

***FIGURES***

***CHAPTER 6***

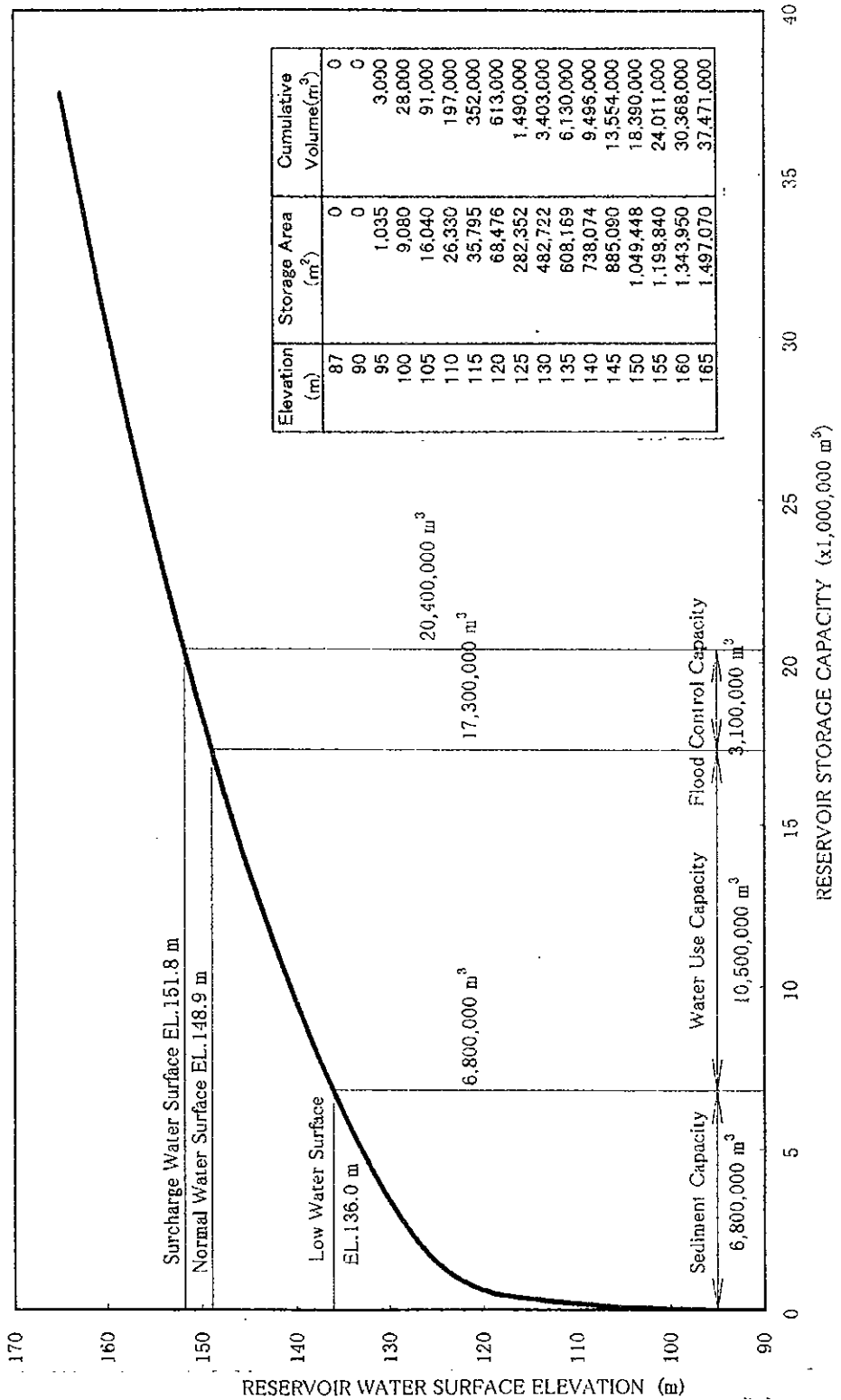
***FORMULATION OF DEFINITIVE PLAN***

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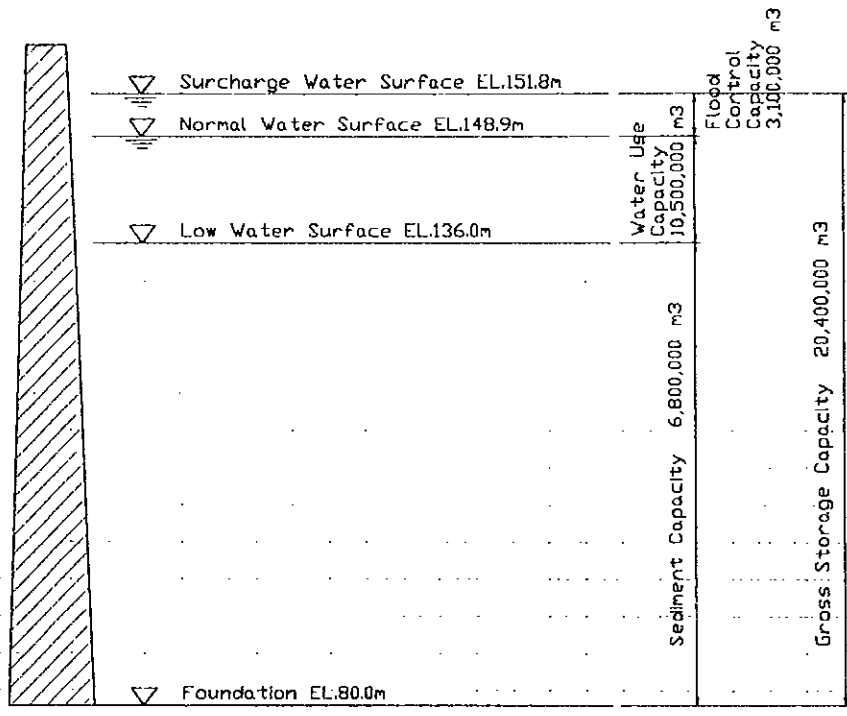
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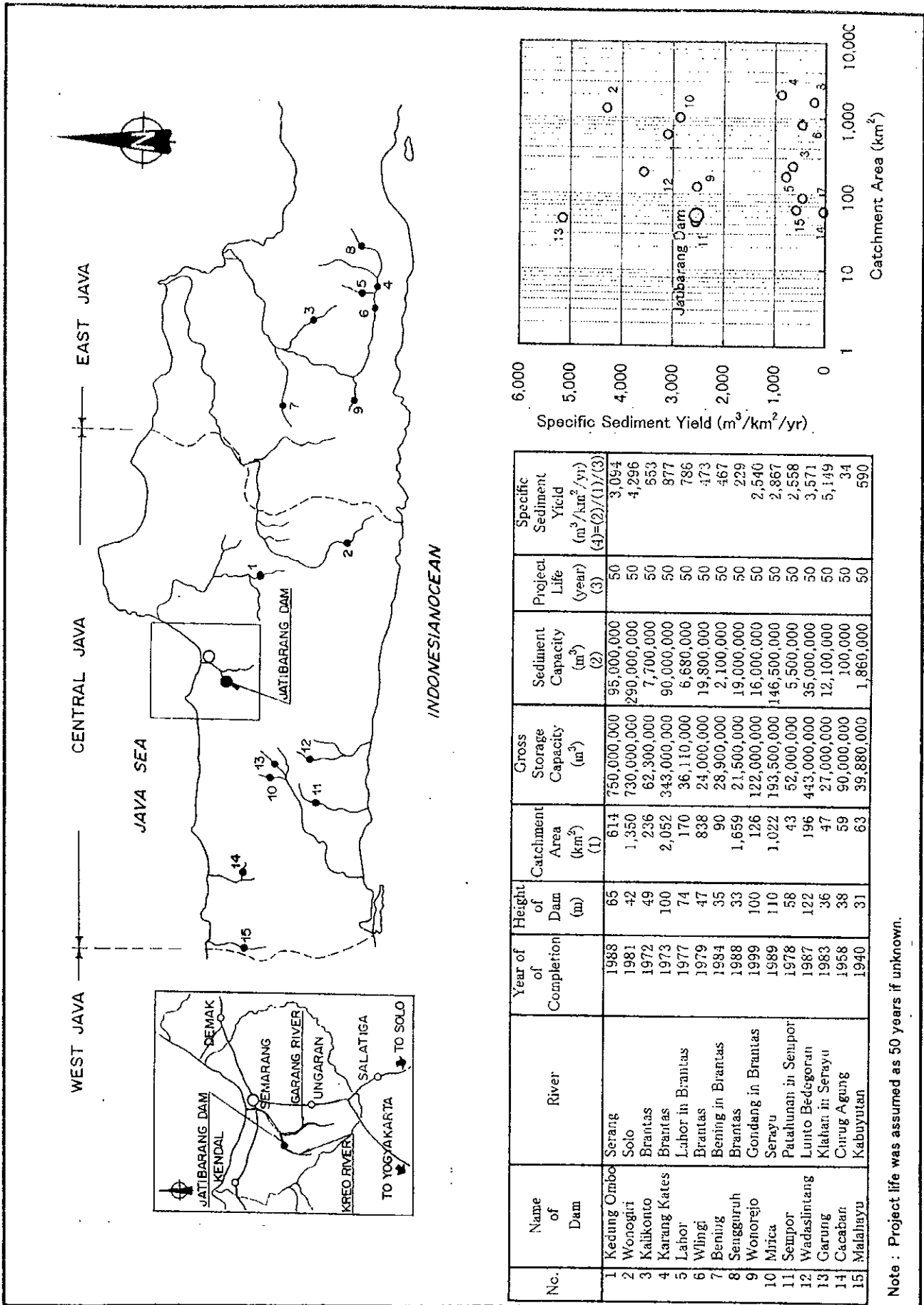


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Fig. 6.2.1 STORAGE-CAPACITY CURVE OF JATIBARANG DAM

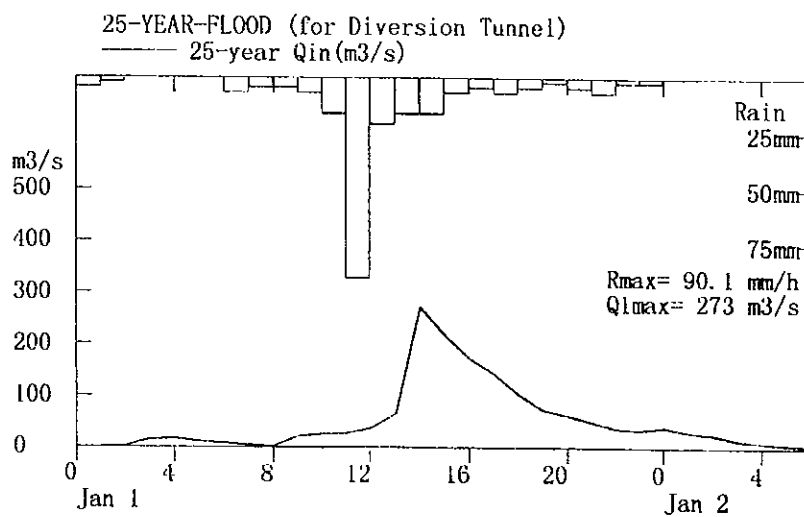
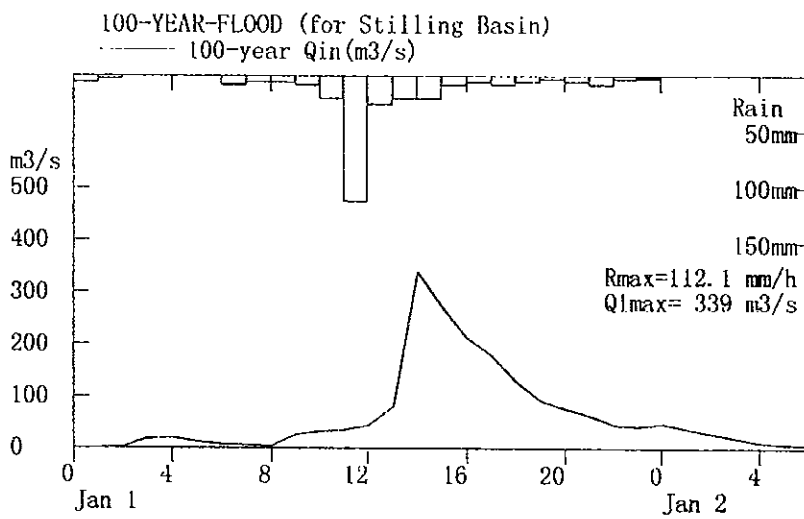
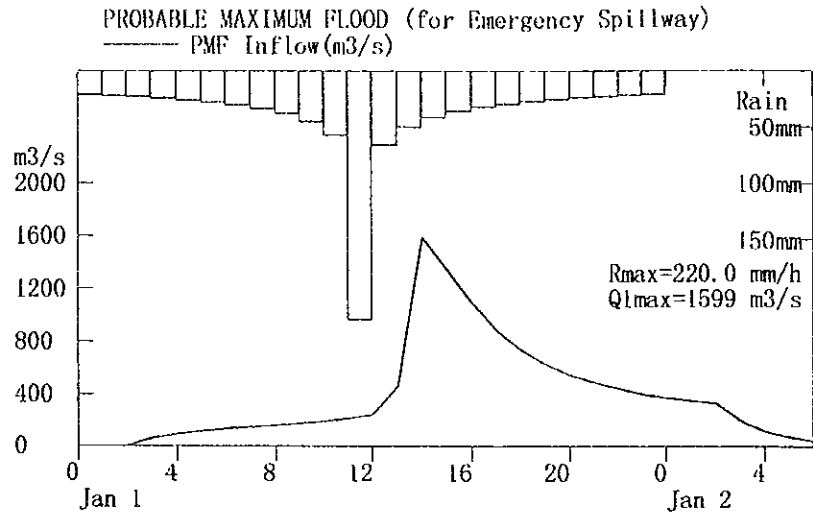




Note : Project life was assumed as 50 years if unknown.

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Fig. 6.2.3  
 SPECIFIC SEDIMENT YIELD ADOPTED IN NEARBY DAM RESERVOIR



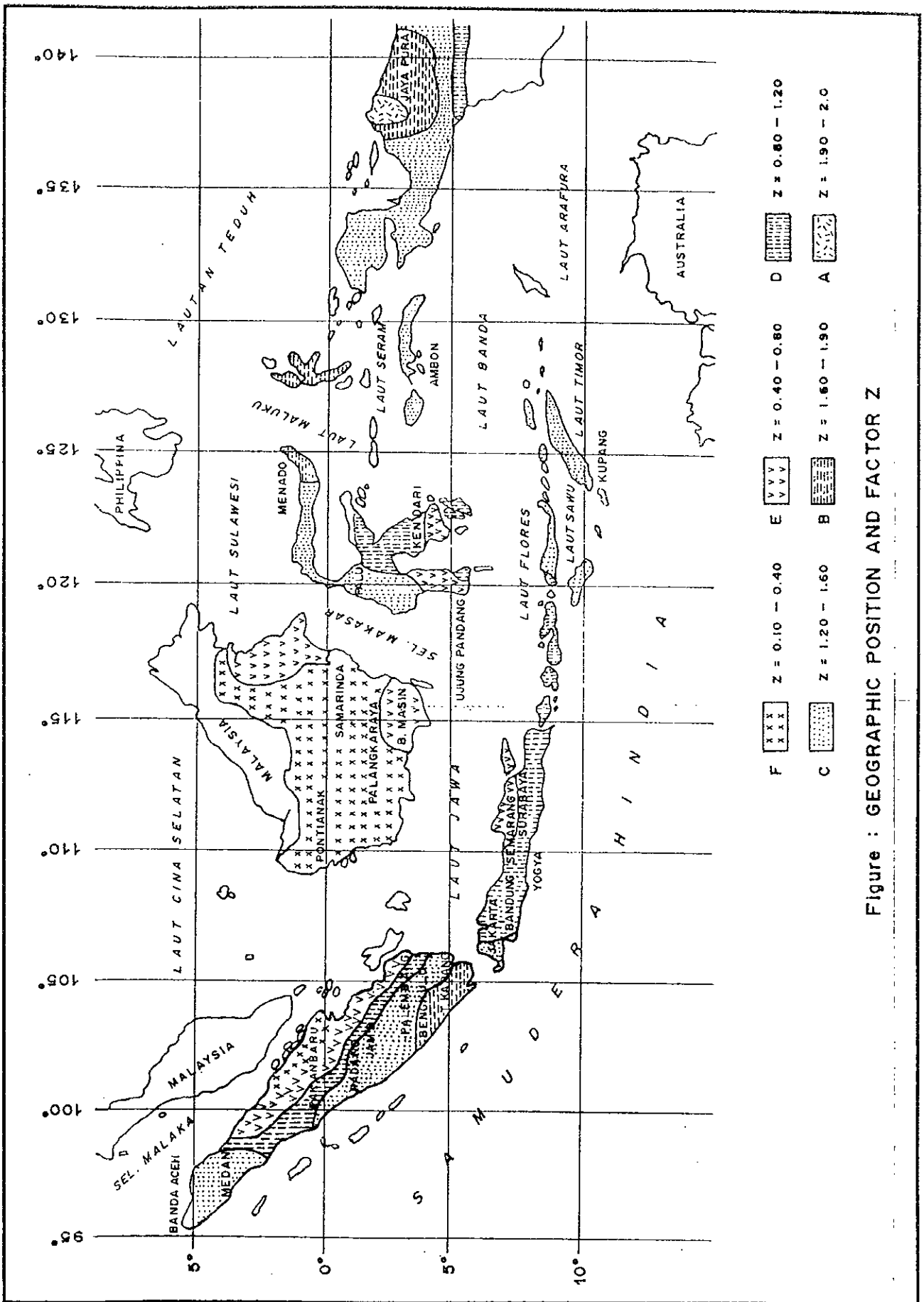


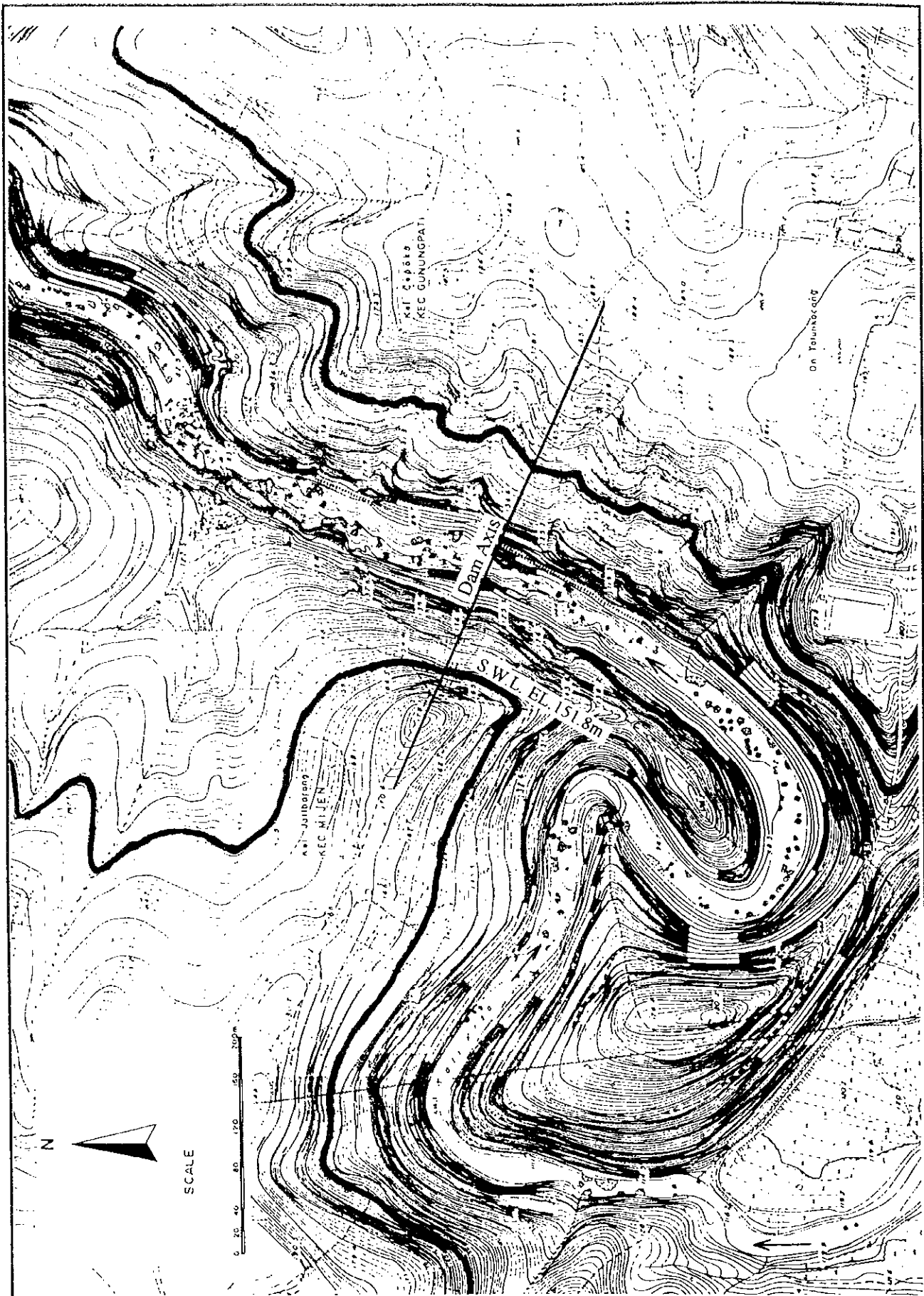
Figure : GEOGRAPHIC POSITION AND FACTOR Z

