Chapter 4 B R I D G E

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4.1 Spillway Bridge

	BILL OF QUANTITIES FOR SPILLWAY BRIDGE (SUPER	STRU	CTURE)	
ltem No.	Description		Unit	Quantity
	Precast Prestressed Concrete Beam including			and the second sec
A.1.1	Tensioning and Erection	1.1	L.S.	te en la ce
	Precast Concrete Diaphragm including.			ł
A.1.2	Tensioning and Erection		L.S.	
A.1.3	Precast Concrete Panel including Erection	1.1	L.S. 🕚	· · ·
A.1.4	Concrete, Type B including Formwork		m ³	<u>)</u> : ≕ 35
	formwork for A.1.4		m^2	102
A.1.5	Deformed Reinforcing Bars		kg	5,296
A.1.6	Asphalt Concrete		tonne	21
A.1.7	Expansion Joint		· m	11
A.1.8	Hand Rail		kg	168
A.1.9	Drain Pipe, PVC Pipe Dia. 100 mm		m	11
A.1.10	Elastometric Bearing Pad (350 x 280 x 73)		No.	6
A.1.11	Rubber Sheet (40 x 10 x 3)		No.	6

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Concrete Volume

			111
	unit		
concrete K400 for girder	cu.m	38	
reinforcing bar	kg	5457	1. J. J. J.
concrete K400 for diaphragm	cu.m	2	
reinforcing bar	kg i	160	
concrete K250 for panel plate	cu.m		ji ta k
reinforcing bar	kg	705	
		6 (14) (1	
concrete K250 for slab+curb	cu.m	35	$\gamma_{i,j}^{(1)} \neq \gamma_{i,j}^{(2)}$
reinforcing bar	kg	5296	
		· · · ·	

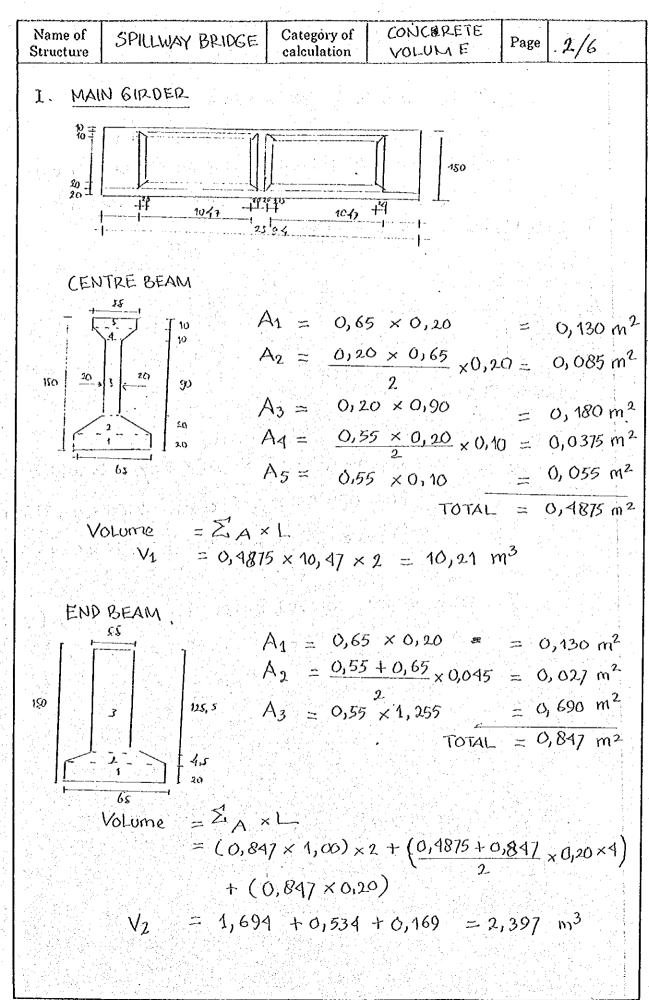
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Name of SPILLWAY BRIDGE Category of CONCRETE Page 3/6
Structure SPILLWAY BRIDGE Category of CONCRETE VOLUME Page 3/6
• TOTAL VOLUME FOR ONE BEAM = V₁ + V₂

$$Z_V = 10, 2.1 + 2,397 = 12,607 \text{ m}^3$$

• TOTAL VOLUME FOR ALL BEAM IN BRIDGE = (V6)
 $V_0 = 3 \times Z_V = 3 \times 12,607 = 37,821 \text{ m}^3$
I. BED PLATE AND PANEL PLATE
 $V_0 = 3 \times Z_V = 3 \times 12,607 = 37,821 \text{ m}^3$
I. BED PLATE AND PANEL PLATE
 $V_0 = 3 \times Z_V = 3 \times 12,607 = 37,821 \text{ m}^3$
FOR ONE PANEL PLATE
 $V_0 = 3 \times Z_V = 3 \times 12,607 = 37,821 \text{ m}^3$
FOR ONE PANEL PLATE
 $V_0 = 2 \times Z_V = 2,309 = 0,1015 \text{ m}^3$
FOR ONE GRIPER (Length = 2480 - 60 = 2120 \text{ m})
 $V_0 = 2,369 \text{ m}^3$
• TOTAL VOLUME FOR ALL PANEL, PLATE IN BRIDGE (Vp)
 $V = 2,369 \text{ m}^3$
• TOTAL VOLUME FOR ALL PANEL, PLATE IN BRIDGE (Vp)
 $V_P = 2 \times V = 2 \times 2,369 = 4,738 \text{ m}^3$
• SLAB
 $V_0 = \frac{11}{24}$
 $V_0 = \frac{11}{25}$
 $V_0 = \frac{11}{25}$
 $V_0 = \frac{11}{25}$
 $V_0 = \frac{11}{25}$
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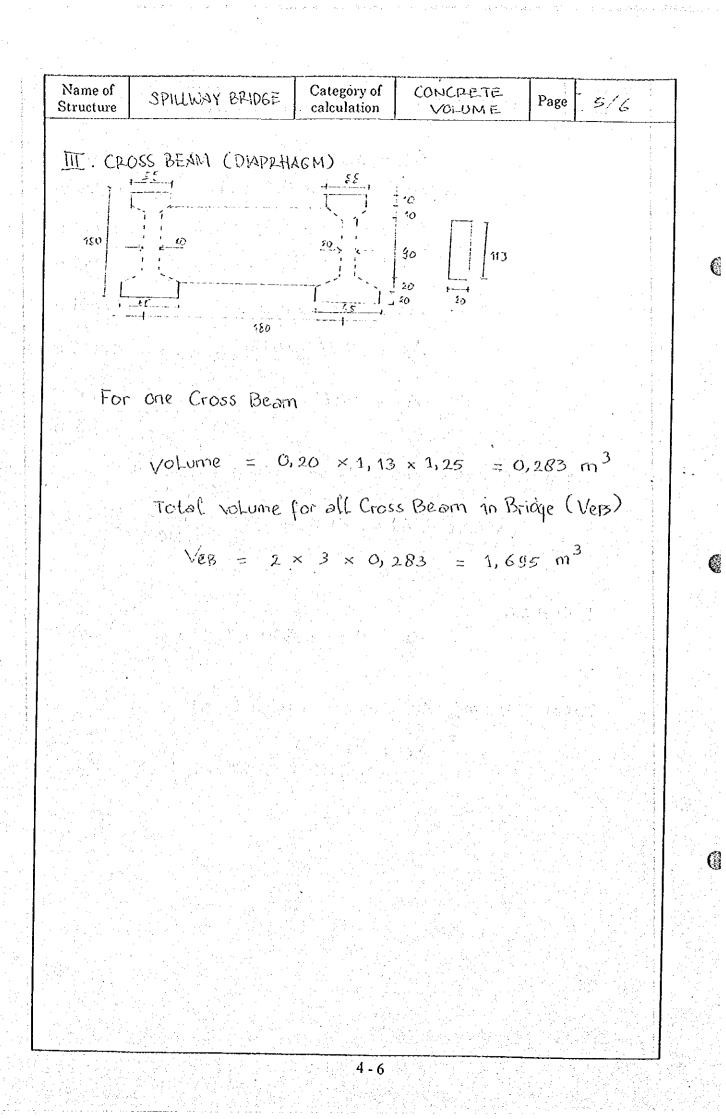
가슴 바람을 한 것은 것은 것은 것 같아요. 이 것은 것 같아요. 아이들 것 같아요.

	·			المناط المراجع المحافظ المحافظ المحافظ والمحافظ والمحافظ والمحافظ والمحافظ والمحافظ والمحافظ والمحافظ والمحافظ	
Name of Structure	SPILLWAY BRA	IDGE	Category of calculation	CONC PRETE VOLUME	Page 4/6
		Aı	= 3,25 x	0.20	$= 0,65 \mathrm{m}^2$
	: .		$= 0,35 \times$		$= 0,0.25 \text{ m}^2$
		A_3	= 0,45 ×	0,27 × 2	$= 0,243 \text{ m}^2$
					$2 = 0,316 \text{ m}^2$
· · ·			$2 = 0,22 \times$	la de la companya de	$= 0,044 m^{2}$
					$2 \approx 0,156 \text{ m}^2$
		A	= 0,08 ×		$= 0.032 \text{ m}^2$
				TOTAL	$= 1,466 \text{ m}^2$
Le	ingth of plate	e (L) = 23,94	-60 = 23;34 = 23,34	
	Volume	(V,) = A ×		x 23,34
ENI	> PLATE				
۰ ، ، ۴ ۰ ، ۰ ، ۰ ، ۰ ، ۰ ۰ - ۰ ، ۰ ، ۰ ، ۰ ،		$\sqrt{2}$	$= 0_1 30 \times 0$	0,30 × 5,44 ×	$2 = 0,979 \text{ m}^3$
Tot	al Volume	AU	plale in B	ridge (VB)	
		VB	$= V_1 + V_2$ = 39,916 +	2 - 0,0 9 79 = 35	5,195 m ³
			- 01)-0		
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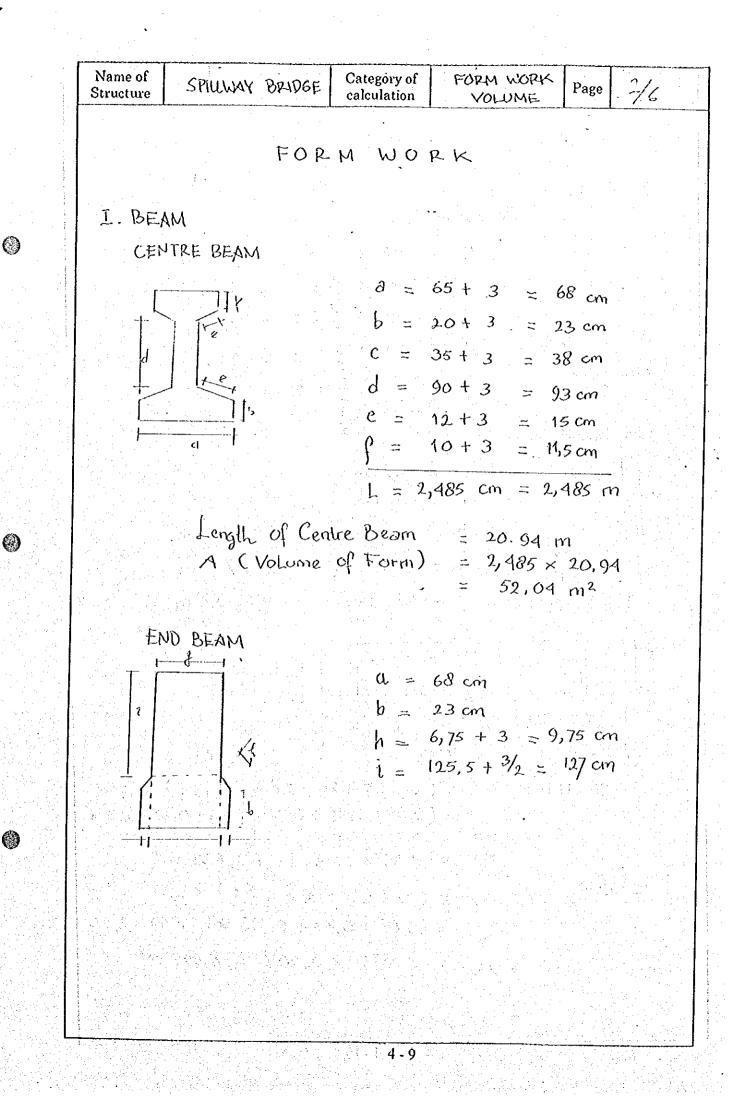
Name of Structure	SPILIWAY BRIDGE	Category of calculation	CONCRETE VOLUME	Page 6/
				· .
Con	icrete Volume			
	, MAIN GIRDER	- = 37,	821 m ³ (K	400)
	· SLAB	A second s	195 m ³ (C	and the second
	· PANEL PLATE	= 4,	738 m ³ (1	c1,250)
	· CROSS BEAM	= 1, 6	.95 m ³ (1	C:400)
	τοτα	1_ = 79, 4	49 m3	

