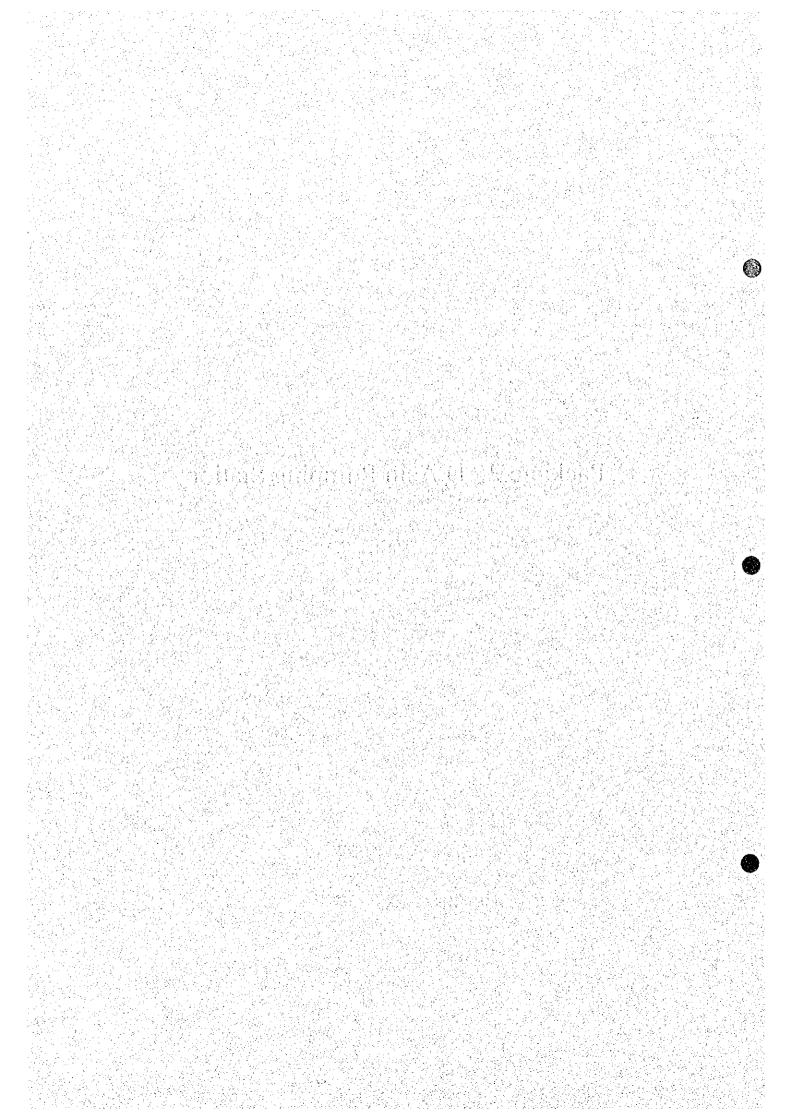
Package 2: D Asin Pumping Station



Name of Structure	ASIN PUMPING STATION	Category Calculation	Work Volume	Page	1/7
	SUMMARY OF ASIN P	UMPING STATIO	ON WORK VOL	UME	
1. STRUC	CTURE EXCAVATION		· · · · · · · · · · · · · · · · · · ·	= 1,264	m³
	CTURE EMBANKMENT	(SANDY SOIL)	1 No. 1	= 913	m³
3. BACK		(ONIND I SOID)		= 104	: m ³
	ING CONCRETE, TYPE			= 66	m m³
i. BB i El	FORM WORK	in the American State of the Control			
5. CONC	RETE FOR STRUCTURE	TVDR C1	e de la companya de La companya de la co	= 17	m ²
J. CONC.	FORM WORK	s, THECT		= 1,120	m ³
6. WET S	TONE MASONRY			= 1,891	m ²
	IONE MASONKI IDARY CONCRETE FOI	B SCBEW (C3)		= 29	m³ 3
A Company of the Company	RETE SHEET PILE (W=5		00)	= 131	m³
		and the second of the second o	υυ) Εναβάλου (1919)	= 484	m'
	SHEET PILE (W=400, L	≔ɔ,ɔ⊍∪) 		= 488	m'
and the secret	RÉTE PILE (Ø 500)			= 3,655	m'
and the second second	N MATTRESS (3,000 ×			= 49	m³
and the second	Y HAND RAIL (TYPE I)			= 1,123	Kg
A	Y HAND RAIL (TYPE II)		= 595	Kg
4.00	ORCING STEEL BAR			= 77,567	Kg
	FOR STRENGTHENING	G OF STOPLOG G	ROVE		
	- 200 - 10)			= 3,533	Kg
and the second second	L BAR (Ø 19, L=1,240)			= 293	Kg
	HOLE Ø 50			= 12	nos
18. POINTI				= 42	m²
Commence of the second	R STOP (W=200, t=20)			= 44	m'
20. SCAFF				= 1,342	m ²
21. FALSE	WORKS			= 941	m ³
		tion of the second of the seco			
					To the con-
				AND THE SECTION	
				entrality of the second of the	
7 4 4					

				A STATE OF THE STA		-	
Name of Structure			Category Calculation	Work Volume	Page	2/7	·
1. \$	STRUCTURE EXCA	VATIO	N				
((a.) Section B-B,	A =	41.625 m ²		i dreft.	•	
			10 m		4		
		٧	$= A \times L = 416.3$	250 m³			
((b.) Section C-C,		88.225 m²			·	
1 -1		L =	7.599 m				•
-1		V	$= A \times L = 670.4$	122 m³	va ⁿ e ⁿ		
(c.) Section D-D,	A ==	55.935+0				
			2	· 图 · 图 · 图 · 图 · 图 · 图 · 图 · 图 · 图 · 图			
- Est			27.967 m ²				
1 - 4 - 4 - 4 - 4 - 4 - 4 - 4 - 4 - 4 -		L =	6.33 m		139.00	Taraba ya 1	•
· (; -			$= A \times L = 177.0$	and the second s			
				$034 \text{ m}^3 \sim 1264 \text{ m}^3$		a texto ix	
2. S	STRUCTURE EMBA		and the second second second	SOIL			
(a.) Section D-D,	A =	$\frac{88.197 + 0}{2}$	e, a Propositive trans.			
		- =	2 44.099 m²				
		ar in the second	2.703 m				
			= A×L = 119.1	98 m³			
(b.) Section E-E,		88.197 m²			the Company of the Co	
		L =	8.998 m				
ing Visit		v	$= A \times L = 793.5$	The state of the s			
		V (a)	+(b) = 912.7	95 m³~913 m³		erinde. Manggapa	
3. B	BACKFILL					10 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	
(;	a.) Section B-B,	A =	4.205 m ²		i Sakata jiga S		
			10 m				
			$= A \times L = 42.0$	50 m ³			
((b.) Section C-C,		6.48 m ²				
			7.599 m				
			$= A \times L = 49.2$	42 m³			
((c.) Section D-D,	$\mathbf{A} = \frac{2}{3}$	$\frac{3.784+0}{2}$				
		=	1.892 m ²				
		L =	6.328 m				
			$= A \times L = 11.9$	73 m³ [+]			
			+(b)+(c) = 103.2				

Name of Structure	ASIN PUMPING STATION	Category Calculation	Work Volume	Page	3/7	7
4. LEV	ELING CONCRETE	ЕТҮРЕ Е	<u> </u>		- J <u></u>	
Volu	$ = 0.10 \times 3 $	35 × 16.00	= 56.00 m ³			
	0.10 × 9	9.50 × 10.00	= 9.50 m ³			
, Settle		num num u からまた History	= 65.50 m ³ ~	66 m³		
5. CON	CRETE FOR STUC	TURE	energy Egyptonis energy	ing samuel North Augus North and Augus		٠.
(a.)	Section B-B (L=10,	000 m)	A Company			
	and the second s	$4 \times 1.0 \times 4.90 \times 10.00$		6.000 m ³		:
	* ;	$4 \times 3.0 \times 0.4 \times 1.0$ $4 \times 2.4 \times 0.4 \times 1.0$		4.800 m ³ 3.840 m ³		
	То	tal for Walls		,	187.360	m³
<u>-</u>		0.8 × 10 × 16	The second secon	8.000 m³		
a section of	the state of the s	$3 \times 1.2 \times 1.0 \times 0.25$ $3 \times 0.65 \times 0.4 \times 4.8$		0.900 m³ 3.744 m³		
		tal for Walls		= -	123.356	m³
· 图图 · 图	TOE =	$\frac{0.4+1.0}{2} \times 0.8 \times (16+1)$	0)		5.040	m³
	estificações de la compansión de la compan					
- 1 4的复数性	Bridge No.1 & 2 =	$3 \times 3.0 \times 0.25 \times 4.8$ $3 \times 2.4 \times 0.25 \times 4.8$		0.800 m³ 7.740 m³		
				=	18.540	m³
J	Total (a) Section B-B			10 to	343.816	m³
(b.)	Section C-C (L=7,5	59 m)	TO SEE SEE SEE SEE		ne New York	
		$4 \times 1.0 \times 6.40 \times 7.599$		4.534 m³		
		4 × 3.0 × 0.4 × 1.0 tal for Walls	= -	4.800 m³ =	189.734	m ³
		1.3 × 16 × 7.599	/	8.059 m³	105.751	,,,
		$3 \times 1.4 \times 0.3$		1.260 m³		
	and the control of th	tal for Walls			156.799	m³
	TOE =	$\frac{0.4+1.0}{2}$ × 7.599		= 1	5.319	m^3
	Bridge No.1 & 2 =	2 3 × 3 0 × 0 4 × 4 80			17.280	n13
	Fotal (b) Section C-C				179.398	
					177.570	111
(c.)	Section D-D (L ₄ =					
		9.727 m)		richer Programme Programme	100 410	
		$4 \times 1.0 \times 5.427 \times 8.403$ $0.924 \times 16.0 \times 8.403$			182.412 124.230	
	TOE	$\frac{0.4+1.0}{2} \times 0.8 \times 9.727$			5.447	
		 2				
	Fotal (c) Section D-D		A CONTRACTOR OF STREET	: =	323.089	m,

Name of Structure	ASIN PUMPING STATION	Category Calculation	Work Volume	Page	4/7
(d.)	Section E-E (L=2,00 m)) ,	e e e e e e e e e e e e e e e e e e e		
-		$1.0 \times 4.80 \times 2.00$	a La santa de la composição	28.800 m ³	•
		25 × 4.8 × 2.0 For Walls		11.760 m ³	40.560 m³
	Bottom Slab = 1.3	× 16.225 × 2.0 × 16.225 × 2.0			16.225 m ³ 42.185 m ³
-	TOE $= \frac{0.4}{}$	$\frac{+1.0}{2} \times 0.8 \times (16+1)$	0)		0.560 m ³
1	otal (d) Section E-E			==	99.530 m³
(e.)	Section F-F (L=7,00 m)				
-		$1.0 \times 1.90 \times 7.0$ $5 \times 1.90 \times 7.0$		39.900 m ³ 16.293 m ³	
		or Walls	Militaria (T.) Malarat, ita	10.293 III ²	56.193 m³
		× 16.225 × 7.0		, =	56.788 m³
See All and		$\frac{+1.0}{2} \times 0.8 \times (7+16)$.225)	; =	13.006 m ³
Т	otal (e) Section F-F		e i se e di jira di. Bilinga i e di jira di		125.987 m ³
and the second of	Deck Slab = $0.5 \times 10 \times 9$				47.500 m ³
Till the state of	otal (a)+(b)+(c)+(d)+(e)	+(f)	in the state of th	= 11	[19.320 m ³]
6. WET	STONE MASONRY				
V =	$\frac{0.3+1.15}{2} \times 1.9 \times (10+9)$	5)	= 28.714	m³	
7. SECC	NDARY CONCRETE F	OR SCREW			
V = {	$3\times4\times2.0-\left(\frac{1}{2}\times\pi\times1.5^2\right)$	×3)}×9.727	= 130.314	m³	
8. CON	CRETE SHEET PILE (W	V = 500, $t = 220$, $L = 20$	= 13,000)		
V =	$28 + \frac{16}{0.4} \times 8$		= 484	m'	
9. STEE	L SHEET PILE (W = 40	0; t = 5,300)			
(52 +	$\left(\frac{16}{0.4}\right) \times 5.3$		= 488	m'	
10. CON	CRETE PILE Ø500				
a. Sec	tion B-B : $9 \times 6 \times 21.6$		= 1,166.4	m'	
1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1	tion C-C : $10 \times 5 \times 20$		= 1,000.0	Z - 4	
c. Sec	tion D-D : (6×21.9)+(6×	×23.6)+(6×25.2)	= 424.2		

