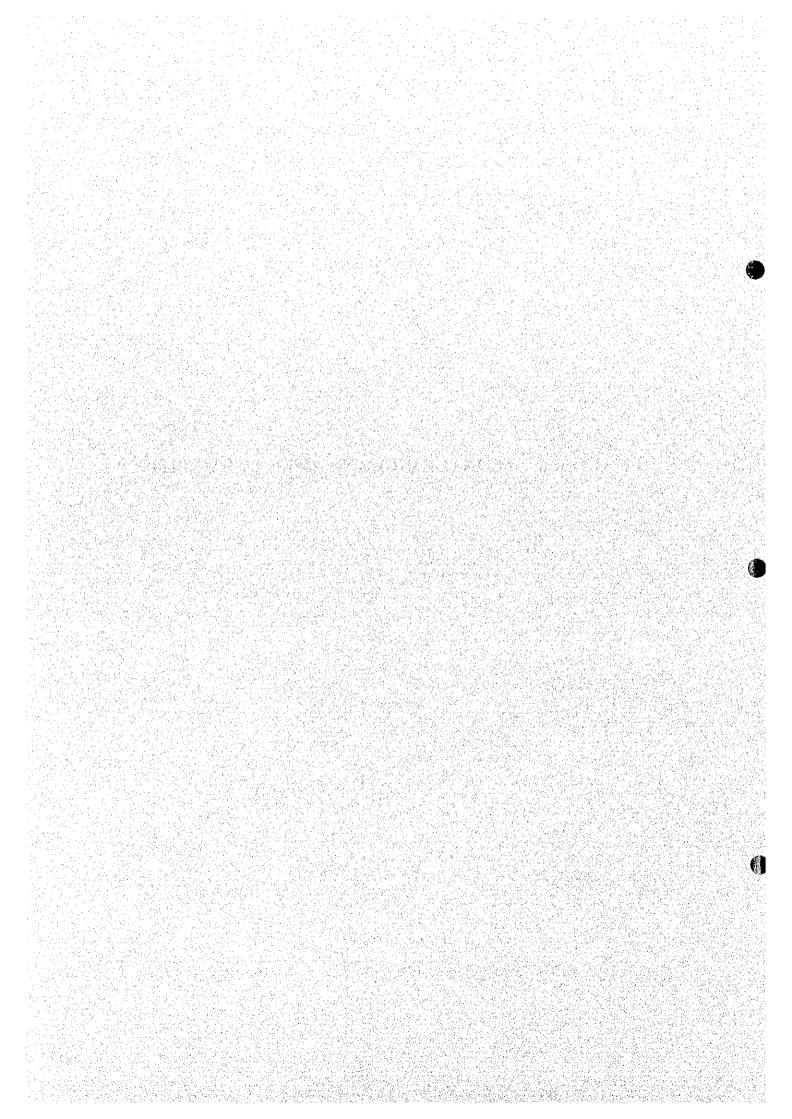
CHAPTER 6 MAINTENANCE AND AMENITY FACILITIES



6.1 Mooring Facility TYPE OF WORK: MOORING FACILITIES LOCATION:

LOCATION		PER 1 PLACE
CALCULATION		RESULT
STRUCTURAL EXCAVATION		
$A = (6.28 + 7.78) \times \frac{1}{2} \times 3.00$	$= 21.090 \text{ m}^2$	
$V_1 = 21.09 \times 10.00$	= 210.900	
$V_2 = \frac{1}{2} \times 6.00 \times 3.00 \times 6.00$	= 54.000	
$V_3 = (1.50 + 0.90) \times 1/2 \times 0.60 \times (5.00 + 1.414)$		
x 6.00) x 2	= 19.416	
$V_4 = 1.118 \times 6.00 \times 1.00 \times (5.00 + 1.414)$	100 001	
x 6.00) x 2	= 180.901	
$V_5 = 0.60 \times 1.00 \times 5.00 \times 2$	= 6.000	
V = (1.20 2.90) - 1/ - 0.60 - 10.00	12.000	
$V_6 = (1.20 + 2.80) \times \frac{1}{2} \times 0.60 \times 10.00$	= 12.000	
V = (4.80 6.10) 17 0.50 10.00	- 27.250	
$V_7 = (4.80 + 6.10) \times \% \times 0.50 \times 10.00$	= 27.250	
$V_8 = 1.202 \times 4.50 \times 0.50 \times 10.00$	= 27.045	
V ₈ = 1.202 x 4.30 x 0.30 x 10.00	- 27.043	
ΤΟΤΑΙ Υ	= 537.510	537.510 m ³
BACKFILL WITH SELECTED SOIL	337.310	337.310 111
$A_1 = (0.50 + 1.10) \times \frac{1}{2} \times 0.60$	= 0.480	
$A_2 = (0.50 + 1.00) \times \frac{1}{2} \times 0.50$	= 0.375	
$A_1 = (0.50 + 0.80) \times \frac{1}{2} \times 0.30$	= 0.195	
TOTAL A	$= 1.050 \text{ m}^2$	
$V = 1.05 \times 10.00$	= 10.500	10.500 m ³
GRAVEL BEDDING		
$V = (6.31 \times 0.25 + 0.50 \times 0.25) \times 10.00$	= 17.025	17.025 m ³
RUBBLE STONE FILLING		
	1.075	1 0753
$V = \frac{1}{2} \times 0.50 \times 0.75 \times 10.00$	= 1.875	1.875 m ³
CADYON ACAMODES		
GABION MATTRESS		
$A \rightarrow 0.50 \times 2.00$	$= 1.50 \text{ m}^2$	
$A = 0.50 \times 3.00$ $V = 1.50 \times 10.00$	$= 1.50 \text{ m}^2$ = 15.000	15.000 m ³
$\mathbf{v} = \mathbf{x} \mathbf{J} \mathbf{u} \mathbf{x} \mathbf{T} \mathbf{u} \mathbf{u} \mathbf{v}$	10.000	12,000 111

	·		 	T			·	·	 and any the shortest	****		 Posteriore	 					
?) PLACE	RESULT						264.000 m								183.000m			
	CALCULATION	- PC SHEET PILE	$L_1 = 6.00 \text{ m/pile}$ $L_2 = 4.00 \text{ m/pile}$		1 = (6.00 m/pile x 18 piles + 4.00 m/pile x 6 piles) x 2		= 264.000	• DRIVING	$L_1 = 4.50^{\text{m}}/\text{pile} \times 12 \text{ pile} \times 2 = 708.000$		12 = 300 m/pile x 3piles x 2 = 18.000	13 = 3.50 m/pile x 3piles x 2 = 21.000	14 = 3.00 m/pile x 6 piles x 2 = 36.000		TOTAL = 183.000			
MOORING FACILITIES	YPE OF WORK: PC SHEET PILE	LOCATION		F SH 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	EET P CO, w *		144				6,000 6,000 NCRET	200		20 146				
	TYP	707								6	- 3							

TYPE OF WORK: MOORING FACILITIES

LOCATION

1

LOCATION :		PER 1 PLACE
CALCULATION		RESULT
PC SHEET PILE		
		is stigated as f
PC SHEET PILE		
$L_1 = 6.00 \text{ m/pile}$ $L_2 = 4.00 \text{ m/pile}$		
L ₁ = 0.00 m7 pne L ₂ = 4.00 m7pne		
L = (6.00 m / pile x 18 piles + 4.00 m / pile x)		
6 piles) x 2	= 264.000	264.000 m
DRIVING		
$L_1 = 4.50 \text{ m/pile x } 12 \text{ piles x } 2$	= 108.000	
T = 2.00 = 7.11 = 2.11 = 10.		
$L_2 = 3.00 \text{ m} / \text{pile x 3 piles x 2}$	= 18.000	
$L_3 = 3.50 \text{ m/pile x 3 piles x 2}$	= 21.000	
D3 Sist in pile x 5 piles x 2	21,000	
$L_4 = 3.00 \text{m/pile x 6 piles x 2}$	= 36.000	A CONTRACTOR
TOTALL	= 183.000	183.000 m
COPING CONCRETE		
CONCRETE TYPE C1		
	= 4.875	4.875 m ³
1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1	1.0/3	1.073 111
GRAVEL BEDDING		
$V = (6.50 + 6.00) \times 2 \times 0.10 \times 0.24$	= 0.600	0.600 m^3
FORM (H < 4.0 m)		s i sail a gái
$A_1 = (0.50 \times 2 + 0.14) \times (6.50 + 6.00) \times 2$	= 28.500	
$A_2 = 0.50 \times 0.50 \times 2 \times 2$	= 1.000	20.500 2
TOTAL A	= 29.500	29.500 m ²
REINFORCING BAR		
KENT OKCING BAK		
D13 (W = 1.04 kg.f/m)		
$n_1 = 7 \text{ bars}$		
$W_1 = (6.50 + 6.00 - 0.05 \times 2) \times 7 \text{ bars } \times 1.04$	= 90.272	A STAN STAN

TYPE OF WORK : MOORING FACILITIES LOCATION :

