

Name of Structure	BARU CONVEYANCE CHANNEL INLET	Category Calculation	REINFORCING BAR VOLUME	Page	5/17	
				·	<u> </u>	

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3. Reinforcing Bar

a. Box Culvert Reinforcing Bar

· · · · · ·		<u> </u>	BAR W	EIGTH		· · ·	to see at se
TYPE	DIA	LENGTH	NUMBER	WEIGHT PER m (kgf/m)	WEIGHT PER BAR (kgf/m)	WEIGHT (kgf)	SHAPE
\$1	D 13	5380	39	1.040	5.595	218.213	
S 2	D 13	2520	- 77	1.040	2.621	201.812	
<u>\$3</u>	D 13	970	78	1.040	1.009	78.686	
<u>\$4</u>	D 13	2300	78	1.040	2.392	186.576	
<u>\$5</u>	D 13	8820	22	1.040	9.173	201.806	
<u>\$6</u>	D 13	870	54	1.040	0.905	48.859	
W1	D 13	2550	78	1.040	2.652	206.856	
· W 2	D 13	8820	22	1.040	9.173	201.802	<u>-</u>
W 3	D 13	400	108	1.040	0.416	44.928	
· F1	D 13	5430	39	1.040	5.647	220.241	
<u>F2</u>	D 13	2520	. 77	1.040	2.621	201.802	·
F 3	D 13	1010	78	1.040	1.050	81.931	
F 4	D 13	2240	78	1.040	2.330	181.709	
F5	D 13	8820	22	1.040	9.173	201.802	
F6	D 13	920	54	1.040	0.957	51.667	
L		· · · ·	ur un	an ware i	Tolal =	2,328.690	

b. Wing Wall Reinforcing Bar

	4 6 1	ан 1911 - 1911 - 1911 - 1911 - 1911 - 1911 - 1911 - 1911 - 1911 - 1911 - 1911 - 1911 - 1911 - 1911 - 1911 - 1911 -		e et la elui			19 ^{19 1}
····		· · · · · · · · · · · · · · · · · · ·	BAR WE	IGTH			
TYPE	DIA	LENGTH	NUMBER	WEIGHT PER m (kgf/m)	WEIGHT PER BAR (kgl/m)	WEIGHT (kgf)	SHAPE
WW 1	D 13	2,040	21	1.040	2.122	44.562	<u> </u>
WW 2	D 13	2,040	21	1.040	2.122	44.562	L
WW 3	D 13	2,990	42	1.040	3.110	130.603	
WW 4	D 13	2,990	42	1.040	3.110	130.603	
WW 5	D 13	4,670	16	1.040	4.857	77.712	4
WW 5a	D 13	4,200	20	1.040	4.368	87.360	
WW 6	D 13	840	21	1.040	0.874	18.354	
WW 7	D 13	840	21	1.040	0.874	18.354]
WW 8	D 13	903	21	1.040	0.939	19.740	
WW 9	D 13	1,003	63	1.040	1.043	65 709	
WW 10	D 13	7,820	12	1.040	8.133	97.596	······································
WW 10a	D 13	500	4	1.040	0.520	2.080	19 <u></u> 11
WW 11	D 13	5,415	2	1.040	5.632	11.264	
					Total =	748.499	

Total Reinforcing Bar a + b =

2,328.69 + 748.499

= 3,077.189 kg

Name of Structure		I CONVEY ANNEL IN			ategory lculation		NFORCING R VOLUME	Page	6/17
	oncrete I . Top Co		ment B	aru Reta	rding Ponc	1			
50 20	0 50		Туре	Dia (mm)	Length (m)	No.	Weight Per m	Weight Per Bar	Total (kg)
			1	13	7.5	6 x 2	1.040	7.80	187.2
695		.	2	10	1.56	26x2	0.617	0.963	50.05
		<u>6Ø13</u> 10-300		ا <u>ــــــــــــــــــــــــــــــــــــ</u>	· · · ·			Total	237.25
50 JC		<u> </u>							
100 A. A.	Base Cor	ncrete				· · · · · · · · · · · · · · · · · · ·			
200	300		Тур	e Dia	Lengt	n No	u la se se di		Total
50				(mm			Per m	Per Bar	(kg)
250	$\langle $	<u>6Ø13</u>		13	10	6 x 1		10.40	124.80
150		Ø 10-300	2	10	1.51	34x	2 0.617	0.932	63.35
sotter [•				· · · · ·		· · · · ·		Total	188.154
50	00 50 Partition	n Wall							
50		şe ta se	Туре	e Dia	Lengt	No.	Weight	Weight	Total
400		<u>Ø 13</u>		(mm) (m)		Per m	Per Bar	(kg)
		0-300	1	13	6.5	6 x 2	2 1.040	6.76	81.12
50			2	10	1.26	22x2	2 0.617	0.777	34.21
50 20	0 50							Total	115.33
Total	1+2+	3							
23	7.25 + 1	88.154 +	115.33			===	540.73 k	g	
Total Re	a service a								
3,	077.189	+ 540.73					3,617.92 k	g	
		1997 - 1917 1917 - 1917	1 - 1 - 1 - 1 - 1 - 1						

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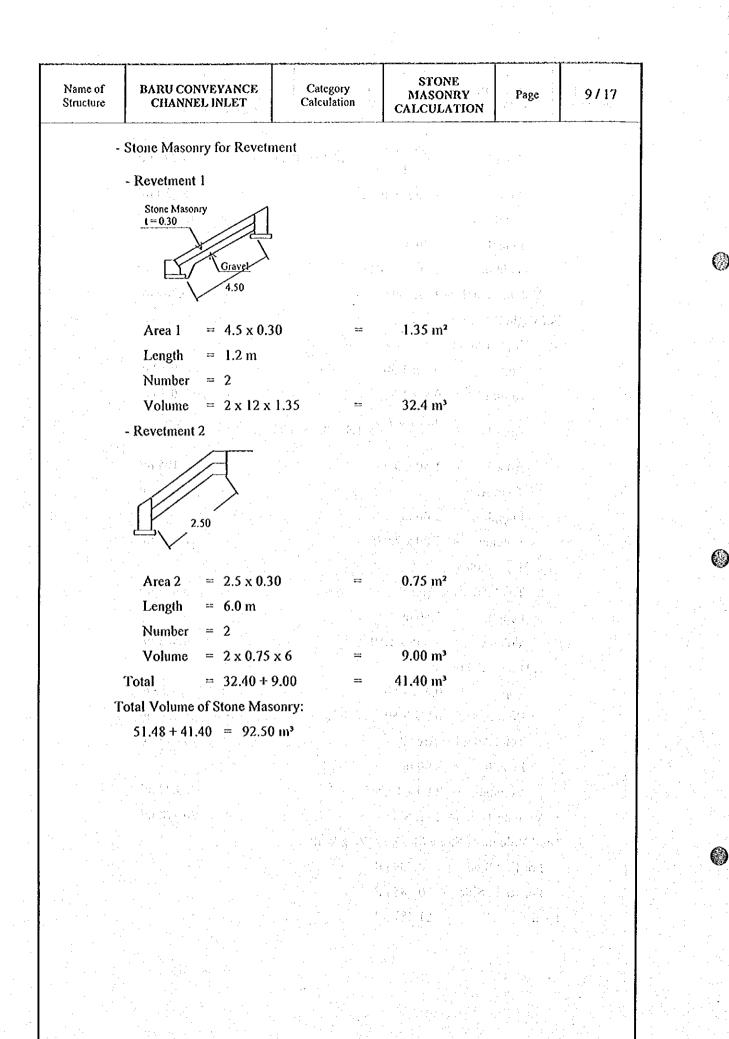
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		7/17		Page	STONE MASONRY CALCULATION	Category Calculation	BARU CONVEYANCE CHANNEL INLET	Name of Structure
		· · ·		n de _{la} companya	neg daer vooren en ster af steren en s	n an	one Masonry	4. St
		:			.e. t		Wall	`
							0.311 Note:	
	÷					= 1.50 m	9.35 H ₁₌₁	
÷ .	•	14	11	· · ·		= 2.00 m	H_{2-2}	
(. •	· · · · · · · · · · · · · · · · · · ·	= 3.50 m	H ₃₋₃	
								n a saya 🖡 a sa
• •							1	H
							2	1.50
•								1.50 L
								· · · · · · ·
•	÷.,	. •					0.5H	
. 1							or Left Side	Fc
n de la composition Composition							a. $H_{1.1} = 1.50 \text{ m}$	
	· ·			0.50 m	=	.50	$Top = 0.3 \times 1$	
•	·	Alexandria Alexandria Alexandria		0.75 m		.50	Bottom = 0.5×1	
e e Le g				.875 m²		0.75 x (1.50 – 0.10)	Area 1 = $\frac{0.5+6}{0.5+6}$	an a Catalona An San An
•		e Nu den i		.073 114	U	x (1.50 - 0.10)	2	
				.035 m²	− 0	.35	0.1 x 0	
· · ·				.125 m²	= 1	0.75	Area 2 = 1.50 x	
•			· · · ·	.035 m ²	= 2		Total Area	
	2				· · · · · · · · · · · · · · · · · · ·		Length $= 2.00 \text{ m}$	
	+ 1 -			4.07 m³			Volume = 2.00 x	
• •							b. H ₂₋₂ = 2.00	
				0.60 m	=	.00	Top = 0.3×2	
				1.00 m			Bottom = 0.5×2	
				1.00 11			and the second	
	 			1.52 m²	, 	<u>.00</u> x 1.90	Area 1 = $\frac{0.6+1}{2}$	
			1910 - 1	0252		35	. –	
(5 A. A			.035 m ²			0.1×0	
			с. С	1.50 m ²	- ゴー・オート・キス	U.73	Area 2 = 1.50 x	
	ен. 1. с			.055 m²	= 3		Total Area	
	·. ·.			a frei Aliana Alfrei Aliana		and the second state of second	Length $= 1.50 \text{ m}$	
to transi Attention			1.1 . 1	.583 m³	= 4	1.50	Volume $=$ 3.055 x	
le e							c. $H_{3.3} = 3.50 \text{ m}$	
				.050 m	=	50	Top = 0.3×3	• • • •
				1.75 m	· · · · · · · · · · · · · · · · · · ·	50	Bottom = 0.5×3	

Name of Structure	BARU CONVEYANCE CHANNEL INLET	Category Calculation	STONE MASONRY CALCULATIO	Page	8/17
	Area 1 = $\frac{1.05 + 2}{2}$	1.75 x 3.40 + 0.1	0 x 0.35 =	4.795 m²	
	Area 2 = 1.50 x 3	3.50	=	5.250 m²	
	Total Area		=	10.045 m²	
	Length = 1.20 m				
	Volume = 1.20 x	10.045	=	12.054 m ³	
	Volume Total for Left Si	de	. =	20.709 m ³	
F	or Right Side				
	a. $H_{1-1} = 1.50 \text{ m}$	·			
	Top = 0.3 x 1.	.50		0.50 m	
	Bottom = 0.5×1 .	.50		0.75 m	
	Area 1 = $\frac{0.5+0}{2}$	<u>.75</u> x 1.4 + 0.1 x	0.35 =	0.91 m²	
	Area 2 = 1.50×6	0.75	=	1.125 m ²	
·	Total Area		=	2.035 m²	
	Length = 3.00 m				
• •	Volume = 3.00×2	2.035		6.105 m³	
	b. $H_{2-2} = 2.00$	· · · ·			
	Total Area 1 + Area 2		=	3.055 m²	
· · · ·	Length $= 1.50$ m	<u>.</u>			
	Volume = 3.055 x	1.50	=	4.583 m³	
	c. $H_{3-3} = 3.50 \text{ m}$				а. С
• •	Top = 0.3×3 .	50	to transformation and the second s	1.050 m	
· ·	Bottom = 0.5×3 .	50		1.75 m	
	Total Area 1 + Area 2			10.045 m²	
	Length = 2.00 m		· ·		
	Volume = 2.00×1	0.045	=	20.09 m³	
	Volume Total for Left Sid	de	=	30.778 m³	•
Т	otal Volume of Stone Masc	onry Wing Wall:			
	For Left Side $= 20.709$	and the second			
	For Right Side = 30.78	8 m³			
т	otal = 51.48				•