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Fig. 6.2.5 EARTHQUAKE ZONE MAP

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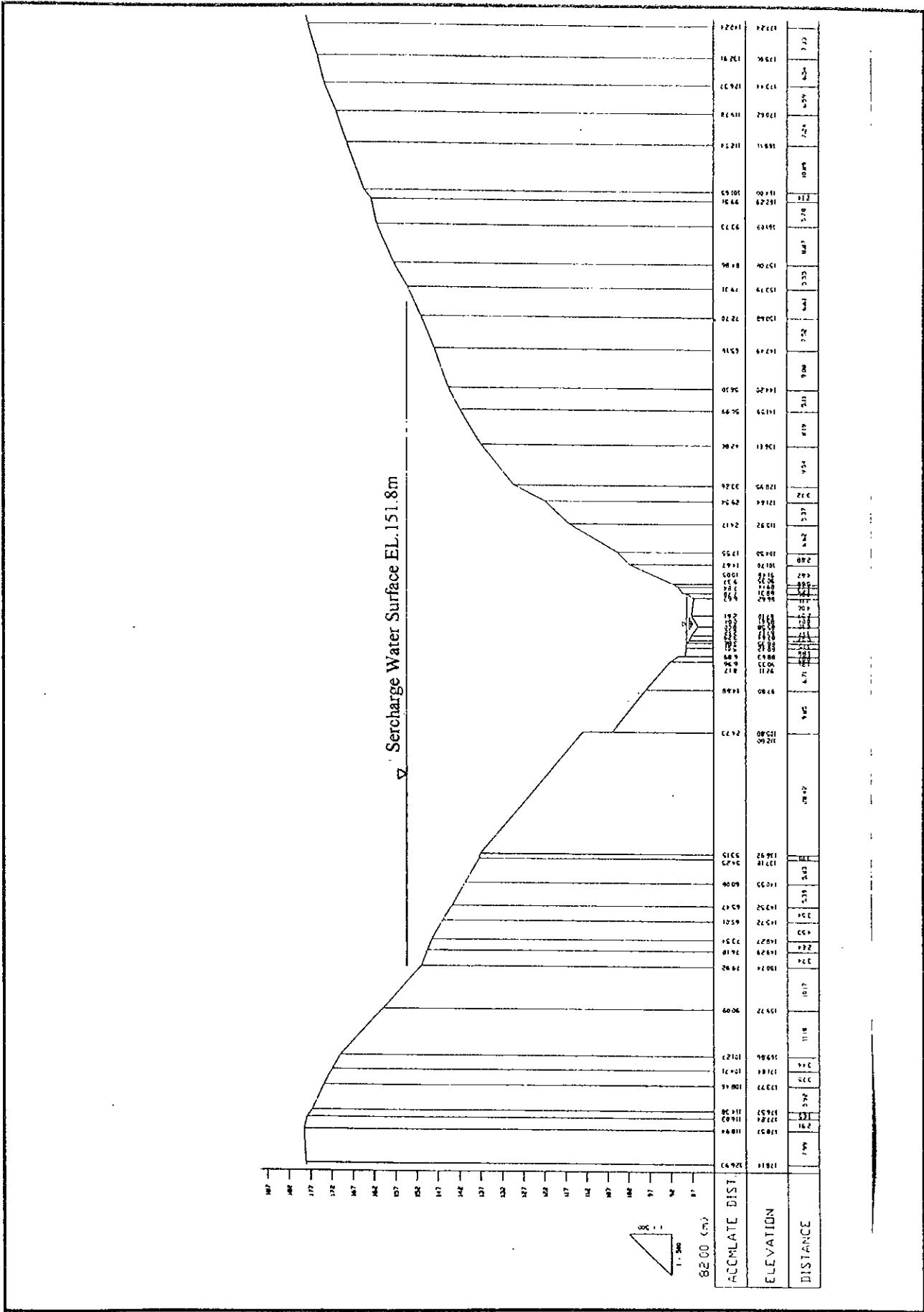




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Fig. 6.2.6  
AXIS OF JATIBARANG DAM .

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## CALCULATION OF WATER WAVE IN RESERVOIR

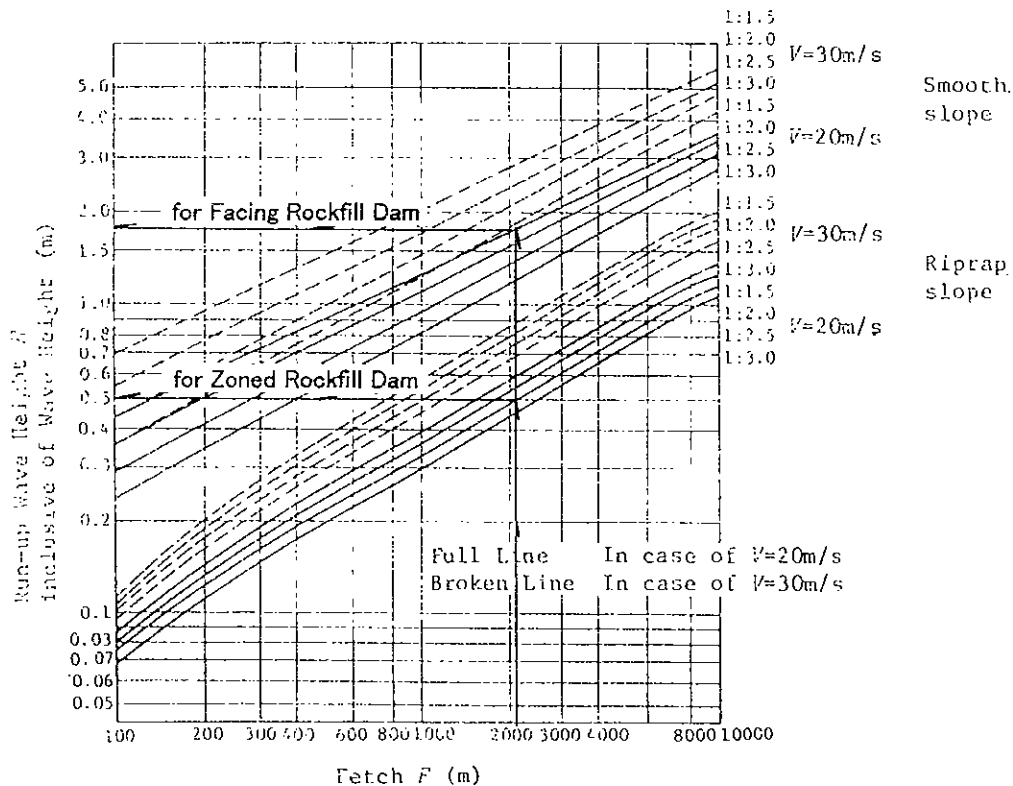
### 1. WAVE DUE TO WIND

Dam Type	Symbol	Value	Remarks
Zoned Rockfill	Wind Velocity	V	20 m/s
	Maximum Fetch	F	2,100 m
	Upstream Slope		1 : 2.6
	Height of Wave by Wind	hw	0.5 m
Facing Rockfill	Wind Velocity	V	20 m/s
	Maximum Fetch	F	2,100 m
	Upstream Slope		1 : 1.5
	Height of Wave by Wind	hw	1.8 m

### 2. WAVE DUE TO SEISMIC MOTION

Dam Type	Symbol	Value	Remarks
Zoned Rockfill	Earthquake Coefficient	k	0.18
Facing Rockfill	Seismic Frequency	t	1 sec
	Reservoir Depth	Ho	68.9 m
	Height of Wave by Earthquake	he	0.8 m

refer to Table 2.2.3  
Ho = NWL 148.9 - Base 80.0m  
he = k\*t\*(9.8\*Ho)<sup>0.5</sup>/2/3.14=0.745m



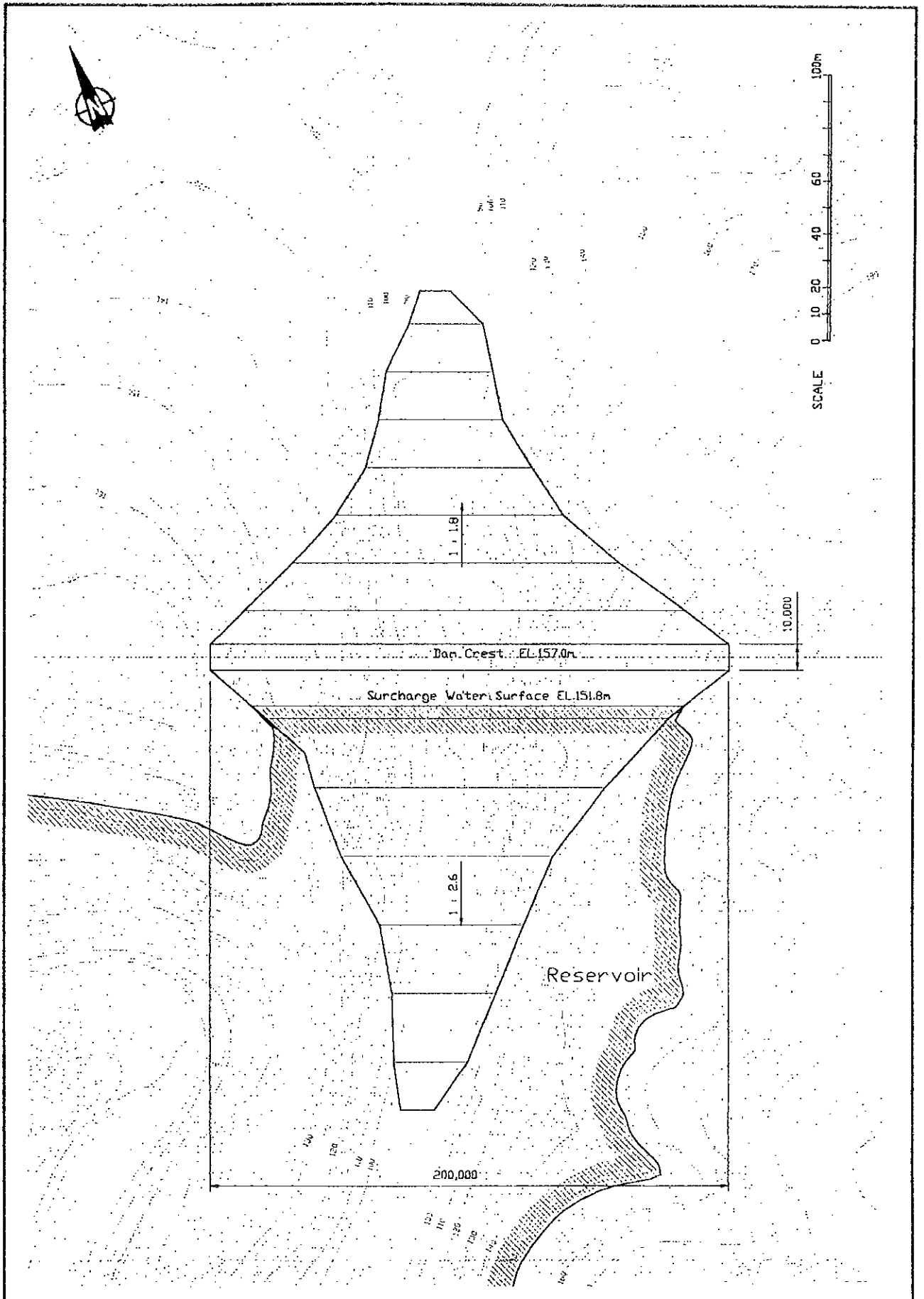
Run-up Wave Height (inclusive of Wave Height) obtained by a combined use  
of the S.M.B. Method and Saville's Method

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DRAINAGE AND WATER RESOURCES DEVELOPMENT  
IN SEMARANG IN THE REPUBLIC OF INDONESIA

Fig. 6.2.8

CALCULATION OF WATER WAVE IN RESERVOIR

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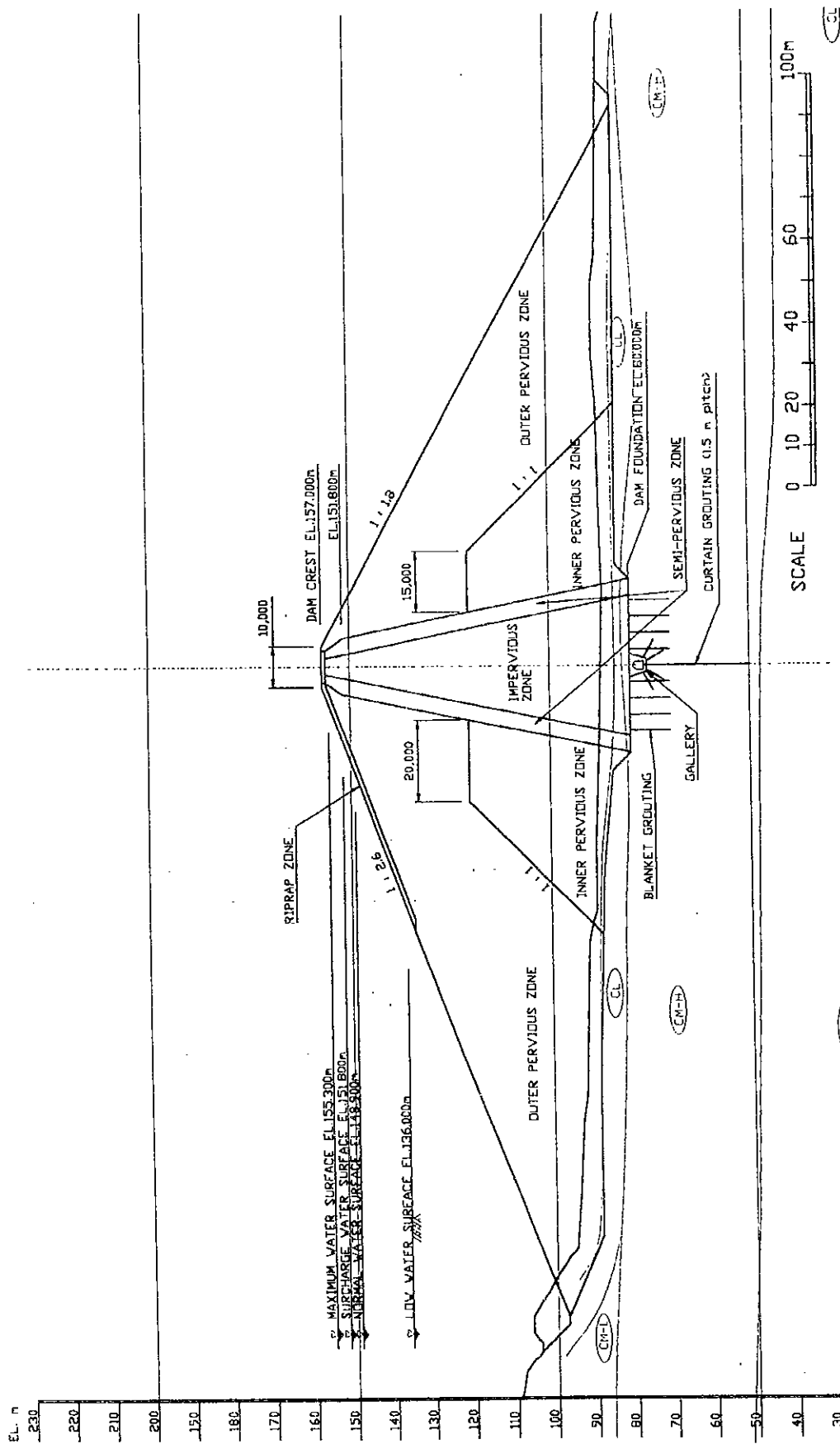


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Fig. 6.2.9

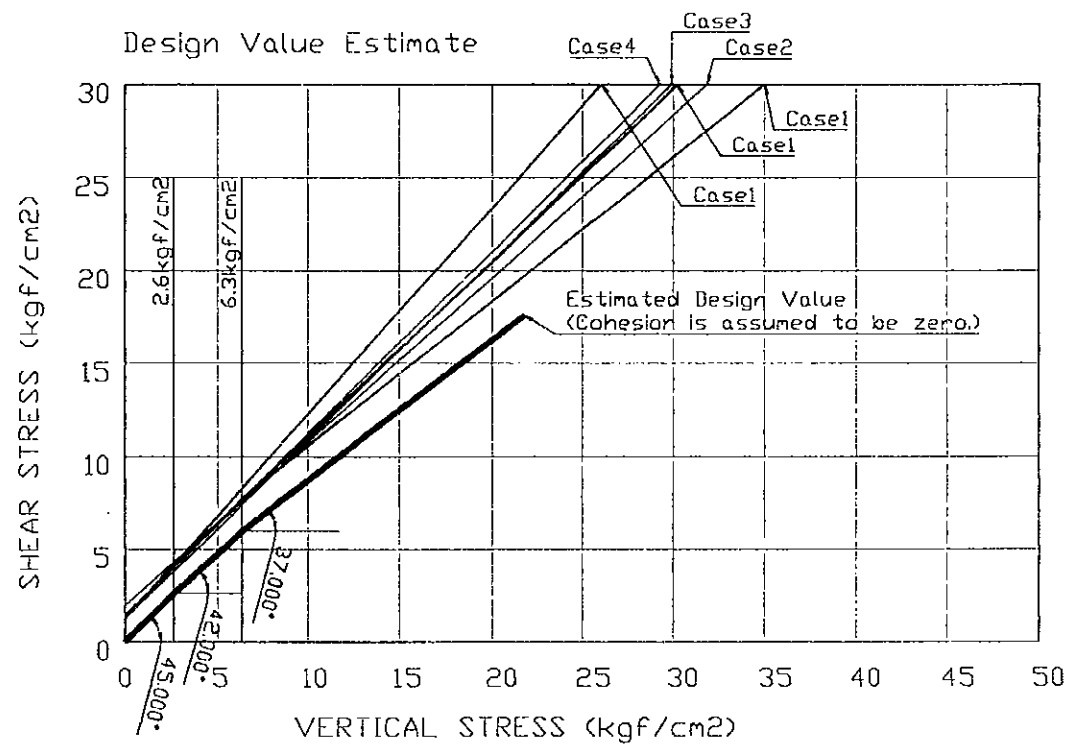
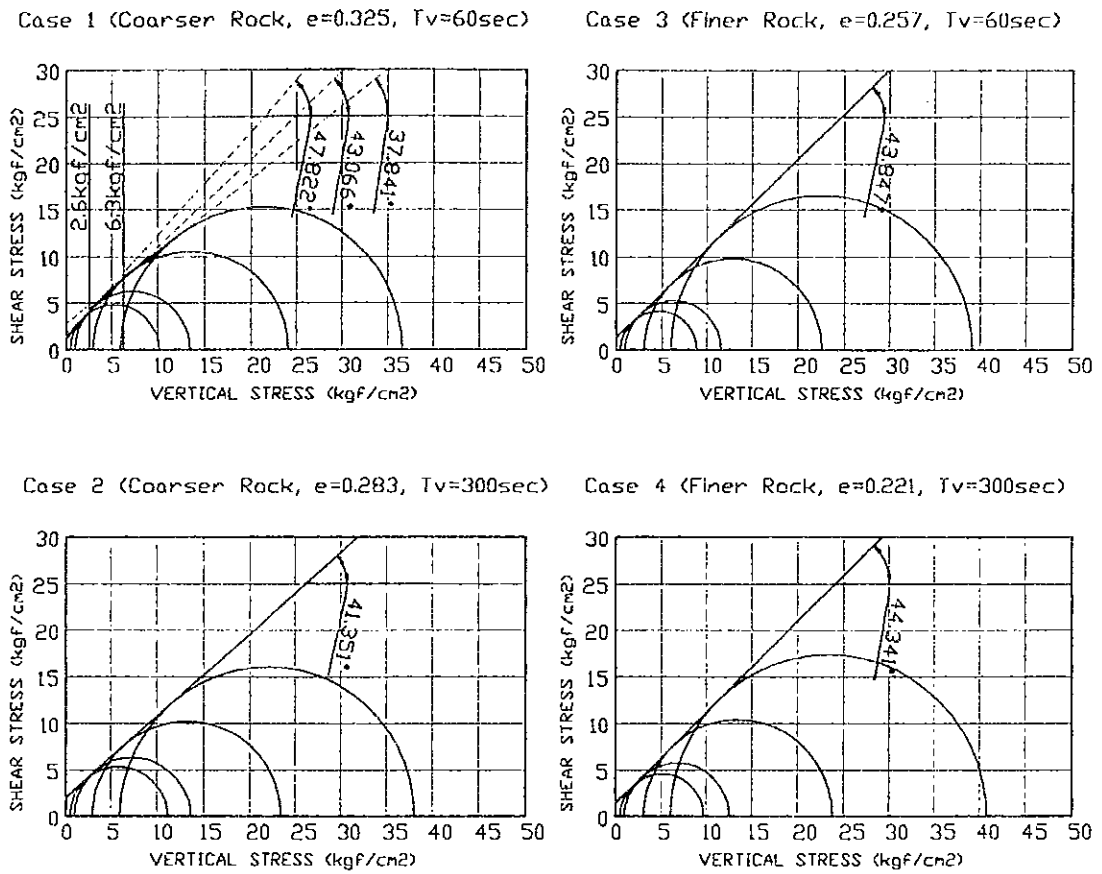
PLAN OF CENTER CORE ROCKFILL DAM