Name of Structure	ASIN PUMPING STATION	Category Calculation	Work Volume	Page	5/7
d. Se	ction E-E : (4×7×25.8)	+(3×4×28.5)	= 1,064.4	<u>m'</u>	
Total	Length of Concrete Pile	Ø 500	= 3,655.0	m'	
11. GAB	SION MATTRESS (3,000	0 × 1,500 × 500)			11
V =	$2\times16.225\times3\times0.5$		= 48.675	m³	
12. FOR	M WORK	•			٠.
	Section B-B (L=10 m)		Aby a substitute of the second)7 241 m²
	$0 \times (5.7 + 0.8) + 10 \times 5.7$		6 .)).241 III
>	$\sqrt{0.15^2 + 0.15^2} + 10 \times \sqrt{0.4 + 4.9 \times 6 \times 0.65 \times 4.9}$ + $\frac{1}{12}\pi \times 0.5 \times 4.9 = 507.2$	$+3\times\pi\times0.5\times4.9$		Asur [©]	
b. S	ection C-C (L=7.599 m)			= 43	31.374 m ²
	$7.599 \times (7.7 + 0.8) + 7.599$	<u>an i</u> an ing pakalahan ing Pa	电光电阻 医多色性 化二氯化二甲基甲基甲基甲基甲基甲基甲基甲基甲基甲基甲基甲基甲基甲基甲基甲基甲基甲基甲基		
	$+7.599 \times 6 \times \sqrt{0.15^2 + 0.1}$				
	ection D-D (L=8.403 m)		A READ THE REST		3.386 m ²
1.0	$3.403 \times (6.351 + 0.8) + 8.4$ $-8.403 \times \sqrt{0.4^2 + 0.8^2} + 3$	Applications of the application of the			
	ection E-E (L=2.0 m)				16.480 m²
	$0.0 \times (5.6 + 0.8) + 2.0 \times 5.6$	5+2.0×4.3×6+2.	0×4.0×3		70.400 III
	$4\times3\times2.0\times\sqrt{0.15^2+0}.$	<u> </u>	ささだけ あたば ほうしゃ	m^2	•
e. S	ection F-F (L=6.403 m)			= 31	7.488 m²
6	$0.403 \times (3.2 + 0.8) + 6.403$	×3.2 + 6.403×1.9	× 6		
+	$-6.403 \times 4.0 \times 3 + 4 \times 3 \times 6$	$5.403 \times \sqrt{0.15^2 + 0.}$	15 ²	yara Mara	
+	$-6.403 \times \sqrt{0.4^2 + 0.8^2} =$	317.488 m²			
f S	lab (9 × 10 m)				
<u>2</u>	$\times 10 \times 0.5 + 9.5 \times 0.5$			=	14.75 m ²
1	otal Form Work (a)+(t	o)+(c)+(d)+(e)+(f)		= 1,89	0.719 m ²
13. DOW	EL BAR				
$\left\{ \left(\frac{16}{0.5} \right) \right\}$	$\left(\frac{9.5}{5}+1\right)+\left(\frac{9.5}{0.5}+1\right)\times 2\times 1$.24×2.23	= 293.11 kg	,	
14. WEE	P HOLE				
$\left(\frac{21.5}{2.0}\right)$	$\left(\frac{3}{1}+1\right)$ nos		= 12.00 no	os	

		and the state of t	ور سور ور و	
Name of Structure	ASIN PUMPING STATION	Category Calculation	Work Volume	Page 6/7
15. POIN	TING			Construction of the
21.8	$\times \sqrt{1.7^2 + 0.85^2}$		= 41.43 m	2
16. FOR	M WORK (For Concrete	Type E)		n dagaman ang ta
2 × 0.	1 × (16 + 35 + 16) + 0.1	× 35	= 16.9 m	2
17. SAFI	ETY HANDRAIL			
(Ø42.	7 mm; $t = 2.3 mm$; weight	ght = 2.29 kg/m)	-	
(Ø60.	5 mm; $t = 4.0 \text{ mm}$; weig	ght = 5.57 kg/m)		to taking by all the second
(a.)	Туре І	$= \left(\frac{1}{2} \right) \right) \right) \right) \right)}{1} \right) \right)}{1} \right)} \right)} \right)} \right)} \right)} \right)} \right)} \right)} \right)}} \right)}}}}}}}}$		
	- Handrail Pipe			to the second
	L=6.6+16+4×3×4.0+;	3×4.0+7×5.15	= 118.65 m	
	Weight of handrail p	ipe = (2.29 + 5.57)	< 118.65 = 932.59 k	(1.20 t) (1.20 t) (g
Paga Alda (se). Alda Alda (se).	- Column Pipe			
	$L=0.8\times \left\{ \left(\frac{6.6}{1.66} + 1 \right) + \right.$	$\left(\frac{16}{1.66}+1\right)+\left(4\times3\right)$	(4)+(4×3)+(4×7)} = 83.20 m
	Weight of handrail p	ipe = 83.20 × 2.29 =	= 190.53 kg	
	Total Weight of Typ			gg trata
(b.)	Гуре II	en e		
	· Handrail Pipe			
	$L = 8 \times 8.3 = 66.4 \text{ m}$			
	Weight of handrail pi	ipe = (2.29 + 5.57)	c 66.4 = 521.90 kg	
	Column Pipe			
i de Salatió La	$L = 0.8 \times 8 \times 5 = 32 \text{ m}$	n		
	Weight of handrail pi	$pe = 32 \times 2.29 = 73$	3.28 kg	
	Total Weight of Typ	e II = 595.18 kg		
18. WAT	ER STOP		(18) 11 人名斯内特斯斯 12: 13: 13: 13: 13: 13: 13: 13: 13: 13: 13	
16+4	\times 7.05 = 44.2 m ²			
19. ANCI	IOR BAR			
L = 30	00; D = 19 mm; Weight =	= 2 23 ko/m		
	$N = 4 \times 10$		= 120 nos	
	3×10>		= 180 nos	1. 在建筑的。
	3×10>		= 90 nos	
	3×3×	and the second	= 27 nos	
	Total		$= \frac{27 \text{ nos}}{417 \text{ nos}}$	

Name of Structure		PUMPING ATION	Category Calculation	Work Volume	Page	7/7
20. Aì	NCHOR PAI)				
					er Strand (1997) Geografia	
			× 1.05 × 2.23	= 140.49 kg		
L:	= 650; D = 19	9 mm	1 1	CHARLES		
, N	$= 10 \times 6 = 60$	$nos \rightarrow W = 60$	\times 0.65 \times 2.23	= 86.97 kg		
	a da firma da	Total of we	ight	= 227.46 kg		:
21 RF	INFORCING	GRAR		Balantan mengerak		
21. K		JDAK				i. i
а. 3	Structure rein	nforcing		= 77,567	kg	
Ь.	Anchor Bar		$(1+\varepsilon)_{i\in\Omega}\Omega^{i}$	= 278	kg	
с.	Anchor Pad		ing distribution of the state o	= 228	<u>kg</u>	
То	tal of Steel R	einforcing		= 78,073	kg	
					A Company	
					ty seed for a	
e grande e Grande						
						٠.
					•	
						• •
1					±	
			A DESTRUCTION OF THE SECOND SE			
	The second second second second		and the second of the second of the second	the contract of the contract o		

Name of STATION, FOR STATION, FOR SCAFOLDING AND FORM SUPPORT Calculation FORM SUPPORT Work Volume Page 1/2

SUMMARY OF SCAFOLDING AND FORM SUPPORT VOLUME FOR ASIN RIVER DRAINAGE SYSTEM IMPROVEMENT.

		Vo	olume
No	Structure	Scafolding (m²)	Form Support (m³)
1	Asin Pumping Station	1342	941
2	Asin Pumping Station, Gate	732	254
3	Asin Box Culvert	1843	1358
4	Asin Box Culvert Inlet Structure	54	11.5
5	Asin Box Culvert Outlet Structure	62	16
6	Secondary Channel Outlet on Asin River		34
7	Fuel Tank Box for Asin Pumping Station	133	62
8	Asin Pumping Station Bridge, Sub Structure	166	-
9	Asin No.1 Bridge, Sub Structure	293	10 10 10 10 10 10 10 10 10 10 10 10 10 1
10	Asin No.2 Bridge, Sub Structure	251	
	Total	4876	2676

Name of Structure	ASIN PUMPING STATION, FOR SCAFOLDING AND FORM SUPPORT	Category Calculation	Work Volume	Page	2/2
1. SC	CAFFOLDING				<u>. </u>
(a)	Section A - A (L=10.0 m)		Carlo Specific		
	$(6.5 + 5.7 + 6 \times 4.9) \times 10.0$		= (416.00 m²	
	$\left\{ \left(\frac{1}{2} \times 3 \times \pi \times \frac{1.0}{2} \right) + \left(\frac{1}{4} \times $	$\pi \times \frac{1.0}{2}$ $\times 4.9$	<u></u>	13.47 m²	
n .	Total (a)		egy of Marchael	= 4	 129.47 m²
(b)	Section C - C (L=7.599 m)		•	•	.:
	$(8.5 + 7.7 + 6 \times 6.4) \times 7.599$		•	= 4	14.905 m²
(c)	Section D - D (L=7.901 m)				
	$(7.151 + 6.351 + 6 \times 5.427)$	× 7.901		= 30	53.952 m²
(d)	Section E – E (L=2.0 m)				
	$(6.4 + 5.6 + 6 \times 4.3) \times 2.0$			==	75.60 m ²
(e)	Section F - F (L=7.0 m)				
	(4.0 + 3.2)× 7	•	= (50.400 m²	
	$\left\{ \left(\frac{1}{2} \times \pi \times \frac{1.225}{2} \right) + \left(\frac{1}{2} \times 2 \times 2 \times 2 \right) \right\}$	$\pi \times \frac{1.0}{2} + \left(\frac{1}{2} \times \pi \right)$	$\times \frac{1.0}{2}$ $\times 2.4 =$	7.021 m²	
•	Total (e)			=	57.421 m
	Total (a)+(b)+(c)+(d)+(e)			=1,3	41.348 m
2. SU	JPPORT AREA			1 1	
(a)	Section B - B & C - C				
	$(3 \times 4.0 \times 3.0 \times 4.9) + (3 \times 4)$	$.0 \times 5.4 \times 6.4)$	= 591.12	m^3	
(b)	Section E – E & F – F				

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(b) Section E – E & F – F $(3 \times 4.0 \times 4.8 \times 3.298) + 3 \times 4.0 \times 1.9 \times 7.0)$ = 349.56 m³ Total (a)+(b) = 940.68 m³ Name of Structure INSPECTION BRIDGE Category Calculation Work Volume Page 1/3

SUMMARY OF WORK VOLUME

1. CONCRETE, TYPE B

= 54.38 m³

2. REINFORCING BAR, U 30

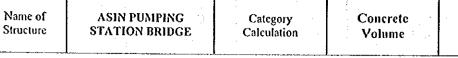
= 2,968 Kg

Name of Structure	INSPECTION BRIDGE	Category Calculation	Work Volume	Page	2/3
D.7 Maint	tenance Bridge		100	1.	1.00
	rete (B)				ville en la companya de la companya
N	<u>0.1</u> No.2	•	*	. :	
		$\supset \gamma$			
$\left \begin{array}{c} \\ \end{array} \right $					
			Pumping Station		
		Asin	r umping Station		
		>			
(B_1)					
		$\sum_{i} f_i$			
A					
					4.
A	sin Pumping Station Ga	nte			
Bridge of No	<u>.1</u>				
(B_1) $0.4+$	$\frac{0.65}{} \times 3.00 \times 5.50$		= 8	.66	
\bigcirc v. \land	3.00 × 5.00		= 6	.00	
	3.00 × 5.50		= 6	.60	
Bridge of No				20	
	$3.00 \times 5.50 \times 2$ 3.00×5.00			.20	
\cup			— D	.00	
	in Pump Station Gate				
	$3.00\times5.80\times2$.92	
Volume D.7.1			= 54	.38 m³	

