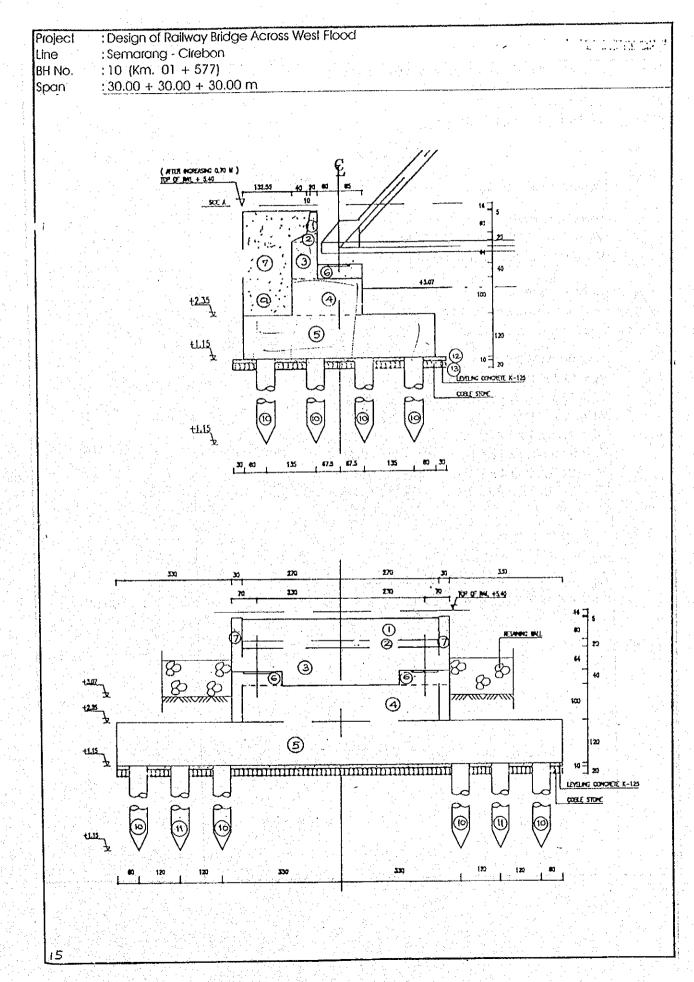


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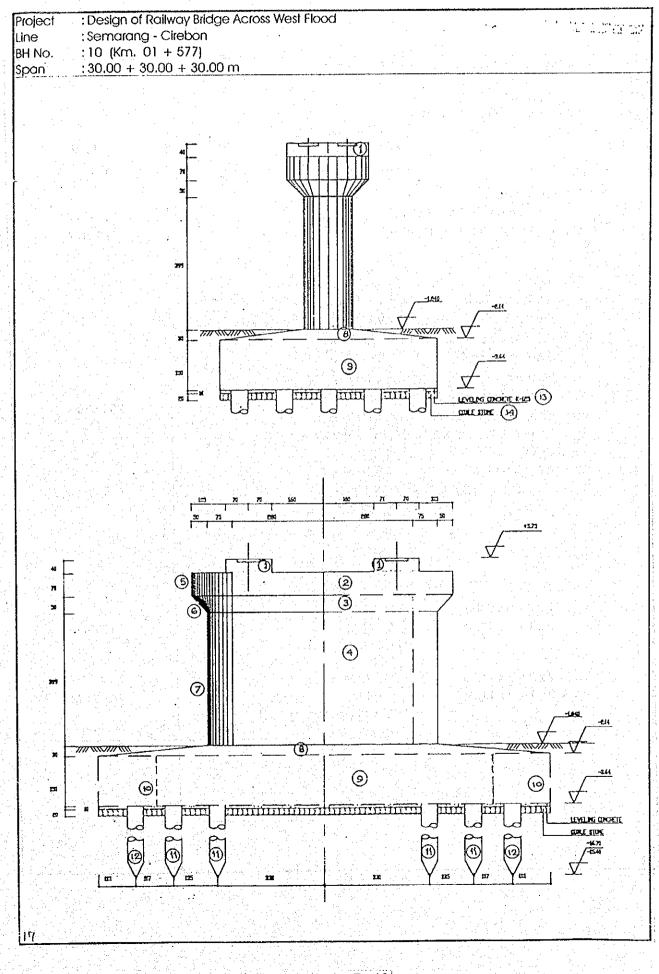
۴	TOTAL	1.137 M <sup>3</sup>
•		
. *		
· .		
	비금 방송 방송 홍수의 중 전 감독에서 많아요?	
	Digging for abutment work = $10 \times 2,20 \times 13 = 286 \text{ M}^3$	286 M <sup>3</sup>
<u>∕c∖</u>	Abutment Sm Side	
∧	Pier masonry = $2 \times 5.8 \times 2.6 \times 5 = 151 \text{M}^3$	151M3
n en	Digging = $2 \times 2 \times 8 \times 14 = 448 \text{M}^3$ Dior mercentric 2 × 6 × 2 × 6 × 14 = 151×4 <sup>3</sup>	448M3
B	Pier Cn and Sm Side	
	Abutment masonry = $1 \times 6 \times 2 = 12 \text{ M}^3$	240 Cm <sup>3</sup> 12 M <sup>3</sup>
	Cirebon Side Soil digging: $8 \times 12 \times 2,50 = 240 \text{ Cm}^3$	
	Demolishing	
	Soil Excavation	an la companya di serie di ser
No.	Calculation	Total
san	: 30.00 + 30.00 + 30.00 m	en de la companya de La companya de la comp
H No.	: 10 (Km. 01 + 577)	