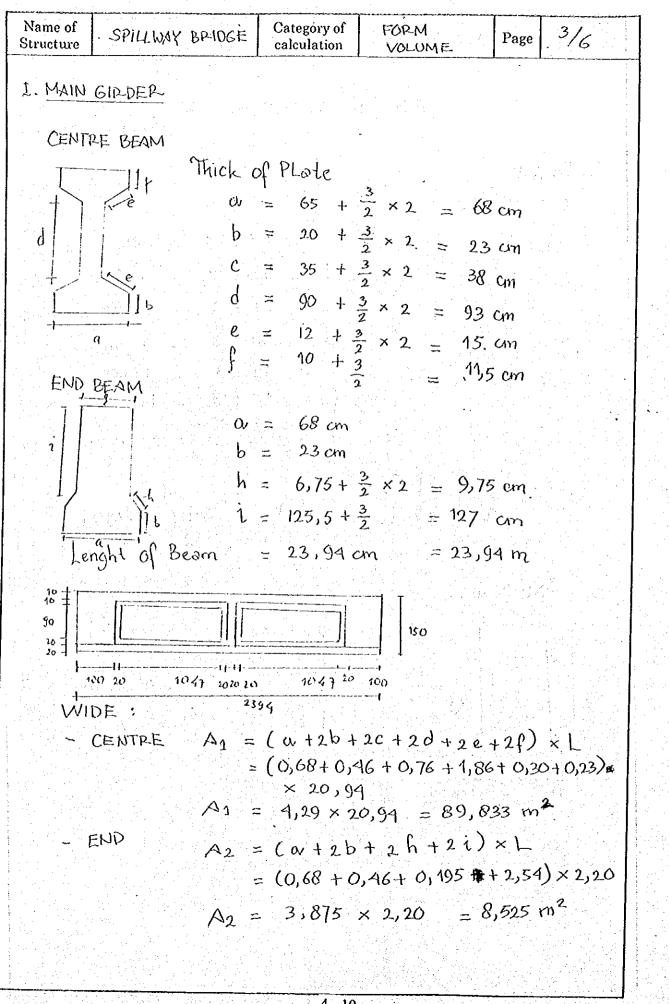
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• Concrete Quality K. 400 = 39,516 m³ • Concrete Quality K. 250 = 39,933 m³.

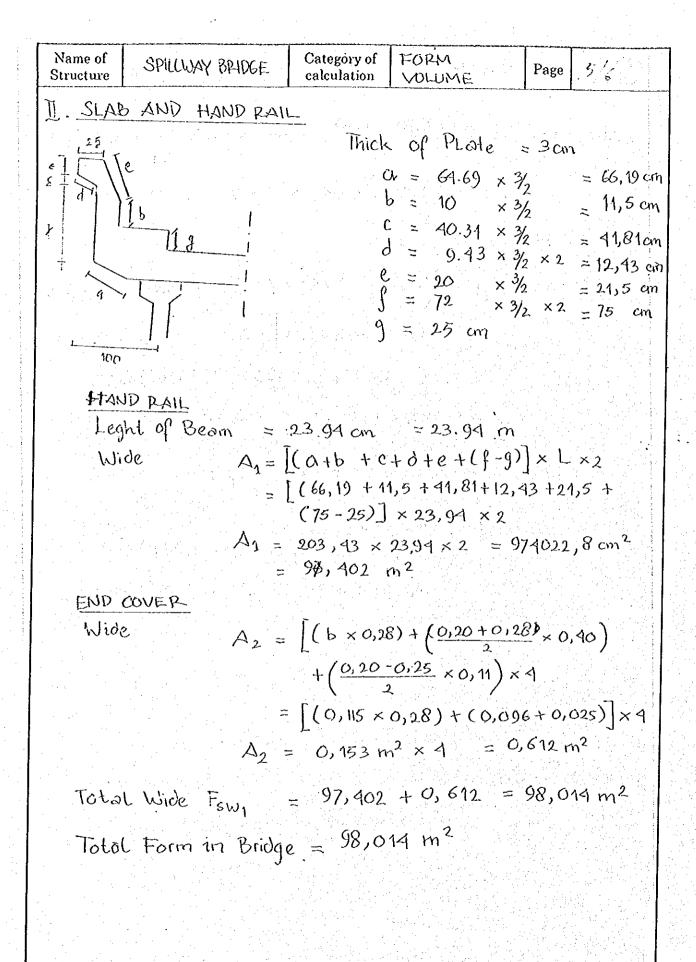
Name of Structure			Category of calculation	<u> </u>	Page	1/6
		SUMMAR	Y OF BRO	AD FORM	WORK	
	- SLa - Side - Pan	n Girder b Walk el PLat prhògm		290.91 4.38 98.01 9.1.47	m² m²	
	~ (716)		= Total =	22.30	m2	
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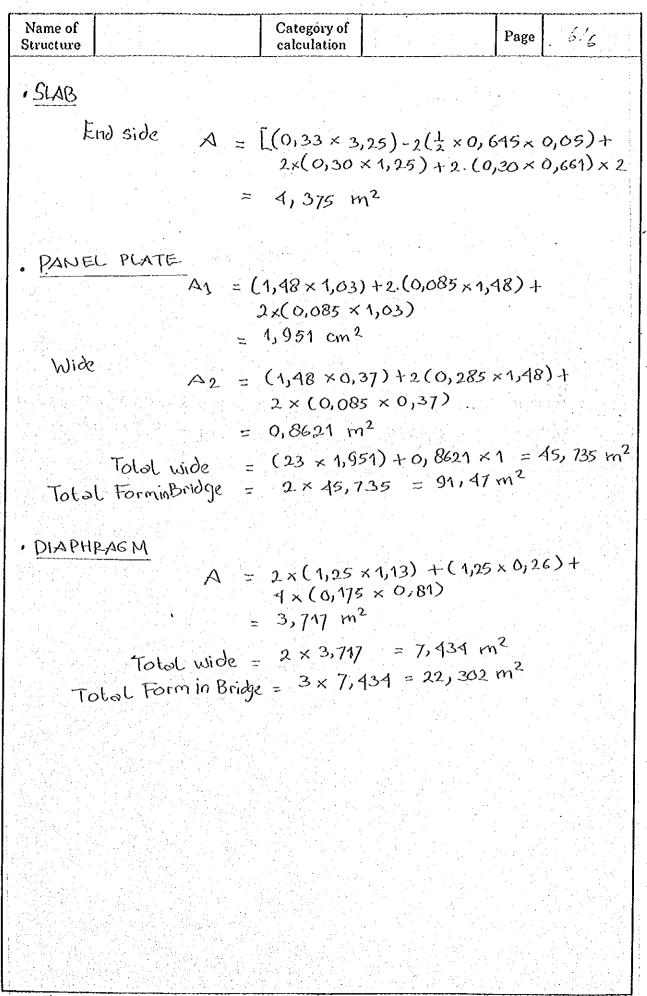
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Name of Category of SPILLWAY BRIDGE 1/1 Page Structure calculation ASPHALT PAUEMENT Length of beam = 2394 cm = 23,94 mThick of asphalt = 5 cm = 0,05 m0,050 --- 210 if sem 150 2,500 500 $B_{road} = (0,05 \times 5,00) + 2(2 \times 2,5 \times 0,05) = 0,375 m^2$ Volume = 0, 375 m² × 23, 94 m = 8,98 m³ 4 - 14

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		unit	amount
main girder		kg	5,457
panel plate		kg	705
diaphragm		kg .	160
	total	kg	6,322
· ·		•	
slab		kg	4,451
handrail and c	urb	kg	845
na se	total	kg	5,296
PC cable			
K1 12 strand	(D=12.7mm)	kg	1,147
K2 7 strand ([kg	667
K3 12 strand	(D=12.7mm)	kg	1,151
	diaphragm (D=12.5mm)	kg -	23
	total	kg	2,988

Reinforcing Bar Calculation (Spillway Bridge)