	P	ER 1 PLACE
CALCULATION		RESULT
D10 (W = 0.617 kg.f/m)		
$n_2 = (6.50 + 6.00 - 0.05 \times 2) : 0.30 + 1$		
(0.00 0.00 0.00 x 2) . 0.50 1 1	+ 42 vais	
$L = 0.40 \times 3$	= 1.200 m / bar	
	1.200 III / Uai	
$W_2 = 42 \times 1.20 \times 0.617 \times 2$	= 62.194	
	02.154	
TOTAL W	= 152.466 kg.f	0.152 tf
	152.400 kg.1	0.132 ti
BASE CONCRETE		
CONCRETE TYPE C1		And the second of the
$V = 3.15 \text{ m}^3 / 10.00 \text{ m} \times 10.00 \text{ m}$	= 3.150	3.150 m ³
		J.130 III
GRAVEL BEDDING		
$V = 0.80 \text{ m}^3 / 10.00 \text{ m} \times 10.00 \text{ m}$	= 0.800	0.800 m ³
V.00 III / 10.00 III X 10.00 III		0.800 Hi
FORM (H < 4.0 m)		
$A = 13.45 \text{ m}^2 / 10.00 \text{ m} \times 10.00 \text{ m}$	= 13.450	13.450 m ²
7. 15.45 iii / 10.00 iii X 10.00 iii	- 13.430	13.450 III
REINFORCING BAR		
$W = 0.039 \text{ tf} / 10.00 \text{ m} \times 10.00 \text{ m}$	- 0.020	0.020 46
W = 0.039 H / 10.00 HI X 10.00 HI	= 0.039	0.039 tf
LOG PILE (Dia 150, L = 2.0 m / pile)		
	- 10.000	10.000
L - 10.00 m / 10.00 m x 10.00 m	= 10.000	10.000 m
CONCRETE STEPS		
CONCRETESTERS		
CONCRETE TYPE D		
$V = \{(6.31 + 1.00) \times 0.50 - \frac{1}{2} \times 0.30 \times 0.45$		Mark a constitution of the second
x 10} x 10.00	= 29.800	20.000 3
FORM (H < 4.0 m)	= 29.800	29.800 m ³
	- 20.000	
$A_1 = 0.30 \times 10.00 \times 10$	= 30.000	
$A_2 = 0.25 \times 10.00$	= 2.500	20.500. 2
TOTAL A	= 32,500	32.500 m ²
DEINICODONIC DAD		
REINFORCING BAR		
D12 (W = 1.04 kg f / m)		
D13 (W = 1.04 kg.f/m)		
$n_1 = (6.31 + 0.45 - 0.05 \times 2) : 0.30 + 1$	= 24 bars	
I - (10.00; 0.05 - 0)	0.00	
$L = (10.00 - 0.05 \times 2)$	= 9.90 m / bar	
W = 24 - 0.00 - 1.04	0.47 + 0.45	
$W_1 = 24 \times 9.90 \times 1.04$	= 247.104	

TYPE OF WORK : MOORING FACILITIES LOCATION :

		PER 1 PLACE
CALCULATION		RESULT
$n_2 = (10.00 - 0.05 \times 2) : 0.30 + 1$	= 34 bars	
$L = (6.31 + 0.45 - 0.05 \times 2)$	= 6.660 m / bar	
	005 100	
$W_2 = 6.66 \times 34 \times 1.04$	= 235.498	
TOTAL W	= 482.602 kg.f	0.483 tf
	- 402.002 kg.1	0.485 (1
RUBBLE STONE BEDDING		
$A_1 = 1.118 \times 6.00 \times 0.40 + \frac{1}{2} \times 0.40 \times 0.80$	$= 4.283 \text{ m}^2$	
$V_1 = 4.283 \times 5.00 \times 2$	= 42.830	
		and the second s
$A_2 = \frac{1}{2} \times (1.118 \times 6.00)^2$	$= 22.499 \text{ m}^2$	
$V_2 = 22.499 \times 0.40 \times 2$	= 17.999	
	- (0.930	(0.0003
TOTAL V	= 60.829	60.829 m ³
STONE FACING		
STONE FACING		
$A_1 = (1.50 + 0.90) \times \frac{1}{2} \times 0.60$	$= 0.720 \text{ m}^2$	
$A_2 = 1.118 \times 6.00 \times 0.60$	$= 4.025 \text{ m}^2$	
$A_3 = 1.00 \times 0.60$	$= 0.600 \text{ m}^2$	
	6015	
TOTAL A	$= 5.345 \text{ m}^2$	
$V_1 = 5.345 \times 5.00 \times 2$	= 53.450	
Silb of a Silb of the silb of		
$V_2 = \{ \frac{1}{2} \times (1.118 \times 6.00)^2 \times 0.60 + 0.720 \times 0.00 + 0.$		
1.118 x 6.00} x 2	= 36.658	
		. M. Sira, ilaanisa
Mile per la TOTÀL V	$= 90.108 \mathrm{m}^3$	90.108 m ³
		atjanija i sa kalina a ligit aa

6.2 Riverside Approach Steps

TYPE OF WORK : LOCATION :

APPROACH STEPS TYPE - WA (RIVER SIDE)

CALCULATION		RESULT
El comprisoner la rivio de la comprisoner la compri		
STRUCTURAL EXCAVATION		
		7,5 4475.7
$A_1 = (1.4 + 2.0) \times 0.6 : 2$	= 102	
A ₁ = (1.4 + 2.0) x 0.0 . 2	= 1.02	
$V_1 = 1.02 \times 3.40$	= 3.468	
$A_2 = (1.4 + 2.0) \times 0.6 : 2$	= 1.02	
		1. 1. 1. 1. 1. 1. 1
$V_2 = 1.02 \times 0.8$	= 0,816	
ΤΟΤΑΙ		4004
TOTAL	= 4.284	4.284 m ³
BACKFILL WITH SELECTED SOIL		
$A_1 = (0.6 + 1.2) \times 0.6 : 2$	= 0.54	
		\$4 - 11 To 12
$V_1 = 0.54 \times 3.4$	= 1.836	
A (0 (1 1 0)		
$A_2 = (0.6 + 1.2) \times 0.6 : 2$	= 0.54	
$V_2 = 0.54 \times 0.8 \times 2$	= 0.864	
12 0.54 12 0.6 12	<u> </u>	The graph of the
TOTAL	= 2.700	2.700 m ³
		\$ 1000
		A G
		grand and sold
		<u> </u>
		14 1 1 1 1 1 1 1
and the second of the second o		

TYPE OF WORK: LOCATION: APPROACH STEPS TYPE - WA (RIVER SIDE)

CALCULATION		RESULT
CALCOLATION		KESULI
♂ WET STONE MASONRY		
WEI STUNE MASUNKY		
1. 0.8 x 0.2 x 1.0 =	0.16	
0 00 10	0.16	<u> </u>
2. 0.8 x 0.2 x 1.6 = 3. 0.8 x 0.2 x 2.2 =	0.256 0.352	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1
4. 0.8 x 0.2 x 2.8	0.352	
5. 0.8 x 0.7 x 3.4	0.544	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1
	V.JTT	
	1.76	1,760 m ³
		1.700 III
□ GRAVEL BEDDING		
		1
$V = 0.1 \times 0.9 \times 3.6$	= 0.324	0.324 m ³
☐ CEMENT MORTAR POINTING		
• SIDE		
0.2 x 1.0	0.2	
0.2 x 1.6 =	0.32	
0.2 x 2.2 =	0.44	
0.2 x 2.8 =	0.56	
0.2 x 3.4 =	0.68	
The state of the	2 200	
TOTAL:	2.200	
• FRONT		
0.8 x 1.0 x 2 =	1.600	
• TOP		
3.4 x 0.8	2.720	
TOTAL =	6.52	6.520 m ²
5 JOINT FILTER		
$A = 0.2 \times 1.0 + 0.2 \times 1.6 + 0.2 \times 2.2 + 0.2 \times 2.8 + 0.7$	x 3.4	
		14 14 20 14 14 14 14 14 14 14 14 14 14 14 14 14
	2.2	2.200 m ²

RESULT					3.256 kgf				
(RIVER SIDE) CALCULATION D 16(1,580 kgf/m)	8.0 - 5.0 -	S + 2 = 0, 4 × 3	(3) = 0, 4 × 3.	707/1 = 3.2	W= 3,2 x 1,580 = 5.056				
TYPE OF WORK: ANCHOR BARS LOCATION: (RIVER SIDE)	ODD TO THE TANK THE T	100 000 000 000 000 000 000 000 000 000	00c1 6c2, 6 00c	100 100 100 100 100 100 100 100 100 100	SECTION B-B	3,400	 © \\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\	8	SECTION A-A

TYPE OF WORK:

LOCATION : APPROACH STEPS TYPE – WA (LAND SIDE)

CALCULATION		RESULT
☐ STRUCTURAL EXCAVATION		
4 (1750 + 1050) - 04 - 2	0.0	
$A_1 = (1.750 + 1.250) \times 0.4 : 2$	= 0.6	
$V_1 = 0.6 \times 5.8$	= 3.48	1 4 4 1 1 1 A
$A_2 = (0.6 + 1.0) \times 0.4 : 2$	= 0.32	
$V_2 = 0.32 \times 1.250 \times$	⇒ 0.800	
$TOTAL = V_1 + V_2$	= 4.28	4.280 m ³
5 BACKFILL WITH SELECTED SOIL		
$A_1 = (1.0 + 0.6) \times 0.4 : 2$	= 0.32	
$V_1 = 0.32 \times 6.00$	= 1.92	
The state of the s	3/	
$A_2 = (0.6 + 1.0) \times 0.4 : 2$	= 0.32	
$V_2 = 0.32 \times 0.6 \times 2$	= 0.384	
$TOTAL = V_1 + V_2$	= 2.304	2.304 m ³
		2,007 441
		jerništi kalbina Para i pravi sa
	and the second of the second o	
		get in the same that

TYPE OF WORK : LOCATION : APPROACH STEPS TYPE - WA (LAND SIDE)

CALCULATION		RESULT
5 WET STONE MASONRY		
WEI STOILE MASONKY		and the second
BLOCK		
1 00		
1. 0.8 x 0.2 x 1.0	= 0.16	
2. 0.8 x 0.2 x 1.6	= 0.256	
	= 0.256	21.15.3.4
3. 0.8 x 0.2 x 2.2	= 0.352	
4 08 00 00		
4. 0.8 x 0.2 x 2.8	= 0.448	
5. 0.8 x 0.2 x 3.4	= 0.544	
	- 0.344	
6. 0.8 x 0.2 x 4.0	= 0.640	
7. 0.8 x 0.2 x 4.6		
7. 0.0 A 0.2 X 4,0	= 0.736	
8. 0.8 x 0.2 x 5.2	= 0.832	
	0.832	
9. (0.8 x 0.67) x 0.367 : 2 x 5.8	= 1.565	
TOTAL:	= 5.533	
	3.333	5.533 m ³
	1000 X 1000 X 100	
GRAVEL BEDDING		
$V = (0.76 + 0.73) \times 0.1 : 2 \times 6.0$	= 0.447	0.447 m ³
<u>and the state of </u>		

