
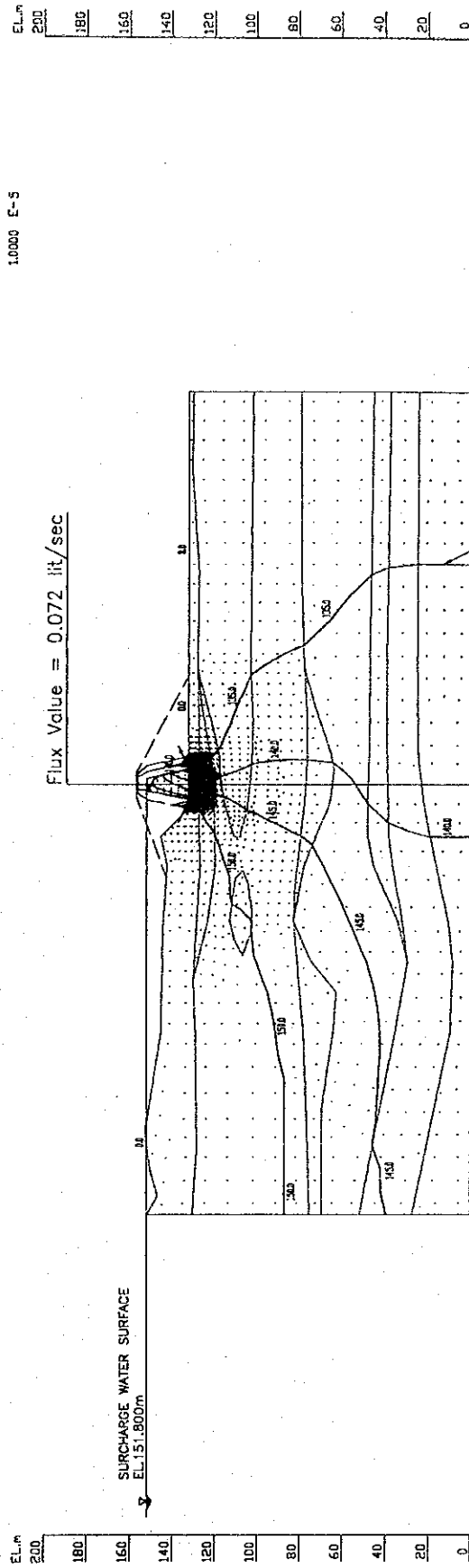


# RESULTS OF SEEPAGE ANALYSIS

CASE 1-5  
E-E Section, without grout

|  |                              |
|--|------------------------------|
| Escape Gradient Downstream of Impervious Zone    | 1.895                        |
| Exit Gradient at Downstream Toe of Pervious Zone | 0.014                        |
| Maximum Flow Velocity                            | $1.269 \times 10^{-5}$ m/sec |

  
 GED. SCALE (m)  
 20000 E+1  
 VEC. SCALE (m/sec)  
 10000 E-5



THE DETAILED DESIGN OF FLOOD CONTROL, URBAN DRAINAGE AND WATER RESOURCES DEVELOPMENT IN SEMARANG IN THE REPUBLIC OF INDONESIA


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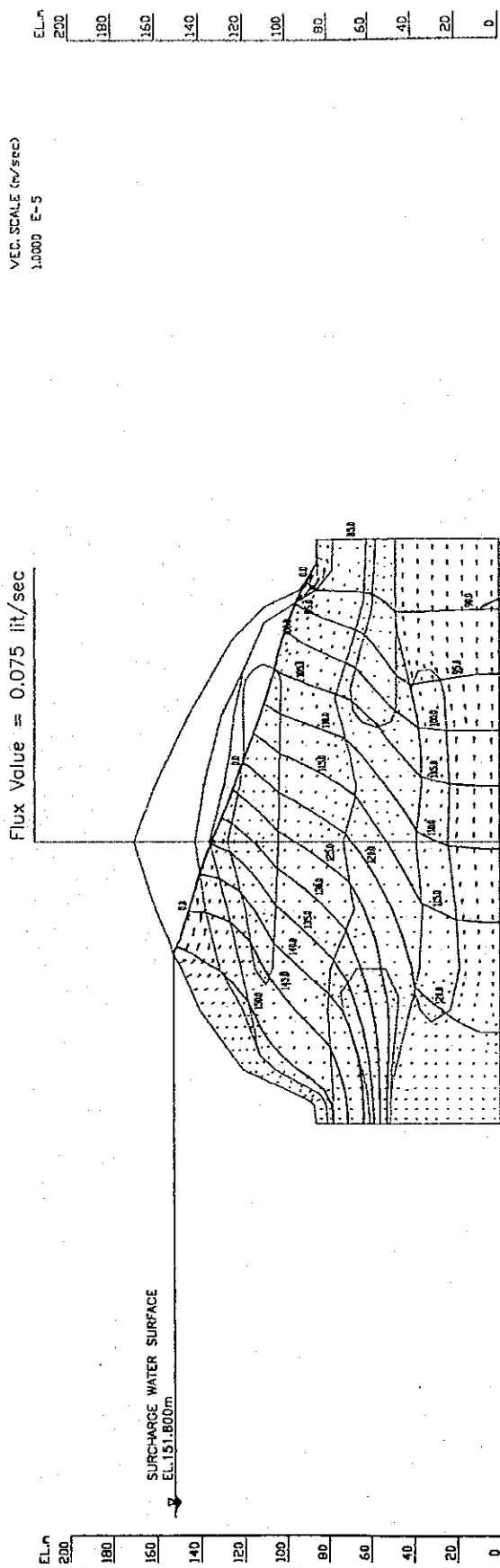
Fig. 7.2.23 (5/8)  
RESULTS OF SEEPAGE ANALYSIS (WITHOUT GROUT)

# RESULTS OF SEEPAGE ANALYSIS

CASE 1-6  
F-F Section, without grout

|   |                              |
|---|------------------------------|
| Escape Gradient Downstream of Impervious Zone | -                            |
| Exit Gradient at Downstream River             | 0.450                        |
| Maximum Flow Velocity                         | $3.017 \times 10^{-6}$ m/sec |

  
 GEO. SCALE (m)  
 2,000 E+1  
 VEC. SCALE (m/sec)  
 1,0000 E+5



THE DETAILED DESIGN OF FLOOD CONTROL, URBAN DRAINAGE AND WATER RESOURCES DEVELOPMENT IN SEMARANG IN THE REPUBLIC OF INDONESIA


JAPAN INTERNATIONAL COOPERATION AGENCY

Fig. 7.2.23 (6/8)  
RESULTS OF SEEPAGE ANALYSIS (WITHOUT GROUT)

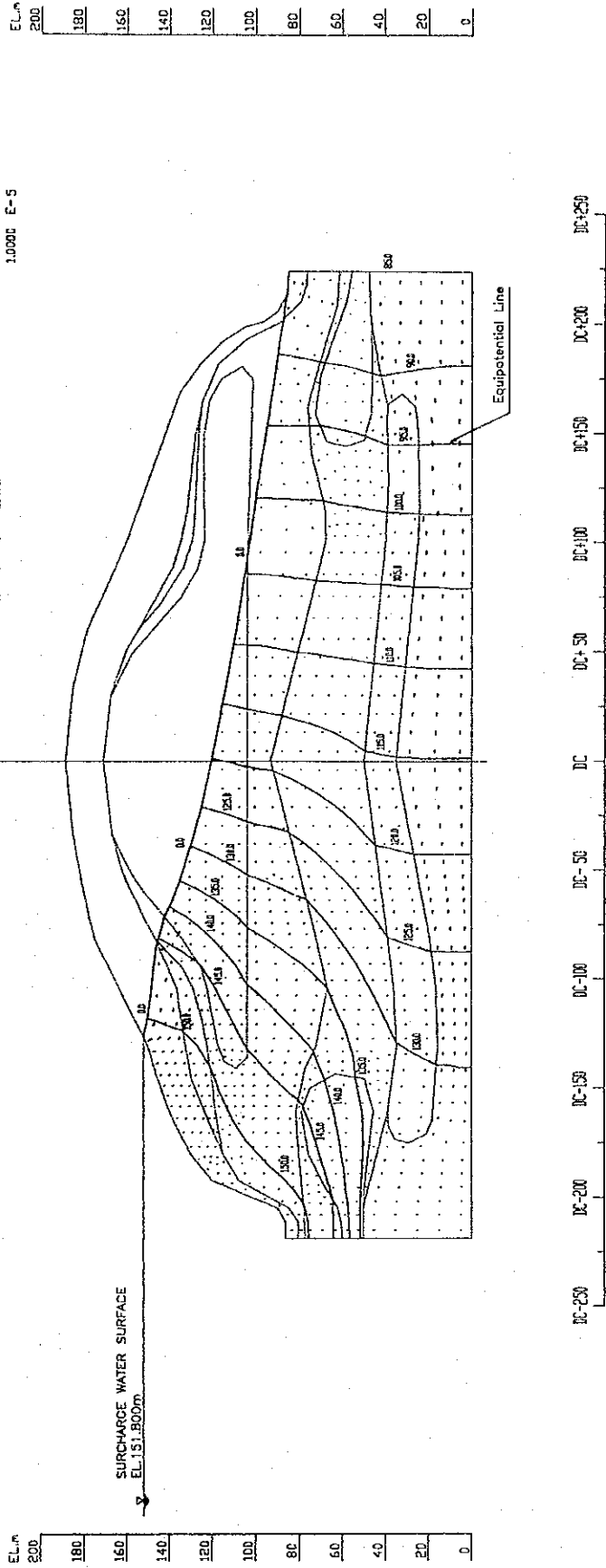
# RESULTS OF SEEPAGE ANALYSIS

CASE 1-7  
G-G Section, without grout

|   |                              |
|---|------------------------------|
| Escape Gradient Downstream of Impervious Zone | -                            |
| Exit Gradient at Downstream River             | 0.082                        |
| Maximum Flow Velocity                         | $1.099 \times 10^{-5}$ m/sec |

  
 GED. SCALE (m)  
 2,0000 E+1  
 VEC. SCALE (m/sec)  
 1,0000 E-5

Flux Value = 0.044 lit/sec



THE DETAILED DESIGN OF FLOOD CONTROL, URBAN DRAINAGE AND WATER RESOURCES DEVELOPMENT IN SEMARANG IN THE REPUBLIC OF INDONESIA

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Fig. 7.2.23 (7/8)

RESULTS OF SEEPAGE ANALYSIS (WITHOUT GROUT)

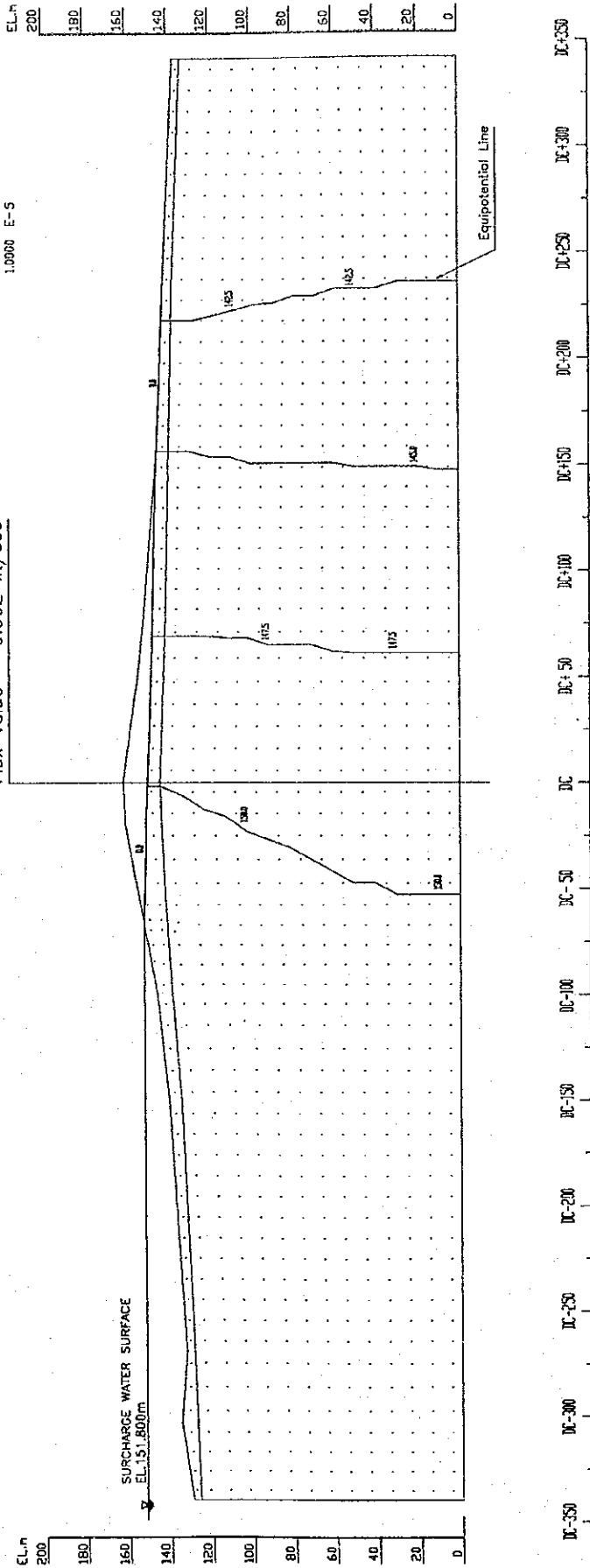
RESULTS OF SEEPAGE ANALYSIS

CASE 1-8  
H-H Section, without grout

|   |                              |
|---|------------------------------|
| Escape Gradient Downstream of Impervious Zone | -                            |
| Exit Gradient at Downstream River             | 0.003                        |
| Maximum Flow Velocity                         | $2.933 \times 10^{-7}$ m/sec |

GEO. SCALE (H)  
2,0000 E+1  
VEC. SCALE (V/SEC)  
1,0000 E-5

Flux Value = 0.002 lit/sec



THE DETAILED DESIGN OF FLOOD CONTROL, URBAN DRAINAGE AND WATER RESOURCES DEVELOPMENT IN SEMARANG IN THE REPUBLIC OF INDONESIA


JAPAN INTERNATIONAL COOPERATION AGENCY

Fig. 7.2.23 (8/8)  
RESULTS OF SEEPAGE ANALYSIS (WITHOUT GROUT)

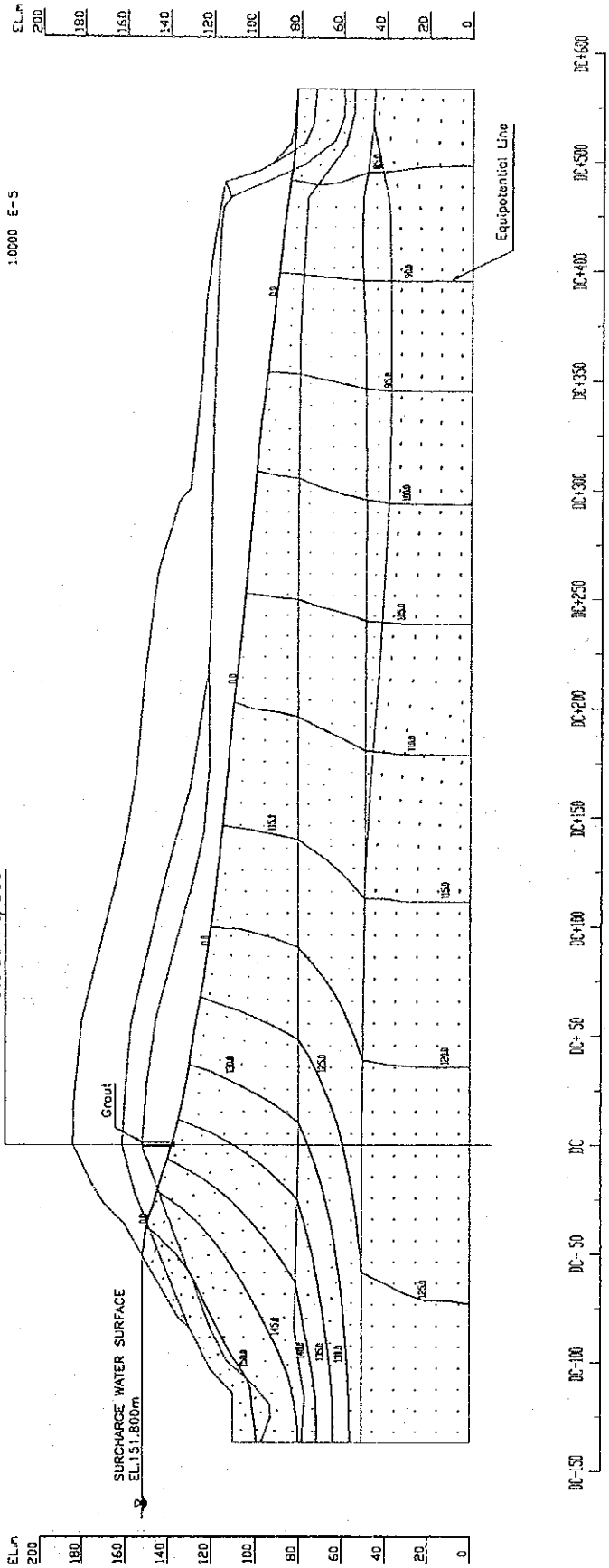
# RESULTS OF SEEPAGE ANALYSIS

CASE 2-1  
A-A Section, with grout

|   |                              |
|---|------------------------------|
| Escape Gradient Downstream of Impervious Zone | -                            |
| Exit Gradient at Downstream River             | 0.026                        |
| Maximum Flow Velocity                         | $8.151 \times 10^{-7}$ m/sec |


  
 GED. SCALE (H)  
 2.0000 E+1  
 VEC. SCALE (M/SEC)  
 1.0000 E-5

Flux Value = 0.028 lit/sec



THE DETAILED DESIGN OF FLOOD CONTROL, URBAN DRAINAGE AND WATER RESOURCES DEVELOPMENT IN SEMARANG IN THE REPUBLIC OF INDONESIA

JAPAN INTERNATIONAL COOPERATION AGENCY


Fig. 7.2.24 (1/7)

RESULTS OF SEEPAGE ANALYSIS (WITH GROUT)

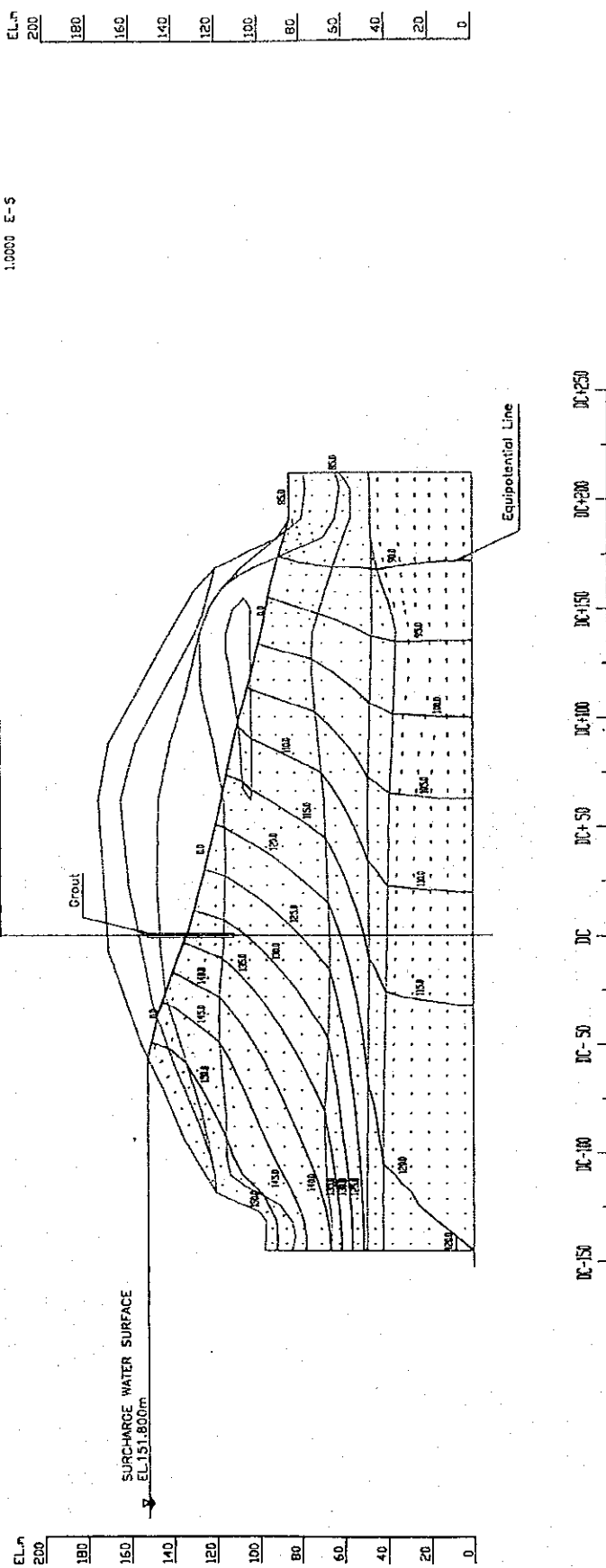
# RESULTS OF SEEPAGE ANALYSIS

CASE 2-2  
B-B Section, with grout

|   |                              |
|---|------------------------------|
| Escape Gradient Downstream of Impervious Zone | -                            |
| Exit Gradient of Downstream River             | 0.207                        |
| Maximum Flow Velocity                         | 1.388x10 <sup>-6</sup> m/sec |


  
 GEO. SCALE (m)  
 20000 E+1  
 VEC. SCALE (m/sec)  
 1.0000 E-5

Flux Value = 0.047 lit/sec



THE DETAILED DESIGN OF FLOOD CONTROL, URBAN DRAINAGE AND WATER RESOURCES DEVELOPMENT IN SEMARANG IN THE REPUBLIC OF INDONESIA

JAPAN INTERNATIONAL COOPERATION AGENCY


Fig. 7.2.24 (2/T)

RESULTS OF SEEPAGE ANALYSIS (WITH GROUT)

# RESULTS OF SEEPAGE ANALYSIS

CASE 2-3  
C-C Section, with grout

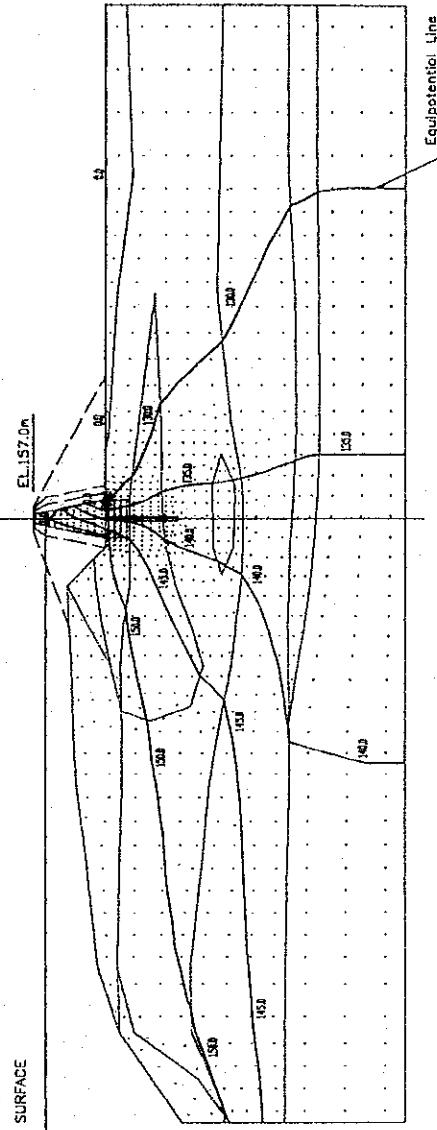
|  |                              |
|--|------------------------------|
| Escape Gradient Downstream of Impervious Zone    | 1.972                        |
| Exit Gradient at Downstream Toe of Pervious Zone | 0.009                        |
| Maximum Flow Velocity                            | 1.321x10 <sup>-6</sup> m/sec |

  
 GEO. SCALE (H)  
 2,0000 E+1  
 VEC. SCALE (V/sec)  
 1,0000 E-5

EL.m  
200  
180  
160  
140  
120  
100  
80  
60  
40  
20  
0

EL.m  
200  
180  
160  
140  
120  
100  
80  
60  
40  
20  
0

Flux Value = 0.028 lit/sec



DC-300 DC-250 DC-200 DC-150 DC-100 DC-50 DC DC+50 DC+100 DC+150 DC+200 DC+250 DC+300

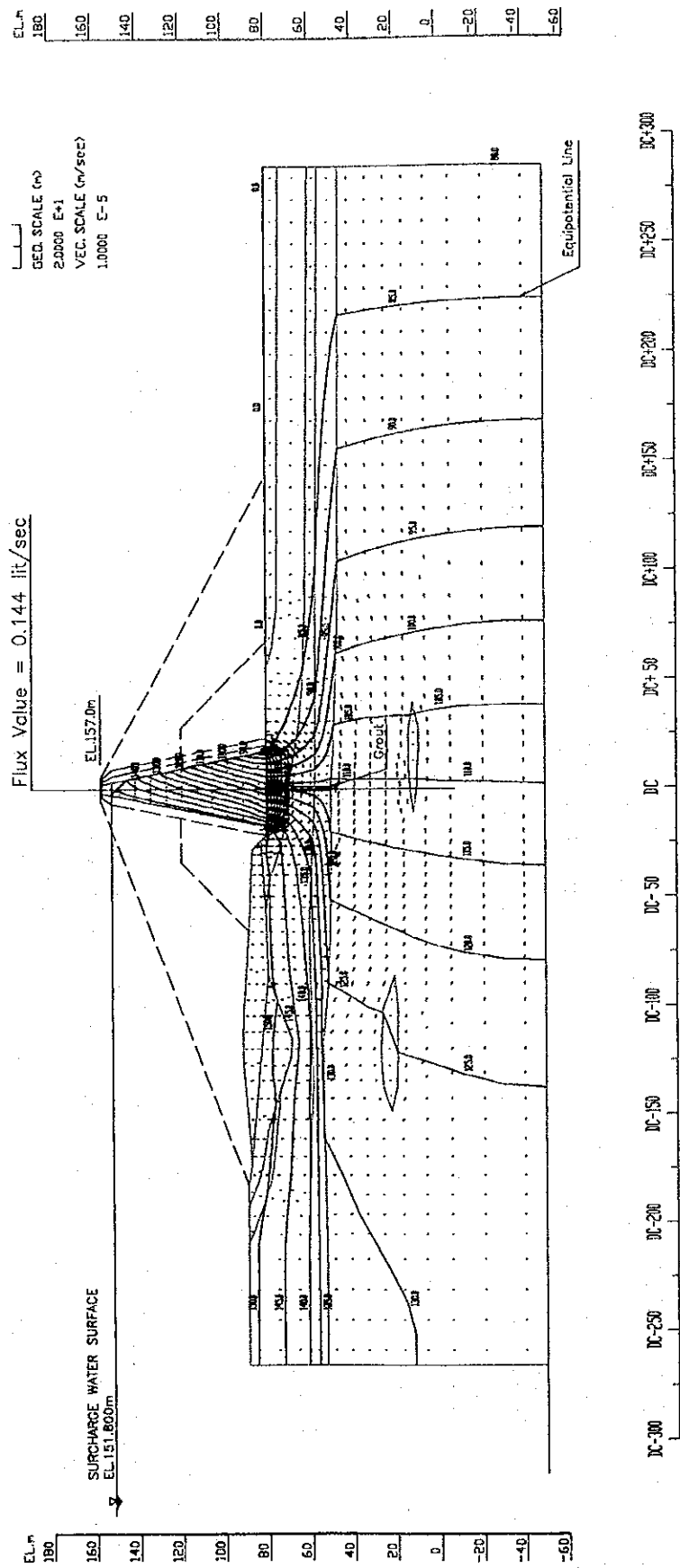
THE DETAILED DESIGN OF FLOOD CONTROL, URBAN DRAINAGE AND WATER RESOURCES DEVELOPMENT IN SEMARANG IN THE REPUBLIC OF INDONESIA  
JAPAN INTERNATIONAL COOPERATION AGENCY

Fig. 7.2.24 (3/7)  
RESULTS OF SEEPAGE ANALYSIS (WITH GROUT)

# RESULTS OF SEEPAGE ANALYSIS

CASE 2-4  
D-D Section, with grout

|  |                              |
|--|------------------------------|
| Escape Gradient Downstream of Impervious Zone    | 0.479                        |
| Exit Gradient at Downstream Toe of Pervious Zone | 0.031                        |
| Maximum Flow Velocity                            | $5.414 \times 10^{-5}$ m/sec |



THE DETAILED DESIGN OF FLOOD CONTROL, URBAN DRAINAGE AND WATER RESOURCES DEVELOPMENT IN SEMARANG IN THE REPUBLIC OF INDONESIA

JAPAN INTERNATIONAL COOPERATION AGENCY

Fig. 7.2.24 (4/7)


RESULTS OF SEEPAGE ANALYSIS (WITH GROUT)

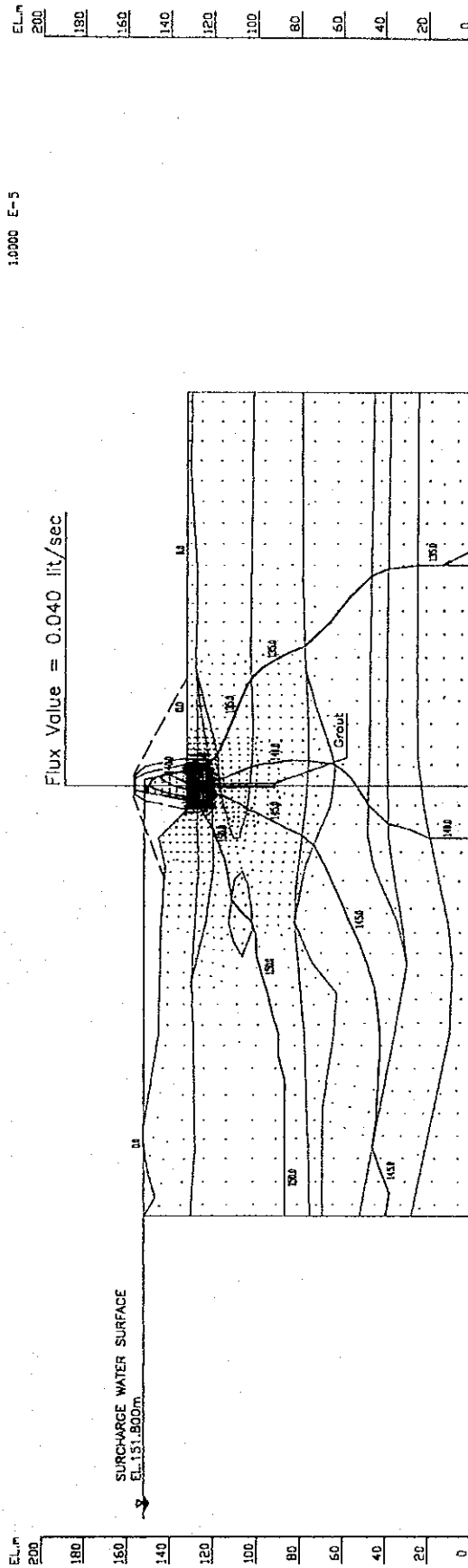


# RESULTS OF SEEPAGE ANALYSIS

CASE 2-5  
E-E Section, with grout

|  |                              |
|--|------------------------------|
| Escape Gradient Downstream of Impervious Zone    | 0.441                        |
| Exit Gradient at Downstream Toe of Pervious Zone | 0.014                        |
| Maximum Flow Velocity                            | $3.157 \times 10^{-8}$ m/sec |

  
 GEO. SCALE (m)  
 2.0000 E+1  
 VEC. SCALE (m/sec)  
 1.0000 E-5



THE DETAILED DESIGN OF FLOOD CONTROL, URBAN DRAINAGE AND WATER RESOURCES DEVELOPMENT IN SEMARANG IN THE REPUBLIC OF INDONESIA

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Fig. 7.2.24 (5/7)  
RESULTS OF SEEPAGE ANALYSIS (WITH GROUT)

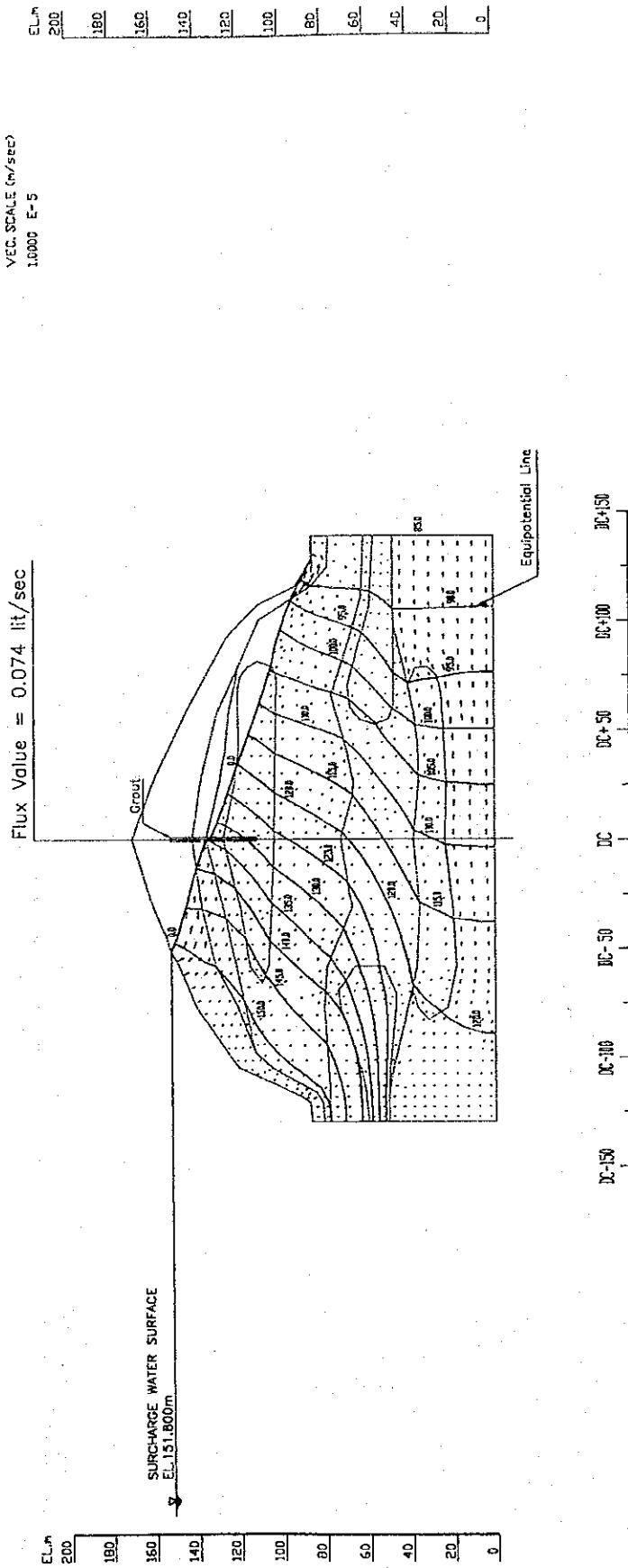
# RESULTS OF SEEPAGE ANALYSIS

CASE 2-6

F-F Section, with grout

|   |                              |
|---|------------------------------|
| Escape Gradient Downstream of Impervious Zone | -                            |
| Exit Gradient at Downstream River             | 0.437                        |
| Maximum Flow Velocity                         | 2.926x10 <sup>-6</sup> m/sec |