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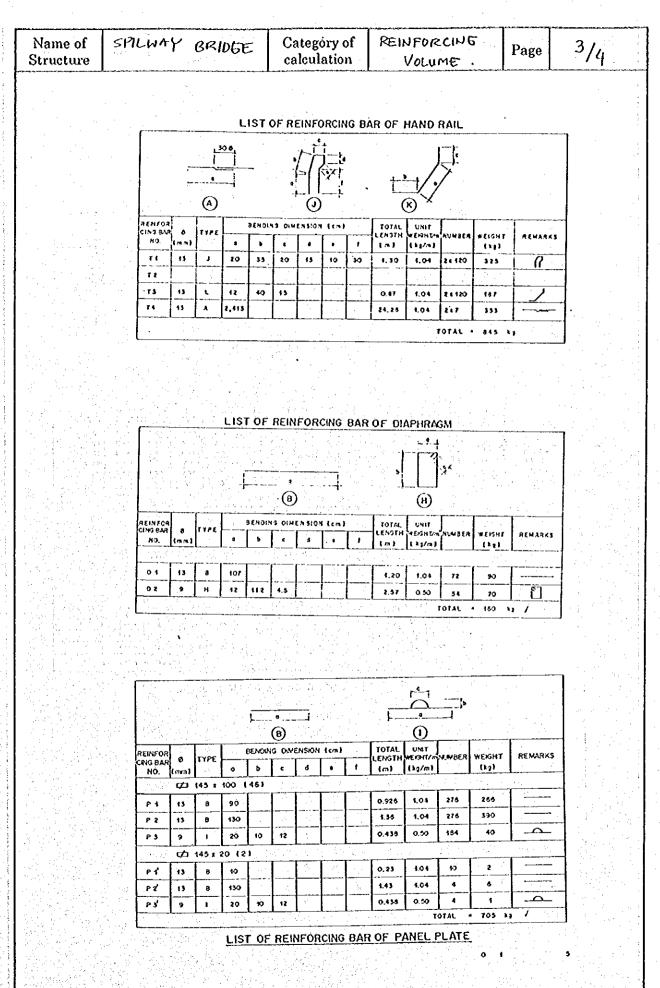
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Name of Structure	SPILWAY BRIDGE	Category of	KEIN FORCING	Page	2.14
onnorme		calculation	Volume.		~14
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			and and a second se Second second second Second second		
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	CINS BUR 0 TYPE	DIMENSION LEAD	TOTAL UNIT LENGTH WEIGHT NUMBER WEIGHT	ÁÉMARKS	
с. ж	NO (ma) - 6 3	* * * *	1m) [19/m] [19]		
	31 43 0 57 55 82 43 E 62 486	35 20	1.97 1.04 3±(11 682	25	-
	82 13 E 12 181 .83 13 C 47 4	12 20	3.74 4.04 31411 1293 4.19 1.04 3195 353		· · · · · · · · · · · · · · · · · · ·
	84 13 A 2,385		24.25 1.04 314 303		1
	85 13 A 2,234		22.73 1.04 32 10 709		1
	85 13 6 55 10		0.75 1.04 311 2	<u> </u>	1
	87 15 A 2,305 B5 13 A 2,306		24.34 1.58 318 923 24.25 1.04 312 1.151	·	
	89 43 N 47 142	65	24.25 1.04 312 1 156 3.96 1.04 3118 220		la en se l'ét
	310 (3 8 92 25		130 1.04 3110 44		j dage til stad
· · · ·	811 tO F 16 4		0.24 0.617 3x240 407	C	
•	B12 16 A 2,586 B13 13 I 6 6		24.34 1.58 324 482		
ta de la composición de la composición Composición de la composición de la comp	A13 43 I 6 6 B14 43 J 7 10	38	2.36 1.04 612 30 0.95 1.04 614 28	N N	1
	815 13 K 30. 18		3.391 1.04 626 127	NNN	
	818 13 8 15		0.90 1.04 612 11		18676.18
and the second second	<u>- 817 13 8 30 </u>		210 1.04 814 13	L	
	L		TOTAL + 5 457		1
	LIST OF RE	INFORCING BAR O	F MAIN GIRDER		<u> </u>
				-ख	n
1.1.1					
×					
		· · · · · · · · · · · · · · · · · · ·			
-		and the second			
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	· · · · · · · · · · · · · · · · · · ·	CIMENSIONIES TOTAL U	NIT WEIGHT		
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	33 D+5 B 130	╏╸╼╼┠──╼╂╼──┨╶╍╍╌╾┨╺	.58 2.054 230 472	-1	
	34 D15 B 360	3.60 1	58 3.588 121 688	-	
	3.5 D 16 8 115	1,15 1	58 1.817 12 22		
	56 D10 C 27 22	5 1.06 (56 0.508 28 17		
· · ·	\$7 0 0 25 10	5 0.55 0	.56 0.308 44 14 🗋		
	38 013 A 2423	24.23 1	.04 23.199 28 706	-1	
	\$ 9 D13 A 2423	24.23 1	04 25 199 28 106	그 영상	
			TOTAL + 4451 12		
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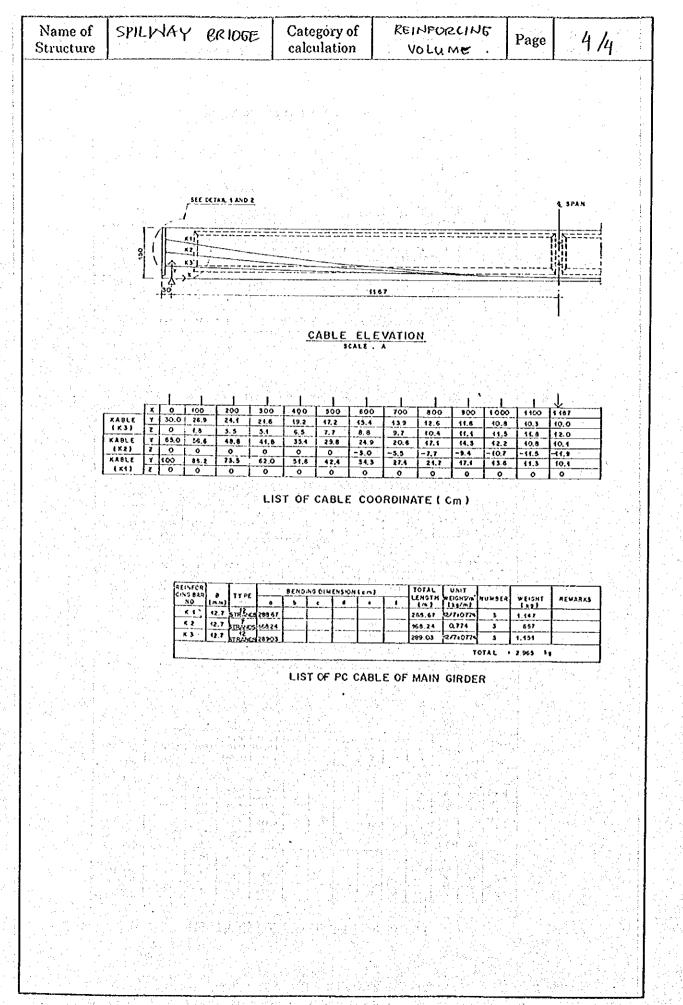


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4.2 Approach Bridge to Goa Kreo Cave

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Item No.	Description	Unit	Quantity
A.1	Preparatory Works		
A.1.1	Temporary Construction Road	L.S.	
A.2	Bridge and Approach Road		
1 1	(Superstructure)		
A.2.1	Concrete, Type B including Formwork	m ³	99
A.2.2	Deformed Reinforcing Bars	kg	17,610
	formwork for A.1.4	m ²	663
A.2.3	Asphalt Concrete	tonne	23
A.2.4	Expansion Joint	m	· · · · 10
A.2.5	Hand Rail	kg	476
A.2.6	Drain Pipe, PVC Pipe Dia. 100 mm	<u> </u>	48
A.2.7	Elastometric Bearing Pad (316 x 316)	No.	16
	(Substructure and Approach Road)		
A.2.8	Stripping of Top Soil	m ³	47
A.2.9	Excavation	m°	1,023
A.2.10	Backfilling	m ³	879
A.2.11	Embankment	m ³	103
A.2.12	Concrete, Type C-1 including Formwork	m ³	155
	formwork	m ²	410
21 문문 전	scaffolding	m ²	43
A.2.13	Deformed Reinforcing Bars	kg	11,473
A.2.14	Leveling Concrete, Type E	m ³	8
A.2.15	Wet Stone Masonry	In ³	136
A.2.16	Weep Hole, Dia. 50 mm	No.	70
A.2.17	Asphalt Concrete	tonne	18
A.2.18	Gravell	m ³	31
A.3	Gate Relocation		
A.3.1	Demolition of Existing Gate	L.S.	
A.3.2	Excavation	m ³	14
A.3.3	Concrete, Type C-1 including Formwork	m	6
	formwork	m ²	2
A.3.4	Wet Stone Masonry Reconstruction	m ³	7
A.3.5	Temple Stone Reconstruction	m ³	21

BILL OF QUANTITIES FOR APPROACH BRIDGE TO GOA KREO CAVE

THE SUMMARY TABLE of SUPERSTRUCTURE

			Con	Concrete	Reinforc		PC cable	si e	Form	pavem	Expansi	Road	Drain	Bearin	Bearing shoe	Note
NAME and KIND	rd KINL		400	250	ement	7T12.7	12712.7	1T12.5			on joint	rail	pipe	size	Number	
			m3	m3	kg	kg	kg	kg	m2		ផ	B	: E	a a		
Spillway	F-3	Pc									•					
Gua kureo		Rc	}	24.833	4.404	1		1	165.67	2.550	4.00	34.00	12.00	316×316	2	
	5	Rc	 	24.833	4.404	l	1	-	165.67	2.550	2.5	34.00	12.00	316x316	4	
	က	Rc	 	24.833	24.833 4.404	I	ļ		165.67	2:550	. 2-D	34.00		910%910	4	
	4	Rc	1	24.333	24.333 4.404	1	1 1 1	1	165.67	2.550	2.0	.	12.00	3164316	4	
	total			<u> 99.</u> 33 2	17.616				662.68-		10.00	136.00	48.80		. 16 .	
simon weir	1	Rc						- - - -				(361983)				
	2	Pc											16 17 17 17		2 	
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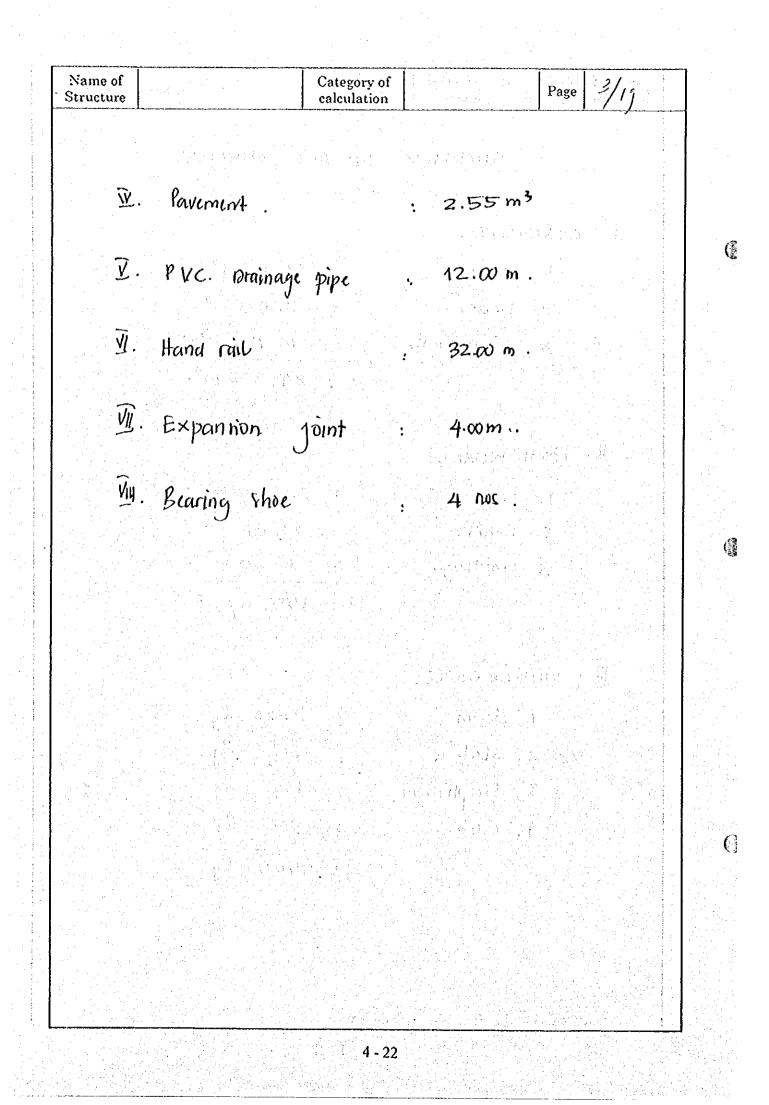
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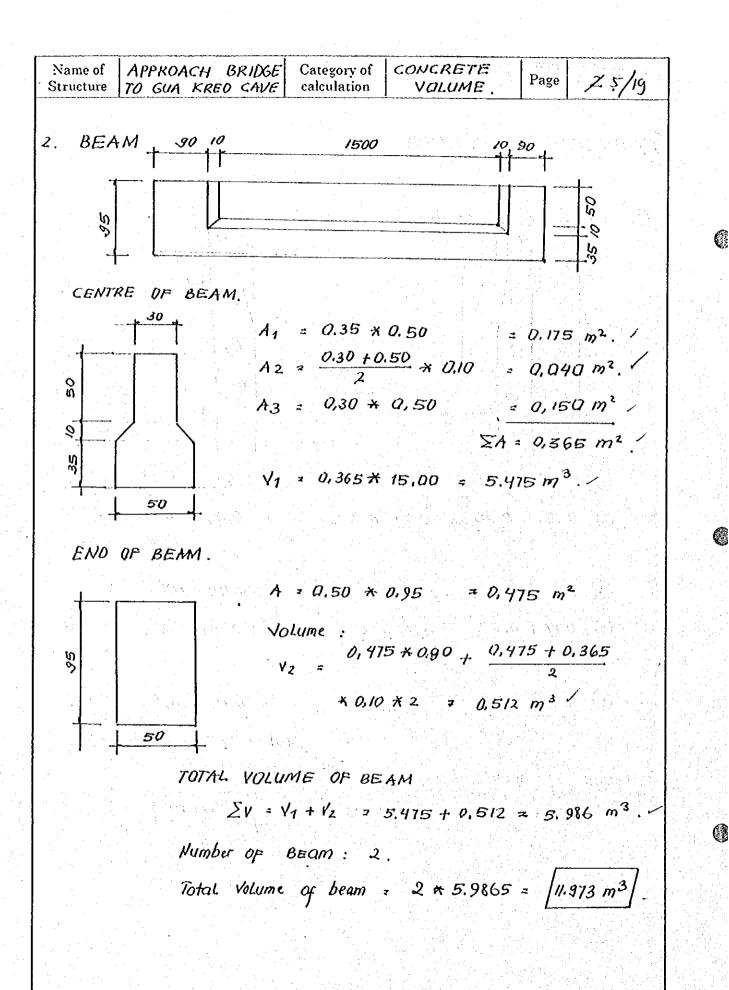
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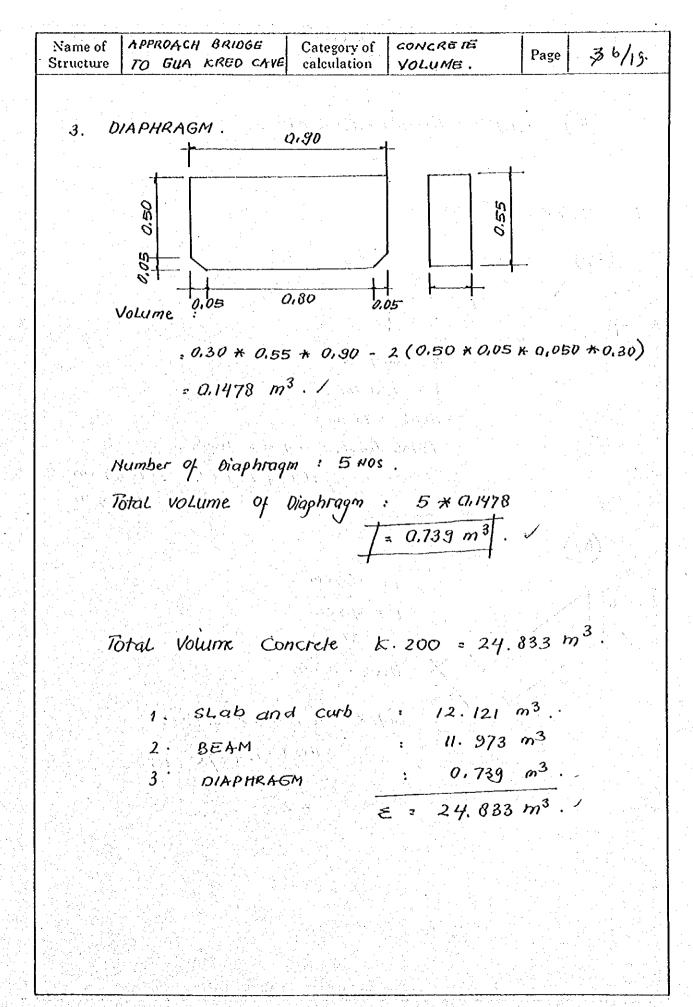
Name of Structure	APPROACH BRIDGE ID KREO CAVE	Category of calculation				Page	2/19
			<u></u>			<u> </u>	· · · · ·
	SUMMARY		I.I.	Qu	ለተጠላ		
	2 (. · · · ·) - 1	~ <u></u>					
		•		1		an a	
	CONCRETE.		•		: *		-
	1. Shab		12	121	m ³ ·		
	2. Beam.	3	- 11	973	m ³ .		
	3. Diaphragm	• •	0.	739	m ³		
		٤ :					
			· · ·	· .	· · ·		
٦	Form work.			(1994) 			
• •**	FORM WORK .				1		
	1. Slab			246		,	
	2. Beam.	and the second		. 128		· · · ·	
	3. Dinpthaym		6	30	m ² .		
		- 2 :	165	5,6	7 m².		
					.	•	
Ĩ	REINFORCINE.		· .			· · · ·	
			- F	• - •	0	1.	
	1. Beam .		- , - t.,	23.		 	
	2. slab.			2	~ ~ ·		
	3. Diaphraph	1. :	19.2	1	in the second		
	4. auto.		93	0	lig .		
		2.	440	74	Reg .		
					v Series (V) Series (Series (i de su L'he su L'he su fe	