		RESULT																		11.323 m2							
				5,0 =	= 0.32	440 =	= 0.56	= 0.68	= 0.80	= 0.92	+01 =	= 0.389	= 5,349		7,304			= 4.640		× 11, 323							
		CALCULATION											70741		11.6 + 0.067) x0.8			8.		707AL					The state of the s		
	S10E)		ES	0,2 x 1,0	0.2 x / 8	0.2 x 2.2	0,2×2.8	0.2 x 3.4	0.8×4.0	0.0 X X Q	0,2 × 5.2	0.067 × 5.8		FRONT	A = (1.6 + 0.0		Q	A: 5.8 x 0	The state of the s	- Control of the Cont		and the state of t					
	4. (LAND		SIDES											· ZR			· 70P	7									
	APPROACH STEPS TYPE - N.A. (LAND SIDE)	PLASTERIUG				URIVER SIDE 3		STONE MASONRY	•••	50't 000 202 (3) 1		007	OCC.	Ann Out								C Cove					
	CH STEPS	CEMENT MORTAR I		SIDE)		VAHIDS uou		FLOOD WALLCROWN		55	.001	-	000	ANCHOR BARB	พ 600 + ๆ - รีการ์การ์การ์การ์การ์การ์การ์การ์การ์การ์	SECTION B-B			\$ 800	000/2.				88			
3	APPROA	CEMEN	A second	(LAND SIDE		000	LOUNT FILLER 1-10 mm	WET STONE MASONRY	6	OII NOC				26. 25.	L. LEZISTIN	38			Ý,	,				5800		a de de de la companya de de de	
		YPE OF WORK:	LOCATION				, shano SIOE I				002 1			000						-	&	91 LS	7	8 ,			
		L.	07										6	- 12													

TYPE OF WORK : LOCATION : APPROACH STEPS TYPE - WA (LAND SIDE)

1	ting a second control of the	CALCULATION	the state of the s	RESULT
5 J(DINT FILTER			
5				
	0.2 x 1.0	•••	0.2	
	0.2 x 1.6		0.2	
	0.2 x 2.2		0.44	
	0.2 x 2.8		0.44	
	0.2 x 3.4			
	0.2 x 4.0		0.68	
			0.80	19 To
	0.2 x 4.6	25	0.92	4 4 10.51
* 21 T	0.2 x 5.2		1.04	1 1 1 1 1 1
<u> </u>	0.367 x 5.8		2.129	
		un de la companya de Transportation de la companya de la		
·				<u> Liberali ye</u>
	y company the property	TOTAL:	7.089	7.089 m²
<u> </u>				
<u> </u>				
. 2.		and the state of t		
A P	NCHOR BARS			
31.57				
D	16 (1.580 kgf/m)			
	e de la participación de la composición			18 Table 18
1.	$L_1 = 0.4 \times 2$	n na servenje i stanjeni i pravije i servenje i servenje i servenje i servenje i servenje i servenje i servenj	0.8	
2.	$L_2 = 0.4 \times 5$	=	2.0	
A				
3.	$L_3 = 0.4 \times 9$	Salah da 😑 🖆 🗎	3.6	
<u> </u>	$L_3 = 0.4 \times 9$		3.6	
·			3.6 6.4	
<u> </u>	$L = L_1 + L_2 + L_3$		6.4	10.112 kgf
<u> </u>	$L = L_1 + L_2 + L_3$			10.112 kgf
<u> </u>	$L = L_1 + L_2 + L_3$		6.4	10.112 kgf
<u> </u>	$L = L_1 + L_2 + L_3$		6.4	10.112 kgf
<u> </u>	$L = L_1 + L_2 + L_3$		6.4	10.112 kgf
<u> </u>	$L = L_1 + L_2 + L_3$		6.4	10.112 kgf
<u> </u>	$ \begin{array}{rcl} L &=& L_1 + L_2 + L_3 \\ N &=& 6.4 \times 1.580 \end{array} $		6.4 10.112	
3.	$\begin{array}{cccccccccccccccccccccccccccccccccccc$		6.4 10.112	
3.	$ \begin{array}{rcl} L & = & L_1 + L_2 + L_3 \\ N & = & 6.4 \times 1.580 \end{array} $		6.4 10.112	
3.	$L = L_1 + L_2 + L_3$ $N = 6.4 \times 1.580$		6.4 10.112	
3.	$L = L_1 + L_2 + L_3$ $N = 6.4 \times 1.580$		6.4 10.112	
3.	$L = L_1 + L_2 + L_3$ $N = 6.4 \times 1.580$		6.4 10.112	
3.	$L = L_1 + L_2 + L_3$ $N = 6.4 \times 1.580$		6.4 10.112	
3.	$ \begin{array}{rcl} L &=& L_1 + L_2 + L_3 \\ N &=& 6.4 \times 1.580 \end{array} $		6.4 10.112	
3.	$L = L_1 + L_2 + L_3$ $N = 6.4 \times 1.580$		6.4 10.112	
3.	$L = L_1 + L_2 + L_3$ $N = 6.4 \times 1.580$		6.4 10.112	
3.	$L = L_1 + L_2 + L_3$ $N = 6.4 \times 1.580$		6.4 10.112	
3.	$L = L_1 + L_2 + L_3$ $N = 6.4 \times 1.580$		6.4 10.112	
3.	$L = L_1 + L_2 + L_3$ $N = 6.4 \times 1.580$		6.4 10.112	

TYPE OF WORK : CONCRETE STEPS
LOCATION : APPROACH STEPS (TYPE-WA-2) LOCATION

		PER 1 PLACE
CALCULATION		RESULT
STRUCTURAL EXCAVATION		
$A_1 = (1.20 + 4.30) \times \frac{1}{2} \times 1.20$	= 3.300	
$A_1 - (1.20 \pm 4.30) \times 72 \times 1.20$	_ 3.300	
$A_2 = (1.118 \times 6.30 + 0.70) \times 0.60$	= 4.646	
$A_3 = (0.80 + 1.80) \times \frac{1}{2} \times 0.80$	= 1.040	
TOTALA	$= 8.986 \text{ m}^2$	
$V = 8.986 \times 1.80$	= 16.175	16.175 m ³
BACKFILL WITH SELECTED SOIL		
$A_1 = (0.50 + 1.20) \times \frac{1}{2} \times 0.70$	= 0.595	
$A_2 = (1.50 + 1.90) \times \frac{1}{2} \times 0.30$	= 0.510	
(0.00) 1100 1700 000		
$A_1 = (0.50 + 1.10) \times \frac{1}{2} \times 0.60$	= 0.480	
TOTAL A	$= 1.585 \text{ m}^2$	
	ja varaka esti jarras tilatera e	
$V = 1.585 \times 1.80$	= 2.853	2.853 m ³
OD AMEL DEDIVING		
GRAVEL BEDDING		
Average Height: h = 2.650 m		. Diese bereit
$A = (1.118 \times 6.30 + 0.70) \times 0.25$	= 1.936	
$V = 1.936 \times 1.20$	= 2.323	2.323 m ³
1 - 1.930 X1.20	- 2.323	Z.JEJ III
CONCRETE TYPE D		
Average Height: h = 2.650 m		
CONCRETE TYPE D		
$V = \{(1.118 \times 6.30 + 0.70) \times 0.35 - \frac{1}{2} \times 0.40$		
x 0.20 x14}	= 2.580	2.580 m ³
FORM (H < 4.0 m)	= 3.360	3.360 m ²
$A = 0.20 \times 1.20 \times 14$	- 3.30U	3.300 m

TYPE OF WORK : CONCRETE STEPS LOCATION : APPROACH STEPS (TYPE-WA-2)

PER 1 PLACE

CALCULATION	11.		RESULT
REINFORCING BAR (D13, W = 1.04 kg.f/m			sa Malaysay.
$n_1 = (1.118 \times 6.30 + 0.70 - 0.05 \times 2) : 0.30 + 1$	-	27 bars	
$L_1 = (1.20 - 0.05 \times 2)$	=	1.10 m / bar	
		20.000	
$W_1 = 27 \times 1.10 \times 1.04$	=_	30.888	
(1.00 0.05 0.00 1.1	_		
$n_2 = (1.20 - 0.05 \times 2) : 0.30 + 1$		6 bars	
$L_2 = 1.118 \times 6.30 + 0.70 - 0.05 \times 2$		7.643 m / bar	
$W_2 = 7.643 \times 6 \times 1.04$	7	47.692	
	1 h.	70.5001	0.050.00
TOTAL W	=	78.580 kg.f	0.079 tf
SIDE WALL			
DIDE HARD	<u> </u>		
CONCRETE TYPE C1			
$V = 1.233 \text{m}^3 / 2.936 \text{m} \times 7.743$	=	3.252	3.252 m ³
	- 1 - 1	and the second of the second o	
GRAVEL BEDDING		0.000	2,872 m ³
$V = 1.089 \text{ m}^3 / 2.936 \text{ m x } 7.743$	=	2.872	2.8/2 M
FORM (H < 4.0 m)	7 -		
$A = 11.782 \text{m}^2 / 2.936 \text{m} \times 7.743$	= -	31.072	31.072 m ²
REINFORCING BAR	61 . ·		
$W = 0.059 \text{ tf} / 2.936 \text{ m} \times 7.743$	=_	0.156	0.156 tf
JOINT FILLER		<u>operatorial de la Carolina. Establik</u> Haritana di Angelong di Politania (h. 1888).	
the state of the s	=	5.420	5.420 m ²
	. %		
BASE CONCRETE			
	10.5		
CONCRETE TYPE C1 $V = 3.150 \text{ m}^3 / 10.00 \text{ m x } 1.80$		0.567	0.567 m ³
V = 3.150 m / 10.00 m x 1.80	·	<u> 0.307 </u>	0.307 m
GRAVEL BEDDING	* *		
[=	0.144	0.144 m ³
FORM (H < 4.0 m)	1		
$A = 13.240 \text{ m}^2 / 10.00 \text{ m} \times 1.80$	=	2.383	2.383 m ²
and the first term of the second of the seco		ing the second s	

TYPE OF WORK : CONCRETE STEPS LOCATION : APPROACH STEPS : APPROACH STEPS (TYPE-WA-2)