Name of Structure	INSPECTION BRIDGE	Category Calculation	Work Volume	Page	3/3
D.7.2 Rein	forcing Bar (SII U 30)				
Bridge of N	0.1	e e			
(B_1) (B_2) (B_2)	2.89 + 84) × 2		12-43 13-15 14-15	746	
(C) (Z	262 + 78) × 1			340	
Bridge of N	0.2				
$\left(\mathrm{B_2}\right)$ (289	+84) × 2		=	746	
(C) (262	+ 78) × 1	a kilony i k	=	340	
Bridge of A	sin Pump Station Gate				
(A) (305	+93)×2		<u> </u>	796	
Volume D.7.	1		_	2,968 kg	

Name of Structure	ASIN PUMPING STATION BRIDGE	Category Calculation	Concrete Volume	Page	1/5
				i satur karini.	
		MARY OF QUAN SUPER STRUC			
	<u> </u>	OUBROING	LUKE	* · · · · · · · · · · · · · · · · · · ·	
1. S	TRUCTURE CONCRETE	K400	· 	33.32 m ³	
	TRUCTURE CONCRETE		——————————————————————————————————————	36.60 m ³	
	EINFORCING STEEL	K230	===	10.486 m ³	11
	C CABLE K1 Ø12.7 7 STF	RANDS =	385.0 kg	10.700 m	
	C CABLE K2 Ø12.7 12 ST		660.0 kg		
	C CABLE K1 Ø12.7 7 STF	the second	386.0 kg		ใหม่ อได้ เกม
	C CABLE MONO STRANI		22.5 kg		
	AL DESCRIPTION OF THE PROPERTY	TOTAL		1453.5 kg	.*
5. B	RIDGE RAILLING	en e	: =	43.60 m	
6. E	XPANSION JOINT			10.88 m	
7. B	EARING SHOE AND RUE	BBER SHEET		6.00 piec	es
8. P	VC DRAINAGE PIPE Ø10	cm	=	10.50 m'	
9. A	SPHALT PAVEMENT AC	ON TOP OF SLA	B =	6.81 m ³	
10. F	ORM WORK	as I temperation		456.45 m ²	
	and the second s	Tigar Salahiri Pilipi			

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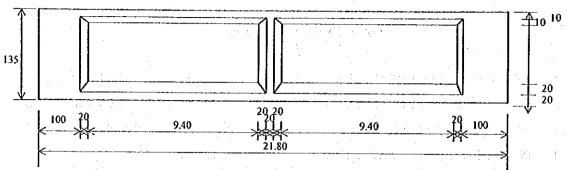


Wide

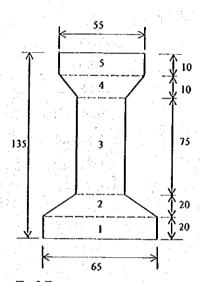
Page

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1. Main Girder K400



Center Beam



$$A_{1} = 0.65 \times 0.20 = 0.1300 \text{ m}^{2}$$

$$A_{2} = \frac{0.20 + 0.65}{2} \times 0.20 = 0.0850 \text{ m}^{2}$$

$$A_{3} = 0.20 \times 0.75 = 0.1500 \text{ m}^{2}$$

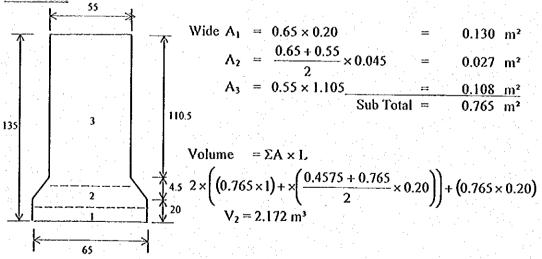
$$A_{4} = \frac{0.55 + 0.20}{2} \times 0.10 = 0.0375 \text{ m}^{2}$$

$$A_{5} = 0.55 \times 0.10 = 0.130 \text{ m}^{2}$$

$$\text{Sub Total} = 0.4575 \text{ m}^{2}$$

Length of Center Beam = 18.80 m Volume : $\Sigma A \times L$ $V_1: 0.4575 \times 18.80 = 8.601 \text{ m}^3$

End Beam



- Volume Total one beam = $V_1 + V_2$ $V = 8.601 + 2.172 = 10.773 \text{ m}^3$
- Total Volume for all Beam in Bridge (V_G) $V_G = 3 \times \Sigma V = 3 \times 10.003 = 32.319 \text{ m}^3$

Name of Structure	ASIN PUMPING STATION BRIDGE SUB STRUCTURE	Category Calculation	Form Work Volume	Page	1/5
	SUM	MARY OF WOR	K VOLUME		·
1.	Structural Excavation	grant to a silver	ing a gas page.	302	m^3
2.	Backfill		and the second second	265	m³
3.	PC Pile			900	m
4.	Concrete Type C ₁			193	m³
5.	Form Work for C ₁		na ang kabangan sa Salah sa S Salah sa Salah sa Sa	271	m²
6.	Concrete Type E		# 14 - 원양론 :	1 1142 9	m³
7.]	Form Work for E			4	m²
8.	Deformed Reinforcing B	ar		13782	kg
					X.
				e Magaza	
			en in State en de Communication		
				ing page	

	ASIN PUMPING)			
Name of Structure	STATION BRIDGE, FOR SCAFOLDING AND FORM SUPPORT	Category Calculation	Work Volume	Page	1/2

SUMMARY OF SCAFOLDING AND FORM SUPPORT VOLUME FOR ASIN RIVER DRAINAGE SYSTEM IMPROVEMENT.

-		Volume		
No	Structure	Scafolding (m²)	Form Support (m³)	
1	Asin Pumping Station	1342	941	
2	Asin Pumping Station, Gate	732	254	
3	Asin Box Culvert	1843	1358	
4	Asin Box Culvert Inlet Structure	54	11	
5	Asin Box Culvert Outlet Structure	62	1647	
6	Secondary Channel Outlet on Asin River		. 4, 34	
7	Fuel Tank Box for Asin Pumping Station	133	62	
8	Asin Pumping Station Bridge, Sub Structure	166	· · · · · ·	
9	Asin No.1 Bridge, Sub Structure	293	<u>-</u>	
10	Asin No.2 Bridge, Sub Structure	251		
	Total	4876	2676	

Name of Structure	ASIN PUMPING STATION BRIDGE, FOR SCAFOLDING AND FORM SUPPORT	Category Calculation	Work Volume	Page	2/2
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ZELTERA ENGLA EL ESPECIA DE LA

1. Scafolding Area

$$(2 \times 5.9 \times 5.6) + (2 \times 5.9 \times 1.43) = 82.954 \text{ m}^2$$

For both side abutment = $2 \times 82.954 = 165.908 \text{ m}^2$

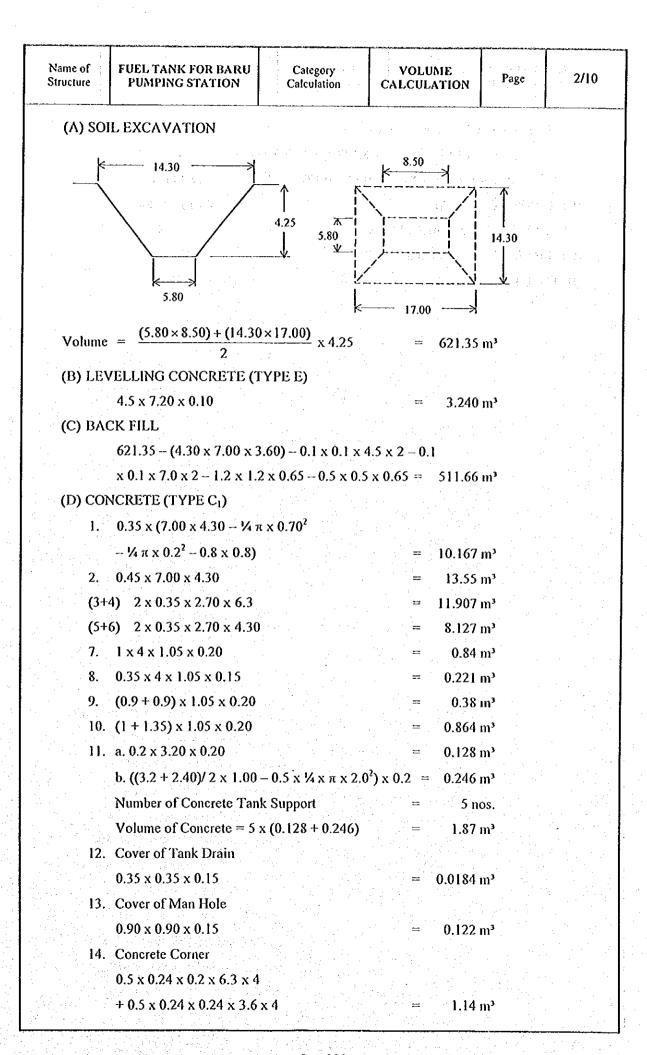
2. Form Support Area

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Name of Structure	FUEL TANK FOR ASIN PUMPING STATION	Category Calculation	VOLUME CALCULATION	Page	1/10
Structure	PUMPING STATION	Calculation	CALCULATION		

SUMMARY OF WORK VOLUME

1.	SOIL EXCAVATION	=	621.35 m ³
2.	BACK FILL	==	511.66 m ³
3.	LEVELING CONCRETE, TYPE E	=	3.24 m ³
4.	CONCRETE FOR STRUCTURE, TYPE CI	==	49.21 m ³
5.	REINFORCING BAR	· ==	6,810.034 Kg
6.	FORM WORK FOR CONCRETE, TYPE C1	=	$166.69 \mathrm{m}^2$
7	FORM WORK FOR CONCRETE TYPE F		3 m ²



Name of Structure	FUEL TANK FOR BARU PUMPING STATION	Category Calculation	VOLUME CALCULATION	Page	3/10
To	l	C.			
10	10.167 + 13.55 + 11.907				N 14 1
		And the second second			
	+ 0.38 + 0.864 + 1.87 + (J.U184 + U.122 +			
	INFORCING BAR		= 6,810.0		
	EL TANK AND ACCESSO	ORY	= 1	Set	
(G) GR	OUNDING BC 50 mm		= 1	Set	
(H) FU	EL PIPE SET		=	Set	
			The English		
			1.79年基本企	PARITIES.	
				Mataa	M (. G
				- 34,445 - 34,445	
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	43 122				
	The state of the s				
				and the second of	
	·····································				
			in a place of the		

Name of Structure	FUEL TANK FOR BARU PUMPING STATION	Category Calculation	FORM WO VOLUMI	RK p	Page 10
FO	RM WORK				
(A)) Wall Slab				
	2 x 3.5 x 4.30 + 2 x 3.5 x	7.0	=	79.10 m²	
	2 x 2.4 x 3.30 + 2 x 2.4 x	6.0	=	44.64 m²	
: '	Total A		= 1	23.74 m²	
(B)	Top Slab				
	3.30 x 6.0 - 2 x 0.8 x 0.8 ·	- 0.20 x 0.20	21	18.48 m²	
(C)	Fuel Pipe Hole		n de la companya de Companya de la companya de la compa		
	4 x 1.20 x 0.75 + 4 x 0.8 x			6.72 m²	
(D)	Man Hole			gyra a lia, a	
	4 x 1.20 x 0.85 + 4 x 0.8 x	$\times 0.75 \pm 4 \times 1.0 \times$	0.10 = = 1	6.88 m ²	
(E)	Main Hole				
	$4 \times 0.50 \times 0.85 + 4 \times 0.20$	$x 1.10 + 4 \times 0.35$	x 0.10 =	2.72 m²	
(F)	Slope Concrete in the Cor	ner			
	$(\sqrt{0.15^2+0.15^2})$ x (4 x 6	.15 + 4 x 3.45)	=	8.15 m ²	
Tot	al Form Work		= 1	66.69 m²	