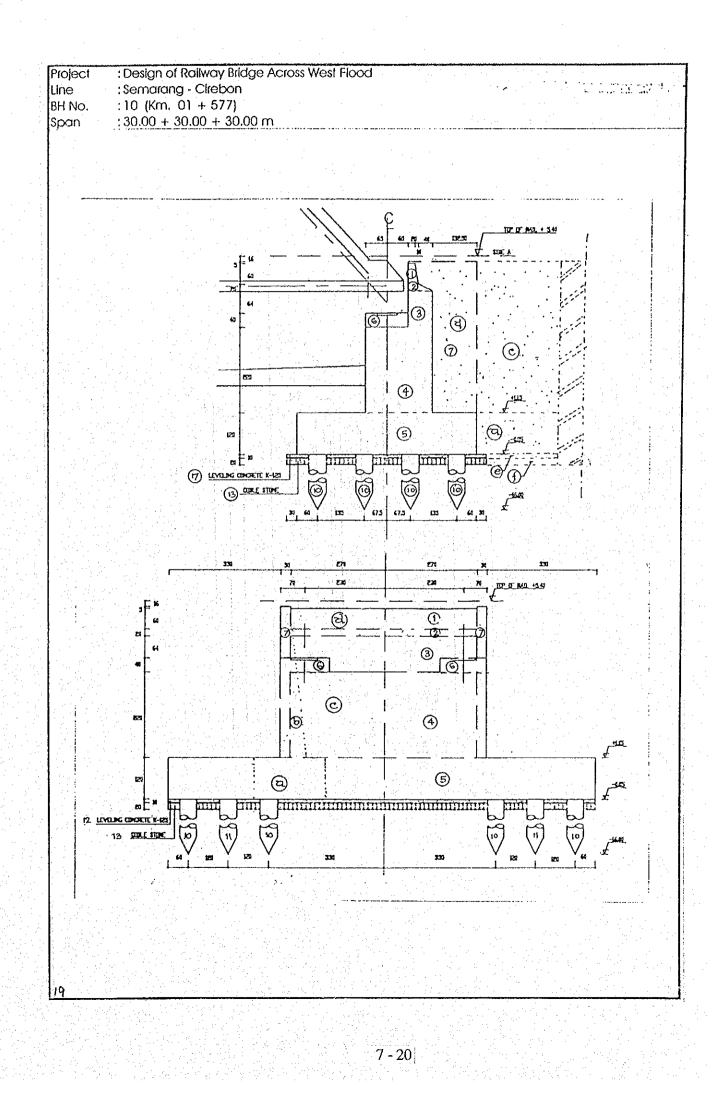
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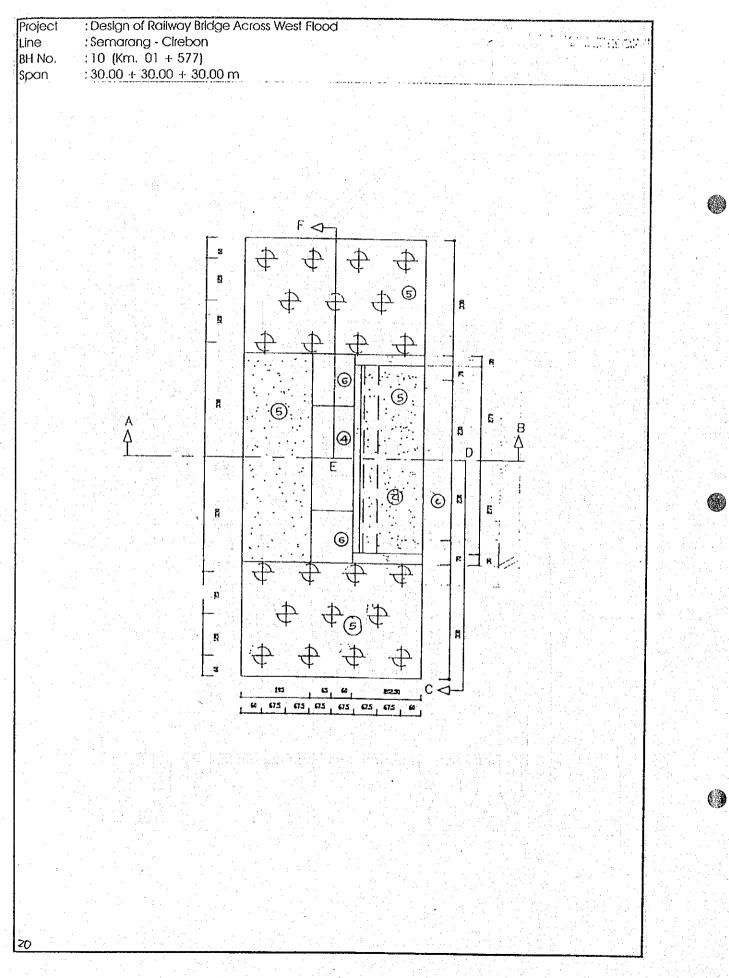
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Project	: Design of Railway Bridge Across West Flood		· · · · · · ·
Line	: Semarang - Cirebon		
BH No.	:10 (Km. 01 + 577)		
Span	: 30.00 + 30.00 + 30.00 m		<u> </u>
No.	Calculation		Total
1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 -	Abutment Cirebon Side		te de la construcción de la constru La construcción de la construcción d
	Leveling concrete		7 677 1 43
•	$V_{12} = 0.1 \times 5.85 \times 12.6 = 7.371 \text{ M}^3$		7,371 M <sup>3</sup>
	$V_{13} = 0.2 \times 5.85 \times 12.6 = 14.742 M^3$	·	14,742 M <sup>3</sup>
$\wedge$			47 40 43
<u>ZA</u>	$= 2,84 \times 1,32 \times 4,60 = 17,42 \text{ M}^{-3}$		17,42 M <sup>3</sup>
	1. A set of the set		<u> </u>
	Abutment Cn Side		
	· 같이 가지 않는 것 같은 것 같		
	$V_1 = (0,2 + 0,3) \times 0,5 \times 0,6 \times 5,4 = 0,810 \text{ M}^3$		e la facilita de la comunicación de Comunicación de la comunicación de l
	$V2 = (0,3 + 0,7) \times 0,5 \times 0,2 \times 5,4 = 0,540 \text{ M}^3$		
	$V3 = 0.6 \times 1.04 \times 5.4 = 3.370 \text{ M}^3$		
	$V4 = 1,95 \times 1 \times 5,4$ V4 = 1,95 × 1 × 5,4		00 7 10 13
	$V_5 = 5,25 \times 1,2 \times 12,60 = 79,380 \text{ M}^3$		99,540 M <sup>3</sup>
	$V6 = 2 \times 1,25 \times 0,4 \times 1,4$ = 1,400 M <sup>3</sup>		
	$V7 = 2 \times 2,025 \times 2,89 \times 0,3 = 3,510 \text{ M}^3$		
	$= 99,540 \text{ M}^3$		
	= <b>YY</b> , <b>JU</b> M		
· · · ·		·	<u></u>
	Pile		
	$V_{10} = 2 \times 4 \times 2 \times 10,45 \text{ M}^1 = 167,20 \text{ M}^1$	i de la constaté. El se constaté de la c	
	$V_{11} = 2 \chi 2 \times 2 \times 10.45 M^1 = 41.80 M^1$		209,00 M <sup>1</sup>
na frains a	$= 209,00 \text{ M}^{1}$		207,00 141
	<i>≕ ∠</i> ∪γ,∪∪  V		
н н. 1. н.	1. Second states and the second states in the second states of the second states in the second state in the second states in the second state in the second states in the seco		
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Project	: Design of Railway Bridge Across West Flood	
Line	: Semarang - Cirebon	
BH No.	: 10 (Km. 01 + 577)	
Span	: 30.00 + 30.00 + 30.00 m	······································
No.	Calculation	Tedet
140.		Total
1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 -	Leveling concrete	
	$V_{13} = 0.1 \times 6.6 \times 11.90 = 7.85 \text{ M}^3$	7,85 M <sup>3</sup>
	$V_{14} = 0.2 \times 6.6 \times 11.90 = 15.70 \text{ M}^3$	
		15,70M <sup>3</sup>
1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1		······································
a de la de	Pier I == II	
	$V_1 = 2 \times 1.4 \times 2.7 \times 0.4 = 3.042 \text{ M}^3$	
	$V_2 = 5.6 \times 2.5 \times 0.7$ = 9,800 M <sup>3</sup>	
	$V_3 = 0.5 (5.6 \times 2.5 + 5.6 \times 1.5) = 5,500 \text{ M}^3$	
	$V_4 = 5.6 \times 3.99 \times 1.5$ = 33.516 M <sup>3</sup>	an shi ta shi ka sha shi sa shi s
	$V_4 = 0.0 \times 3.77 \times 1.3$ $V_5 = 2 \times 0.7 \times 3.14 \times 1.25^2$ = 6,869 M <sup>3</sup>	
	· · · · · · · · · · · · · · · · · · ·	
	$V_6 = 0.5 (3.14 \times (1.25 + 0.752)) \times 0.5 = 3.336 \text{ M}^3$	206,530 M <sup>3</sup>
	$V7 = 2 \times 3.14 \times 0.75^2 \times 3.99 = 14,095 \text{ M}^3$	
	$V_8 = 0.5 (5.6 \times 1.5 + 2 \times 3.14 \times 0.75^2 + 9.9 \times 6.6 + 0.5 \times 1.5 \times 10^{-10} \text{ m}^{-10} $	
	$(6,6 \times 2) \times 0,3 = 12,580 \text{ M3}$	
	$V_{9} = 9.9 \times 1.5 \times 6.6 = 98.10 \text{ M}^{3}$	
	$V_{10} = 2 \times 0.5 \times 6.6 \times 2 \times 1.5 = 19,800 \text{ M}^3$	
	$= 206,530 \mathrm{M}^3$	
· · · .	Pile	
$(x_1, \dots, x_n) \in \mathbb{R}^n$		
· · ·	$V_{11} = 2 \times 2 \times 5 \times 13,06$ = 261,2 M	
	$V_{12} = 2 \times 1 \times 2 \times 13,06$ = 52.24 M	313,44 M
	=313,44 M (Cirebon Side)	
	$V_{11} = 2 \times 2 \times 5 \times 11,76 = 235,20 \text{ M}$	
• • •	$V_{12} = 2 \times 1 \times 2 \times 11,76$ = 47,04 M	282,24 M
	= 282,24 M (Semarang Side)	
· · · · · · · · · · · · · · · · · · ·	n <u>en en e</u>	
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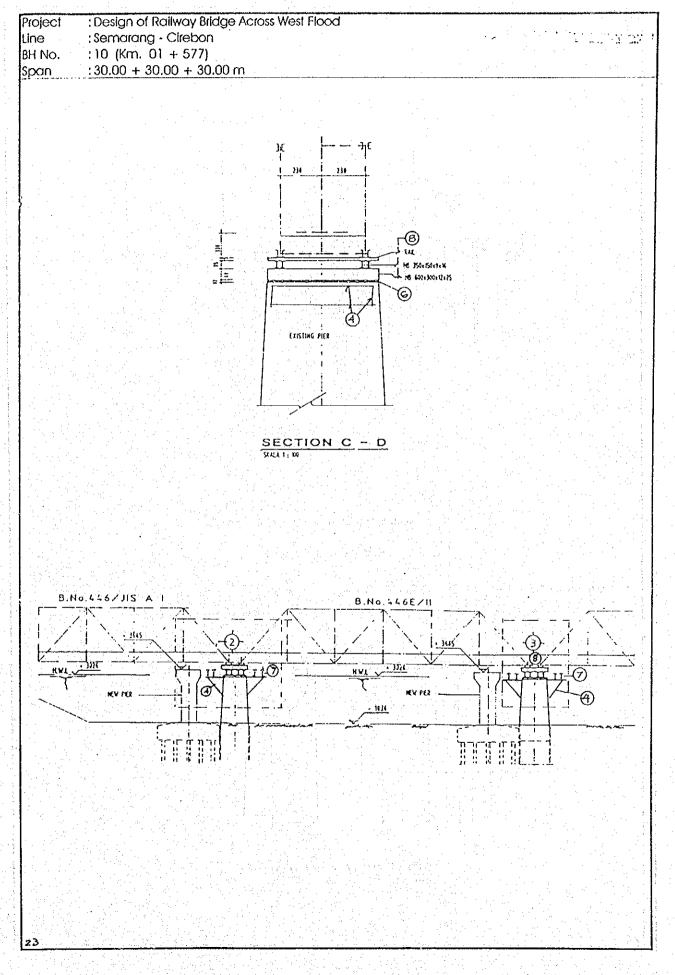