  
 GEO. SCALE (m)  
 2,000 E+1  
 VEC. SCALE (m/sec)  
 1,000 E-5



THE DETAILED DESIGN OF FLOOD CONTROL, URBAN DRAINAGE AND WATER RESOURCES DEVELOPMENT IN SEMARANG IN THE REPUBLIC OF INDONESIA

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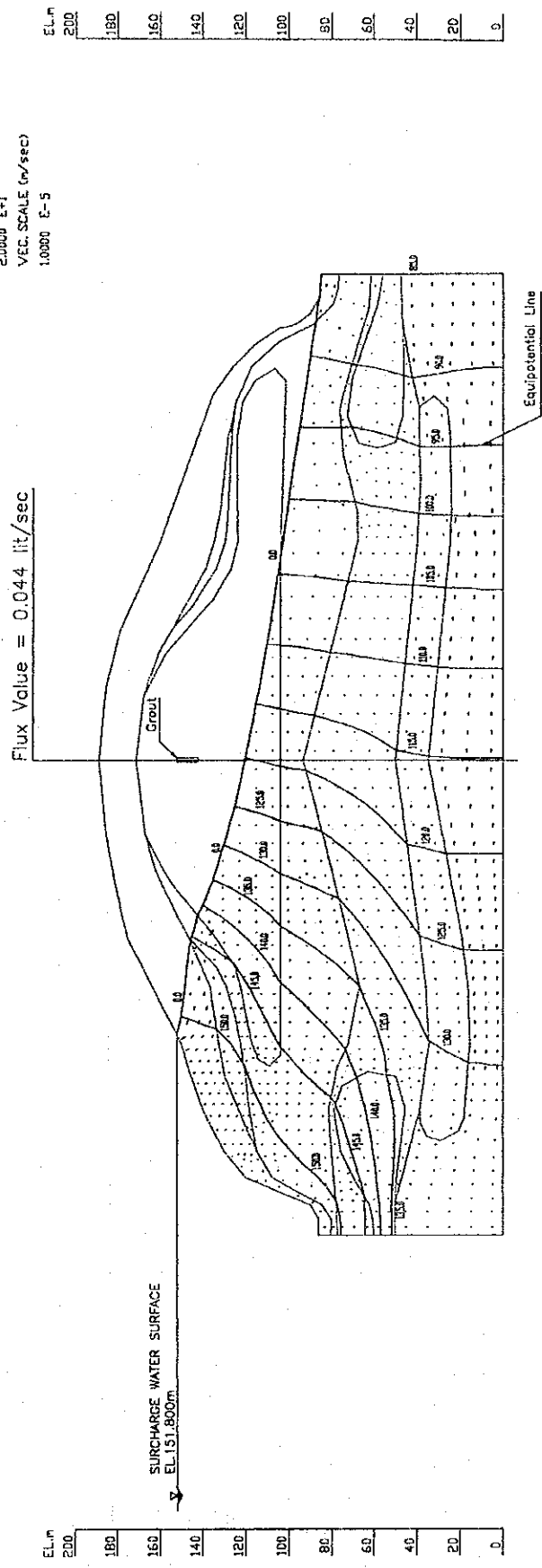
Fig. 7.24 (6/7)  
RESULTS OF SEEPAGE ANALYSIS (WITH GROUT)

# RESULTS OF SEEPAGE ANALYSIS

CASE 2-7  
G-G Section, with grout

|   |                              |
|---|------------------------------|
| Escape Gradient Downstream of Impervious Zone | -                            |
| Exit Gradient at Downstream River             | 0.082                        |
| Maximum Flow Velocity                         | $1.099 \times 10^{-6}$ m/sec |

GEO. SCALE (H)  
2,0000 E+1  
VEC. SCALE (V/SEC)  
1,0000 E-5



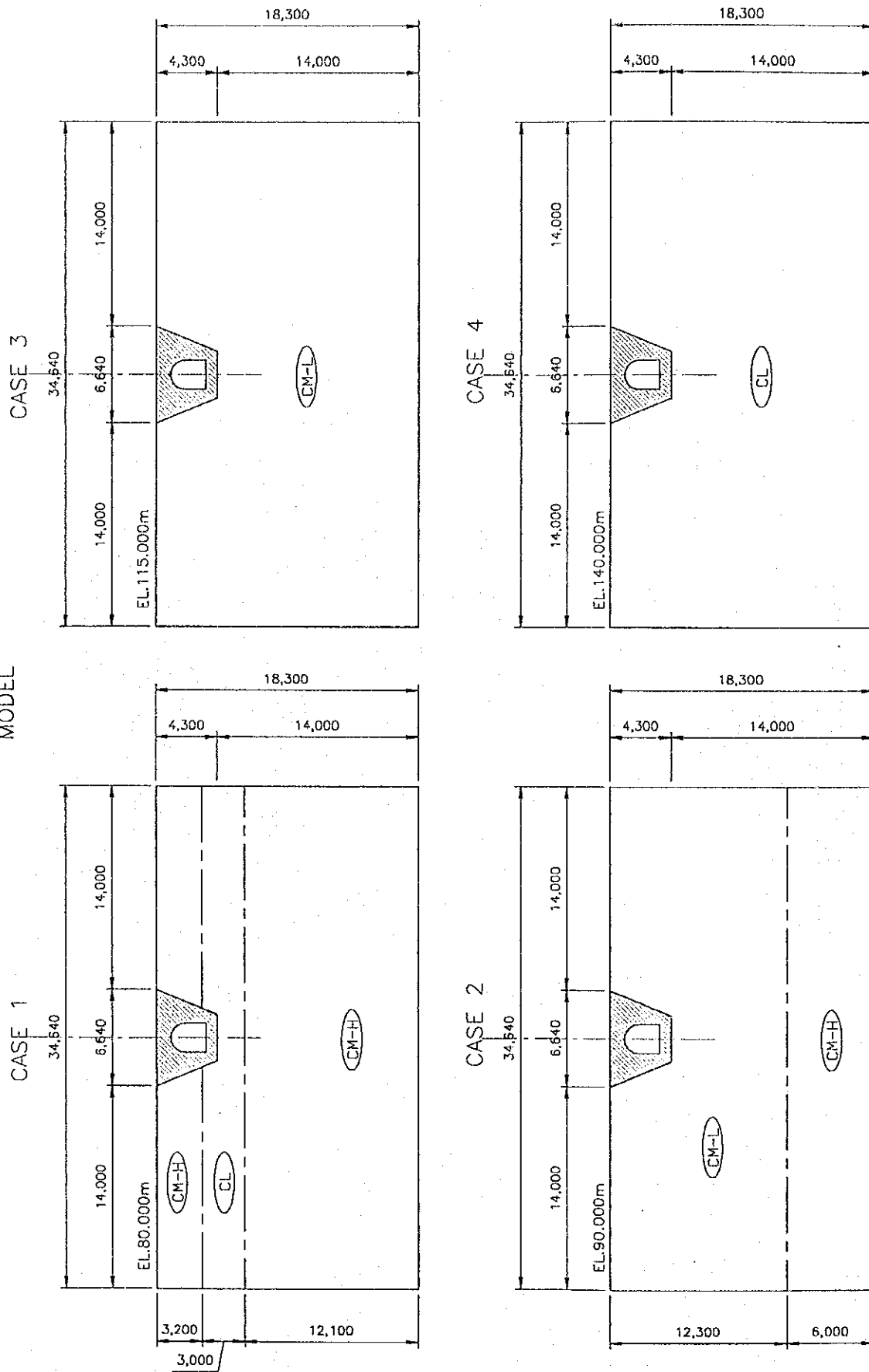
THE DETAILED DESIGN OF FLOOD CONTROL, URBAN DRAINAGE AND WATER RESOURCES DEVELOPMENT IN SEMARANG IN THE REPUBLIC OF INDONESIA

JAPAN INTERNATIONAL COOPERATION AGENCY

Fig. 7.2.24 (77)  
RESULTS OF SEEPAGE ANALYSIS (WITH GROUT)

STRESS-STRAIN ANALYSIS FOR GALLERY

MODEL



THE DETAILED DESIGN OF FLOOD CONTROL, URBAN DRAINAGE AND WATER RESOURCES DEVELOPMENT IN SEMARANG IN THE REPUBLIC OF INDONESIA

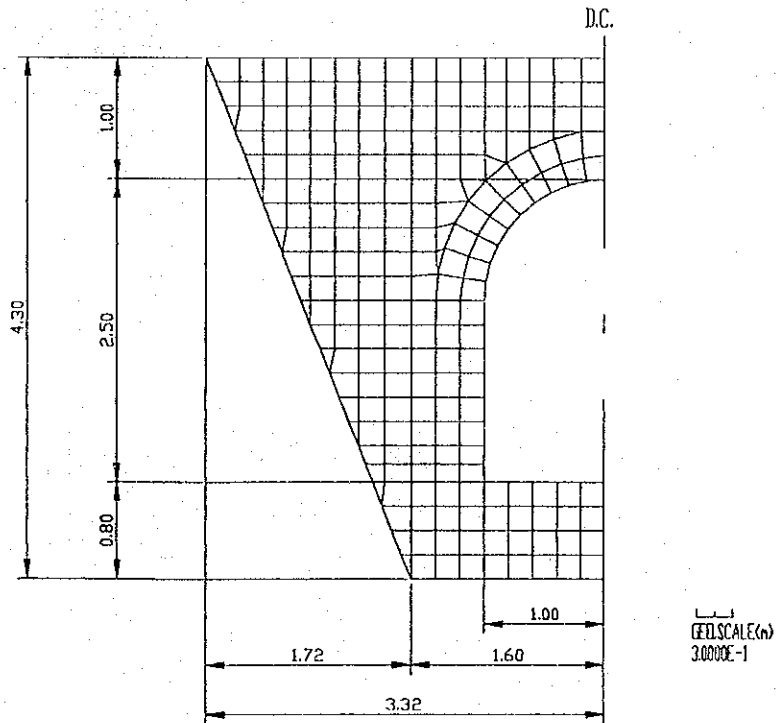
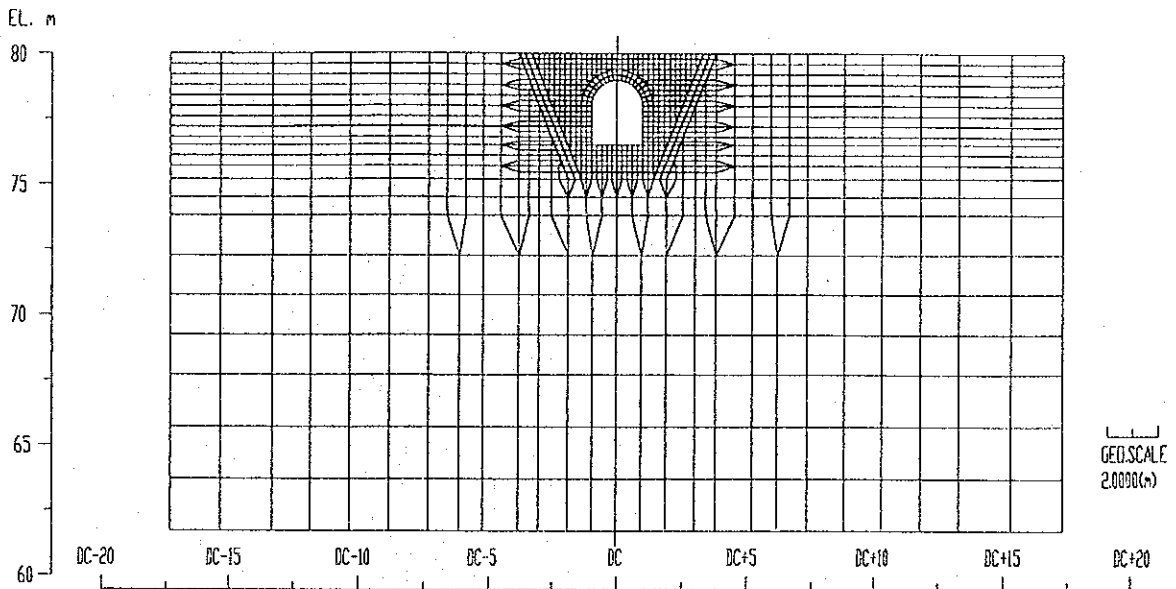
JAPAN INTERNATIONAL COOPERATION AGENCY

Fig. 7.2.25

SECTION OF STRESS-STRAIN ANALYSIS FOR GALLERY

# STRESS-STRAIN ANALYSIS FOR GALLERY

## Finite Element Mesh



THE DETAILED DESIGN OF FLOOD CONTROL, URBAN DRAINAGE AND WATER RESOURCES DEVELOPMENT IN SEMARANG IN THE REPUBLIC OF INDONESIA

JAPAN INTERNATIONAL COOPERATION AGENCY

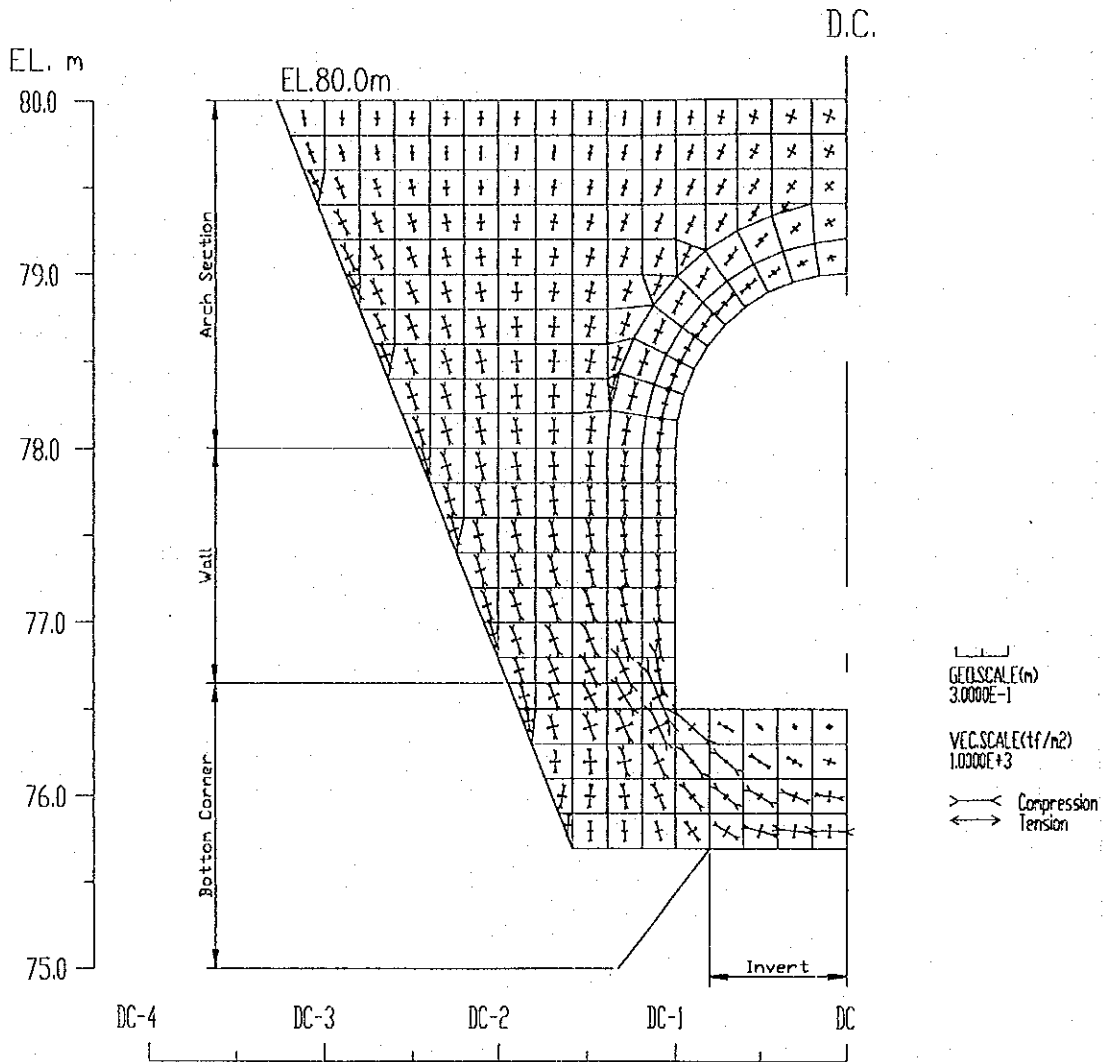
Fig. 7.2.26

FINITE ELEMENT MESH OF STRESS-STRAIN ANALYSIS FOR GALLERY

STRESS-STRAIN ANALYSIS FOR GALLERY

CASE 1 (TOP ELEVATION EL.80.0m)

Principal Stress Vector



| Section       | Maximum Principal Stress(tf/m <sup>2</sup> ) |         |
|---------------|--|---------|
|               | Compression                                  | Tension |
| Arch Section  | 425.9  | -121.1  |
| Wall          | 674.2  | -       |
| Bottom Corner | 969.6  | -8.6    |
| Invert        | 588.6  | -123.9  |

THE DETAILED DESIGN OF FLOOD CONTROL, URBAN DRAINAGE AND WATER RESOURCES DEVELOPMENT IN SEMARANG IN THE REPUBLIC OF INDONESIA

JAPAN INTERNATIONAL COOPERATION AGENCY

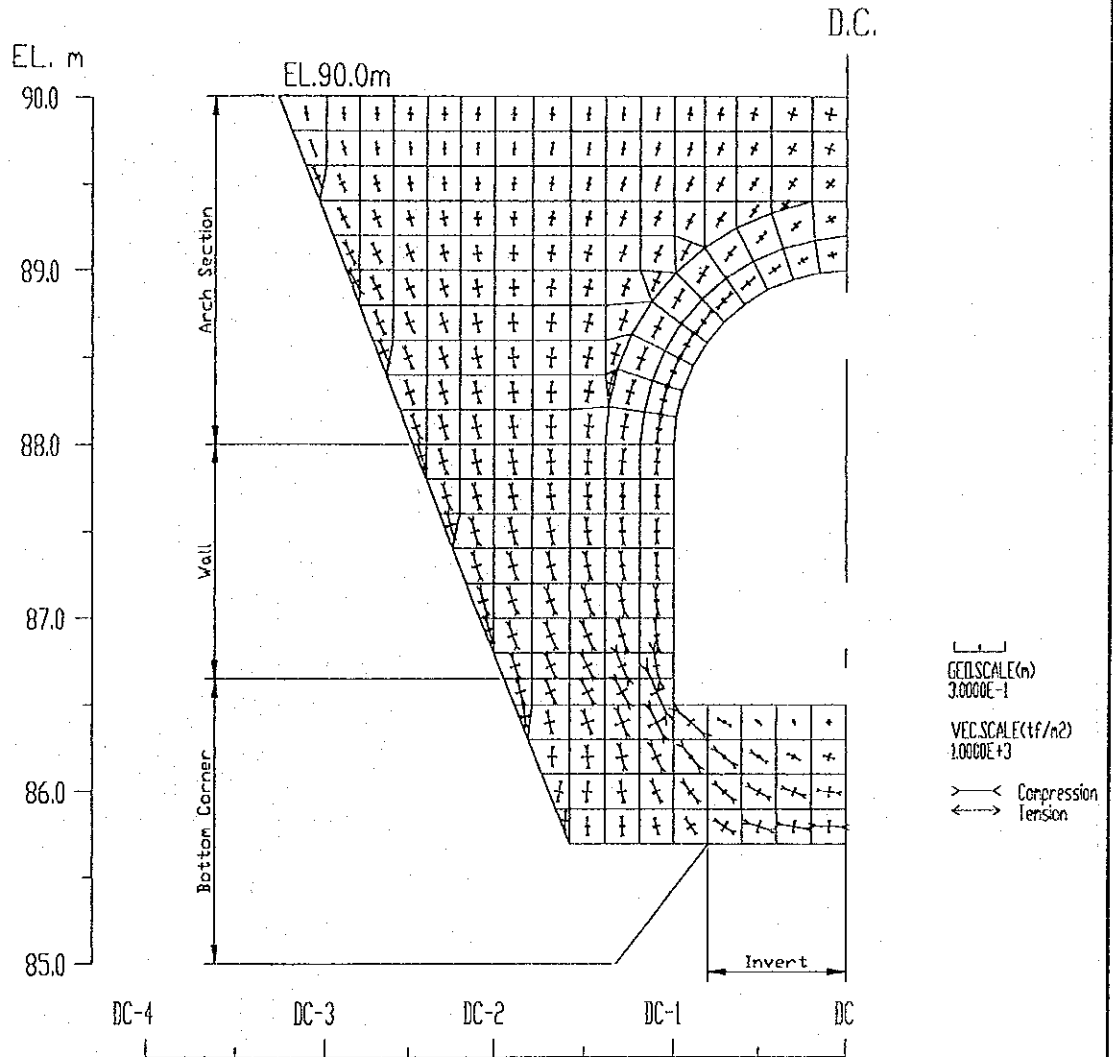
Fig. 7.2.27 (1/4)

**RESULTS OF STRESS-STRAIN ANALYSIS FOR GALLERY (VECTORS)**

# STRESS-STRAIN ANALYSIS FOR GALLERY

CASE 2 (TOP ELEVATION EL.90.0m)

Principal Stress Vector



| Section       | Maximum Principal Stress(tf/m <sup>2</sup> ) |         |
|---------------|--|---------|
|               | Compression                                  | Tension |
| Arch Section  | 372.0  | -112.4  |
| Wall          | 578.2  | -       |
| Bottom Corner | 837.2  | -1.9    |
| Invert        | 480.5  | -88.6   |

THE DETAILED DESIGN OF FLOOD CONTROL, URBAN DRAINAGE AND WATER RESOURCES DEVELOPMENT IN SEMARANG IN THE REPUBLIC OF INDONESIA

JAPAN INTERNATIONAL COOPERATION AGENCY

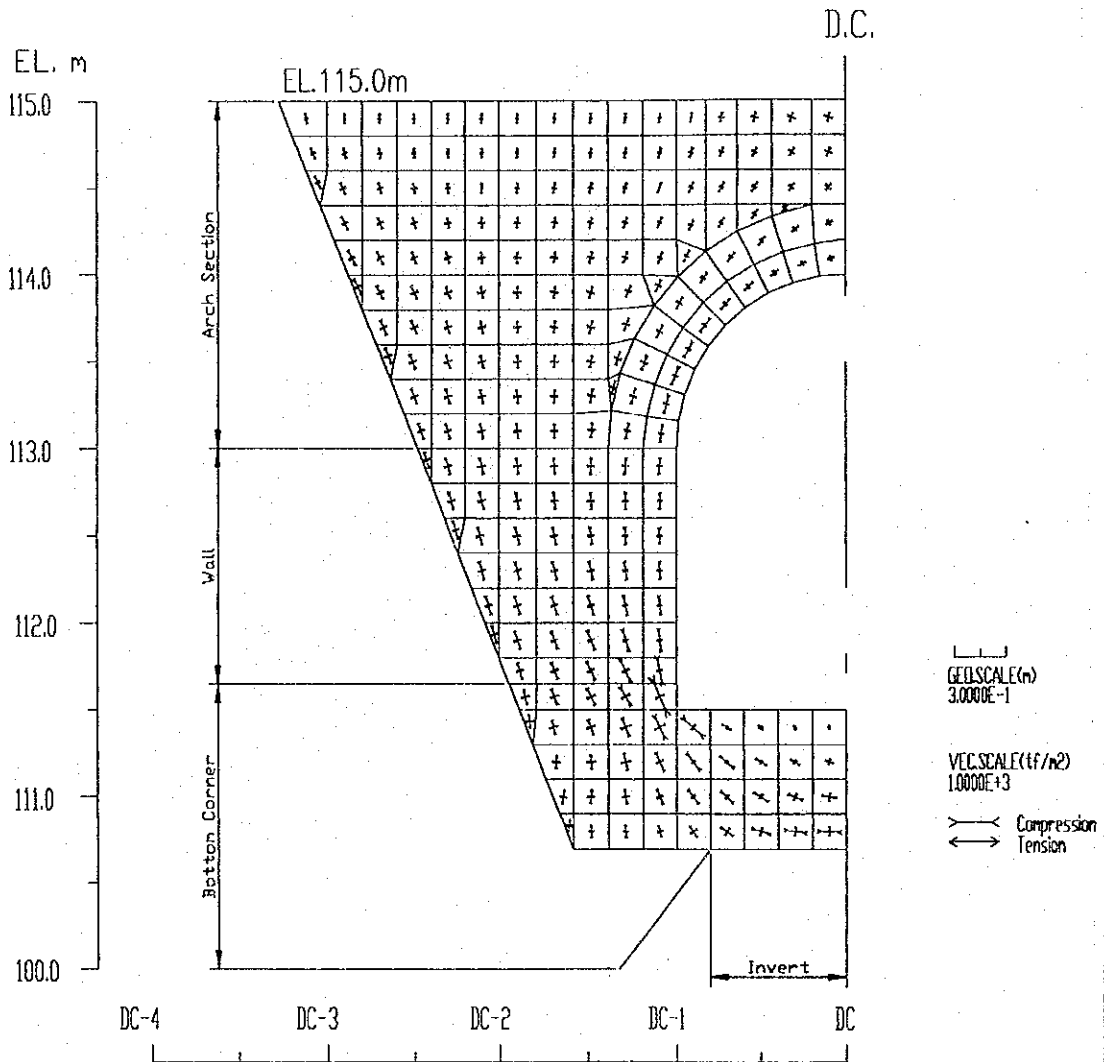
Fig. 7.2.27 (2/4)

RESULTS OF STRESS-STRAIN ANALYSIS FOR GALLERY (VECTORS)

STRESS-STRAIN ANALYSIS FOR GALLERY

CASE 3 (TOP ELEVATION EL.115.0m)

Principal Stress Vector



| Section       | Maximum Principal Stress(tf/m <sup>2</sup> ) |         |
|---------------|--|---------|
|               | Compression                                  | Tension |
| Arch Section  | 235.8  | -68.7   |
| Wall          | 366.5  | -       |
| Bottom Corner | 529.1  | -1.7    |
| Invert        | 302.4  | -57.4   |

THE DETAILED DESIGN OF FLOOD CONTROL, URBAN DRAINAGE AND WATER RESOURCES DEVELOPMENT IN SEMARANG IN THE REPUBLIC OF INDONESIA

JAPAN INTERNATIONAL COOPERATION AGENCY

Fig. 7.2.27 (3/4)

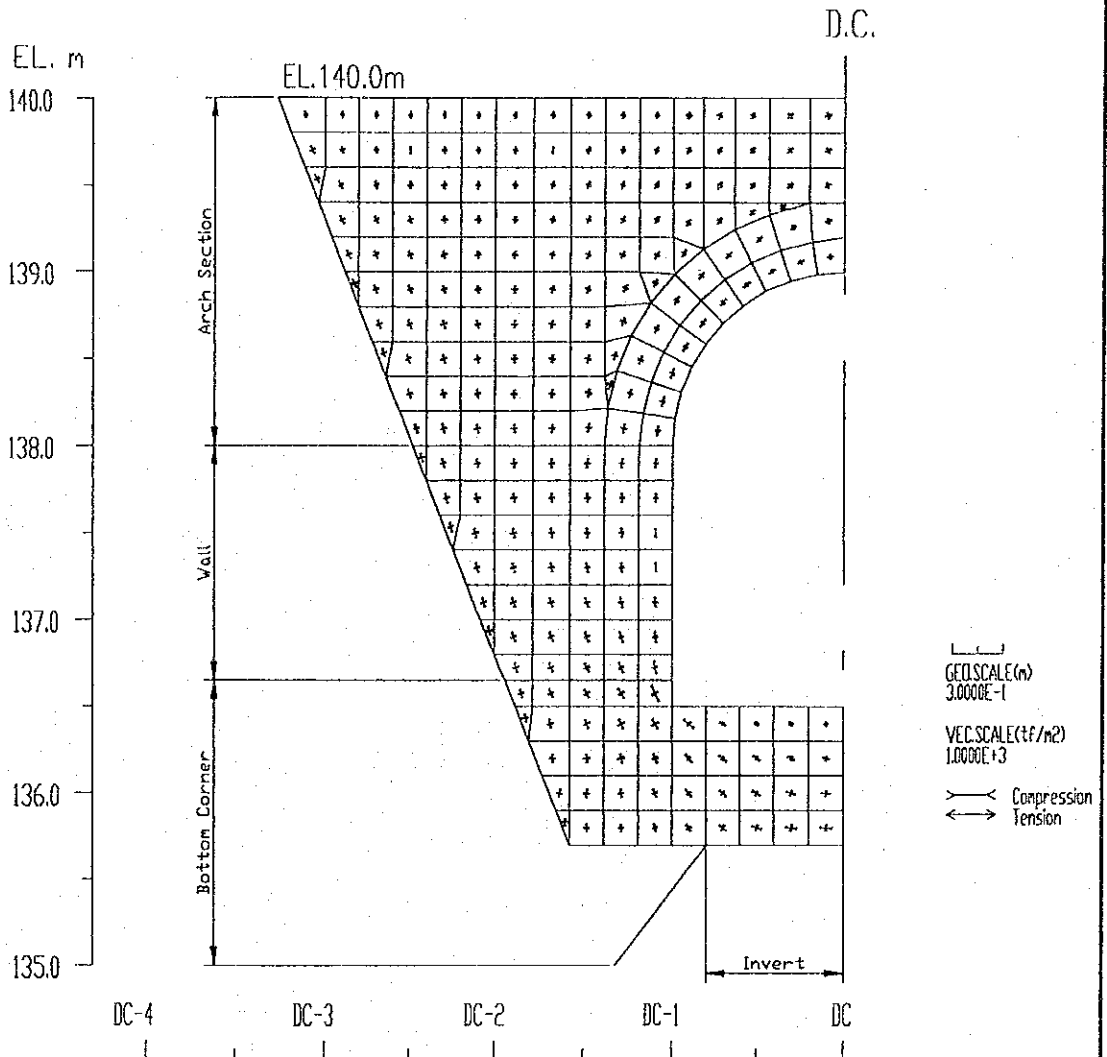
RESULTS OF STRESS-STRAIN ANALYSIS FOR GALLERY (VECTORS)



STRESS-STRAIN ANALYSIS FOR GALLERY

CASE 4 (TOP ELEVATION EL.140.0m)

Principal Stress Vector



| Section       | Maximum Principal Stress(tf/m <sup>2</sup> ) |         |
|---------------|--|---------|
|               | Compression                                  | Tension |
| Arch Section  | 96.0   | -18.9   |
| Wall          | 150.4  | -       |
| Bottom Corner | 222.3  | -2.4    |
| Invert        | 131.8  | -18.6   |

THE DETAILED DESIGN OF FLOOD CONTROL, URBAN DRAINAGE AND WATER RESOURCES DEVELOPMENT IN SEMARANG IN THE REPUBLIC OF INDONESIA

JAPAN INTERNATIONAL COOPERATION AGENCY

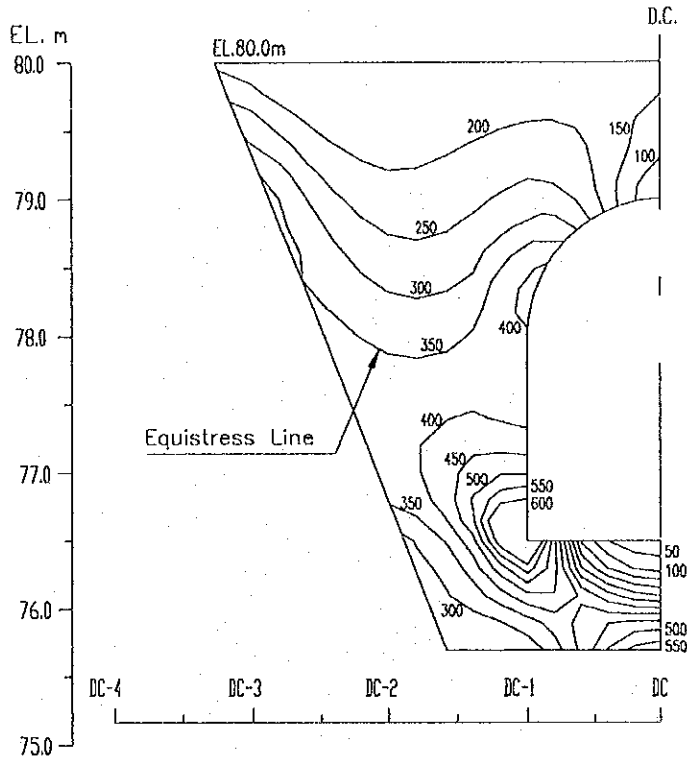
Fig. 7.2.27 (4/4)

RESULTS OF STRESS-STRAIN ANALYSIS FOR GALLERY (VECTORS)