APPROACH BRIDGE CONCRETE Name of Category of X. 4/9 Page Structure TO GUA LEGO CANE calculation VOLUME . DCONCRETE VOLUME (1) slab and curb. 17 8 Ø Ŝ Ī ს 3 0 9 Ô _{ch}ũ_c **(**2**)** ហ្ $\textcircled{\baselinetwidth}{\b$ () 0.20 * 0.200 = 0.40 m² (2) $0,15 \neq 0,20 \neq 0,22 \neq 2 = 0,077 m^2 \sqrt{}$ = 0,044 m². (3) 0.22 × 0,10 × 2 $(4) \quad 0.17 + 0.22 \\ * 0.40 * 2 = 0.156 m^2.$ $\underbrace{\begin{array}{c} \textcircled{6} \\ 0.20 \\ 2 \end{array}}_{2} \underbrace{\begin{array}{c} 0.25 \\ * \\ 0.08 \\ * 2 \end{array}}_{2} \underbrace{\begin{array}{c} 0.08 \\ * 2 \end{array}}_{2} \underbrace{\begin{array}{c} 0.036 \\ m^{2} \end{array}}_{2} - \underbrace{$ iotal = 0.713 m². / Length of spans L = 17.00 m. Volume E.A.L = 0,713 × 17 = 12.121 m³

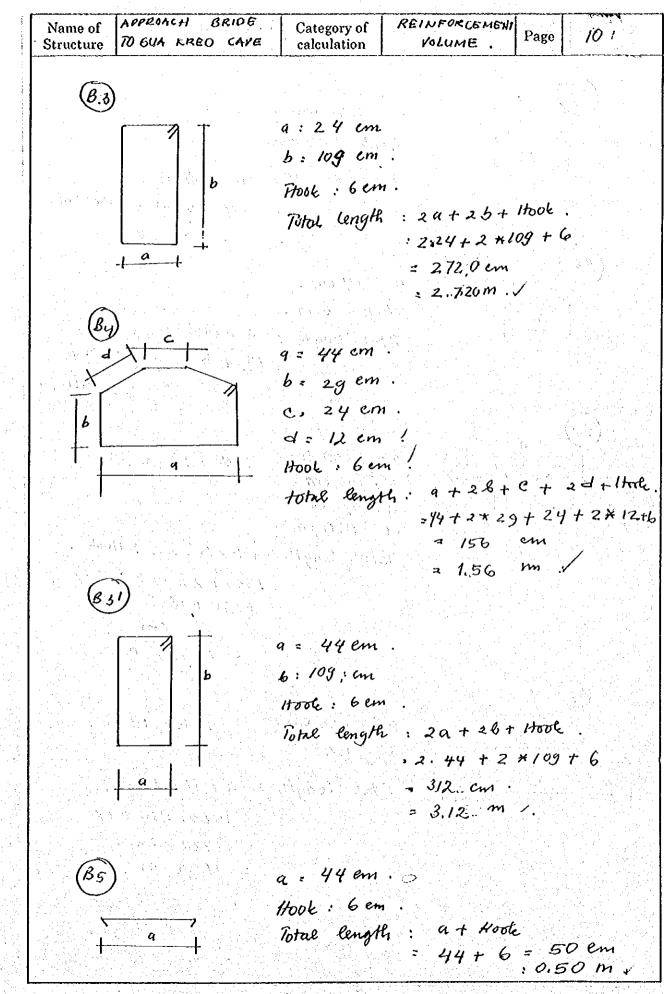
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Name of Category of -91 Page Structure calculation BRIDGE REINFORCEMENT. Ī BEAM : 1. C B, 0 α. a = 1694 cm ... Hook : 45 cm . . Total length: a + d + hoole. 1684+150+45 = 1889 cm - 18.89 m Bz 9 = 160 cm . 6 = 40 cm c = 37.5 cm. Hook: Is em . q Topl. 4+b+e+ 1took - 160 + 40 + 37.5 + 15 = 252, 5 cm = 2.53 m . / ()4 - 26



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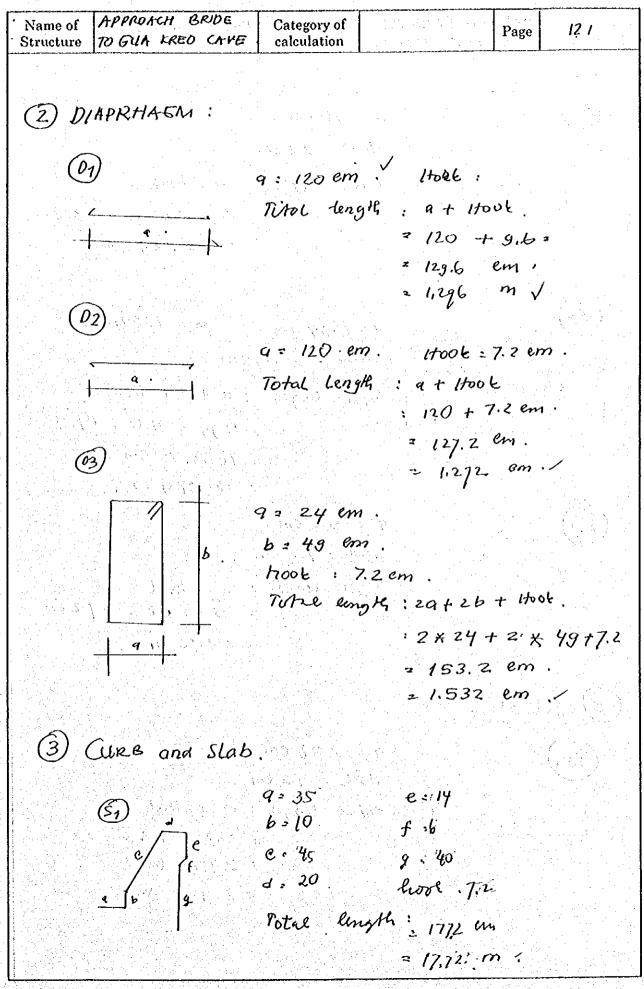
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APPROACH Bridge Keinporcement Name of Category of 111 Page to GUA KRED CAVE calculation a Miga Structure Volume a: 24 em. Avok . 6 cm . q. Total length : a + Hook . 24+6 m = 30 cm. = 0.3 M. J $q = 94 \, em$. Hook : 6em . Btal length: a + Itook. : 8g. + 6 em = 95 em. = 0.95-111. в.ь a a = 1500 cm. Hode : 18 em. b = 14. cm. c. 37 em. d= 150 cm. Total length: 4+2b+2c + 1toole = 1500 + 2 * 14 + 2 * 87 +150 + 18 = 1870 Cm ~ 18.70 m B7 4: 1560 cm. Hook : 18 cm. d , 150 q Total length: at d+ Hook. 1560+150+18 1728 em. 17.28 M

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Name of Structure	-	CH BRIDGE KRED CAUF		REINFORCEMENT VOLUME.	Page	13
	· · ·					
(Sz)			9:16 em			
· · · · · ·		-	Hook 7.20			
	<u>q</u> .			n: at hook		
				: 16 + 7.2	· · · ·	1
				: 0.232 n	1.1	
$\overline{\mathbf{C}}$						
(S3)		d.	a: 1694 en	n. d: 19	50 em	
			17006 · 1	service and the set of the set		
	9		Total length	a + d + d	ltoo b	
				= 1694 t ^I E	50 +	14.4
				= 1858,40	가 가 봐	
				2 18,584	m ,	
(54)			9 = 50 em			
r	a.	X	Houle: 7.2			
	Κ.,		Total lengt	$\frac{1}{5} = \frac{1}{50} + 7.2$		7.2 cm .
				= 0,572 m	. /	
$\overline{\mathbf{a}}$						
(3) 5	Lab					
(Tua)			9=2388	m .	N	
			Itoole : 7.2			
	۹.		total leng	h: a + trok		
•		T av		= 238 + 7. = 2452 lm		
				= 2.452 M		

Reinforcement APPROACH BRIDGE Name of Category of 14 Page Structure TO GUA KRED CAVE calculation Volyme . Tub 9:200 em . b= 24 cm. 11006: 7.2 cm total length : a + 2b + Hook a = 200 + 24 + 2 + 7.2 2 255,2 em = 2,552 m Tp L q= 1694 em. Hook: 21.6 em. d: 150 cm à Total length: a+d+ Hook 1694 + 150 + 21.6 2 1851.2 em = 18.512 m EXPANSION JOINT WIR CLOHNE PRESS. TYPE 9: 200 em Number of capannion joint : 5 Total length: 5 * 2 * 200 = 2000 en = 20 m.