Project Line	: Design of Railway Bridge Across Wes : Semarang - Cirebon	t Flood	
BH NO.	: 10 (Km. 01 + 577)		
Span	: 30.00 + 30.00 + 30.00 m		in an
No.		ation	Τοτοι
	Ballast box connector		
	Foundation = $2 \times 1.75 \times 2.2 \times 1.2 = 9$	,24 M <sup>3</sup>	9,24 M <sup>3</sup>
B	Cobble stone = $0,5(5,4 + 4,5) \times 3,6$	x 3,5 = 62,37 M <sup>3</sup>	62,37 M <sup>3</sup>
	$e = 2 \times 0.1 \times 02.0 \times 1.75 = 0.7 M^3$		0.7 M <sup>3</sup>
	$j = 2 \times 0.2 \times 2.0 \times 1.75 = 1.4 \text{ M}^3$		1,4 M <sup>3</sup>
	Abutment Sm Side	0.010.143	
	$V_1 = (0.2 + 0.3) \times 0.5 \times 0.6 \times 5.4$	$= 0.810 \text{ M}^3$	
	$V_2 = (0.3 + 0.6) \times 0.5 \times 0.2 \times 5.4$	$= 0.486 \text{ M}^3$	
	$V_3 = 0.6 \times 1.04 \times 5.4$	$= 3.370 M^3$	
	$V_4 = 1.95 \times 2.2 \times 5.4$	$= 23,166 \text{ M}^3$	113,501 M <sup>3</sup>
	$V_5 = 5,25 \times 12,60 \times 1,2$	$= 79,380 \text{ M}^3$	
	$V_{6} = 2 \times 1,25 \times 0,4 \times 1,4$	$= 1,400 \text{ M}^3$	
	V <sub>7</sub> = 2 x 2,025 x 4,09 x 0,3	$= 4.969 \text{ M}^3$	
		113,501 M <sup>3</sup>	
	Pile		
	$V_{10} = 2 \times 4 \times 2 \times 17.20$	= 275,20 M	
	$V_{10} = 2 \times 4 \times 2 \times 17.20$ $V_{11} = 2 \times 3 \times 1 \times 17.20$	= 273,20  M = 103,20 M	378,40 M
	Υ <sub>11</sub> - 2 × 3 × 1 × 17,20	= <u>103,20 M</u> = 378,40 M	
	Leveling		
	$V_{12} = 0.10 \times 12.6 \times 5.85$	= 7.371 M	
	$V_{13} = 0.20 \times 12.6 \times 5.85$	<u>= 14.742 M</u>	22.113 M
	Total	=22.113 M	
71			

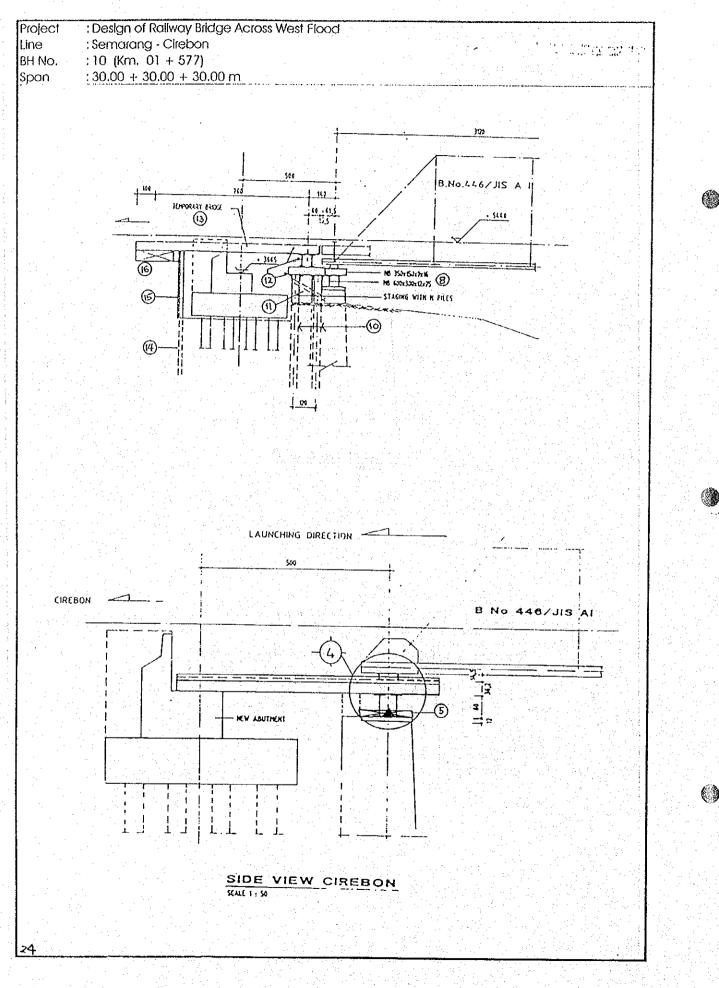
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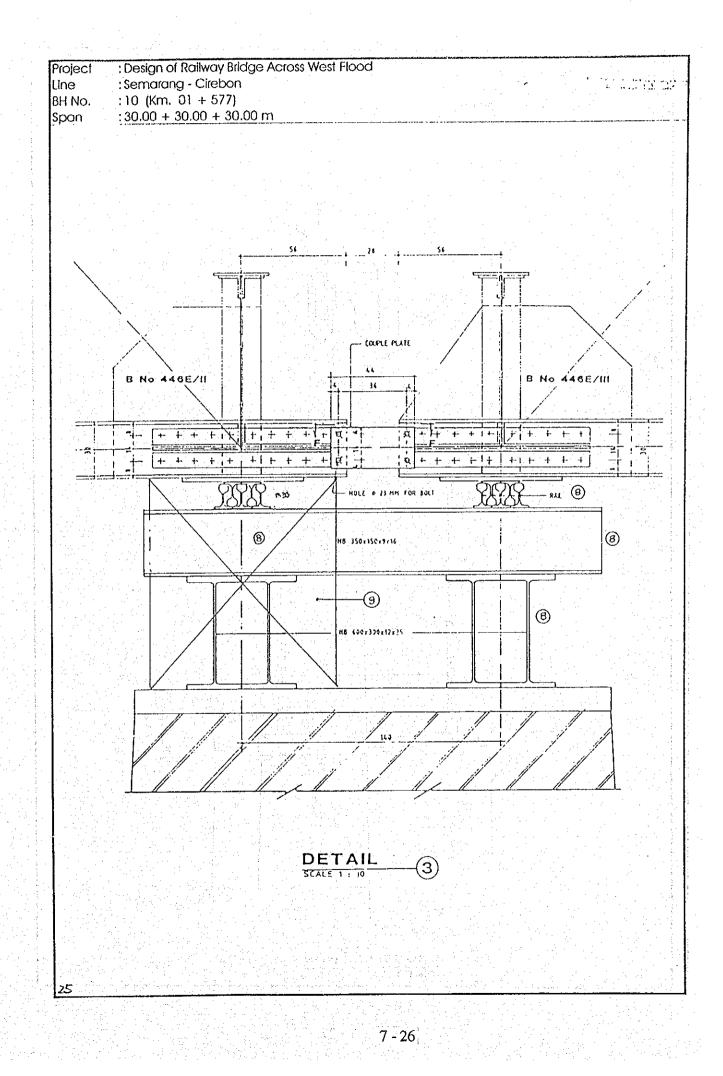
Project Line BH No. Span	: Design of Railway Bridge Ac : Semarang - Cirebon : 10 (Km. 01 + 577) : 30.00 + 30.00 + 30.00 m		
No.		Colculation	Total
	Recapitulation		
]	Leveling concrete		
	- Abutment Cirebon Side	= 7.371 M3 = 14.742 M3	
	- Pier Cirebon Side	= 7.850 M3 = 15.700 M3	
	- Pier Semarang Side	= 7.850 M3 = 15.700 M3	91.326 M <sup>3</sup>
	- Abutment Semarang Side Total	= 7.371 M3 = <u>14.742 M3</u> = 91.326 M3	
2	Concrete K. 225		
	- Abutment Cirebon Side - Pier Cirebon Side - Pier Semarang Side - Abutment Semarang Total	$= 99.540 \text{ M3} \\= 206.530 \text{ M3} \\= 206.530 \text{ M3} \\= 113.501 \text{ M3} \\= 626.101 \text{ M3}$	625.101 M <sup>3</sup>
3	Pile		
	- Abutment Cirebon Side - Pier Cirebon Side - Pier Semarang Side - Abutment Semarang Side Total	= 209.00 = 313.44 = 284.24 <u>= 378.40</u> = 1183.00 M	1183.00 M