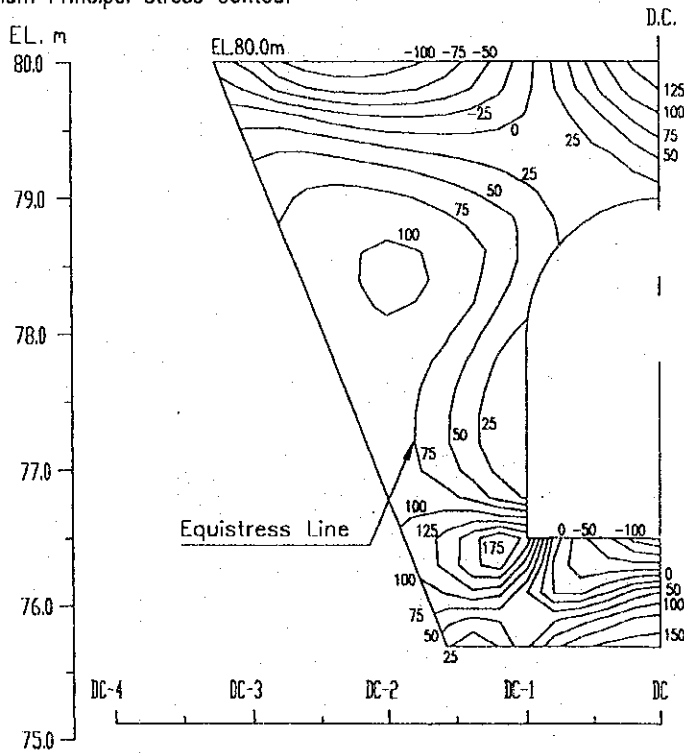
# STRESS-STRAIN ANALYSIS FOR GALLERY

## CASE 1 (TOP ELEVATION EL.80.0m)

Maximum Principal Stress Contour



Minimum Principal Stress Contour



THE DETAILED DESIGN OF FLOOD CONTROL, URBAN DRAINAGE AND WATER RESOURCES DEVELOPMENT IN SEMARANG IN THE REPUBLIC OF INDONESIA

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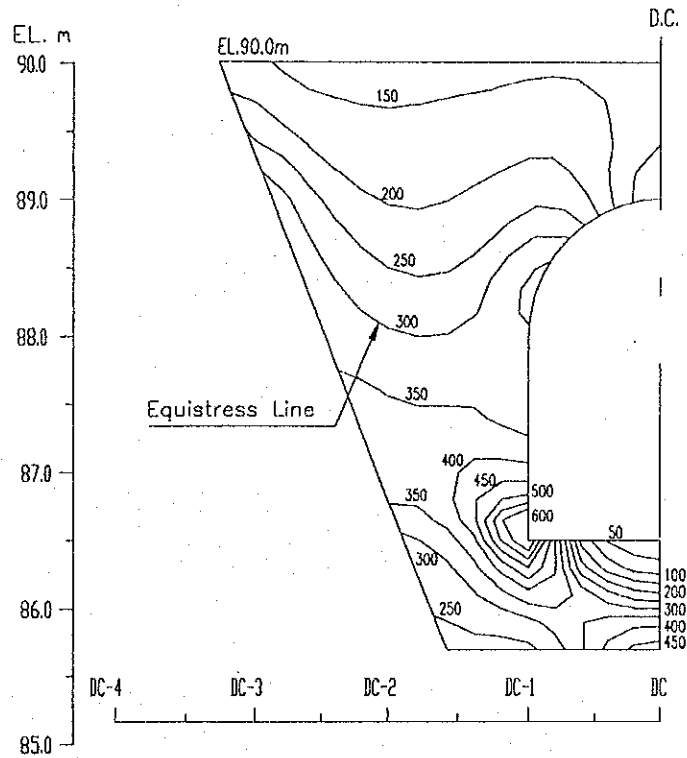
Fig. 7.2.28 (1/4)

### RESULTS OF STRESS-STRAIN ANALYSIS FOR GALLERY (CONTOURS)

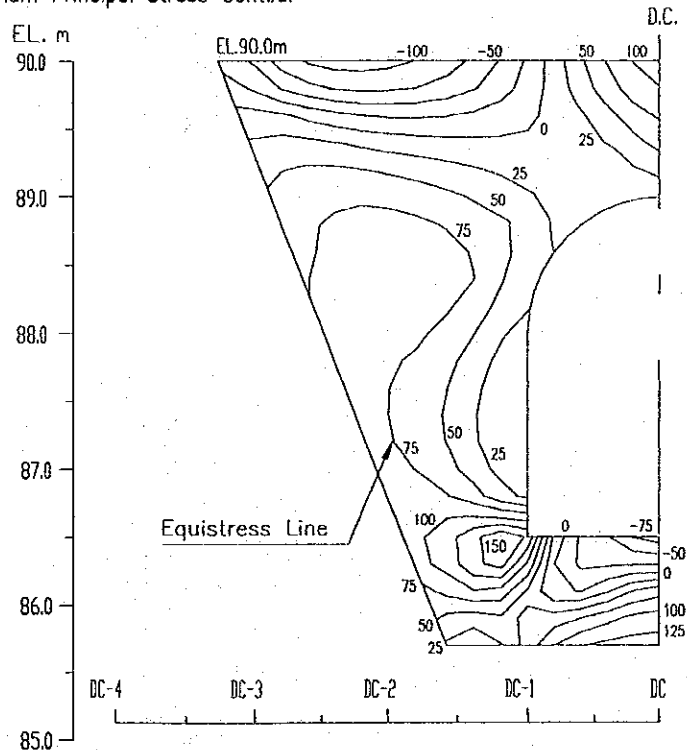
# STRESS-STRAIN ANALYSIS FOR GALLERY

## CASE 2 (TOP ELEVATION EL.90.0m)

Maximum Principal Stress Contour



Minimum Principal Stress Contour



THE DETAILED DESIGN OF FLOOD CONTROL, URBAN DRAINAGE AND WATER RESOURCES DEVELOPMENT IN SEMARANG IN THE REPUBLIC OF INDONESIA

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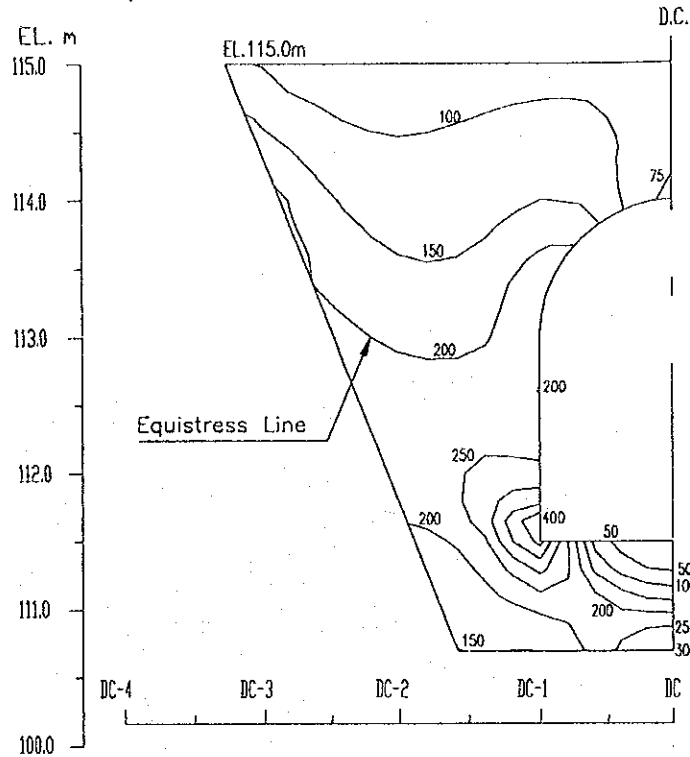
Fig. 7.2.28 (2/4)

RESULTS OF STRESS-STRAIN ANALYSIS FOR GALLERY (CONTOURS)

STRESS-STRAIN ANALYSIS FOR GALLERY

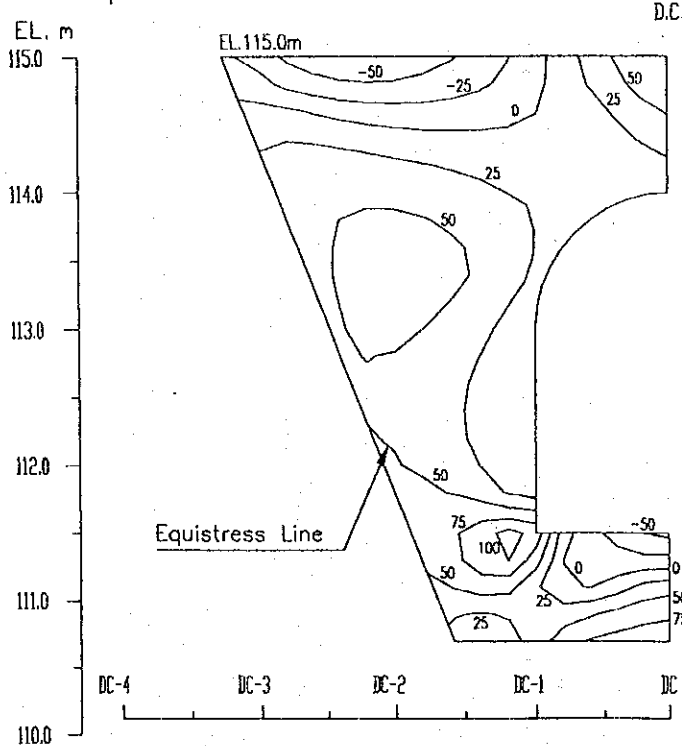
CASE 3 (TOP ELEVATION EL.115.0m)

Maximum Principal Stress Contour



GEO.SCALE(m)  
3.0000E-1  
UNIT (tf/m<sup>2</sup>)

Minimum Principal Stress Contour



GEO.SCALE(m)  
3.0000E-1  
UNIT (tf/m<sup>2</sup>)

THE DETAILED DESIGN OF FLOOD CONTROL, URBAN DRAINAGE AND WATER RESOURCES DEVELOPMENT IN SEMARANG IN THE REPUBLIC OF INDONESIA

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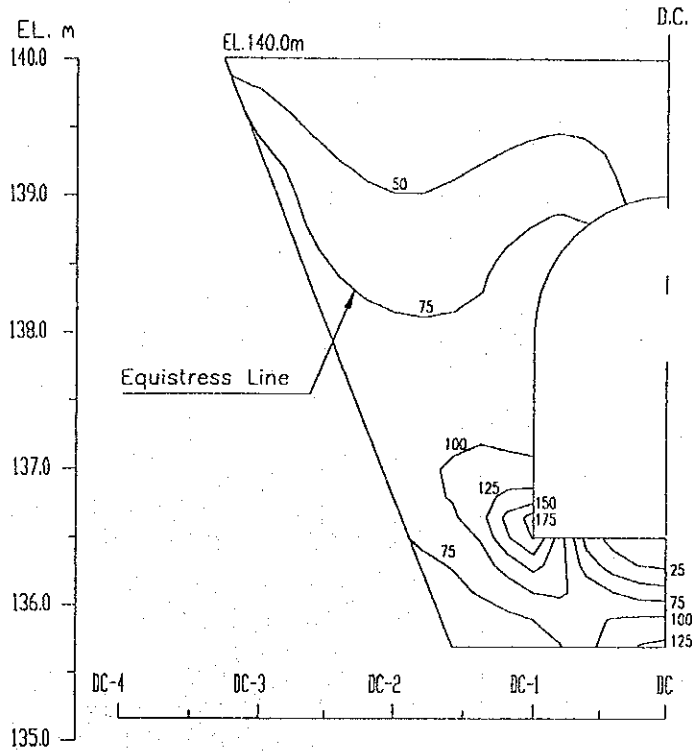
Fig. 7.2.28 (3/4)

RESULTS OF STRESS-STRAIN ANALYSIS FOR GALLERY (CONTOURS)

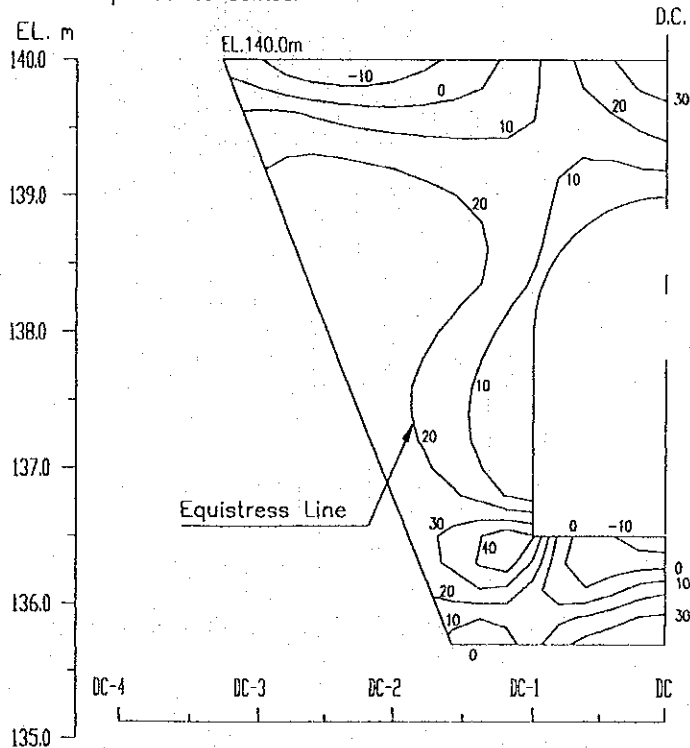
# STRESS-STRAIN ANALYSIS FOR GALLERY

## CASE 4 (TOP ELEVATION EL.140.0m)

Maximum Principal Stress Contour



Minimum Principal Stress Contour



THE DETAILED DESIGN OF FLOOD CONTROL, URBAN DRAINAGE AND WATER RESOURCES DEVELOPMENT IN SEMARANG IN THE REPUBLIC OF INDONESIA

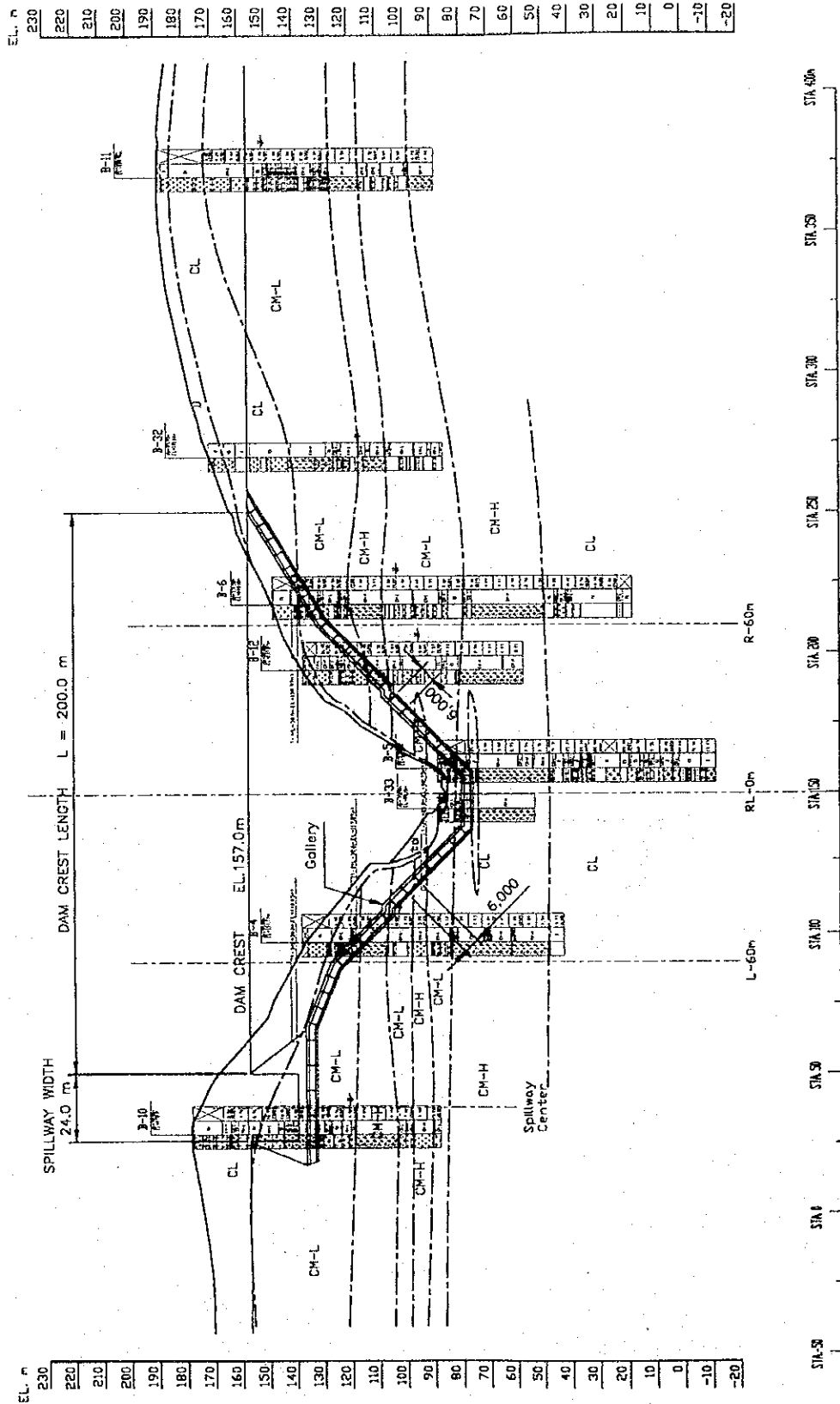
JAPAN INTERNATIONAL COOPERATION AGENCY

Fig. 7.2.28 (4/4)

RESULTS OF STRESS-STRAIN ANALYSIS FOR GALLERY (CONTOURS)

DEFORMATION ANALYSIS FOR GALLERY

Geological Section Along Dam Axis



THE DETAILED DESIGN OF FLOOD CONTROL, URBAN DRAINAGE AND WATER RESOURCES DEVELOPMENT IN SEMARANG IN THE REPUBLIC OF INDONESIA

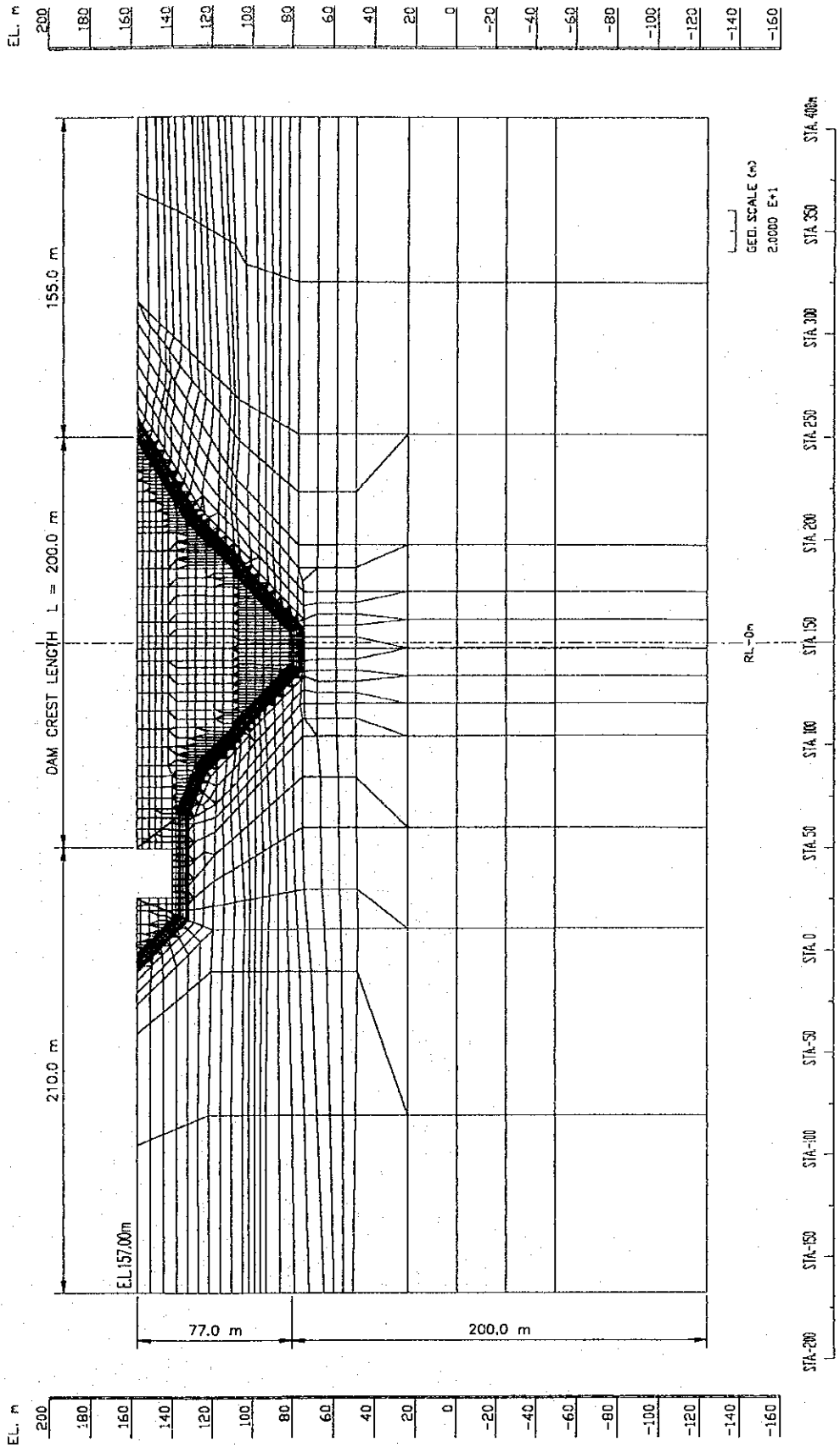
JAPAN INTERNATIONAL COOPERATION AGENCY

Fig. 7.2.29

LONGITUDINAL SECTION OF DEFORMATION ANALYSIS FOR GALLERY

DEFORMATION ANALYSIS FOR GALLERY

Finite Element Mesh



THE DETAILED DESIGN OF FLOOD CONTROL, URBAN DRAINAGE AND WATER RESOURCES DEVELOPMENT IN SEMARANG IN THE REPUBLIC OF INDONESIA

JAPAN INTERNATIONAL COOPERATION AGENCY

Fig. 7.2.30

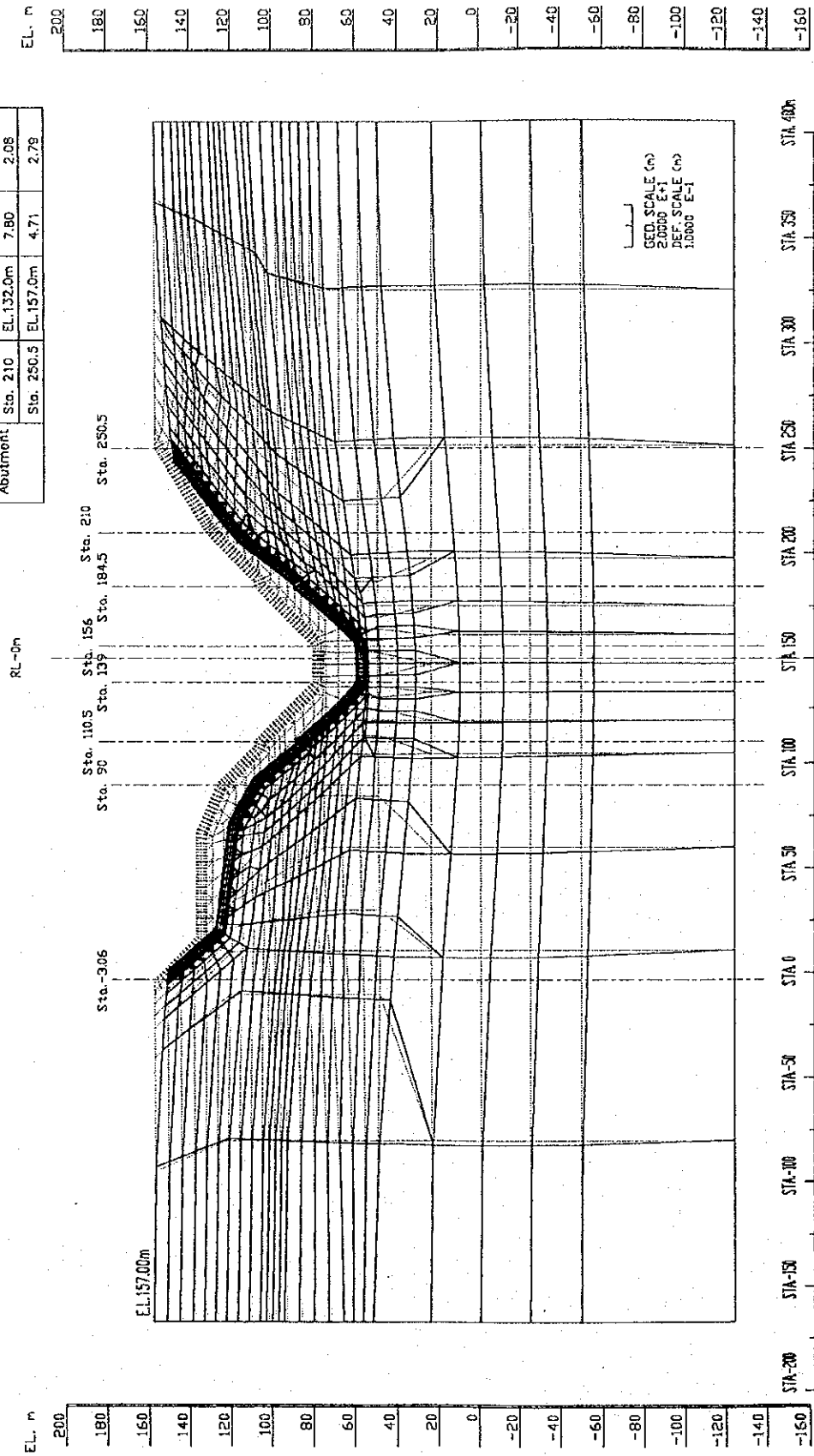
FINITE ELEMENT MESH OF DEFORMATION ANALYSIS FOR GALLERY

# DEFORMATION ANALYSIS FOR GALLERY

Deformation

Displacement (cm)

| Location       | Displacement (cm) |            |
|----------------|-------------------|------------|
|                | Vertical          | Horizontal |
| Left Abutment  | Sta. -3.06        | 3.47       |
|                | Sta. 90           | 9.20       |
|                | Sta. 110.5        | 10.36      |
| Right Abutment | Sta. 139          | 10.50      |
|                | Sta. 156          | 10.27      |
|                | Sta. 184.5        | 9.70       |
|                | Sta. 210          | 7.80       |
|                | Sta. 250.5        | 4.71       |



THE DETAILED DESIGN OF FLOOD CONTROL, URBAN DRAINAGE AND WATER RESOURCES DEVELOPMENT IN SEMARANG IN THE REPUBLIC OF INDONESIA

JAPAN INTERNATIONAL COOPERATION AGENCY

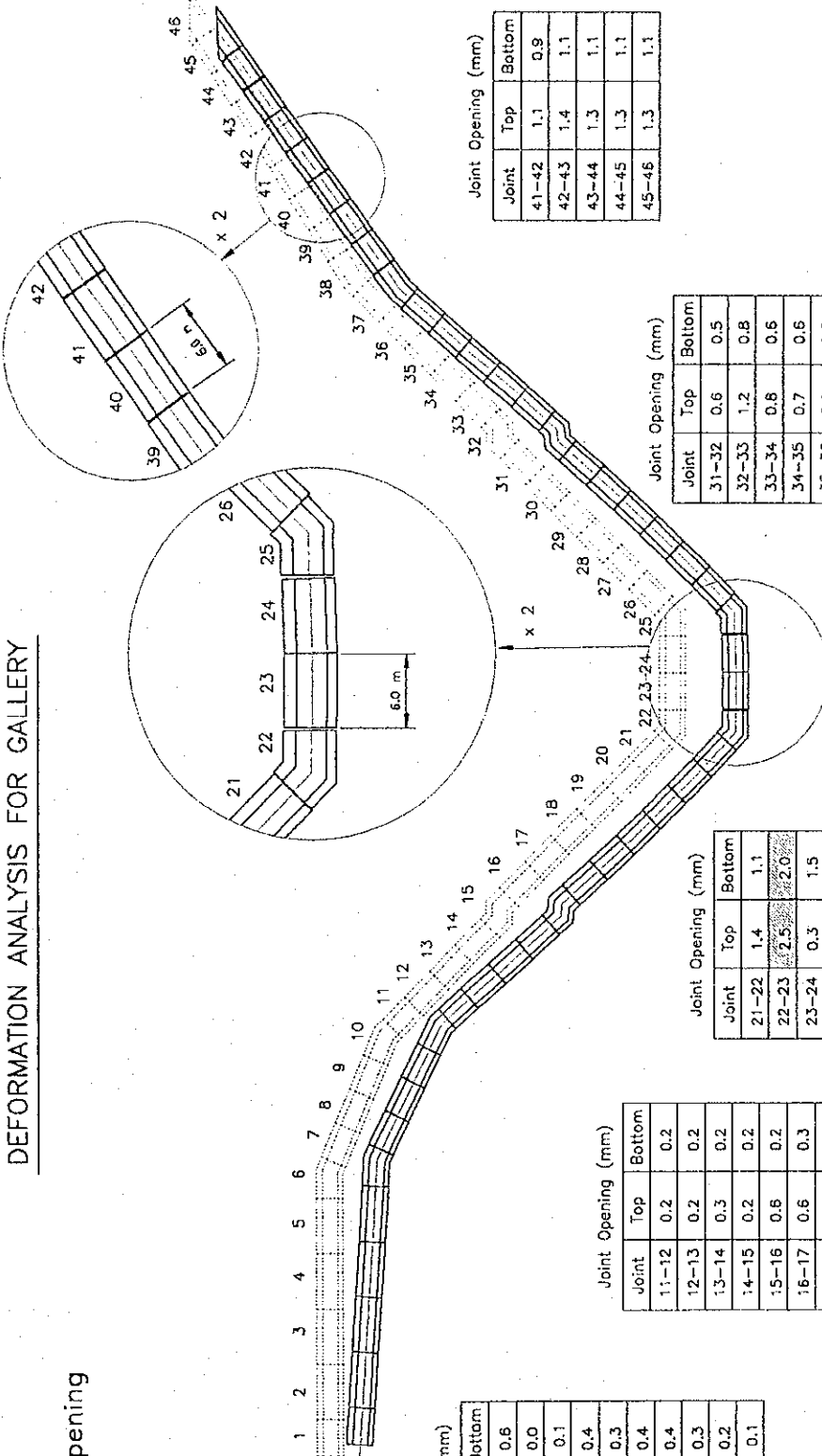
Fig. 7.231  
RESULTS OF DEFORMATION ANALYSIS FOR GALLERY (DISPLACEMENT)



DEFORMATION ANALYSIS FOR GALLERY

Joint Opening

| EL. m | 160 | 150 | 140 | 130 | 120 | 110 | 100 | 90 | 80 |
|-------|-----|-----|-----|-----|-----|-----|-----|----|----|
|-------|-----|-----|-----|-----|-----|-----|-----|----|----|



| Joint Opening (mm) |        |
|--------------------|--------|
| Joint              | Bottom |
| 1-2                | 0.6    |
| 2-3                | 0.0    |
| 3-4                | 0.1    |
| 4-5                | 0.4    |
| 5-6                | 0.3    |
| 6-7                | 0.4    |
| 7-8                | 0.4    |
| 8-9                | 0.3    |
| 9-10               | 0.2    |
| 10-11              | 0.1    |

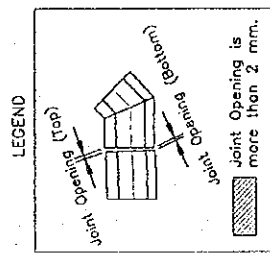
| Joint Opening (mm) |        |
|--------------------|--------|
| Joint              | Bottom |
| 11-12              | 0.2    |
| 12-13              | 0.2    |
| 13-14              | 0.3    |
| 14-15              | 0.2    |
| 15-16              | 0.8    |
| 16-17              | 0.6    |
| 17-18              | 0.7    |
| 18-19              | 0.7    |
| 19-20              | 0.7    |
| 20-21              | 1.0    |

| Joint Opening (mm) |        |
|--------------------|--------|
| Joint              | Bottom |
| 21-22              | 1.4    |
| 22-23              | 2.5    |
| 23-24              | 0.3    |
| 24-25              | 3.6    |
| 25-26              | 2.6    |
| 26-27              | 0.6    |
| 27-28              | 0.5    |
| 28-29              | 0.4    |
| 29-30              | 0.5    |
| 30-31              | 0.5    |

| Joint Opening (mm) |        |
|--------------------|--------|
| Joint              | Bottom |
| 31-32              | 0.6    |
| 32-33              | 1.2    |
| 33-34              | 0.6    |
| 34-35              | 0.7    |
| 35-36              | 0.9    |
| 36-37              | 1.0    |
| 37-38              | 1.0    |
| 38-39              | 1.5    |
| 39-40              | 1.3    |
| 40-41              | 1.5    |

| Joint Opening (mm) |        |
|--------------------|--------|
| Joint              | Bottom |
| 41-42              | 1.1    |
| 42-43              | 1.4    |
| 43-44              | 1.3    |
| 44-45              | 1.3    |
| 45-46              | 1.3    |