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Name of Structure	BARU CONVEYANCE CHANNEL INLET	Category Calculation	GRAVEL BACK FILLING	Page	10/17
5. B	ack Filling Gravel			see g	
		(1) A = 0.30 x	x 4.50 = 1.3	5 m²	
$\frac{\text{Gravel Filling}}{t=0.30}$		(3) A =	$\frac{\times 0.60}{2} = 0.0$	3 m²	
	2.50	Total A	= 1.38	60 m²	
	4.50	Number $= 2$			· · ·
Y		Length $= 12 \text{ m}$			
		Volume = 1.380		2 m ³	·
	(2) A = 0.30×2.50)	= 0.75	60 m²	
	Number $= 2$				1000 - 1000 1000 - 1000
	Length = 6 m				· .
	Volume = $0.75 \times 2 \times 2$		= 9.(00 m³	
	Total Volume Gravel F	the second s		n dati 1 da il	· · ·
	= 33.12 + 9.0	00	42.1 •	3 m ³	
			an a		
		· · · · · · · · · · · · · · · · · · ·			

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Name of Structure	BARU CONVEYANCE CHANNEL INLET	Category Calculation	EXCAVATION	Page	12/17
	Total A		= 12.7	5 m²	
	Vol. = 12.75 x 2		= 24.5	0 m³	-
	Total Excavation for Wi	ing Wall:			1
T	Vol. = 13.75 + 13. otal Volume Excavation:	50 + 48.60 + 24.50	= 100.35	0 m ³	:
	Vol. = 227.82 + 10	00.35	= 328.1	7 m³	-
7. Ba	ick Filling			a An an	-
-]	For Box Culvert		а Т		-
	= 227.82 - (2	.85 x 2.70 x 9)	= 158.56	5 m ³	
-]	For Wing Wall				-
	= 100.35 - (0	.9 x 5) - (1.6 x 3) -	(3.6 x 5.4)		-
	- (4.9 x 2)		= 61.8	1 m ³	
	stal Rock Filling	。 注意:1931年(1931年)。		, ha e f	
	= 158.565 + 6		= 220.37	5 m³	

Name of Structure	BARU CONVEYANCE CHANNEL INLET	Category Calculation	ROA PAVEMI LOG PI	ENT, 💈	Page	13 / 17
8. P	VC Weep Hole: Ø 50 mm	1			• • •	
	Number of Hole/ m len Length of Revetment	gth = 7 holes = 13 m		tin te		
	Total hole	$= 10 \times 2 \times 7$	=	1401	oles	
9. L	og Pile Ø 150, L = 3,000					· ·
	Length of Str.	$= 6.5 + 2.0 \times 2$	+ 5.0 =	15.5	50 m 💠 🚎	1
· ·	Log Pile	$= 15 \times 2 \times 3.0$	=	9	0 m²	
10. 0	Cobble Stone					
	Section $1 - 1 = (3)$	0 + 2.0) x 1.70 x 0.1	5 =	1.27	5 m³	
	Section $2-2 = (1.$	5 + 1.5) x 2.20 x 0.1	5 =	0.99	0 m³	
•	Section $3 - 3 = (3.$	0 + 2.2) x 3.20 x 0.1	5 da s ≞ d	2.49	6 m³	
· .	Section $4-4 = (1)$	0 + 1.0) x 3.70 x 0.1	5 =	1.10	0 m³	
	Total			5.87	1 m ³	· · · · ·
11.1	Pointing					
•	$=\sqrt{3.4^2+6}$	8^{2} x (8.5 + 11)	=	148.2	S m²	

Name of Structure	BARU CONVE CHANNEL IN	1	Category Calculation	FORM W	ORK	Page	14/17
FOR	RM WORK	n transt Barna	n an	*		· · · · · · · · · · · · · · · · · · ·	.
. Tig k Bo	1. Wall:	2.75 m x	2 x 8.6 m	. =	47.3	0 m²	
	•	1.70 m x	2 x 8.6 m	=:2	29.2	4 m ²	· ·
		0.45 m x	: 4 x 9.00 m	en. 200 🖛 e	16.2	0 m²	
· · · · ·		Sub Tota	at a fui	이 너 ㅋ :	92.7	4 m²	
· · · ·	2. Top Slab:	1.70 m x	9.00 m		15.3	0 m²	E
	3. Wing Wall:	4 - 2 - 2	×	e galerin et syn Sin de service			
i en		(5) 2.5	m x 4.10 m x 4	1. 	41.0	0 m²	en e
	•	(6) <u>2.5</u>	$\frac{m+1.5 m}{2} \ge 0.75 r$	n x 4 =	6.0	0 m²	
n 1997 - Den 1997 - Den		(7) 0.75	5 m x 2.00 m x 2	이 아이 같이요. 1993년 - 동네 문	3.0	0 m²	
		(8) 1.30) m x 2.00 m x 2		7.2	0 m²	an an Arta. An Arta Arta
		Sub Tota	1	=	57.2	0 m²	
1	Total	= 92.7	74 + 15.30 + 57.20	=	164.9	4 m²	
	4. Concrete for	1. S.					
i aletter Viteri	Top Cor	crete			12.57¥		
			+ 0.70 m) x 7.5 x 2	=	18.7	5 m²	
		- ·· .	2 x 7.5 m x 2	=	3.0	0 m²	· · ·
•	Base Co	ncrete					
ta san		(0.5 + 0.	55) x 10 x 2	=	21.0	0 m ²	· · · · · ·
· . ·	an a	0.1 m x 2	2 x 10 x 2	· =	4.0	0 m²	
	Partition						14 17
	· · · · ·	0.5 x 2 x	6.5 x 2	: =	13.1	0 m²	
		1	2 x 6.5 x 2	-	2.6	0 m²	
	Total				61.77	5 m²	
	Grand Total	= 164	.94 + 61.775	=	226	.675	
$\{ (1,2) \in \mathbb{N} \}$:

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Name of Structure	SCAFOLDING AND FROM SUPPORT, FOR BANDAR HARJO DRAINAGE SYSTEM	Category of calculation	WORK VOLUME	Page	15/17

SUMMARY OF SCAFOLDING AND FORM SUPPORT VOLUME, FOR BANDARHARJO DRAINAGE SYSTEM

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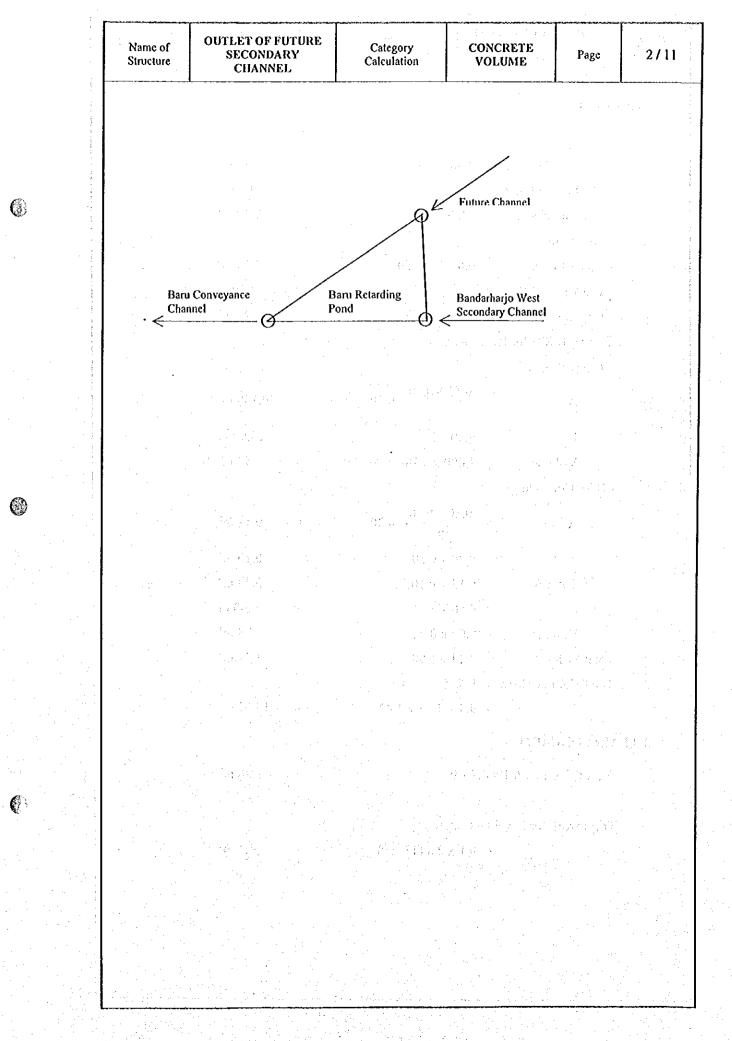
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No.	VOLUME	SCAFOLDING (m²)	FORM SUPPORT (m³)
• 1	BARU PUMPING STATION	1,049	549
2	BARU PUMPING STATION GATE	350	120
3	BARU CONVEYANCE CHANNEL	6,574	2,768
4	BARU CONVEYANCE CHANNEL INLET STRUCTURE	150	35
5	BARU CONVEYANCE CHANNEL OUTLET STRUCTURE	106	20
6	BANDARHARJO EAST SECONDARY CHANNEL	1,166	491
7	BANDARHARIO EAST SECONDARY CHANNEL OUTLET STRUCTURE	90	31
8	BARU RETARDING POND INLET STRUCTURE No. 1		77
9	BARU RETARDING POND INLET STRUCTURE No. 2		42
10	FUEL TANK BOX FOR BARU PUMPING STATION	133	62
	TOTAL	9,618	4,195

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	Name of Structure	BARU CONVEYANCE CH. INLET STR.; FOR SCAFFOLDING AND FORM SUPPORT	Category of calculation	VOLUME CALCULATION	Page	16/17
9 1 1 1		DING AREA		an a		
	A. Box C {(2 x	2.75) + (2 x 2.0)} x 8.65	=	82.175 m ²		
	B. Wing	Wall		e Na Stationa		
C		.85 – 2 x 2.0) + (2 x 2.5 x 4	.85)	an an taon an	· • • •	- -
	+ (3.0	x 1.55) + (2 x 0.35 x 4.85)		67.06 m ²		-
	Total		=	149.235 m ²		
	2 FORM SU				1911 L	
	2. FORM SU 2.0 x 2.0 x		· . =	36.60 m ³		
(andar gʻoʻrata alagi Tari Alagi atalogi atalogi Alagi atalogi ata		
	and Alian Alian ang ang ang ang ang ang ang ang ang a					
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			*			
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					· · .	
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Name of Structure	OUTLET OF FUTURE SECONDARY CHANNEL	Category Calculation	CONCRETE VOLUME	Page	1/11 3 1/11 3 2 (4 g)	
	OUTLET WORK ()F FUTURE SEC(net.		
		<u>,</u>				
1. CO	NCRETE K 225		=]	15.07 m ³		
2. LE	VELING CONCRETE		an a	1.28 m ³	йн н М	
FO	RM WORK FOR LEVEL	ING CONCRETE		4.4 m ²		
3. RE	INFORCING BAR		=	837 Kg		
4. ST	ONE MASONRY		=	137 m ³	2	
5. BA	CK FILL GRAVEL			17.5 m ³		
6. ST	RUCTURE EXCAVATIO	N	=	378 m ³		
7. BA	CK FILLING		= 20	8.63 m ³	· · · · ·	
8. LO	G PILE Ø 15 cm, L=3.0 n	n	=	156 m'		
9. WE	EP HOLE PVC Ø 50 cm			42 nos.		
	RM WORK		≕ 9	5.91 m ²		
· · · · · · · · · · · · · · · · · · ·	BBLE STONE			5.57 m ³		



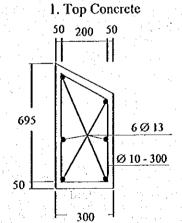
Structure	SECONDARY CHANNEL	Category Calculation	CONCI		Page	3/11
CONCRET	E K 225		· · ·		· · .	· · ·
Slab						
- Area	a of Slab = 2.6	50 x 9.0		23.40	m²	
- Thio	kness of Slab		=	0.40	m	
- Voli	ume of Slab = 23.	.40 x 0.40	=	9.40	m³	
Parapet	Wall					
- Area	a of Parapet = 0.8	x 2 x 2.60	=	4.16	m²	2
- Thic	kness of Parapet		. =	0.30	m	
- Volı	ume of Parapet = 0.3	x 4.16	·	1.25	m ³	
Concre	te K 225 for Revetme	nt			•	
- Тор	Concrete:			1. S.	· •.	
	$A = \frac{0.5}{2}$	$\frac{55+0.70}{2}$ × 0.30	=	0.188	n²	
	L = 6.5	0 x 2	=	13.00	m	
	Volume $= 13.$	00 x 0.188	·	2.44 1	11 ³	
- Base	Concrete:			1 A.		
	$A = \frac{0.5}{2}$	$\frac{50+0.30}{2} \times 0.30$	=	0.12 ı	n²	
	0.2	0 x 0.50	=	0.10 1	n²	
· .	Total A = 0.12	2 + 0.10	. ==	0.22 ו	n²	
	L = 4.5	0 x 2	. =	9.00	m	
	Volume = 9.00	0 x 0.22	= `	1.98 r	n ³	
Total	Volume = 2.44	4 + 1.98	_ =	4.42 r	n ³	
Total	Volume Concrete K 2	25		۰ ۲۰ ۰ - ۲۰		
	= 9.4	+ 1.25 + 4.42	=	15.07 r	n ³	
2. PLAIN CO	ONCRETE				al Salah di Maran	
		•	1911 - 1913 - 1914 1			
0.1 X 0	0.5 x 13 + 0.1 x 0.7 x 9	9		1.28 п	n'	
Form v	work for Leveling Co	ncrete				
· ·	= 0.1	x 2 x (13 + 9)	=	4.4 n	1 ²	
					riti. Linguna di	