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Fig. 6.2.10  
TYPICAL CROSS SECTION OF CENTER CORE ROCKFILL DAM



NOTE : e=Void Ratio, Tv=Time of Vibrating Compaction

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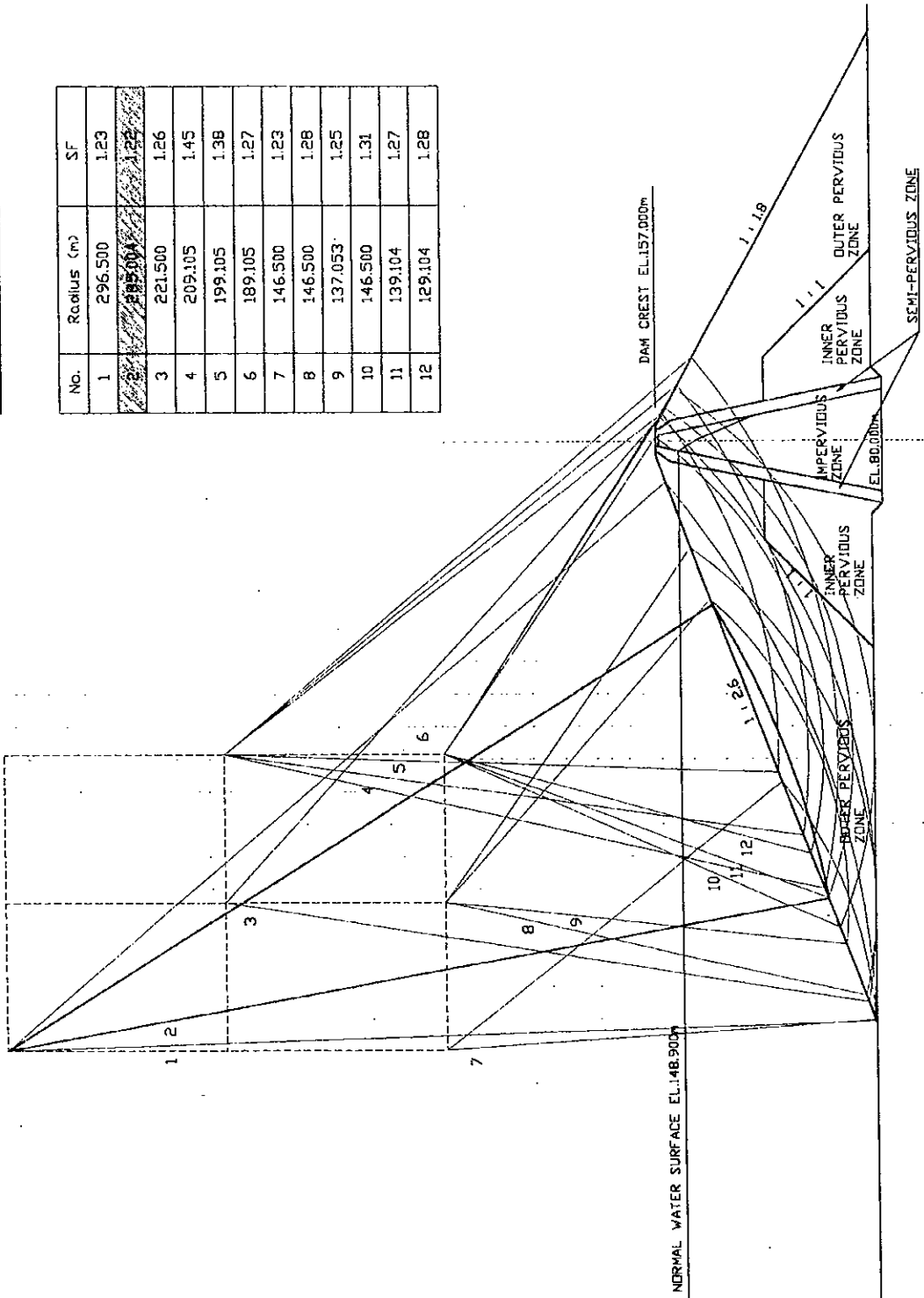
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Fig. 6.2.11 INTERNAL FRICTION ANGLE OF ROCK IN PERVIOUS ZONE

EL.m 400  
390  
380  
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90  
80  
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60  
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Water Level	Normal Water Surface EL.148.9m
Seismic Coefficient	0.18

No.	Radius (m)	SF
1	296.500	1.23
2	296.500	1.22
3	221.500	1.26
4	209.105	1.45
5	199.105	1.38
6	189.105	1.27
7	146.500	1.23
8	146.500	1.28
9	137.053	1.25
10	146.500	1.31
11	139.104	1.27
12	129.104	1.28



EL.m 400  
390  
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THE DETAILED DESIGN OF FLOOD CONTROL, URBAN DRAINAGE AND WATER RESOURCES DEVELOPMENT IN SEMARANG IN THE REPUBLIC OF INDONESIA

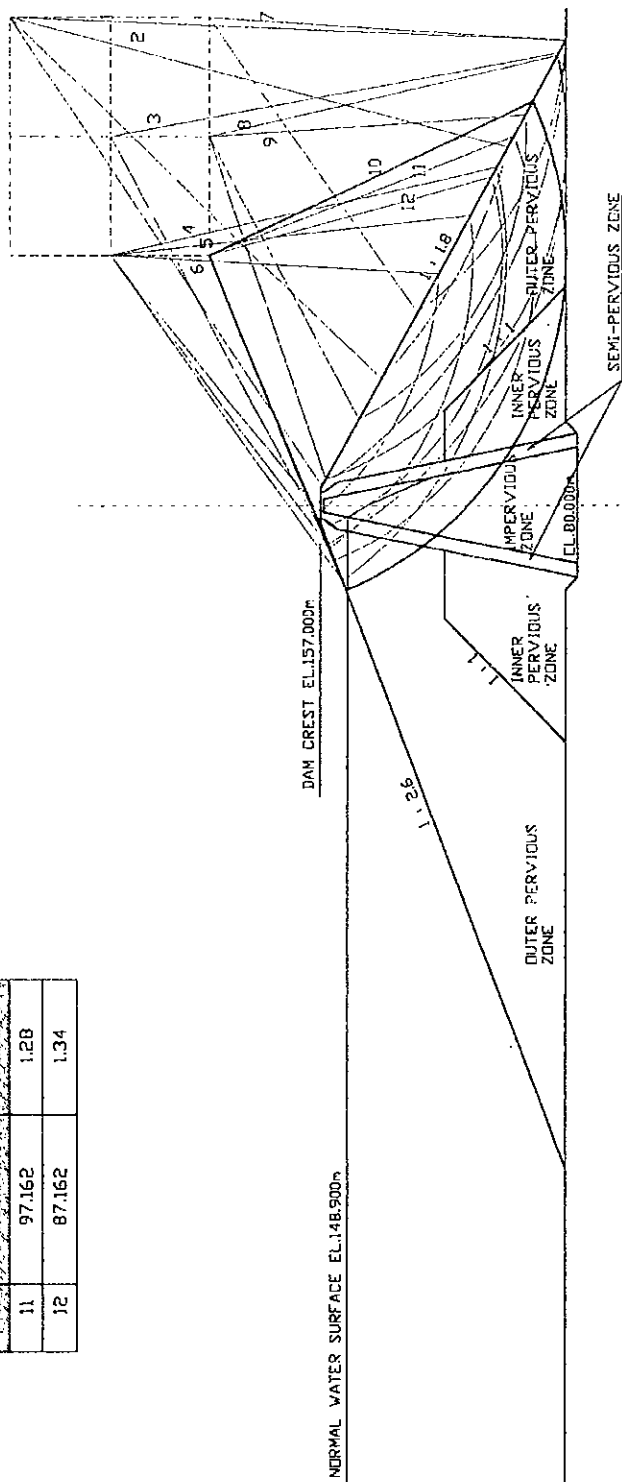
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Fig. 6.2.12  
UPSTREAM SLOPE STABILITY ANALYSIS OF CENTER CORE ROCKFILL DAM

EL.m  
400  
390  
380  
370  
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350  
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330  
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190  
180  
170  
160  
150  
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120  
110  
100  
90  
80  
70  
60  
50

Water Level	Normal Water Surface EL.148.9m
Seismic Coefficient	0.18

No.	Radius (m)	SF
1	166.500	1.28
2	153.642	1.24
3	136.500	1.28
4	118.387	1.29
5	108.387	1.34
6	98.387	1.30
7	106.500	1.28
8	106.500	1.34
9	94.279	1.32
10	106.500	1.23
11	97.162	1.28
12	87.162	1.34



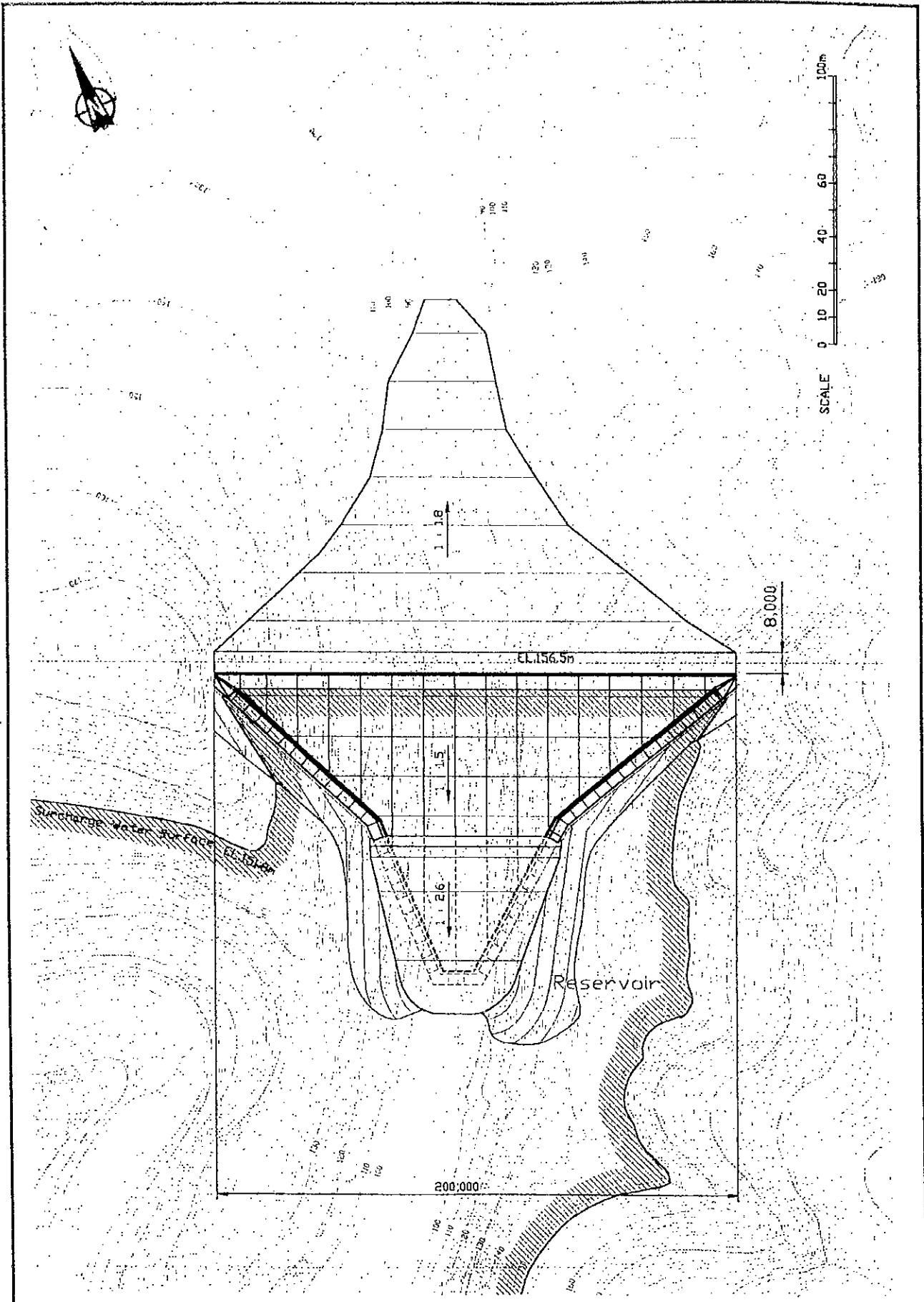
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THE DETAILED DESIGN OF FLOOD CONTROL, URBAN DRAINAGE AND WATER RESOURCES DEVELOPMENT IN SEMARANG IN THE REPUBLIC OF INDONESIA

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Fig. 6.2.13  
DOWNSTREAM SLOPE STABILITY ANALYSIS OF CENTER CORE ROCKFILL DAM

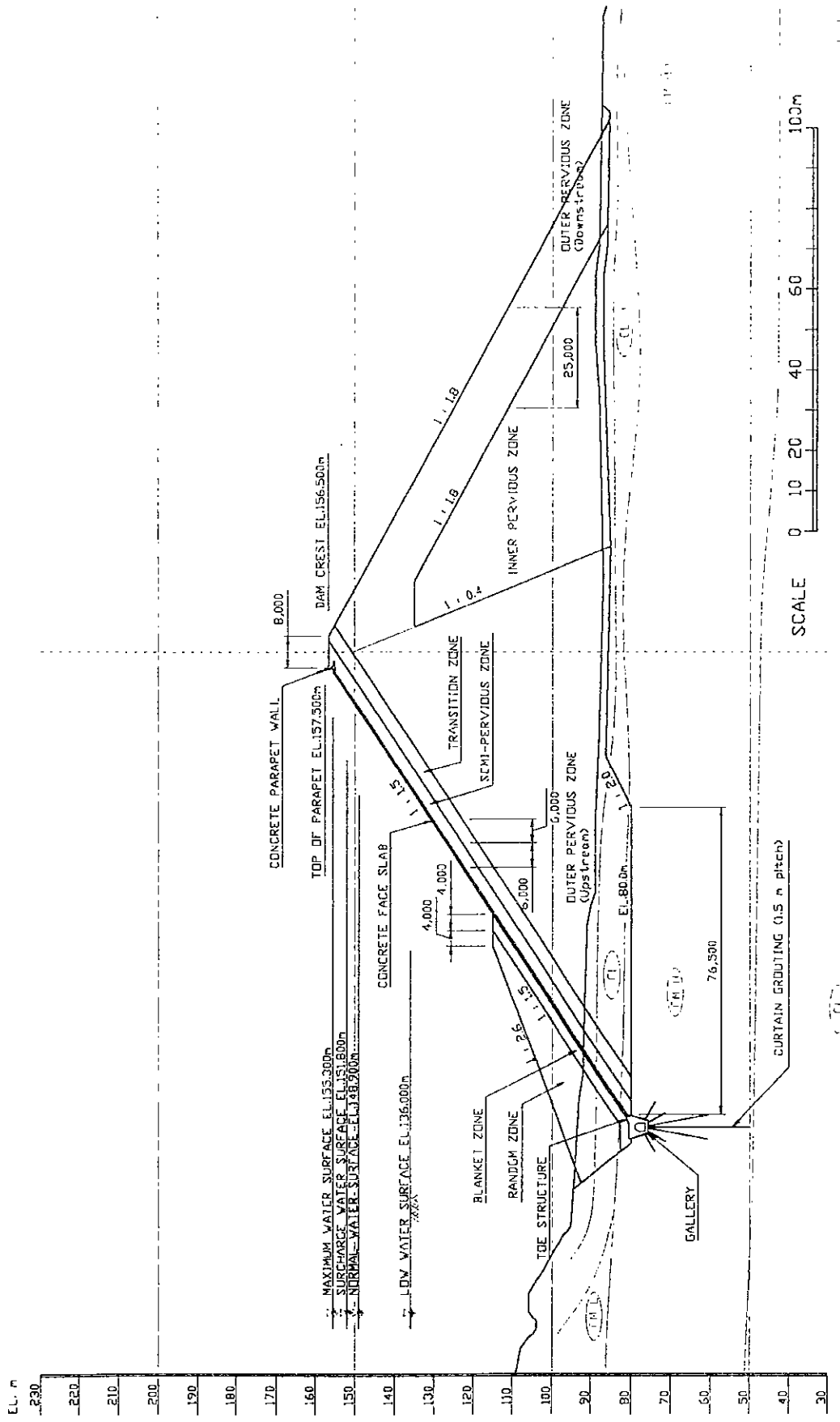




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Fig. 6.2.14  
PLAN OF CONCRETE FACE ROCKFILL DAM