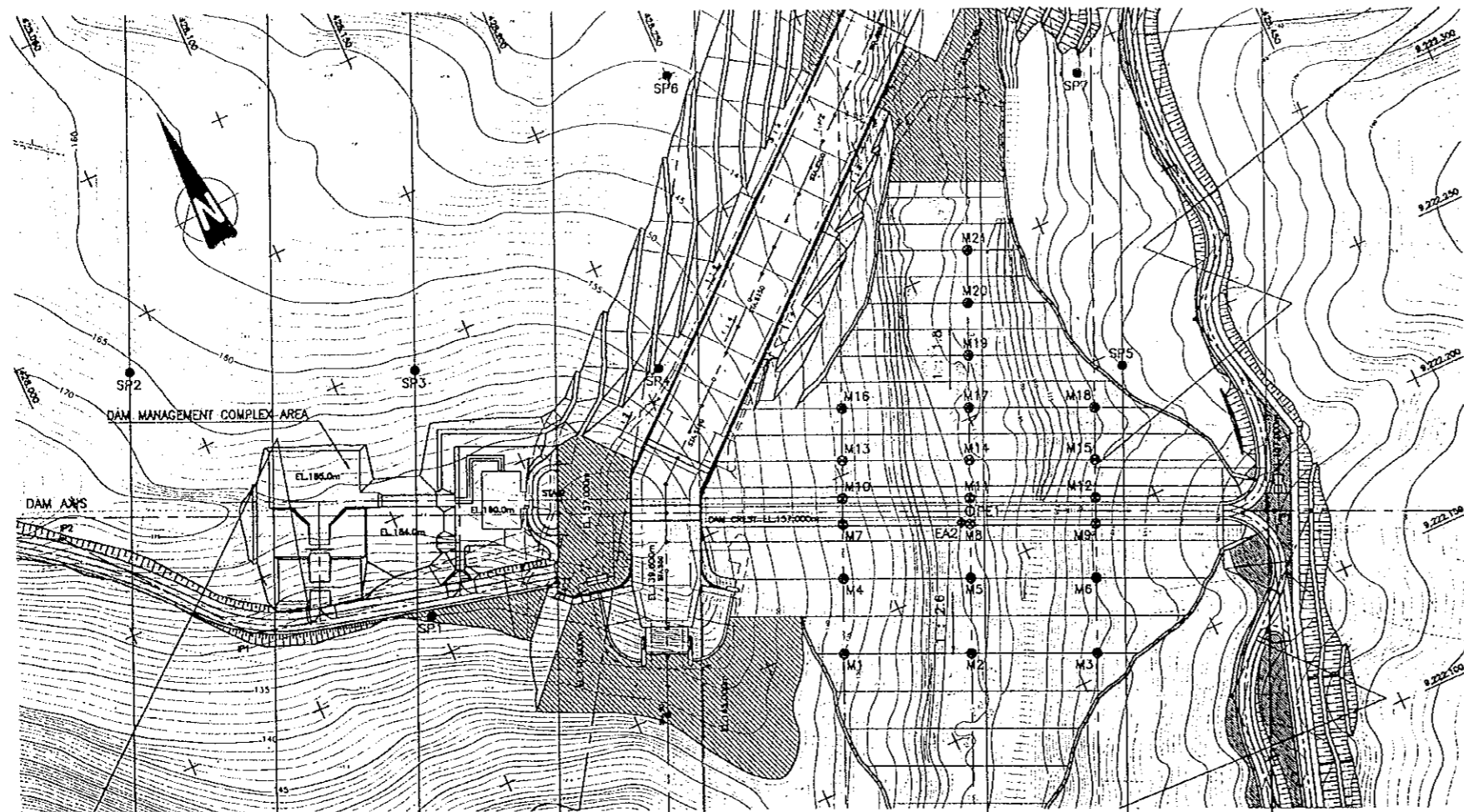
GED. SCALE (m)  
 1:10000 E+1  
 DEF. SCALE (m)  
 1:10000 E-1



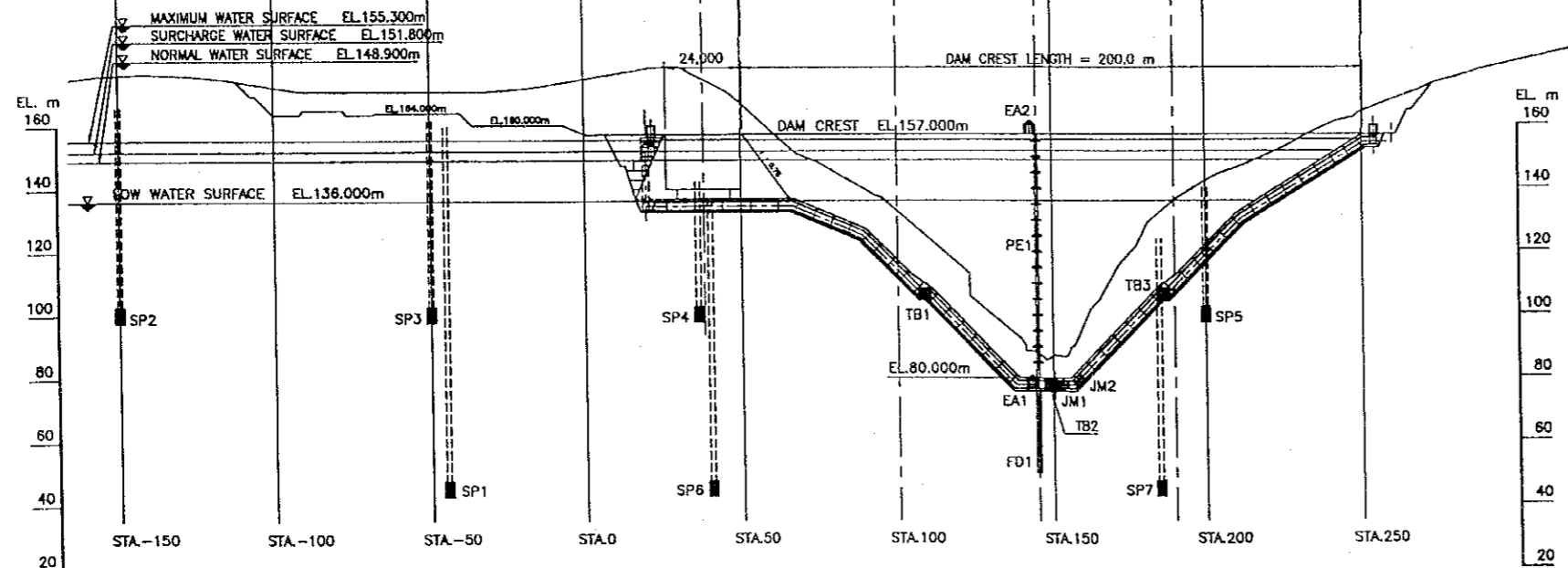
THE DETAILED DESIGN OF FLOOD CONTROL, URBAN DRAINAGE AND WATER RESOURCES DEVELOPMENT IN SEMARANG IN THE REPUBLIC OF INDONESIA

JAPAN INTERNATIONAL COOPERATION AGENCY

Fig. 7.2.32 RESULTS OF DEFORMATION ANALYSIS FOR GALLERY (JOINT OPENING)



PLAN



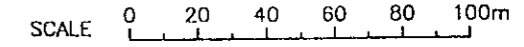
LONGITUDINAL PROFILE

- NOTES**
1. ALL DIMENSIONS ARE IN MILLIMETERS, UNLESS OTHERWISE NOTED.
  2. INSTRUMENTATION LOCATIONS AND ELEVATION ARE APPROXIMATE AND WILL BE FINALIZED BY THE ENGINEER AS PER SITE CONDITIONS.

**REFERENCE DRAWINGS**

JD-P1-ED-In-2 INSTRUMENTATION - SECTIONS

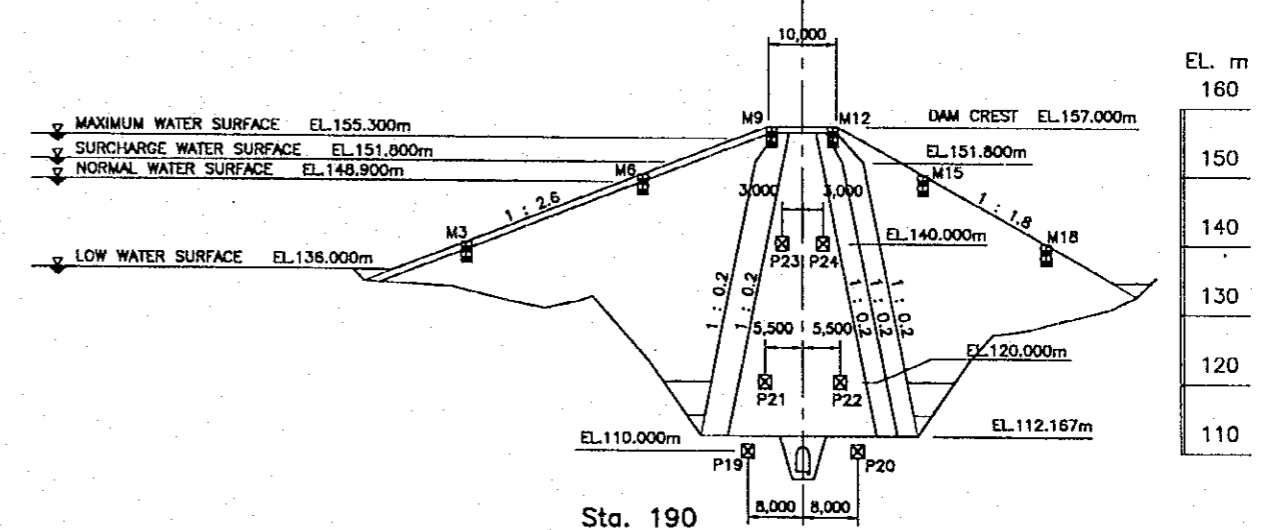
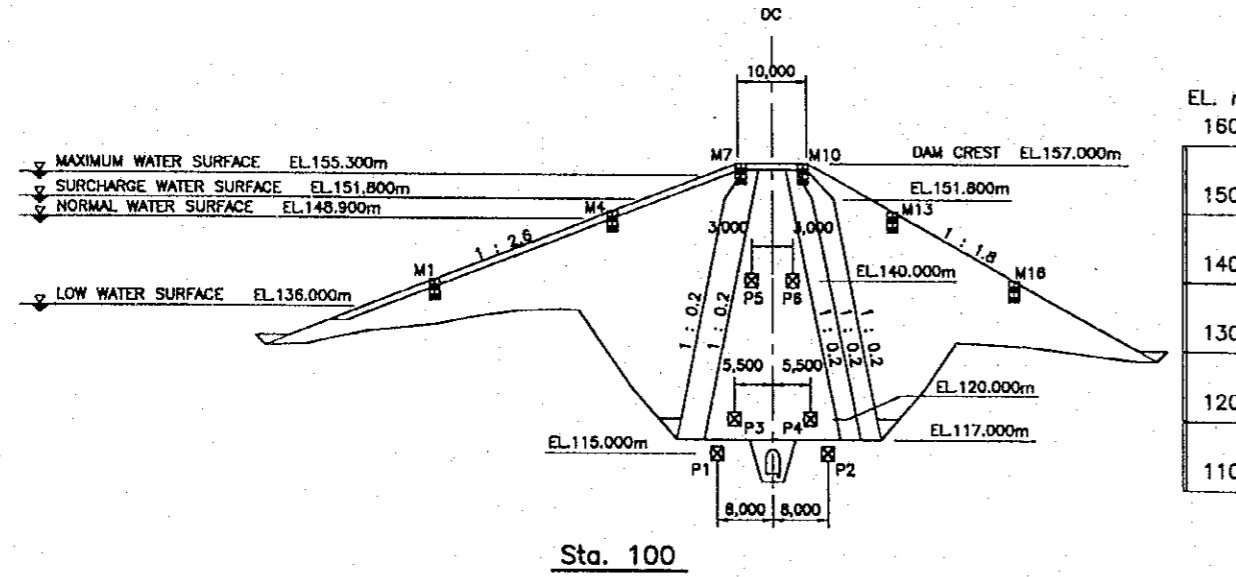
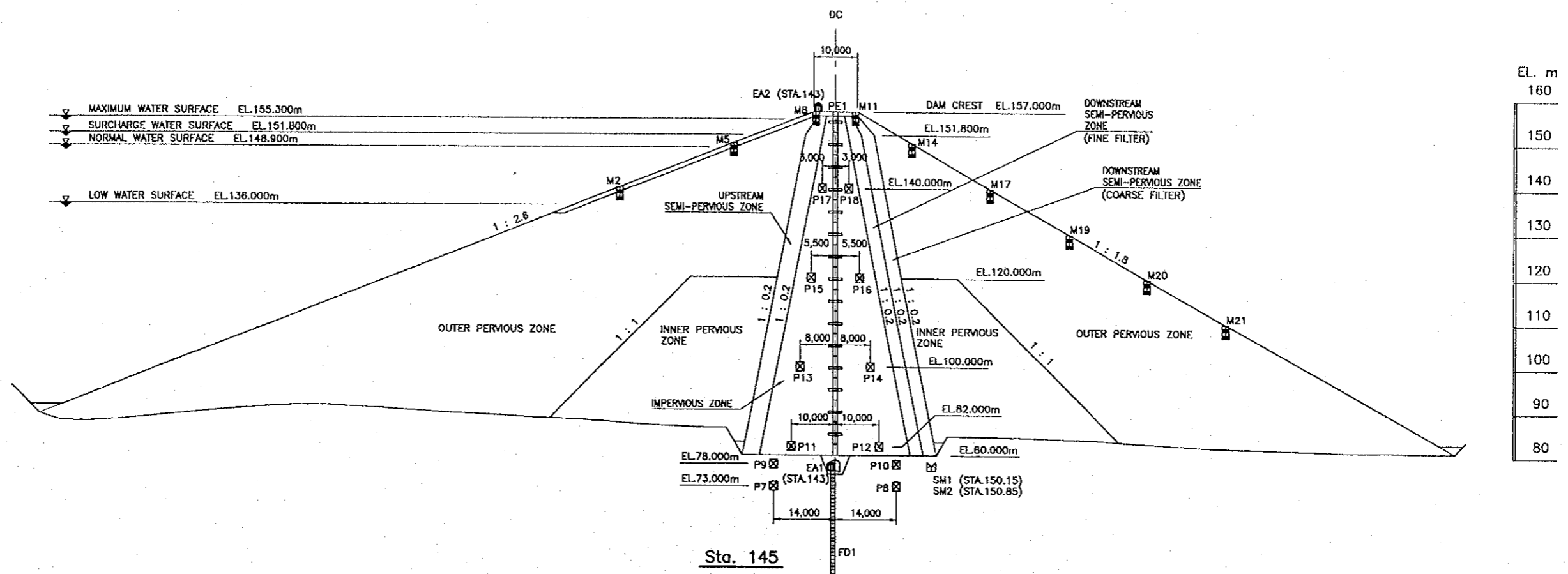
| TYPE OF INSTRUMENT              | SYMBOL |           | DESIGNATION | LOCATION |                      | ELEVATION                 | READOUT POINT | REMARKS                           |
|---------------------------------|--------|-----------|-------------|----------|----------------------|---------------------------|---------------|-----------------------------------|
|                                 | PLAN   | SECTION   |             | STA.     | OFFSET FROM DAM AXIS |                           |               |                                   |
| SURFACE MOVEMENT MARKER         | ⊙      | E         | M1          | 100      | 49.2m U/S            | EL.140.000m               | --            |                                   |
|                                 |        |           | M2          | 145      | 49.2m U/S            | EL.140.000m               | --            |                                   |
|                                 |        |           | M3          | 190      | 49.2m U/S            | EL.140.000m               | --            |                                   |
|                                 |        |           | M4          | 100      | 23.2m U/S            | EL.150.000m               | --            |                                   |
|                                 |        |           | M5          | 145      | 23.2m U/S            | EL.150.000m               | --            |                                   |
|                                 |        |           | M6          | 190      | 23.2m U/S            | EL.150.000m               | --            |                                   |
|                                 |        |           | M7          | 100      | 4.5m U/S             | EL.157.000m               | --            |                                   |
|                                 |        |           | M8          | 145      | 4.5m U/S             | EL.157.000m               | --            |                                   |
|                                 |        |           | M9          | 190      | 4.5m U/S             | EL.157.000m               | --            |                                   |
|                                 |        |           | M10         | 100      | 4.5m D/S             | EL.157.000m               | --            |                                   |
|                                 |        |           | M11         | 145      | 4.5m D/S             | EL.157.000m               | --            |                                   |
|                                 |        |           | M12         | 190      | 4.5m D/S             | EL.157.000m               | --            |                                   |
|                                 |        |           | M13         | 100      | 17.6m U/S            | EL.150.000m               | --            |                                   |
|                                 |        |           | M14         | 145      | 17.6m U/S            | EL.150.000m               | --            |                                   |
|                                 |        |           | M15         | 190      | 17.6m U/S            | EL.150.000m               | --            |                                   |
|                                 |        |           | M16         | 100      | 35.6m U/S            | EL.140.000m               | --            |                                   |
|                                 |        |           | M17         | 145      | 35.6m U/S            | EL.140.000m               | --            |                                   |
|                                 |        |           | M18         | 190      | 35.6m U/S            | EL.140.000m               | --            |                                   |
|                                 |        |           | M19         | 190      | 53.8m U/S            | EL.130.000m               | --            |                                   |
|                                 |        |           | M20         | 190      | 71.6m U/S            | EL.120.000m               | --            |                                   |
| M21                             | 190    | 89.6m U/S | EL.110.000m | --       |                      |                           |               |                                   |
| CASAGRANDE STANDPIPE PIEZOMETER | ●      | II        | SP1         | -45      | 35.0m U/S            | EL.45.000m                | --            |                                   |
|                                 |        |           | SP2         | -150     | 50.0m D/S            | EL.100.000m               | --            |                                   |
|                                 |        |           | SP3         | -50      | 50.0m D/S            | EL.100.000m               | --            | ELEVATION REFER TO PIEZOMETER TIP |
|                                 |        |           | SP4         | 38       | 50.0m D/S            | EL.100.000m               | --            |                                   |
|                                 |        |           | SP5         | 200      | 50.0m D/S            | EL.100.000m               | --            |                                   |
|                                 |        |           | SP6         | 40       | 150.0m D/S           | EL.45.000m                | --            |                                   |
|                                 |        |           | SP7         | 185      | 150.0m D/S           | EL.45.000m                | --            |                                   |
| ELECTRICAL PIEZOMETER           | ⊗      | E         | P1          | 100      | 8.0m U/S             | EL.115.000m               | TB1           |                                   |
|                                 |        |           | P2          | 100      | 8.0m D/S             | EL.115.000m               | TB1           |                                   |
|                                 |        |           | P3          | 100      | 5.5m U/S             | EL.120.000m               | TB1           |                                   |
|                                 |        |           | P4          | 100      | 5.5m D/S             | EL.120.000m               | TB1           |                                   |
|                                 |        |           | P5          | 100      | 3.0m U/S             | EL.140.000m               | TB1           |                                   |
|                                 |        |           | P6          | 100      | 3.0m D/S             | EL.140.000m               | TB1           |                                   |
|                                 |        |           | P7          | 145      | 14.0m U/S            | EL.73.000m                | TB2           |                                   |
|                                 |        |           | P8          | 145      | 14.0m D/S            | EL.73.000m                | TB2           |                                   |
|                                 |        |           | P9          | 145      | 14.0m U/S            | EL.78.000m                | TB2           |                                   |
|                                 |        |           | P10         | 145      | 14.0m D/S            | EL.78.000m                | TB2           |                                   |
|                                 |        |           | P11         | 145      | 10.0m U/S            | EL.82.000m                | TB2           |                                   |
|                                 |        |           | P12         | 145      | 10.0m D/S            | EL.82.000m                | TB2           |                                   |
|                                 |        |           | P13         | 145      | 8.0m U/S             | EL.100.000m               | TB3           | ELEVATION REFER TO PIEZOMETER TIP |
|                                 |        |           | P14         | 145      | 8.0m D/S             | EL.100.000m               | TB3           |                                   |
|                                 |        |           | P15         | 145      | 5.5m U/S             | EL.120.000m               | TB3           |                                   |
|                                 |        |           | P16         | 145      | 5.5m D/S             | EL.120.000m               | TB3           |                                   |
|                                 |        |           | P17         | 145      | 3.0m U/S             | EL.140.000m               | TB3           |                                   |
|                                 |        |           | P18         | 145      | 3.0m D/S             | EL.140.000m               | TB3           |                                   |
|                                 |        |           | P19         | 190      | 8.0m U/S             | EL.110.000m               | TB3           |                                   |
|                                 |        |           | P20         | 190      | 8.0m D/S             | EL.110.000m               | TB3           |                                   |
|                                 |        |           | P21         | 190      | 5.5m U/S             | EL.120.000m               | TB3           |                                   |
|                                 |        |           | P22         | 190      | 5.5m D/S             | EL.120.000m               | TB3           |                                   |
|                                 |        |           | P23         | 190      | 3.0m U/S             | EL.140.000m               | TB3           |                                   |
|                                 |        |           | P24         | 190      | 3.0m D/S             | EL.140.000m               | TB3           |                                   |
| PROBE EXTENSOMETER              | ○      | ⊥         | PE1         | 145      | DAM AXIS             | EL.80.000m<br>EL.157.000m | --            |                                   |
| FOUNDATION DEFORMATION METER    |        | ⊥         | FD1         | 145      | 0.5m U/S             | EL.78.500m<br>EL.50.000m  | TB2           |                                   |
| TRI-AXIAL JOINT METER           |        | ⊥         | JM1         | 153.5    | 3.0m U/S             | EL.79.500m                | TB2           |                                   |
|                                 |        | ⊥         | JM2         | 158.5    | 3.0m D/S             | EL.80.000m                | TB2           |                                   |
| SEEPAGE MEASURING DEVICE        |        | E         | SM1         | 150.15   | 21.4m D/S            | EL.78.500m                | --            |                                   |
|                                 |        | E         | SM2         | 150.85   | 21.4m D/S            | EL.78.500m                | --            |                                   |
| STRONG MOTION ACCELEROGRAPH     | ◆      | E         | EA1         | 143      | 0.75m U/S            | EL.78.500m                | --            |                                   |
|                                 |        | E         | EA2         | 143      | 4.0m U/S             | EL.157.000m               | --            |                                   |
| TERMINAL BOX                    |        | E         | TB1         | 108.5    | 1.0m D/S             | EL.105.800m               | --            | 6 POINTS                          |
|                                 |        | E         | TB2         | 149      | 1.0m D/S             | EL.77.300m                | --            | 9 POINTS                          |
|                                 |        | E         | TB3         | 186.5    | 1.0m D/S             | EL.105.800m               | --            | 12 POINTS                         |



THE DETAILED DESIGN OF FLOOD CONTROL, URBAN DRAINAGE AND WATER RESOURCES DEVELOPMENT IN SEMARANG IN THE REPUBLIC OF INDONESIA

JAPAN INTERNATIONAL COOPERATION AGENCY

Fig. 7.2.33 (1/2)  
LAYOUT OF INSTRUMENTS



**NOTES**

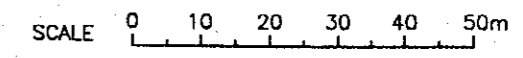
1. ALL DIMENSIONS ARE IN MILLIMETERS, UNLESS OTHERWISE NOTED.
2. SECTIONS ARE REFERRED FROM DRAWING NO. JD-P1-ED-In-1.
3. INSTRUMENTATION LOCATIONS AND ELEVATION ARE APPROXIMATE AND WILL BE FINALIZED BY THE ENGINEER AS PER SITE CONDITIONS.

**REFERENCE DRAWINGS**

JD-P1-ED-In-1 INSTRUMENTATION - LAYOUT PLAN AND PROFILE

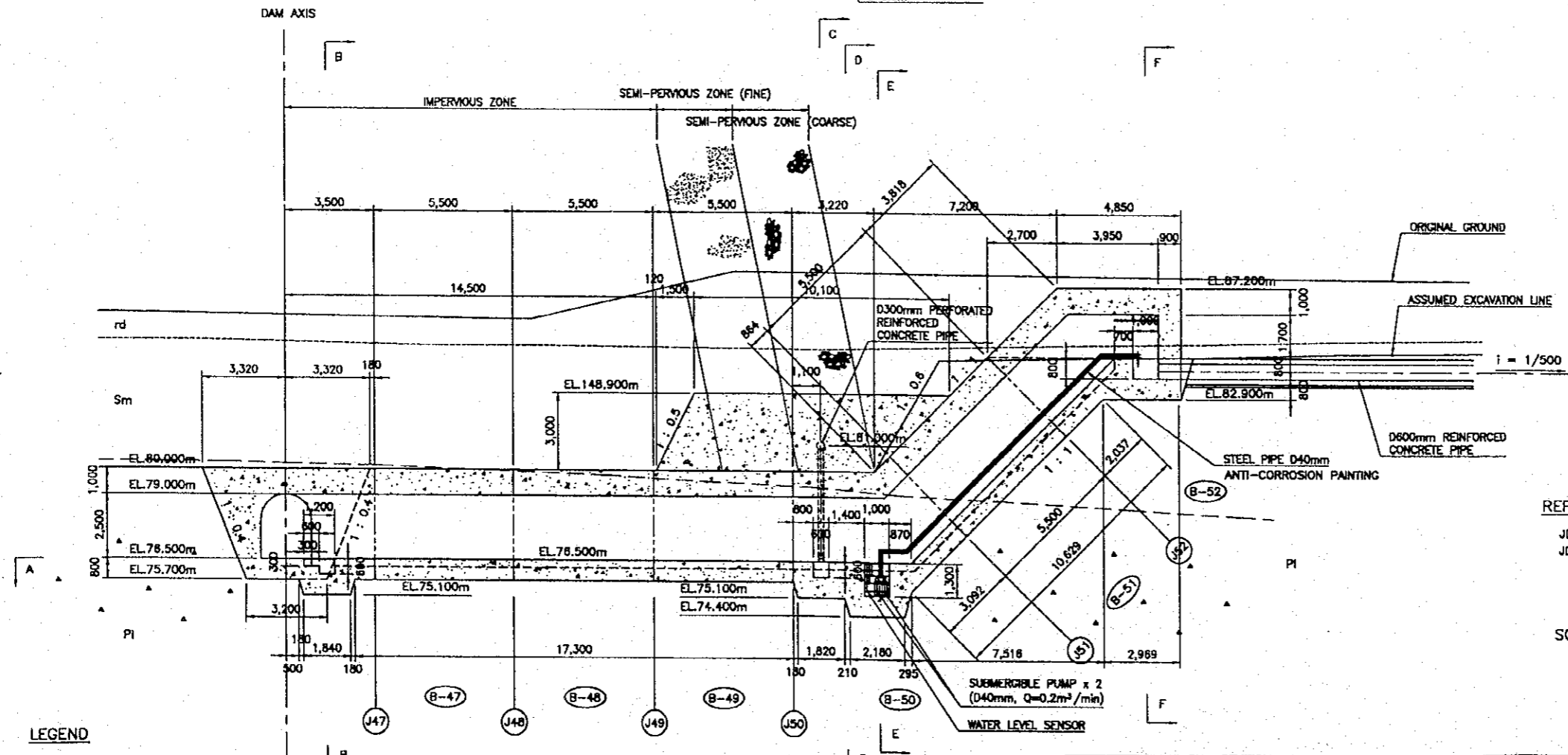
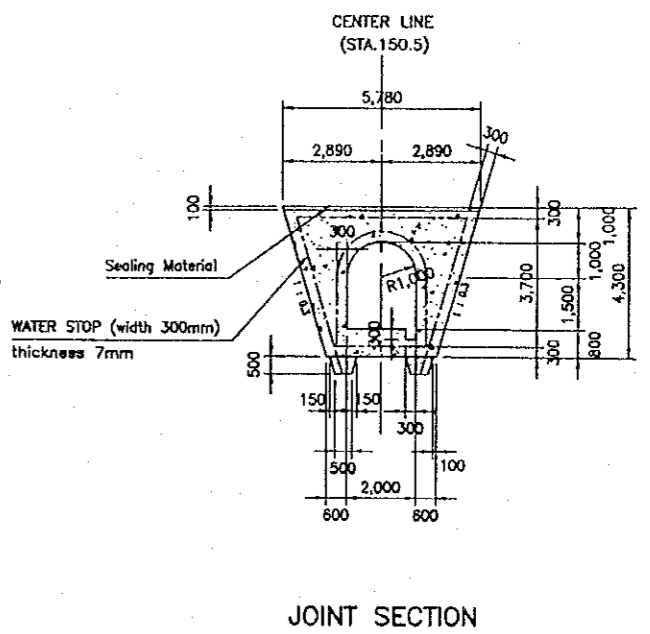
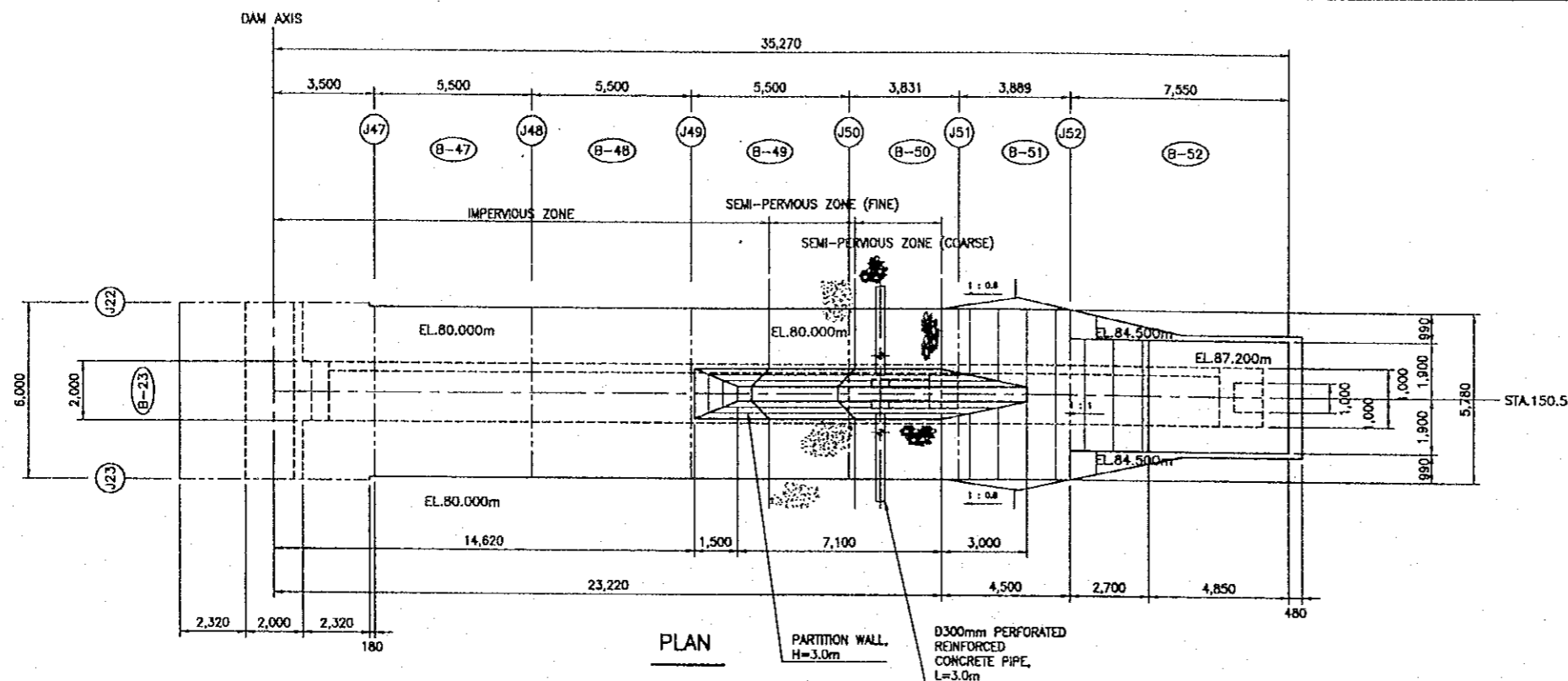
**LEGEND**

| TYPE OF INSTRUMENT           | SYMBOL | DESIGNATION |
|------------------------------|--------|-------------|
| SURFACE MOVEMENT MARKER      | ⊞      | M           |
| ELECTRICAL PIEZOMETER        | ⊠      | P           |
| PROVE EXTENSOMETER           | ⊥      | PE          |
| FOUNDATION DEFORMATION METER | ⊥      | FD          |
| SEEPAGE MEASURING DEVICE     | ⊞      | SM          |
| STRONG MOTION ACCELEROGRAPH  | ⊞      | EA          |



THE DETAILED DESIGN OF FLOOD CONTROL, URBAN DRAINAGE AND WATER RESOURCES DEVELOPMENT IN SEMARANG IN THE REPUBLIC OF INDONESIA  
 JAPAN INTERNATIONAL COOPERATION AGENCY

Fig. 7.2.33 (2/2)  
 LAYOUT OF INSTRUMENTS



REFERENCE DRAWINGS  
 JD-P1-ED-Ga-1 GALLERY - LAYOUT PLAN AND PROFILE  
 JD-P1-ED-Ga-5 GALLERY - STRUCTURAL DETAILS (4/4)

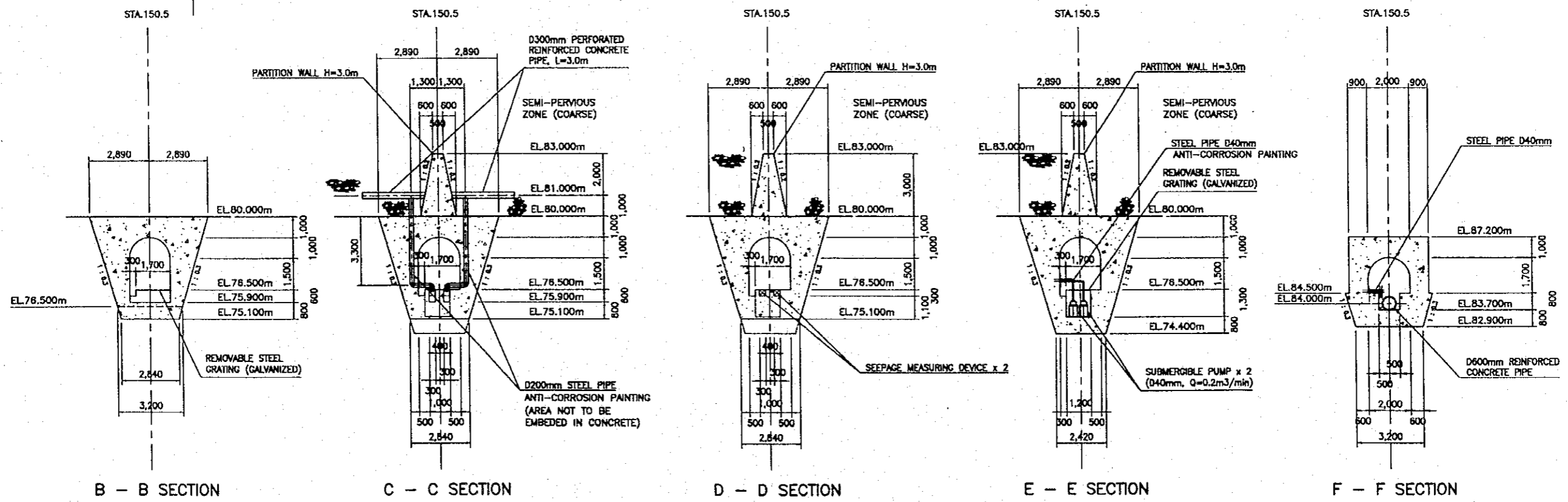
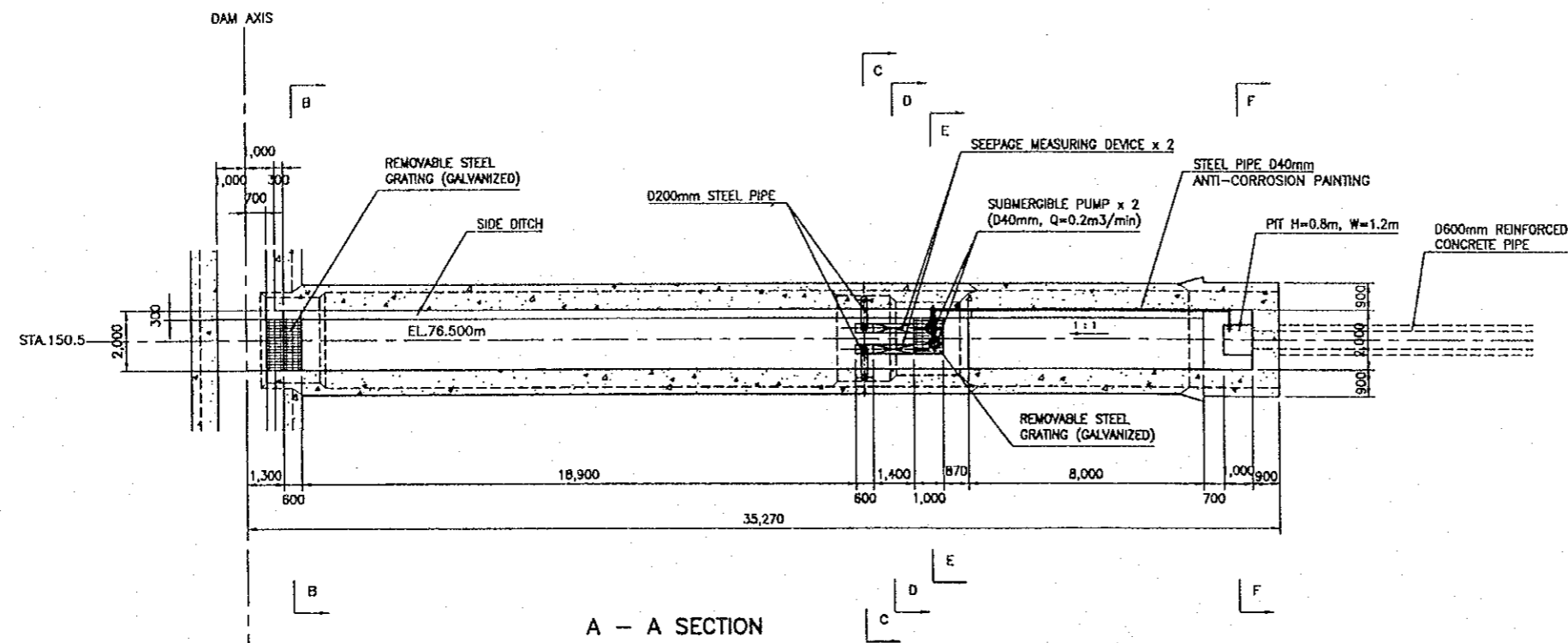
SCALE 0 2 4 6 8 10m