CALCULATIO	N	RESULT
REINFORCING BAR	11	KEGGET
$W = 0.101 \text{ tf} / 10.00 \text{ m} \times 1.80$	= 0.018	0.018 tf
W - 0.101 U / 10.00 III X 1.80	- 0.018	0.010 11
TOP CONCRETE		
TOP CONCRETE		
CONCRETE TYPE C1		
$V = 2.10 \text{m}^3 / 10.00 \text{m} \times 1.80$	= 0.378	0.378 m ³
v = 2,10 m / 10,00 m x 1.80	= 0.378	0.378 m
ODAVICE DEDDDIO		
GRAVEL BEDDING		
$V = 0.80 \text{ m}^3 / 10.00 \text{ m} \times 1.80$	= 0.144	0.144 m ³
	gradie (f. 17 defender – a fragter et en 1964). Er fragt fan de 1963.	
FORM (H < 4.0 m)		
$A = 14.210 \text{ m}^2 / 10.00 \text{ m} \times 1.80$	= 2.558	2.558 m ²
REINFORCING BAR	(1997年) 《本联》是古典基督作为代表的诗句(1907年)	
$W = 0.098 \text{ tf} / 10.00 \text{ m} \times 1.80$	= 0.018	0.018 tf
OINT FILLER		
$\Lambda = 3.120 \text{ m}^2 / 10.00 \text{ m} \times 1.80$	= 0.562	0.562 m ²
		till til sammer som fill til state og til sta
		and the second second
o translation of the contract	美国 建铁铁铁铁 医二氢二甲二基甲	
		(44) (1) (4) (44444 (1) (1)
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die Maria de la Carte de l La companya de la Carte de		
		

TYPE OF WORK : CONCRETE STEPS
LOCATION : APPROACH STEPS (TYPE-WB)

	I	PER 1 PLACE
CALCULATION		RESULT
STRUCTURAL EXCAVATION		
$A_1 = \frac{1}{2} \times 1.80 \times 0.90$	= 0.810	
$A_2 = (1.118 \times 2.00 + 0.70) \times 0.60$	= 1.762	
A = (0.80 ± 1.80 ± 1/ ± 0.00		
$A_3 = (0.80 + 1.80) \times \frac{1}{2} \times 0.80$	= 1.040	
$V_1 = 3.612 \times 3.60$	$= 3.612 \text{ m}^2$	
V) - 3.012 x 3.00	= 13.003	
$V_2 = \{\frac{1}{2} \times 0.60 \times 1.20 + (2.795 + 1.00) \times 0.50\}$		
x 2.00 x 2	= 9.030	
TOTAL V	= 22.033	22.033 m ³
	- 22.033	22:033 III
BACKFILL WITH SELECTED SOIL		
$A_1 = (0.50 + 1.10) \times \frac{1}{2} \times 0.60$	= 0.480	
$A_2 = (0.50 + 0.80) \times \frac{1}{2} \times 0.30$	= 0.195	
$V_1 = 0.480 \times 3.60$	= 1.728	
$V_2 = 0.195 \times 2.00 \times 2$	= 0.780	
TOTAL V	= 2.508	2.508 m ³
GRAVEL BEDDING		the control
A - (1 110 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0		
$A = (1.118 \times 2.00 + 0.70) \times 0.25$	= 0.734	
$V = 0.734 \times 3.00$		
Y - 0.734 X 3.00	= 2.202	2.202 m ³
RUBBLE STONE FILLING	<u>interpretation of the state of</u>	
ALL DE LA COLLEGE LE		
$V = \frac{1}{2} \times 0.60 \times 1.20 \times 7.60$	= 2.736	2.736 m ³
	2.750	2.730 III
GABION CYLINDER Ø 500		
$V = \frac{\pi}{4} \times 0.50^2 \times (2.795 + 1.00) \times 4.00 \times 2$	= 5.961	5.961 m ³
SOIL FILLING	<u>er in transport en de la fina de desertes, estre de la colonia.</u> La fina de la colonia de la	
$V = (2.795 + 1.00) \times 2.00 \times 2 - 5.961$	= 1.629	1.629 m ³

TYPE OF WORK : CONCRETE STEPS LOCATION : APPROACH STEPS (TYPE-WB)

CALCUM LOVO		PER 1 PLACE
CALCULATION		RESULT
CONCRETE TYPE D		
CONCRETE TYPE D		
$V = \{(1.118 \times 2.00 + 0.70) \times 0.35 - \frac{1}{2} \times 0.40$		
x 0.20 x 5} x 3.00	= 2.483	2.483 m ³
FORM (H < 4.0 m)		
$A = 0.20 \times 3.00 \times 5$	= 3.000	3.000 m ²
REINFORCING BAR (D13, $W = 1.04 \text{ kg.f/m}$		
$n_1 = (1.118 \times 2.00 + 0.70 - 0.05 \times 2) : 0.30 + 1$		
$L_1 = (3.00 - 0.05 \times 2)$	= 2.90 m / bar	
$W_1 = 11 \times 2.90 \times 1.04$	= 33.176	
$n_2 = (3.00 - 0.05 \times 2) : 0.30 + 1$	≑ 11 bars	
11/2 (3.00 0.03 K 2) . 0.50 + 1	- 11 Outs	
$L_2 = 1.118 \times 2.00 + 0.70 - 0.05 \times 2$	= 2.836 m / bar	
1.110 K 2.00 + 0.70 0.03 K 2	2,030 III / Odi	
$W_2 = 2.836 \times 11 \times 1.04$	= 32.444	
	32.777	
TOTAL W	= 65.620 kg.f	0.066 tf
SIDE WALL	031020 Kg.x	0.000 ta
CONCRETE TYPE C1		
	= 1.233	1.233 m ³
	1.255	1123511
GRAVEL BEDDING		
$V = \{(0.10 \times 0.30) + (0.10 + 0.211) \times \frac{1}{2} \times 10^{-10} \times 10^{-$		
$(2.236 + 0.70) \times 2$	= 1.089	1.089 m ³
		11007.11
FORM (H < 4.0 m)		
$A = \{(2.236 + 2.420) \times \frac{1}{2} + (0.70 + 0.535) \times \frac{1}{2} + (0.70 + 0.535$		
½} x 2 x 2	= 11.782	11.782 m ²
REINFORCING BAR		
D13 ($W = 1.04 \text{ kg.f/m}$)		
$n_1 = 6$ bars		
$L_1 = (2.236 + 0.70 - 0.05 \times 2)$	= 2.836 m / bar	
<u> </u>		
$W_1 = 6 \times 2.836 \times 1.04 \times 2$	= 35.393	

 $W_2 = 11 \times 2.057 \times 0.617$

TYPE OF WORK : CONCRETE STEPS
LOCATION : APPROACH STEPS (TYPE-WB)

	P	ER 1 PLACE
CALCULATION		RESULT
D10 (W = 0.617 kg.f/m)		
$n_2 = (2.236 + 0.70 \times 2) : 0.30 + 1$		
$L_2 = (0.20 \times 2 + 0.60 \times 2 + 15 \times 0.01)$	= 1.750 m / bar	
$W_2 = 11 \times 1.75 \times 0.617 \times 2$	= 23.755	
TOTAL W	7 = 59.148 kg.f	0.059 tf
JOINT FILLER		
$A = (1.118 \times 2.00 + 0.70) \times 0.35 \times 2$	= 2.055	2.055 m ²
BASE CONCRETE		
COMONEME ENTRE OF		
CONCRETE TYPE C1	0.000	0.000 3
$V = (0.50 + 0.75) \times \frac{1}{2} \times 0.50 \times 3.00$	= 0.938	0.938 m ³
GRAVEL BEDDING		
	= 0.224	0.224 m ³
$V = (3.00 \pm 0.10 \times 2) \times 0.10 \times 0.70$	- 0.224	0.224 III
FORM (H < 4.0 m)		
$A = (0.50 + 0.75) \times 3.00 + (0.50 + 0.75) \times \frac{1}{2}$	en e	
x 0.50 x 2	= 4.375	4.375 m ²
REINFORCING BAR		
D13 ($W = 1.04 \text{ kg.f/m}$)		
$n_1 = 6 \text{bars}$		
$L_1 = (3.00 - 0.05 \times 2)$	= 2.900 m / bar	
$W_1 = 6 \times 2.900 \times 1.04$	= 18.096	
D10 (W = 0.617 kg.f/m)		
$n_2 = (3.00 - 0.05 \times 2) : 0.30 + 1$	÷ 11 bars	
		建设建设
$L_2 = (0.40 + 0.43 + 0.63 + 0.447 + 15 \times 0.01)$	= 2.057 m / bar	
the state of the Article State of the Control of the State of the Stat		

TOTAL W

13.961

32.057 kg.f

0.032 tf

TYPE OF WORK : CONCRETE STEPS

LOCATION : APPROACH STEPS (TYPE-WB)

PER 1 PLACE CALCULATION RESULT LOG PILE = 2.00 m / pile x 3 piles6.000 6.000 m TOP CONCRETE CONCRETE TYPE CI $V = 2.10 \text{ m}^3 / 10.00 \text{ m} \times 3.60$ 0.756 $0.756 \,\mathrm{m}^3$ GRAVEL BEDDING $V = 0.80 \text{ m}^3 / 10.00 \text{ m} \times 3.60$ 0.288 0.288 m^3 FORM (H < 4.0 m) $A = 14.210 \text{ m}^2 / 10.00 \text{ m x } 3.60$ 5.116 5.116 m² REINFORCING BAR W = 0.098 tf / 10.00 m x 3.600.035 0.035 tf JOINT FILLER $A = 3.120 \text{ m}^2 / 10.00 \text{ m} \times 3.60$ = 1.123 1.123 m²

TYPE OF WORK : CONCRETE STEPS
LOCATION : APPROACH STEPS (TYPE-WB)