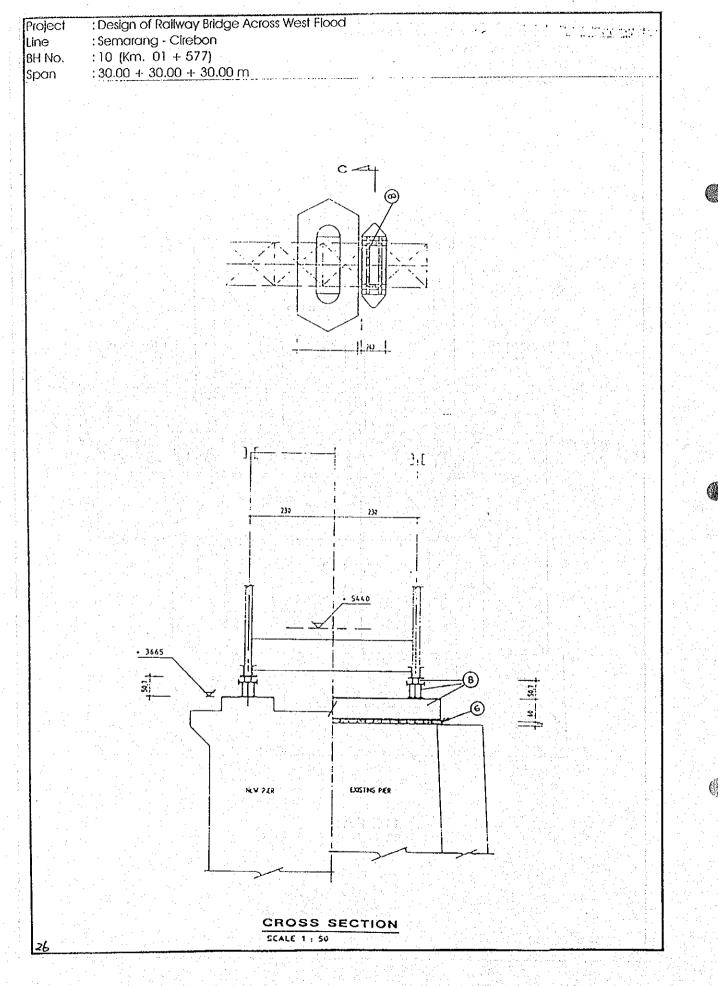
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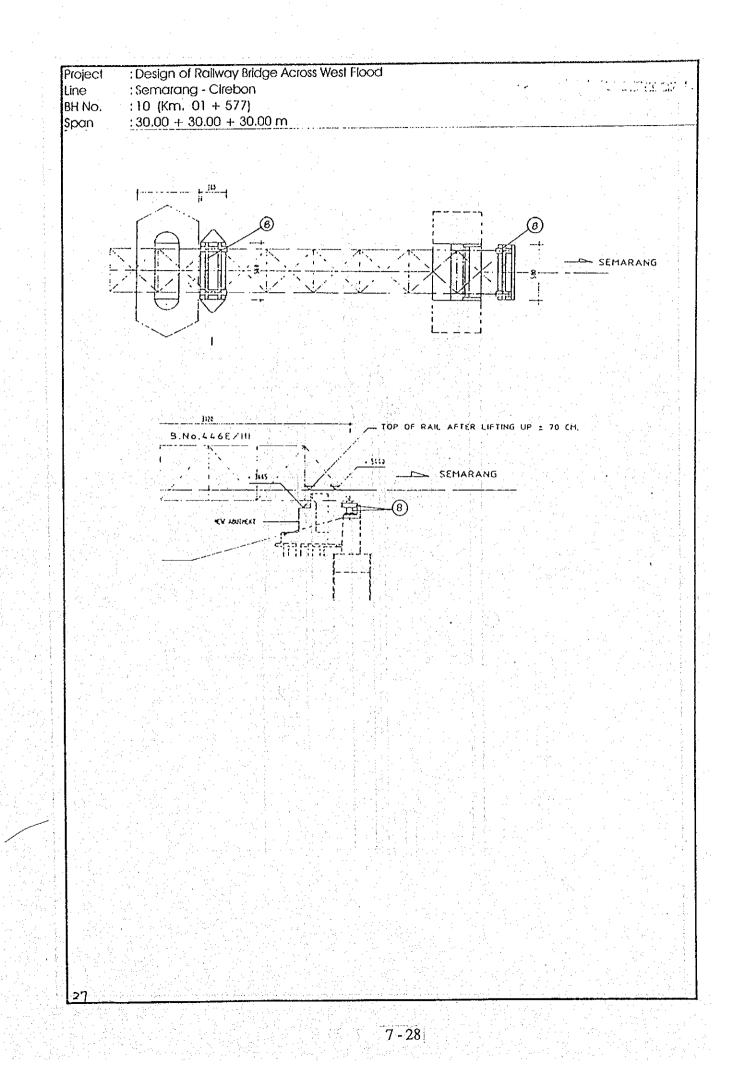


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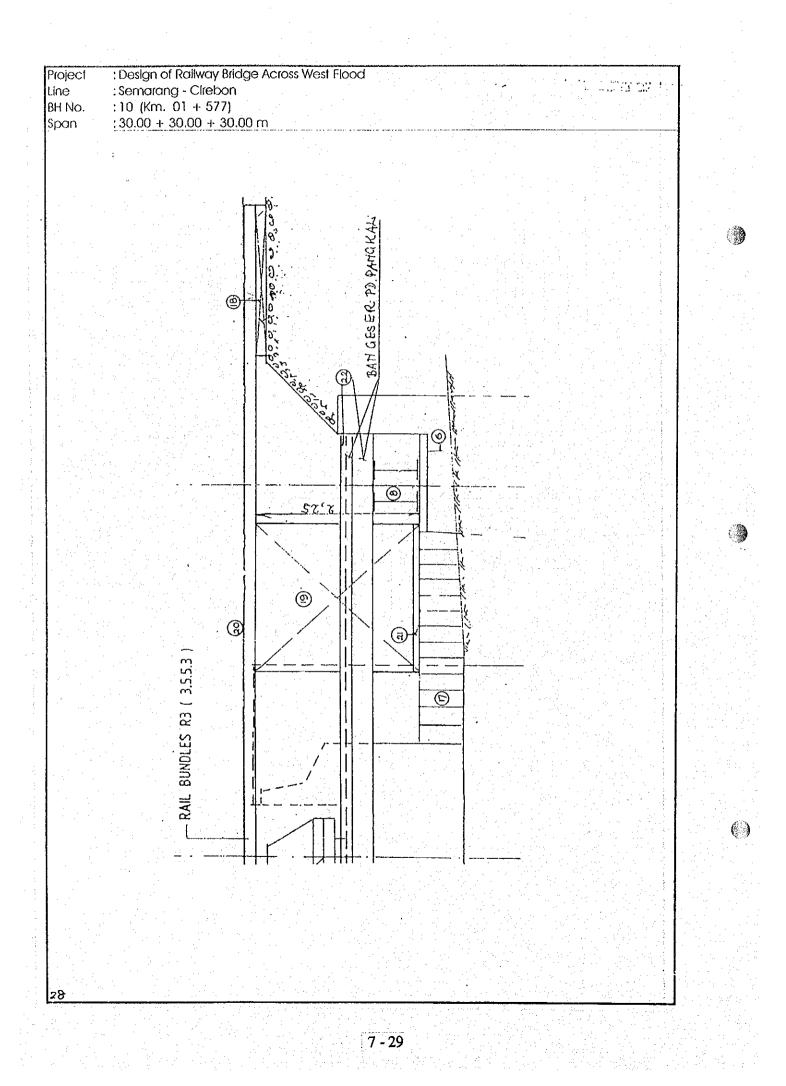