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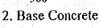
	me of acture	SEC	T OF FUTUR CONDARY HANNEL		tegory culation	REINFORCING BAR VOLUME	1 Page	4/1
• .	3. REINF	ORCIN	G BAR					
	Slab F	einforci	ng	1			1.2	
	1.2	I.1]	L1	1.2	1.2		I.1
	n n n N	Shape	1	Shape 2		LI Shape 3	1.3 Shape 4	
	Туре	Dia	Length (mm)	Number	Weight/m kg/m	Weight/bar (kg)	Weight (kg)	Shape
	S1	13	3,060	71	1.040	3.182	225.95	1
[S2	13	2,620	37	1.040	2.725	100.817	2
	S 3	13	8,820	20	1.040	9.173	183.46	2
. ·	S4	13	400	12	1.040	0.416	4.99	3
Ì	S5	13	2,040	20	1.040	2.121	42.432	4
			······					

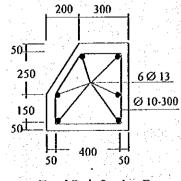
C. Concrete for Revetment Baru Retarding Pond



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11.1	1					
Туре	Dia	Length	No.	Weight	Weight	Total
	(mm)	<u>(m)</u>		Per m	Per Bar	(kg)
1	13	13	6 x 2	1.040	13.52	27.04
2	10	1.56	44x2	0.617	0.963	83.5
• • •	1	• • • • •	••••••••••••••••••••••••••••••••••••••	· · · · · · · · · · · · · · · · · · ·	Total =	110.54
· · · ·						L

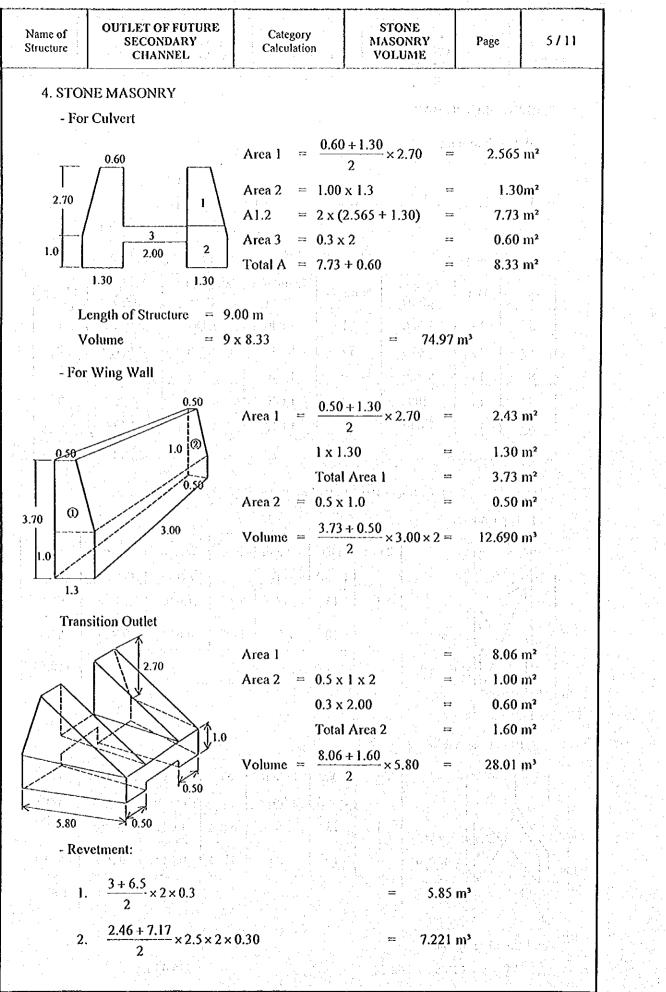




Total Reinforcing Bar 557.65 + 110.54 + 168.24

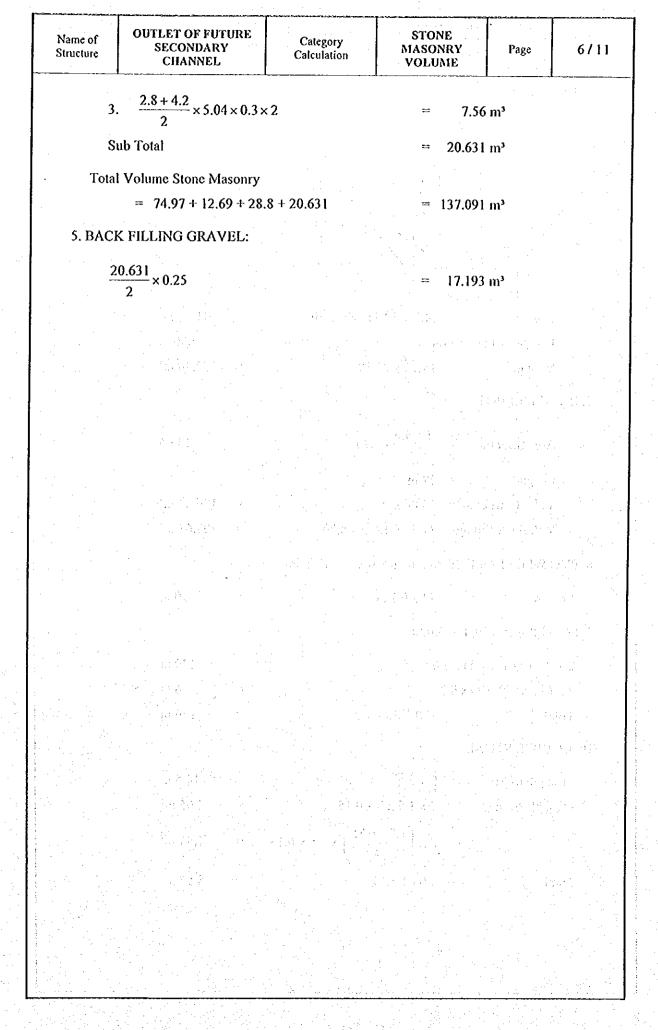
Weight Total Турс Dia Length No. Weight Per Bar (m) Per m (kg) (mm) 9.36 1 6 x 2 1.040 112.32 9 13 2 0.932 55.92 31x2 0.617 10 1.56 Total ≕ 168.24

836.43 kg



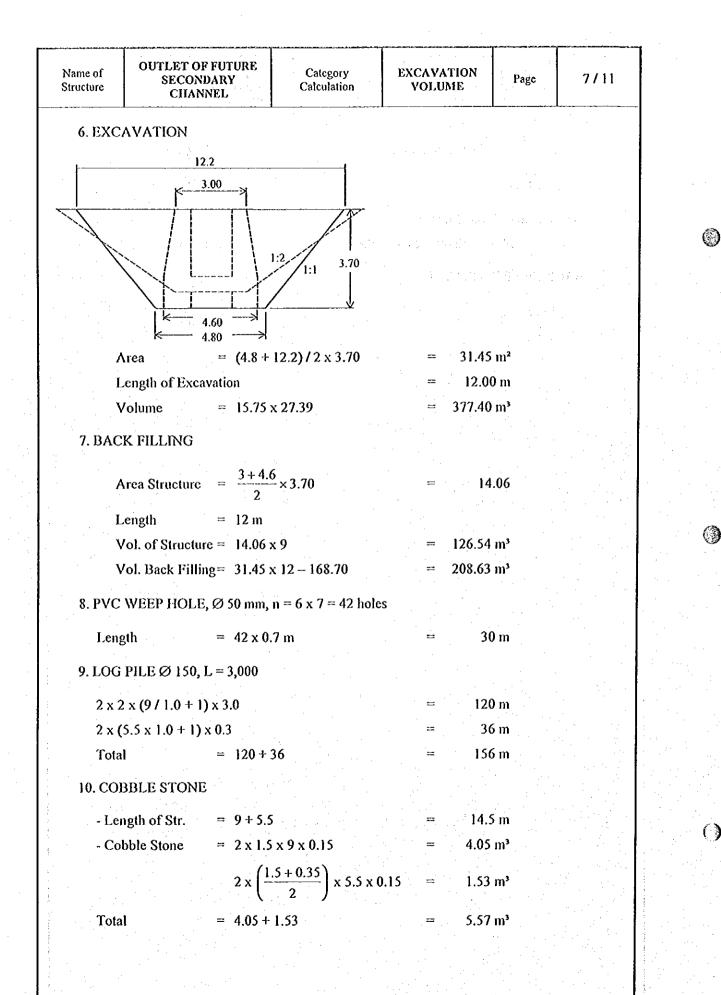
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Name of Structure	OUTLET OF FUTURE SECONDARY CHANNEL	Category Calculation	FORM WORK VOLUME	Page	8/11	
11. Forn		n an				
· .	··· · ·					
a. Sla	ab Concrete			1	2	
2.	00 m x 9.00 m ==	18.00 m ²			÷	
1.	20 m x 2.60 x 2 =	6.24 m²		e a tradición.		
0.	30 m x 2.60 x 2 =	4.16 m²				
Sı	ub Total =	28.40 m ²			·	
b. Co	oncrete for Revetment					
- Tor) Concrete		en operations de la constant de la constant de la constant de la constant de la c			
	.695 + 0.95) x 13 x 2 =	32.37 m ²				
0.	$10 \times 2 \times 13 \times 2 =$	5.20 m²				
Sı	ıb Total =	37.57 m²				
- Bas	e Concrete					
(0	.55 + 0.5) x 13 x 2 ==	27.3 m²				
0.	$10 \ge 2 \ge 6.6 \ge 2$ =	2.64 m²				-
Sı	ıb Total 🗧	29.94 m²		1.		••
Total	Form Work					
28	3.40 + 37.57 + 29.94 =	95.91 m²				•
e da Alexane. Estat						•

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Name of Structure	SUPPORT FOR BANDAR	Category of calculation	WORK VOLU	JME Page	9/1
:	SUMMARY OF SCAFOLDIN FOR BANDARHAR	G AND FOR IJO DRAINA	M SUPPORT VO .GE SYSTEM	LUME,	•
No.	VOLU	ME SC	AFOLÐING (m²)	FORM SUP (m³)	PORT
1	BARU PUMPING STATION		1,049	549	
2	BARU PUMPING STATION GATE		350	120	
3	BARU CONVEYANCE CHANNEL		6,574	2,768	
4	BARU CONVEYANCE CHANNEL INLET STRUCTURE		150	35	
5	BARU CONVEYANCE CHANNEL OUTLET STRUCTURE		106 (1990) 104	20	
6	BANDARHARIO EAST SECONDA CHANNEL		1,166	491	
7	BANDARHARJO EAST SECONDA CHANNEL OUTLET STRUCTURE	RY	90	31	
8	BARU RETARDING POND INLET STRUCTURE No. 1			77	
9	BARU RETARDING POND INLET STRUCTURE No. 2			42	
10	FUEL TANK BOX FOR BARU PUMPING STATION		133	62	
	TOTAL		9,618	4,195	

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	Name of Structure	BARU RETARDING POND INLET STRUCTURE NO.2 FOR SCAFFOLDING AND	Category Calculation	WORK VOLUME	Page	10/11
1 		FORM SUPPORT			L	
:		FFOLDING AREA			. '	
÷	1.001		•		·	
:	2. FOR	M SUPPORT AREA			· · ·	
8					· · ·	
	. ··· 9	$0.0 \ge 2.0 \ge 2.30 = 41.4$	40 m ³			
	· · · · ·				1.	·. ·
			·	and a second parts		
	1			والمراجعين فكرار والأرا		
				an an an an Anna an An	na an an an Arainneach an A Arainneach an Arainneach an A	н. 1
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	5				n en sen de jarren. Ne sen en se Ne sen en sen	
				an a	an na 1999. I	
				· · · · · · · · · · · · · · · · · · ·		
				11月1日日本皇,11月1日 11月		6
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V					• • • •	
						e egel
	·. ·				e en seg	er großen.
			. •			
			•	÷.	, i farfi a santa ang	:
	•••					
			· · · · ·		en en gr	
				· ·		
			e de la construcción de la construc		:	n an training An training training
					· . · ·	
					. 197	
			the second se	and the second	and the second	