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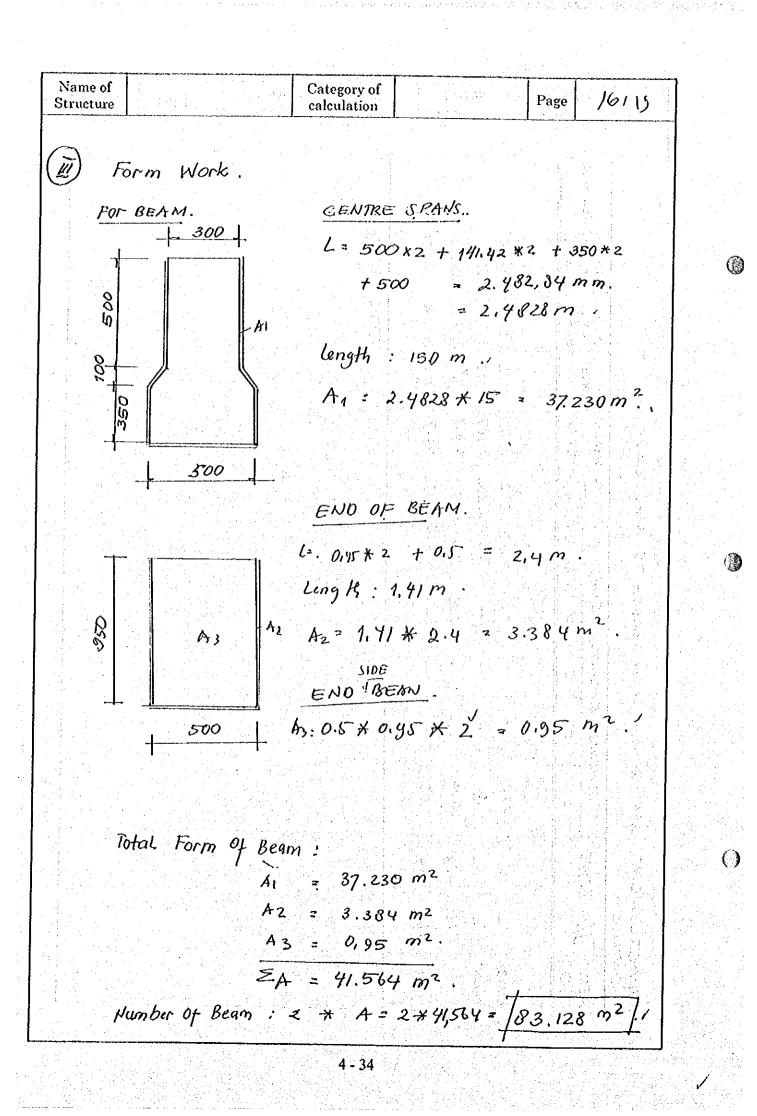
ne of cture		ррдол 5 GU			210E	Sec. 1997	Ca ca	atego Icul	ory o atior	f			R E 11 DR CE		Page		19	5/1
	KT MAKK					· · · ·					.	- 2257			العبر.	· · · · · · · · · · · · · · · · · · ·		
752102	WG/G7/ C K29).	1:454.53	155.85	341. 81	107.63	136.04	.97.33	37.70	4.682	46.08	85.156		81.752	11.283	40.767	187.04	134.67	430.25
0V V	0 F B F A M	ત	า	Ч	3	N	Ч	ч	ત	ы	~ ~	 	ŝ	h	h	4	۲.	1
0-1071114	Vaamber Vaamber	10	8	102	7 7	102	158	102	4	N	1		¢	Ч	Q	80		26
WEIGHT	(Kg/m)	3.850	3.850	0.616	0,616	0.616	0.616	0.616	D.616	0.616	0.616		1.577	0.887	0.887	0.837	0.887	0,357
· · · · · · · · · · · · · · · · · · ·	LENGH	18.890	2.530	2.720	3.120	1.560	0.50	0.300	0.950	18.700	17.280		1236	1.272	1.032	2.452	2.552	18.656
	hook	45 -	j,	Q	٩	ى	Q	9	9	18	18		2. 9	72	7.2	7.2	7.2	21.6
U Ccm)	0	150		1	1	32	1	1	I	ŝ	150		١	1	1.11 (S. 17) 1. (T. 17) 2. (T. 17)	1	1	150
NOISNEWIO	U	1 1	8 M	1	1	24	1		J	87	1	X	1	1	1	1	1 . 1 .	1
	9	1	05	501	109	52	. 1	1	1	14	1		1	l	64	1	24	١
BENDING	0	1634	160	24	44	44	44	24	\$8	1500	1500		120	120	24	238	200	1634
TVPF	, , ,	8,	83	â	B3,	84	ß5	ê B	BG	Bć'	87		<i>D</i> 7	02	03	Tua	Tub	70
	0/4	22	22	10	2	Q	5	01	á	01	9		9	/2	3	12	12	12
REINE				BEAM										OIAP.		SLAB		

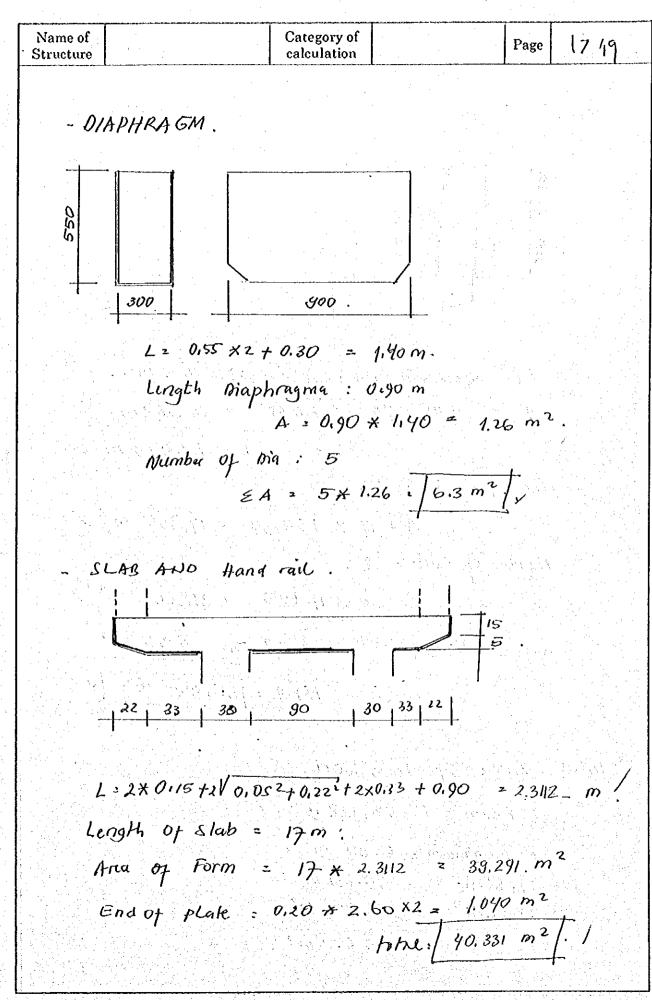
4 - 32

Name Structi		APPROACH BRIDE TO GUA KRED CAVE	Category of calculation	CONCRETE REINFORCEMENT	Page	HT.15	19
	· .						r
kreo cave	REWARK		230 7707		•		
	TOTAL	540.687 70.79 230.78 87.27	4402.07				
	V v v O a v V v	N N N N	10742				
	NUMBER	/72 /72 7 86					
	WEIGHT (Ky/m)						
	TOTAL LENGTH	(m) 1.772 0.232 18.584 0.572					
0+ 1-	n) (n	1.2 7.2 7.2 7.2 7.2 7.2					
REINFORCEMENT BAR OF APPROACH BRIDGE CONTINUE	N Ccm)						
	DIMENSION	, , , , , , ,					
	BENDING DI						
	BEND	170 170 1694 50					
	TYPE	25 25 25 25					
	0/4	2 2 2 2					
	REINF	CURB/					

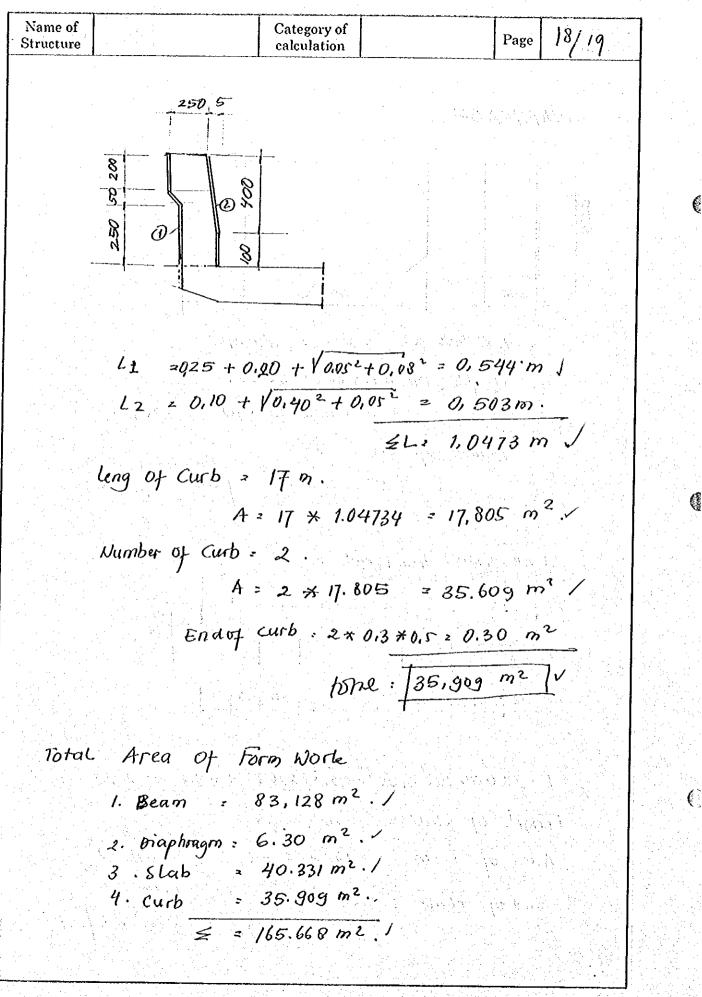
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4-36

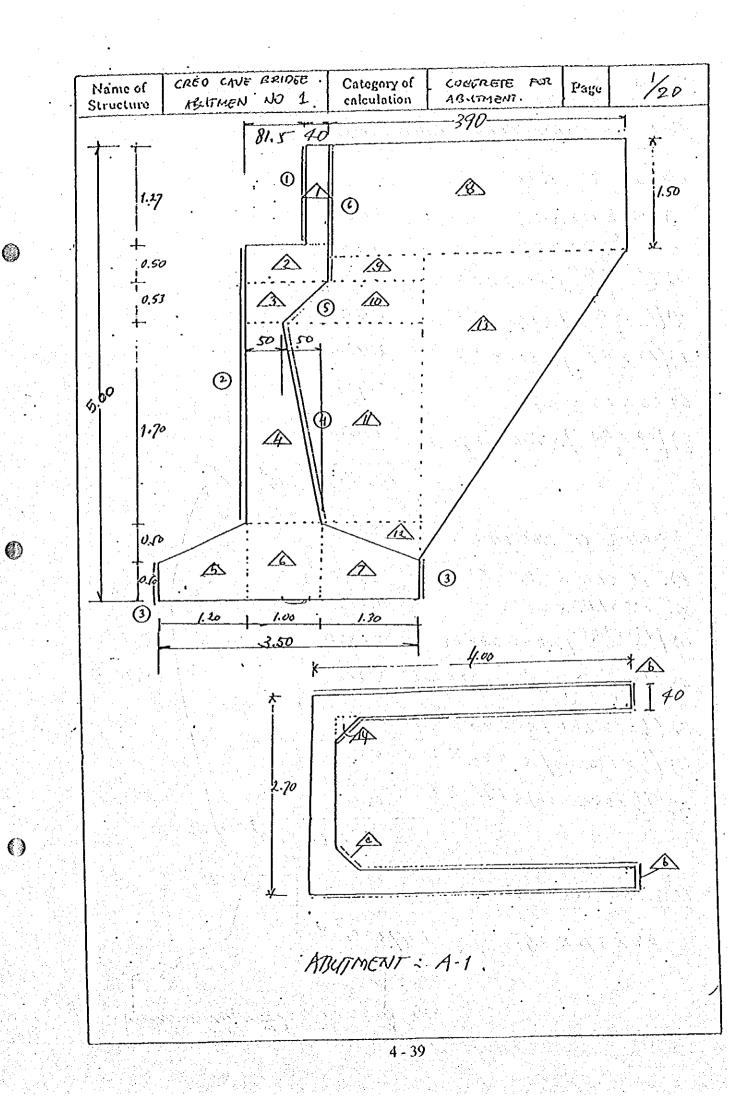
Name of Structure	APROACH BRIDGE TO GOA KRED CAVE	Category of calculation	YOLUME OF PAVEMENT.	Page	19/19
V	Pavement :				
	Width of 1	hattic	: 2.00 m .		
	length		17.00 m		
	A : (0,03 + .Volume : 1	0,05 //2 * 2 7.00 * 0.08	$= \frac{1.36 \text{ m}^3}{1.36 \text{ m}^3}$		
¥ ·	PVC Drainage pip	こうようき きちょう おうきか			
	PVC Ø 10				
		: 1.20 M G : 1.20 *	5*2 1 1	2,00 M	
Ţ. Į	Number of	ridge : 1 handrail : hand rail	7 m : 2 → Mght = : 2 * 17 =	e Lefth 34 r	7
Ð	. €×panhion joir	#: 2*	2. = 4m		
	. Bearing Shi	0e:2*	2 = 4nos		