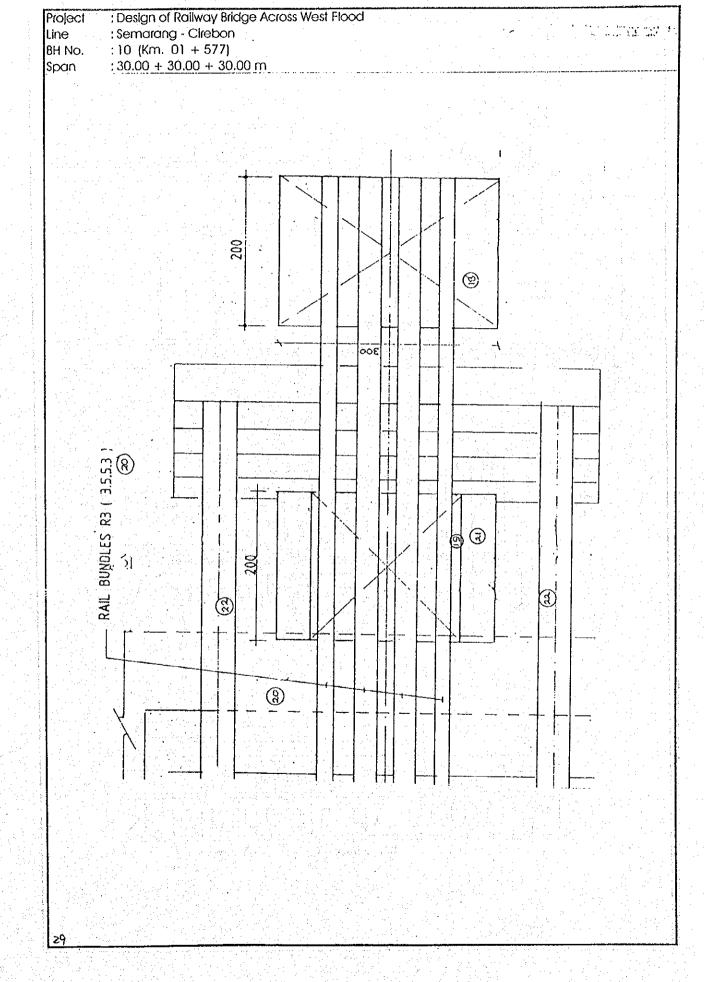




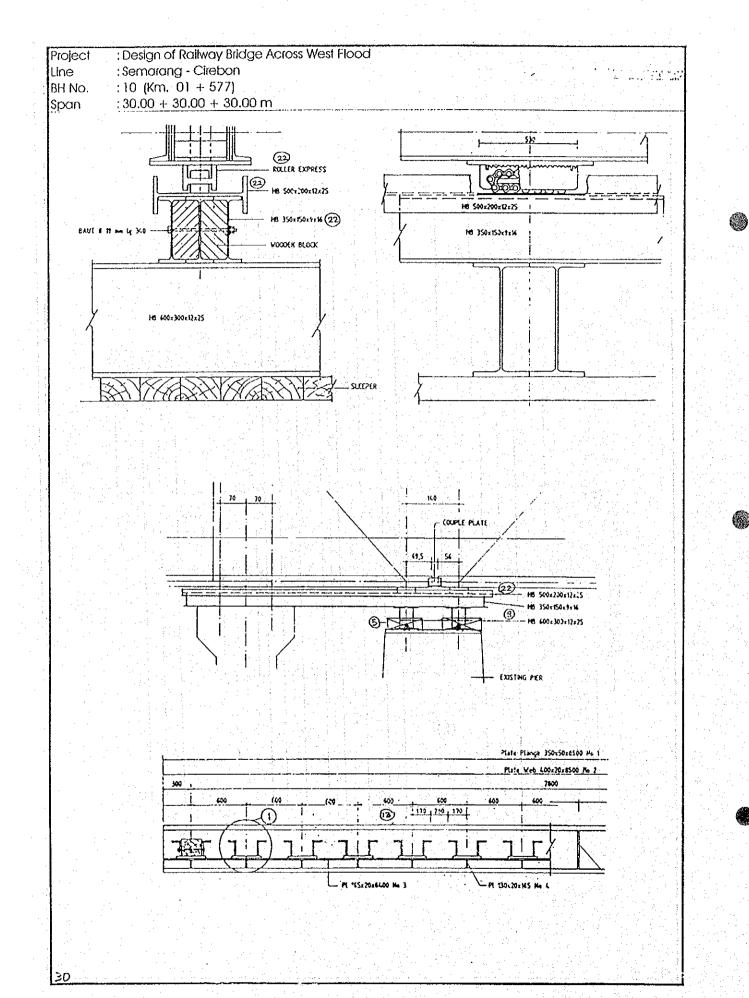


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Project Line	: Design of Railway Bridge Across West Flood	<u></u>
BH No.	: Semarang - Cirebon : 10 (Km, 01 + 577)	
Span	30.00 + 30.00 + 30.00 m	
		1
No.		Total
	PLAT FORM including the materials	
	At pler No. 2 = 2 Unit Abutment Cn Side = 1 Unit	
	At pier No. $3 = 2$ Unit Abutment Sm Side = 1 Unit	6 Unit
	and the second secon	
$\sqrt{5}$	At Cn side abutment = 2 Pieces = $1/2 \times 3 \times 2 \times 2 = 6$ Bars	
	At pier No. 2 Cm side = 4 Pieces = $1/2 \times 3 \times 2 \times 4 = 12$ Bars	12 Piece
	At pier No. 3 Cm side = 4 Pieces = $1/2 \times 3 \times 2 \times 4 = 12$ Bars At Sm side abutment = 2 Pieces = $1/2 \times 3 \times 2 \times 4 = 6$ Bars	36 Bars
	$\frac{1}{2} = \frac{1}{2} \times \frac{1}$	
	Sleeper matress above pier / abutment	
a an		
$\sqrt{6}$	At Cn side abutment = $1 \times 5.8 = 5.8 \text{ m}^2$	
	At pier No. 2 = $2.6 \times 5.8 = 15.08 \text{ m}^2$	41,76 m <sup>2</sup>
	At pier No. $3 = 2.6 \times 5.8 = 15.08 \text{ m}^2$	
	Sleeper = $26 + 1/2 \times 26 + 1/2 \times 26 + 26 = 78$ Bars	78 Bars
t sata		
	Preparing cross supporter	
	Before tt's set, cross supporter prepared in front of pier / abutment on the platform	
<u> </u>	- Cn side abutment	
	HB. $600 \times 300 \times 12 \times 15$ , Length 5,8 m x 2 Bars x 0,177 Ton = 2,053 Ton	
	HB. 300 x 150 x 9 x 16, Length 1,30 m x 4 Bars x 0,0774 Ton = 0,403 Ton R33 rail, Length 5,8 m x 5 Bars x 0,033 Ton = 0,957 Ton	
	R33 rail, Length 5,8 m x 5 Bars x 0,033 Ton $= 0.957$ Ton 3,413 Ton	
	- At pier No, 2	1
	HB. 600 x 300 x 12 x 25, Length 5,8 m x 4 Bars x 0,177 Ton = 4,106 Ton	20,476 Tor
		==,
	HB. 350 x 150 x 9 x 16, Length 2,60 m x 4 Bars x 0,0774 Ton = 0,805 Ton	and the second second
	HB. $350 \times 150 \times 9 \times 16$ , Length 2,60 m x 4 Bars x 0,0774 Ton = 0,805 Ton   R33 rail, Length 5,8 m x 5 Bars x 0,033 Ton x 2	and Angelia ang
	HB. $350 \times 150 \times 9 \times 16$ , Length 2,60 m x 4 Bars x 0,0774 Ton = 0,805 Ton   R33 rail, Length 5,8 m x 5 Bars x 0,033 Ton x 2 $=$ 1,914 Ton $6,825$ Ton	
	HB. $350 \times 150 \times 9 \times 16$ , Length 2,60 m x 4 Bars x 0,0774 Ton = 0,805 Ton   R33 rail, Length 5,8 m x 5 Bars x 0,033 Ton x 2	