Nam Struc		BARU CONVEYANCE CHANNEL (BOX CULVERT)	Category of calculation	SUMM	ARY	Page	1.	/1
					la - S S - S - S	:		
SUMM	IARY	Y OF CHANNEL WORK AND	D INSPECTION	ROAD VC	DLUME	CALCU	LATI	ION
			· .			 100 	ka si	
I.	<u>CII</u>	ANNEL WORKS						-
	1.	Dewatering		<u> </u>	LS			
	2.	Structure Excavation		í, i m i	14,030		m ³	
	3.	Structure Excavation with Sho	ring		8,544		m ³	
•	4.	Bock Fill with Sandy Soil		#	16,110)	m ³	
	5.	Concrete Structure (Type C1)			2981	.5	m³	
	6.	Leveling concrete (Type E)		≓	240	.63	m³	
	7.	Water Stop with Rubber Filler	· · ·	=	381	.50	m ʻ	
	8.	Form Work for Concrete Type	Cl	=	10,989	.43	m²	
	9.	Form work for Concrete Type	E	. =	166		m²	
1. A.							·	
Ш.	RE	NFORCING BAR					•	•
	1.	Box Culvert = 79650 + 54450)	. ==	134,100	ч. 1. т. т.	kg	
	2.	Man Hole Block Type A			25,710		kg	
	3.	Man Hole Block Type B			65,410	· · · · · · · · · · · · · · · · · · ·	kg	
	· · ·		·	=	225,220		kg	
				۰ ۲۰۰۰ ۲۰۰۰			•	
111.	INS	PECTION ROAD			÷.,			
	۱.	Embankment		=	1,944	· ·	m ³	
	2.	Sand Bedding	r.	. =	79	· .	m ³	
	3.	Concrete Conbloc		. =	1,315		m ²	
	4.	Cement Mortar	. '	• •	3		m ³	
	5.	Concrete Kerb		=	32		m³	
	6.	Aggregate Class A			225		m ³	

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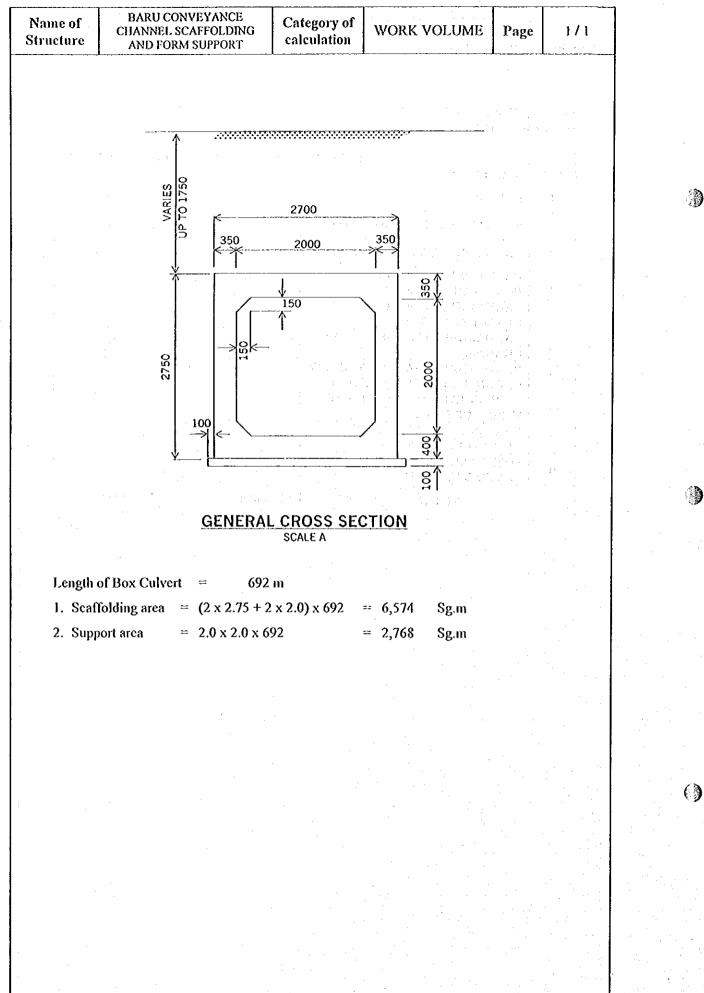
Name of	
Structure	

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Page

SUMMARY SCAFFOLDING AND FORM SUPPORT VOLUME, FOR BANDARHARJO DRAINAGE SYSTEM

No.	VOLUME	SCAFFOLDING (m²)	FORM SUPPORT (m ³)
1	BARU PUMPING STATION	1,049	549
2	BARU PUMPING STATION GATE	350	120
3	BARU CONVEYANCE CHANNEL	6,574	2,768
4	BARU CONVEYANCE CHANNEL INLET STRUCTURE	150	35
5	BARU CONVEYANCE CHANNEL OUTLET STRUCTURE	106	20
6	BANDARHARJO EAT SECONDARY CHANNEL	1,166	491
7	BANDARHARJO EAST SECONDARY CHANNEL OUTLET STRUCTURE	90	31
8	BARU RETARDING POND INLET STRUCTURE No. 1	-	77
9	BARU RETARDING POND INLET STRUCTURE No. 2		42
10	FUEL TANK BOX FOR BARU PUMPING STATION	133	62
	TOTAL	9,618	4,195



Package 3: I Bandarharjo East Secondary Channel

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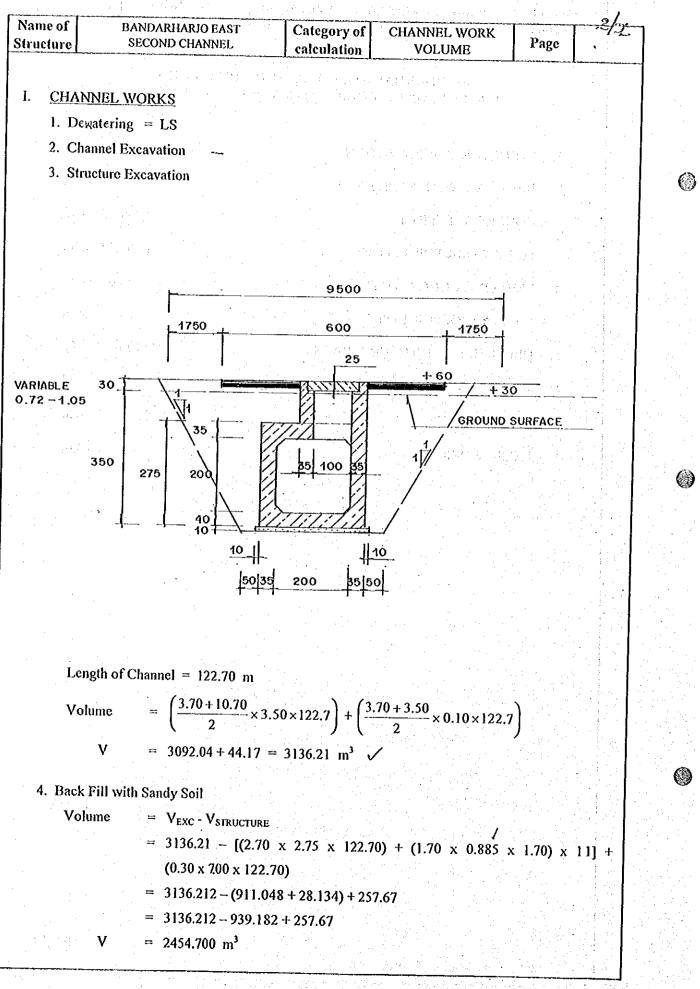
Name of Structure	BANDARHARJO EAST SECONDARY CHANNEL	Category of calculation	WORKS VOLUM		age	1/7
	<u>BANDARHARJO E.</u> SUMMARY OF CHANNEJ	AST SECONDA L WORK VOLI	<u>RY CHANN</u> JME CALCU	<u>EL</u> ILATION		
		· .		2 126 21		
	UCTURAL EXCAVATION X FILL WITH SANDY SOII	,		3,136.21 2,YJ47		€ V
3. CON	ICRETE TYPE C1		=	439.45	cu.m	1
FOR	M WORK FOR TYPE CI	· .	· = .	1,554.49	sq.m	
4. LEV	ELING CONCRETE TYPE I	3	=	35.64	cu.m	1
FOR	M WORK FOR TYPE E			24.54	sq.m	
5. DEF	ORMED REINFORCING BA	ARS	, i a i	32,875	Kg	1
6. WA	TER STOP, $W = 200$		=	65	m'	V
7. SCA	FFOLDING		=	1,166	sq.m	· · · · ·
8. FAI	.SE WORK			491	cu.m	
					· · ·	

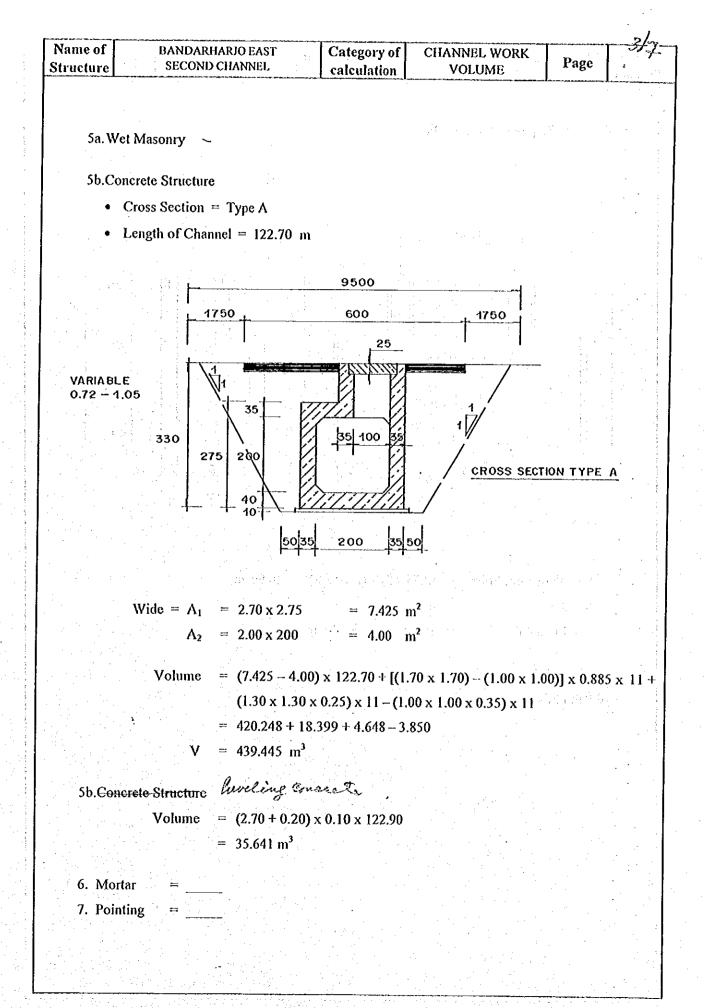
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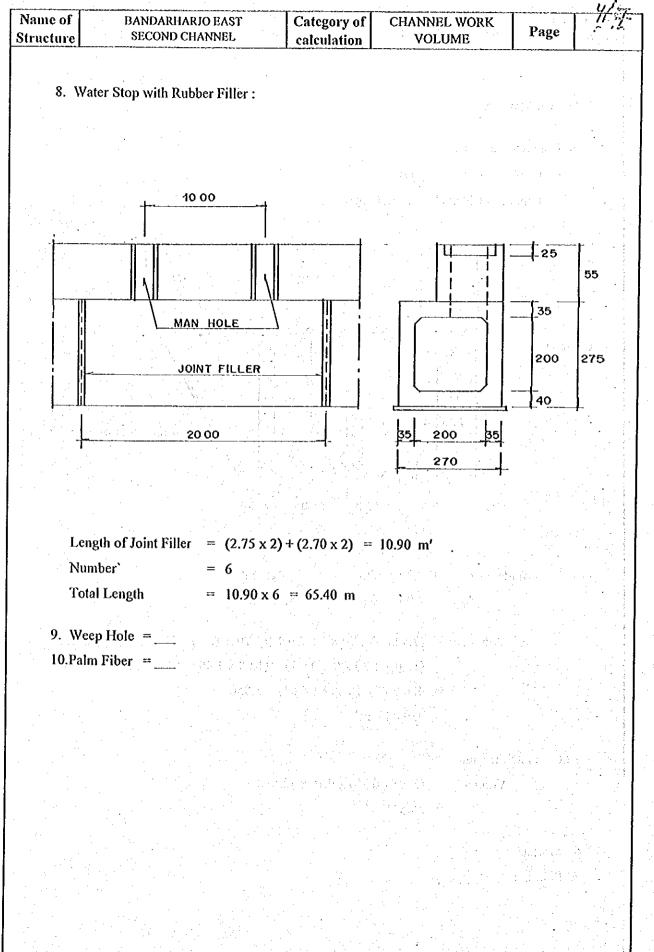
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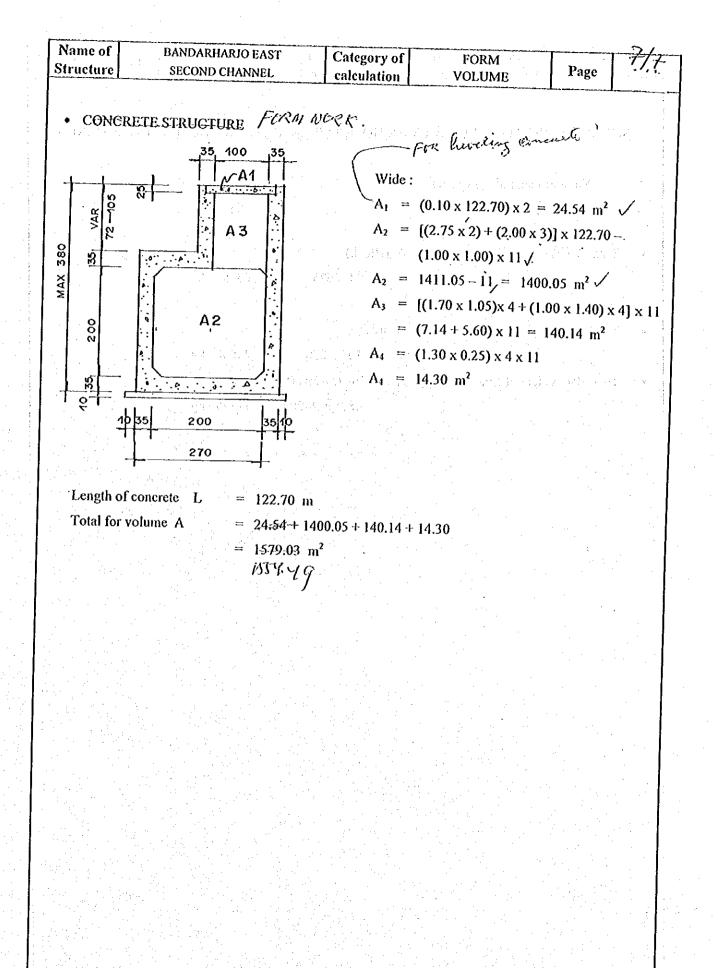