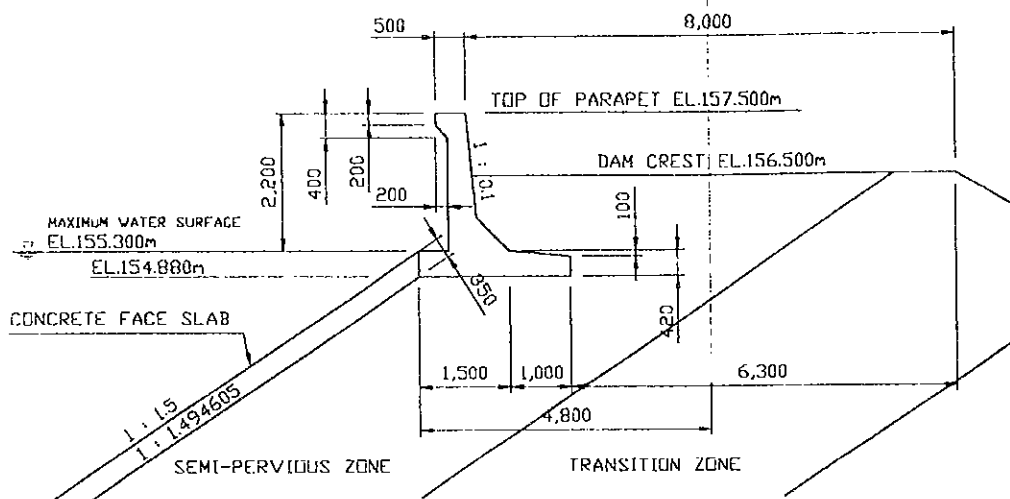
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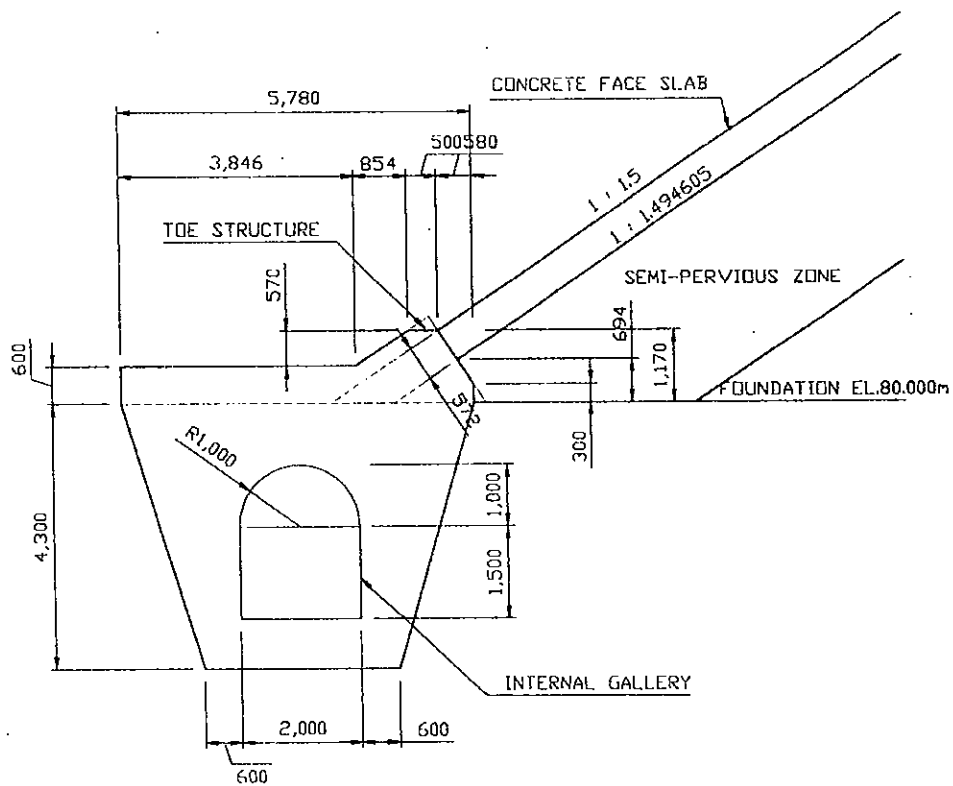
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Fig. 6.2.15  
TYPICAL CROSS SECTION OF CONCRETE FACE ROCKFILL DAM

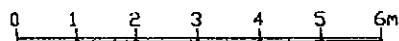
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PARAPET WALL



TOE STRUCTURE AND GALLERY



60221  
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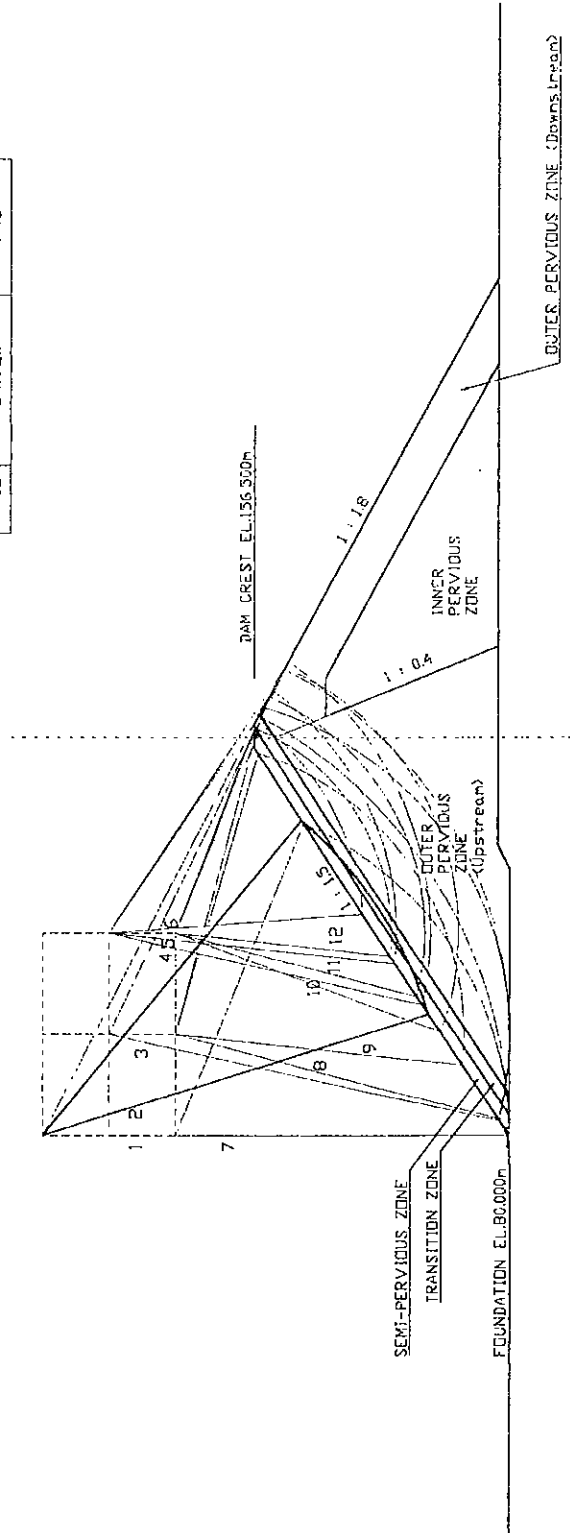
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Fig. 6.2.16  
 TYPICAL CROSS SECTION OF PARAPET WALL AND TOE SLAB

EL.m 400  
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Water Level	- (End of Construction)
Seismic Coefficient	0.09 (50%)

No.	Radius (m)	SF
1	140.000	1.36
2	121.487	1.27
3	120.000	1.61
4	96.564	1.86
5	86.564	1.67
6	76.564	1.43
7	100.000	1.36
8	100.000	1.50
9	86.564	1.40
10	84.923	1.66
11	74.923	1.63
12	64.923	1.45



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THE DETAILED DESIGN OF FLOOD CONTROL, URBAN DRAINAGE AND WATER RESOURCES DEVELOPMENT IN SEMARANG IN THE REPUBLIC OF INDONESIA

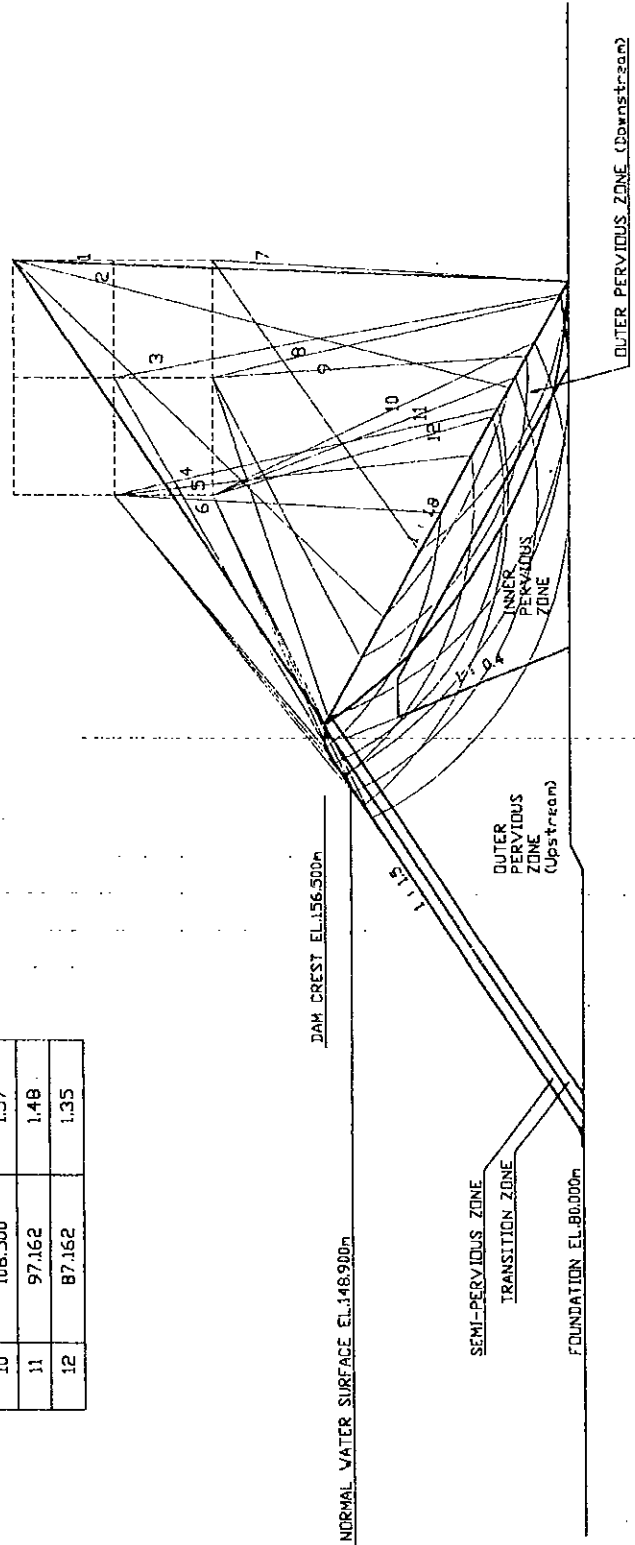
Fig. 6.2.17  
UPSTREAM SLOPE STABILITY ANALYSIS OF CONCRETE FACE ROCKFILL DAM

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Water Level	Normal Water Surface EL.148.9m
Seismic Coefficient	0.18

No.	Radius (m)	SF
2	153.642	1.24
3	136.500	1.31
4	118.387	1.56
5	108.387	1.43
6	98.387	1.33
7	106.500	1.28
8	106.500	1.26
9	94.279	1.26
10	106.500	1.57
11	97.162	1.48
12	87.162	1.35



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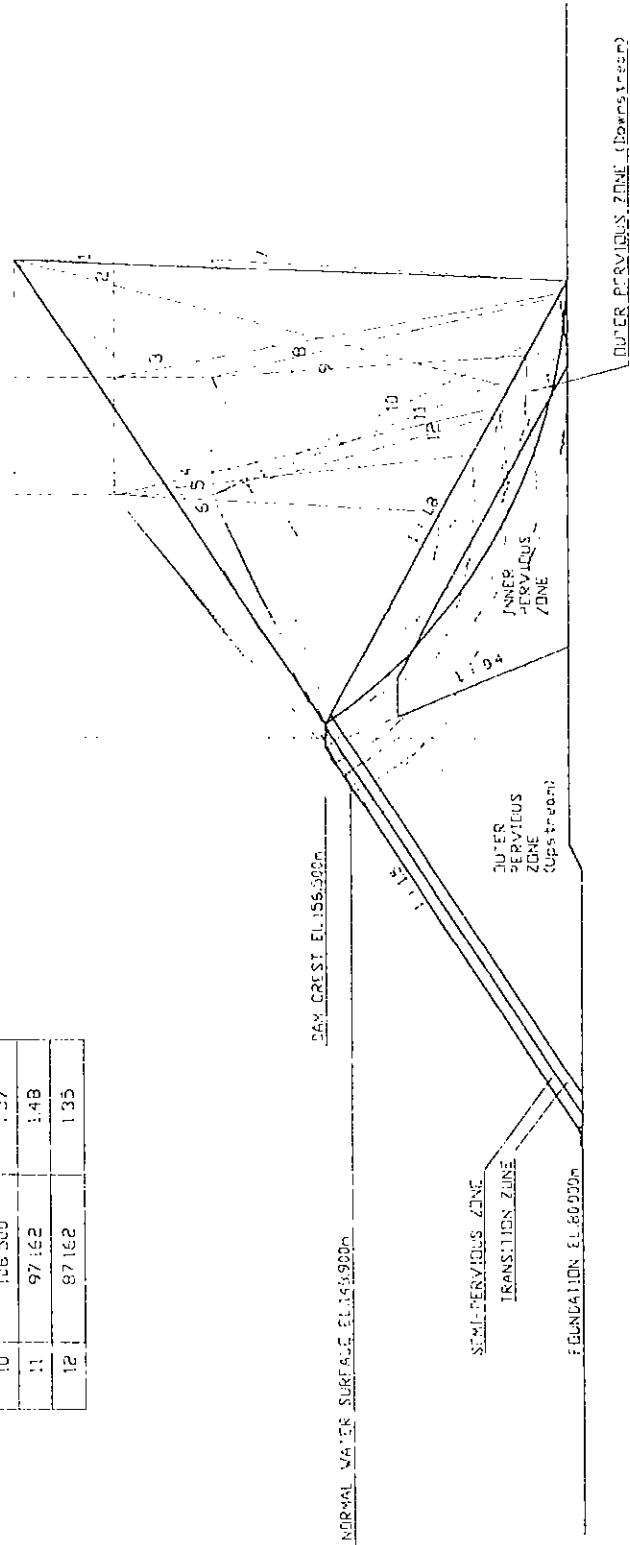
Fig. 6.2.18  
DOWNSTREAM SLOPE STABILITY ANALYSIS OF CONCRETE FACE ROCKFILL DAM

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Water Level	Normal Water Surface EL.148.9m
Seismic Coefficient	0.18

No.	Radius (m)	SF
1	156.500	1.22
2	153.642	1.24
3	136.500	1.31
4	118.397	1.55
5	108.287	1.43
6	98.387	1.33
7	106.500	1.28
8	106.500	1.25
9	94.279	1.26
10	106.500	1.57
11	97.162	1.48
12	87.162	1.35

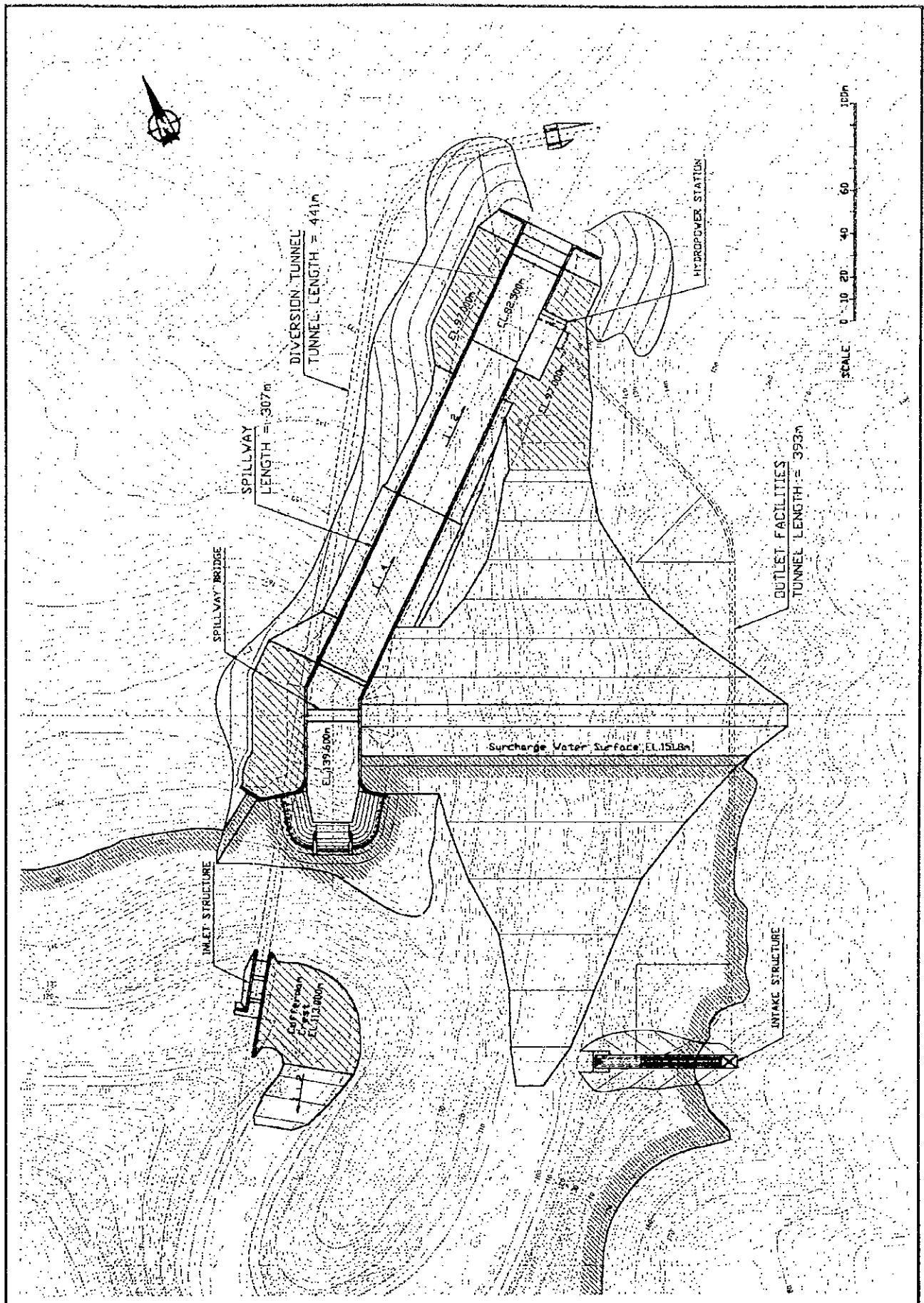


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THE DETAILED DESIGN OF FLOOD CONTROL, URBAN DRAINAGE AND WATER RESOURCES DEVELOPMENT IN SEMARANG IN THE REPUBLIC OF INDONESIA

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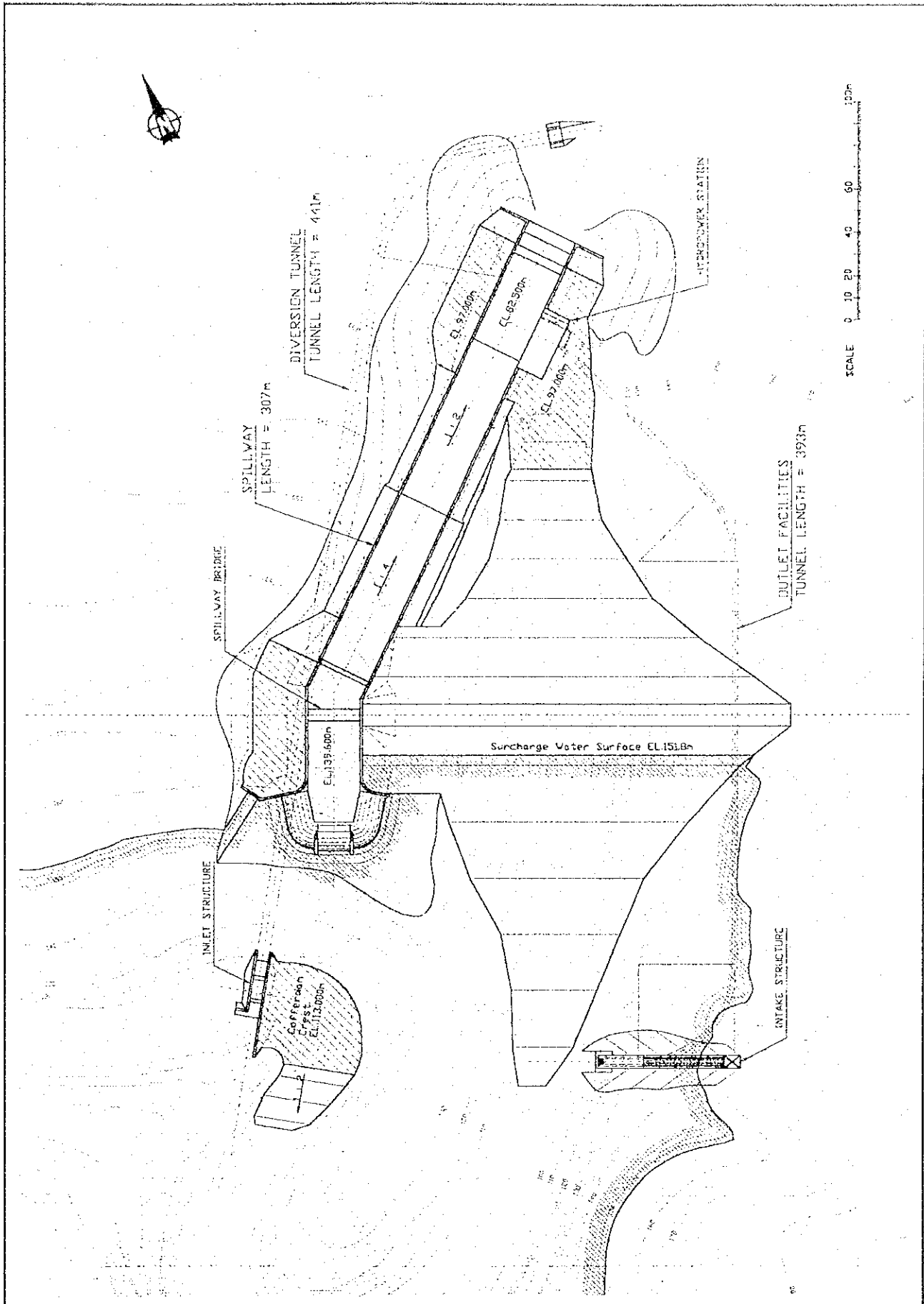
Fig. 6.2.18  
DOWNSTREAM SLOPE STABILITY ANALYSIS OF CONCRETE FACE ROCKFILL DAM