Project	: Design of Railway Bridge Across West Flood	
ine	; Semarang - Clrebon	
3H No,	: 10 (Km. 01 + 577)	
Span	: <u>30.00 + 30.00 + 30.00 m</u>	
No.	Calculation	Total
	Bridge roising for 70 Cm height	
	Implemented in 6 steps It's needed the sleeper : - At abutment 2 x 1/2 Bar x 4 Layer x 3 Rows x 2 = 24 Bars 2 x 1Bar x 4 Layer x 3 Rows x 2 = 48 Bars - At pier 2 x 1/2 Bar x 4 Layer x 3 Rows x 4 = 48 Bars 2 x 1Bar x 4 Layer x 3 Rows x 4 = 96 Bars	6 Steps 206 Bars
	Piling HB. 400 x 300 x 12 x 15	
$\underline{10}$	10a. Pile, Length 8 m x 4 Bars x 0,152 Ton = 4,864 Ton 10b. Pile, Length 8m x 4 Bars x 0,152 Ton = 4,864 Ton	8 Bars 9,728 Ton
	Setting steel stiffener	
$\triangle$	$11^{\circ} + 11^{\circ} = LS. 100 \times 100 \times 10$ , Length 2m x 4 Bars x 2 x 0,0151 Ton	0,242 Ton
	Setting cross girder	
	12a HB. 600 x 300 x 12 x 25, Length 10 M x 2 Bars x 0,177 Ton = 3,54 Ton 12b HB. 400 x 300 x 12 x 25, Length 2 M x 4 Bars x 0,152 Ton = 1,216 Ton	4,756 Ton
	Temporary bridge	
$\sqrt{13}$	Appropriate with weight list in the main drawing	10,559 Jon
	Rail piling for bulkhead	
$\Lambda$	Rail piling for strenghten the bulkhead	2 Pieces

roject	: Design of Railway Bridge Across West Flood	
ine	: Semarang - Cirebon	and a second
ih No.	: 10 (Km. 01 + 577)	
pan	: 30.00 + 30.00 + 30.00 m	· · · · · · · · · · · · · · · · · · ·
No.	Calculation	Total
	Setting the bulkhead	
	Consist of wooden plate size 8 x 12 x 300 Cm, Height 3,5 m = 0,08 x 3 x 3,5 = 0,84 m3	0,84 m <sup>3</sup>
	Construct temporary abutment	
	Temporary abutment for temporary bridge Sleeper = 14 + 6 x 3 = 32 Bars	1 Piece 32 Bors
	Fill work	
	Made for sleeper mattress base of rail bundles - support = $0.6 \times 2.8 \times 5 = 8.4 \text{ m}3$	8,4 m <sup>3</sup>
	Temporary abutment	
<u>A</u>	Made for rail bundles position = 1 Piece Sleeper = $15 + 6 = 21$ Bars	1 Piece 21 Bars
	Sleeper mattress for rail bundles	
	Made for supporting in the middle of rail bundles, height= 2,25 M Sleeper = $3 \times 17$ Layer = $51$ bars	2,25 M 51 Bars
	Rail Bundles	
20	Consist of 10 m length of R3, for 16 Bars Weight = $16 \times 10 \times 0.033$ Ton = 5,280 Ton Become 4 lines = $3 + 5 + 5 + 3$ bars	4 Lines 5,280 Ton

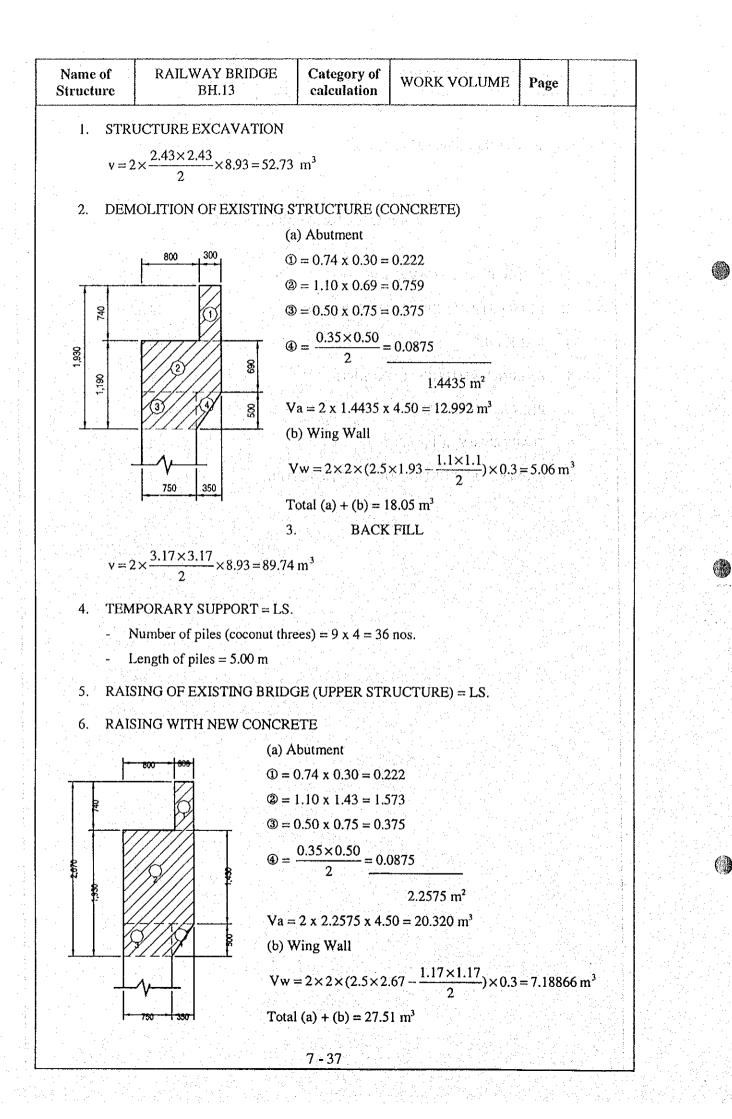
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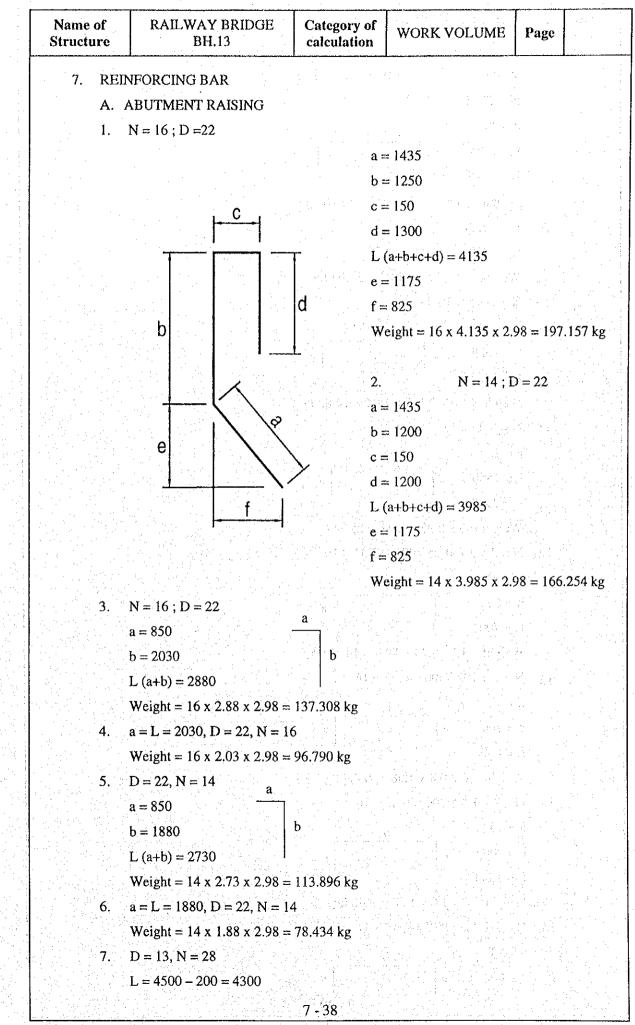
ject	: Design of Railway Bridge Across West Flood	in here in a set of
e	Semarang - Cirebon	
No.	: 10 (Km. 01 + 577)	
n	: 30.00 + 30.00 + 30.00 m	이네 안 같은 것같이?
		· · · · · · · · · · · · · · · · · · ·
NO.	Calculation	Total
т. 1 т. т.		
	Setting sleeper matress	
~		
21	For base stapling of rail bindles	14 Bars
	Sleeper = 14 Bars = $2 \times 3 \text{ m} = 6 \text{ m}^2$	6 m²
	Construct shifting tire	
22	Shifting tire made from HB.	
••••••••••••	Al Sm side abutment = 2 Unit	
	HB. 350 x 150 x 9 x 16, Length 7 m x 3 Bors x 2 = 3,251 Ton	
	HB. 500 x 200 x 12 x 25, Length 7 m x 1 Bor x 2 = 1,785 Ton	8 Unit
5. <u>.</u>	At Pier I = 2 Unit	OUIII
		しょう うち しょうようかん たいしょう
· · · ·	HB. $350 \times 150 \times 9 \times 16$ Length 8 m x 3 Bors x 2 = 3.751 Top	01 504 Too
	HB. $350 \times 150 \times 9 \times 16$ ; Length 8 m x 3 Bors x 2 = 3,751 Ton HB. $500 \times 200 \times 12 \times 25$ Length 8 m x 1 Bors x 2 = 2.041 Top	21,584 Ton
	HB. 500 x 200 x 12 x 25, Length 8 m x 1 Bar x 2 = 2,041 Ton	21,584 Ton
	HB, 500 x 200 x 12 x 25, Length 8 m x 1 Bar x 2 = 2,041 for At Pier II = 2 Unit = 5,756 for	21,584 Ton
	HB. 500 x 200 x 12 x 25, Length 8 m x 1 Bar x 2 = 2,041 Ton	21,584 Ton
	HB, 500 x 200 x 12 x 25, Length 8 m x 1 Bar x 2 = 2,041 for At Pier II = 2 Unit = 5,756 for	21,584 Ton
	HB, 500 x 200 x 12 x 25, Length 8 m x 1 Bar x 2 = 2,041 for At Pier II = 2 Unit = 5,756 for	21,584 Ton
	HB, 500 x 200 x 12 x 25, Length 8 m x 1 Bar x 2 = 2,041 for At Pier II = 2 Unit = 5,756 for	21,584 Ton
	HB, 500 x 200 x 12 x 25, Length 8 m x 1 Bar x 2 = 2,041 for At Pier II = 2 Unit = 5,756 for	21,584 Ton
	HB, 500 x 200 x 12 x 25, Length 8 m x 1 Bar x 2 = 2,041 for At Pier II = 2 Unit = 5,756 for	21,584 Ton
	HB, 500 x 200 x 12 x 25, Length 8 m x 1 Bar x 2 = 2,041 for At Pier II = 2 Unit = 5,756 for	21,584 Ton
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	HB, 500 x 200 x 12 x 25, Length 8 m x 1 Bar x 2 = 2,041 for At Pier II = 2 Unit = 5,756 for	21,584 Ton
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	HB, 500 x 200 x 12 x 25, Length 8 m x 1 Bar x 2 = 2,041 for At Pier II = 2 Unit = 5,756 for	21.584 Ton
	HB, 500 x 200 x 12 x 25, Length 8 m x 1 Bar x 2 = 2,041 for At Pier II = 2 Unit = 5,756 for	21.584 Ton
	HB, 500 x 200 x 12 x 25, Length 8 m x 1 Bar x 2 = 2,041 for At Pier II = 2 Unit = 5,756 for	21,584 Ton
	HB, 500 x 200 x 12 x 25, Length 8 m x 1 Bar x 2 = 2,041 for At Pier II = 2 Unit = 5,756 for	21,584 Ton
	HB, 500 x 200 x 12 x 25, Length 8 m x 1 Bar x 2 = 2,041 for At Pier II = 2 Unit = 5,756 for	21,584 Ton

