land	%	10				915,200	915,200	10,467,600	
Indirect Cost											
Site Expense			%	15	0.8		0.2	16,233,360	0	4,058,340	
Profit and Overhead Cost			%	10	0.8		0.2	12,443,576	0	3,111,394	
		Miscellaneous	L.S.					64	0	66	
Total for	1 L.S.							38,746,200	10,067,200	122,313,400	
Unit Cost for	1 L.S.							38,746,200	10,067,200	122,313,400	

Table 4.2.5 (13/119) CALCULATION SHEET FOR UNIT COST OF EACH PAYMENT ITEM OF THREE PACKAGES

ID No.	Working Name	Calculation	Quantity	Remarks	Unit Cost			Cost			Remarks
U-P2-Bq-6	Drawings		1 L.S.		PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Direct Cost											
Labour											
	L-2-34	Cad Operator	day	400	0	0	54700	0	0	21,880,000	
	L-2-35	Draft Man	day	400	0	0	39000	0	0	15,600,000	
Material											
	M-L-15	Drawing Paper (A1)	sheet	200	3000	0	2000	1,600,000	0	400,000	
	M-L-16	Blue Copy (A1)	sheet	200	0	2500	2500	0	500,000	500,000	
Others											
		Tools	%	20				320,000	100,000	7,676,000	Computer, Plotter, Drafter and etc.
Indirect Cost											
		Site Expense	%	15	0.8		0.2	5,829,120	0	1,457,280	
		Profit and Overhead Cost	%	10	0.8		0.2	4,468,992	0	1,117,248	
		Miscellaneous	L.S.					88	0	72	Round Up
Total for 1 L.S.											
								12,218,200	600,000	48,630,600	
Unit Cost for 1 L.S.											
								12,218,200	600,000	48,630,600	

ID No.	Working Name	Calculation	Quantity	Remarks	Unit Cost			Cost			Remarks
U-P2-Bq-7	Surveying		1 L.S.		PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Direct Cost											
		Drawing Plan of River Scale 1:500	ha	10			112,000	0	0	1,120,000	
		River Cross Section Survey Scale 1:100									
		a. Surveying	section	20			232000	0	0	4,640,000	
		b. Drawing & Processing	section	20			62000	0	0	1,240,000	
Indirect Cost											
		Site Expense	%	15	0.8		0.2	840,000	0	210,000	
		Profit and Overhead Cost	%	10	0.8		0.2	644,000	0	161,000	
		Miscellaneous	L.S.					0	0	0	Round Up
Total for 1 L.S.											
								1,484,000	0	7,371,000	
Unit Cost for 1 L.S.											
								1,484,000	0	7,371,000	

Table 4.2.5 (I4/I19) CALCULATION SHEET FOR UNIT COST OF EACH PAYMENT ITEM OF THREE PACKAGES

ID No.	Working Name		Calculation Qu.L.S.							Remarks	
U-P2-Bq-9	Demolition of Existing Pumping Stations		I No.								
Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Direct Cost											
Equipment		Demolition of Brick Masonry	m3	21.22							
	A-2-2-7	Compressor; 3.5-3.7 m3/min	daily	1.5915	120290.344	4032	92494.8987	191,442	6,417	147,206	
	A-2-2-16	Generator; 125 kVA	daily	1.5915	271912.19	15120	209095.974	432,748	24,063	332,776	
	A-2-2-35	Pick Hammer	daily	3.183	5716.91089	0	2030.08672	18,197	0	6,462	
Labour	L-2-1	Foreman	day	2.122	0	0	48800	0	0	103,554	
	L-2-23	Common Labour	day	3.183	0	0	35100	0	0	111,723	
Equipment		Demolition of Column Concrete	m3	2.14							
	A-2-1-7	Backhoe; 0.6 m3	hourly	19.7736	125542.908	2040	90965.0765	2,482,435	40,338	1,798,707	
	A-2-2-10	Concrete Breaker; 20 kg	daily	2.8248	9136.07656	0	3181.31237	25,808	0	8,987	
	A-2-2-16	Generator; 125 kVA	daily	1.4124	271912.19	15120	209095.974	384,049	21,355	295,327	
	A-2-2-7	Compressor; 3.5-3.7 m3/min	daily	1.4124	120290.344	4032	92494.8987	169,898	5,695	130,640	
Labour	L-2-1	Foreman	day	0.9416	0	0	48800	0	0	45,950	
	L-2-2	Operator	day	3.103	0	0	46900	0	0	145,531	
	L-2-23	Common Labour	day	2.9104	0	0	35100	0	0	102,155	
Others		Small Tools	%	2				61,244	1,348	50,546	
Equipment		Accumulation & Loading Truck	m3	23.36							
	A-2-1-7	Backhoe; 0.6 m3	hourly	4.672	125542.908	2040	90965.0765	586,536	9,531	424,989	
Labour	L-2-1	Foreman	day	0.4672	0	0	48800	0	0	22,799	
	L-2-23	Common Labour	day	0.7008	0	0	35100	0	0	24,598	
Equipment		Dump Truck for garvaging:	m3	23.36							
	A-2-1-48	Dumptruck; 10 ton	hourly	10.44192	77,269	3,060	70,744	806,837	31,952	738,704	
Equipment		Demolition of Roofing :	m2	92.25							
	A-2-1-75	Truck Crane; 4.9 ton, Oil Pressure	hourly	27.675	55145.8852	720	48324.4743	1,526,162	19,926	1,337,380	
Labour	L-2-1	Foreman	day	4.6125	0	0	48800	0	0	225,090	
	L-2-12	Carpenter	day	4.6125	0	0	39000	0	0	179,888	
	L-2-23	Common Labour	day	9.225	0	0	35100	0	0	323,798	
Equipment		Removing pumps	unit	16							
	A-2-1-54	Dumptruck; 4 ton Truck Crane; 11(10) ton, Oil Pressure	hourly	32	30,204	1,376	28,632	966,517	44,032	916,217	
	A-2-1-71	Pressure	hourly	48	99321.89	1020	85928.7745	4,767,451	48,960	4,124,581	
Labour	L-2-1	Foreman	day	1.6	0	0	48800	0	0	78,080	
	L-2-23	Common Labour	day	16	0	0	35100	0	0	561,600	
Indirect Cost											
Site Expense			%	15	0.8		0.2	2,989,227	0	747,307	
Profit and Overhead Cost			%	10	0.8		0.2	2,291,741	0	572,935	
		Miscellaneous	L.S.					7	82	72 Round Up	
Total for	I No.							17,700,300	253,700	13,557,600	
Unit Cost for	I No.							17,700,300	253,700	13,557,600	

- * 1 : Demolition of Brick Masonry unit 10 m3
 Foreman : 1 day
 Pick Hammer : 1.5 day
 Generator : 0.75 day
 Common Labor : 1.5 day
 Air Compressor : 0.75 day
- * 2 : Demolition of Column Concrete unit 10 m3
 Foreman : 4.4 day
 Operator : 14.5 day
 Concrete Breaker : 13.2 day
 Backhoe : 92.4 hours
 Small Tools : 2 % of costs above
 Common Labor : 13.6 day
 Air Compressor : 6.6 day
 Generator : 6.6 day
- * 3 : Accumulation & Loading Truck unit 10 m3
 Foreman : 0.2 day
 Backhoe : 2 hours
 Common Labor : 0.3 day
- * 4 : Dump Truck for garvaging: unit 10 m3
 10 km/rnd / 30 Km/hr + 20 mnts loss = 0.67 hours
 10 ton / 1.5 ton/m3 = 6.7 m3/dump
 10 m3 / 6.7 m3 x 0.67 hours = 4.47 hours
- * 5 : Demolition of Roofing : unit 10 m2
 Foreman : 0.5 day
 Carpenter : 0.5 day
 Truck Crane : 3 hours
 Common Labor : 1 day
- * 6 : Removing pumps unit 1 unit
 Foreman : 0.1 day
 Truck Crane : 3 hours
 Common Labor : 1 day
 Dump Truck : 2 hours
- * 7 : Quantities
 Demolition of Brick Masonry : 21.22 m3
 Demolition of Column Concrete : 2.14 m3
 Accumulation & Loading Truck : 23.36 m3
 Dump Truck for garvaging : 23.36 m3
 Demolition of Roofing : 92.25 m2
 Removing pumps : 16 units

Table 4.2.5 (16/119) CALCULATION SHEET FOR UNIT COST OF EACH PAYMENT ITEM OF THREE PACKAGES

ID No.	Working Name	Calculation Quantity	Remarks								
U-P2-Bq-14	Coffering and Dewatering	1 L.S.									
Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Direct Cost											
Material											
	M-E-31	Steel Sheet Pile (Purchasing)	ton	76.8	5700000	0	300000	437,760,000	0	23,040,000	for Relocation
	M-E-31	Steel Sheet Pile (Purchasing)	ton	144	5700000	0	300000	820,800,000	0	43,200,000	for Improvement
	M-B-9	Soil for Backfilling	m ³	289	0	400	7600	0	115,600	2,196,400	for Earth Filling
Working Base Cost											
	CW-1-1	Backfill (Soil) A	m ³	289	6076	87	5043	1,755,964	25,143	1,457,427	for Earth Filling
	CW-1-65	Spreading by Swamp Bulldozer	m ³	289	4284.3203	54.264	4047.348	1,238,169	15,682	1,169,684	for Earth Filling
	CW-1-48	Excavation C	m ³	289	3943	65	2857	1,139,527	18,785	825,673	for Earth Filling
	CW-3-9	Driving In of Steel Sheet Pile (Type-II)	m	1000	9909.0668	76.17755	8578.669	9,909,067	76,178	8,578,669	for Relocation
	CW-3-10	Pulling Out of Steel Sheet Pile (Type-II)	m	1000	9754	67	8548	9,754,000	67,000	8,548,000	for Relocation
	CW-3-9	Driving In of Steel Sheet Pile (Type-II)	m	1875	9909.0668	76.17755	8578.669	18,579,500	142,833	16,085,005	for Improvement
	CW-3-10	Pulling Out of Steel Sheet Pile (Type-II)	m	1875	9754	67	8548	18,288,750	125,625	16,027,500	for Improvement
	CW-4-23	Temporary Dewatering by D160mm	day	210	319831.08	15124	234050.7	67,164,527	3,176,040	49,150,644	for Relocation
	CW-4-23	Temporary Dewatering by D160mm	day	540	319831.08	15124	234050.7	172,708,785	8,166,960	126,387,369	for Improvement
Indirect Cost											
	Site Expense		%	15	0.8		0.2	224,123,340	0	56,030,835	
	Profit and Overhead Cost		%	10	0.8		0.2	171,827,894	0	42,956,974	
	Miscellaneous		L.S.					77	54	21	
Total for	1 L.S.							1,955,049,600	11,929,900	395,654,200	
Unit Cost for	1 L.S.							1,955,049,600	11,929,900	395,654,200	

- Earthfill
- * 1 : Volume of Earthfill (V1) : 289 m³ from Construction Planning
- Steel Sheet Pile for Relocation of Semarang River
- * 2 : Total Length of Driving (L1) : 5 m long x 200 pieces = 1000 m
 - * 3 : Purchasing Steel Sheet Pile (L2) : 8 m long x 200 pieces x 48 kg/m = 76.8 ton
- Steel Sheet Pile for Semarang River Improvement
- * 4 : Total Length of Driving (L3) : 5 m long x 375 pieces = 1875 m
 - * 5 : Purchasing Steel Sheet Pile (L4) : 8 m long x 375 pieces x 48 kg/m = 144 ton
- Dewatering for Relocation of Semarang River
- * 6 : D160mm 7 month x 30 days/month = 210 days
- Dewatering for Semarang River Improvement
- * 7 : D160mm 18 month x 30 days/month = 540 days

ID No.	Working Name	Calculation Quantity	Remarks								
U-P2-Bq-14	Temporary Construction Road	1 L.S.									
Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Material											
	M-B-13	Solid Soil	m ³	2147	0	600	11400	0	1,288,200	24,475,800	for Embankment
	M-B-12	Crushed Stone for Pavement and Concrete	m ³	295	0	3250	61750	0	958,750	18,216,250	for Embankment
Working Base Cost											
	CW-1-46	Excavation A	m ³	1254	2361	39	1711	2,960,694	48,906	2,145,594	
	CW-1-56	Spreading and Compaction for Gravel Pavement	m ³	295	5117.1681	43.2	16431.17	1,509,565	12,744	4,847,194	for Embankment
	CW-1-59	Spreading and Compaction-D	m ³	2147	1509	19	1473	3,239,823	40,793	3,162,531	for Embankment
Indirect Cost											
	Site Expense		%	15	0.8		0.2	7,548,821	0	1,887,205	
	Profit and Overhead Cost		%	10	0.8		0.2	5,787,430	0	1,446,857	
	Miscellaneous		L.S.					68	7	68	Round Up
Total for	1 L.S.							21,046,400	2,349,400	56,181,500	
Unit Cost for	1 L.S.							21,046,400	2,349,400	56,181,500	

- for Embankment
- * 1 : Temporary Road Body Volume (V1) : 2147 m³ from Construction Planning
 - * 2 : Gravel Pavement Volume (V2) : 0.15 m thick x 3.0 m wide = 0.45 m³/m
Purchased Crushed Stone : 0.45 m³/m / 0.9 loss = 0.5 m³/m
 - * 3 : Temporary Road Length (l1) : 583 m from Construction Planning roundup 590 m
- Excavation
- * 4 : Excavation Volume (V3) : 1254 m³ from Construction Planning