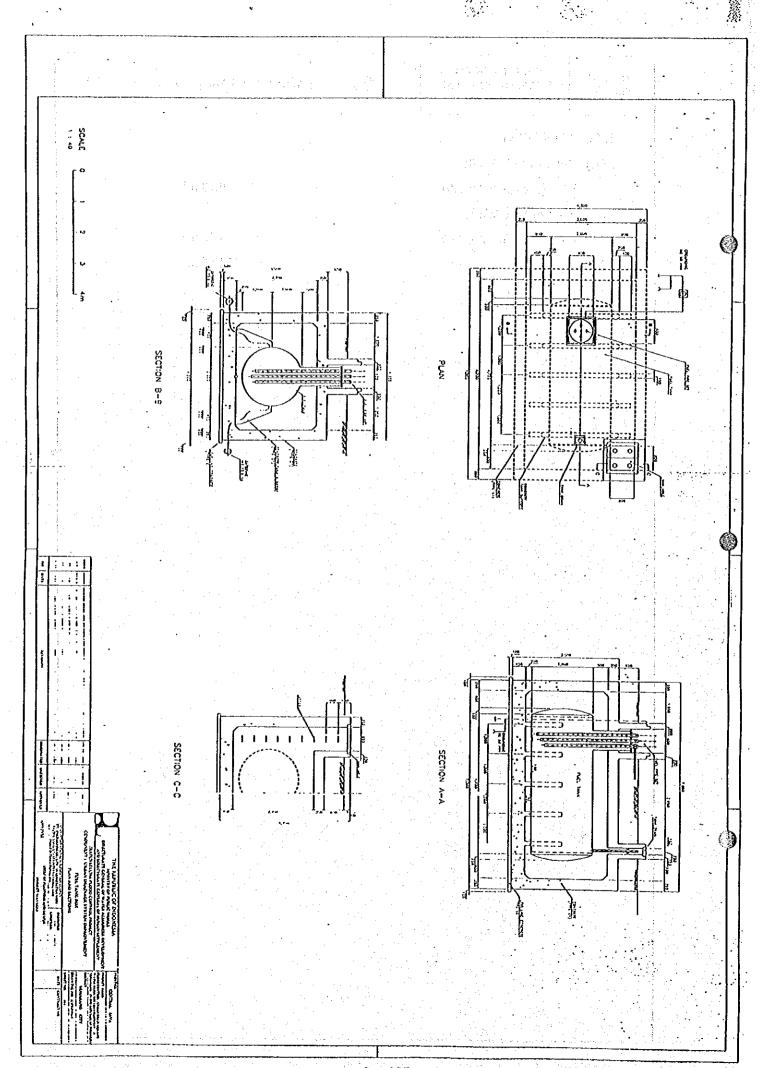
Name of SCAFOLDING AND FORM SUPPORT, FOR Structure ASIN DRAINAGE SYSTEM

SCAFOLDING AND Category WORK VOLUME Page 1/1

SUMMARY OF SCAFOLDING AND FORM SUPPORT VOLUME, FOR ASIN DRAINAGE SYSTEM

No.	VOLUME STRUCTURE	SCAFFOLDING (m²)	FORM SUPPORT (m³)
1	ASIN PUMPING STATION	1342	941
2	ASIN PUMPING STA. GATE	732	254
3	ASIN BOX CULVERT	1843	1358
4	ASIN BOX CULVERT INLET STRUCTURE	541.97 () + 1.97 () + 54	11
5	ASIN BOX CULVERT OUTLET STRUCTURE	(a et) 62	
6	SECONDARY CHANNEL OUTLET STRUCTURE ON ASIN RIVER		34
7	FUEL TANK BOX FOR ASIN PUMPING STATION	133	62
8	ASIN PUMPING STATION BRIDGE, SUB STRUCTURE	166	
9	ASIN NO. 1 BRIDGE, SUB STRUCTURE	293	
10	ASIN NO. 1 BRIDGE, SUB STRUCTURE	251	
	TOTAL	4625	2676

Name of Structure	FUEL TANK BOX SCAFFOLDING AND FORM SUPPORT	Category Calculation	WORK VOLUME	Page	1/1
1. SCAF	FOLDING				
(A) O	uter of Side Walls				
	5 x (2 x 4.3 + 2 x 7.0)		= 79.10	m²	
	ner of Side Walls		•		
	7 x (2 x 6.3 + 2 x 3.6)		= 53.46	m²	
Total /			= 132.56	m²	
2. SUPP	ORT AREA				
6.	3 x 3.6 x 2.7		= 61.24	m³	
				ra .	
				. •	
and the second					

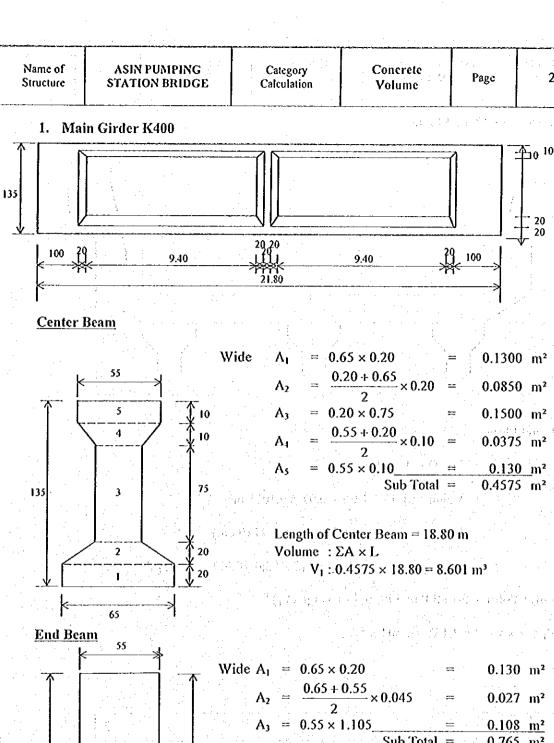


Name of Structure	FUEL TANK BOX SCAFFOLDING AND FORM SUPPORT	Category Calculation	WORK VOLUME	Page	1/1
1. SCAI	FOLDING		•	L	
(A) O	uter of Side Walls	· · · · · · · · · · · · · · · · · · ·			
	5 x (2 x 4.3 + 2 x 7.0)		= 79.10	m²	
(B) In	ner of Side Walls	i.		gradient gebook	,
2.	$7 \times (2 \times 6.3 + 2 \times 3.6)$		53.46	m² - 5 - 11 ()	
Total A	\+B		= 132.56	m²	: .
2. SUPP	ORT AREA		Supplied to the supplied of		
	3 x 3.6 x 2.7		= 61.24	m ³	
			en geraal en de		ja ja
				9 11 - 37 3 <u>11</u>	
				4.7	
					4.
		•			
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	ta mendên ew li birin. De li birin û birinê birinê				
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Name of Structure	ASIN PUMPING STATION BRIDGE	Category Calculation	Concrete Volume	Page	1/5
				36 (4)	·

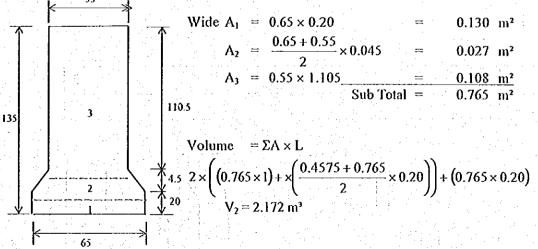
SUMMARY OF QUANTITIES OF SUPER STRUCTURE

1.	STRUCTURE CONCRETE K400		=	33.32 m ³
2.	STRUCTURE CONCRETE K250		: = ; `	36.60 m ³
3.	REINFORCING STEEL		=	10.486 m ³
4.	PC CABLE K1 Ø12.7 7 STRANDS ==	385.0	kg	en de la companie de La companie de la co
	PC CABLE K2 Ø12.7 12 STRANDS =	660.0	kg	
	PC CABLE K1 Ø12.7 7 STRANDS =	386.0	kg	e familie de la company
	PC CABLE MONO STRAND CABLE/FS =	22.5	kg	
	TOTAL		=	1453.5 kg
5.	BRIDGE RAILLING		==	43.60 m
6.	EXPANSION JOINT		==	10.88 m
7.	BEARING SHOE AND RUBBER SHEET		=	6.00 pieces
8.	PVC DRAINAGE PIPE Ø10 cm		:	10.50 m'
9.	ASPHALT PAVEMENT AC ON TOP OF SLAB		=	6.81 m ³
10.	FORM WORK		<u>.</u>	456,45 m ²

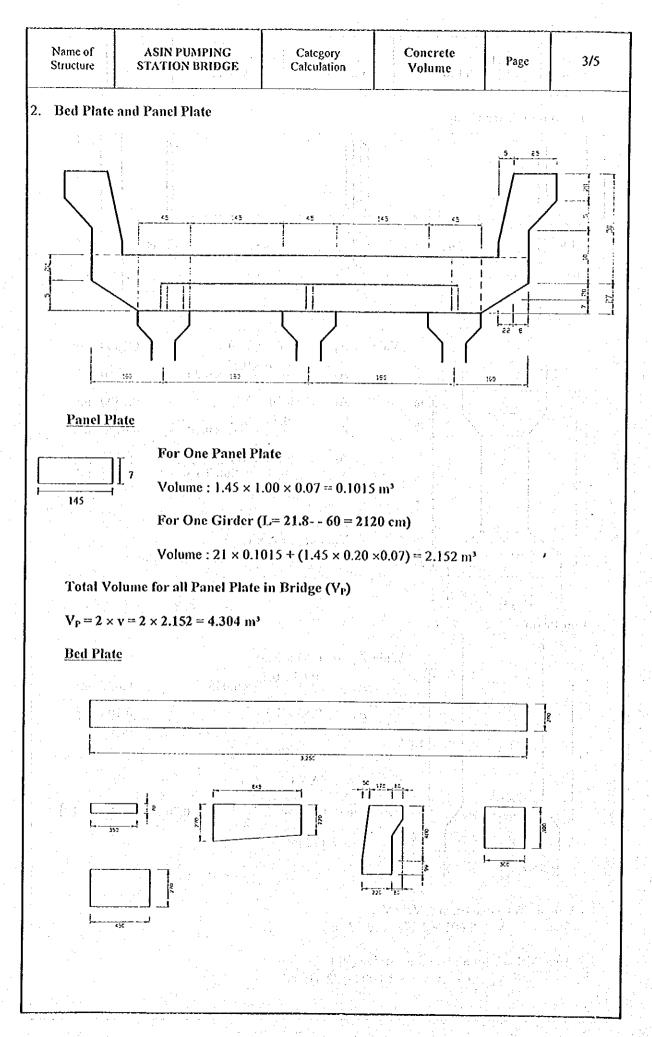


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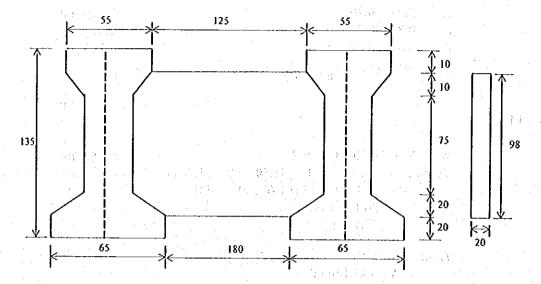
- Volume Total one beam = $V_1 + V_2$ $V = 8.601 + 2.172 = 10.773 \text{ m}^3$
- Total Volume for all Beam in Bridge (V_G) $V_G = 3 \times \Sigma V = 3 \times 10.003 = 32.319 \text{ m}^3$