LEGEND  
 rd : RIVER DEPOSIT  
 Sm : MIDDLE SEDIMENTARY ROCK UNIT  
 PI : LOWER PYROCLASTIC ROCK UNIT

LONGITUDINAL PROFILE (STA.150.5)

THE DETAILED DESIGN OF FLOOD CONTROL, URBAN DRAINAGE AND WATER RESOURCES DEVELOPMENT IN SEMARANG IN THE REPUBLIC OF INDONESIA  
 JAPAN INTERNATIONAL COOPERATION AGENCY

Fig. 7.234 (1/2)  
 LAYOUT OF SEEPAGE MEASURING DEVICES



**REFERENCE DRAWINGS**

- JD-P1-ED-Ga-1 GALLERY - LAYOUT PLAN AND PROFILE
- JD-P1-ED-Ga-4 GALLERY - STRUCTURAL DETAILS (3/4)

SCALE 0 2 4 6 8 10m

THE DETAILED DESIGN OF FLOOD CONTROL, URBAN DRAINAGE AND WATER RESOURCES DEVELOPMENT IN SEMARANG IN THE REPUBLIC OF INDONESIA

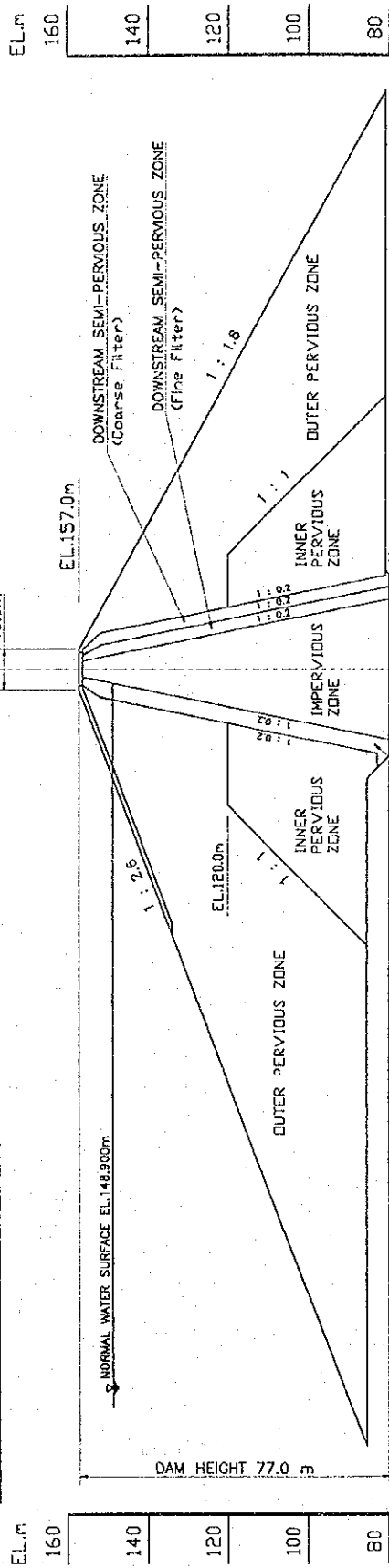
Fig. 7.2.34 (2/2)  
LAYOUT OF SEEPAGE MEASURING DEVICES

JAPAN INTERNATIONAL COOPERATION AGENCY

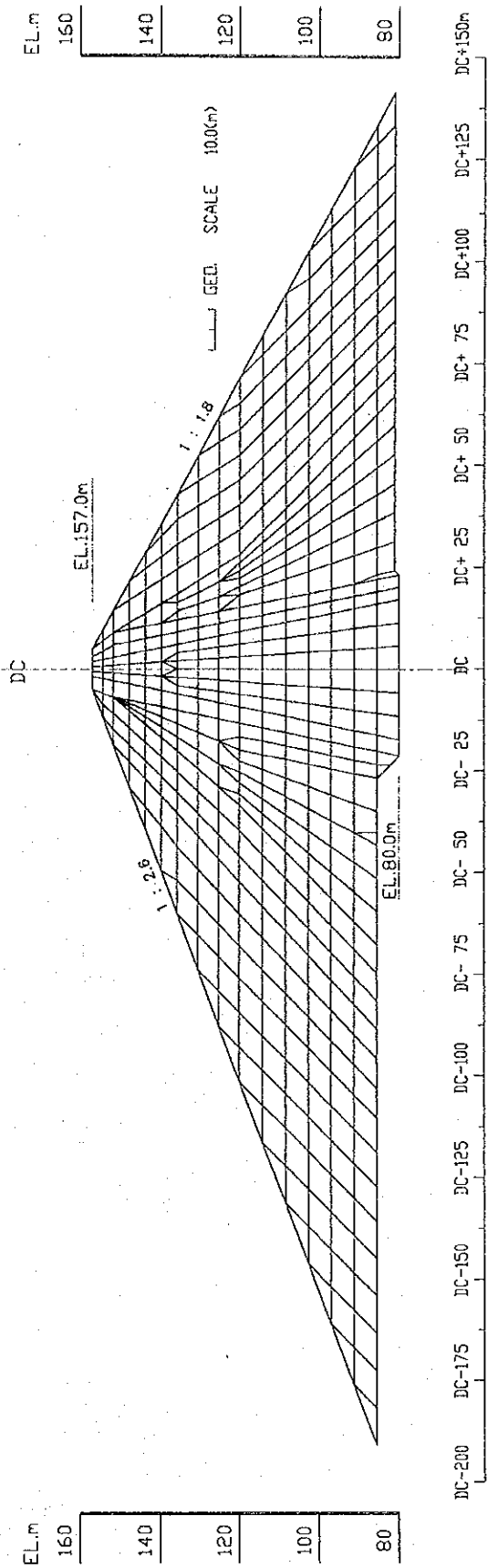


STATIC ANALYSIS FOR DAM

Cross Section (Sta. 140)



Finite Element Mesh



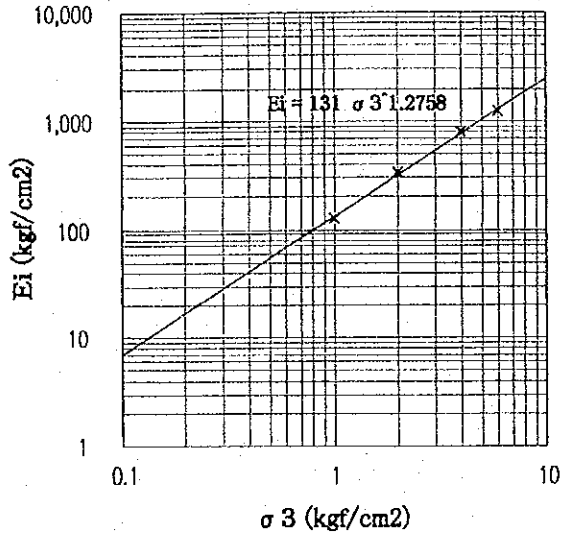
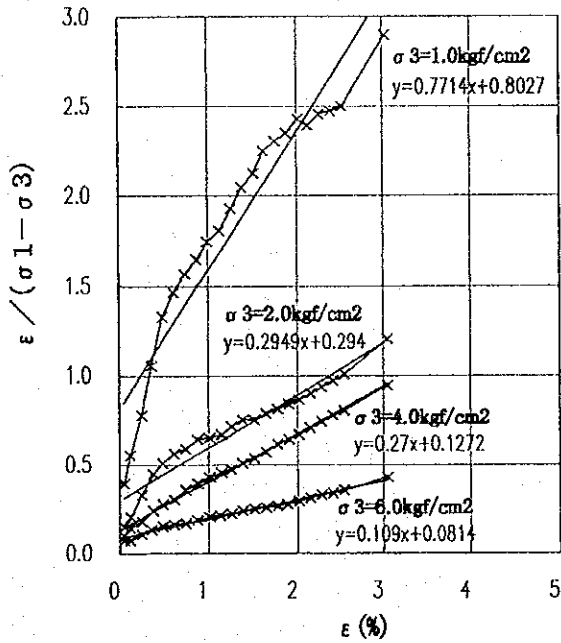
THE DETAILED DESIGN OF FLOOD CONTROL, URBAN DRAINAGE AND WATER RESOURCES DEVELOPMENT IN SEMARANG IN THE REPUBLIC OF INDONESIA

JAPAN INTERNATIONAL COOPERATION AGENCY

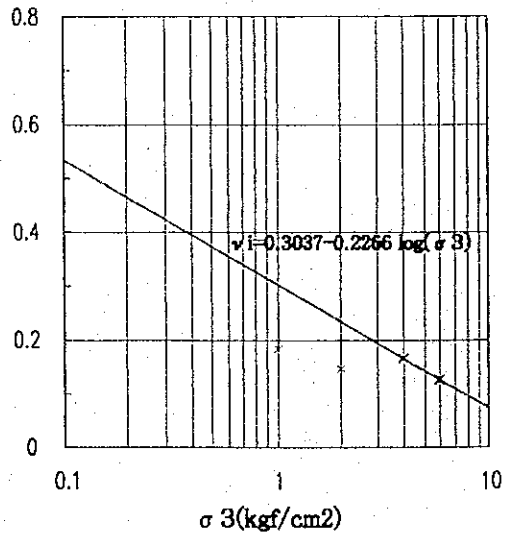
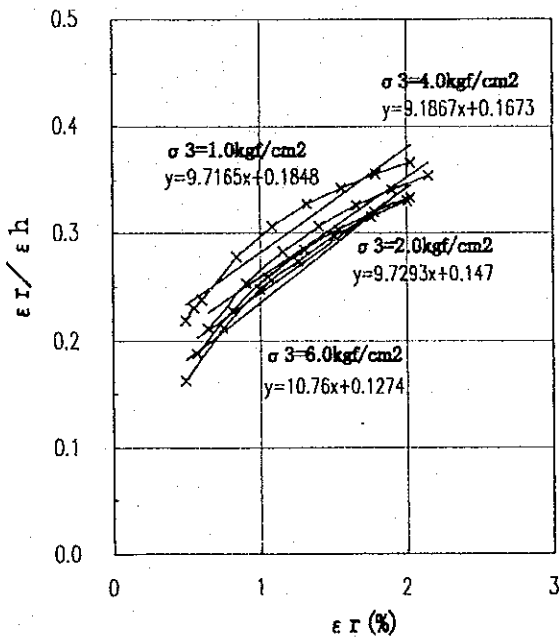
Fig. 73.1

FINITE ELEMENT MESH OF STATIC ANALYSIS FOR DAM

PROPERTIES OF IMPERVIOUS MATERIAL



ELASTIC MODULUS



POISSON'S RATIO

THE DETAILED DESIGN OF FLOOD CONTROL, URBAN DRAINAGE AND WATER RESOURCES DEVELOPMENT IN SEMARANG IN THE REPUBLIC OF INDONESIA

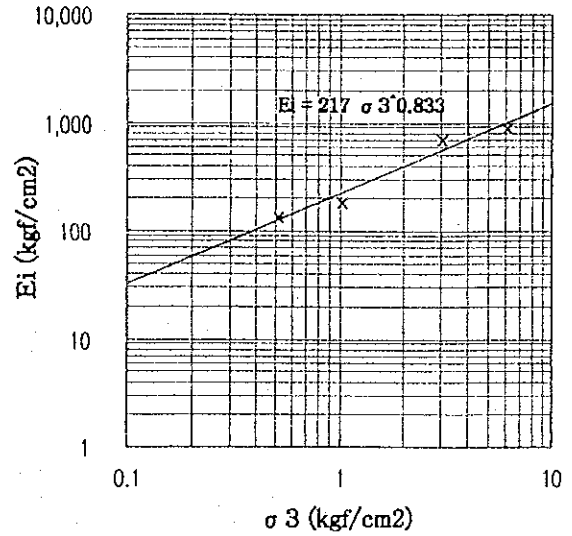
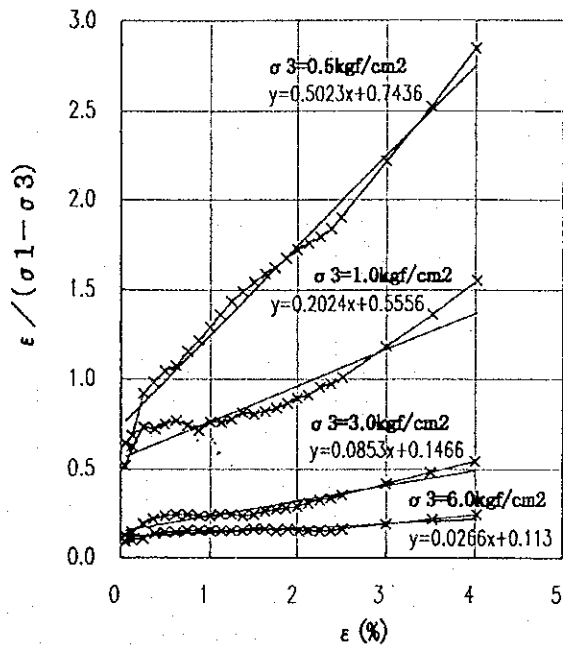
JAPAN INTERNATIONAL COOPERATION AGENCY

Fig.7.3.2 (1/3)

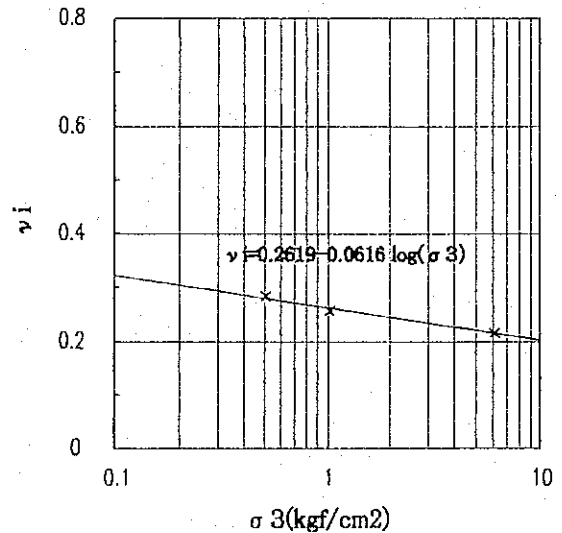
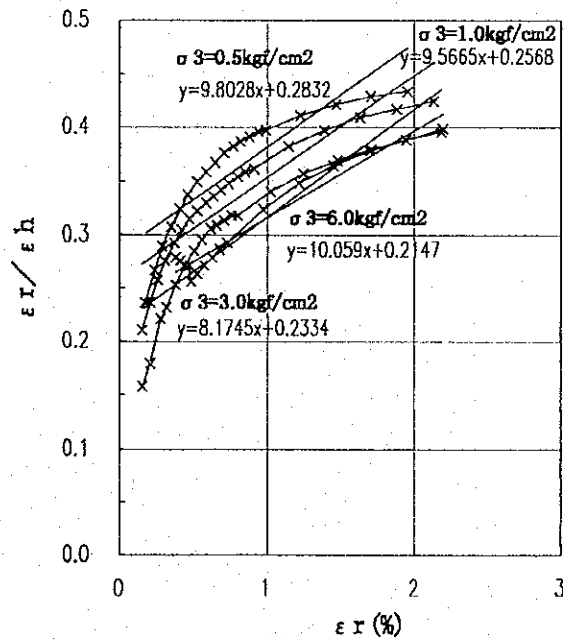
DETERMINATION PROCEDURE OF STATIC MATERIAL PROPERTIES



PROPERTIES OF SEMI-PERVIOUS MATERIAL



ELASTIC MODULUS



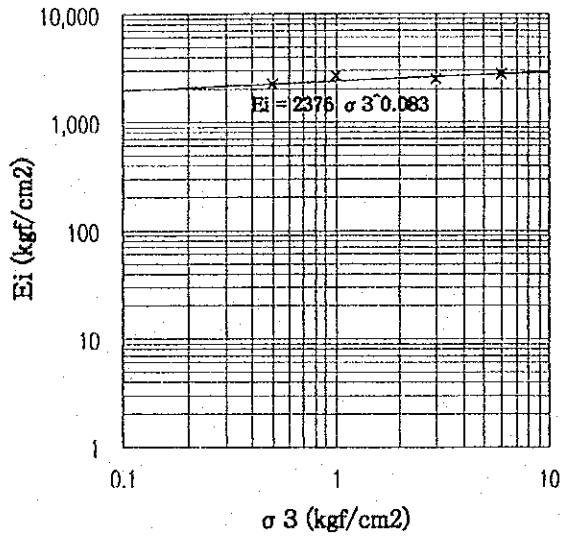
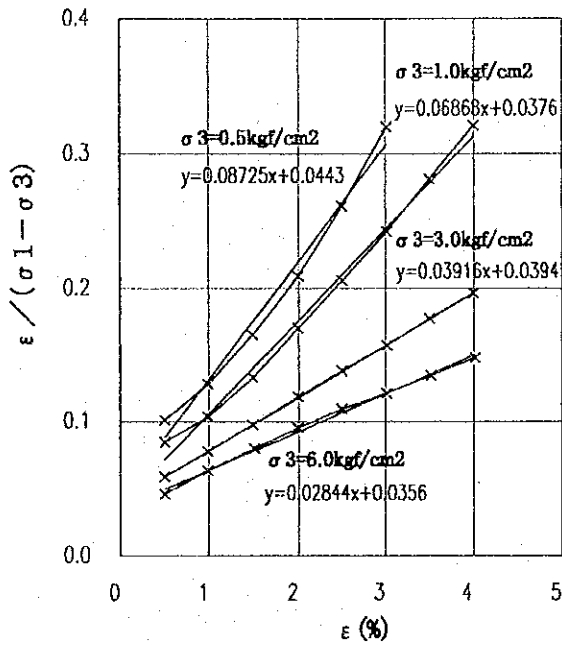
POISSON'S RATIO

THE DETAILED DESIGN OF FLOOD CONTROL, URBAN DRAINAGE AND WATER RESOURCES DEVELOPMENT IN SEMARANG IN THE REPUBLIC OF INDONESIA

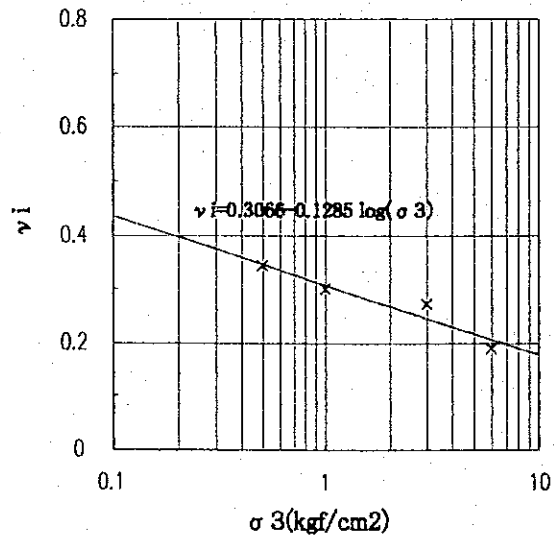
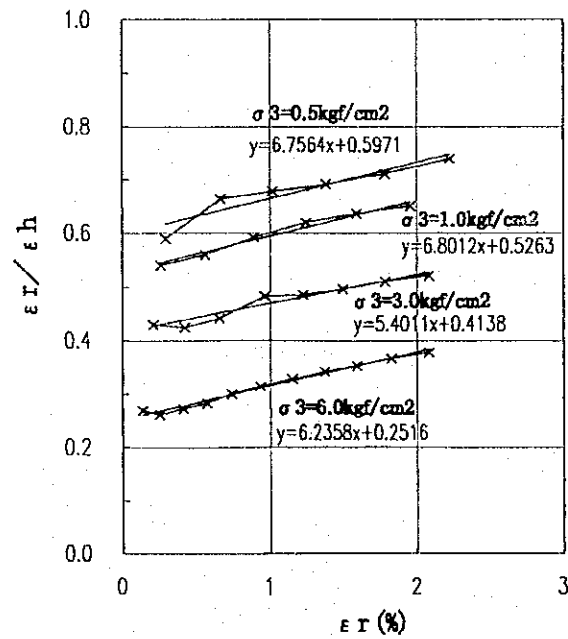
JAPAN INTERNATIONAL COOPERATION AGENCY

Fig. 7.3.2 (2/3)  
DETERMINATION PROCEDURE OF STATIC MATERIAL PROPERTIES

PROPERTIES OF PERVIOUS MATERIAL



ELASTIC MODULUS



POISSON'S RATIO

THE DETAILED DESIGN OF FLOOD CONTROL, URBAN DRAINAGE AND WATER RESOURCES DEVELOPMENT IN SEMARANG IN THE REPUBLIC OF INDONESIA

JAPAN INTERNATIONAL COOPERATION AGENCY

Fig. 7.3.2 (3/3)

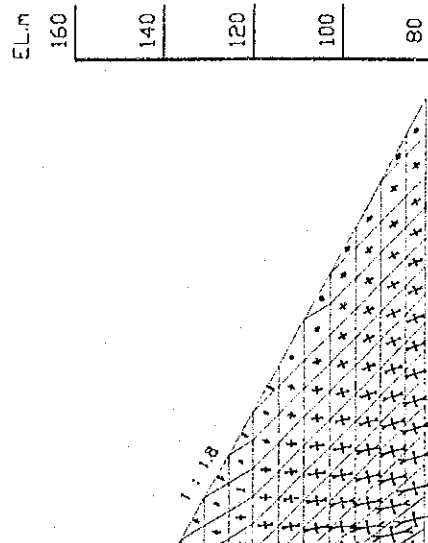
DETERMINATION PROCEDURE OF STATIC MATERIAL PROPERTIES

STATIC ANALYSIS FOR DAM

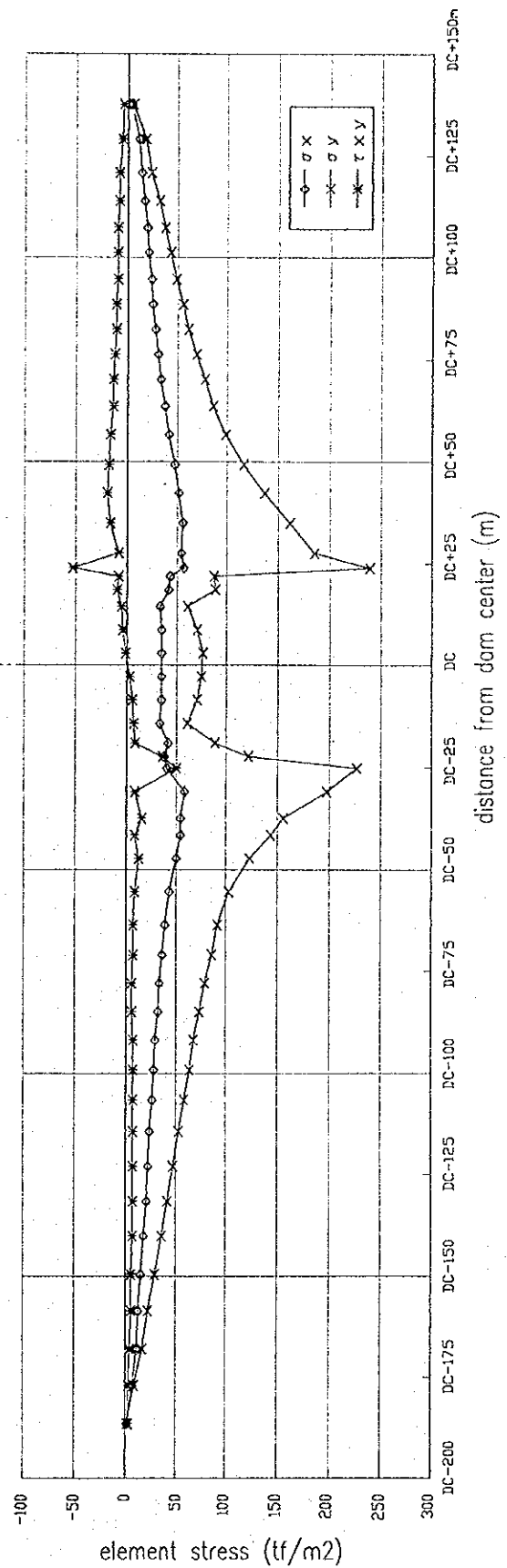
At End of Construction

Stress Vectors

| EL.m | Zone                                   | Maximum Principal Stress (tf/m <sup>2</sup> ) | Stress |
|------|--|---|--------|
| 160  | Upstream Pervious Zone                 | 233.9   |        |
| 140  | Upstream Semi-Pervious Zone            | 89.2  |        |
|      | Impervious Zone                        | 75.3  |        |
| 120  | Downstream Semi-Pervious Zone (fine)   | 90.9  |        |
|      | Downstream Semi-Pervious Zone (coarse) | 88.8  |        |
| 80   | Downstream Pervious Zone               | 253.9   |        |



Stresses on Foundation



THE DETAILED DESIGN OF FLOOD CONTROL, URBAN DRAINAGE AND WATER RESOURCES DEVELOPMENT IN SEMARANG IN THE REPUBLIC OF INDONESIA

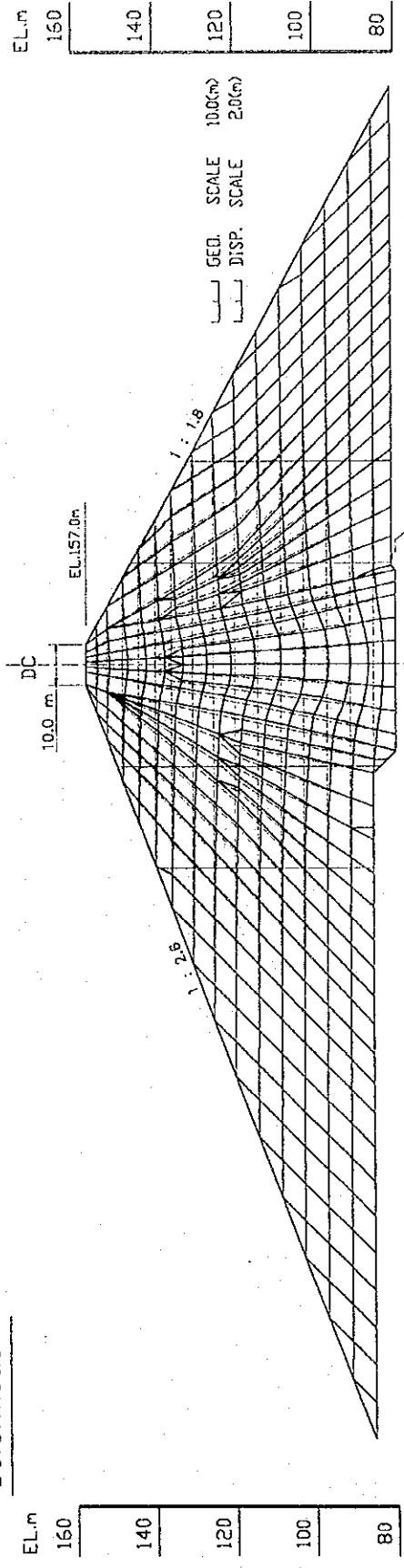
JAPAN INTERNATIONAL COOPERATION AGENCY

Fig. 7.3.3 RESULTS OF STATIC ANALYSIS FOR DAM (END OF CONSTRUCTION, STRESSES)

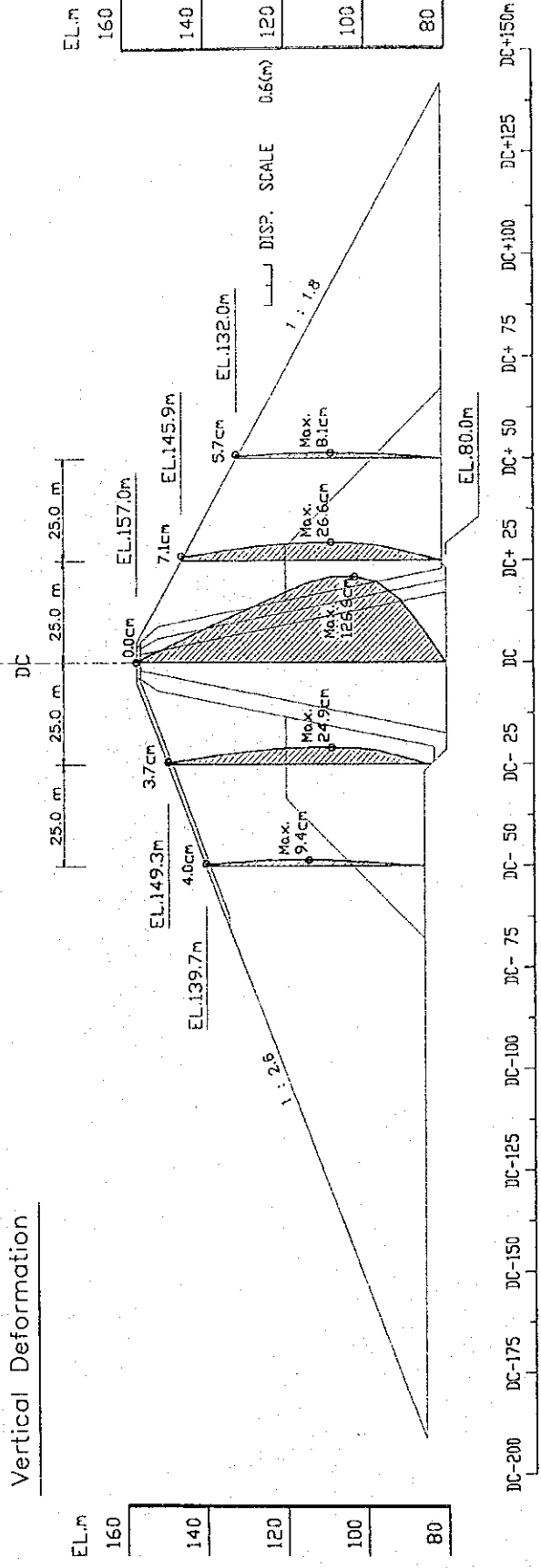
STATIC ANALYSIS FOR DAM

At End of Construction

Deformation



Vertical Deformation



THE DETAILED DESIGN OF FLOOD CONTROL, URBAN DRAINAGE AND WATER RESOURCES DEVELOPMENT IN SEMARANG IN THE REPUBLIC OF INDONESIA

JAPAN INTERNATIONAL COOPERATION AGENCY

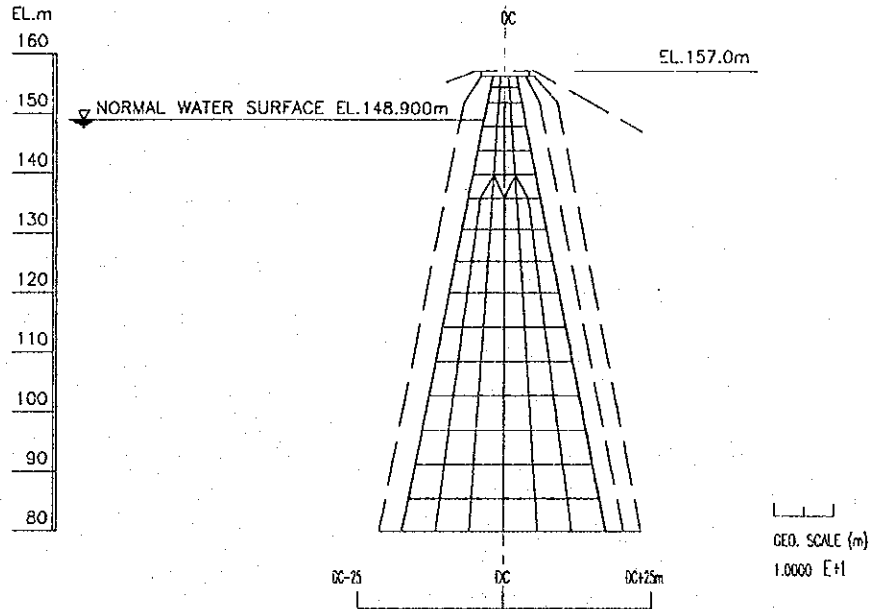
Fig. 7.3.4 RESULTS OF STATIC ANALYSIS FOR DAM (END OF CONSTRUCTION, DISPLACEMENTS)