Table 4.2.5 (17/119) CALCULATION SHEET FOR UNIT COST OF EACH PAYMENT ITEM OF THREE PACKAGES

ID No.	Working Name		Calculation Quantity			Remarks					
U-P2-Bq-15	Clearing of Garbage		1 L.S.								
Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Direct Cost											
Equipment	A-2-1-54	Dumptruck; 4 ton	hourly	3.32	30203.663	1376	28631.77	100,276	4,568	95,057	
	A-2-1-7	Backhoe; 0.6 m3	hourly	7	125542.91	2040	90965.08	878,800	14,280	636,756	
Labour											
	L-2-1	Foreman	day	1	0	0	48800	0	0	48,800	
	L-2-23	Common Labour	day	10	0	0	35100	0	0	351,000	
Material											
		Plastic Bag	sheet	100	0	0	1000	0	0	100,000	
Indirect Cost											
Site Expense			%	15	0.8		0.2	267,545	0	66,886	
Profit and Overhead Cost			%	10	0.8		0.2	205,117	0	51,279	
		Miscellaneous	L.S.					61	52	21	Round Up
Total for	1 L.S.							1,451,800	18,900	1,349,800	
Unit Cost for	1 L.S.							1,451,800	18,900	1,349,800	

* 1: Dump Truck : 4 Truck (Assumption)
 10 km/rnd / 4 Truck x 20 km/hr + 20 mnts(loss)= 0.83 hours
 = 3.32 hours

ID No.	Working Name		Calculation Quantity			Remarks					
U-P2-Bq-17	Common channel excavation including hauling and spoiling		100 m3								
Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Direct Cost											
Equipment	A-2-1-48	Dumptruck; 10 ton	hourly	17.78	77268.996	3060	70744.12	1,373,843	54,407	1,257,830	from Hauling
Working Base Cost											
	CW-1-5	Spreading A	m3	100	2941	35	2823	294,100	3,500	282,300	at the disposal site
	CW-1-47	Excavation B	m3	100	2951	48	2138	295,100	4,800	213,800	for Dredging
Indirect Cost											
Site Expense			%	15	0.8		0.2	453,562	0	113,390	
Profit and Overhead Cost			%	10	0.8		0.2	347,731	0	86,933	
		Miscellaneous	L.S.					65	93	46	
Total for	100 m3							2,764,400	62,800	1,954,300	
Unit Cost for	1 m3							27,644	628	19,543	

* 1: Dump Truck for Hauling: 107 m3 / 6.5 m3/truck = 16.46 Truck
 15 km/rnd / 20 km/hr + 20 mnts(loss)= 1.08 hours
 16.46 Truck x 1.08 hours = 17.78 hours

* 7: Spreading at Disposal Site 100 m3

Table 4.2.5 (18/119) CALCULATION SHEET FOR UNIT COST OF EACH PAYMENT ITEM OF THREE PACKAGES

ID No.	Working Name	Calculation Quantity	Remarks								
U-P2-Bq-18	Excavation below water level including heaving and treatment of contaminated soil	100 m ³									
Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Direct Cost											
Equipment											
	A-2-1-84	Tugboat; 15 ton	hourly	1.17	129433.06	4440	140042.2	151,437	5,195	163,849	from Ship
	A-2-2-41	Drifter Air Type : 150kg class	daily	4.99	257007.06	0	88827.32	1,282,465	0	443,248	from Ship
	A-2-2-37	Pontoon Barge; 100 ton	daily	1.17	314821.29	0	237790.6	368,341	0	278,215	from Ship
	A-2-1-7	Backhoe; 0.6 m3	hourly	2.82	125542.91	2040	90965.08	354,031	5,753	256,522	for Transportation
	A-2-1-1	Backhoe; 2 m3 Long Arm	hourly	1.17	512434.99	4440	355749.2	599,549	5,195	416,227	from Ship
Labour											
	L-2-1	Foreman	day	0.008686	0	0	48800	0	0	424	for Disposal Site
	L-2-23	Common Labour	day	0.028952	0	0	35100	0	0	1,016	for Disposal Site
	L-2-1	Foreman	day	0.17	0	0	48800	0	0	8,295	for Excavation by ship
	L-2-23	Common Labour	day	0.34	0	0	35100	0	0	11,934	for Excavation by ship
Equipment											
	A-2-1-48	Dumptruck; 10 ton	hourly	13.66	77268.996	3060	70744.12	1,055,494	41,800	966,365	from Hauling
	A-2-2-2	Stabilizer	hourly	0.683761	743275.52	1092	502607.8	508,223	747	343,663	for Mixing
Labour											
	L-2-23	Common Labour	day	6.666667	0	0	35100	0	0	234,000	for Spreading of Concrete
Material											
		Drain Pipe	m	0.173712	108000	0	12000	18,761	0	2,085	for Hauling
		Water Proof Sheet for Disposal Site	m ²	0.008686	27000	0	3000	235	0	26	for Disposal Site
		Water Proof Sheet for Dump Truck	sheet	0.347423	0	0	50000	0	0	17,371	for Hauling
	M-B-3	Sand for Mortar (Masonry)	m ³	21.74986	0	2250	42750	0	48,937	929,806	for Backfill of Drain
	M-C-1	Portland Cement	kg	7000	0	100	400	0	700,000	2,800,000	for Backfill of Drain
	M-B-5	Cobble Stone	m ³	0.496526	0	1850	35150	0	919	17,453	for Backfill of Drain
	M-B-13	Solid Soil	m ³	33.42373	0	600	11400	0	20,054	381,030	for Banking
Working Base Cost											
	CW-1-64	Excavation by Backhoe 0.35m ³	m ³	0.496526	2687.7729	45.24	1953.753	1,335	22	970	for Excavation
	CW-1-64	Excavation by Backhoe 0.35m ³	m ³	0.551695	2687.7729	45.24	1953.753	1,483	25	1,078	for Backfill
	CW-1-65	Spreading by Swamp Bulldozer	m ³	108	4284.3203	54.264	4047.348	462,707	5,861	437,114	for Spreading
	CW-1-5	Spreading A	m ³	80.74117	2941	35	2823	237,460	2,826	227,932	for Disposal Site
	CW-1-10	Slope Clearing for Embankment 2	m ²	3.420382	3265	54	2660	11,168	185	9,098	for Final Spreading
	CW-1-5	Spreading A	m ³	20.26636	2941	35	2823	59,603	709	57,212	for Final Spreading
	CW-1-1	Backfill (Soil) A	m ³	30.57788	6076	87	5043	185,791	2,660	154,204	for Banking
		Spreading and Compaction for Earth									
	CW-1-58	Filling	m ³	30.57788	2833.8029	36.252	2632.618	86,652	1,109	80,500	for Banking
	CW-1-47	Excavation B	m ³	1.022583	2951	48	2138	3,018	49	2,186	for Pit
		Concrete Work for Small Structure :									
	CW-1-21	Type-D	m ³	0.348871	120	42570	193500	42	14,851	67,507	for Pit
	CW-1-23	Form Work A	m ²	0.318471	60	0	44798	19	0	14,267	for Pit
	CW-1-29	Reinforcing Bar Setup I	t	0.006977	0	3120900	3325100	0	21,776	23,201	for Pit
	CW-1-15	Gravel Bedding	m ³	0.056167	0	1360	31260	0	76	1,756	for Pit
	CW-1-2	Backfill (Soil) B	m ³	0.132021	7022	103	6326	927	14	835	for Pit
Indirect Cost											
		Site Expense	%	15	0.8		0.2	1,754,027	0	438,507	
		Profit and Overhead Cost	%	10	0.8		0.2	1,344,754	0	336,188	
		Miscellaneous	L.S.					81	38	15	
Total for 100 m³											
								8,487,600	878,800	9,124,100	
Unit Cost for 1 m³											
								84,876	8,788	91,241	

- * 1 : Dump Truck for Hauling: 107 m³ / 6.5 m³/truck = 16.46 Truck
 10 km/hr + 20 km/hr = 20 mnts(loss) = 0.83 hours
 16.46 Truck x 0.83 hours = 13.66 hours
- Water Proof Sheet
 10 m²/truck x 20 trucks x 3 changes = 600
 Unit Quantity 600 m² / 172700 = 0.003474 m³
- * 2 : Excavation/Backfill by Backhoe 0.35m³ for construction of drain
 Dimension of Drain 0.5 m wide x 0.5 m depth x 490 x 350 = 857.5 m length
 x 2 / 100 m pitch = 857.5 m³
 Unit excavation/Backfilling Volume 857.5 m³ / 172700 = 0.004965 m³
 Cobble Stone 0.004965 / 0.9 loss = 0.005517
- * 3 : Spreading by Swamp Bull 100 m³ x 1.2 (L) x 0.9 (C) = 108 m³
- * 4 : Banking 52808 m³ from Construction Planning
 Unit Volume 52808 / 172700 = 0.305779 m³
 Soil (52808 - 857.5) / 172700 = 0.9 loss = 57722.7778 m³
 Unit Volume 57722.78 / 172700 = 0.334237 m³
- * 5 : Labor Rate and Cement:
 Cement : 70 kg/m³ is necessary
 Labor : 2 person/party for Spreading at 50kg (1bag) of cement
 It takes 10 minutes.
 Hence, 70 kg/m³ / 50 kg/bag x 2 person/party = 0.066667 person/m³
 x 10 minutes / 60 minutes/hour / 7 hours/day
- * 6 : Mixing
 $Q = \frac{60 \times q \times E}{Cm} \text{ (m³/hour)}$ Cm = (0.027 x 0.79 x 3 round = 4.8 minutes
 q = 3 m wide x 0.2 m depth x 30 m long = 18 m³/time
- * 7 : Final Spreading
 500 m x 350 m x 0.2 m depth = 35000 m³
 35000 m³ / 172700 = 0.202664 m³
- * 8 : Backhoe & Tugboat & Pontoon
 1 hour = 3600 q = 1.9 f = 1
 E = work efficiency = 0.5 Cm = 40 Q = 85.5
 Hence, Driving Time = 100 m³ / 85.5 = 1.17 hour/100m³
 = 0.17 day/100m³
- * 9 : Backhoe for Transportation
 1 hour = 3600 q = 0.59 f = 0.83333333
 E = work efficiency = 0.6 Cm = 30 Q = 35.4
 Hence, Driving Time = 100 m³ / 35.4 = 2.82 hour/100m³

Table 4.2.5 (19/119) CALCULATION SHEET FOR UNIT COST OF EACH PAYMENT ITEM OF THREE PACKAGES

* 10 :	Soil Carriage Ship:	1.17	hour *	2.82	hour *	1	hour for Moving =	4.99	day/100m3
* 11 :	Labor Rate :		Foreman :	1	person/party		Hence,	0.17	
			Common Labor :	2	person/party		Hence,	0.34	
* 9 :	Water Proof Sheet for Disposal Site :	82167	m2 /	172700	m3 =		0.475778807		
	including Drain Pipe and Coarse Sand		Foreman :	3	persons x	5 days /	172700	m3 =	8.68558E-05
* 10 :	Spreading for Disposal Site :	139440	m2		from Construction Planning /	5 days /	172700	m3 =	0.000289519
* 11 :	Slope Clearing :	5907	m2		from Construction Planning /		172700	m3 =	0.807411697
* 12 :	Drain Pipe :	300	m		from Construction Planning /		172700	m3 =	0.034203822
* 13 :	Coarse Sand :	37562	m3 (from Construction Planning) /				172700	m3 =	0.001737116
Reservoir Pit								m3 =	0.217498552
* 14 :	Excavation :	1766	m3 (from Construction Planning) /				172700	m3 =	0.010225825
* 15 :	Concrete :	602.5	m3 (from Construction Planning) /				172700	m3 =	0.003488709
* 16 :	Formwork :	550	m2 (from Construction Planning) /				172700	m3 =	0.003184713
* 17 :	Reinforcing Bar :	12050	kg (from Construction Planning) /				172700	m3 =	0.069774175
* 18 :	Backfilling Gravel :	97	m3 (from Construction Planning) /				172700	m3 =	0.000561668
* 19 :	Backfilling :	228	m3 (from Construction Planning) /				172700	m3 =	0.001320208

U-P2-Bq-20 Structural Excavation is equal unit cost with U-P1-Bq-16.

ID No.	Working Name			Calculation Quantity	Remarks
U-P2-Bq-21	Backfill with Cobble			100 m3	
U-P2-Bq-62	Backfill with Cobble	U-P3-Bq-22	Backfill with Cobble		
U-P2-Bq-73	Backfill with Cobble				
U-P2-Bq-306	Backfill with Cobble	U-P3-Bq-198	Backfill with Cobble		

Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Direct Cost											
Equipment	A-2-1-7	Backhoe; 0.6 m3	hourly	4	125542.91	2040	90965.08	502,172	8,160	363,860	
	A-2-1-88	Vibrating Roller; 0.8-1.1 ton (Hand Guide)	hourly	7	17057.227	144	14790.56	119,401	1,008	103,534	
Labour	L-2-23	Common Labour	day	4	0	0	35100	0	0	140,400	
Material	M-B-5	Cobble Stone	m3	111.11	0	1850	35150	0	205,554	3,905,517	
Working Base Cost	CW-1-8	Tamper Loading	m3	30	1760	60	2600	17,600	600	26,000	
Indirect Cost											
Site Expense			%	15	0.8		0.2	647,257	0	161,814	
Profit and Overhead Cost			%	10	0.8		0.2	496,230	0	124,038	
		Miscellaneous	L.S.					41	79	18	Round Up
Total for		100 m3						1,782,700	215,400	4,825,200	
Unit Cost for		1 m3						17,827	2,154	48,252	

- * 1 : Cobble Stone 100 m3 / 0.9 loss = 111.11 m3
- * 2 : All rates are quoted from Japanese Standard.