$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	4/5		Page	Concrete Volume	Category Calculation	ASIN PUMPING STATION BRIDGE	Name of Structure
$\begin{array}{cccccccccccccccccccccccccccccccccccc$				New York			Wide
$\begin{array}{cccccccccccccccccccccccccccccccccccc$		m²	0.650	=		$A_1 = 3.25 \times 0.20$	
End Plate Wide $A = 0.30 \times 0.30 \times 2$ Volume $= \Sigma A \times L \rightarrow L = 21.80 - 60 = 21.20 \text{ m}$ $= (1.234 \times 21.20) + (0.180 - 5.44)$ $= 26.161 + 0.979$ V $= 27.140 \text{ m}^3$ Total Volume Slab in Bridge (Vs) $V_g = 27.140 \text{ m}^3$ $A_1 = \frac{0.22 \times 0.10 \times 2}{2} = 0.0044 \text{ m}^2$ $A_2 = \frac{0.17 \times 0.22}{2} \times 0.40 \times 2 = 0.156 \text{ m}^2$ $A_3 = \frac{0.20 + 0.25}{2} \times 0.08 \times 2 = 0.036 \text{ m}^2$ Concrete Volume Handrail: $= \Sigma A \times L$ $= 0.236 \times 21.80 = 5.145 \text{ m}^3$ Total Volume of Handrail in Bridge (V _{LAR}) $V_{LAR} = 5.145 \text{ m}^3$		m²	0.025	. ==	×1	$A_2 = 0.35 \times 0.07$	
End Plate Wide $A = 0.30 \times 0.30 \times 2$ $Volume = \Sigma A \times L \rightarrow L = 21.80 - 60 = 21.20 \text{ m}$ $= (1.234 \times 21.20) + (0.180 - 5.44)$ $= 26.161 + 0.979$ $V = 27.140 \text{ m}^3$ Total Volume Slab in Bridge (V _S) $V_g = 27.140 \text{ m}^3$ $A_1 = 0.22 \times 0.10 \times 2$ $V_g = 27.140 \text{ m}^3$ $A_5 = 0.22 \times 0.10 \times 2$ $V_g = 27.140 \text{ m}^3$ $A_6 = 0.17 \times 0.22 \times 0.40 \times 2$ $A_7 = 0.20 + 0.25 \times 0.08 \times 2$ $A_7 = 0.236 \text{ m}^2$ $Concrete Volume Handrail: = \Sigma A \times L = 0.236 \times 21.80 = 5.145 \text{ m}^3 Total Volume of Handrail in Bridge (VLAR) V_{LAR} = 5.145 \text{ m}^3$		m²	0.243	=		<u> </u>	
End Plate Wide $A = 0.30 \times 0.30 \times 2$ Volume = $\Sigma A \times L \rightarrow L = 21.80 - 60 = 21.20 \text{ m}$ = $(1.234 \times 21.20) + (0.180 - 5.44)$ = $26.161 + 0.979$ V = 27.140 m^3 Total Volume Slab in Bridge (V _S) $V_g = 27.140 \text{ m}^3$ Handrail Wide = $A_5 = 0.22 \times 0.10 \times 2$ = 0.044 m^2 $A_6 = \frac{0.17 \times 0.22}{2} \times 0.40 \times 2$ = 0.156 m^2 $A_7 = \frac{0.20 + 0.25}{2} \times 0.08 \times 2$ = 0.036 m^2 Concrete Volume Handrail: = $\Sigma A \times L$ = $0.236 \times 21.80 = 5.145 \text{ m}^3$ Total Volume of Handrail in Bridge (V _{LAR}) $V_{LAR} = 5.145 \text{ m}^3$		3	0.216	<u>. </u>	v 0 645 v 2	0.27 ± 0.22	
End Plate Wide $A = 0.30 \times 0.30 \times 2$ Volume $= \Sigma A \times L \rightarrow L = 21.80 - 60 = 21.20 \text{ m}$ $= (1.234 \times 21.20) + (0.180 - 5.44)$ $= 26.161 + 0.979$ V $= 27.140 \text{ m}^3$ Total Volume Slab in Bridge (V _S) $V_g = 27.140 \text{ m}^3$ $A_5 = 0.22 \times 0.10 \times 2 = 0.044 \text{ m}^2$ $A_6 = \frac{0.17 \times 0.22}{2} \times 0.40 \times 2 = 0.156 \text{ m}^2$ $A_7 = \frac{0.20 + 0.25}{2} \times 0.08 \times 2 = 0.036 \text{ m}^2$ $= 0.236 \times 21.80 = 5.145 \text{ m}^3$ Total Volume of Handrail in Bridge (V _{LAR}) $V_{LAR} = 5.145 \text{ m}^3$		1111"	0.310	-	X 0.043 X Z	2	
Wide A = $0.30 \times 0.30 \times 2$ = 0.180 m^2 Volume = $\Sigma A \times L \rightarrow L = 21.80 - 60 = 21.20 \text{ m}$ = $(1.234 \times 21.20) + (0.180 - 5.44)$ = $26.161 + 0.979$ V = 27.140 m^3 Total Volume Slab in Bridge (V _S) $V_g = 27.140 \text{ m}^3$ $V_g = 27.140 \text{ m}^3$ $V_g = 27.140 \text{ m}^3$ $A_5 = 0.22 \times 0.10 \times 2 = 0.044 \text{ m}^2$ $A_6 = \frac{0.17 \times 0.22}{2} \times 0.40 \times 2 = 0.156 \text{ m}^2$ $A_7 = \frac{0.20 + 0.25}{2} \times 0.08 \times 2 = 0.036 \text{ m}^2$ $= 0.236 \text{ m}^2$ Concrete Volume Handrail: = $\Sigma A \times L$ = $0.236 \times 21.80 = 5.145 \text{ m}^3$		m²	1.254	4 = ;			n 1 n 1
Volume = $\Sigma A \times L \rightarrow L = 21.80 - 60 = 21.20 \text{ m}$ = $(1.234 \times 21.20) + (0.180 - 5.44)$ = $26.161 + 0.979$ V = 27.140 m^3 Total Volume Slab in Bridge (V _S) $V_g = 27.140 \text{ m}^3$ Handrail Wide = $A_5 = 0.22 \times 0.10 \times 2$ = 0.044 m^2 $A_6 = \frac{0.17 \times 0.22}{2} \times 0.40 \times 2$ = 0.156 m^2 $A_7 = \frac{0.20 + 0.25}{2} \times 0.08 \times 2$ = 0.036 m^2 = 0.236 m^2 Concrete Volume Handrail: = $\Sigma A \times L$ = $0.236 \times 21.80 = 5.145 \text{ m}^3$ Total Volume of Handrail in Bridge (V _{LAR}) $V_{LAR} = 5.145 \text{ m}^3$	· -				} - •		End Plate
$Volume = \Sigma A \times L \rightarrow L = 21.80 - 60 = 21.20 \text{ m}$ $= (1.234 \times 21.20) + (0.180 - 5.44)$ $= 26.161 + 0.979$ $V = 27.140 \text{ m}^3$ $V_g = 27.140 \text{ m}^3$ $V_g = 27.140 \text{ m}^3$ $V_g = 27.140 \text{ m}^3$ $A_5 = 0.22 \times 0.10 \times 2 = 0.044 \text{ m}^2$ $A_6 = \frac{0.17 \times 0.22}{2} \times 0.40 \times 2 = 0.156 \text{ m}^2$ $A_7 = \frac{0.20 + 0.25}{2} \times 0.08 \times 2 = \frac{0.036 \text{ m}^2}{2} = 0.236 \text{ m}^2$ $= 0.236 \times 21.80 = 5.145 \text{ m}^3$ $V_{LAR} = 5.145 \text{ m}^3$			 0.100		1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	Wide A - 030	1.5
$= (1.234 \times 21.20) + (0.180 - 5.44)$ $= 26.161 + 0.979$ $V = 27.140 \text{ m}^{3}$ $V_{g} = 27.140 \text{ m}^{3}$ $V_{g} = 27.140 \text{ m}^{2}$ $V_{g} = 27.140 \text{ m}^$		m-	0.160		, .	The second secon	
$V = 26.161 + 0.979$ $V = 27.140 \text{ m}^{3}$ $V_{g} = 27.140 \text{ m}^{3}$ $V_$						•	
Total Volume Slab in Bridge (V _S) $V_g = 27.140 \text{ m}^3$ $\frac{\text{Handrail}}{\text{Wide}} = A_5 = 0.22 \times 0.10 \times 2 = 0.044 \text{ m}^2$ $A_6 = \frac{0.17 \times 0.22}{2} \times 0.40 \times 2 = 0.156 \text{ m}^2$ $A_7 = \frac{0.20 + 0.25}{2} \times 0.08 \times 2 = \frac{0.036 \text{ m}^2}{2} = 0.236 \text{ m}^2$ $= 0.236 \times 21.80 = 5.145 \text{ m}^3$ $V_{LAR} = 5.145 \text{ m}^3$		14	;				
$V_{g} = 27.140 \text{ m}^{3}$ $\frac{\text{Handrail}}{\text{Wide}} \qquad A_{5} = 0.22 \times 0.10 \times 2 \qquad = 0.044 \text{ m}^{2}$ $A_{6} = \frac{0.17 \times 0.22}{2} \times 0.40 \times 2 \qquad = 0.156 \text{ m}^{2}$ $A_{7} = \frac{0.20 + 0.25}{2} \times 0.08 \times 2 \qquad = 0.036 \text{ m}^{2}$ $= 0.236 \text{ m}^{2}$ $= 0.236 \times 21.80 = 5.145 \text{ m}^{3}$ $V_{LAR} = 5.145 \text{ m}^{3}$	1,		į.		0 m³	V = 27.14	
$V_{g} = 27.140 \text{ m}^{3}$ $\frac{\text{Handrail}}{\text{Wide}} \qquad A_{5} = 0.22 \times 0.10 \times 2 \qquad = 0.044 \text{ m}^{2}$ $A_{6} = \frac{0.17 \times 0.22}{2} \times 0.40 \times 2 \qquad = 0.156 \text{ m}^{2}$ $A_{7} = \frac{0.20 + 0.25}{2} \times 0.08 \times 2 \qquad = 0.036 \text{ m}^{2}$ $= 0.236 \text{ m}^{2}$ $= 0.236 \times 21.80 = 5.145 \text{ m}^{3}$ $V_{LAR} = 5.145 \text{ m}^{3}$							
$\begin{array}{cccccccccccccccccccccccccccccccccccc$:			b in Bridge (V _S)	Total Volume Sla	
Wide = $A_5 = 0.22 \times 0.10 \times 2$ = 0.044 m ² $A_6 = \frac{0.17 \times 0.22}{2} \times 0.40 \times 2$ = 0.156 m ² $A_7 = \frac{0.20 + 0.25}{2} \times 0.08 \times 2$ = 0.036 m ² $= 0.236 \text{ m}^2$ Concrete Volume Handrail: $= \sum A \times L$ $= 0.236 \times 21.80 = 5.145 \text{ m}^3$ Total Volume of Handrail in Bridge (V _{LAR}) $V_{LAR} = 5.145 \text{ m}^3$					140 m³	$V_g = 27.$	
Wide = $A_5 = 0.22 \times 0.10 \times 2$ = 0.044 m ² $A_6 = \frac{0.17 \times 0.22}{2} \times 0.40 \times 2$ = 0.156 m ² $A_7 = \frac{0.20 + 0.25}{2} \times 0.08 \times 2$ = 0.036 m ² $= 0.236 \text{ m}^2$ Concrete Volume Handrail: $= \sum A \times L$ $= 0.236 \times 21.80 = 5.145 \text{ m}^3$ Total Volume of Handrail in Bridge (V _{LAR}) $V_{LAR} = 5.145 \text{ m}^3$			- 1				Handrail
$A_{6} = \frac{0.17 \times 0.22}{2} \times 0.40 \times 2 = 0.156 \text{ m}^{2}$ $A_{7} = \frac{0.20 + 0.25}{2} \times 0.08 \times 2 = 0.036 \text{ m}^{2}$ $= 0.236 \text{ m}^{2}$ $= 0.236 \times 21.80 = 5.145 \text{ m}^{3}$ $V_{LAR} = 5.145 \text{ m}^{3}$		m²	0.044	1.34 1	× 0.10 × 2	$A_5 = 0.22$	
$A_{7} = \frac{0.20 + 0.25}{2} \times 0.08 \times 2 \qquad = \frac{0.036 \text{ m}^{2}}{2}$ $= 0.236 \text{ m}^{2}$ $= 0.236 \times 21.80 = 5.145 \text{ m}^{3}$ $V_{LAR} = 5.145 \text{ m}^{3}$					7×0.22	. _ 0.1	,60, 17G ⁵⁰
$A_7 = \frac{0.20 + 0.25}{2} \times 0.08 \times 2 = \frac{0.036 \text{ m}^2}{2}$ $= 0.236 \text{ m}^2$ $= 0.236 \times 21.80 = 5.145 \text{ m}^3$ $V_{LAR} = 5.145 \text{ m}^3$	1. 11	m² .	0.156		${2}$ × 0.40×2	A ₆ =	
Concrete Volume Handrail: $= \sum A \times L$ $= 0.236 \times 21.80 = 5.145 \text{ m}^{3}$ Total Volume of Handrail in Bridge (V_{LAR}) $V_{LAR} = 5.145 \text{ m}^{3}$	1 .	4. f. ·	\$ [8] 3 F	1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1	0+025	0.2	
Concrete Volume Handrail: $= \sum A \times L$ $= 0.236 \times 21.80 = 5.145 \text{ m}^{3}$ Total Volume of Handrail in Bridge (V_{LAR}) $V_{LAR} = 5.145 \text{ m}^{3}$		m²	0.036	= <u>=</u>	$\times 0.08 \times 2$	$A_7 = \frac{3.2}{2}$	
Concrete Volume Handrail: $= \sum A \times L$ $= 0.236 \times 21.80 = 5.145 \text{ m}^{3}$ Total Volume of Handrail in Bridge (V_{LAR}) $V_{LAR} = 5.145 \text{ m}^{3}$		m²	0.236		Z		
$= \sum A \times L$ $= 0.236 \times 21.80 = 5.145 \text{ m}^3$ Total Volume of Handrail in Bridge (V _{LAR}) $V_{LAR} = 5.145 \text{ m}^3$					nndrail:	Concrete Volume Ha	<u> </u>
$= 0.236 \times 21.80 = 5.145 \text{ m}^3$ $\text{Total Volume of Handrail in Bridge (V_{LAR})}$ $V_{LAR} = 5.145 \text{ m}^3$						= ΣΑνΙ	
Total Volume of Handrail in Bridge (V_{LAR}) $V_{LAR} = 5.145 \text{ m}^3$							1 - 20 - 220 - 1
$V_{LAR} = 5.145 \text{ m}^3$			÷		$.80 = 5.145 \text{ m}^3$	$= 0.236 \times 21$	
$V_{LAR} = 5.145 \text{ m}^3$							
$V_{LAR} = 5.145 \text{ m}^3$					in Bridge (V. 15)	I Volume of Handrail i	Tota
					and the second s		
Total Slab (V_S) and Handrail (V_{LAR}) = 32.285 m ³					m²	V _{LAR} = 3.143	
Total Slab (V_S) and Handrail (V_{LAR}) = 32.285 m ³							
					ail $(V_{LAR}) = 32.285 \text{ m}^3$	l Slab (V _S) and Handra	Tota
	•						
		:					
建光线 医克克特 人名英格兰 医克克氏虫素 自身权益 医皮肤 人名英格兰 医二十二十二十二十二十二十二十二十二十二十二十二十二十二十二十二十二十二十二十							
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Name of ASIN PUMPING Category Concrete Structure STATION BRIDGE Calculation Volume	l Page	5/5