# STRESS-STRAIN ANALYSIS FOR DAM

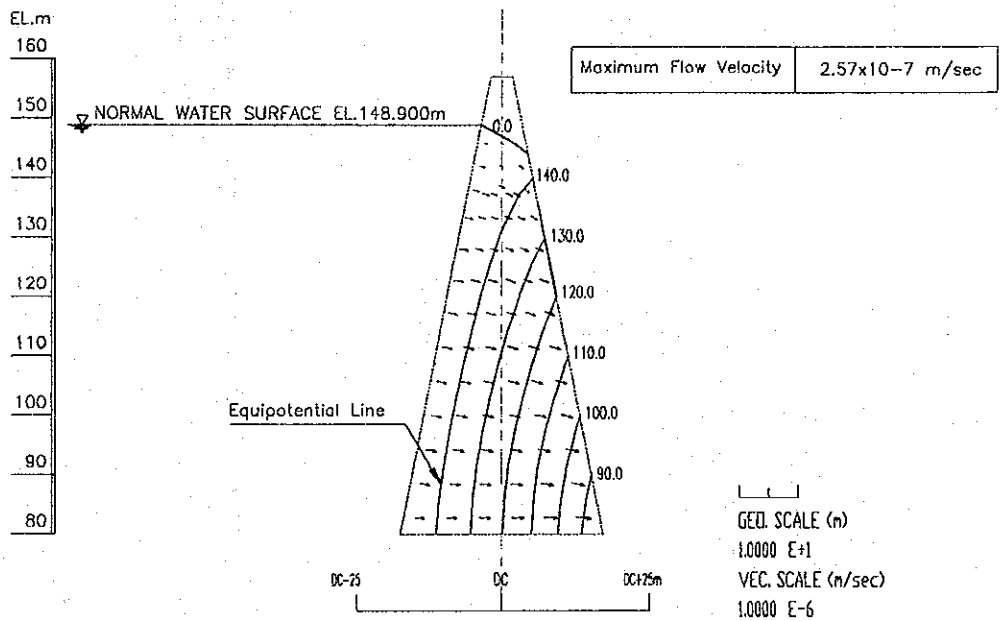
After Reservoir Filling

## Seepage Analysis

### Finite Element Mesh



### Flow Velocity



THE DETAILED DESIGN OF FLOOD CONTROL, URBAN DRAINAGE AND WATER RESOURCES DEVELOPMENT IN SEMARANG IN THE REPUBLIC OF INDONESIA

JAPAN INTERNATIONAL COOPERATION AGENCY

Fig. 7.3.5

**RESULTS OF SEEPAGE ANALYSIS FOR STATIC ANALYSIS**

# STATIC ANALYSIS FOR DAM

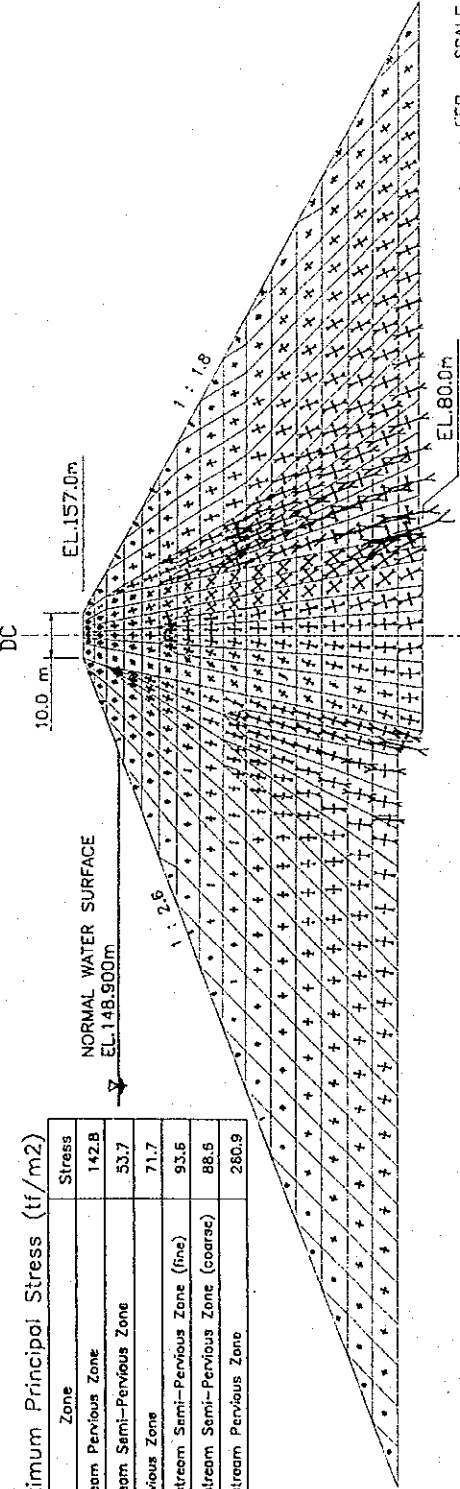
After Reservoir Filling

## Stress Vectors

| Zone                                   | Stress (tf/m <sup>2</sup> ) |
|--|-----------------------------|
| Upstream Pervious Zone                 | 142.8                       |
| Upstream Semi-Pervious Zone            | 53.7                        |
| Impervious Zone                        | 71.7                        |
| Downstream Semi-Pervious Zone (fine)   | 93.5                        |
| Downstream Semi-Pervious Zone (coarse) | 88.5                        |
| Downstream Pervious Zone               | 280.9                       |

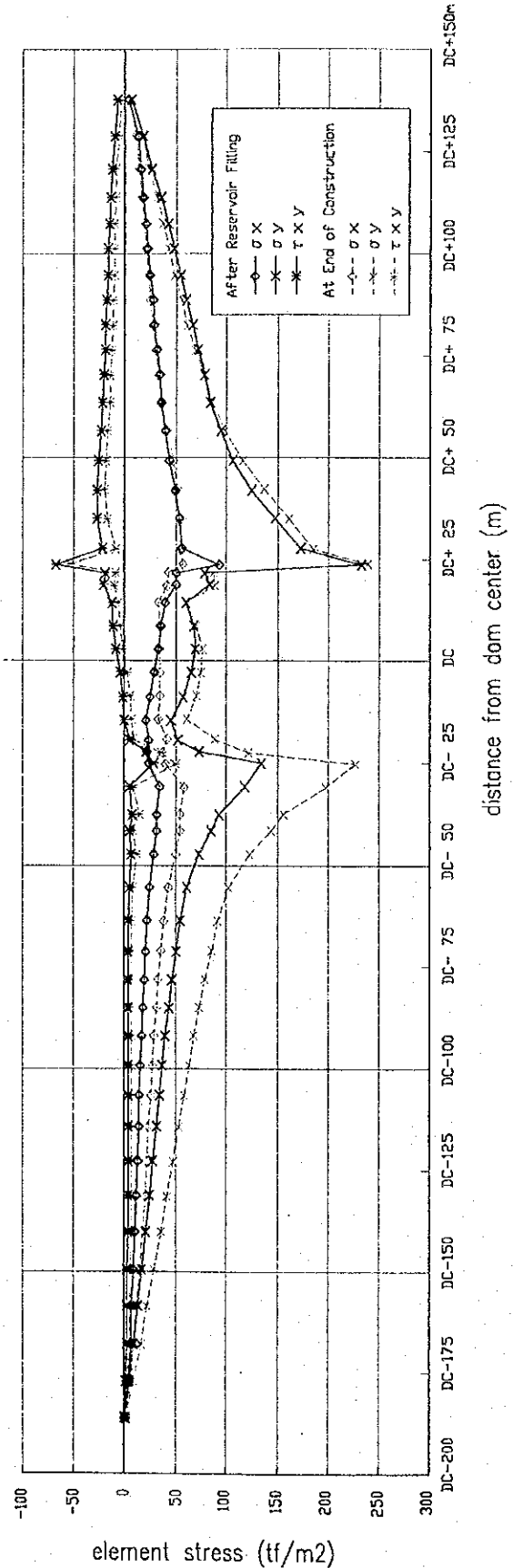
| EL.m | 160 | 140 | 120 | 100 | 80 |
|------|-----|-----|-----|-----|----|
|------|-----|-----|-----|-----|----|

| EL.m | 160 | 140 | 120 | 100 | 80 |
|------|-----|-----|-----|-----|----|
|------|-----|-----|-----|-----|----|



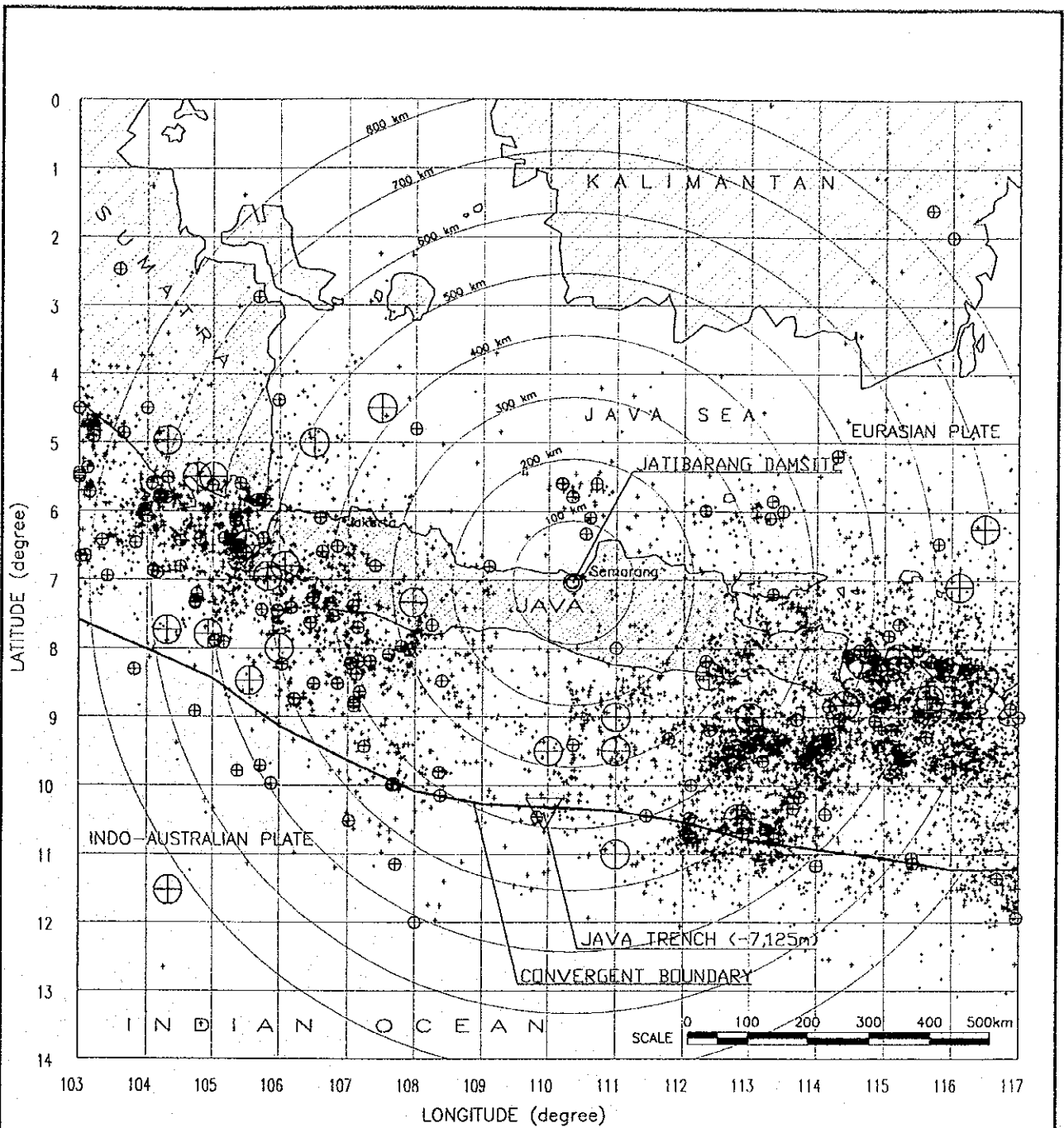
GED. SCALE 100(m)  
STRESS SCALE 200.0(tf/m<sup>2</sup>)

## Stresses on Foundation



THE DETAILED DESIGN OF FLOOD CONTROL, URBAN DRAINAGE AND WATER RESOURCES DEVELOPMENT IN SEMARANG IN THE REPUBLIC OF INDONESIA  
JAPAN INTERNATIONAL COOPERATION AGENCY

Fig. 73.6 RESULTS OF STATIC ANALYSIS FOR DAM (AFTER RESERVOIR FILLING, STRESSES)



LEGEND

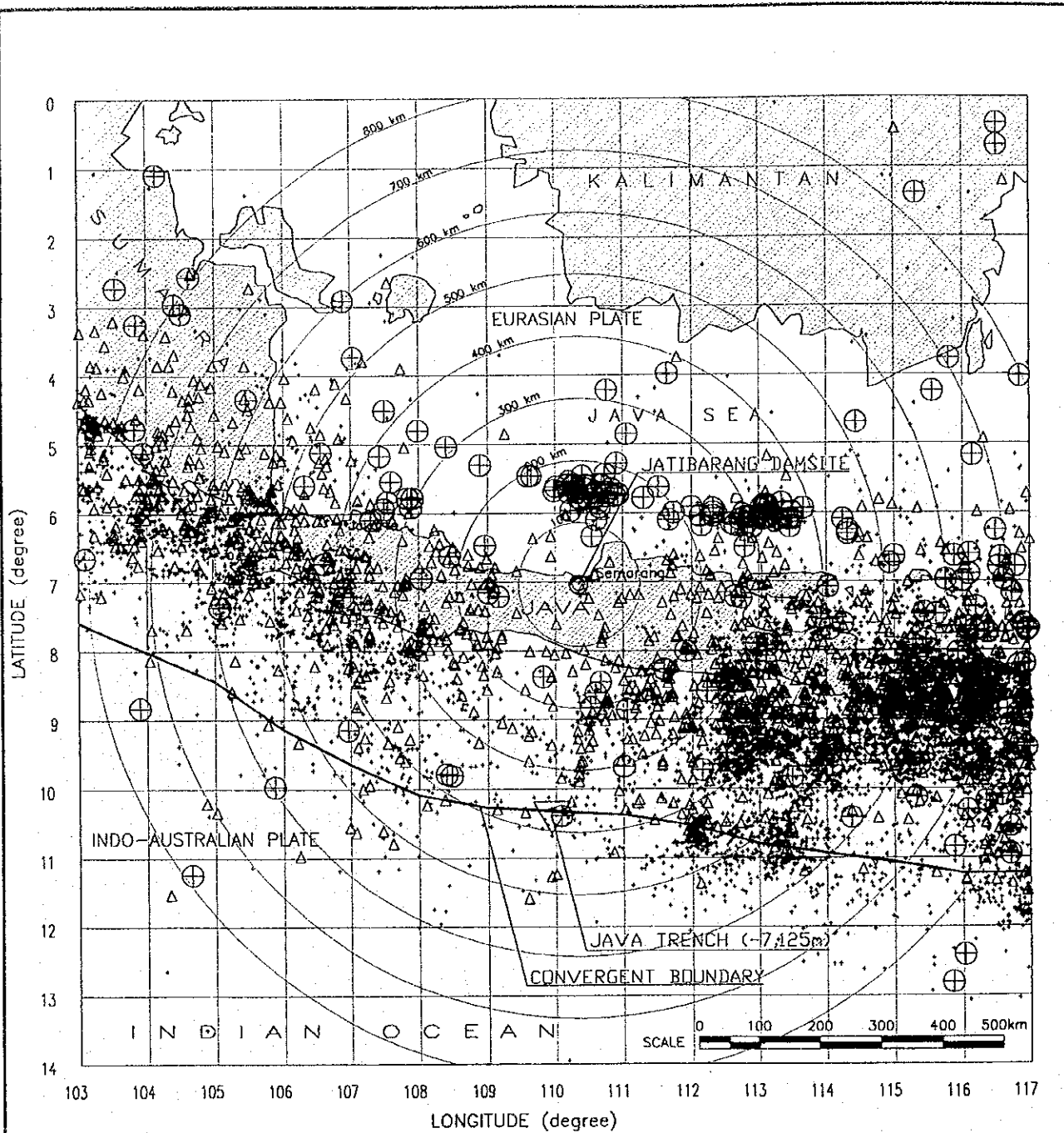
|                                |   |           |   |           |
|--------------------------------|---|-----------|---|-----------|
| MAGNITUDE OF EARTHQUAKE        | ·   | 4.0 - 4.9 | ⊕ | 6.0 - 6.9 |
|                                | +   | 5.0 - 5.9 | ⊕ | 7.0 - 7.9 |
| LOCATION OF JATIBARANG DAMSITE | LATITUDE 7° 2' 10" S<br>LONGITUDE 110° 21' 3" E |           |   |           |

\* About 7,200 seismic records from 1900 to 1999, qualified by Meteorological and Geophysical Agency.

THE DETAILED DESIGN OF FLOOD CONTROL, URBAN DRAINAGE AND WATER RESOURCES DEVELOPMENT IN SEMARANG IN THE REPUBLIC OF INDONESIA

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Fig.7.3.7  
**HISTORICAL SEISMIC DATA AROUND JAVA ISLAND (BY MAGNITUDE)**



LEGEND

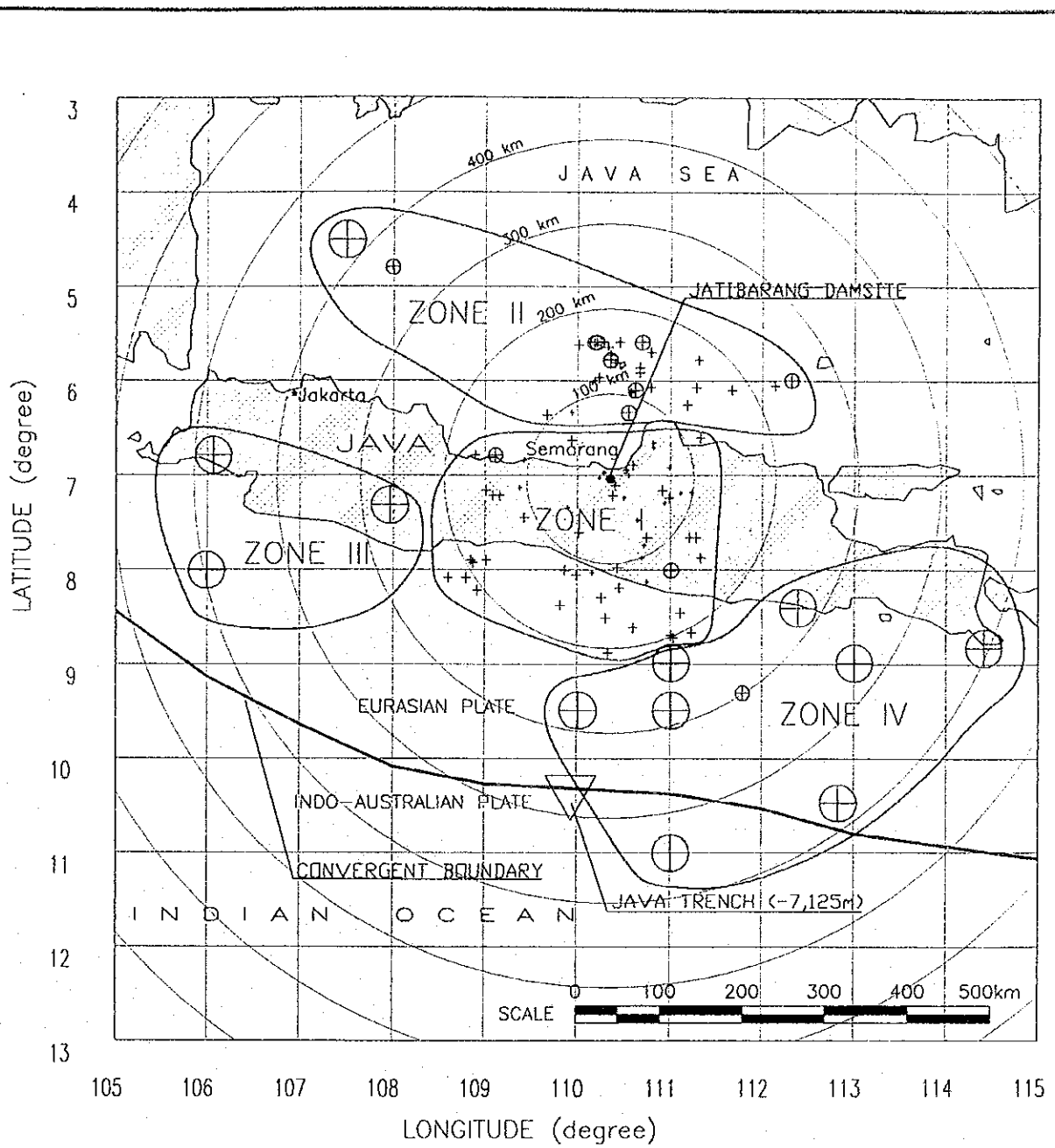
|                                |   |
|--------------------------------|---|
| DEPTH OF EPICENTER             | • 0 - 99 km                                     |
|                                | △ 100 - 299 km                                  |
|                                | ⊕ 300 km -                                      |
| LOCATION OF JATIBARANG DAMSITE | LATITUDE 7° 2' 10" S<br>LONGITUDE 110° 21' 3" E |

\* About 7,200 seismic records from 1900 to 1999, qualified by Meteorological and Geophysical Agency.

THE DETAILED DESIGN OF FLOOD CONTROL, URBAN DRAINAGE AND WATER RESOURCES DEVELOPMENT IN SEMARANG IN THE REPUBLIC OF INDONESIA  
JAPAN INTERNATIONAL COOPERATION AGENCY

Fig. 7.3.8  
**HISTORICAL SEISMIC DATA AROUND JAVA ISLAND (BY DEPTH)**





LEGEND

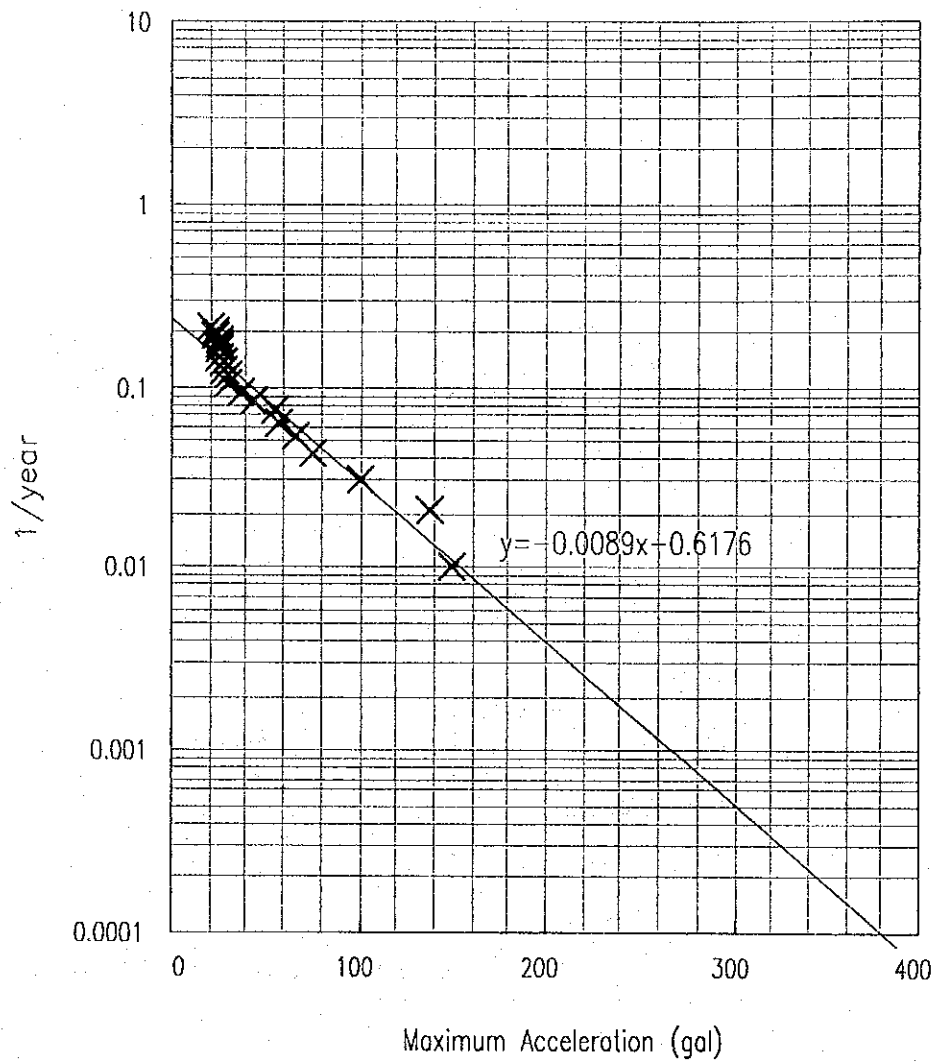
|                                |   |             |
|--------------------------------|---|-------------|
| MAGNITUDE OF EARTHQUAKE        | • 4.0 - 4.9                                     | ⊕ 6.0 - 6.9 |
|                                | + 5.0 - 5.9                                     | ⊕ 7.0 - 7.9 |
| LOCATION OF JATIBARANG DAMSITE | LATITUDE 7° 2' 10" S<br>LONGITUDE 110° 21' 3" E |             |

\* 100 seismic records from 1900 to 1999, influenced to Jatibarang Damsite.

THE DETAILED DESIGN OF FLOOD CONTROL, URBAN DRAINAGE AND WATER RESOURCES DEVELOPMENT IN SEMARANG IN THE REPUBLIC OF INDONESIA

Fig. 7.3.9  
**HISTORICAL SEISMIC DATA AROUND JATIBARANG DAMSITE**

JAPAN INTERNATIONAL COOPERATION AGENCY



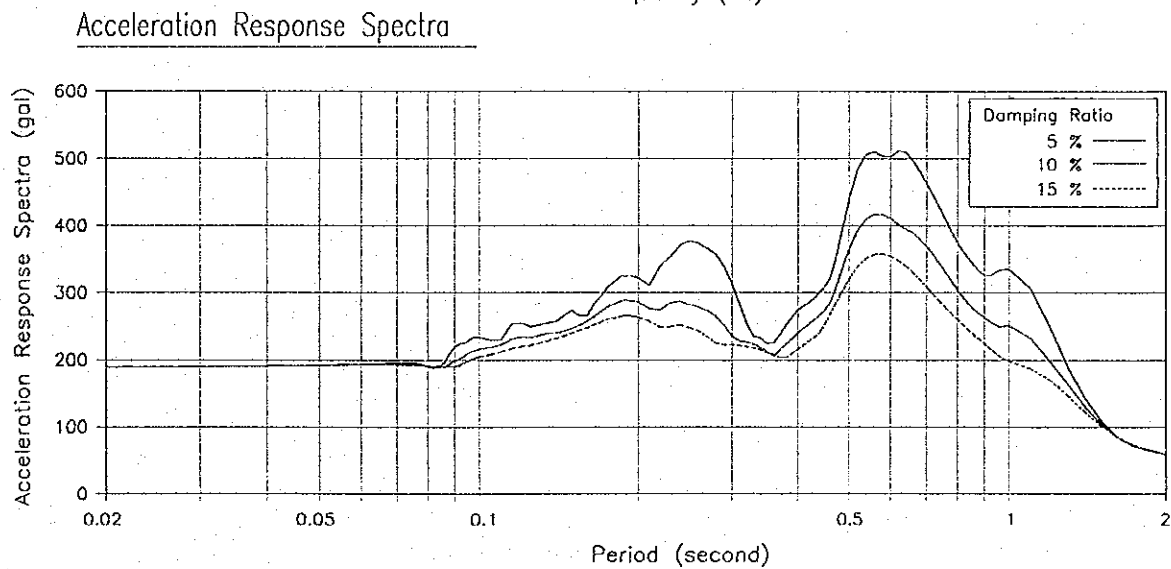
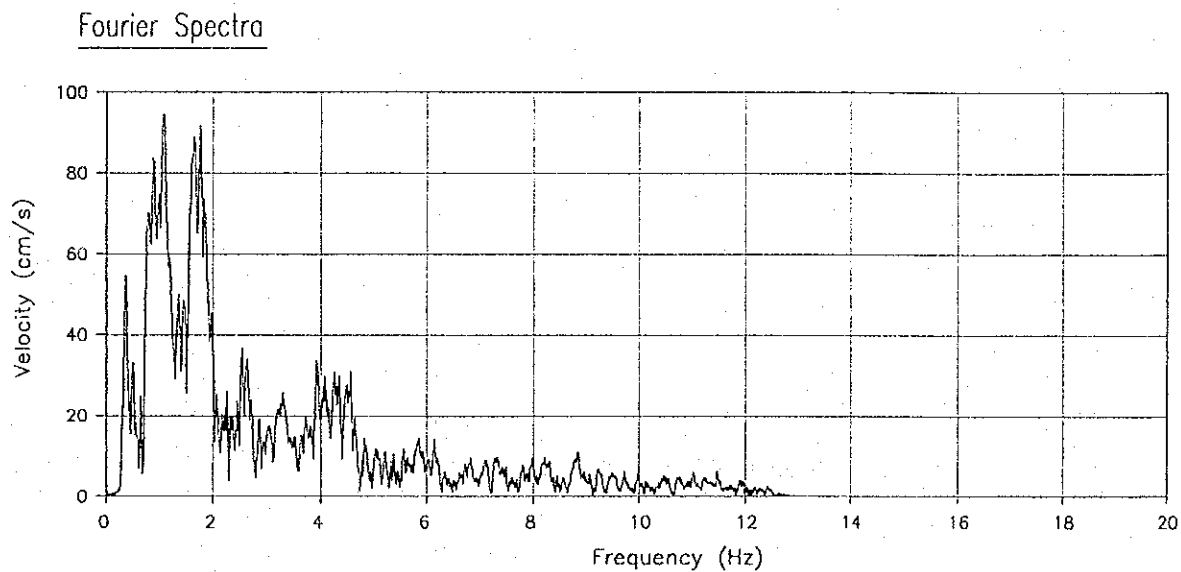
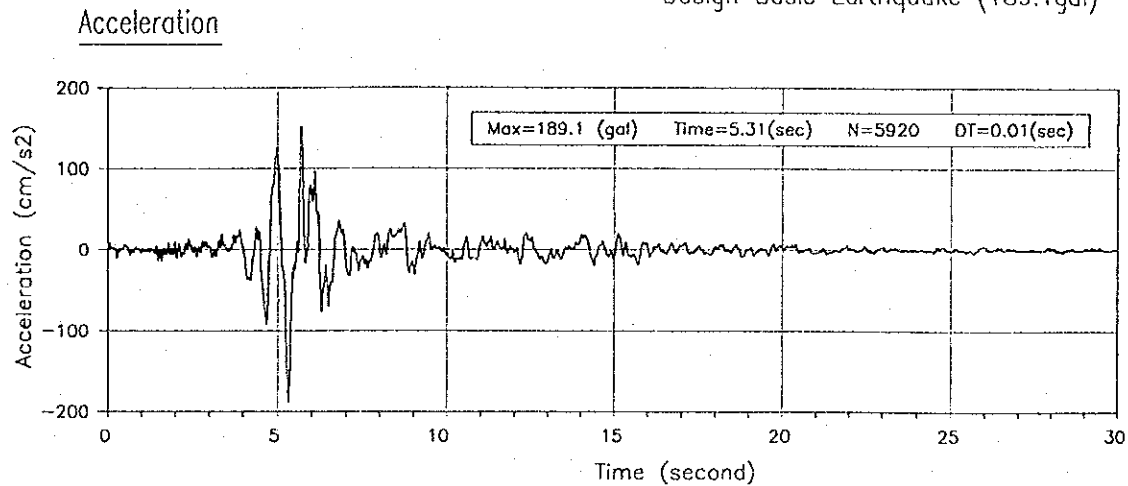
| Return Period<br>(year) | Maximum Acceleration<br>(gal) | Design Earthquake           |
|-------------------------|-------------------------------|-----------------------------|
| 100                     | 155.3                         |                             |
| 200                     | 189.1                         | Design Basis Earthquake     |
| 500                     | 233.9                         |                             |
| 1,000                   | 267.7                         |                             |
| 5,000                   | 346.2                         |                             |
| 10,000                  | 380.0                         | Maximum Credible Earthquake |

Fig. 7.3.10

**PROBABILISTIC RISK ANALYSIS OF EARTHQUAKE AT JATIBARANG DAMSITE**

# DYNAMIC ANALYSIS FOR DAM

Input Earthquake Motion  
Design Basis Earthquake (189.1gal)



THE DETAILED DESIGN OF FLOOD CONTROL, URBAN DRAINAGE AND WATER RESOURCES DEVELOPMENT IN SEMARANG IN THE REPUBLIC OF INDONESIA

JAPAN INTERNATIONAL COOPERATION AGENCY

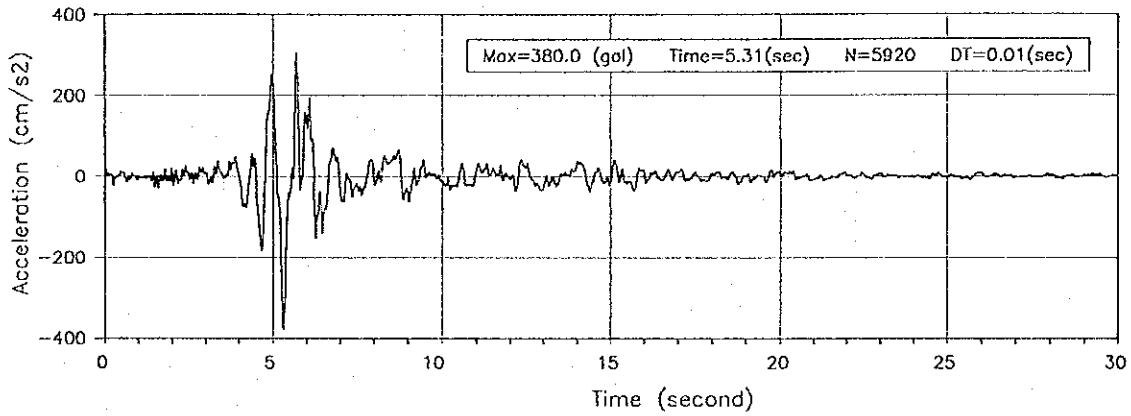
Fig. 7.3.11 (1/2)  
INPUT EARTHQUAKE MOTION OF DBE AND MCE