ID No.	Working Name			Calculation Quantity	Remarks
U-P2-Bq-22	Backfill with Gravel	U-P3-Bq-23	Backfill with Gravel	10 m3	
U-P2-Bq-63	Backfill with Gravel	U-P3-Bq-199	Backfill with Gravel		
U-P2-Bq-74	Backfill with Gravel	U-P3-Bq-211	Backfill with Gravel		
U-P2-Bq-307	Backfill with Gravel	U-P3-Bq-222	Backfill with Gravel		
		U-P3-Bq-232	Backfill with Gravel		

Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Direct Cost											
Working Base Cost	CW-1-16	Backfilling Gravel A	m3	10	0	4060	83560	0	40600	835600	
Indirect Cost											
Site Expense			%	15	0.8		0.2	105,144	0	26,286	
Profit and Overhead Cost			%	10	0.8		0.2	80,610	0	20,153	
		Miscellaneous	L.S.					46	0	61	Round Up
Total for		10 m3						185,800	40,600	882,100	
Unit Cost for		1 m3						18,580	4,060	88,210	

Table 4.2.5 (20/119) CALCULATION SHEET FOR UNIT COST OF EACH PAYMENT ITEM OF THREE PACKAGES

ID No.	Working Name	Calculation Quantity	Remarks
U-P2-Bq-23	Backfill with Sandy Soil	10 m3	
U-P2-Bq-75	Backfill with Sandy Soil		
U-P2-Bq-95	Backfill with Sandy Soil		
U-P2-Bq-126	Backfill with Sandy Soil		
U-P2-Bq-151	Backfill with Sandy Soil		
U-P2-Bq-254	Backfill with Sandy Soil		
U-P2-Bq-271	Backfill with Sandy Soil		
U-P2-Bq-308	Backfill with Sandy Soil		
U-P3-Bq-24	Backfill with Sandy Soil		
U-P3-Bq-53	Backfill with Sandy Soil		
U-P3-Bq-160	Backfill with Sandy Soil		
U-P3-Bq-166	Backfill with Sandy Soil		
U-P3-Bq-200	Backfill with Sandy Soil		
U-P3-Bq-252	Backfill with Sandy Soil		
U-P3-Bq-269	Backfill with Sandy Soil		
U-P3-Bq-287	Backfill with Sandy Soil		

Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Direct Cost											
Material	M-B-4	Sand for Filling and Base Course	m3	11.11	0	1350	25650	0	14,999	284,972	
Working Base Cost											
	CW-1-1	Backfill (Soil) A	m3	4	6076	87	5043	24,304	348	20,172	
	CW-1-2	Backfill (Soil) B	m3	3	7022	103	6326	21,066	309	18,978	
	CW-1-3	Backfill (Soil) C	m3	2	6392	98	6338	12,784	196	12,678	
	CW-1-4	Backfill (Soil) D	m3	2	6038	132	7114	12,076	264	14,228	
Indirect Cost											
Site Expense			%	15	0.8		0.2	52,485	0	13,121	
Profit and Overhead Cost			%	10	0.8		0.2	40,238	0	10,060	
		Miscellaneous	L.S.					47	85	94	Round Up
Total for	10 m3							163,000	16,200	374,300	
Unit Cost for	1 m3							16,300	1,620	37,430	

*1 : Sand : 10 m3 / 0.9 = 11.11 m3
 *2 : All Rates are quoted from Japanese Standard

ID No.	Working Name	Calculation Quantity	Remarks
U-P2-Bq-24	Concrete, Type C1 including Formwork	10 m3	

Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Direct Cost											
Equipment	A-2-1-79	Truck Mixer; 4.5 m3	hourly	3.17	77957.876	1560	60651.61	247,126	4,945	192,266	
Working Base Cost											
	CW-1-23	Form Work A	m2	54.65	60	0	44798	3,279	0	2,448,171	
	CW-2-40	Breaking-up the Concrete Form	m2	54.65	0	0	3700	0	0	202,202	
	CW-1-60	Concrete Work for Type-C by Shoot Hopper	m3	10	120	43660	197860	1,200	436,600	1,978,600	
Indirect Cost											
Site Expense			%	15	0.8		0.2	661,727	0	165,432	
Profit and Overhead Cost			%	10	0.8		0.2	507,324	0	126,831	
		Miscellaneous	L.S.					44	55	99	Round Up
Total for	10 m3							1,420,700	441,600	3,113,600	
Unit Cost for	1 m3							142,070	44,160	311,360	

*1 : Total Concrete Volume : 228 m3
 *2 : Total Formwork Area : 1246 m2
 Average Formwork Area : 54.64912 m2/unit m3
 *3 : Truck Mixer : 10.7 m3 / 4.5 m3/truck = 2.38 Truck
 10 km/rnd / 30 km/hr + 60 mnts(loss) = 1.33 hours
 2.38 Truck x 1.33 hours = 3.17 hours

ID U-P2-Bq-24 Con Fm 276 2004

ID No.	Working Name	Calculation Quantity	Remarks
U-P2-Bq-25	Concrete, Type E including Formwork	10 m3	

Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Direct Cost											
Equipment	A-2-1-79	Truck Mixer; 4.5 m3	hourly	3.02	77957.876	1560	60651.61	235,433	4,711	183,168	
Working Base Cost											
	CW-1-28	Form Work F	m2	32.65625	0	0	36510	0	0	1,192,280	
	CW-1-22	Concrete Work for Levelling	m3	10	120	37130	158740	1,200	371,300	1,587,400	
Indirect Cost											
Site Expense			%	15	0.8		0.2	429,059	0	107,265	
Profit and Overhead Cost			%	10	0.8		0.2	328,945	0	82,236	
		Miscellaneous	L.S.					63	89	51	Round Up
Total for	10 m3							994,700	376,100	3,152,400	
Unit Cost for	1 m3							99,470	37,610	315,240	

*1 : Form Work Area : Total Form work area 209 / Concrete Volume 64 = From Work Area m2 / concrete 1m3 3.265625 m2/m3
 *2 : Dump Truck : 10.2 m3 / 4.5 m3/truck = 2.27 Truck
 10 km/rnd / 30 km/hr + 60 mnts(loss) = 1.33 hours
 2.27 Truck x 1.33 hours = 3.02 hours

Table 4.2.5 (21/119) CALCULATION SHEET FOR UNIT COST OF EACH PAYMENT ITEM OF THREE PACKAGES

ID No.	Working Name	Calculation Quantity	Remarks
U-P2-Bq-26	Deformed Reinforcing Bars	1000 kg	
U-P2-Bq-66	Deformed Reinforcing Bars		
U-P3-Bq-27	Deformed Reinforcing Bars		
U-P3-Bq-204	Deformed Reinforcing Bars		

Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Direct Cost											
Working Base Cost											
	CW-1-31	Reinforcing Bar Setup 2	t	1	0	28088.10	2992590	0	2,808,810	2,992,590	
Indirect Cost											
		Site Expense	%	15	0.8		0.2	696,168	0	174,042	
		Profit and Overhead Cost	%	10	0.8		0.2	533,729	0	133,432	
		Miscellaneous	L.S.					3	90	36	Round Up
Total for								1,229,900	2,808,900	3,300,100	
Unit Cost for								1,230	2,809	3,300	

U-P2-Bq-27 Wet Stone Masonry is equal unit cost with U-P1-Bq-26.

U-P2-Bq-28 Pointing is equal unit cost with U-P1-Bq-23.

ID No.	Working Name	Calculation Quantity	Remarks
U-P2-Bq-29	Weep Hole, Dia.50mm	10 m	
U-P2-Bq-68	Weep Hole, Dia.50mm		
U-P2-Bq-79	Weep Hole, Dia.50mm		
U-P2-Bq-104	Weep Hole, Dia.50mm		
U-P2-Bq-134	Weep Hole, Dia.50mm		
U-P2-Bq-159	Weep Hole, Dia.50mm		
U-P2-Bq-188	Weep Hole, Dia.50mm		
U-P2-Bq-214	Weep Hole, Dia.50mm		
U-P2-Bq-314	Weep Hole, Dia.50mm		
U-P3-Bq-30	Weep Hole, Dia. 50 mm		
U-P3-Bq-57.1	Weep Hole, Dia. 50 mm		
U-P3-Bq-81	Weep Hole, Dia. 50 mm		
U-P3-Bq-113.1	Weep Hole, Dia. 50 mm		
U-P3-Bq-206	Weep Hole, Dia. 50 mm		
U-P3-Bq-217	Weep Hole, Dia. 50 mm		
U-P3-Bq-228	Weep Hole, Dia. 50 mm		
U-P3-Bq-238	Weep Hole, Dia. 50 mm		
U-P3-Bq-272.1	Weep Hole, Dia. 50 mm		

Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Direct Cost											
Labour											
	L-2-1	Foreman	day	3	0	0	48800	0	0	146,400	
	L-2-28	Chief of Concrete Worker	day	0.2	0	0	58600	0	0	11,720	
	L-2-17	Concrete Worker	day	1.75	0	0	39000	0	0	68,250	
	L-2-23	Common Labour	day	3	0	0	35100	0	0	105,300	
Material											
	M-G-4	PVC Pipe, Dia. 50mm	m	11.11	0	0	2340	0	25,997	60,661	
	M-G-13	Geotextile	m2	0.13	7837.5	0	412.5	1,019	0	54	
Working Base Cost											
Indirect Cost											
		Site Expense	%	15	0.8		0.2	50,328	0	12,582	
		Profit and Overhead Cost	%	10	0.8		0.2	38,585	0	9,646	
		Miscellaneous	L.S.					68	3	88	Round Up
Total for								90,000	26,000	414,700	
Unit Cost for								9,000	2,600	41,470	
Unit Cost for								3,147	909	14,500	

*1	Composition of Manpowers is quoted from Working Cost of PU										
*2	PVC pipe	1 m /	0.9	x	10 m	=				11.11	
*3	Palm Fiber	0.01 m2 /	0.75	x	10 m	=				0.13	

ID No.	Working Name	Calculation Quantity	Remarks
U-P2-Bq-30	Log Pile, Dia.150 mm, L=3.0m	12 m	
U-P2-Bq-103	Log Pile, Dia. 150mm, L=3.0m		
U-P3-Bq-206.1	Log Pile, Dia. 150 mm, L = 3.0 m		

Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Direct Cost											
Material											
	M-D-1	Log Pile, Dia. 15cm	m	12	0	0	10000	0	0	120,000	
Working Base Cost											
	CW-3-23	Driving In of Log Pile L=3.0m	piece	4	29500.914	437.433	22371.15	118,004	1,750	89,485	
Indirect Cost											
		Site Expense	%	15	0.8		0.2	39,509	0	9,877	
		Profit and Overhead Cost	%	10	0.8		0.2	30,290	0	7,572	
		Miscellaneous	L.S.					98	50	66	Round Up
Total for								187,900	1,800	227,000	
Unit Cost for								15,658	150	18,917	

Table 4.2.5 (22/119) CALCULATION SHEET FOR UNIT COST OF EACH PAYMENT ITEM OF THREE PACKAGES

ID No. Working Name Calculation Quantity Remarks
 U-P2-Bq-31 Gabion Mattress (t=500mm (Galvanized)) 10 m³
 U-P2-Bq-209 Gabion Mattress, t=500 mm (Galvanized)
 U-P2-Bq-281 Gabion Mattress (t=500 mm (Galvanized))
 U-P3-Bq-110.1 Gabion Mattress, t=500 mm (Galvanized)
 U-P3-Bq-172.1 Gabion Mattress, t= 500 mm (Galvanized)

Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Direct Cost											
Equipment	A-2-1-7	Backhoe; 0.6 m ³	hourly	2.7	125542.91	2040	90965.08	338,966	5,508	245,606	
Labour	L-2-27	Chief of Mason	day	0.45	0	0	58600	0	0	26,370	
	L-2-11	Mason	day	0.48	0	0	39000	0	0	18,720	
	L-2-23	Common Labour	day	1.7	0	0	35100	0	0	59,670	
Material	M-E-68	Gabion Mattress; 2.7mm, 3.0x1.0x0.5m, Galvanized	pcs	15	327037.5	0	17212.5	4,905,563	0	258,188	
	M-B-7	Boulder	m ³	10	0	2500	47500	0	25,000	475,000	
Indirect Cost											
Site Expense			%	15	0.8		0.2	763,031	0	190,758	
Profit and Overhead Cost			%	10	0.8		0.2	584,990	0	146,248	
		Miscellaneous	L.S.					51	92	42	Round Up
Total for		10 m ³						6,592,600	30,600	1,420,600	
Unit Cost for		1 m ³						659,260	3,060	142,060	

*1 All composition numbers are quoted from Japanese Standard.