Approach Bridge to Goa Kreo Cave (Sub-structure/Aproach Road) Work Quantity

			-			-			
gravel		۳ ۲	0	0	() () () () () () () () () () () () () (- 1974) - 1974) - O -	0	30.78	30.78
asphalt		tonne	0	0	0	0	0	17.71	17.71
ween hole	(PVC 50)	No.	0	0	0	0	0	70	70
wet stone masonry		m³	42.17	86.33	0.00	0.00	0.00	7.80	136.3
· •		m3	1.07	1.07	1.37	2.26	1.76	0.00	7.53
scaffolding		m²	00.0	0.00	0.00	27.58	14.98	0.00	42.56
form work		m²	130.06	130.06	38.60	43.20	39.80	27.84	409.56
reinforcing bars		kg	1,819.19	1.819.19	1,595,15	3,363.33	2,587.11	289.15	11,473.12
concrete	(C-1)	° ٤	30.10	30.10	19.19	43.67	30.68	1.04	154.784
embankmen t		°e	0.00	0.00	0.00	0.00	0.00	103.29	103.29
backfilling		ືຍ	92.40	92.40	155.78	245.63	293.27	0.00	879.48
excavation		"E	124.48	146.35	168.52	273.21	310.72	0:00	1023.28 879.48
ripping of top soil		, E	0	0	• 0	0	0	47.26	47.26
		unit	Abutment A-1	Abutment A-2	Pier P-1	Pier P-2	Pier P-3	Approach Road	total
	of top excavation backfilling t concrete bars form work scaffolding concrete masonry ween hole concrete	stripping of top embankmen reinforcing reinforcing leveling wet stone asphalt soil excavation backfilling t concrete bars form work scaffolding concrete concrete (PVC 50)	excavationbackfillingembankmen treinforcing concretelevelingwet stoneasphaltexcavationbackfillingtconcretebarsform work scaffoldingconcretemasonryweep holeconcrete m^3 m^3 m^3 m^3 m^3 m^2 m^2 m^3 montonne	stripping of top soilexcavation excavationembankmen embankmenreinforcing harsleveling form workwet stone scaffoldingasphalt concrete m^3 m^2 m^2 m^2 m^3 m^3 $no.$ 0124.4892.400.0030.101,819.19130.060.001.0742.1700	stripping of topembankmenreinforcingreinforcinglevelingwet stoneasphaltsoilexcavationbackfillingtconcretebarsform workscaffoldingconcreteasphalt m^3 m^3 m^3 m^3 m^3 m^2 m^2 m^2 m^3 m^3 No. m^3 m^3 m^3 m^3 m^3 m^2 m^2 m^2 m^3 m^3 No. 0 124.4892.400.0030.101,819.19130.060.001.0742.1700 0 146.3592.400.0030.101.319.19130.060.001.0786.3300	stripping of top soiltexovation excavationbackfilling ttreinforcing barsleveling form workleveling scaffoldingteveling masonryweep hole concreteasphait asphait m^3 m^3 m^3 m^3 m^3 m^3 $No.$ $concrete$ 0 124.4892.400.0030.101.819.19130.060.00 1.07 42.17 0 0 0 146.3592.400.0030.101.819.19130.060.00 1.07 86.33 0 0 0 146.3592.400.0030.101.819.19130.060.00 1.07 86.33 0 0 0 168.52155.780.0019.191.595.1538.600.00 1.37 0.00 0 0	stripping of top soilackfilling excavationembankmen backfillingreinforcing toncreteleveling barsket stone barsasphalt concrete m^3 m^3 m^3 m^3 m^3 m^3 No.(PVC 50) m^3 m^3 m^3 m^3 m^3 No.tonne 0 124.4892.400.0030.101,819.19130.060.001.07 42.17 00 0 146.3592.400.0030.101,819.19130.060.001.07 42.17 00 0 146.3592.400.0030.101,819.19130.060.001.07 42.17 00 0 168.52155.780.0030.101,819.19130.060.001.07 86.33 00 0 168.52155.780.0030.101.819.19130.060.001.07 86.33 00 0 168.52155.780.0030.103.36.3343.202.7582.260.0000	Stripping of top soli embankmen reinforcing to reinforcing scaffolding leveling to eveling set score ver stone masonry asphait set scaffolding set score to asphait (PVC 50) asphait scaffolding set score concrete asphait scaffolding set score (E) assonry wees hole concrete asphait concrete m^3 m^3 m^3 m^3 m^3 m^3 No. (PVC 50) $nore$ 0 124.48 92.40 0.00 30.10 1,819.19 130.06 0.00 1.07 42.17 0 0 0 124.48 92.40 0.00 30.10 1,819.19 130.06 0.00 1.07 42.17 0 0 0 146.35 92.40 0.00 30.10 1,819.19 130.06 0.00 1.07 42.17 0 0 0 146.35 92.40 0.00 1.310.06 0.00 1.07 86.33 0 0 0 0 273.11 235.33 43.20 </td <td>Strupting of top soil exervation excavation backfilling t concrete bars reinforcing form work seaficient correte bars leveling form work seaficient correte masonry essenty wees hole (PVC 50) asphalt unit m^3 m^3 m^2 m^2 m^2 m^3 No. Abutment A-1 0 124.48 92.40 0.00 30.10 1,319.19 130.06 0.00 1,07 42.17 0 0 Abutment A-1 0 146.35 92.40 0.00 30.10 1,319.19 130.06 0.00 1,07 42.17 0 0 0 Abutment A-2 0 146.35 92.40 0.00 30.10 1,319.19 130.06 0.00 1,07 86.33 0</td>	Strupting of top soil exervation excavation backfilling t concrete bars reinforcing form work seaficient correte bars leveling form work seaficient correte masonry essenty wees hole (PVC 50) asphalt unit m^3 m^3 m^2 m^2 m^2 m^3 No. Abutment A-1 0 124.48 92.40 0.00 30.10 1,319.19 130.06 0.00 1,07 42.17 0 0 Abutment A-1 0 146.35 92.40 0.00 30.10 1,319.19 130.06 0.00 1,07 42.17 0 0 0 Abutment A-2 0 146.35 92.40 0.00 30.10 1,319.19 130.06 0.00 1,07 86.33 0

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KRED CAVE BRIDGE CINCKETE FOR Name of Category of 3/20 Page Structure ABUTMENT. No. 1. ABUTMENT. calculation BILL OF GUANTITY ABUTMENT A-1 : CONCRETE - K225 1) 0.4× 1.22 × 2.7 1.3716 2) 1.215 x 0.5 X 2.7 = 1.6403 3) $\left(\frac{1.215+0.5}{1}\right) \times 0.53 \times 2.7 = 1.2271$ 4)[1.0 + 0.5] × 1.70 × 2.7 = 3.4425 $\int \frac{1.0 + 0.5}{2} \int x 1.20 \times 2.7 = 2.4300$ = 2.700 1 6) 1.0 × 1.0 × 2.7 $7 = \frac{1.0 + 0.5}{2} \times 1.30 \times 2.7 = 2.6325$ 15.4440, m3 VA CONCRETE OF WING WALL P). 4.0 x.1.10 x 0.40 x2 = 4.800 gy. 0.27 x1.115 x . 0 40 x 2 = 0.2560 10).[1.20+1.185.] x0.53x0.40x2 = 0.6328 30,1008 11). [1.80 + 1.30] x 2.70 × 0,40 × 2 = 3, 34 8 12). (1×1.30×0.50/× 0.40×2 = 0.260 13). (1 × 2.80×40/ ×. 0,40 × 2 = 4,490 14). (1 xo. 40x0. xo) x J. so X2 = 0.280 = 14,6568 1m³ LEVELING CONDRETE $0.10 \times 3.70 \times 2.90 = 1.07.3 \text{ m}^3 \text{ / }$ 0.10] VIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIII 3.70 4 - 40

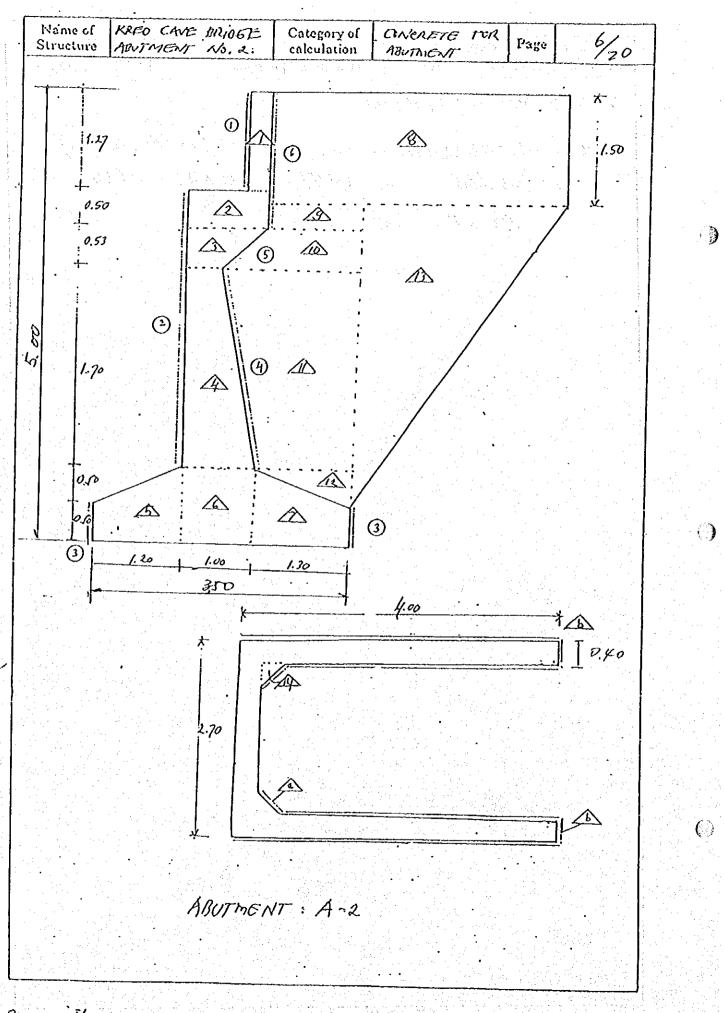
KRED CAVE BRIDGE FORM MOSA Name of Category of 3/20 Page FOR ABUTMENT Structure ABUTRIENT NO . 1. calculation FORM AREA OF ADUMENT () 1,27 × 2,70 = 3.429 (2) $3.73 \times 2.70 = 10.071^{1}$ (3) $0.50 \times 2.70 \times 2 = 2.700^{1-1}$ (4) $2.77 \times 2.70 = .7.425^{1}$ (5) $0.81 \times 2.70 = 2.187^{1-1}$ () 1.17 x 2.70 = 4.779 1 = 30,591 . 12.1 1 0.3 × 1.27×2 = 1.016 2. 1.115 × 0.5×2 = 1.215 3 1.115+0,5 × 0.53 × 2= 0, 909 A 0.571.0 x 217 x2 = 4.0500 A 05+ 1.0 × 1.20 × 2 = 1. POO. = 2.000 1.0×1.0×2 A 0.5 +1.0 × 1.30 ×2 = 1.910. = 12.940 m² FORM AREA OF WING WALL : (4,0×1.50×2)×2 = 24.00 = 1:280 (0.27 X1.181X2/X 2. A (0.4VE X 5.10 x2.] X 2 = 12.445 $\Delta \left[0.4x (1.10 + 5.0] \right] \chi^2 = 5.200 \frac{110}{7}$ $= 86.507 M^2$ TOTAL FORM AMEA: 30.591 + 12.9Kg + 86.529 = 130.060 m2. 4 - 41