DYNAMIC ANALYSIS FOR DAM

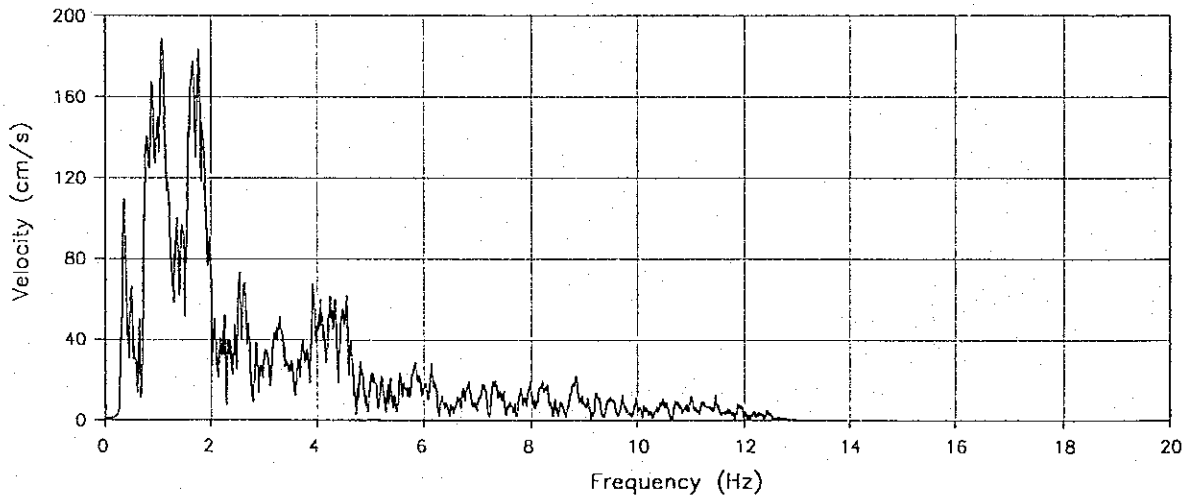
INPUT EARTHQUAKE MOTION

Maximum Credible Earthquake (380.0gal)

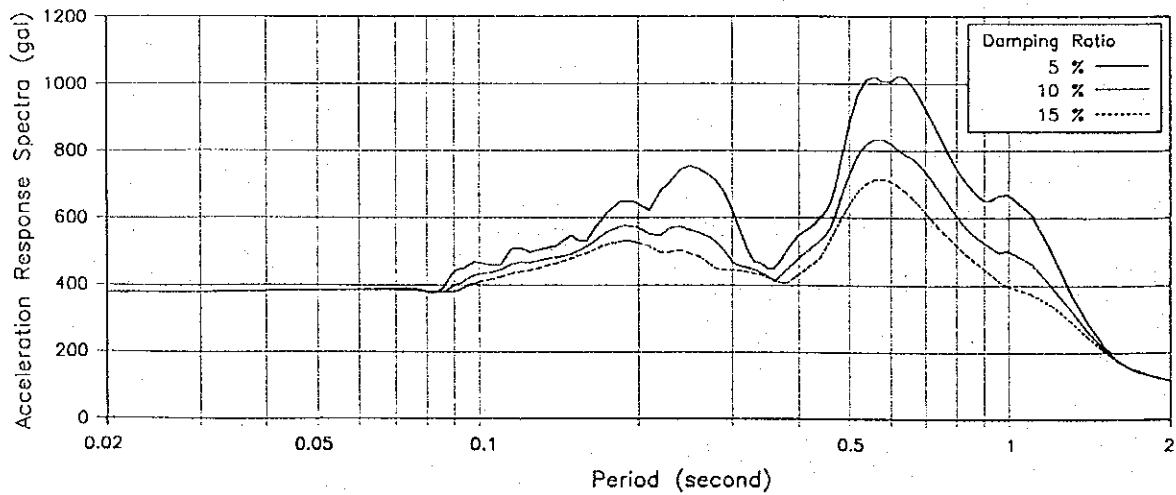
Acceleration



Fourier Spectra



Acceleration Response Spectra



THE DETAILED DESIGN OF FLOOD CONTROL, URBAN DRAINAGE AND WATER RESOURCES DEVELOPMENT IN SEMARANG IN THE REPUBLIC OF INDONESIA

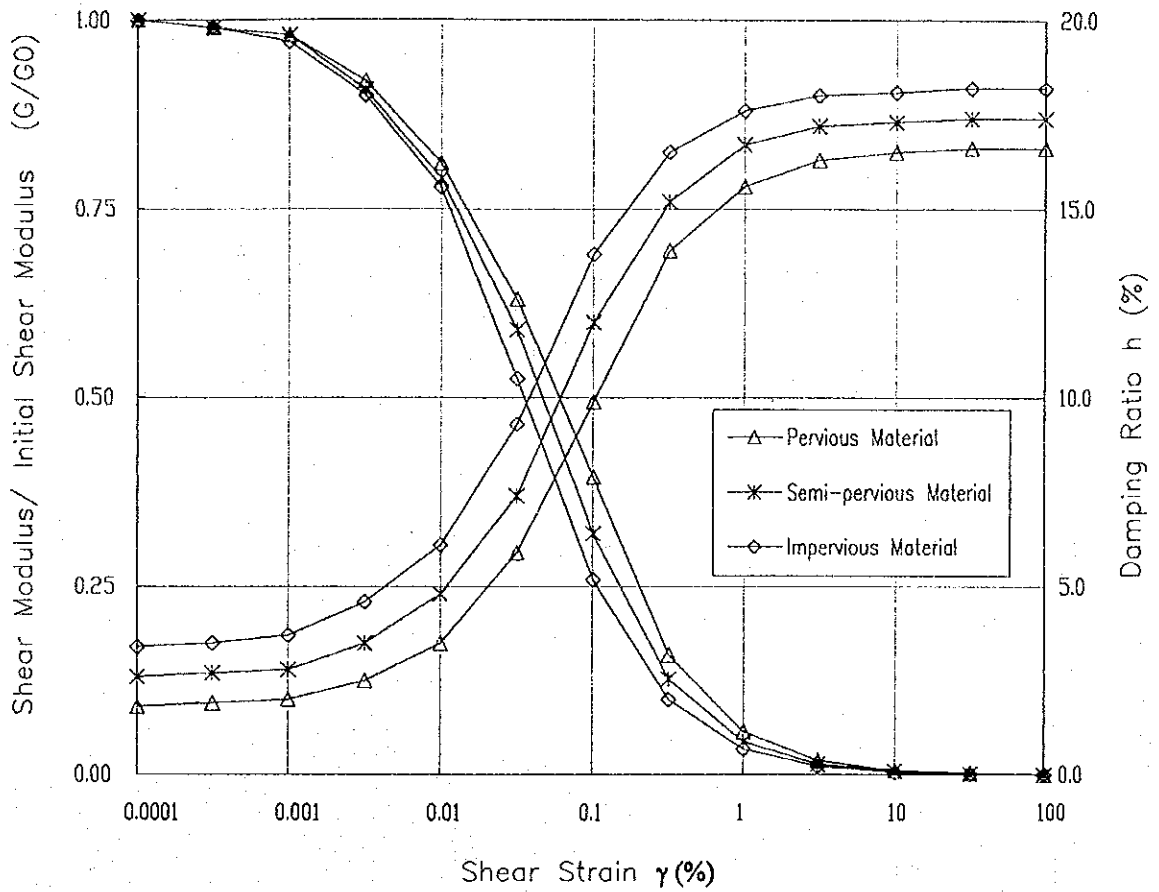
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Fig. 7.3.11 (2/2)

INPUT EARTHQUAKE MOTION OF DBE AND MCE

DYNAMIC ANALYSIS FOR DAM

Shear Strain Dependent Curve



Relationship between G/G<sub>0</sub>, h and shear Strain

| Shear Strain $\gamma$ (%) | Pervious Material |       | Semi-pervious Material |       | Impervious Material |       |
|---------------------------|-------------------|-------|------------------------|-------|---------------------|-------|
|                           | G/G <sub>0</sub>  | h (%) | G/G <sub>0</sub>       | h (%) | G/G <sub>0</sub>    | h (%) |
| $10^{-4}$                 | 1.000             | 1.8   | 1.000                  | 2.6   | 1.000               | 3.4   |
| $10^{-3.5}$               | 0.990             | 1.9   | 0.990                  | 2.7   | 0.990               | 3.5   |
| $10^{-3}$                 | 0.980             | 2.0   | 0.980                  | 2.8   | 0.972               | 3.7   |
| $10^{-2.5}$               | 0.920             | 2.5   | 0.908                  | 3.5   | 0.900               | 4.6   |
| $10^{-2}$                 | 0.810             | 3.5   | 0.790                  | 4.8   | 0.778               | 6.1   |
| $10^{-1.5}$               | 0.630             | 5.9   | 0.590                  | 7.4   | 0.525               | 9.3   |
| $10^{-1}$                 | 0.395             | 9.9   | 0.320                  | 12.0  | 0.259               | 13.8  |
| $10^{-0.5}$               | 0.159             | 13.9  | 0.127                  | 15.2  | 0.100               | 16.5  |
| $10^0$                    | 0.057             | 15.6  | 0.044                  | 16.7  | 0.034               | 17.6  |
| $10^{0.5}$                | 0.019             | 16.3  | 0.014                  | 17.2  | 0.011               | 18.0  |
| $10^1$                    | 0.006             | 16.5  | 0.005                  | 17.3  | 0.003               | 18.1  |
| $10^{1.5}$                | 0.002             | 16.6  | 0.001                  | 17.4  | 0.001               | 18.2  |
| $10^2$                    | 0.000             | 16.6  | 0.000                  | 17.4  | 0.000               | 18.2  |

THE DETAILED DESIGN OF FLOOD CONTROL, URBAN DRAINAGE AND WATER RESOURCES DEVELOPMENT IN SEMARANG IN THE REPUBLIC OF INDONESIA

JAPAN INTERNATIONAL COOPERATION AGENCY

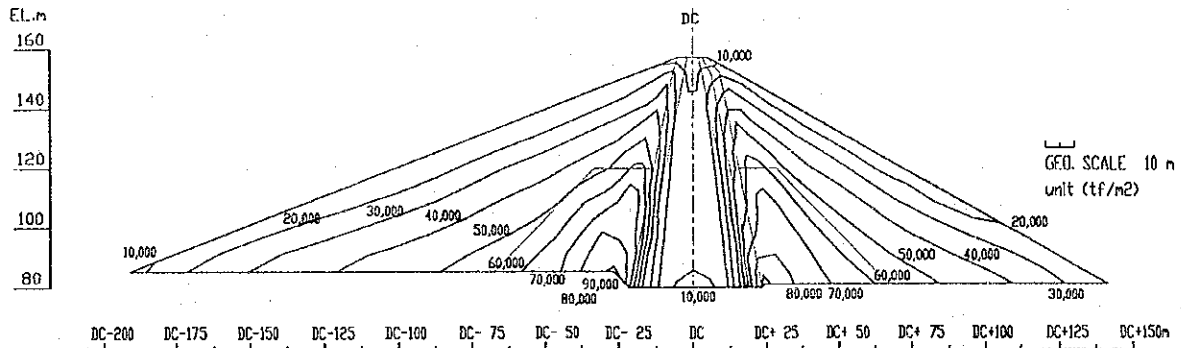
Fig. 7.3.12

STRAIN DEPENDENT SHEAR MODULUS AND DAMPING RATIO

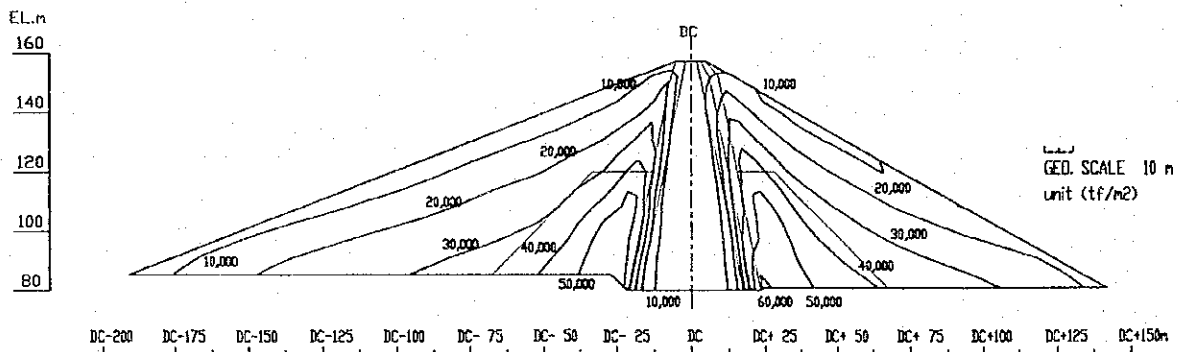
# DYNAMIC ANALYSIS FOR DAM

## Shear Modulus Contour

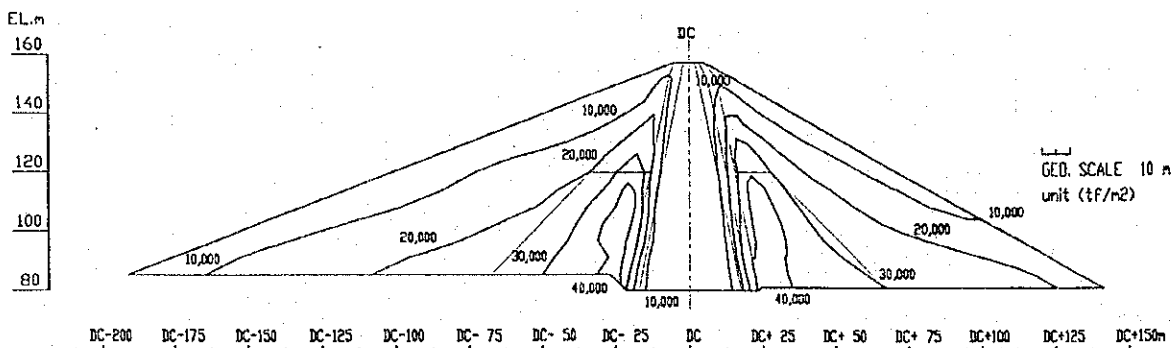
### Initial Shear Modulus



### Final Shear Modulus after Design Basis Earthquake (189.1 gal)



### Final Shear Modulus after Maximum Credible Earthquake (380.0 gal)



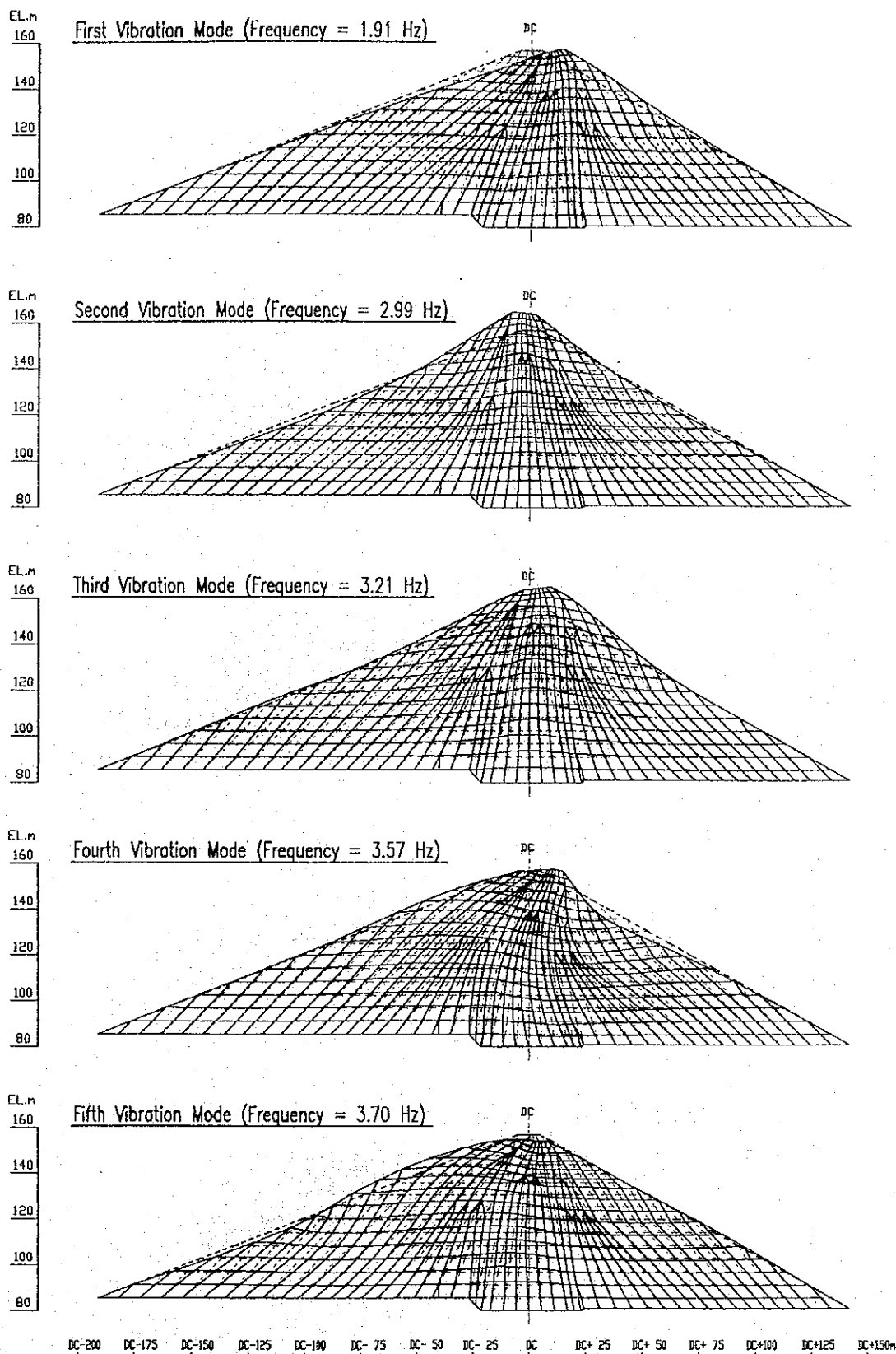
THE DETAILED DESIGN OF FLOOD CONTROL, URBAN DRAINAGE AND WATER RESOURCES DEVELOPMENT IN SEMARANG IN THE REPUBLIC OF INDONESIA

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Fig. 7.3.13  
RESULTS OF DYNAMIC ANALYSIS (SHEAR MODULUS CONTOUR)

# DYNAMIC ANALYSIS FOR DAM

Vibration Mode Shape  
subjected to Initial Shear Modulus



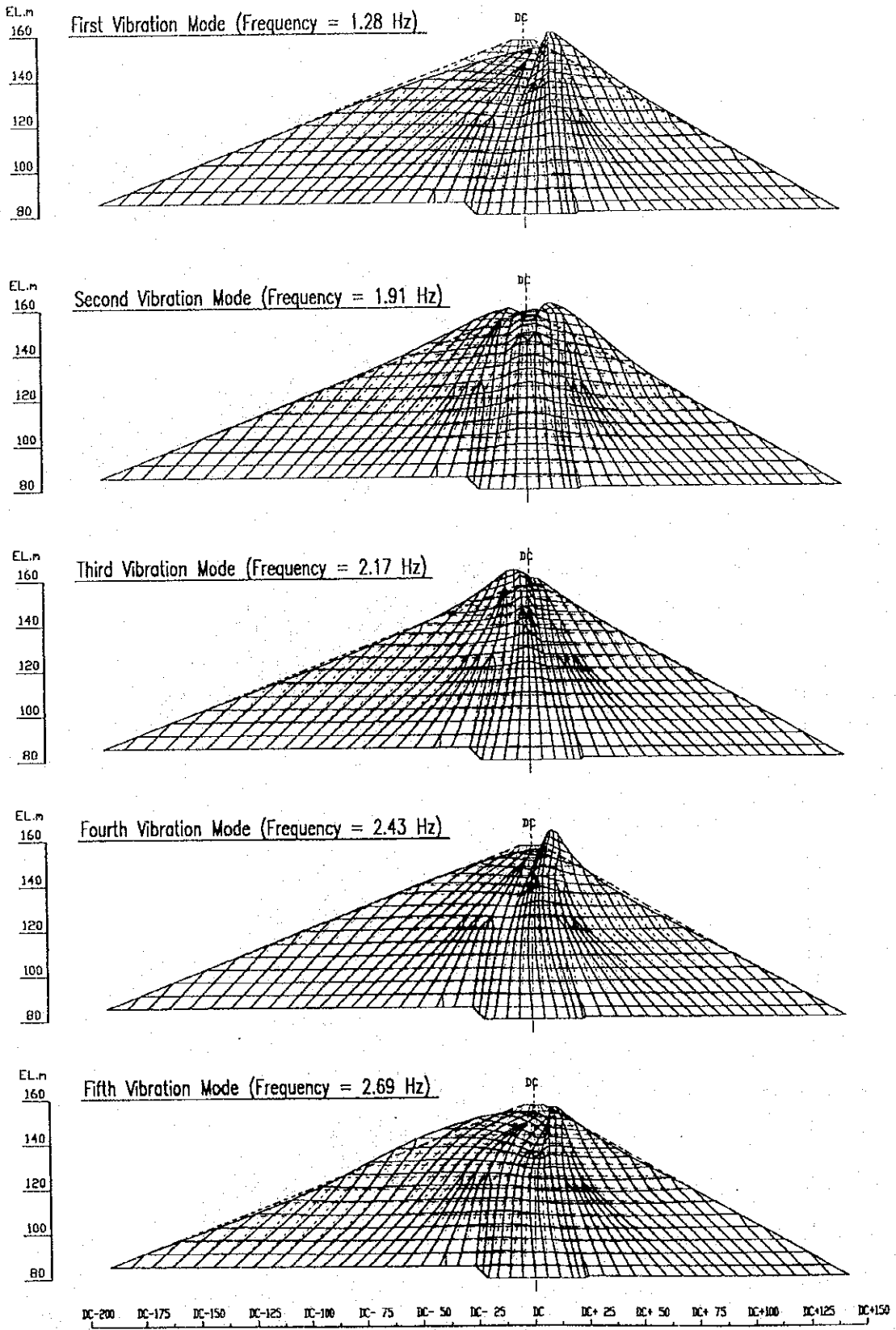
THE DETAILED DESIGN OF FLOOD CONTROL, URBAN DRAINAGE AND WATER RESOURCES DEVELOPMENT IN SEMARANG IN THE REPUBLIC OF INDONESIA

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Fig. 7.3.14 (1/3)  
**RESULTS OF DYNAMIC ANALYSIS  
(VIBRATION MODE SHAPE)**

DYNAMIC ANALYSIS FOR DAM

Vibration Mode Shape  
after Design Basis Earthquake (189.1gal)



THE DETAILED DESIGN OF FLOOD CONTROL, URBAN DRAINAGE AND WATER RESOURCES DEVELOPMENT IN SEMARANG IN THE REPUBLIC OF INDONESIA

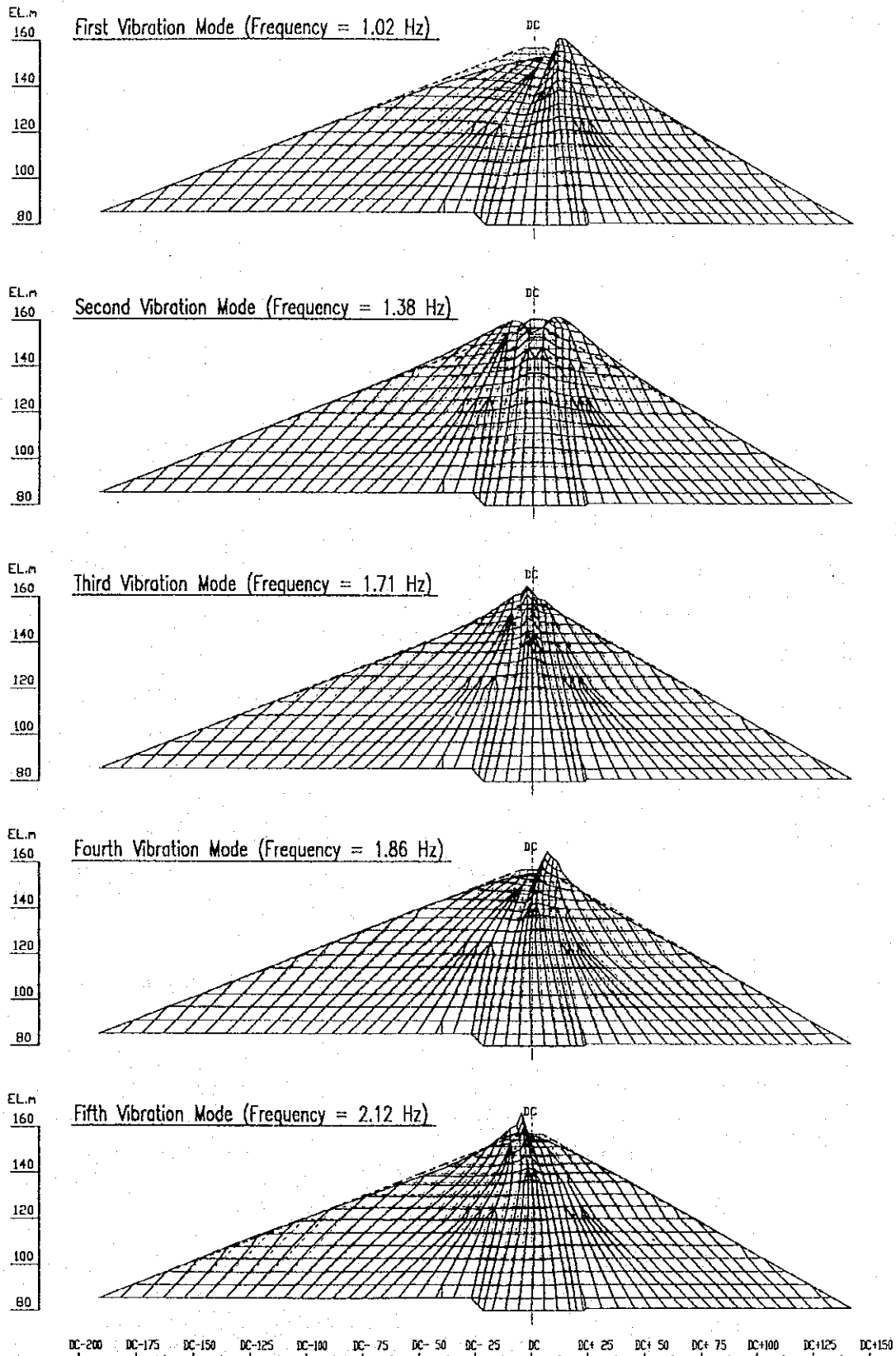
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Fig. 7.3.14 (2/3)  
RESULTS OF DYNAMIC ANALYSIS  
(VIBRATION MODE SHAPE)



DYNAMIC ANALYSIS FOR DAM

Vibration Mode Shape  
after Maximum Credible Earthquake (380.0gal)



THE DETAILED DESIGN OF FLOOD CONTROL, URBAN DRAINAGE AND WATER RESOURCES DEVELOPMENT IN SEMARANG IN THE REPUBLIC OF INDONESIA

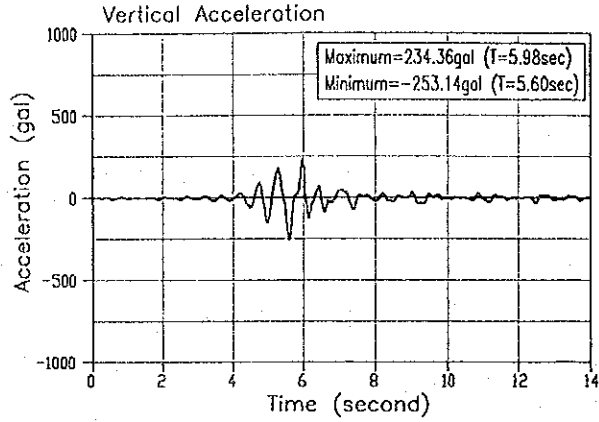
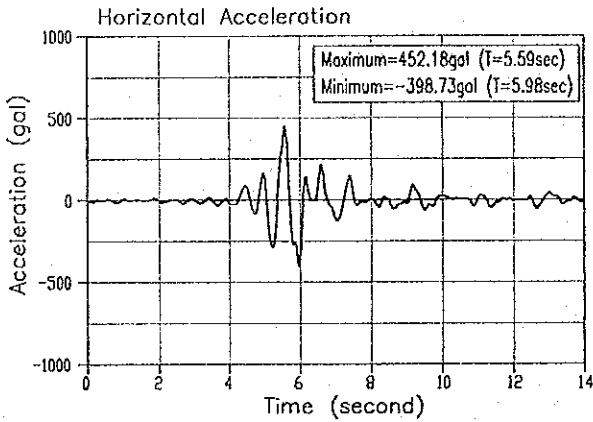
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Fig. 7.3.14 (3/3)  
RESULTS OF DYNAMIC ANALYSIS  
(VIBRATION MODE SHAPE)

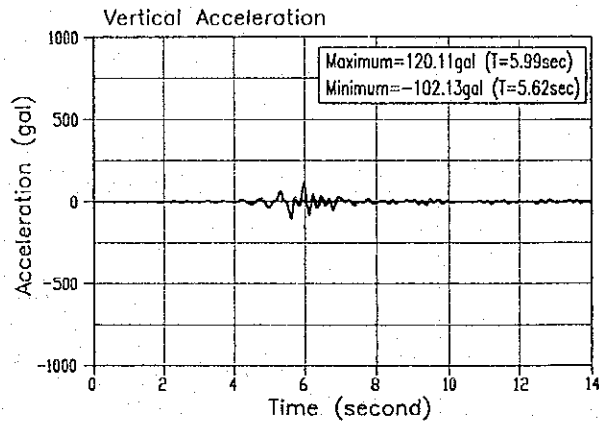
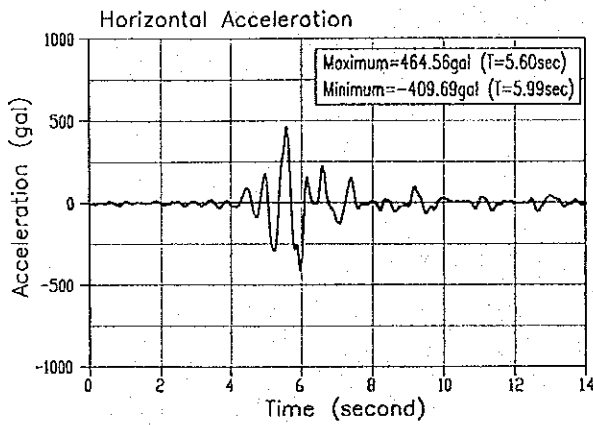
# DYNAMIC ANALYSIS FOR DAM

Crest Acceleration  
Design Basis Earthquake (189.1gal)

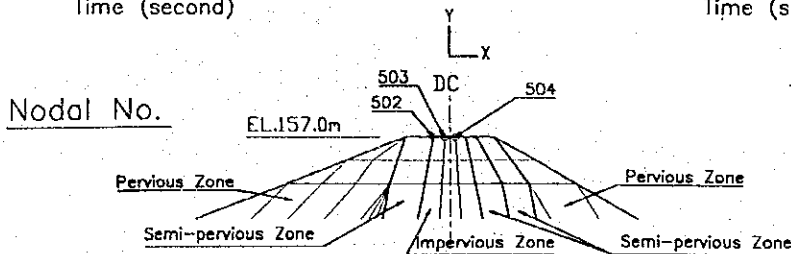
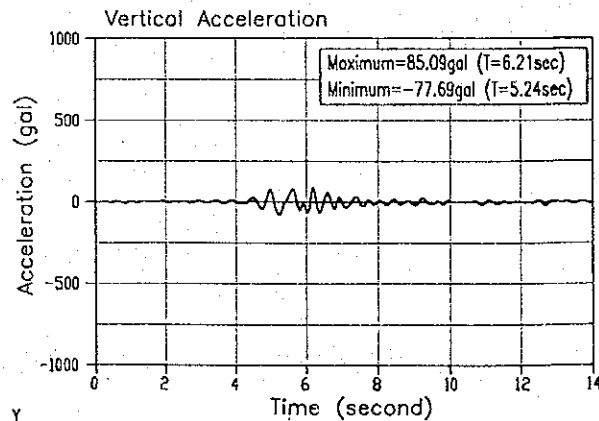
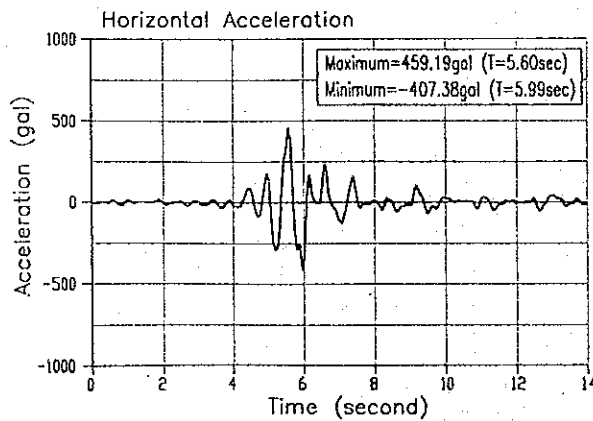
## Nodal No.502



## Nodal No.503



## Nodal No.504



THE DETAILED DESIGN OF FLOOD CONTROL, URBAN DRAINAGE AND WATER RESOURCES DEVELOPMENT IN SEMARANG IN THE REPUBLIC OF INDONESIA