3. Cross Beam / Diapraghm K400



For One Cross Beam Volume = $0.20 \times 0.98 \times 1.25 = 0.245 \text{ m}^3$

Total volume Cross Beam in Bridge (V_{CB}) $V_{CB} = 2 \times 3 \times 0.245 = 1.470 \text{ m}^3$

Carlotter and the

Name of Structure	ASIN PUMPING STATION BRIDGE	Category Calculation	Reinforcing Volume	Page	1/4

SUMMARY OF REINFORCING BAR

	·				
-	MAIN GIRDER		. =	4,488	kg
-	SLAB		=	4,485	kg
-	HAND RAIL AND KERB		= .	757	kg
-	PANEL PLATE			610	kg
-	DIAPRAGHM		=	146	kg
		TOTAL		10,486	kg
-	PC CABLE		en e		
	K1 Ø12.7 7 STRANDS		=	385.0	kg
	K2 Ø12.7 12 STRANDS	And the second		660.0	kg
	K1 Ø12.7 7 STRANDS		=	386.0	kg
	DIAPRAGHM Ø12.5		=	22.5	kg
		ТОТАІ	==	1 453 5	ko

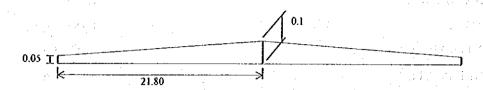
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	'	'				
Name of Structure		in a gradual state of the state	Category Calculation	Pavement Volume	Page	. 1/1

7. Pavement

Span of bridge: 21.80 m

Thick of Asphalt at side: 5 cm



$$A = 0.05 + \frac{2}{100} \times 2.5 = 0.10 \text{ m}$$

Volume =
$$\frac{(0.1 + 0.05)}{2} \times 2.5 \times 2 = 0.375$$

= $0.375 \times 21.80 = 8.175 \text{ m}^3$

Name of Structure	ASIN PUMPING STATION BRIDGE SUPER STRUCTURE	Category Calculation	Form Work Volume	Page	1/5
	SUMMA	RY OF BOARD FO	DRM WORK	: *	
- МА	IN GIRDER			450.50	
- SLA			=====================================	459.50 m	
	E WALK		≓ ;	4.38 m	•
			₩ 1.44 1.44	89.31 m ²	
. *	EL PLATE		= ↓.	83.20 m	
- DIA	PRAGHM		. =	20.05 m ²	
		TOTAL	=	656.44 m	
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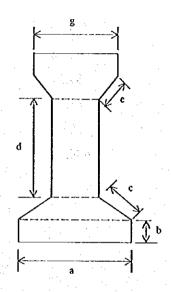
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Name of	ASIN PUMPING STATION BRIDGE	Category	Form Work	Page	2/5
Structure	SUPER STRUCTURE	Calculation	Volume		273

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1. Main Girder

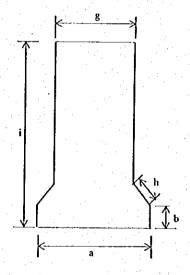
Center Beam



Thick of Plate = 3 cm

a =
$$65 + \frac{3}{2} \times 2$$
 = 68 cm
b = $20 + \frac{3}{2} \times 2$ = 23 cm
c = $35 + \frac{3}{2} \times 2$ = 38 cm
d = $75 + \frac{3}{2} \times 2$ = 78 cm
e = $12 + \frac{3}{2} \times 2$ = 15 cm
f = $10 + \frac{3}{2}$ = 11.5 cm

End Beam



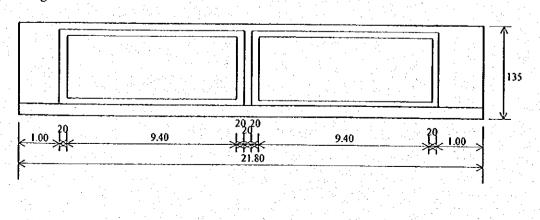
$$b = 23 cm$$

h =
$$6.75 + \frac{3}{2} \times 2$$
 = 9.75 cm

$$i = 110.5 + \frac{3}{2} = 112 \text{ cm}$$

$$g = 55 cm$$

Length of Beam = 21.80 m



Name of Structure	ASIN PUMPING STATION BRIDGE SUPER STRUCTURE	Category Calculation	Form Work Volume	Page	3/5
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Wide

- Center
$$A_1 = (a + 2b + 2c + 2d + 2e + 2f) \times L$$
$$= (0.68 + 0.46 + 0.76 + 1.56 + 0.30 + 0.23) \times 18.80$$
$$= 3.99 \times 18.80$$
$$= 75.012 \text{ m}^2$$

- End
$$A_2 = (a + 2b + 2h + 2i) \times L$$

= $(0.68 + 0.46 + 0.76 + 0.195 + 2.24) \times 2.20$
= 3.575×2.20
= 7.865 m^2

- Between
$$A_3 = \left[\left(\frac{0.75 + 1.05}{2} \times 0.20 + 2 \times 1/2 \times 0.20 \times 0.20 \right) \times 4 \right] \times 2$$

$$= (0.18 + 0.04) \times 4 \times 2$$

$$= 1.76 \text{ m}^2$$
Therefore End Beam

Cover End Beam

$$A_4 = \left[\left(0.68 \times 0.23 \right) + \left(\frac{0.68 + 0.58}{2} \times 0.20 \right) + \left(0.58 \times 1.12 \right) \right] \times 2$$

$$= \left(0.1564 + 0.126 + 0.6496 \right) \times 2$$

$$= 0.932 \times 2$$

$$= 1.864 \text{ m}^2$$

Total Form in Bridge (FG)

$$F_G = \Sigma A \times 3$$

= (75.012 + 7.865 + 1.76 + 1.864)× 3
= 86.501 × 3
= 259.503 m²

Name of Structure

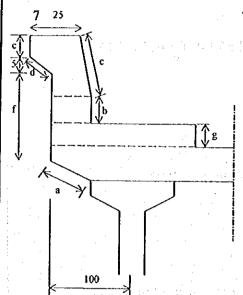
ASIN PUMPING STATION BRIDGE SUPER STRUCTURE

Category Calculation Form Work Volume

Page

4/5

2. Bed Plate and Hand Rail + Side Walk



Thick of Plate = 3 cm

a =
$$64.69 + \frac{3}{2}$$
 = 66.19 cm
b = $10 + \frac{3}{2}$ = 11.50 cm
c = $40.31 + \frac{3}{2}$ = 41.81 cm
d = $9.43 + \frac{3}{2} \times 2$ = 12.43 cm
e = $20 + \frac{3}{2}$ = 21.50 cm
f = $72 + \frac{3}{2} \times 2$ = 75.00 cm
g = 25.00 cm

Handrail + Sidewalk

Length of Beam = 21.80 m

Wide:
$$A_1 = [a+b+c+d+e+(f-g)] \times L \times 2$$

= $[66.19+11.5+41.81+12.43+21.5+(75-25)] \times 2180 \times 2$
= $8869548 \text{ cm}^2 \sim 88.70 \text{ m}^2$

End Cover

Bridge:
$$A_2 = \left[\left(b \times 0.28 \right) + \left(\frac{0.20 + 0.28}{2} \times 0.40 \right) + \left(\frac{0.20 + 0.25}{2} \times 0.11 \right) \right] \times 4$$

$$= 0.153 \times 4$$

$$= 0.612 \text{ m}^2$$

Total Wide $F_{SW1} = 88.70 + 0.612 = 89.312 \text{ m}^2$

Total Wide in Bridge = 89.312 m²

Slab: End of Slab

A =
$$[(0.33 \times 3.25) - 2 \times (1/2 \times 0.645 \times 0.05) + 2 \times (0.30 \times 1.25) + 2(0.30 \times 0.661)] \times 2$$

= 2.187×2
A = 4.375 m^3

Name of Structure ASIN PUMPING STATION BRIDGE SUPER STRUCTURE

Category Calculation Form Work Volume

Page

5/5

Panel Plate

Thick of Plate = 3 cm

Thick of Plate = 7 cm



145 × 100

Broad

 $A_1 = (1.48 \times 1.03) + 2 \times (0.0085 \times 1.48) + 2 \times (0.085 \times 1.03)$ = 1.5244 + 0.2516 + 0.1751

 $= 1.951 \text{ m}^2$

Broad

 145×20

 $A_2 = (1.48 \times 0.23) + 2 \times (0.085 \times 1.48) + 2 \times (0.085 \times 0.23)$

= 0.3404 + 0.2516 + 0.0391

 $= 0.6311 \text{ m}^2$

Total Broad = $(21 \times 1.9511) + 0.6311 = 41.602 \text{ m}^2$

Total Broad in Bridge $F_P = 2 \times 41.602 = 83.204 \text{ m}^2$

Cross Beam

Broad:

 $A = 2 \times (1.25 \times 0.98) + (1.25 \times 0.26) + 4 \times (0.175 \times 0.81)$