THE DETAILED DESIGN OF FLOOD CONTROL, URBAN DRAINAGE AND WATER RESOURCES DEVELOPMENT IN SEMARANG IN THE REPUBLIC OF INDONESIA

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Fig. 6.2.19  
LAYOUT OF APPURTENANT STRUCTURES IN ALTERNATIVE L 1



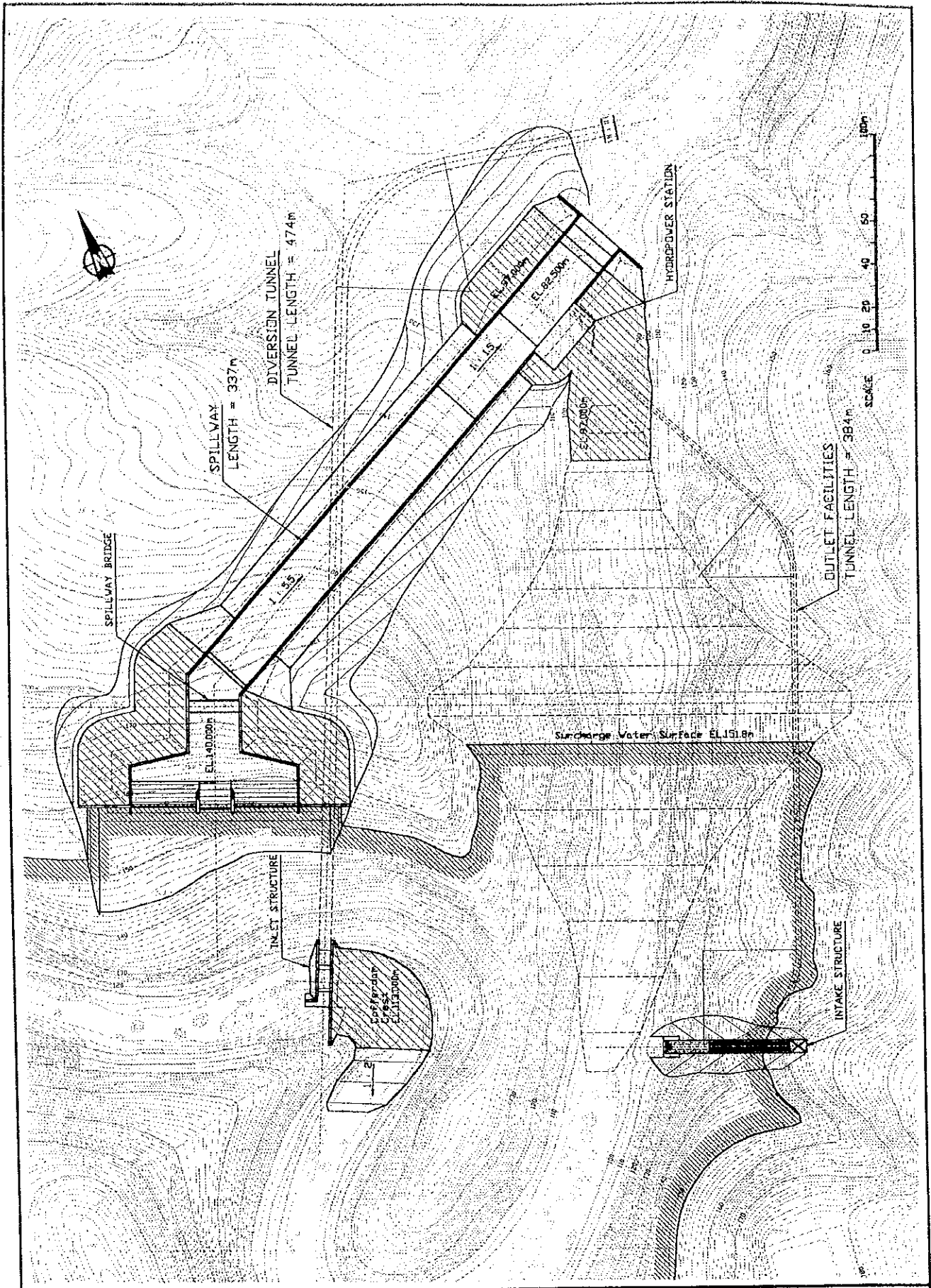
SCALE 0 10 20 40 60 80m

THE DETAILED DESIGN OF FLOOD CONTROL, URBAN DRAINAGE AND WATER RESOURCES DEVELOPMENT IN SEMARANG IN THE REPUBLIC OF INDONESIA

Fig. 6.2.19 LAYOUT OF APPURTENANT STRUCTURES IN ALTERNATIVE L 1

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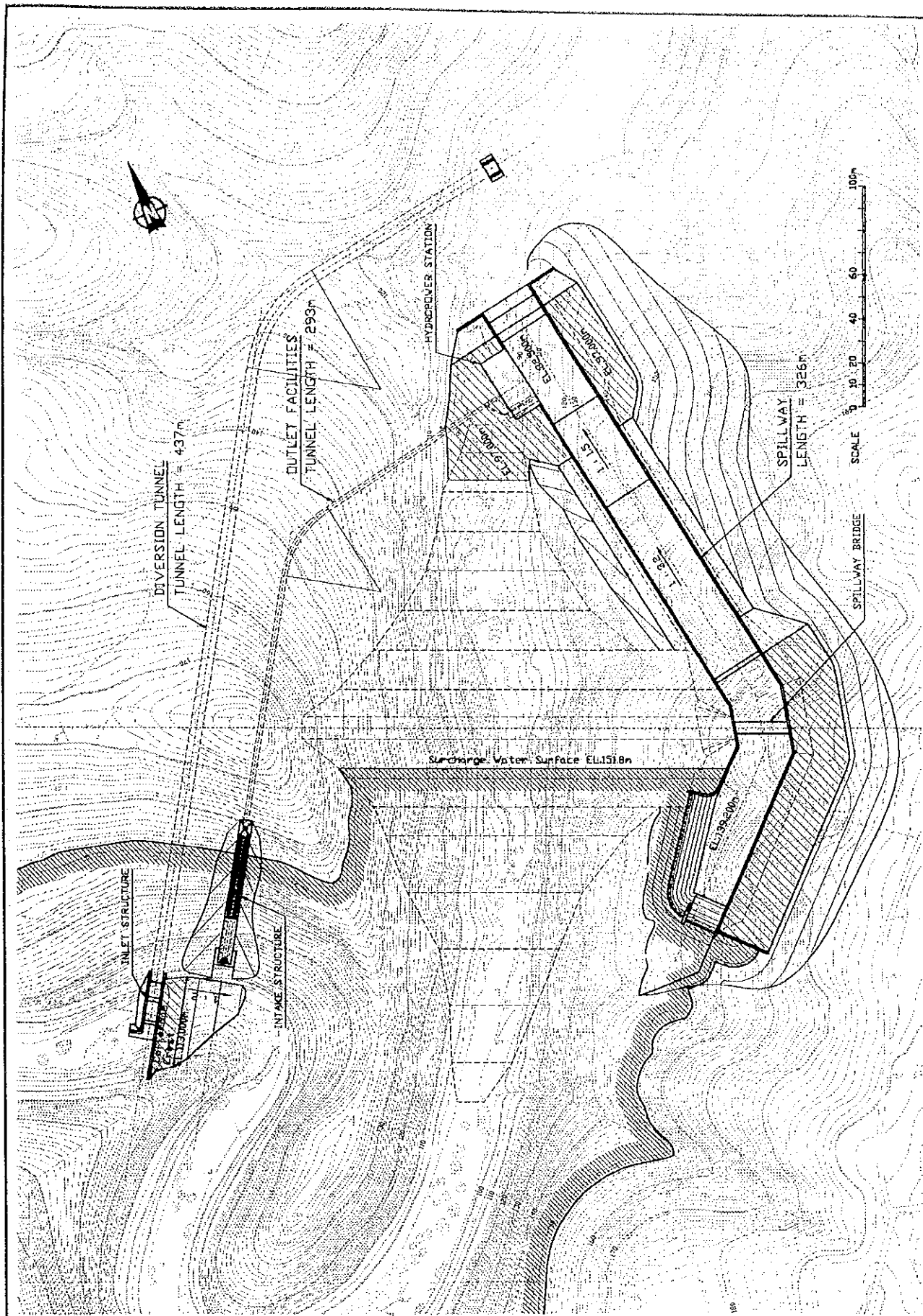




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Fig. 6.2.20  
LAYOUT OF APPURTENANT STRUCTURES IN ALTERNATIVE L 2

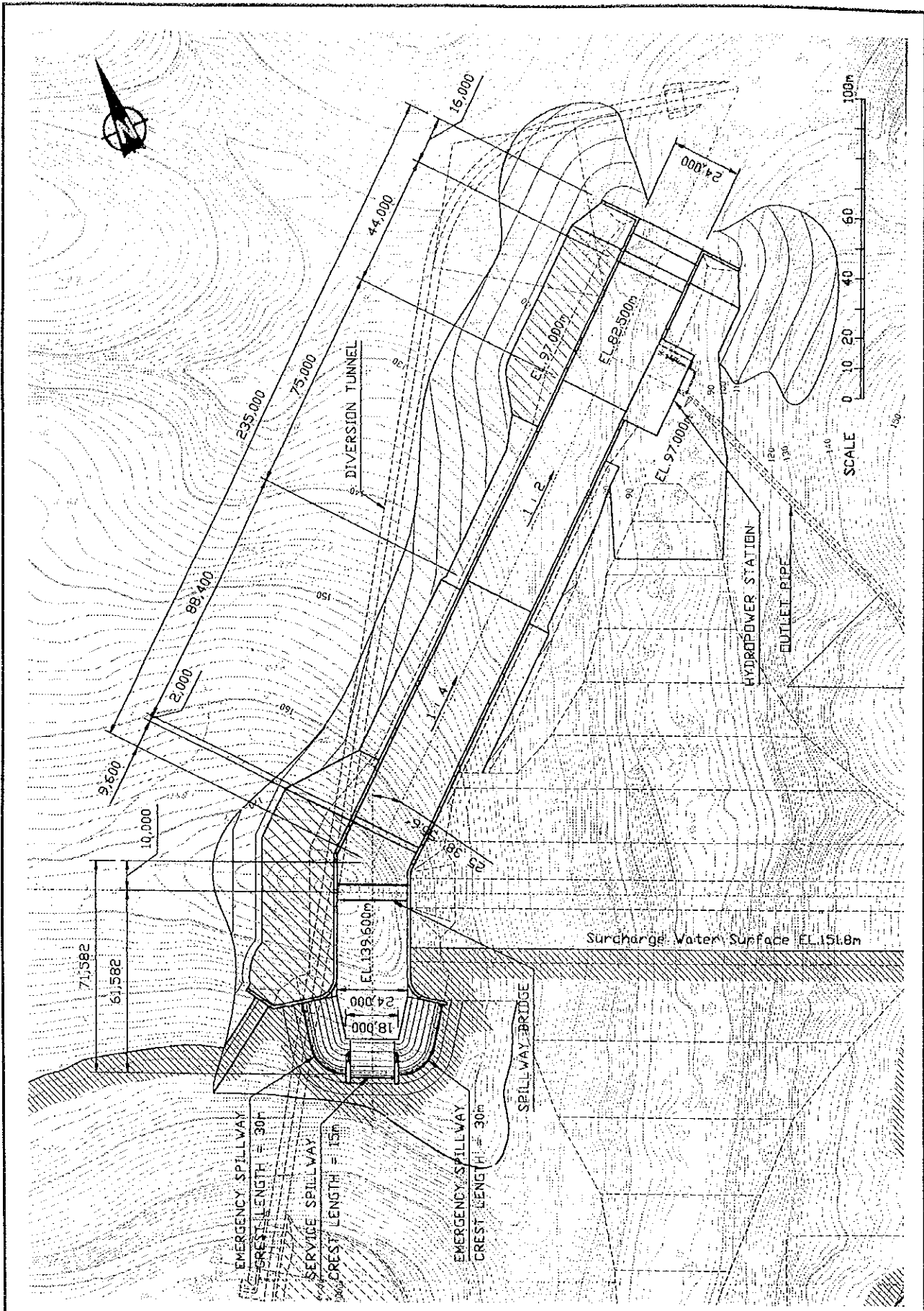
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THE DETAILED DESIGN OF FLOOD CONTROL, URBAN DRAINAGE AND WATER RESOURCES DEVELOPMENT IN SEMARANG IN THE REPUBLIC OF INDONESIA

Fig. 6.2.21  
LAYOUT OF APPURTENANT STRUCTURES IN ALTERNATIVE R 1

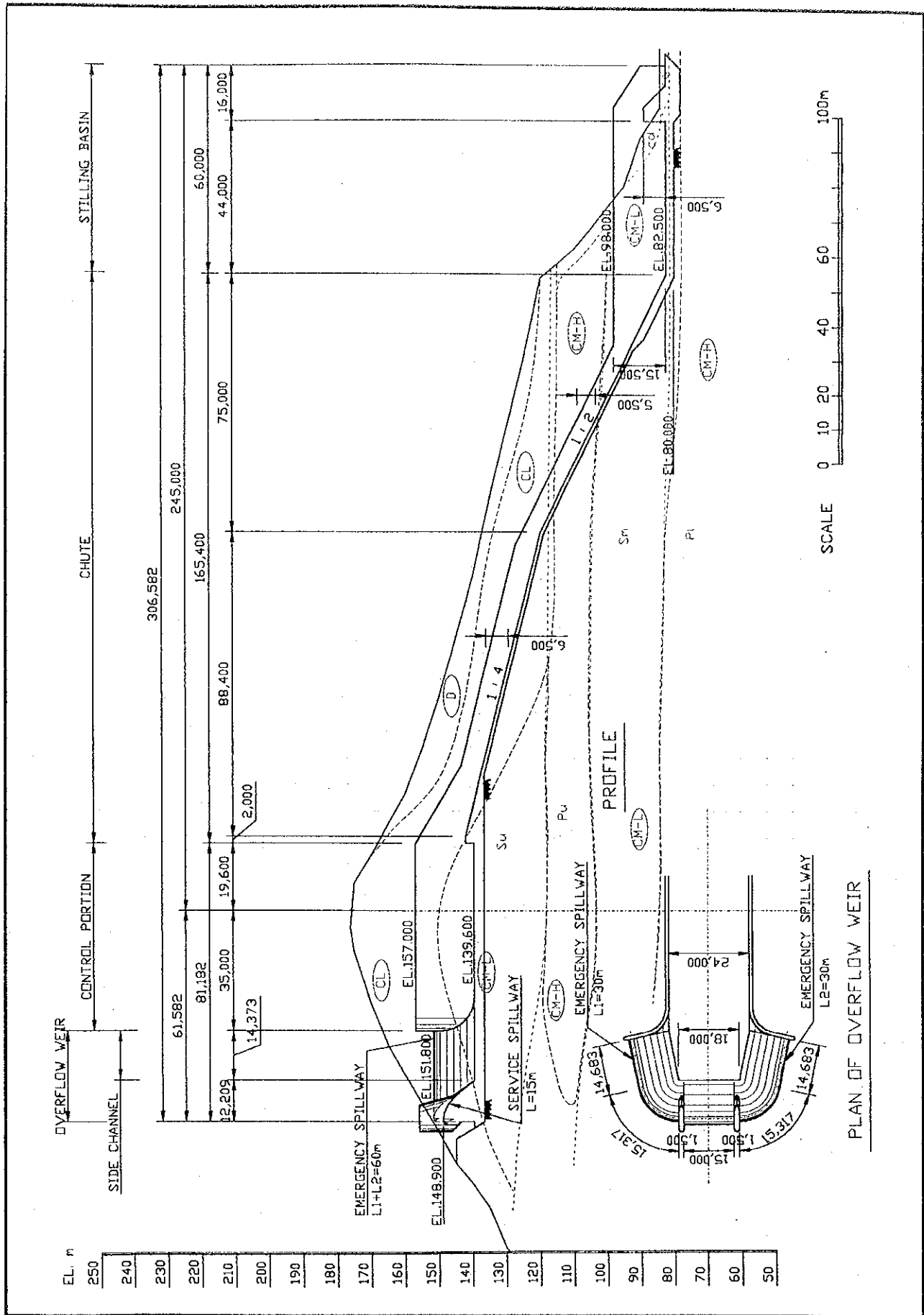
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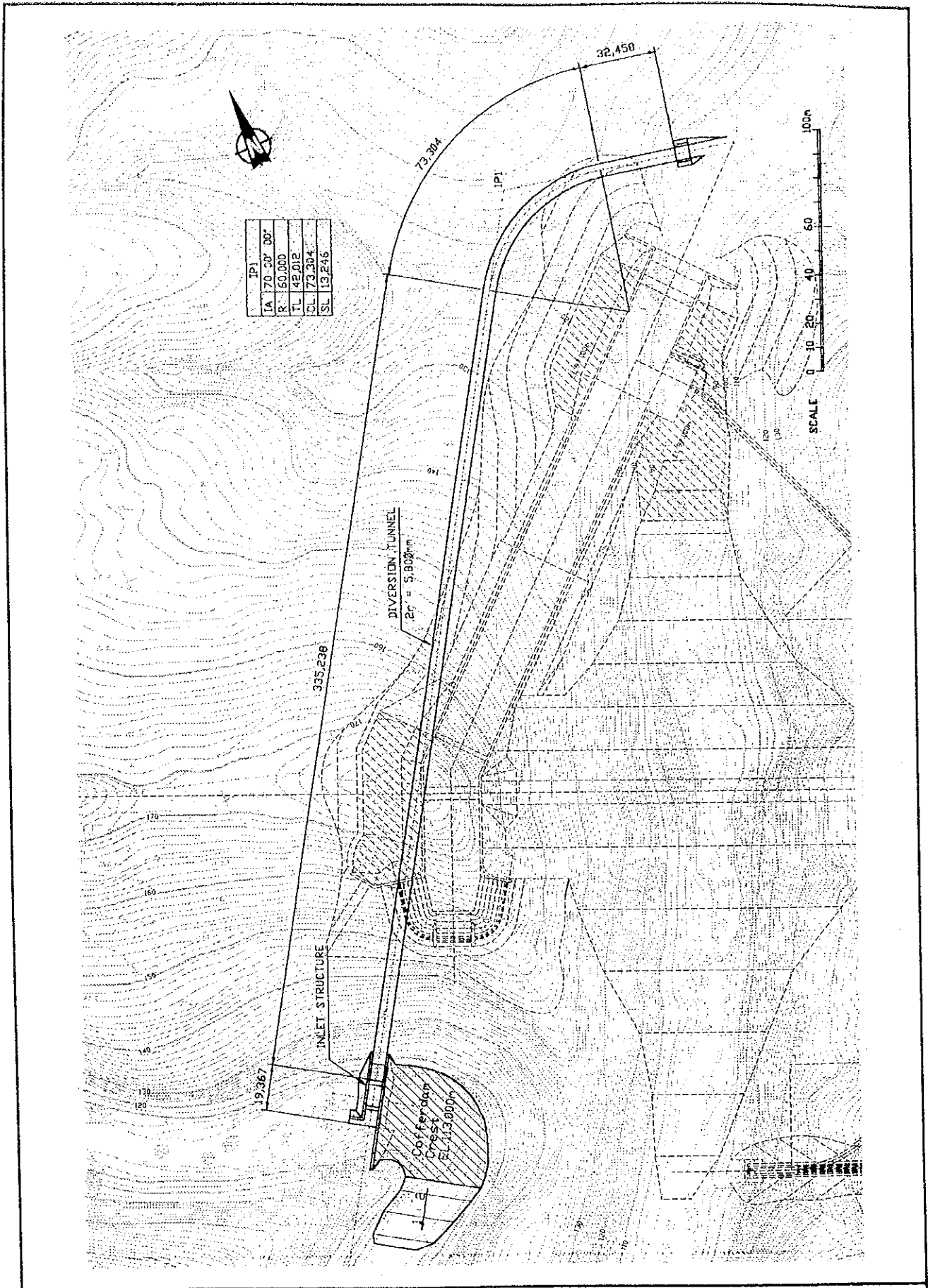
Fig. 6.2.22  
PLAN OF SPILLWAY