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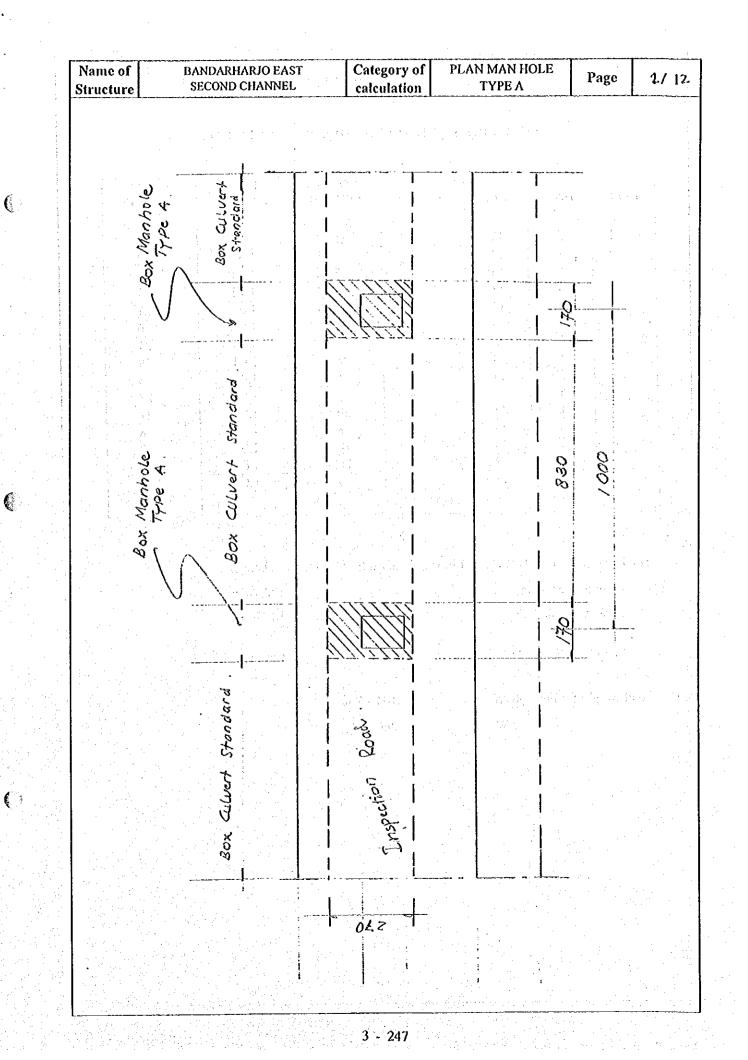




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Name of Structure	BANDARHARJO EAST SECOND CHANNEL	Category of calculation	REINFORCING MAN HOLE VOLUME	Page 1 / 12
SHUCILIE				L
- 	•	- 		
<u>SUMMA</u>	RY REINFORCING BAR MAN	HOLE TYPE	A AND BOX CULVER	(STANDARD)
		. *		
- Nu	umber man hole (Type A)		= 11 pieces <	
-	angth of Box Culvert		= 104 m'	
- 10				
• Total B	ar Weight of Man Hole Type A	(Block) :		
10000		$= 11 \times 625$	= 6875 kg	
× .		- 11 X 025	- 0075 Kg	
	1993年末1月1日(1月1日)(1月1日)) 1993年末日(1月1日)(1月1日)(1月1日)(1月1日)(1月1日)(1月1日)(1月1日)(1月1日)(1月1日)(1月1日)(1月1日)(1月1日)(1月1日)(1月1日)(1月1日)(1月1日)(1月1日)(1			
Total B	Bar Weight of Box Culvert (Stand	1. Sec. 1. Sec		
		= 104 x 250	= 26000 kg	
Total B	lar Weight of Man Hole Type A a	and Box Culvert	(Standard) :	
		= 6875 + 260	00 = 32875 kg	
				an dia kaominina dia 41 Manjara dia mandri kaominina
		an an an an Araba. An an Araba		



Name of Structure			RHARJO EA D CHANNE		Category of calculation		RCING BOX	Page	3 / 12
		· .	•	· · · · ·	· · · · · · ·				
		DA	DWEIOI						7
		DE	u weigr	TI FURS	l'ANDARD I	SOX CUL	<u>VERT / m'</u>		
			· · ·			•		•	-
TY	PE	SHAPE	DIA	NUMBE	R LENG (m)	тн Гу	UNIT VEIGHT (Kg/m)	TOTAL WEIGHT (Kg)	
S	1	5	13	4	21.5		1.04		-
	2	1	13	8	20.1		1.04	22	-
	3	6	13	8	7.7		1.04		-
	4	4	13	8	18.4	0	1.04	19	-
	5	<u> </u>	13	20	20.0	0	1.04	21	
117	6	3	13	6	5.2	2	1.04	6	-
W	1	1	13	8	20.4	0	1.04	21	-
	2		13	28	28.0	0	1.04	29	-
F	3	2	13	12	4.8		1.04	5	1.
<u>r</u>	$\frac{1}{2}$	5	13	4	21.72		1.04	23	
	2 3	1	13	8	20.10		1.04	21	
-	3	6	13	8	8.08		1.04	8	1
	5		13	8	18.00		1.04	19	1
	6	3	13	20	20.00		1.04	21] : :
	Ľ			6	5.52	?	1.04	6	
				TOTAL		a da seta. Na		250 kg	
				rt in Bandai	harjo East Se	condary ch	annel :		
-	Lengt	th of channe	1	=	<u>e Australia</u>	= 122	.70 m		
-]	Lengt	h of man ho	ole	=	11 x 1.70	- = 18.7	'0 m	· · · · · · · · · · · · · · · · · · ·	
··]	Lengt	h of standar	d box culv	ert		= 104.	00 m		Norman Norman Norman

	I otal weight to	r box culvert		=	104 x 250
• •		Σ₩	;	=	26000 kg

	ie of cture		HARJO EAS		Category of alculation		ORCING MA		Page	4/
		BAI	WEIGH	r for on	E BLOCK N	MAN H	OLE TYPE	<u>A</u> : :	Ar a	.1 .1
			.*				e Bellester			
	түре	SHAPE	DIA	NUMBEI	₹ TOTA LENG (m)	TH	UNIT WEIGHT (Kg/m)		TOTA WEIGI (Kg)	
ĺ	S 1	. 5	13	4	28.42	24	1.04		30	
ł	2	1	13	8	17.9	52	1.04		19	
ł	3	6	13	8	6.59	96	1.04		7	
ł	4	4	13	8	15.64	40	1.04		16	1.5
ľ	5	1	13	20	22.00	00	1.04		23	
ľ	6	3	13	6	3.6	54	1.04	·	- 4	
t	W 1 1	1	13	. 8	34.6	80	1.04		36	
Ī	2	1	13	28	47.60	00	1.04		50	
Ì	3	2	13	12	8.10	60	1.04		9	
Ì	F : 1	5	13	4	36.92	24	1.04		38	
·	2	1	13	8	34.2	72	1.04		36	
Ì	. 3	6	13	8	13.7	36	1.04		14	
	4	4	13	8	30.6		1.04		32	
. [5	1	13	20	34.0		1.04		35	
	6	3	13	6	9.3		1.04		10	
	M I	- 3 -	13	4	30.0		1.04		31	
•	2	3	13	3	16.2		1.04	<u>^</u>	17	· · · .
. [- 3	5	13	33	86.7		1.40		91	14 .
	4	4	13	15	22.7		1.04		24	
	- 5	. 7 :	13	13	25.0		1.04		26	
·	- 6	7	13	<u> </u>	21.7		1.04		23	
	7	<u>− 1 2 2</u>	<u>* 13 4</u>		3.5		1.04		4	
·	8	1	13	4	4.8		1.04		5	
	C 1	<u> </u>	19	7	12.4		2.23		28	
·	2	1	13	4	5.1		1.04		5	
ļ	3	1	13		3.0		1.04		3	
	. 4	2	13		5.5		1.04		6	
	. 5	6	19	2 TOTAI	1.5	40	2.23		<u> </u>	