PER 1 PLACE

CALCULATION		RESULT
STRUCTURAL EXCAVATION		
$A_1 = \frac{1}{2} \times 1.80 \times 0.90$	= 0.810	
A = (1.118 + 2.00 + 0.70) + 0.60	17(0	
$A_2 = (1.118 \times 2.00 + 0.70) \times 0.60$	= 1.762	
$A_3 = (0.80 + 1.80) \times \frac{1}{2} \times 0.80$	= 1.040	
TOTAL A	and the second of the second o	
$V_1 = 3.612 \times 3.60$	= 13.003	
		4.174.45
$V_2 = \{ \frac{1}{2} \times 0.60 \times 1.20 + (2.795 + 1.00) \times 0.50 \}$		
x 2.00 x 2	= 9.030	
TOTAL V	= 22.033	22.033 m ³
BACKFILL WITH SELECTED SOIL	i kanada (h. 1966). Biliar Englis (h. 1964). Biliar	
DACINILLI WITH SELECTED SOIL		
$A_1 = (0.50 + 1.10) \times \frac{1}{2} \times 0.60$	= 0.480	a en de do el el da Religio de la da
$A_2 = (0.50 + 0.80) \times \frac{1}{2} \times 0.30$	= 0.195	
$V_1 = 0.480 \times 3.60$	= 1.728	
W - 0.106 - 0.00 - 0	0.000	
$V_2 = 0.195 \times 2.00 \times 2$ TOTAL V	= 0.780 $= 2.508$	0.500 3
IOIAL V	- 2.308	2.508 m ³
GRAVEL BEDDING		
$A = (1.118 \times 2.00 + 0.70) \times 0.25$	= 0.734	
$V = 0.734 \times 3.00$	= 2.202	2.202 m ³
RUBBLE STONE FILLING		
ROBBLE STONE FILLING		
$V = \frac{1}{2} \times 0.60 \times 1.20 \times 7.60$	= 2.736	2.736 m ³
A TAX OLD A TAX A	2.130 (mg) (mg)	2.730 111
GABION CYLINDER Ø 500		
$V = \frac{\pi}{4} \times 0.50^2 \times (2.795 + 1.00) \times 4.00 \times 2$	= 5.961	5,961 m ³
SOIL FILLING		
$V = (2.795 + 1.00) \times 2.00 \times 2 - 5.961$	= 1.629	1.629 m³

TYPE OF WORK : CONCRETE STEPS
LOCATION : APPROACH STEPS (TYPE-GB-1)

		PER 1 PLACE
	CALCULATION	RESULT
STRUCTURAL EXCAVATION		
$A_1 = \frac{1}{2} \times 1.80 \times 0.90$	= 0.810	
72 × 1.00 × 0.50	V.01V	
$A_2 = (1.118 \times 4.800 + 0.70) \times 0.60$	= 3.640	
$A_3 = (0.80 + 1.80) \times \frac{1}{2} \times 0.80$	= 1.040 TOTAL A = 5.490 m ²	
$V_1 = 5.490 \times 3.60$	= 19.764	
$V_2 = {\frac{1}{2} \times 0.60 \times 1.20 + (5.925 + 1.00)}$		
x 2.00 x 2	= 15.290	
	TOTALV = 35.054	35.054 m ³
BACKFILL WITH SELECTED SO	iL	
$A_1 = (0.50 + 1.10) \times \frac{1}{2} \times 0.60$	= 0.480	
$A_2 = (0.50 + 0.80) \times \frac{1}{2} \times 0.30$	= 0.195	
$A_2 = (0.30 \pm 0.80) \times 72 \times 0.30$		
$V_1 = 0.480 \times 3.60$	= 1.728	
$V_2 = 0.195 \times 2.00 \times 2$	= 0.780 TOTAL V = 2.508	2.508 m ³
	101AL V = 2.308	2.308 m
GRAVEL BEDDING	1	
$A = (1.118 \times 4.80 + 0.75) \times 0.25$	= 1,517	
$V = 1.517 \times 3.00$	= 4.551	4.551 m ³
BASE CONCRETE		
CONCRETE TWO CI		
CONCRETE TYPE C1 V = 0.938		0.938 m ³
GRAVEL BEDDING		
V = 0.224		0.224 m ³
FORM (U < 4.0 m)		
FORM $(H < 4.0 \text{ m})$ A = 4.375		4.375 m ²
	<u> 1900 m. n. 1900 m. n. 1900 m. 1900 m. n. 1900 m. n. 1900 m. 1900 m. n. 1900 m. 1900 m. n. 1900 m. 1900 m. n. 1900 m. 190</u>	

TYPE OF WORK : CONCRETE STEPS
LOCATION : APPROACH STEPS (TYPE-GB-1)

PER 1 PLACE

CALCULATION	RESULT
REINFORCING BAR	
W = 0.032	0.032 tf
	The state of the s
LOGPILE	
L = 6.000	6.000 m
TOP CONCRETE	
CONCRETE TYPE CI	0.000
V = 0.756	0.756 m ³
GRAVEL BEDDING	0.288 m ³
V = 0.288	U.200 III
FORM (U < 4.0 m)	The second of th
FORM (H < 4.0 m) A = 5.116	5.116 m ²
REINFORCING BAR	
W = 0.035	0.035 tf
JOINT FILLER	
A = 1.123	1.123 m ²
	gay v state <u>state</u>
SIDE WALL	
CONCRETE TYPE C1	
$V = 1.233 \text{ m}^3 / 2.936 \text{ m} \times 6.066 = 2.547$	2.547 m ³
GRAVEL BEDDING $V = 1.089 \text{ m}^3/2.936 \text{ m} \times 6.066$ = 2.250	2.250 m ³
$V = 1.089 \mathrm{m}^3 / 2.936 \mathrm{m} \mathrm{x} 6.066 \qquad = 2.250$	2.230 III
FORM (II < 4.0 ~~)	
FORM (H < 4.0 m) A = $11.782 \text{ m}^2/2.936 \text{ m} \times 6.066$ = 24.343	24.343 m ²
$A = 11.782 \text{ m}^2 / 2.936 \text{ m x } 6.066 = 24.343$	27.373.11
REINFORCING BAR	
$W = 0.059 \text{ tf} / 2.936 \text{ m} \times 6.066$ = 0.122	0.122 tf
JOINT FILLER	
$A = 2.055 \text{ m}^2 / 2.936 \text{ m} \times 6.066 = 4.246$	4.246 m ²
	12. Ann an
	And the second s

TYPE OF WORK : CONCRETE STEPS LOCATION : APPROACH STEPS (TYPE-GB-1)

PER	1	PI.	A	CF
1 1./1				·

CALCULATION	RESULT
CONCRETE TYPE D	
$V = \{(1.118 \times 4.80 + 0.70) \times 0.35 - \frac{1}{2} \times 0.40$	100
$x 0.20 \times 12 = 4.930$	
FORM (H < 4.0 m)	
$A = 0.20 \times 3.00 \times 12 = 7.200$	7.200 m ²
REINFORCING BAR (D13, $W = 1.04 \text{ kg.f/m}$)	
$n_1 = (1.118 \times 4.80 + 0.70 - 0.05 \times 2) / 0.30 + 1 = 21 \text{ bars}$	
$L_1 = (3.00 - 0.05 \times 2)$ = 2.90 m/ bar	
$W_1 = 21 \times 2.90 \times 1.04 = 63.336$	
$n_2 = (3.00 - 0.05 \times 2) / 0.30 + 1$ = 11 bars	
$L_2 = 1.118 \times 4.80 + 0.70 - 0.05 \times 2 = 5.966 \text{ m/ bar}$	
$W_2 = 11 \times 5.966 \times 1.04 = 68.251$	
TOTAL W - 121 6071 - 6	0.120 #
TOTAL W = 131.587 kg.f	0.132 tf
RUBBLE STONE FILLING	
NOBBLE 51 ONE PROBLEMS	
$V = \frac{1}{2} \times 0.60 \times 1.20 \times 7.60 = 2.736$	2.736 m ³
- 2.750	2.750 III
GABION CYLINDER Ø 500	
$V = \frac{\pi}{4} \times 0.50^2 \times (5.925 + 1.00) \times 4.00 \times 2 = 10.878$	10.878 m ³
$Y = \frac{7}{4} \times 0.30 \times (3.923 + 1.00) \times 4.00 \times 2$	10.070 H
SOIL FILLING	
$V = (5.925 + 1.00) \times 0.50 \times 2 - 10.878 = 2.972$	2.972 m ³
$Y = (3.923 \pm 1.00) \times 0.30 \times 2 - 10.878 = 2.972$	4.714 III
	(2)
and distributed and the property of the first of the state of	A service was a St.

TYPE OF WORK: LOCATION:

CALCULATION		RESULT
	43 - 4 - 1 - 1 - 1 - 1	
5 STRUCTURAL EXCAVATION		
$A = (2.3 + 2.7) \times 0.6 : 2 =$: 1.500	
$A = 1.3 \times 2.9$	3.770	
$A = (6.3 + 3.25) \times 1.0 : 2 =$	4.775	
$A = (0.3 + 3.23) \times 1.0 \times 2$	4.773	
$A = (2.8 + 1.2) \times 0.7 \cdot 2 =$	1.400	
TOTAL =	11.445	
V = 11.445 x 3.6	41.202	41.202 m ³
		57357 <u>1</u>
ina di Partino de Maria de la proposición de la confesión de la confesión de la confesión de la confesión de l La confesión de la confesión d		
DACKER A MARKA CON ECTED COM		
5 BACKFILL WITH SELECTED SOIL		
$(0.5 + 1.3) \times 0.8 : 2$	0.720	
$(0.5 + 1.2) \times 0.7 : 2 =$	0.595	
$(0.4 + 1.4) \times 1.0 : 2 - 0.5 \times 1.0$	0.400	
TOTAL	1.715	
$V = 1.715 \times 3.6$	6.174	6.174 m³
	10.174	0.174 111