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WET. TONE MA Pago Name of KRED CAVE BRIDGE Category of 4/20 ABUTMENT. No. 1. Structure calculation JONRY FOR APUTAL STONE MASONRY WET 4.00 0.60 11.60 1=2.00 2.00 Pite drain 111 CROSS REDION I.I. •) VOLUME WET STONE MADNRY : 0-E-TO TO GROTT REPTON I - I O[(0.50+1.00] x(2.0+05 jx 11.60] x 2 = 11.70 m. = 20.416 m³. (2) (0.55× 1.60 × 11.60) ×2 = 42.166 103 14 EMBANK MENT : = 40.950 m³ 1 A (1 x 2.50 x 15.60 / X 2,10 * PIPE DRAIN PUE \$50 mm. Length: 1000 mm 12×2 = 24: piec. RX2 = 24 unite palmpibne. 1 · EXCAVATION (0.50 × 1.80 × 11.60 / x 2 = 20,88 m³. / · EXCAVATION AT MINITMENT TXX 7152.00 Valume: [[1.50+4.5]x 3.50]x 3.70 m³ 350 = 103,600 · m3 · 1 +148.10 3.50 3.50 TOTAL EXCHVATION = 20,88 + 103, 600 . = 124, 420 m31

4 - 42

KRED RAVE BRIDGE ABUTMENT NO. 1. Name of Category of calculation VOLUME OF BACK -5/20 Page Structure VOLUME OF BACKFILLING VOL = VOL. EXCAVATION - VIL. ADVITCONT. (A+A+A+A) = 103.600 - (3.4425+2.43+2.70+2.6325). 92.395 M³. **(**) 4 - 43



Same with aboutment up 1.

ONCRETE FOR Name of KRED CAVE ORIDEE Category of 1/20 Page APUTMENT. Structure ARVIMENT N. 2. calculation BILL OF QUANTITY ABUTMENT A-2: CONCRETE - K225 = 1,3716 1) 0.4 × 1.27 × 2.7 = 1.6403 2) 1.715×0.5×2.7. 3) $(\frac{1,75+0.5}{2}) \times 0.53 \times 2.7 = 1.225/'$ 4) [1.0 + 0.5] x. 1.70 x 2.7 = 3.44251 $\int \frac{1.0 + 0.5}{2} \int x 1.20 \times 2.7 = 2.43001$ = 2.700 1 6) 1.0 × 1.0 × 2.7 2.6325 -7.) [1.0 + 0.5] × 1.30 × 2.7 = 15.4440 m31 CONCRETE OF WING WALL : P). 4.0 x.1.10 x 0.4 x2 = 4.20 / 30/008 gy. 0.27 x 1.115 x 0, 4 x 2 = 0, 256 / 10).[<u>1.00+1.105</u>]x00xx0.4x2 = 0.6328 - $117. \left[\frac{1.80 + 1.30}{2}\right] \times 2.70 \times 0.4 \times 2 = 3.378$ 12). (1 x 1.30 x 0.50 / x 0.40 x 2 = 0,260 $13). \left(\frac{1}{2} \times 2.80 \times \frac{40}{x} \times 0.40 \times 2 = 4.480\right)$ $13). \left(\frac{1}{2} \times 2.80 \times \frac{40}{x} \times 0.43 \times 5.50 \times 2 = 0.880\right)$ $14). \left(\frac{1}{2} \times 0.41 \times 0.43 \times 5.50 \times 2 = 0.880\right)$ = 14.6568 · m3/ LEVELING ONORETE = 1.073 m³/.... 0.10 × 3.70 × 2.90

KRED CAVE BRIDGE 8/20 Name of Category of FORM AREA FOR Page ABUTMENT NS. 2. Structure calculation MinIMENT. FORM AREA OF ADUMENT () 1,27 x 2,70 = 3.429) (2) $3.73 \times 2.70 = 10.071'$ (3) $0.50 \times 2.70 \times 2 = 2.700'$ (4) $2.77 \times 2.70 = .7.425'$ (5) $0.91 \times 2.70 = .2.717'$ · 1.77 × 2.70 = 4.979 = 3C. 53.1 h2.1 1 0.4 x 1.27 x2 ·· = 1,016 $\begin{array}{rcl} \textcircled{(2)} & \rule{(2)} & \rule{$ A 0.5+ 1.0 × 1.20 × 2 = 1. POO 1 = 2.000 * 1.0×1.0×2 $4 \frac{0.7 + 1.0}{2} \times 1.30 \times 2 = 1.950^{1} + \frac{1}{2} = \frac{1}{2}.950^{1} + \frac{1}{2}$ FORM AREA OF WING WALL : ▲ (4,0×1.50×2)×2 = 24.00 ' - 1.280 . A (0.27×1.145×2./x 2 a (1.2+1.181 x 2/x 2x0.13 = 3.16y- $\begin{array}{rcl}
& \underbrace{(1.8 + 1.30}_{X} & \underline{1.7 \times 2} \\ \times & \underbrace{(1.8 + 1.30}_{X} & \underline{1.7 \times 2} \\ & \underline{(1.8 + 1.30 \times 0.50}_{X} & \underline{1.7 \times 2} \\ & \underline{(1.8 + 1.30 \times 0.50}_{X} & \underline{1.7 \times 2} \\ & \underline{(1.8 + 1.30 \times 0.50}_{X} & \underline{1.7 \times 2} \\ & \underline{(1.8 + 1.30 \times 0.50}_{X} & \underline{1.7 \times 2} \\ & \underline{(1.8 + 1.30 \times 0.50}_{X} & \underline{1.7 \times 2} \\ & \underline{(1.8 + 1.30 \times 0.50}_{X} & \underline{1.7 \times 2} \\ & \underline{(1.8 + 1.30 \times 0.50}_{X} & \underline{1.7 \times 2} \\ & \underline{(1.8 + 1.30 \times 0.50}_{X} & \underline{1.7 \times 2} \\ & \underline{(1.8 + 1.30 \times 0.50}_{X} & \underline{1.7 \times 2} \\ & \underline{(1.8 + 1.30 \times 0.50}_{X} & \underline{1.7 \times 2} \\ & \underline{(1.8 + 1.30 \times 0.50}_{X} & \underline{1.7 \times 2} \\ & \underline{(1.8 + 1.30 \times 0.50}_{X} & \underline{(1.8 + 1.30 \times 0.50)}_{X} \\ & \underline{(1.8 + 1.30 \times 0.50}_{X} & \underline{(1.8 + 1.30 \times 0.50)}_{X} \\ & \underline{(1.8 + 1.30 \times 0.50)}_{X} & \underline{(1.8 + 1.30 \times 0.50)}_{X} \\ & \underline{(1.8 + 1.30 \times 0.50)}_{X} & \underline{(1.8 + 1.30 \times 0.50)}_{X} \\ & \underline{(1.8 + 1.30 \times 0.50)}_{X} & \underline{(1.8 + 1.30 \times 0.50)}_{X} \\ & \underline{(1.8 + 1.30 \times 0.50)}_{X} & \underline{(1.8 + 1.30)}_{X} \\ & \underline{(1.8 +$ (} A (04VE X 5.00 +2] X2 = 12485 TOTAL FORM AMEA: 30.541 + 12:140 + 86.529 = 130.060 m2. 4 - 46

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Name of KRED QUE MUDDE 9/20 Category of WET. JTONE MATCH Page Structure ARAMENT NO. 2. calculation Ry. P.R. AlyTMENT VOLUME WET STONE MASONING $\frac{\left[0.5+1.00\right] \times \frac{2.0+0.5}{3} \times 23.75 \times 2 = 44.531$ [0.55×1.60 × 23.95] × 2 = 41,800 86.331 M2. 1 27.77. + ---- + -:-4.00 TIPE ORATIN EVE \$ SUMM LENgth : 1000 MM. 23 X2 = 46 piece 23×2 = 46 unit palm fibre u EXCAVATION OF WET STONE MATENRY: (0.50× 1: Pox 23, 75/x = 42, 75 m3. 1 EXCANATION OF ADVITENT: +15200 3.50 1 -148.10 . 3.50 150 3.50 0.50 3.50; Volume . (11.50+ 4.50 × 3.50) × 7.70 = 103,600 43 PPAL ECVAVATION = 42,75 + 103.600 = 146.350 M3 1

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Name of KRED OWE BRIDGE Category of EMBANKMENT 10 / Structure Page ADITMENT NO.2. 20 calculation FOR ADAMENT. VOLUME OF EMDANK MENT. △ (2.10+ 0,50 x 2,10× 27.75/ = 62.344 M³. V VOLUME OF BACKFILLING =. VOL = W. EXCINITION - M. ABURNENT = 103.600 - (3.4425.725) + 2.437 + 2.6325 / . $= 92.395 m^3.1$ ()4 - 48

WET. STONG MATCH 11/20 KKED CAVE DRIDGE Category of Name of Page PARAPET. Structure calculation LY FOR PARADET VOLUME WET STONE MASONRY FOR PARAPET 5tripping (11,30+23,45)×(3,6+3,2)×0,2=40,26,3 25 Grovel 3.6x (11, 30+23, K\$+8)x0.2 = 30.70 80 Povement 3, 6x (1, 3+23, ×5+8)×0,05= 1, 70 3 400 50 6.400 M VOLUME = (0.PX0. 50 X.4. / * 4 1.400 47. (0.50+0.20 × 0.25/# 4 × 4 = 7.800 43 1. VALME POST CONCRETE VOLUME QONCHETE = (0. 15 X 0. 15 X 1. 45 /x (12 +10). $= 1.044 \cdot 41^{3} \cdot \sqrt{1.000}$ Florm Work = 0.15×1.45×4×(22+10) VOLUME STEEL : = 27.8×m² 145 · \$8-100 : [[0,10 x4] * 4 0.0. + 780] * 15 # 32. 10 = 75.76. . . kg. JUT 1 4\$13 : ([1.60 × 4] + 4, 70.013 * 7810] × 32. = 213.39 4 10 TOTAL STEEL: 75.76+ 2,13.39 = 289.15 4.