= 2.45 + 0.325 + 0.567

 $= 3.342 \text{ m}^2$

Total Broad

 $= 2 \times 3.342 = 6.684 \text{ m}^2$

Total Broad in Bridge $F_{CB} = 3 \times 6.684 = 20.052 \text{ m}^2$

SUBSTRUCTURE QUANTITY CALCULATION ASIN PUMP BRIDGE

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Name of Structure	710N F.	PUMPIN BRIDGE GTURE	ig sia-	Category of calculation	NOKK	VOLWATE-	Page	
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SUBSTRUCTURE QUANTITY CALCULATION

SUMMARY	ASIN PUMPING STATION
STIMMAANV	A SIM PERMPINIT STATION
OMBINION I	

THE RESIDENCE OF THE PROPERTY OF THE	SOMMAK	, ,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	1101111 0111	TING OIL			
			A1	A2		1 1	TOTAL
STATION	· ·		-				
	0-2m	(m³)	84.5	84.5			169
STRUCTURE	2-4m	(m³)	66.8	66.8			133.6
EXCAVATION	>4m	(m³)	0	0			0
	TOTAL	(m ³)	151.3	151.3	er jake bi		302.6
	WATER	(m³)	261.1	261.1			522.2
	IN RIVER	(m³)					0
BLINDING	STONE	(m³)	8.98	8.98			17.96
BACK I	TLE	(m ³)	132.03	132.03	7. 1.		264.06
PC PII	Æ	(m)	450	450			900
STEEL-I	PILE	(m)		· -			
	A-2-5						0
	B-1-2	(m³)					0
CONCRETE	B-1-3	(m³)		· · · · · · · · · · · · · · · · · · ·			0
	B-1-4	(m³)					0
	C-1	(m³)	96.17	96.17			192,34
	E	(m³)	4.49	4.49			8.98
	TYPE-1	(m²)	135.61	135.61			271.22
FORM	TYPE-2	(m²)					0
	TYPE-3	(m²)					0
	TYPE-4	(m²)					0
R-BA	R	(kg)	6891	6891			13782
REMARK	· · · · · · · · · · · · · · · · · · ·						

	PC PILE				<u> </u>		
			A1	A2			TOTAL
STA	rion						
ELEVATION	FL	m					
*	GH	m		: - :			
	HW	m					
	H1	m					
HEIGHT	H2-1	m	1 . V .				
	112-2	m					
	Н3	m					
	Hh	m					1
*	wr	m_				20° 12	
	Α	m					
DIMENSION	В	m					· · · · · · · · · · · · · · · · · · ·
	D1	m	0.5	0.5			
	DL	m					
NUMBER	OF PIER	÷					
PILE	NUMBER	m	25	25			50
	LENGTH	m	18	18			900

· ·	XCAVA	MOF	· · · · · ·		-	
	XCAVA	TON	4.1	A2		TOTAL
			A1	712		
STAT	NOI					
1		(m ²)		*	*	
A*B		(m)				
GH-FL			04.50	84.50		169.00
	0-2m	(m ³)	84.50			133.60
EXCAVATION	2-4m	(m ³)	66.80	66.80		0.00
	>4m	(m ³)	0.00	0.00		302.60
	TOTAL	(m ³)	151.30	151.30		
		1 -	261.10	261.10		522.20
	WATER					17.96
BLINDING S	TONE	(m ³)	8.98	0.00		
STEEL-	PILE	(m)				

3/11

 (m^3) CONCRETE TOTAL A1 A2 **STATION** (m²)LEVELING CONCRETE (E) (m) (m^3) 4.49 4.49 8.98 Ÿ (m^2) al (m²) a2 (m) REFER TO ABUTMENT FOOTING (C-1) h1 (m) h2 (m³) 96.17 96.17 192.34 ٧ (m²) al (m^2) a2 COLUMN (B-1-2) h1 (m) h2 (m) (m^3) C-PIER HEAD (B-1-2) RC PORTAL PC PORTAL

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(1)

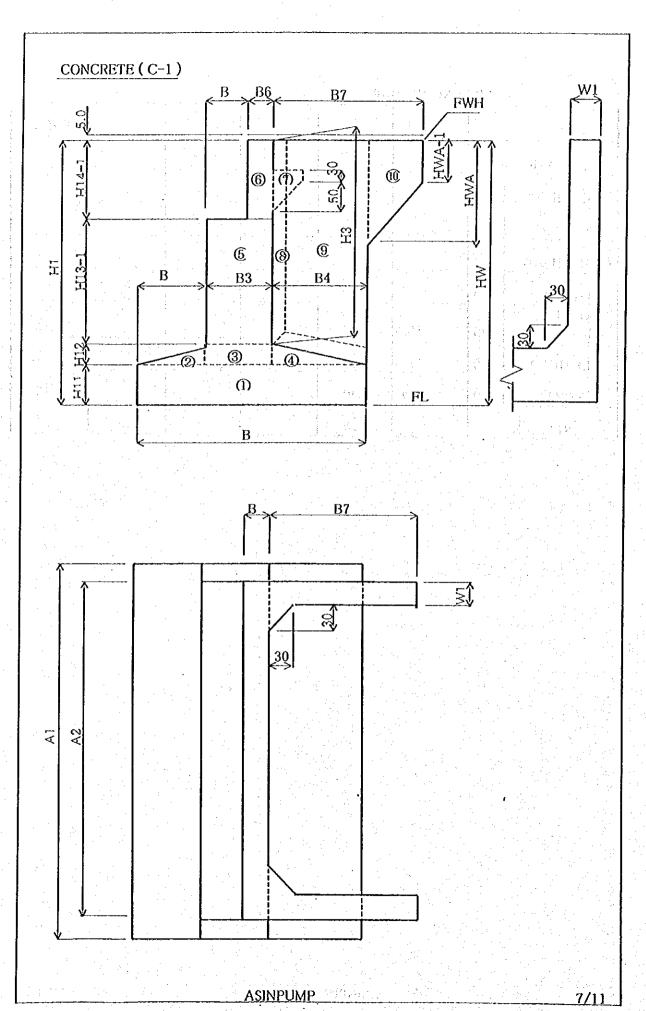
4/11

REMARK

go springerik Francoski salik sa	FORM A	REA	(m²)				
	:		A1	A2			TOTAL
STA	TION						
	al	(m ²)					
	a2	(m²)					
	Λ	(m²)	i		2.7	1 40 4 10 10	
	D2	(m)					
	D3	(m)					
	h1	(m)	\$ 5			1	
	h2	(m)					;
	al	(m ²)					
	a2	(m²)					
	Α	(m²)	135.61	135.61			271,22
C-PIER HEAD	TYPE-1						
	TYPE-4						
RC PORTAL	ТҮРЕ-6				3 2 4 7 7 1 3 4 4 7 7 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		
PC PORTAL	TYPE-6			\$ 17 . 12. 	£ N = .		
REM	ARK						

REINFORCING BAR

		Al	A2			TOTAL
STATION						
FOOTING	(kg)			i, saar ee ee	-	
COLUMN	(kg)					
C-PIER HEAD	(kg)			√ 1 ∀ 		
OTHER	(kg)					
TOTAL of STRUCTURE	(kg)	5166	5166			10332
PILE HEAD	(kg)	1725	1725			3450
TOTAL	(kg)	6891	6891			13782
REMARK	4		1. 7. 2.			



ABUTMENT A-1

CONCRETE (C-1)

	····									······					Τ		~
	Vı	า	=	C	*	Bn	*	Hnn	*	An		: :	· VOLUM	Ε	L	ni	
4																	
						m		m ·		m		 	<u>m3</u>		n	nn	n
	1		=	1.0	*	6.50	*	1.20	*	6.50		=	50.70	m3	1	11	1
	2		=	0.5	*	6.50	*	0.00	*	6.50		=	0.00	m3	2	12	1
	3		:=	1.0	*	1.43	*	0.00	*	6.50		=	1		3	12	1
	4	. ,		0.5	*	2.57	*	0.00	*	6.50		=			4	12	1
	5			1.0	*	1.43	*	4.96		5.60					3	13	1
	<u> </u>		_	1.0		1.40							*.		1	10	1
	6	1	=	1.0	*	0.50	*	1.84	*	5.60		=	5.16	m3	6	14	1_
	7	-		. 141			·			5.60		H =		m3			
	8		=	0.5	*	0.30	*	0.30	*	6.80	*	2 =	0.61	m3			
	9	-	=		*		*		*		*	=	0.00	m3			
	10	1	=		*		*		*			-		m3			:
	11		- =		*		*		*		*		0.00			:	:
	12	7	=	į	*	31	*		*		*		, v				
					<u></u>				-	. Express							
	<u> </u>				-							V =		m3	J		

RUBBER SHEET

()

 $0.28 * 0.35 * 5 = 0.49 \text{ m}^2$

t = 73 mm

ABUTMENT A-1

FORM AREA

,	Vn -	=	С	*	N		*	Bn,An	*	Hnn				=	VOLUME		ni	
						٠,		m		m			· · ·		m ²	n	nn	n
	1	· · =	1.0	*	2		*	6.50	*	1.20		F1 (1		· =	15.60 m ²	1	11	1
		=	1.0	*	2		*	6.50	*	1.20	·		:		15.60 m ²	1	11	1
	2	=	0.5	*	2	1	*	2.50	*	141			<u>.</u>	=	0.00 m ²	2	12	1
	3	=	1.0	*	2	:	*	1.43	*			10 100		- =	0.00 m ²	3	12	1
	4	=	0.5	*	2	- 1. T	*	2.57	*					=	0.00 m ²	4	12	1
	5	= .	1.0	*	2		*	1.43	*	4.96		1	-	=	14.18 m ²	3	13	1
		=	1.0	*	2		*	5.60	*	4.96			·	=	55.52 m ²	3	13	1
	6	=	1.0	*	2	,	*	1.84	*	1.84		* - 1	<u> </u>	,: = ·	6.79 m ²	6	14	1
-		- 12 - - 2 - - 2	1.0	*	2		*	5.60	*	1.84	<u> </u>			=	20.64 m ²	6	14	1
	7	; =					·				<u> </u>	· · ·			m ²			
_		=			0.3	3	*	1.414	==	0.42	· · · · ·		s 11					
_	8	=	<u>:</u>	· ·	2		*	0.42	*	2.00				· =	1.68 m ²			
_	9	==	<u>.</u>		4		*	0.70	*	2.00				=	5.60 m ²			
	0	=		- : :		ř ()	:		. :	<u></u>			· .		m ²			
	1	=				. •	*_		*				1916	=	0.00 m ²	1.		
Ĺ	2	=					*	· · · · · · · · · · · · · · · · · · ·	*					=	0.00 m ²			
_											· · ·		·····			. :		
										<u> </u>				/ =	135.61 m ²			

LEVELING CONCRETE (CLASS E)

$$V = (B1 + 0.20)*(A1 + 0.20)* 0.10$$

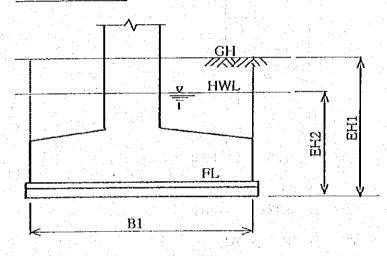
= $(6.50 + 0.20)*(6.50 + 0.20)* 0.10$ = $4.49 \text{ m}3$

(A-1,A-2 = 8.98 m3)

BLINDING STONE

(A-1,A-2 = 17.96 m3)