THE DETAILED DESIGN OF FLOOD CONTROL, URBAN DRAINAGE AND WATER RESOURCES DEVELOPMENT IN SEMARANG IN THE REPUBLIC OF INDONESIA

Fig. 6.2.23  
PROFILE OF SPILLWAY

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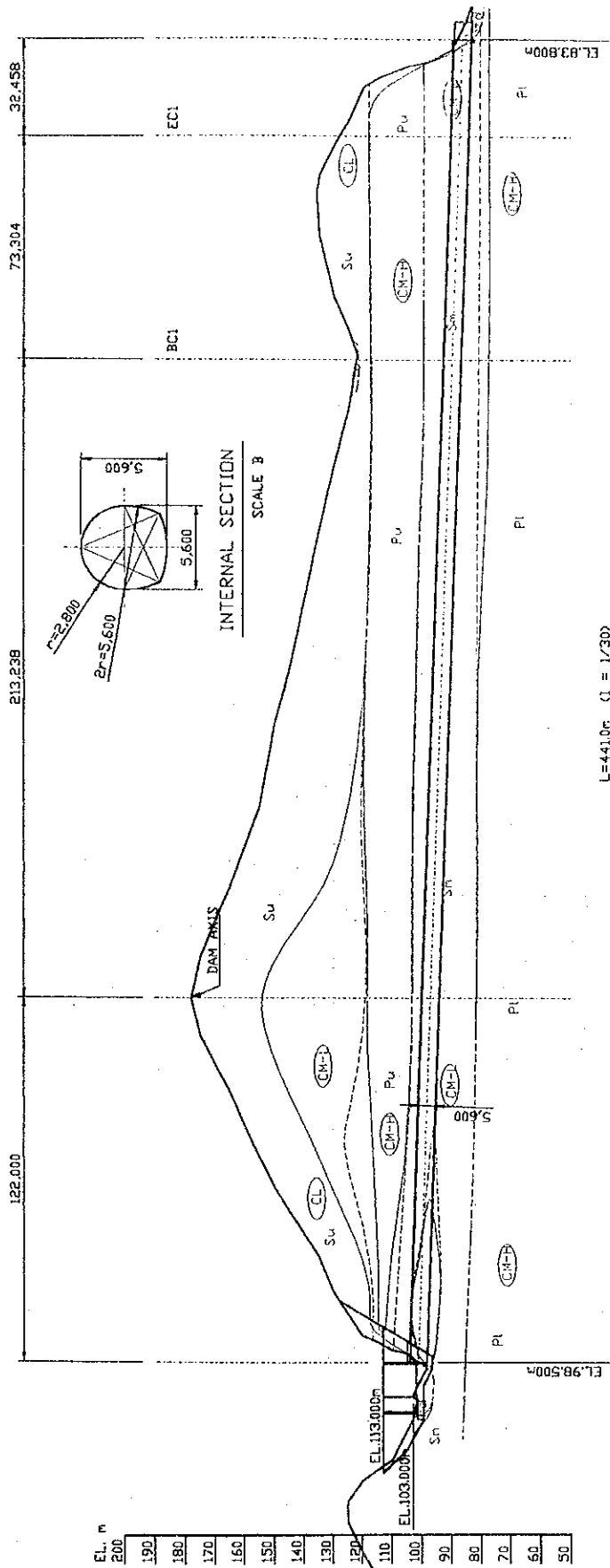


THE DETAILED DESIGN OF FLOOD CONTROL, URBAN DRAINAGE AND WATER RESOURCES DEVELOPMENT IN SEMARANG IN THE REPUBLIC OF INDONESIA

Fig. 6.2.24  
PLAN OF DIVERSION TUNNEL

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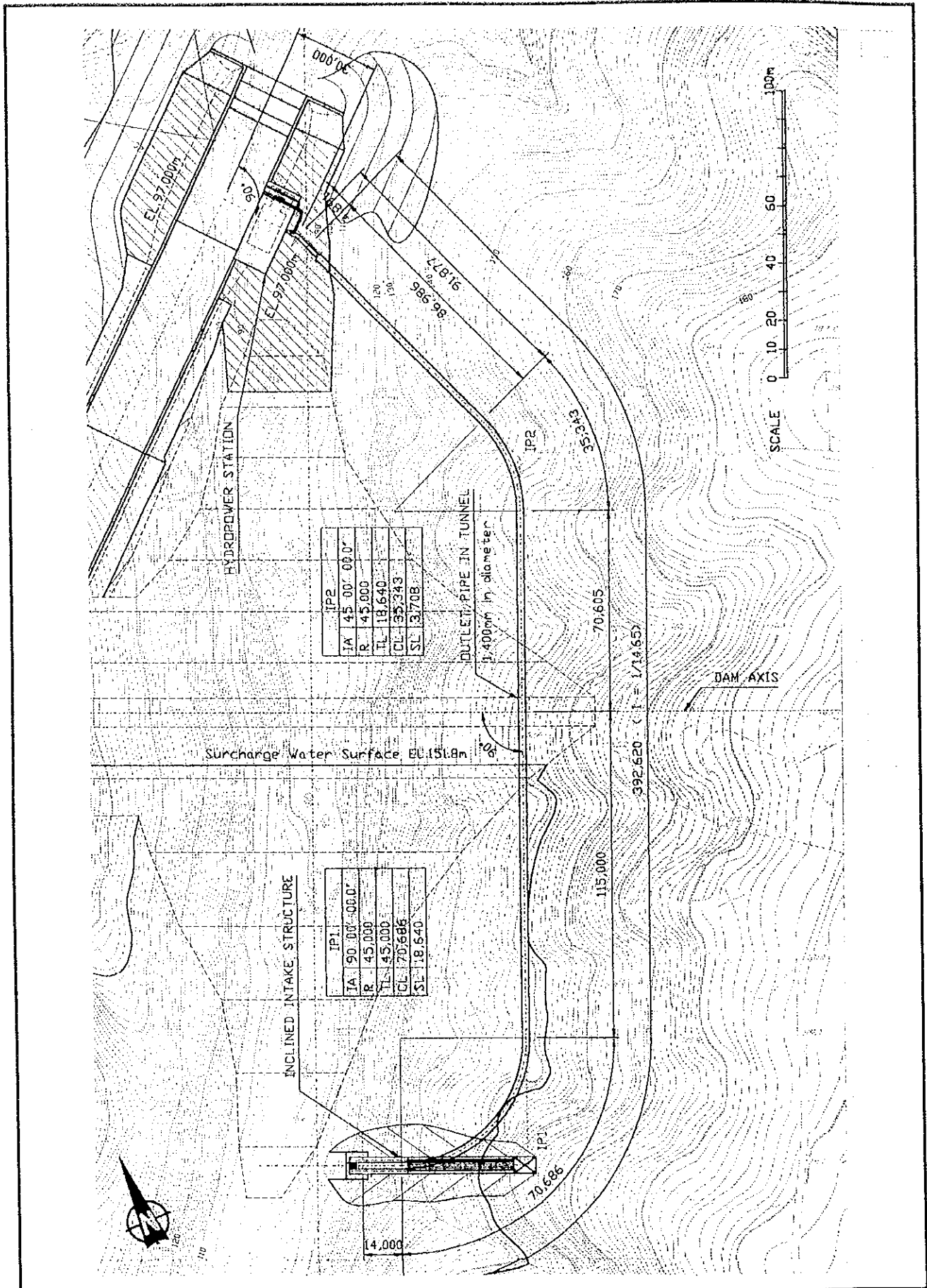
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SL	113.246



THE DETAILED DESIGN OF FLOOD CONTROL, URBAN DRAINAGE AND WATER RESOURCES DEVELOPMENT IN SEMARANG IN THE REPUBLIC OF INDONESIA

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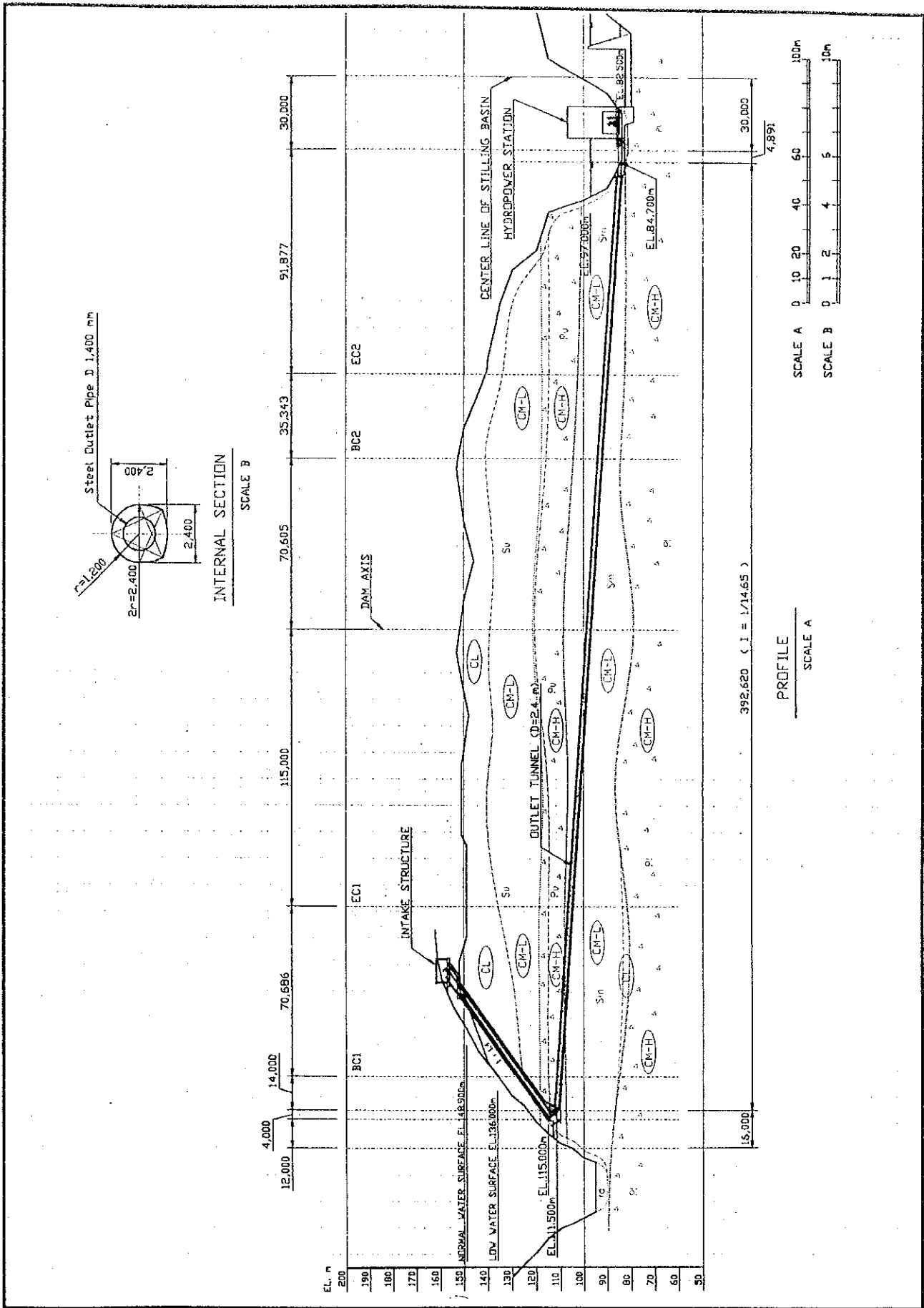
Fig. 6.2.25  
PROFILE OF DIVERSION TUNNEL



THE DETAILED DESIGN OF FLOOD CONTROL, URBAN DRAINAGE AND WATER RESOURCES DEVELOPMENT IN SEMARANG IN THE REPUBLIC OF INDONESIA

Fig. 6.2.26  
PLAN OF OUTLET FACILITIES

JAPAN INTERNATIONAL COOPERATION AGENCY

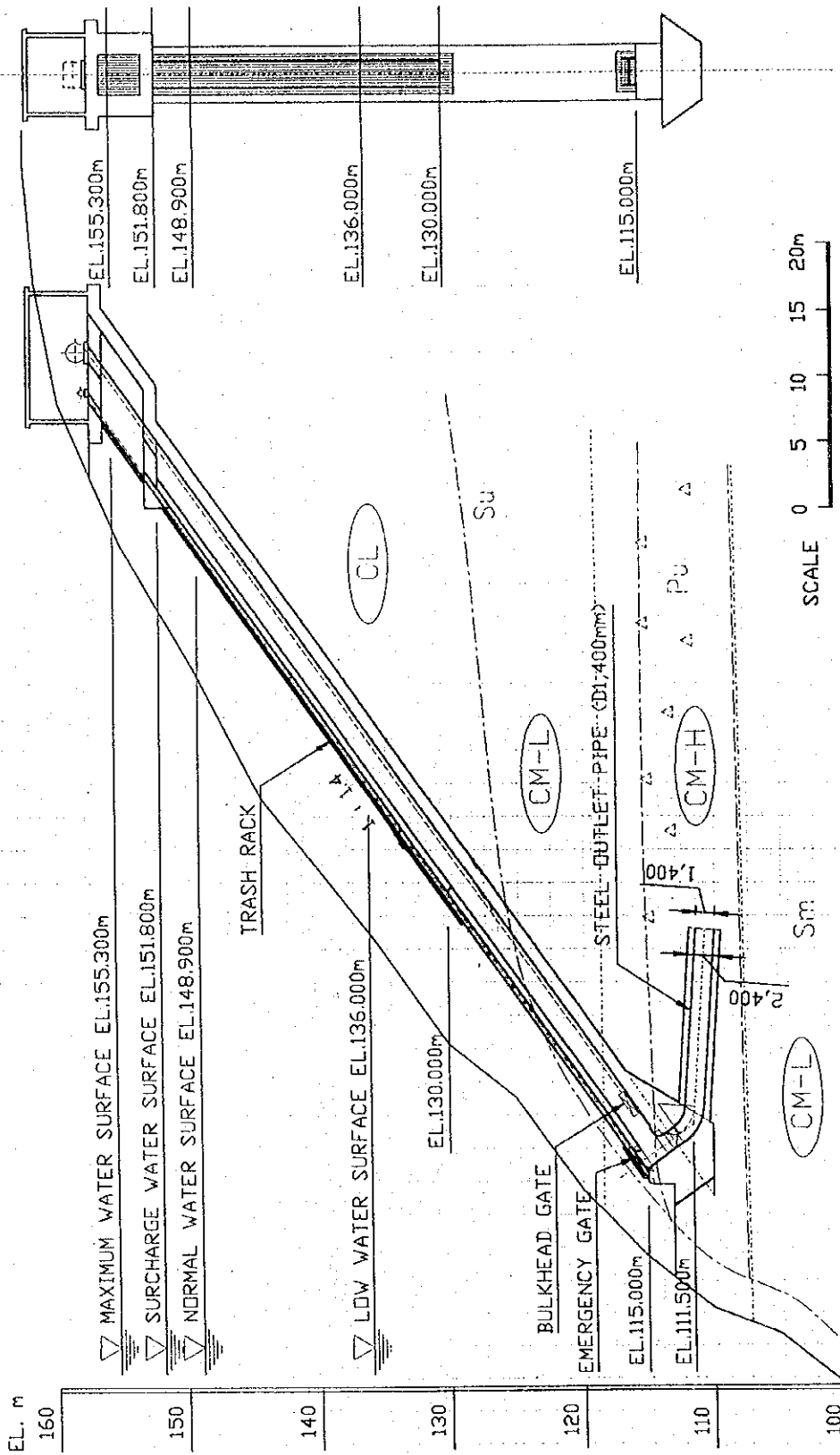


THE DETAILED DESIGN OF FLOOD CONTROL, URBAN DRAINAGE AND WATER RESOURCES DEVELOPMENT IN SEMARANG IN THE REPUBLIC OF INDONESIA

Fig. 6.2.27  
PROFILE OF OUTLET FACILITIES

JAPAN INTERNATIONAL COOPERATION AGENCY





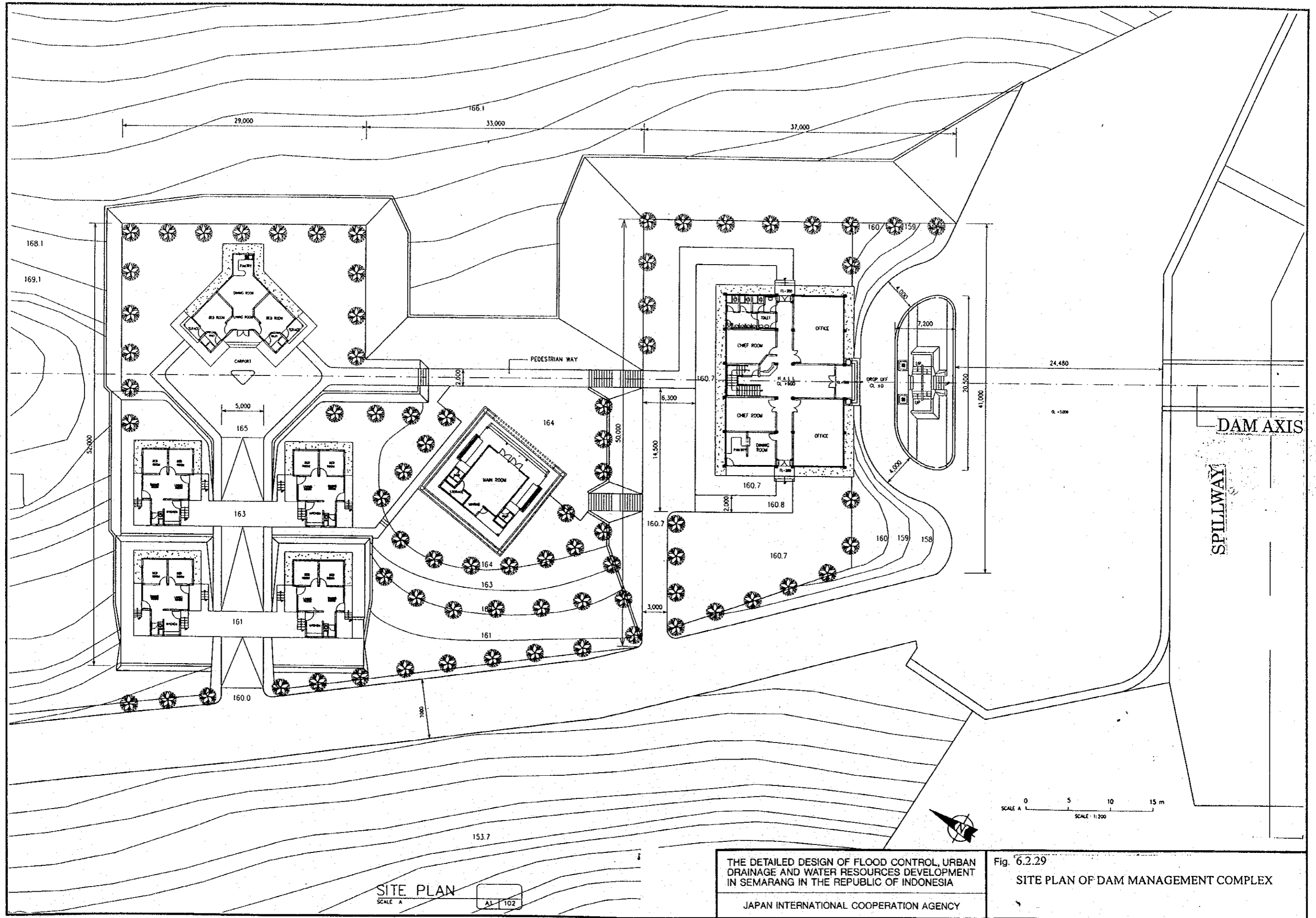
THE DETAILED DESIGN OF FLOOD CONTROL, URBAN DRAINAGE AND WATER RESOURCES DEVELOPMENT IN SEMARANG IN THE REPUBLIC OF INDONESIA