Bar Shorted Consequent for One Man Hole

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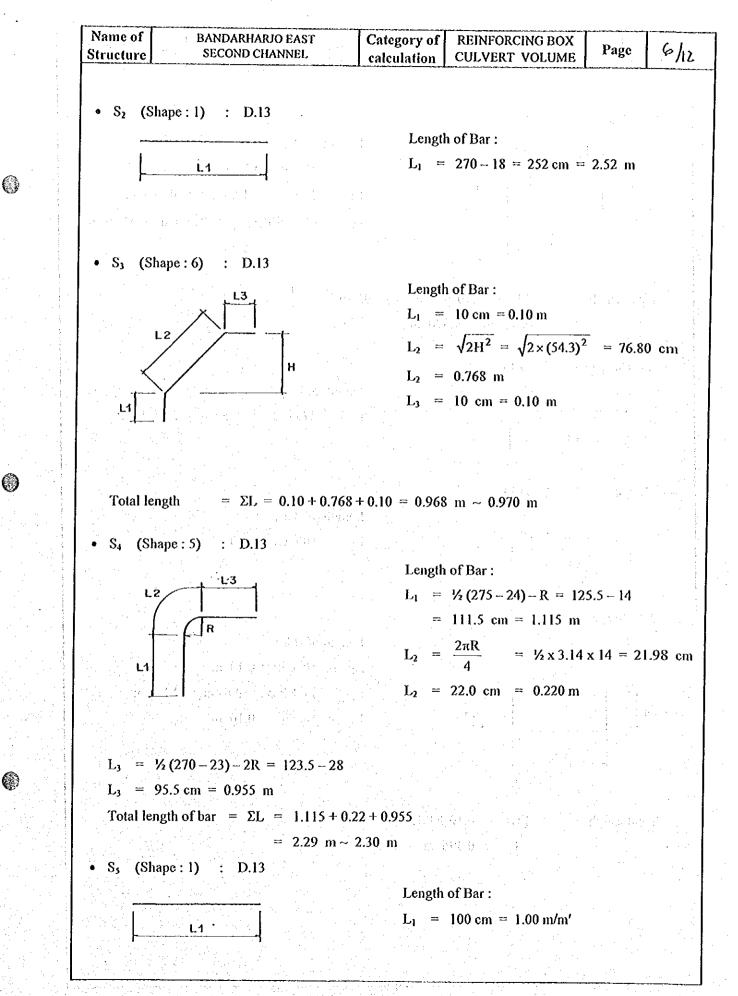
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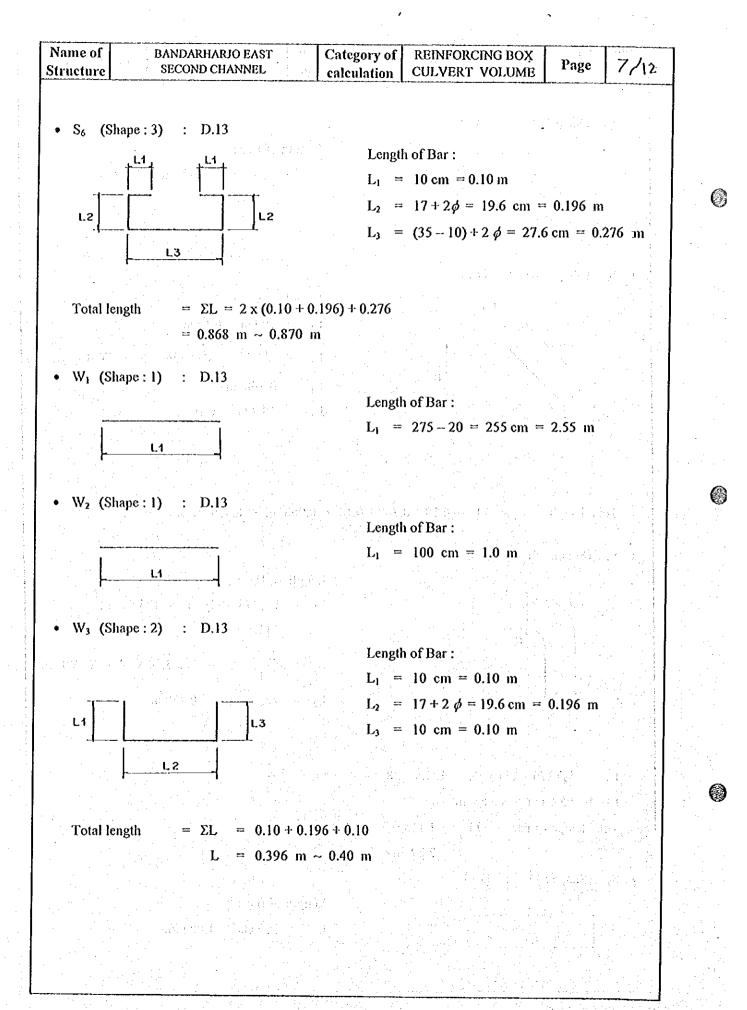
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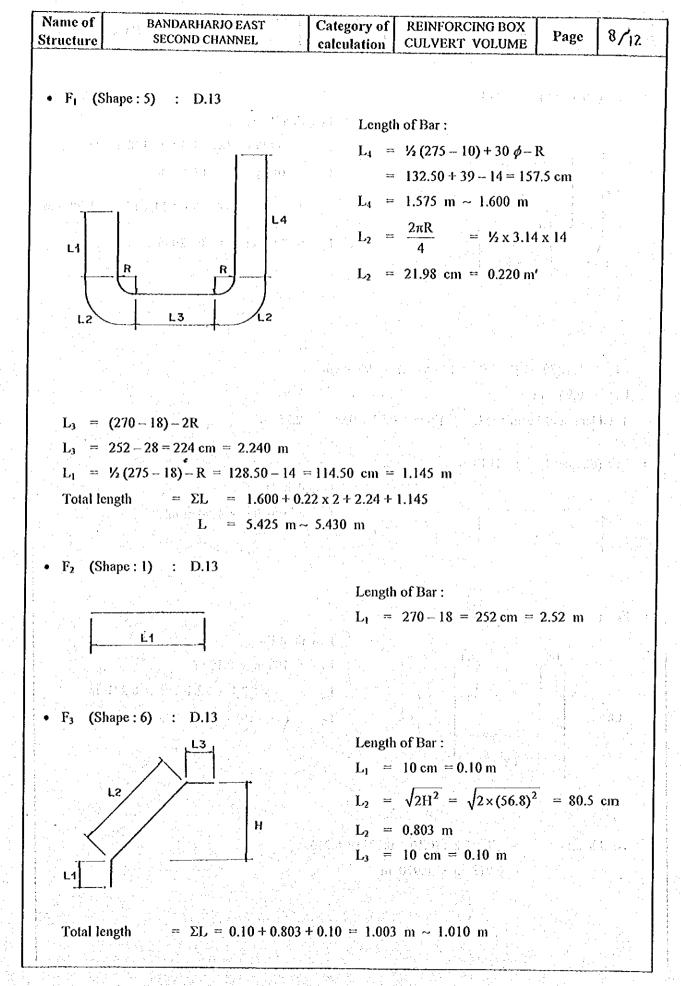
S ₁	==	(1.00 + 2 x 0.10) x 4	=	4.8	m
S ₂	=	1.20 x 8	=	9.60	m
S3	=	0.970 x 4	=	3.88	m
S ₄	=	2.30 x 4		9.20	m
Ss	=	(1.20 x 10)		12.00	m
S6	=	0.870 x 6	=	5.22	m
				1.1	

Structure		calculation	HOLE	VOLUME	Page	5/12
	t n. C. O Died Mais Hal	· · ·			· .	
	orcing Bar for One Block Man Hol					
·		5.72 x 1.70 =				
- S	The second se	$0.56 \ge 1.70 =$				•
- S	$b_3 = (0.970 \times 8) - 3.88 = 3.$	88 x 1.70 =		m		
- S	$b_4 = (2.30 \times 8) - 9.20 = 9.$	20 x 1.70 =	15.640	m		
- S	$s_s = (1.70 \times 20) - 12.00$		22.000	m		
- S	$B_6 = (0.870 \times 6) \times 1.70 - 5.22$		3.654	m		
·	$Y_1 = 2.55 \times 8 \times 1.70$		34.680	m		
· · · · · · · · · · · · · · · · · · ·	$V_2 = 1.00 \ge 28 \ge 1.70$		47.600	m		el el el
- N	$V_3 = 0.40 \times 12 \times 1.70$		8.160	m		
- F	$F_1 = 5.43 \times 4 \times 1.70$	na tenan yang sebagai yang sebag Sebagai yang sebagai	36.924	m		
- F	$F_2 = 2.52 \times 8 \times 1.70$	e e de la compañía d	34.272	m to the		
1 - F - F - F -	$F_3 = 1.01 \times 8 \times 1.70$	=	13.736	m		
	$E_4 = 2.25 \times 8 \times 1.70$		30.600	m		
1	$F_{5} = 1.70 \times 20$					
	$F_6 = 0.920 \times 6 \times 1.70$			11 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		
	6 - 0.720 X 0 X 1.70		7.501			
• S ₁ (S	hape:5) : D.13					
		Lengt	h of Bar :			
n de ser de la composition de la compos En la composition de la			1. S. 1. 1. #	- 10) + 30 <i>ø</i> -	R	
L2/			an tas an tara a	+ 39 – 14 = 15	and the second	
	$-\left(\begin{bmatrix} R & R \end{bmatrix} \right) - +$		= 1.575 n	are a constant	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	
	Le la constante de la constante	+ L ₁ =	الأربي جافل أرار	•		
L1		L ₂ =	$=\frac{2\pi R}{4}$	$= \frac{1}{2} \times 3.1$	4 x 14	
			- 21.09 -	0.000 -		
· 1		L2 =	= 21.98 C	m = 0.220 n	11	
L ₃ =	(270 – 18) – 2R					
L₃ ≒	252 - 28 = 224 cm = 2.240 m					
L ₄ =	$\frac{1}{2}(275-23) - R = 126 - 14 =$	112 cm = 1.12	!0 m			
Total I	$ength = \Sigma L = 1.575 + 0$).22 x 2 + 2.24 +	+ 1.12			
	L = 5.375 m	n el de su etter a la e				

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Structure	BANDARHARJO EAST SECOND CHANNEL	Category of	REINFORCING BOX CULVERT VOLUMB	Page	9/12
aucure		calculation	COLVERT YOLUMB		-/14
• F4 (Sh	nape: 5) : D.13			an gan	
	• 1 ⁴ 1 4 1 4	Lengt	h of Bar :		
r	÷ ₁ , (nativitation)	L ₁ =	½ (275 – 18) – 1½R =	128.5 - 2	1
		$L_1 =$	107.5 cm = 1.075 m		
L1		La =	$\frac{2\pi R}{2\pi R} = \frac{1}{2} \times 3.14$		1.00
		$L_2 =$	$\frac{2\pi x}{4} = \frac{1}{2} \times 3.14$	$1 \times 14 = 2$	1.98 cm
		L ₂ =	22.0 cm = 0.220 m		
rs/	L3	ar an			
· · ·				,	
-				· · · ·	
L ₃ =	$\frac{1}{2}(270-23)-2R = 123.5-28$	= 95.5 cm			
· · · · ·	0.955 m				
	ngth of beam = $\Sigma L = 1.075 + 0$	$.22 \pm 0.955 = 2$	2.25 m		
	<u> </u>			na an an an Anna Anna Anna Anna Anna Ann	
• F5 (Sl	nape:1) : D.13				
	a a de la companya d A companya de la comp A companya de la comp		of Bar :		
	1		おもちがあり オート 見しり オート		
		$L_1 =$	100 cm = 1.00 m/m'		
	L1	L ₁ =	100 cm = 1.00 m/m'		
	<u>L1</u>		100 cm = 1.00 m/m'		
	L 1	1	100 cm = 1.00 m/m'		
• F& (St		La La La = 21 (1) (1) (1) (1) (1) (1) (1) (1) (1) (1	100 cm = 1.00 m/m'		
• F ₆ (St	L 1 nape: 3) : D.13				
• F ₆ (St		Lengt	ı of Bar :		
• F ₆ (Sł		Lengt L ₁ =	n of Bar : 10 cm = 0.10 m	= 0.221	
T	hape: 3) : D.13	Length $L_1 = L_2 = L_2$	n of Bar : 10 cm = 0.10 m 19.5 + 2 ϕ = 22.1 cm	198 - A. C.	
• F ₆ (Sł		Length $L_1 = L_2 = L_2$	n of Bar : 10 cm = 0.10 m	198 - A. C.	
T	hape: 3) : D.13	Length $L_1 = L_2 = L_2$	n of Bar : 10 cm = 0.10 m 19.5 + 2 ϕ = 22.1 cm	198 - A. A.	
T	hape: 3) : D.13	Length $L_1 = L_2 = L_2$	n of Bar : 10 cm = 0.10 m 19.5 + 2 ϕ = 22.1 cm	198 - A. A.	
T	hape: 3) : D.13	Length $L_1 = L_2 = L_2$	n of Bar : 10 cm = 0.10 m 19.5 + 2 ϕ = 22.1 cm	198 - A. A.	
T	hape: 3) : D.13	Length $L_1 = L_2 = L_2$	n of Bar : 10 cm = 0.10 m 19.5 + 2 ϕ = 22.1 cm	198 - A. A.	
T	hape: 3) : D.13	$LengtlL_1 =L_2 =L_3 =$	n of Bar : 10 cm = 0.10 m 19.5 + 2 ϕ = 22.1 cm	198 - A. A.	
L2	hape: 3) : D.13	Lengtl $L_1 = L_2 = L_3 = L_3 = 0.221$	n of Bar : 10 cm = 0.10 m 19.5 + 2 ϕ = 22.1 cm	198 - A. A.	
L2	hape: 3) : D.13 L^{1} L^{2} L^{3} hgth = $\Sigma L = 2 \times (0.10 + 0.00)$	Lengtl $L_1 = L_2 = L_3 = L_3 = 0.221$	n of Bar : 10 cm = 0.10 m 19.5 + 2 ϕ = 22.1 cm	198 - A. A.	
L2	hape: 3) : D.13 $ \begin{array}{c} L1 \\ L3 \\ L3 \\ L3 \\ L2 \\ L2$	Lengti $L_1 = L_2 = L_3 = L_3 = 0.221) + 0.276$	n of Bar : 10 cm = 0.10 m 19.5 + 2 ϕ = 22.1 cm	198 - A. A.	