EXCAVATION _



EH1=GH-FL+0.3

EH2=HWL-FL+0.3

GH = -2.700

HWL = -0.100

FL = -5.980

3.58EH1 =

EH2 =6.18

B1 - A1 Hn

: V1 $6.50 * 6.50 * 2.00 = 84.5 \text{ m}^3$ 1. EXC.<2m

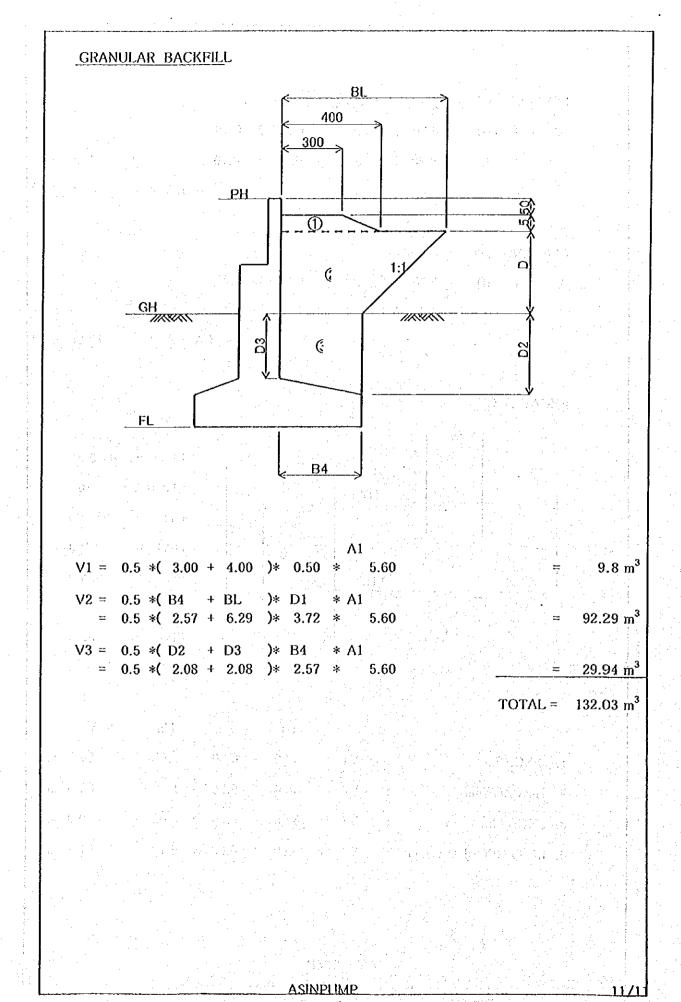
66.8 m³ V2 = 6.50 * 6.50 * 1.58 =2. 2m<EXC.<4m

V3 = 0.0 m^3 6.50 * 6.50 * 0.00 =3. EXC.>=4m

261.1 m³ V4 = 6.50 * 6.50 * 6.18 =4. INTO GROUND WATE:

ASINPUMP

10/1



Name of Structure	ASIN PUMPING STATION BRIDGE, FOR SCAFOLDING AND	Category Calculation	Work Volume	Page	1/2
	FORM SUPPORT				

SUMMARY OF SCAFOLDING AND FORM SUPPORT VOLUME FOR ASIN RIVER DRAINAGE SYSTEM IMPROVEMENT.

		Vo	lume
No	Structure	Scafolding (m²)	Form Support (m³)
1	Asin Pumping Station	1342	941
2	Asin Pumping Station, Gate	732	254
3	Asin Box Culvert	1843	1358
4	Asin Box Culvert Inlet Structure	54	11
5	Asin Box Culvert Outlet Structure	62	16
6	Secondary Channel Outlet on Asin River	-	34
7	Fuel Tank Box for Asin Pumping Station	133	62
8	Asin Pumping Station Bridge, Sub Structure	166	<u>-</u>
9	Asin No.1 Bridge, Sub Structure	293	-
10	Asin No.2 Bridge, Sub Structure	251	-
- 1	Total	4876	2676

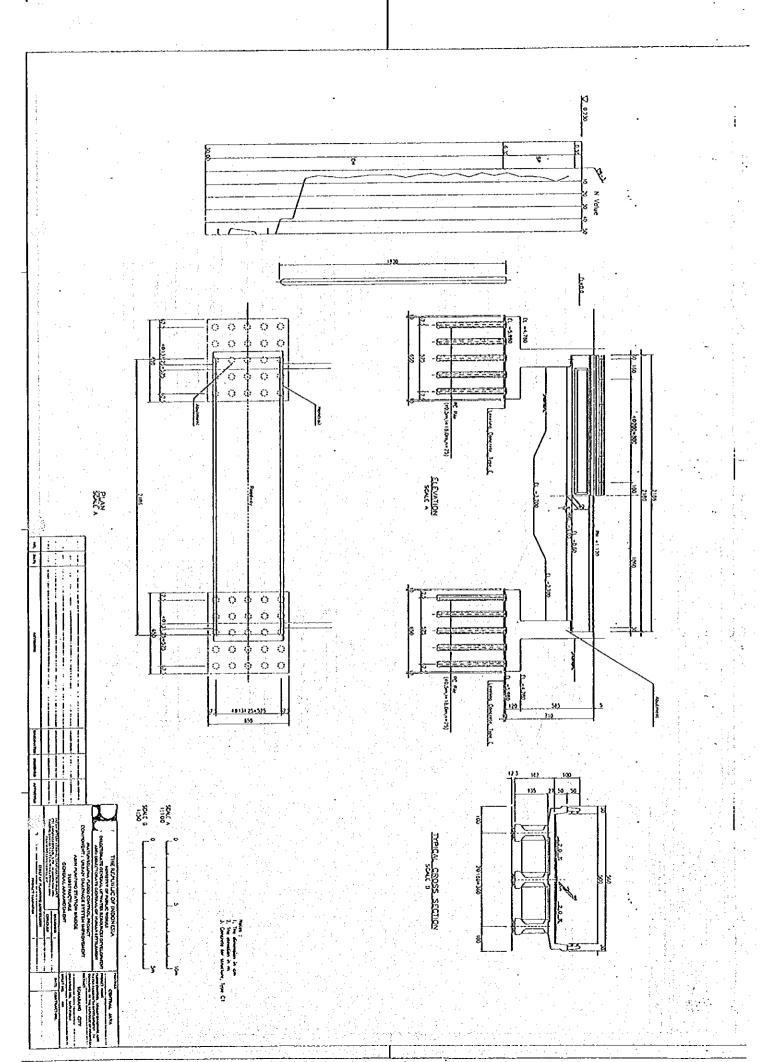
Monac	ASIN PUMPING		14 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		
Name of Structure	STATION BRIDGE, FOR SCAFOLDING AND	Category Calculation	Work Volume	Page	2/2
	FORM SUPPORT	:		12. A	

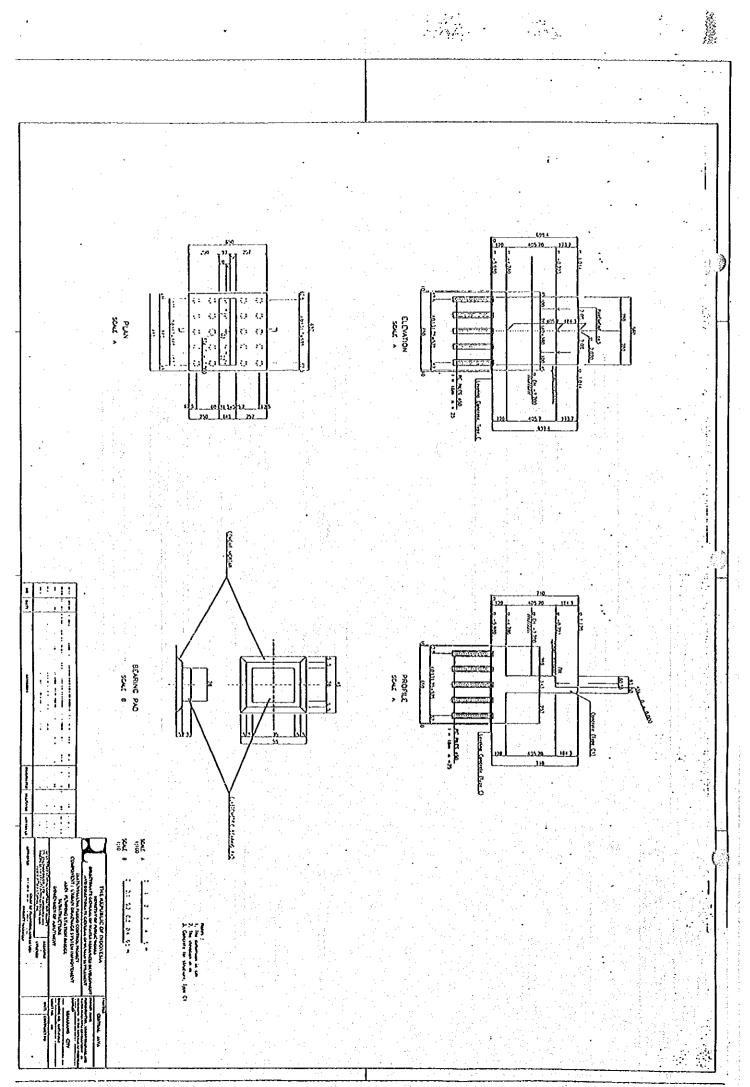
1. Scafolding Area

$$(2 \times 5.9 \times 5.6) + (2 \times 5.9 \times 1.43) = 82.954 \text{ m}^2$$

For both side abutment = $2 \times 82.954 = 165.908 \text{ m}^2$

2. Form Support Area



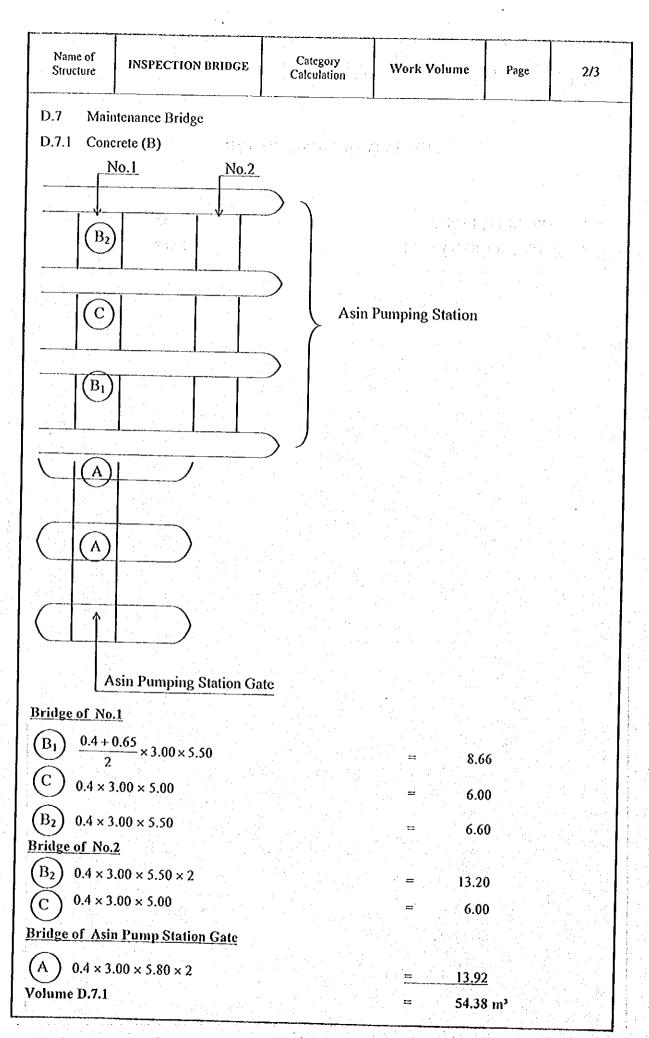


Name of Structure	INSPECTION BRIDGE	Category Calculation	Work Volume	Page	1/3
	'		i i		

SUMMARY OF WORK VOLUME

- 1. CONCRETE, TYPE B
- 2. REINFORCING BAR, U 30

- = 54.38 m³
- = 2,968 Kg



	STATE OF THE PARTY						
Name of Structure	INSPECTION BRIDGE	Category Calculation	Work Volume	Page	3/3		
D.7.2 Reinforcing Bar (SII U 30)							
Bridge of N	<u>0.1</u>						
			•				
$(B_1)(B_2)$ (2	2.89 + 84) × 2		==	746	,		
(C) (2	262 + 78) × 1		= ;	340			
Bridge of N	<u>o.2</u>						
(B_2) (289	+ 84) × 2		, =	746			
(262)	+ 78) × 1		. =	340			
Bridge of As	sin Pump Station Gate				·		
(305	+ 93) × 2		==	<u>796</u>			
Volume D.7.	1		= 2,	968 kg			