Fig. 6.2.28

PROFILE OF INCLINED INTAKE STRUCTURE

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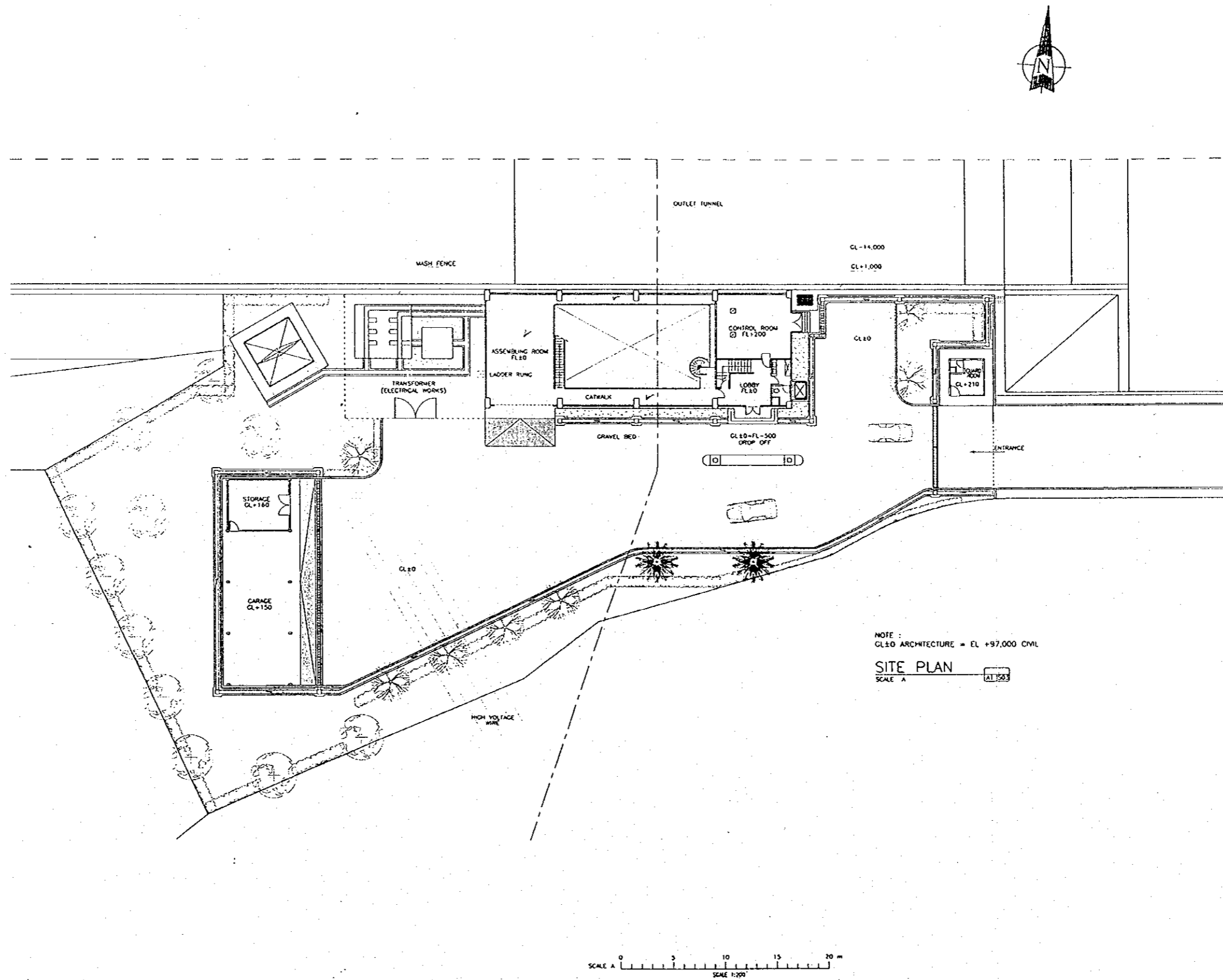




SITE PLAN  
SCALE A

THE DETAILED DESIGN OF FLOOD CONTROL, URBAN DRAINAGE AND WATER RESOURCES DEVELOPMENT IN SEMARANG IN THE REPUBLIC OF INDONESIA  
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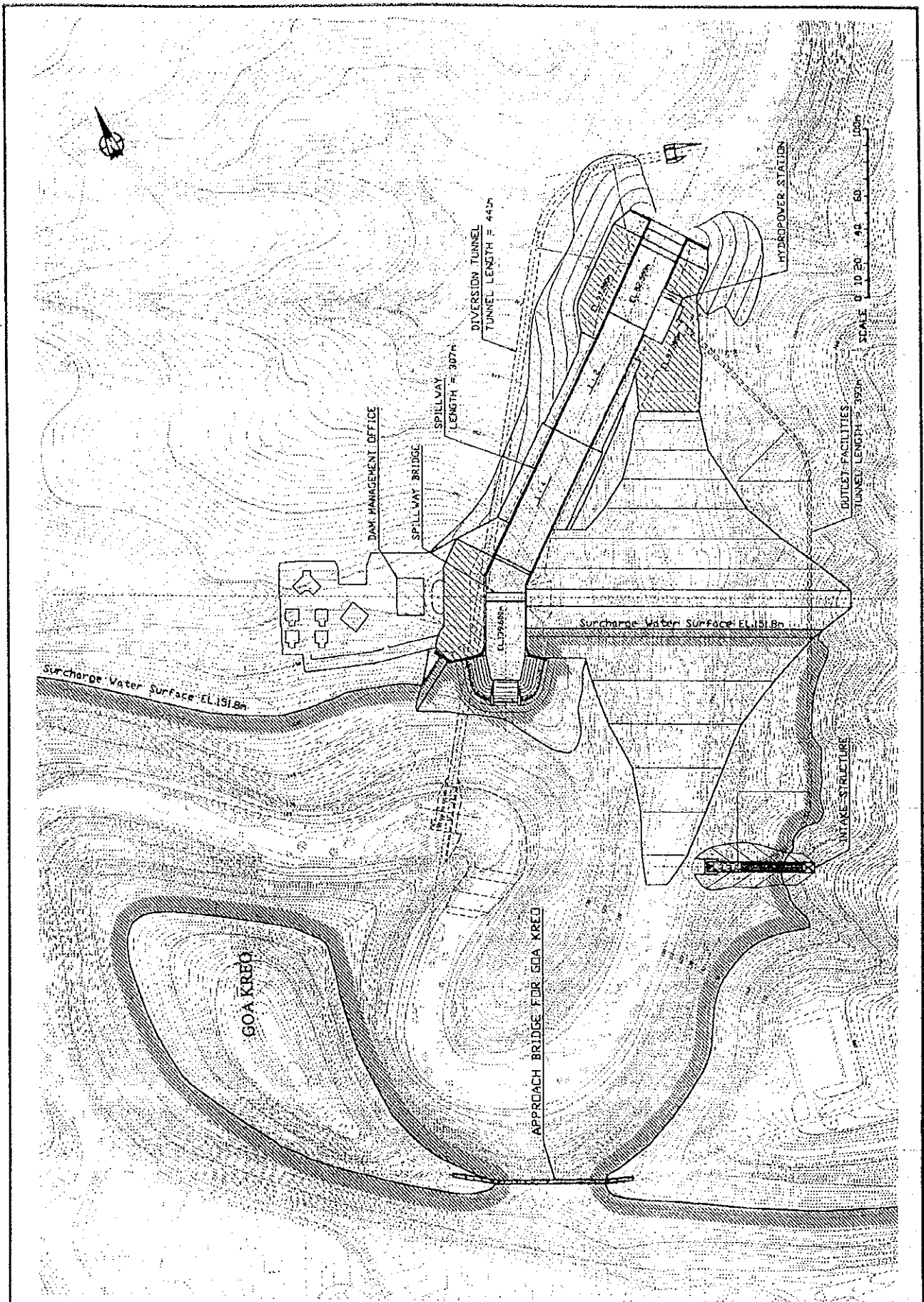
Fig. 6.2.29  
SITE PLAN OF DAM MANAGEMENT COMPLEX



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 IN SEMARANG IN THE REPUBLIC OF INDONESIA

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Fig. 6.2.30  
 SITE PLAN OF HIDRO POWER STATION COMPLEX



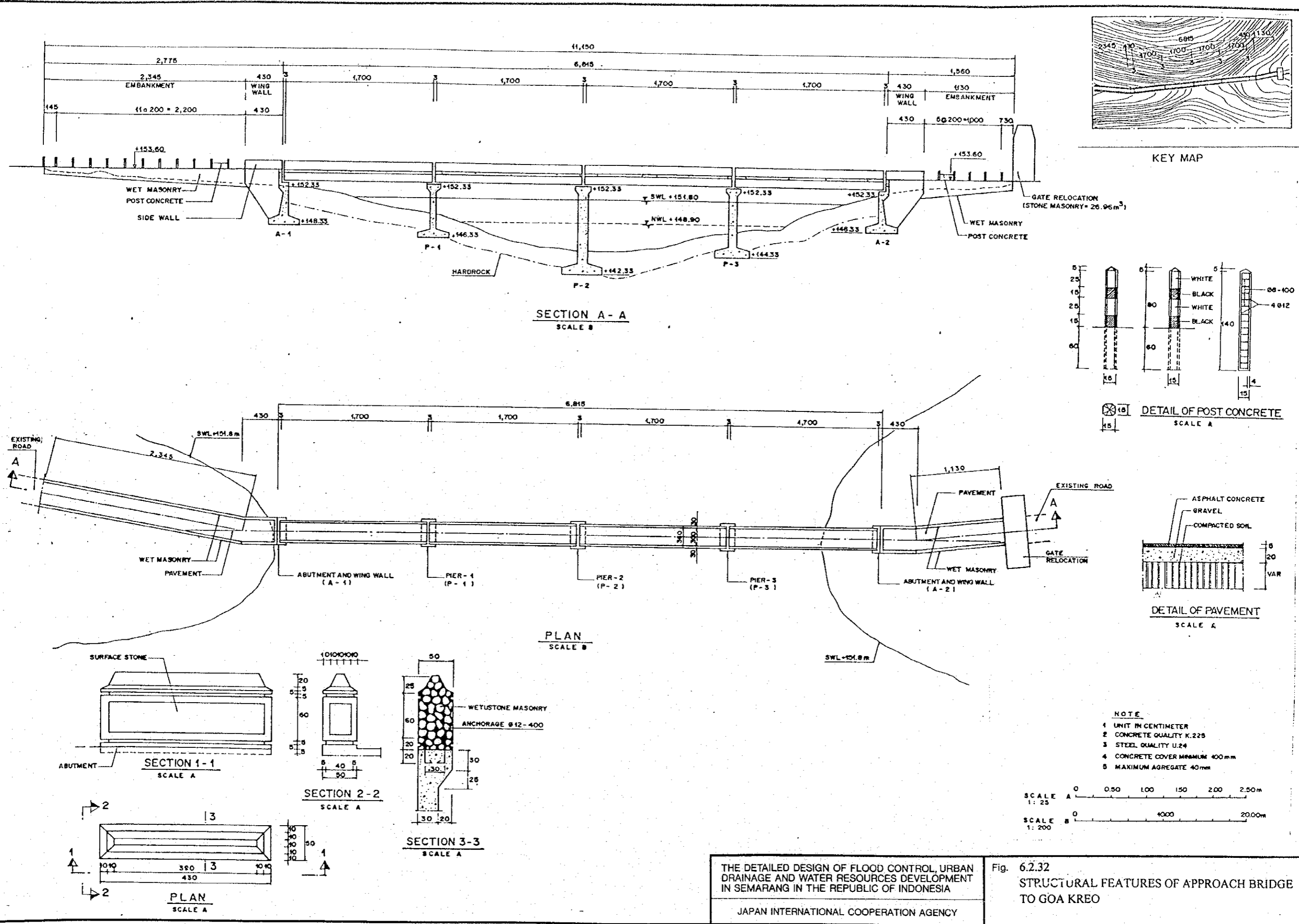
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Fig. 6.2.31

LOCATION OF APPROACH BRIDGE TO GOA KREO

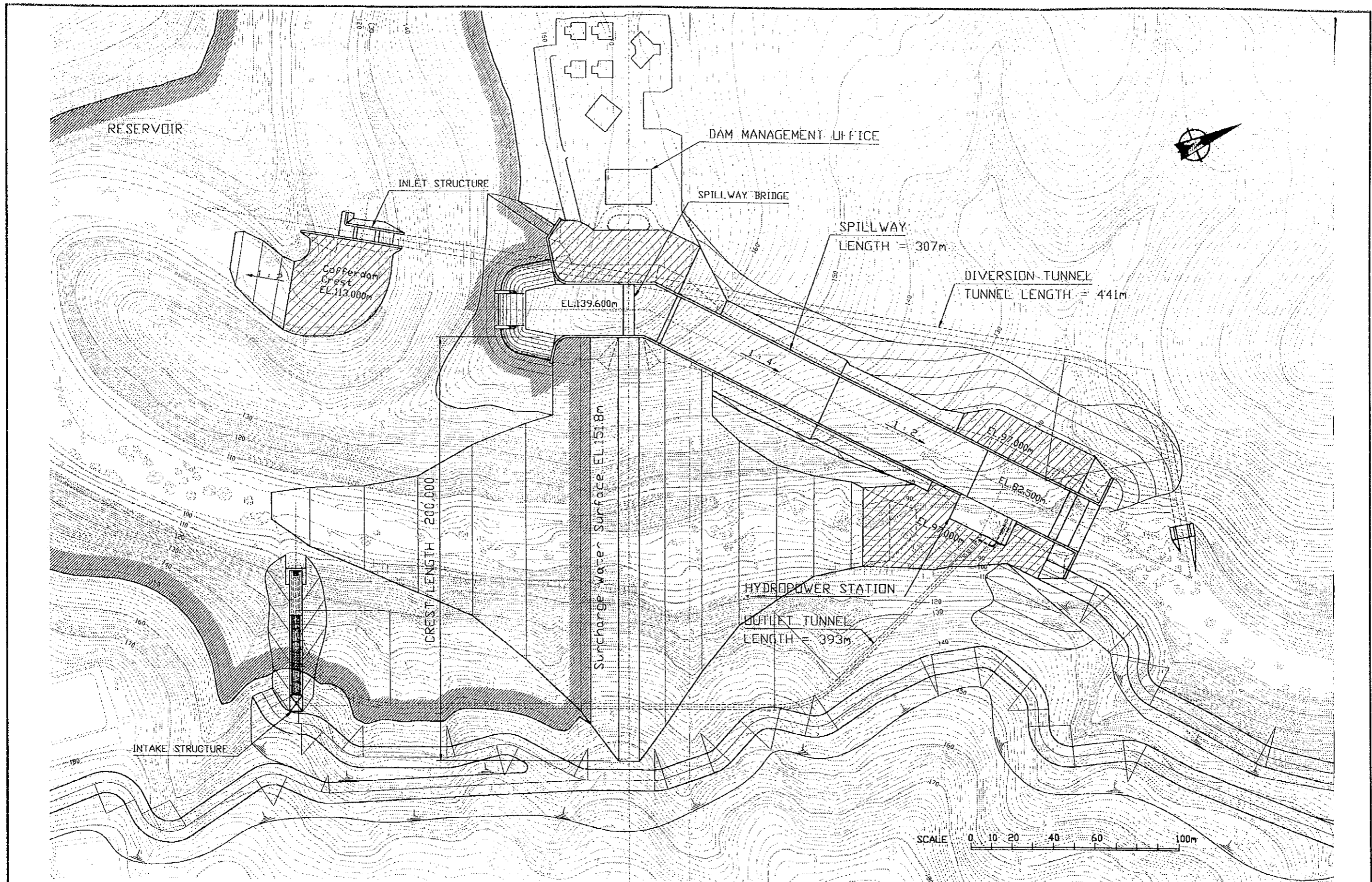




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Fig. 6.2.32  
STRUCTURAL FEATURES OF APPROACH BRIDGE TO GOA KREO

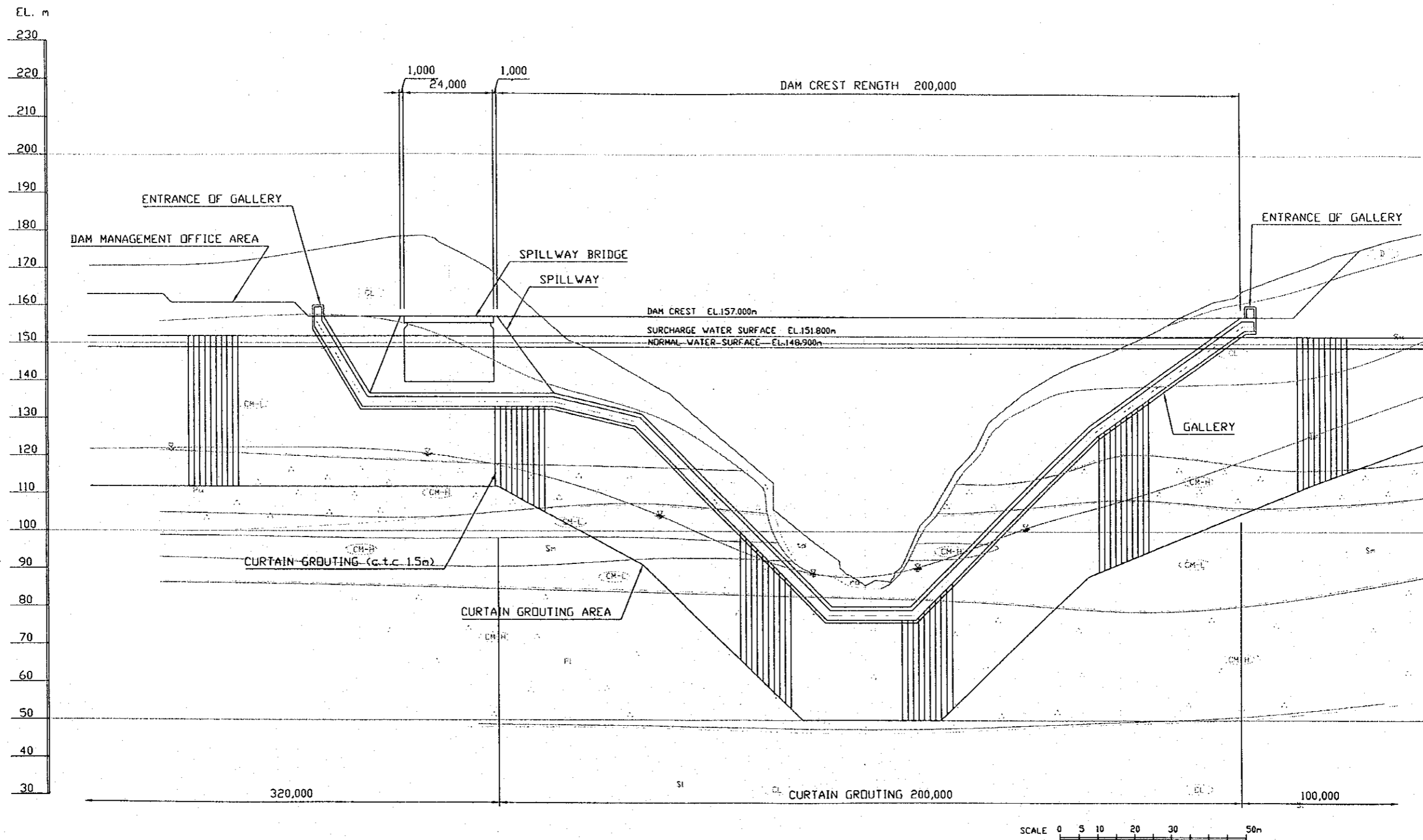


THE DETAILED DESIGN OF FLOOD CONTROL, URBAN DRAINAGE AND WATER RESOURCES DEVELOPMENT IN SEMARANG IN THE REPUBLIC OF INDONESIA