ID No. Working Name Calculation Quantity Remarks
 U-P2-Bq-33 Furnishing and Driving PC Sheet Pile (t=220 mm) 10 m
 U-P2-Bq-82 Furnishing and Driving PC Sheet Pile (t=220 mm) U-P3-Bq-95 Furnishing and Driving PC Sheet Pile (t=220 mm)
 U-P2-Bq-208 Furnishing and Driving PC Sheet Piles (t=220 mm) U-P3-Bq-110 Furnishing and Driving PC Sheet Pile (t=220 mm)
 U-P2-Bq-316 Furnishing and Driving PC Sheet Pile (t=220 mm)
 U-P3-Bq-32 Furnishing and Driving PC Sheet Pile (t=220 mm)

Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Direct Cost											
Material	M-C-48	Prestressed Concrete Sheet Pile (B=0.5m, t=0.22m)	m	10	190000	0	10000	1,900,000	0	100,000	
Working Base Cost	CW-3-11	Driving In of Concrete Sheet Pile (t=22)	m	10	15343	136	12963	153,430	1,360	129,630	
Indirect Cost			%	15	0.8		0.2	274,130	0	68,533	
Profit and Overhead Cost			%	10	0.8		0.2	210,167	0	52,542	
		Miscellaneous	L.S.					73	40	96	Round Up
Total for		10 m						2,537,800	1,400	350,800	
Unit Cost for		1 m						253,780	140	35,080	

Length of Sheet Pile	Length of Driving
1	10
2	0
3	0
4	0
5	0
6	0
Total	10
Ratio	10

ID No. Working Name Calculation Quantity Remarks
 U-P2-Bq-34 Concrete, Type C1 including Formwork 10 m³

Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Direct Cost											
Equipment	A-2-1-79	Truck Mixer; 4.5 m ³	hourly	3.17	77957.876	1560	60651.61	247,126	4,945	192,266	
Working Base Cost	CW-1-23	Form Work A	m ²	74.36	60	0	44798	4,462	0	3,331,133	
	CW-2-40	Breaking-up the Concrete Form	m ²	74.36	0	0	3700	0	0	275,128	
	CW-1-60	Hopper	m ³	10	120	43660	197860	1,200	436,600	1,978,600	
Indirect Cost			%	15	0.8		0.2	776,575	0	194,144	
Profit and Overhead Cost			%	10	0.8		0.2	595,374	0	148,844	
		Miscellaneous	L.S.					62	55	85	Round Up
Total for		10 m ³						1,624,800	441,600	6,120,200	
Unit Cost for		1 m ³						162,480	44,160	612,020	

*1: Total Concrete Volume : 39 m³
 *2: Total Formwork Area : 290 m²
 Average Formwork Area : 74.36 m²/unit m³
 *3: Truck Mixer : 10.7 m³ / 4.5 m³/truck = 2.38 Truck
 10 km/rnd / 30 km/hr + 60 min(=loss) = 1.33 hours
 2.38 Truck x 1.33 hours = 3.17 hours

Table 4.2.5 (23/119) CALCULATION SHEET FOR UNIT COST OF EACH PAYMENT ITEM OF THREE PACKAGES

ID No.	Working Name	Calculation Quantity	Remarks
U-P2-Bq-35	Deformed Reinforcing Bars	U-P3-Bq-34	Deformed Reinforcing Bars
U-P2-Bq-84	Deformed Reinforcing Bars	U-P3-Bq-48	Deformed Reinforcing Bars
U-P2-Bq-167	Deformed Reinforcing Bars	U-P3-Bq-91	Deformed Reinforcing Bars
U-P2-Bq-175	Deformed Reinforcing Bars	U-P3-Bq-214	Deformed Reinforcing Bars
U-P2-Bq-182	Deformed Reinforcing Bars	U-P2-Bq-312	Deformed Reinforcing Bars

Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Direct Cost											
Working Base Cost											
	CW-1-29	Reinforcing Bar Setup 1	t	1	0	3120900	3325100	0	3,120,900	3,325,100	
Indirect Cost											
		Site Expense	%	15	0.8		0.2	773,520	0	193,380	
Profit and Overhead Cost											
		Miscellaneous	L.S.					48	0	62	Round Up
Total for 1000 kg								1,366,600	3,120,900	3,666,800	
Unit Cost for 1 kg								1,367	3,121	3,667	

- U-P2-Bq-37 Structural Excavation is equal unit cost with U-P1-Bq-16.
- U-P2-Bq-38 Backfill with Selected Soil is equal unit cost with U-P1-Bq-17.
- U-P2-Bq-39 Sand Bedding is equal unit cost with U-P1-Bq-19.
- U-P2-Bq-40 Wet Stone Masonry is equal unit cost with U-P1-Bq-20.
- U-P2-Bq-41 Joint Filler, 10mm thick (Elastic Material) is equal unit cost with U-P1-Bq-21.
- U-P2-Bq-42 Pointing is equal unit cost with U-P1-Bq-23.

ID No.	Working Name	Calculation Quantity	Remarks
U-P2-Bq-44	Stripping of Top Soil	10 m3	
U-P2-Bq-318	Stripping of Top Soil		
U-P3-Bq-18	Stripping of Top Soil		
U-P3-Bq-240	Stripping of Top Soil		
U-P3-Bq-276	Stripping of Top Soil		

Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Direct Cost											
Equipment											
	A-2-1-48	Dumptruck; 10 ton	hourly	0.87	77268.996	3060	70744.12	67,224	2,662	61,547	
Working Base Cost											
	CW-1-46	Excavation A	m3	10	2361	39	1711	23,610	390	17,110	
	CW-1-5	Spreading A	m3	10	2941	35	2823	29,410	350	28,230	
Indirect Cost											
		Site Expense	%	15	0.8		0.2	27,664	0	6,916	
Profit and Overhead Cost											
		Miscellaneous	L.S.					83	98	94	Round Up
Total for 10 m3								169,200	3,500	119,200	
Unit Cost for 1 m3								16,920	350	11,920	

*1: Soil Volume : 10 m3
 *2: Dump Truck : 10 ton/dump / 1.5 m3/ton = 6.67 m3/dump
 10 km/trd / 40 km/hr + 20 mins(loss) = 0.58 hours
 10 m3 / 6.67 m3/dump = 1.5 dp/10m3
 1.5 dp/10m3 x 0.58 hours = 0.87 hours

ID No.	Working Name	Calculation Quantity	Remarks
U-P2-Bq-45	Embankment	U-P3-Bq-19	Embankment
U-P2-Bq-127	Embankment	U-P3-Bq-102	Embankment
U-P2-Bq-152	Embankment	U-P3-Bq-241	Embankment
U-P2-Bq-198	Embankment	U-P3-Bq-258	Embankment
U-P2-Bq-302	Embankment	U-P3-Bq-277	Embankment
U-P2-Bq-319	Embankment		

Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Direct Cost											
Material											
	M-B-4	Sand for Filling and Base Course	m3	3.33	0	1350	25650	0	4,496	85,415	
	M-B-2	Coarse Aggregate	m3	1.11	0	2600	49400	0	2,886	54,834	
	M-B-9	Soil for Backfilling	m3	6.67	0	400	7600	0	2,668	50,692	
Working Base Cost											
	CW-1-9	Slope Clearing for Embankment 1	m2	50	2674	35	2902	133,700	1,750	145,100	
	CW-1-55	Spreading and Compaction-A	m3	10	1900.3129	22.6272	1938.609	19,003	226	19,386	
Indirect Cost											
		Site Expense	%	15	0.8		0.2	62,419	0	15,605	
Profit and Overhead Cost											
		Miscellaneous	L.S.					24	74	5	Round Up
Total for 10 m3								263,000	12,100	383,000	
Unit Cost for 1 m3								26,300	1,210	38,300	

*1: Sand : 3 m3 / 0.9 = 3.33 m3
 *2: Aggregate : 1 m3 / 0.9 = 1.11 m3
 *3: Soil : 6 m3 / 0.9 = 6.67 m3
 *4: Slope Clearing Work : 1000 m2 / 200 m3 x 10m3 = 50 m2/m3

Table 4.2.5 (24/119) CALCULATION SHEET FOR UNIT COST OF EACH PAYMENT ITEM OF THREE PACKAGES

ID No. Working Name Calculation Quantity Remarks
 U-P2-Bq-46 Aggregate Class A U-P3-Bq-244 Aggregate Class A 10 m3
 U-P2-Bq-90 Aggregate Class A U-P3-Bq-261 Aggregate Class A
 U-P2-Bq-108 Aggregate Class A U-P3-Bq-280 Aggregate Class A
 U-P2-Bq-320 Aggregate Class A

Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks	
					PF/C	IF/C	L/C	PF/C	IF/C	L/C		
Direct Cost												
Equipment												
	A-2-1-96	Wheel Loader; 1.2 m3	hourly	0.78	72,404	1,116	58,629	56,475	870	45,731		
	A-2-1-48	Dumptruck; 10 ton	hourly	0.54	77268.996	3060	70744.12	41,725	1,652	38,202		
	A-2-1-108	Motorgrader; 2.8 m Vibrating Roller; 0.8-1.1 ton (Hand Guide)	hourly	0.14	104320.17	0	97031.6	14,605	0	13,584		
	A-2-1-88	Guide	hourly	0.12	17057.227	144	14790.56	2,047	17	1,775		
	A-2-1-68	Tire Roller; 8-20 ton	hourly	0.04	81684.162	864	82451.15	3,267	35	3,298		
	A-2-1-104	Water Tanker; 4000 liter	hourly	0.03	46827.594	1076	35137.54	1,405	32	1,054		
Labour												
	L-2-1	Foreman	day	0.11	0	0	48800	0	0	5,368		
	L-2-23	Common Labour	day	0.78	0	0	35100	0	0	27,378		
Material												
	M-B-12	Crushed Stone for Pavement and Concrete	m3	12	0	3250	61750	0	39,000	741,000		
Others												
		Small Tools	%	5				5,976	2,080	43,870		
Indirect Cost												
		Site Expense	%	15	0.8		0.2	130,854	0	32,713		
		Profit and Overhead Cost	%	10	0.8		0.2	100,321	0	25,080		
		Miscellaneous	L.S.					25	13	47	Round Up	
Total for								10 m3		356,700	43,700	979,100
Unit Cost for								1 m3		35,670	4,370	97,910

Analysis of base course aggregate class A 1 M3, 150 mm thick

No	Working Name	Code	Coefficient	Unit	Remark	No	Working Name	Code	Coefficient	Unit
I.	1 Using Equipment					2.3	Motor Grader	E13		
	2 Work location is on the road length						Panjang hamparan Lh			50 m
	3 Existing condition of the road are medium						Effective width of b			2 m
	4 Average distance from base camp to TL			3 Km			Efficiency factor Fa		0.83	
	5 Thickness of base course	t	0.15 m				Average speed v			4 km/hour
	6 Loose factor of the material	Fk	1.2				Number of lintasan n			6 lintasan 3pp
	7 Effective Hour / day	Tk	7 Hour				Cycle time : perataan satu linta T1			0.75 minute
	8 Composition of the material						others T2			1 minute
	-Fine aggregate	Ak	0.64				Ts3			1.75 minute
	-Coarse aggregate	Ah	0.36				Production capaci Q3			71.14285714 m3
							= (Lh*b*Fa*60)/(n*Ts3)			
H.	Material, Equipment and Labor						Equipment coeffit E13			0.014056225 hour
1	Material					2.4	Vibratory Roller	E19		
	-Fine aggregate = (Ak*1 m3 * Fk)	M03	0.768 m3				Average speed v			3 km/hour
	-Coarse aggregate = (Ak*1 m3 * Fk)	M04	0.432 m3				Effective width of b			1.8 m
			1.2 m3				Number of track n			8 lintasan
2	Equipment						Efficiency factor Fa		0.83	
2.1	Wheel Loader	E15					Production capaci Q4			84.0375 m3
	Bucket capacity	V	1.2 m3				= (1000*v*b*Fa)/n			
	Bucket factor	Fb	0.9				Equipment coeffit E19			0.01189945 hour
	Efficiency factor	Fa	0.83			2.5	Pneumatic Tire	E18		
	Cycle time						Average speed v			5 km/hour
	- mixing	T1	2.5 minute				Effective width of b			1.8 m
	-Loading	T2	1 minute				Number of track n			4 lintasan
		Ts1	3.5 minute				Efficiency factor Fa		0.83	
	Production capacity (m3/hour)	Q1	12.80571				Production capaci Q5			280.125 m3
	= (V*Fb*Fa*60)/(Fk*Ts1)						= (1000*v*b*Fa)/n			
	Equipment coefficient/m3 = 1/Q1	E15	0.07809	hour			Equipment coeffit E18			0.003569835 hour
2.2	Dump Truck					2.6	Water Tank Tru	E23		
	Bucket capacity	V	6 m3				Water Tank Volu V			4 m3
	Efficiency factor	Fa	0.83				Water need/m3 of Wc			0.07 m3
	Average loading speed	v1	30 Km/hour				Water tank filling/n			1 times
	Average empty speed	v2	40 Km/hour				Efficiency factor Fa		0.83	
	Efficiency factor	Fa	0.83				Production capaci Q6			47.42857143 m3
	Cycle time :						= (V*n*Fa)/Wc			
	Loading time = (L/v1)*60 minute	T1	6 minute				Equipment coeffit E23			0.021084337 hour
	Empty time = (L/v2)*60 minute	T2	4.5 minute			2.7	Light tools			
	Others	T3	3 minute				Carriage = 3 pieces			
		Ts2	13.5 minute				Shovel = 3 pieces			
	Production capacity (m3/hour)	Q2	18.44444				Barth fork = 2 pieces			
	= (V*Fa*60)/(Fk*Ts2)					2.8	Man Power/ labor			
	Equipment coefficient/m3 = 1/Q1	E15	0.054217				The significant pr Q1			12.80571429 m3/hour
							Aggregat producti Q1			89.64 m3
							Man power			
							-common labour P			7
							-foreman M			1
							Koefisien tenaga/m3 =			
							-common labour = L01			0.546630968 hour
							-foreman = (Tk*ML03			0.078090138 hour