Name of
Structure
 DANDARHARIO EAST
SECOND CHANNEL
 Category of
categold(on)
 REINFORCING MAN
HOLE VOLUME
 Page

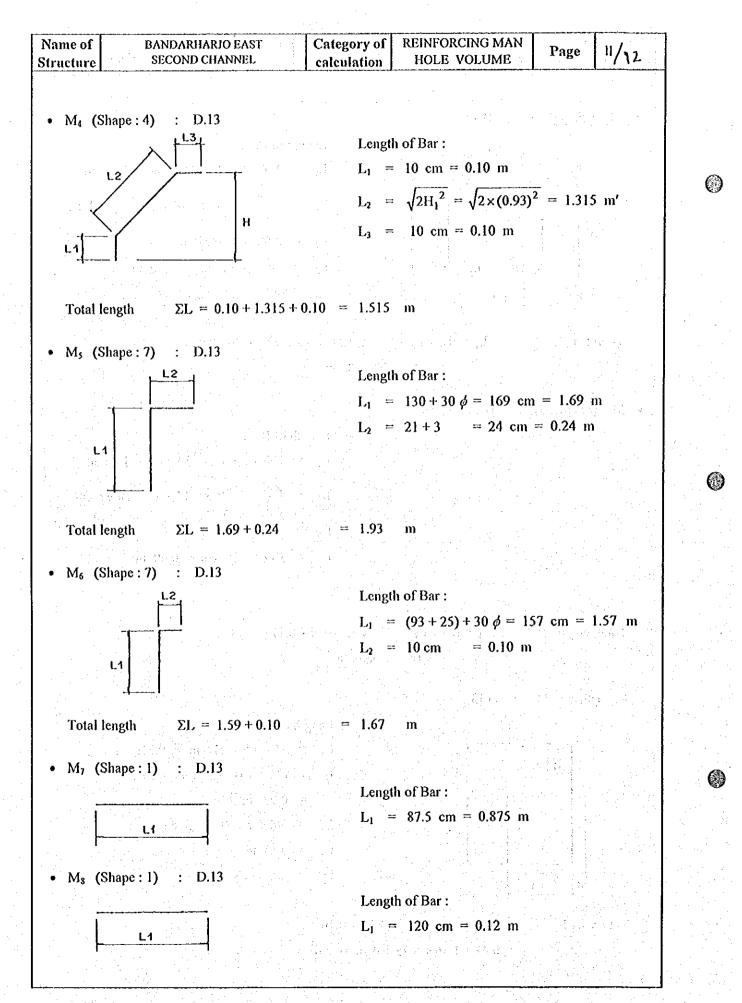
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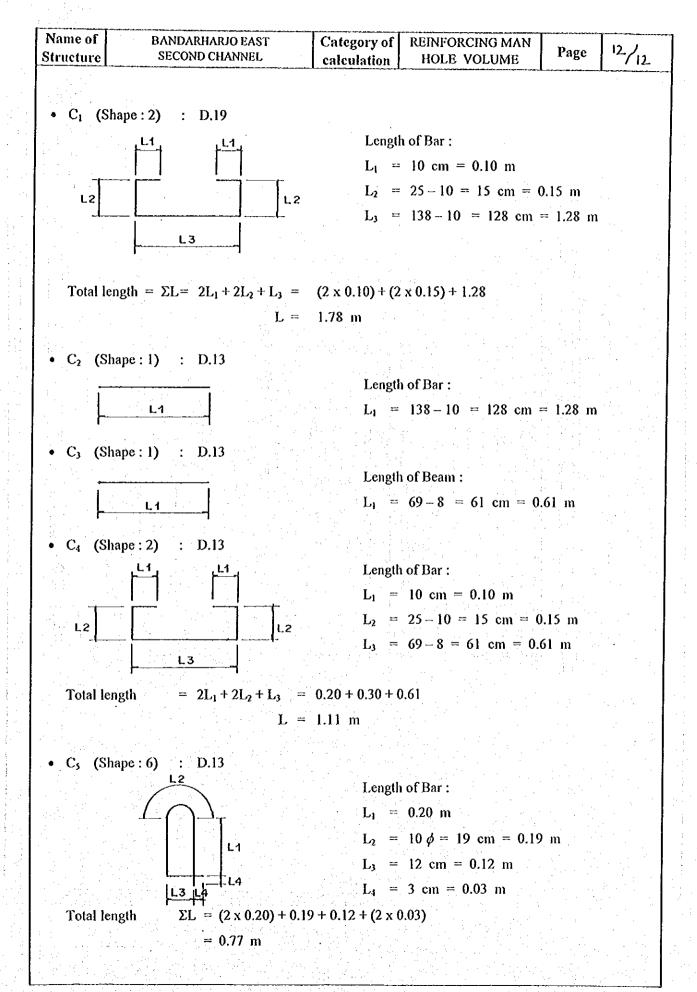
 • M₁ (Shape : 3) : D.13
 L1
 L2
 L3
 L3
 L2
 L3
 L3
 L3
 L3
 L2
 L3
 L3
 L3
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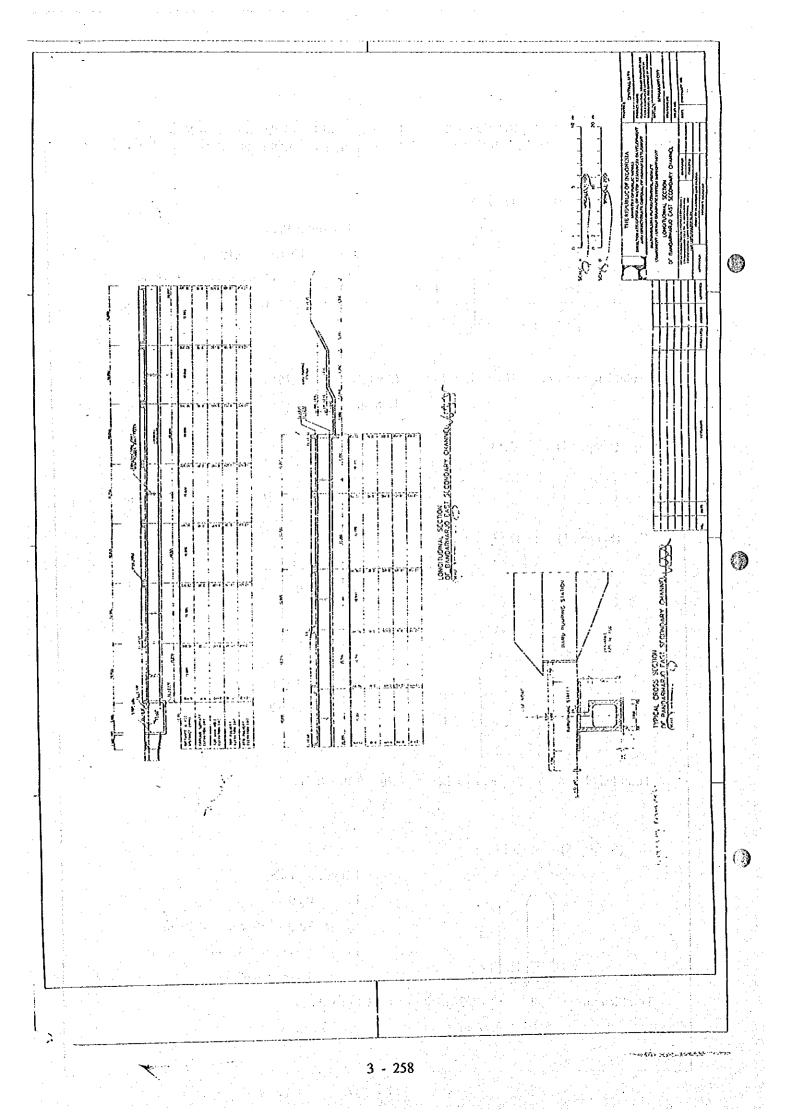
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September 128, 1999

tructure		RJO DRAINAGE SYST Calculation		,,
		MARY OF SCAPOLDING AND FO	RM SUPPORT	NOLLIME
	COP COP	AANDAALMANT ARAMAO C SV	OTEAN	
		BANDARHARIO BRAINAGE SY	<i>S 7 2-11</i>	
	[VOLUME	SCAFOLDING	FORM
	N⁰	STRUCTURE	(14 ²)	SUPPORT (m)
	1	BARU PUMPING STATION	1049	549
	2	BARU PUMPING STA. GATE	350	120
	3	SARU CONVEYANCE CH.	6574	2768
	Y	BARU CONVEYANCE CH INLET STRUCTURE	150	35
	5	BARU CONVEYANCE CH OUTLET STRUCTURE	106	~20
· · · · ·	6	BANDAR HAR JO EAST SE CO- NDARY CHANNEL	1166	491
	7	BANDARHAR JO EAST SECONDA- RY CHANNEL DUTLET STRUCT.		3/
	8	BARU RETARDING POUD IN - LET STRUCTURE Nº 1		77
	\mathbf{V} .	BARU RETARDING FOND IN- LET STRUCTURE Nº 2		42.
		FUEL TANK 80% FOR BARU PUMPING STATION	/33	62
· · · · ·		TOTAL	9618	4195