TYPE OF WORK

LOCATION : APPROACH STEP TYPE - G2

CALCULATION		RESU
NATURE DEPONING		
GRAVEL BEDDING		
REVETMENT		
$V = (0.7 + 9.890) \times 0.25 \times 3.0$	= 7.943	
2) BASE CONCRETE		i jaran da
$V = 0.800 \text{ m}^3 / 10.0 \text{ m} \times 3.6 \text{ m}$	- 0.200	
V - 0.800 iii / 10.0 iii x 3.0 iii	- 0.288	
3) TOP CONCRETE		
$V = 0.625 \mathrm{m}^3 / 10.0 \mathrm{m} \mathrm{x} 3.6 \mathrm{m}$	= 0.225	1.7
	sa ko a alamba labi f	
4) END WALL		1 / /
$V = 0.810 \text{ m}^3/10.0 \text{ m} \times (0.535 + 9.994) \times$	2	
	= 17.056	
	2.1000	
	= 25.512	25.512
	经运动 医电子电子 医多种性 医肾上腺炎	the second second
ONCDETE TVDE D	The state of the s	†
ONCRETE TYPE - D		
ONCRETE TYPE - D		
CONCRETE		
CONCRETE	= 3.807	
CONCRETE $A_1 = (10.872 + 0.7) \times 0.329$		
CONCRETE		
CONCRETE $A_1 = (10.872 + 0.7) \times 0.329$ $A_2 = 0.4 \times 0.2 : 2 \times 19$		
CONCRETE $A_1 = (10.872 + 0.7) \times 0.329$ $A_2 = 0.4 \times 0.2 : 2 \times 19$ $A = A_1 - A_2$	= 0.760	
CONCRETE $A_1 = (10.872 + 0.7) \times 0.329$ $A_2 = 0.4 \times 0.2 : 2 \times 19$ $A = A_1 - A_2$ $V = 3.047 \times 3.0 \text{ m}$	= 0.760	1
CONCRETE $A_1 = (10.872 + 0.7) \times 0.329$ $A_2 = 0.4 \times 0.2 : 2 \times 19$ $A = A_1 \cdot A_2$ $V = 3.047 \times 3.0 \text{ m}$	= 0.760 = 3.047 = 9.141	
CONCRETE $A_1 = (10.872 + 0.7) \times 0.329$ $A_2 = 0.4 \times 0.2 : 2 \times 19$ $A = A_1 - A_2$ $V = 3.047 \times 3.0 \text{ m}$	= 0.760 = 3.047 = 9.141	1
CONCRETE $A_1 = (10.872 + 0.7) \times 0.329$ $A_2 = 0.4 \times 0.2 : 2 \times 19$ $A = A_1 - A_2$ $V = 3.047 \times 3.0 \text{ m}$	= 0.760 = 3.047 = 9.141	
CONCRETE $A_1 = (10.872 + 0.7) \times 0.329$ $A_2 = 0.4 \times 0.2 : 2 \times 19$ $A = A_1 - A_2$ $V = 3.047 \times 3.0 \text{ m}$	= 0.760 = 3.047 = 9.141	
CONCRETE $A_1 = (10.872 + 0.7) \times 0.329$ $A_2 = 0.4 \times 0.2 : 2 \times 19$ $A = A_1 - A_2$ $V = 3.047 \times 3.0 \text{ m}$ • FORM $A = 0.2 \times 3.0 \times 19$	= 0.760 = 3.047 = 9.141	
CONCRETE $A_1 = (10.872 + 0.7) \times 0.329$ $A_2 = 0.4 \times 0.2 : 2 \times 19$ $A = A_1 - A_2$ $V = 3.047 \times 3.0 \text{ m}$	= 0.760 = 3.047 = 9.141	
CONCRETE $A_1 = (10.872 + 0.7) \times 0.329$ $A_2 = 0.4 \times 0.2 : 2 \times 19$ $A = A_1 - A_2$ $V = 3.047 \times 3.0 \text{ m}$ • FORM $A = 0.2 \times 3.0 \times 19$	= 0.760 = 3.047 = 9.141 = 11.4	
CONCRETE $A_1 = (10.872 + 0.7) \times 0.329$ $A_2 = 0.4 \times 0.2 : 2 \times 19$ $A = A_1 - A_2$ $V = 3.047 \times 3.0 \text{ m}$ • FORM $A = 0.2 \times 3.0 \times 19$	= 0.760 = 3.047 = 9.141 = 11.4	9.141
$A_2 = 0.4 \times 0.2 : 2 \times 19$ $A = A_1 - A_2$ $V = 3.047 \times 3.0 \text{ m}$ • FORM $A = 0.2 \times 3.0 \times 19$	= 0.760 = 3.047 = 9.141 = 11.4	11,400

APPROACH STEP TYPE	G 2	F
YPE OF WORK: CONCRETE TYPE C , FORM		170071
OCATION:		
	(7) BASE CONORETE	
300	COUNTY ETE	
0 13 0 10 cre 300	V = 3.150 m2/100m x 3.6m = 1.134	1134m3
8		
200	1 = 13.240m3/10.0 mx 3.6m= 4.766	4.766m2
2 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8		
001	3 TOP CONCRETE	
0 \ 001 001 001 000	· CONJORETE	
CONCRETE (TYPE_C1)	1 = 2.100m3/10,0m x 3,6m = 0.756	D.756m2
人名英格兰 经存货 化二氯化丁二苯 医多种性 医囊性 医二氏性 医动物性动物 医克里克氏试验检尿病 医多种性 医多种性 医多种性 医多种性 医多种性 医多种性 医多种性 医多种性		
	A = 14,400 m / 10,00m X 3.6m = 5,184	S.184m2
300 100 100 100 100 100 100 100 100 100	3 ZWD WALL	
6ABION (TLINDEN 50, 200, 50 (ELASTIC MATERIAL)	· CONCRETE	
	A = (0,7+0,535)x0,7+2 +	
	20 10 10	
300	V = 3,942x 0,3x2 = 2,365	2,365 m3
SOIL FILLING / STATE SOIL FILLING	TORM STATE OF THE	
100 101 101 101 101 101 101 101 101 101	3.942x2 = 7.88+	7.884m2
L		
SIDE WALL		
医乳色色素 医多克氏反射 医克克克克克克克克 法法律法院法律法 法法院 经证据的现在分词 化氯乙二甲烷法 经经济的 经人工工程 计记录器 计记录器 计记录器 计记录器 计记录器 计记录器 计记录器 计记录器		
	一一一十二十二十二十二十二十二十二十二十二十二十二十二十二十二十二十二十二十二	
	1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1	

	RESULT				0.036+F				0.034tf			0,202 +4										:			0.249+f
	CALCULATION		Q BASE CONCRETE		0.101 H/10,0m x 3.60m = 0.036		2) TOP CONCRETE		0,095 tf/10,0m x 3,60m = 0.034		S END WALL	0,101 + 1 / place x 2 = 0,202 ++		4 STEP	1213 (1.04 kgf/m)	* HOUZON74L	1=30-0.05x2 = 2.900	n = (10, 872 + 0, 7 - 0,05x2)	= 39	N=2.8x39 x 1.04 = 117624 but	·VERTICAL	1= 10,872+0,7-0,05x2 = 11,472	n=(30-0,05×2)=0,3+1= 11	W= 11.472 x1/x 1.04 = 131239	TOTAL = 2×8,863
APPROACH STEP TYPE 92	CING	LOCATION	400	0 250 250 50	D 13	000 OOE	GRAVEL BEDDING	033 cic 300	100 100 100 50200 00 00 00 00 00 00 00 00 00 00 00 00	1013 - 010	05	2000 mm	001	000 313 810 \ 001 001 000 000	TOP CONCRETE			300	7 (ELASTIC		002	8 0	() - () - () - () - () - () - () - () -	100 400 1111 CONCRETE (TYPE - C.)	

TYPE OF WORK:

LOCATION : APPROACH STEP TYPE - G2

CALCULATION	RESULT
를 JOINT FILTER	
1) TOP CONCRETE	
1) TOP CONCRETE	
A = 0.35 x 3.0 =	1.05 m ² 1.050 m ²
2) SIDE WALL	
0.7 10.00	
L = 0.7 + 10.062 =	0.762
$A = 10.762 \times 0.35 \times 2 =$	7.533 7.533 m ³
	7.555 III
	 4. 4. 4. 4. 4. 4. 4. 4. 4. 4. 4. 4. 4. 4
☐ GABION CYLINDER	
	<u> Principal de l'Espaigne de la company de l</u>
Ø = 500 mm	
$a = \pi/4 \times 0.5^2$	0.196
$V = 0.196 \times 8 \times (1.0 + 7.826 + 3.0) =$	8.543
	. I i a en este este a la l
5 SOIL FILLING	
5 SOIL FILLING	
$V_1 = 0.5 \times (1.0 + 7.826 + 3.0) \times 4.0 = 2$	3.652
$V_2 = 18.543$	
$V = V_1 - V_2 =$	5.109 m ³

TYPE OF WORK: LOCATION: APPROACH STEP TYPE - G2

CALCULATION	RESULT
	H 12
☐ GABION MATTRESS	
$A = 3.0 \times 0.5 + 1.5 \times 0.5 = 2.25$	
$V = 2.25 \times 3.0 \text{ m} = 6.75$	6.750 m ³
	<u> </u>
	1. 20 7.25
<u>그는</u> 등을 하는데 말하는 사람들은 사람들이 되었다. 그는 사람들은 사람들이 가는 사람들이 되었다. 그는 바다 그는 사람들이 되었다.	
	· · · · · · · · · · · · · · · · · · ·
A DUDDLE CLONE BULLING	
주 RUBBLE STONE FILLING	
다 RUBBLE STONE FILLING	
	1.500 m ³
$A = 1.0 \times 0.5 : 2 \times 2 = 0.5$	1.500 m ³
$A = 1.0 \times 0.5 : 2 \times 2 = 0.5$	1.500 m ³
$A = 1.0 \times 0.5 : 2 \times 2 = 0.5$	1.500 m ³
$A = 1.0 \times 0.5 : 2 \times 2 = 0.5$	1.500 m ³
$A = 1.0 \times 0.5 : 2 \times 2 = 0.5$	1.500 m ³
$A = 1.0 \times 0.5 : 2 \times 2 = 0.5$ $V = 0.5 \times 3.0 \text{ m} = 1.5$	1.500 m ³
$A = 1.0 \times 0.5 : 2 \times 2 = 0.5$ $V = 0.5 \times 3.0 \text{ m} = 1.5$	
$A = 1.0 \times 0.5 : 2 \times 2 = 0.5$ $V = 0.5 \times 3.0 \text{ m} = 1.5$	
$A = 1.0 \times 0.5 : 2 \times 2 = 0.5$ $V = 0.5 \times 3.0 \text{ m} = 1.5$	
$A = 1_{i}0 \times 0.5 : 2 \times 2 = 0.5$ $V = 0.5 \times 3.0 \text{ m} = 1.5$	
$A = 1,0 \times 0.5 : 2 \times 2 = 0.5$ $V = 0.5 \times 3.0 \text{ m} = 1.5$	
$A = 1,0 \times 0.5 : 2 \times 2 = 0.5$ $V = 0.5 \times 3.0 \text{ m} = 1.5$	
$A = 1.0 \times 0.5 : 2 \times 2 = 0.5$ $V = 0.5 \times 3.0 \text{ m} = 1.5$	
$A = 1.0 \times 0.5 : 2 \times 2 = 0.5$ $V = 0.5 \times 3.0 \text{ m} = 1.5$	
$A = 1.0 \times 0.5 : 2 \times 2 = 0.5$ $V = 0.5 \times 3.0 \text{ m} = 1.5$	
$A = 1.0 \times 0.5 : 2 \times 2 = 0.5$ $V = 0.5 \times 3.0 \text{m} = 1.5$	
$A = 1.0 \times 0.5 : 2 \times 2 = 0.5$ $V = 0.5 \times 3.0 \text{ m} = 1.5$	