Name o Structur			Page
7.4 Ab	utment (Location : 2K+332 m)		
		an a	
	SUMMARY OF WOR	<u>RK VOLUME</u>	
1. ;	STRUCTURE EXCAVATION	= 53 m	3
2.	DEMOLITION EXISTING STRUCTURE	= 18 m	3
3.	BACK FILL	≈ 90 m	
		- 90 III	
4. ]	RAISING WITH NEW CONCRETE	= 28 m <sup>2</sup>	3
5. ]	REINFORCING BAR	= 3,468 kg	5
6. *	TEMPORARY SUPPORT	= LS	n an an teachtraige Chailtean agus an a
7. 1	RAISING OF EXISTING UPPER STRUCT	URE = LS	

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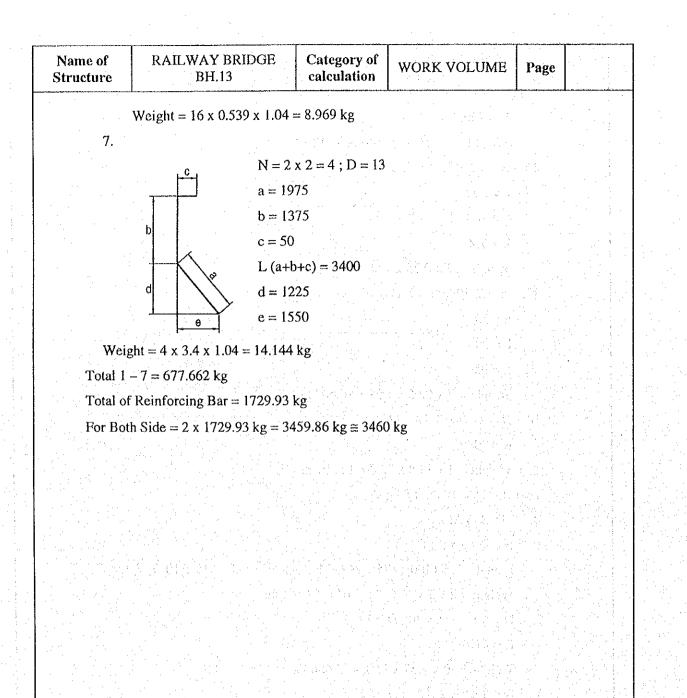




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Name of Structure	RAILWAY BRIDGE BH.13	Category of calculation	WORK VOLUME	Page		
	Weight = $28 \times 4.3 \times 1.04 =$	125.216 kg	n de la companya de Esta de la companya d			
8.	$N = 2 \times 2 = 4, D = 13$		an Ang atalah sa sa sa sa sa sa sa sa sa			• •
	a = 200 b					
· · · ·	b = 1025 a	a				
	L = 1425			4		
	Weight = 4 x 1.425 x 1.04 =	5.928 kg				
9.	D = 13, N = 2					
· · · ·	$L = 2250 + 2 \times 150 = 2550$					a ta t
	Weight = $2 \times 2.55 \times 1.04 = 5$	5.304 kg				
10.	$N = 2 \ge 5 = 10, D = 13$			· · · ·		
	a = 200					
	b = 1025 a	a				n an st
	L = 1425					
	Weight = $10 \times 1.425 \times 1.04$	= 14.82 kg				
11.	N = 5, D = 13					
	a = 200					
	b = 2550 a	a				
	L = 2950					
	Weight = $5 \times 2.95 \times 1.04 = 100$	15.34 kg				
12.	$N = 2 \times 2 \times 1025/300 = 13, 1$	D = 16				
	a == 50 b					
	b = 600 a	a				
	L = 700					
. · · ·	Weight = $13 \times 0.70 \times 1.58 =$	14.378 kg				
13.	N = 2 x 1025/300 = 6, D = 1	6				
	a = 50 b				and and an and an	
	b = 700 a	a				
	L = 800					
	Weight = $6 \times 0.8 \times 1.58 = 7$ .	.584 kg				
14.	$N = 2 \times 1025/300 = 6, D = 1$	6				
and Angel (1997) an an Angel (1997)	a = 50 b	· · · · · · · · · · · · · · · · · · ·				
	b = 900 a	a				
	L = 1,000					
	Weight = $6 \times 1.0 \times 1.58 = 9$ .					n de la de La dela dela La dela dela
15.	$N = 2 \times 2 \times 2250/300 = 30, 1$	D = 16				
	a = 50 b					
	b = 600 a	a				
	经通知的 计算法分子	7 - 39				