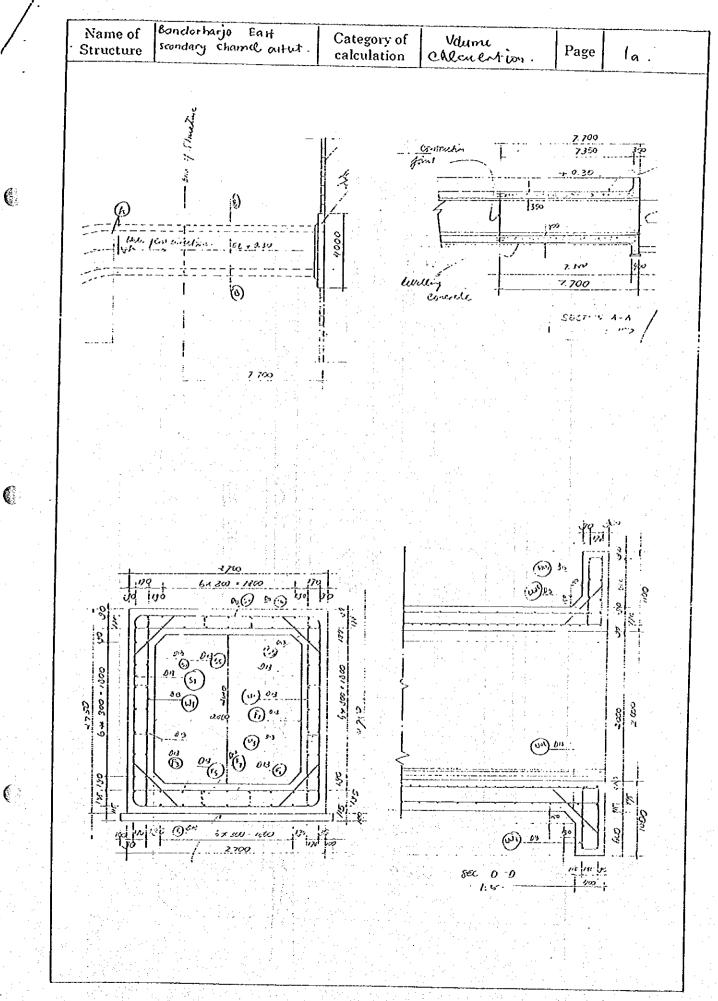
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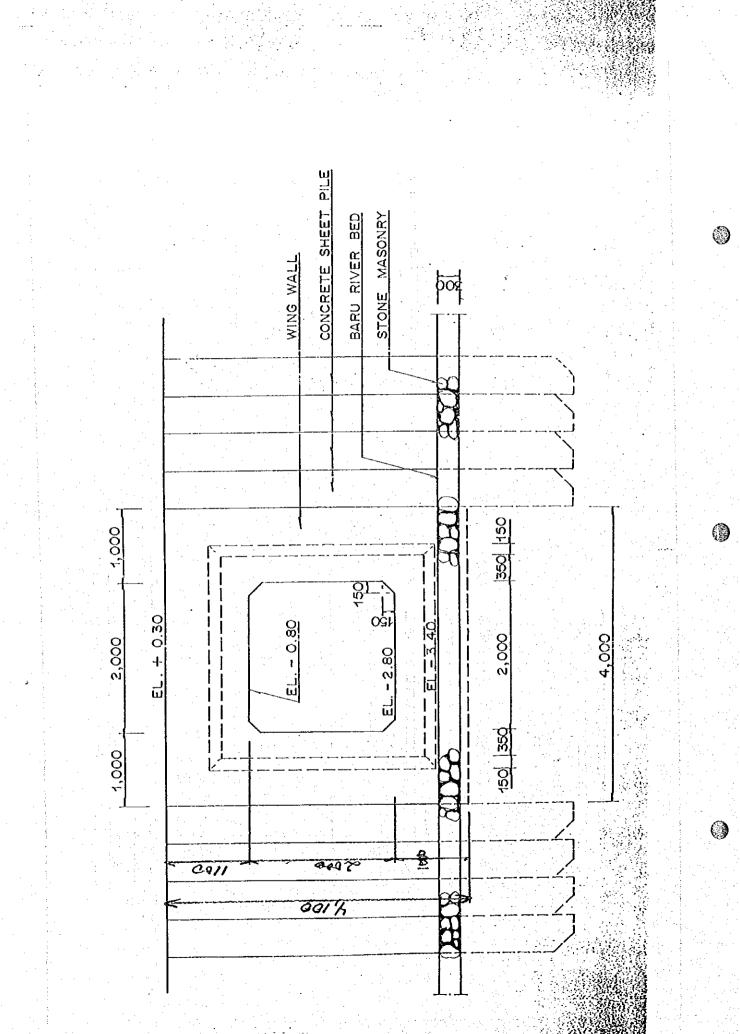
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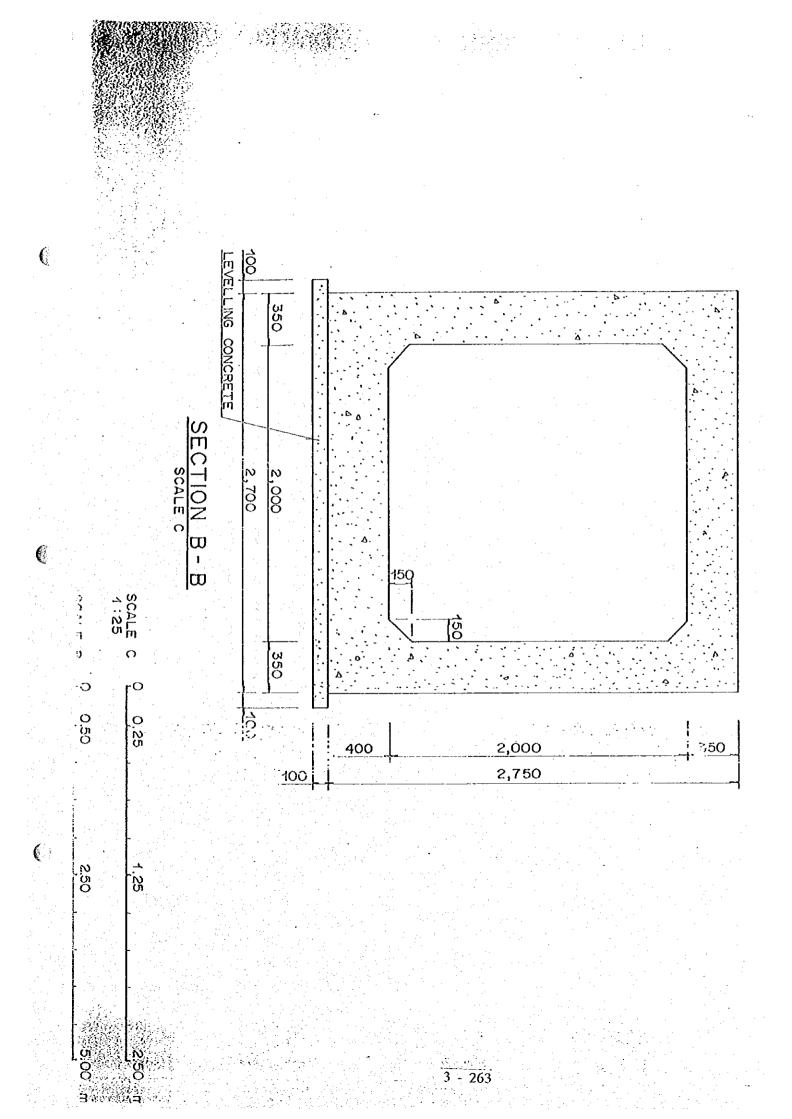
OUTLET BANMARHIIRIO EAST SC SCAPOLDVING AND Name of Category of WORK VOLUME 1 Page FORM SUPPORT Structure calculation 1. SCAFOLDING AREA (Walls of box entorier (1=7.7-0, Y=7.3 m/) (2×2,35+2×2,0)×2,3 = 69.35 Sq.m., Ding Wall (4.1×4.0-2.0×2.0/+(2×0.5×4.1)+(2×0.4×4.10)= 19.78 0 @+10 = 89.13 82 m, 2. FORM SUPPORT AREA 2.0x2.0x7.70 = 30.80 Sym () 3 - 260

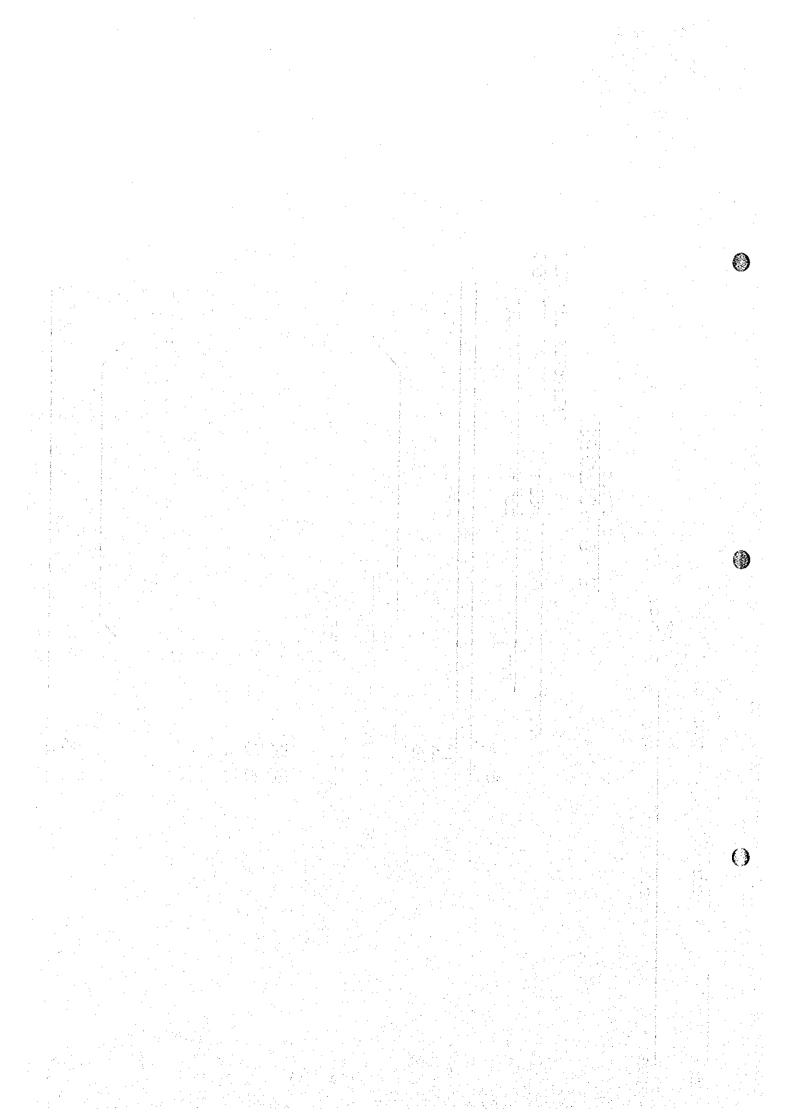


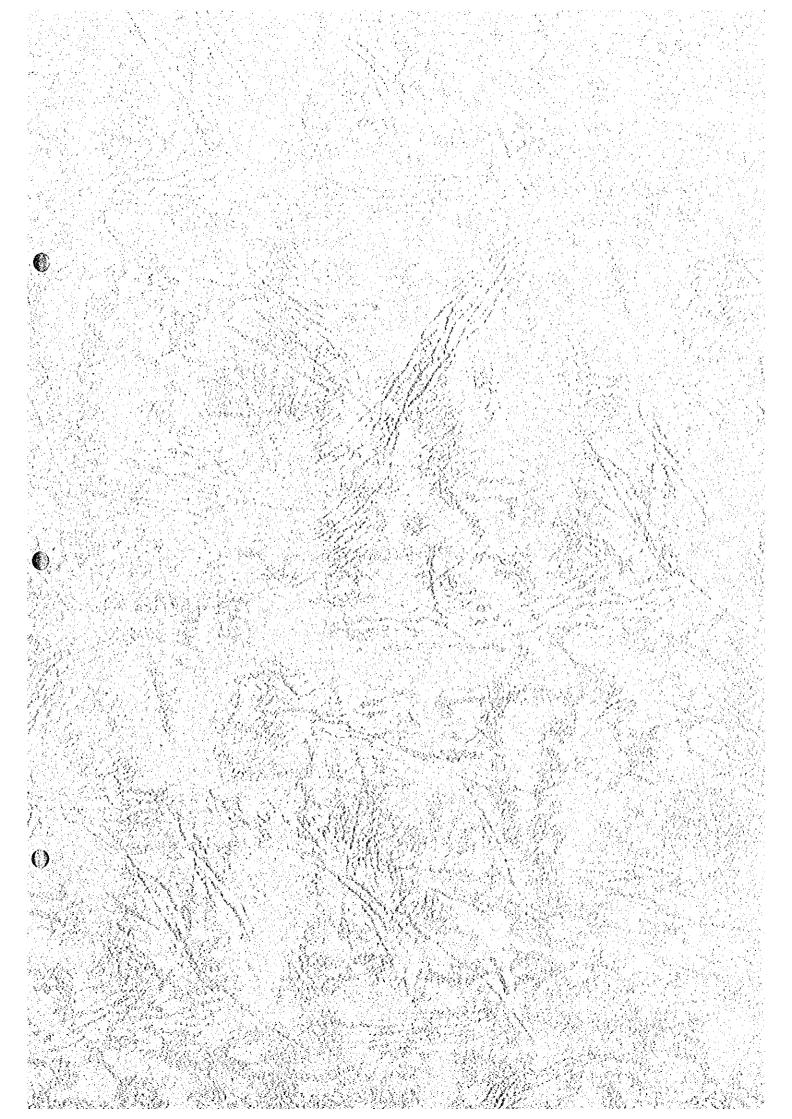
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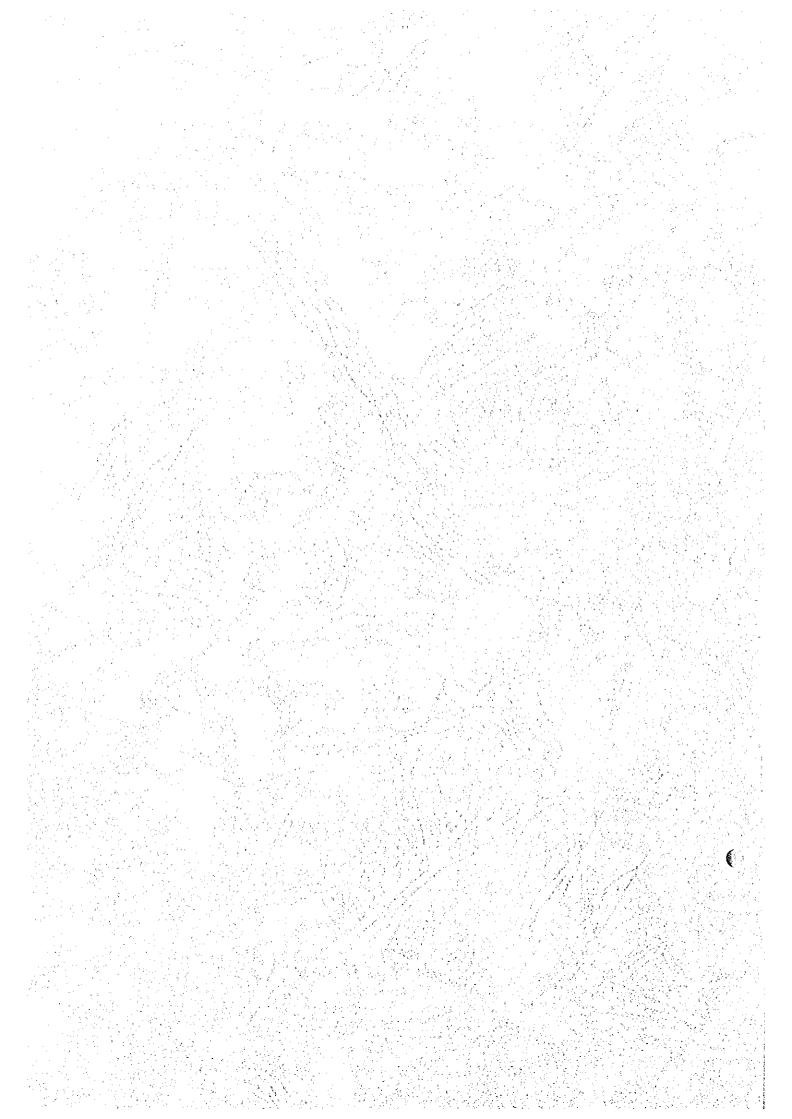


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