TYPE OF WORK : LOCATION : APPROACH STEP TYPE GA-3

CALCULATION		RESULT
☐ STRUCTURAL EXCAVATION		
		<u> </u>
1) (2.9 + 2.2) x 0.6 : 2	= 1.53	
1) (2.5 + 2.2) X 0.0 · 2		
2) 8.6 x 1.3	= 11.18	
3) (2.7 + 4.3) x 0.5 : 2	= 1.75	
4) (1.2 + 2.7) x 0.7 : 2	= 1.365	
	= 15.825	A STATE OF THE STA
		Market 1
V = 15.825 x 1.800	= 28.485	28.485 m ³
BACKFILL WITH SELECTED SOIL		
1) (0.5 + 1.3) x 0.8 : 2	= 0.72	essisinae in 199
2) (0.5 + 1.2) x 0.7 : 2	= 0.595	
3) (1.2 + 1.7) x 0.5 : 2	= 0.725	
	= 2.040	
$V = 2.040 \times 1.800$	= 3.672	3.672 m ³
V - 2.040 X 1.000	- 3.072	3.072 III
		5.57.55 (3.57)
		n line in t
	· · · · · · · · · · · · · · · · · · ·	
		1.4% (A) (1.1% (A)
		<u> </u>

TYPE OF WORK:

LOCATION

APPROACH STEP TYPE GA-3

	CALCULATION	RESULT
□ GRA	AVEL BEDDING	
1)	BASE CONCRETE	
1,		
	$0.800 \mathrm{m}^3/10.0 \mathrm{m} \times 1.8 \mathrm{m}$ = 0.144	0.144, m ³
1245		
2)	TOP CONCRETE	
	$2.100 \mathrm{m}^3 / 10.0 \mathrm{m} \times 1.8 \mathrm{m} = 0.378$	0.378 m³
		0.576 III
3)	SIDE WALL	
	$0.810 \text{ m}^3/10.0 \text{ m x } (10.9 + 0.7) \text{ x 2} = 1.879$	1.879 m ³
4)		
- 7)	international Company of the company	
	$10.9 \times 0.25 \times 1.2 = 3.27$	3.27 m ³
<u> </u>		
•	ing transfer for the contraction of the first property of the contraction of the contraction of the contraction of $f r$	
	elegi varan kantan kalan da kantan da kantan kantan kantan kantan da kantan kantan kantan da kantan kantan da Maja suman Kangga Kantan da Kantan kantan kantan kantan kantan da kantan da kantan da kantan da kantan kantan d	Harris and the Cartestay
	en de la companya de De la companya de la	
라 cor	NCRETE TYPE - D	
占 CON	NCRETE TYPE – D	
	NCRETE TYPE - D CONCRETE	
	CONCRETE	
	CONCRETE $V_1 = (10.9 + 0.7) \times 0.329 \times 1.2 = 4.580$	
	CONCRETE	
	CONCRETE $V_1 = (10.9 + 0.7) \times 0.329 \times 1.2 = 4.580$ $V_2 = 0.4 \times 0.2 \times 23 \times 1.2 = 2.208$	2 270 3
	CONCRETE $V_1 = (10.9 + 0.7) \times 0.329 \times 1.2 = 4.580$ $V_2 = 0.4 \times 0.2 \times 23 \times 1.2 = 2.208$ $V = V_1 - V_2 = 2.372$	2.372 m ³
	CONCRETE $V_1 = (10.9 + 0.7) \times 0.329 \times 1.2 = 4.580$ $V_2 = 0.4 \times 0.2 \times 23 \times 1.2 = 2.208$	2.372 m³
	CONCRETE $V_1 = (10.9 + 0.7) \times 0.329 \times 1.2 = 4.580$ $V_2 = 0.4 \times 0.2 \times 23 \times 1.2 = 2.208$ $V = V_1 - V_2 = 2.372$ FORM	
	CONCRETE $V_{1} = (10.9 + 0.7) \times 0.329 \times 1.2 = 4.580$ $V_{2} = 0.4 \times 0.2 \times 23 \times 1.2 = 2.208$ $V = V_{1} - V_{2} = 2.372$ FORM	
	CONCRETE $V_1 = (10.9 + 0.7) \times 0.329 \times 1.2 = 4.580$ $V_2 = 0.4 \times 0.2 \times 23 \times 1.2 = 2.208$ $V = V_1 - V_2 = 2.372$ FORM $A = 0.2 \times 1.2 \times 23 = 5.52$	5.52 m³
	CONCRETE $V_1 = (10.9 + 0.7) \times 0.329 \times 1.2 = 4.580$ $V_2 = 0.4 \times 0.2 \times 23 \times 1.2 = 2.208$ $V = V_1 - V_2 = 2.372$ FORM $A = 0.2 \times 1.2 \times 23 = 5.52$	
	CONCRETE $V_1 = (10.9 + 0.7) \times 0.329 \times 1.2 = 4.580$ $V_2 = 0.4 \times 0.2 \times 23 \times 1.2 = 2.208$ $V = V_1 - V_2 = 2.372$ FORM $A = 0.2 \times 1.2 \times 23 = 5.52$	5.52 m³
	CONCRETE $V_1 = (10.9 + 0.7) \times 0.329 \times 1.2 = 4.580$ $V_2 = 0.4 \times 0.2 \times 23 \times 1.2 = 2.208$ $V = V_1 - V_2 = 2.372$ FORM $A = 0.2 \times 1.2 \times 23 = 5.52$	5.52 m ³
	CONCRETE $V_1 = (10.9 + 0.7) \times 0.329 \times 1.2 = 4.580$ $V_2 = 0.4 \times 0.2 \times 23 \times 1.2 = 2.208$ $V = V_1 - V_2 = 2.372$ FORM $A = 0.2 \times 1.2 \times 23 = 5.52$	5.52 m³
	CONCRETE $V_1 = (10.9 + 0.7) \times 0.329 \times 1.2 = 4.580$ $V_2 = 0.4 \times 0.2 \times 23 \times 1.2 = 2.208$ $V = V_1 - V_2 = 2.372$ FORM $A = 0.2 \times 1.2 \times 23 = 5.52$	5.52 m ³

	RESULT			S.567m3		2,3834				D,378m3		2,520 m ²				2.436m3		16.240m2						* * * * * * * * * * * * * * * * * * *	
	CALCULATION	CONCRETE		x 1,8m = 0.567	And the second s	m2/10.0m x 1.8m = 2.484		WCRE7E		m = 10.0m x 1.8m = 0.378		14.000 m3/10.0m x 1.8m = 2,520 2		7.7	-72	1x0,7x0,3 = 2,436		10.970.7) XO.7 X 2 = 16.24					e per agreta anticonomica e en enconomica (per en en porte en enconomica en entre de entre en		
GH-3		O BASE CON	· CONCRETE	N = 3.150 m3/10.0m		(ELAST A = 13.240		3 TOP CONCRETE	•	$V = 2.7\infty$	· FORM	A		SIDE WALL	· CONCORTE	V=(10.9+0.7	- FORIY	A = (10.9							
TUYE STEBS	1			电光线表 化多合物 医动物性 医皮肤 医二氏病 医二氏病 医二氏病 医二氏病 医二氏病 医二氏病 医二氏病 医二氏病	300	202020	88			i oc	1000	•					y.	0.00 513 00.0	001		100 400/	CONCRETE (IYRE-CL)	SIDE WALL		
	TYPE OF WORK:	LOCATION:			009	250 250	013	L	000		009	CONCRETE (TYPE - CI)	5-3	33			A SOO B SOO		₩ 2.	SOIT LIA			日本の日本の日本の日本の日本の日本の日本の日本の日本の日本の日本の日本の日本の日		

APPROACH STEPS TYPE GA-			
TYPE OF WORK: DEFORMED REMITORCING BAR	CALCULATION	RESULT	
LOCATIONS			
	() BASE CONCRETE		
000	0.10/11/10.0m X 1.8 m = 0.018	0,018+f.	
250 250 50			
8	2) TOP CONCRETE		
300	0.095 + 1 /10,0m x 1.8m = 0.017	0.017tf	
300			
ـا ـــــــــــــــــــــــــــــــــــ	3) SIDE NATL		
0 00 00 00 100 00 00 100 00 100 100 100			
CONGRETE (TYPE - CI)	0.095 +7 /10,0m x (10,9+0.7)x2 =0,110	P.110 +F	
GABION CYLINDER SO ZOO SO JOINT FILLER I TO MM	S 5720		
	, D/3 (1.04 kgt/m)		
	· +01120N74 L		
001	1=12-0,05x2		
O TO THE TIOS	N= (10.9+0.7-0.65x2) 20.3+1 = 39		
000 1000	818.44 = 40.1 X 88 X 1 X = W	•	
		(No. menur)	
	· VERTICAL		
3 05	1 = 10.9 +0.7 -0.05x2 = 11.5		
ONCARTE	n=(1,2-0,05x2) =0,3+1 = 4		
(1798-0)	W= 11.5 x 4 x 1.04		
GRAVEL BEDDING			
013 013 013	70741 = 92:456	0.0927	
013 010 300		•	

TYPE OF WORK:

LOCATION : APPROACH STEP TYPE GA

CALCULATION		RESULT
5 JOINT FILTER		
5 JOINT FILTER		
t = 10.0 mm ELASTIC MATERIAL		
1) TOP CONCRETE		
0.710 -2410.0		
0.710 m ² /10.0 m x 1.200 m	= 0.445	0.445 m ²
2) SIDE WALL		
L = 10.9 + 0.7	= 11.600	
$A = 11.60 \times 0.35 \times 2$	= 8.120	8.120 m ²
	0.120	0.120 111
		A section of the
☐ GABION CYLINDER		
		#8
• Ø 500 mm	gen en en er i de magey met de de gel.	e Sylver of the
$A = \pi / 4 \times 0.5^2$	= 0.196 m ²	
	- 0.190 m	
$V = 0.196 \times 8 \times (3.0 + 5.4)$	= 13.171	13.171 m ³
		18 July 18 18 18 18 18 18 18 18 18 18 18 18 18
5 SOIL FILLING		
$V_1 = 0.5 \times (3.0 + 5.4) \times 4.0$	= 16.800 m ³	
	10.000 m	
$V_2 = 13.171 \text{ m}^3$		
$V = V_1 - V_2$	2 (00	3 (3.3
$V = V_1 - V_2$	= 3.629	3.629 m ³
The state of the s		
· · · · · · · · · · · · · · · · · · ·		

TYPE OF WORK : LOCATION : APPROACH STEP TYPE GA-4

CALCULATION		RESULT
5 STRUCTURAL EXCAVATION		
B STRUCTURAL EXCAVATION		
i) (2.0 + 2.7) x 0.7 : 2 =	1.645	
2) (1.2 + 0.9) x 0.2 : 2 =	0.210	
		, , , , , , , , , , , , , , , , , , , ,
3) 1.0 x 2.1 =	2,100	
4) (3.9 + 1.9) x 0.7 : 2	2.030	
5 (10 12 2 0 1 0	0.640	
5) (1.9 + 1.3) x 0.4 : 2	0.640	
TOTAL =	6.625	
V = 6.625 x 1.8 =	11.925	11.925 m ³
	<u>and and a substance of the Community of</u>	
5 BACKFILL WITH SELECTED SOIL		
- BACKFILL WITH SELECTED SOIL		
1) (0.5 + 1.0) x 0.9 : 2	0.675	
2) (2.8 + 1.6) x 0.5 : 2 =	1.100	
3) (1.3 + 0.6) x 0.7 : 2 =	0.775	
= 3) (1.3 + 0.6) x 0.7 : 2	0.665	
TOTAL A =	2.440	
$V = 2.440 \times 1.8$	4.392	4.392 m ³
The second of th		

TYPE OF WORK : LOCATION : APPROACH STEP TYPE GA-4

	CALCULATION				RESULT
GRAVEL BEDDING		<u> </u>			
					
1) STEP					
$0.3 \times (5.65 + 0.7) \times$	1.2 m	=	2.286		2.286 m ³
a) Dige collonger					
2) BASE CONCRETE					
0.800 m ³ /10.0 m x 1	.8 m	=======================================	0.144		0.144 m ³
				-	V.1 () III
3) TOP CONCRETE					
0.700 m ³ /10.0 m	10		1.00 A		2122
0.700 In 710.0 in	X 1.8 m		0.126	<u> 4</u> 2 4	0.126 m ³
4) SIDE WALL					
	<u>An Car</u> e to the Analysis				
0.700 m ³ /10.0 m	$x (5.65 + 0.7) \times 2$	=	0.889	Richard Cont.	0.889 m ³
				and the second	
<u> </u>					
			<u> </u>		
CONCRETE TYPE -	D				
CONCRETE TYPE -	D				
	D				
	D				
• CONCRETE					
			2.943		
• CONCRETE $V_1 = 0.366 \times (6.0)$	+ 0.7) x 1.2				
• CONCRETE	+ 0.7) x 1.2		2.943 0.54		
• CONCRETE $V_1 = 0.366 \times (6.0)$	+ 0.7) x 1.2				2.403 m ³
• CONCRETE $V_1 = 0.366 \times (6.0)$ $V_2 = 0.2 \times 0.3$	+ 0.7) x 1.2		0.54		2.403 m³
• CONCRETE $V_1 = 0.366 \times (6.0)$ $V_2 = 0.2 \times 0.3$: $V = V_1 - V_2$	+ 0.7) x 1.2		0.54		2.403 m².
• CONCRETE $V_1 = 0.366 \times (6.0)$ $V_2 = 0.2 \times 0.3 :$ $V = V_1 - V_2$	+ 0.7) x 1.2		0.54		
• CONCRETE $V_1 = 0.366 \times (6.0)$ $V_2 = 0.2 \times 0.3 :$ $V = V_1 - V_2$ • FORM	+ 0.7) x 1.2 2 x 1.2 x 15		0.54 2.403		
• CONCRETE $V_1 = 0.366 \times (6.0)$ $V_2 = 0.2 \times 0.3$: $V = V_1 - V_2$	+ 0.7) x 1.2 2 x 1.2 x 15		0.54		3.600 m²
• CONCRETE $V_1 = 0.366 \times (6.0)$ $V_2 = 0.2 \times 0.3 :$ $V = V_1 - V_2$ • FORM $A = 0.2 \times 1.$	+ 0.7) x 1.2 2 x 1.2 x 15		0.54 2.403		
• CONCRETE $V_1 = 0.366 \times (6.0)$ $V_2 = 0.2 \times 0.3 :$ $V = V_1 - V_2$ • FORM	+ 0.7) x 1.2 2 x 1.2 x 15 2 x 15		0.54 2.403		3.600 m²
• CONCRETE $V_1 = 0.366 \times (6.0)$ $V_2 = 0.2 \times 0.3 :$ $V = V_1 - V_2$ • FORM $A = 0.2 \times 1.$	+ 0.7) x 1.2 2 x 1.2 x 15 2 x 15		0.54 2.403		3.600 m²
• CONCRETE $V_1 = 0.366 \times (6.0)$ $V_2 = 0.2 \times 0.3 :$ $V = V_1 - V_2$ • FORM $A = 0.2 \times 1.$	+ 0.7) x 1.2 2 x 1.2 x 15 2 x 15		0.54 2.403		3.600 m²
• CONCRETE $V_1 = 0.366 \times (6.0)$ $V_2 = 0.2 \times 0.3 :$ $V = V_1 - V_2$ • FORM $A = 0.2 \times 1.$	+ 0.7) x 1.2 2 x 1.2 x 15 2 x 15		0.54 2.403		3.600 m ²
• CONCRETE $V_1 = 0.366 \times (6.0)$ $V_2 = 0.2 \times 0.3 :$ $V = V_1 - V_2$ • FORM $A = 0.2 \times 1.$	+ 0.7) x 1.2 2 x 1.2 x 15 2 x 15		0.54 2.403		3.600 m²

APPROACH STEP TYPE GA	6.4.4	
TYPE OF WORK: CONCRETE TYPE C1	CALCULATION	RESULT
LOCATION		
	O BASE CONCRETE	
20 20 20 20 20 20 20 20 20 20 20 20 20 2	CONCRETE	
6.300	V = 3150 m3/10,0m x 180m = 0.567	0.587 m3
000	The FOR M & Contract of the specimen of the second	
	A = 13.240 m2/000m x 1.80m = 2383	2.383m²
91 000		
BASE CONCRETE	3) TOP CONCRETE	
	CONCRETE	
	V = 2,400 m3/lo.cm × 1,80m = 0,432	0.432m3
	A = 16,000 m 2/10,0m x 1,80m = 2,88	2,880m²
		2 2 2 3 4 4 4
	3 SIDE WALL	
	CONCRETE	
300. 300 300 300 1 1 1 1 1 1 1 1 1 1 1 1 1 1	7) x0, 3 x 0,8x2 = 3,216	3,216m3
0 13 51		
	$A = (6.6+0.7) \times 0.8 \times 4 = 21.44$	21.440m²
000 111 110 000 110 000		
SANGE BEDS NO SON SON SON SON SON SON SON SON SON		
SIDE WALL TOP CONCRETE		
《《···································		

TYDE OF WORK	APPROACH STED TYPE CA-4	NOLLY III O IVO	T = 10 = 0
LOCATION:	DEFORMED REINFORCING BAR		200
		(1) BASE CONCRETE	0/8+6
		3 TOP CONCRETE	
		0.100 +4 /10.0m x 1.8m = 0.018 0.018+f.	18+J.
010 614 500 500 500	\$ 200 m		
000	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	3 SIDE WALL	
001	OHIOGEN TOWNS OF THE PROPERTY	0,100 +1/10,0m x 16,070,7) x2 = 0,067 0,067+	67+£
100 600 IIO	900000	÷	
_	YFE_CI) OF STEPS	4 STEP	
BASE CONCRETE	7. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1.	DI3 Choutest/m)	
		· HOLIZONTAL	
Toon I	00.1	l= 1,2 =0,05x2 = 1,10 m	
20 200 30 (GLASTIC MATCRIAL)	2 St. P. 121-13 10 20 20 20 20 20 20 20 20 20 20 20 20 20	n= (6,0+0,7 -0,05x2)20x311= 23	
	000 300 000 000 000 000 000 000 000 000	W= 11 x 23 x 104 = 26,3/2 2.4	
		· VERTICAL	
000 577 (10)		1 = 6.0+0.7-0.05x2 = 6.60	
100 300 100 117	CONTRACT STORY CONTRACT CONTRA	n=(1,2-0,05x2)-0,3+1 = 4	
SIDE WALL	RETE	W= 66 x 4 x 104 = 29.040 lef	:
SCALE	STATE OF THE STATE		
		7074L = 55.352 0055	55 tf.
		A service of the serv	

RESULT			0,509 m²		S.360m2												
CALCULATION	· 10 mm , Elastic Matterial.	O TOP CONCRETE	A = 4,240 m=/10,0m x 1,2m = 0,509 0.	SIDE WALL	A= (6,0+0,7) x D, 4 x 2 = 5,36 S.												
APPROACH STEP TYPE GA-4 TYPE OF WORK: JOINT FILLER	LOCATION	300	D13 ctc 300	08 08 08 08 08 08 08 08	300/	GRAVEL SEDDING CONCRETE, (TYPE - CI)	SCALE B		SON	0.13		366	S	211 001 001	CONCRETE, LYPE - CI)	SIDE WALL	