Fig. 6.2.33  
PLAN OF JATIBARANG MULTIPURPOSE DAM

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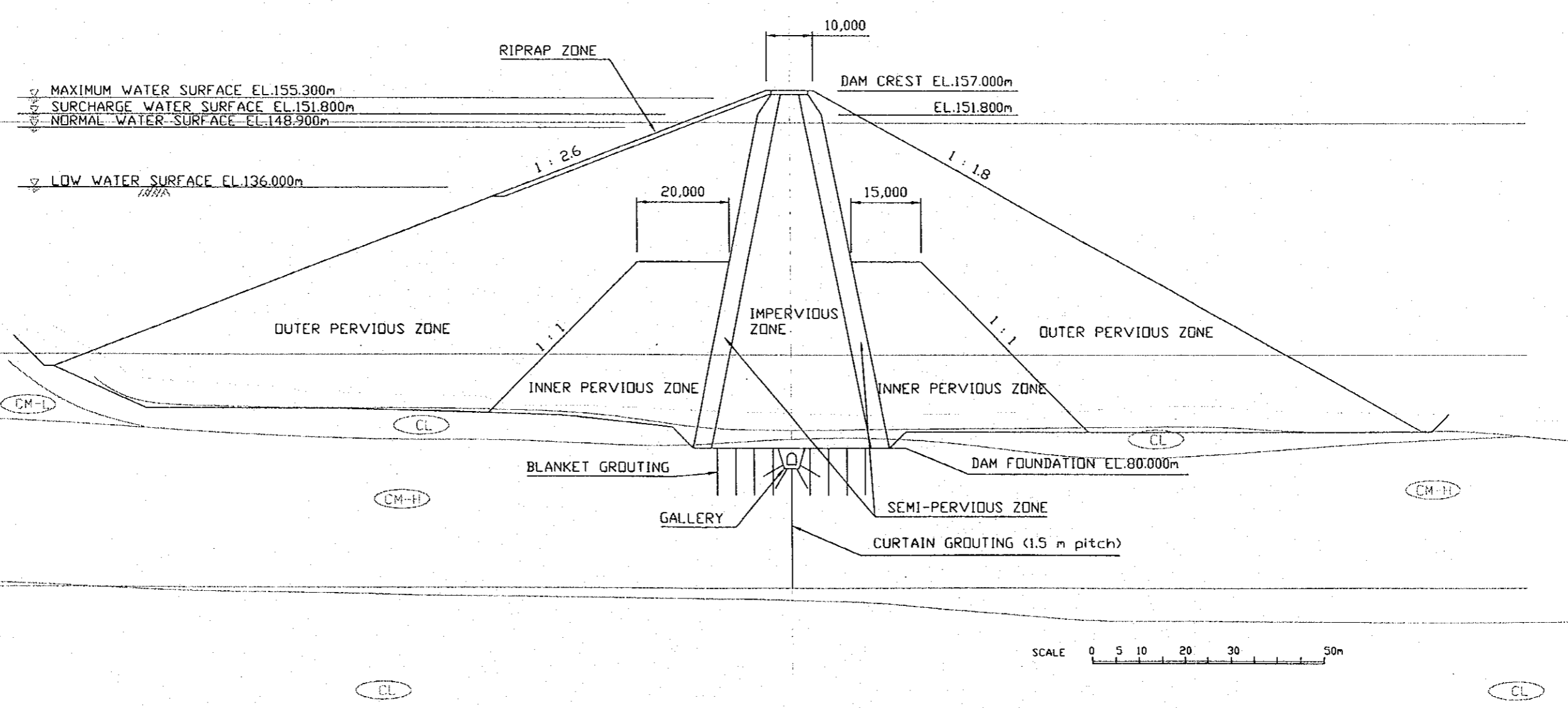


THE DETAILED DESIGN OF FLOOD CONTROL, URBAN DRAINAGE AND WATER RESOURCES DEVELOPMENT IN SEMARANG IN THE REPUBLIC OF INDONESIA

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Fig. 6.2.34  
LONGITUDINAL PROFILE OF JATIBARANG MULTIPURPOSE DAM

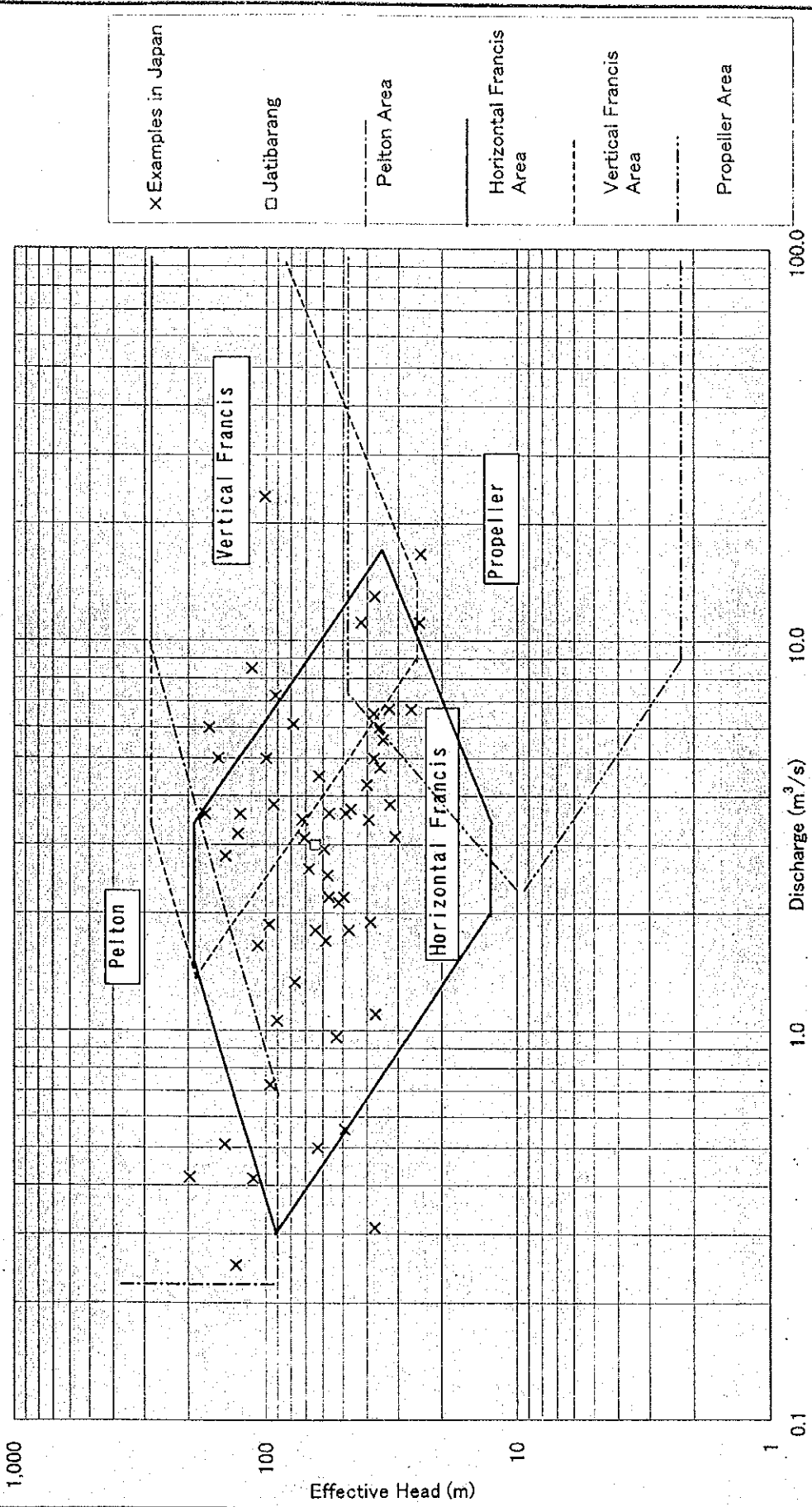
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THE DETAILED DESIGN OF FLOOD CONTROL, URBAN  
 DRAINAGE AND WATER RESOURCES DEVELOPMENT  
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Fig. 6.2.35  
 TYPICAL CROSS SECTION OF JATIBARANG  
 MULTIPURPOSE DAM





x	Examples in Japan
□	Jatibarang
---	Pelton Area
---	Horizontal Francis Area
---	Vertical Francis Area
---	Propeller Area

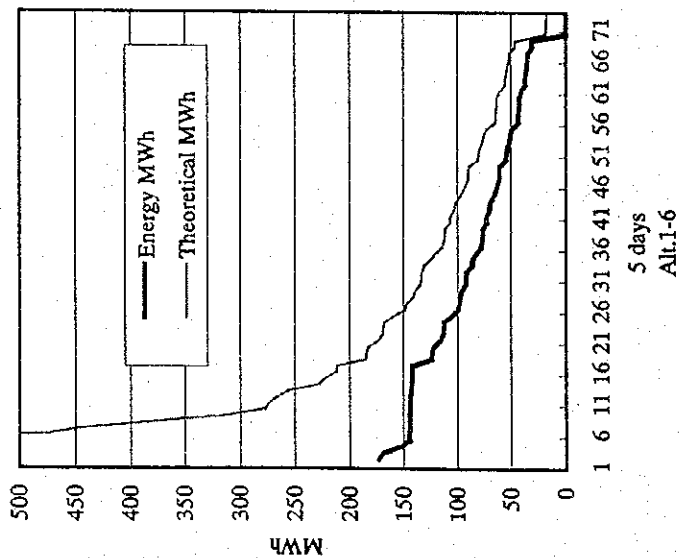
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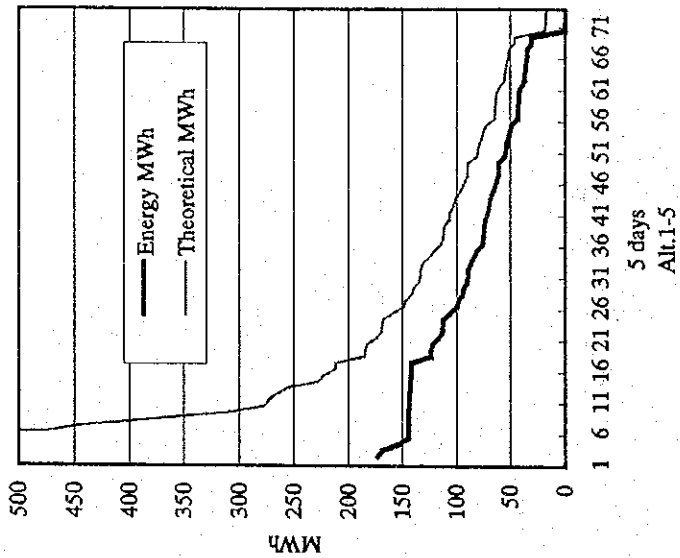
Fig. 6.3.1  
TURBINE TYPE SELECTION DIAGRAM

Energy Production and Allocation of Turbine Discharge for Two Units

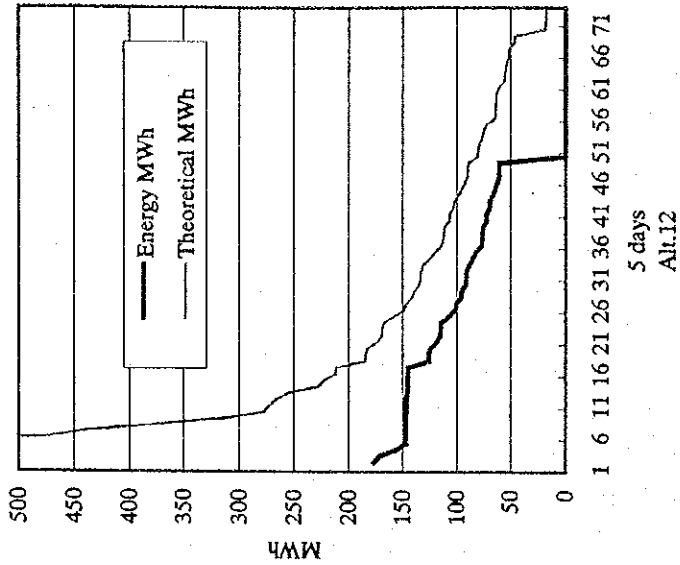
	Discharge(m <sup>3</sup> /s)		Power(kW)		Output(MWh)	
	Unit #1	Unit #2	Unit #1	Unit #2	Energy (Present Stage)	Energy (Future Stage)
Alt.1-6	2.0	1.0	948	463	1,411	8,212
Alt.1-5	1.5	1.5	705	705	1,410	8,183
Alt.12	3.0	0	1,438	0	1,438	8,307



5 days  
Alt.1-6



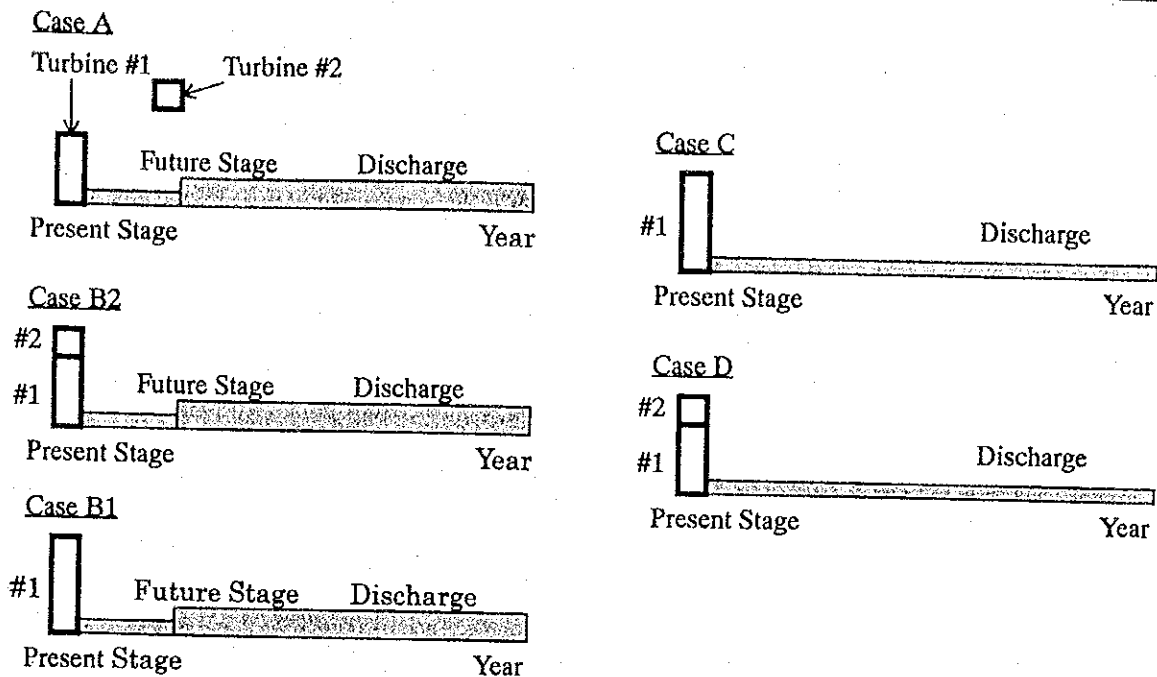
5 days  
Alt.1-5



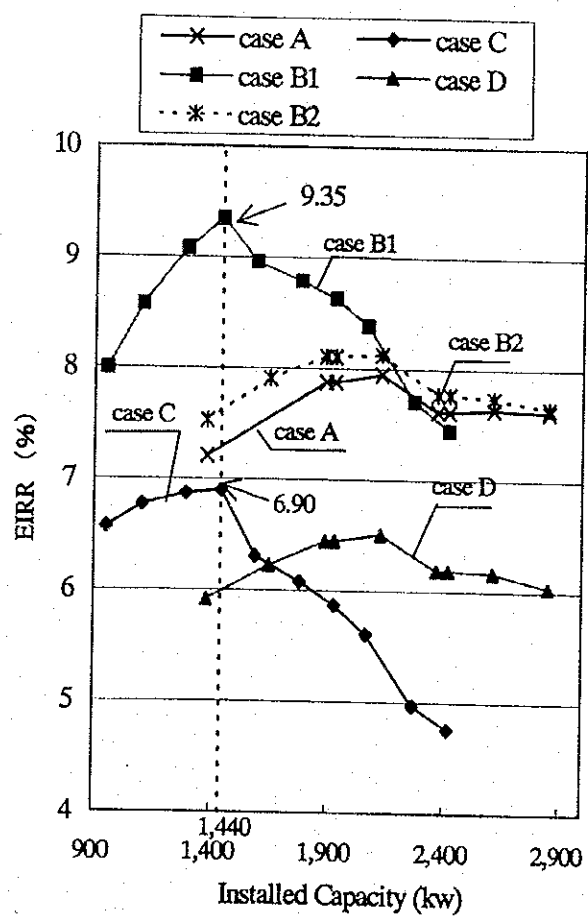
5 days  
Alt.12

Note : Theoretical MWh =  $9.8Q_{out} H_{max} \times 5 \times 24 / 1000$

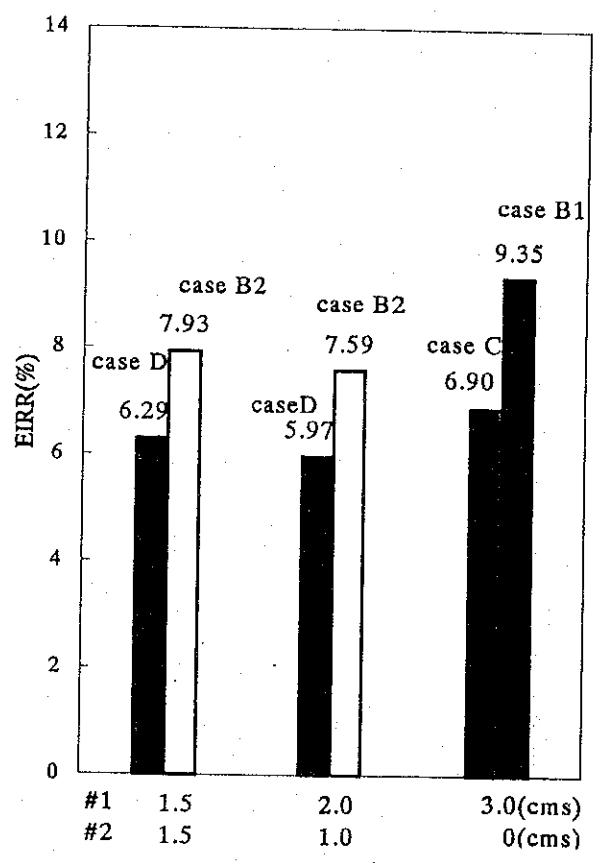
Fig. 6.3.2 ANNUAL DURATION CURVE OF MWH ON FUTURES STAGE OF 1990'S DATA



(a) Stage Development and Turbine Number



(b) Relation between Installed Capacity and EIRR



(c) Variation of EIRR with Changes in Turbine Discharge Allocation