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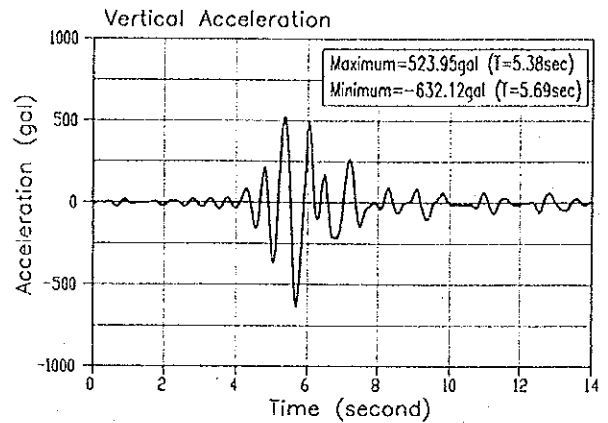
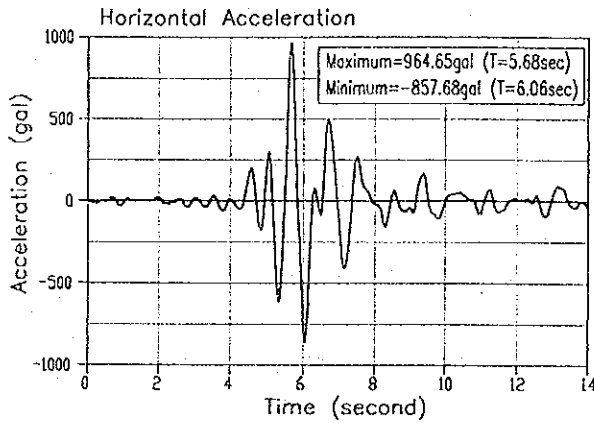
Fig. 7.3.15 (1/4)  
**RESULTS OF DYNAMIC ANALYSIS  
(MAXIMUM ACCELERATION AND  
DISPLACEMENT AT CREST)**

# DYNAMIC ANALYSIS FOR DAM

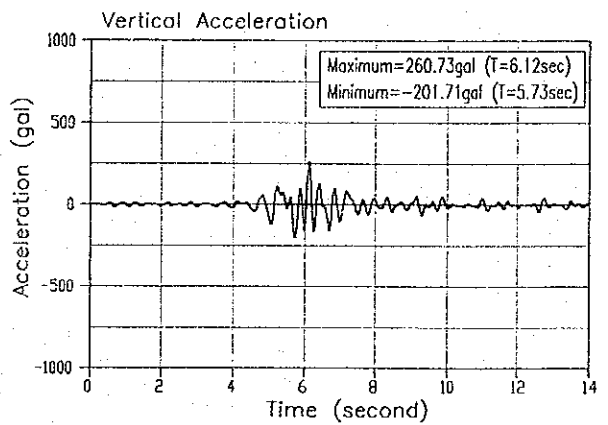
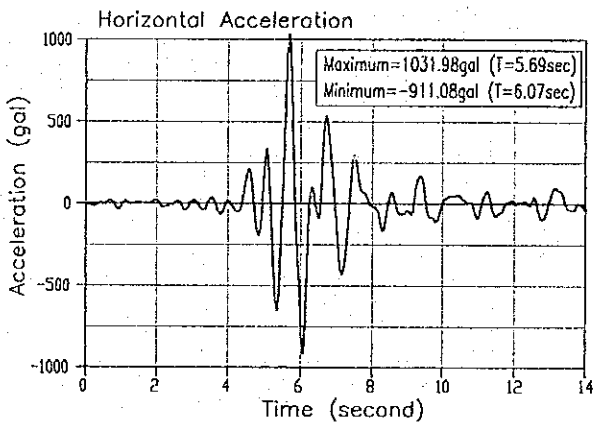
Crest Acceleration

Maximum Credible Earthquake (380.0gal)

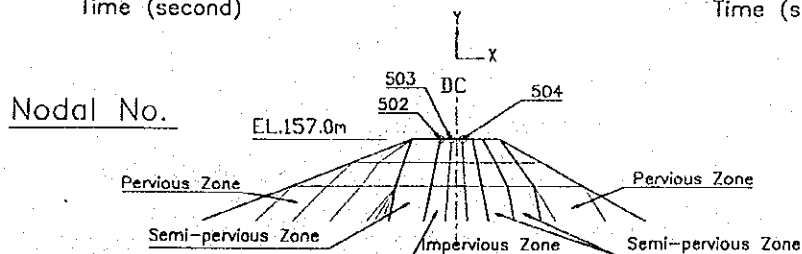
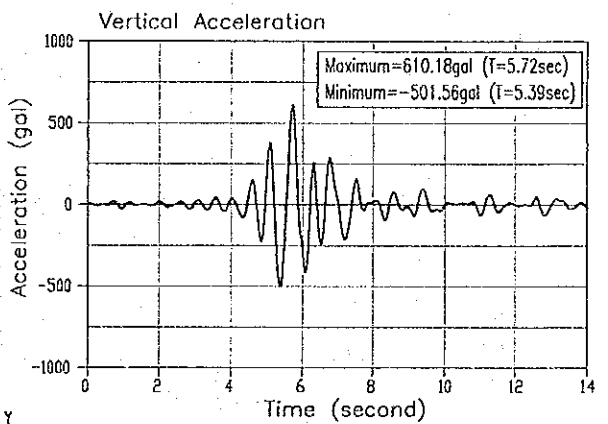
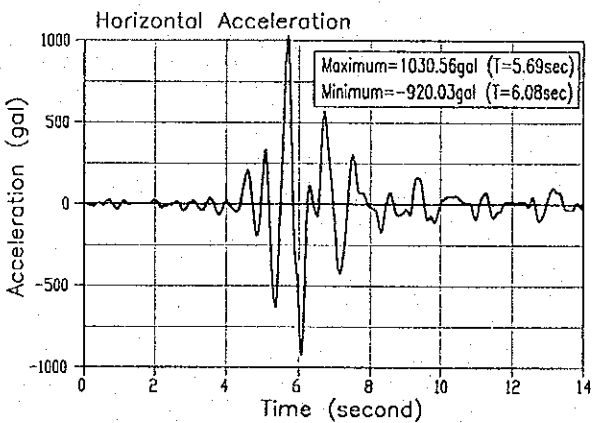
Nodal No.502



Nodal No.503



Nodal No.504



THE DETAILED DESIGN OF FLOOD CONTROL, URBAN DRAINAGE AND WATER RESOURCES DEVELOPMENT IN SEMARANG IN THE REPUBLIC OF INDONESIA

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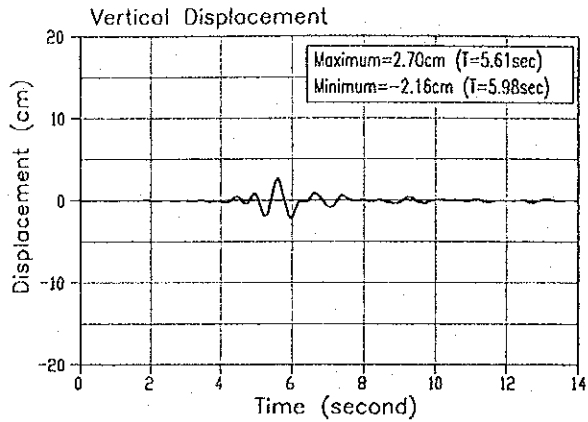
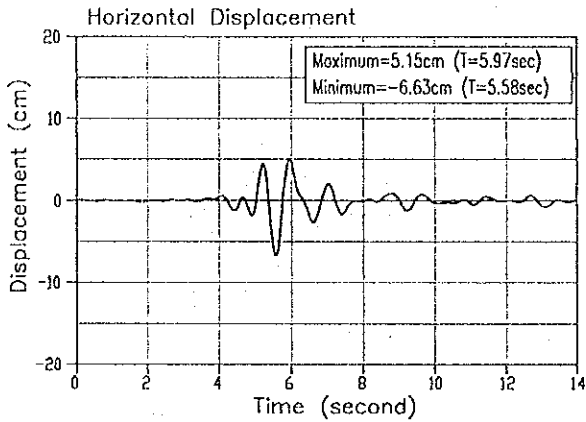
Fig.

**7.3.15 (2/4)**  
**RESULTS OF DYNAMIC ANALYSIS**  
**(MAXIMUM ACCELERATION AND**  
**DISPLACEMENT AT CREST)**

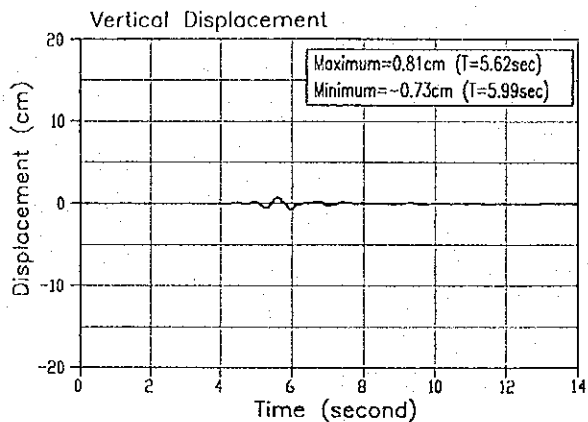
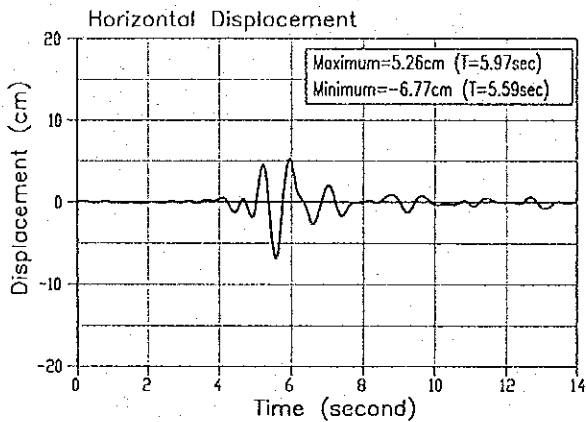
# DYNAMIC ANALYSIS FOR DAM

Crest Displacement  
Design Basis Earthquake (189.1gal)

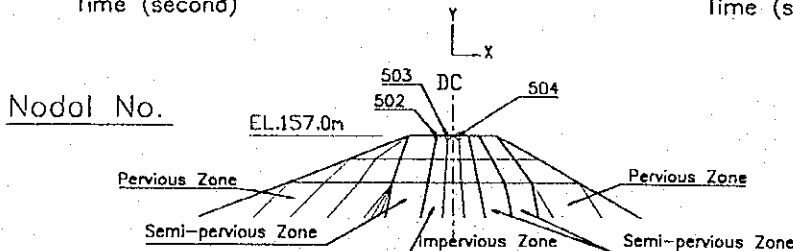
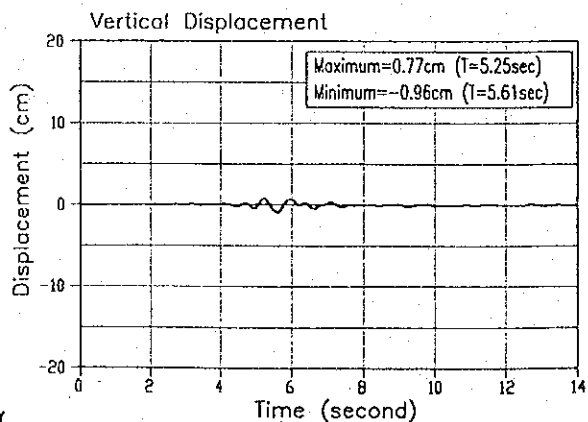
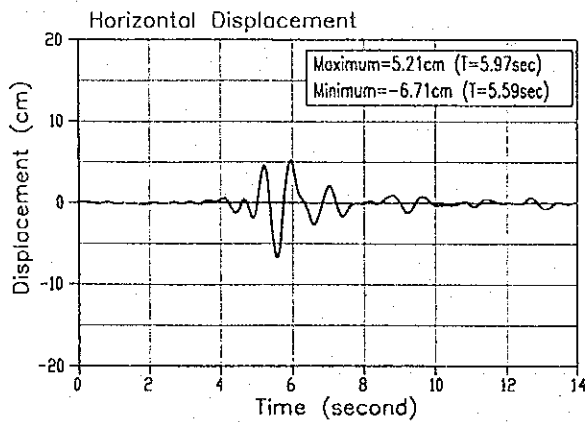
## Nodal No.502



## Nodal No.503



## Nodal No.504



THE DETAILED DESIGN OF FLOOD CONTROL, URBAN DRAINAGE AND WATER RESOURCES DEVELOPMENT IN SEMARANG IN THE REPUBLIC OF INDONESIA

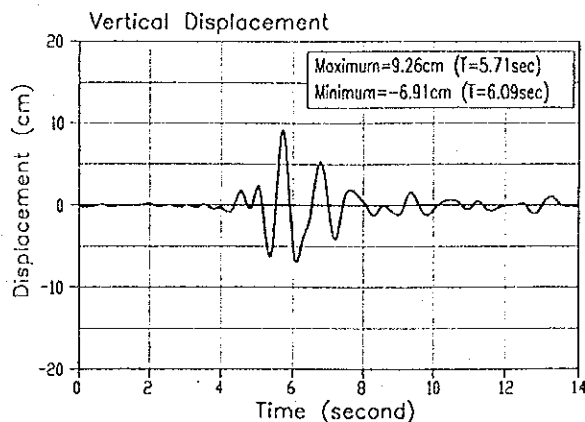
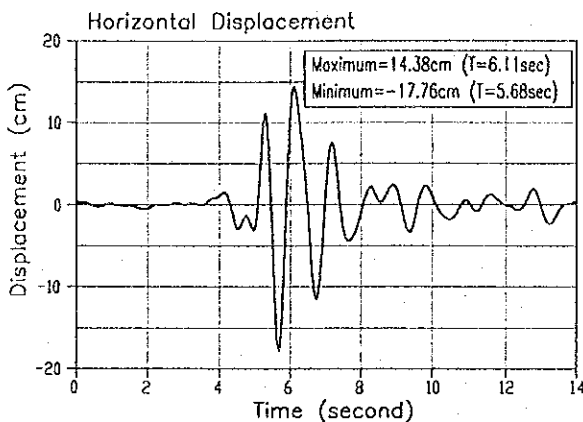
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Fig. 7.3.15 (3/4)  
**RESULTS OF DYNAMIC ANALYSIS  
(MAXIMUM ACCELERATION AND  
DISPLACEMENT AT CREST)**

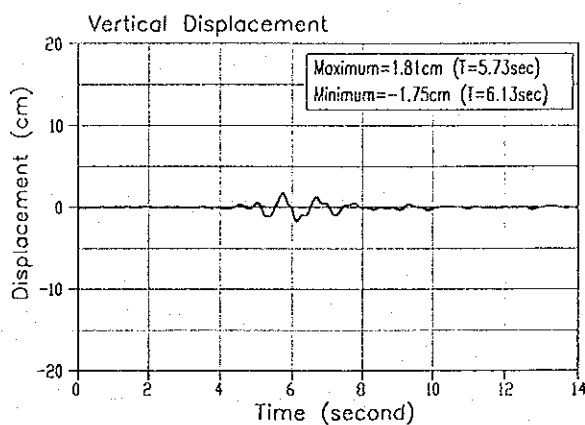
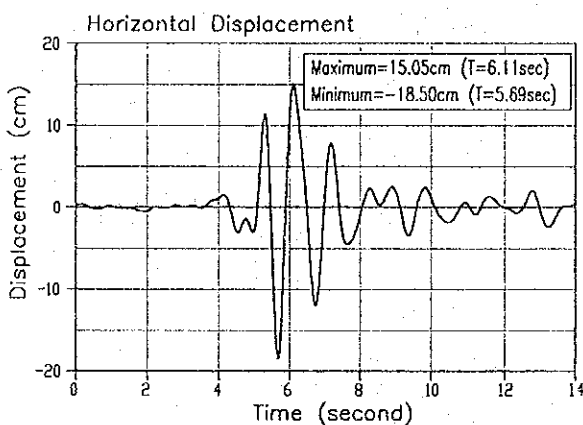
# DYNAMIC ANALYSIS FOR DAM

Crest Displacement  
Maximum Credible Earthquake (380.0gal)

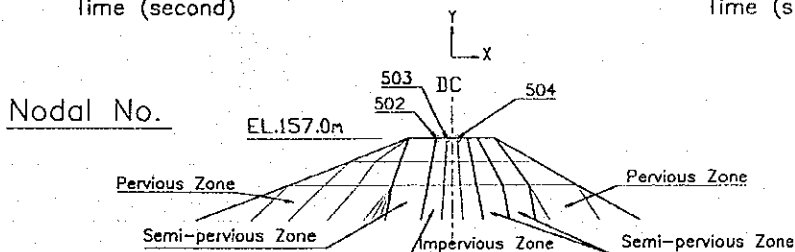
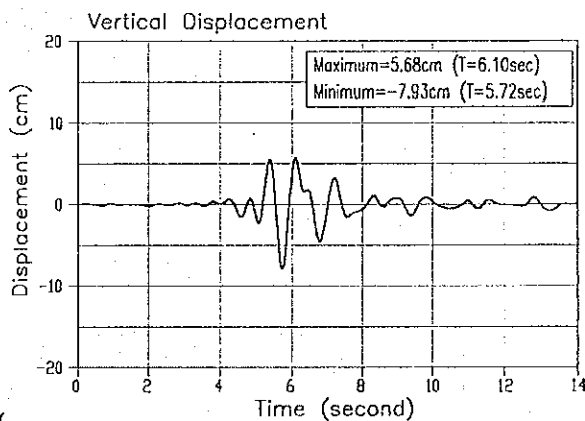
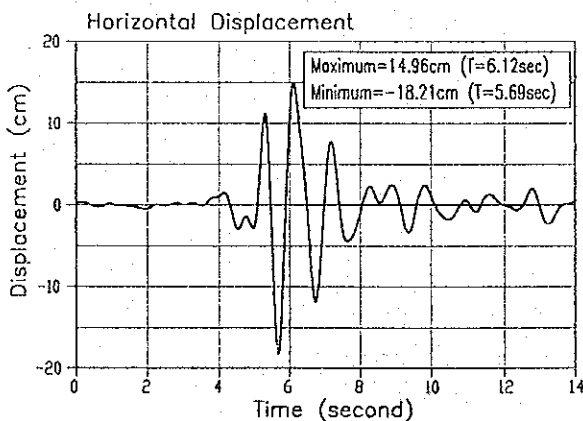
Nodal No.502



Nodal No.503



Nodal No.504



THE DETAILED DESIGN OF FLOOD CONTROL, URBAN DRAINAGE AND WATER RESOURCES DEVELOPMENT IN SEMARANG IN THE REPUBLIC OF INDONESIA

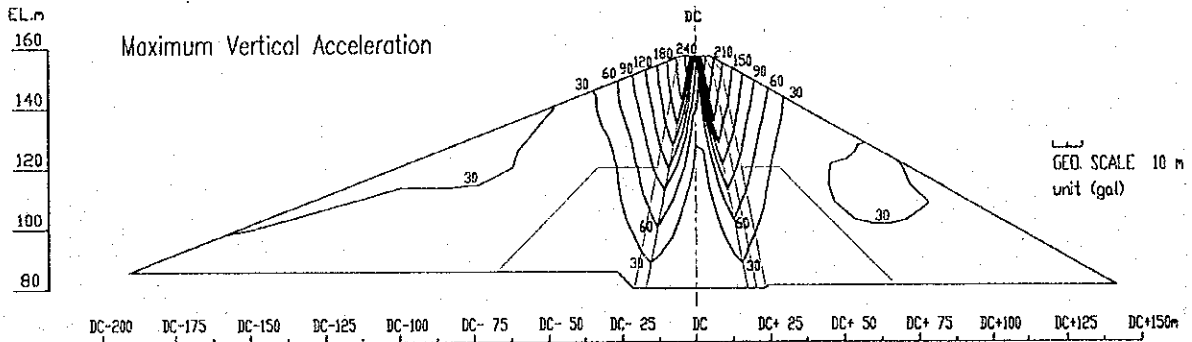
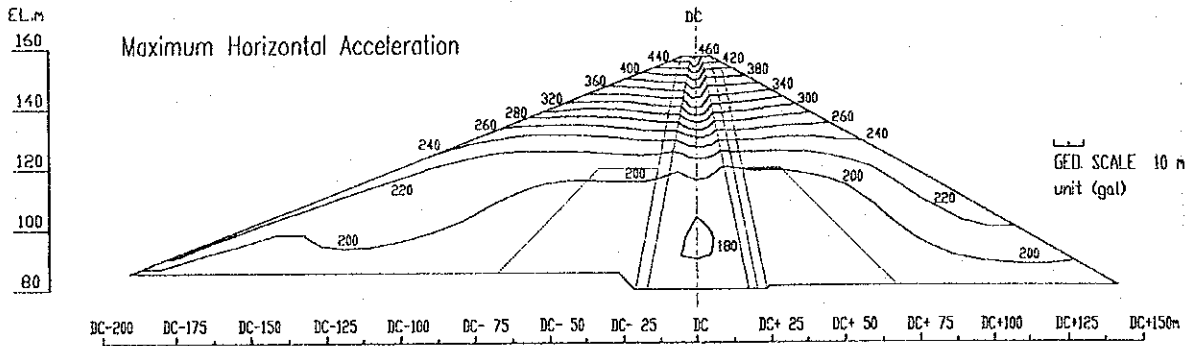
JAPAN INTERNATIONAL COOPERATION AGENCY

Fig. 7.3.15 (4/4)  
**RESULTS OF DYNAMIC ANALYSIS  
(MAXIMUM ACCELERATION AND  
DISPLACEMENT AT CREST)**

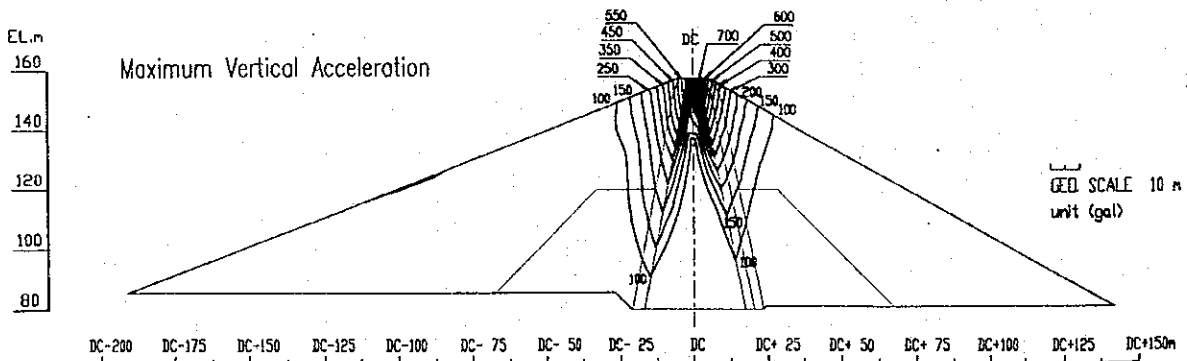
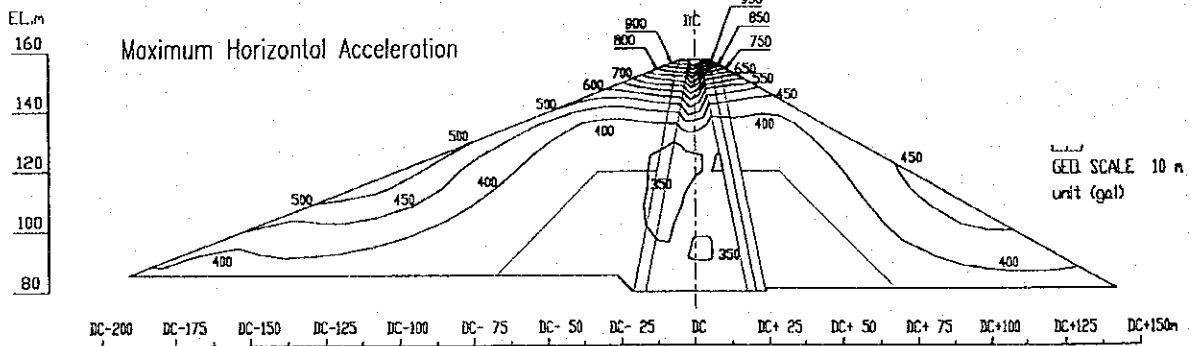
# DYNAMIC ANALYSIS FOR DAM

## Maximum Acceleration Contour

### Design Basis Earthquake (189.1 gal)



### Maximum Credible Earthquake (380.0 gal)



THE DETAILED DESIGN OF FLOOD CONTROL, URBAN DRAINAGE AND WATER RESOURCES DEVELOPMENT IN SEMARANG IN THE REPUBLIC OF INDONESIA

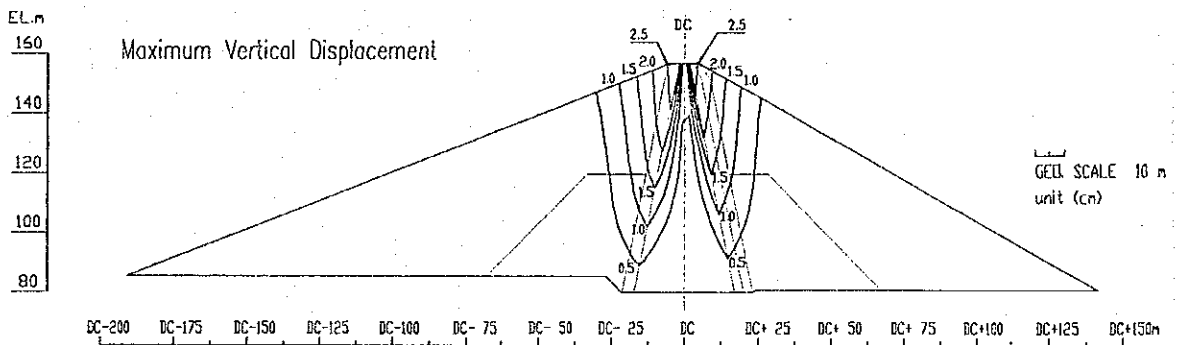
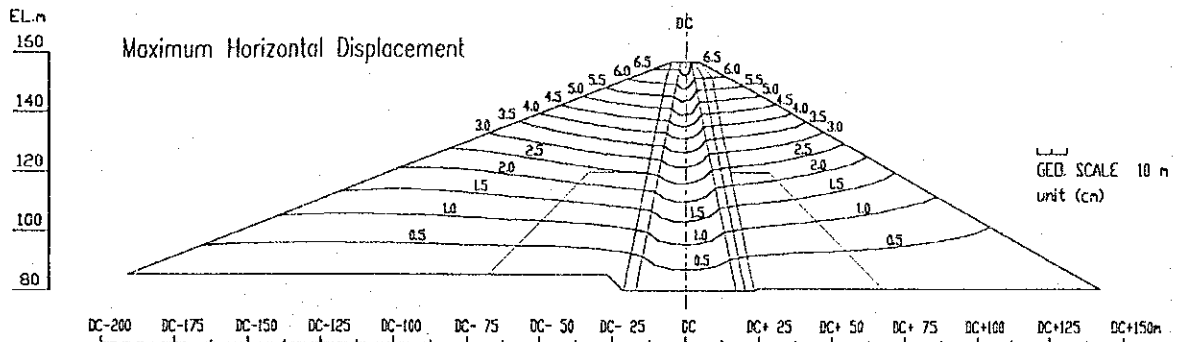
JAPAN INTERNATIONAL COOPERATION AGENCY

Fig. 7.3.16 (1/2)  
RESULTS OF DYNAMIC ANALYSIS  
(MAXIMUM ACCELERATION AND  
DISPLACEMENT CONTOUR)

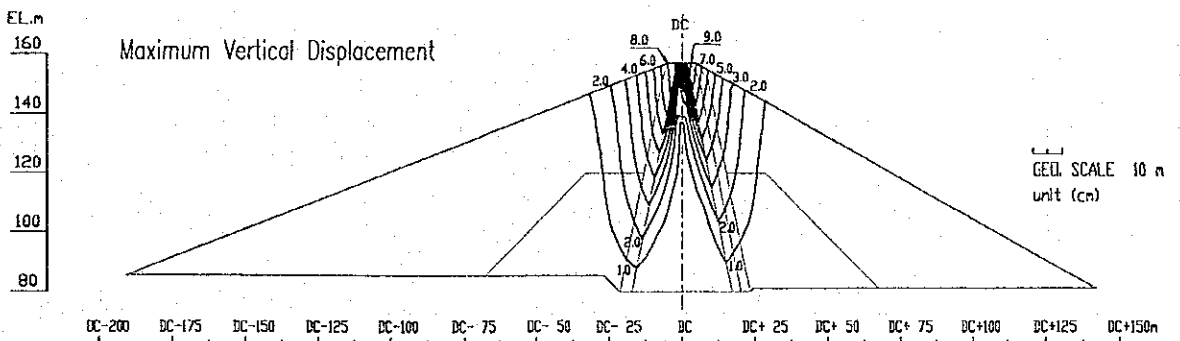
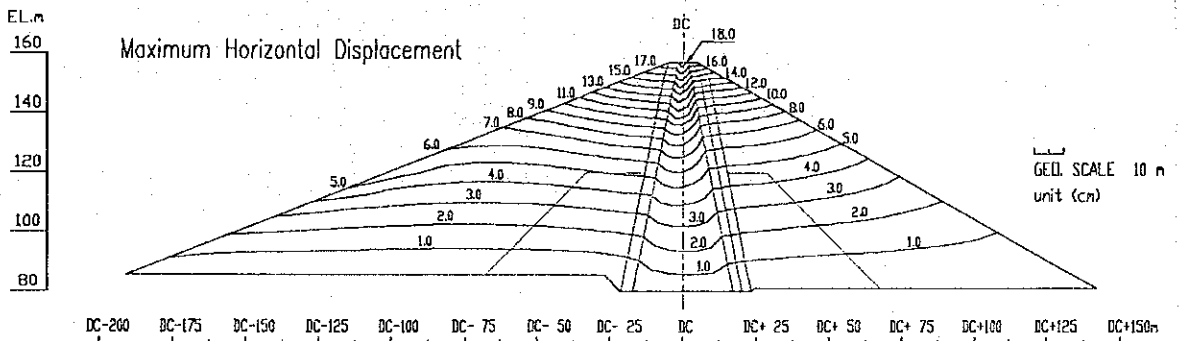
# DYNAMIC ANALYSIS FOR DAM

## Maximum Displacement Contour

### Design Basis Earthquake (189.1 gal)



### Maximum Credible Earthquake (380.0 gal)



THE DETAILED DESIGN OF FLOOD CONTROL, URBAN DRAINAGE AND WATER RESOURCES DEVELOPMENT IN SEMARANG IN THE REPUBLIC OF INDONESIA

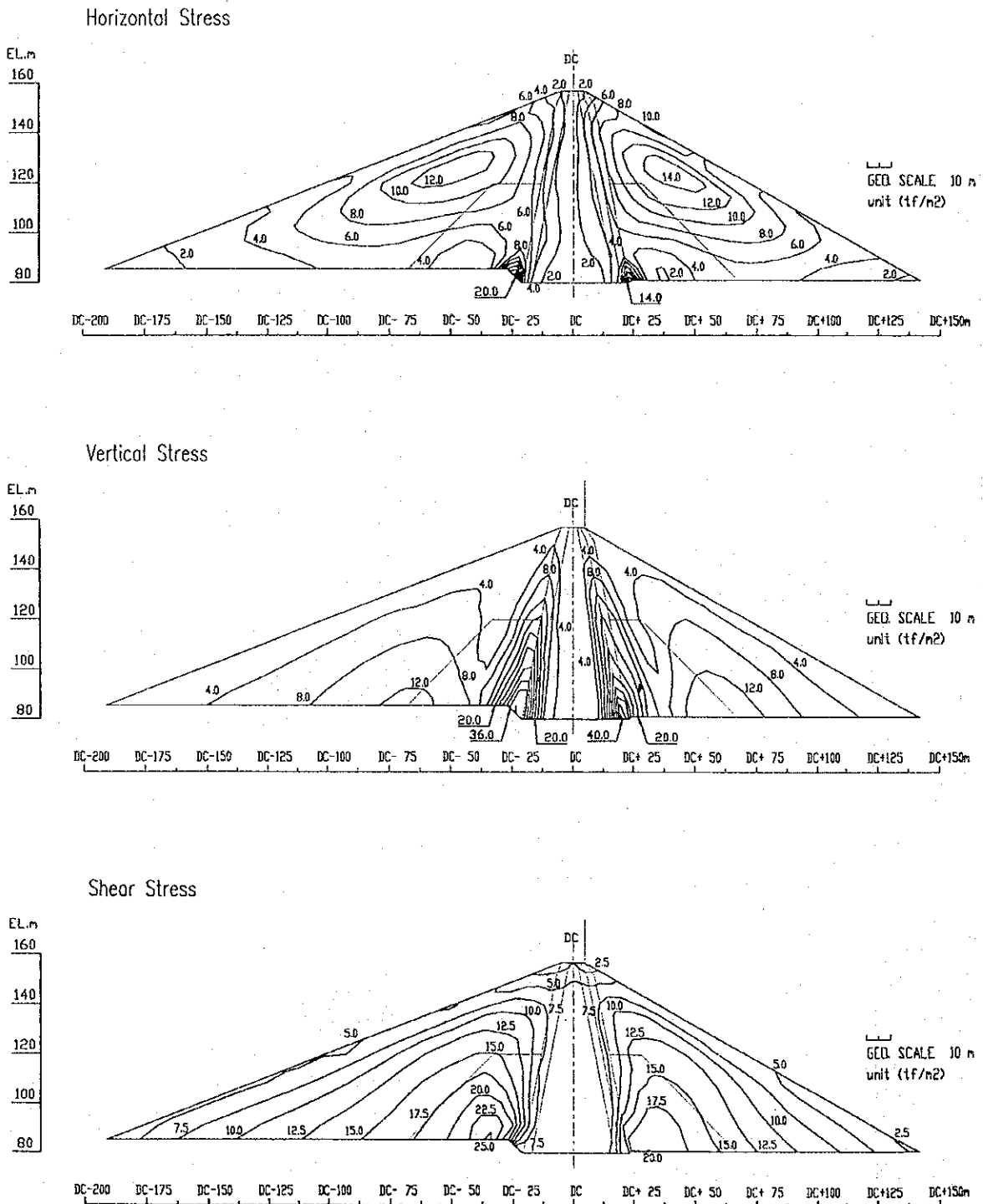
JAPAN INTERNATIONAL COOPERATION AGENCY

Fig. 7.3.16 (2/2)  
**RESULTS OF DYNAMIC ANALYSIS  
(MAXIMUM ACCELERATION AND  
DISPLACEMENT CONTOUR)**

# DYNAMIC ANALYSIS FOR DAM

## Stress Contour

Design Basis Earthquake (189.1 gal)



THE DETAILED DESIGN OF FLOOD CONTROL, URBAN DRAINAGE AND WATER RESOURCES DEVELOPMENT IN SEMARANG IN THE REPUBLIC OF INDONESIA

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Fig. 7.3.17 (1/2)

**RESULTS OF DYNAMIC ANALYSIS (DYNAMIC STRESS CONTOUR)**