Table 4.2.5 (25/119) CALCULATION SHEET FOR UNIT COST OF EACH PAYMENT ITEM OF THREE PACKAGES

ID No.	Working Name		Calculation Quantity	Remarks
U-P2-Bq-47	Aggregate Class B	U-P3-Bq-245	Aggregate Class B	10 m3
U-P2-Bq-91	Aggregate Class B			
U-P2-Bq-109	Aggregate Class B	U-P3-Bq-281	Aggregate Class B	
U-P2-Bq-321	Aggregate Class B			

Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Direct Cost											
Equipment											
	A-2-1-96	Wheel Loader; 1.2 m3	hourly	0.78	72,404	1,116	58,629	56,475	870	45,731	
	A-2-1-48	Dumptruck; 10 ton	hourly	0.54	77268.996	3060	70744.12	41,725	1,652	38,202	
	A-2-1-108	Motorgrader; 2.8 m Vibrating Roller; 0.8-1.1 ton (Hand Guide)	hourly	0.11	104320.17	0	97031.6	11,475	0	10,673	
	A-2-1-88	Tire Roller; 8-20 ton	hourly	0.09	17057.227	144	14790.56	1,535	13	1,331	
	A-2-1-68	Water Tanker; 4000 liter	hourly	0.03	81684.162	864	82451.15	2,451	26	2,474	
	A-2-1-104		hourly	0.03	46827.594	1076	35137.54	1,405	32	1,054	
Labour											
	L-2-1	Foreman	day	0.11	0	0	48800	0	0	5,368	
	L-2-23	Common Labour	day	0.78	0	0	35100	0	0	27,378	
Material											
	M-B-12	Crushed Stone for Pavement and Concrete	m3	12	0	3250	61750	0	39,000	741,000	
Others											
		Small Tools	%	5				5,753	2,080	43,661	
Indirect Cost											
Site Expense											
			%	15	0.8		0.2	129,764	0	32,441	
Profit and Overhead Cost											
		Miscellaneous	L.S.	10	0.8		0.2	99,486	0	24,871	
								31	26	16	Round Up
Total for	10 m3							350,100	43,700	974,200	
Unit Cost for	1 m3							35,010	4,370	97,420	

Analysis of sub base course aggregate class B 1 M3 200 mm thick

No	Working Name	Code	Coeffitien	Unit	Remark	No	Working Name	Code	Coeffitien	Unit
I.						2.3	Motor Grader	E13		
	1 Using Equipment						Panjang hamparan Lh		50	m
	2 Work location is on the road length						Effective width of b		2	m
	3 Existing condition of the road are medium						Efficiency factor Fa		0.83	
	4 Average distance from base camp to tL			3 Km			Average speed v		4	km/hour
	5 Thickness of sub base course t			0.2 m			Number of lintasan n		6	lintasan 3pp
	6 Loose factor of the material Fk			1.2			Cycle time :			
	7 Effective Hour / day Tk			7 Hour			perataan satu linta T1		0.75	minute
	8 Composition of the material						others T2		1	minute
	-Fine aggregate Ak			0.59			Ts3		1.75	minute
	-Coarse aggregate Ah			0.41			Production capaci Q3		94.85714286	m3
II.	Material, Equipment and Labor						= (Lh*b*Fa*60)/(n*Ts3)			
	1 Material						Equipment coeffit E13		<u>0.010542169</u>	hour
	-Fine aggregate = (Ak*1 m3 * Fk) M03			<u>0.708</u> m3		2.4	Vibratory Roller	E19		
	-Coarse aggregate = (Ak*1 m3 * Fk) M04			<u>0.492</u> m3			Average speed v		3	km/hour
				<u>1.2</u> m3			Effective width of b		1.8	m
	2 Equipment						Number of track n		8	lintasan
	2.1 Wheel Loader	E15					Efficiency factor Fa		0.83	
	Bucket capacity V			1.2 m3			Production capaci Q4		112.05	m3
	Bucket factor Fb			0.9			= (1000v*b*t*Fa)/n			
	Efficiency factor Fa			0.83			Equipment coeffit E19		<u>0.008924587</u>	hour
	Cycle time					2.5	Pneumatic Tire	E18		
	- mixing T1			2.5 minute			Average speed v		5	km/hour
	- Loading T2			1 minute			Effective width of b		1.8	m
				3.5 minute			Number of track n		4	lintasan
	Production capacity (m3/hour) Q1			12.80571			Efficiency factor Fa		0.75	
	= (V*Fb*Fa*60)/(Fk*Ts1)						Production capaci Q5		337.5	m3
	Equipment coeffitien/m3 = 1/Q1	E15		<u>0.07809</u>			= (1000v*b*t*Fa)/n			
	2.2 Dump Truck						Equipment coeffit E18		<u>0.002962963</u>	hour
	Bucket capacity V			6 m3		2.6	Water Tank Tru	E23		
	Efficiency factor Fa			0.83			Water Tank Volu V		4	m3
	Average loading speed v1			30 Km/hour			Water need/m3 of Wc		0.07	m3
	Average empty speed v2			40 Km/hour			Water tank filling n		1	times
	Efficiency factor Fa			0.83			Efficiency factor Fa		0.83	
	Cycle time :						Production capaci Q6		47.42857143	m3
	Loading time = (L/v1)*60 minute T1			6 minute			= (V*n*Fa)/Wc			
	Empty time = (L/v2)*60 minute T2			4.5 minute			Equipment coeffit E23		<u>0.021084337</u>	hour
	Others T3			3 minute			2.7 Light tools			
				13.5 minute			Carriage = 3 pieces			
	Production capacity (m3/hour) Q2			18.44444			Shovel = 3 pieces			
	= (V*Fa*60)/(Fk*Ts2)						Earth fork = 2 pieces			
	Equipment coeffitien/m3 = 1/Q1	E15		<u>0.054217</u>			2.8 Man Power/ labor			
							The significant pr:Q1		12.80571429	m3/hour
							Aggregat productiQt		89.64	m3
							Man power			
							-common labour P		7	
							-foreman M		1	
							Koefisien tenaga/m3 =			
							-common labour = L01		<u>0.546630968</u>	hour
							-foreman = (Tk*M.L03		<u>0.078090138</u>	hour

Table 4.2.5 (26/119) CALCULATION SHEET FOR UNIT COST OF EACH PAYMENT ITEM OF THREE PACKAGES

U-P2-Bq-48 Sand Bedding is equal unit cost with U-P1-Bq-33.1.
 U-P2-Bq-49 Concrete Block Pavement is equal unit cost with U-P1-Bq-33.2.
 U-P2-Bq-50 Cement Mortar is equal unit cost with U-P1-Bq-33.3.
 U-P2-Bq-51 Concrete Kerb is equal unit cost with U-P1-Bq-33.4.

ID No. Working Name Calculation Quantity Remarks
 U-P2-Bq-52 Sodding U-P3-Bq-207 Sodding 10 m2
 #REF! #REF! U-P3-Bq-245.5 Sodding

Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Direct Cost											
Working Base Cost											
	CW-1-14	Sodding	m2	10	0	0	5761	0	0	57,610	
Indirect Cost											
Site Expense			%	15	0.8		0.2	6,913	0	1,728	
Profit and Overhead Cost			%	10	0.8		0.2	5,300	0	1,325	
		Miscellaneous	L.S.					87	0	37	Round Up
Total for		10 m2						12,300	0	60,700	
Unit Cost for		1 m2						1,230	0	6,070	

Table 4.2.5 (27/119) CALCULATION SHEET FOR UNIT COST OF EACH PAYMENT ITEM OF THREE PACKAGES

ID No.		Working Name		Calculation Quantity		Remarks		1 L.S.			
Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Direct Cost											
Material											
	M-B-9	Soil for Backfilling	m ³	550.4	0	400	7600	0	220,160	4,183,040	for River Improvement
	M-E-4	Structural Steel(Purchasing), SS41	kg	862.4	5225	0	275	4,506,040	0	237,160	for River Improvement
	M-E-13	Tierod (Purchasing)	kg	900	47500	0	2500	42,750,000	0	2,250,000	for River Improvement
	M-E-78	C-beam (Purchasing), SS41	kg	4848	5225	0	275	25,330,800	0	1,333,200	for River Improvement
	M-E-31	Steel Sheet Pile (Purchasing)	ton	15.36	5700000	0	300000	87,552,000	0	4,608,000	for River Improvement
Working Base Cost											
	CW-4-17	Temporary Double Steel Sheet Pile for Drainage Component	m	80	936106.3	10718.99	818075.7	74,888,506	857,519	65,446,054	for River Improvement
	CW-4-21	Temporary Dewatering by D200mm	day	1080	353884.4	15124	251251.1	382,195,197	16,333,920	271,351,193	for River Improvement
	CW-4-22	Temporary Dewatering by D180mm	day	180	339695.5	15124	244084.3	61,145,198	2,722,320	43,935,167	for River Improvement
	CW-4-23	Temporary Dewatering by D160mm	day	180	319831.1	15124	234050.7	57,569,595	2,722,320	42,129,123	for River Improvement
	CW-4-7	Sand Bags	nos	90	88.5375	758.9625	4199.663	7,968	68,307	377,970	
	CW-3-9	Driving In of Steel Sheet Pile (Type-II)	m	1025	9909.067	76.17755	8578.669	10,156,793	78,082	8,793,136	for Relocation
	CW-3-10	Pulling Out of Steel Sheet Pile (Type-II)	m	1025	9754	67	8548	9,997,850	68,675	8,761,700	for Relocation
	CW-4-23	Temporary Dewatering by D160mm	day	210	319831.1	15124	234050.7	67,164,527	3,176,040	49,150,644	
Indirect Cost											
Site Expense											
			%	15	0.8		0.2	162,248,184	0	40,562,046	
Profit and Overhead Cost											
		Miscellaneous	L.S.	10	0.8		0.2	124,390,275	0	31,097,569	
								67	57	99	
Total for								1 L.S.			
Unit Cost for								1 L.S.			
								1,109,903,000	26,247,400	574,216,100	
								1,109,903,000	26,247,400	574,216,100	

Asin River Improvement

- * 1 : Total Length of Double Steel Sheet Pile : 80 m from Construction Planning
- * 2 : Purchasing Soil : 13.76 m³/m x 40 m = 550.4 (reuse ofr 20m of 2nd year and 20m of 3rd year)
- * 3 : Purchasing Structural Steel : 21.56 kg/m x 40 m = 862.4
- * 4 : Purchasing Tierod : 22.5 kg/m x 40 m = 900
- * 5 : Purchasing C-beam : 121.2 kg/m x 40 m = 4848
- * 6 : Purchasing Steel Sheet Pile : 48 kg/m x 20 m x 2 side x 8 m long = 15.36 ton
- * 7 : Dewatering
 - D200mm 18 months x 30 days x 2 set = 1080
 - D180mm 18 months x 5 days x 2 set = 180
 - D160mm 18 months x 10 days x 1 set = 180

Bridge Work

- * 8 : Total Number of Sand Bags : 90 nos. from Construction Planning
- * 9 : Total Length of Driving (L1) : 5 m long x 205 pieces = 1025 m
- * 10 : Purchasing Steel Sheet Pile (L2) : 0 ton (No purchasing)
- * 11 : Dewatering D160mm 7 months x 30 days x 1 set = 210

ID No.		Working Name		Calculation Quantity		Remarks		1 L.S.			
Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Direct Cost											
Equipment											
	A-2-1-54	Dumptruck; 4 ton	hourly	0	30203.66	1376	28631.77	0	0	0	
	A-2-1-7	Backhos; 0.6 m ³	hourly	7	125542.9	2040	90965.08	878,800	14,280	636,756	
Labour											
	L-2-1	Foreman	day	1	0	0	48800	0	0	48,800	
	L-2-23	Common Labour	day	10	0	0	35100	0	0	351,000	
Material											
		Plastic Bag	sheet	100	0	0	1000	0	0	100,000	
Indirect Cost											
Site Expense											
			%	15	0.8		0.2	243,556	0	60,889	
Profit and Overhead Cost											
		Miscellaneous	L.S.	10	0.8		0.2	186,727	0	46,682	
								17	20	74	Round Up
Total for								1 L.S.			
Unit Cost for								1 L.S.			
								1,309,100	14,300	1,244,200	
								1,309,100	14,300	1,244,200	