	Name of Structure	RAILWAY BRIDGE BH.13	Category of calculation	WORK VOLUME	Page	
an a		L = 700		the state of the A		·
		Weight = $30 \times 0.70 \times 1.58 = 100$	33.18 kg			
	16.	N = 2250/300 = 7, D = 16				
an a		a = 50 b			•	
		b = 700 a	a			
		L = 750				
		Weight = $7 \times 0.75 \times 1.58 = 8$	.295 kg			
	17.	N = 2250/300 = 7, D = 16				
an Angla di Angla Angla di Angla		a = 50b			۰. 	
		b = 900 a	a a	en de la companya de La companya de la comp	n gan in s Rin Maria	
		L = 950				
		Weight = $7 \ge 0.95 \ge 1.58 = 1$	0.507 kg			
		$N = 2 \times 2 = 4, D = 22$				
		L = 2080				
		Weight = $4 \times 2.08 \times 2.98 = 1$	2.397 kg			
	al that is a start of the	1 1 – 18 = 1052.268 kg				
		WING WALL				
	1	$N = 7 \times 2 \times 2$ ; $D = 13$			10000	
		L total = $7 \times 1400 + (75 + 24)$			13832	
		Weight = $2 \times 2 \times 13.832 \times 1$ .	04 = 57.541  kg			
	2.	$N = 5 \times 2 \times 2 = 20$ ; $D = 13$				
		L = 2570 Weight = 20 x 2.57 x 1.04 =	53 156 kg			
	2	$W = 10 \times 2.57 \times 1.04 = 100 \times 1000 \text{ m}$ $N = 5 \times 2 \times 2 = 20 \text{ ; } D = 22 \text{ m}$	JJ.+JU Kg		e e se se se La seconda de seconda	
	3.	$N = 3 \times 2 \times 2 - 20$ , $D = 22$ L = 2300				
		Weight = $20 \times 2.3 \times 2.98 = 1$	37 N8 kg			
	4.	$N = 5 \times 2 \times 2 = 20$ ; $D = 22$	57.00 NB			
		L = 3100				
		Weight = $20 \times 3.10 \times 2.98 =$	184.76 kg		en e	
	5.	$N = 8 \times 2 \times 2 = 32$ ; $D = 22$				
		L total = $8 \times 1550 + (75 + 27)$	75 + 475 + 675 -	+ 875 + 1075 + 1275 +	1475) =	18600
		Weight = $2 \times 2 \times 18.600 \times 2$	date that is a possible of			
	6.					
		a = 50				
		b = 100	<u> </u>			
		c = 239	b			
		a L = 539	α <b>.</b>		e de la constante Sector de la constante Sector de la constante d	
			7 - 40			



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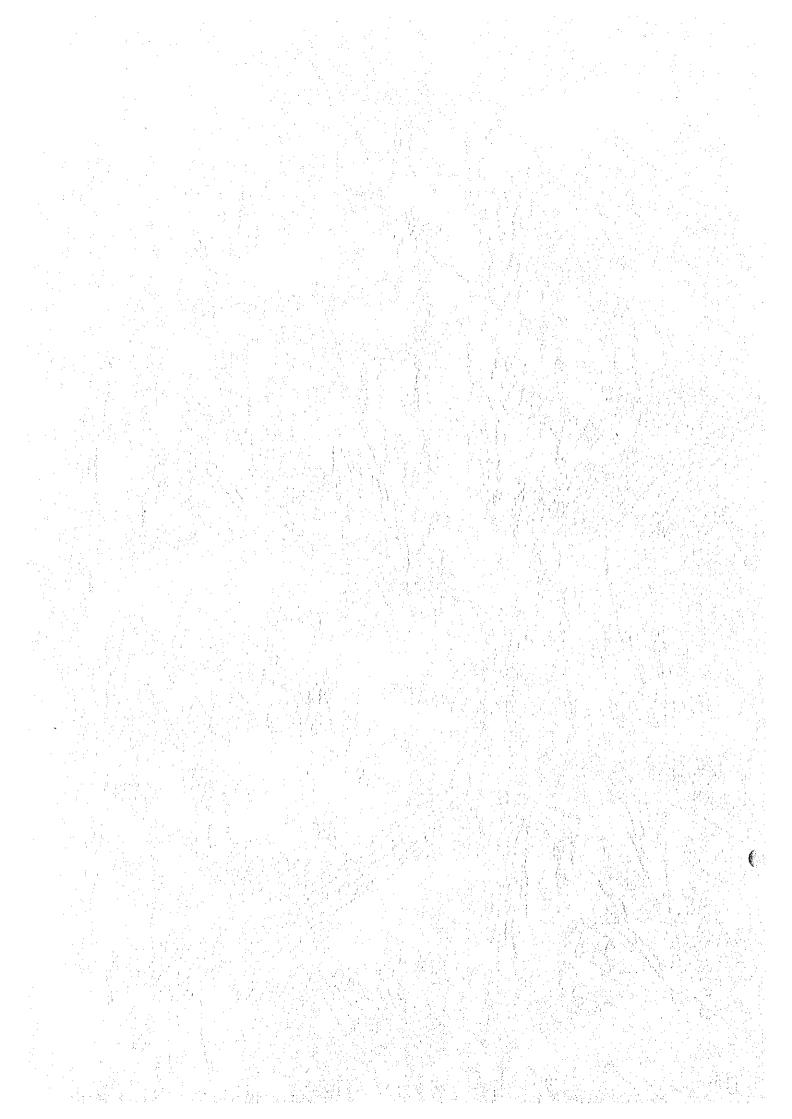
Name of Structure	TEMPORARY SUPPORT FOR RAILWAY BRIDGE BH.13	Category Calculation	WORK VOLUME	Page	
		SUMMAR	X		
1. CO	CONUT TREE PILES L	= 5.50 m; Ø = 30 c	an a satin . am a ⊨ <sub>ala</sub> sa	198 m'	· ·
2. W(	DODEN			10.90 m <sup>3</sup>	•
3. ST	EEL BEAM H - 500 x 30	0 x 16	= 10	6,000 kg	

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TEMPORARYName ofSUPPORT FORStructureRAILWAY BRIDGEBH.13	Category Calculation	WORK VOLUME	Page	
1. COCONUT TREE PILE				
$L = 5.50 \text{ m}; \emptyset = 30 \text{ cm}$			an a	
$N = 4 \times 9 = 36 \text{ nos.}$				
Total Length = $36 \times 5.50$		= 198 m'		
2. WOODEN		사망가 관계 위험 가지 같아요.	REALES 1	
(a) Wooden Sleeper (2,000 x 2	00 x 130)			
N = 2 x (4 x 4 + 4 x 3) + 4	4 x (2 x 3 x 4)	= 152 nos.		
$V = 2 \times 0.2 \times 0.13 \times 152$		= 7.90 m <sup>3</sup>		
(b) Wooden for Pile Cap (2,000	0 x 200 x 200)			
N = 3 x 4		= 12 nos.		
$V = 2 \times 0.2 \times 0.2 \times 12$		$= 0.96 \text{ m}^3$		
(c) Wooden for Ballast Stopper	r (2,250 x 200 x 1	30)		
$N = 2 \times (3.0 / 0.2)$		= 30 nos.		
$V = 2.25 \times 0.2 \times 0.13 \times 30$	)	= 1.76 m <sup>3</sup>		
(d) Wooden Pile Stability				
- For (2,000 x 200 x 130)				
N = 4 x 4 x 2		= 32 nos.		
$V = 1.65 \times 0.07 \times 0.05 \times 3$	32	$= 0.18 \text{ m}^3$		
- For (1,750 x 70 x 50)				
N = 4 x 4 x 1		= 16 nos.		
$V = 1.75 \times 0.07 \times 0.05 \times 1$	6	$= 0.10 \text{ m}^3$		
Sub Total V = $0.18 + 0.1$	0	= 0.28 m <sup>3</sup>		
Total Wooden = $7.90 + 0.9$	6 + 1.76 + 0.28	$= 0.28 \text{ m}^3$		
3. STEEL H - BEAM				
L = 10,000; H – 500 x 300 x 16	; Weight = 200 k	g∕m		
N = 2 x 4 = 8 nos.				
Weight = $10 \times 8 \times 200$		= 16,000 kg		

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