4 - 49

APPROACH BEIDE TU Name of Category of CONCRETE 12 ECA KRED CME. Page volume of Pier 1. Structure calculation 20 + as + 0- go + p.35 + 10.35 0.35 +-152.33 <u>ال</u>ک 0.40 20.40 ·1... 么 3 4.30 h Ð 0.40/8 G (I) 63 3 3,50 H+ 0.10 +-| 0.10 3.52 1 BILL OF QUARTITY OF. PIER-1 CONTRETE - K 225 : () 1.60 × 0.4 × 2.70 = 1.728 (2) $\frac{1.60 \pm 0.50}{2} \times 0.40 \times \frac{2.70 \pm 2.0}{2} = 1.175$ (3) $\frac{0.90 \times 4.30 \times 2.0}{2} = 7.740$ 3. 0.90 x 4.30 x 2,0 $(4) \quad \frac{0.90 + 3.10}{2} \neq 0.40 \times \frac{2.0 + 3.10}{2} = 2.420$ 6.125 (\cdot) 3.10×0.10 × 3.50 () $E = 1.369 \text{ m}^3 \text{ s}^3$ LEVELING. CONCRETE (0.10 x 3.70 x 3.90 4 - 50

ARM AREA VOLUME Page WINCAUL ERIOBE TO Category of Name of 13/20 ETA KRED ENVE CF PIER . 1. calculation Structure FORM AREA OF PIER-1 (P-1). A U.yo x 2.70 x 2 = 2.160 1 2 0.60 x 2.90 x 2: = 3.240 1 3 4.30 x 2.00 x 2 = 17.200 1 A 0.50 x 3.50 x 2 = 3.00 , A 0.40 x 1.60 x 2 = 1.280 1 Do 0.53 x 1.60 x 2 = 1.696 1 A 4.30 × 0.70 × 2 = . 6.0201. 3.5001 0.50×20×2 = 38.5% m² <u>+111</u>.00. EXCAVA TION 4.77 +150.00 + 146.23. 1XA 4.77 450 Volume = 4.50+ 13.04 × (3.50+1.00/x 4.77+3.77. = 168.52 m³ VOLUME OF BACKFILLING Vie = Vie Exervation - Voe Pien-1. (0+9.0). = 168.520 - [(0,7× 2,0× 3,3) + 2,420+ 6.125] = 155.775 W3 J

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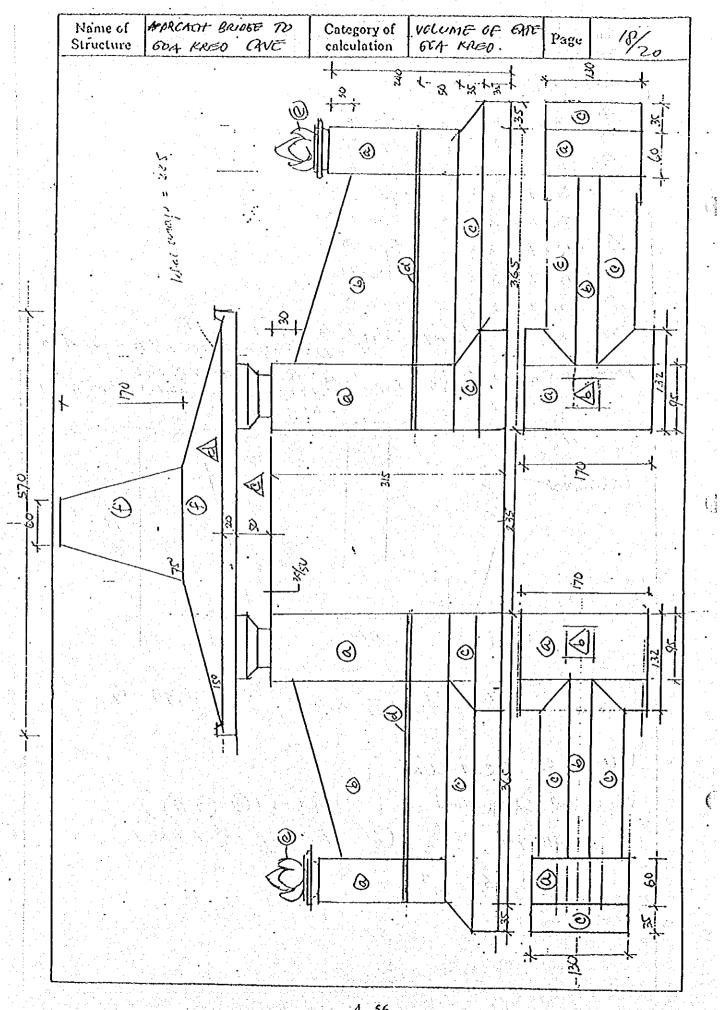
APPRCACIT BRIDEE TO UNDERE VOLIME 14/20 Name of Category of Page Structure ATTA WED GIVE calculation OF MER-2. 0.15 1.30 0.15 -10-31--2.00 10.3,-1 + 152.33 0.40 .: 5 040 10 600 8.00 /27 15: (3) 14. Ā 3 (5) $\mathcal{L}^{\textcircled{}}$ 12.33 0. i 4.0 ĥ BILL OF GUANTITY OF PIER-2: CONCRETE K-225 = 1.728 . (1) 1.60 x 0.40 x 2.70 (2) <u>1.60 + 1.30</u> × 0.40 × <u>2.30 + 2.0</u> = 1.363 . 3 . 1.30 x 8.00 x 2.0 = 20.80D. (4) $\frac{1.30 + 4.55}{2} \times 0.40 \times \frac{2.0 + 3.5}{1} = 3.2181$ () (4.55× 0.80× 4.55 · = 16:562 43.671 m3 LEVELING QHORETE 6 0.10 × 4.75 × 4.25 = .2.256 m³ 4 - 52

FORM AREA VILLAE APRIONELL GRIDEE TO Category of 15/20 Name of Page OF MOR-2. EVA KRED ENE. calculation Structure FORM AMEA OF PIER-2 (P-2). か~ 0.40 x 2.70 x 2 = 2.160 ዀ~ 0.60 x 2.70 x 2 = 3.240 m Δ 8.00 x 2.00 x 2 = 32.000 h~ / 5 600 4 0.800×3.50×2 = ... 0.40×1.60×2 = ٨~ 0.40x 1.60 x 2 = 1.280 26 0.57 × 1.60 × 2 = 1.696 M 1 m~ 1 8.00 x 1.20 x 2 = 19.200 A A w. 0.80 x 3.10 x 2 = 5.600 FORM ANEA OF P2 (HEIGHT & J. POM): A+A+A+A = (JX2X2/4-5,10+JX/2X2+J.6 = 43.20 M2-FORM MEA OF P2 (HEIGHT > 5.00 m]: A+A+A+A+A+A. = 2.16+3.24+1.28+1.696+(3×2×2)+2×1.2×2 (JEAFOLDING) = 27.576 m . EXCANATION. +147.00 +147.00 4.77 +14/2.23 -14:27 4.77 + 5.55 $Valume : \frac{5.55 + 15.09}{2} \times 4.77 \times 5.00 = 273.207 m^{3} \cdot 1$ VOLUME OF DATERFILLING Vol = Vol. Exchumin - Vrl. MEn-2. (0+0.0) [(1.30 × 2.0 × 3,00] + 3,2181 + 16.562] = 273.207 -= 245, 627 m²·1 4 - 53

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andrete vount Page KPPRINTH GROBE TO Name of Category of 16/20 er4 KREG ONE Structure EF PIER-3. calculation 10.29 1.10 0.29 +152.33 2.00 0.75 <u>/:-</u> in 6:10 Ref (\mathbf{J}) : 21 C 0.40 \odot 0.70 (6) 44.33 f10-4.00 ĥ íf BILL OF QUANTITY OF PIER-3 ONTRETE K-225. 1.60 x 0.40 x 2.70 = 1.728 (1) $\frac{1.60 + 1.10}{2} \times 0.40 \times \frac{2.70 + 2.0}{2} = 1.269$ 3 1.10 x 6.10 x 2.0 = 13.420 (y) 1.10 + 4.00 x 0.40 x 2.0 + 4.00 = 3.060 $= 11200 \\ = 30.677 m^3 +$ 1 4.00 × 0. 70 × 4.00 ()LEVELING CONCRETE = 1.764.m². 6. 0.10 x 4.20 x 4.20 4 - 54

APPROACH BRIDGE TO FERN ARCA VOLUME 11/20 Category of Name of Page BOA KRED ONE OF- PIEN-3 calculation Structure FORM AREA OF PIER-3 $(1) 0.40 \times 2.70 \times 2 = 2.160 m^{-1}$ $(2) 0.60 \times 2.70 \times 2' = 3.240 m^{-1}$ 3 6.10 x 2.00 x 2 = 24. 400 h~1 Ly 0.70x 3.50 x 2 = 4.900 h~1 \$ 0.40× 1.60 × 2 = 1.280. m (0.53 x 1.60 x 2 = 1.696 m A 6.10 × 1.00 × 2 = 12.210. m € 0.70× 3.50 × 2 = 4.900 N-1 FORM MEA OF P3 (HEIGHT & J. M/: B+A+A+A = JX2X2+ 4.9+ JX1X2+ 4.90 - 39.20 m FORM AREA OF P3 (HEIGHT > 5.00 / = D. + D + D + D + D'+4) - (SCAFOLDING / - (S = 14.936 m +150.00 TXX. 755 EX EAVATION-5.77. + 144.23 5:77. 1.00 15.17 = 310,715 m³ / V.007 16.54 x 5.77 X 5.00 Volume : VOLUME OF BACKFILLING Vie = Val. Etchnation - Vie. MOR-3 (Ot(9+6)) = 310, 715 - { (1.1× 1, 45× 2]+ 3.060+ 11.200] = 293,265 m3.



4 - 56

AIMCADT BRIDGE DO Name of VELUME OF SATE Category of 19/20. Page Structure OVE GTA KRED GCA- KRED calculation VOLUME OF GATE GOA KRED : H.40 m3 -> EXCAVATION 2 ·) CONCRETE m 0.60 A TAPENED FOOTING m A COLUMN 0.7 5 & BEAM 0.41 m3. **-**A JUNB OF ROOF 4.26 m3. -6. 02 m².1 -TOTAL. CONCRETE of TEMPLE STONE FOR NRFACE STRUCTURE m³/ M³/ M³/ a column 13.92 \simeq 2.24 (B) WALL = 5.11 @ TAPERES (UMPAR) <u>____</u> M 20.60 (D) LIST (ORNAMEN) = 2.00 4nit @ RATE PLOWER ORNAMEN = hi .28 Y3 SURFACT OF NOOF (F) 2 M3 1 03-FOUNDATION OF WET STONE 672 MADONRY

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All ROADT BRIDGE TO Name of Category of VOLIME OF CATE 20/20 Page Structure GTA KRED CAVE calculation GOA- KRED VOLUME OF GATE 1. GALIAN : $4 \times 1, 2 \times 2 \times 1 = 14.40 \text{ m}^3$ 400 y + 180 y 0.60 ms. 2 DONDASI FONT PLAT (BETCH). = 0,30 × 1×1×2 - 30×30 30 A KOLOKA = 0,30×0,30×4,15×2 = 0,747 m³
A BALOK = 0,30×0,50×2,70 = 0,405 M. 1 A PLAT MTRP 570 : 220 $= 1,50 \times 0,90 \times \frac{1,70}{2} = 1,1475 m^{3}$ (1,50 × 0,90) + (5,70 × 2,25) ×0,60 = 4,2625 m³. 1 3. BARICANDI ? @ KOLOMA = 1,70 × 0,95 × 3,15 ×2 = 10,1745 m3 r 1,30 × 0,60 × 2,40 × 2 = 3,744 11 $m = 13,9125 m^{3}$ (b) DINDING = $(\frac{2,15 + 1,40}{2} \times 2,11 \times 2) = 13,9125 \text{ M}.$ = 7,455 $\times 0,30$ = 2,240 M^2 (c) UNIPAR = 3,65 $\times 0,70 \times 1 \times 2$ = 5,710 M^3 (i) UMIPAR (ORNATUEN/LIST = (3,65+3,65+1,70+1,30)×2 = 20,60 m. 1 @ ORNATIEN MELATI = 2 64. () PELAPIS ATTAP = (0,60 + 1,12) × 1,70 × 2 + (0,90 × 1,70 × 2) $\#\left[\frac{n q n + 2 + 2 5}{2}\right] \times 2 + 5 \times 2 + \#\left[\frac{1 + 5 + 5 + 7 0}{2}\right] \times 2 + 5 \times 2 = 22,445 m^{2} - 5$ 4. PONDASI BATU BELAH = (1170×132×1) + (173×1,3×1)×2 = 6,72m/

work quantity of gate

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	unit	amount
excavation	cu.m	14.40
concrete (K225)	cu.m	6.02
formwork	sq.m	2.40
wet stone masonry	cu.m	6.72
temple stone	cu.m	21.27
and the second		

 2 e^{-ij}



(2)