Table 4.2.5 (28/119) CALCULATION SHEET FOR UNIT COST OF EACH PAYMENT ITEM OF THREE PACKAGES

ID No.	Working Name	Calculation Quantity	Remarks									
U-P2-Bq-57	Demolition of Existing Revetment	I L.S.										
Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks	
					PF/C	IF/C	I/C	PF/C	IF/C	I/C		
Direct Cost												
Equipment												
	A-2-1-7	Backhoe; 0.6 m3	hourly	325.00	125,543	2,040	90,965	40,801,445	663,000	29,563,650		
	A-2-2-35	Pick Hammer	daily	565.22	5716.911	0	2030.087	3,231,297	0	1,147,440		
	A-2-1-48	Dumptruck; 10 ton	hourly	763.75	77269	3060	70744.12	59,014,196	2,337,075	54,030,822		
	A-2-2-17	Generator; 15 kVA	daily	141.30	82875	1800	52496.05	11,710,238	254,340	7,417,692		
Labour												
	L-2-1	Foreman	day	141.30	0	0	48800	0	0	6,895,652		
	L-2-10	Drill Worker	day	565.22	0	0	39000	0	0	22,043,478		
	L-2-23	Common Labour	day	565.22	0	0	35100	0	0	19,839,130		
Working Base Cost												
	CW-1-54	Excavation I	m3	100	5072	83	3675	507,200	8,300	367,500		
Indirect Cost												
		Site Expense	%	15	0.8		0.2	31,179,895	0	7,794,974		
		Profit and Overhead Cost	%	10	0.8		0.2	20,786,596	0	5,196,649		
		Miscellaneous	L.S.					133	285	12	Round Up	
Total for								1 L.S.				
Unit Cost for								1 L.S.				
								167,231,000	3,263,000	154,297,000		
								167,231,000	3,263,000	154,297,000		

Manpower Composition; Foreman : man/day
 Common Labor : man/day
 Drill Worker : man/day

* 1 : Length of Revetment Demolished L = 1200 m x 2 sides = 2400 m
 * 2 : Area of Assumed of Revetment A1 = 1.625 m Hence, Volume = 3900 m3

* 3	0.5m3 x	$\frac{T_a}{T \times 60}$	x Composition of Manpower	=	Foreman	Drill Worker	Common
* 4	1m3 x	$\frac{T_b}{60}$	= Dump Truck		141.30	565.22	565.22
* 5	0.5m3 x	$\frac{T_c}{T \times 60}$	= Generator				
* 6	0.5m3 x	$\frac{T_c}{60}$	= Backhoe				

* 7 : Average Daily Working Time of Generator, Labor, Breaker
 $T = \frac{690}{100} = 6.9$ (hour/day)

Working Time by Hand Breaker / 1m3 (Ta) = 30 minutes/m3

Working Time by Dump Truck / 1m3 (Tb) = $\left(\frac{5 \text{ km(one way)} \times 2}{20 \text{ minutes / } \frac{30}{10} \text{ km/hour}} \right) = 11.75 \text{ minutes/m3}$

Working Time by Backhoe and Pick Hammer / 1m3 (Tc) = 10 minutes/m3

Table 4.2.5 (29/119) CALCULATION SHEET FOR UNIT COST OF EACH PAYMENT ITEM OF THREE PACKAGES

ID No.	Working Name	Calculation Quantity	Remarks								
U-P2-Bq-59	Common channel excavation including hauling and treatment of contaminated soil	100 m3									
Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Direct Cost											
Equipment											
	A-2-1-48	Dumptruck; 10 ton	hourly	21.89	77269	3060	70744.12	1,691,418	66,983	1,548,589	from Hauling
	A-2-2-2	Stabilizer	hourly	0.683761	743275.5	1092	502607.8	508,223	747	343,663	for Mixing
Labour											
	L-2-1	Foreman	day	0.008686	0	0	48800	0	0	424	for water proof sheet
	L-2-23	Common Labour	day	0.028952	0	0	35100	0	0	1,016	for water proof sheet
	L-2-23	Common Labour	day	6.666667	0	0	35100	0	0	234,000	for Spreading of Concrete
Material											
		Drain Pipe	m	0.173712	108000	0	12000	18,761	0	2,085	for Hauling
		Water Proof Sheet for Disposal Site	m2	47.57788	27000	0	3000	1,284,603	0	142,734	for Disposal Site
		Water Proof Sheet for Dump Truck	sheet	0.347423	0	0	50000	0	0	17,371	for Hauling
	M-B-3	Sand for Mortar (Masonry)	m3	21.74986	0	2250	42750	0	48,937	929,806	for Backfill of Drain
	M-C-1	Portland Cement	kg	7000	0	100	400	0	700,000	2,800,000	for Backfill of Drain
	M-B-5	Cobble Stone	m3	0.496526	0	1850	35150	0	919	17,453	for Backfill of Drain
	M-B-13	Solid Soil	m3	33.42373	0	600	11400	0	20,054	381,030	for Banking
Working Base Cost											
	CW-1-64	Excavation by Backhoe 0.35m3	m3	0.496526	2687.773	45.24	1953.753	1,335	22	970	for Excavation
	CW-1-64	Excavation by Backhoe 0.35m3	m3	0.551695	2687.773	45.24	1953.753	1,483	25	1,078	for Backfill
	CW-1-65	Spreading by Swamp Bulldozer	m3	108	4284.32	54.264	4047.348	462,707	5,861	437,114	for Spreading
	CW-1-5	Spreading A	m3	80.74117	2941	35	2823	237,460	2,826	227,932	for Disposal Site
		Slope Clearing for									
	CW-1-10	Embankment 2	m2	3.420382	3265	54	2660	11,168	185	9,098	for Disposal Site
	CW-1-5	Spreading A	m3	20.26636	2941	35	2823	59,603	709	57,212	for Final Spreading
	CW-1-1	Backfill (Soil) A	m3	30.57788	6076	87	5043	185,791	2,660	154,204	for Banking
		Spreading and Compaction for									
	CW-1-58	Earth Filling	m3	30.57788	2833.803	36.252	2632.618	86,652	1,109	80,500	for Banking
	CW-1-48	Excavation C	m3	100	3943	65	2837	394,300	6,500	285,700	for Dredging
	CW-1-47	Excavation B	m3	1.022583	2951	48	2138	3,018	49	2,186	for Pit
		Concrete Work for Small									
	CW-1-21	Structure : Type-D	m3	0.348871	120	42570	193500	42	14,851	67,507	for Pit
	CW-1-23	Form Work A	m2	0.318471	60	0	44798	19	0	14,267	for Pit
	CW-1-29	Reinforcing Bar Setup 1	t	0.006977	0	3120900	3325100	0	21,776	23,201	for Pit
	CW-1-15	Gravel Bedding	m3	0.056167	0	1360	31260	0	76	1,756	for Pit
	CW-1-2	Backfill (Soil) B	m3	0.132021	7022	103	6326	927	14	835	for Pit
Indirect Cost											
Site Expense											
			%	15	0.8		0.2	1,634,825	0	408,706	
Profit and Overhead Cost											
		Miscellaneous	L.S.	10	0.8		0.2	1,253,366	0	313,341	
								1	97	22	
Total for 100 m3											
								7,835,700	894,400	8,503,800	
Unit Cost for 1 m3											
								78,357	8,944	85,038	

Table 4.2.5 (30/119) CALCULATION SHEET FOR UNIT COST OF EACH PAYMENT ITEM OF THREE PACKAGES

* 1:	Dump Truck for Hauling:	107 m3	/	6.5 m3/truck	=	16.46 Truck	
		20 km/rnd /		20 km/hr +	20 mnts(loss)=	1.33 hours	
		16.46 Truck	x	1.33 hours	=	21.89 hours	
	Water Proof Sheet	10 m2/truckx	20	trucks x	3 changes =	600	
	Unit Quantity	600 m2 /	172700	=	0.003474	m3	
* 2:	Excavation/Backfill by Backhoe	0.35m3 for construction of drain					
	Dimension of Drain	0.5 m wide x	0.5	m depthx(490	x	350)m length
		x	2	/	100 m pitch	=	857.5 m3
	Unit excavation/Backfilling Volume	857.5 m3 /	172700	=	0.004965	m3	
	Cobble Stone	0.004965 /	0.9	loss =	0.005517		
* 3:	Spreading by Swamp Bull	100 m3 x	1.2 (L) x	0.9 (C) =		108 m3	
* 4:	Banking	52808 m3	from Construction Planning				
	Unit Volume	52808 /	172700	=	0.305779	m3	
	Soil	(52808	-	857.5)/	0.9 loss =	57722.77778 m3	
	Unit Volume	57722.78 /	172700	=	0.334237	m3	
* 5:	Labor Rate and Cement:	Cement : 70 kg/m3 is necessary					
	Labor :	2 person/party for Spreading at 50kg (1bag) of cement					
		It takes	10 minutes.				
	Hence,	70 kg/m3 /	50 kg/bag x	2 person/party			
	x	10 minutes/	60 minutes/hour /	7 hours/day			
	=	0.066667 person/m3					
* 6:	Mixing	Q = $\frac{60 \times q \times E}{Cm}$ (m3/hour)	Cm = (0.027	x	30 m +		
		q =	3 m wide x	0.2 m depth x	30 m long =	4.8 minutes	
		Q =	146.25 m3/hour	Hence, Tm =	0.006837607 hour/m3	18 m3/aim	
* 7:	Final Spreading	500 m x	350 m x	0.2 m depth =	35000 m3		
		35000 m3 /	172700	=	0.202664	m3	
* 8:	Water Proof Sheet for Disposal Site:	82167 m2 /	172700 m3 =	0.475778807			
	including Drain Pipe and	Foreman :	3 persons x	5 days /	172700	m3 =	8.68558E-05
	Coarse Sand	Common Labor :	10 persons x	5 days /	172700	m3 =	0.000289519
* 9:	Spreading for Disposal Site :	139440 m2	from Construction Planning /	172700	m3 =	0.807411697	
* 10:	Slope Clearing :	5907 m2	from Construction Planning /	172700	m3 =	0.034203822	
* 11:	Drain Pipe :	300 m	from Construction Planning /	172700	m3 =	0.001737116	
* 12:	Coarse Sand :	37562 m3 (from Construction Planning) /	172700	m3 =	0.217498552		
Reservoir Pit							
* 13:	Excavation :	1766 m3 (from Construction Planning) /	172700	m3 =	0.010225825		
* 14:	Concrete :	602.5 m3 (from Construction Planning) /	172700	m3 =	0.003488709		
* 15:	Formwork :	550 m2 (from Construction Planning) /	172700	m3 =	0.003184713		
* 16:	Reinforcing Bar :	12050 kg (from Construction Planning) /	172700	m3 =	0.069774175		
* 17:	Backfilling Gravel :	97 m3 (from Construction Planning) /	172700	m3 =	0.000561668		
* 18:	Backfilling :	228 m3 (from Construction Planning) /	172700	m3 =	0.001320208		

Table 4.2.5 (31/119) CALCULATION SHEET FOR UNIT COST OF EACH PAYMENT ITEM OF THREE PACKAGES

U-P2-Bq-61 Structural Excavation is equal unit cost with U-P1-Bq-16.
 U-P2-Bq-62 Backfill with Cobble is equal unit cost with U-P2-Bq-21.
 U-P2-Bq-63 Backfill with Gravel is equal unit cost with U-P2-Bq-22.

ID No. Working Name Calculation Quantity Remarks
 U-P2-Bq-64 Concrete, Type C1 including Formwork 10 m3

Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Direct Cost											
Equipment	A-2-1-79	Truck Mixer; 4.5 m3	hourly	3.02	77957.88	1560	60651.61	235,433	4,711	183,168	
Working Base Cost											
	CW-1-23	Form Work A	m2	52.42	60	0	44798	3,145	0	2,348,247	
	CW-2-40	Breaking-up the Concrete Form Concrete Work for Reinforced	m2	52.42	0	0	3700	0	0	193,949	
	CW-1-20	Concrete C1 by Pump	m3	10	20270	41770	183330	202,700	417,700	1,833,300	
Indirect Cost											
Site Expense			%	15	0.8		0.2	650,682	0	162,671	
Profit and Overhead Cost			%	10	0.8		0.2	498,856	0	124,714	
		Miscellaneous	L.S.					83	89	52	Round Up
Total for		10 m3						1,590,900	422,500	4,846,100	
Unit Cost for		1 m3						159,090	42,250	484,610	

*1 : Total Concrete Volume : 1013 m3
 *2 : Total Formwork Area : 5310 m2
 Average Formwork Area : 52.42 m2/unit m3
 *3 : Truck Mixer : 10.2 m3 / 4.5 m3/truck = 2.27 Truck
 10 km/rnd / 30 km/hr + 60 mnts(loss) = 1.33 hours
 2.27 Truck x 1.33 hours = 3.02 hours

ID No. Working Name Calculation Quantity Remarks
 U-P2-Bq-65 Concrete, Type E including Formwork 10 m3

Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Direct Cost											
Equipment	A-2-1-79	Truck Mixer; 4.5 m3	hourly	3.02	77957.88	1560	60651.61	235,433	4,711	183,168	
Working Base Cost											
	CW-1-28	Form Work F	m2	29.96528	0	0	36510	0	0	1,094,032	
	CW-1-22	Concrete Work for Levelling Concrete	m3	10	120	37130	158740	1,200	371,300	1,587,400	
Indirect Cost											
Site Expense			%	15	0.8		0.2	417,269	0	104,317	
Profit and Overhead Cost			%	10	0.8		0.2	319,906	0	79,977	
		Miscellaneous	L.S.					91	89	6	Round Up
Total for		10 m3						973,900	376,100	3,048,900	
Unit Cost for		1 m3						97,390	37,610	304,890	

*1 : Form Work Area : Total Form work area 863 / Concrete Volume 288 = Form Work Area m2 / concrete 1m3 = 2.996528 m2/m3
 *2 : Dump Truck : 10.2 m3 / 4.5 m3/truck = 2.27 Truck
 10 km/rnd / 30 km/hr + 60 mnts(loss) = 1.33 hours
 2.27 Truck x 1.33 hours = 3.02 hours

U-P2-Bq-66 Deformed Reinforcing Bars
 U-P2-Bq-67 Wet Stone Masonry is equal unit cost with U-P1-Bq-20.
 U-P2-Bq-68 Weep Hole, Dia.50mm is equal unit cost with U-P2-Bq-29.

Table 4.2.5 (32/119) CALCULATION SHEET FOR UNIT COST OF EACH PAYMENT ITEM OF THREE PACKAGES

ID No.		Working Name		Calculation Quantity			Remarks				
U-P2-Bq-69		Log Pile, Dia. 150mm, L=4m		12 m							
U-P2-Bq-80		Log Pile, Dia. 150mm, L=4m									
Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Direct Cost											
Material											
	M-D-1	Log Pile, Dia. 15cm	m	12	0	0	10000	0	0	120,000	
Working Base Cost											
	CW-3-24	Driving In of Log Pile L=4.0m	piece	3	33998.99	504.1296	25782.13	101,997	1,512	77,346	
Indirect Cost											
Site Expense			%	15	0.8		0.2	36,103	0	9,026	
Profit and Overhead Cost			%	10	0.8		0.2	27,679	0	6,920	
Miscellaneous			L.S.					22	88	8	Round Up
Total for		12 m						165,800	1,600	213,300	
Unit Cost for		1 m						13,817	133	17,775	

- U-P2-Bq-70 Joint Filler, 10mm thick (Elastic Material) is equal unit cost with U-P2-Bq-21.
- U-P2-Bq-72 Structural Excavation is equal unit cost with U-P1-Bq-16.
- U-P2-Bq-73 Backfill with Cobble is equal unit cost with U-P2-Bq-21.
- U-P2-Bq-74 Backfill with Gravel is equal unit cost with U-P2-Bq-22.
- U-P2-Bq-75 Backfill with Sandy Soil is equal unit cost with U-P2-Bq-23.

ID No.		Working Name		Calculation Quantity			Remarks				
U-P2-Bq-76		Concrete, Type E including Formwork		10 m3							
Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Direct Cost											
Equipment											
	A-2-1-79	Truck Mixer; 4.5 m3	hourly	3.02	77957.88	1560	60651.61	235,433	4,711	183,168	
Working Base Cost											
	CW-1-28	Form Work F	m2	6.728972	0	0	36510	0	0	245,675	
	CW-1-22	Concrete Work for Levelling Concrete	m3	10	120	37130	158740	1,200	371,300	1,587,400	
Indirect Cost											
Site Expense			%	15	0.8		0.2	315,466	0	78,867	
Profit and Overhead Cost			%	10	0.8		0.2	241,858	0	60,464	
Miscellaneous			L.S.					43	89	26	Round Up
Total for		10 m3						794,000	376,100	2,155,600	
Unit Cost for		1 m3						79,400	37,610	215,560	

*1 : Form Work Area : Total Form work area / Concrete Volume = Form Work Area m2 / concrete 1m3
 432 / 642 = 0.672897 m2/m3

*2 : Dump Truck : 10.2 m3 / 4.5 m3/truck = 2.27 Truck
 10 km/rnd / 30 km/hr + 60 mnts(loss) = 1.33 hours
 2.27 Truck x 1.33 hours = 3.02 hours

- U-P2-Bq-77 Wet Stone Masonry is equal unit cost with U-P1-Bq-20.
- U-P2-Bq-78 Pointing is equal unit cost with U-P1-Bq-23.
- U-P2-Bq-79 Weep Hole, Dia.50mm is equal unit cost with U-P2-Bq-29.
- U-P2-Bq-82 Furnishing and Driving PC Sheet Pile (t=220 mm) is equal unit cost with U-P2-Bq-33.

Table 4.2.5 (33/119) CALCULATION SHEET FOR UNIT COST OF EACH PAYMENT ITEM OF THREE PACKAGES

ID No.	Working Name	Calculation Quantity	Remarks								
U-P2-Bq-83	Concrete, Type C1 including Formwork	10 m ³									
Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Direct Cost											
Equipment	A-2-1-79	Truck Mixer; 4.5 m ³	hourly	3.17	77957.88	1560	60651.61	247,126	4,945	192,266	
Working Base Cost											
	CW-1-23	Form Work A	m ²	71.43	60	0	44798	4,286	0	3,199,857	
	CW-2-40	Breaking-up the Concrete Form Concrete Work for Type-C by Shoot Hopper	m ²	71.43	0	0	3700	0	0	264,286	
	CW-1-60		m ³	10	120	43660	197860	1,200	436,600	1,978,600	
Indirect Cost											
Site Expense			%	15	0.8		0.2	759,500	0	189,875	
Profit and Overhead Cost			%	10	0.8		0.2	582,283	0	145,571	
		Miscellaneous	L.S.					5	55	46	Round Up
Total for 10 m³											
								1,594,400	441,600	5,970,500	
Unit Cost for 1 m³											
								159,440	44,160	597,050	

*1 : Total Concrete Volume : 14 m³
 *2 : Total Formwork Area : 100 m²
 Average Formwork Area : 71.43 m²/unit m³
 *3 : Truck Mixer : 10.7 m³ / 4.5 m³/truck = 2.38 Truck
 10 km/trnd / 30 km/hr + 60 mnts(loss)= 1.33 hours
 2.38 Truck x 1.33 hours = 3.17 hours

- U-P2-Bq-84 Deformed Reinforcing Bars is equal unit cost with U-P2-Bq-35.
- U-P2-Bq-86 Sand Bedding is equal unit cost with U-P1-Bq-19.
- U-P2-Bq-87 Concete Block Pavement
- U-P2-Bq-88 Cement Mortar
- U-P2-Bq-89 Concrete Kerb is equal unit cost with U-P1-Bq-33.4.
- U-P2-Bq-90 Aggregate Class A is equal unit cost with U-P1-Bq-32.
- U-P2-Bq-91 Aggregate Class B is equal unit cost with U-P1-Bq-33.

ID No.	Working Name	Calculation Quantity	Remarks								
U-P2-Bq-93	Coffering and Dewatering	1 L.S.									
Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Direct Cost											
Material	M-E-31	Steel Sheet Pile (Purchasing)	ton	6.912	5700000	0	300000	39,398,400	0	2,073,600	
Working Base Cost											
	CW-4-7	Sand Bags	nos	462	88.5375	758.9625	4199.663	40,904	350,641	1,940,244	
	CW-3-9	Driving In of Steel Sheet Pile (Type-II)	m	90	9909.067	76.17755	8578.669	891,816	6,856	772,080	
	CW-3-10	Pulling Out of Steel Sheet Pile (Type-II)	m	90	9754	67	8548	877,860	6,030	769,320	
	CW-4-23	Temporary Dewatering by D160mm	day	50	319831.1	15124	234050.7	15,991,554	756,200	11,702,534	
Indirect Cost											
Site Expense			%	15	0.8		0.2	9,069,365	0	2,267,341	
Profit and Overhead Cost			%	10	0.8		0.2	6,953,180	0	1,738,295	
		Miscellaneous	L.S.					21	73	85	
Total for 1 L.S.											
								73,223,100	1,119,800	21,263,500	
Unit Cost for 1 L.S.											
								73,223,100	1,119,800	21,263,500	

Sand bag
 * 1 : Total number of Sand Bags : 462 nos. from Construction Planning
 Steel Sheet Pile
 * 2 : Total Length of Driving (L1) : 5 m long x 18 pieces = 90 m
 * 3 : Purchasing Steel Sheet Pile (L2) : 8 m long x 18 pieces x 48 kg/m = 6.912 ton
 Dewatering for Relocation of Semarang River
 * 4 : D160mm : 5 months x 10 days/month = 50 days

Table 4.2.5 (34/119) CALCULATION SHEET FOR UNIT COST OF EACH PAYMENT ITEM OF THREE PACKAGES

ID No.	Working Name		Calculation Quantity							Remarks		
U-P2-Bq-94	Structural Excavation with Shoring		3 m									
Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks	
					PF/C	IF/C	L/C	PF/C	IF/C	L/C		
Direct Cost												
Material												
	M-E-9	H-beam (Purchasing), SS41	kg	297	5225	0	275	1,551,825	0	81,675		
	M-E-31	Steel Sheet Pile (Purchasing)	ton	1.08	5700000	0	300000	6,156,000	0	324,000		
Equipment												
	A-2-1-48	Dumptruck; 10 ton	hourly	3.36	77269	3060	70744.12	259,624	10,282	237,700		
Working Base Cost												
	CW-1-5	Spreading A	m3	27	2941	35	2823	79,407	945	76,221	for Reclamation Site	
	CW-1-46	Excavation A	m3	33.75	2361	39	1711	79,684	1,316	57,746		
	CW-1-47	Excavation B	m3	33.75	2951	48	2138	99,596	1,620	72,158		
	CW-1-6	Manpower Excavation	m3	1	0	0	15800	0	0	15,800		
	CW-3-17	Wale Work-B (Temporary)	ton	1.485	558950	3200	581050	830,041	4,752	862,859		
	CW-3-9	Driving In of Steel Sheet Pile (Type-II)	m	45	9909.067	76.17755	8578.669	445,908	3,428	386,040		
	CW-3-10	Pulling Out of Steel Sheet Pile (Type-II)	m	45	9754	67	8548	438,930	3,015	384,660		
Indirect Cost												
	Site Expense		%	15	0.8		0.2	1,495,828	0	373,957		
	Profit and Overhead Cost		%	10	0.8		0.2	1,146,801	0	286,700		
	Miscellaneous		L.S.					56	42	83	Round Up	
Total for								12,583,700	25,400	3,159,600		
Unit Cost for								1 m	4,194,567	8,467	1,053,200	
Unit Cost for								1 m3	186,425	376	46,809	

- * 1 : Calculation Length : 3 m
- * 2 : Excavation Volume : 5 m width x 4.5 m high x 3 m long = 67.5 m3
- * 3 : Backfill Volume : 67.5 m3 - 9 m3 x 3 m long = 40.5 m3
- * 4 : Soil Waste Volume : 67.5 m3 - 40.5 m3 = 27 m3
- * 5 : Dump Truck : 10 ton/dump / 1.5 m3/ton = 6.67 m3/dump
15 km/rnd / 30 km/hr + 20 mnts(loss) = 0.83 hours
27 m3 / 6.67 m3/dump = 4.05 dp/10m3
4.05 dp/10m3 x 0.83 hours = 3.36 hours
- * 6 : Spreading : 27 m3
- * 7 : Steel Sheet Pile : 48 kg/m x 15 m long x 3 m distnc / 0.4 m wide = 5400 kg
48 kg/m x 45 m in total = 5.4 ton / 5 times = 1.08 ton
- * 8 : Steel in General : 11 m length in total x 135 kg/m3 = 1485 kg
1485 kg / 5 times = 297 kg

U-P2-Bq-95 Backfill with Sandy Soil is equal unit cost with U-P2-Bq-23.

ID No.	Working Name		Calculation Quantity							Remarks		
U-P2-Bq-96	Concrete, Type C1 including Formwork, Scaffolding, and Falsework		10 m3									
Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks	
					PF/C	IF/C	L/C	PF/C	IF/C	L/C		
Direct Cost												
Equipment												
	A-2-1-79	Truck Mixer; 4.5 m3	hourly	3.02	77957.88	1560	60651.61	235,433	4,711	183,168		
Working Base Cost												
	CW-1-24	Form Work B	m2	29.19	10030	75	52910	292,787	2,189	1,544,501		
	CW-2-40	Breaking-up the Concrete Form	m2	29.19	0	0	3700	0	0	108,007		
	CW-1-42	Tublar Scaffolding for Re-Con IV	m2	19.80789	24970	70	23610	494,603	1,387	467,664		
	CW-1-44	Frame Support	m3	14.00404	11370	50	22310	159,226	700	312,430		
	CW-1-20	Concrete Work for Reinforced Concrete C1 by Pump	m3	10	20270	41770	183330	202,700	417,700	1,833,300		
Indirect Cost												
	Site Expense		%	15	0.8		0.2	751,261	0	187,815		
	Profit and Overhead Cost		%	10	0.8		0.2	575,967	0	143,992		
	Miscellaneous		L.S.					24	13	23	Round Up	
Total for								2,712,000	426,700	4,780,900		
Unit Cost for								1 m3	271,200	42,670	478,090	

- * 1 : Total Concrete Volume : 989 m3
- * 2 : Total Scaffolding Area : 1959 m2
- Average Scaffolding Area : 19.80789 m2/unit m3
- Total Formwork Area : 2887 m2
- Average Formwork Area : 29.19110 m2/unit m3
- * 3 : Truck Mixer : 10.2 m3 / 4.5 m3/truck = 2.27 Truck
10 km/rnd / 30 km/hr + 60 mnts(loss) = 1.33 hours
2.27 Truck x 1.33 hours = 3.02 hours

Table 4.2.5 (35/119) CALCULATION SHEET FOR UNIT COST OF EACH PAYMENT ITEM OF THREE PACKAGES

ID No.	Working Name		Calculation Quantity							Remarks	
U-P2-Bq-97	Concrete, Type E including Formwork		10 m ³								
Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Direct Cost											
Equipment	A-2-1-79	Truck Mixer; 4.5 m ³	hourly	3.02	77957.88	1560	60651.61	235,433	4,711	183,168	
Working Base Cost											
	CW-1-28	Form Work F Concrete Work for Levelling	m ²	5.652174	0	0	36510	0	0	206,361	
	CW-1-22	Concrete	m ³	10	120	37130	158740	1,200	371,300	1,587,400	
Indirect Cost											
Site Expense			%	15	0.8		0.2	310,749	0	77,687	
Profit and Overhead Cost			%	10	0.8		0.2	238,241	0	59,560	
		Miscellaneous	L.S.					78	89	24	Round Up
Total for		10 m ³						785,700	376,100	2,114,200	
Unit Cost for		1 m ³						78,570	37,610	211,420	

*1: Form Work Area : Total Form work area / Concrete Volume = Form Work Area m² / concrete 1m³
 52 / 92 = 0.565217 m²/m³

*2: Dump Truck : 10.2 m³ / 4.5 m³/truck = 2.27 Truck
 10 km/rnd / 30 km/hr + 60 mnts(loss) = 1.33 hours
 2.27 Truck x 1.33 hours = 3.02 hours

ID No.	Working Name		Calculation Quantity							Remarks	
U-P2-Bq-98	Deformed Reinforcing Bars		1000 kg								
U-P2-Bq-115	Deformed Reinforcing Bars										
U-P2-Bq-132	Deformed Reinforcing Bars										
U-P2-Bq-141	Deformed Reinforcing Bars										
U-P2-Bq-157	Deformed Reinforcing Bars										
U-P2-Bq-202	Deformed Reinforcing Bars										
U-P2-Bq-244	Deformed Reinforcing Bars										
U-P2-Bq-259	Deformed Reinforcing Bars										
U-P2-Bq-275	Deformed Reinforcing Bars										
U-P3-Bq-105	Deformed Reinforcing Bars										
U-P3-Bq-255	Deformed Reinforcing Bars										
U-P3-Bq-290	Deformed Reinforcing Bars										
Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Direct Cost											
Working Base Cost											
	CW-1-32	Reinforcing Bar Setup by using Crane 2	1	1	123300	2809800	3097935	123,300	2,809,800	3,097,935	
Indirect Cost											
Site Expense			%	15	0.8		0.2	723,724	0	180,931	
Profit and Overhead Cost			%	10	0.8		0.2	554,855	0	138,714	
		Miscellaneous	L.S.					21	0	20	Round Up
Total for		1000 kg						1,401,900	2,809,800	3,417,600	
Unit Cost for		1 kg						1,402	2,810	3,418	

ID No.	Working Name		Calculation Quantity							Remarks	
U-P2-Bq-99	Water Stop, 200 mm wide		100 m								
U-P2-Bq-203	Water Stop, 200 mm wide										
U-P2-Bq-276	Water Stop, 200 mm wide										
U-P3-Bq-106	Water Stop, 200 mm wide										
U-P3-Bq-170	Water Stop, 200 mm wide										
U-P3-Bq-256	Water Stop 200 mm wide										
U-P3-Bq-273	Water Stop, 200 mm wide										
U-P3-Bq-291	Water Stop, 200 mm wide										
Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Direct Cost											
Labour	L-2-1	Foreman	day	0.25	0	0	48800	0	0	12,200	
	L-2-6	Welder	day	0.25	0	0	39000	0	0	9,750	
	L-2-23	Common Labour	day	1	0	0	35100	0	0	35,100	
Material	M-G-14	Waterstop; B=200mm	m	105.26	47500	0	2500	4,999,850	0	263,150	
Others											
		Small Tools	%	1				0	0	0	Welder Machine and etc.
Indirect Cost											
Site Expense			%	15	0.8		0.2	638,406	0	159,602	
Profit and Overhead Cost			%	10	0.8		0.2	489,445	0	122,361	
		Miscellaneous	L.S.					99	0	37	Round Up
Total for		100 m						6,127,800	0	602,200	
Unit Cost for		1 m						61,278	0	6,022	

*1: Manpower ; Foreman : 1 man/day / 400 m/day x 100 m = 0.25
 Common Labor : 4 man/day / 400 m/day x 100 m = 1
 Welder : 1 man/day / 400 m/day x 100 m = 0.25
 Common Labor : 1 manpower = 100 m/day
 *2: Water Stop : 100 m / 0.95 = 105.26

Table 4.2.5 (36/119) CALCULATION SHEET FOR UNIT COST OF EACH PAYMENT ITEM OF THREE PACKAGES

ID No.		Working Name		Calculation Quantity			Remarks		
U-P2-Bq-100		Gravel Bedding		10 m3					
U-P2-Bq-186		Gravel Bedding							
U-P3-Bq-79		Gravel Bedding							

Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Direct Cost											
Working Base Cost											
	CW-1-15	Gravel Bedding	m3	10	0	1360	31260	0	13,600	312,600	
Indirect Cost											
		Site Expense	%	15	0.8		0.2	39,144	0	9,786	
		Profit and Overhead Cost	%	10	0.8		0.2	30,010	0	7,503	
		Miscellaneous	L.S.					46	0	11	Round Up
Total for		10 m3						69,200	13,600	329,900	
Unit Cost for		1 m3						6,920	1,360	32,990	

ID No.		Working Name		Calculation Quantity			Remarks		
U-P2-Bq-101		Cobble Stone		10 m3					
U-P2-Bq-187		Cobble Stone							
U-P3-Bq-80		Cobble Stone							
U-P3-Bq-211.1		Cobble Stone							
U-P3-Bq-222.1		Cobble Stone							
U-P3-Bq-232.1		Cobble Stone							
U-P3-Bq-269.1		Cobble Stone							

Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Direct Cost											
Equipment											
	A-2-1-7	Backhoe; 0.6 m3	hourly	0.31	125542.9	2040	90965.08	38,918	632	28,199	
Labour											
	L-2-1	Foreman	day	0.05	0	0	48800	0	0	2,440	
	L-2-23	Common Labour	day	0.2	0	0	35100	0	0	7,020	
Material											
	M-B-5	Cobble Stone	m3	11.11	0	1850	35150	0	20,554	390,517	
Indirect Cost											
		Site Expense	%	15	0.8		0.2	58,594	0	14,648	
		Profit and Overhead Cost	%	10	0.8		0.2	44,922	0	11,230	
		Miscellaneous	L.S.					66	14	45	Round Up
Total for		10 m3						142,500	21,200	454,100	
Unit Cost for		1 m3						14,250	2,120	45,410	

*1 : Manpower ; Foreman : 1 man/day / 200 m3/day x 10 m2 = 0.05
Common Labor : 4 man/day / 200 m3/day x 10 m2 = 0.2
Common Labor : 1 manpower = 50 m3/day

*2 : Backhoe : 3600 x q x f x E
Cm
f : 0.9 q : 0.59 E : 0.5 Cm : 30
Hence, Volume = 31.86 m3/hour

- U-P2-Bq-102 Wet Stone Masonry is equal unit cost with U-P1-Bq-20.
- U-P2-Bq-103 Log Pile, Dia. 150mm, L=3.0m
- U-P2-Bq-104 Weep Hole, Dia. 50mm is equal unit cost with U-P2-Bq-29.
- U-P2-Bq-105 Pointing

Table 4.2.5 (37/119) CALCULATION SHEET FOR UNIT COST OF EACH PAYMENT ITEM OF THREE PACKAGES

Major Item	ID No.	Description	Unit	Quantity	Unit Cost			Cost			Remarks
					PF/C	IF/C	L/C	PF/C	IF/C	L/C	
Direct Cost											
Equipment											
	A-2-1-96	Wheel Loader; 1.2 m3	hourly	0.02	72,404	1,116	58,629	1,448	22	1,173	
	A-2-2-16	Generator; 125 kVA	daily	0.01	271912.2	15120	209096	2,719	151	2,091	
	A-2-1-55	Dumptruck; 8 ton	hourly	0.15	58770.27	2200	53720.55	8,316	330	8,058	
	A-2-2-63	Asphalt Finisher 2.4m	hourly	0.03	170054.4	0	150540.1	5,102	0	4,516	
	A-2-1-68	Tire Roller; 8-20 ton	hourly	0.02	81684.16	864	82451.15	1,634	17	1,649	
Labour											
	L-2-1	Foreman	day	0.01	0	0	48800	0	0	488	
	L-2-22	Asphalt Walker	day	0.06	0	0	35100	0	0	2,106	
Material											
	M-B-2	Coarse Aggregate	m3	0.420406	0	2600	49400	0	1,093	20,768	
	M-B-1	Fine Aggregate (washed sand)	m3	0.185625	0	2100	39900	0	390	7,406	
		Filler	kg	60.01875	0	0	135.35	0	0	8,124	lessthan0.075mm
	M-C-4	Asphalt	kg	82.6875	0	450	1050	0	37,209	86,822	
Others											
		Small Tools	%	5				986	1,961	7,160	
Indirect Cost											
		Site Expense	%	15	0.8		0.2	25,469	0	6,267	
Profit and Overhead Cost											
		Miscellaneous	L.S.	10	0.8		0.2	19,526	0	4,881	
								1	26	91	Round Up
Total for 10 m2								65,700	41,200	161,700	
Unit Cost for 1 m2								6,570	4,120	16,170	
Unit Cost for 1 ton (1.23ton including 10%loss)								874	548	2,151	

Analysis of surface course (AC) 1 M2, 50 mm thick

No	Working Name	Code	Coefficien	Unit	Remark	No	Working Name	Code	Coefficien	Unit
I.										
1	Using equipment					2.4	Dump Truck	E09		
2	Work location is on the road length						Bucket capacity	V		4 m3
3	Existing condition of the road are medium						Efficiency factor	Fa	0.83	
4	Average distance from base cam L			1 Km			Average loading spv1			30 Km/hour
5	Thickness of surface course (AC)			0.05 m			Average empty spv2			40 Km/hour
6	Effective Hour / day	Tk		7 Hour			AMP capacity/Bat Q2b			0.5 ton
7	Loss factor of the material						Time to prepare 1 Tb			1 minute
	-Aggregate	Fh1		1.1			Cycle time :			
	-Asphalt	Fh2		1.05			Loading time = (VT1			8 minute
8	Composition of the material						Transportation tin T2			2 minute
	-Fine aggregate 30 - 50 %	FA		0.27			Waiting+dumping T3			15 minute
	-Coarse aggregate 39 - 59 %	CA		0.6115			Turning back time T4			1.5 minute
	-Filler fraction 4.5 - 7.5 %	FF		0.0485			Ts2			26.5 minute
	Asphalt minimum 6.7 %	As		0.07			Production capaciti Q4			66.81761006 m2
9	Unit Weight of Material						= (V*Fa*60)/(D1*Ts2*t)			
	-AC	D1		2.25 ton/m3			Equipment coeffiti E09			0.014966114 hour
	-Coarse aggregate & Fine aggred	D2		1.8 ton/m3		2.5	Asphalt Finisher	E02		
	-Filler fraction	D3		2 ton/m3			Production capaciti V			40 ton/hour
	-Asphalt	D4		1.03 ton/m3			Efficiency factor Fa			0.83
							Production capaciti Q5			295.1111111 m2
							Equipment coeffiti E02			0.003388554 hour
II.										
	Material, equipment, and labour					2.6	Tandem Roller	E17		
1	Material				ton		Average speed v			2.5 km/hour
	Coarse aggregate = (CA*(D1*t)/M03		0.042041	m3	0.075673		Effective width of b			1.8 m
	Fine aggregate = (FA*(D1*t)/M04		0.018563	m3	0.033413		Number of track n			4 track
	Filler = (FF*(D1*t)/M05		6.001875	kg	0.006002		Efficiency factor Fa			0.83
	Asphalt = (AS*(D1*t)/M10		8.26875	kg	0.008269		Production capaciti Q6			933.75 m2
					0.123356		= (1000v*b*(Fa)/(n*t)			
							Equipment coeffiti E19			0.00107095 hour
2	Equipment					2.7	Pneumatic Tire R	E18		
2.1	Wheel Loader	E15					Average speed v			3.5 km/hour
	Bucket capacity	V		1.2 m3			Effective width of b			1.8 m
	Bucket factor	Fb		0.9			Number of track n			8 track
	Efficiency factor	Fa		0.83			Efficiency factor Fa			0.83
	Cycle time						Production capaciti Q7			653.625 m2
	- Loading	T1		1.5 minute			= (1000v*b*(Fa)/(n*t)			
	-Others	T2		0.5 minute			Equipment coeffiti E14			0.001529929 hour
		Ts1		2 minute						
	Production capacity (m2/hour) Q1			430.272 m2		2.8	Light Tools			
	= (D2*V*Fb*Fa*60)/(D1*t*Ts1)						Pole			
	Equipment coefficient/m2 = 1/Q1E15			0.002324 hour			Carriage			
2.2	Asphalt Mixing Plant	E01					Shovel			
	Production capacity	V		30 ton/hour			Earth fork			
	Efficiency factor	Fa		0.83			Control stick for pavement thickness			
	Production capacity/hour = V*F*Q2			221.3333 m2		2.9	Man Power/labour			
	Equipment coefficient/m2 = 1/Q2E01			0.004518 hour			The Significant Pr-Q1			221.3333333 m2/hour
2.3	Generator Set	E12					AC production/da-Qt			1549.333333 m2
	Production capacity = AMP	Q3		221.3333 m2			Man Power			
	Equipment coefficient/m2 = 1/Q3E01			0.004518 hour			-Common Labor P			10
							-Foreman M			1
							Man Power Coefficient			
							-Common Labor L01			0.045180723
							-Foreman L03			0.004518072 hour